

GENERAL STRUCTURAL NOTES

1. **CODES:**
- A. ALL WORK SHALL CONFORM TO THE FLORIDA BUILDING CODE, 2017 EDITION AND ALL OTHER APPLICABLE LOCAL CODES.
- B. ALL STANDARDS REFERENCED IN THESE DRAWINGS SHALL REFER TO THE EDITIONS OF SUCH STANDARDS AS LISTED IN FBC 2017, CHAPTER 35, "REFERENCED STANDARDS".
2. **DESIGN CRITERIA:**
- A. ROOF LOADS (GRAVITY):
NON-OCCUPIABLE ROOF AREA LIVE LOAD: 30 PSF
OCCUPIABLE ROOF AREA LIVE LOAD: 100 PSF
SUPERIMPOSED DEAD LOAD: 10 PSF (@ ROOF)
10 PSF (@ CEILING)
- B. FLOOR LOADS (GRAVITY):
LIVE LOAD FOR ALL AREAS EXCEPT BALCONIES: 40 PSF
BALCONY LIVE LOAD: 60 PSF
SUPERIMPOSED DEAD LOAD: 25 PSF
- C. WIND LOADS: IN ACCORDANCE WITH ASCE 7-10 [RISK CATEGORY II; 170 MPH ULTIMATE DESIGN WIND SPEED; EXPOSURE D; INTERNAL PRESSURE COEFFICIENTS, ± 0.18]. SEE CALCULATIONS FOR ADDITIONAL INFORMATION.
- D. FLOOD DESIGN CRITERIA:
1. FLOOD ZONE: VE
2. FLOOD ELEVATIONS:
a. BASE FLOOD ELEVATION (B.F.E.) = +10.00 FT. N.G.V.D.
b. DESIGN FLOOD ELEVATION (D.F.E.) = B.F.E. + 1.00 FT. = +10.00 FT.+1.00 = +11.00 FT. N.G.V.D.
3. DESIGN STANDARD:
a. ASCE 24 (WITH MODIFICATIONS INDICATED IN FBC 1612.4.1).
4. FLOOD DESIGN APPROACH:
a. THE ELEVATION OF THE BOTTOM OF LOWEST HORIZONTAL STRUCTURAL MEMBER IS AT OR ABOVE D.F.E. FOR VE AND COASTAL A ZONES. THE BUILDING IS DESIGNED IN ACCORDANCE WITH ASCE 24 (INCLUDING PILE FOUNDATION AND SUPERSTRUCTURE) SO THAT IT IS ANCHORED TO RESIST FLOTATION, COLLAPSE AND LATERAL MOVEMENT DUE TO THE EFFECTS OF WIND AND FLOOD LOADS ACTING SIMULTANEOUSLY ON ALL BUILDING COMPONENTS AND OTHER LOAD REQUIREMENTS OF CHAPTER 16 OF THE FBC.
- b. DRY FLOODPROOFING:
DRY FLOODPROOFING IS DESIGNED IN ACCORDANCE WITH ASCE 24. SEE PLAN FOR LOCATIONS.
3. **BUILDING PERMIT:**
- A. OBTAIN BUILDING PERMIT.
- B. COMPLY WITH THE REQUIREMENTS OF THE BUILDING PERMIT AND WITH OTHER REQUIREMENTS OF THE PERMITTING AUTHORITY.
- C. IF CHANGES TO STRUCTURAL DESIGN ARE ISSUED BY THE ENGINEER, SUBMIT CHANGES TO THE BUILDING DEPARTMENT FOR REVIEW AND APPROVAL. MAINTAIN PERMIT APPROVALS CONCURRENT WITH CONSTRUCTION.
4. **CONSTRUCTION DOCUMENTS:**
- A. EXAMINE AND STUDY ALL CONSTRUCTION DOCUMENTS PRIOR TO COMMENCEMENT OF WORK. DIRECT ANY QUESTIONS TO THE ENGINEER.
5. **ELEVATION DATUM:**
- ALL ELEVATIONS ON THESE STRUCTURAL DRAWINGS REFER TO TOP OF INTERIOR GROUND FLOOR SLAB = -0'-2", -0'-2"=14.51 FT N.G.V.D. (+12.93 N.A.V.D.)
6. **COORDINATION AND DIMENSIONS:**
- A. COORDINATE ALL DIMENSIONS AND ELEVATIONS BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO PROCEEDING WITH THE WORK. VERIFY ALL DIMENSIONS AND CONDITIONS RELATED TO EXISTING CONDITIONS IN THE FIELD PRIOR TO CONSTRUCTION. BRING ANY DISCREPANCIES TO THE IMMEDIATE ATTENTION OF THE ENGINEER. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALE. SCALE IS FOR GUIDELINE PURPOSES ONLY. IF DIMENSIONS ARE UNCLEAR, DO NOT SCALE. REQUEST CLARIFICATION FROM THE ENGINEER.
- B. COORDINATE THE STRUCTURAL WORK WITH THE WORK OF ALL OTHER TRADES.
- C. COORDINATE THE STRUCTURAL WORK WITH SLAB DEPRESSIONS, MECHANICAL OPENINGS, PIPING AND CONDUITS AS SHOWN ON OTHER DRAWINGS.
- D. COORDINATE ALL DIMENSIONS RELATIVE TO MANUFACTURED ITEMS, SUCH AS ELEVATORS, ESCALATORS, DOORS, WINDOWS, ACCESS PANELS AND HEATING, VENTILATING AND AIR-CONDITIONING EQUIPMENT.
7. **CONFLICTS IN DOCUMENTS:**
- IF CONFLICTS OCCUR IN OR BETWEEN ARCHITECTURAL AND ENGINEERING DOCUMENTS, BETWEEN DOCUMENTS AND FIELD CONDITIONS OR OTHERWISE, IMMEDIATELY CONTACT THE ENGINEER FOR CLARIFICATION AND DIRECTION BEFORE PROCEEDING. COORDINATE ALL DIMENSIONS BETWEEN ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO PROCEEDING WITH THE WORK.
8. **METHODS & SAFETY:**
- A. THE CONTRACTOR IS RESPONSIBLE FOR ALL METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION. PROVIDE APPROPRIATE SUPERVISION THROUGHOUT THE PROJECT. CONSTRUCTION SITE SAFETY, INCLUDING ALL ADEQUATE TEMPORARY BRACING AND SHORING, IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO OBTAIN THE NECESSARY PROFESSIONAL SERVICES TO DETERMINE THE NECESSARY METHODS AND SUPPORTS REGARDING FORMING AND CONSTRUCTION LOADS. TEMPORARY BRACING AND SHORING SHALL BE DESIGNED TO RESIST ALL CONSTRUCTION LOADS INCLUDING THE WEIGHTS OF ALL SUPPORTED MATERIALS PLUS A LIVE LOAD OF 50 PSF ON HORIZONTAL SURFACES. MAINTAIN TEMPORARY BRACING AND RETAIN IN PLACE UNTIL PERMANENT STRUCTURAL SYSTEMS ARE CAPABLE OF RESISTING ALL CONSTRUCTION PHASE LOADS.
- B. CONTRACTOR IS SOLELY RESPONSIBLE FOR THE SAFE AND APPROPRIATE USE OF ALL PRODUCTS AND MATERIALS. STRICTLY CONFORM TO ALL OF THE MANUFACTURERS' PROVIDERS' OR INDUSTRY'S RESTRICTIONS, RECOMMENDATIONS, PRECAUTIONS AND PROTECTIONS (INCLUDING AS INDICATED IN THE MATERIAL/PRODUCT SAFETY DATA SHEET) FOR EACH PRODUCT'S OR MATERIAL'S STORAGE, HANDLING, USE, APPLICATION, CLEAN-UP AND DISPOSAL.
9. **PROTECTION OF EXISTING CONSTRUCTION:**
- A. DO NOT DAMAGE EXISTING CONSTRUCTION WHICH IS TO REMAIN. LOCATE AND PROTECT CONCEALED PIPES, CONDUITS AND OTHER EXISTING CONSTRUCTION PRIOR TO DEMOLITION AND TAKE APPROPRIATE ACTION TO PROTECT THEM AND TO PROVIDE FOR SAFETY.
10. **CONSTRUCTION INSPECTIONS:**
- A. NOTIFY THE BUILDING INSPECTOR FOR INSPECTION OF ALL STRUCTURAL ELEMENTS. VERIFY THAT EACH AND EVERY STRUCTURAL ITEM HAS BEEN ACCEPTED BY THE INSPECTOR PRIOR TO PROCEEDING WITH SUBSEQUENT WORK AND/OR CONCEALING ANY STRUCTURAL ITEM. ANY STRUCTURAL ITEM WHICH HAS NOT BEEN SPECIFICALLY ACCEPTED BY THE INSPECTOR AND/OR ANY CONCEALING CONSTRUCTION WILL BE SUBJECT TO REMOVAL AND RECONSTRUCTION.
- B. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT ALL REQUIRED STRUCTURAL INSPECTIONS HAVE BEEN PERFORMED AND THAT THEY INDICATE ACCEPTANCE.
11. **SPECIAL INSPECTION:**
- BY PROVISION OF THE FLORIDA BUILDING CODE 2017 AND THE BROWARD COUNTY ADMINISTRATIVE PROVISIONS (SECTION 110.10), SPECIAL INSPECTION IS REQUIRED FOR INSTALLATION OF STRUCTURAL PILES. REINFORCED MASONRY, WELDING AND HIGH STRENGTH BOLTING FOR STRUCTURAL STEEL AND AS MAY OTHERWISE BE REQUIRED BY THE BUILDING OFFICIAL. NOTIFY SPECIAL INSPECTOR FOR INSPECTION OF ALL COMPONENTS OF THESE SYSTEMS PRIOR TO THEIR CONCEALMENT BY OTHER CONSTRUCTION. DO NOT CONCEAL ANY COMPONENT OF THESE SYSTEMS UNTIL ACCEPTED BY SPECIAL INSPECTOR AND UNTIL SO INDICATED ON THE SPECIAL INSPECTION LOG. THE SPECIAL INSPECTION LOG WILL BE PREPARED BY SPECIAL INSPECTOR AND GIVEN TO CONTRACTOR FOR KEEPING ON SITE. KEEP THE SPECIAL INSPECTION LOG IN A CLEAN DRY AREA AT THE SITE AND MAKE IT AVAILABLE TO THE SPECIAL INSPECTOR AND TO THE MUNICIPAL BUILDING INSPECTOR UPON REQUEST. THE SPECIAL INSPECTOR WILL MAKE ENTRIES ON THE LOG FOR EACH SITE VISIT AND WILL INDICATE ACCEPTANCE OR REJECTION OF THE ITEMS OBSERVED. ACCEPTANCE APPLIES ONLY TO THOSE SPECIFIC ITEMS SO INDICATED. NO OTHER ITEMS OR AREAS ARE TO BE ASSUMED TO BE ACCEPTED. ANY COMPONENTS OF THE STRUCTURAL SYSTEMS WHICH HAVE BEEN CONCEALED WITHOUT SPECIFIC ACCEPTANCE BY THE SPECIAL INSPECTOR WILL BE REJECTED, AND ANY CONCEALING CONSTRUCTION MUST BE REMOVED. UPON ACCEPTANCE OF ALL COMPONENTS OF ALL SYSTEMS, RETURN SPECIAL INSPECTION LOG TO THE SPECIAL INSPECTOR. COMPLY WITH ALL REQUIREMENTS OF THE BUILDING OFFICIAL.
12. **SHOP DRAWINGS:**
- A. SUBMIT TO THE ENGINEER COMPLETE SHOP DRAWINGS AS REQUIRED BY THESE CONTRACT DOCUMENTS.
- B. CHECK ALL SHOP DRAWINGS FOR COMPLIANCE AND COMPLETENESS PRIOR TO SUBMITTAL. ALL SHOP DRAWINGS SHALL BEAR EVIDENCE OF CONTRACTORS REVIEW AND APPROVAL.
- C. REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS FOR CONFORMANCE WITH DESIGN CONCEPT AND INFORMATION INDICATED IN CONTRACT DOCUMENTS, ACCURACY, COMPLETENESS, DIMENSIONS, QUANTITIES, SAFETY PRECAUTIONS, CONSTRUCTION MEANS AND METHODS, SEQUENCE OF CONSTRUCTION, COORDINATION WITH OTHER TRADES AND PERFORMANCE OF SYSTEMS REMAIN THE RESPONSIBILITY OF THE CONTRACTOR.
- D. REVIEW BY THE ENGINEER IS NOT FOR THE PURPOSE OF APPROVING CHANGES OR SUBSTITUTIONS.

13. **SOILS:**
- A. INFORMATION ABOUT THE SOILS ON THIS SITE IS PRESENTED IN A REPORT PREPARED BY NUTTING ENGINEERS OF FLORIDA, INC., DATED APRIL 30, 2019 AND ADDENDUM No.1 DATED JUNE 10, 2019. REVIEW THIS REPORT AND FOLLOW ITS RECOMMENDATIONS UNLESS OTHERWISE INDICATED IN THE CONTRACT DOCUMENTS. CONTRACTOR MAY OBTAIN SUPPLEMENTAL INFORMATION THROUGH ADDITIONAL SOILS EXPLORATION, TESTING AND ANALYSIS IF DESIRED. COORDINATE WITH THE OWNER FOR SITE ACCESS.
- B. ACCORDING TO THE ABOVE REFERENCED REPORT, THE SOILS AT THE BORING LOCATIONS GENERALLY CONSIST OF:
- MEDIUM DENSE BROWN TO GRAY SAND WITH SHELL FRAGMENTS (BEACH SAND), FROM +0'-0" (EXIST. GRADE) TO 23'-0".
- HARD GRAY LIMESTONE WITH SOME POROUS LENSES, FROM 23'-0" TO 50'-0"
14. **SOILS -GENERAL:**
- A. PRIOR TO COMMENCING WORK, VERIFY THAT METHODS AND PROCEDURES WILL NOT CAUSE DAMAGE TO NEARBY EXISTING STRUCTURES.
- B. PRIOR TO START OF WORK, THOROUGHLY PHOTOGRAPH AND/OR VIDEO RECORD EXISTING NEARBY CONSTRUCTION AND SITE CONDITIONS ALL AROUND THE BUILDING SITE (INCLUDING AN APPROPRIATE DISTANCE BEYOND THE PROPOSED CONSTRUCTION). MAKE SPECIFIC NOTE OF CRACKS, SETTLEMENTS OR OTHER DEFECTS IN EXISTING CONSTRUCTION. SUBMIT RECORD TO ENGINEER PRIOR TO THE START OF WORK.
- C. EXCAVATIONS:
1. COMPLY WITH ALL EXCAVATION SAFETY REGULATIONS AND STANDARDS. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO ASSURE EXCAVATION SAFETY AND STABILITY AT ALL TIMES. CONTRACTOR SHALL HIRE A FLORIDA REGISTERED ENGINEER FOR CONSULTATION, DESIGN AND INSPECTION AS MAY BE APPROPRIATE FOR LATERAL SUPPORT OF SIDES OF EXCAVATIONS. ASSURE THAT CONSTANT ADEQUATE SOIL SUPPORT IS PROVIDED TO ALL NEARBY EXISTING CONSTRUCTION AND SITE GRADES AT ALL TIMES. DO NOT ALLOW ANY SOIL SETTLEMENT OR LATERAL MOVEMENT THAT COULD CAUSE DAMAGE TO NEARBY EXISTING CONSTRUCTION OR SITE GRADES.
2. WHERE EXCAVATION MAY REDUCE LATERAL SUPPORT FOR EXISTING SOILS ON WHICH EXISTING FOUNDATIONS OF NEARBY STRUCTURES RELY AND WHERE OTHER OPERATIONS, SUCH AS DEWATERING, SOIL COMPACTION AND PILE DRIVING, MAY NEGATIVELY AFFECT THE PERFORMANCE OF EXISTING SOILS ON WHICH EXISTING FOUNDATIONS AND NEARBY STRUCTURES RELY, CONTRACTOR SHALL HIRE A REGISTERED ENGINEER TO ASSESS EXISTING SOILS AND EXISTING STRUCTURES. CONTRACTOR'S ENGINEER SHALL DETERMINE THE REQUIREMENTS FOR PROTECTION OF THE EXISTING STRUCTURES AND GRADES. SUCH PROTECTION MAY INCLUDE MODIFICATION OF PROPOSED PROCEDURES, UNDERPINNING, SOIL IMPROVEMENT, BRACING, SHEETING, OR OTHER APPROPRIATE MEANS AS DETERMINED BY CONTRACTOR'S ENGINEER AND AS ACCEPTABLE TO THE BUILDING OFFICIAL.

15. AUGER CAST PILES:
- A. PILES INDICATED IN THESE DRAWINGS AS 35T:
- | SYMBOL ON PLANS | DIAMETER | ALLOWABLE COMPRESSION CAPACITY | ALLOWABLE TENSION CAPACITY | ALLOWABLE LATERAL CAPACITY | DEPTH BELOW P.C. | CONCRETE GROUT STRENGTH | REINFORCEMENT |
|-----------------|----------|--------------------------------|----------------------------|----------------------------|------------------|-------------------------|---|
| | 14"Ø | 35 TONS | 18 TONS | 7.5 TONS | 27'-0" | 5,000 P.S.I. | #8@9 VERTICAL BARS AND (48) #3 TIES AT 5" O.C. FOR A LENGTH OF 20 FEET FROM THE BOTT. OF THE GRADE BM. OR PILE CAP/SLAB AND #3 @ 12" O.C. BALANCE |
- B. ALL PILES SHALL EXTEND AT LEAST 7 FEET INTO THE LIMEROCK, AN APPROXIMATE DEPTH OF 27 FEET FROM BOTTOM OF PILE CAP. BASE BID SHALL INCLUDE PILES WITH TIPS UP TO 30 FEET BELOW BOTTOM OF PILE CAP. PROVIDE UNIT PRICE FOR EACH LINEAR FOOT OF PILE DEEPER THAN 30 FEET BELOW BOTTOM OF PILE CAP.
- C. SEE AUGER PILE SCHEDULE ABOVE FOR PILE REINFORCEMENT. INSTALL SPACERS TO ASSURE PROPER ALIGNMENT. TAKE CARE TO PROTECT THE PILE. CONDUCT AUGERING AND CASTING OF CONCRETE GROUT IN A MANNER THAT ASSURES A MINIMUM 14-INCH DIAMETER CROSS SECTION AT ALL POINTS AND A PILE FREE OF VOIDS. A MINIMUM GROUT FACTOR OF 1.25 SHALL BE ACHIEVED FOR EACH PILE. IF GROUT PUMPING AND/OR AUGER RETRIEVAL OPERATIONS ARE STOPPED AT ANY TIME DURING THE FORMATION OF A PILE, REAUGER BORE HOLE AND REFORM THE PILE. IF THE CONCRETE LEVEL DROPS, IF THE RATE OF GROUT-TAKE CHANGES MARKEDLY, OR IF THE INJECTION PRESSURE MARKEDLY DECREASES AT ANY TIME, THE PILE SHALL BE REJECTED AND REPLACED. PILES SHALL BE PLUMB WITHIN 1-3/4 INCHES. PILES SHALL BE CAST AND THE GROUT SHALL HAVE ACHIEVED INITIAL SET PRIOR TO AUGERING ADDITIONAL PILES WITHIN TEN FEET OF PREVIOUSLY CAST PILES.
- D. SUBMIT CONCRETE GROUT MIX DESIGN TO THE ENGINEER FOR APPROVAL PRIOR TO START OF WORK.
- E. UNDERGROUND UTILITIES: UNDERGROUND UTILITIES EXIST WITHIN THE CONSTRUCTION AREAS. LOCATE UTILITIES BY GROUND PENETRATING RADAR, HAND EXCAVATION OR OTHER APPROPRIATE METHOD AND IDENTIFY THEM ON SHOP DRAWINGS. IF UTILITIES ARE FOUND TO CONFLICT WITH PROPOSED PILE LOCATIONS, NOTIFY ENGINEER FOR FURTHER DIRECTION. DO NOT DAMAGE EXISTING UTILITIES.
- F. TAKE CONCRETE GROUT SAMPLES AT THE TIME OF CONCRETE PLACEMENT. TAKE SAMPLES AT LEAST ONCE PER DAY AND AT LEAST ONCE FOR EACH 50 CUBIC YARDS OF GROUT PLACED. TEST SAMPLES FOR COMPRESSION AT 28 DAYS. THE AVERAGE OF ALL TESTS SHALL EXCEED 5000 PSI WITH NO ONE TEST FALLING BELOW 4700 PSI. SUBMIT TEST REPORTS TO THE ENGINEER.
- G. INSPECTION: ALL AUGER CAST PILE INSTALLATION SHALL BE MONITORED BY AN INSPECTOR AS REQUIRED BY SECTION 1910.3.3 PARAGRAPHS 1922.1.20 AND 21 OF THE F.B.C., 2014. INSPECTOR SHALL SUBMIT TO ENGINEER AND TO THE BUILDING DEPARTMENT A STATEMENT OF COMPLIANCE (SIGNED AND SEALED BY A FLORIDA-REGISTERED ENGINEER) BY AT THE COMPLETION OF PILE INSTALLATION, INCLUDING CERTIFICATION OF ALLOWABLE PILE LOAD CAPACITIES OF 35 TONS COMPRESSION, 15 TONS TENSION AND 7.5 TONS LATERAL.
- H. SURVEY AND TOLERANCES:
1. SURVEY LOCATIONS OF ALL PILE TOPS. SUBMIT SURVEY PLAN TO ENGINEER.
2. TOLERANCES:
a. VARIATION FROM LOCATION: CENTER OF PILE TOPS: 3 INCHES
b. VARIATION FROM PLUMB OR BATTER ANGLE: 1 INCH IN 5 FT. LENGTH

16. **TERMITE PROTECTION:**
- A. EXCEPT FOR AREAS OF EXISTING CONSTRUCTION, PROVIDE TERMITE PROTECTION FOR ALL AREAS OF CONSTRUCTION.
- B. CONFORM TO THE REQUIREMENTS OF SECTION 1816 OF THE FLORIDA BUILDING CODE, 2017
- C. PROVIDE TREATMENT CERTIFICATE TO THE BUILDING DEPARTMENT.
17. **UNDER-SLAB VAPOR-GAS RETARDER:**
- A. PLACE VAPOR-GAS RETARDER SHEET CONTINUOUSLY UNDER ALL CONCRETE SLABS PLACED ON GROUND.
- B. COMPLY WITH ASTM E1745 (LATEST EDITION), CLASS C OR BETTER, WITH MAXIMUM WATER VAPOR PERMEANCE OF 0.1 PERMS (AS DETERMINED IN ACCORDANCE WITH ASTM E96 OR ASTM E154). COORDINATE WITH PROPOSED FLOOR FINISH PRODUCTS, AND PROVIDE A VAPOR-GAS RETARDER SYSTEM WITH A PERMEANCE OF LESS THAN 0.1 PERMS IF RECOMMENDED OR REQUIRED BY THE FLOOR FINISH PRODUCT MANUFACTURER.
- C. SUBMIT PRODUCT DATA SHEETS FOR VAPOR-GAS RETARDER SHEET AND ALL ACCESSORIES, TAPE, ADHESIVES, ETC. AS REQUIRED FOR A COMPLETE INSTALLATION IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND REQUIREMENTS.
- D. COMPLY WITH THE REQUIREMENTS OF THE FLORIDA BUILDING CODE, APPENDICES B, C AND/OR E AS APPLICABLE TO THE PROJECT.
- E. LAP SPlice ALL JOINTS A MINIMUM OF 12-INCHES (OR OTHERWISE AS REQUIRED BY SHEET MANUFACTURER). SEAL ALL LAP JOINTS CONTINUOUSLY W/ PRESSURE-SENSITIVE, MINIMUM 2-INCH WIDE VINYL TAPE (OR OTHERWISE AS REQUIRED BY SHEET MANUFACTURER). SEAL AROUND ALL PENETRATIONS OF VAPOR-GAS RETARDER SHEET BY EXTENDING SHEET TO WITHIN 1/2-INCH OF PENETRATING OBJECT AND SEALING TO OBJECT ALL AROUND WITH MINIMUM 2-INCH WIDE PRESSURE-SENSITIVE VINYL TAPE (OR OTHERWISE AS REQUIRED BY SHEET MANUFACTURER). EXTEND VAPOR-GAS RETARDER SHEET MINIMUM 1-INCH ON TO STEM WALL, GRADE BEAM OR FOOTING (OR OTHERWISE AS REQUIRED BY SHEET MANUFACTURER). IF THIS IS NOT POSSIBLE, ASSURE SHEET CONTACT WITH VERTICAL STRUCTURAL MEMBER AND EXTEND VERTICALLY MINIMUM 1-INCH UP VERTICAL STRUCTURAL MEMBER (BUT NOT MORE THAN ONE HALF OF SLAB THICKNESS AND SEAL WITH MINIMUM 2-INCH WIDE PRESSURE-SENSITIVE VINYL TAPE) (OR OTHERWISE AS REQUIRED BY SHEET MANUFACTURER).
- F. DO NOT DAMAGE SHEET. SUPPORT REINFORCEMENT ON CONCRETE BRICKS OR 6"x6" ASPHALTIC HARDBOARD PADS OR OTHER DEVICES RECOMMENDED BY SHEET MANUFACTURER. EXAMINE VAPOR-GAS RETARDER SHEET PRIOR TO PLACING CONCRETE, AND REPAIR ANY HOLES OR TEARS IN SHEET BY INSTALLING AN OVERLAY SHEET EXTENDING MINIMUM 12-INCHES BEYOND DAMAGED AREA AND SEALED TO LOWER SHEET WITH PRESSURE-SENSITIVE, MINIMUM 2-INCH WIDE VINYL TAPE (OR OTHERWISE AS REQUIRED BY SHEET MANUFACTURER). DO NOT DAMAGE VAPOR-GAS RETARDER SHEET AFTER REPAIRS AND WHILE CASTING CONCRETE.
- G. AFTER SLAB-ON-GROUND HAS CURED, EXAMINE SLAB FOR CRACKS. ALL CRACKS GREATER THAN 1/32" WIDE AND ALL SLAB JOINTS (CONTROL JOINTS AND EXPANSION JOINTS) AND ALL PENETRATIONS SHALL BE SEALED USING A POLYURETHANE, POLYSULFIDE OR EPOXY SEALANT CONFORMING TO ASTM C920. INSTALL SEALANT IN ACCORDANCE WITH ASTM C1193.

18. **CONCRETE (CAST-IN-PLACE):**
- A. STANDARDS: ACI 301, ACI 347, ACI 307, ACI 207, ACI 117, ACI 308.1 & ACI 318 (EXCEPT AS MODIFIED BY FBC 2017, SECTION 1905).
- B. CONCRETE MIXES
1. SUBMIT CONCRETE MIX DESIGNS TO ENGINEER FOR REVIEW PRIOR TO USE.
2. PROPORTION ALL NORMAL-WEIGHT CONCRETE IN ACCORDANCE WITH ACI 301 TO ATTAIN THE FOLLOWING PROPERTIES:
- | CONCRETE LOCATION | COMPRESSIVE STRENGTH | MAXIMUM WATER / CEMENT RATIO | NOTES |
|-------------------|----------------------|------------------------------|-------|
| ALL | 5,000 PSI @ 28 DAYS | 0.40 | |
3. SLUMP SHALL BE 4" (±1") FOR REGULAR MIXES AND NOT GREATER THAN 9" FOR MIXES WITH WATER-REDUCING ADMIXTURES. ADD NO WATER TO THE CONCRETE AT THE SITE, UNLESS OTHERWISE APPROVED BY THE ENGINEER IN ADVANCE AND ONLY IF TEST SAMPLES ARE TAKEN AFTER ADDITION OF THE APPROVED WATER.
4. CEMENTITIOUS MATERIALS:
- a. CEMENT: ASTM C150, TYPE I, UNLESS OTHERWISE NOTED. BLENDED HYDRAULIC CEMENTS (ASTM C595) MAY BE USED WITH PRIOR APPROVAL BY THE ENGINEER.
- b. FLY ASH: ASTM C618, CLASS F.
- c. GROUND GRANULATED BLAST-FURNACE SLAG: ASTM C989 GRADE 100 OR 120
- d. LIMIT PERCENTAGE OF FLY ASH, SLAG OR COMBINATION FLY ASH AND SLAG TO 25% BY WEIGHT OF TOTAL CEMENTITIOUS MATERIALS. UNLESS OTHERWISE NOTED OR APPROVED BY THE ENGINEER.
5. FOR INTERIOR SLABS ON VAPOR-GAS RETARDER SHEET ON GROUND, MAXIMUM WATER CONTENT SHALL BE 33 GALLONS PER CUBIC YARD.
6. LARGE-AGGREGATE PUMP MIXES WITH WATER-REDUCING ADMIXTURES MAY BE USED WITH PRIOR APPROVAL OF THE ENGINEER. PEA ROCK PUMP MIXES WILL NOT BE APPROVED.
7. COARSE AGGREGATE SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-33.
8. WATER: ASTM C-1602.
9. READY-MIX CONCRETE: ASTM C-94.
10. LIMIT WATER SOLUBLE CHLORIDE IONS TO MAXIMUM PERCENTAGE BY WEIGHT OF CEMENT PERMITTED BY ACI 318 OR PCI MNL 116.
- C. CONTRACT AN INDEPENDENT TESTING LABORATORY TO PERFORM THE CONCRETE CYLINDER SAMPLING AND TESTING AS REQUIRED BY SECTION 26.12 OF ACI 318-14. SUBMIT TEST REPORTS TO THE ENGINEER TIMELY.
- D. PROVIDE ALL FORMING AND TEMPORARY SHORING.
- E. DO NOT EMBED PIPES OR CONDUITS EXCEEDING 1/3 THE SLAB THICKNESS IN OUTSIDE DIAMETER IN THE CONCRETE FLOOR WITHOUT THE WRITTEN APPROVAL FROM THE ENGINEER. WHERE PIPES OR CONDUITS ARE PERMITTED, PLACE NO CLOSER THAN THREE DIAMETERS O.C. AND LOCATE SO AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE.
- F. REINFORCEMENT
1. STANDARD: ASTM A-615, GRADE 60.
2. GALVANIZED: ASTM A-615, GRADE 60, ASTM A767 CLASS 1 ZINC COATED AFTER FABRICATION AND BENDING. REPAIR ALL DAMAGED AND UNCOATED AREAS OF HOT-DIP GALVANIZED COATINGS IN ACCORDANCE WITH ASTM A-780. USE GALVANIZED OR EPOXY-COATED WIRE REINFORCEMENT OR ALL PLASTIC REINFORCEMENT SUPPORTS. USE GALVANIZED THE WIRES.
3. WELDED WIRE FABRIC (W.W.F.): ASTM A1064, PLAIN, GALVANIZED FOR ALL EXTERIOR LOCATIONS AND WHERE OTHERWISE INDICATED IN THESE DRAWINGS.
4. REINFORCEMENT PLACEMENT TOLERANCES: COMPLY WITH SECTION 2.2 OF ACI 117-10.
- G. CONCRETE COVER OVER REINFORCEMENT: UNLESS OTHERWISE INDICATED IN THESE DRAWINGS, PROVIDE CONCRETE COVER OVER REINFORCEMENT AS FOLLOWS:
1. 3 INCHES WHERE CAST AGAINST EARTH
2. 2 INCHES FOR NO. 6 AND LARGER AND 1 - 1/2 INCHES FOR NO. 5 AND SMALLER DIAMETER BARS WHERE CAST IN FORMS OR ON VAPOR-GAS RETARDER SHEET AND PERMANENTLY EXPOSED TO WEATHER OR EARTH
3. 1-1/2 INCHES FOR INTERIOR COLUMNS AND BEAMS
4. 3/4-INCH FOR INTERIOR WALLS, SLABS, JOISTS AND STAIRS
- H. STEEL REINFORCEMENT SHOP DRAWINGS:
- SUBMIT REINFORCEMENT PLACING DRAWINGS TO ENGINEER THAT DETAIL BENDING AND PLACEMENT. INCLUDE BAR SIZES, MATERIALS, MATERIAL AND GRADE. INCLUDE DRAWINGS, SCHEDULES AND DIAGRAMAS NECESSARY TO CLEARLY INDICATE ALL BARS, ARRANGEMENT, SPICES, SPACING AND SUPPORTS.
- I. CASTING TOLERANCES: THESE TOLERANCES ARE A MINIMUM STANDARD FOR GENERAL STRUCTURAL PERFORMANCE. PROVIDE MORE STRINGENT TOLERANCES WHERE REQUIRED ELSEWHERE IN THE CONTRACT DOCUMENTS FOR AESTHETICS, FINISH SYSTEMS, EQUIPMENT OR OTHER CONSTRUCTION.
1. COMPLY WITH ALL REQUIREMENTS OF ACI 117-10
2. FLOOR SLABS: "FLAT" IS DEFINED IN 4.5.7 OF ACI 117-10, WITH MAXIMUM 3/16 INCH GAP UNDER A 10 ft. STRAIGHTEDGE PLACED ANYWHERE ON THE SLAB AND ALLOWING IT TO REST UPON ANY TWO HIGH SPOTS. PROVIDE STRICTER STANDARD OF FLATNESS IF RECOMMENDED BY FLOOR FINISH MANUFACTURER OR OTHER NON-STRUCTURAL ITEMS.
- J. MAKE CONSTRUCTION JOINTS IN THE MANNER AND LOCATIONS INDICATED IN THESE DRAWINGS OR AS APPROVED BY THE ENGINEER IN ADVANCE. APPLY ENGINEER-APPROVED BONDING AGENT TO PREVIOUSLY-CAST CONCRETE SURFACES AT VERTICAL CONSTRUCTION JOINTS (SUCH AS BEAM-COLUMN JOINT).
- K. EMBEDMENT OF REINFORCEMENT: UNLESS OTHERWISE INDICATED IN THESE DRAWINGS, EMBED ALL REINFORCING BARS TO THE FAR SIDE (LESS APPROPRIATE COVERAGE) OF CONNECTING AND SUPPORTING MEMBERS.
- L. REINFORCING LAP SPICES
1. UNLESS OTHERWISE INDICATED OR APPROVED BY THE ENGINEER IN ADVANCE, MAKE SPICES FOR REINFORCING BARS BY LAPPING BARS. UNLESS OTHERWISE NOTED, LAP SPICE COLUMN AND WALL BARS ABOVE FLOOR AND FOOTING LEVELS. UNLESS OTHERWISE NOTED FOR BEAMS AND SLABS, PLACE LAP SPICES FOR TOP BARS WITHIN THE MIDDLE THIRD OF THE SPANS AND PLACE LAP SPICES FOR BOTTOM BARS AND INTERMEDIATE BARS CENTERED OVER SUPPORTS, OR WHEN THIS IS NOT POSSIBLE, PLACE THEM WITHIN ONE THIRD OF THE SPAN LENGTH FROM THE CENTERLINE OF THE SUPPORTING COLUMN OR WALL. U.O.N., LENGTHS OF TENSION LAP SPICES SHALL BE IN ACCORDANCE WITH "TENSION DEVELOPMENT AND LAP SPICE LENGTHS FOR REINFORCING BARS" ON DRAWING No. S-1.02.
2. LAP SPICE WELDED WIRE FABRIC MINIMUM ONE FULL MESH DIMENSION.
- M. SAWCUT CONSTRUCTION JOINTS AS SOON AS THE CONCRETE SURFACE WILL NOT BE DAMAGED BY SAWCUTTING, AND NO MORE THAN TWELVE HOURS AFTER CONCRETE PLACEMENT. MAKE SAW-CUT JOINTS WHERE INDICATED.
- N. FINISH ALL CONCRETE IN ACCORDANCE WITH ACI 117-10 AND ACI 301-05, AS INDICATED IN THESE DRAWINGS AND AS APPROPRIATE FOR THE ARCHITECTURAL FINISHES INDICATED IN THE ARCHITECTURAL DRAWINGS. GIVE A LIGHT BROOM FINISH TO ALL EXTERIOR CONCRETE WALKING SURFACES AND TO ALL SURFACES WHICH ARE TO RECEIVE A TILE OR STONE FINISH.
- O. CURE ALL CONCRETE FOR MINIMUM OF SEVEN DAYS. COMPLY WITH ACI 308.1. THE CURING SHALL ENTAIL MAINTENANCE OF THE MOISTURE IN THE CONCRETE. GENERALLY, THIS IS ACCOMPLISHED BY TREATING EXPOSED CONCRETE SURFACES WITH A CHEMICAL CURING COMPOUND IMMEDIATELY AFTER FINISHING AND IMMEDIATELY AFTER REMOVAL OF FORMS. FORMS SHALL BE KEPT MOIST BY FREQUENT WATER SPRAYING PRIOR TO REMOVAL. CURING COMPOUND SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-309. VERIFY COMPATIBILITY OF CURING COMPOUND WITH PROPOSED FINISHES, OR COMPLETELY REMOVE CURING COMPOUND PRIOR TO APPLYING FINISHES. CONTRACTOR MAY USE OTHER CURING METHODS IN ACCORDANCE WITH ACI 308.1.
- P. REINFORCING STEEL ALLOWANCE: PROVIDE AN ALLOWANCE OF 1.0% OF THE TOTAL WEIGHT OF REINFORCING STEEL FOR THE PROJECT (AS DETERMINED FROM THE DRAWINGS) FOR THE ENGINEER TO DIRECT ITS USE AT THE DISCRETION OF THE ENGINEER. PROVIDE A CREDIT TO THE OWNER FOR ANY PORTION OF THIS ALLOWANCE NOT USED.
- Q. ALLOWANCE FOR MITIGATION OF CONCRETE CRACKS: CRACK MITIGATION TECHNIQUES HAVE BEEN INCORPORATED IN THE DESIGN OF THIS PROJECT. HOWEVER, CONCRETE HAS AN INHERENT TENDENCY TO DEVELOP CRACKS. RELATIVELY NARROW CRACKS DO NOT NORMALLY SIGNIFICANTLY IMPAIR THE STRENGTH PERFORMANCE OF THE CONCRETE MEMBERS. IT MAY BE DESIRED TO SEAL OR EPOXY-FILL CRACKS TO PREVENT AIR AND WATER INFILTRATION. APPROXIMATELY TWO YEARS AFTER THE LAST CASTING OF STRUCTURAL CONCRETE, THE CONTRACTOR OR OWNER SHALL RETAIN A PROFESSIONAL ENGINEER TO EXAMINE THE STRUCTURAL CONCRETE MEMBERS, DETERMINE IF CRACKS ARE PRESENT AND DETERMINE IF SEALING AND/OR EPOXY FILLING ARE WARRANTED. CONTRACTOR'S/OWNER'S ENGINEER SHALL PROVIDE SPECIFICATIONS (AND DRAWINGS IF APPROPRIATE) FOR ACTIONS TO BE TAKEN. CONTRACTOR OR OWNER SHALL ASSURE THE RECOMMENDED ACTIONS ARE ACCOMPLISHED.

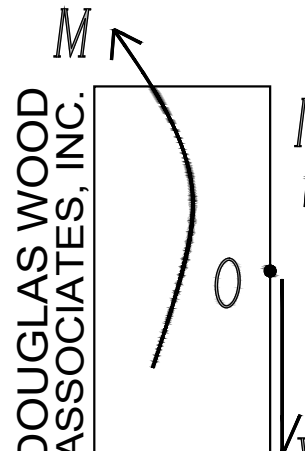
ABBREVIATION	DESCRIPTION	ABBREVIATION	DESCRIPTION
#5	REBAR SIZE	INFO.	INFORMATION
ADD.	ADDITIONAL	INT.	INTERMEDIATE
ADJ.	ADJACENT	K	KIP (1000 LBS)
A.B.F.	ABOVE FINISHED FLOOR	LB. (S)	POUNDS (S)
AHU.	AIR HANDLING UNIT	L.P.	LOW POINT
ALUM.	ALUMINUM	LWC.	LIGHTWEIGHT CONCRETE
ANCH.	ANCHOR	MAX.	MAXIMUM
APPROX.	APPROXIMATE	MECH.	MECHANICAL
ARCH.	ARCHITECTURAL	MFR.	MANUFACTURER
B.E.	BOTH ENDS	MIN.	MINIMUM
B.F.	BOTH FACES	MISC.	MISCELLANEOUS
B.W.	BOTH WAYS (PERPENDICULAR)	MW- X	C.M.U. WALL MARK
BLDG.	BUILDING	N/A	NOT APPLICABLE
BLK.	BLOCKING	N.I.C.	NOT IN CONTRACT
BM.	BEAM	N.T.S.	NOT TO SCALE
BOTT.	BOTTOM	NWC	NORMAL WEIGHT CONCRETE
BRDG.	BRIDGING	O.C.	ON CENTER
BRG.	BEARING	OPNG. (S)	OPENING (S)
BTWN.	BETWEEN	OPP. HAND	OPPOSITE HAND
CANT.	CANTILEVER	PCF	POUNDS PER CUBIC FOOT
C.I.P.	CAST IN PLACE CONCRETE	P/C	PRECAST
C.J.	CONSTRUCTION JOINT OR CONTROL JOINT	P.C.J.	PRECAST JOIST
C.M.U.	CONCRETE MASONRY UNIT	P.C.	PILECAP
C/C	CENTER TO CENTER	PL	PLATE
CL.	CLIMB	PL.F.	POUNDS PER LINEAR FOOT
CLR.	CLEAR DISTANCE OF ELEMENTS	P.P.T.	PRESSURE PRESERVATIVE TREATED
COL.	COLUMN	PRELIM.	PRELIMINARY
CONC.	CONCRETE	PROP.	PROPERTY
CONN.	CONNECTION	PSF	POUNDS PER SQUARE FOOT
CONSTR.	CONSTRUCTION	PSI	POUNDS PER SQUARE INCH
COORD.	COORDINATE	CONT.	CONTINUOUS CONDITION
CORR.	CORRUGATED	COORD.	COORDINATE
CTR.	CENTER	REQD.	REQUIRED
db	BAR DIAMETER	REV.	REVISION
DIA.	DIAMETER	RTU.	ROOF TOP UNIT
DIAG.	DIAGONAL	SCHED.	SCHEDULE
DIM.	DIMENSION	SECT.	SECTION
DIST.	DISTANCE	S.H.	SHEAR HEAD
DN.	DOWN	SMT.	SHEET
DTL.	DETAIL	SIM.	SIMILAR
DWG.	DRAWING	SQ.	SPECIFICATIONS
E.A.	EACH	STAGG.	SQUARE
EL.	EXPANSION JOINT	STD.	STANDARD
E.L.	ELEVATION	STIFF.	STIFFENER
EMBED.	EMBEDMENT	STL.	STEEL
ENGR.	ENGINEER	STIRR.	STIRRUP
E.O.R.	ENGINEER-OF-RECORD	STRUCT.	STRUCTURE OR STRUCTURAL
E.Q.	EQUAL	SW- X	SHREWWALL MARK
EQUIP.	EQUIPMENT	SYM.	SYMMETRICAL CONDITION
E.W.	EACH WAY	T.O.	TOP OF
EXIST.	EXISTING	T.O.B	TOP OF BEAM
EXT.	EXTERIOR	T.O.C	TOP OF CONCRETE
FAB.	FABRICATE	T.O.S	TOP OF SLAB
FDN.	FOUNDATION	T.O.W.	TOP OF WALL
FIN.	FINISH (ED)	TYP.	TYPICAL
FLR.	FLOOR	U.N.O.	UNLESS NOTED OTHERWISE
FTG.	FOOTING	U.O.N.	UNLESS OTHERWISE NOTED
FUTR.	FUTURE	VERT.	VERTICAL
GEN.	GENERAL	V.I.F.	VERIFY IN FIELD
G.L.	GRID LINE	W/P	WATER PROOFING
GALV.	GALVANIZED	WD.	WOOD
H.P.	HIGH POINT	WT.	WEIGHT
HORIZ.	HORIZONTAL	W.W.F	WELDED WIRE FABRIC

DWG. No.	STRUCTURAL DRAWING INDEX
S-1.00	GENERAL STRUCTURAL NOTES, ABBREVIATION LEGEND & STRUCTURAL DRAWING INDEX
S-1.01	CONT. GENERAL STRUCTURAL NOTES
S-1.02	SCHEDULES AND TYP. DETAILS
S-1.03	SCHEDULES AND TYP. DETAILS
S-1.04	SHEAR WALL REINFORCEMENT
S-1.05	TYP. DETAILS
S-1.06	TYPICAL PILE CAP REINFORCEMENT DETAILS
S-2.00	FOUNDATION PLAN
S-2.01A	PARTIAL GROUND FLOOR FRAMING PLAN
S-2.01B	PARTIAL GROUND FLOOR FRAMING PLAN
S-2.02	SECOND FLOOR FRAMING PLAN
S-2.03	THIRD FLOOR FRAMING PLAN
S-2.04	ROOF FRAMING PLAN
S-2.05	HIGH ROOF FRAMING PLAN
S-3.00	SECTIONS AND DETAILS
S-3.01	SECTIONS AND DETAILS
S-3.02	SECTIONS AND DETAILS
S-3.03	SECTIONS AND DETAILS
S-3.04	SECTIONS AND DETAILS
S-4.00	DESIGN WIND PRESSURES FOR ROOFING SYSTEM
S-4.01	DESIGN WIND PRESSURES FOR DOORS, WINDOWS & CLADDING
S-4.02	DESIGN WIND PRESSURES FOR DOORS, WINDOWS & CLADDING
S-4.03	DESIGN WIND PRESSURES FOR DOORS, WINDOWS & CLADDING

RNA

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GREENPARK GROUP

PROJECT NUMBER: 19023
GENERAL STRUCT.
NOTES,
ABBREVIATION
LEGEND & STRUCT.
DWG. INDEX

DESIGNED BY:

DATE:

ISSUED FOR

- ☐ REVIEW & COMMENTS
☐ REWORK
☐ NOT FOR CONSTRUCTION
☒ PERMIT
☐ CONSTRUCTION

S-1.00

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11/15/2019

CONSTRUCTION DOCUMENTS

CONT. GENERAL STRUCTURAL NOTES

19. REINFORCED CONCRETE MASONRY:

- A. HOLLOW CONCRETE MASONRY UNITS: ASTM C90 "STANDARD SPECIFICATIONS FOR HOLLOW LOAD BEARING CONCRETE MASONRY UNITS." MINIMUM ALLOWABLE COMPRESSIVE STRESS ON NET AREA SHALL BE $f_c = 2,000$ PSI. OR AS OTHERWISE REQUIRED. C.M.U. SHALL MEET THE MINIMUM EQUIVALENT THICKNESS FOR 2-HOUR FIRE-RATED WALLS AS INDICATED IN TABLE 721.1(2) OF THE FLORIDA BUILDING CODE 2017. SUBMIT CERTIFICATES OF FIRE RATING FROM C.M.U. MANUFACTURER.
- B. REINFORCED CONCRETE MASONRY PRISM STRENGTH SHALL BE MINIMUM $f_c = 2,000$ PSI, AND SHALL BE VERIFIED BY CERTIFICATION OF MASONRY UNIT STRENGTH AND FIELD SAMPLING AND LABORATORY TESTING OF MORTAR AND GROUT. ALTERNATIVELY, PRISM STRENGTH MAY BE VERIFIED BY PRISM TESTING IN ACCORDANCE WITH ASTM E447, METHOD B AND AS OTHERWISE MODIFIED IN ACI 530.1, 1.4.
- C. MORTAR SHALL COMPLY WITH ASTM C270, TYPE M OR S.
- D. REINFORCEMENT BARS: ASTM A615, GRADE 60.
- E. PLACE CONCRETE MASONRY UNITS: IN STANDARD RUNNING BOND PATTERN. BOND MASONRY WITH OVERLAPPING UNITS AT WALL CORNERS AND INTERSECTIONS, UNLESS OTHERWISE NOTED. ALL MORTAR JOINTS SHALL BE 3/8" THICK, WHERE PERMANENTLY EXPOSED TO VIEW. TOOL MORTAR JOINTS WITH ROUNDED JOINTER. FOR OTHER LOCATIONS, STRIKE JOINTS FLUSH WITH C.M.U. FACES. FULLY MORTAR FACE SHELLS AT ALL HORIZONTAL AND VERTICAL JOINTS. FULLY MORTAR WEB SHELLS AS WELL AT FOUNDATION. FULLY MORTAR WEB SHELLS AT ALL CELLS WHICH ARE TO BE GROUTED. UNUSED MORTAR SHALL BE DISCARDED WITHIN 2 1/2 HOURS AFTER INITIAL MIXING.
- F. WHEREVER ANCHOR BOLTS IS TO BE SET IN MASONRY, FILL C.M.U. CELLS WITH CONCRETE GROUT FOR MINIMUM 8" INCHES ALL AROUND BOLT.
- G. INSTALL HOT DIPPED GALVANIZED (EXTERIOR WALLS) OR MILL GALVANIZED COATED (INTERIOR WALLS) STEEL HORIZONTAL JOINT REINFORCEMENT (No. 9 GAGE, LADDER TYPE) AT EVERY COURSE IN PARAPET WALLS AND AT EVERY OTHER COURSE AT ALL OTHER WALLS (U.O.N.). EXTEND HORIZONTAL JOINT REINFORCEMENT MINIMUM 4" INCHES INTO ALL CONCRETE THE COLUMNS AND WALLS. PROVIDE PREFABRICATED "L" AND "T" SHAPED REINFORCEMENT UNITS AT ALL CORNERS AND INTERSECTIONS RESPECTIVELY. LAP SPlice REINFORCEMENT MINIMUM SIX INCHES.
- H. GROUT: MINIMUM $f_c = 3000$ PSI (OR AS OTHERWISE REQUIRED TO ACHIEVE SPECIFIED f_c WHEN VERIFYING MASONRY STRENGTH BY PRISM TESTING) CONCRETE GROUT CONFORMING TO THE REQUIREMENTS OF ASTM C476.
- I. COMPLY WITH THE REQUIREMENTS OF SECTION 2122 OF THE FLORIDA BUILDING CODE 2017 AND THE REQUIREMENTS OF ACI 530 AND ACI 530.1.
- J. LINTELS: ANY C.M.U. WALL OPENINGS WHICH ARE NOT OTHERWISE FRAMED WITH CONCRETE SHALL HAVE 8"x8" CONCRETE LINTELS OR GROUTED LINTEL BLOCK WITH 2#5 BOTTOM. EXTEND REINFORCEMENT MINIMUM 6" PAST OPENING AND CAST LINTEL WITH MINIMUM 8" BEARING ON ADJACENT COLUMNS OR REINFORCED CELLS.
- K. SILLS: UNLESS OTHERWISE FRAMED IN CONCRETE, PROVIDE 8" X 8" CONCRETE (FORMED OR BOND-BEAM BLOCK) WITH 2#5 MID-DEPTH AT ALL SILLS. EXTEND REINFORCEMENT MINIMUM 6" INTO ADJACENT COLUMN OR REINFORCED CELL AND CAST CONCRETE/GROUT MINIMUM 8" INTO ADJACENT COLUMN OR REINFORCED CELL.
- L. REINFORCEMENT PLACEMENT:
1. PLACE ALL HORIZONTAL BARS IN BOND BEAM UNITS. WHEN TWO BARS ARE USED, STAGGER LAPS A MINIMUM OF 5'-0".
 2. VERTICAL REINFORCING SHALL BE HELD IN POSITION USING REBAR POSITIONERS AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 200 BAR DIAMETERS.
 3. FOR PARTIALLY GROUTED WALLS, METAL LATH STRIPS, CELL CAPS OR EQUAL SHALL BE USED AS GROUT STOPS AT THE BOTTOM OF STRUCTURAL CONCRETE BEARING ON C.M.U. AND AT THE BOTTOM OF C.M.U. COURSES TO BE FILLED FOR THE EMBEDMENT OF HORIZONTAL BARS. DO NOT USE PAPER OR OTHER UNAPPROVED MATERIAL.
 4. LAP SPlice BARS ONLY WHERE NECESSARY. ALL LAP SPlice LENGTHS SHALL BE A MINIMUM OF THOSE IN "REBAR DEVELOPMENT AND LAP SPlice LENGTH "Ld" SCHEDULE FOR MASONRY" ON DRAWING S-1.01.
- M. PROVIDE CLEANOUT HOLES AT BOTTOMS OF ALL VERTICALLY REINFORCED CELLS. REMOVE ALL DEBRIS AND EXCESS MORTAR. INSPECT FOR COMPLETE GROUT PLACEMENT UPON REMOVAL OF FORM OVER CLEANOUT HOLE.
- N. GROUT LIFT HEIGHT: PLACE GROUT IN LIFTS NOT EXCEEDING 5 FEET AND A MAXIMUM PLACEMENT OF 12 FEET 8 INCHES.
- O. GROUT CONSOLIDATION: CONSOLIDATE ALL GROUT BY MECHANICAL VIBRATION USING A 3/4" Ø HEAD LOW-VELOCITY VIBRATOR FOR A COUPLE OF SECONDS IN EACH FILLED CELL. RECONSOLIDATION BY VIBRATION MUST BE DONE AFTER THE INITIAL WATER LOSS AND BEFORE INITIAL SET HAS OCCURRED. WHEN GROUTING IS STOPPED FOR A PERIOD OF 1 HOUR OR LONGER, FORM HORIZONTAL CONSTRUCTION JOINTS BY STOPPING THE GROUT POUR 1-1/2 INCHES MINIMUM BELOW THE UPPERMOST UNIT, EXCEPT AT TOP OF WALL.
- P. PROVIDE MIN. 20 GAGE, GALVANIZED STEEL CONTINUOUS DOVETAIL SLOTS IN ADJACENT CONCRETE COLUMNS WHERE C.M.U. IS INSTALLED AFTER COLUMNS. ANCHOR C.M.U. TO CONCRETE COLUMNS USING MIN. 16 GAGE GALVANIZED CORRUGATED STEEL DOVETAIL MASONRY ANCHORS AT EVERY OTHER COURSE OF C.M.U. IF DOVETAIL SLOTS ARE DAMAGED OR MISPLACED AND CANNOT BE USED, REMOVE SLOTS, PATCH CONCRETE AND INSTALL MASONRY ANCHORS WITH MIN. (1) 3/4" Ø TAPCON SCREW WITH MIN. 1 3/4" EMBED. CAST ALL THE COLUMNS (AND WALLS WHICH ARE) AS THE COLUMNS, TIE BEAMS, LINTELS AND SILLS DIRECTLY INTO OR ON TO C.M.U. UNITS SO THAT THEY ARE FULLY ENGAGED WITH C.M.U. USE FABRICATED NON-CORROSIVE CELL CAPS WHERE CONCRETE IS CAST ON TOPS OF UN-GROUTED CELLS.
- R. DO NOT EMBED PIPES, CONDUIT, OR ANY OTHER CONSTRUCTION IN MASONRY WALLS WITHOUT PRIOR APPROVAL OF ENGINEER.

20. STRUCTURAL STEEL:

- A. MATERIALS:
1. STRUCTURAL TUBING: ASTM A500, GRADE B
 2. STEEL PIPE: ASTM A53, TYPE E OR S, GRADE B
 3. W-SHAPES: ASTM A992
 4. ALL OTHER STRUCTURAL STEEL: ASTM A36 (U.O.N.)
 5. ANCHOR BOLTS: ASTM A307
 6. THREADED RODS: ASTM A36
 7. HEADED SHEAR STUDS: AWS D1.1, TYPE B
 8. ALL OTHER BOLTS: ASTM A325-09
 9. NUTS: ASTM A563, GRADE C
 10. WASHERS: ASTM F436, TYPE I
- B. SHOP DRAWINGS: SUBMIT COMPLETE SHOP DRAWINGS FOR STRUCTURAL STEEL FOR REVIEW BY ENGINEER PRIOR TO FABRICATION.
- C. STANDARDS:
1. AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL BUILDINGS."
 2. AWS D1-1, E-70 SERIES ELECTRODES. ALL WELDERS SHALL BE CERTIFIED BY AWS FOR THE MATERIALS AND FOR THE WELD TYPES, SIZES AND ORIENTATIONS INDICATED IN THESE DRAWINGS. SUBMIT WELDER CERTIFICATIONS TO ENGINEER.
- E. CORROSION CONTROL: EXCEPT WHERE STRUCTURAL STEEL IS INDICATED TO BE GALVANIZED, WHERE STRUCTURAL STEEL IS TO BE EMBEDDED IN CONCRETE, WHERE STRUCTURAL STEEL IS TO BE FIELD WELDED AND WHERE STRUCTURAL STEEL IS TO RECEIVE DIRECTLY APPLIED FIREPROOFING MATERIALS, SHOP PAINT ALL STRUCTURAL STEEL WITH RUST-INHIBITIVE PRIMER. TOUCH-UP PRIMER AFTER ERECTION.
- F. CORROSION CONTROL: HOT DIP GALVANIZE STRUCTURAL STEEL (AND ALL ADJOINING CLIPS AND BRACKETS) AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 AND A563. WELDED JOINTS SHALL BE MADE WITH LOW-SILICON ELECTRODES (<0.30%) AND ALL SLAG AND FLUX FROM WELDS MUST BE REMOVED PRIOR TO GALVANIZING. FOR WELDING OF GALVANIZED MEMBERS, REMOVE THE ZINC COATING AT LEAST ONE TO FOUR INCHES FROM BOTH SIDES OF THE INTENDED WELD ZONE ON BOTH PIECES PRIOR TO WELDING ACCORDING TO AWS D-19.0. TOUCH-UP AND REPAIR ALL DAMAGED AND UNCOATED AREAS OF HOT-DIP GALVANIZED COATINGS AFTER ERECTION AND/OR WELDING WITH ZINC-RICH PAINT IN ACCORDANCE WITH ASTM A-780.
- G. QUALITY CONTROL (QC) AND QUALITY ASSURANCE (QA) OF WELDED AND BOLTED CONNECTIONS SHALL BE AS REQUIRED BY AISC 360, CHAPTER N, AND ITS REFERENCED STANDARDS. CONTRACTOR SHALL INCLUDE IN BASE BID ALL ASSOCIATED COSTS OF QC INSPECTION TASKS. CONTRACTOR SHALL COVER THE ASSOCIATED COSTS OF QA INSPECTION TASKS AND NON-DESTRUCTIVE TESTING, WHICH SHALL INCLUDE 10% OF CJP GROOVE WELDS BY ULTRASONIC OR RADIOGRAPHIC METHOD, AND 10% OF FILLET WELDS BY MAGNETIC PARTICLE OR DYE PENETRANT METHOD. REPAIR OF NON-CONFORMANCES SHALL BE AT CONTRACTOR'S EXPENSE.

21. FASTENERS AND ANCHORS:

- A. FASTENERS AND ANCHORS SHALL BE OF THE TYPE AND SIZE INDICATED IN THESE DRAWINGS. USE THE SPECIFIC MANUFACTURER AND MODEL WHERE INDICATED. STRICTLY FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR INSTALLATION.
- B. ALL FASTENERS, INCLUDING EXPANSION ANCHORS, SLEEVE ANCHORS, STRAPS, NAILS, SCREWS, ETC. SHALL BE GALVANIZED, OR OTHERWISE COATED FOR CORROSION CONTROL BY A METHOD APPROVED BY THE ENGINEER.

22. EPOXY ANCHORS:

USE TWO COMPONENT EPOXY BASED PRODUCT SUPPLIED AS A SIDE-BY-SIDE CARTRIDGE SYSTEM AND USE TWO COMPONENT EPOXY BASED PRODUCT SUPPLIED AS A SIDE-BY-SIDE CARTRIDGE SYSTEM AND SUBSTITUTE. FOLLOW RECOMMENDATIONS AND SPECIFICATIONS FOR PREPARATION AND INSTALLATION INDICATED IN PRODUCT'S ICC-ES EVALUATION REPORT (ESR) OR STATE OR COUNTY PRODUCT APPROVAL, INCLUDING THE NUMBER OF TIMES HOLES SHALL BE WIRE-BRUSHED AND BLOWN WITH COMPRESSED AIR. ALL HOLES MUST BE OBSERVED BY THE PROJECT INSPECTOR PRIOR TO INSTALLATION OF EPOXY. ONCE EPOXY CURING TIME HAS ELAPSED, SET CONNECTION TO SPECIFIED MINIMUM INSTALLATION TORQUE USING AN ADJUSTABLE TORQUE-LIMITING WRENCH. DO NOT OVER-TORQUE ANCHORS NOR USE AIR-POWERED OR ELECTRIC WRENCHES.

23. DELEGATED ENGINEERED SYSTEMS:

- A. DELEGATED ENGINEERED SYSTEMS FOR THIS PROJECT CONSIST OF THE FOLLOWING:
1. PREFABRICATED STEEL STAIRS (REFER TO "GENERAL STRUCTURAL NOTES" No. 24 FOR ADDITIONAL INFORMATION).
 2. THE DELEGATED ENGINEER SHALL CONFORM TO CURRENT RULES ESTABLISHED BY THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS.
 3. THE DELEGATED ENGINEER SHALL BE RETAINED BY THE DELEGATED SYSTEM SUPPLIER OR MAY BE AN EMPLOYEE OF THE SUPPLIER.
 4. INCLUDE ALL COSTS FOR THE DELEGATED ENGINEER IN THE CONSTRUCTION BID.
 5. DELEGATED ENGINEER SHALL DESIGN DELEGATED SYSTEM IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2017, ALL OTHER APPLICABLE CODES AND ORDINANCES AND REFERENCED STANDARDS AND AT LEAST IN ACCORDANCE WITH THE STANDARD OF CARE EXERCISED BY OTHER PROFESSIONALS ON SIMILAR PROJECTS. DELEGATED ENGINEER MUST BE A LICENSED ENGINEER IN THE STATE OF FLORIDA AND MUST BE EXPERIENCED AND COMPETENT IN THE DESIGN OF THE DELEGATED SYSTEM.
 6. DELEGATED ENGINEER SHALL ANALYZE AND DESIGN THE DELEGATED SYSTEM AND PREPARE SHOP DRAWINGS FOR THE DELEGATED SYSTEM. SHOP DRAWINGS SHALL INDICATE LAYOUT, DIMENSIONS, MATERIALS, MATERIALS PROPERTIES, MEMBER SIZES, CONNECTIONS, ANCHORAGES, INSTALLATION INFORMATION AND ALL OTHER INFORMATION TO CLEARLY CONVEY THE SYSTEM IN ITS ENTIRETY AND TO ASSURE PROPER FABRICATION AND INSTALLATION. THE DELEGATED ENGINEER SHALL SUBMIT SIGNED AND SEALED DRAWINGS AND DESIGN CALCULATIONS TO THE ENGINEER-OF-RECORD AND TO THE BUILDING DEPARTMENT. DO NOT INSTALL ANY COMPONENTS OF THE DELEGATED SYSTEM UNTIL DRAWINGS AND CALCULATIONS HAVE BEEN REVIEWED AND ACCEPTED BY BOTH THE ENGINEER-OF-RECORD AND THE BUILDING DEPARTMENT.

24. PREFABRICATED STEEL STAIRS

- A. PREFABRICATED STEEL STAIRS ARE A DELEGATED SYSTEM (REFER TO "GENERAL STRUCTURAL NOTES" No. 23 FOR ADDITIONAL INFORMATION).
- B. REFER TO ARCHITECTURAL DRAWINGS FOR ARCHITECTURAL INFORMATION, INCLUDING BUT NOT LIMITED TO LAYOUT, RISE AND RUN OF TREADS, WIDTHS, LANDING DIMENSIONS, HANDRAILS, GUARDRAILS AND HEADROOM CLEARANCES.
- C. DESIGN FOR A SUPERIMPOSED LIVE LOAD OF 40 PSF.
- D. CONFORM TO THE REQUIREMENTS OF "GENERAL STRUCTURAL NOTES" No. 20, STRUCTURAL STEEL.

25. LIGHTWEIGHT CONCRETE:

- A. THE EXTENT OF THE LIGHTWEIGHT CONCRETE IS INDICATED IN THESE DRAWINGS.
- B. PROPORTION LIGHTWEIGHT CONCRETE TO ATTAIN A 28-DAY COMPRESSIVE STRENGTH OF 5000 PSI AND A MAXIMUM DENSITY OF 115 POUNDS PER CUBIC FOOT.
- C. LIGHTWEIGHT AGGREGATE SHALL CONFORM TO THE REQUIREMENTS OF ASTM C330.
- D. THE REQUIREMENTS OF NOTE NO. 18 "CONCRETE CAST-IN-PLACE", ALSO APPLY TO LIGHTWEIGHT CONCRETE.

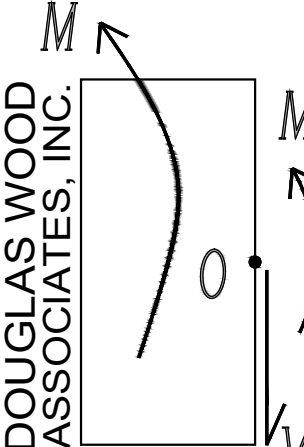
26. SHOTCRETE:

- A. STANDARDS: ACI 506R AND ACI 506.2
- B. INSTALLER QUALIFICATION: SHOTCRETING CONTRACTOR SHALL HAVE A MINIMUM OF FIVE YEARS OF SUCCESSFUL EXPERIENCE IN SHOTCRETING. SHOTCRETE NOZZLE OPERATOR SHALL BE CERTIFIED IN ACCORDANCE WITH ACI CP-60 AND SHALL HAVE COMPLETED AT LEAST ONE SIMILAR APPLICATION AS A NOZZLE OPERATOR ON A SIMILAR PROJECT. A QUALIFIED SHOTCRETING CREW SHALL ATTAIN MEAN CORE GRADES NOT EXCEEDING 2.5, IN ACCORDANCE WITH ACI 506.2, ON PRECONSTRUCTION TESTS.
- C. MATERIALS: SHOTCRETE PROPORTIONS SHALL BE SELECTED THAT ALLOW SUITABLE PLACEMENT PROCEDURES USING THE DELIVERY EQUIPMENT SELECTED AND SHALL RESULT IN FINISHED IN-PLACE HARDENED SHOTCRETE MEETING THE STRENGTH REQUIREMENTS SPECIFIED BELOW.
1. CEMENT: ASTM C150 OR C595.
 2. AGGREGATE: ASTM C33, GRADING AS RECOMMENDED IN ACI 506R FOR COMBINED AGGREGATE GRADING WITH MAXIMUM SIZE AGGREGATE PASSING A 1/2" SIEVE.
3. REINFORCEMENT:
- a. STEEL REINFORCEMENT SHOP DRAWINGS: SUBMIT REINFORCEMENT PLACING DRAWINGS TO ENGINEER THAT DETAIL BENDING AND PLACEMENT. INCLUDE BAR SIZES, LENGTHS, MATERIAL AND GRADE. INCLUDE DRAWINGS, SCHEDULES AND DIAGRAM AS NECESSARY TO CLEARLY INDICATE ALL BARS, ARRANGEMENT, SPLICES, SPACING AND SUPPORTS. INDICATE LOCATIONS OF PROPOSED CONSTRUCTION JOINTS.
 - b. SIZE: THE MAXIMUM SIZE OF REINFORCEMENT SHALL BE NO. 5 BARS UNLESS OTHERWISE SPECIFIED IN THESE DRAWINGS, IN WHICH CASE, IT SHALL BE DEMONSTRATED BY PRE-CONSTRUCTION TESTS THAT ADEQUATE ENCASEMENT OF LARGER BARS WILL BE ACHIEVED.
 - c. CLEARANCE: WHEN NO. 5 OR SMALLER BARS ARE USED, THERE SHALL BE A MINIMUM CLEARANCE BETWEEN PARALLEL REINFORCEMENT BARS OF 2-1/2 INCHES. WHEN BARS LARGER THAN NO. 5 ARE PERMITTED, THERE SHALL BE A MINIMUM CLEARANCE BETWEEN PARALLEL BARS EQUAL TO SIX DIAMETERS OF THE BARS USED. WHEN TWO CURTAINS OF STEEL ARE PROVIDED, THE CURTAIN NEARER THE NOZZLE SHALL HAVE A MINIMUM SPACING EQUAL TO 12 BAR DIAMETERS AND THE REMAINING CURTAIN SHALL HAVE A MINIMUM SPACING OF SIX BAR DIAMETERS. EXCEPTION: IF THE ABOVE SAID CLEARANCES CANNOT BE GEOMETRICALLY ACHIEVED, DEMONSTRATE BY PRE-CONSTRUCTION TESTS THAT ADEQUATE ENCASEMENT OF THE BARS USED IN THE DESIGN WILL BE ACHIEVED. SUBMIT TEST RESULTS TO ENGINEER AND TO BUILDING OFFICIAL FOR APPROVAL PRIOR TO COMMENCEMENT OF WORK.
 - d. SPLICES: LAP SPLICES OF REINFORCING BARS SHALL UTILIZE THE NON-CONTACT LAP SPlice METHOD WITH A MINIMUM CLEARANCE OF 2 INCHES BETWEEN BARS. EXCEPTION: THE USE OF CONTACT LAP SPLICES IS PERMITTED WHEN IT HAS BEEN DEMONSTRATED BY PRE-CONSTRUCTION TESTS THAT ADEQUATE ENCASEMENT OF THE BARS USED IN THE DESIGN WILL BE ACHIEVED, AND PROVIDED THAT THE SPlice IS ORIENTED SO THAT A PLANE THROUGH THE CENTER OF THE SPliced BARS IS PERPENDICULAR TO THE SURFACE OF THE SHOTCRETE. SUBMIT TEST RESULTS TO ENGINEER AND TO BUILDING OFFICIAL FOR APPROVAL PRIOR TO COMMENCEMENT OF WORK.
4. ADD AIR-ENTRAINING ADMIXTURE AT MANUFACTURER'S PRESCRIBED RATE TO RESULT IN NORMAL-WEIGHT WET-MIX SHOTCRETE HAVING AN AIR CONTENT BEFORE PUMPING OF 7 PERCENT WITH A TOLERANCE OF PLUS OR MINUS 1-1/2 PERCENT.
5. REFER TO GENERAL STRUCTURAL NOTE "CONCRETE (CAST-IN-PLACE)" FOR ADDITIONAL INFORMATION.
- D. STRENGTH: $f_c = 5000$ PSI AT 28 DAYS (OR AS OTHERWISE NOTED).
- E. PRE-CONSTRUCTION SAMPLE: EACH SHOTCRETING CREW SHALL CREATE A SAMPLE PANEL AT LEAST TEN DAYS PRIOR TO THE COMMENCEMENT OF THE WORK. THE SAMPLE PANEL SHALL BE REPRESENTATIVE OF THE PROJECT AND SIMULATE JOB CONDITIONS AS CLOSELY AS POSSIBLE. THE SAMPLE PANELS SHALL HAVE MINIMUM DIMENSIONS OF 24"x24". THE PANEL THICKNESS AND REINFORCING SHALL REPRODUCE THE THICKEST AND MOST CONGESTED AREA (INCLUDING LAP SPLICES) SPECIFIED IN THESE STRUCTURAL DRAWINGS. IT SHALL BE SHOT AT THE SAME ANGLE AND WITH THE SAME CONCRETE MIX DESIGN THAT WILL BE USED ON THE PROJECT. THE EQUIPMENT USED IN PRE-CONSTRUCTION TESTING SHALL BE THE SAME EQUIPMENT USED IN THE WORK REQUIRING SUCH TESTING. UNLESS SUBSTITUTE EQUIPMENT IS APPROVED BY THE BUILDING OFFICIAL, SUBMIT SHOP DRAWING FOR SAMPLE PANEL FOR REVIEW AND APPROVAL BY THE ENGINEER ALONG WITH REINFORCEMENT SHOP DRAWING.
- F. PRE-CONSTRUCTION TESTING: FROM EACH TEST PANEL, TESTING AGENCY SHALL OBTAIN SIX TEST SPECIMENS: ONE SET OF THREE SPECIMENS UNREINFORCED, AND ONE SET OF THREE SPECIMENS REINFORCED. AGENCY WILL PERFORM THE FOLLOWING:
1. STRENGTH TESTING: TEST EACH SET OF UNREINFORCED SPECIMENS FOR COMPRESSIVE STRENGTH ACCORDING TO ASTM C 42.
 2. CORE GRADING: VISUALLY INSPECT EACH SET OF REINFORCED SHOTCRETE CORES TAKEN FROM TEST PANELS AND DETERMINE MEAN CORE GRADES ACCORDING TO ACI 506.2.
 3. REPORTS OF PRE-CONSTRUCTION TESTS SHALL BE SUBMITTED TO ENGINEER AND TO BUILDING OFFICIAL FOR REVIEW AND APPROVAL PRIOR TO COMMENCEMENT OF WORK.
 4. FORMING: TWO ADJACENT SIDES OF EACH COLUMN (OR BEAM) SHALL BE FORMED. THE OTHER TWO SIDES SHALL NOT BE FORMED.
- H. APPLICATION:
1. APPLY SHOTCRETE ACCORDING TO ACI 506.2. APPLY DRY-MIX SHOTCRETE MATERIALS WITHIN 45 MINUTES AFTER PRE-PANING. APPLY SHOTCRETE MATERIALS WITHIN 90 MINUTES AFTER BATCHING. DEPOSIT SHOTCRETE CONTINUOUSLY IN MULTIPLE PASSES, TO REQUIRED THICKNESS, WITHOUT COLD JOINTS AND LAMINATIONS DEVELOPING. REMOVE REBOUND FROM THE WORK AREA AS IT OCCURS. DO NOT REUSE REBOUND. DO NOT DISTURB SHOTCRETE SURFACES BEFORE BEGINNING FINISHING OPERATIONS.
 2. INSTALLATION TOLERANCES: PLACE SHOTCRETE WITHOUT EXCEEDING INSTALLATION TOLERANCES PERMITTED BY ACI 117, INCREASED BY A FACTOR OF TWO (UNLESS STRICTER TOLERANCES ARE REQUIRED APPROPRIATE FOR ARCHITECTURAL FINISHES OR OTHER SYSTEMS).
 3. IN-PLACE SHOTCRETE THAT EXHIBITS SAGS, SLOUGHS, SEGREGATION, HONEYCOMBING, SAND POCKETS OR OTHER OBVIOUS DEFECTS SHALL BE REMOVED AND REPLACED. REMOVE SHOTCRETE ABOVE SAGS AND SLOUGHS AND REPLACE WHILE STILL PLASTIC.
 4. HOT-WEATHER SHOTCRETING: MIX, PLACE, AND PROTECT SHOTCRETE ACCORDING TO ACI 305.1 WHEN HOT-WEATHER CONDITIONS AND HIGH TEMPERATURES WOULD SERIOUSLY IMPAIR QUALITY AND STRENGTH OF SHOTCRETE, AND AS FOLLOWS:
 - a. COOL INGREDIENTS BEFORE MIXING TO MAINTAIN, AT TIME OF PLACEMENT, SHOTCRETE TEMPERATURE BELOW 100 DEG F FOR DRY MIX AND BELOW 90 DEG F FOR WET MIX.
 - b. REDUCE TEMPERATURE OF REINFORCING STEEL AND RECEIVING SURFACES BELOW 100 DEG F BEFORE SHOTCRETING.
- I. CONSTRUCTION JOINTS: EXCEPT WHERE PERMITTED HEREIN, UNFINISHED WORK SHALL NOT BE ALLOWED TO STAND FOR MORE THAN 30 MINUTES UNLESS EDGES ARE SLOPED TO A THIN EDGE. LOCATE AND INSTALL CONSTRUCTION JOINTS TAPERED TO A 1:1 SLOPE WHERE JOINT IS NOT SUBJECT TO COMPRESSION LOADS. FOR STRUCTURAL ELEMENTS THAT WILL BE UNDER COMPRESSION (COLUMNS AND WALLS) AND FOR CONSTRUCTION JOINTS SHOWN ON THE APPROVED CONSTRUCTION DOCUMENTS, SQUARE JOINTS ARE PERMITTED. BEFORE PLACING ADDITIONAL MATERIAL ADJACENT TO PREVIOUSLY APPLIED WORK, SLOPING AND SQUARE EDGES SHALL BE CLEANED AND WETTED. CONTINUE REINFORCEMENT THROUGH CONSTRUCTION JOINTS UNLESS OTHERWISE INDICATED.
- J. CURING:
1. INITIAL CURING: SHOTCRETE SHALL BE KEPT CONTINUOUSLY MOIST FOR 24 HOURS AFTER SHOTCRETING IS COMPLETE OR SHALL BE SEALED WITH AN APPROVED CURING COMPOUND.
 2. FINAL CURING: FINAL CURING SHALL CONTINUE FOR SEVEN DAYS AFTER SHOTCRETING, OR FOR THREE DAYS IF HIGH-EARLY-STRENGTH CEMENT IS USED, OR UNTIL THE SPECIFIED STRENGTH IS OBTAINED. FINAL CURING SHALL CONSIST OF THE INITIAL CURING PROCESS OR THE SHOTCRETE SHALL BE COVERED WITH AN APPROVED MOISTURE-RETAINING COVER.
- K. QUALITY CONTROL: IN ADDITION TO THE PRE-CONSTRUCTION SAMPLE, EACH SHOTCRETING CREW SHALL CREATE A SAMPLE, 24"x24"x6", EACH DAY, BUT NOT LESS THAN ONE FOR EACH 50 CUBIC YARDS OF SHOTCRETE. PANELS SHALL BE SHOT IN THE SAME POSITION AS THE WORK, DURING THE COURSE OF THE WORK AND BY THE NOZZLEMEN DOING THE WORK. THE CONDITIONS UNDER WHICH THE PANELS ARE CURED SHALL BE THE SAME AS THE WORK. TAKE CORE SAMPLES FROM THESE DAILY SAMPLE PANELS AT 7 AND 28 DAYS AND TEST IN ACCORDANCE WITH ASTM C42. SUBMIT TEST REPORTS TO ENGINEER.
1. STRENGTH TESTS: STRENGTH TESTS FOR SHOTCRETE SHALL BE MADE BY AN APPROVED AGENCY ON SPECIMENS THAT ARE REPRESENTATIVE OF THE WORK AND WHICH HAVE BEEN WATER SOAKED FOR AT LEAST 24 HOURS PRIOR TO TESTING. WHEN THE MAXIMUM-SIZE AGGREGATE IS LARGER THAN 3/8 INCH, SPECIMENS SHALL CONSIST OF NOT LESS THAN THREE 3-INCH-DIAMETER CORES. WHEN THE MAXIMUM-SIZE AGGREGATE IS 3/8 INCH OR SMALLER, SPECIMENS SHALL CONSIST OF NOT LESS THAN THREE 1-INCH-DIAMETER CORES.
2. ACCEPTANCE CRITERIA: THE AVERAGE COMPRESSIVE STRENGTH OF THREE CORES FROM A SINGLE TEST PANEL SHALL EQUAL OR EXCEED 0.85 f_c WITH NO SINGLE CORE LESS THAN 0.75 f_c TO CHECK ACCURACY, LOCATIONS REPRESENTED BY ERRATIC CORE STRENGTHS SHALL BE RETESTED.

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GREENPARK GROUP

PROJECT NUMBER: 19023

CONT. GENERAL
STRUCTURAL
NOTES

DESIGNED BY:

DATE:

ISSUED FOR

- ☐ REVIEW & COMMENTS
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CONSTRUCTION DOCUMENTS

11/15/2019

FOOTING SCHEDULE							
MARK	SIZE	THICKNESS	BOTT. REINFORCEMENT		TOP REINFORCEMENT		REMARKS
			SHORT WAY	LONG WAY	SHORT WAY	LONG WAY	
WF-36	3'-0" x CONT.	16"	#5 @ 10" O.C.	#5 @ 10" O.C.	--	--	--
WF-48	4'-0" x CONT.	16"	#5 @ 10" O.C.	#5 @ 10" O.C.	--	--	--

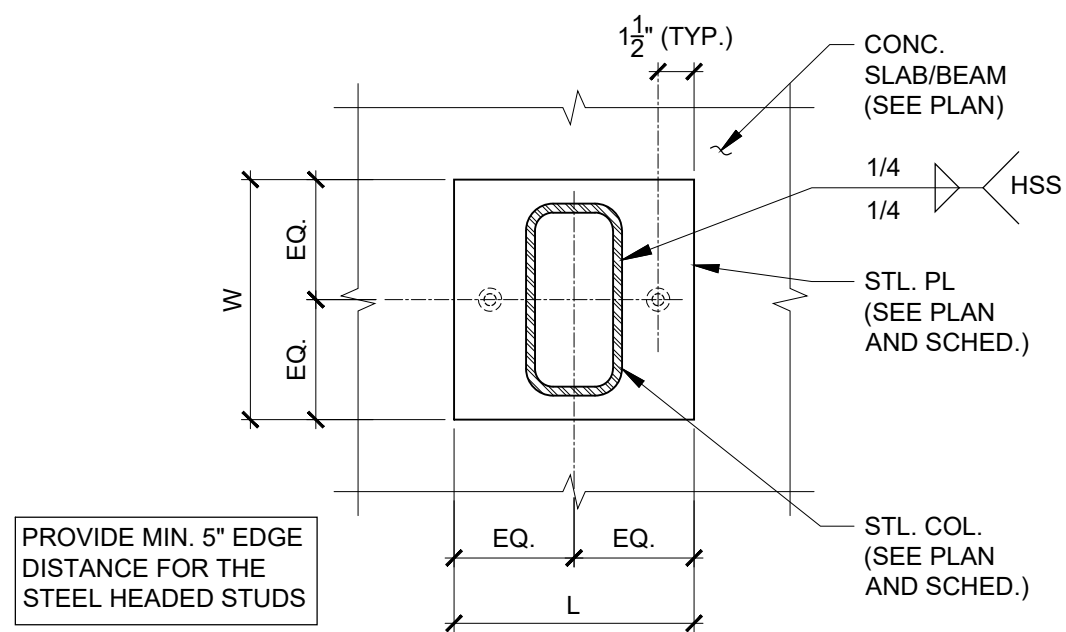
FOOTING NOTES:
1. AT ALL FOOTING CORNERS AND INTERSECTIONS, PROVIDE 2#5 L-SHAPED CORNER BARS WITH 30-INCH LONG LEGS.

STEEL COLUMN SCHEDULE				
MARK	SHAPE	BASE PLATE	CAP PLATE	REMARKS
ST1	HSS8x6x3/8	BP3	CP3	BEARING MULLION
ST2	HSS5x5x1/8	BP4	CP4	NON-BEARING MULLION
ST3	HSS5x5x3/16	BP2	CP2	NON-BEARING MULLION
ST4	HSS8x4x1/8	BP1	CP1	NON-BEARING MULLION
ST5	HSS10x4x1/8	BP1	CP1	NON-BEARING MULLION
ST6	HSS10x4x1/8	BP1	CP1	NON-BEARING MULLION
ST7	HSS4x4x1/4	BP2	CP2 / *CP3	NON-BEARING MULLION CP3 AT ROOF ONLY
ST8	HSS8x6x1/4	BP3	CP3	BEARING MULLION
ST9	HSS8x4x3/8	BP1	*	BEARING MULLION, *SEE SECTION 4/S-3.02

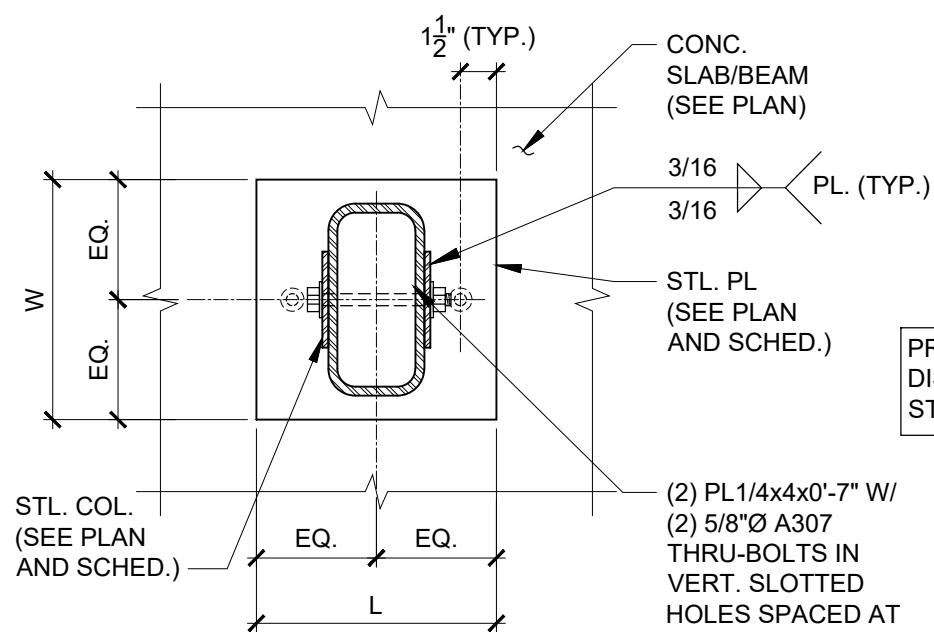
STEEL COLUMNS NOTES:
1. ALL NON LOAD-BEARING MEMBERS SHALL BE ERECTED AFTER MAIN FRAMING HAS BEEN UNSHORED.
2. FOR STEEL COLUMN BASE/CAP PLATES, SEE SCHEDULE.
3. ALL STEEL COLUMN SHALL BE HOT DIPPED GALVANIZED.

COLUMN BASE/CAP PLATE SCHEDULE				
MARK	SIZE (W (in) x L (in))	THICKNESS (in)	ANCHOR	DETAIL
BP1	10" x 10"	5/8"	(2) 5/8"Ø x 5" LONG WELDED HEADED STUDS (AWS D1.1 GR. B - ANCHORS)	BPA
CP1	10" x 10"	5/8"	(2) 5/8"Ø x 5" LONG WELDED HEADED STUDS (AWS D1.1 GR. B - ANCHORS)	CPA
BP2	6" x 10"	5/8"	(2) 5/8"Ø x 5" LONG WELDED HEADED STUDS (AWS D1.1 GR. B - ANCHORS)	BPA
CP2	6" x 10"	5/8"	(2) 5/8"Ø x 5" LONG WELDED HEADED STUDS (AWS D1.1 GR. B - ANCHORS)	CPA
BP3	9" x 10"	5/8"	(2) 5/8"Ø x 5" LONG WELDED HEADED STUDS (AWS D1.1 GR. B - ANCHORS)	BPB
CP3	9" x 10"	5/8"	(2) 5/8"Ø x 5" LONG WELDED HEADED STUDS (AWS D1.1 GR. B - ANCHORS)	CPB
BP4	6" x 10"	5/8"	(2) 5/8"Ø x 5" LONG WELDED HEADED STUDS (AWS D1.1 GR. B - ANCHORS)	BPB
CP4	6" x 10"	5/8"	(2) 5/8"Ø x 5" LONG WELDED HEADED STUDS (AWS D1.1 GR. B - ANCHORS)	CPB

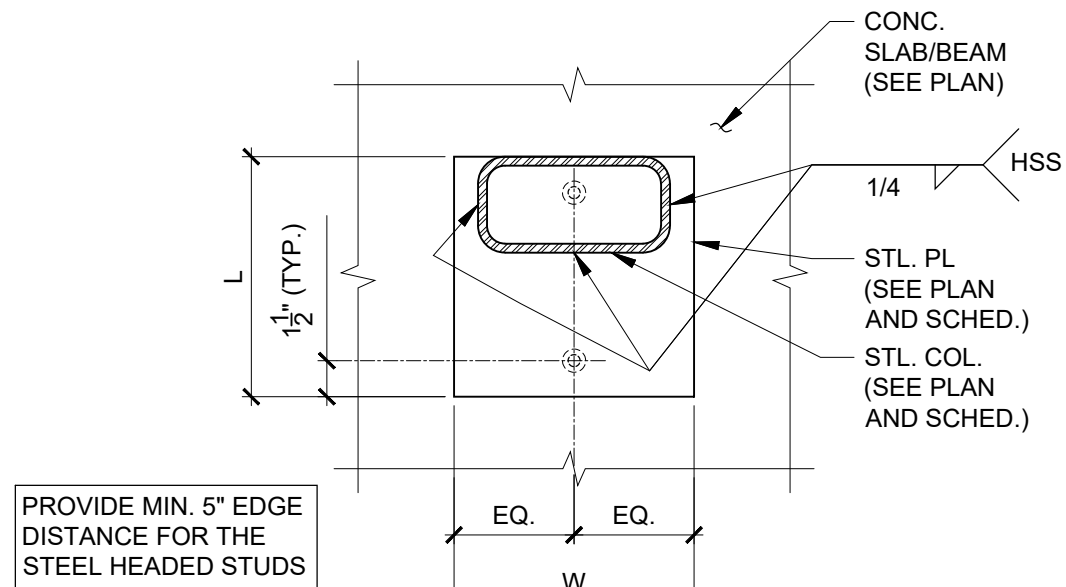
NOTE:
1. ALL THE STEEL PLATES SHALL BE EMBEDDED IN CONCRETE.
2. ALL STEEL PLATES, ANCHORS AND HARDWARE SHALL BE HOT-DIPPED GALVANIZED.



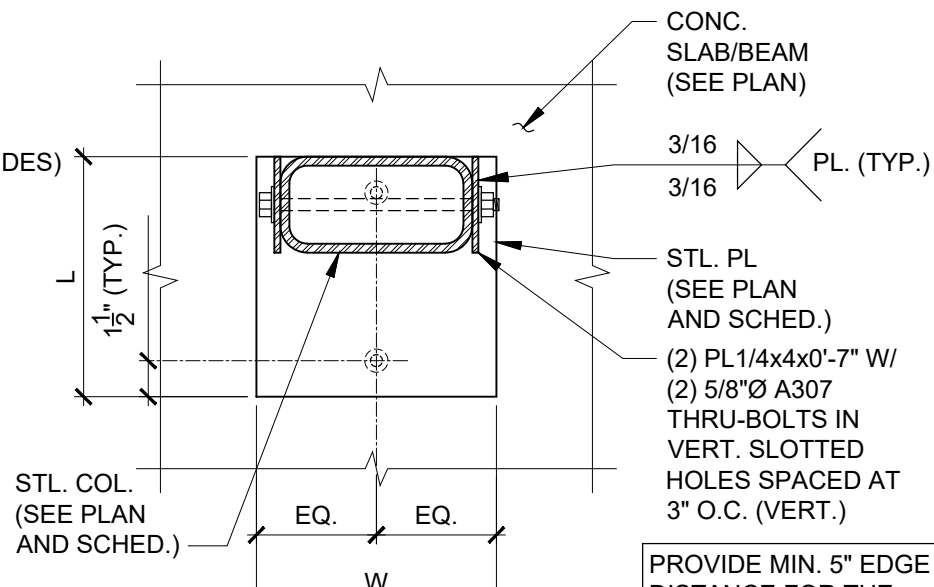
DETAIL "BPA"
1-1/2" = 1'-0"



DETAIL "CPA"
1-1/2" = 1'-0"



DETAIL "BPB"
1-1/2" = 1'-0"



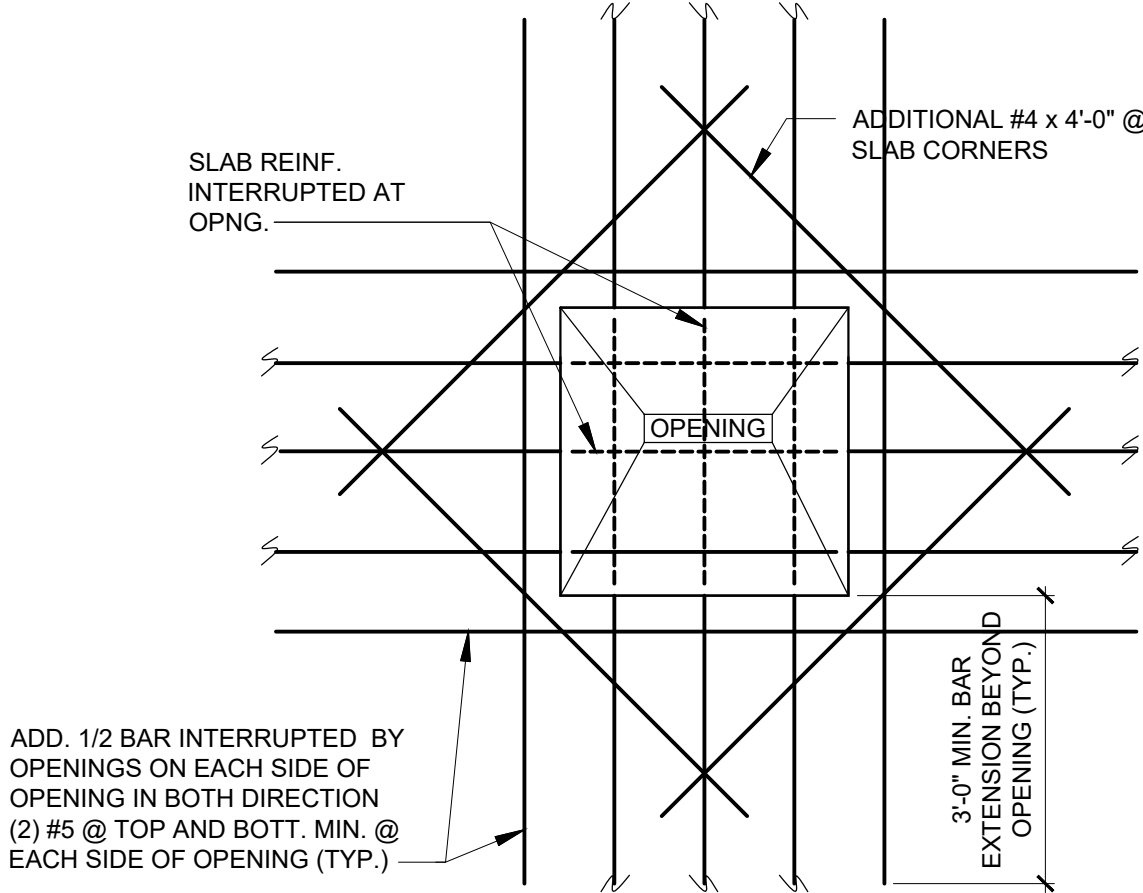
DETAIL "CPB"
1-1/2" = 1'-0"

COLUMN SCHEDULE				
MARK	SIZE WxD (IN.)	REINFORCEMENT		REMARKS
		VERTICAL	TIES	
C1	8x12	4#5	#3 @ 8" O.C.	SEE DETAIL - "A"
C1A	8x12	4#5	#3 @ 2-1/2" O.C.	SEE DETAIL - "A"
C2	8x20	6#5	#3 @ 2-1/2" O.C.	SEE DETAIL - "B"
C3	8x10	4#5	#3 @ 8" O.C.	SEE DETAIL - "A"
TC1	8x12	4#5	#3 @ 8" O.C.	SEE DETAIL - "A"
TC2	8x14	4#5	#3 @ 8" O.C.	SEE DETAIL - "A"
TC3	8x18	6#5	#3 @ 2-1/2" O.C.	SEE DETAIL - "B"
TC4	8x20	6#5	#3 @ 8" O.C.	SEE DETAIL - "B"
TC4A	8x20	6#5	#3 @ 2-1/2" O.C.	SEE DETAIL - "B"
TC5	8x11x12	6#5	#3 @ 2-1/2" O.C.	SEE DETAIL "D"
TC6	8x16	6#5	#3 @ 2-1/2" O.C.	SEE DETAIL - "C"
TC7	L-SHAPE 8x20/8x15-1/2	8#5	#3 @ 2-1/2" O.C.	SEE DETAIL - "E"
NC1	8x12	4#4	#3 @ 8" O.C.	SEE DETAIL - "A", NON-BEARING CONCRETE COLUMN
NC2	8x18	6#4	#3 @ 8" O.C.	SEE DETAIL - "B", NON-BEARING CONCRETE COLUMN
NC3	8x14	6#4	#3 @ 8" O.C.	SEE DETAIL - "A", NON-BEARING CONCRETE COLUMN
NC4	8x20	6#5	#3 @ 8" O.C.	SEE DETAIL - "B", NON-BEARING CONCRETE COLUMN

COLUMN NOTES:
1. HOOKED DOWELS, SAME SIZE AND NUMBER AS VERTICAL REINFORCEMENT, SHALL BE EMBEDDED TO BOTTOM OF FOOTING BELOW AND SHALL LAP MINIMUM 30 BAR DIAMETERS WITH VERTICAL REINFORCEMENT IN COLUMNS.
2. HOOK ALL VERTICAL COLUMN REINFORCEMENT AT TOP.

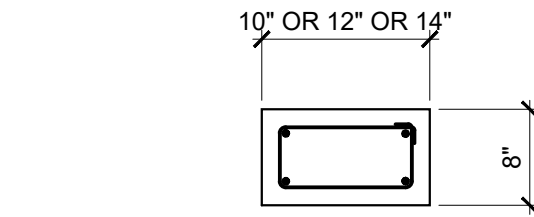
REBAR DEVELOPMENT & LAP SPLICE SCHEDULE fc'= 5000 PSI				
fc'= 5000 PSI	#6 & SMALLER		#7 & LARGER	
	TOP BARS (2)	OTHER BARS	TOP BARS (2)	OTHER BARS
DEVELOPMENT LENGTH & CLASS "A" SPLICE LENGTH	45 db	34 db	55 db	43 db
CLASS "B" SPLICE LENGTH	58 db	45 db	72 db	55 db

NOTES:
1. "db" DENOTES BAR DIAMETER.
2. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12 IN. OF CONCRETE CAST IN THE MEMBER BELOW THE SPLICE.
3. ALL SPLICES SHALL BE CLASS "B" SPLICE U.O.N.
4. DEVELOPMENT AND LAP SPLICE LENGTHS SHOWN IN SCHEDULE ABOVE ARE BASED ON CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN db, CLEAR COVER NOT LESS THAN db, AND STIRRUPS OR TIES THROUGHOUT THE DEVELOPMENT OR SPLICE LENGTH NOT LESS THAN THE CODE MINIMUM, OR CLEAR SPACING OF BARS BEING DEVELOPED OR SPLICED NOT LESS THAN 2db AND CLEAR COVER NOT LESS THAN db. FOR OTHER CASES, INCREASE LENGTHS GIVEN IN SCHEDULE ABOVE BY 50%.
5. FOR LIGHTWEIGHT AGGREGATE CONCRETE , MULTIPLY THE TABULATED VALUABLES BY 1.33.
6. INCREASE THE LENGTH OF LAP BY 20% FOR A 3-BAR BUNDLE AND 33% FOR A 4-BAR BUNDLE. INDIVIDUAL BARS WITHIN A BUNDLE TERMINATED WITHIN THE SPAN OF FEXURAL MEMBERS SHALL TERMINATE AT DIFFERENT POINTS WITH AT LEAST 40DB STAGGER.
7. USE MECHANICAL COUPLERS FOR BARS LARGER THAN #11. DO NOT LAP SPLICE BARS LARGER THAN #11.
8. DO NOT USE NON-CONTACT LAP SPLICES UNLESS PREVIOUSLY APPROVED BY THE ENGINEER.

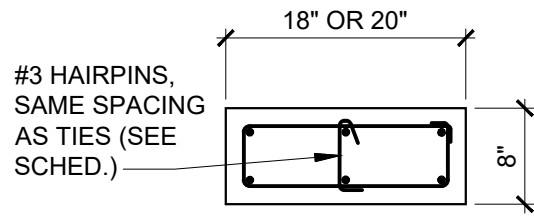


TYPICAL PLAN VIEW AT
OPENINGS IN STRUCTURAL
CONCRETE SLABS

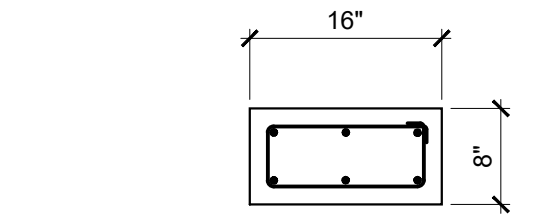
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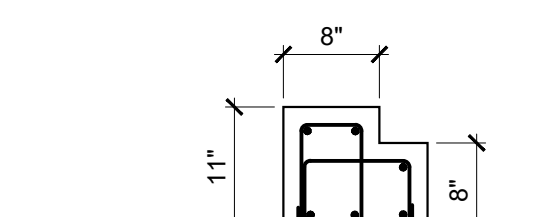
DETAIL "A" (4) BARS
SCALE: 3/4"=1'-0"



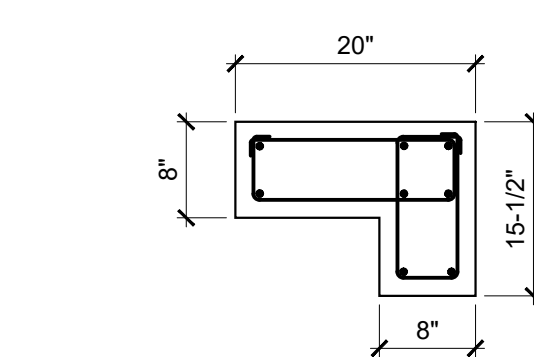
DETAIL "B" (6) BARS
SCALE: 3/4"=1'-0"



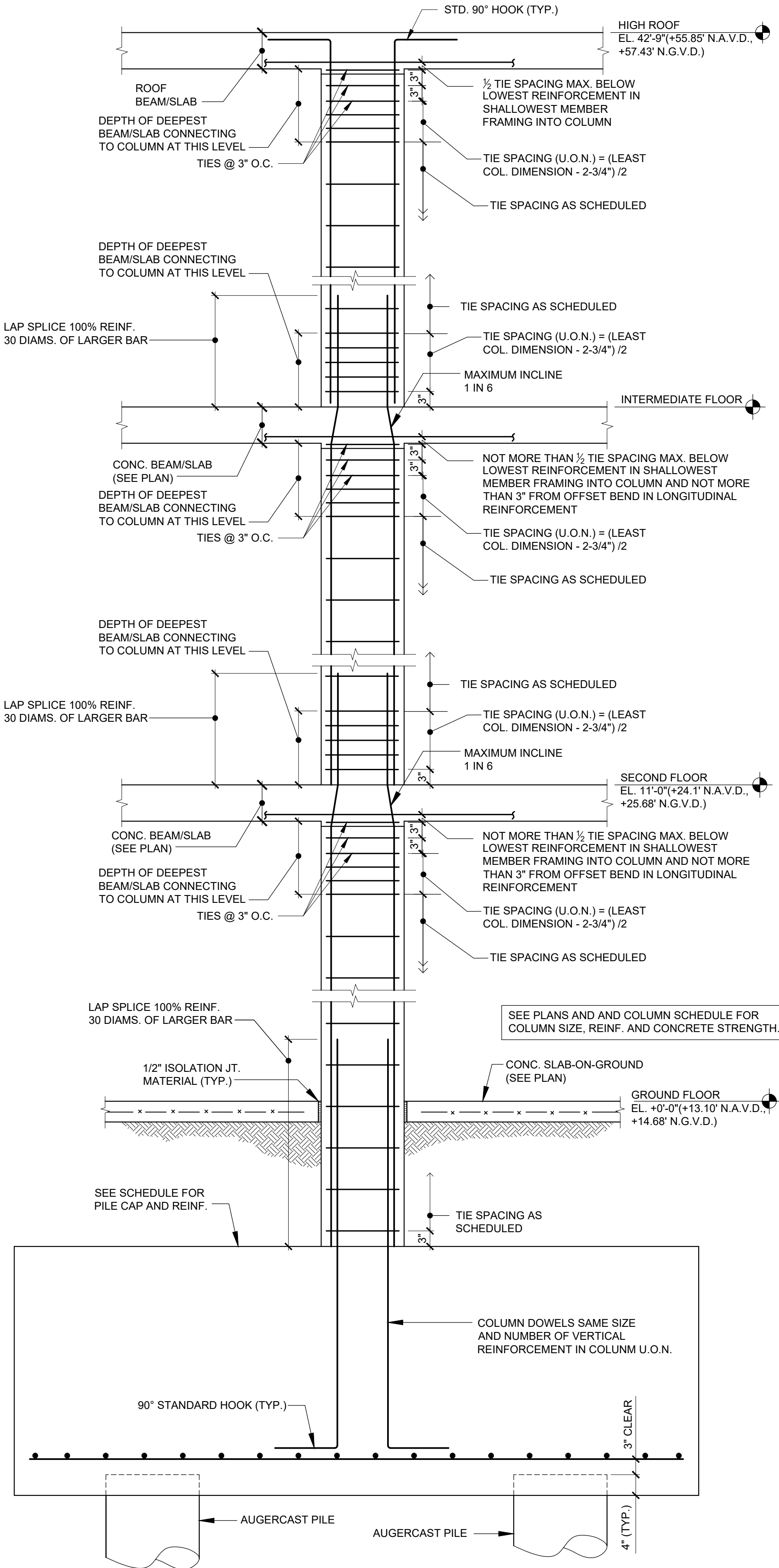
DETAIL "C" (6) BARS
SCALE: 3/4"=1'-0"



DETAIL "D" (6) BARS
SCALE: 3/4"=1'-0"



DETAIL "E" (8) BARS
SCALE: 3/4"=1'-0"



TYPICAL CONCRETE COLUMN REINFORCEMENT
ELEVATION DIAGRAM

N.T.S.

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GREENPARK GROUP

PROJECT NUMBER: 19023

SCHEDULES AND
TYP. DETAILS

DESIGNED BY:

DATE:
REVISION 01-31-2020

ISSUED FOR
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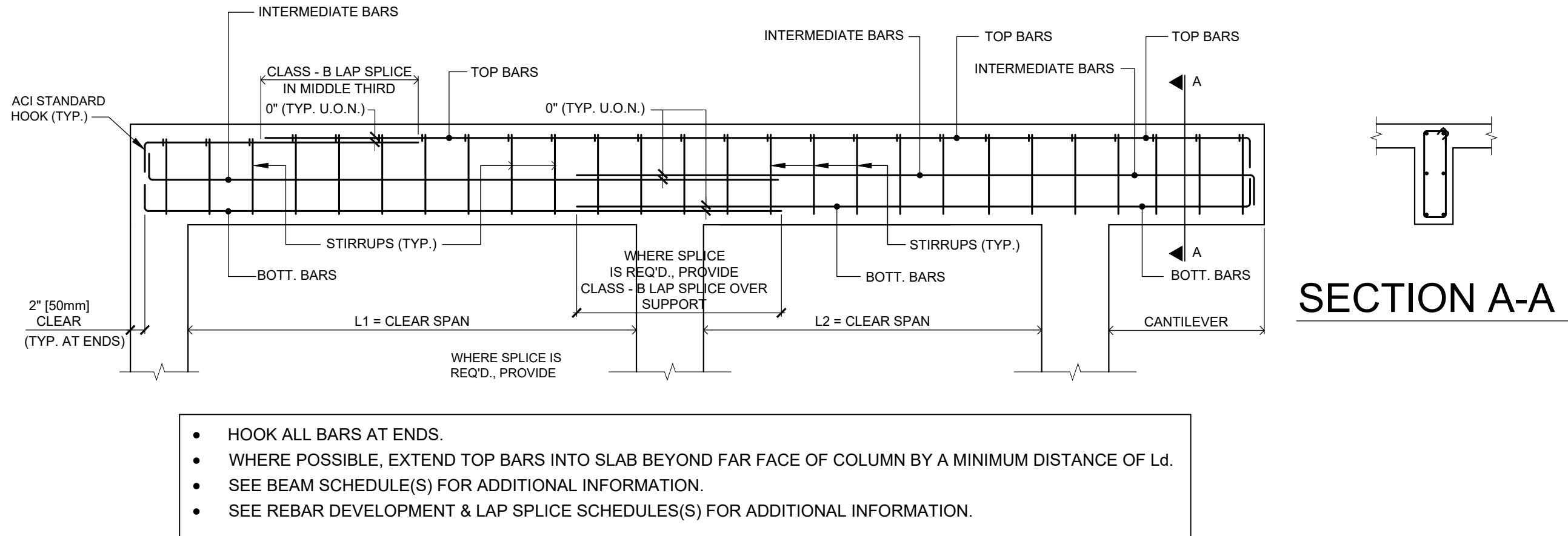
11/15/2019

BEAM SCHEDULE						
MARK	SIZE WxD (IN.)	REINFORCEMENT				REMARKS
		TOP	INT.	BOTT.	STIRRUPS	
GB-1	16x30	3#6	--	3#6	#3 @ 6" O.C.	--
GB-2	16x30	5#6	--	5#6	#3 @ 6" O.C.	--
GB-3	16x30	3#6	--	*8#9	#4 @ 6" O.C.	* PLACE BARS IN TWO LAYERS
GB-4	16x30	3#6	--	*6#9	#4 @ 6" O.C.	* PLACE BARS IN TWO LAYERS
GB-5	16x30	*10#8	--	*10#8	#4 @ 4" O.C.	* PLACE BARS IN TWO LAYERS
GB-6	16x30	5#9	--	6#9	#4 @ 4" O.C.	--
GB-7	16x30	5#6	--	6#6	#3 @ 6" O.C.	--
1B-1	8x20	2#6	2#6 E.F.	2#6	#3 @ 2-1/2" O.C.	--
2B-1	8x25	2#5	--	2#5	#3 @ 10" O.C.	--
2B-2	8x27	2#6	--	2#7	#3 @ 12" O.C.	--
2B-3	8x12	2#5	--	2#5	#3 @ 4" O.C.	--
2B-4	8x20	2#5	--	2#5	#3 @ 8" O.C.	--
2B-5	12x24	2#5	--	2#7	#3 @ 10" O.C.	--
2B-6	15x15	3#5	--	3#5	#3 @ 6" O.C.	--
2B-7	8x21	2#5	--	2#5	#3 @ 8" O.C.	--
2B-8	8x15	2#5	--	2#5	#3 @ 8" O.C.	--
2B-9	15x23	4#6	--	6#6	#3 @ 10" O.C.	--
2B-10	15x22	3#5	--	3#5	#3 @ 8" O.C.	--
2B-11	8x27	4#7*	2#5	4#7*	#3 @ 3" O.C.	* PLACE BARS IN TWO LAYERS
2B-12	12x24	2#5	--	3#8	#3 @ 10" O.C.	--
2B-13	8x23	2#6	--	2#6	#3 @ 10" O.C.	--
2B-14	8x19	2#5	--	2#5	#3 @ 10" O.C.	--
3B-1	10x24	2#7	--	2#9	#3 @ 8" O.C.	--
3B-2	8x12	2#5	--	2#5	#3 @4" O.C.	--
3B-3	8x20	2#5	--	2#7	#3 @8" O.C.	--
3B-4	12x12	2#5	--	2#5	#3 @4" O.C.	--
RB-1	10x22	2#6	--	3#7	#3 @8" O.C.	--
RB-2	8x12	2#5	--	2#5	#3 @4" O.C.	--
RB-3	8x20	2#5	--	2#7	#3 @8" O.C.	--
RB-4	12x12	2#7	--	2#6	#3 @4" O.C.	--
HRB-1	8x12	2#5	--	2#5	#3 @4" O.C.	--
TB-1	8x12	2#5	--	2#5	#3 @ 10" O.C.	--

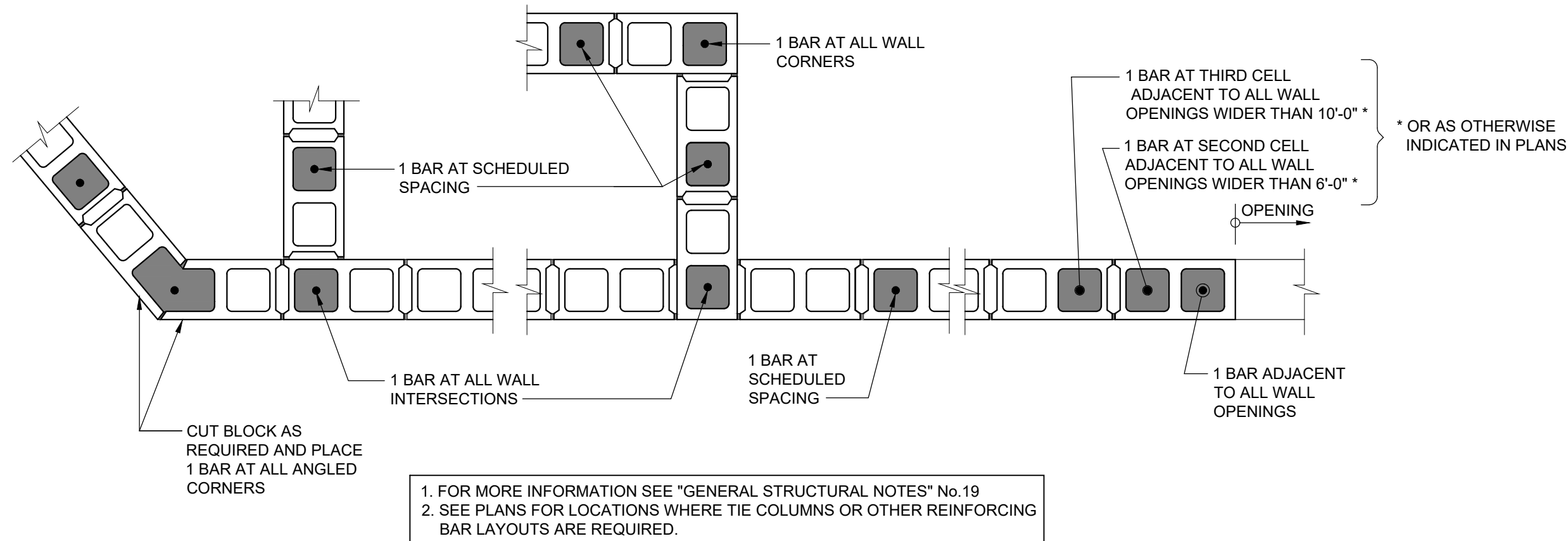
BEAM NOTES:
2. HOOK ALL TOP BARS AT ENDS.
3. ALL TOP AND BOTTOM BARS SHALL BE CONTINUOUS, U.O.N. WHERE NECESSARY, LAP SPLICE TOP BARS IN MIDDLE THIRD OF SPAN AND LAP SPLICE BOTTOM BARS AT SUPPORTS. LAP SPLICES SHALL BE MINIMUM 48 BAR DIAMETERS.

C.M.U. WALL SCHEDULE				
WALL	THICKNESS (NOMINAL)	f'm (psi)	VERTICAL REINFORCEMENT	REMARKS
MW-1	12"	2,000	#4 @ 8" O.C.	NON-BEARING C.M.U. WALL
MW-2	8"	2,000	#4 @ 24" O.C.	PARAPET WALL/ROOF BEARING WALL
MW-3	8"	2,000	#4 @ 8" O.C.	NON-BEARING C.M.U. WALL
MW-4	12"	2,000	#4 @ 32" O.C.	PARAPET WALL
BMW-1	8"	2,000	#5 @ 40" O.C.	BREAKAWAY WALL
BMW-2	8"	2,000	#5 @ 48" O.C.	BREAKAWAY FENCE WALL

C.M.U. WALL NOTES:
1. PROVIDE VERTICAL REINFORCEMENT IN GROUT-FILLED CELLS AT SPECIFIED SPACING IN ADDITION TO ENDS, CORNERS, JAMBS OF OPENINGS, INTERSECTIONS AND WHERE OTHERWISE INDICATED.
2. PROVIDE HORIZ. JOINT REINF. @ EVERY OTHER COURSE.
3. SEE "GENERAL STRUCTURAL NOTES" No. 19.
4. CAST ADJACENT STRUCTURAL CONCRETE AS TIE COLUMNS (AND WALL), TIE BEAMS (AND SLABS) (U.O.N.).



CONTINUOUSLY REINFORCED BEAM



TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS

3/4" = 1'-0"

MASONRY (f'm=2,000 PSI) REBAR DEVELOPMENT & LAP SPLICE LENGTH (Ld) SCHEDULE		
BAR SIZE	WALL THICKNESS	
	8" C.M.U.	12" C.M.U.
#3	1'-3"	1'-3"
#4	1'-8"	1'-8"
#5	2'-1"	2'-1"
#6	2'-7"	2'-6"
#7	2'-11"	2'-11"
#8	4'-2"	3'-7"
#9	5'-5"	4'-5"

CONC. WALL SCHEDULE				
WALL	THICKNESS (NOMINAL)	HORIZONTAL REINFORCEMENT	VERTICAL REINFORCEMENT	REMARKS
W-1	5"	#3 @ 12" O.C. AT MID-DEPTH	#3 @ 12" O.C. AT MID-DEPTH	EXPENDABLE SHOTCRETE WALL

MASONRY SHEAR WALL SCHEDULE				
WALL	THICKNESS (NOMINAL)	f'm (psi)	VERTICAL REINFORCEMENT	REMARKS
MSW-1	8"	2,000	#5 @ 32" O.C.	--
MSW-2	8"	2,000	#7 @ 32" O.C.	--
MSW-3	8"	2,000	#6 @ 32" O.C.	--

C.M.U. WALL NOTES:
1. PROVIDE VERTICAL REINFORCEMENT IN GROUT-FILLED CELLS AT SPECIFIED SPACING IN ADDITION TO ENDS, CORNERS, JAMBS OF OPENINGS, INTERSECTIONS AND WHERE OTHERWISE INDICATED.
2. PROVIDE HORIZ. JOINT REINF. @ EVERY OTHER COURSE.
3. SEE "GENERAL STRUCTURAL NOTES" No. 19.
4. CAST ADJACENT STRUCTURAL CONCRETE AS TIE COLUMNS (AND WALL), TIE BEAMS (AND SLABS) (U.O.N.).

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GREENPARK GROUP

PROJECT NUMBER: 19023

SCHEDULES AND
TYPICAL DETAILS

DESIGNED BY:

DATE:

ISSUED FOR
☐ PREVIEW & COMMENTS
☐ BIDDING
☒ NOT FOR CONSTRUCTION
☐ PERMIT
☐ CONSTRUCTION

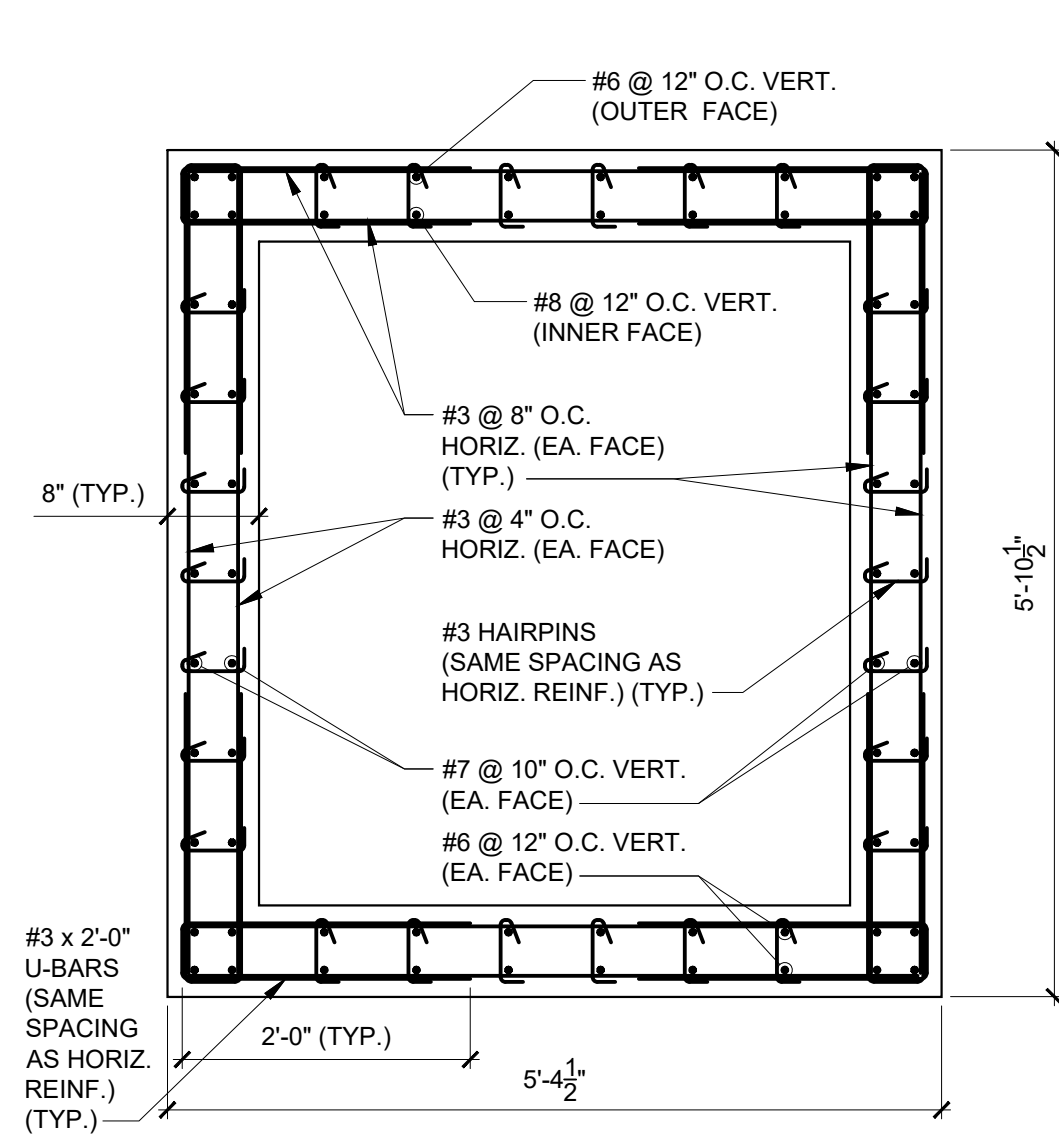
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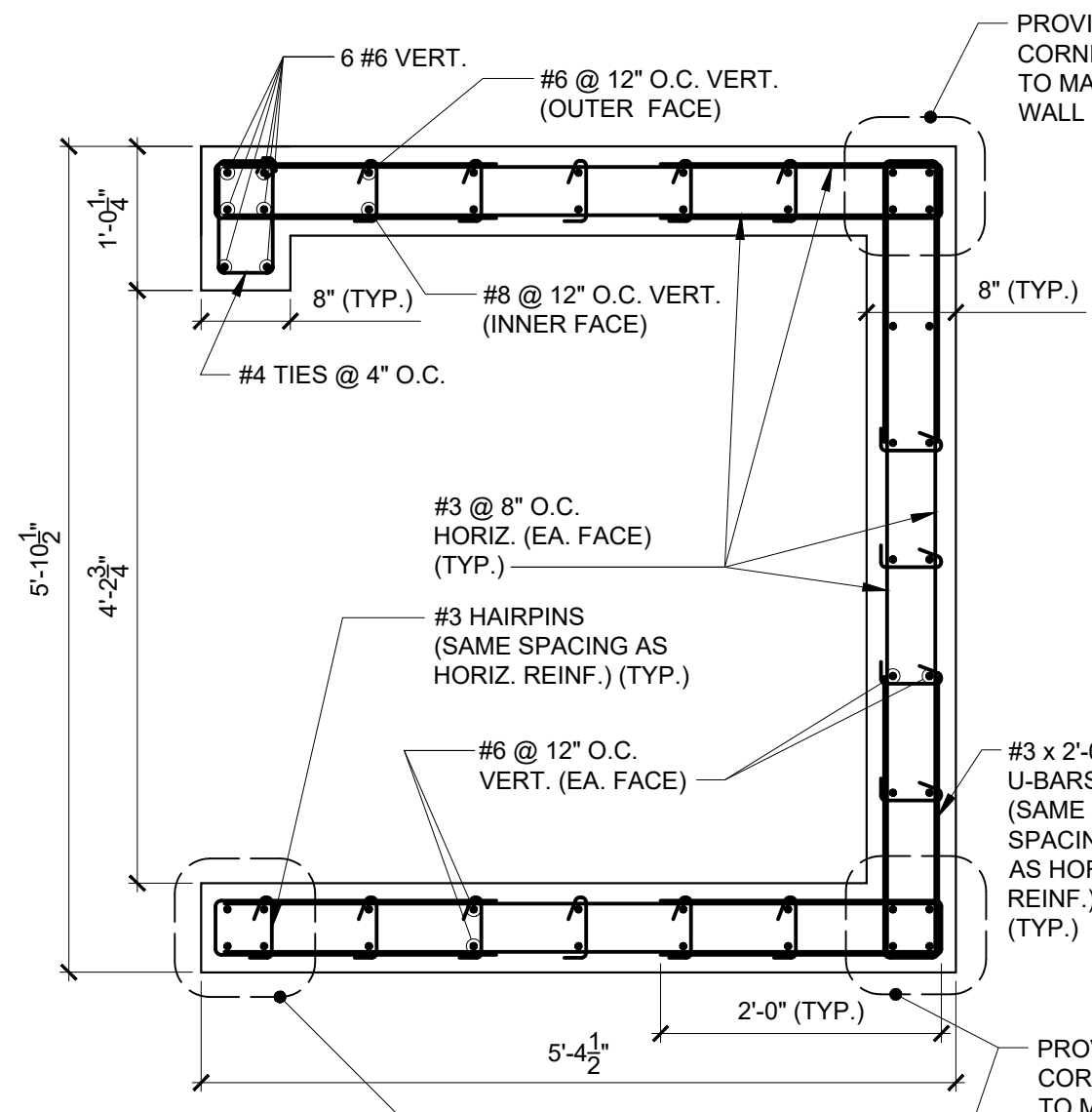
11/15/2019

CONSTRUCTION
DOCUMENTS



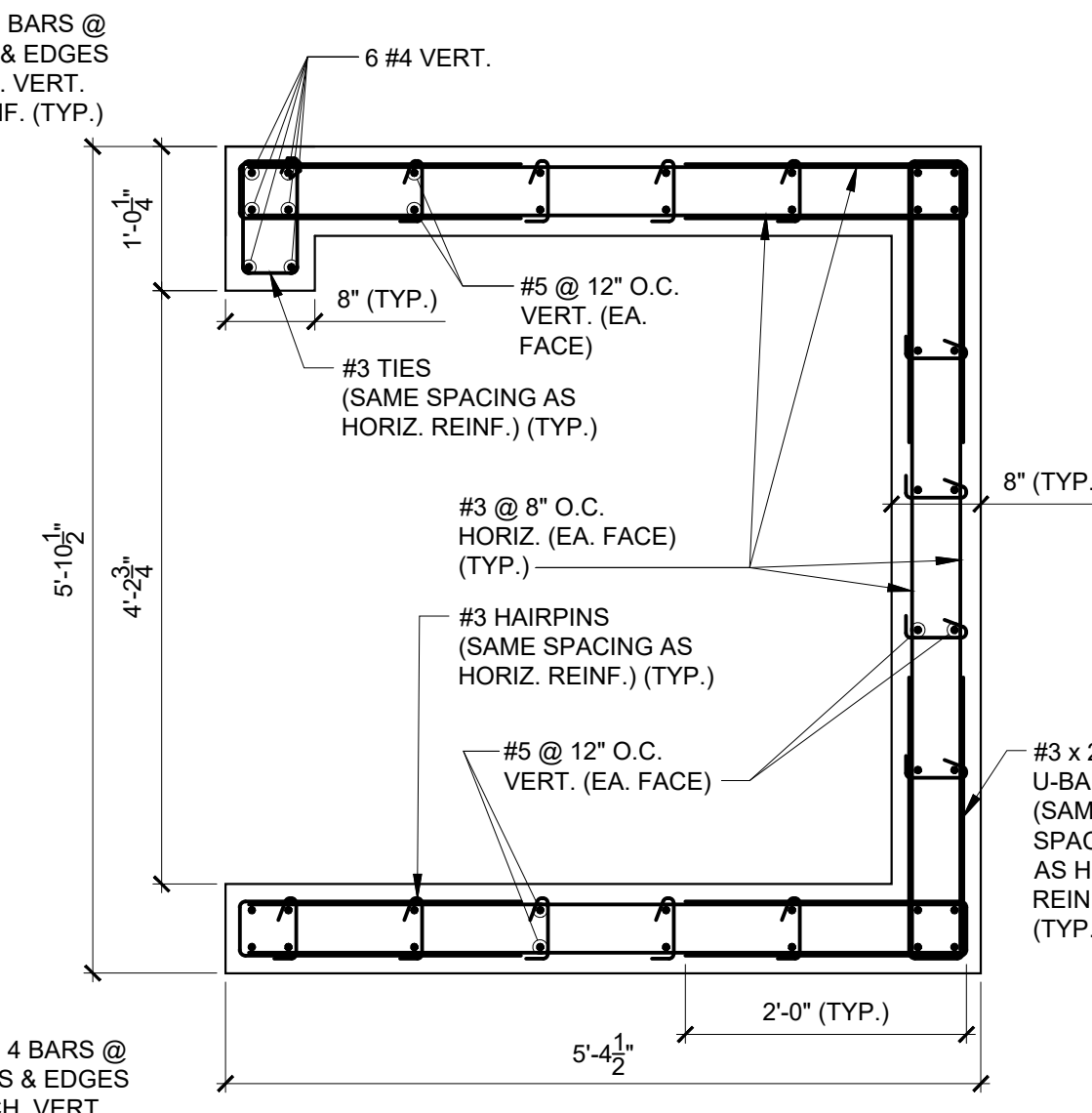
SW#1 - FOUNDATION TO
GROUND FLOOR

3/4" = 1'-0"



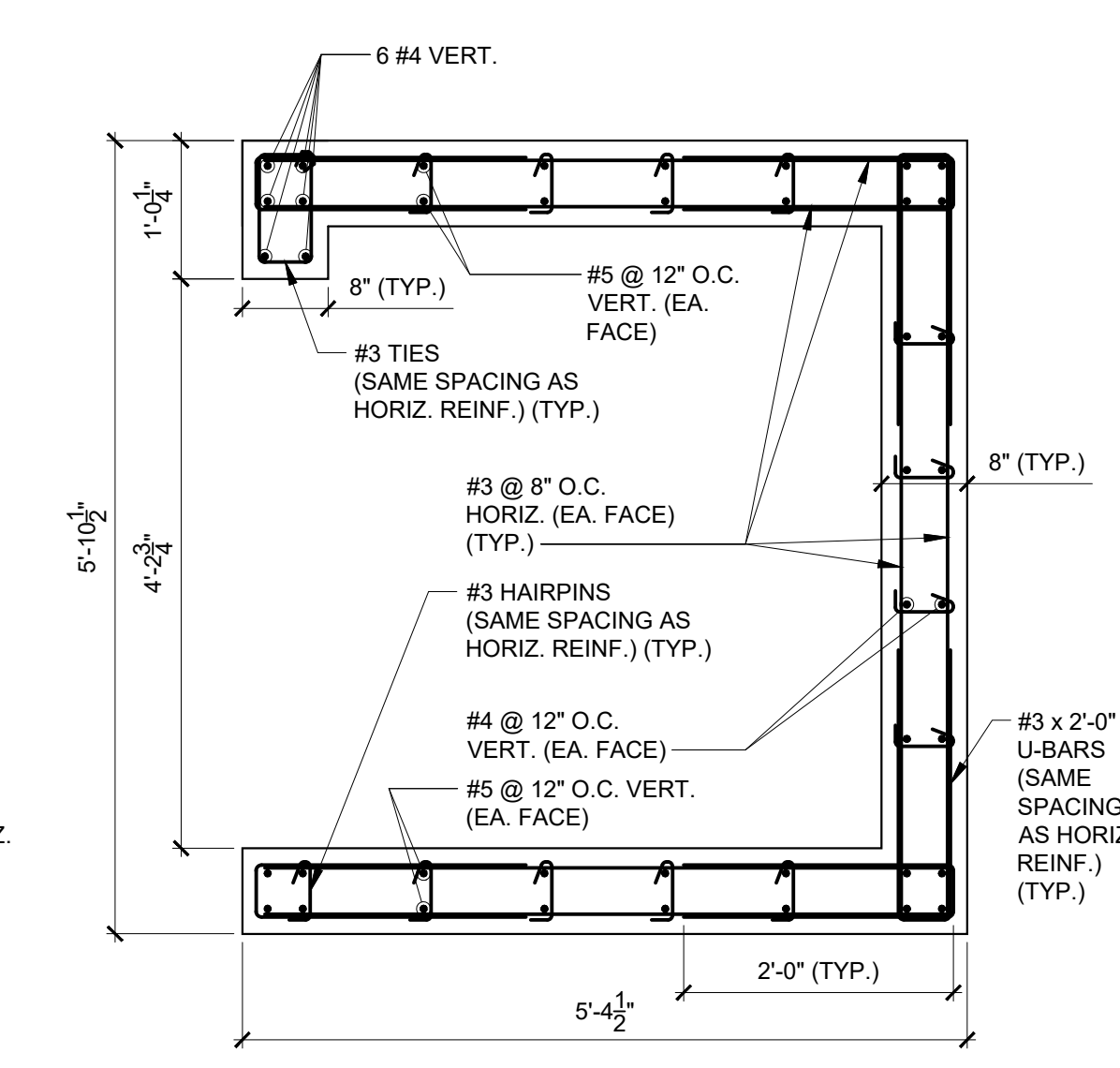
SW#1 - GROUND FLOOR TO
SECOND FLOOR

3/4" = 1'-0"



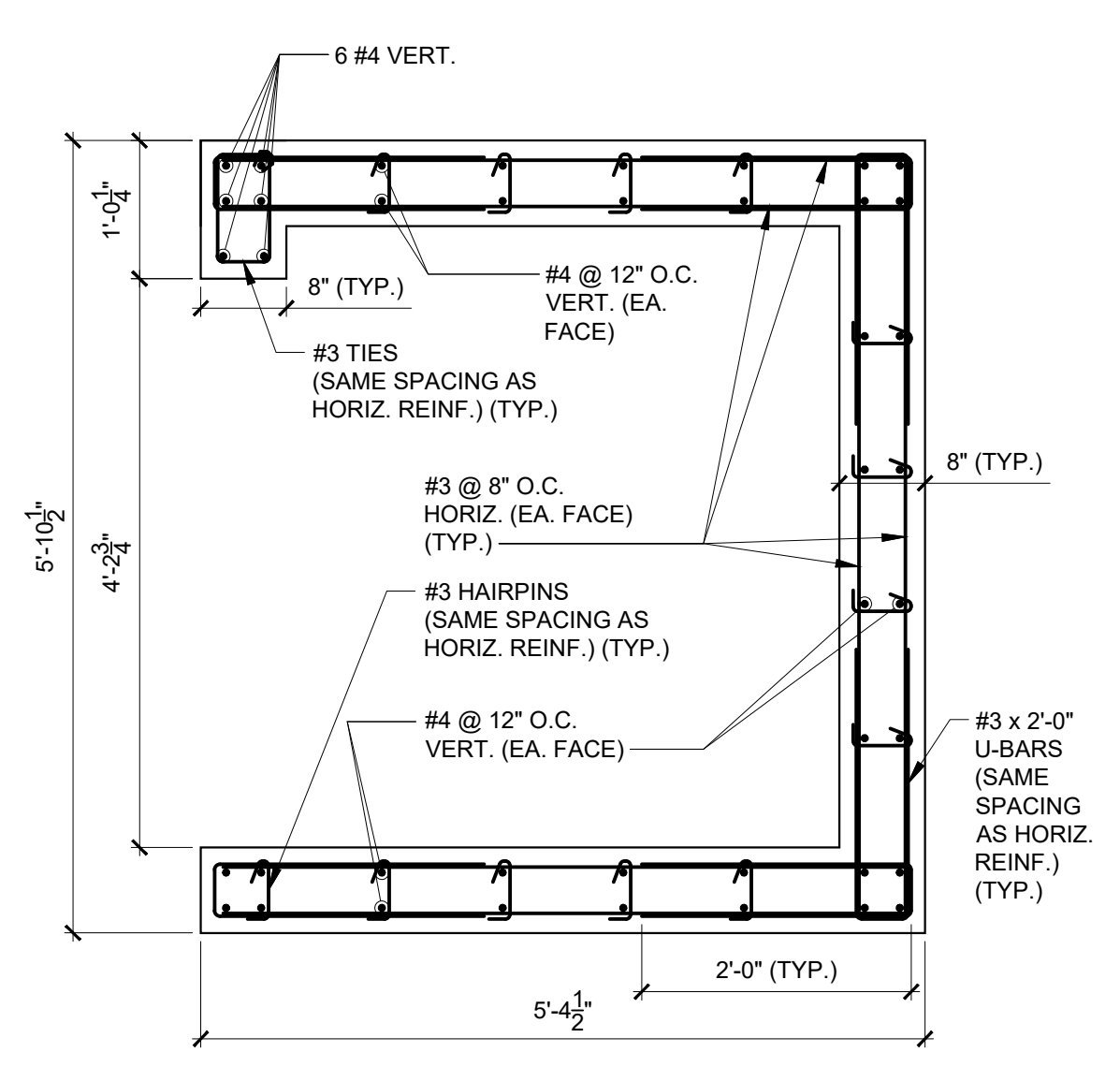
SW#1 - SECOND FLOOR TO
THIRD FLOOR

3/4" = 1'-0"



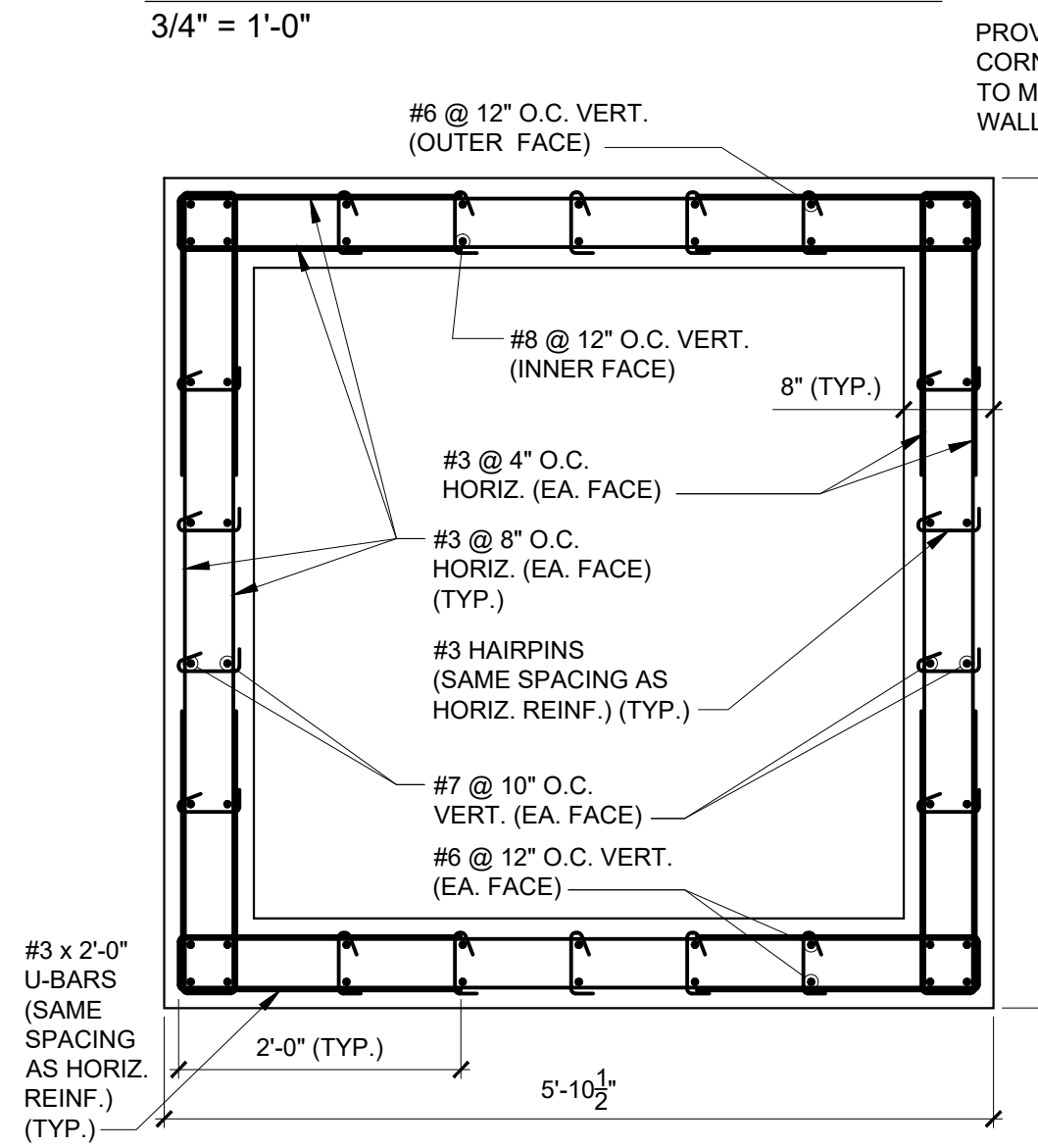
SW#1 - THIRD FLOOR TO
ROOF

3/4" = 1'-0"



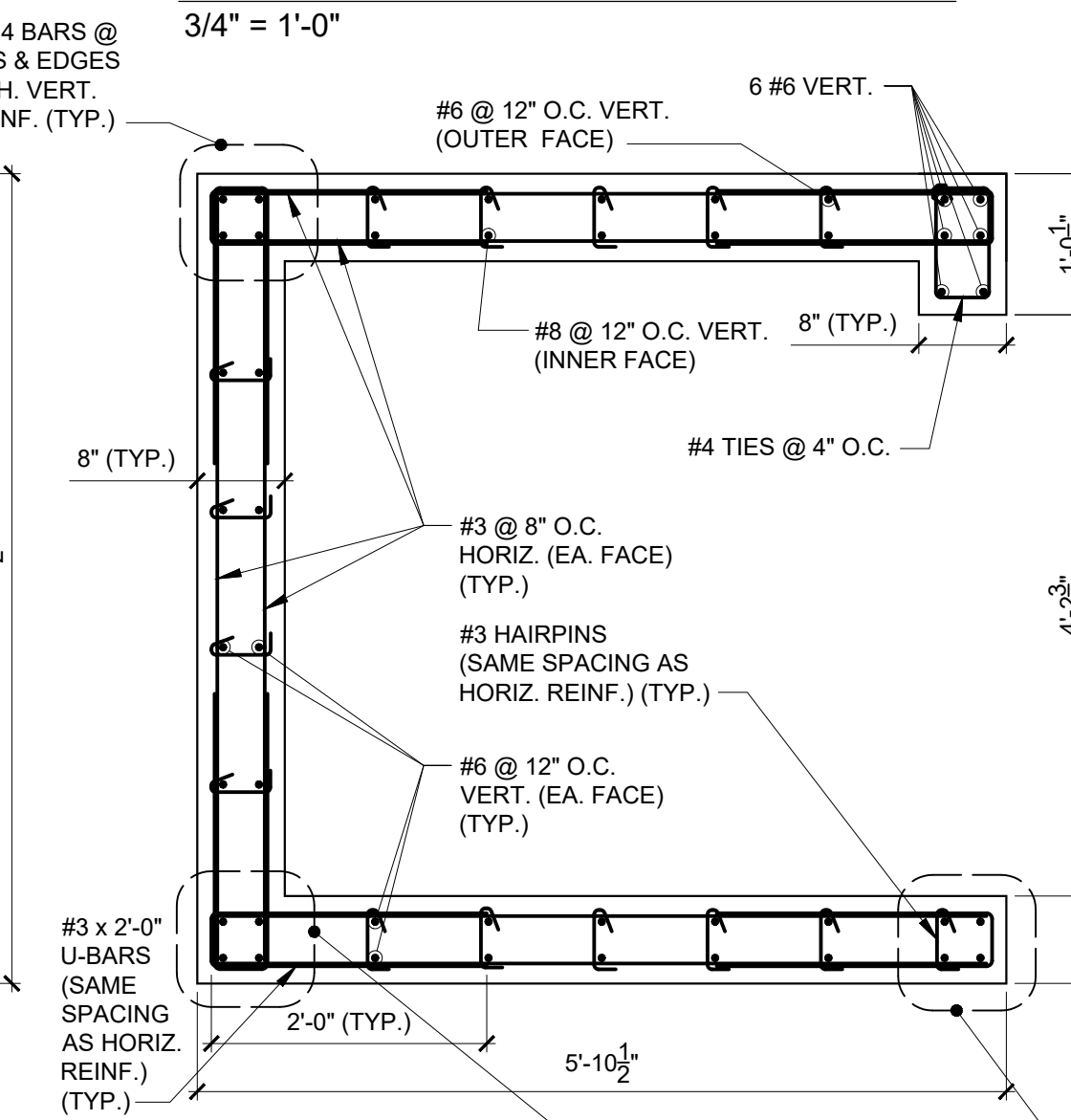
SW#1 - ROOF TO HIGH
ROOF

3/4" = 1'-0"



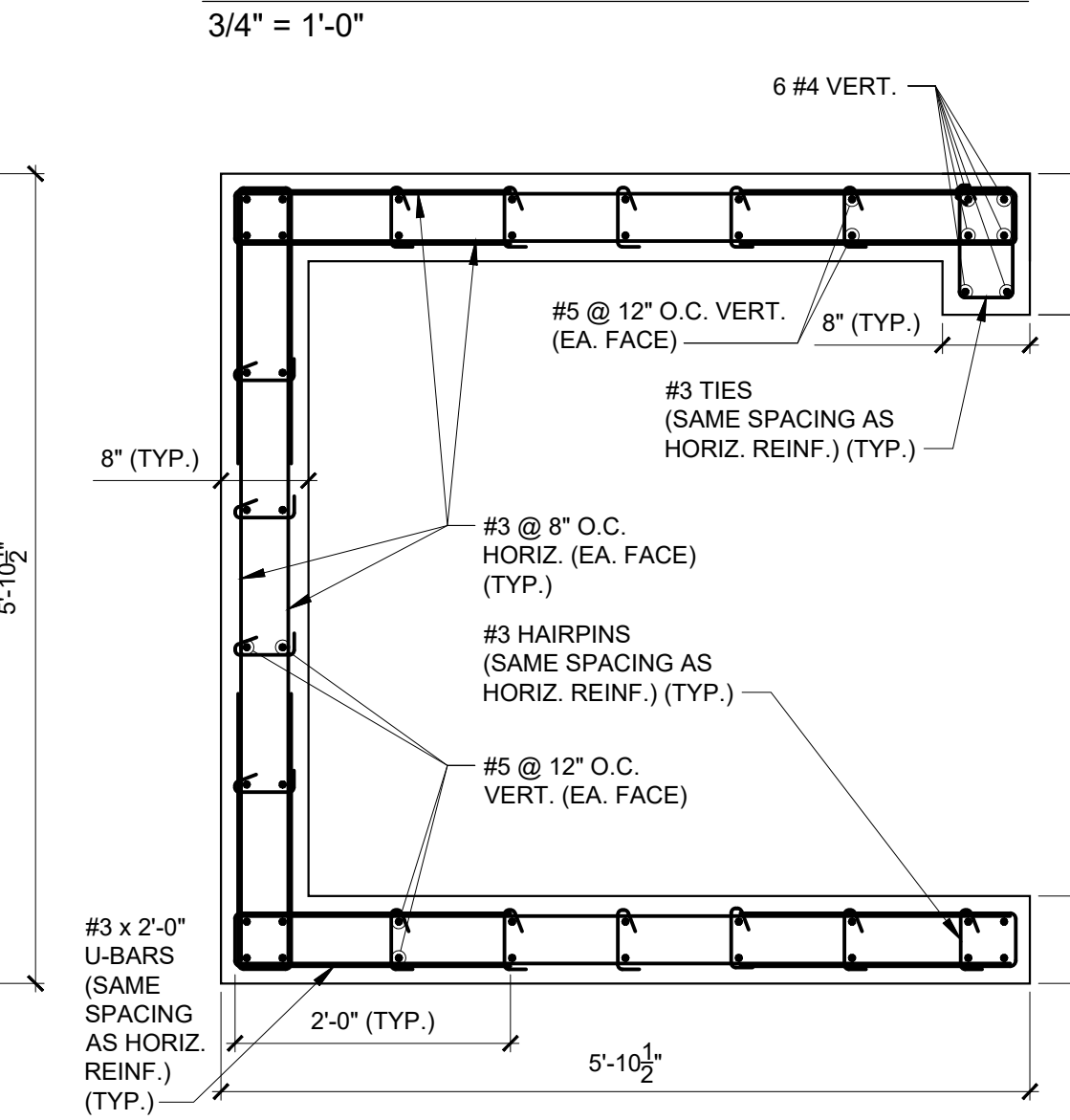
SW#2 - FOUNDATION TO
GROUND FLOOR

3/4" = 1'-0"



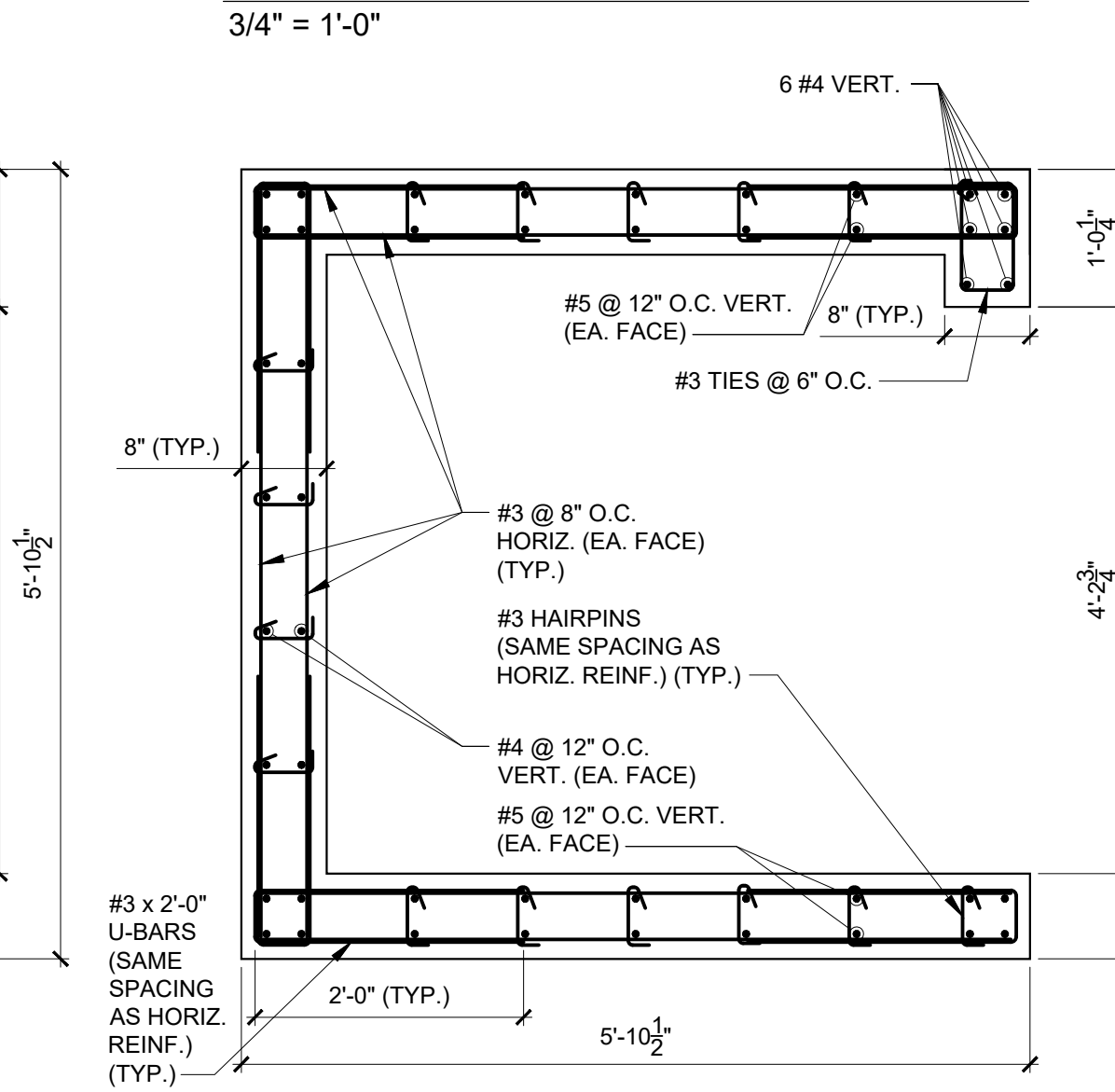
SW#2 - GROUND FLOOR TO
SECOND FLOOR

3/4" = 1'-0"



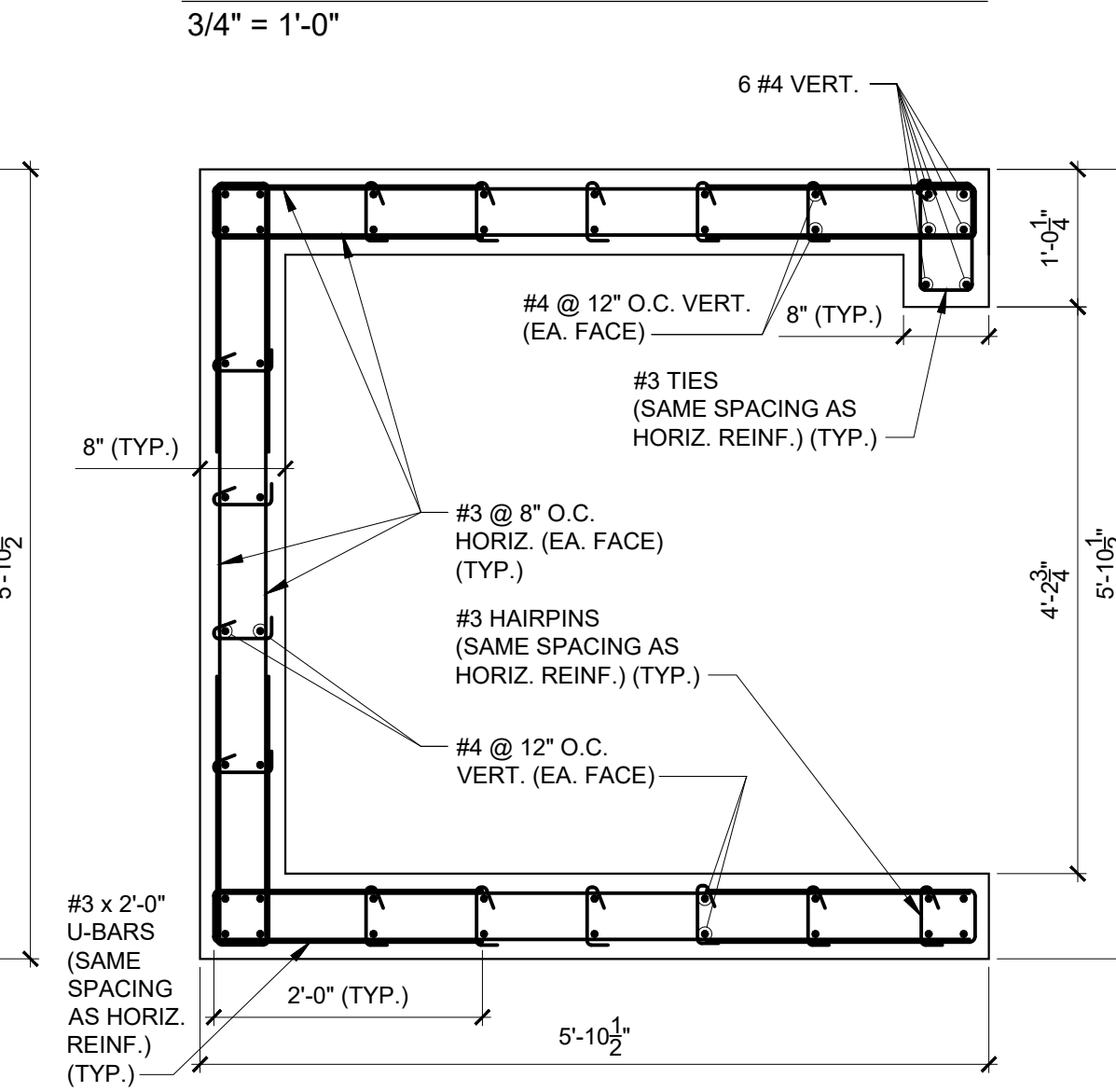
SW#2 - SECOND FLOOR TO
THIRD FLOOR

3/4" = 1'-0"



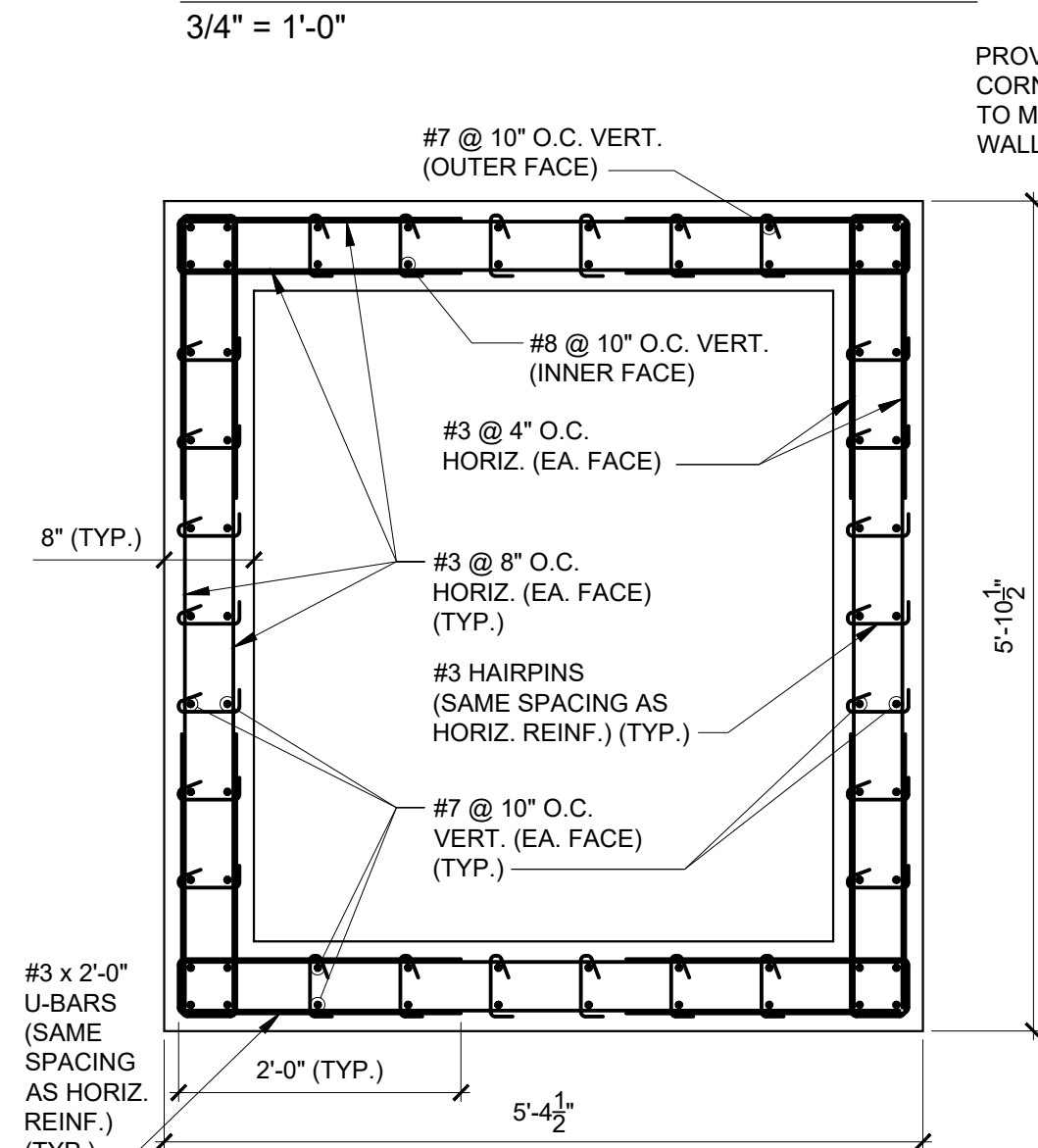
SW#2 - THIRD FLOOR TO
ROOF

3/4" = 1'-0"



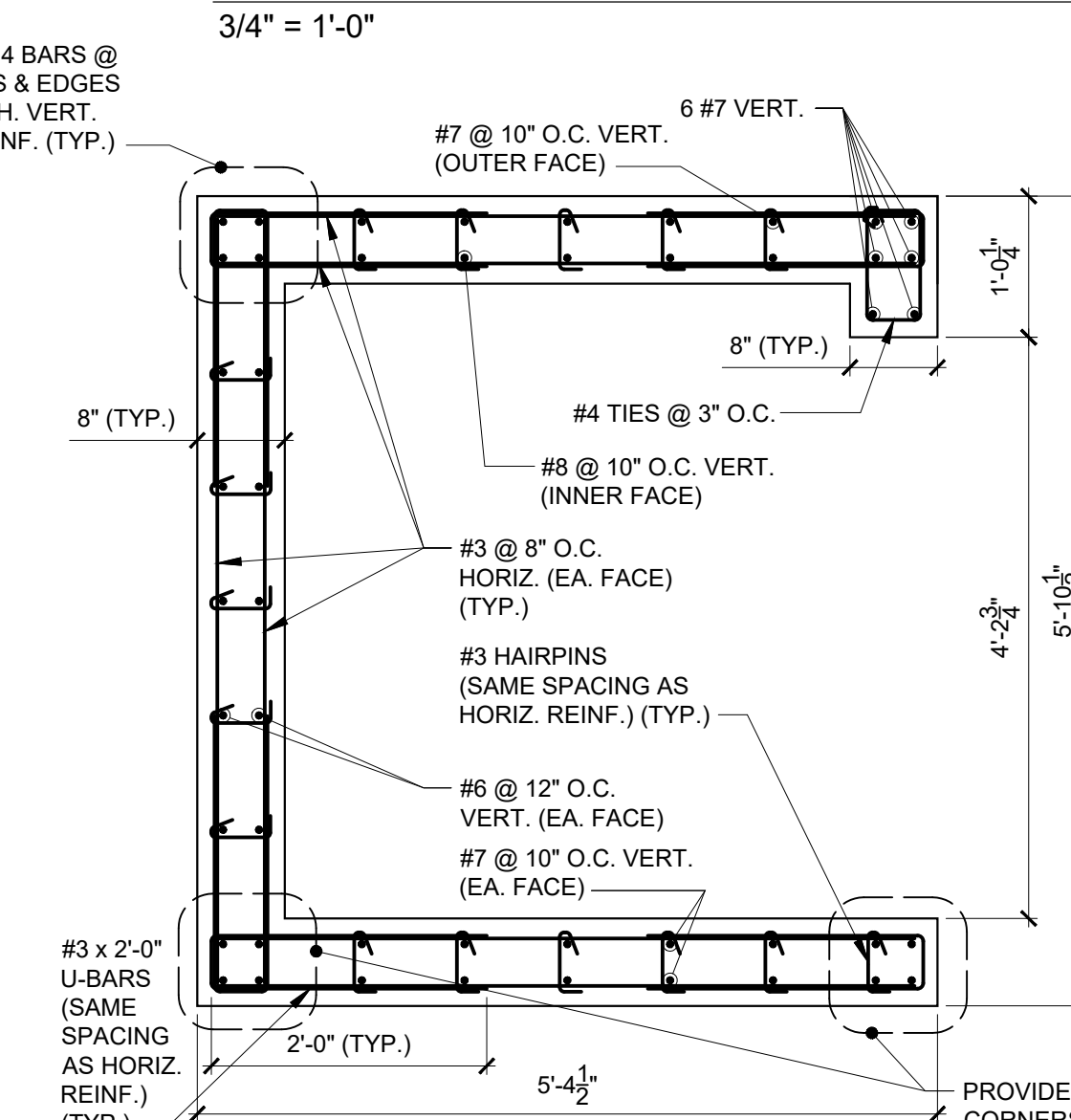
SW#2 - ROOF TO HIGH
ROOF

3/4" = 1'-0"



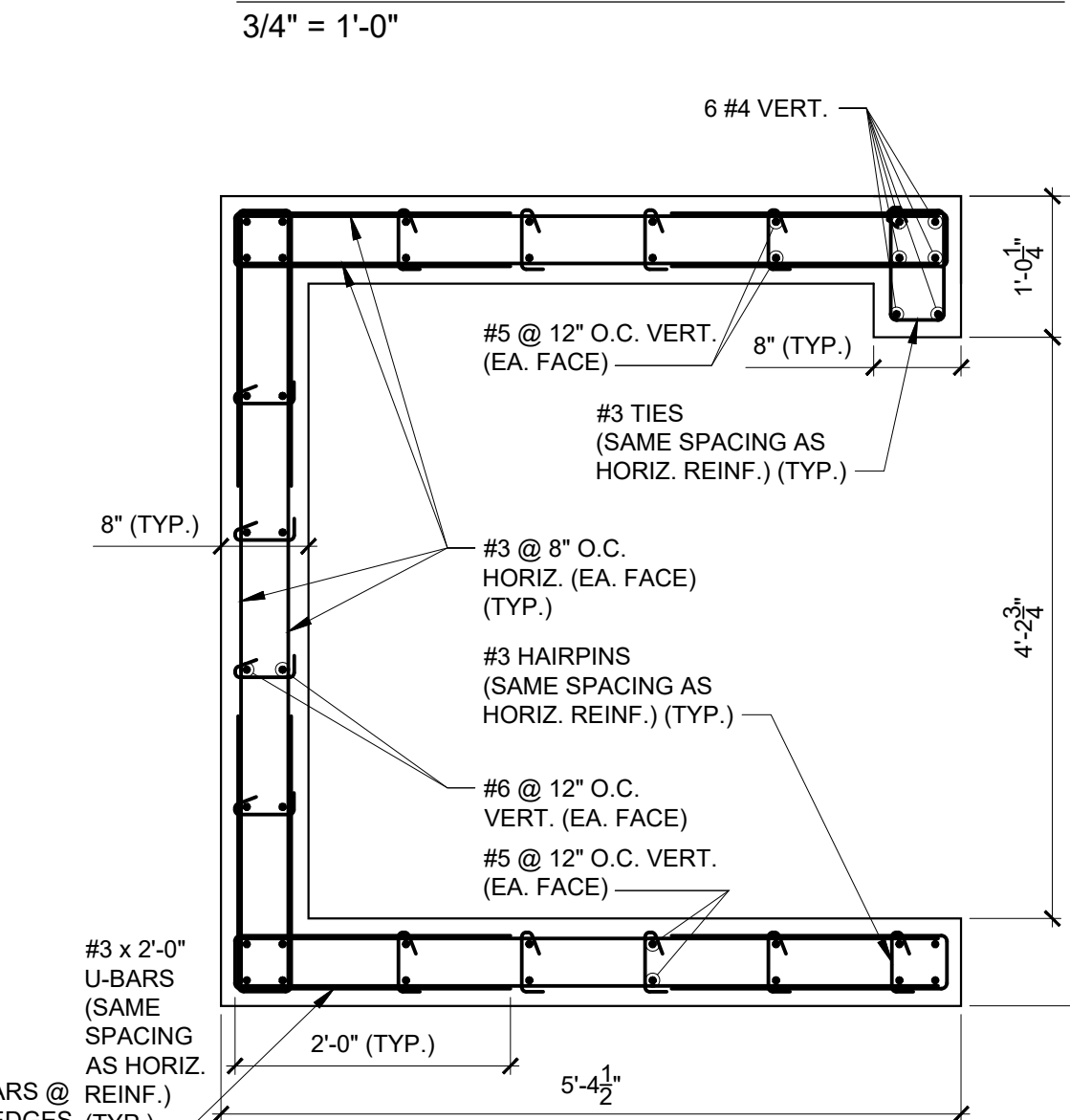
SW#3 - FOUNDATION TO
GROUND FLOOR

3/4" = 1'-0"



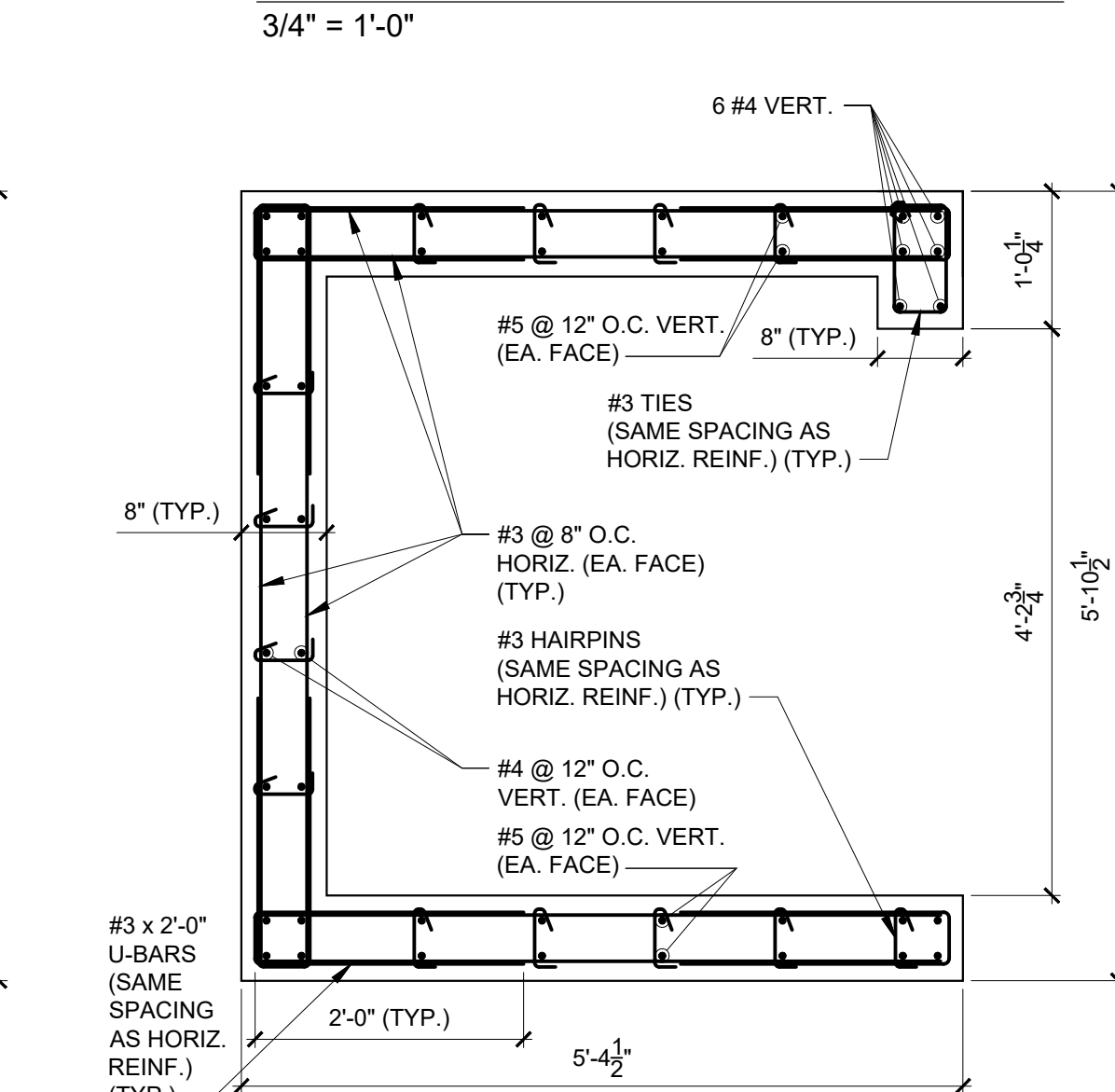
SW#3 - GROUND FLOOR TO
SECOND FLOOR

3/4" = 1'-0"



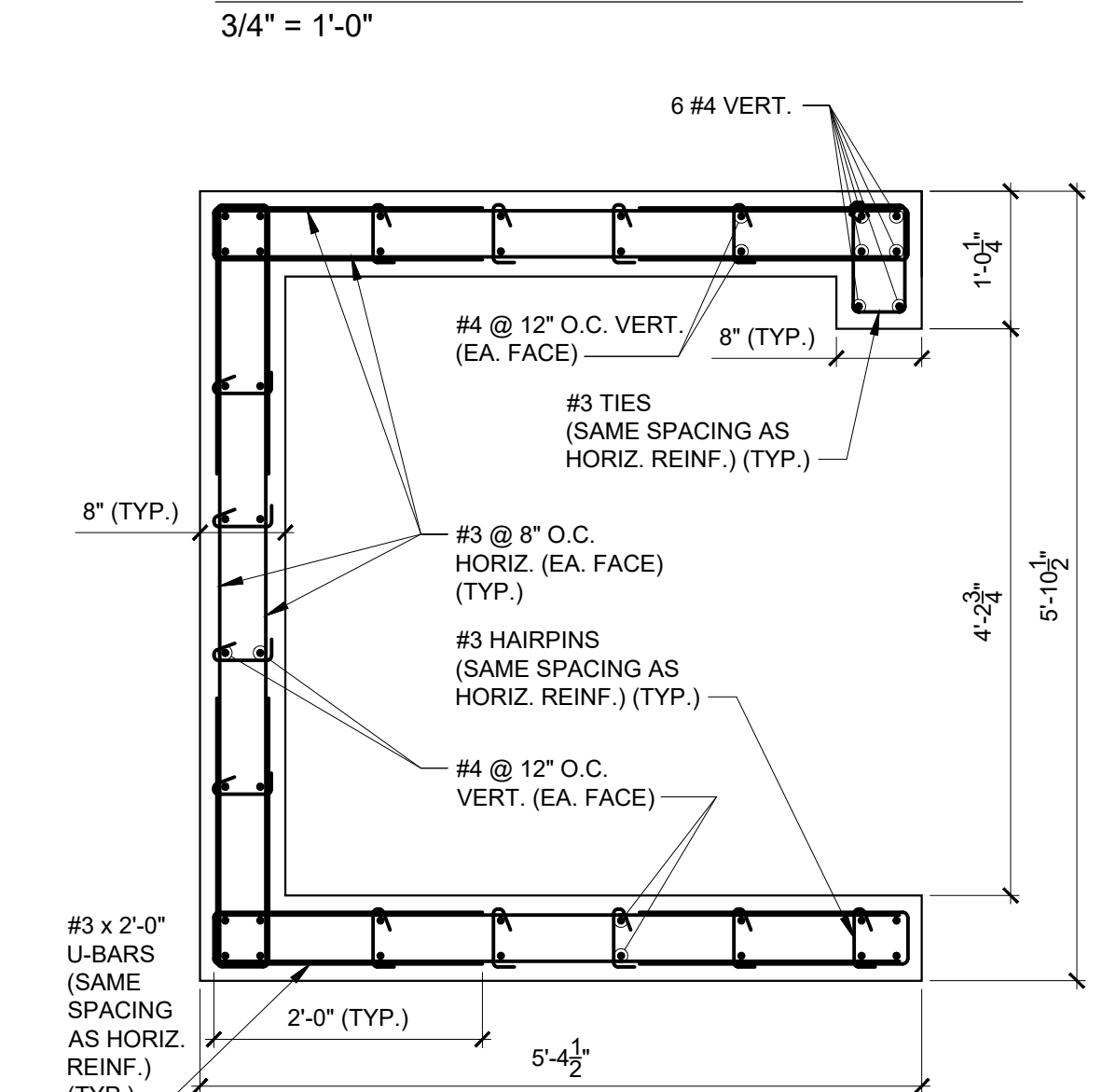
SW#3 - SECOND FLOOR TO
THIRD FLOOR

3/4" = 1'-0"



SW#3 - THIRD FLOOR TO
ROOF

3/4" = 1'-0"



SW#3 - ROOF TO HIGH
ROOF

3/4" = 1'-0"

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GREENPARK GROUP

PROJECT NUMBER: 19023

SHEAR WALL
DETAILS

DESIGNED BY:

DATE:

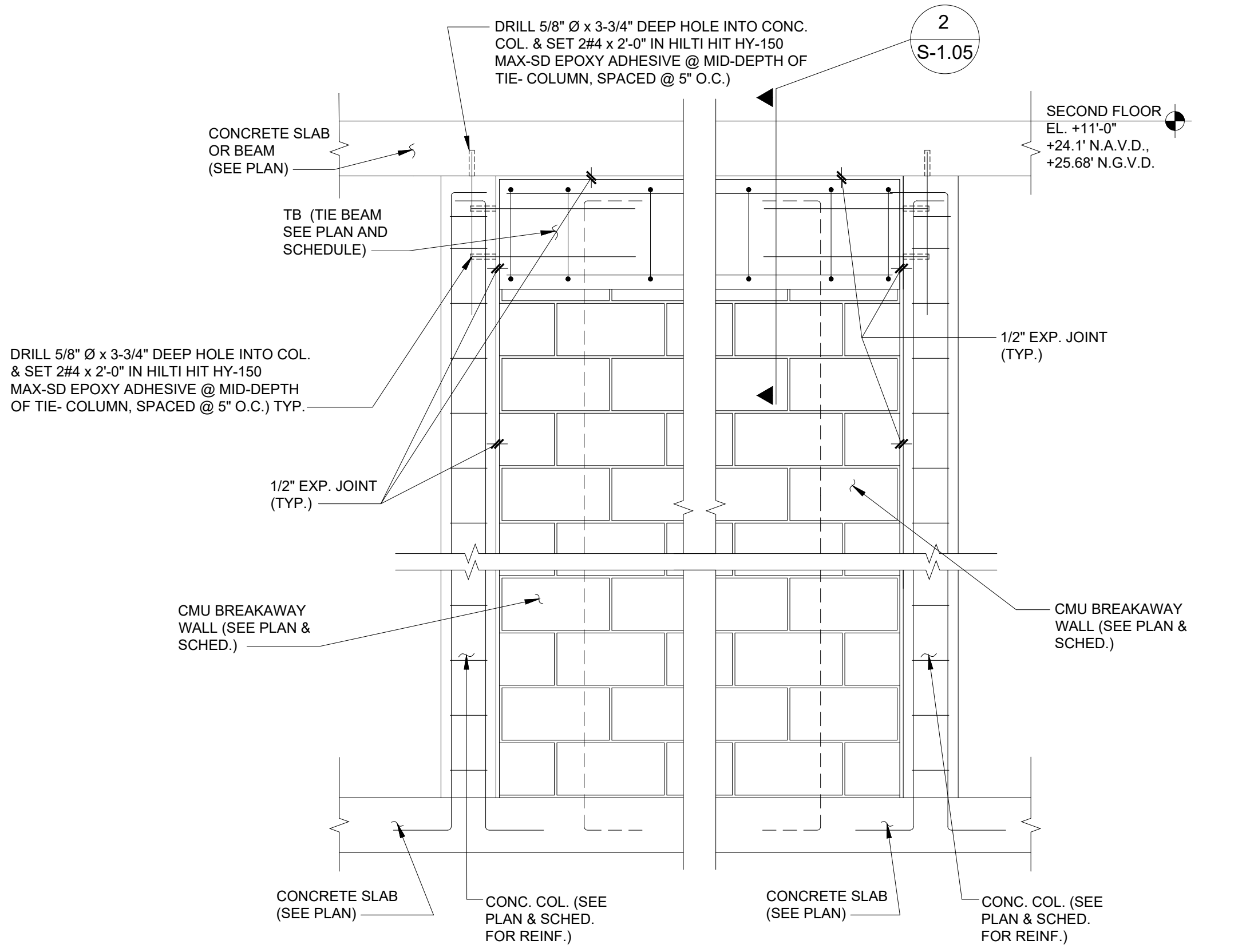
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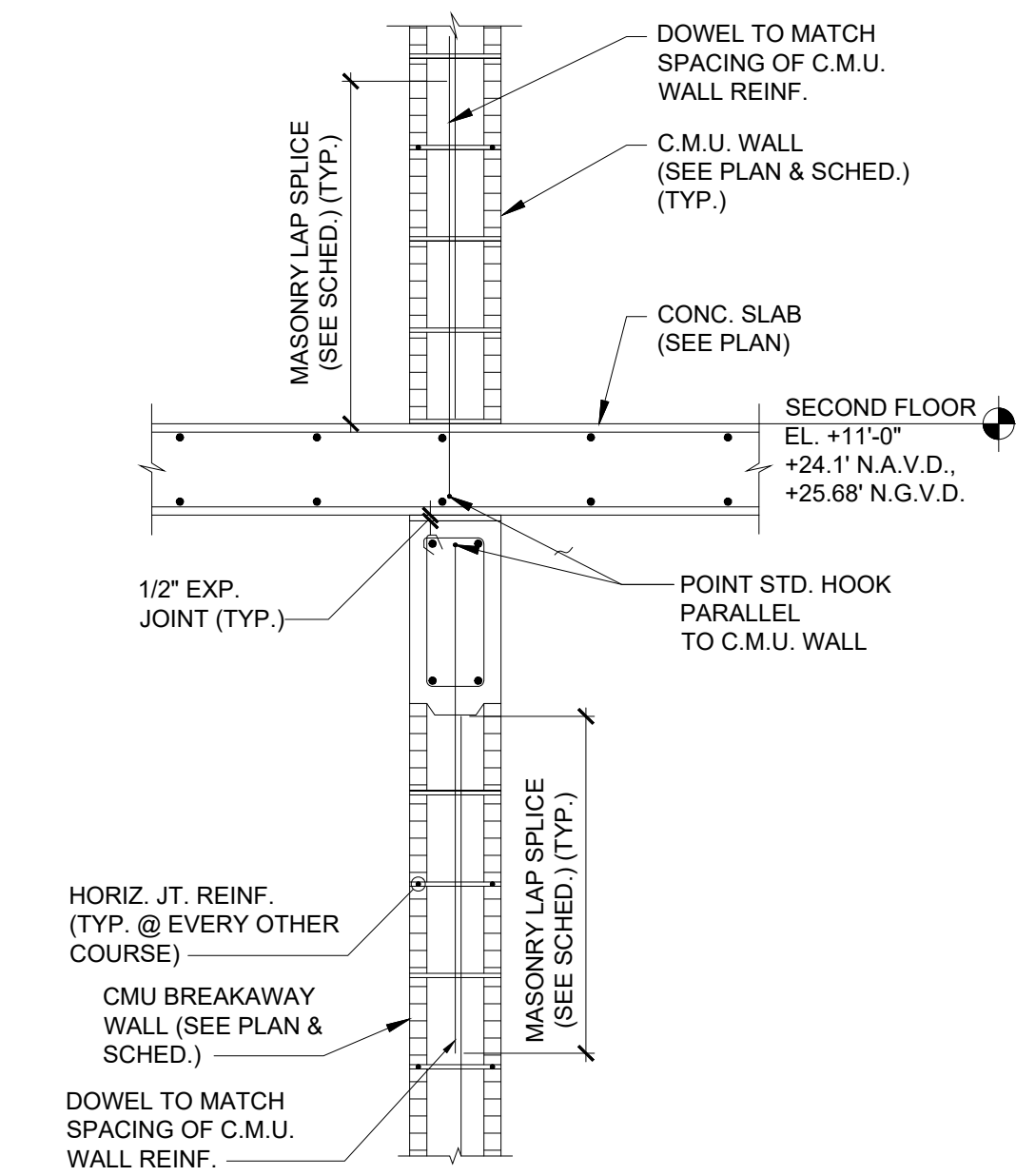
11/15/2018



TYPICAL BREAK AWAY WALL & COL. ELEVATION
AT CONCRETE BEAMS OR SLABS

3/4" = 1'-0"

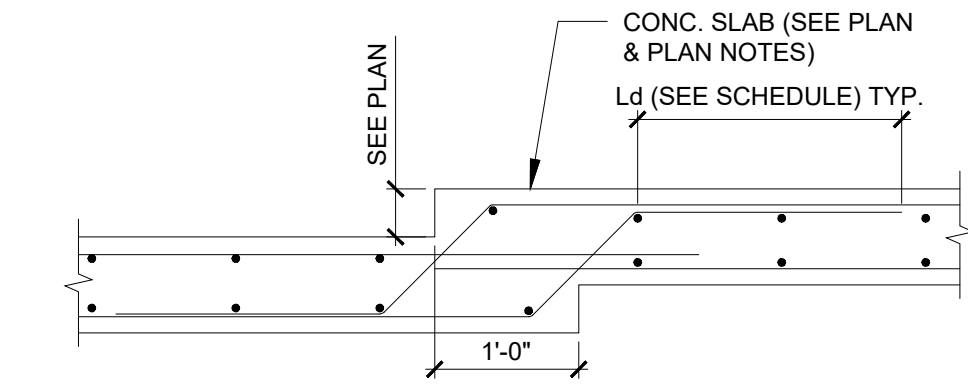
1
S-1.05



TYP. NON-BEARING WALL
(BREAKAWAY WALL)

3/4" = 1'-0"

2
S-1.05



TYPICAL STEP IN SLAB DETAIL

3/4" = 1'-0"

3
S-1.05

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GREENPARK GROUP

PROJECT NUMBER: 19023

TYPICAL DETAILS

DESIGNED BY:

DATE:

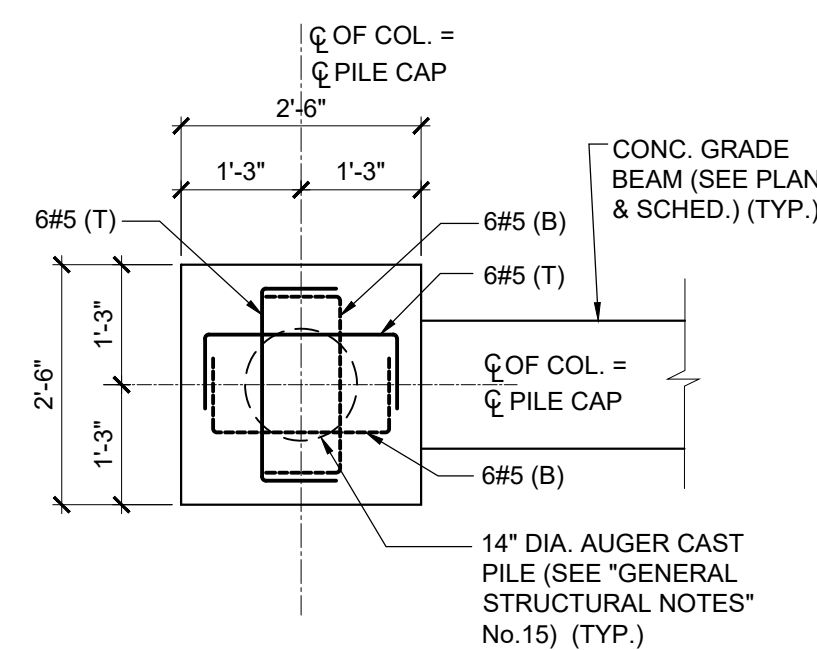
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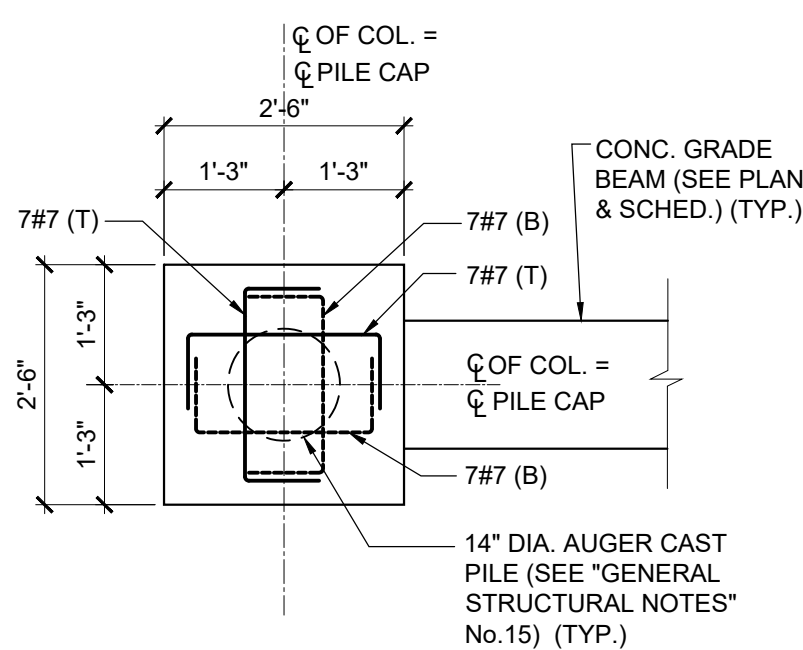
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11/15/2018



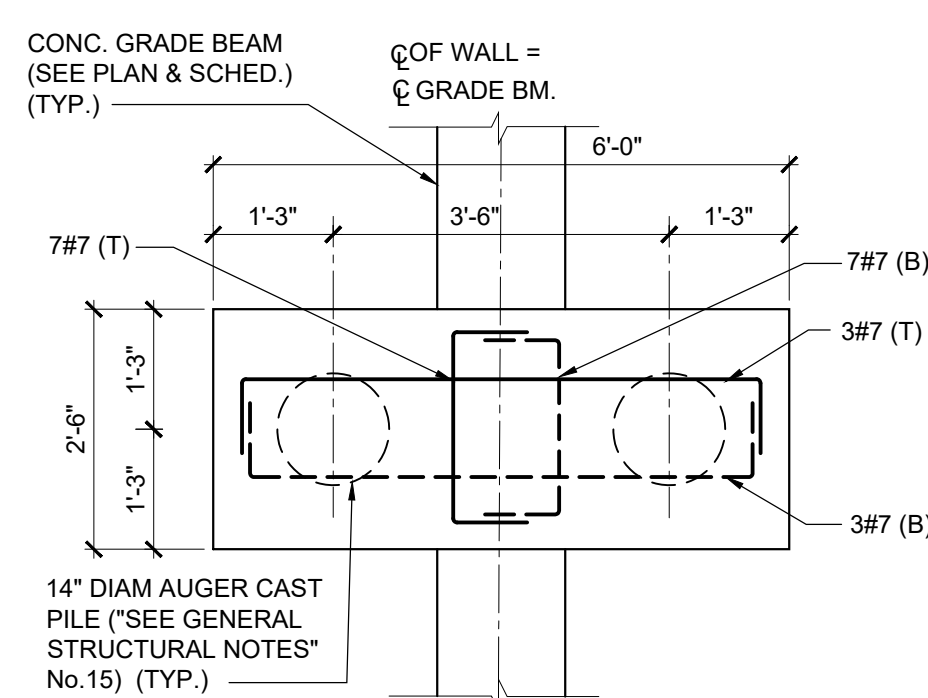
1PC-A

1/2" = 1'-0" DEPTH = 30 INCHES



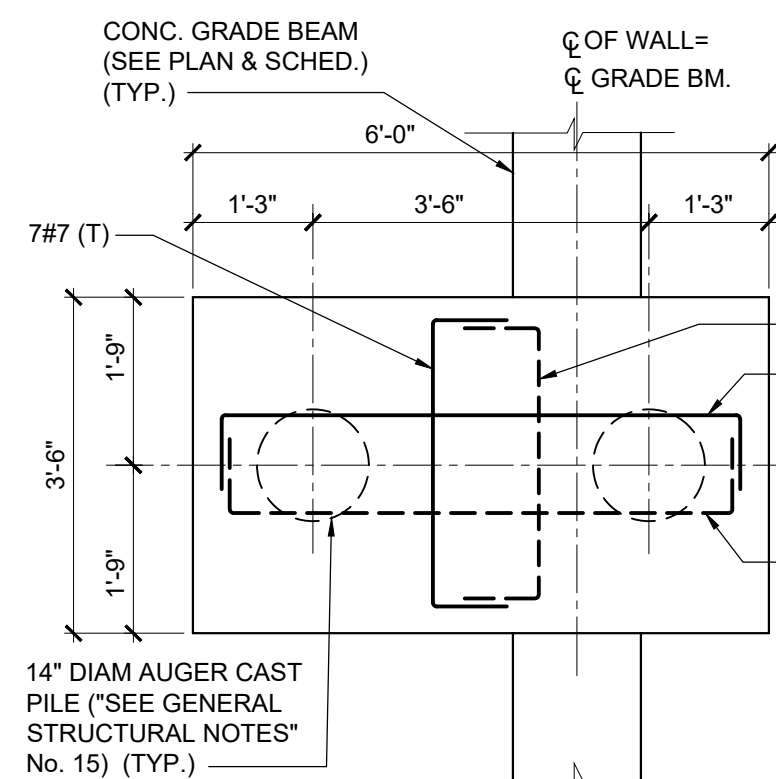
1PC-B

1/2" = 1'-0" DEPTH = 30 INCHES



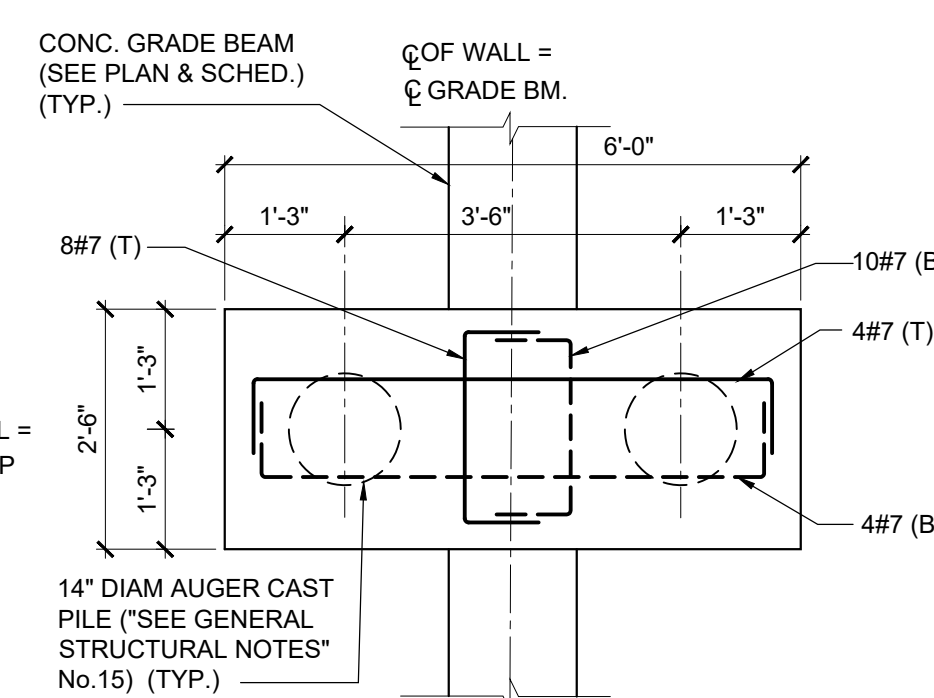
2PC-A

1/2" = 1'-0" DEPTH = 30 INCHES



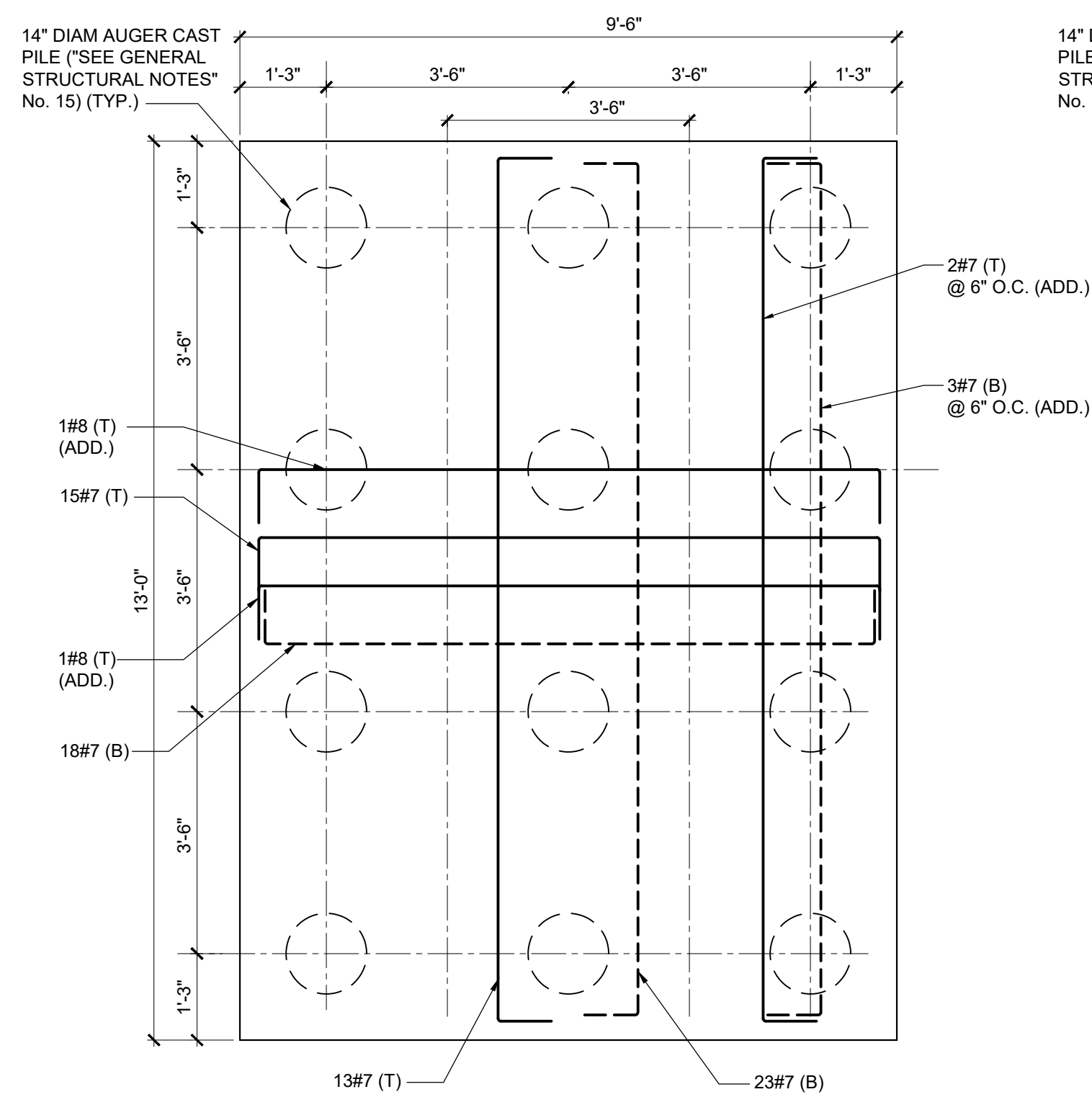
2PC-B

1/2" = 1'-0" DEPTH = 30 INCHES



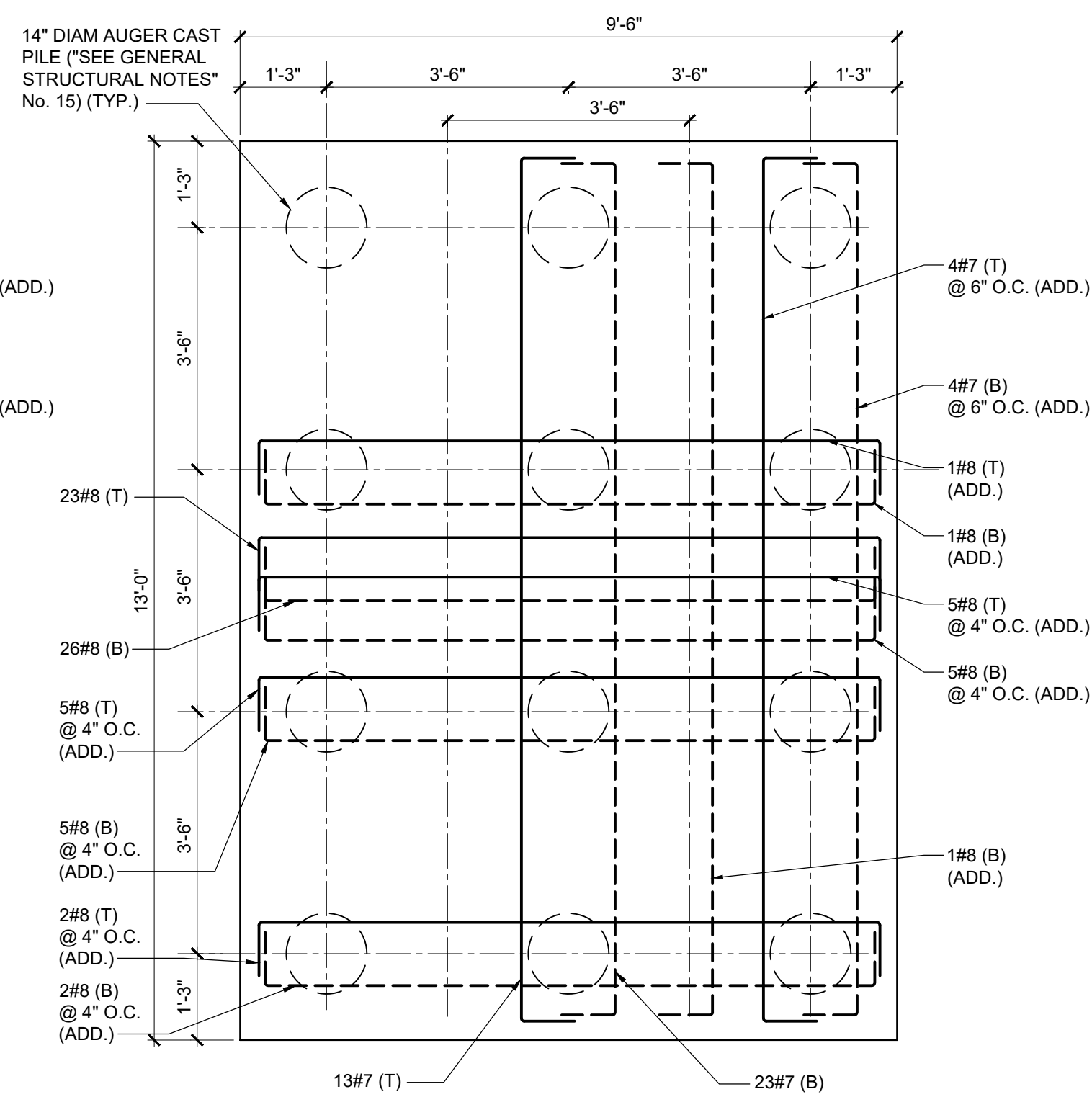
2PC-C

1/2" = 1'-0" DEPTH = 30 INCHES



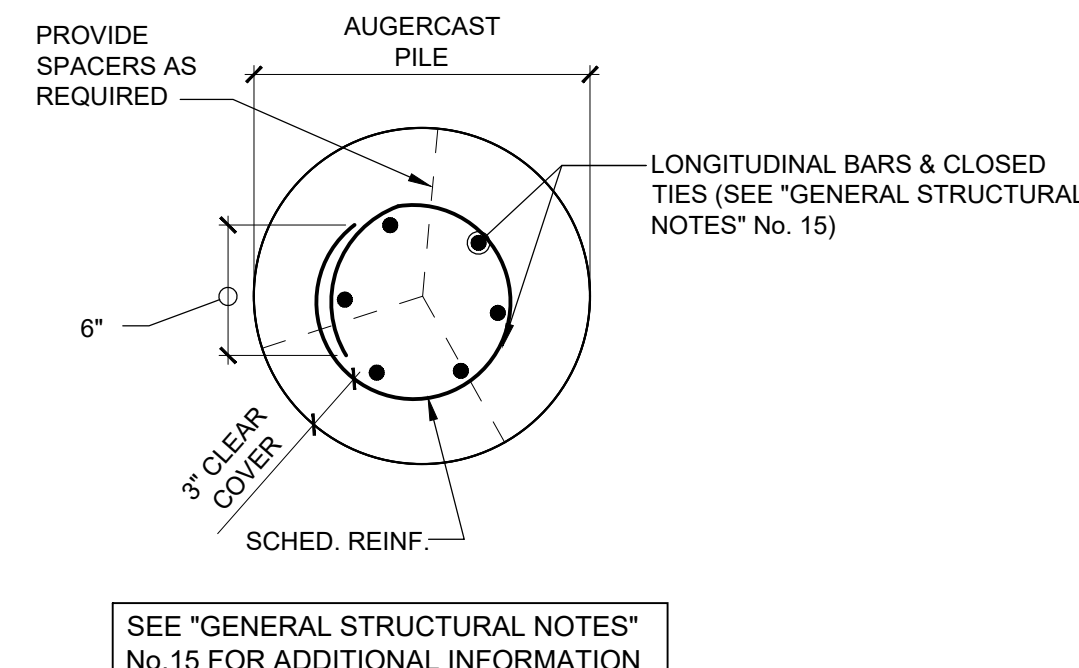
12PC-A

1/2" = 1'-0" DEPTH = 30 INCHES



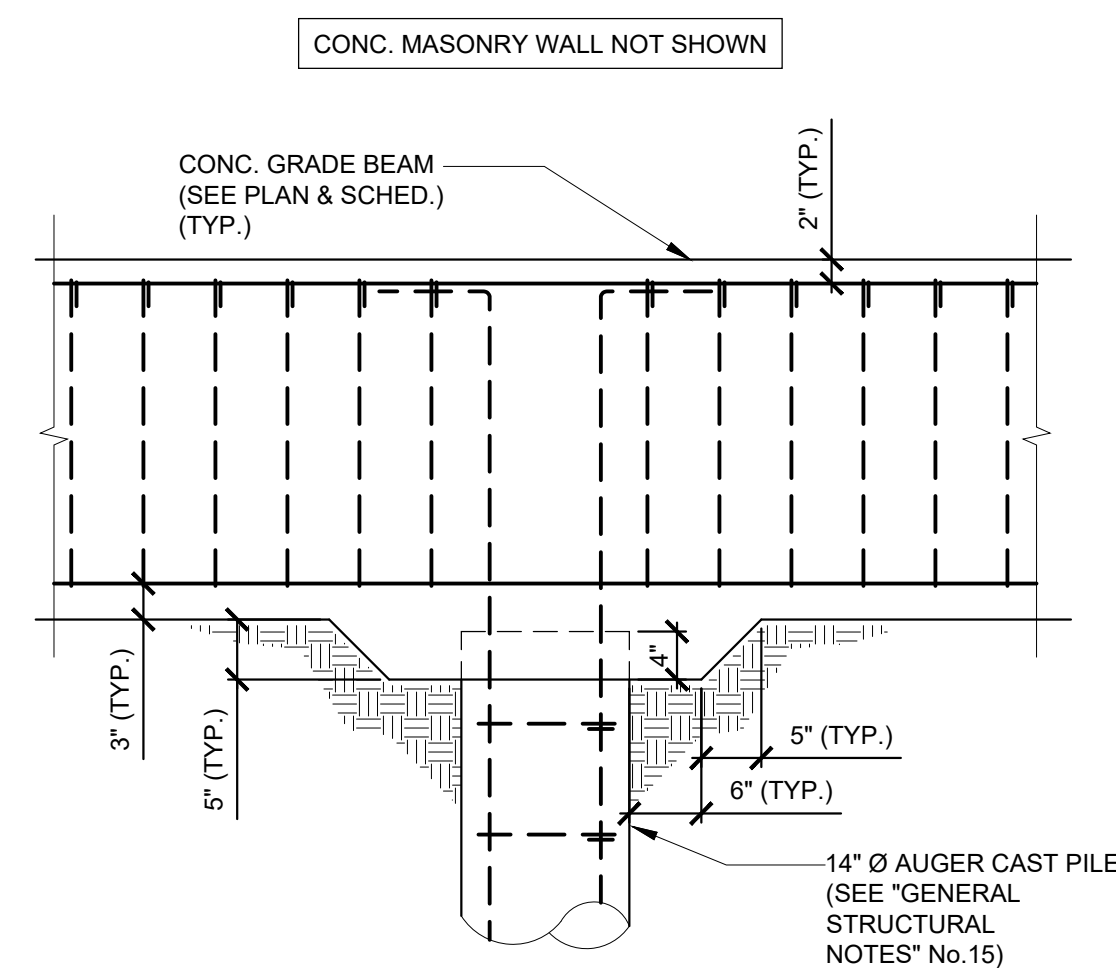
12PC-B

1/2" = 1'-0" DEPTH = 30 INCHES



TYP. AUGERCAST PILE REINF. DETAIL

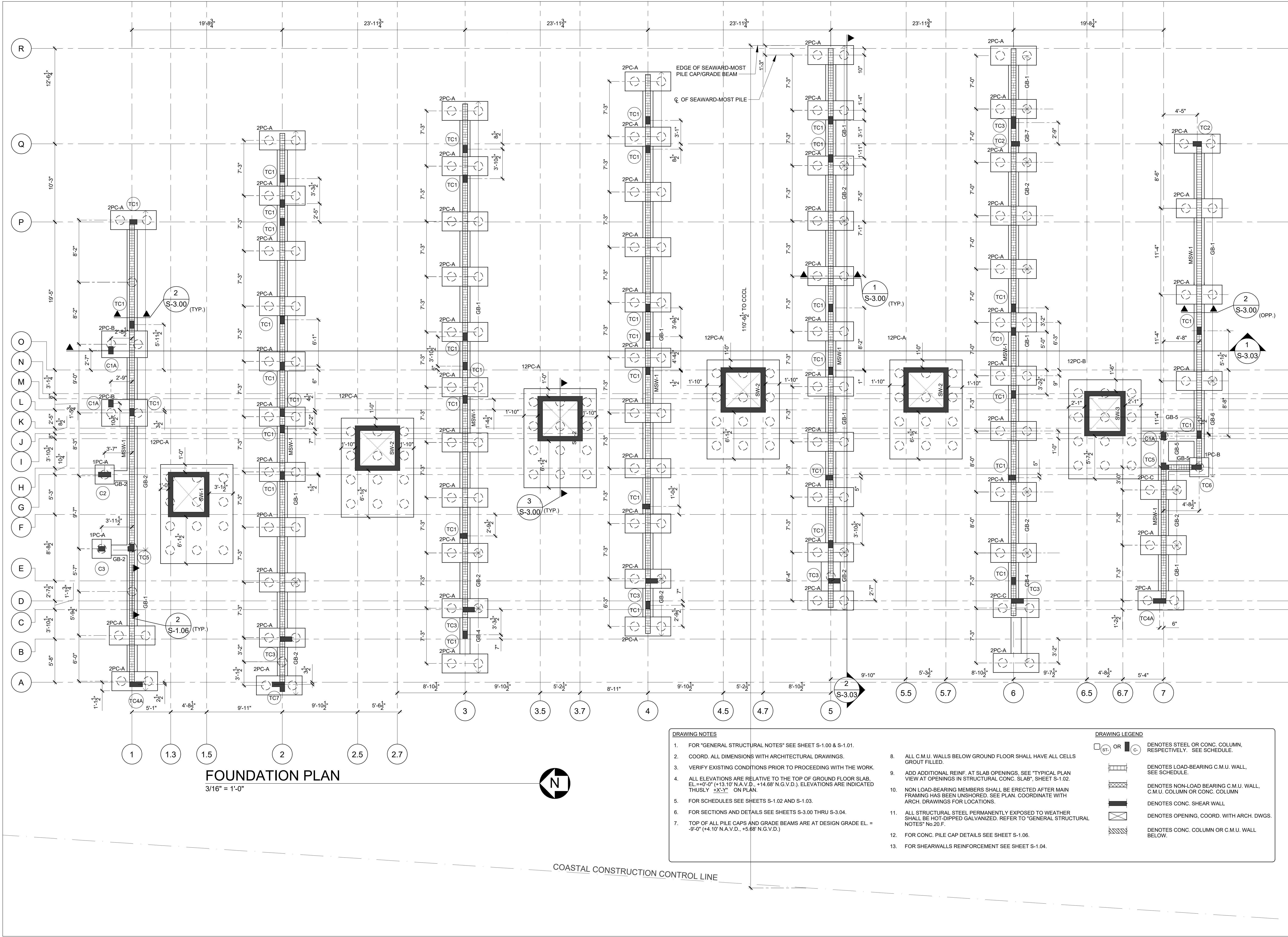
1-1/2" = 1'-0"

1
S-1.06

TYP. G.B. HAUNCH SECTION

3/4" = 1'-0"

2
S-1.06



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GREENPARK GROUP

PROJECT NUMBER: 19023

FOUNDATION PLAN

DESIGNED BY:

DATE:

ISSUED FOR

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S-2.00

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DRAWING NOTES

1. FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01.

2. COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

3. VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK.

4. ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB, EL.=+0'-0" (+13.10' N.A.V.D., +14.68' N.G.V.D.). ELEVATIONS ARE INDICATED THUSLY: +X'-Y" ON PLAN.

5. FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03.

6. FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.

7. TOP OF ALL PILE CAPS AND GRADE BEAMS ARE AT DESIGN GRADE EL. = -9'-0" (+4.10' N.A.V.D., +5.68' N.G.V.D.)

8. ALL C.M.U. WALLS BELOW GROUND FLOOR SHALL HAVE ALL CELLS GROUT FILLED.

9. ADD ADDITIONAL REINF. AT SLAB OPENINGS. SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB", SHEET S-1.02.

10. NON LOAD-BEARING MEMBERS SHALL BE ERECTED AFTER MAIN FRAMING HAS BEEN UNSHORED. SEE PLAN, COORDINATE WITH ARCH. DRAWINGS FOR LOCATIONS.

11. ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED. REFER TO "GENERAL STRUCTURAL NOTES" No.20.F.

12. FOR CONC. PILE CAP DETAILS SEE SHEET S-1.06.

13. FOR SHEARWALLS REINFORCEMENT SEE SHEET S-1.04.

DRAWING LEGEND

☐ ST. OR ☐ C.

DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE.

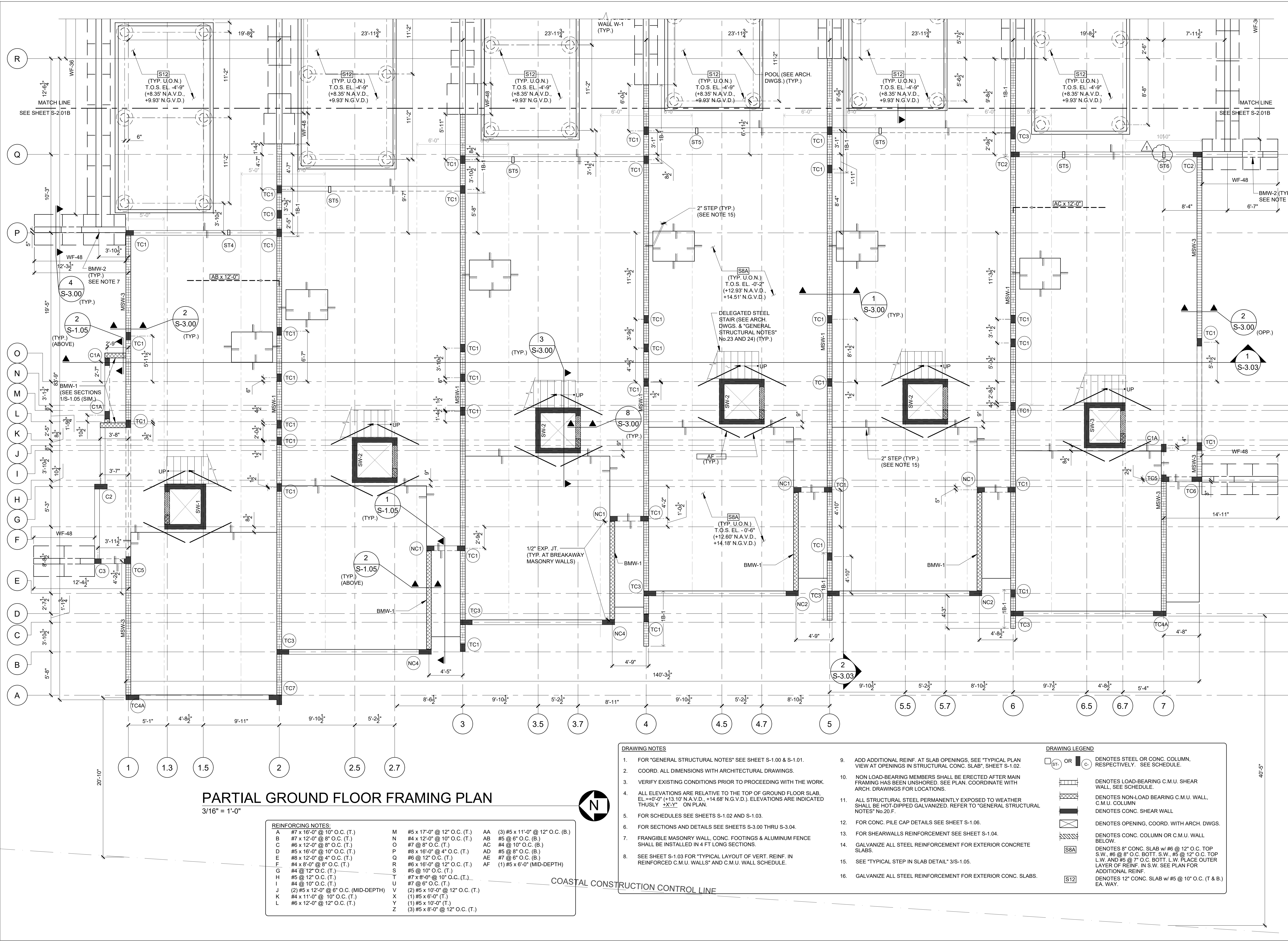
DENOTES LOAD-BEARING C.M.U. WALL, SEE SCHEDULE.

DENOTES NON-LOAD BEARING C.M.U. WALL, C.M.U. COLUMN OR CONC. COLUMN

DENOTES CONC. SHEAR WALL

DENOTES OPENING, COORD. WITH ARCH. DWGS.

DENOTES CONC. COLUMN OR C.M.U. WALL BELOW.



PARTIAL GROUND FLOOR FRAMING PLAN
3/16" = 1'-0"

REINFORCING NOTES:			
A	#7 x 16'-0" @ 10' O.C. (T.)	M	#5 x 17'-0" @ 12" O.C. (T.)
B	#7 x 12'-0" @ 8" O.C. (T.)	N	#4 x 12'-0" @ 10" O.C. (T.)
C	#6 x 12'-0" @ 8" O.C. (T.)	O	#7 @ 8" O.C. (T.)
D	#5 x 16'-0" @ 10" O.C. (T.)	P	#8 x 16'-0" @ 4" O.C. (T.)
E	#8 x 12'-0" @ 4" O.C. (T.)	Q	#6 @ 12" O.C. (T.)
F	#4 x 8'-0" @ 8" O.C. (T.)	R	#6 x 16'-0" @ 12" O.C. (T.)
G	#4 @ 12" O.C. (T.)	S	#5 @ 10" O.C. (T.)
H	#5 @ 12" O.C. (T.)	T	#7 x 8'-0" @ 10" O.C. (T.)
I	#4 @ 10" O.C. (T.)	U	#7 @ 6" O.C. (T.)
J	(2) #5 x 12'-0" @ 8" O.C. (MID-DEPTH)	V	(2) #5 x 10'-0" @ 12" O.C. (T.)
K	#4 x 11'-0" @ 10" O.C. (T.)	X	(1) #5 x 6'-0" (T.)
L	#6 x 12'-0" @ 12" O.C. (T.)	Y	(1) #5 x 10'-0" (T.)
		Z	(3) #5 x 8'-0" @ 12" O.C. (T.)

- DRAWING NOTES**
- FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01.
 - COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
 - VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK.
 - ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB, EL.=+0'-0" (+13.10' N.A.V.D., +14.68' N.G.V.D.). ELEVATIONS ARE INDICATED THUSLY "+X'-Y" ON PLAN.
 - FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03.
 - FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.
 - FRANGIBLE MASONRY WALL, CONC. FOOTINGS & ALUMINUM FENCE SHALL BE INSTALLED IN 4 FT LONG SECTIONS.
 - SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE.
 - ADD ADDITIONAL REINF. AT SLAB OPENINGS. SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB", SHEET S-1.02.
 - NON LOAD-BEARING MEMBERS SHALL BE ERRECTED AFTER MAIN FRAMING HAS BEEN UNSHORED. SEE PLAN. COORDINATE WITH ARCH. DRAWINGS FOR LOCATIONS.
 - ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED. REFER TO "GENERAL STRUCTURAL NOTES" No.20.F.
 - FOR CONC. PILE CAP DETAILS SEE SHEET S-1.06.
 - FOR SHEARWALLS REINFORCEMENT SEE SHEET S-1.04.
 - GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONCRETE SLABS.
 - SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05.
 - GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS.

- DRAWING LEGEND**
- | | | | |
|--|----|--|---|
| | OR | | DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE. |
| | | | DENOTES LOAD-BEARING C.M.U. SHEAR WALL. SEE SCHEDULE. |
| | | | DENOTES NON-LOAD BEARING C.M.U. WALL, C.M.U. COLUMN |
| | | | DENOTES CONC. SHEAR WALL |
| | | | DENOTES OPENING, COORD. WITH ARCH. DWGS. |
| | | | DENOTES CONC. COLUMN OR C.M.U. WALL BELOW. |
| | | | DENOTES 8" CONC. SLAB w/ #6 @ 12" O.C. TOP S.W., #6 @ 8" O.C. BOTT. S.W., #5 @ 12" O.C. TOP L.W. AND #5 @ 7" O.C. BOTT. L.W. PLACE OUTER LAYER OF REINF. IN S.W. SEE PLAN FOR ADDITIONAL REINF. |
| | | | DENOTES 12" CONC. SLAB w/ #5 @ 10" O.C. (T & B) EA. WAY. |

RNA

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700 Briny Ave, Pompano Beach, FL 33062
GREENPARK GROUP

PROJECT NUMBER: 19023

PARTIAL GROUND FLOOR FRAMING PLAN

DESIGNED BY:

DATE:
REVISION 01-31-2020

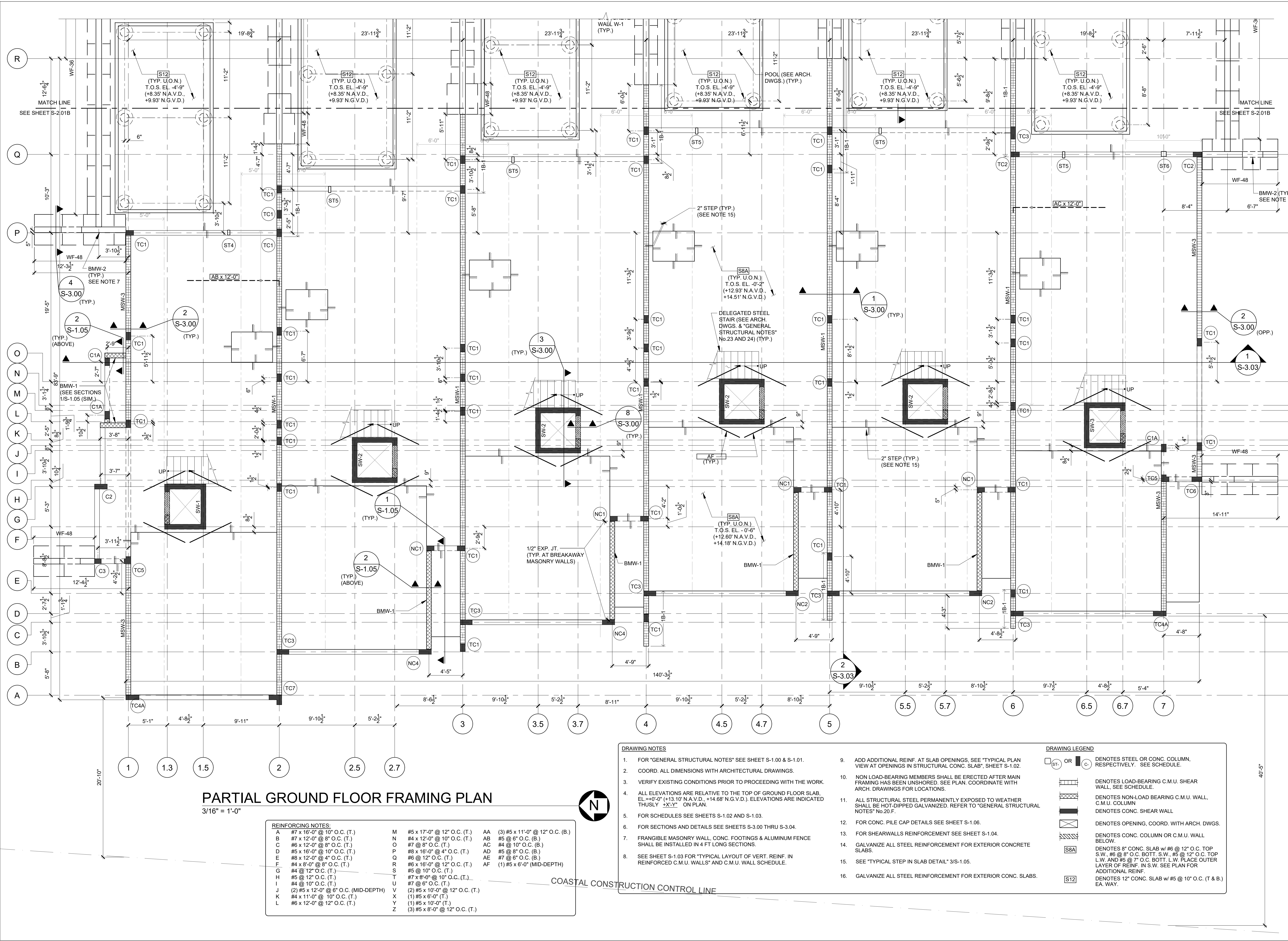
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11/15/2019



PARTIAL GROUND FLOOR FRAMING PLAN
3/16" = 1'-0"

REINFORCING NOTES:			
A	#7 x 16'-0" @ 10' O.C. (T.)	M	#5 x 17'-0" @ 12" O.C. (T.)
B	#7 x 12'-0" @ 8" O.C. (T.)	N	#4 x 12'-0" @ 10" O.C. (T.)
C	#6 x 12'-0" @ 8" O.C. (T.)	O	#7 @ 8" O.C. (T.)
D	#5 x 16'-0" @ 10" O.C. (T.)	P	#8 x 16'-0" @ 4" O.C. (T.)
E	#8 x 12'-0" @ 4" O.C. (T.)	Q	#6 @ 12" O.C. (T.)
F	#4 x 8'-0" @ 8" O.C. (T.)	R	#6 x 16'-0" @ 12" O.C. (T.)
G	#4 @ 12" O.C. (T.)	S	#5 @ 10" O.C. (T.)
H	#5 @ 12" O.C. (T.)	T	#7 x 8'-0" @ 10" O.C. (T.)
I	#4 @ 10" O.C. (T.)	U	#7 @ 6" O.C. (T.)
J	(2) #5 x 12'-0" @ 8" O.C. (MID-DEPTH)	V	(2) #5 x 10'-0" @ 12" O.C. (T.)
K	#4 x 11'-0" @ 10" O.C. (T.)	X	(1) #5 x 6'-0" (T.)
L	#6 x 12'-0" @ 12" O.C. (T.)	Y	(1) #5 x 10'-0" (T.)
		Z	(3) #5 x 8'-0" @ 12" O.C. (T.)

DRAWING NOTES

- FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01.
- COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK.
- ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB, EL.=+0'-0" (+13.10' N.A.V.D., +14.68' N.G.V.D.). ELEVATIONS ARE INDICATED THUSLY "+X'-Y" ON PLAN.
- FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03.
- FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.
- FRANGIBLE MASONRY WALL, CONC. FOOTINGS & ALUMINUM FENCE SHALL BE INSTALLED IN 4 FT LONG SECTIONS.
- SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE.

- ADD ADDITIONAL REINF. AT SLAB OPENINGS. SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB", SHEET S-1.02.
- NON LOAD-BEARING MEMBERS SHALL BE ERRECTED AFTER MAIN FRAMING HAS BEEN UNSHORED. SEE PLAN. COORDINATE WITH ARCH. DRAWINGS FOR LOCATIONS.
- ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED. REFER TO "GENERAL STRUCTURAL NOTES" No.20.F.
- FOR CONC. PILE CAP DETAILS SEE SHEET S-1.06.
- FOR SHEARWALLS REINFORCEMENT SEE SHEET S-1.04.
- GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONCRETE SLABS.
- SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05.
- GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS.

DRAWING LEGEND

- OR ○ DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE.
- ▤ DENOTES LOAD-BEARING C.M.U. SHEAR WALL. SEE SCHEDULE.
- ▥ DENOTES NON-LOAD BEARING C.M.U. WALL, C.M.U. COLUMN.
- ▧ DENOTES CONC. SHEAR WALL.
- ▨ DENOTES OPENING, COORD. WITH ARCH. DWGS.
- ▩ DENOTES CONC. COLUMN OR C.M.U. WALL BELOW.
- DENOTES 8" CONC. SLAB w/ #6 @ 12" O.C. TOP S.W., #6 @ 8" O.C. BOTT. S.W., #5 @ 12" O.C. TOP L.W. AND #5 @ 7" O.C. BOTT. L.W. PLACE OUTER LAYER OF REINF. IN S.W. SEE PLAN FOR ADDITIONAL REINF.
- DENOTES 12" CONC. SLAB w/ #5 @ 10" O.C. (T & B) EA. WAY.

RNA

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700 Briny Ave, Pompano Beach, FL 33062

GREENPARK GROUP

PROJECT NUMBER: 19023

PARTIAL GROUND FLOOR FRAMING PLAN

DESIGNED BY:

DATE:

ISSUED FOR

- REVIEW & COMMENTS
- BIDDING
- NOT FOR CONSTRUCTION
- PERMIT
- CONSTRUCTION

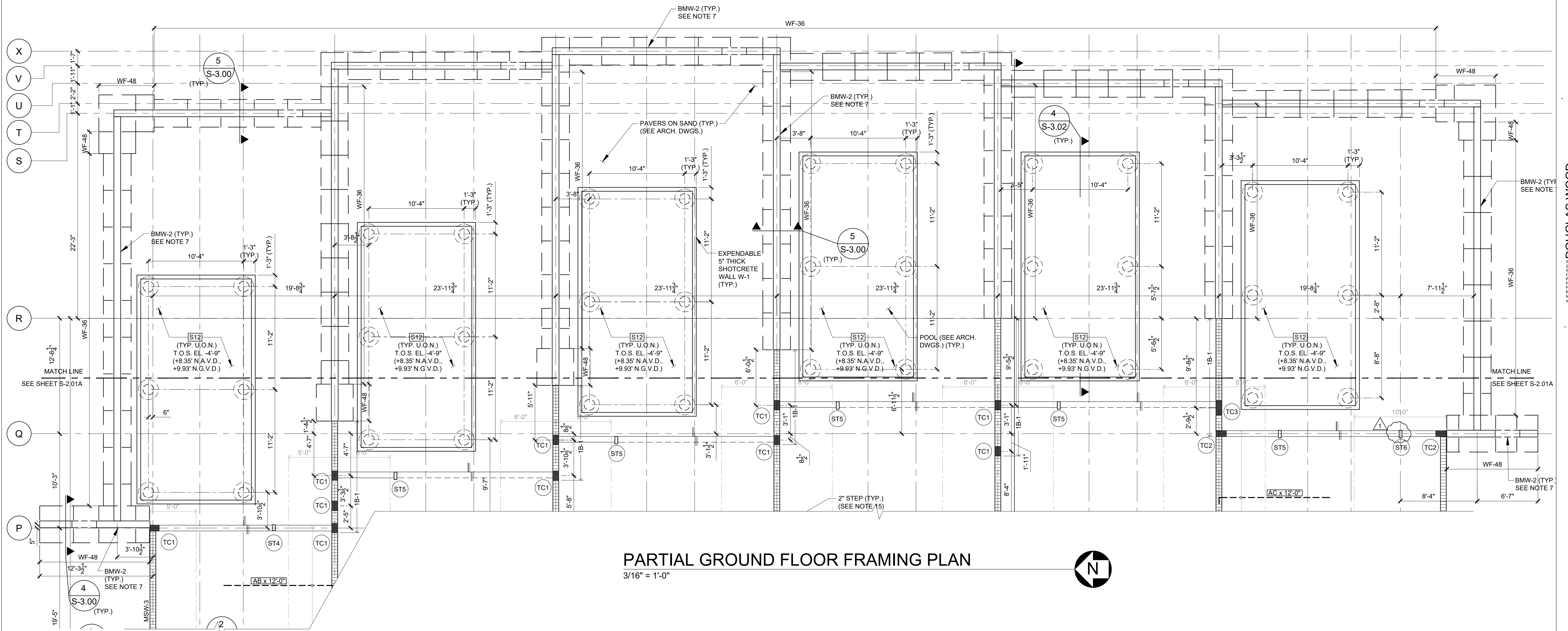
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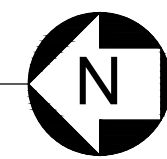
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CONSTRUCTION DOCUMENTS

11/15/2019



PARTIAL GROUND FLOOR FRAMING PLAN

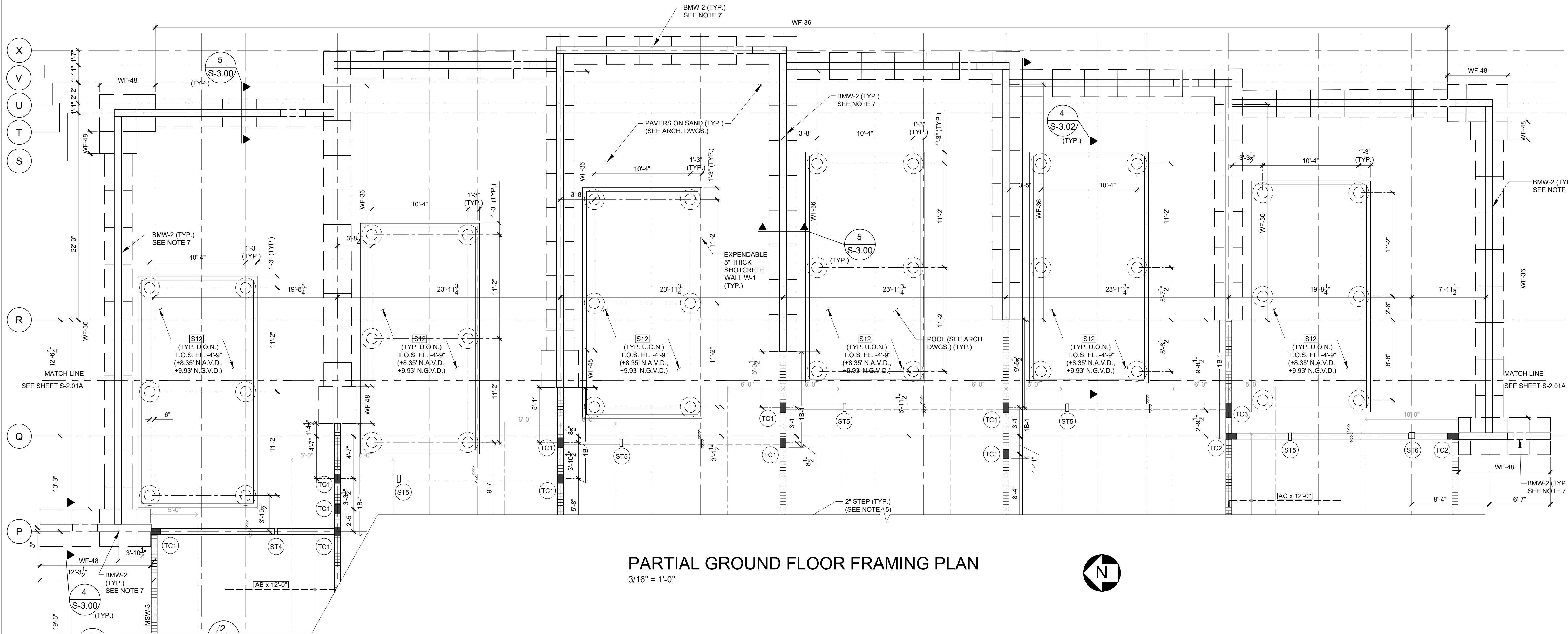
$$\frac{3}{16}'' = 1'-0''$$


DRAWING NOTES

- | | | | |
|----|---|-----|---|
| 1. | FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01. | 9. | ADD ADDITIONAL REINF. AT SLAB OPENINGS. SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB", SHEET S-1.02. |
| 2. | COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS. | 10. | NON LOAD-BEARING MEMBERS SHALL BE ERECTED AFTER MAIN FRAMING HAS BEEN UNSHORED. SEE PLAN. COORDINATE WITH ARCH. DRAWINGS FOR LOCATIONS. |
| 3. | VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK. | 11. | ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED. REFER TO "GENERAL STRUCTURAL NOTES" No 20.F. |
| 4. | ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB, EL.=0'+0" (+13.10' N.A.V.D., +14.68' N.G.V.D.). ELEVATIONS ARE INDICATED THUSLY $+X'-X''$ ON PLAN. | 12. | FOR CONC. PILE CAP DETAILS SEE SHEET S-1.06. |
| 5. | FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03. | 13. | FOR SHEARWALLS REINFORCEMENT SEE SHEET S-1.04. |
| 6. | FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04. | 14. | GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONCRETE SLABS. |
| 7. | FRANGIBLE MASONRY WALL, CONC. FOOTINGS & ALUMINUM FENCE SHALL BE INSTALLED IN A 4 FT LONG SECTIONS. | 15. | SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05. |
| 8. | SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE. | 16. | GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS. |

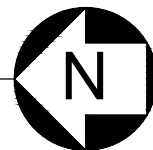
DRAWING LEGEND

- | | | | |
|---|----|---|---|
|  | OR |  | DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE. |
|  | | | DENOTES LOAD-BEARING C.M.U. SHEAR WALL, SEE SCHEDULE. |
|  | | | DENOTES NON-LOAD BEARING C.M.U. WALL, C.M.U. COLUMN |
|  | | | DENOTES CONC. SHEAR WALL |
|  | | | DENOTES OPENING, COORD. WITH ARCH. DWGS. |
|  | | | DENOTES CONC. COLUMN OR C.M.U. WALL BELOW. |
|  | | | DENOTES 8" CONC. SLAB w/ #6 @ 12" O.C. TOP T.S.W. #6 @ 8" O.C. BOTT. T.S.W. #5 @ 12" O.C. TOP T.W. AND #5 @ 7" O.C. BOTT. T.W. PLACE OUTER LAYER OF REINF. IN S.W. SEE PLAN FOR ADDITIONAL REINF. |
|  | | | DENOTES 12" CONC. SLAB w/ #5 @ 10" O.C. (T & B.) EA. WAY. |



PARTIAL GROUND FLOOR FRAMING PLAN

3/16" = 1'-0"

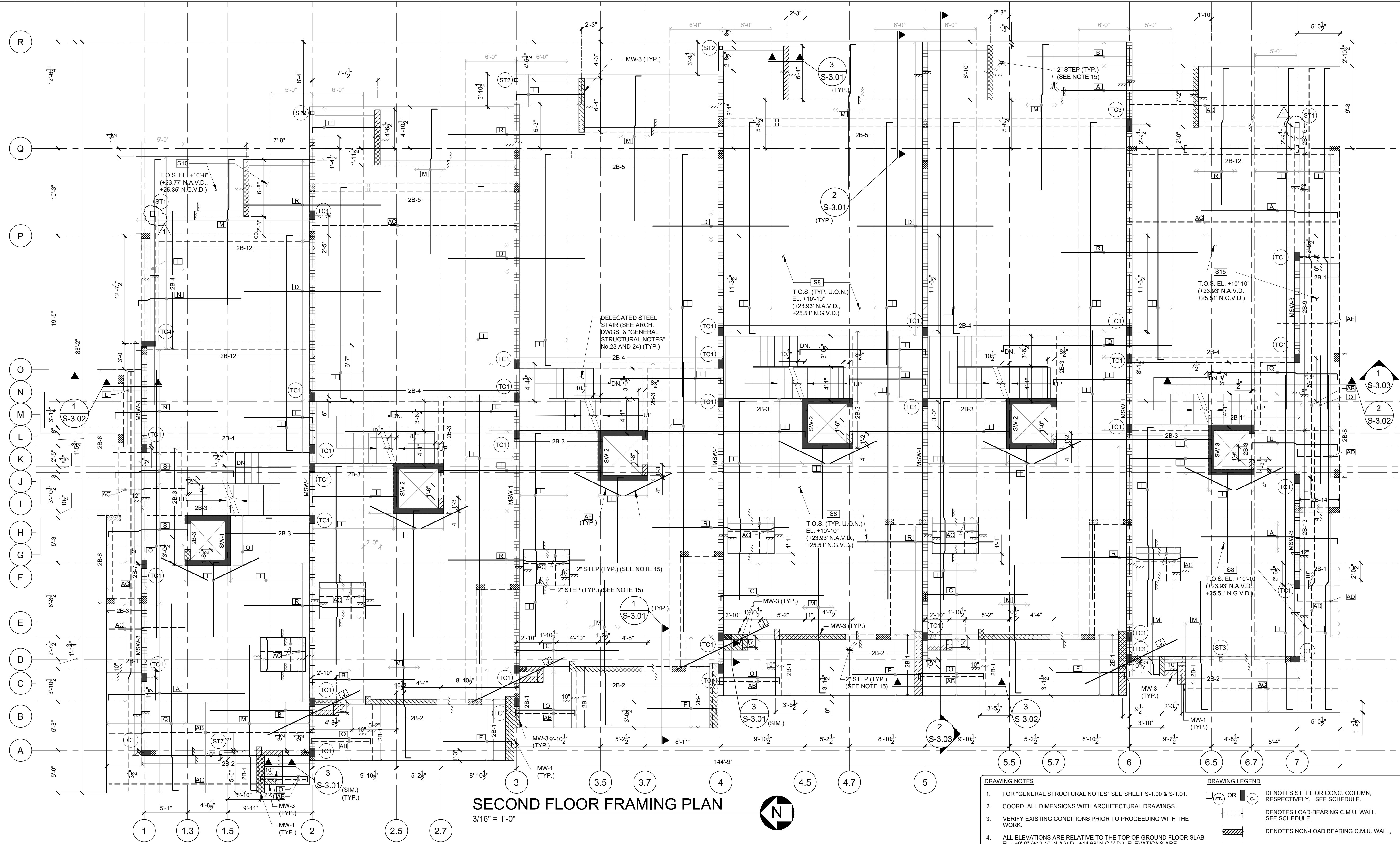


DRAWING NOTES

- FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01.
- COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK.
- ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB, EL.=+0'-0" (+13.10' N.A.V.D., +14.68' N.G.V.D.). ELEVATIONS ARE INDICATED THUSLY +X'-Y" ON PLAN.
- FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03.
- FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.
- FRANGIBLE MASONRY WALL, CONC. FOOTINGS & ALUMINUM FENCE SHALL BE INSTALLED IN 4 FT LONG SECTIONS.
- SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE.

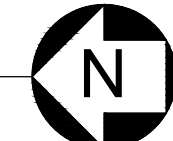
DRAWING LEGEND

- ☐ ST OR ☐ C DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE.
- ☐ DENOTES LOAD-BEARING C.M.U. SHEAR WALL. SEE SCHEDULE.
- ☐ DENOTES NON-LOAD BEARING C.M.U. WALL, C.M.U. COLUMN.
- ☐ DENOTES CONC. SHEAR WALL.
- ☐ DENOTES OPENING, COORD. WITH ARCH. DWGS.
- ☐ DENOTES CONC. COLUMN OR C.M.U. WALL BELOW.
- ☐ S8A DENOTES 8" CONC. SLAB w/ #6 @ 12" O.C. TOP S.W., #6 @ 8" O.C. BOTT. S.W., #5 @ 12" O.C. TOP L.W. AND #5 @ 7" O.C. BOTT. L.W. PLACE OUTER LAYER OF REINF. IN S.W. SEE PLAN FOR ADDITIONAL REINF.
- ☐ S12 DENOTES 12" CONC. SLAB w/ #5 @ 10" O.C. (T & B.) EA. WAY.



SECOND FLOOR FRAMING PLAN

3/16" = 1'-0"



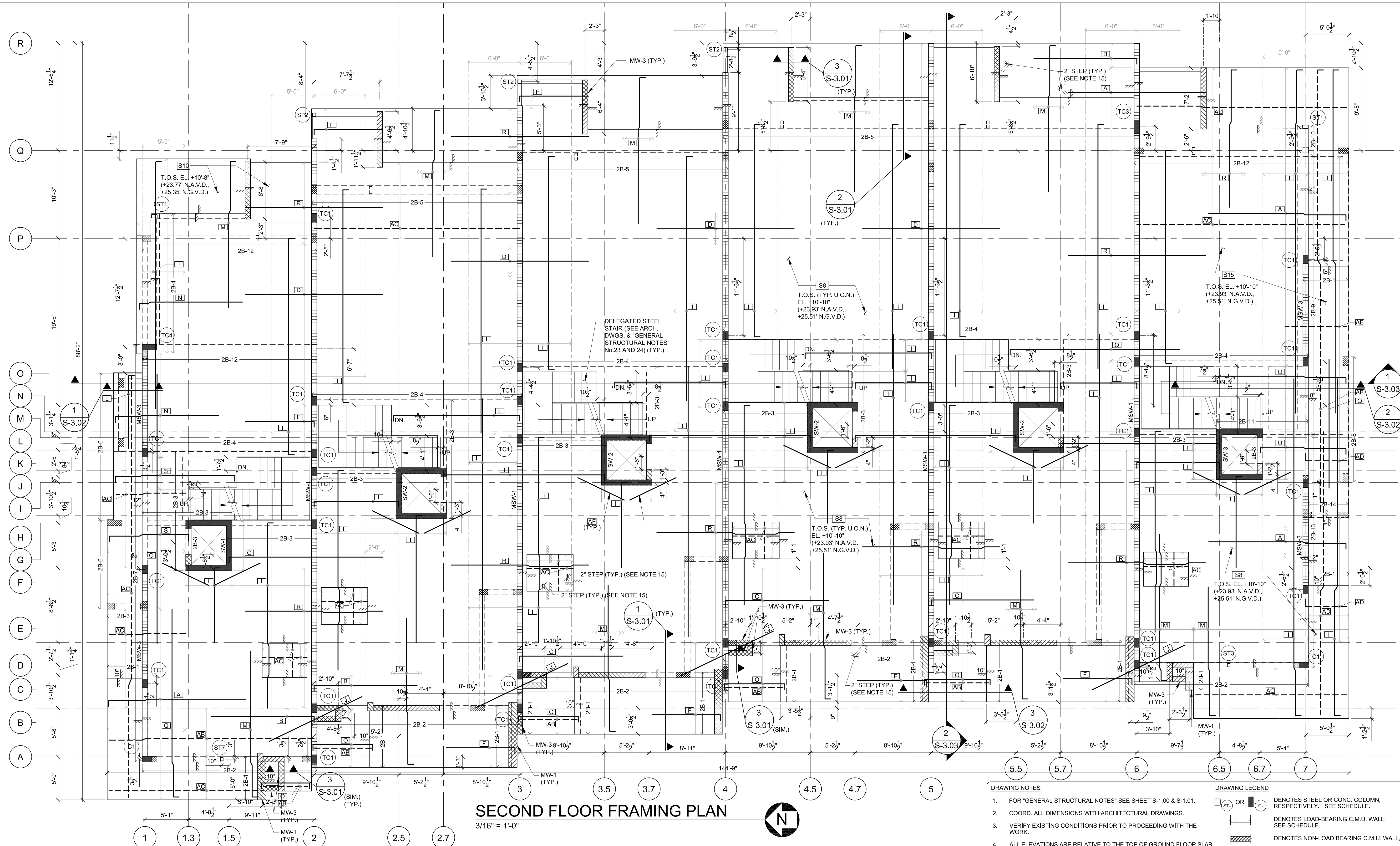
REINFORCING NOTES:			
A	#7 x 16'-0" @ 10" O.C. (T.)	AA	(3) #5 x 11'-0" @ 12" O.C. (B.)
B	#7 x 12'-0" @ 8" O.C. (T.)	AB	#5 @ 6" O.C. (B.)
C	#6 x 12'-0" @ 8" O.C. (T.)	AC	#4 @ 10" O.C. (B.)
D	#5 x 16'-0" @ 10" O.C. (T.)	AD	#5 @ 8" O.C. (B.)
E	#8 x 12'-0" @ 4" O.C. (T.)	AE	#7 @ 6" O.C. (B.)
F	#4 x 8'-0" @ 8" O.C. (T.)	AF	(1) #5 x 6'-0" (MID-DEPTH)
G	#4 @ 12" O.C. (T.)		
H	#5 @ 12" O.C. (T.)		
I	#4 @ 10" O.C. (T.)		
J	(2) #5 x 12'-0" @ 6" O.C. (MID-DEPTH)		
K	#4 x 11'-0" @ 10" O.C. (T.)		
L	#6 x 12'-0" @ 12" O.C. (T.)		
M	#5 x 17'-0" @ 12" O.C. (T.)		
N	#4 x 12'-0" @ 10" O.C. (T.)		
O	#7 @ 8" O.C. (T.)		
P	#8 x 16'-0" @ 4" O.C. (T.)		
Q	#6 @ 12" O.C. (T.)		
R	#6 x 16'-0" @ 12" O.C. (T.)		
S	#5 @ 10" O.C. (T.)		
T	#7 x 8'-0" @ 10" O.C. (T.)		
U	#7 @ 6" O.C. (T.)		
V	(2) #5 x 10'-0" @ 12" O.C. (T.)		
X	(1) #5 x 6'-0" (T.)		
Y	(1) #5 x 10'-0" (T.)		
Z	(3) #5 x 8'-0" @ 12" O.C. (T.)		

DRAWING NOTES

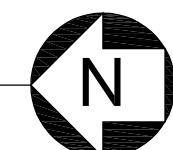
- FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01.
- COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK.
- ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB, EL. = +0'-0" (+13.10' N.A.V.D., +14.68' N.G.V.D.). ELEVATIONS ARE INDICATED THUSLY ±X'-Y" ON PLAN.
- FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03.
- FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.
- SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE.
- ADD ADDITIONAL REINF. AT SLAB OPENINGS. SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB", SHEET S-1.02.
- NON LOAD-BEARING MEMBERS SHALL BE ERECTED AFTER MAIN FRAMING HAS BEEN UNSHORED. SEE PLAN, COORDINATE WITH ARCH. DRAWINGS FOR LOCATIONS.
- ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED. REFER TO "GENERAL STRUCTURAL NOTES" No.20.F.
- FOR CONC. PILE CAP DETAILS SEE SHEET S-1.06.
- FOR SHEARWALL REINFORCEMENT SEE SHEET S-1.04.
- FOR CONC. SLAB REINF., PLACE OUTER BAR LAYERS IN SHORT DIRECTION.
- GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS.
- SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05.

DRAWING LEGEND

- ST OR C DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE.
- DENOTES LOAD-BEARING C.M.U. WALL, SEE SCHEDULE.
- DENOTES NON-LOAD BEARING C.M.U. WALL.
- DENOTES CONC. SHEAR WALL
- DENOTES OPENING, COORD. WITH ARCH. DWGS.
- DENOTES CONC. COLUMN OR C.M.U. WALL BELOW.
- S5 DENOTES 5" CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY.
- S5LW DENOTES 5" LIGHTWEIGHT CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY.
- S8 DENOTES 8" CONC. SLAB W/ #4 @ 12" O.C. (B.) LONG WAY & #5 @ 12" O.C. (B.) SHORT WAY. SEE PLAN FOR ADD. REINF.
- S10 DENOTES 10" CONC. SLAB W/ #5 @ 12" O.C. (B.) LONG WAY AND #6 @ 12" O.C. (B.) SHORT WAY.
- S15 DENOTES 15" CONC. SLAB W/ #7 @ 12" O.C. (B.) SHORT WAY AND #5 @ 10" O.C. (B.) LONG WAY.



SECOND FLOOR FRAMING PLAN

$$\overline{3/16'' = 1'-0''}$$


REINFORCING NOTES

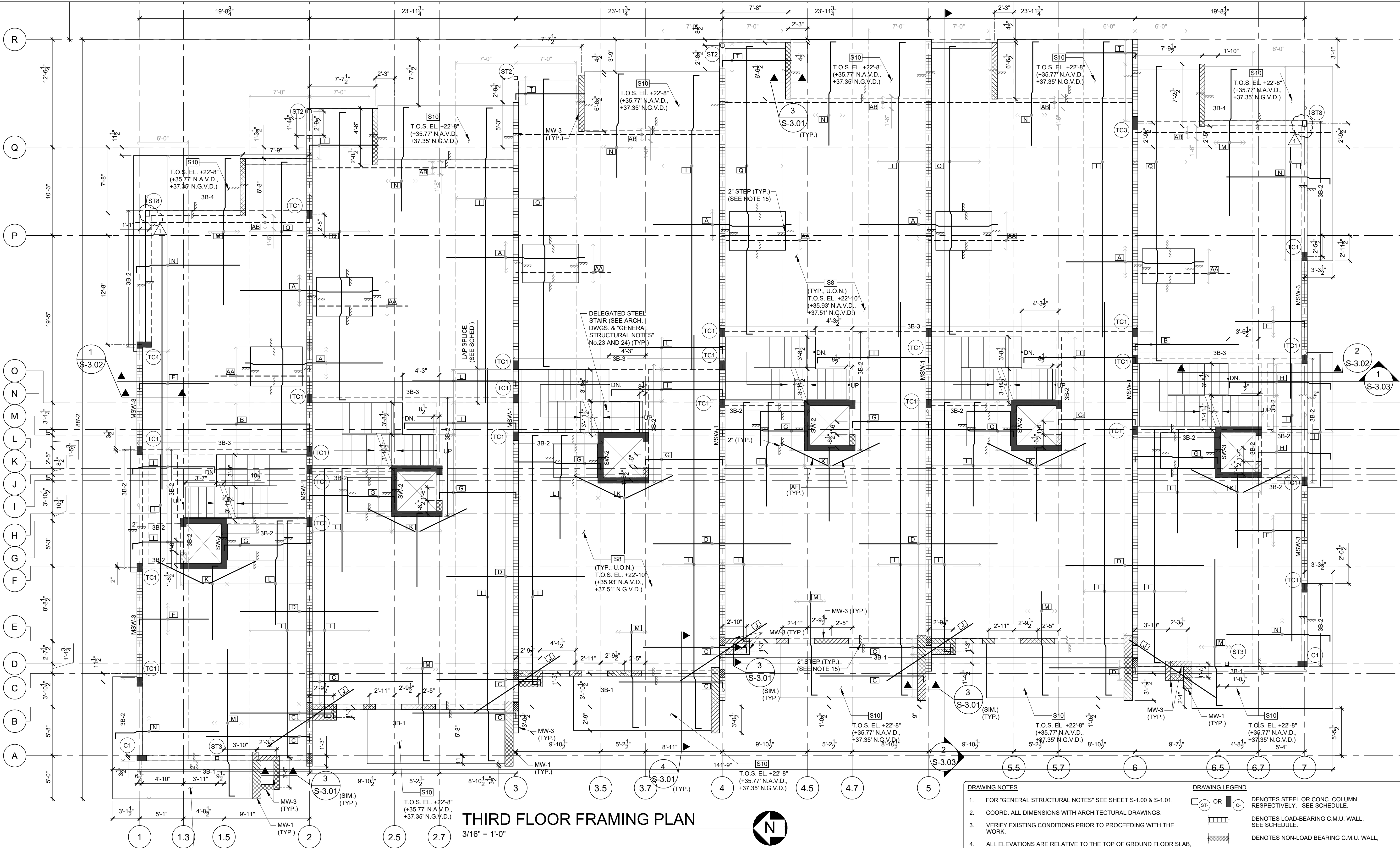
A	#7 x 16'-0" @ 10° O.C. (T.)	AA	(3) #5 x 11'-0" @ 12° O.C. (B.)
B	#7 x 12'-0" @ 8° O.C. (T.)	AB	#5 @ 8° O.C. (B.)
C	#7 x 12'-0" @ 8° O.C. (T.)	AC	#4 @ 10° O.C. (B.)
D	#5 x 16'-0" @ 10° O.C. (T.)	AD	#5 @ 8° O.C. (B.)
E	#8 x 12'-0" @ 4° O.C. (T.)	AE	#7 @ 6° O.C. (B.)
F	#4 x 8'-0" @ 8° O.C. (T.)	AF	(1) #5 x 6'-0" (MID-DEPTH)
G	#4 x 12'-0" @ 8° O.C. (T.)		
H	#5 x 12'-0" @ 8° O.C. (T.)		
I	#4 @ 10° O.C. (T.)		
J	(2) #5 x 12'-0" @ 6° O.C. (MID-DEPTH)		
K	#4 x 11'-0" @ 10° O.C. (T.)		
L	#6 x 12'-0" @ 12° O.C. (T.)		
M	#5 x 17'-0" @ 12° O.C. (T.)		
N	#4 x 12'-0" @ 10° O.C. (T.)		
O	#7 @ 8° O.C. (T.)		
P	#8 x 16'-0" @ 4° O.C. (T.)		
Q	#6 @ 12° O.C. (T.)		
R	#6 x 16'-0" @ 12° O.C. (T.)		
S	#5 @ 10° O.C. (T.)		
T	#7 x 8'-0" @ 10° O.C. (T.)		
U	#7 @ 6° O.C. (T.)		
V	(2) #5 x 10'-0" @ 12° O.C. (T.)		
X	(1) #5 x 8'-0" (T.)		
Y	(1) #5 x 10'-0" (T.)		
Z	(3) #5 x 8'-0" @ 12° O.C. (T.)		

DRAWING NOTES:

1. FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01.
2. COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
3. VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK.
4. ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB ELEV. $+0.00$ (+1.18' N.A.V.D., +1.48' U.S.V.D.). ELEVATIONS ARE INDICATED THUSLY "+X'-Y" ON PLAN.
5. FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03.
6. FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.
7. SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE.
8. ADD ADDITIONAL REINF. AT SLAB OPENINGS. SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB" SHEET S-1.02.
9. NON LOAD-BEARING MEMBERS SHALL BE ERRECTED AFTER MAIN FRAMING HAS BEEN UNSHORED. SEE PLAN, COORDINATE WITH ARCH. DRAWINGS FOR LOCATIONS.
10. ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED. REFER TO "GENERAL STRUCTURAL NOTES" No.20.
11. FOR CONC. PILE CAP DETAILS SEE SHEET S-1.06.
12. FOR SHEARWALL REINFORCEMENT SEE SHEET S-1.04.
13. FOR CONC. SLAB REINF., PLACE OUTER BAR LAYERS IN SHORT DIRECTION.
14. GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS.
15. SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05.

DRAWING LEGEND

	ST- OR C- DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE.
	DENOTES LOAD-BEARING C.M.U. WALL, SEE SCHEDULE.
	DENOTES NON-LOAD BEARING C.M.U. WALL.
	DENOTES CONC. SHEAR WALL
	DENOTES OPENING, COORD. WITH ARCH. DWGS.
	DENOTES CONC. COLUMN OR C.M.U. WALL BELOW.
	S5 DENOTES 5" CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY.
	S5LW DENOTES 5" LIGHTWEIGHT CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY.
	S8 DENOTES 8" CONC. SLAB W/ #4 @ 12" O.C. (B.) LONG WAY & #5 @ 12" O.C. (B.) SHORT WAY. SEE PLAN FOR ADD. REINF.
	S10 DENOTES 10" CONC. SLAB W/ #5 @ 12" O.C. (B.) LONG WAY AND #6 @ 12" O.C. (B.) SHORT WAY.
	S15 DENOTES 15" CONC. SLAB W/ #7 @ 12" O.C. (B.) SHORT WAY AND #5 @ 10" O.C. (B.) LONG WAY.



THIRD FLOOR FRAMING PLAN

3/16" = 1'-0"

REINFORCING NOTES:			
A	#7 x 16'-0" @ 10' O.C. (T.)	AA	(3) #5 x 11'-0" @ 12" O.C. (B.)
B	#7 x 12'-0" @ 8' O.C. (T.)	AB	#5 @ 6" O.C. (B.)
C	#6 x 12'-0" @ 8' O.C. (T.)	AC	#4 @ 10' O.C. (B.)
D	#5 x 16'-0" @ 10' O.C. (T.)	AD	#5 @ 8' O.C. (B.)
E	#5 x 12'-0" @ 4' O.C. (T.)	AE	#7 @ 8' O.C. (B.)
F	#4 x 8'-0" @ 8' O.C. (T.)	AF	(1) #5 x 6'-0" (MID-DEPTH)
G	#4 @ 12' O.C. (T.)		
H	#5 @ 12' O.C. (T.)		
I	#4 @ 10' O.C. (T.)		
J	(2) #5 x 12'-0" @ 6' O.C. (MID-DEPTH)		
K	#4 x 11'-0" @ 10' O.C. (T.)		
L	#6 x 12'-0" @ 12' O.C. (T.)		
M	#5 x 17'-0" @ 12' O.C. (T.)		
N	#4 x 12'-0" @ 10' O.C. (T.)		
O	#7 @ 8' O.C. (T.)		
P	#8 x 16'-0" @ 4' O.C. (T.)		
Q	#6 @ 12' O.C. (T.)		
R	#5 x 16'-0" @ 12' O.C. (T.)		
S	#5 @ 10' O.C. (T.)		
T	#7 x 8'-0" @ 10' O.C. (T.)		
U	#7 @ 6' O.C. (T.)		
V	(2) #5 x 10'-0" @ 12' O.C. (T.)		
X	(1) #5 x 6'-0" (T.)		
Y	(1) #5 x 10'-0" (T.)		
Z	(3) #5 x 8'-0" @ 12' O.C. (T.)		

DRAWING NOTES

- FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01.
- COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK.
- ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB, EL.=+0'-0" (+13.10' N.A.V.D., +14.68' N.G.V.D.). ELEVATIONS ARE INDICATED THUSLY: "X'-Y" ON PLAN.
- FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03.
- FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.
- SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE.
- ADD ADDITIONAL REINF. AT SLAB OPENINGS, SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB", SHEET S-1.02.
- NON LOAD-BEARING MEMBERS SHALL BE ERECTED AFTER MAIN FRAMING HAS BEEN UNSHORED. SEE PLAN, COORDINATE WITH ARCH. DRAWINGS FOR LOCATIONS.
- ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED. REFER TO "GENERAL STRUCTURAL NOTES" No.20.F.
- FOR CONC. PILE CAP DETAILS SEE SHEET S-1.06.
- FOR SHEARWALL REINFORCEMENT SEE SHEET S-1.04.
- FOR CONC. SLAB REINF., PLACE OUTER BAR LAYERS IN SHORT DIRECTION.
- GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS.
- SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05.

DRAWING LEGEND

- | | | | |
|--|----|--|---|
| | OR | | DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE. |
| | | | DENOTES LOAD-BEARING C.M.U. WALL, SEE SCHEDULE. |
| | | | DENOTES NON-LOAD BEARING C.M.U. WALL. |
| | | | DENOTES CONC. SHEAR WALL |
| | | | DENOTES OPENING, COORD. WITH ARCH. DWGS. |
| | | | DENOTES CONC. COLUMN OR C.M.U. WALL BELOW. |
| | | | DENOTES 5" CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY. |
| | | | DENOTES 5" LIGHTWEIGHT CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY. |
| | | | DENOTES 8" CONC. SLAB W/ #4 @ 12" O.C. (B.) LONG WAY & #5 @ 12" O.C. (B.) SHORT WAY. SEE PLAN FOR ADD. REINF. |
| | | | DENOTES 10" CONC. SLAB W/ #5 @ 12" O.C. (B.) LONG WAY AND #6 @ 12" O.C. (B.) SHORT WAY. |
| | | | DENOTES 15" CONC. SLAB W/ #7 @ 12" O.C. (B.) LONG WAY AND #5 @ 10" O.C. (B.) LONG WAY. |

RNA

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www.douglaswood.biz

700 Briny Ave, Pompano Beach, FL 33062

GREENPARK GROUP

PROJECT NUMBER: 19023

THIRD FLOOR
FRAMING PLAN

DESIGNED BY:

DATE:

REVISION 01-31-2020

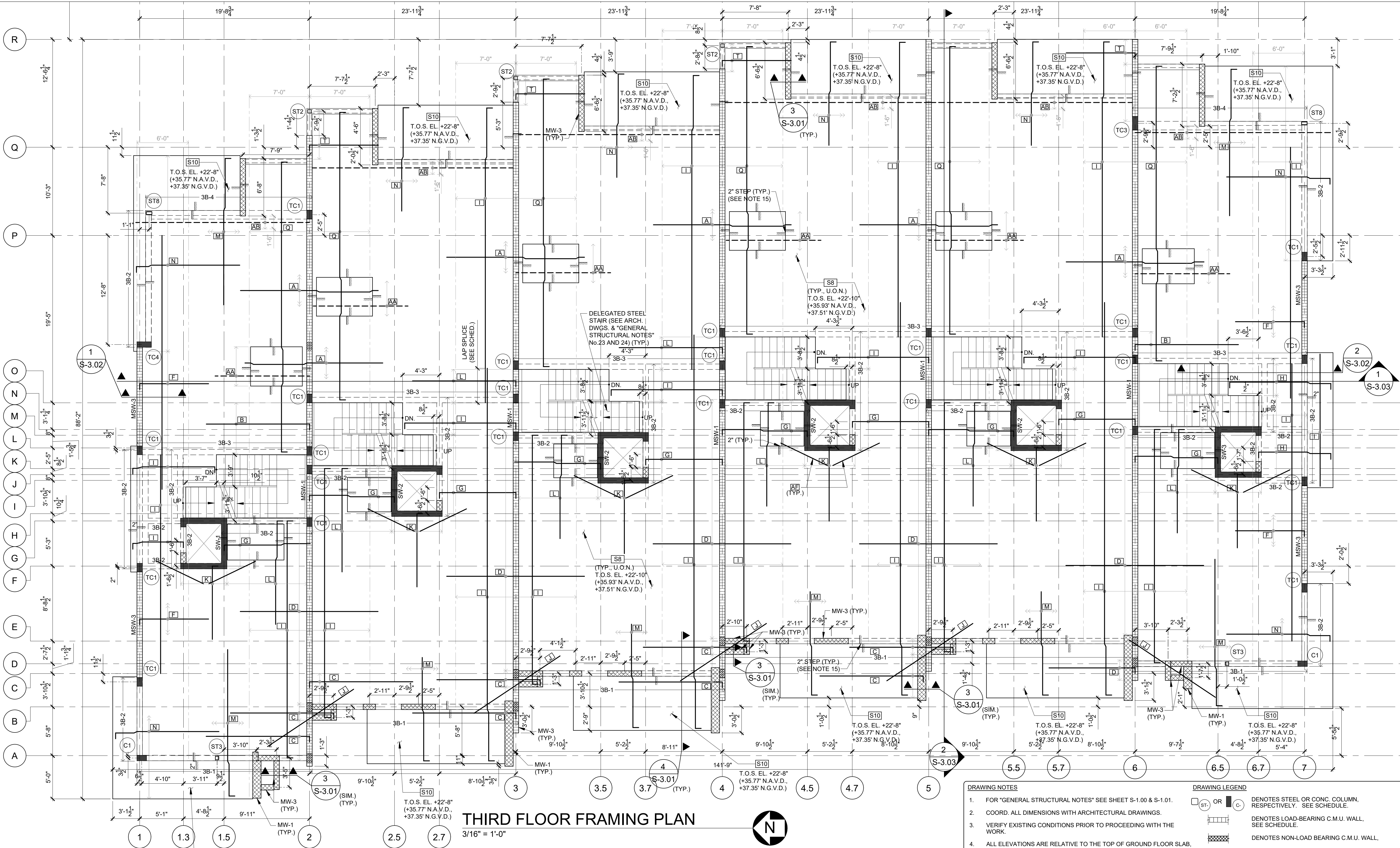
ISSUED FOR

- REVIEW & COMMENTS
- BIDDING
- NOT FOR CONSTRUCTION
- PERMIT
- CONSTRUCTION

S-2.03

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THIRD FLOOR FRAMING PLAN

3/16" = 1'-0"



REINFORCING NOTES:

A	#7 x 16'-0" @ 10" O.C. (T.)	AA	(3) #5 x 11'-0" @ 12" O.C. (B.)
B	#7 x 12'-0" @ 8" O.C. (T.)	AB	#5 @ 6" O.C. (B.)
C	#6 x 12'-0" @ 8" O.C. (T.)	AC	#4 @ 10" O.C. (B.)
D	#5 x 16'-0" @ 10" O.C. (T.)	AD	#5 @ 8" O.C. (B.)
E	#5 x 12'-0" @ 4" O.C. (T.)	AE	#7 @ 8" O.C. (B.)
F	#4 x 8'-0" @ 8" O.C. (T.)	AF	(1) #5 x 6'-0" (MID-DEPTH)
G	#4 @ 12" O.C. (T.)		
H	#5 @ 12" O.C. (T.)		
I	#4 @ 10" O.C. (T.)		
J	(2) #5 x 12'-0" @ 6" O.C. (MID-DEPTH)		
K	#4 x 11'-0" @ 10" O.C. (T.)		
L	#6 x 12'-0" @ 12" O.C. (T.)		
M	#5 x 17'-0" @ 12" O.C. (T.)		
N	#4 x 12'-0" @ 10" O.C. (T.)		
O	#7 @ 8" O.C. (T.)		
P	#8 x 16'-0" @ 4" O.C. (T.)		
Q	#6 @ 12" O.C. (T.)		
R	#5 x 16'-0" @ 12" O.C. (T.)		
S	#5 @ 10" O.C. (T.)		
T	#7 x 8'-0" @ 10" O.C. (T.)		
U	#7 @ 6" O.C. (T.)		
V	(2) #5 x 10'-0" @ 12" O.C. (T.)		
X	(1) #5 x 6'-0" (T.)		
Y	(1) #5 x 10'-0" (T.)		
Z	(3) #5 x 8'-0" @ 12" O.C. (T.)		

DRAWING NOTES

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- COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
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- FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.
- SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE.
- ADD ADDITIONAL REINF. AT SLAB OPENINGS, SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB", SHEET S-1.02.
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- GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS.
- SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05.

DRAWING LEGEND

	OR		DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE.
			DENOTES LOAD-BEARING C.M.U. WALL, SEE SCHEDULE.
			DENOTES NON-LOAD BEARING C.M.U. WALL.
			DENOTES CONC. SHEAR WALL
			DENOTES OPENING, COORD. WITH ARCH. DWGS.
			DENOTES CONC. COLUMN OR C.M.U. WALL BELOW.
			DENOTES 5" CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY.
			DENOTES 5" LIGHTWEIGHT CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY.
			DENOTES 8" CONC. SLAB W/ #4 @ 12" O.C. (B.) LONG WAY & #5 @ 12" O.C. (B.) SHORT WAY. SEE PLAN FOR ADD. REINF.
			DENOTES 10" CONC. SLAB W/ #5 @ 12" O.C. (B.) LONG WAY AND #6 @ 12" O.C. (B.) SHORT WAY.
			DENOTES 15" CONC. SLAB W/ #7 @ 12" O.C. (B.) SHORT WAY AND #5 @ 10" O.C. (B.) LONG WAY.

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E: dwood@douglaswood.biz
www.douglaswood.biz

700 Briny Ave, Pompano Beach, FL 33062

GREENPARK GROUP

PROJECT NUMBER: 19023

THIRD FLOOR FRAMING PLAN

DESIGNED BY:

DATE:

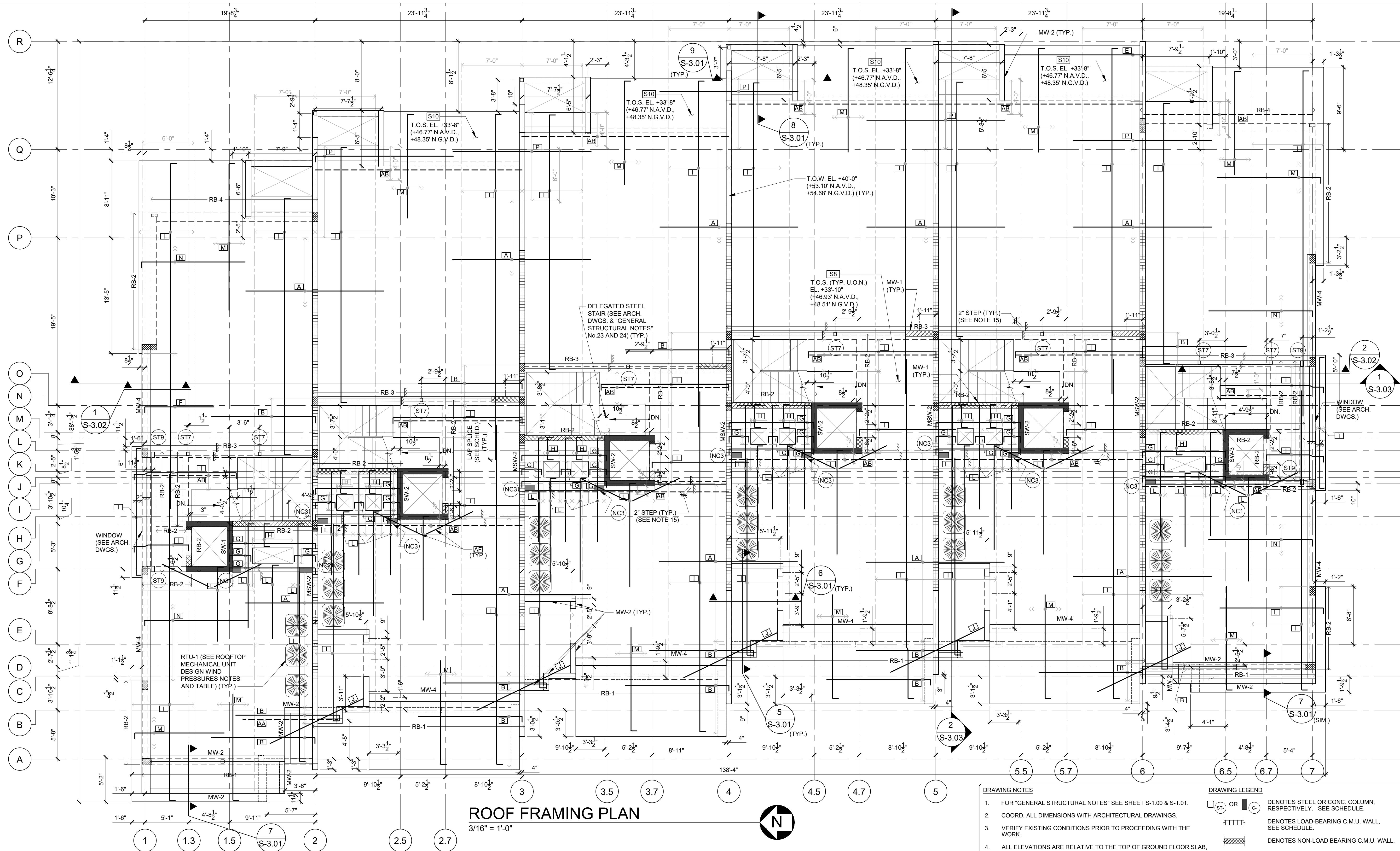
ISSUED FOR

- ☐ REVIEW & COMMENTS
- ☐ BIDDING
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S-2.03

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ROOFTOP MECHANICAL UNIT DESIGN WIND PRESSURES (ASD)		
UNIT	VERT. WIND PRESSURE (P.S.F.)	HORIZ. WIND PRESSURE (P.S.F.)
RTU-1	71 PSF	89 PSF

ROOFTOP MECHANICAL UNIT DESIGN WIND PRESSURES NOTES:
1. DESIGN WIND PRESSURES INDICATED IN TABLE ABOVE ARE: IN ACCORDANCE WITH ASCE 7-10 ROOFTOP TOP STRUCTURES AND EQUIPMENT [RISK CATEGORY II], 170 MPH ULTIMATE WIND SPEED; EXPOSURE D AND IN ACCORDANCE WITH F.B.C. (2017) SECTION 162.0.6 WITH A $GCF = 1.9$ FOR THE HORIZONTAL WIND PRESSURE AND A $GCF = 1.5$ FOR THE VERTICAL (UPLIFT) WIND PRESSURE. ALLOWABLE PRESSURES ARE INDICATED IN TABLE. FOR ULTIMATE PRESSURES MULTIPLY BY 1.6 LOAD FACTOR.

2. ROOFTOP UNIT SUPPORT STANDS & CURBS AND DUCT SUPPORTS SHALL BE DESIGNED BY AN ENGINEER HIRED BY THE MECHANICAL CONTRACTOR USING THE WIND PRESSURES INDICATED IN THE TABLE ABOVE WHICH SHALL SUBMIT SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS OR FLORIDA PRODUCT APPROVAL FOR SUBMISSION TO THE BUILDING DEPARTMENT FOR REVIEW AND APPROVAL.

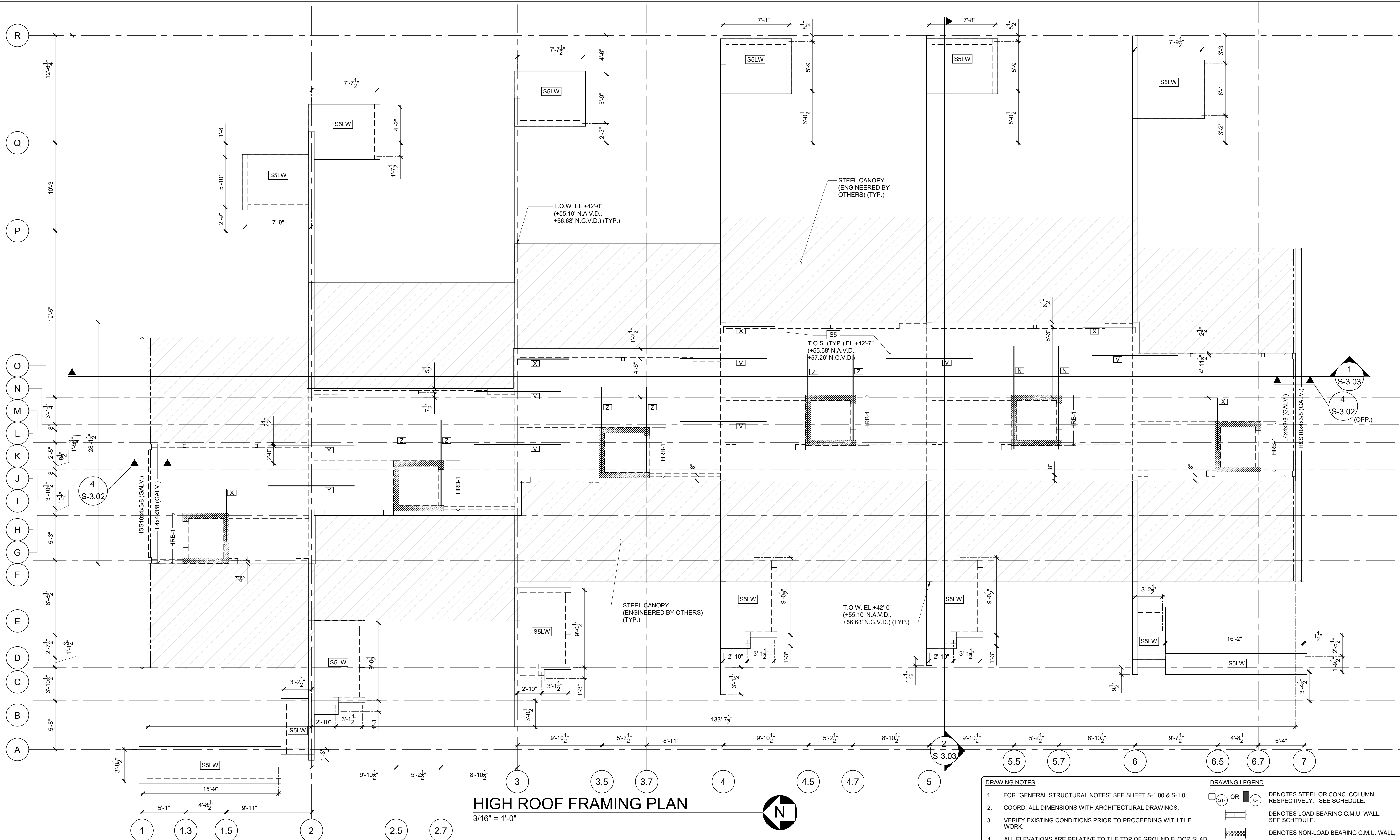
REINFORCING NOTES:		
A	#7 x 16'-0" @ 10" O.C. (T.)	AA (3) #5 x 11'-0" @ 12" O.C. (B.)
B	#7 x 12'-0" @ 8" O.C. (T.)	AB #5 @ 6" O.C. (B.)
C	#6 x 12'-0" @ 8" O.C. (T.)	AC #4 @ 12" O.C. (B.)
D	#5 x 16'-0" @ 10" O.C. (T.)	AD #5 @ 8" O.C. (B.)
E	#5 x 12'-0" @ 8" O.C. (T.)	AE #7 @ 8" O.C. (B.)
F	#4 x 8'-0" @ 8" O.C. (T.)	AF (1) #5 x 6'-0" (MID-DEPTH)
G	#4 @ 12" O.C. (T.)	
H	#5 @ 12" O.C. (T.)	
I	#4 @ 10" O.C. (T.)	
J	(2) #5 x 12'-0" @ 8" O.C. (MID-DEPTH)	
K	#4 x 11'-0" @ 10" O.C. (T.)	
L	#6 x 12'-0" @ 12" O.C. (T.)	
M	#5 x 17'-0" @ 12" O.C. (T.)	
N	#4 x 12'-0" @ 10" O.C. (T.)	
O	#7 @ 8" O.C. (T.)	
P	#6 x 16'-0" @ 4" O.C. (T.)	
Q	#6 @ 12" O.C. (T.)	
R	#6 x 16'-0" @ 12" O.C. (T.)	
S	#5 @ 10" O.C. (T.)	
T	#7 x 8'-0" @ 10" O.C. (T.)	
U	#7 @ 8" O.C. (T.)	
V	(2) #5 x 10'-0" @ 12" O.C. (T.)	
X	(1) #5 x 6'-0" (T.)	
Y	(1) #5 x 10'-0" (T.)	
Z	(3) #5 x 8'-0" @ 12" O.C. (T.)	

DRAWING NOTES

1. FOR "GENERAL STRUCTURAL NOTES" SEE SHEET S-1.00 & S-1.01.
2. COORD. ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
3. VERIFY EXISTING CONDITIONS PRIOR TO PROCEEDING WITH THE WORK.
4. ALL ELEVATIONS ARE RELATIVE TO THE TOP OF GROUND FLOOR SLAB, $EL.=+0'-0"$ (+13.10' N.A.V.D., +14.68' N.G.V.D.). ELEVATIONS ARE INDICATED THUSLY " $\pm X'-Y"$ " ON PLAN.
5. FOR SCHEDULES SEE SHEETS S-1.02 AND S-1.03.
6. FOR SECTIONS AND DETAILS SEE SHEETS S-3.00 THRU S-3.04.
7. SEE SHEET S-1.03 FOR "TYPICAL LAYOUT OF VERT. REINF. IN REINFORCED C.M.U. WALLS" AND C.M.U. WALL SCHEDULE.
8. ADD ADDITIONAL REINF. AT SLAB OPENINGS, SEE "TYPICAL PLAN VIEW AT OPENINGS IN STRUCTURAL CONC. SLAB", SHEET S-1.02.
9. NON LOAD-BEARING MEMBERS SHALL BE ERRECTED AFTER MAIN FRAMING HAS BEEN UNSHORED. SEE PLAN. COORDINATE WITH ARCH. DRAWINGS FOR LOCATIONS.
10. ALL STRUCTURAL STEEL PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED. REFER TO "GENERAL STRUCTURAL NOTES" No.20.F.
11. FOR CONC. PILE CAP DETAILS SEE SHEET S-1.06.
12. FOR SHEARWALL REINFORCEMENT SEE SHEET S-1.04.
13. FOR CONC. SLAB REINF., PLACE OUTER BAR LAYERS IN SHORT DIRECTION.
14. GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS.
15. SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05.

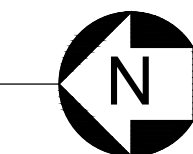
DRAWING LEGEND

  	    	DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE. DENOTES LOAD-BEARING C.M.U. WALL, SEE SCHEDULE. DENOTES NON-LOAD BEARING C.M.U. WALL, DENOTES CONC. SHEAR WALL DENOTES OPENING, COORD. WITH ARCH. DWGS DENOTES CONC. COLUMN OR C.M.U. WALL BELOW.
		DENOTES 5" CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY DENOTES 5" LIGHTWEIGHT CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY.
		DENOTES 8" CONC. SLAB W/ #4 @ 12" O.C. (B.) LONG WAY & #5 @ 12" O.C. (B.) SHORT WAY. SEE PLAN FOR ADD. REINF. DENOTES 10" CONC. SLAB W/ #5 @ 12" O.C. (B.) LONG WAY & #5 @ 12" O.C. (B.) SHORT WAY.
		DENOTES 15" CONC. SLAB W/ #7 @ 12" O.C. (B.) SHORT WAY & #5 @ 10" O.C. (B.) LONG WAY.



HIGH ROOF FRAMING PLAN

3/16" = 1'-0"



REINFORCING NOTES:

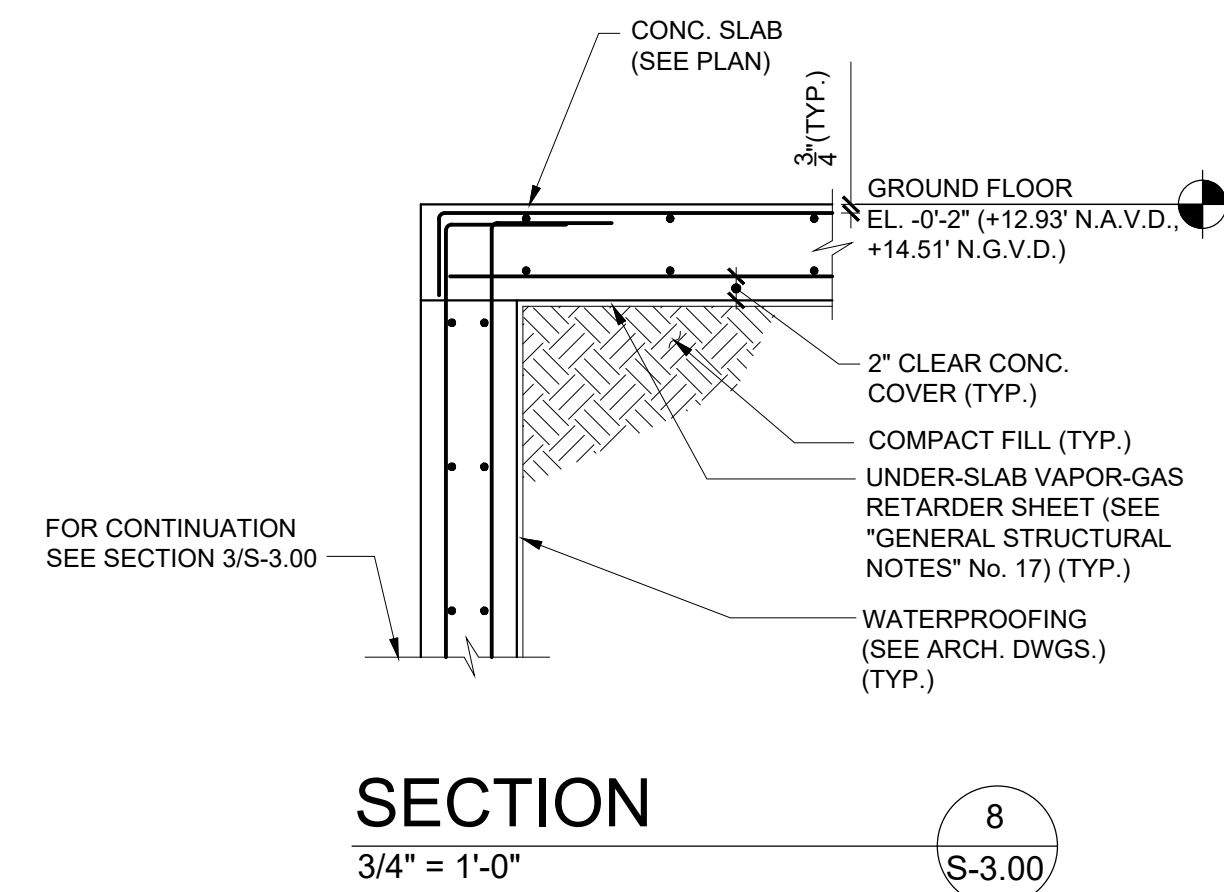
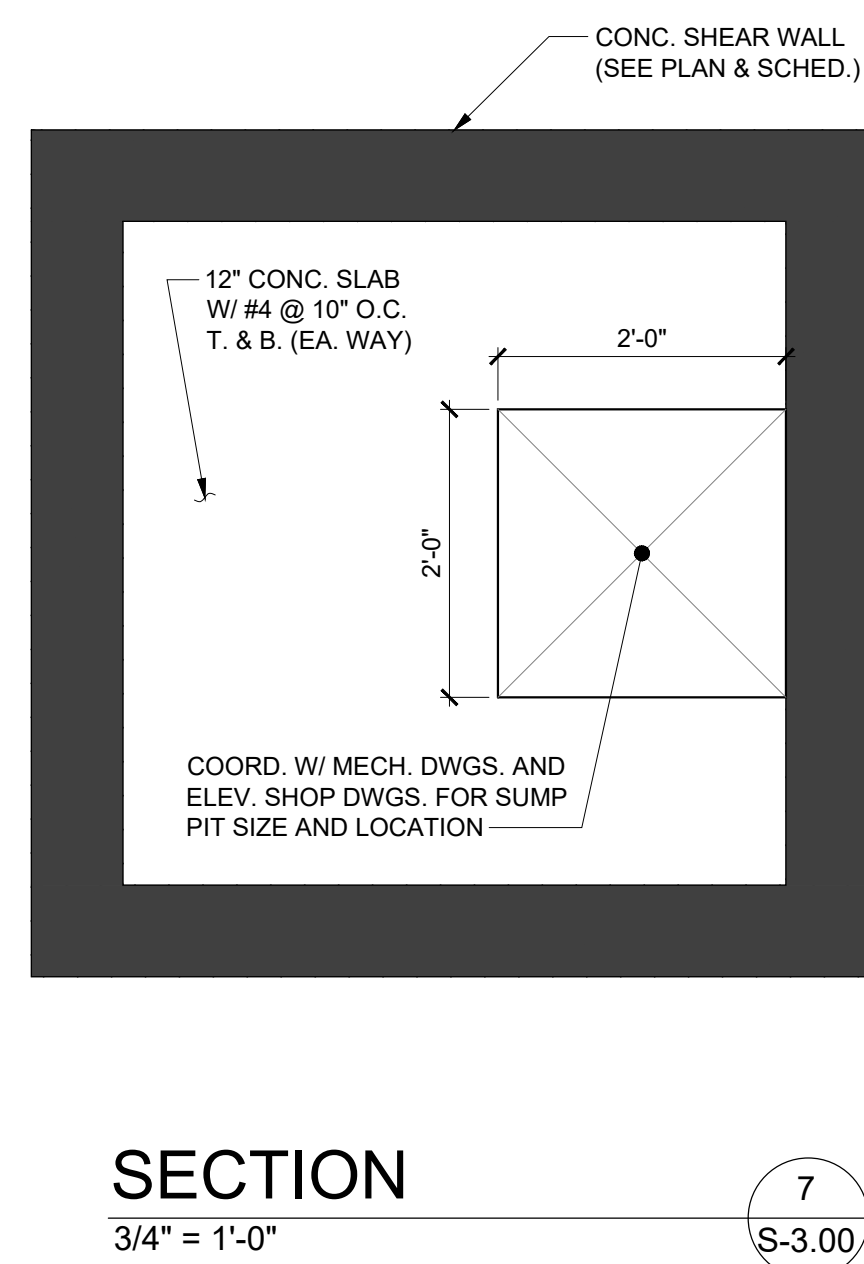
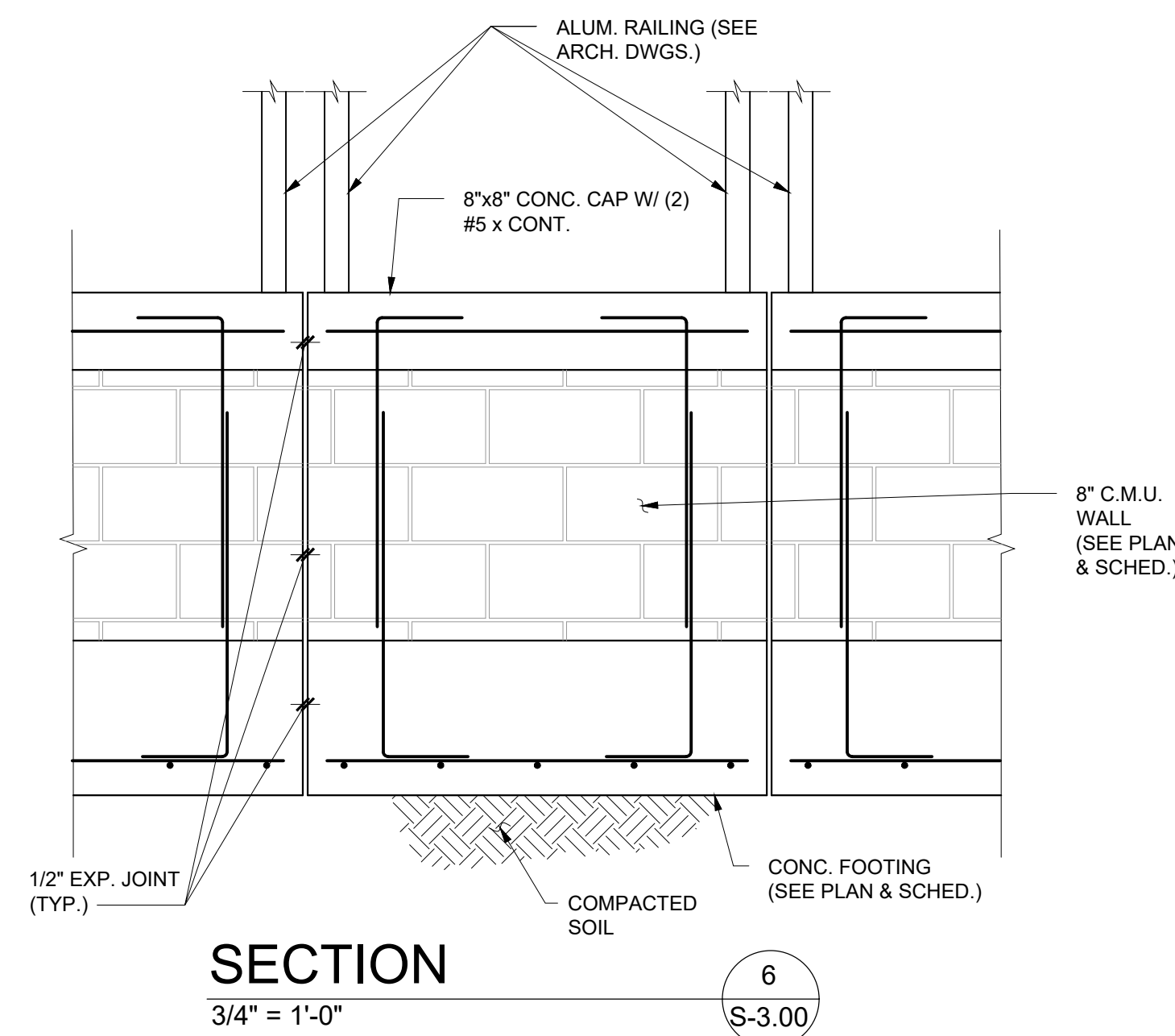
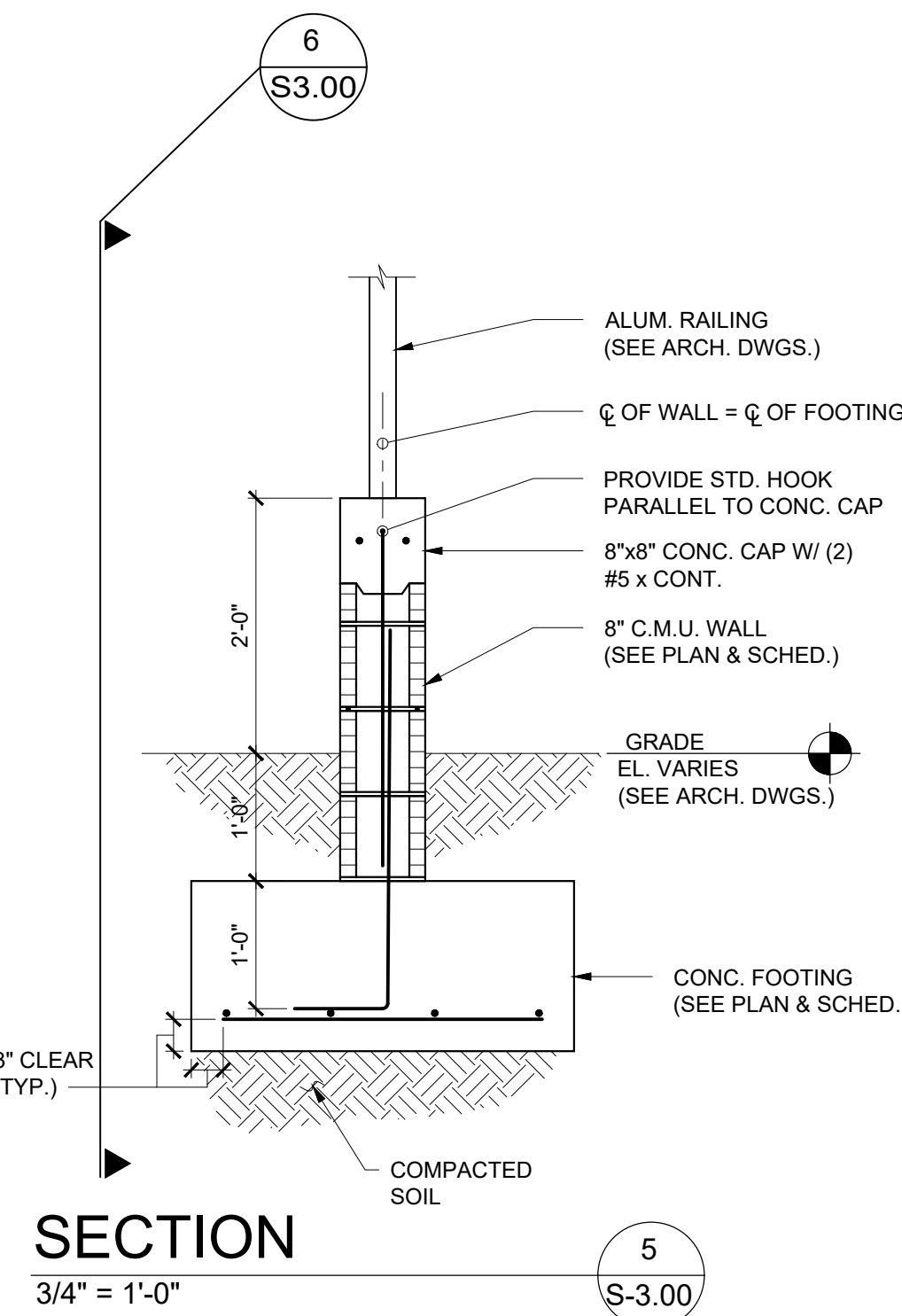
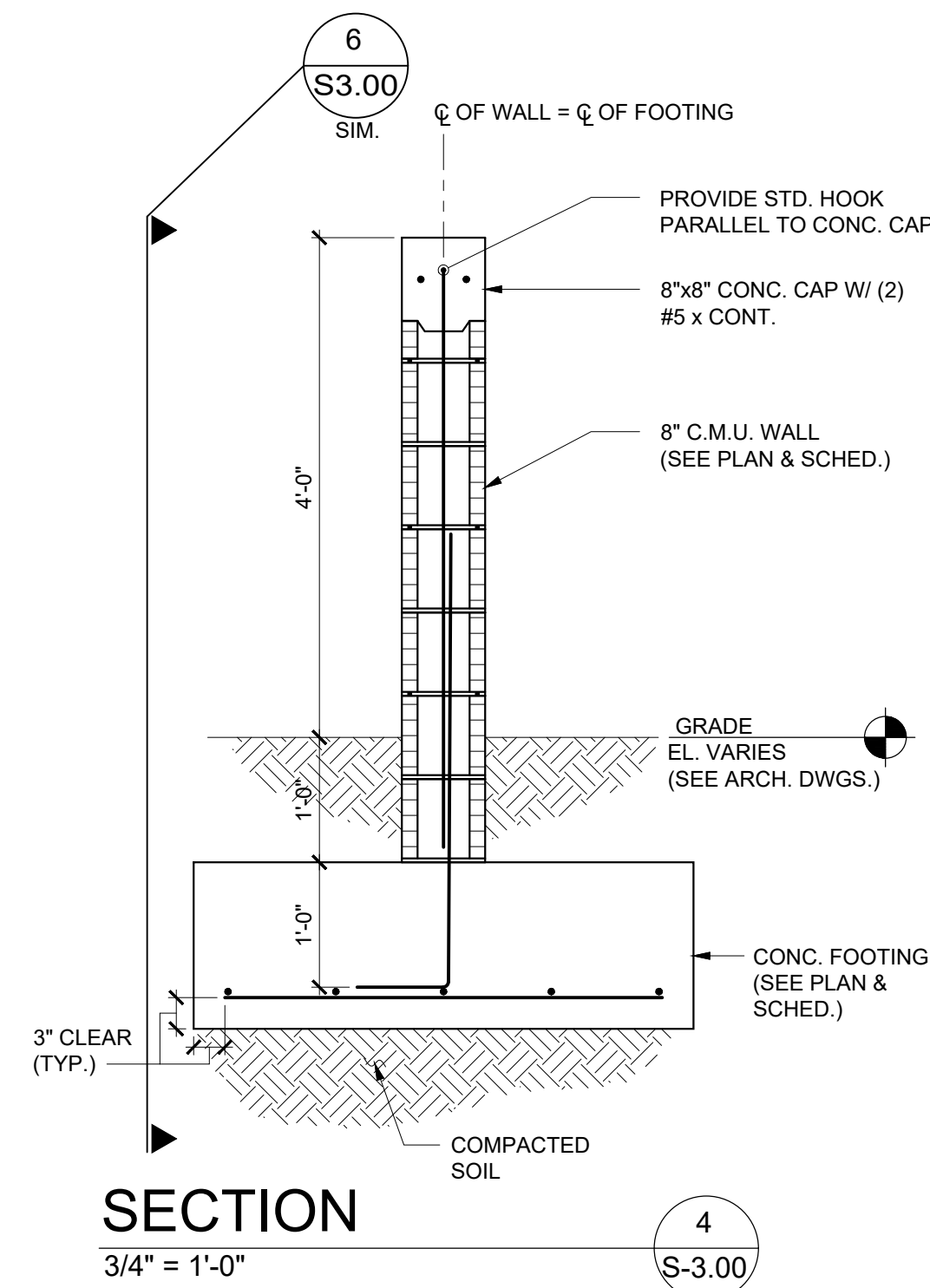
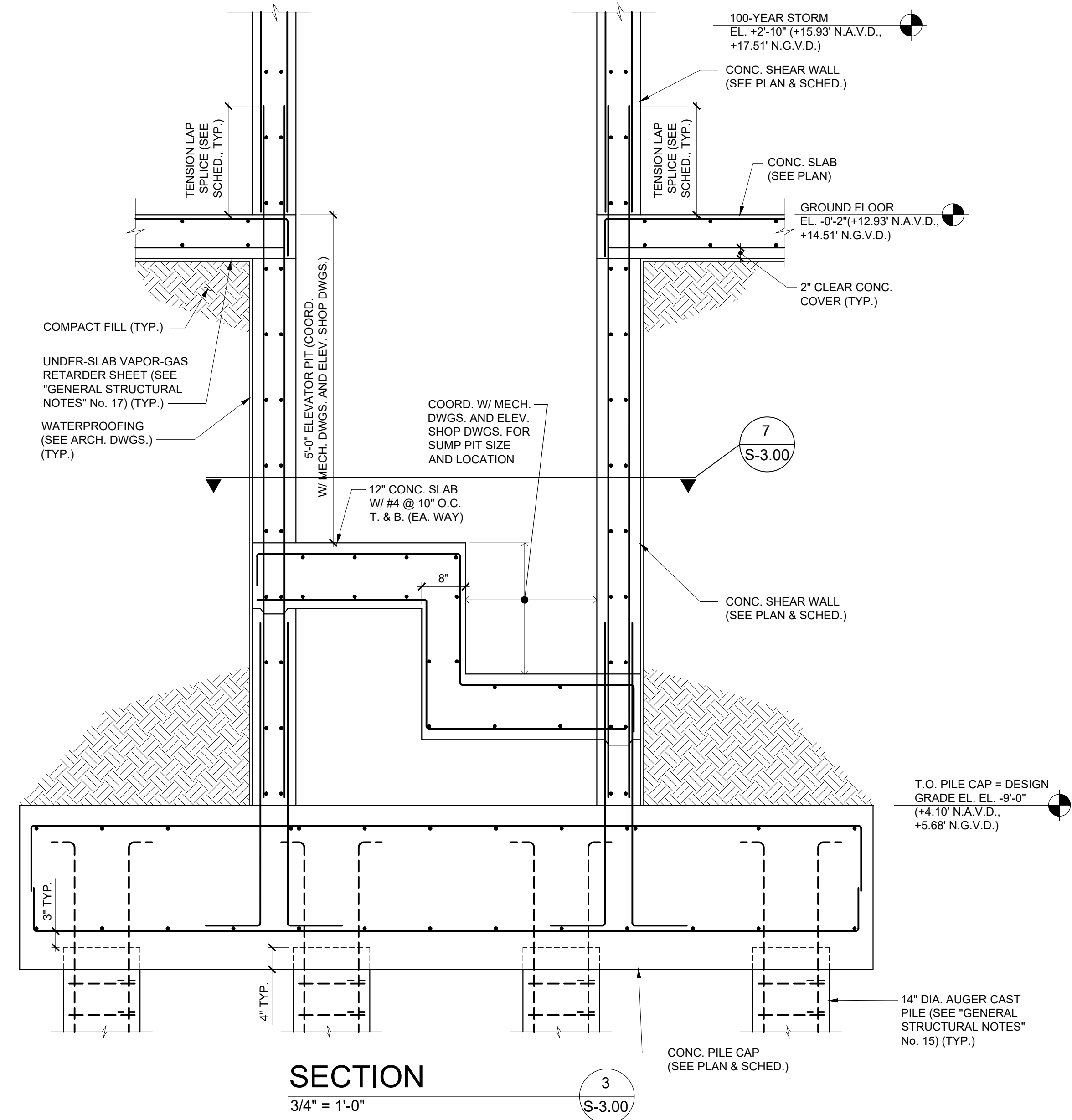
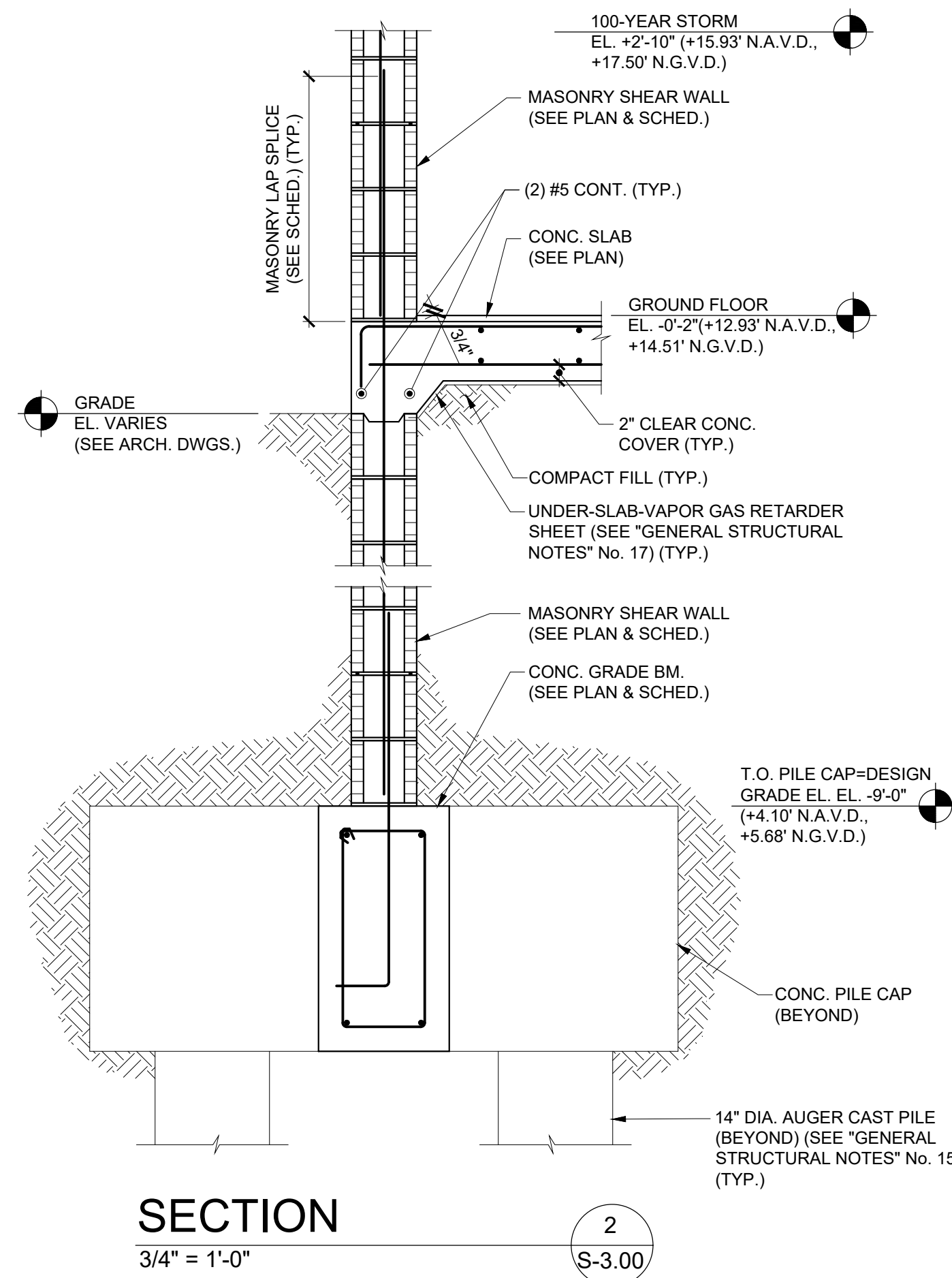
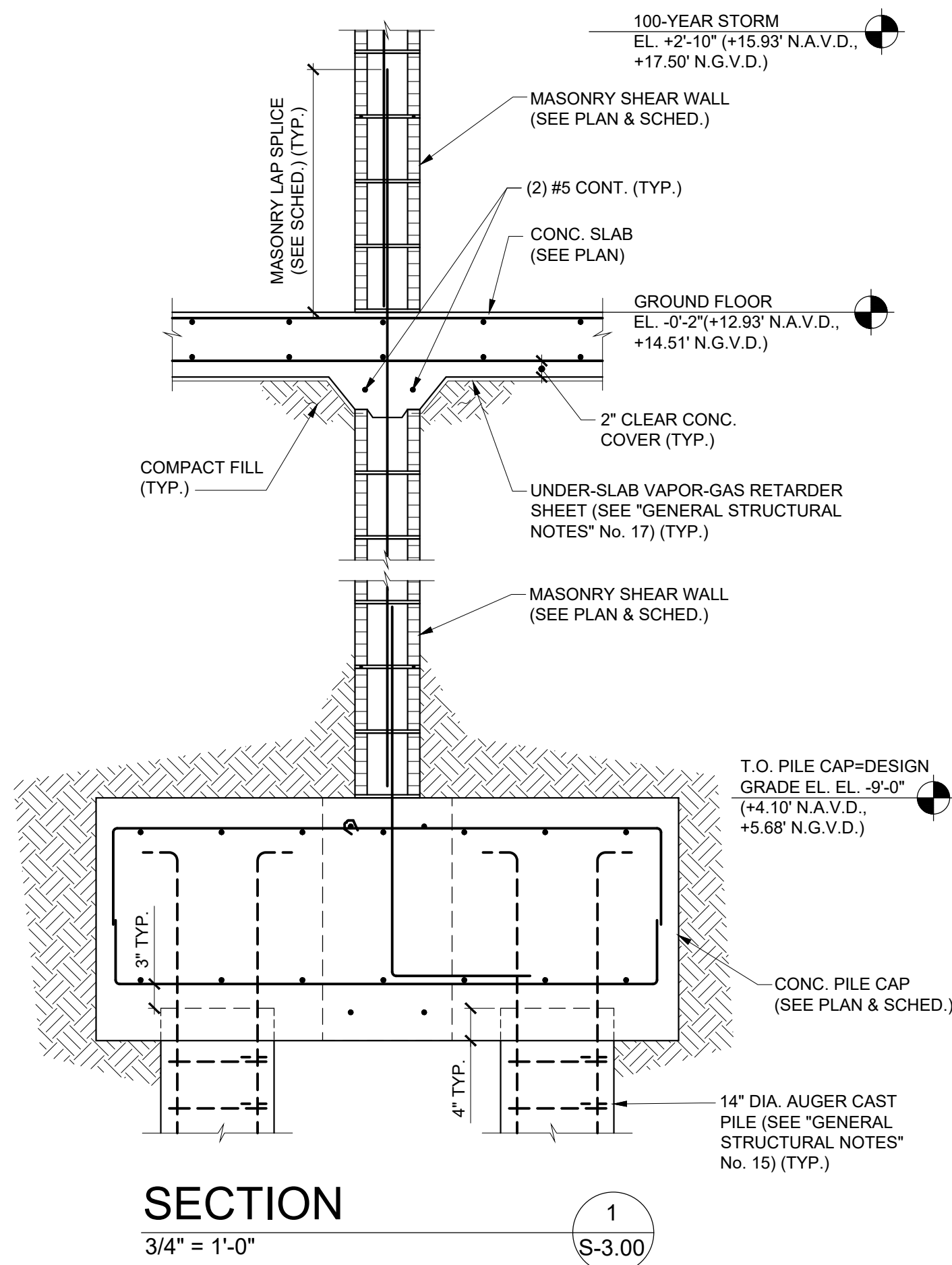
A	#7 x 16'-0" @ 10" O.C. (T.)	AA	(3) #5 x 11'-0" @ 12" O.C. (B.)
B	#7 x 12'-0" @ 8" O.C. (T.)	AB	#5 @ 6" O.C. (B.)
C	#6 x 12'-0" @ 8" O.C. (T.)	AC	#4 @ 10" O.C. (B.)
D	#5 x 16'-0" @ 10" O.C. (T.)	AD	#5 @ 8" O.C. (B.)
E	#8 x 12'-0" @ 4" O.C. (T.)	AE	#7 @ 8" O.C. (B.)
F	#4 x 8'-0" @ 8" O.C. (T.)	AF	(1) #5 x 6'-0" (MID-DEPTH)
G	#4 @ 12" O.C. (T.)		
H	#5 @ 12" O.C. (T.)		
I	#4 @ 10" O.C. (T.)		
J	(2) #5 x 12'-0" @ 8" O.C. (MID-DEPTH)		
K	#4 x 11'-0" @ 10" O.C. (T.)		
L	#6 x 12'-0" @ 12" O.C. (T.)		
M	#5 x 17'-0" @ 12" O.C. (T.)		
N	#4 x 12'-0" @ 10" O.C. (T.)		
O	#7 @ 8" O.C. (T.)		
P	#5 x 16'-0" @ 4" O.C. (T.)		
Q	#6 @ 12" O.C. (T.)		
R	#6 x 16'-0" @ 12" O.C. (T.)		
S	#5 @ 10" O.C. (T.)		
T	#7 x 8'-0" @ 10" O.C. (T.)		
U	#7 @ 6" O.C. (T.)		
V	(2) #5 x 10'-0" @ 12" O.C. (T.)		
X	(1) #5 x 6'-0" (T.)		
Y	(1) #5 x 10'-0" (T.)		
Z	(3) #5 x 8'-0" @ 12" O.C. (T.)		

DRAWING NOTES

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- GALVANIZE ALL STEEL REINFORCEMENT FOR EXTERIOR CONC. SLABS.
- SEE "TYPICAL STEP IN SLAB DETAIL" 3/S-1.05.

DRAWING LEGEND

- | | |
|---|---|
| ☐ ST OR ☐ C | DENOTES STEEL OR CONC. COLUMN, RESPECTIVELY. SEE SCHEDULE. |
| | DENOTES LOAD-BEARING C.M.U. WALL, SEE SCHEDULE. |
| | DENOTES NON-LOAD BEARING C.M.U. WALL. |
| | DENOTES CONC. SHEAR WALL |
| | DENOTES OPENING, COORD. WITH ARCH. DWGS. |
| | DENOTES CONC. COLUMN OR C.M.U. WALL BELOW. |
| SSL | DENOTES 5" CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY. |
| SSLW | DENOTES 5" LIGHTWEIGHT CONC. SLAB W/ #4 @ 12" O.C. (T. & B.) EA. WAY. |
| S8 | DENOTES 8" CONC. SLAB W/ #4 @ 12" O.C. (B.) LONG WAY & #5 @ 12" O.C. (B.) SHORT WAY. SEE PLAN FOR ADD. REINF. |
| S10 | DENOTES 10" CONC. SLAB W/ #5 @ 12" O.C. (B.) LONG WAY AND #6 @ 12" O.C. (B.) SHORT WAY. |
| S15 | DENOTES 15" CONC. SLAB W/ #7 @ 12" O.C. (B.) SHORT WAY AND #5 @ 10" O.C. (B.) LONG WAY. |



NOTE: FRANGIBLE MASONRY WALL AND CONC. FOOTINGS SHALL BE INSTALLED IN MAX. 4 FT LONG SECTIONS.

NOTE: FRANGIBLE MASONRY WALL, CONC. FOOTINGS AND ALUMINUM FENCE SHALL BE INSTALLED IN MAX. 4 FT LONG SECTIONS.

NOTE: FRANGIBLE MASONRY WALL, CONC. FOOTINGS AND ALUMINUM FENCE SHALL BE INSTALLED IN MAX. 4 FT LONG SECTIONS.

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GREENPARK GROUP

PROJECT NUMBER: 19023

SECTIONS AND DETAILS

DESIGNED BY:

DATE:

ISSUED FOR

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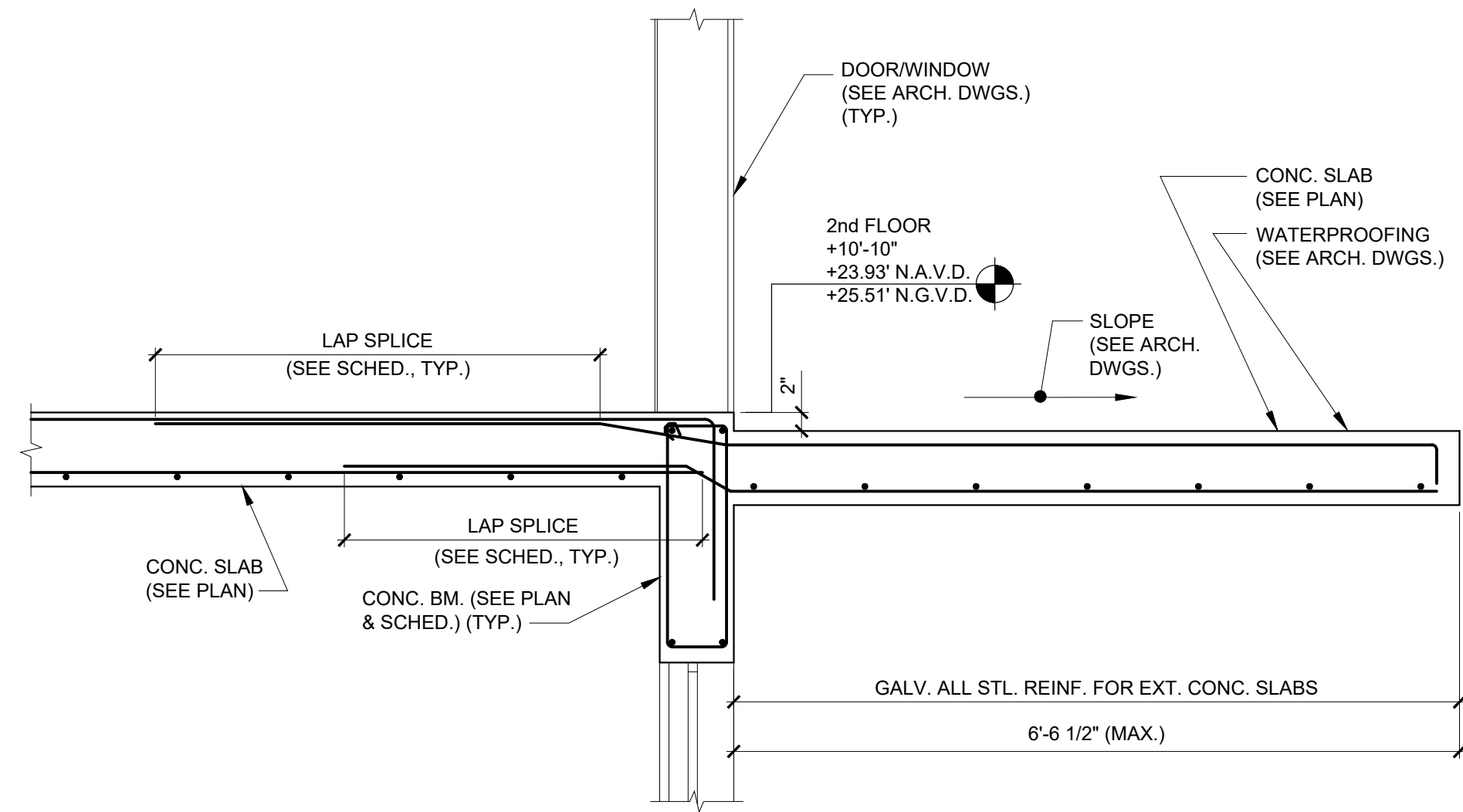
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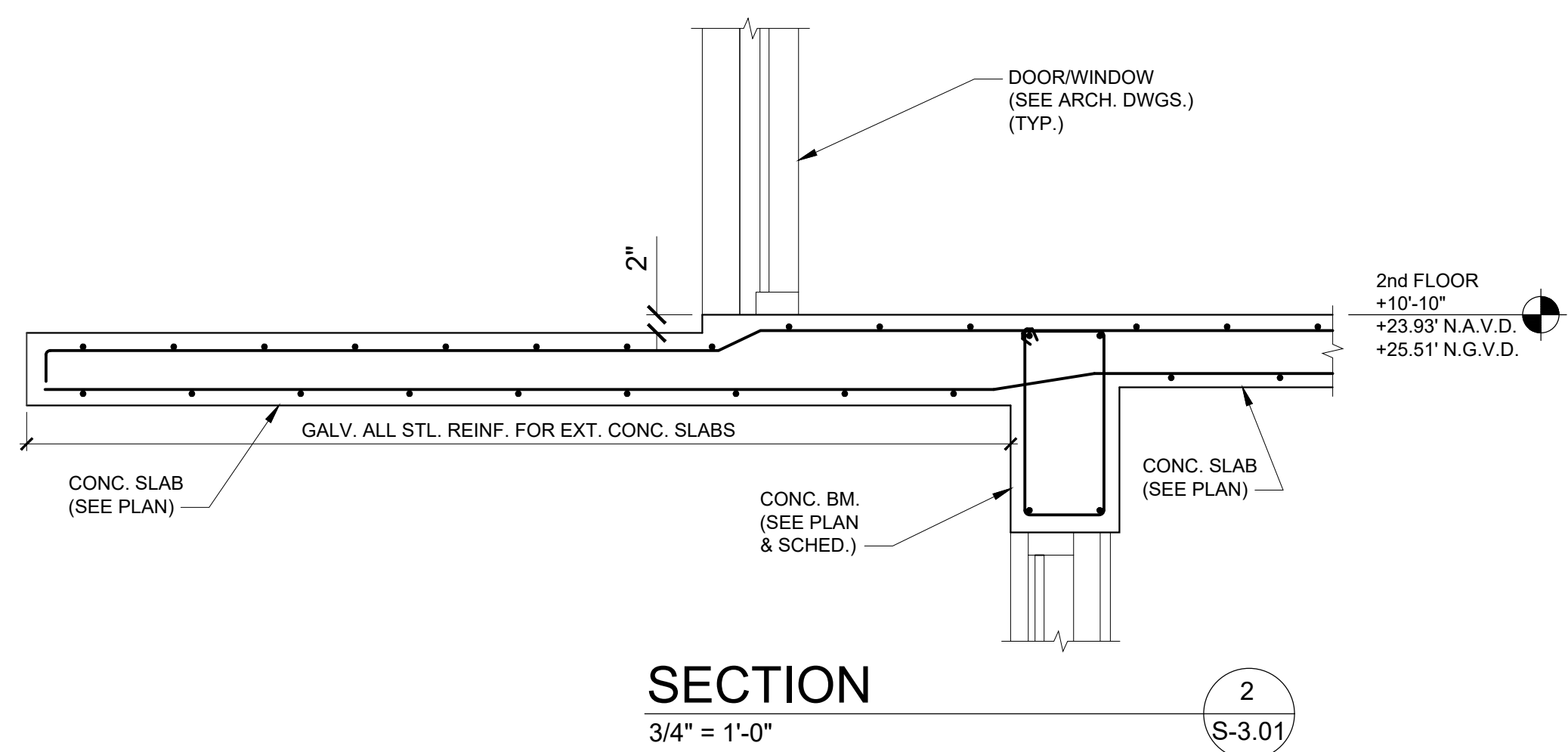
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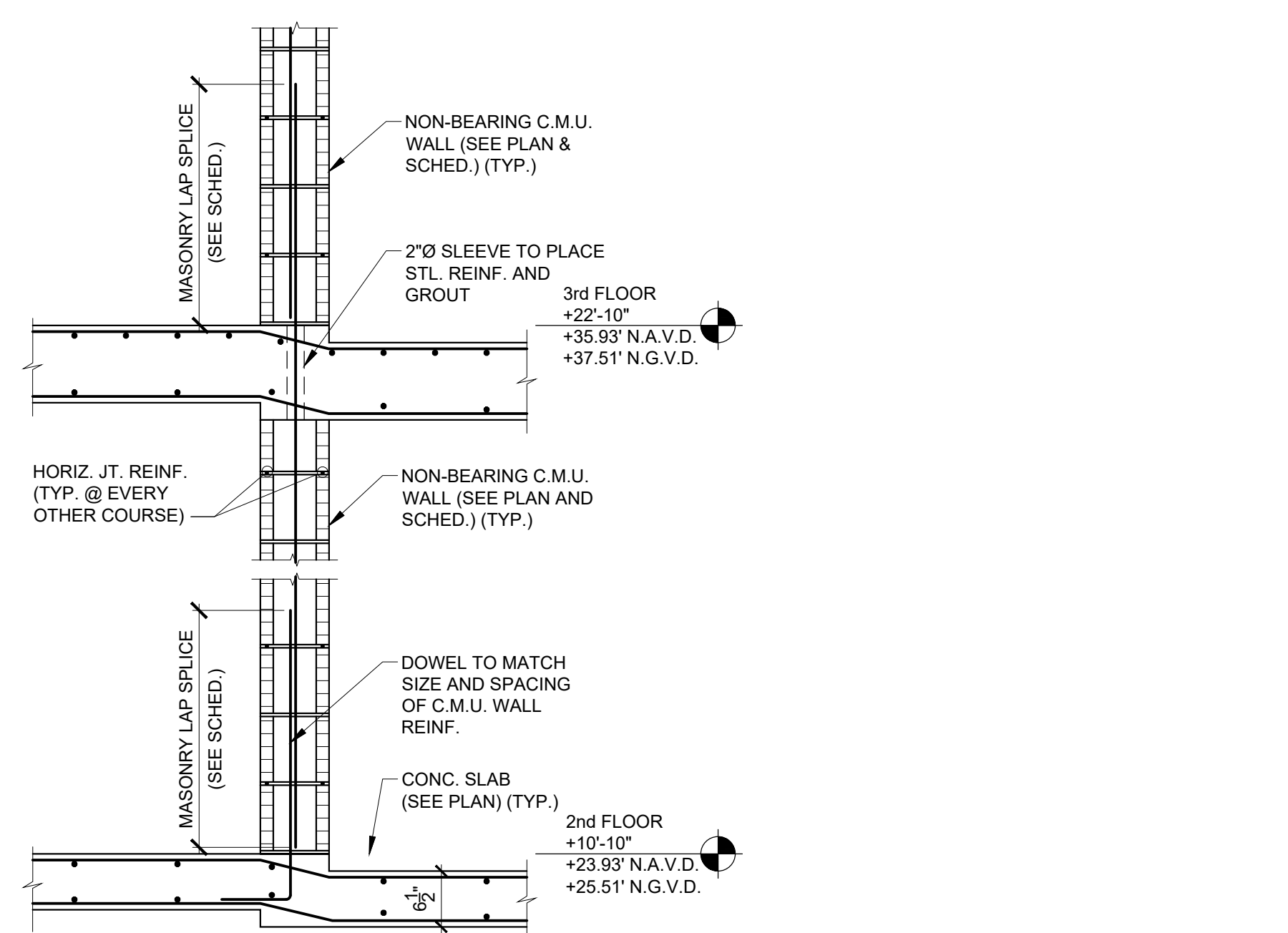
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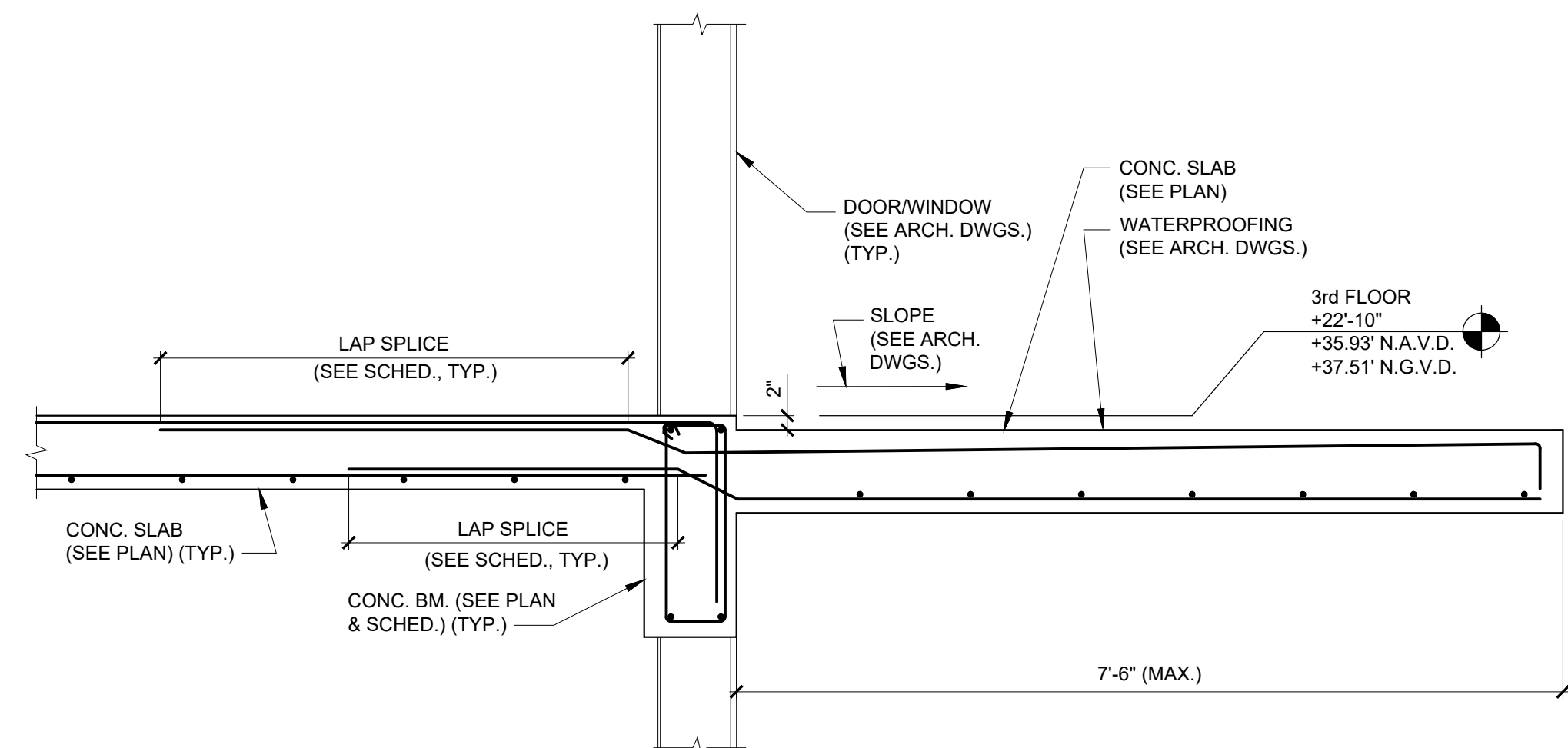
SECTION
3/4" = 1'-0"



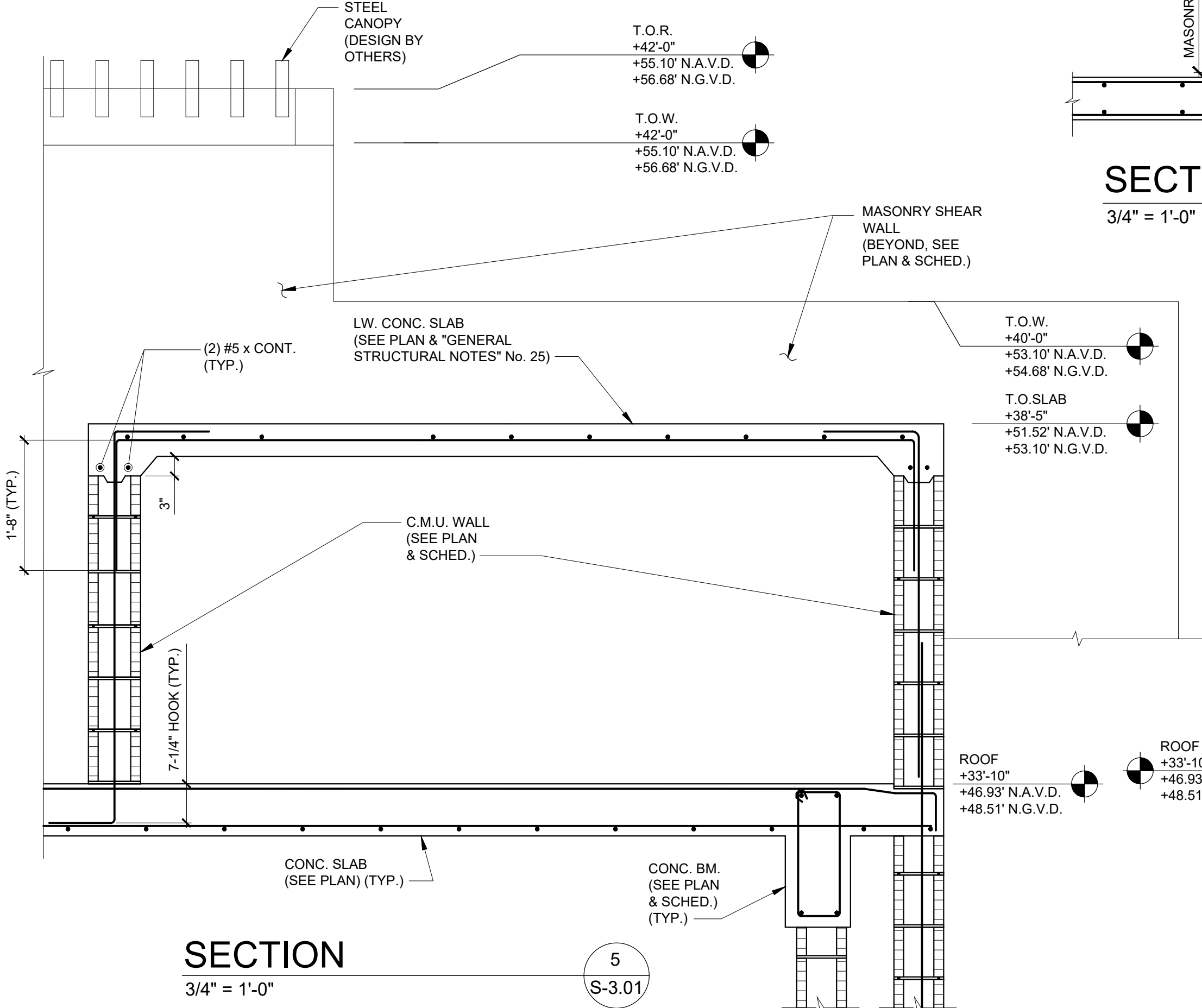
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3/4" = 1'-0"



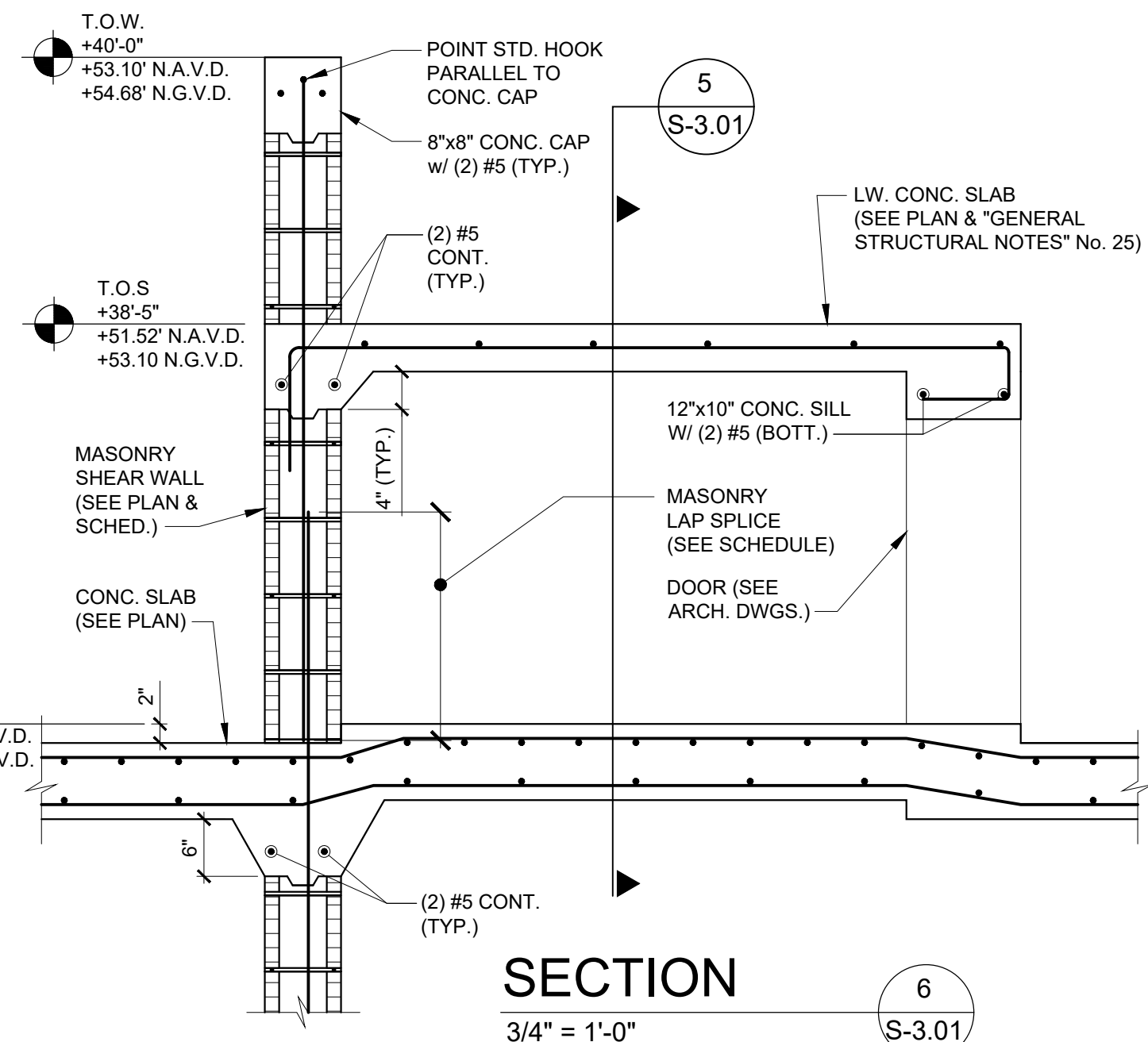
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3/4" = 1'-0"



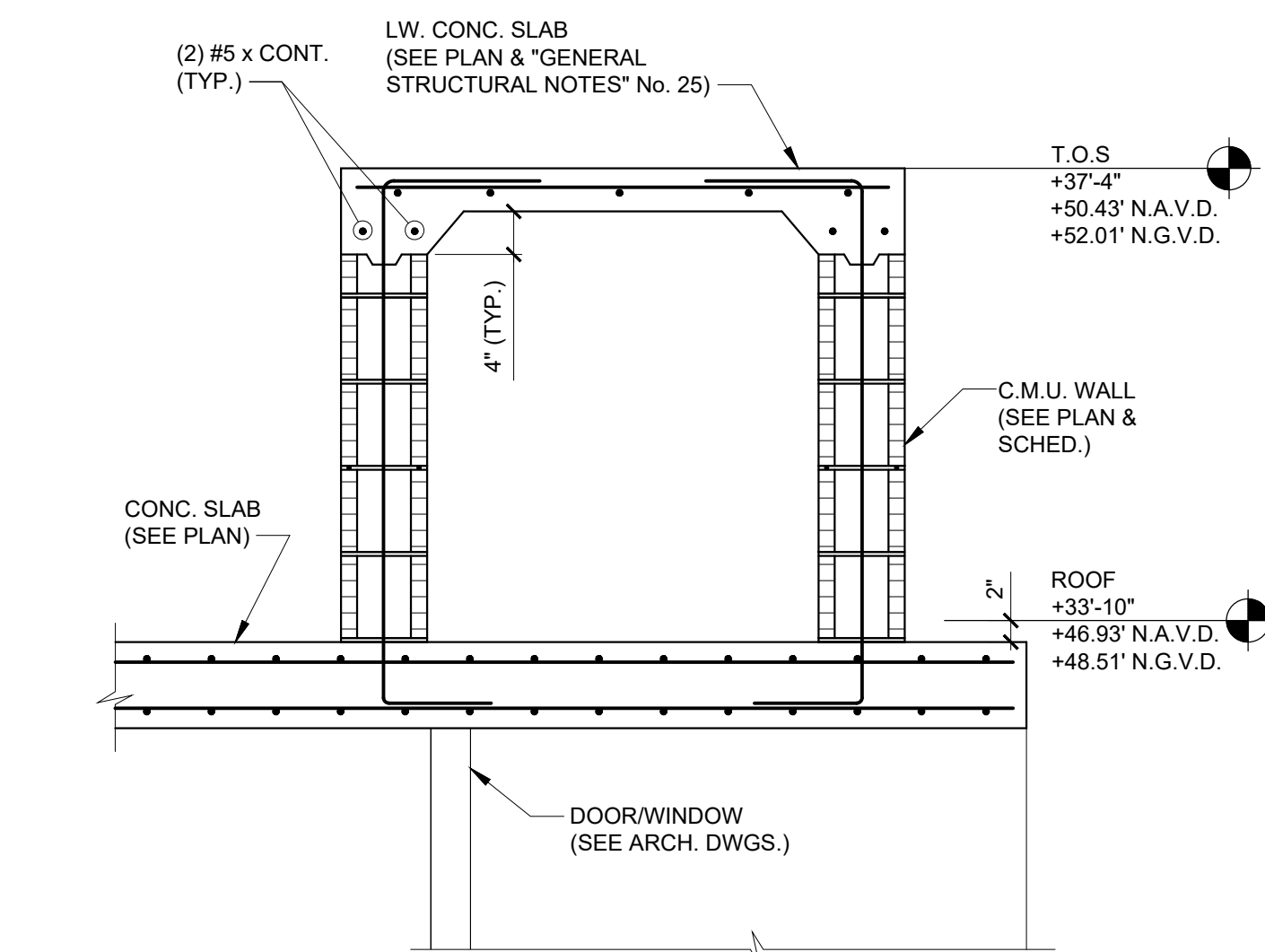
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3/4" = 1'-0"



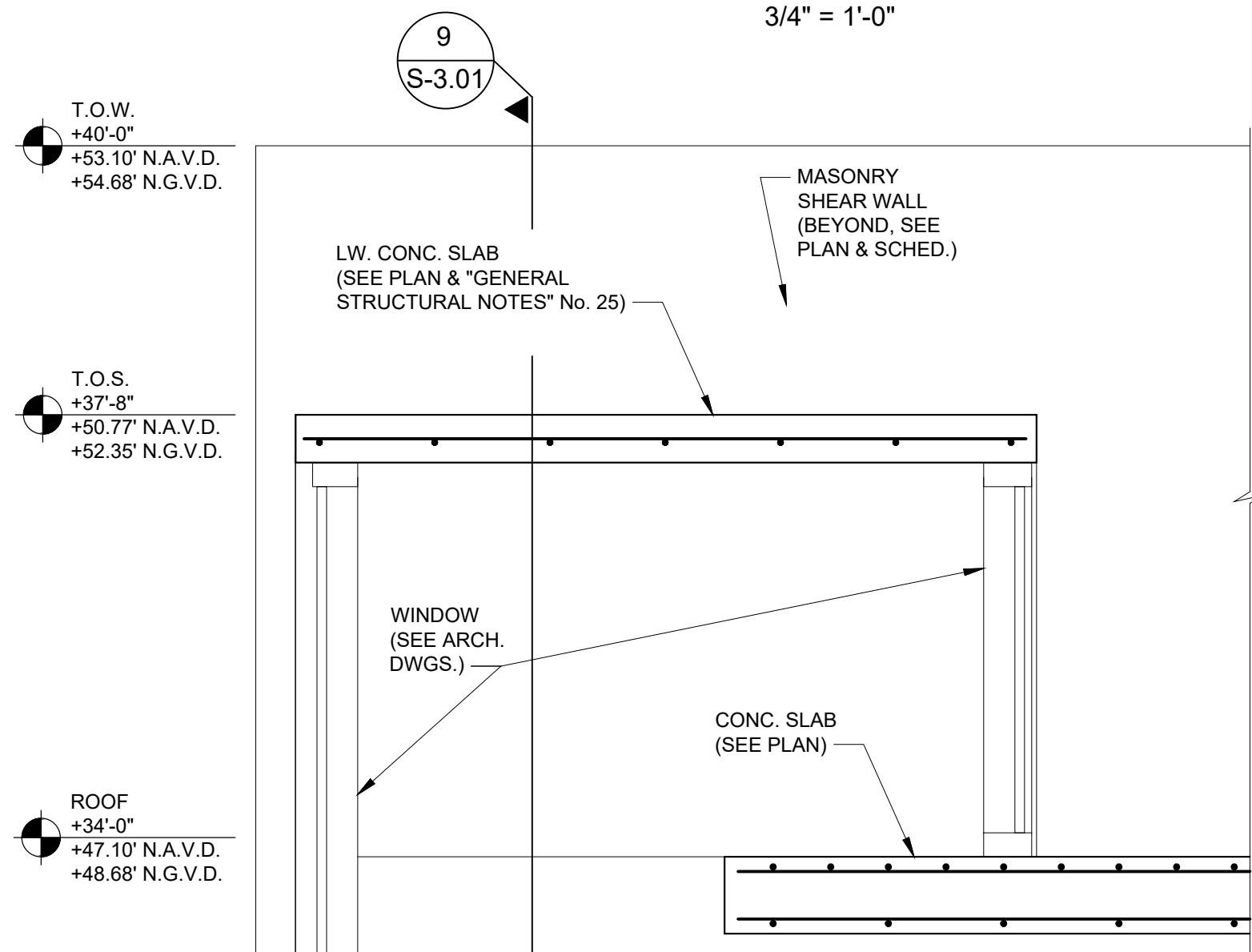
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3/4" = 1'-0"



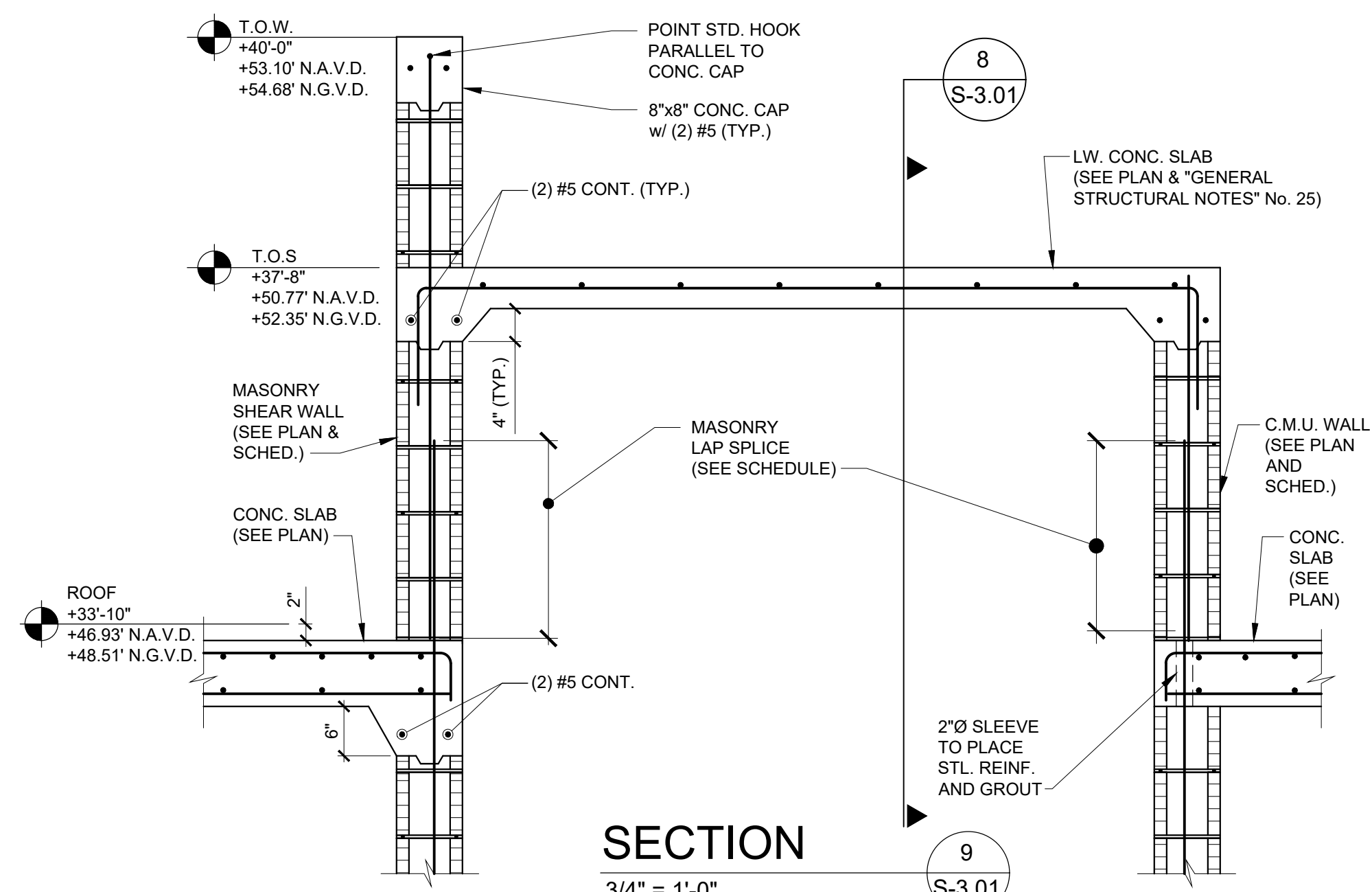
SECTION
3/4" = 1'-0"



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3/4" = 1'-0"

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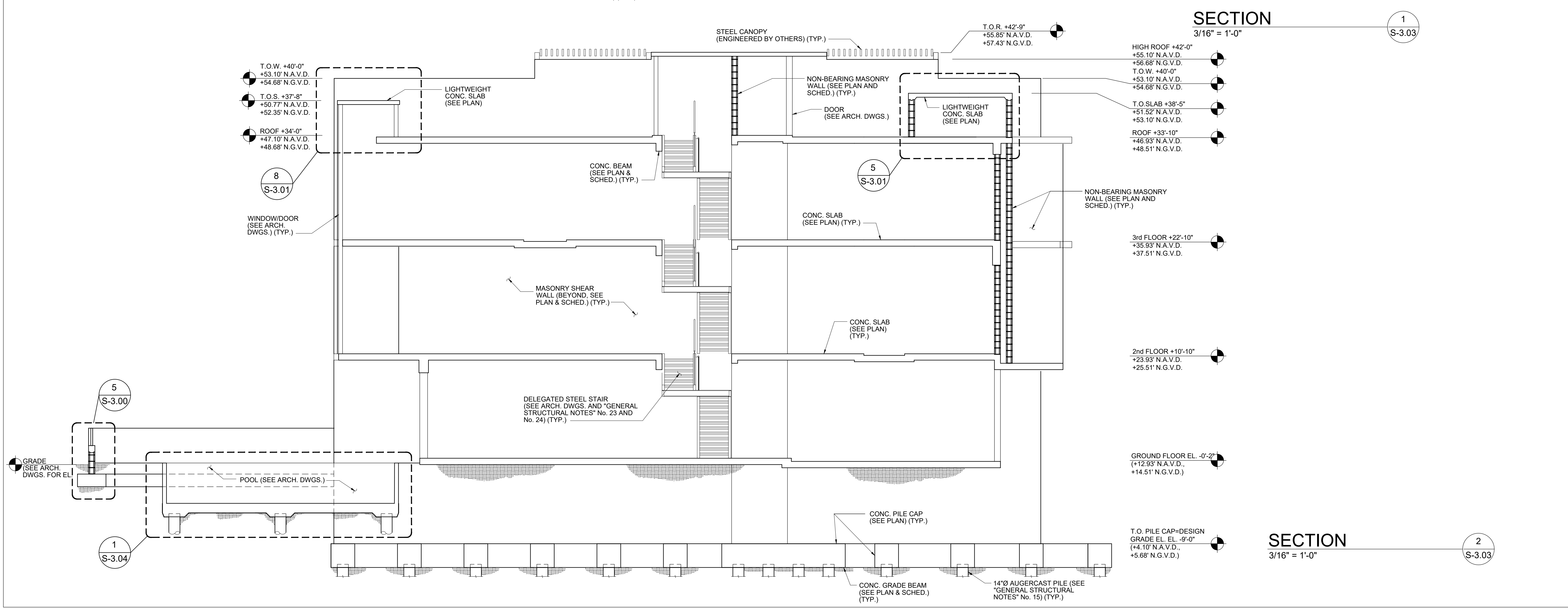
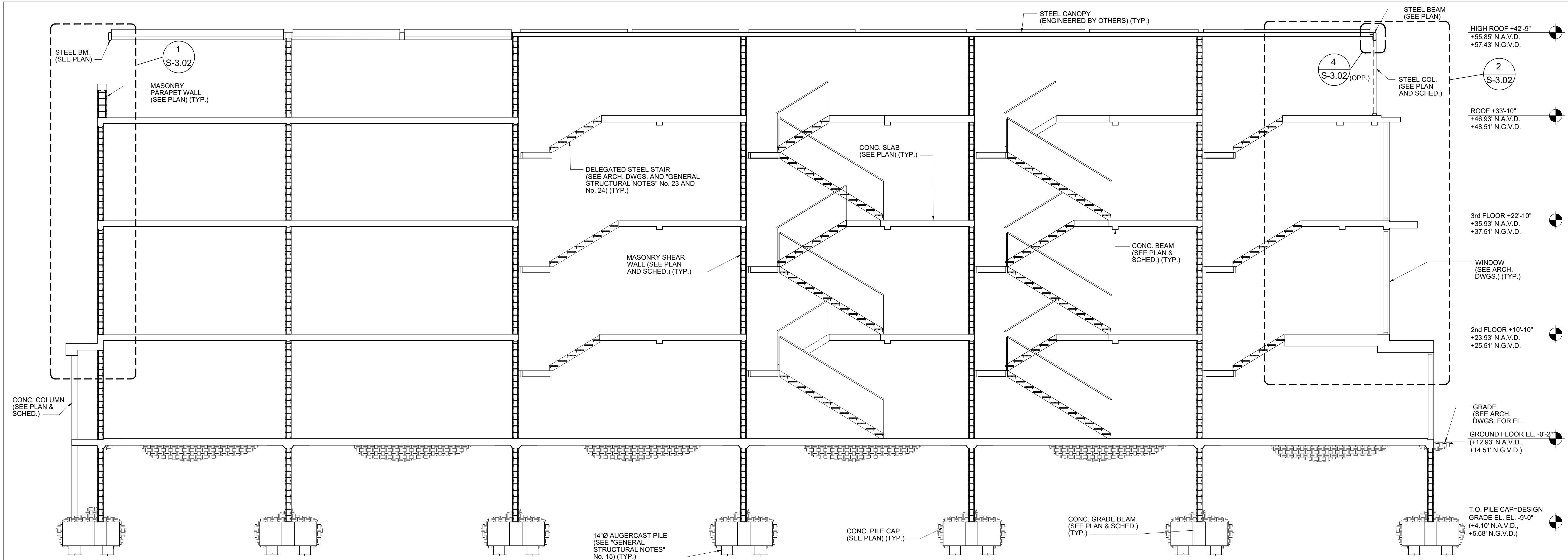
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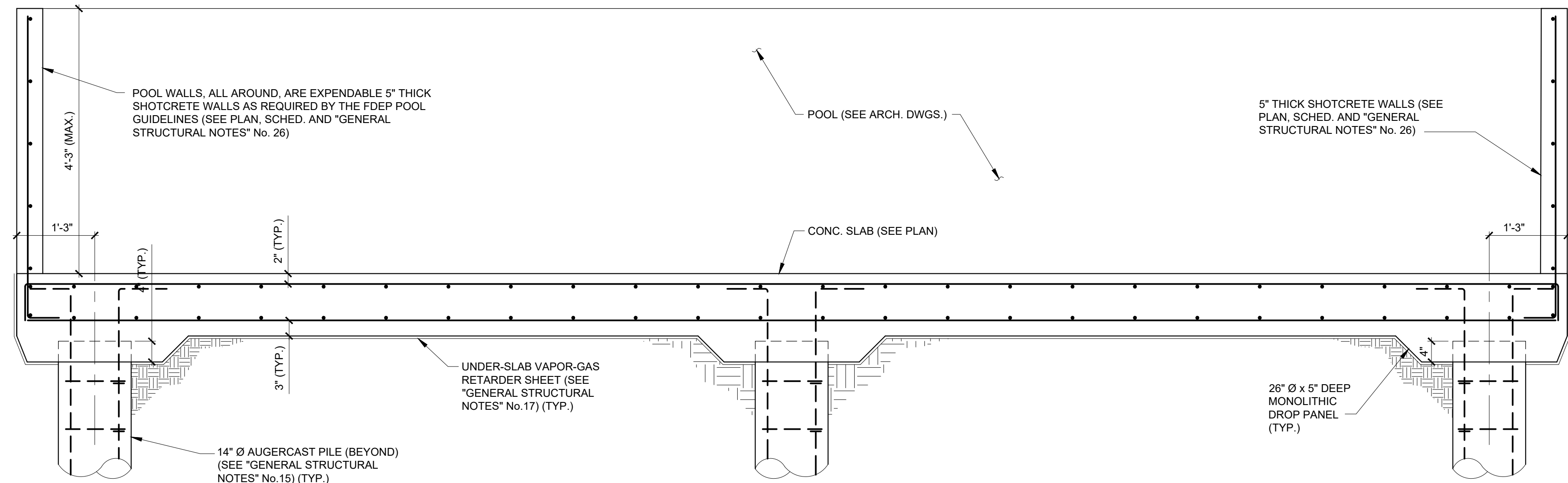
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FINISH GRADE
EL. -0'-4" (+12.60' N.A.V.D.,
+14.18' N.G.V.D.)

SECTION
3/4" = 1'-0"

1
S-3.04

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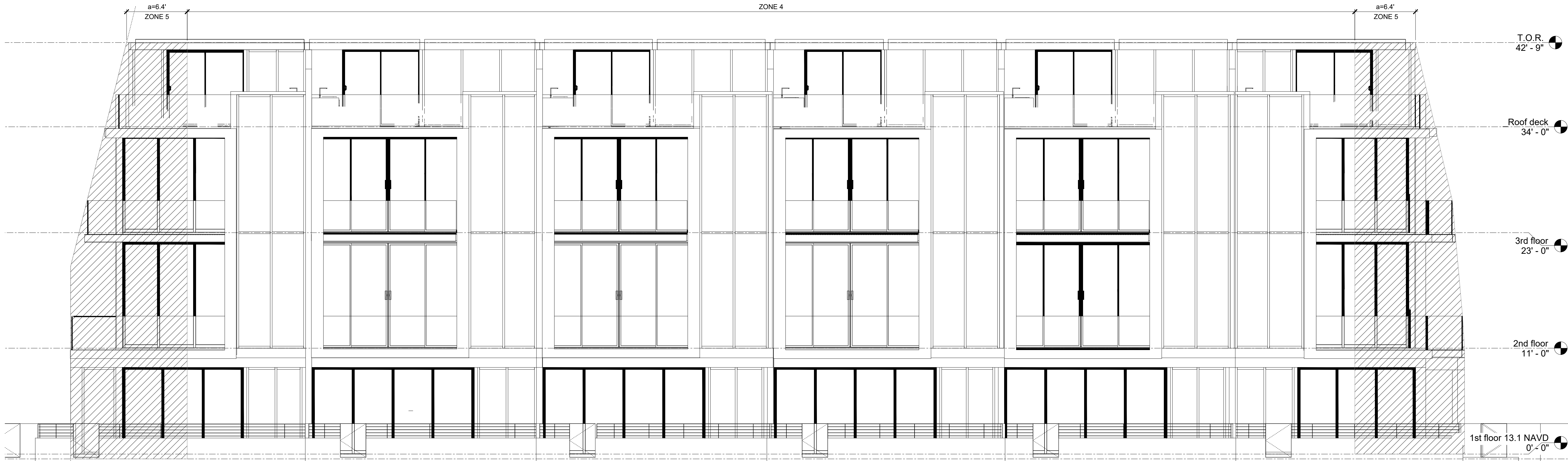
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DESIGN WIND PRESSURES FOR ROOFING SYSTEM					
DESIGN WIND LOAD		NEGATIVE (PSF)		POSITIVE (PSF)	
		P _{ULT}	P _{ASD}	P _{ULT}	P _{ASD}
	ZONE 1	-91.7	-55.0	37.3	22.4
	ZONE 2	-153.9	-92.3	83.9	50.3
	ZONE 3	-153.9	-92.3	83.9	50.3

ROOFING NOTES:
DESIGN WIND PRESSURES (PERPENDICULAR TO SURFACE) FOR ROOFING ARE INDICATED ON ROOF PLAN (NEGATIVE VALUES = WIND PRESSURE AWAY FROM SURFACE) IN ACCORDANCE WITH ASCE 7-10 COMPONENTS & CLADDING [RISK CATEGORY II, 170 MPH ULTIMATE WIND SPEED; DIRECTIONALITY FACTOR $K_d = 0.85$; EXPOSURE D; INTERNAL PRESSURE COEFFICIENTS = ± 0.18]. BOTH ULTIMATE PRESSURES AND ALLOWABLE PRESSURES ($0.6 \times \text{PULT}$) ARE INDICATED ON ROOF PLAN.

 $a = 6.4 \text{ ft}$



EAST ELEVATION

3/16" = 1'-0"

DESIGN WIND PRESSURES FOR DOORS, WINDOWS & CLADDING																					
AREA (SQFT)		ZONE 4										ZONE 5									
		10 - 20	20 - 30	30 - 40	40 - 50	50 - 60	60 - 70	70 - 80	80 - 90	90 - 100	>100	10 - 20	20 - 30	30 - 40	40 - 50	50 - 60	60 - 70	70 - 80	80 - 90	90 - 100	>100
DESIGN WIND PRESSURES																					
ULTIMATE (PSF)	NEGATIVE	-90.9	-87.2	-85.1	-83.5	-82.3	-81.3	-80.5	-79.8	-79.2	-78.6	-111.9	-104.5	-100.1	-97.1	-94.7	-92.7	-91.1	-89.6	-88.4	-87.2
	POSITIVE	83.9	80.2	78.1	76.5	75.3	74.3	73.5	72.8	72.2	71.6	83.9	80.2	78.1	76.5	75.3	74.3	73.5	72.8	72.2	71.6
ALLOWABLE (PSF)	NEGATIVE	-54.5	-52.3	-51.1	-50.1	-49.4	-48.8	-48.3	-47.9	-47.5	-47.2	-67.1	-62.7	-60.1	-58.3	-56.8	-55.6	-54.7	-53.8	-53.0	-52.3
	POSITIVE	50.3	48.1	46.9	45.9	45.2	44.6	44.1	43.7	43.3	43.0	50.3	48.1	46.9	45.9	45.2	44.6	44.1	43.7	43.3	43.0

EXTERIOR WINDOWS, DOOR AND CLADDING NOTES:
DESIGN WIND PRESSURES (PERPENDICULAR TO SURFACE) FOR ALL EXTERIOR WINDOWS, DOORS AND CLADDING ARE INDICATED ON ELEVATIONS (NEGATIVE VALUES = WIND PRESSURE AWAY FROM SURFACE & POSITIVE VALUES = WIND PRESSURE TOWARDS SURFACE) IN ACCORDANCE WITH ASCE 7-10 COMPONENTS & CLADDING [RISK CATEGORY II, 170 mph ULTIMATE WIND SPEED, DIRECTIONALITY FACTOR Kd = 0.85; EXPOSURE D; INTERNAL PRESSURE COEFFICIENTS = ± 0.18]. BOTH ULTIMATE PRESSURES AND ALLOWABLE PRESSURES (0.6 x Psf) ARE INDICATED ON ELEVATIONS AND SCHEDULE ABOVE.

a = 6.4 ft

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PROJECT NUMBER: 19023

DESIGN WIND
PRESSURES FOR
DOORS, WINDOWS
& CLADDING

DESIGNED BY:

DATE:

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WEST ELEVATION

3/16" = 1'-0"

DESIGN WIND PRESSURES		ZONE 4														ZONE 5									
		10 - 20	20 - 30	30 - 40	40 - 50	50 - 60	60 - 70	70 - 80	80 - 90	90 - 100	>100	10 - 20	20 - 30	30 - 40	40 - 50	50 - 60	60 - 70	70 - 80	80 - 90	90 - 100	>100				
ULTIMATE (PSF)	NEGATIVE	-90.9	-87.2	-85.1	-83.5	-82.3	-81.3	-80.5	-79.8	-79.2	-78.6	-111.9	-104.5	-100.1	-97.1	-94.7	-92.7	-91.1	-89.6	-88.4	-87.2				
	POSITIVE	83.9	80.2	78.1	76.5	75.3	74.3	73.5	72.8	72.2	71.6	83.9	80.2	78.1	76.5	75.3	74.3	73.5	72.8	72.2	71.6				
ALLOWABLE (PSF)	NEGATIVE	-54.5	-52.3	-51.1	-50.1	-49.4	-48.8	-48.3	-47.9	-47.5	-47.2	-67.1	-62.7	-60.1	-58.3	-56.8	-55.6	-54.7	-53.8	-53.0	-52.3				
	POSITIVE	50.3	48.1	46.9	45.9	45.2	44.6	44.1	43.7	43.3	43.0	50.3	48.1	46.9	45.9	45.2	44.6	44.1	43.7	43.3	43.0				

EXTERIOR WINDOWS, DOOR AND CLADDING NOTES:
DESIGN WIND PRESSURES (PERPENDICULAR TO SURFACE) FOR ALL EXTERIOR WINDOWS, DOORS AND CLADDING ARE INDICATED ON ELEVATIONS (NEGATIVE VALUES = WIND PRESSURE AWAY FROM SURFACE & POSITIVE VALUES = WIND PRESSURE TOWARDS SURFACE) IN ACCORDANCE WITH ASCE 7-10 COMPONENTS & CLADDING [RISK CATEGORY II]; 170 mph ULTIMATE WIND SPEED; DIRECTIONALITY FACTOR Kd = 0.85; EXPOSURE D; INTERNAL PRESSURE COEFFICIENTS = ± 0.18; BOTH ULTIMATE PRESSURES AND ALLOWABLE PRESSURES (0.6 x Pult) ARE INDICATED ON ELEVATIONS AND SCHEDULE ABOVE.

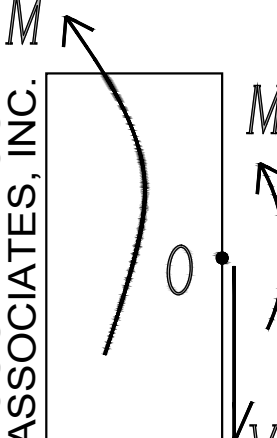
a = 6.4 ft

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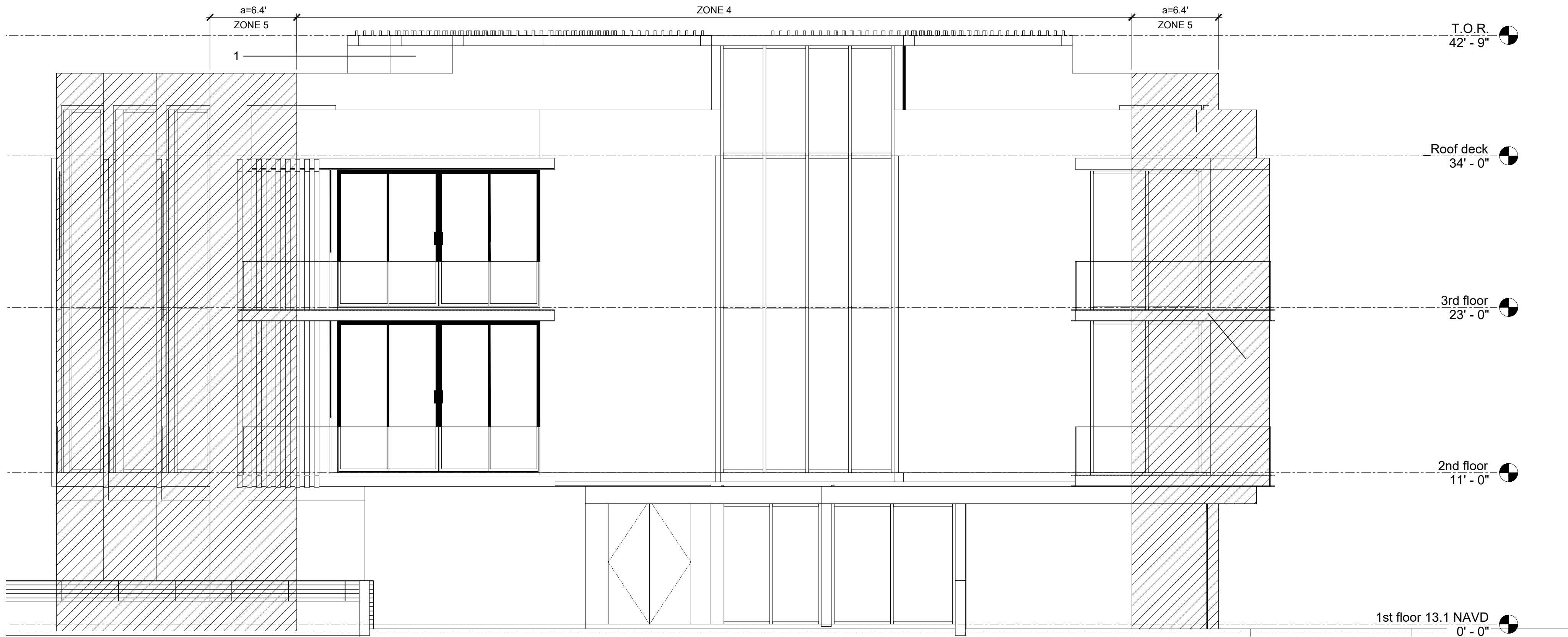
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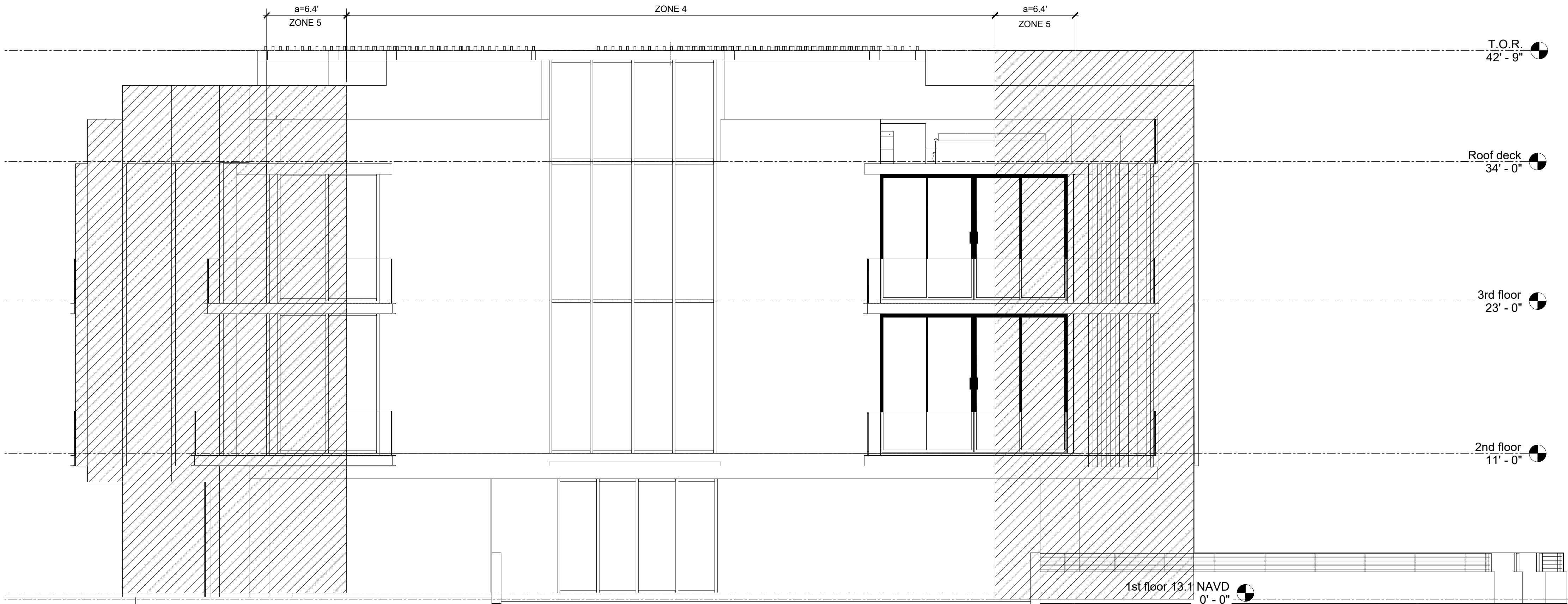
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NORTH ELEVATION

3/16" = 1'-0"



SOUTH ELEVATION

3/16" = 1'-0"

AREA (SQFT)		ZONE 4												ZONE 5											
		10 - 20	20 - 30	30 - 40	40 - 50	50 - 60	60 - 70	70 - 80	80 - 90	90 - 100	>100	10 - 20	20 - 30	30 - 40	40 - 50	50 - 60	60 - 70	70 - 80	80 - 90	90 - 100	>100				
DESIGN WIND PRESSURES	ULTIMATE (PSF)	NEGATIVE	-90.9	-87.2	-85.1	-83.5	-82.3	-81.3	-80.5	-79.8	-79.2	-78.6	-111.9	-104.5	-100.1	-97.1	-94.7	-92.7	-91.1	-89.6	-88.4	-87.2			
	POSITIVE	POSITIVE	83.9	80.2	78.1	76.5	75.3	74.3	73.5	72.8	72.2	71.6	83.9	80.2	78.1	76.5	75.3	74.3	73.5	72.8	72.2	71.6			
ALLOWABLE (PSF)	NEGATIVE	NEGATIVE	-54.5	-52.3	-51.1	-50.1	-49.4	-48.8	-48.3	-47.9	-47.5	-47.2	-67.1	-62.7	-60.1	-58.3	-56.8	-55.6	-54.7	-53.8	-53.0	-52.3			
	POSITIVE	POSITIVE	50.3	48.1	46.9	45.9	45.2	44.6	44.1	43.7	43.3	43.0	50.3	48.1	46.9	45.9	45.2	44.6	44.1	43.7	43.3	43.0			

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a = 6.4 ft

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