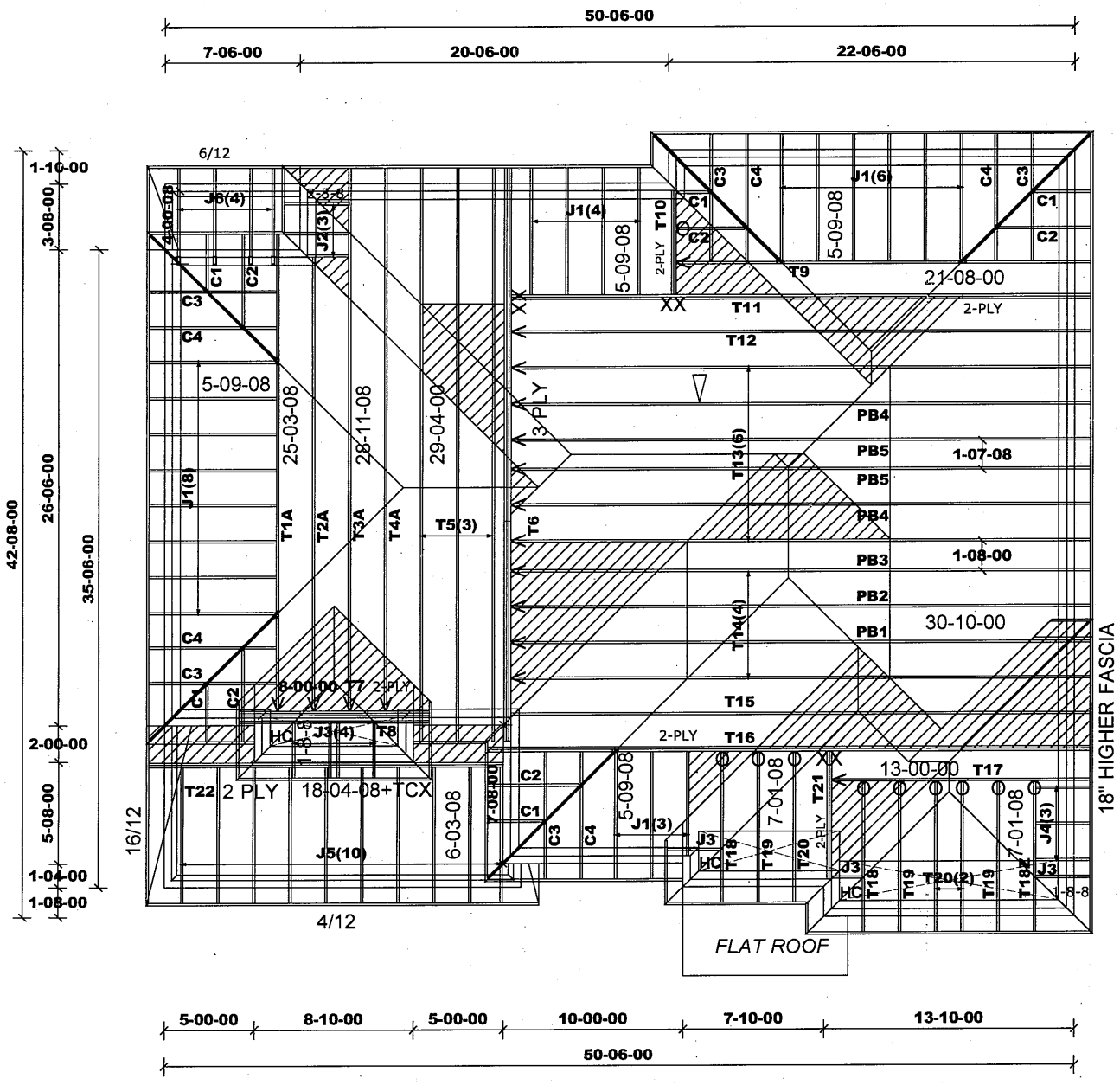



ALL ROOF PITCHES ARE 8/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012 ( 2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

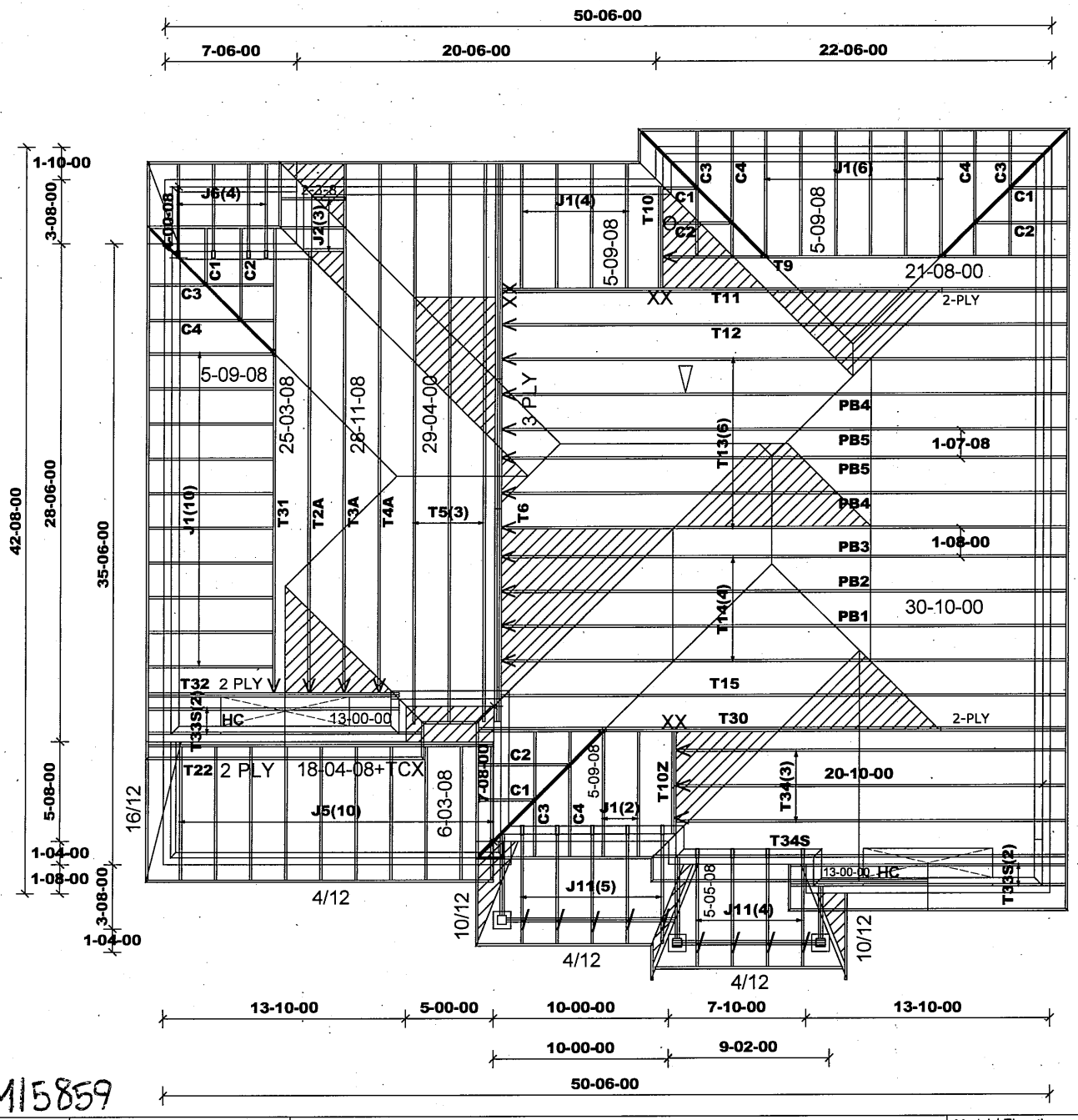
**HC: 18" RAISED CEILING & PLATE**

M15859

| | | | |
|---|---|---|---|
| TAMARACK ROOF TRUSSES INC. | Job Track: 53256 | Builder / Location: GREEN PARK HOMES / RICHMOND HILL | Model / Elevation: ROSE 9A / 1 |
| | Plan Log: 207884 | Project: TRINIGROUP DEVELOPMENTS | |
| | Layout ID: 436956 | Date: 2023-12-21 Sales: Rick DiCiano Designer:LC/YPG | |



Mitek ver 8.6.3.353




ALL ROOF PITCHES ARE 8/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (/)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012 ( 2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

**HC: 18" RAISED CEILING & PLATE**

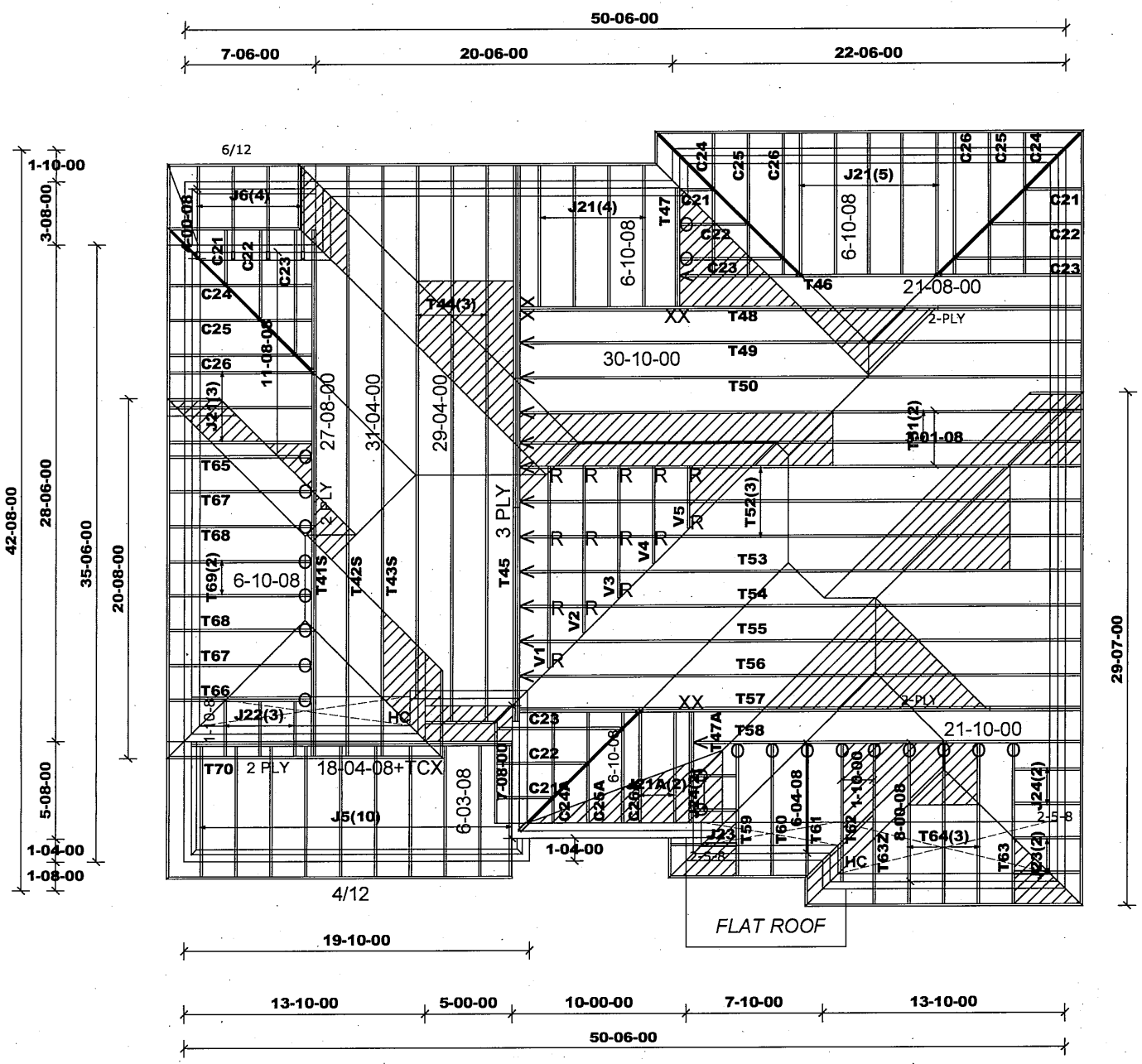


M15859

| | | | |
|---|---|---|---|
| TAMARACK ROOF TRUSSES INC. | Job Track: 53256 | Builder / Location: GREEN PARK HOMES / RICHMOND HILL | Model / Elevation: ROSE 9A / 2 |
| | Plan Log: 207884 | Project: TRINIGROUP DEVELOPMENTS | |
| | Layout ID: 436957 | Date: 2023-12-21 Sales: Rick DiCiano Designer: LC/YPG | |



Mitek ver 8.6.3.353




ALL ROOF PITCHES ARE 6/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
VTCR - (R)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012 ( 2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

**HC: 18" RAISED CEILING & PLATE**



M15859

| | | | |
|---|---|---|---|
| TAMARACK ROOF TRUSSES INC. | Job Track: 53256 | Builder / Location: GREEN PARK HOMES / RICHMOND HILL | Model / Elevation: ROSE 9A / 3 |
| | Plan Log: 207884 | Project: TRINIGROUP DEVELOPMENTS | |
| | Layout ID: 436958 | Date: 2023-12-21 Sales: Rick DiCiano Designer:LC/YPG | |



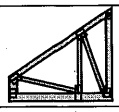
Mitek ver 8.6.3.353

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436956 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 1 of 3 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 1 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## *Roof Trusses*

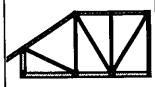
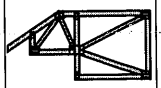
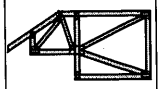
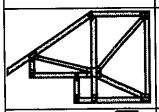
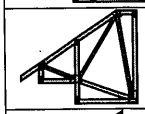
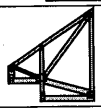
| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 1 | T1A Hip Girder | 8 /12 | 25-03-08 | 5-03-02 | 2 x 4<br>2 x 6 | 1-03-08 | 1-04-13<br>1-07-13 | 126.76<br>79.50 | | |
|  | 1 | T2A Hip | 8 /12 | 25-03-08 | 6-07-02 | 2 x 4 | 1-03-08 | 1-04-13<br>1-07-13 | 112.39<br>72.50 | | |
|  | 1 | T3A Roof Special Girder | 8 /12 | 28-11-08 | 7-11-02 | 2 x 4 | 1-03-08 | 1-04-13<br>1-07-13 | 127.54<br>83.17 | | |
|  | 1 | T4A Roof Special | 8 /12 | 28-11-08 | 9-03-02 | 2 x 4 | 1-03-08 | 1-04-13<br>1-07-13 | 140.85<br>88.83 | | |
|  | 3 | T5 Roof Special | 8 /12 | 29-04-00 | 9-11-08 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 409.09<br>262.50 | | |
|  | 1<br>3-ply | T6 Hip Girder | 8 /12 | 29-04-00 | 8-08-08 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 514.01<br>310.00 | | |
|  | 1<br>2-ply | T7 Hip Girder | 8 /12 | 8-00-00 | 4-03-13 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 2-10-13<br>2-10-13 | 107.99<br>68.67 | | |
|  | 1 | T8 Hip Girder | 8 /12 | 8-00-00 | 2-06-08 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 34.91<br>21.83 | | |
|  | 1 | T9 Hip Girder | 8 /12 | 21-08-00 | 5-03-02 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 109.39<br>67.83 | | |
|  | 1<br>2-ply | T10 Monopitch Girder | 8 /12 | 5-09-08 | 5-03-02 | 2 x 4<br>2 x 6 | | 1-04-13<br>5-03-02 | 67.68<br>43.33 | | |
|  | 1<br>2-ply | T11 Half Hip Girder | 8 /12 | 30-10-00 | 5-03-02 | 2 x 4<br>2 x 6 | 1-03-08 | 1-04-13<br>5-03-02 | 299.9<br>185.33 | | |
|  | 1 | T12 Half Hip | 8 /12 | 30-10-00 | 6-07-02 | 2 x 4 | 1-03-08 | 1-04-13<br>6-07-02 | 137.99<br>88.33 | | |
|  | 6 | T13 Half Hip | 8 /12 | 30-10-00 | 7-11-02 | 2 x 4 | 1-03-08 | 1-04-13<br>7-11-02 | 831.64<br>517.00 | | |
|  | 4 | T14 Hip | 8 /12 | 30-10-00 | 7-11-02 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 553.34<br>350.67 | | |

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436956 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 2 of 3 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 1 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## *Roof Trusses*

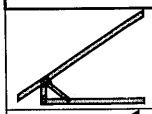
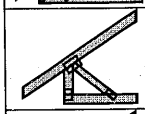
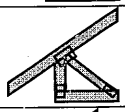
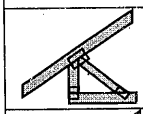
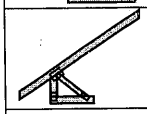
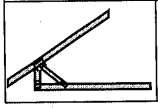
| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 1 | T15 Hip | 8 /12 | 30-10-00 | 6-07-02 | 2 x 4 | 1-03-08 1-03-08 | 1-04-13 1-04-13 | 138.17 87.33 | | |
|  | 1 2-ply | T16 Hip Girder | 8 /12 | 30-10-00 | 5-03-02 | 2 x 4 2 x 6 | 1-03-08 1-03-08 | 1-04-13 1-04-13 | 301.8 184.67 | | |
|  | 1 | T17 Half Hip Girder | 8 /12 | 13-00-00 | 6-06-08 | 2 x 4 2 x 6 | 1-03-08 | 2-10-13 6-06-08 | 77.17 48.33 | | |
|  | 2 | T18 Half Hip Girder | 8 /12 | 7-01-08 | 4-00-08 | 2 x 4 | 1-03-08 | 1-04-13 4-00-08 | 79.1 53.67 | | |
|  | 1 | T18Z Half Hip Girder | 8 /12 | 7-01-08 | 4-00-08 | 2 x 4 | 1-03-08 | 1-04-13 4-00-08 | 39.62 26.83 | | |
|  | 3 | T19 Half Hip | 8 /12 | 7-01-08 | 5-04-08 | 2 x 4 | 1-03-08 | 1-04-13 5-04-08 | 131.24 87.50 | | |
|  | 3 | T20 Half Hip | 8 /12 | 7-01-08 | 6-08-08 | 2 x 4 | 1-03-08 | 1-04-13 6-08-08 | 145.6 98.00 | | |
|  | 1 2-ply | T21 Monopitch Girder | 8 /12 | 7-01-08 | 7-07-13 | 2 x 4 2 x 6 | | 1-04-13 7-07-13 | 106.97 68.33 | | |
|  | 1 2-ply | T22 Half Hip Girder | 16 /12 | 18-04-08 | 3-00-06 | 2 x 6 2 x 4 | 1-03-08 | 1-08-15 2-05-02 | 174.18 110.33 | | |
|  | 1 | PB1 Piggyback | 8 /12 | 11-03-01 | 1-04-00 | 2 x 4 | | | 31.85 21.50 | | |
|  | 1 | PB2 Piggyback | 8 /12 | 11-03-01 | 2-08-00 | 2 x 4 | | | 31.75 20.00 | | |
|  | 1 | PB3 Piggyback | 8 /12 | 11-03-01 | 3-09-00 | 2 x 4 | | | 28.93 17.50 | | |
|  | 2 | PB4 Piggyback | 8 /12 | 21-00-08 | 1-04-00 | 2 x 4 | | 1-00-06 | 130.94 85.00 | | |
|  | 2 | PB5 Piggyback | 8 /12 | 21-00-08 | 2-08-00 | 2 x 4 | | 1-00-06 | 139.43 87.33 | | |

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436956 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 3 of 3 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 1 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## *Roof Trusses*

| PROFILE | QTY PLY | MARK TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT RIGHT | HEEL HEIGHT LEFT RIGHT | LBS. BFT. | BUNDLE # STACK # | LOAD BY REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 21 | J1 Jack-Open | 8 /12 | 5-09-08 | 5-03-02 | 2 x 4 | 1-03-08 | 1-04-13 5-03-02 | 398.61 245.00 | | |
|  | 3 | J2 Jack-Open | 8 /12 | 2-03-08 | 2-11-02 | 2 x 4 | 1-03-08 | 1-04-13 2-11-02 | 30.58 21.00 | | |
|  | 7 | J3 Jack-Open | 8 /12 | 1-08-08 | 2-06-08 | 2 x 4 | 1-03-08 | 1-04-13 2-06-08 | 61.1 39.67 | | |
|  | 3 | J4 Jack-Open | 8 /12 | 1-08-08 | 4-00-08 | 2 x 4 | 1-03-08 | 2-10-13 4-00-08 | 34.11 22.50 | | |
|  | 10 | J5 Jack-Partial | 4 /12 | 6-03-08 | 2-10-00 | 2 x 4 | 1-03-08 | 3-15 2-05-02 | 189.33 126.67 | | |
|  | 4 | J6 Jack-Open | 6 /12 | 4-00-08 | 2-11-15 | 2 x 4 | 1-03-08 | 6-12 2-07-00 | 48.96 34.67 | | |
|  | 5 | C1 Jack-Open | 8 /12 | 1-09-07 | 2-07-02 | 2 x 4 | 1-03-08 1-01 | 1-04-13 2-07-02 | 45.13 28.33 | | |
|  | 5 | C2 Jack-Open | 8 /12 | 1-10-08 | 3-11-02 | 2 x 4 | 1-03-08 1-10-15 | 1-04-13 2-07-13 | 58.84 38.33 | | |
|  | 5 | C3 Jack-Open | 8 /12 | 1-09-07 | 2-07-02 | 2 x 4 | 1-03-08 4-00-01 | 1-04-13 2-07-02 | 67.46 41.67 | | |
|  | 5 | C4 Jack-Open | 8 /12 | 3-09-07 | 3-11-02 | 2 x 4 | 1-03-08 2-00-01 | 1-04-13 3-11-02 | 81.17 51.67 | | |

TOTAL # TRUSS= **121** TOTAL BFT OF ALL TRUSSES= **3885.32** BFT. TOTAL WEIGHT OF ALL TRSSES **6145.56** LBS

## *HARDWARE*

| QTY | TYPE | MODEL | LENGTH |
|---|---|---|---|
| 3 | Hardware | HGUS26-2 | |
| 16 | Hardware | LJS26DS | |
| 10 | Hardware | LUS24 | |

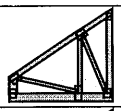
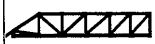
TOTAL NUMBER OF ITEMS= **29**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436957 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 1 of 3 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 2 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## *Roof Trusses*

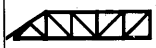
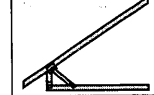
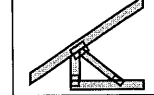
| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 1 | T2A Hip | 8 /12 | 25-03-08 | 6-07-02 | 2 x 4 | 1-03-08 | 1-04-13<br>1-07-13 | 112.39<br>72.50 | | |
|  | 1 | T3A Roof Special Girder | 8 /12 | 28-11-08 | 7-11-02 | 2 x 4 | 1-03-08 | 1-04-13<br>1-07-13 | 127.54<br>83.17 | | |
|  | 1 | T4A Roof Special | 8 /12 | 28-11-08 | 9-03-02 | 2 x 4 | 1-03-08 | 1-04-13<br>1-07-13 | 136.03<br>86.17 | | |
|  | 3 | T5 Roof Special | 8 /12 | 29-04-00 | 9-11-08 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 409.09<br>262.50 | | |
|  | 1<br>3-ply | T6 Hip Girder | 8 /12 | 29-04-00 | 8-08-08 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 514.01<br>310.00 | | |
|  | 1 | T9 Hip Girder | 8 /12 | 21-08-00 | 5-03-02 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 109.39<br>67.83 | | |
|  | 1<br>2-ply | T10 Monopitch Girder | 8 /12 | 5-09-08 | 5-03-02 | 2 x 4<br>2 x 6 | | 1-04-13<br>5-03-02 | 67.63<br>43.33 | | |
|  | 1<br>2-ply | T10Z Monopitch Girder | 8 /12 | 5-09-08 | 5-03-02 | 2 x 4<br>2 x 6 | | 1-04-13<br>5-03-02 | 67.68<br>43.33 | | |
|  | 1<br>2-ply | T11 Half Hip Girder | 8 /12 | 30-10-00 | 5-03-02 | 2 x 4<br>2 x 6 | 1-03-08 | 1-04-13<br>5-03-02 | 299.9<br>185.33 | | |
|  | 1 | T12 Half Hip | 8 /12 | 30-10-00 | 6-07-02 | 2 x 4 | 1-03-08 | 1-04-13<br>6-07-02 | 137.99<br>88.33 | | |
|  | 6 | T13 Half Hip | 8 /12 | 30-10-00 | 7-11-02 | 2 x 4 | 1-03-08 | 1-04-13<br>7-11-02 | 831.64<br>517.00 | | |
|  | 4 | T14 Hip | 8 /12 | 30-10-00 | 7-11-02 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 553.34<br>350.67 | | |
|  | 1 | T15 Hip | 8 /12 | 30-10-00 | 6-07-02 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 138.17<br>87.33 | | |
|  | 1<br>2-ply | T22 Half Hip Girder | 16 /12 | 18-04-08 | 3-00-06 | 2 x 6<br>2 x 4 | 1-03-08 | 1-08-15<br>2-05-02 | 174.18<br>110.33 | | |

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436957 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 2 of 3 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 2 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## *Roof Trusses*

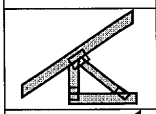
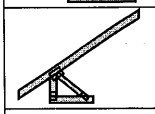
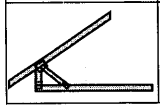
| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 1<br>2-ply | T30<br>Hip Girder | 8 /12 | 30-10-00 | 5-03-02 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 296.84<br>179.67 | | |
|  | 1 | T31<br>Half Hip<br>Girder | 8 /12 | 25-03-08 | 5-03-02 | 2 x 4<br>2 x 6 | 1-03-08 | 1-04-13<br>5-03-02 | 127.08<br>78.33 | | |
|  | 1<br>2-ply | T32<br>Common<br>Girder | 8 /12 | 13-00-00 | 5-08-13 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 162.89<br>98.67 | | |
|  | 4 | T33S<br>Roof Special | 8 /12 | 13-00-00 | 5-08-13 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 256.21<br>164.00 | | |
|  | 3 | T34<br>Common | 8 /12 | 20-10-00 | 8-04-02 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 283.76<br>176.00 | | |
|  | 1 | T34S<br>Roof Special | 8 /12 | 20-10-00 | 8-04-02 | 2 x 4 | 1-03-08<br>1-03-08 | 1-04-13<br>1-04-13 | 103.69<br>66.33 | | |
|  | 1 | PB1<br>Piggyback | 8 /12 | 11-03-01 | 1-04-00 | 2 x 4 | | | 31.85<br>21.50 | | |
|  | 1 | PB2<br>Piggyback | 8 /12 | 11-03-01 | 2-08-00 | 2 x 4 | | | 31.75<br>20.00 | | |
|  | 1 | PB3<br>Piggyback | 8 /12 | 11-03-01 | 3-09-00 | 2 x 4 | | | 28.93<br>17.50 | | |
|  | 2 | PB4<br>Piggyback | 8 /12 | 21-00-08 | 1-04-00 | 2 x 4 | | 1-00-06 | 130.94<br>85.00 | | |
|  | 2 | PB5<br>Piggyback | 8 /12 | 21-00-08 | 2-08-00 | 2 x 4 | | 1-00-06 | 139.43<br>87.33 | | |
|  | 22 | J1<br>Jack-Open | 8 /12 | 5-09-08 | 5-03-02 | 2 x 4 | 1-03-08 | 1-04-13<br>5-03-02 | 417.6<br>256.67 | | |
|  | 3 | J2<br>Jack-Open | 8 /12 | 2-03-08 | 2-11-02 | 2 x 4 | 1-03-08 | 1-04-13<br>2-11-02 | 30.58<br>21.00 | | |
|  | 10 | J5<br>Jack-Partial | 4 /12 | 6-03-08 | 2-10-00 | 2 x 4 | 1-03-08 | 3-15<br>2-05-02 | 189.33<br>126.67 | | |

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436957 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 3 of 3 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 2 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## Roof Trusses

| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 4 | J6 Jack-Open | 6 /12 | 4-00-08 | 2-11-15 | 2 x 4 | 1-03-08 | 6-12<br>2-07-00 | 48.96<br>34.67 | | |
|  | 9 | J11 Jack-Open | 4 /12 | 5-05-08 | 2-06-11 | 2 x 4 | 1-03-08 | 3-15<br>2-01-12 | 130.04<br>84.00 | | |
|  | 4 | C1 Jack-Open | 8 /12 | 1-09-07 | 2-07-02 | 2 x 4 | 1-03-08<br>1-01 | 1-04-13<br>2-07-02 | 36.11<br>22.67 | | |
|  | 4 | C2 Jack-Open | 8 /12 | 1-10-08 | 3-11-02 | 2 x 4 | 1-03-08<br>1-10-15 | 1-04-13<br>2-07-13 | 47.07<br>30.67 | | |
|  | 4 | C3 Jack-Open | 8 /12 | 1-09-07 | 2-07-02 | 2 x 4 | 1-03-08<br>4-00-01 | 1-04-13<br>2-07-02 | 53.97<br>33.33 | | |
|  | 4 | C4 Jack-Open | 8 /12 | 3-09-07 | 3-11-02 | 2 x 4 | 1-03-08<br>2-00-01 | 1-04-13<br>3-11-02 | 64.93<br>41.33 | | |

TOTAL # TRUSS= **114** TOTAL BFT OF ALL TRUSSES= **3953.16** BFT. TOTAL WEIGHT OF ALL TRSSES **6290.95** LBS

## HARDWARE

| QTY | TYPE | MODEL | LENGTH |
|---|---|---|---|
| 9 | Hardware | H2.5T | |
| 3 | Hardware | HGUS26-2 | |
| 18 | Hardware | LJS26DS | |
| 1 | Hardware | LUS24 | |

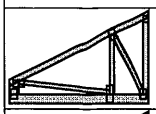
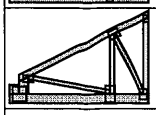
TOTAL NUMBER OF ITEMS= **31**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436958 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 1 of 4 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 3 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## *Roof Trusses*

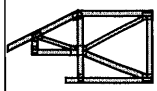
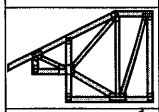
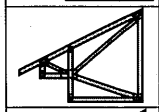
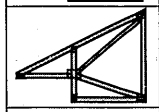
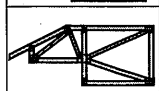
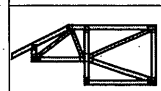
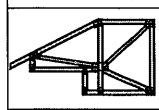
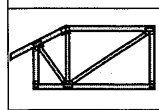
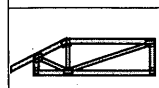
| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 1<br>2-ply | T41S<br>Roof Special Girder | 6 /12 | 27-08-00 | 5-07-04 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-02-00<br>1-02-00 | 284.01<br>180.33 | | |
|  | 1 | T42S<br>Roof Special | 6 /12 | 31-04-00 | 5-07-04 | 2 x 4 | 1-03-08<br>1-03-08 | 1-02-00<br>1-02-00 | 140.78<br>94.17 | | |
|  | 1 | T43S<br>Roof Special | 6 /12 | 31-04-00 | 6-07-04 | 2 x 4 | 1-03-08<br>1-03-08 | 1-02-00<br>1-02-00 | 135.57<br>87.83 | | |
|  | 3 | T44<br>Roof Special | 6 /12 | 29-04-00 | 7-07-00 | 2 x 4 | 1-03-08<br>1-03-08 | 1-02-00<br>1-02-00 | 356.68<br>226.50 | | |
|  | 1<br>3-ply | T45<br>Hip Girder | 6 /12 | 29-04-00 | 6-07-12 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-02-00<br>1-02-00 | 458.39<br>282.00 | | |
|  | 1 | T46<br>Hip Girder | 6 /12 | 21-08-00 | 4-07-04 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 1-02-00<br>1-02-00 | 106.15<br>67.00 | | |
|  | 1<br>2-ply | T47<br>Monopitch Girder | 6 /12 | 6-10-08 | 4-07-04 | 2 x 4<br>2 x 6 | | 1-02-00<br>4-07-04 | 71.09<br>44.00 | | |
|  | 1<br>2-ply | T47A<br>Monopitch Girder | 6 /12 | 6-05-00 | 4-07-04 | 2 x 4<br>2 x 6 | | 1-04-12<br>4-07-04 | 71.71<br>46.00 | | |
|  | 1<br>2-ply | T48<br>Half Hip Girder | 6 /12 | 30-10-00 | 4-07-04 | 2 x 4<br>2 x 6 | 1-03-08 | 1-02-00<br>4-07-04 | 292.01<br>180.67 | | |
|  | 1 | T49<br>Half Hip | 6 /12 | 30-10-00 | 5-07-04 | 2 x 4 | 1-03-08 | 1-02-00<br>5-07-04 | 129.7<br>83.00 | | |
|  | 1 | T50<br>Half Hip | 6 /12 | 30-10-00 | 6-07-04 | 2 x 4 | 1-03-08 | 1-02-00<br>6-07-04 | 129.38<br>81.17 | | |
|  | 2 | T51<br>Hip | 6 /12 | 30-10-00 | 7-07-04 | 2 x 4 | 1-03-08 | 1-02-00<br>6-10-00 | 299.68<br>189.67 | | |
|  | 3 | T52<br>Roof Special | 6 /12 | 30-10-00 | 8-10-08 | 2 x 4 | 1-03-08 | 2-08-00<br>1-02-00 | 402.54<br>251.50 | | |
|  | 1 | T53<br>Roof Special | 6 /12 | 30-10-00 | 8-07-04 | 2 x 4 | 1-03-08 | 2-08-00<br>1-02-00 | 143.35<br>92.17 | | |

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436958 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 2 of 4 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 3 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## *Roof Trusses*

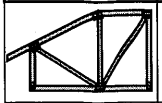
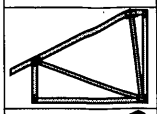
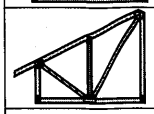
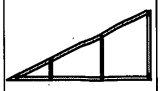
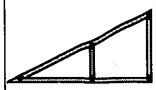
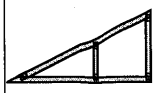
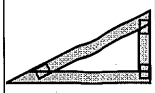
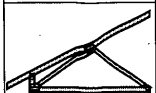
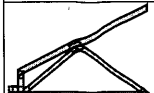
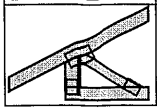
| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 1 | T54<br>Hip | 6 /12 | 30-10-00 | 7-07-04 | 2 x 4 | 1-03-08 | 2-08-00<br>1-02-00 | 135.18<br>85.50 | | |
|  | 1 | T55<br>Hip | 6 /12 | 30-10-00 | 6-07-04 | 2 x 4 | 1-03-08 | 2-08-00<br>1-02-00 | 129.18<br>83.00 | | |
|  | 1 | T56<br>Hip | 6 /12 | 30-10-00 | 5-07-04 | 2 x 4 | 1-03-08 | 2-08-00<br>1-02-00 | 126.21<br>82.00 | | |
|  | 1<br>2-ply | T57<br>Hip Girder | 6 /12 | 30-10-00 | 4-07-04 | 2 x 4<br>2 x 6 | 1-03-08 | 2-08-00<br>1-02-00 | 290.56<br>180.67 | | |
|  | 1 | T58<br>Hip Girder | 6 /12 | 20-10-00 | 5-10-04 | 2 x 4<br>2 x 6 | 1-03-08<br>1-03-08 | 2-08-00<br>2-08-00 | 117.77<br>75.33 | | |
|  | 1 | T59<br>Half Hip<br>Girder | 6 /12 | 6-04-08 | 3-10-12 | 2 x 4 | 1-03-08 | 1-02-00<br>3-10-12 | 34.9<br>23.50 | | |
|  | 1 | T60<br>Half Hip | 6 /12 | 6-04-08 | 4-10-12 | 2 x 4 | 1-03-08 | 1-02-00<br>4-10-12 | 40.91<br>27.33 | | |
|  | 1 | T61<br>Monopitch | 6 /12 | 6-04-08 | 5-10-04 | 2 x 4 | 1-03-08 | 1-02-00<br>5-10-04 | 38.09<br>25.33 | | |
|  | 1 | T62<br>Monopitch | 6 /12 | 8-00-08 | 5-10-04 | 2 x 4 | | 4-00<br>5-10-04 | 38.33<br>25.67 | | |
|  | 1 | T63<br>Half Hip<br>Girder | 6 /12 | 8-00-08 | 3-10-12 | 2 x 4 | 1-03-08 | 1-02-00<br>3-10-12 | 41.27<br>28.17 | | |
|  | 1 | T63Z<br>Half Hip | 6 /12 | 8-00-08 | 3-10-12 | 2 x 4 | 1-03-08 | 1-02-00<br>3-10-12 | 41.27<br>28.17 | | |
|  | 3 | T64<br>Half Hip | 6 /12 | 8-00-08 | 4-10-12 | 2 x 4 | 1-03-08 | 1-02-00<br>4-10-12 | 133.52<br>91.00 | | |
|  | 1 | T65<br>Half Hip | 6 /12 | 6-10-08 | 3-07-04 | 2 x 4 | 1-03-08 | 2-08-00<br>3-07-04 | 32.77<br>21.83 | | |
|  | 1 | T66<br>Half Hip<br>Girder | 6 /12 | 6-10-08 | 2-01-04 | 2 x 4 | 1-03-08 | 1-02-00<br>2-01-04 | 26.79<br>17.17 | | |

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436958 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 3 of 4 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 3 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## *Roof Trusses*

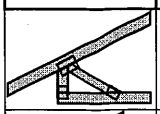
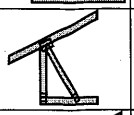
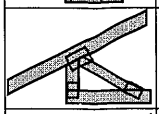
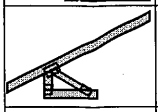
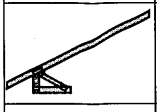
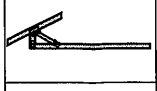
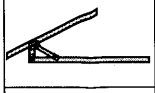
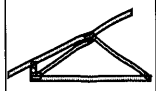
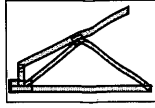
| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 2 | T67<br>Half Hip | 6 /12 | 6-10-08 | 4-07-04 | 2 x 4 | 1-03-08 | 2-08-00<br>4-07-04 | 71.31<br>47.00 | | |
|  | 2 | T68<br>Half Hip | 6 /12 | 6-10-08 | 5-07-04 | 2 x 4 | 1-03-08 | 2-08-00<br>5-07-04 | 71.8<br>47.67 | | |
|  | 2 | T69<br>Common | 6 /12 | 6-10-08 | 5-11-00 | 2 x 4 | 1-03-08 | 2-08-00<br>5-08-12 | 76.09<br>48.33 | | |
|  | 1<br>2-ply | T70<br>Flat Girder | 0 /12 | 18-04-08 | 2-05-02 | 2 x 6<br>2 x 4 | | 2-05-02<br>2-05-02 | 164.86<br>101.67 | | |
|  | 1 | V1<br>Valley | 6 /12 | 11-07-00 | 5-07-04 | 2 x 4 | | 5-07-04 | 36.57<br>23.67 | | |
|  | 1 | V2<br>Valley | 6 /12 | 9-07-00 | 4-09-08 | 2 x 4 | | 4-09-08 | 28.88<br>18.83 | | |
|  | 1 | V3<br>Valley | 6 /12 | 7-07-00 | 3-09-08 | 2 x 4 | | 3-09-08 | 22.5<br>14.33 | | |
|  | 1 | V4<br>Valley | 6 /12 | 5-07-00 | 2-09-08 | 2 x 4 | | 2-09-08 | 15.12<br>10.00 | | |
|  | 1 | V5<br>Valley | 6 /12 | 3-07-00 | 1-09-08 | 2 x 4 | | 1-09-08 | 9.15<br>6.67 | | |
|  | 10 | J5<br>Jack-Partial | 4 /12 | 6-03-08 | 2-10-00 | 2 x 4 | 1-03-08 | 3-15<br>2-05-02 | 189.33<br>126.67 | | |
|  | 4 | J6<br>Jack-Open | 6 /12 | 4-00-08 | 2-11-15 | 2 x 4 | 1-03-08 | 6-12<br>2-07-00 | 48.96<br>34.67 | | |
|  | 12 | J21<br>Jack-Partial | 6 /12 | 6-10-08 | 4-07-04 | 2 x 4 | 1-03-08 | 1-02-00<br>4-07-04 | 308.82<br>198.00 | | |
|  | 2 | J21A<br>Jack-Partial | 6 /12 | 6-05-00 | 4-07-04 | 2 x 4 | | 1-04-12<br>4-07-04 | 47.96<br>31.67 | | |
|  | 3 | J22<br>Jack-Open | 6 /12 | 1-10-08 | 2-01-04 | 2 x 4 | 1-03-08 | 1-02-00<br>2-01-04 | 25.25<br>17.00 | | |

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

# DELIVERY SHIPLIST

| | | | |
|---|---|---|---|
| Lumber Yard: | TAMARACK LUMBER | Job Track: | 53256 |
| Builder: | GREEN PARK HOMES | PlanLog: | 207884 |
| Project: | TRINIGROUP DEVELOPMENTS | Layout ID: | 436958 |
| Location: | RICHMOND HILL | Ref # | |
| Model: | ROSE 9A | Page: | 4 of 4 |
| Lot #: | | Date: | 12-21-2023 |
| Elevation: | 3 | Designer: | |
| | | Sales Rep: | Rick DiCiano |

## Roof Trusses

| PROFILE | QTY / PLY | MARK / TYPE | PITCH | SPAN | HEIGHT | LUMBER | OVERHANG LEFT / RIGHT | HEEL HEIGHT LEFT / RIGHT | LBS. / BFT. | BUNDLE # / STACK # | LOAD BY / REMARKS |
|---|---|---|---|---|---|---|---|---|---|---|---|
|  | 3 | J23<br>Jack-Open | 6 /12 | 2-05-08 | 2-04-12 | 2 x 4 | 1-03-08 | 1-02-00<br>2-04-12 | 29.48<br>21.00 | | |
|  | 4 | J24<br>Jack-Open | 6 /12 | 2-05-08 | 3-10-12 | 2 x 4 | 1-03-08 | 2-08-00<br>3-10-12 | 49.36<br>35.33 | | |
|  | 4 | C21<br>Jack-Open | 6 /12 | 1-09-07 | 2-00-12 | 2 x 4 | 1-03-08<br>1-01 | 1-02-00<br>2-00-12 | 33.22<br>22.67 | | |
|  | 4 | C22<br>Jack-Open | 6 /12 | 1-10-08 | 3-00-12 | 2 x 4 | 1-03-08<br>1-10-15 | 1-02-00<br>2-01-04 | 43.41<br>28.00 | | |
|  | 4 | C23<br>Jack-Open | 6 /12 | 1-10-08 | 4-00-12 | 2 x 4 | 1-03-08<br>3-10-15 | 1-02-00<br>2-01-04 | 53.61<br>36.00 | | |
|  | 3 | C24<br>Jack-Open | 6 /12 | 1-09-07 | 2-00-12 | 2 x 4 | 1-03-08<br>5-01-01 | 1-02-00<br>2-00-12 | 42.02<br>27.00 | | |
|  | 1 | C24A<br>Jack-Open | 6 /12 | 6-05-00 | 2-00-12 | 2 x 4 | | 1-04-12<br>4-07-04 | 12.31<br>8.33 | | |
|  | 3 | C25<br>Jack-Open | 6 /12 | 3-09-07 | 3-00-12 | 2 x 4 | 1-03-08<br>3-01-01 | 1-02-00<br>3-00-12 | 49.67<br>31.00 | | |
|  | 1 | C25A<br>Jack-Open | 6 /12 | 6-05-00 | 3-00-12 | 2 x 4 | | 1-04-12<br>4-07-04 | 14.86<br>9.67 | | |
|  | 3 | C26<br>Jack-Open | 6 /12 | 5-09-07 | 4-00-12 | 2 x 4 | 1-03-08<br>1-01-01 | 1-02-00<br>4-00-12 | 73.04<br>47.50 | | |
|  | 1 | C26A<br>Jack-Open | 6 /12 | 6-05-00 | 4-00-12 | 2 x 4 | | 1-04-12<br>4-07-04 | 22.59<br>15.17 | | |

TOTAL # TRUSS= **117** TOTAL BFT OF ALL TRUSSES= **3708.53** BFT. TOTAL WEIGHT OF ALL TRSSES **5787.99** LBS

## HARDWARE

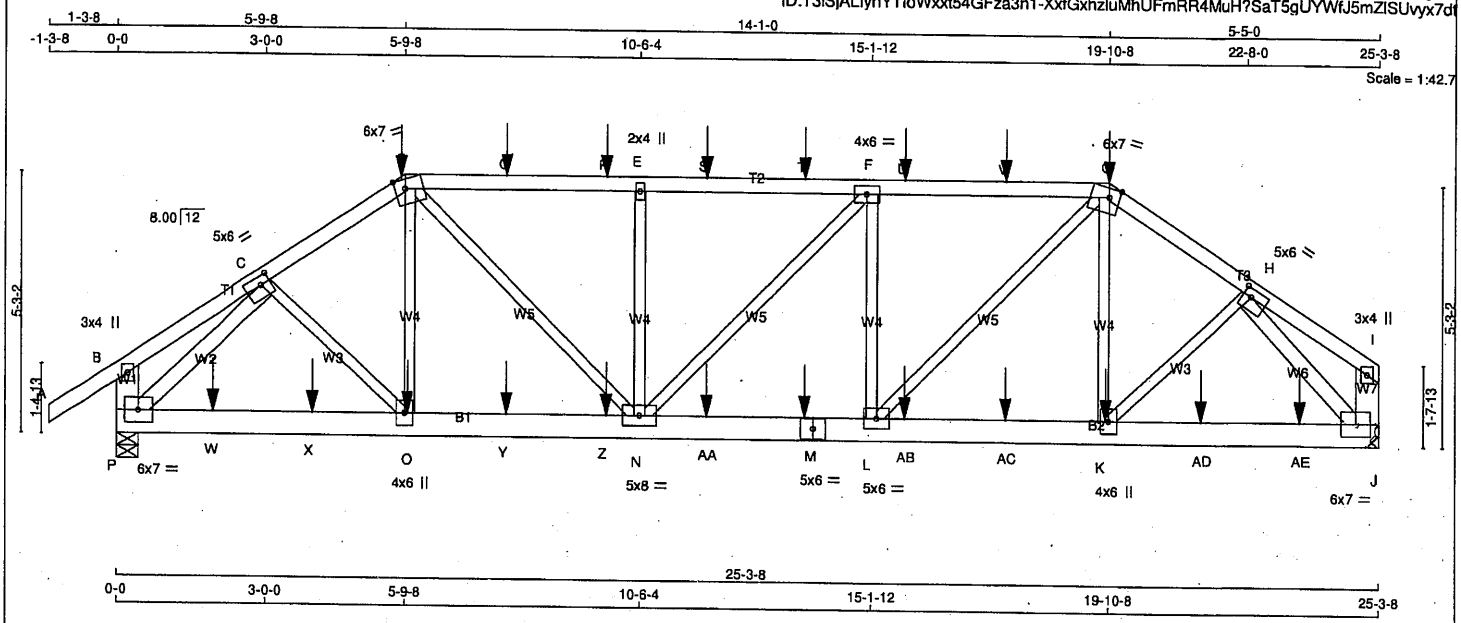
| QTY | TYPE | MODEL | LENGTH |
|---|---|---|---|
| 3 | Hardware | HGUS26-2 | |
| 14 | Hardware | LJS26DS | |
| 20 | Hardware | LUS24 | |
| 15 | | VTCR | |

TOTAL NUMBER OF ITEMS= **52**

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T1A | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:22 2023 Page 1
ID:T3iSjALIyhYTIoWxxt54GFza3n1-XxfGxhzluMhUFmRR4MuH?SaT5gUYWfJ5mZlSUvyx7dt




TOTAL WEIGHT = 127 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - G | 2x4 | DRY | No.2 | SPF |
| G - I | 2x4 | DRY | No.2 | SPF |
| P - B | 2x6 | DRY | No.2 | SPF |
| J - I | 2x6 | DRY | No.2 | SPF |
| P - M | 2x6 | DRY | No.2 | SPF |
| M - J | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - J | 2x4 | DRY | No.2 | SPF |
| P - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | 2.00 | 2.25 |
| D | TTWW-m | MT20 | 6.0 | 7.0 | Edge | 2.50 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TMWW-t | MT20 | 4.0 | 6.0 | | |
| G | TTWW-m | MT20 | 6.0 | 7.0 | Edge | 2.50 |
| H | TMWW-t | MT20 | 5.0 | 6.0 | 2.00 | 2.00 |
| I | TMV+p | MT20 | 3.0 | 4.0 | | |
| J | BMVW1-t | MT20 | 6.0 | 7.0 | | |
| K | BMWW+t | MT20 | 4.0 | 6.0 | | |
| L | BMWW-t | MT20 | 5.0 | 6.0 | | |
| M | BS-t | MT20 | 5.0 | 6.0 | | |
| N | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| O | BMWW+t | MT20 | 4.0 | 6.0 | | |
| P | BMVW1-t | MT20 | 6.0 | 7.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311444

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| J | 2618 | 0 | 2618 | 0 | 0 | MECHANICAL | |
| P | 2708 | 0 | 2708 | 0 | 0 | 5-8 | 2-15 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-13.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| J | 1853 | 1206 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 647 / 0 | 0 / 0 |
| P | 1914 | 1260 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 654 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 |
| B- C | 0 / 12 | -91.8 | -91.8 | 0.10 (1) | 10.00 |
| C- D | -3230 / 0 | -91.8 | -91.8 | 0.25 (1) | 3.67 |
| D- Q | -3634 / 0 | -91.8 | -91.8 | 0.86 (1) | 2.62 |
| Q- R | -3634 / 0 | -91.8 | -91.8 | 0.86 (1) | 2.62 |
| R- E | -3634 / 0 | -91.8 | -91.8 | 0.86 (1) | 2.62 |
| E- S | -3634 / 0 | -91.8 | -91.8 | 0.87 (1) | 2.58 |
| S- T | -3634 / 0 | -91.8 | -91.8 | 0.87 (1) | 2.58 |
| T- F | -3634 / 0 | -91.8 | -91.8 | 0.87 (1) | 2.58 |
| F- U | -3610 / 0 | -91.8 | -91.8 | 0.88 (1) | 2.59 |
| U- V | -3610 / 0 | -91.8 | -91.8 | 0.88 (1) | 2.59 |
| V- G | -3610 / 0 | -91.8 | -91.8 | 0.88 (1) | 2.59 |
| G- H | -3097 / 0 | -91.8 | -91.8 | 0.24 (1) | 3.75 |
| H- I | 0 / 13 | -91.8 | -91.8 | 0.09 (1) | 10.00 |
| P- B | -236 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 |
| J- I | -92 / 0 | 0.0 | 0.0 | 0.01 (1) | 7.81 |
| P- W | 0 / 2369 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| W- X | 0 / 2369 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| X- O | 0 / 2369 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| O- Y | 0 / 2670 | -18.5 | -18.5 | 0.41 (1) | 10.00 |
| Y- Z | 0 / 2670 | -18.5 | -18.5 | 0.41 (1) | 10.00 |
| Z- N | 0 / 2670 | -18.5 | -18.5 | 0.41 (1) | 10.00 |
| N-AA | 0 / 3611 | -18.5 | -18.5 | 0.55 (1) | 10.00 |
| AA- M | 0 / 3611 | -18.5 | -18.5 | 0.55 (1) | 10.00 |
| M- L | 0 / 3611 | -18.5 | -18.5 | 0.55 (1) | 10.00 |
| L-AB | 0 / 2557 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| AB-AC | 0 / 2557 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| AC- K | 0 / 2557 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| K-AD | 0 / 2108 | -18.5 | -18.5 | 0.34 (1) | 10.00 |
| AD-AE | 0 / 2108 | -18.5 | -18.5 | 0.34 (1) | 10.00 |
| AE- J | 0 / 2106 | -18.5 | -18.5 | 0.34 (1) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| O- D | -57 / 127 | 0.05 (4) |
| K- G | -223 / 68 | 0.09 (1) |
| K- H | 0 / 632 | 0.16 (1) |
| H- J | -3233 / 0 | 0.57 (1) |
| C- O | 0 / 421 | 0.10 (1) |
| P- C | -3370 / 0 | 0.65 (1) |
| L- G | 0 / 1513 | 0.37 (1) |
| D- N | 0 / 1385 | 0.34 (1) |
| L- F | -925 / 0 | 0.37 (1) |
| N- E | -893 / 0 | 0.35 (1) |
| N- F | 0 / 34 | 0.01 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| D | 5-9-8 | -327 | -327 | --- | FRONT | VERT | TOTAL | --- | C1 |
| G | 19-10-8 | -327 | -327 | --- | FRONT | VERT | TOTAL | --- | C1 |
| K | 19-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.88/1.00 (F-G:1) , BC=0.55/1.00 (L-N:1) , WB=0.65/1.00 (C-P:1) , SSI=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (G) (INPUT = 0.90 )
JSI METAL= 0.77 (C) (INPUT = 0.95 )



CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T1A | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| M | 13-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| O | 5-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Q | 7-10-4 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| R | 9-10-4 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| S | 11-10-4 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| T | 13-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| U | 15-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| V | 17-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| W | 1-11-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| X | 3-11-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Y | 7-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Z | 9-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AA | 11-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AB | 15-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AC | 17-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AD | 21-8-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AE | 23-8-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311444

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T2A | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:23 2023 Page 1
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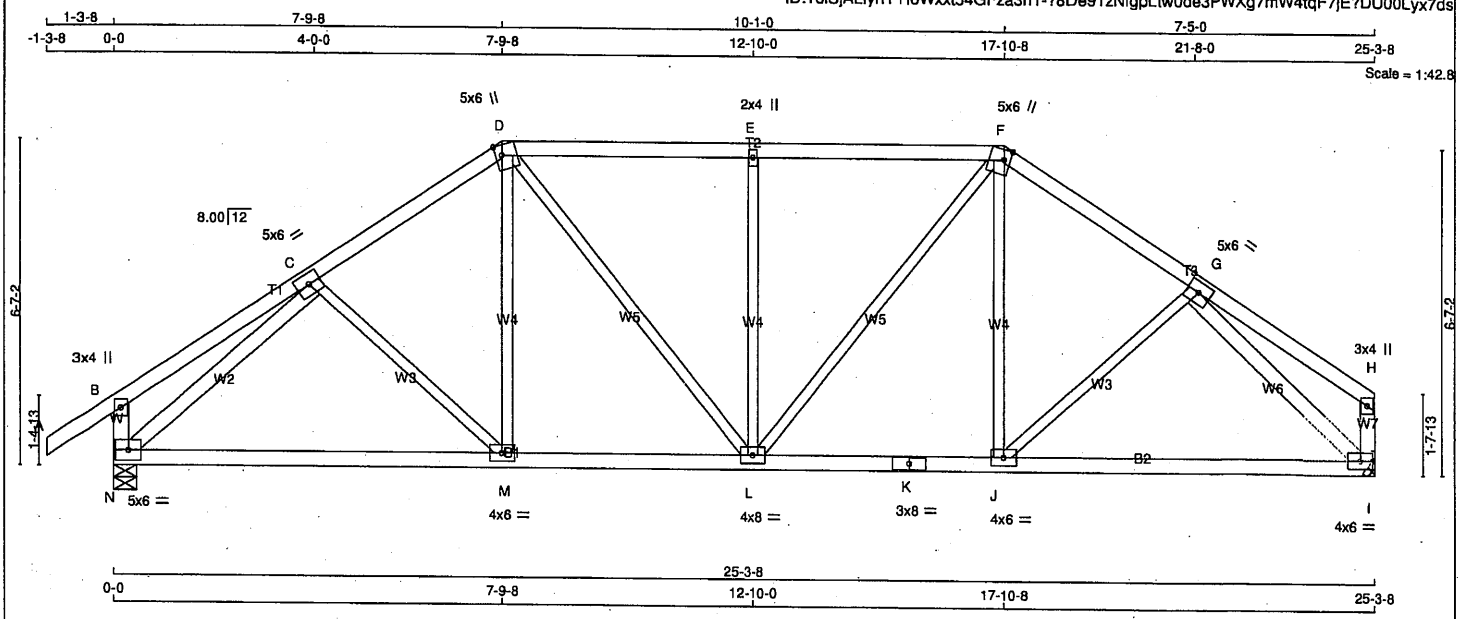


TOTAL WEIGHT = 112 lb
[M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| N - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| N - K | 2x4 | DRY | No.2 | SPF |
| K - I | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| N - C | 2x4 | DRY | No.2 | SPF |
| G - I | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | | |
| D | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| G | TMWW-t | MT20 | 5.0 | 6.0 | | |
| H | TMV+p | MT20 | 3.0 | 4.0 | | |
| I | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| J | BMWW-t | MT20 | 4.0 | 6.0 | | |
| K | BS-t | MT20 | 3.0 | 8.0 | | |
| L | BMWWW-t | MT20 | 4.0 | 6.0 | | |
| M | BMWW-t | MT20 | 4.0 | 6.0 | | |
| N | BMVW1-t | MT20 | 5.0 | 6.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| N | 1520 | 0 | 1520 | 0 | 0 | 5-8 | 1-10 |
| I | 1394 | 0 | 1394 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| N | 1073 | 717 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 355 / 0 | 0 / 0 |
| I | 986 | 647 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 339 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | C- M | -78 / 27 | 0.04 (1) | |
| B- C | 0 / 22 | -91.8 | -91.8 | 0.22 (1) | 10.00 | M- D | 0 / 205 | 0.05 (4) | |
| C- D | -1478 / 0 | -91.8 | -91.8 | 0.19 (1) | 5.21 | D- L | 0 / 367 | 0.08 (1) | |
| D- E | -1442 / 0 | -91.8 | -91.8 | 0.32 (1) | 5.09 | L- E | -564 / 0 | 0.40 (1) | |
| E- F | -1442 / 0 | -91.8 | -91.8 | 0.32 (1) | 5.09 | L- F | 0 / 422 | 0.09 (1) | |
| F- G | -1436 / 0 | -91.8 | -91.8 | 0.18 (1) | 5.28 | J- F | 0 / 136 | 0.04 (4) | |
| G- H | 0 / 23 | -91.8 | -91.8 | 0.20 (1) | 10.00 | J- G | 0 / 55 | 0.02 (4) | |
| N- B | -264 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | N- C | -1742 / 0 | 0.58 (1) | |
| I- H | -121 / 0 | 0.0 | 0.0 | 0.01 (1) | 7.81 | G- I | -1684 / 0 | 0.50 (1) | |
| N- M | 0 / 1266 | -18.5 | -18.5 | 0.35 (4) | 10.00 | | | | |
| M- L | 0 / 1213 | -18.5 | -18.5 | 0.36 (4) | 10.00 | | | | |
| L- K | 0 / 1178 | -18.5 | -18.5 | 0.33 (4) | 10.00 | | | | |
| K- J | 0 / 1178 | -18.5 | -18.5 | 0.33 (4) | 10.00 | | | | |
| J- I | 0 / 1164 | -18.5 | -18.5 | 0.32 (4) | 10.00 | | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.36/1.00 (L-M:4) , WB=0.58/1.00 (C-N:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (I) (INPUT = 0.90 )
JSI METAL= 0.39 (C) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311445

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T3A | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:24 2023 Page 1
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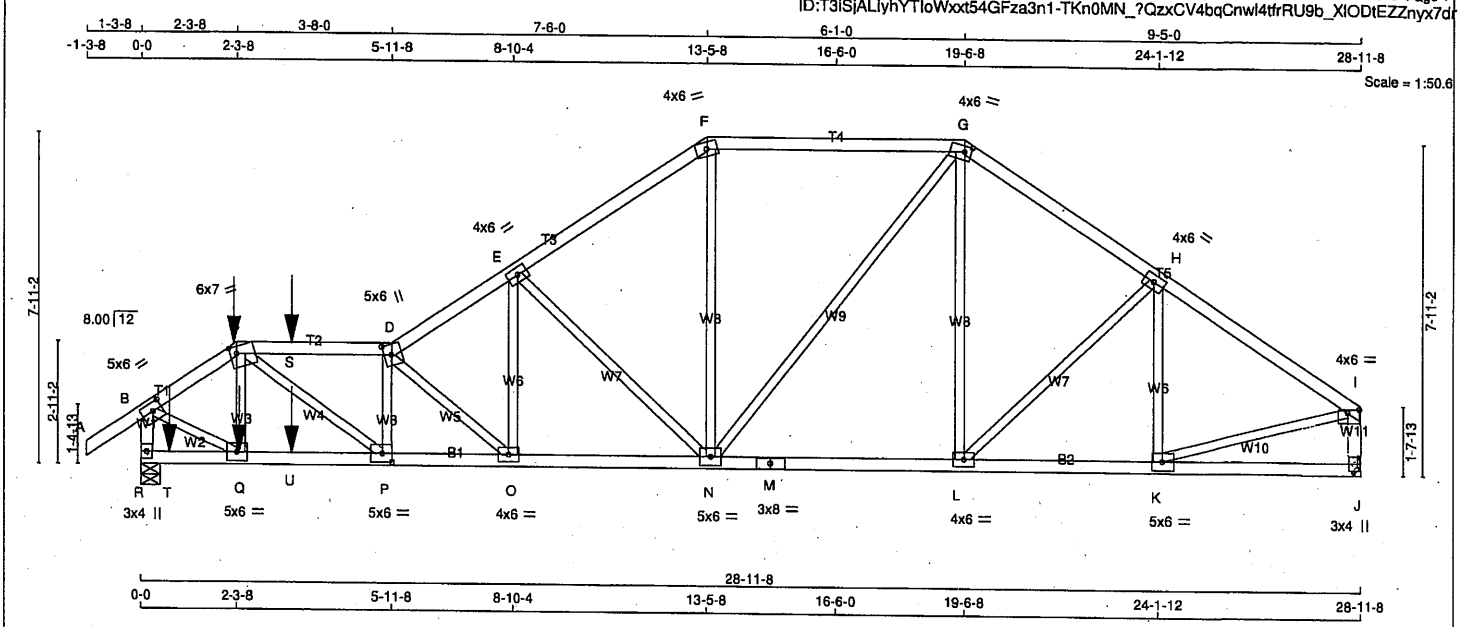


**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - G | 2x4 | DRY | No.2 | SPF |
| G - I | 2x4 | DRY | No.2 | SPF |
| R - B | 2x4 | DRY | No.2 | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| R - M | 2x4 | DRY | No.2 | SPF |
| M - J | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.25 | 2.75 |
| C | TTWW-m | MT20 | 6.0 | 7.0 | 2.00 | 2.00 |
| D | TTWW+m | MT20 | 5.0 | 6.0 | 3.00 | 2.00 |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TTW-m | MT20 | 4.0 | 6.0 | | |
| G | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.00 |
| H | TMWW-t | MT20 | 4.0 | 6.0 | | |
| I | TMVW-p | MT20 | 4.0 | 6.0 | 1.00 | 3.25 |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |
| K | BMWW-t | MT20 | 5.0 | 6.0 | | |
| L | BMWW-t | MT20 | 4.0 | 6.0 | | |
| M | BS-t | MT20 | 3.0 | 8.0 | | |
| N | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| O | BMWW-t | MT20 | 4.0 | 6.0 | | |
| P | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| Q | BMWW-t | MT20 | 5.0 | 6.0 | | |
| R | BMV1+p | MT20 | 3.0 | 4.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311446

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| R | 1811 | 0 | 1811 | 0 | 0 | 5-8 | 2-6 |
| J | 1604 | 0 | 1604 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-12.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| R | 1279 | 851 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 428 / 0 | 0 / 0 |
| J | 1135 | 745 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 390 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | | 10.00 |
| B- C | -1681 / 0 | -91.8 | -91.8 | 0.13 (1) | | 4.98 |
| C- S | -2895 / 0 | -91.8 | -91.8 | 0.38 (1) | | 3.69 |
| S- D | -2895 / 0 | -91.8 | -91.8 | 0.38 (1) | | 3.69 |
| D- E | -2650 / 0 | -91.8 | -91.8 | 0.37 (1) | | 3.84 |
| E- F | -1853 / 0 | -91.8 | -91.8 | 0.35 (1) | | 4.55 |
| F- G | -1530 / 0 | -91.8 | -91.8 | 0.76 (1) | | 4.13 |
| G- H | -1647 / 0 | -91.8 | -91.8 | 0.45 (1) | | 4.61 |
| H- I | -1750 / 0 | -91.8 | -91.8 | 0.46 (1) | | 4.49 |
| R- B | -1799 / 0 | 0.0 | 0.0 | 0.20 (1) | | 6.19 |
| J- I | -1566 / 0 | 0.0 | 0.0 | 0.18 (1) | | 6.55 |
| R- T | 0 / 0 | -18.5 | -18.5 | 0.06 (4) | | 10.00 |
| T- Q | 0 / 0 | -18.5 | -18.5 | 0.06 (4) | | 10.00 |
| Q- U | 0 / 1373 | -18.5 | -18.5 | 0.30 (1) | | 10.00 |
| U- P | 0 / 1373 | -18.5 | -18.5 | 0.30 (1) | | 10.00 |
| P- O | 0 / 2939 | -18.5 | -18.5 | 0.58 (1) | | 10.00 |
| O- N | 0 / 2237 | -18.5 | -18.5 | 0.47 (1) | | 10.00 |
| N- M | 0 / 1347 | -18.5 | -18.5 | 0.32 (1) | | 10.00 |
| M- L | 0 / 1347 | -18.5 | -18.5 | 0.32 (1) | | 10.00 |
| L- K | 0 / 1479 | -18.5 | -18.5 | 0.32 (1) | | 10.00 |
| K- J | 0 / 0 | -18.5 | -18.5 | 0.09 (4) | | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| Q- C | -537 / 0 | 0.10 (1) |
| C- P | 0 / 1890 | 0.47 (1) |
| P- D | -1092 / 0 | 0.21 (1) |
| D- O | -946 / 0 | 0.26 (1) |
| O- E | 0 / 709 | 0.18 (1) |
| E- N | -994 / 0 | 0.78 (1) |
| N- F | 0 / 567 | 0.14 (1) |
| L- G | 0 / 230 | 0.06 (1) |
| L- H | -187 / 0 | 0.15 (1) |
| K- H | -318 / 0 | 0.12 (1) |
| B- Q | 0 / 1530 | 0.38 (1) |
| K- I | 0 / 1534 | 0.38 (1) |
| N- G | 0 / 296 | 0.07 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| C | 2-3-8 | -7 | -7 | --- | FRONT | VERT | DEAD | --- | C1 |
| C | 2-3-8 | -15 | -15 | --- | FRONT | VERT | TOTAL | --- | C1 |
| C | 2-3-8 | -29 | -29 | --- | FRONT | VERT | SNOW | --- | C1 |
| Q | 2-4-4 | -2 | -2 | --- | FRONT | VERT | TOTAL | --- | C1 |
| S | 3-7-4 | -9 | -9 | --- | FRONT | VERT | TOTAL | --- | C1 |
| T | 8-4 | -4 | -4 | --- | FRONT | VERT | TOTAL | --- | C1 |
| U | 3-7-4 | -2 | -2 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 128 lb [M]

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.76/1.00 (F-G:1) , BC=0.58/1.00 (O-P:1) , WB=0.78/1.00 (E-N:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90 )
JSI METAL= 0.41 (B) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T3A | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.


Structural component only
DWG# T-2311446

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T4A | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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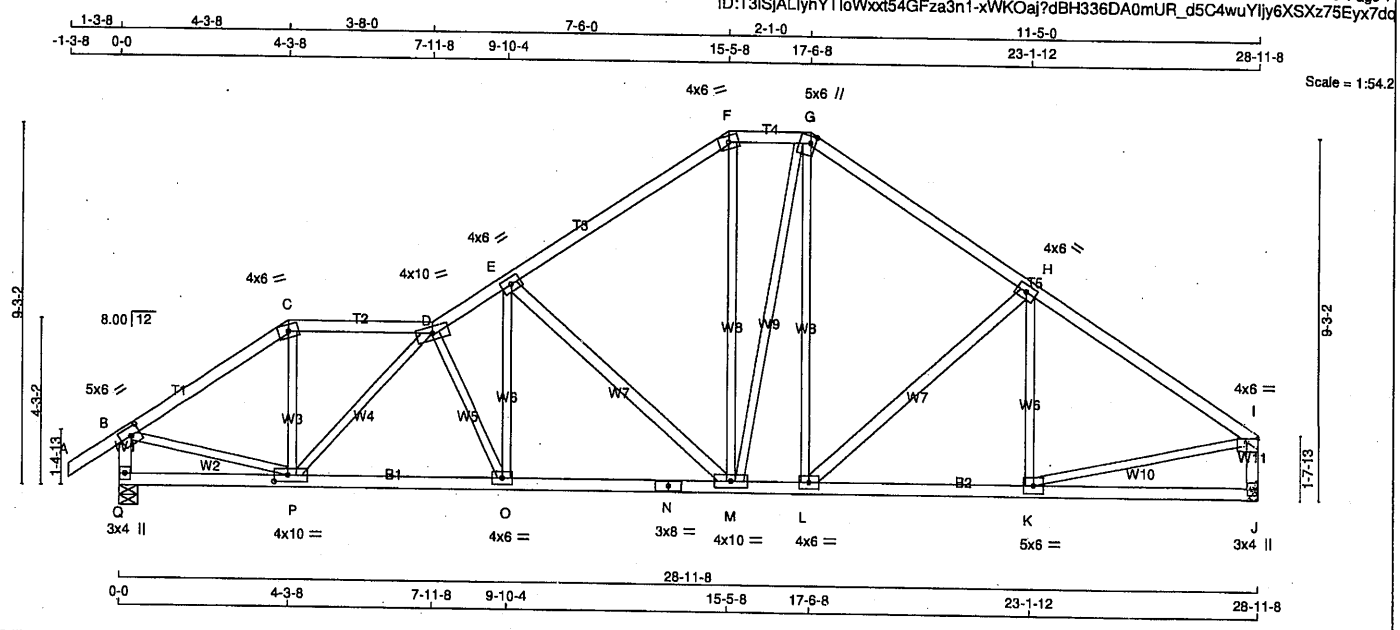


TOTAL WEIGHT = 141 lb [M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - G | 2x4 | DRY | No.2 | SPF |
| G - I | 2x4 | DRY | No.2 | SPF |
| Q - B | 2x4 | DRY | No.2 | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| Q - N | 2x4 | DRY | No.2 | SPF |
| N - J | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| L - H | 2x4 | DRY | No.2 | SPF |
| E - M | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| C | TTW-m | MT20 | 4.0 | 6.0 | | |
| D | TTWW-m | MT20 | 4.0 | 10.0 | | |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TTW-m | MT20 | 4.0 | 6.0 | | |
| G | TTWW+m | MT20 | 5.0 | 6.0 | 2.25 | 1.50 |
| H | TMWW-t | MT20 | 4.0 | 6.0 | | |
| I | TMVW-p | MT20 | 4.0 | 6.0 | 1.00 | 3.25 |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |
| K | BMWW-t | MT20 | 5.0 | 6.0 | | |
| L | BMWW-t | MT20 | 4.0 | 6.0 | | |
| M | BMWWW-t | MT20 | 4.0 | 10.0 | | |
| N | BS-t | MT20 | 3.0 | 8.0 | | |
| O | BMWW-t | MT20 | 4.0 | 6.0 | | |
| P | BMWWW-t | MT20 | 4.0 | 10.0 | 2.00 | 4.00 |
| Q | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| Q | 1722 | 0 | 1722 | 0 | 0 | 5-8 | 2-2 |
| J | 1596 | 0 | 1596 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-12.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| Q | 1216 | 811 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 405 / 0 | 0 / 0 |
| J | 1129 | 741 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 388 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT:
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

CHORDS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | | 10.00 |
| B- C | -1803 / 0 | -91.8 | -91.8 | 0.26 (1) | | 4.74 |
| C- D | -1526 / 0 | -91.8 | -91.8 | 0.18 (1) | | 5.15 |
| D- E | -2378 / 0 | -91.8 | -91.8 | 0.34 (1) | | 4.11 |
| E- F | -1568 / 0 | -91.8 | -91.8 | 0.38 (1) | | 4.86 |
| F- G | -1287 / 0 | -91.8 | -91.8 | 0.07 (1) | | 5.63 |
| G- H | -1519 / 0 | -91.8 | -91.8 | 0.43 (1) | | 4.85 |
| H- I | -1774 / 0 | -91.8 | -91.8 | 0.45 (1) | | 4.53 |
| Q- B | -1688 / 0 | 0.0 | 0.0 | 0.17 (1) | | 6.39 |
| J- I | -1552 / 0 | 0.0 | 0.0 | 0.16 (1) | | 6.61 |
| Q- P | 0 / 0 | -18.5 | -18.5 | 0.09 (4) | | 10.00 |
| P- O | 0 / 2322 | -18.5 | -18.5 | 0.42 (1) | | 10.00 |
| O- N | 0 / 2052 | -18.5 | -18.5 | 0.39 (1) | | 10.00 |
| N- M | 0 / 2052 | -18.5 | -18.5 | 0.39 (1) | | 10.00 |
| M- L | 0 / 1236 | -18.5 | -18.5 | 0.26 (1) | | 10.00 |
| L- K | 0 / 1505 | -18.5 | -18.5 | 0.32 (1) | | 10.00 |
| K- J | 0 / 0 | -18.5 | -18.5 | 0.15 (4) | | 10.00 |

WEBS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| P- C | 0 / 639 | 0.14 (1) |
| M- F | 0 / 553 | 0.12 (1) |
| M- G | 0 / 238 | 0.05 (1) |
| L- G | 0 / 304 | 0.07 (1) |
| L- H | -369 / 0 | 0.32 (1) |
| K- H | -223 / 23 | 0.10 (1) |
| B- P | 0 / 1541 | 0.35 (1) |
| K- I | 0 / 1544 | 0.35 (1) |
| E- M | -1044 / 0 | 0.91 (1) |
| O- E | 0 / 661 | 0.15 (1) |
| D- O | -623 / 0 | 0.20 (1) |
| P- D | -1202 / 0 | 0.58 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.45/1.00 (H-I:1) , BC=0.42/1.00 (O-P:1) , WB=0.91/1.00 (E-M:1) , SSI=0.21/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.90 )
JSI METAL= 0.63 (N) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311447

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T5 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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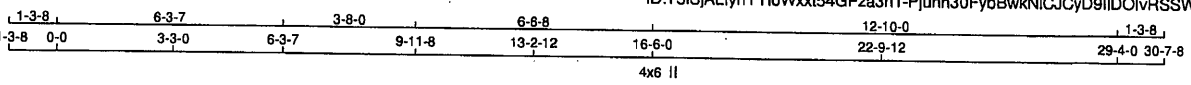


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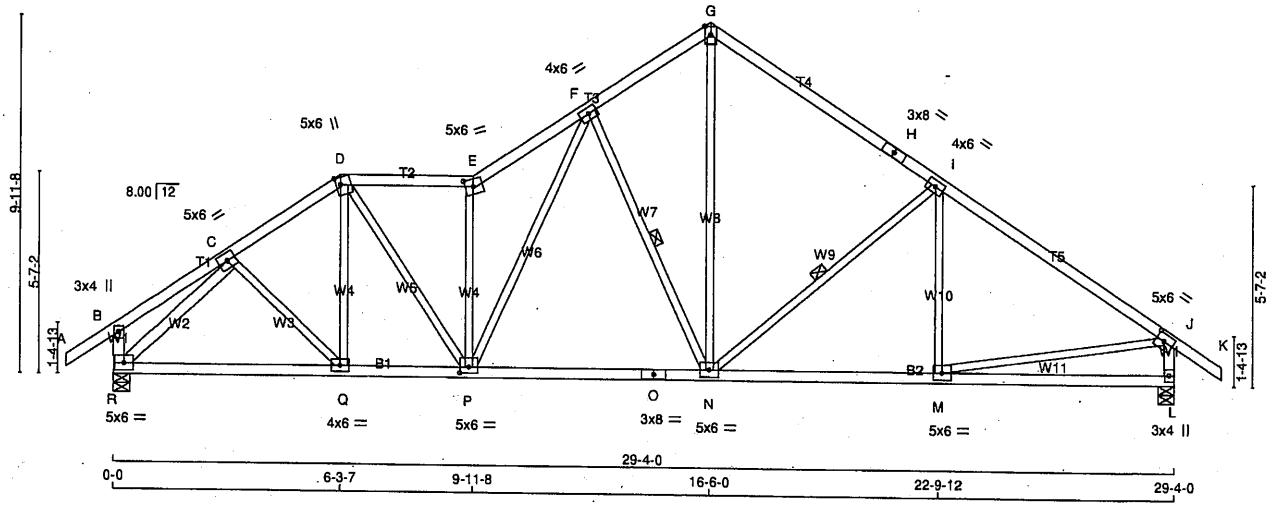


## LUMBER

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - E | 2x4 | DRY | No.2 | SPF |
| E - G | 2x4 | DRY | No.2 | SPF |
| G - H | 2x4 | DRY | No.2 | SPF |
| H - K | 2x4 | DRY | No.2 | SPF |
| R - B | 2x4 | DRY | No.2 | SPF |
| L - J | 2x4 | DRY | No.2 | SPF |
| R - O | 2x4 | DRY | No.2 | SPF |
| O - L | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| R - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

## PLATES (table is in inches)

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | | |
| D | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| E | TTW-m | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| F | TMWW-t | MT20 | 4.0 | 6.0 | | |
| G | TTW+p | MT20 | 4.0 | 6.0 | Edge | |
| H | TS-t | MT20 | 3.0 | 8.0 | | |
| I | TMWW-t | MT20 | 4.0 | 6.0 | | |
| J | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| L | BMV1+p | MT20 | 3.0 | 4.0 | | |
| M | BMWW-t | MT20 | 5.0 | 6.0 | | |
| N | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| O | BS-t | MT20 | 3.0 | 6.0 | | |
| P | BMWWW-t | MT20 | 5.0 | 6.0 | 2.00 | 3.00 |
| Q | BMWW-t | MT20 | 4.0 | 6.0 | | |
| R | BMVW1-t | MT20 | 5.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311448

## DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 3 X 136 = 409 lb [M][F]

### BEARINGS

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| R | 1743 | 0 | 1743 | 0 | 0 | 5-8 | 1-14 |
| L | 1743 | 0 | 1743 | 0 | 0 | 5-8 | 2-3 |

### UNFACTORED REACTIONS

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| R | 1230 | 821 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 410 / 0 | 0 / 0 |
| L | 1230 | 821 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 410 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

### BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

### LOADING

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX. CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | 0 / 16 | -91.8 | -91.8 | 0.13 (1) | 10.00 |
| C- D | -1850 / 0 | -91.8 | -91.8 | 0.14 (1) | 4.83 |
| D- E | -2000 / 0 | -91.8 | -91.8 | 0.20 (1) | 4.61 |
| E- F | -2417 / 0 | -91.8 | -91.8 | 0.19 (1) | 4.27 |
| F- G | -1485 / 0 | -91.8 | -91.8 | 0.15 (1) | 5.24 |
| G- H | -1506 / 0 | -91.8 | -91.8 | 0.54 (1) | 4.69 |
| H- I | -1506 / 0 | -91.8 | -91.8 | 0.54 (1) | 4.69 |
| I- J | -1892 / 0 | -91.8 | -91.8 | 0.60 (1) | 4.23 |
| J- K | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| R- B | -240 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 |
| L- J | -1692 / 0 | 0.0 | 0.0 | 0.17 (1) | 6.39 |
| R- Q | 0 / 1437 | -18.5 | -18.5 | 0.32 (1) | 10.00 |
| Q- P | 0 / 1524 | -18.5 | -18.5 | 0.33 (1) | 10.00 |
| P- O | 0 / 1591 | -18.5 | -18.5 | 0.36 (1) | 10.00 |
| O- N | 0 / 1591 | -18.5 | -18.5 | 0.36 (1) | 10.00 |
| N- M | 0 / 1606 | -18.5 | -18.5 | 0.36 (1) | 10.00 |
| M- L | 0 / 0 | -18.5 | -18.5 | 0.17 (4) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| C- Q | 0 / 120 | 0.03 (4) |
| Q- D | 0 / 64 | 0.02 (4) |
| D- P | 0 / 853 | 0.19 (1) |
| P- E | -1624 / 0 | 0.77 (1) |
| P- F | 0 / 1080 | 0.24 (1) |
| F- N | -906 / 0 | 0.41 (1) |
| N- G | 0 / 1291 | 0.29 (1) |
| N- I | -510 / 0 | 0.24 (1) |
| M- I | -140 / 56 | 0.07 (1) |
| R- C | -2043 / 0 | 0.46 (1) |
| M- J | 0 / 1628 | 0.37 (1) |

### DESIGN CRITERIA

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.60/1.00 (I-J:1) , BC=0.36/1.00 (M-N:1) , WB=0.77/1.00 (E-P:1) , SSI=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (N) (INPUT = 0.90 )
JSI METAL= 0.51 (E) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T6 | 1 | 3 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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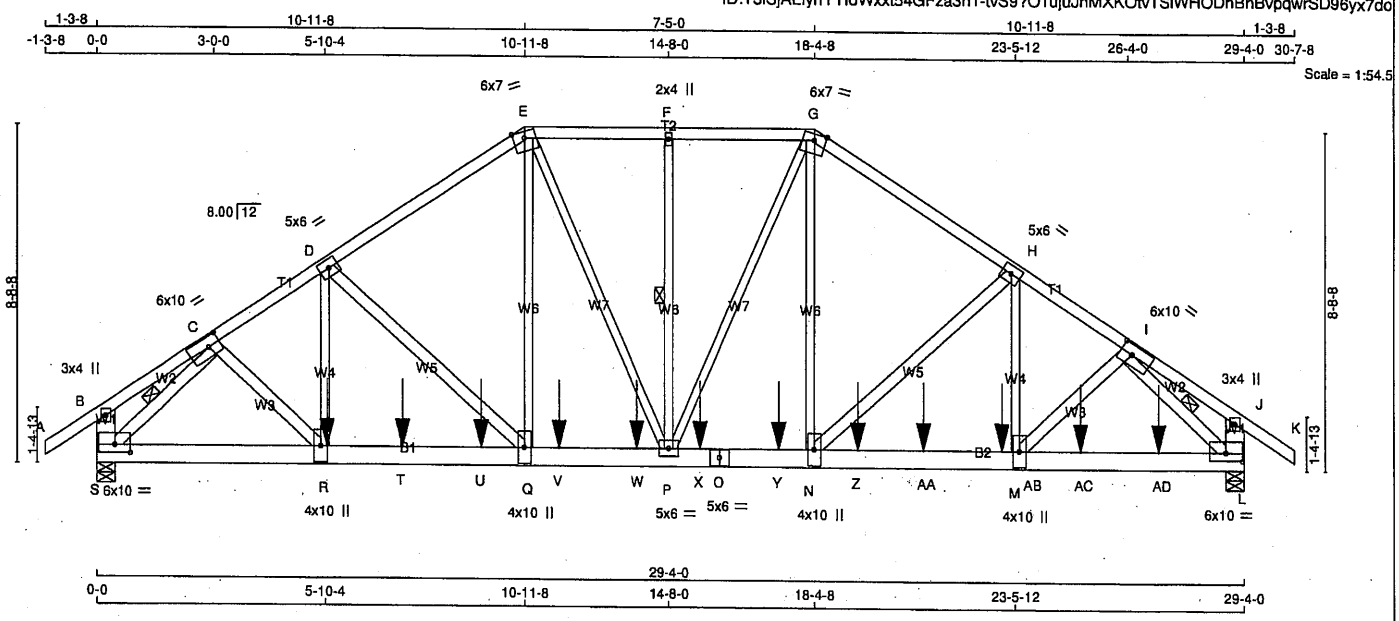


TOTAL WEIGHT = 3 X 171 = 514 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - E | 2x4 | DRY | No.2 | SPF |
| E - G | 2x4 | DRY | No.2 | SPF |
| G - K | 2x4 | DRY | No.2 | SPF |
| S - B | 2x6 | DRY | No.2 | SPF |
| L - J | 2x6 | DRY | No.2 | SPF |
| S - O | 2x6 | DRY | 2100F 1.8E | SPF |
| O - L | 2x6 | DRY | 2100F 1.8E | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| D - Q | 2x4 | DRY | No.2 | SPF |
| N - H | 2x4 | DRY | No.2 | SPF |
| C - R | 2x4 | DRY | No.2 | SPF |
| S - C | 2x4 | DRY | No.2 | SPF |
| M - I | 2x4 | DRY | No.2 | SPF |
| I - L | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A-E | 1 | 12 | TOP |
| E-G | 1 | 12 | TOP |
| G-K | 1 | 12 | TOP |
| S-B | 2 | 12 | TOP |
| L-J | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| S-O | 2 | 8 | SIDE(590.6) |
| O-L | 2 | 8 | SIDE(910.0) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| D-R | 1 | 4 | SIDE(670.8) |
| 2x3 | 1 | 4 | |
| Q-E | 1 | 6 | |
| P-E | 1 | 6 | |
| F-P | 1 | 6 | |
| P-G | 1 | 6 | |
| N-G | 1 | 6 | |
| 2x4 | 1 | 6 | |

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.


Structural component only
DWG# T-2311449

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| L | 13950 | 0 | 13950 | 0 | 0 | 5-8 | 5-1 |
| S | 12357 | 0 | 12357 | 0 | 0 | 5-8 | 4-7 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| L | 9843 | 6587 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 3256 / 0 | 0 / 0 |
| S | 8723 | 5815 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 2908 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P, C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. | FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A-B | 0 / 35 | -91.8 | -91.8 | 0.04 (1) | 10.00 | |
| B-C | -22 / 0 | -91.8 | -91.8 | 0.07 (1) | 6.25 | |
| C-D | -17175 / 0 | -91.8 | -91.8 | 0.46 (1) | 2.64 | |
| D-E | -14791 / 0 | -91.8 | -91.8 | 0.59 (1) | 2.92 | |
| E-F | -13119 / 0 | -91.8 | -91.8 | 0.28 (1) | 3.21 | |
| F-G | -13119 / 0 | -91.8 | -91.8 | 0.28 (1) | 3.21 | |
| G-H | -14811 / 0 | -91.8 | -91.8 | 0.60 (1) | 2.91 | |
| H-I | -17646 / 0 | -91.8 | -91.8 | 0.48 (1) | 2.58 | |
| I-J | -23 / 0 | -91.8 | -91.8 | 0.07 (1) | 6.25 | |
| J-K | 0 / 35 | -91.8 | -91.8 | 0.04 (1) | 10.00 | |
| S-B | -297 / 0 | 0.0 | 0.0 | 0.01 (1) | 7.81 | |
| L-J | -300 / 0 | 0.0 | 0.0 | 0.01 (1) | 7.81 | |
| S-R | 0 / 11904 | -18.5 | -18.5 | 0.23 (1) | 10.00 | |
| R-T | 0 / 14333 | -18.5 | -18.5 | 0.47 (1) | 10.00 | |
| T-U | 0 / 14333 | -18.5 | -18.5 | 0.47 (1) | 10.00 | |
| U-Q | 0 / 14333 | -18.5 | -18.5 | 0.47 (1) | 10.00 | |
| Q-V | 0 / 12352 | -18.5 | -18.5 | 0.40 (1) | 10.00 | |
| V-W | 0 / 12352 | -18.5 | -18.5 | 0.40 (1) | 10.00 | |
| W-P | 0 / 12352 | -18.5 | -18.5 | 0.40 (1) | 10.00 | |
| P-X | 0 / 12370 | -18.5 | -18.5 | 0.37 (1) | 10.00 | |
| X-O | 0 / 12370 | -18.5 | -18.5 | 0.37 (1) | 10.00 | |
| O-Y | 0 / 12370 | -18.5 | -18.5 | 0.37 (1) | 10.00 | |
| Y-N | 0 / 12370 | -18.5 | -18.5 | 0.37 (1) | 10.00 | |
| N-Z | 0 / 14726 | -18.5 | -18.5 | 0.55 (1) | 10.00 | |
| Z-AA | 0 / 14726 | -18.5 | -18.5 | 0.55 (1) | 10.00 | |
| AA-AB | 0 / 14726 | -18.5 | -18.5 | 0.55 (1) | 10.00 | |
| AB-M | 0 / 14726 | -18.5 | -18.5 | 0.55 (1) | 10.00 | |
| M-AC | 0 / 12226 | -18.5 | -18.5 | 0.52 (1) | 10.00 | |
| AC-AD | 0 / 12226 | -18.5 | -18.5 | 0.52 (1) | 10.00 | |
| AD-L | 0 / 12226 | -18.5 | -18.5 | 0.52 (1) | 10.00 | |

| WEBS MEMB. | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| R-D | 0 / 2778 | 0.21 (1) |
| D-Q | -2824 / 0 | 0.65 (1) |
| Q-E | 0 / 6096 | 0.46 (1) |
| E-P | 0 / 1883 | 0.14 (1) |
| P-F | -279 / 0 | 0.04 (1) |
| P-G | 0 / 1839 | 0.14 (1) |
| N-G | 0 / 6148 | 0.46 (1) |
| N-H | -3340 / 0 | 0.76 (1) |
| M-H | 0 / 3379 | 0.25 (1) |
| C-R | 0 / 3395 | 0.18 (1) |
| S-C | -16949 / 0 | 0.68 (1) |
| M-I | 0 / 3495 | 0.19 (1) |
| I-L | -17408 / 0 | 0.69 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| R | 5-11-0 | -2699 | -2699 | --- | BACK | VERT | TOTAL | --- | C1 |
| T | 7-10-4 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| U | 9-10-4 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.60/1.00 (G-H:1) , BC=0.55/1.00 (M-N:1) , WB=0.76/1.00 (H-N:1) , SSI=0.67/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90 )
JSI METAL= 0.84 (I) (INPUT = 0.95 )



CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T6 | 1 | 3 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



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GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 6.0 | 10.0 | 3.00 | 3.75 |
| D | TMWW-t | MT20 | 5.0 | 6.0 | | |
| E | TTWW-m | MT20 | 6.0 | 7.0 | Edge | |
| F | TMW+w | MT20 | 2.0 | 4.0 | | |
| G | TTWW-m | MT20 | 6.0 | 7.0 | Edge | |
| H | TMWW-t | MT20 | 5.0 | 6.0 | | |
| I | TMWW-t | MT20 | 6.0 | 10.0 | 3.00 | 3.75 |
| J | TMV+p | MT20 | 3.0 | 4.0 | | |
| L | BMVW1-t | MT20 | 6.0 | 10.0 | 2.50 | 5.00 |
| M, N, Q, R | | | | | | |
| M | BMWW+t | MT20 | 4.0 | 10.0 | | |
| O | BS-t | MT20 | 5.0 | 6.0 | | |
| P | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| S | BMVW1-t | MT20 | 6.0 | 10.0 | 2.50 | 5.00 |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| V | 11-10-4 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| W | 13-10-4 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| X | 15-5-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| Y | 17-5-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| Z | 19-5-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| AA | 21-1-12 | -1274 | -1274 | --- | BACK | VERT | TOTAL | --- | C1 |
| AB | 23-1-12 | -1274 | -1274 | --- | BACK | VERT | TOTAL | --- | C1 |
| AC | 25-1-12 | -1274 | -1274 | --- | BACK | VERT | TOTAL | --- | C1 |
| AD | 27-1-12 | -1274 | -1274 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311449

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T7 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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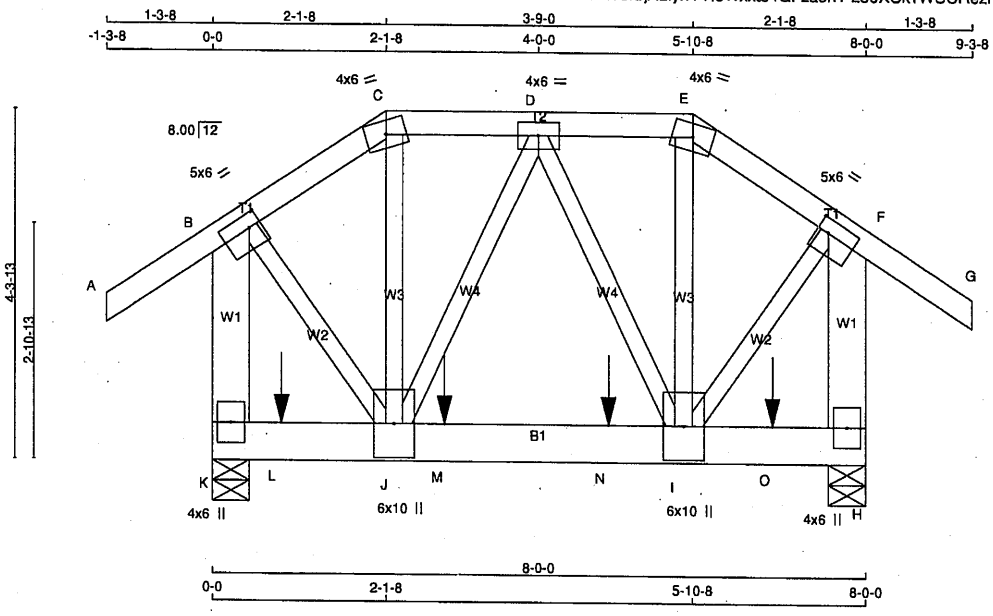


TOTAL WEIGHT = 2 X 54 = 108 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| E - G | 2x4 | DRY | No.2 | SPF |
| K - B | 2x6 | DRY | No.2 | SPF |
| H - F | 2x6 | DRY | No.2 | SPF |
| K - H | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- E | 1 | 12 | TOP |
| E- G | 1 | 12 | TOP |
| K- B | 2 | 12 | TOP |
| H- F | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| K- H | 2 | 12 | SIDE(10.5) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.


Structural component only
DWG# T-2311450

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| K | 4700 | 0 | 4700 | 0 | 0 | 5-8 | 2-9 |
| H | 3725 | 0 | 3725 | 0 | 0 | 5-8 | 2-0 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| K | 3326 | 2168 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 1159 / 0 | 0 / 0 |
| H | 2635 | 1721 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 914 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| FR-TO | | | | | | FR-TO | | |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 | J- C | 0 / 1005 | 0.12 (1) |
| B- C | -2154 / 0 | -91.8 | -91.8 | 0.05 (1) | 6.01 | I- E | 0 / 938 | 0.12 (1) |
| C- D | -1813 / 0 | -91.8 | -91.8 | 0.04 (1) | 6.25 | B- J | 0 / 2682 | 0.33 (1) |
| D- E | -1710 / 0 | -91.8 | -91.8 | 0.04 (1) | 6.25 | I- F | 0 / 2530 | 0.31 (1) |
| E- F | -2032 / 0 | -91.8 | -91.8 | 0.05 (1) | 6.14 | J- D | 0 / 47 | 0.01 (4) |
| F- G | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 | D- I | -204 / 0 | 0.03 (1) |
| K- B | -3427 / 0 | 0.0 | 0.0 | 0.18 (1) | 7.54 | | | |
| H- F | -3246 / 0 | 0.0 | 0.0 | 0.17 (1) | 7.69 | | | |
| K- L | 0 / 0 | -37.3 | -37.3 | 0.39 (1) | 10.00 | | | |
| L- J | 0 / 0 | -37.3 | -37.3 | 0.39 (1) | 10.00 | | | |
| J- M | 0 / 1793 | -37.3 | -37.3 | 0.42 (1) | 10.00 | | | |
| M- N | 0 / 1793 | -37.3 | -37.3 | 0.42 (1) | 10.00 | | | |
| N- I | 0 / 1793 | -37.3 | -37.3 | 0.42 (1) | 10.00 | | | |
| I- O | 0 / 0 | -37.3 | -37.3 | 0.27 (1) | 10.00 | | | |
| O- H | 0 / 0 | -37.3 | -37.3 | 0.27 (1) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| L | 10-4 | -1839 | -1839 | --- | BACK | VERT | TOTAL | --- | C1 |
| M | 2-10-4 | -971 | -971 | --- | BACK | VERT | TOTAL | --- | C1 |
| N | 4-10-4 | -1120 | -1120 | --- | BACK | VERT | TOTAL | --- | C1 |
| O | 6-10-4 | -1114 | -1114 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.18/1.00 (B-K:1) , BC=0.42/1.00 (I-J:1) , WB=0.33/1.00 (B-J:1) , SSI=0.54/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (I) (INPUT = 0.90 )
JSI METAL= 0.33 (B) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T7 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



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**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.00 | 1.75 |
| C | TTW-m | MT20 | 4.0 | 6.0 | | |
| D | TMWW-t | MT20 | 4.0 | 6.0 | | |
| E | TTW-m | MT20 | 4.0 | 6.0 | | |
| F | TMVW-t | MT20 | 5.0 | 6.0 | 2.00 | 1.75 |
| H | BMV1+p | MT20 | 4.0 | 6.0 | | |
| I | BMWWW+t | MT20 | 6.0 | 10.0 | | |
| J | BMWWW+t | MT20 | 6.0 | 10.0 | | |
| K | BMV1+p | MT20 | 4.0 | 6.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311450

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T8 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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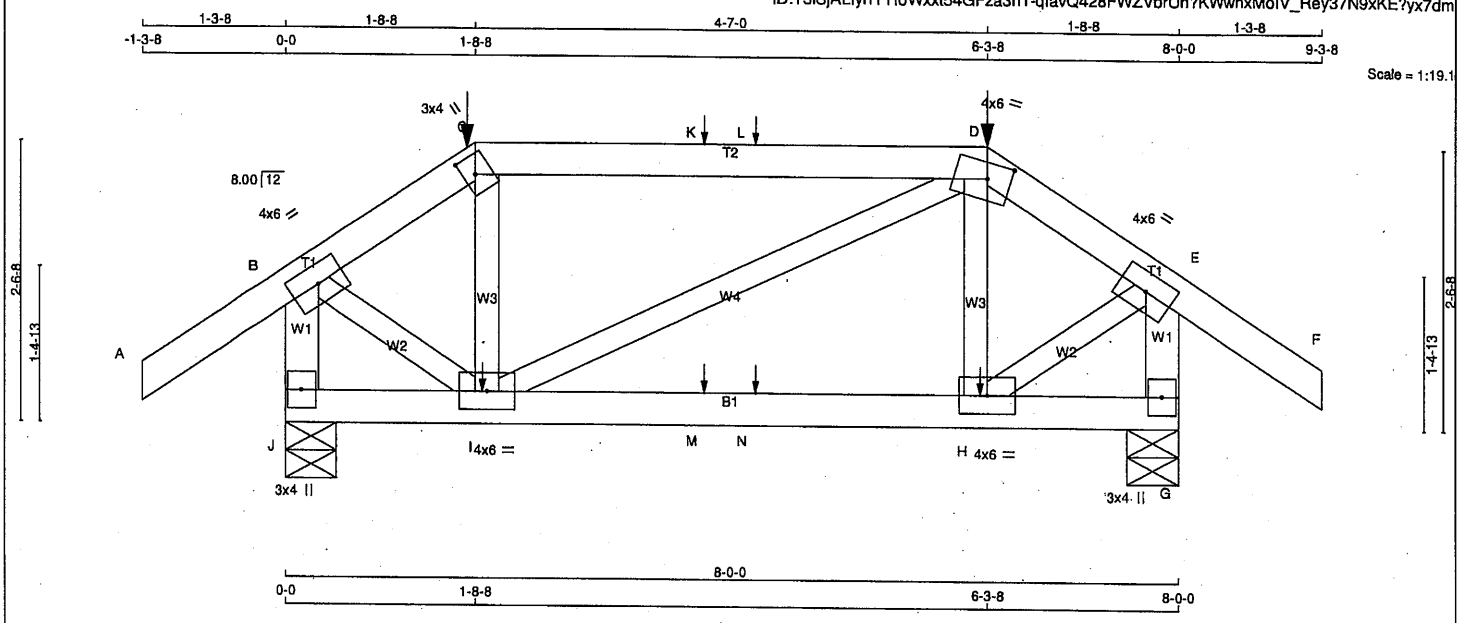


TOTAL WEIGHT = 35 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| J - B | 2x4 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |
| J - G | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TTW+h | MT20 | 3.0 | 4.0 | 2.00 | 1.25 |
| D | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.50 |
| E | TMVW-t | MT20 | 4.0 | 6.0 | | |
| G | BMV1+p | MT20 | 3.0 | 4.0 | | |
| H | BMWW-t | MT20 | 4.0 | 6.0 | | |
| I | BMWWW-t | MT20 | 4.0 | 6.0 | | |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311451

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| J | 596 | 0 | 596 | 0 | 0 | 5-8 | 1-8 |
| G | 596 | 0 | 596 | 0 | 0 | 5-8 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| J | 419 | 291 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 128 / 0 | 0 / 0 |
| G | 419 | 291 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 128 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (7)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 1 | 0.14 (1) | 10.00 | I- C | -89 / 24 | 0.02 (1) |
| B- C | -347 / 0 | -91.8 | -91.8 | 1 | 0.13 (1) | 6.25 | I- D | 0 / 0 | 0.00 (1) |
| C- K | -260 / 0 | -91.8 | -91.8 | 1 | 0.37 (1) | 6.25 | H- D | -89 / 24 | 0.02 (1) |
| K- L | -260 / 0 | -91.8 | -91.8 | 1 | 0.37 (1) | 6.25 | B- I | 0 / 307 | 0.08 (1) |
| L- D | -260 / 0 | -91.8 | -91.8 | 1 | 0.37 (1) | 6.25 | H- E | 0 / 308 | 0.08 (1) |
| D- E | -347 / 0 | -91.8 | -91.8 | 1 | 0.13 (1) | 6.25 | | | |
| E- F | 0 / 35 | -91.8 | -91.8 | 1 | 0.14 (1) | 10.00 | | | |
| J- B | -590 / 0 | 0.0 | 0.0 | 1 | 0.07 (1) | 7.81 | | | |
| G- E | -591 / 0 | 0.0 | 0.0 | 1 | 0.07 (1) | 7.81 | | | |
| J- I | 0 / 0 | -18.5 | -18.5 | 1 | 0.06 (4) | 10.00 | | | |
| I- M | 0 / 260 | -18.5 | -18.5 | 1 | 0.09 (4) | 10.00 | | | |
| M- N | 0 / 260 | -18.5 | -18.5 | 1 | 0.09 (4) | 10.00 | | | |
| N- H | 0 / 260 | -18.5 | -18.5 | 1 | 0.09 (4) | 10.00 | | | |
| H- G | 0 / 0 | -18.5 | -18.5 | 1 | 0.06 (4) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR: | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| C | 1-8-8 | -4 | -4 | --- | FRONT | VERT | DEAD | --- | C1 |
| C | 1-8-8 | 1 | 1 | 79 | FRONT | VERT | TOTAL | --- | C1 |
| C | 1-8-8 | -16 | -16 | --- | FRONT | VERT | SNOW | --- | C1 |
| D | 6-3-8 | -4 | -4 | --- | FRONT | VERT | DEAD | --- | C1 |
| D | 6-3-8 | 1 | 1 | 79 | FRONT | VERT | TOTAL | --- | C1 |
| D | 6-3-8 | -16 | -16 | --- | FRONT | VERT | SNOW | --- | C1 |
| H | 6-2-12 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| I | 1-9-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| K | 3-9-4 | 1 | 1 | 79 | FRONT | VERT | TOTAL | --- | C1 |
| L | 4-2-12 | 1 | 1 | 79 | FRONT | VERT | TOTAL | --- | C1 |
| M | 3-9-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| N | 4-2-12 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.09/1.00 (H-I:4) , WB=0.08/1.00 (E-H:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T8 | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



ID:T3iSjALIyhYTIoWxxt54GFza3n1-qlavQ428FWZVbrUn?KWwnxMoIV_Rey37N9xKE?yx7dm

JSI GRIP= 0.48 (C) (INPUT = 0.90 )
JSI METAL= 0.10 (E) (INPUT = 0.95 )


Structural component only
DWG# T-2311451

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T9 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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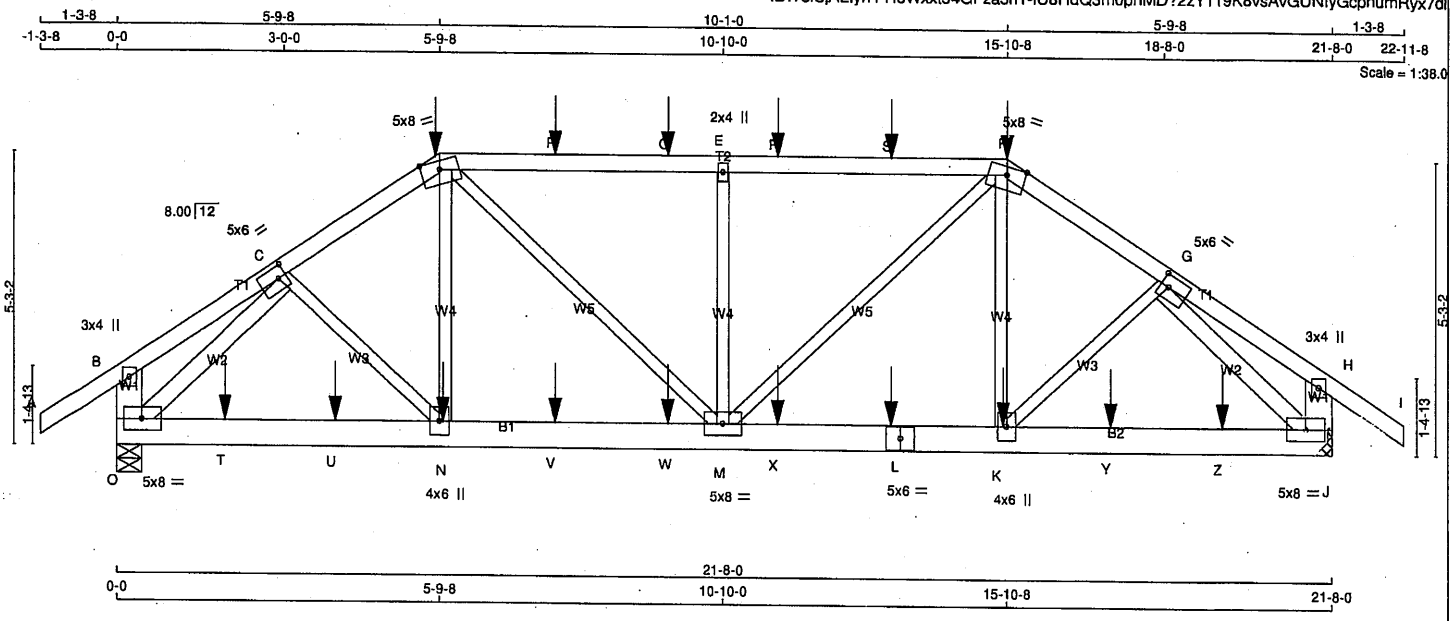


TOTAL WEIGHT = 109 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | 1650F 1.5E | SPF |
| F - I | 2x4 | DRY | No.2 | SPF |
| O - B | 2x6 | DRY | No.2 | SPF |
| J - H | 2x6 | DRY | No.2 | SPF |
| O - L | 2x6 | DRY | No.2 | SPF |
| L - J | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| O - C | 2x4 | DRY | No.2 | SPF |
| G - J | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| D | TTWW-m | MT20 | 5.0 | 8.0 | Edge | |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TTWW-m | MT20 | 5.0 | 8.0 | Edge | |
| G | TMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| H | TMV+p | MT20 | 3.0 | 4.0 | | |
| J | BMVW1-t | MT20 | 5.0 | 8.0 | | |
| K | BMWW+t | MT20 | 4.0 | 6.0 | | |
| L | BS-t | MT20 | 5.0 | 6.0 | | |
| M | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| N | BMWW+t | MT20 | 4.0 | 6.0 | | |
| O | BMVW1-t | MT20 | 5.0 | 8.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311452

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| O | 2317 | 0 | 2317 | 0 | 0 | 5-8 | 2-8 |
| J | 2317 | 0 | 2317 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| O | 1638 | 1079 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 559 / 0 | 0 / 0 |
| J | 1638 | 1079 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 559 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 1 | 0.14 (1) | 10.00 |
| B- C | 0 / 13 | -91.8 | -91.8 | 1 | 0.10 (1) | 10.00 |
| C- D | -2669 / 0 | -91.8 | -91.8 | 1 | 0.21 (1) | 4.04 |
| D- P | -2830 / 0 | -91.8 | -91.8 | 1 | 0.81 (1) | 3.45 |
| P- Q | -2830 / 0 | -91.8 | -91.8 | 1 | 0.81 (1) | 3.45 |
| Q- E | -2830 / 0 | -91.8 | -91.8 | 1 | 0.81 (1) | 3.45 |
| E- R | -2830 / 0 | -91.8 | -91.8 | 1 | 0.81 (1) | 3.45 |
| R- S | -2830 / 0 | -91.8 | -91.8 | 1 | 0.81 (1) | 3.45 |
| S- F | -2830 / 0 | -91.8 | -91.8 | 1 | 0.81 (1) | 3.45 |
| F- G | -2670 / 0 | -91.8 | -91.8 | 1 | 0.21 (1) | 4.04 |
| G- H | 0 / 13 | -91.8 | -91.8 | 1 | 0.10 (1) | 10.00 |
| H- I | 0 / 35 | -91.8 | -91.8 | 1 | 0.14 (1) | 10.00 |
| O- B | -235 / 0 | 0.0 | 0.0 | 1 | 0.02 (1) | 7.81 |
| J- H | -235 / 0 | 0.0 | 0.0 | 1 | 0.02 (1) | 7.81 |
| O- T | 0 / 1984 | -18.5 | -18.5 | 1 | 0.34 (1) | 10.00 |
| T- U | 0 / 1984 | -18.5 | -18.5 | 1 | 0.34 (1) | 10.00 |
| U- N | 0 / 1984 | -18.5 | -18.5 | 1 | 0.34 (1) | 10.00 |
| N- V | 0 / 2206 | -18.5 | -18.5 | 1 | 0.36 (1) | 10.00 |
| V- W | 0 / 2206 | -18.5 | -18.5 | 1 | 0.36 (1) | 10.00 |
| W- M | 0 / 2206 | -18.5 | -18.5 | 1 | 0.36 (1) | 10.00 |
| M- X | 0 / 2206 | -18.5 | -18.5 | 1 | 0.36 (1) | 10.00 |
| X- L | 0 / 2206 | -18.5 | -18.5 | 1 | 0.36 (1) | 10.00 |
| L- K | 0 / 2206 | -18.5 | -18.5 | 1 | 0.36 (1) | 10.00 |
| K- Y | 0 / 1985 | -18.5 | -18.5 | 1 | 0.34 (1) | 10.00 |
| Y- Z | 0 / 1985 | -18.5 | -18.5 | 1 | 0.34 (1) | 10.00 |
| Z- J | 0 / 1985 | -18.5 | -18.5 | 1 | 0.34 (1) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| C- N | 0 / 307 | 0.08 (1) |
| N- D | 0 / 160 | 0.06 (4) |
| K- F | 0 / 161 | 0.06 (4) |
| K- G | 0 / 308 | 0.08 (1) |
| O- C | -2823 / 0 | 0.55 (1) |
| G- J | -2824 / 0 | 0.55 (1) |
| M- F | 0 / 869 | 0.22 (1) |
| D- M | 0 / 870 | 0.22 (1) |
| M- E | -1081 / 0 | 0.43 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| D | 5-9-8 | -327 | -327 | --- | FRONT | VERT | TOTAL | --- | C1 |
| F | 15-10-8 | -327 | -327 | --- | FRONT | VERT | TOTAL | --- | C1 |
| K | 15-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| L | 13-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| N | 5-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| P | 7-10-4 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Q | 9-10-4 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| R | 11-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.81/1.00 (D-E:1) , BC=0.36/1.00 (M-N:1) , WB=0.55/1.00 (G-J:1) , SSI=0.47/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90 )
JSI METAL= 0.64 (C) (INPUT = 0.95 )



CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T9 | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| S | 13-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| T | 1-11-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| U | 3-11-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| V | 7-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| W | 9-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| X | 11-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Y | 17-8-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Z | 19-8-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311452

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T10 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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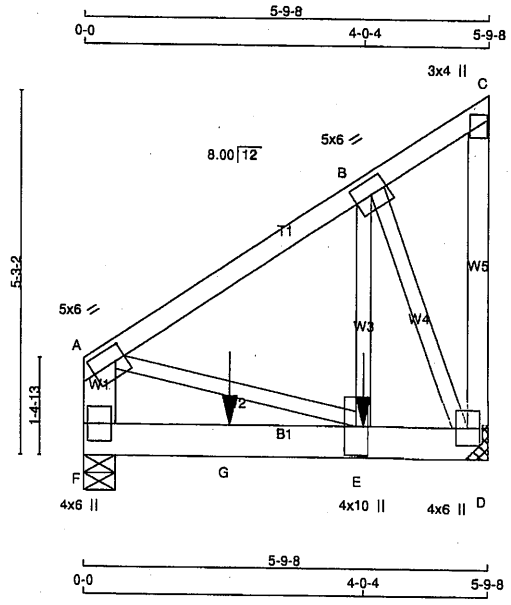


Scale = 1:30.6

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| D - C | 2x4 | DRY | No.2 | SPF |
| F - A | 2x6 | DRY | No.2 | SPF |
| F - D | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- D | 1 | 12 | TOP |
| F- A | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| F- D | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| B- E | 1 | 4 | SIDE(416.6) |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

**PLATES** (table is in inches)

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| B | TMWW-t | MT20 | 5.0 | 6.0 | | |


Structural component only
DWG# T-2311453

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| D | 2025 | 0 | 2025 | 0 | 0 | IN-SX | IN-SX |
| F | 1215 | 0 | 1215 | 0 | 0 | MECHANICAL | |
| | | | | | | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| D | 1428 | 958 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 471 / 0 | 0 / 0 |
| F | 855 | 584 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 270 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | -1105 / 0 | -91.8 | -91.8 | 0.11 (1) | | 6.25 | E- B | 0 / 2202 | 0.27 (1) |
| B- C | -34 / 0 | -91.8 | -91.8 | 0.08 (1) | | 6.25 | B- D | -2149 / 0 | 0.30 (1) |
| D- C | -19 / 0 | 0.0 | 0.0 | 0.00 (1) | | 7.81 | A- E | 0 / 960 | 0.12 (1) |
| F- A | -1010 / 0 | 0.0 | 0.0 | 0.04 (1) | | 7.81 | | | |
| F- G | 0 / 0 | -18.5 | -18.5 | 0.14 (1) | | 10.00 | | | |
| G- E | 0 / 0 | -18.5 | -18.5 | 0.14 (1) | | 10.00 | | | |
| E- D | 0 / 932 | -18.5 | -18.5 | 0.10 (1) | | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| E | 4-0-4 | -1623 | -1623 | --- | BACK | VERT | TOTAL | --- | C1 |
| G | 2-1-4 | -208 | -208 | --- | TOP | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 34 = 68 lb [M]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT. CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.14/1.00 (E-F:1) , WB=0.30/1.00 (B-D:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90 )
JSI METAL= 0.37 (E) (INPUT = 0.95 )



CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T10 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



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**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| C | TMV+p | MT20 | 3.0 | 4.0 | | |
| D | BMVW1+p | MT20 | 4.0 | 6.0 | | |
| E | BMWW+t | MT20 | 4.0 | 10.0 | | |
| F | BMV1+p | MT20 | 4.0 | 6.0 | | |

**NOTES-** (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311453

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T10Z | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 16:43:58 2023 Page 1
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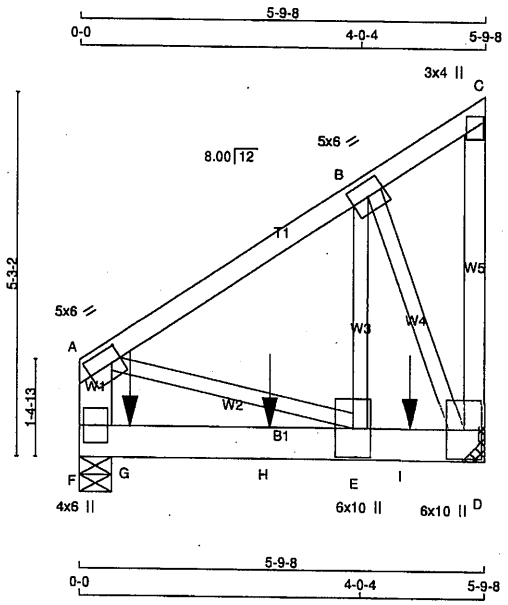


Scale = 1:30.6

TOTAL WEIGHT = 2 X 34 = 68 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| D - C | 2x4 | DRY | No.2 | SPF |
| F - A | 2x6 | DRY | No.2 | SPF |
| F - D | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- D | 1 | 12 | TOP |
| F- A | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| F- D | 2 | 12 | SIDE(0.0) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| B | TMWW-t | MT20 | 5.0 | 6.0 | | |
| C | TMV+p | MT20 | 3.0 | 4.0 | | |


Structural component only
DWG# T-2311468

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| D | 2095 | 0 | 2095 | 0 | 0 | MECHANICAL | |
| F | 2314 | 0 | 2314 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| D | 1476 | 1001 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 475 / 0 | 0 / 0 |
| F | 1630 | 1105 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 525 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | -1151 / 0 | -91.8 | -91.8 | 0.11 (1) | 6.25 | E- B | 0 / 2308 | 0.29 (1) |
| B- C | -34 / 0 | -91.8 | -91.8 | 0.08 (1) | 6.25 | B- D | -2237 / 0 | 0.32 (1) |
| D- C | -20 / 0 | 0.0 | 0.0 | 0.00 (1) | 7.81 | A- E | 0 / 1000 | 0.12 (1) |
| F- A | -1045 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | | | |
| F- G | 0 / 0 | -18.5 | -18.5 | 0.34 (1) | 10.00 | | | |
| G- H | 0 / 0 | -18.5 | -18.5 | 0.34 (1) | 10.00 | | | |
| H- E | 0 / 0 | -18.5 | -18.5 | 0.34 (1) | 10.00 | | | |
| E- I | 0 / 970 | -18.5 | -18.5 | 0.36 (1) | 10.00 | | | |
| I- D | 0 / 970 | -18.5 | -18.5 | 0.36 (1) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| G | 8-12 | -886 | -886 | --- | FRONT | VERT | TOTAL | --- | C1 |
| H | 2-8-12 | -884 | -884 | --- | FRONT | VERT | TOTAL | --- | C1 |
| I | 4-8-12 | -884 | -884 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.36/1.00 (D-E:1) , WB=0.32/1.00 (B-D:1) , SSI=0.44/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90 )
JSI METAL= 0.23 (B) (INPUT = 0.95 )



CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T10Z | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| D | BMVW1+p | MT20 | 6.0 | 10.0 | | |
| E | BMWW+t | MT20 | 6.0 | 10.0 | | |
| F | BMV1+p | MT20 | 4.0 | 6.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311468

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T11 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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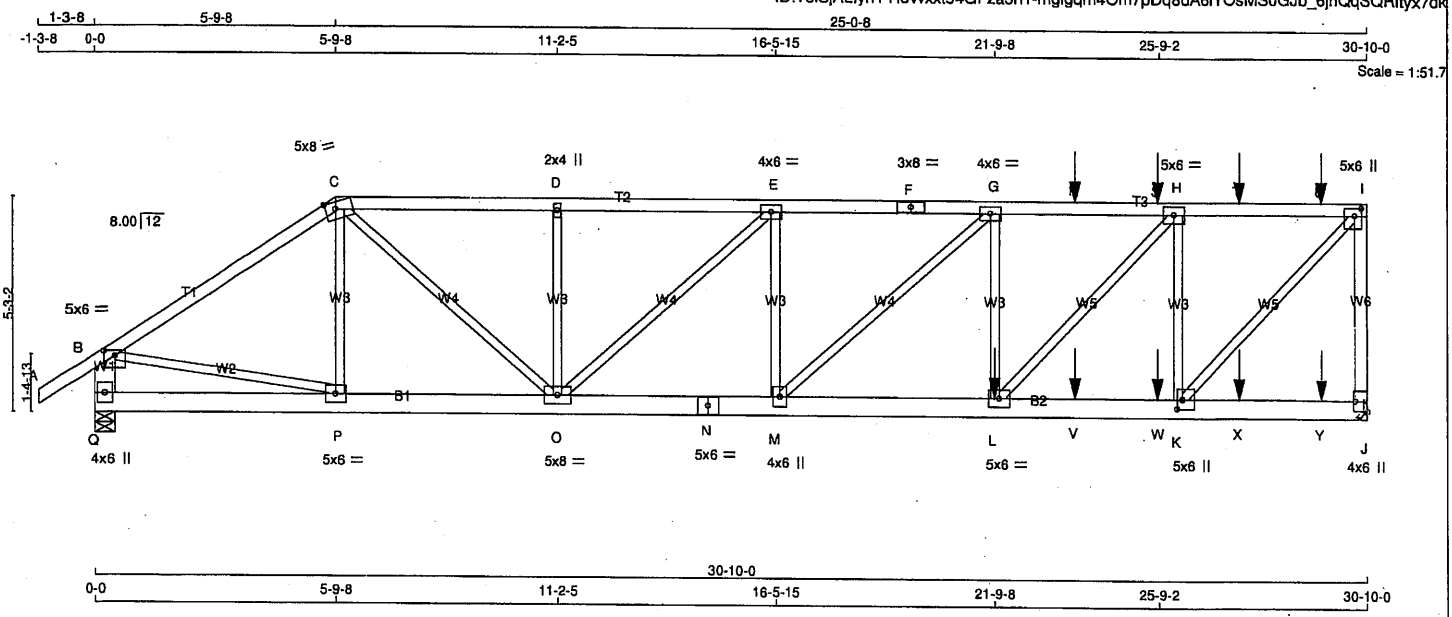





TOTAL WEIGHT = 2 X 150 = 300 lb

[M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - F | 2x4 | DRY | No.2 | SPF |
| F - I | 2x4 | DRY | No.2 | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| Q - B | 2x6 | DRY | No.2 | SPF |
| Q - N | 2x6 | DRY | No.2 | SPF |
| N - J | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- F | 1 | 12 | TOP |
| F- I | 1 | 12 | SIDE(0.0) |
| I- J | 1 | 12 | TOP |
| Q- B | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| Q- N | 2 | 12 | TOP |
| N- J | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| G- L | 1 | 6 | SIDE(340.4) |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.


Structural component only
DWG# T-2311454

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| J | 3843 | 0 | 3843 | 0 | 0 | MECHANICAL | |
| Q | 2525 | 0 | 2525 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-12.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| J | 2714 | 1802 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 912 / 0 | 0 / 0 |
| Q | 1782 | 1192 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 590 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 | |
| B- C | -2997 / 0 | -91.8 | -91.8 | 0.39 (1) | 4.93 | |
| C- D | -4126 / 0 | -91.8 | -91.8 | 0.30 (1) | 4.43 | |
| D- E | -4126 / 0 | -91.8 | -91.8 | 0.30 (1) | 4.43 | |
| E- F | -5071 / 0 | -91.8 | -91.8 | 0.29 (1) | 4.09 | |
| F- G | -5071 / 0 | -91.8 | -91.8 | 0.29 (1) | 4.09 | |
| G- R | -5352 / 0 | -91.8 | -91.8 | 0.44 (1) | 3.78 | |
| R- S | -5352 / 0 | -91.8 | -91.8 | 0.44 (1) | 3.78 | |
| S- H | -5352 / 0 | -91.8 | -91.8 | 0.44 (1) | 3.78 | |
| H- T | -3261 / 0 | -91.8 | -91.8 | 0.35 (1) | 4.76 | |
| T- U | -3261 / 0 | -91.8 | -91.8 | 0.35 (1) | 4.76 | |
| U- I | -3261 / 0 | -91.8 | -91.8 | 0.35 (1) | 4.76 | |
| J- I | -3772 / 0 | 0.0 | 0.0 | 0.85 (1) | 8.07 | |
| Q- B | -2479 / 0 | 0.0 | 0.0 | 0.09 (1) | 7.81 | |
| Q- P | 0 / 0 | -18.5 | -18.5 | 0.04 (4) | 10.00 | |
| P- O | 0 / 2484 | -18.5 | -18.5 | 0.19 (1) | 10.00 | |
| O- N | 0 / 5071 | -18.5 | -18.5 | 0.36 (1) | 10.00 | |
| N- M | 0 / 5071 | -18.5 | -18.5 | 0.36 (1) | 10.00 | |
| M- L | 0 / 5352 | -18.5 | -18.5 | 0.40 (1) | 10.00 | |
| L- V | 0 / 3261 | -18.5 | -18.5 | 0.27 (1) | 10.00 | |
| V- W | 0 / 3261 | -18.5 | -18.5 | 0.27 (1) | 10.00 | |
| W- K | 0 / 3261 | -18.5 | -18.5 | 0.27 (1) | 10.00 | |
| K- X | 0 / 0 | -18.5 | -18.5 | 0.04 (4) | 10.00 | |
| X- Y | 0 / 0 | -18.5 | -18.5 | 0.04 (4) | 10.00 | |
| Y- J | 0 / 0 | -18.5 | -18.5 | 0.04 (4) | 10.00 | |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| P- C | -292 / 0 | 0.06 (1) |
| K- H | -3239 / 0 | 0.64 (1) |
| B- P | 0 / 2526 | 0.31 (1) |
| L- G | -258 / 0 | 0.05 (1) |
| L- H | 0 / 3114 | 0.39 (1) |
| K- I | 0 / 4750 | 0.59 (1) |
| M- G | -382 / 0 | 0.19 (1) |
| C- O | 0 / 2211 | 0.27 (1) |
| M- E | 0 / 396 | 0.05 (1) |
| O- D | -539 / 0 | 0.11 (1) |
| O- E | -1285 / 0 | 0.63 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| L | 21-9-8 | -1414 | -1414 | --- | FRONT | VERT | TOTAL | --- | C1 |
| R | 23-8-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| S | 25-8-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| T | 27-8-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| U | 29-8-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| V | 23-8-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| W | 25-8-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| X | 27-8-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Y | 29-8-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.85/1.00 (I-J:1) , BC=0.40/1.00 (L-M:1) , WB=0.64/1.00 (H-K:1) , SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.90 )
JSI METAL= 0.57 (I) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T11 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



ID:T3iSjALIyhYTIoWxxt54GFza3n1-mgiggm4Om7pDq8dA6lYOsMS0GJb_6jhQqSQRltyx7dk

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-p | MT20 | 5.0 | 6.0 | 1.50 | 3.00 |
| C | TTWW-m | MT20 | 5.0 | 8.0 | 2.00 | 3.00 |
| D | TMW+w | MT20 | 2.0 | 4.0 | | |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TS-t | MT20 | 3.0 | 8.0 | | |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TMWW-t | MT20 | 5.0 | 6.0 | | |
| I | TMVW+p | MT20 | 5.0 | 6.0 | 2.25 | 2.00 |
| J | BMV1+p | MT20 | 4.0 | 6.0 | 3.00 | Edge |
| K | BMWW+t | MT20 | 5.0 | 6.0 | 2.75 | 1.50 |
| L | BMWW-t | MT20 | 5.0 | 6.0 | | |
| M | BMWW+t | MT20 | 4.0 | 6.0 | | |
| N | BS-t | MT20 | 5.0 | 6.0 | | |
| O | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| P | BMWW-t | MT20 | 5.0 | 6.0 | | |
| Q | BMV1+p | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.


Structural component only
DWG# T-2311454

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T12 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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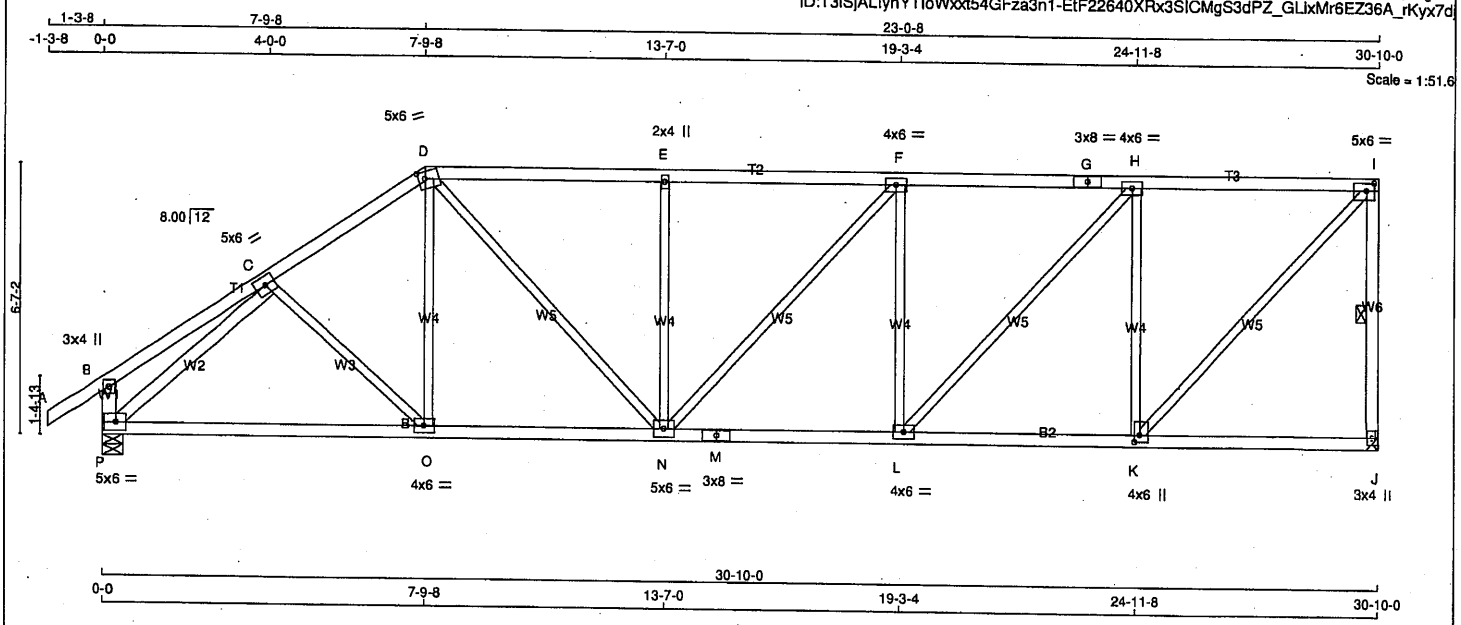


TOTAL WEIGHT = 138 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - G | 2x4 | DRY | No.2 | SPF |
| G - I | 2x4 | DRY | No.2 | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| P - B | 2x4 | DRY | No.2 | SPF |
| P - M | 2x4 | DRY | No.2 | SPF |
| M - J | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| P - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | | |
| D | TTWW-m | MT20 | 5.0 | 6.0 | 2.00 | 2.00 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TMWW-t | MT20 | 4.0 | 6.0 | | |
| G | TS-t | MT20 | 3.0 | 8.0 | | |
| H | TMWW-t | MT20 | 4.0 | 6.0 | | |
| I | TMVW-t | MT20 | 5.0 | 6.0 | 2.25 | 2.25 |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |
| K | BMWW+t | MT20 | 4.0 | 6.0 | 2.00 | 1.50 |
| L | BMWW-t | MT20 | 4.0 | 6.0 | | |
| M | BS-t | MT20 | 3.0 | 8.0 | | |
| N | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| O | BMWW-t | MT20 | 4.0 | 6.0 | | |
| P | BMVW1-t | MT20 | 5.0 | 6.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only.
DWG# T-2311455

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| J | 1700 | 0 | 1700 | 0 | 0 | MECHANICAL | |
| P | 1826 | 0 | 1826 | 0 | 0 | 5-8 | 2-0 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| J | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |
| P | 1289 | 859 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 430 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A-B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | C- O | 0 / 54 | 0.02 (4) |
| B-C | 0 / 22 | -91.8 | -91.8 | 0.21 (1) | 10.00 | | O- D | 0 / 152 | 0.05 (4) |
| C-D | -1934 / 0 | -91.8 | -91.8 | 0.28 (1) | 4.58 | | P- C | -2185 / 0 | 0.73 (1) |
| D-E | -2103 / 0 | -91.8 | -91.8 | 0.57 (1) | 4.06 | | K- I | 0 / 1969 | 0.44 (1) |
| E-F | -2103 / 0 | -91.8 | -91.8 | 0.57 (1) | 4.06 | | D- N | 0 / 756 | 0.17 (1) |
| F-G | -1987 / 0 | -91.8 | -91.8 | 0.57 (1) | 4.14 | | K- H | -1321 / 0 | 0.94 (1) |
| G-H | -1987 / 0 | -91.8 | -91.8 | 0.57 (1) | 4.14 | | N- E | -595 / 0 | 0.43 (1) |
| H-I | -1343 / 0 | -91.8 | -91.8 | 0.51 (1) | 4.91 | | L- H | 0 / 962 | 0.22 (1) |
| J-I | -1655 / 0 | 0.0 | 0.0 | 0.33 (1) | 5.15 | | N- F | 0 / 173 | 0.04 (1) |
| P-B | -265 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | | L- F | -609 / 0 | 0.44 (1) |
| P-O | 0 / 1589 | -18.5 | -18.5 | 0.39 (1) | 10.00 | | | | |
| O-N | 0 / 1591 | -18.5 | -18.5 | 0.39 (1) | 10.00 | | | | |
| N-M | 0 / 1987 | -18.5 | -18.5 | 0.38 (1) | 10.00 | | | | |
| M-L | 0 / 1987 | -18.5 | -18.5 | 0.38 (1) | 10.00 | | | | |
| L-K | 0 / 1343 | -18.5 | -18.5 | 0.29 (1) | 10.00 | | | | |
| K-J | 0 / 0 | -18.5 | -18.5 | 0.15 (4) | 10.00 | | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.57/1.00 (D-E:1) , BC=0.39/1.00 (N-C:1) , WB=0.94/1.00 (H-K:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90 )
JSI METAL= 0.62 (M) (INPUT = 0.95 )

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T13 | 6 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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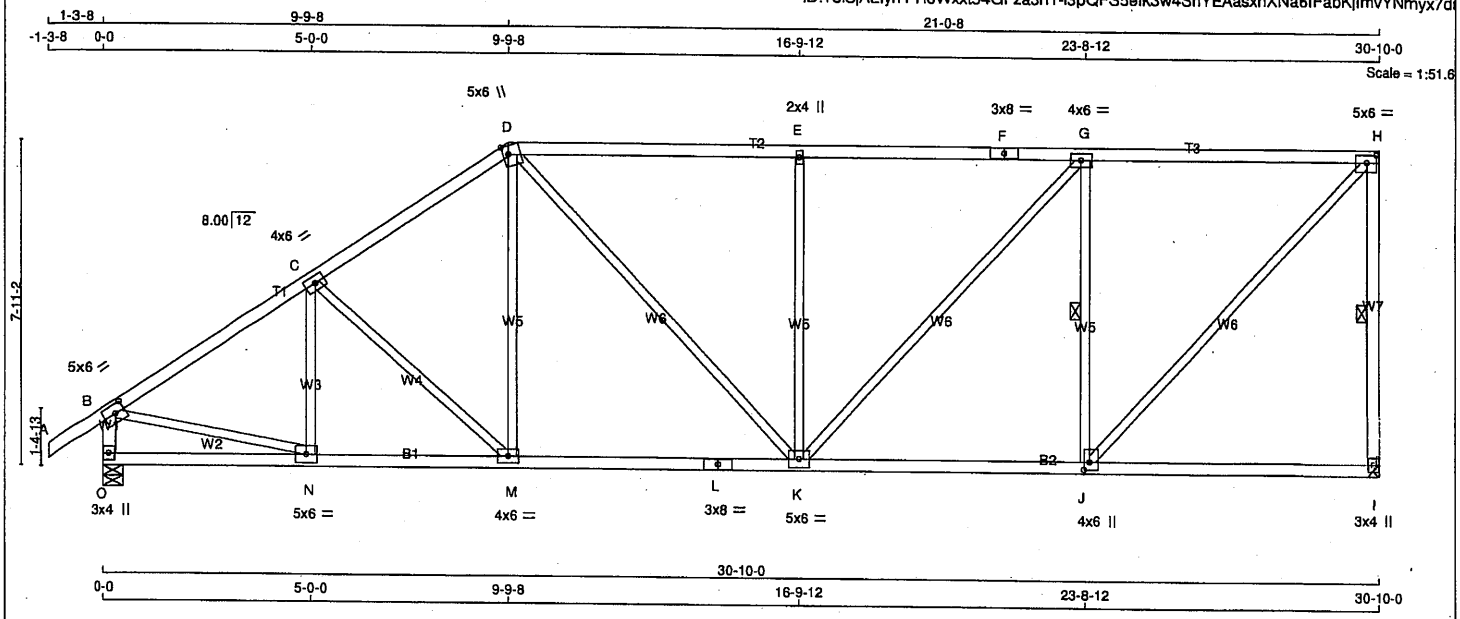


TOTAL WEIGHT = 6 X 139 = 832 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| O - B | 2x4 | DRY | No.2 | SPF |
| O - L | 2x4 | DRY | No.2 | SPF |
| L - I | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TS-t | MT20 | 3.0 | 8.0 | | |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TMVW-t | MT20 | 5.0 | 6.0 | 2.25 | 2.75 |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |
| J | BMWW+t | MT20 | 4.0 | 6.0 | 2.25 | 1.50 |
| K | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| L | BS-t | MT20 | 3.0 | 8.0 | | |
| M | BMWW-t | MT20 | 4.0 | 6.0 | | |
| N | BMWW-t | MT20 | 5.0 | 6.0 | | |
| O | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| I | 1700 | 0 | 1700 | 0 | 0 | MECHANICAL | |
| O | 1826 | 0 | 1826 | 0 | 0 | 5-8 | 2-7 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| I | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |
| O | 1289 | 859 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 430 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | N- C | -274 / 0 | 0.09 (1) |
| B- C | -1987 / 0 | -91.8 | -91.8 | 0.47 (1) | 4.30 | C- M | -245 / 0 | 0.19 (1) |
| C- D | -1829 / 0 | -91.8 | -91.8 | 0.45 (1) | 4.46 | M- D | 0 / 281 | 0.06 (1) |
| D- E | -1766 / 0 | -91.8 | -91.8 | 0.78 (1) | 4.03 | B- N | 0 / 1716 | 0.39 (1) |
| E- F | -1766 / 0 | -91.8 | -91.8 | 0.80 (1) | 3.98 | J- H | 0 / 1890 | 0.43 (1) |
| F- G | -1766 / 0 | -91.8 | -91.8 | 0.80 (1) | 3.98 | D- K | 0 / 395 | 0.09 (1) |
| G- H | -1287 / 0 | -91.8 | -91.8 | 0.73 (1) | 4.59 | J- G | -1237 / 0 | 0.49 (1) |
| I- H | -1647 / 0 | 0.0 | 0.0 | 0.47 (1) | 5.16 | K- E | -695 / 0 | 0.83 (1) |
| O- B | -1784 / 0 | 0.0 | 0.0 | 0.18 (1) | 6.26 | K- G | 0 / 714 | 0.16 (1) |
| O- N | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | |
| N- M | 0 / 1677 | -18.5 | -18.5 | 0.35 (1) | 10.00 | | | |
| M- L | 0 / 1499 | -18.5 | -18.5 | 0.32 (1) | 10.00 | | | |
| L- K | 0 / 1499 | -18.5 | -18.5 | 0.32 (1) | 10.00 | | | |
| K- J | 0 / 1287 | -18.5 | -18.5 | 0.33 (1) | 10.00 | | | |
| J- I | 0 / 0 | -18.5 | -18.5 | 0.22 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.80/1.00 (E-G:1) , BC=0.35/1.00 (M-N:1) , WB=0.83/1.00 (E-K:1) , SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90 )
JSI METAL= 0.57 (J) (INPUT = 0.95 )


Structural component only
DWG# T-2311456

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T14 | 4 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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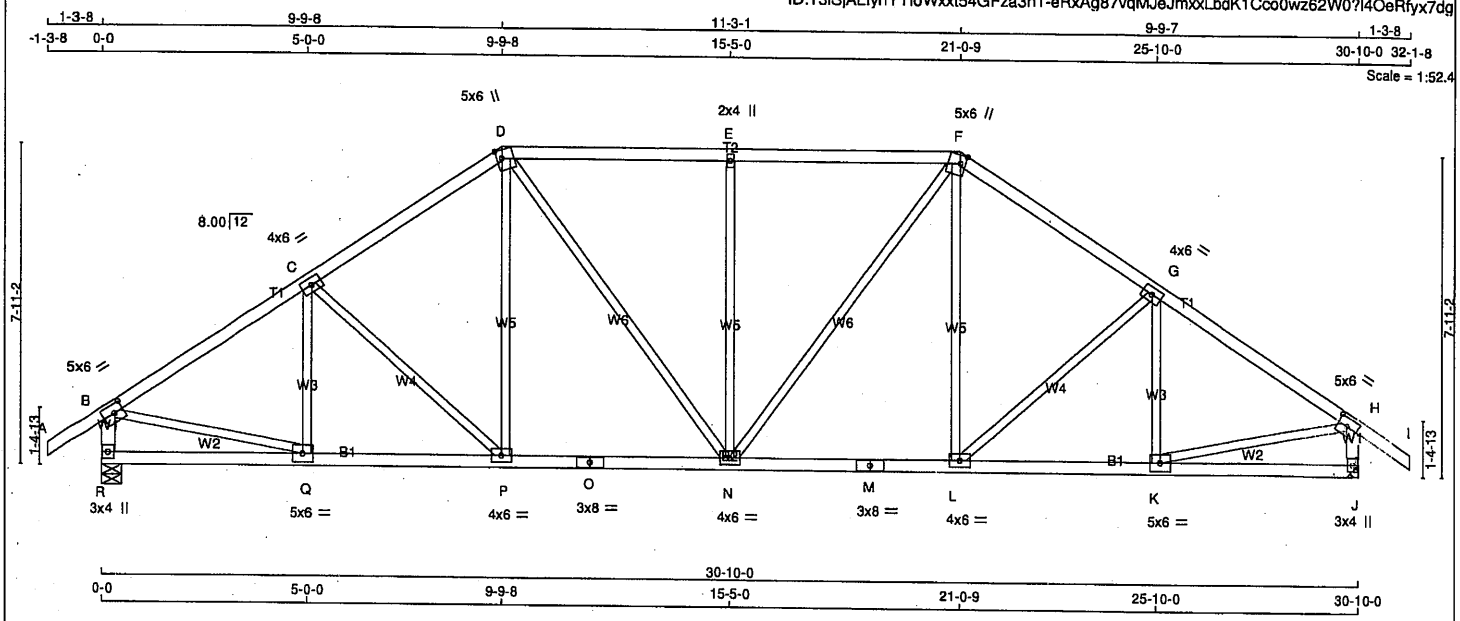


TOTAL WEIGHT = 4 X 138 = 553 lb
[M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - I | 2x4 | DRY | No.2 | SPF |
| R - B | 2x4 | DRY | No.2 | SPF |
| J - H | 2x4 | DRY | No.2 | SPF |
| R - O | 2x4 | DRY | No.2 | SPF |
| O - M | 2x4 | DRY | No.2 | SPF |
| M - J | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |
| K | BMWW-t | MT20 | 5.0 | 6.0 | | |
| L | BMWW-t | MT20 | 4.0 | 6.0 | | |
| M | BS-t | MT20 | 3.0 | 8.0 | | |
| N | BMWWW-t | MT20 | 4.0 | 6.0 | | |
| O | BS-t | MT20 | 3.0 | 8.0 | | |
| P | BMWW-t | MT20 | 4.0 | 6.0 | | |
| Q | BMWW-t | MT20 | 5.0 | 6.0 | | |
| R | BMV1+p | MT20 | 3.0 | 4.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311457

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| R | 1826 | 0 | 1826 | 0 | 0 | 5-8 | 2-7 |
| J | 1826 | 0 | 1826 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-7.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| R | 1289 | 859 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 430 / 0 | 0 / 0 |
| J | 1289 | 859 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 430 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. | FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. | FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | A-B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | | 10.00 | Q-C | | -268 / 0 | 0.09 (1) |
| | B-C | -1988 / 0 | -91.8 | -91.8 | 0.34 (1) | | 4.47 | C-P | | -250 / 0 | 0.20 (1) |
| | C-D | -1827 / 0 | -91.8 | -91.8 | 0.33 (1) | | 4.64 | P-D | | 0 / 266 | 0.06 (1) |
| | D-E | -1767 / 0 | -91.8 | -91.8 | 0.41 (1) | | 4.57 | D-N | | 0 / 456 | 0.10 (1) |
| | E-F | -1767 / 0 | -91.8 | -91.8 | 0.41 (1) | | 4.57 | N-E | | -631 / 0 | 0.75 (1) |
| | F-G | -1827 / 0 | -91.8 | -91.8 | 0.33 (1) | | 4.64 | N-F | | 0 / 456 | 0.10 (1) |
| | G-H | -1988 / 0 | -91.8 | -91.8 | 0.34 (1) | | 4.47 | L-F | | 0 / 266 | 0.06 (1) |
| | H-I | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | | 10.00 | L-G | | -250 / 0 | 0.20 (1) |
| | R-B | -1785 / 0 | 0.0 | 0.0 | 0.18 (1) | | 6.25 | K-G | | -268 / 0 | 0.09 (1) |
| | J-H | -1785 / 0 | 0.0 | 0.0 | 0.18 (1) | | 6.25 | B-Q | | 0 / 1717 | 0.39 (1) |
| | | | | | | | | K-H | | 0 / 1717 | 0.39 (1) |
| | R-Q | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | | 10.00 | | | | |
| | Q-P | 0 / 1678 | -18.5 | -18.5 | 0.33 (1) | | 10.00 | | | | |
| | P-O | 0 / 1496 | -18.5 | -18.5 | 0.30 (1) | | 10.00 | | | | |
| | O-N | 0 / 1496 | -18.5 | -18.5 | 0.30 (1) | | 10.00 | | | | |
| | N-M | 0 / 1496 | -18.5 | -18.5 | 0.30 (1) | | 10.00 | | | | |
| | M-L | 0 / 1496 | -18.5 | -18.5 | 0.30 (1) | | 10.00 | | | | |
| | L-K | 0 / 1678 | -18.5 | -18.5 | 0.33 (1) | | 10.00 | | | | |
| | K-J | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | | 10.00 | | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.33/1.00 (P-Q:1) , WB=0.75/1.00 (E-N:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90 )
JSI METAL= 0.50 (H) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T15 | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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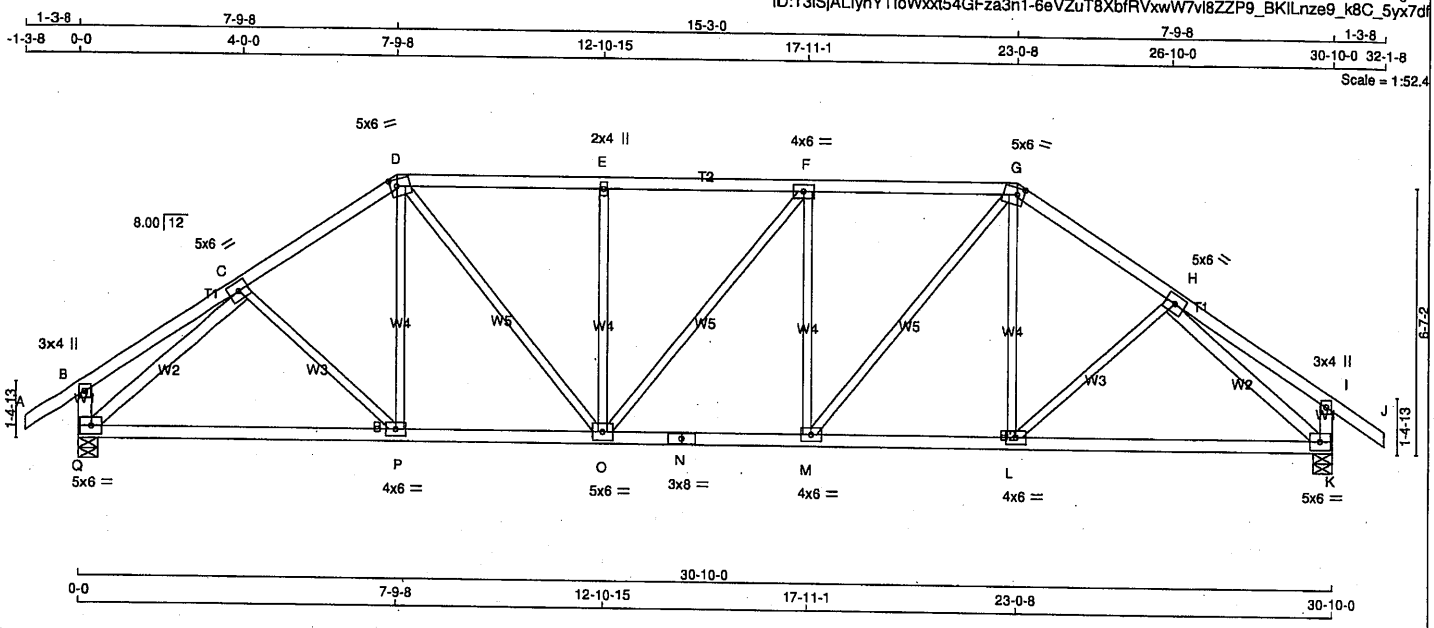


TOTAL WEIGHT = 138 lb [M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - G | 2x4 | DRY | No.2 | SPF |
| G - J | 2x4 | DRY | No.2 | SPF |
| Q - B | 2x4 | DRY | No.2 | SPF |
| K - I | 2x4 | DRY | No.2 | SPF |
| Q - N | 2x4 | DRY | No.2 | SPF |
| N - K | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| Q - C | 2x4 | DRY | No.2 | SPF |
| H - K | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | | |
| D | TTWW-m | MT20 | 5.0 | 6.0 | 2.00 | 2.00 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TMWW-t | MT20 | 4.0 | 6.0 | | |
| G | TTWW-m | MT20 | 5.0 | 6.0 | 2.00 | 2.00 |
| H | TMWW-t | MT20 | 5.0 | 6.0 | | |
| I | TMV+p | MT20 | 3.0 | 4.0 | | |
| K | BMVW1-t | MT20 | 5.0 | 6.0 | | |
| L, M, P | | | | | | |
| L | BMWW-t | MT20 | 4.0 | 6.0 | | |
| N | BS-t | MT20 | 3.0 | 8.0 | | |
| O | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| Q | BMVW1-t | MT20 | 5.0 | 6.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| Q | 1826 | 0 | 1826 | 0 | 0 | IN-SX | IN-SX |
| K | 1826 | 0 | 1826 | 0 | 0 | 5-8 | 2-0 |
| | | | | | | 5-8 | 2-0 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| Q | 1289 | 859 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 430 / 0 | 0 / 0 |
| K | 1289 | 859 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 430 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

CHORDS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 FROM | TO | MAX (PLF) CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | 0 / 22 | -91.8 | -91.8 | 0.21 (1) | 10.00 |
| C- D | -1934 / 0 | -91.8 | -91.8 | 0.21 (1) | 4.67 |
| D- E | -2061 / 0 | -91.8 | -91.8 | 0.32 (1) | 4.42 |
| E- F | -2061 / 0 | -91.8 | -91.8 | 0.30 (1) | 4.45 |
| F- G | -2063 / 0 | -91.8 | -91.8 | 0.32 (1) | 4.42 |
| G- H | -1934 / 0 | -91.8 | -91.8 | 0.21 (1) | 4.67 |
| H- I | 0 / 22 | -91.8 | -91.8 | 0.21 (1) | 10.00 |
| I- J | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| Q- B | -265 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 |
| K- I | -265 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 |
| Q- P | 0 / 1588 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| P- O | 0 / 1591 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| O- N | 0 / 2063 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| N- M | 0 / 2063 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| M- L | 0 / 1590 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| L- K | 0 / 1588 | -18.5 | -18.5 | 0.39 (1) | 10.00 |

WEBS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| C- P | 0 / 53 | 0.02 (4) |
| P- D | 0 / 149 | 0.05 (4) |
| L- G | 0 / 148 | 0.05 (4) |
| L- H | 0 / 53 | 0.02 (4) |
| Q- C | -2185 / 0 | 0.73 (1) |
| H- K | -2185 / 0 | 0.73 (1) |
| M- G | 0 / 749 | 0.17 (1) |
| D- O | 0 / 746 | 0.17 (1) |
| M- F | -502 / 0 | 0.36 (1) |
| O- E | -501 / 0 | 0.36 (1) |
| O- F | -3 / 0 | 0.00 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.39/1.00 (O-P:1) , WB=0.73/1.00 (C-Q:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90 )
JSI METAL= 0.68 (N) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311458

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T16 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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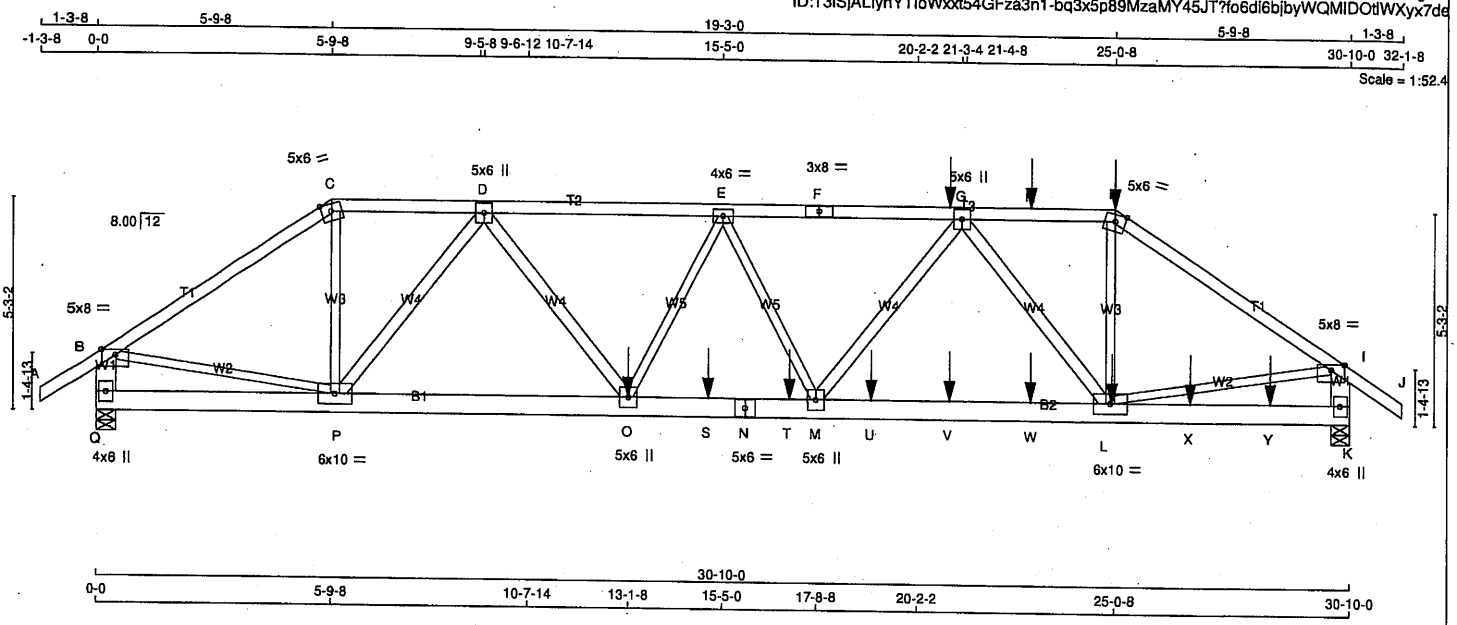


**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| H - J | 2x4 | DRY | No.2 | SPF |
| Q - B | 2x6 | DRY | No.2 | SPF |
| K - I | 2x6 | DRY | No.2 | SPF |
| Q - N | 2x6 | DRY | No.2 | SPF |
| N - K | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| P - D | 2x4 | DRY | No.2 | SPF |
| D - O | 2x4 | DRY | No.2 | SPF |
| G - L | 2x4 | DRY | No.2 | SPF |
| M - G | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A-C | 1 | 12 | TOP |
| C-F | 1 | 12 | TOP |
| F-H | 1 | 12 | SIDE(61.0) |
| H-J | 1 | 12 | SIDE(61.0) |
| Q-B | 2 | 12 | TOP |
| K-I | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| Q-N | 2 | 12 | SIDE(0.0) |
| N-K | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| L-H | 1 | 6 | SIDE(2.2) |
| 2x3 | 1 | 6 | |
| 2x4 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.


Structural component only
DWG# T-2311459

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| Q | 3577 | 0 | 3577 | 0 | 0 | 5-8 | IN-SX 1-15 |
| K | 3989 | 0 | 3989 | 0 | 0 | 5-8 | IN-SX 2-3 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| Q | 2522 | 1699 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 822 / 0 | 0 / 0 |
| K | 2815 | 1877 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 938 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. | FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 FROM (PLF) | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A-B | | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 |
| B-C | | -4505 / 0 | -91.8 | -91.8 | 0.49 (1) | 4.11 |
| C-D | | -3829 / 0 | -91.8 | -91.8 | 0.21 (1) | 4.64 |
| D-E | | -7305 / 0 | -91.8 | -91.8 | 0.52 (1) | 3.32 |
| E-F | | -7095 / 0 | -91.8 | -91.8 | 0.50 (1) | 3.38 |
| F-G | | -7095 / 0 | -91.8 | -91.8 | 0.50 (1) | 3.38 |
| G-R | | -4288 / 0 | -91.8 | -91.8 | 0.26 (1) | 4.36 |
| R-H | | -4288 / 0 | -91.8 | -91.8 | 0.26 (1) | 4.36 |
| H-I | | -5069 / 0 | -91.8 | -91.8 | 0.54 (1) | 3.88 |
| I-J | | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 |
| Q-B | | -3520 / 0 | 0.0 | 0.0 | 0.13 (1) | 7.46 |
| K-I | | -3912 / 0 | 0.0 | 0.0 | 0.14 (1) | 7.16 |
| Q-P | | 0 / 0 | -18.5 | -18.5 | 0.04 (4) | 10.00 |
| P-O | | 0 / 5741 | -18.5 | -18.5 | 0.41 (1) | 10.00 |
| O-S | | 0 / 7328 | -18.5 | -18.5 | 0.62 (1) | 10.00 |
| S-N | | 0 / 7328 | -18.5 | -18.5 | 0.62 (1) | 10.00 |
| N-T | | 0 / 7328 | -18.5 | -18.5 | 0.62 (1) | 10.00 |
| T-M | | 0 / 7328 | -18.5 | -18.5 | 0.62 (1) | 10.00 |
| M-U | | 0 / 5971 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| U-V | | 0 / 5971 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| V-W | | 0 / 5971 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| W-L | | 0 / 5971 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| L-X | | 0 / 0 | -18.5 | -18.5 | 0.09 (4) | 10.00 |
| X-Y | | 0 / 0 | -18.5 | -18.5 | 0.09 (4) | 10.00 |
| Y-K | | 0 / 0 | -18.5 | -18.5 | 0.09 (4) | 10.00 |

| WEBS MEMB. | FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|
| P-C | | 0 / 2105 | 0.26 (1) |
| L-H | | 0 / 1875 | 0.23 (1) |
| B-P | | 0 / 3795 | 0.47 (1) |
| L-I | | 0 / 4270 | 0.53 (1) |
| O-E | | -55 / 11 | 0.01 (1) |
| E-M | | -548 / 0 | 0.14 (1) |
| P-D | | -3239 / 0 | 0.76 (1) |
| D-O | | 0 / 2655 | 0.23 (1) |
| G-L | | -2851 / 0 | 0.67 (1) |
| M-G | | 0 / 1908 | 0.17 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| G | 20-11-12 | -120 | -120 | --- | BACK | VERT | TOTAL | --- | C1 |
| H | 25-0-8 | -329 | -329 | --- | BACK | VERT | TOTAL | --- | C1 |
| L | 24-11-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |
| O | 13-1-8 | -1256 | -1256 | --- | BACK | VERT | TOTAL | --- | C1 |
| R | 22-11-12 | -120 | -120 | --- | BACK | VERT | TOTAL | --- | C1 |
| S | 15-0-12 | -263 | -263 | --- | BACK | VERT | TOTAL | --- | C1 |
| T | 17-0-12 | -272 | -272 | --- | BACK | VERT | TOTAL | --- | C1 |
| U | 19-0-12 | -260 | -260 | --- | BACK | VERT | TOTAL | --- | C1 |
| V | 20-11-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |
| W | 22-11-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |
| X | 26-10-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |
| Y | 28-10-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |

TOTAL WEIGHT = 2 X 151 = 302 lb [M]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.54/1.00 (H-I:1) , BC=0.62/1.00 (M-O:1) , WB=0.76/1.00 (D-P:1) , SSI=0.20/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1967 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (N) (INPUT = 0.90 )
JSI METAL= 0.77 (N) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T16 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| C | TTW-m | MT20 | 5.0 | 6.0 | Edge | |
| D | TMWW+t | MT20 | 5.0 | 6.0 | | |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TS-t | MT20 | 3.0 | 8.0 | | |
| G | TMWW+t | MT20 | 5.0 | 6.0 | | |
| H | TTW-m | MT20 | 5.0 | 6.0 | Edge | |
| I | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| K | BMV1+p | MT20 | 4.0 | 6.0 | | |
| L | BMWWW-t | MT20 | 6.0 | 10.0 | | |
| M | BMWW+t | MT20 | 5.0 | 6.0 | | |
| N | BS-t | MT20 | 5.0 | 6.0 | | |
| O | BMWW+t | MT20 | 5.0 | 6.0 | | |
| P | BMWWW-t | MT20 | 6.0 | 10.0 | | |
| Q | BMV1+p | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES- (1)**
1) Lateral braces to be a minimum of 2X4 SPF #2.

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.


Structural component only
DWG# T-2311459

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T17 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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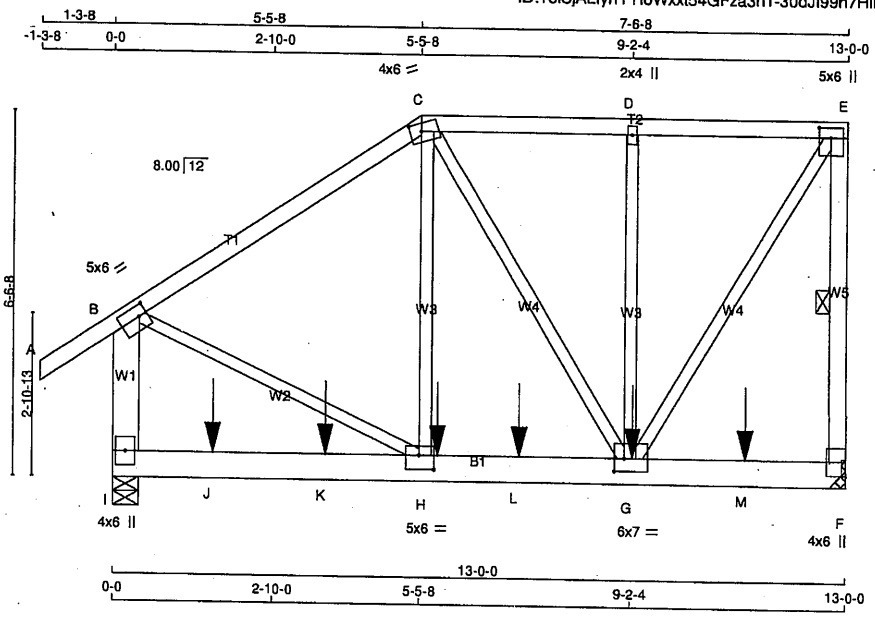


**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| I - B | 2x6 | DRY | No.2 | SPF |
| I - F | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.25 | 1.75 |
| C | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.50 |
| D | TMW+w | MT20 | 2.0 | 4.0 | | |
| E | TMVW+p | MT20 | 5.0 | 6.0 | 2.25 | 2.50 |
| F | BMV1+p | MT20 | 4.0 | 6.0 | 3.00 | Edge |
| G | BMWWW-t | MT20 | 6.0 | 7.0 | 2.75 | 2.00 |
| H | BMWW-t | MT20 | 5.0 | 6.0 | 3.00 | 3.00 |
| I | BMV1+p | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311460

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 1839 | 0 | 1839 | 0 | 0 | MECHANICAL | |
| I | 1982 | 0 | 1982 | 0 | 0 | 5-8 | 2-2 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 2-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 1297 | 873 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 424 / 0 | 0 / 0 |
| I | 1395 | 950 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 445 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 | H- C | 0 / 727 | 0.18 (1) | |
| B- C | -1359 / 0 | -91.8 | -91.8 | 0.62 (1) | 4.69 | C- G | -419 / 0 | 0.40 (1) | |
| C- D | -924 / 0 | -91.8 | -91.8 | 0.23 (1) | 6.10 | G- D | -418 / 0 | 0.28 (1) | |
| D- E | -924 / 0 | -91.8 | -91.8 | 0.23 (1) | 6.10 | G- E | 0 / 1757 | 0.43 (1) | |
| F- E | -1632 / 0 | 0.0 | 0.0 | 0.33 (1) | 5.15 | B- H | 0 / 1238 | 0.31 (1) | |
| I- B | -1639 / 0 | 0.0 | 0.0 | 0.17 (1) | 7.66 | | | | |
| I- J | 0 / 0 | -18.5 | -18.5 | 0.43 (1) | 10.00 | | | | |
| J- K | 0 / 0 | -18.5 | -18.5 | 0.43 (1) | 10.00 | | | | |
| K- H | 0 / 0 | -18.5 | -18.5 | 0.43 (1) | 10.00 | | | | |
| H- L | 0 / 1142 | -18.5 | -18.5 | 0.50 (1) | 10.00 | | | | |
| L- G | 0 / 1142 | -18.5 | -18.5 | 0.50 (1) | 10.00 | | | | |
| G- M | 0 / 0 | -18.5 | -18.5 | 0.25 (1) | 10.00 | | | | |
| M- F | 0 / 0 | -18.5 | -18.5 | 0.25 (1) | 10.00 | | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| G | 9-2-12 | -272 | -272 | --- | BACK | VERT | TOTAL | --- | C1 |
| H | 5-9-4 | -263 | -263 | --- | BACK | VERT | TOTAL | --- | C1 |
| J | 1-9-4 | -261 | -261 | --- | BACK | VERT | TOTAL | --- | C1 |
| K | 3-9-4 | -272 | -272 | --- | BACK | VERT | TOTAL | --- | C1 |
| L | 7-2-12 | -263 | -263 | --- | BACK | VERT | TOTAL | --- | C1 |
| M | 11-2-12 | -260 | -260 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 77 lb [M]

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.50/1.00 (G-H:1) , WB=0.43/1.00 (E-G:1) , SSI=0.34/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (G) (INPUT = 0.90 )
JSI METAL= 0.36 (C) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T18 | 2 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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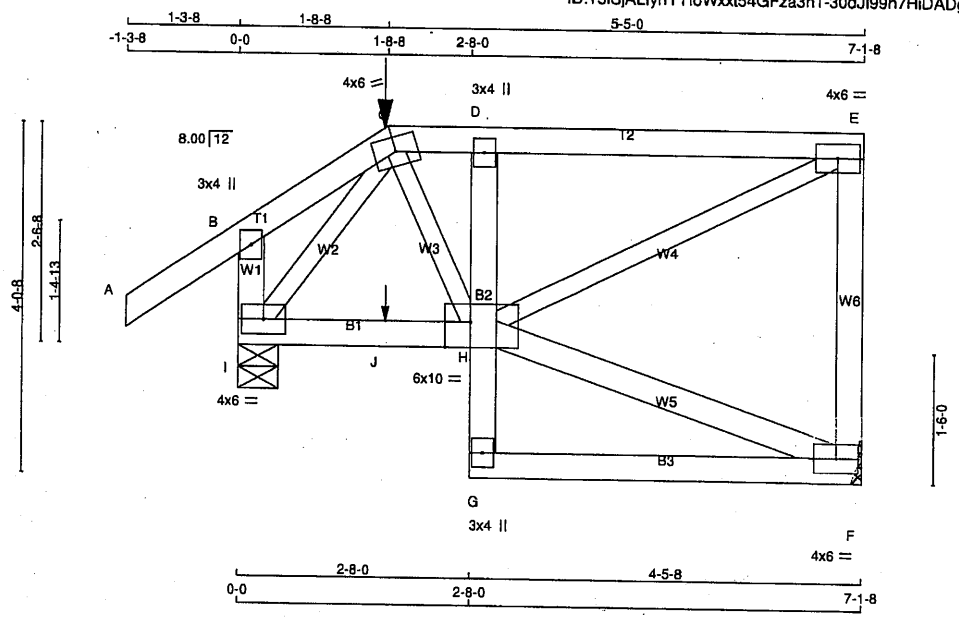


TOTAL WEIGHT = 2 X 40 = 79 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| I - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| G - D | 2x4 | DRY | No.2 | SPF |
| G - F | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TTWW-m | MT20 | 4.0 | 6.0 | | |
| D | TMV+p | MT20 | 3.0 | 4.0 | | |
| E | TMVW-t | MT20 | 4.0 | 6.0 | | |
| F | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| G | BMV+p | MT20 | 3.0 | 4.0 | | |
| H | BVMWWW-I | MT20 | 6.0 | 10.0 | Edge | 3.50 |
| I | BMVW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| F | 388 | 0 | 388 | 0 | 0 | IN-SX | IN-SX |
| I | 553 | 0 | 553 | 0 | 0 | MECHANICAL 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 274 | 179 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 95 / 0 | 0 / 0 |
| I | 388 | 271 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 117 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (7)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT: LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 | |
| B- C | -27 / 0 | -91.8 | -91.8 | 0.13 (1) | 6.25 | |
| C- D | -357 / 0 | -91.8 | -91.8 | 0.24 (5) | 6.25 | |
| D- E | -351 / 0 | -91.8 | -91.8 | 0.24 (5) | 6.25 | |
| F- E | -345 / 0 | 0.0 | 0.0 | 0.09 (1) | 7.81 | |
| I- B | -256 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | |
| I- J | 0 / 211 | -18.5 | -18.5 | 0.07 (4) | 10.00 | |
| J- H | 0 / 211 | -18.5 | -18.5 | 0.07 (4) | 10.00 | |
| G- H | 0 / 45 | 0.0 | 0.0 | 0.03 (1) | 10.00 | |
| H- D | -430 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | |
| G- F | 0 / 9 | -18.5 | -18.5 | 0.11 (4) | 10.00 | |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| C- H | 0 / 375 | 0.09 (5) |
| H- F | -9 / 0 | 0.00 (1) |
| H- E | 0 / 396 | 0.10 (1) |
| I- C | -344 / 0 | 0.06 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| C | 1-9-0 | -4 | -4 | --- | FRONT | VERT | DEAD | --- | C1 |
| C | 1-9-0 | 1 | 1 | 79 | BACK | VERT | TOTAL | --- | C1 |
| C | 1-9-0 | -16 | -16 | --- | FRONT | VERT | SNOW | --- | C1 |
| J | 1-8-4 | 1 | 1 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT).
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.24/1.00 (D-E:5) , BC=0.11/1.00 (F-G:4) , WB=0.10/1.00 (E-H:1) , SSI=0.20/1.00 (D-E:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311461

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T18 | 2 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



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JSI GRIP= 0.60 (C) (INPUT = 0.90 )
JSI METAL= 0.11 (C) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311461

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T18Z | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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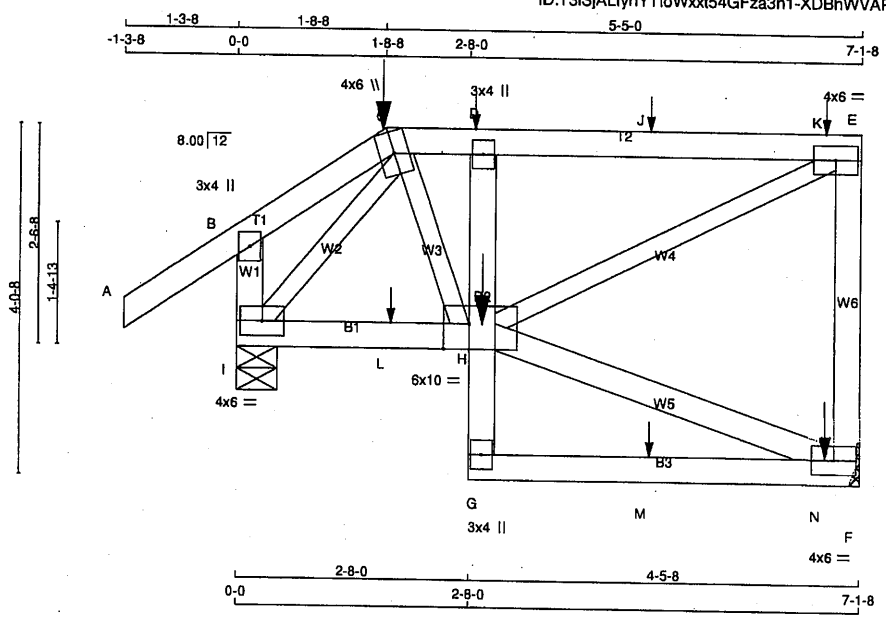


TOTAL WEIGHT = 40 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| I - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| G - D | 2x4 | DRY | No.2 | SPF |
| G - F | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TTWW+m | MT20 | 4.0 | 6.0 | | |
| D | TMV+p | MT20 | 3.0 | 4.0 | | |
| E | TMVW-t | MT20 | 4.0 | 6.0 | | |
| F | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| G | BMV+p | MT20 | 3.0 | 4.0 | | |
| H | BVMWWW-I | MT20 | 6.0 | 10.0 | Edge | 3.50 |
| I | BMVW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311462

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| F | 390 | 0 | 390 | 0 | 0 | IN-SX | IN-SX |
| I | 555 | 0 | 555 | 0 | 0 | MECHANICAL 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 276 | 179 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 97 / 0 | 0 / 0 |
| I | 390 | 271 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 118 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (7)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 | C- H | 0 / 344 | 0.09 (1) |
| B- C | -27 / 0 | -91.8 | -91.8 | 0.13 (1) | 6.25 | H- F | -9 / 0 | 0.00 (1) |
| C- D | -360 / 0 | -91.8 | -91.8 | 0.23 (1) | 6.25 | H- E | 0 / 399 | 0.10 (1) |
| D- J | -354 / 0 | -91.8 | -91.8 | 0.24 (1) | 6.25 | I- C | -347 / 0 | 0.07 (1) |
| J- K | -354 / 0 | -91.8 | -91.8 | 0.24 (1) | 6.25 | | | |
| K- E | -354 / 0 | -91.8 | -91.8 | 0.24 (1) | 6.25 | | | |
| F- E | -346 / 0 | 0.0 | 0.0 | 0.09 (1) | 7.81 | | | |
| I- B | -256 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | | | |
| I- L | 0 / 213 | -18.5 | -18.5 | 0.07 (4) | 10.00 | | | |
| L- H | 0 / 213 | -18.5 | -18.5 | 0.07 (4) | 10.00 | | | |
| G- H | 0 / 45 | 0.0 | 0.0 | 0.03 (1) | 10.00 | | | |
| H- D | -429 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | | | |
| G- M | 0 / 9 | -18.5 | -18.5 | 0.11 (4) | 10.00 | | | |
| M- N | 0 / 9 | -18.5 | -18.5 | 0.11 (4) | 10.00 | | | |
| N- F | 0 / 9 | -18.5 | -18.5 | 0.11 (4) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| C | 1-9-0 | -4 | -4 | --- | FRONT | VERT | DEAD | --- | C1 |
| C | 1-9-0 | 1 | 1 | 79 | FRONT | VERT | TOTAL | --- | C1 |
| C | 1-9-0 | -16 | -16 | --- | FRONT | VERT | SNOW | --- | C1 |
| D | 2-8-12 | 1 | 1 | 79 | FRONT | VERT | TOTAL | --- | C1 |
| H | 2-9-12 | -3 | -3 | --- | FRONT | VERT | TOTAL | --- | C1 |
| J | 4-8-12 | 1 | 1 | 79 | FRONT | VERT | TOTAL | --- | C1 |
| K | 6-8-12 | 1 | 1 | 64 | FRONT | VERT | TOTAL | --- | C1 |
| L | 1-9-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| M | 4-8-12 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| N | 6-8-12 | -1 | -1 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.24/1.00 (D-E:1) , BC=0.11/1.00 (F-G:4) , WB=0.10/1.00 (E-H:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T18Z | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:39 2023 Page 2
ID:T3iSjALlyhYTIoWxxt54GFza3n1-XDBhWVAPuaq4oNEiaQhGB2nXnXPN_UBbgiMsbQyx7dc

JSI GRIP= 0.28 (C) (INPUT = 0.90 )
JSI METAL= 0.15 (C) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311462

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T19 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:40 2023 Page 1
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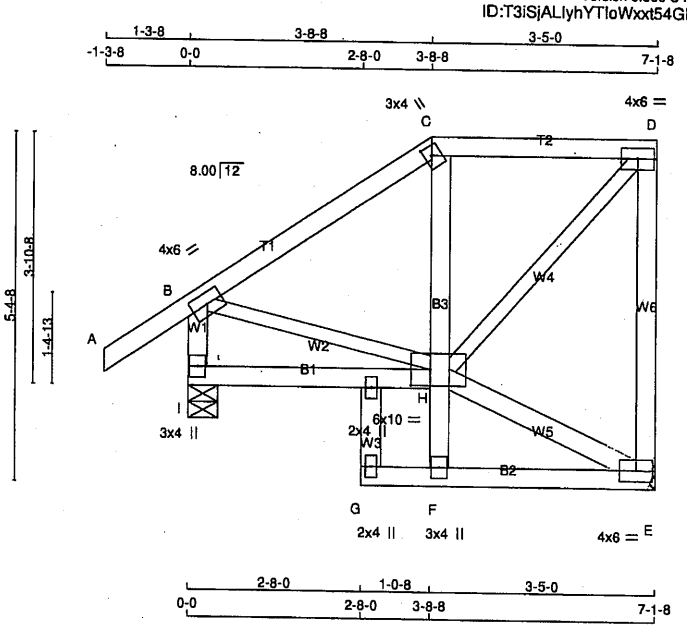


Scale = 1:32.6

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| I - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |
| F - C | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - E | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TTV+h | MT20 | 3.0 | 4.0 | 1.75 | 1.25 |
| D | TMVW-t | MT20 | 4.0 | 6.0 | | |
| E | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| F | BMV+p | MT20 | 3.0 | 4.0 | | |
| G | NP+w | MT20 | 2.0 | 4.0 | | |
| H | BVMWWW-I | MT20 | 6.0 | 10.0 | 3.00 | 3.50 |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |
| J | NP+w | MT20 | 2.0 | 4.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| E | 405 | 0 | 405 | 0 | 0 | MECHANICAL | |
| I | 529 | 0 | 529 | 0 | 0 | IN-SX 5-8 | IN-SX 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 287 | 182 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 105 / 0 | 0 / 0 |
| I | 373 | 253 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 120 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 1 | 0.12 (1) | 10.00 | B- H | 0 / 215 | 0.05 (1) |
| B- C | -250 / 0 | -91.8 | -91.8 | 1 | 0.16 (1) | 6.25 | H- E | -5 / 0 | 0.00 (1) |
| C- D | -198 / 0 | -91.8 | -91.8 | 1 | 0.14 (1) | 6.25 | H- D | 0 / 293 | 0.07 (1) |
| E- D | -372 / 0 | 0.0 | 0.0 | 1 | 0.19 (1) | 7.81 | | | |
| I- B | -493 / 0 | 0.0 | 0.0 | 1 | 0.05 (1) | 7.81 | | | |
| I- H | 0 / 0 | -18.5 | -18.5 | 4 | 0.08 (4) | 10.00 | | | |
| G- F | 0 / 0 | -18.5 | -18.5 | 4 | 0.03 (4) | 10.00 | | | |
| F- E | 0 / 4 | -18.5 | -18.5 | 4 | 0.06 (4) | 10.00 | | | |
| F- H | 0 / 58 | 0.0 | 0.0 | 4 | 0.02 (4) | 10.00 | | | |
| H- C | -189 / 0 | 0.0 | 0.0 | 1 | 0.01 (1) | 7.81 | | | |

TOTAL WEIGHT = 3 X 44 = 131 lb
[M][F]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.19/1.00 (D-E:1) , BC=0.08/1.00 (H-I:4) , WB=0.07/1.00 (D-H:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.90 )
JSI METAL= 0.09 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311463

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T20 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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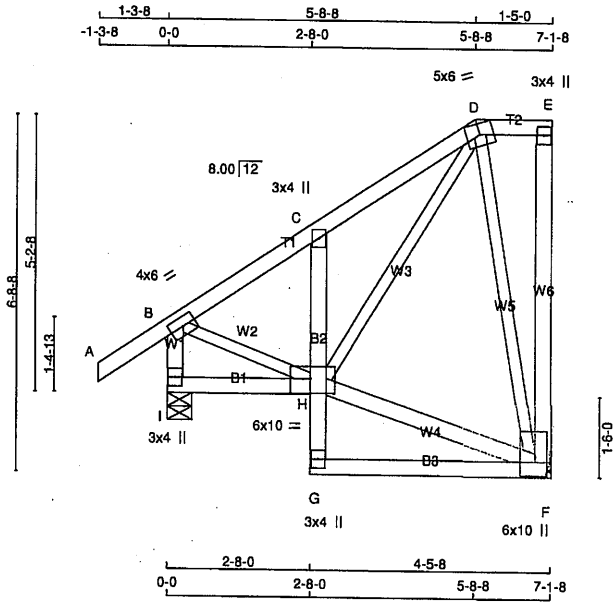


Scale = 1:39.7

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - E | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| I - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| G - C | 2x4 | DRY | No.2 | SPF |
| G - F | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TMV+p | MT20 | 3.0 | 4.0 | | |
| D | TTWW-m | MT20 | 5.0 | 6.0 | | |
| E | TMV+p | MT20 | 3.0 | 4.0 | | |
| F | BMVWW1+p | MT20 | 6.0 | 10.0 | 3.25 | 2.50 |
| G | BMV+p | MT20 | 3.0 | 4.0 | | |
| H | BVMWWW-I | MT20 | 6.0 | 10.0 | 3.25 | 4.50 |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311464

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| F | 393 | 0 | 393 | 0 | 0 | MECHANICAL | |
| I | 519 | 0 | 519 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 278 | 182 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 95 / 0 | 0 / 0 |
| I | 364 | 253 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 112 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | | 10.00 |
| B- C | -294 / 0 | -91.8 | -91.8 | 0.09 (1) | | 6.25 |
| C- D | -310 / 0 | -91.8 | -91.8 | 0.09 (1) | | 6.25 |
| D- E | 0 / 0 | -91.8 | -91.8 | 0.03 (1) | | 10.00 |
| F- E | -63 / 0 | 0.0 | 0.0 | 0.06 (1) | | 7.81 |
| I- B | -493 / 0 | 0.0 | 0.0 | 0.05 (1) | | 7.81 |
| I- H | 0 / 0 | -18.5 | -18.5 | 0.04 (4) | | 10.00 |
| G- H | 0 / 45 | 0.0 | 0.0 | 0.03 (1) | | 10.00 |
| H- C | -314 / 0 | 0.0 | 0.0 | 0.02 (1) | | 7.81 |
| G- F | 0 / 10 | -18.5 | -18.5 | 0.10 (4) | | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| B- H | 0 / 278 | 0.06 (1) |
| H- F | 0 / 59 | 0.01 (1) |
| D- F | -316 / 0 | 0.23 (1) |
| H- D | 0 / 349 | 0.08 (1) |

TOTAL WEIGHT = 3 X 49 = 146 lb
[M][F]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.10/1.00 (F-G:4) , WB=0.23/1.00 (D-F:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (D) (INPUT = 0.90 )
JSI METAL= 0.10 (C) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T21 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:42 2023 Page 1
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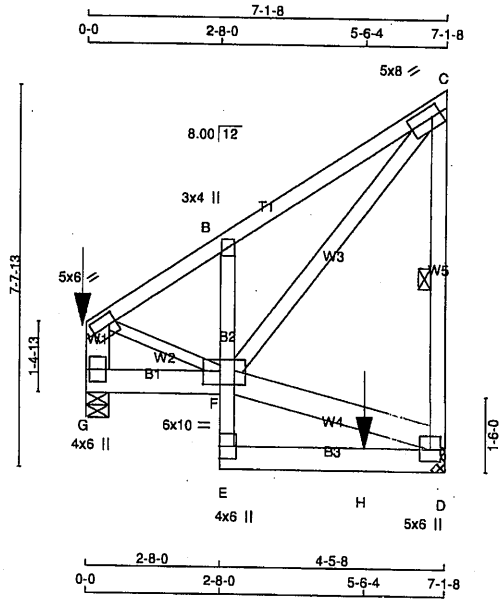


TOTAL WEIGHT = 2 X 53 = 107 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| D - C | 2x4 | DRY | No.2 | SPF |
| G - A | 2x6 | DRY | No.2 | SPF |
| G - F | 2x6 | DRY | No.2 | SPF |
| E - B | 2x4 | DRY | No.2 | SPF |
| E - D | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| F - D | 2x6 | DRY | No.2 | SPF |
| F - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- D | 1 | 12 | TOP |
| G- A | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| G- F | 2 | 12 | TOP |
| E- D | 2 | 12 | SIDE(0.0) |
| B- E | 1 | 12 | TOP |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| 2x3 | 1 | 6 | |
| 2x4 | 1 | 6 | |
| 2x6 | 2 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.


Structural component only
DWG# T-2311465

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| D | 1804 | 0 | 1804 | 0 | 0 | MECHANICAL | |
| G | 1036 | 0 | 1036 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| D | 1271 | 859 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 412 / 0 | 0 / 0 |
| G | 728 | 501 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 227 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | -800 / 0 | -91.8 | -91.8 | 0.11 (1) | 6.25 | | A- F | 0 / 732 | 0.09 (1) |
| B- C | -819 / 0 | -91.8 | -91.8 | 0.12 (1) | 6.25 | | F- D | -26 / 0 | 0.00 (1) |
| D- C | -612 / 0 | 0.0 | 0.0 | 0.08 (1) | 6.25 | | F- C | 0 / 1109 | 0.10 (1) |
| G- A | -1010 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | | | | |
| G- F | 0 / 0 | -18.5 | -18.5 | 0.01 (4) | 10.00 | | | | |
| E- F | 0 / 717 | 0.0 | 0.0 | 0.09 (1) | 10.00 | | | | |
| F- B | -398 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | | | | |
| E- H | 0 / 25 | -18.5 | -18.5 | 0.69 (1) | 10.00 | | | | |
| H- D | 0 / 25 | -18.5 | -18.5 | 0.69 (1) | 10.00 | | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| A | 0-0 | -162 | -162 | --- | TOP | VERT | TOTAL | --- | C1 |
| H | 5-6-4 | -1282 | -1282 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.12/1.00 (B-C:1) , BC=0.69/1.00 (D-E:1) , WB=0.10/1.00 (C-F:1) , SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (A) (INPUT = 0.90 )
JSI METAL= 0.11 (A) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T21 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington


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**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMVW-t | MT20 | 5.0 | 8.0 | 2.50 | 2.25 |
| D | BMVW1+p | MT20 | 5.0 | 6.0 | 2.75 | 2.50 |
| E | BMV+p | MT20 | 4.0 | 6.0 | | |
| F | BVMWWW-I | MT20 | 6.0 | 10.0 | 3.25 | 4.00 |
| G | BMV1+p | MT20 | 4.0 | 6.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311465

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T22 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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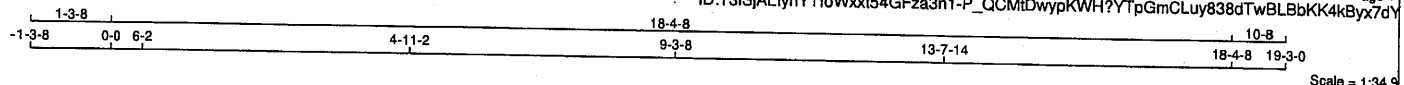


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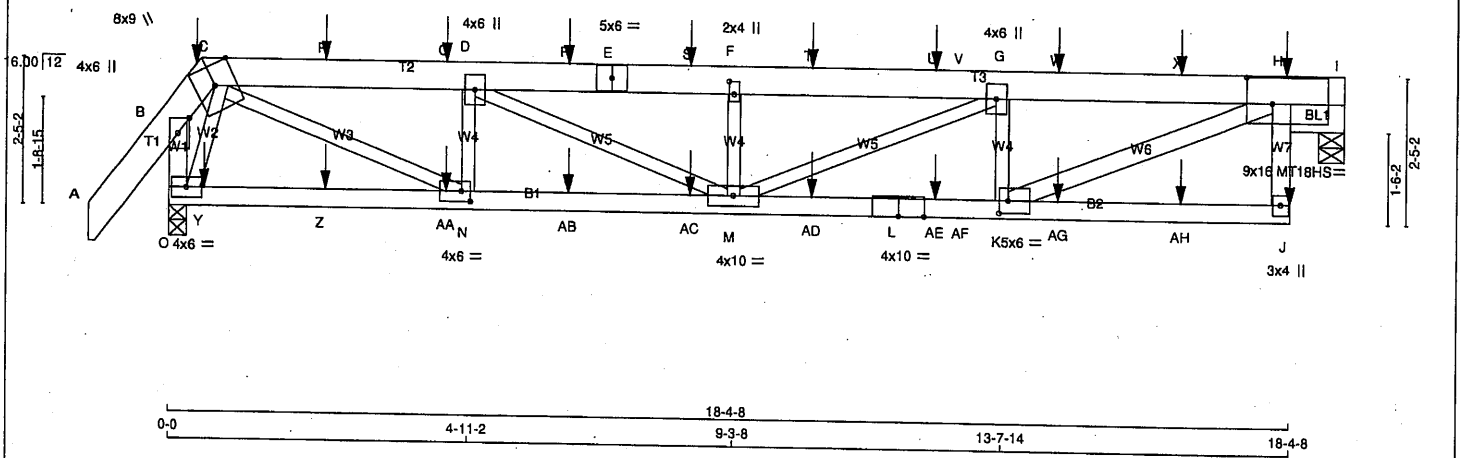





TOTAL WEIGHT = 2 X 87 = 174 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x6 | DRY | No.2 | SPF |
| C - E | 2x6 | DRY | 2100F 1.8E | SPF |
| E - I | 2x6 | DRY | 2100F 1.8E | SPF |
| J - H | 2x4 | DRY | No.2 | SPF |
| O - B | 2x4 | DRY | No.2 | SPF |
| O - L | 2x4 | DRY | No.2 | SPF |
| L - J | 2x4 | DRY | No.2 | SPF |
| BEARING BLOCKS | | | | |
| BL1 | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| K - H | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 2 | 12 | SIDE(13.8) |
| C- E | 2 | 12 | SIDE(175.2) |
| E- I | 2 | 12 | SIDE(166.9) |
| H- J | 1 | 12 | SIDE(8.2) |
| O- B | 1 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| O- L | 1 | 12 | SIDE(54.7) |
| L- J | 1 | 12 | SIDE(56.4) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| D- N | 1 | 6 | SIDE(7.6) |
| 2x3 | 1 | 6 | |
| 2x4 | 1 | 6 | |
| H- I | 2 | 6 | SIDE(20.9) |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.


Structural component only
DWG# T-2311466

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| O | 2827 | 0 | 2827 | 0 | 0 | 3-8 | 1-9 |
| I | 2596 | 0 | 2596 | 0 | 0 | 5-0 | 5-0 |

(** SEE "BEARING NOTE" **)

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): I

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| O | 1995 | 1332 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 663 / 0 | 0 / 0 |
| I | 1834 | 1213 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 621 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 78 | -137.7 | -137.7 | 0.06 (1) | 10.00 | |
| B- C | -175 / 0 | -137.7 | -137.7 | 0.05 (1) | 6.25 | |
| C- P | -4522 / 0 | -137.7 | -137.7 | 0.08 (1) | 6.25 | |
| P- Q | -4522 / 0 | -137.7 | -137.7 | 0.08 (1) | 6.25 | |
| Q- D | -4522 / 0 | -137.7 | -137.7 | 0.08 (1) | 6.25 | |
| D- R | -5688 / 0 | -91.8 | -91.8 | 0.08 (1) | 5.85 | |
| R- E | -5688 / 0 | -91.8 | -91.8 | 0.08 (1) | 5.85 | |
| E- S | -5688 / 0 | -91.8 | -91.8 | 0.08 (1) | 5.85 | |
| S- F | -5688 / 0 | -91.8 | -91.8 | 0.08 (1) | 5.85 | |
| F- T | -5688 / 0 | -91.8 | -91.8 | 0.14 (1) | 5.77 | |
| T- U | -5688 / 0 | -91.8 | -91.8 | 0.14 (1) | 5.77 | |
| U- V | -5688 / 0 | -91.8 | -91.8 | 0.14 (1) | 5.77 | |
| V- G | -5688 / 0 | -137.7 | -137.7 | 0.14 (1) | 5.77 | |
| G- W | -5264 / 0 | -137.7 | -137.7 | 0.54 (1) | 5.27 | |
| W- X | -5264 / 0 | -137.7 | -137.7 | 0.54 (1) | 5.27 | |
| X- H | -5264 / 0 | -137.7 | -137.7 | 0.54 (1) | 5.27 | |
| H- I | -1034 / 0 | -137.7 | -137.7 | 0.21 (1) | 6.25 | |
| J- H | 0 / 448 | 0.0 | 0.0 | 0.04 (1) | 10.00 | |
| O- B | -456 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | |
| O- Y | 0 / 615 | -27.7 | -27.7 | 0.30 (1) | 10.00 | |
| Y- Z | 0 / 615 | -27.7 | -27.7 | 0.30 (1) | 10.00 | |
| Z-AA | 0 / 615 | -27.7 | -27.7 | 0.30 (1) | 10.00 | |
| AA- N | 0 / 615 | -27.7 | -27.7 | 0.30 (1) | 10.00 | |
| N-AB | 0 / 4522 | -18.5 | -18.5 | 0.63 (1) | 10.00 | |
| AB-AC | 0 / 4522 | -18.5 | -18.5 | 0.63 (1) | 10.00 | |
| AC- M | 0 / 4522 | -18.5 | -18.5 | 0.63 (1) | 10.00 | |
| M-AD | 0 / 5264 | -18.5 | -18.5 | 0.68 (1) | 10.00 | |
| AD- L | 0 / 5264 | -18.5 | -18.5 | 0.68 (1) | 10.00 | |
| L-AE | 0 / 5264 | -18.5 | -18.5 | 0.68 (1) | 10.00 | |
| AE-AF | 0 / 5264 | -18.5 | -18.5 | 0.68 (1) | 10.00 | |
| AF- K | 0 / 5264 | -27.7 | -27.7 | 0.68 (1) | 10.00 | |
| K-AG | -32 / 0 | -27.7 | -27.7 | 0.24 (1) | 6.25 | |
| AG-AH | -32 / 0 | -27.7 | -27.7 | 0.24 (1) | 6.25 | |
| AH- J | -32 / 0 | -27.7 | -27.7 | 0.24 (1) | 6.25 | |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| N- D | -1096 / 0 | 0.09 (1) |
| K- H | 0 / 5804 | 0.51 (1) |
| D- M | 0 / 1288 | 0.16 (1) |
| K- G | -1638 / 0 | 0.14 (1) |
| M- F | -212 / 0 | 0.02 (1) |
| M- G | 0 / 468 | 0.06 (1) |
| O- C | -2110 / 0 | 0.18 (1) |
| C- N | 0 / 4328 | 0.54 (1) |

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 964 (0.23")

CSI: TC=0.54/1.00 (G-H:1) , BC=0.68/1.00 (K-M:1) , WB=0.54/1.00 (C-N:1) , SSI=0.39/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1967 | 1873 |
| MT18HS | 586 | 403 | 2455 | 1382 | 3163 | 3004 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | T22 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 4.0 | 6.0 | 3.00 | Edge |
| C | TTWW+m | MT20 | 8.0 | 9.0 | Edge | 4.25 |
| D | TMWW+t | MT20 | 4.0 | 6.0 | | |
| E | TS-t | MT20 | 5.0 | 6.0 | | |
| F | TMW+w | MT20 | 2.0 | 4.0 | 2.50 | 1.00 |
| G | TMWW+t | MT20 | 4.0 | 6.0 | | |
| H | TMWWK-t | MT18HS | 9.0 | 16.0 | 5.25 | 5.25 |
| J | BMV+p | MT20 | 3.0 | 4.0 | | |
| K | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| L | BS-t | MT20 | 4.0 | 10.0 | | |
| M | BMWWW-t | MT20 | 4.0 | 10.0 | | |
| N | BMWW-t | MT20 | 4.0 | 6.0 | 2.00 | 1.75 |
| O | BMVW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| C | 7-8 | -19 | -19 | --- | FRONT | VERT | TOTAL | --- | C1 |
| H | 18-3-12 | -8 | -8 | --- | FRONT | VERT | TOTAL | --- | C1 |
| J | 18-4-8 | -176 | -176 | --- | FRONT | VERT | TOTAL | --- | C1 |
| P | 2-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Q | 4-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| R | 6-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| S | 8-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| T | 10-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| U | 12-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| W | 14-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| X | 16-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Y | 6-14 | -172 | -172 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Z | 2-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AA | 4-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AB | 6-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AC | 8-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AD | 10-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AE | 12-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AG | 14-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AH | 16-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.79 (N) (INPUT = 0.90 )
JSI METAL= 0.63 (L) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311466

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T30 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 16:43:59 2023 Page 1
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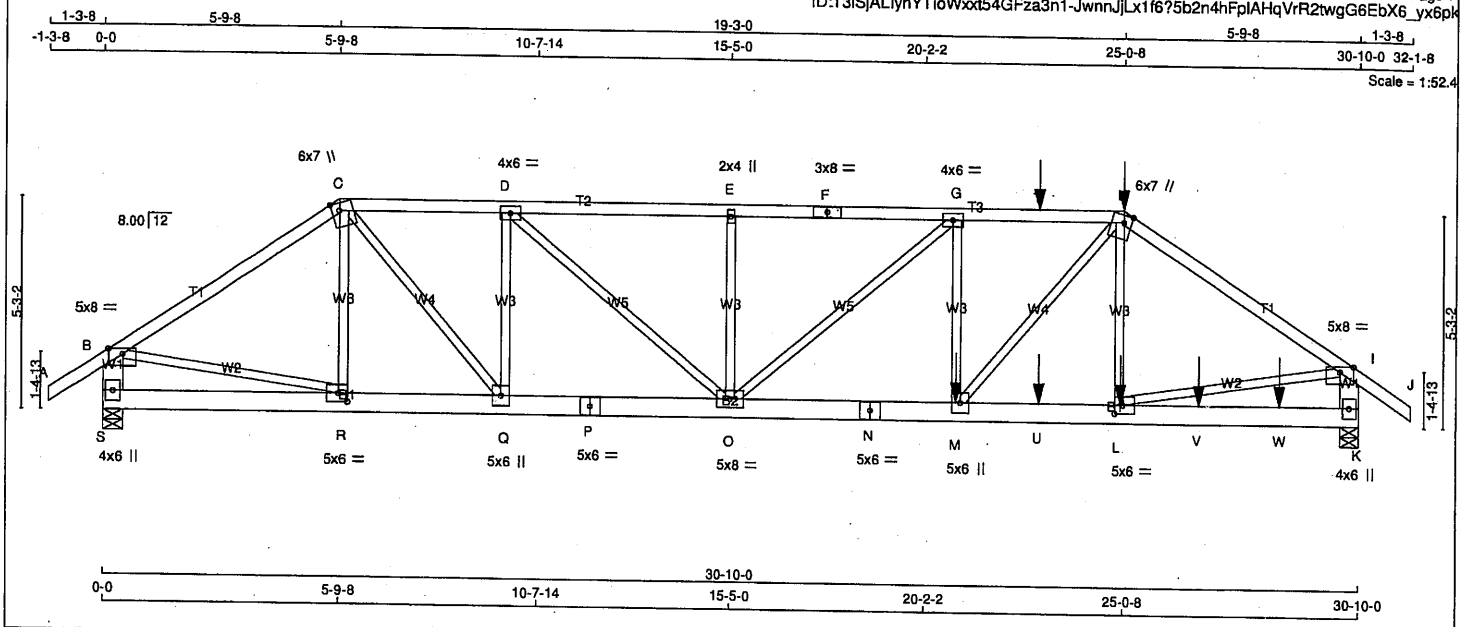





**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| H - J | 2x4 | DRY | No.2 | SPF |
| S - B | 2x6 | DRY | No.2 | SPF |
| K - I | 2x6 | DRY | No.2 | SPF |
| S - P | 2x6 | DRY | No.2 | SPF |
| P - N | 2x6 | DRY | No.2 | SPF |
| N - K | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- F | 1 | 12 | TOP |
| F- H | 1 | 12 | SIDE(61.0) |
| H- J | 1 | 12 | SIDE(61.0) |
| S- B | 2 | 12 | TOP |
| K- I | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| S- P | 2 | 12 | TOP |
| P- N | 2 | 12 | TOP |
| N- K | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| L- H | 1 | 6 | SIDE(2.2) |
| G- M | 1 | 6 | SIDE(358.6) |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.


Structural component only
DWG# T-2311469

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| S | 2647 | 0 | 2647 | 0 | 0 | 5-8 | 1-8 |
| K | 3871 | 0 | 3871 | 0 | 0 | 5-8 | 2-2 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| S | 1867 | 1254 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 613 / 0 | 0 / 0 |
| K | 2732 | 1824 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 908 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 | |
| B- C | -3165 / 0 | -91.8 | -91.8 | 0.40 (1) | 4.82 | |
| C- D | -4035 / 0 | -91.8 | -91.8 | 0.22 (1) | 4.53 | |
| D- E | -5309 / 0 | -91.8 | -91.8 | 0.32 (1) | 3.98 | |
| E- F | -5309 / 0 | -91.8 | -91.8 | 0.35 (1) | 3.95 | |
| F- G | -5309 / 0 | -91.8 | -91.8 | 0.35 (1) | 3.95 | |
| G- T | -5881 / 0 | -91.8 | -91.8 | 0.34 (1) | 3.74 | |
| T- H | -5881 / 0 | -91.8 | -91.8 | 0.34 (1) | 3.74 | |
| H- I | -4883 / 0 | -91.8 | -91.8 | 0.52 (1) | 3.96 | |
| I- J | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 | |
| S- B | -2597 / 0 | 0.0 | 0.0 | 0.09 (1) | 7.81 | |
| K- I | -3793 / 0 | 0.0 | 0.0 | 0.14 (1) | 7.25 | |
| S- R | 0 / 0 | -18.5 | -18.5 | 0.04 (4) | 10.00 | |
| R- Q | 0 / 2623 | -18.5 | -18.5 | 0.18 (1) | 10.00 | |
| Q- P | 0 / 4035 | -18.5 | -18.5 | 0.29 (1) | 10.00 | |
| P- O | 0 / 4035 | -18.5 | -18.5 | 0.29 (1) | 10.00 | |
| O- N | 0 / 5881 | -18.5 | -18.5 | 0.44 (1) | 10.00 | |
| N- M | 0 / 5881 | -18.5 | -18.5 | 0.44 (1) | 10.00 | |
| M- U | 0 / 4048 | -18.5 | -18.5 | 0.31 (1) | 10.00 | |
| U- L | 0 / 4048 | -18.5 | -18.5 | 0.31 (1) | 10.00 | |
| L- V | 0 / 0 | -18.5 | -18.5 | 0.07 (4) | 10.00 | |
| V- W | 0 / 0 | -18.5 | -18.5 | 0.07 (4) | 10.00 | |
| W- K | 0 / 0 | -18.5 | -18.5 | 0.07 (4) | 10.00 | |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| R- C | -343 / 0 | 0.07 (1) |
| C- Q | 0 / 2199 | 0.27 (1) |
| Q- D | -1598 / 0 | 0.32 (1) |
| D- O | 0 / 1698 | 0.21 (1) |
| O- E | -506 / 0 | 0.10 (1) |
| O- G | -762 / 0 | 0.40 (1) |
| M- G | -67 / 45 | 0.01 (1) |
| M- H | 0 / 2853 | 0.35 (1) |
| L- H | -450 / 21 | 0.09 (1) |
| B- R | 0 / 2668 | 0.33 (1) |
| L- I | 0 / 4115 | 0.51 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| H | 25-0-8 | -329 | -329 | --- | BACK | VERT | TOTAL | --- | C1 |
| L | 24-11-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |
| M | 20-11-8 | -1461 | -1461 | --- | BACK | VERT | TOTAL | --- | C1 |
| T | 22-11-12 | -120 | -120 | --- | BACK | VERT | TOTAL | --- | C1 |
| U | 22-11-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |
| V | 26-10-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |
| W | 28-10-12 | -28 | -28 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 148 = 297 lb [M]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 5.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.52/1.00 (H-I:1) , BC=0.44/1.00 (M-O:1) , WB=0.51/1.00 (I-L:1) , SSI=0.14/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90 )
JSI METAL= 0.56 (P) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T30 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| C | TTWW+m | MT20 | 6.0 | 7.0 | 2.25 | 2.25 |
| D | TMWW-t | MT20 | 4.0 | 6.0 | | |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TS-t | MT20 | 3.0 | 8.0 | | |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TTWW+m | MT20 | 6.0 | 7.0 | 2.25 | 2.25 |
| I | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| K | BMV1+p | MT20 | 4.0 | 6.0 | | |
| L | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.50 |
| M | BMWW+t | MT20 | 5.0 | 6.0 | | |
| N | BS-t | MT20 | 5.0 | 6.0 | | |
| O | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| P | BS-t | MT20 | 5.0 | 6.0 | | |
| Q | BMWW+t | MT20 | 5.0 | 6.0 | | |
| R | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.50 |
| S | BMV1+p | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311469

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T31 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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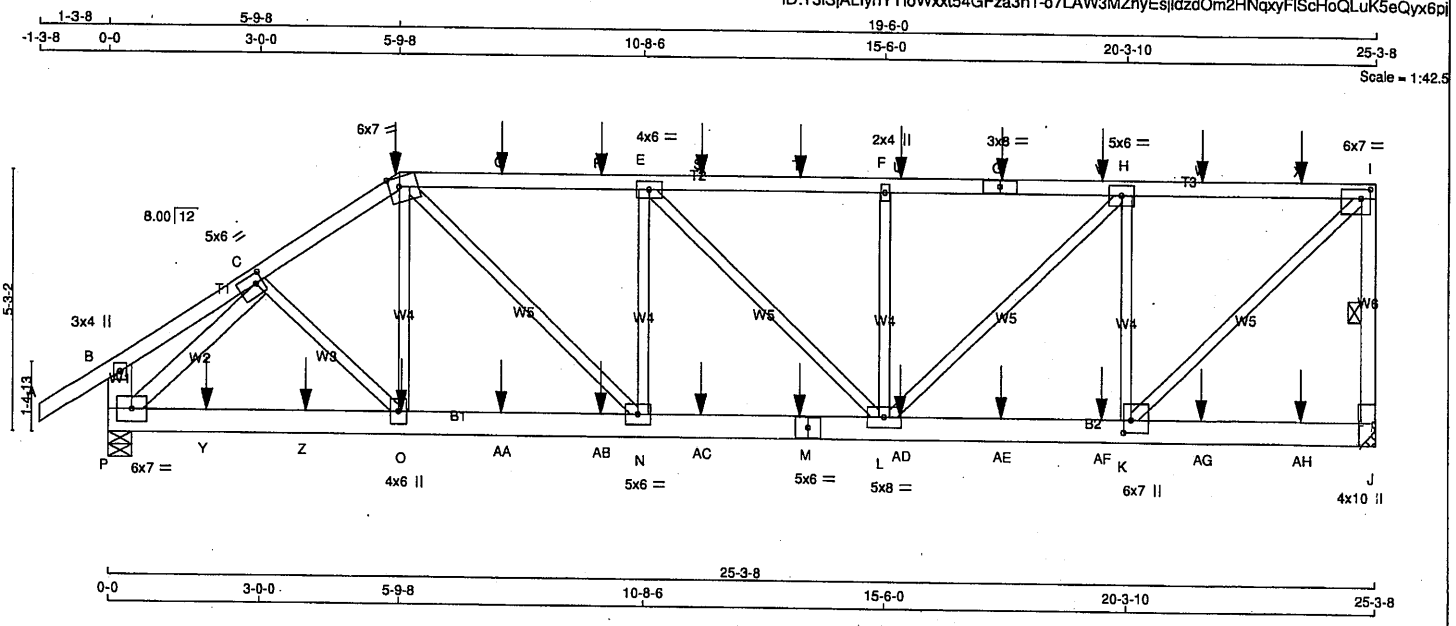


TOTAL WEIGHT = 127 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - G | 2x4 | DRY | 1650F 1.5E | SPF |
| G - I | 2x4 | DRY | 1650F 1.5E | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| P - B | 2x6 | DRY | No.2 | SPF |
| P - M | 2x6 | DRY | No.2 | SPF |
| M - J | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| P - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | 2.25 | 1.75 |
| D | TTWW-m | MT20 | 6.0 | 7.0 | Edge | 2.50 |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TMW+w | MT20 | 2.0 | 4.0 | | |
| G | TS-t | MT20 | 3.0 | 8.0 | | |
| H | TMWW-t | MT20 | 5.0 | 6.0 | | |
| I | TMVW-t | MT20 | 6.0 | 7.0 | 2.25 | 2.25 |
| J | BMV1+t | MT20 | 4.0 | 10.0 | Edge | 0.50 |
| K | BMWW+t | MT20 | 6.0 | 7.0 | 3.00 | 1.75 |
| L | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| M | BS-t | MT20 | 5.0 | 6.0 | | |
| N | BMWW-t | MT20 | 5.0 | 6.0 | | |
| O | BMWW+t | MT20 | 4.0 | 6.0 | | |
| P | BMVW1-t | MT20 | 6.0 | 7.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311470

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| J | 2699 | 0 | 2699 | 0 | 0 | IN-SX | IN-SX |
| P | 2687 | 0 | 2687 | 0 | 0 | MECHANICAL 5-8 | 2-15 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| J | 1908 | 1254 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 654 / 0 | 0 / 0 |
| P | 1900 | 1250 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 650 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 |
| B- C | 0 / 12 | -91.8 | -91.8 | 0.10 (1) | 10.00 |
| C- D | -3199 / 0 | -91.8 | -91.8 | 0.25 (1) | 3.69 |
| D- Q | -3618 / 0 | -91.8 | -91.8 | 0.73 (1) | 3.33 |
| Q- R | -3618 / 0 | -91.8 | -91.8 | 0.73 (1) | 3.33 |
| R- E | -3618 / 0 | -91.8 | -91.8 | 0.73 (1) | 3.33 |
| E- S | -3452 / 0 | -91.8 | -91.8 | 0.71 (1) | 3.41 |
| S- T | -3452 / 0 | -91.8 | -91.8 | 0.71 (1) | 3.41 |
| T- F | -3452 / 0 | -91.8 | -91.8 | 0.71 (1) | 3.41 |
| F- U | -3452 / 0 | -91.8 | -91.8 | 0.72 (1) | 3.39 |
| U- G | -3452 / 0 | -91.8 | -91.8 | 0.72 (1) | 3.39 |
| G- V | -3452 / 0 | -91.8 | -91.8 | 0.72 (1) | 3.39 |
| V- H | -3452 / 0 | -91.8 | -91.8 | 0.72 (1) | 3.39 |
| H- W | -2361 / 0 | -91.8 | -91.8 | 0.66 (1) | 4.09 |
| W- X | -2361 / 0 | -91.8 | -91.8 | 0.66 (1) | 4.09 |
| X- I | -2361 / 0 | -91.8 | -91.8 | 0.66 (1) | 4.09 |
| J- I | -2627 / 0 | 0.0 | 0.0 | 0.40 (1) | 4.19 |
| P- B | -236 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 |
| P- Y | 0 / 2347 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| Y- Z | 0 / 2347 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| Z- O | 0 / 2347 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| O-AA | 0 / 2644 | -18.5 | -18.5 | 0.41 (1) | 10.00 |
| AA-AB | 0 / 2644 | -18.5 | -18.5 | 0.41 (1) | 10.00 |
| AB- N | 0 / 2644 | -18.5 | -18.5 | 0.41 (1) | 10.00 |
| N-AC | 0 / 3618 | -18.5 | -18.5 | 0.55 (1) | 10.00 |
| AC- M | 0 / 3618 | -18.5 | -18.5 | 0.55 (1) | 10.00 |
| M- L | 0 / 3618 | -18.5 | -18.5 | 0.55 (1) | 10.00 |
| L-AD | 0 / 2361 | -18.5 | -18.5 | 0.38 (1) | 10.00 |
| AD-AE | 0 / 2361 | -18.5 | -18.5 | 0.38 (1) | 10.00 |
| AE-AF | 0 / 2361 | -18.5 | -18.5 | 0.38 (1) | 10.00 |
| AF- K | 0 / 2361 | -18.5 | -18.5 | 0.38 (1) | 10.00 |
| K-AG | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| O- D | -64 / 127 | 0.05 (4) |
| C- O | 0 / 415 | 0.10 (1) |
| P- C | -3340 / 0 | 0.65 (1) |
| K- I | 0 / 3306 | 0.82 (1) |
| D- N | 0 / 1373 | 0.34 (1) |
| K- H | -2110 / 0 | 0.84 (1) |
| N- E | -803 / 0 | 0.32 (1) |
| L- H | 0 / 1556 | 0.39 (1) |
| E- L | -237 / 0 | 0.20 (1) |
| L- F | -791 / 0 | 0.31 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.73/1.00 (D-E:1) , BC=0.55/1.00 (L-N:1) , WB=0.84/1.00 (H-K:1) , SSI=0.46/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (L) (INPUT = 0.90 )
JSI METAL= 0.76 (C) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T31 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| AG-AH | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | |
| AH- J | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| D | 5-9-8 | -335 | -335 | --- | FRONT | VERT | TOTAL | --- | C1 |
| G | 17-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| M | 13-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| O | 5-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Q | 7-10-4 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| R | 9-10-4 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| S | 11-10-4 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| T | 13-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| U | 15-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| V | 19-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| W | 21-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| X | 23-9-12 | -120 | -120 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Y | 1-11-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Z | 3-11-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AA | 7-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AB | 9-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AC | 11-10-4 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AD | 15-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AE | 17-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AF | 19-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AG | 21-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AH | 23-9-12 | -28 | -28 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.


Structural component only
DWG# T-2311470

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T32 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 16:44:01 2023 Page 1
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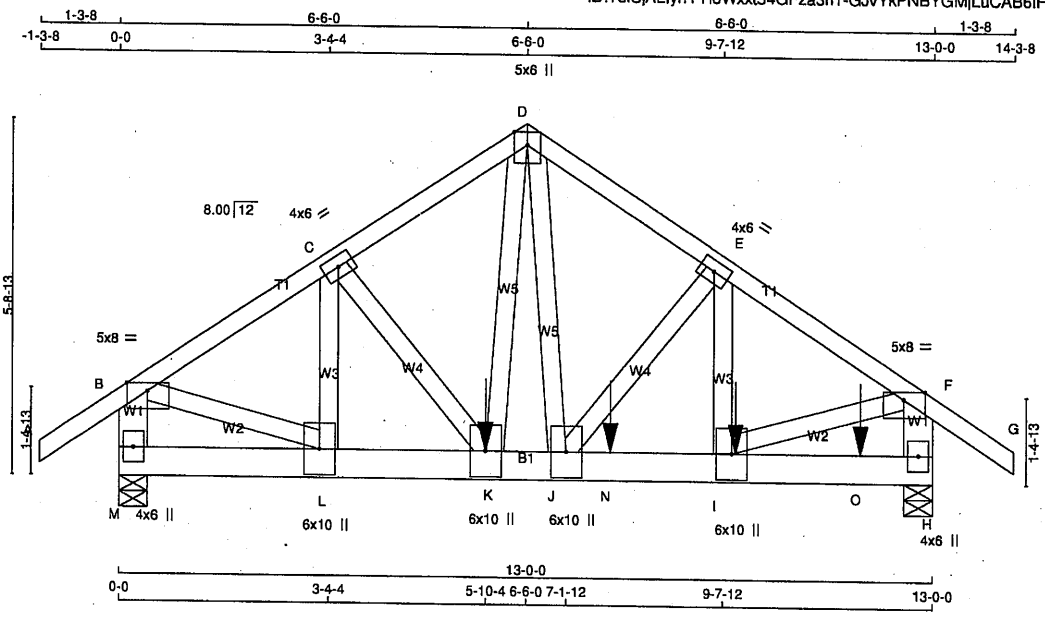


TOTAL WEIGHT = 2 X 81 = 163 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - G | 2x4 | DRY | No.2 | SPF |
| M - B | 2x6 | DRY | No.2 | SPF |
| H - F | 2x6 | DRY | No.2 | SPF |
| M - H | 2x6 | DRY | No.2 | SPF |
| ALL WEBS | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- D | 1 | 12 | TOP |
| D- G | 1 | 12 | TOP |
| M- B | 2 | 12 | TOP |
| H- F | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| M- H | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| D- K | 1 | 5 | SIDE(516.1) |
| I- E | 1 | 6 | SIDE(230.7) |
| 2x4 | 1 | 5 | |
| B- L | 1 | 6 | |
| L- C | 1 | 6 | |
| C- K | 1 | 6 | |
| F- I | 1 | 6 | |
| E- J | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311471

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| M | 3382 | 0 | 3382 | 0 | 0 | 5-8 | 1-13 |
| H | 5523 | 0 | 5523 | 0 | 0 | 5-8 | 3-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| M | 2387 | 1593 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 794 / 0 | 0 / 0 |
| H | 3899 | 2599 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 1299 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| FR-TO | | | | | | | | |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 | K- D | 0 / 1984 | 0.18 (1) |
| B- C | -3816 / 0 | -91.8 | -91.8 | 0.12 (1) | 4.75 | D- J | 0 / 2465 | 0.22 (1) |
| C- D | -4198 / 0 | -91.8 | -91.8 | 0.13 (1) | 4.57 | B- L | 0 / 3324 | 0.29 (1) |
| D- E | -4251 / 0 | -91.8 | -91.8 | 0.13 (1) | 4.53 | L- C | -907 / 0 | 0.07 (1) |
| E- F | -5340 / 0 | -91.8 | -91.8 | 0.18 (1) | 4.07 | C- K | 0 / 467 | 0.04 (1) |
| F- G | 0 / 35 | -91.8 | -91.8 | 0.07 (1) | 10.00 | I- F | 0 / 4644 | 0.41 (1) |
| M- B | -3327 / 0 | 0.0 | 0.0 | 0.12 (1) | 7.62 | I- E | 0 / 1416 | 0.13 (1) |
| H- F | -4552 / 0 | 0.0 | 0.0 | 0.16 (1) | 6.75 | J- E | -1464 / 0 | 0.15 (1) |
| M- L | 0 / 0 | -18.5 | -18.5 | 0.03 (1) | 10.00 | | | |
| L- K | 0 / 3186 | -18.5 | -18.5 | 0.25 (1) | 10.00 | | | |
| K- J | 0 / 3290 | -18.5 | -18.5 | 0.32 (1) | 10.00 | | | |
| J- N | 0 / 4452 | -18.5 | -18.5 | 0.48 (1) | 10.00 | | | |
| N- I | 0 / 4452 | -18.5 | -18.5 | 0.48 (1) | 10.00 | | | |
| I- O | 0 / 0 | -18.5 | -18.5 | 0.40 (1) | 10.00 | | | |
| O- H | 0 / 0 | -18.5 | -18.5 | 0.40 (1) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| I | 9-10-4 | -1120 | -1120 | --- | BACK | VERT | TOTAL | --- | C1 |
| K | 5-10-4 | -1893 | -1893 | --- | BACK | VERT | TOTAL | --- | C1 |
| N | 7-10-4 | -971 | -971 | --- | BACK | VERT | TOTAL | --- | C1 |
| O | 11-10-4 | -1114 | -1114 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.18/1.00 (E-F:1) , BC=0.48/1.00 (I-J:1) , WB=0.41/1.00 (F-I:1) , SSI=0.32/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.90 )
JSI METAL= 0.67 (L) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T32 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



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**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW+p | MT20 | 5.0 | 6.0 | Edge | |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| H | BMV1+p | MT20 | 4.0 | 6.0 | | |
| I, J, K, L | | | | | | |
| I | BMWW+t | MT20 | 6.0 | 10.0 | | |
| M | BMV1+p | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100069024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311471

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T33S | 4 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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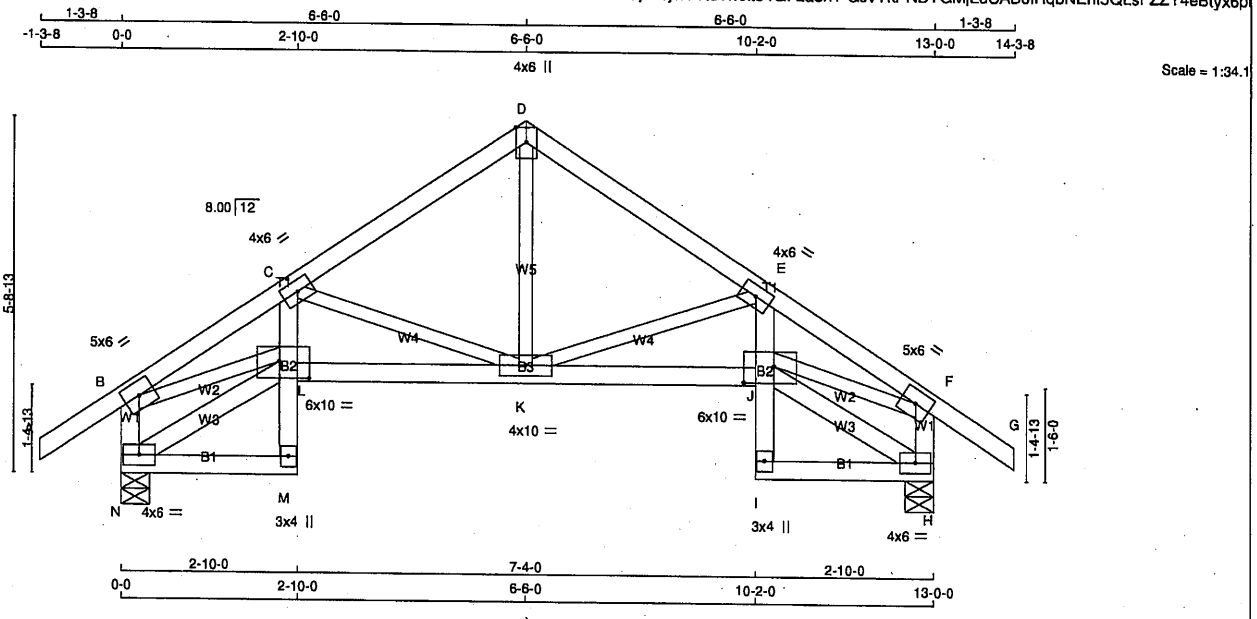


TOTAL WEIGHT = 4 X 64 = 256 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - G | 2x4 | DRY | No.2 | SPF |
| N - B | 2x4 | DRY | No.2 | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |
| N - M | 2x4 | DRY | No.2 | SPF |
| M - C | 2x4 | DRY | No.2 | SPF |
| L - J | 2x4 | DRY | No.2 | SPF |
| I - E | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| N - L | 2x4 | DRY | No.2 | SPF |
| J - H | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | | |
| C | TMVW-t | MT20 | 4.0 | 6.0 | | |
| D | TTW+p | MT20 | 4.0 | 6.0 | Edge | |
| E | TMVW-t | MT20 | 4.0 | 6.0 | | |
| F | TMVW-t | MT20 | 5.0 | 6.0 | | |
| H | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| I | BMV+p | MT20 | 3.0 | 4.0 | | |
| J | BVMWW-t | MT20 | 6.0 | 10.0 | 3.25 | 5.75 |
| K | BMWWW-t | MT20 | 4.0 | 10.0 | | |
| L | BVMWW-t | MT20 | 6.0 | 10.0 | 3.25 | 5.75 |
| M | BMV+p | MT20 | 3.0 | 4.0 | | |
| N | BMVW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| N | 1046 | 0 | 1046 | 0 | 0 | 5-8 | 1-8 |
| H | 1046 | 0 | 1046 | 0 | 0 | 5-8 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| N | 756 | 403 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 353 / 0 | 0 / 0 |
| H | 756 | 403 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 353 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 | K- D | 0 / 825 | 0.22 (4) | |
| B- C | -1783 / 0 | -91.8 | -91.8 | 0.17 (1) | 4.81 | C- K | -746 / 0 | 0.22 (1) | |
| C- D | -1032 / 0 | -91.8 | -91.8 | 0.21 (1) | 5.89 | K- E | -746 / 0 | 0.22 (1) | |
| D- E | -1032 / 0 | -91.8 | -91.8 | 0.21 (1) | 5.89 | N- L | -55 / 0 | 0.01 (1) | |
| E- F | -1783 / 0 | -91.8 | -91.8 | 0.17 (1) | 4.81 | B- L | 0 / 1516 | 0.38 (1) | |
| F- G | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 | J- H | -55 / 0 | 0.01 (1) | |
| N- B | -983 / 0 | 0.0 | 0.0 | 0.11 (1) | 7.81 | J- F | 0 / 1516 | 0.38 (1) | |
| H- F | -983 / 0 | 0.0 | 0.0 | 0.11 (1) | 7.81 | | | | |
| N- M | 0 / 48 | -18.5 | -44.3 | 0.08 (4) | 10.00 | | | | |
| M- L | 0 / 54 | 0.0 | 0.0 | 0.12 (1) | 10.00 | | | | |
| L- C | 0 / 334 | 0.0 | 0.0 | 0.17 (1) | 10.00 | | | | |
| L- K | 0 / 1550 | -44.3 | -81.0 | 0.51 (4) | 10.00 | | | | |
| K- J | 0 / 1550 | -81.0 | -44.3 | 0.51 (4) | 10.00 | | | | |
| I- J | 0 / 54 | 0.0 | 0.0 | 0.12 (1) | 10.00 | | | | |
| J- E | 0 / 334 | 0.0 | 0.0 | 0.17 (1) | 10.00 | | | | |
| I- H | 0 / 48 | -44.3 | -18.5 | 0.08 (4) | 10.00 | | | | |

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.51/1.00 (J-K:4) , WB=0.38/1.00 (F-J:1) , SSI=0.24/1.00 (K-L:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (K) (INPUT = 0.90 )
JSI METAL= 0.44 (B) (INPUT = 0.95 )


Structural component only
DWG# T-2311472

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T34 | 3 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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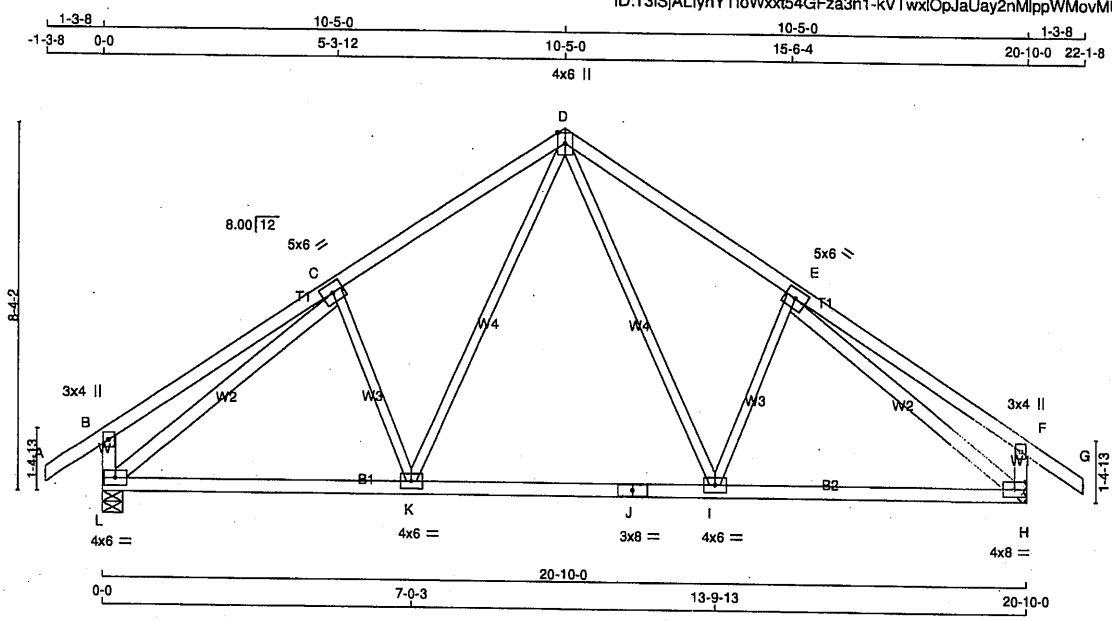



TOTAL WEIGHT = 3 X 95 = 284 lb [M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - G | 2x4 | DRY | No.2 | SPF |
| L - B | 2x4 | DRY | No.2 | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |
| L - J | 2x4 | DRY | No.2 | SPF |
| J - H | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| L - C | 2x4 | DRY | No.2 | SPF |
| E - H | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | | |
| D | TTWW+p | MT20 | 4.0 | 6.0 | Edge | |
| E | TMWW-t | MT20 | 5.0 | 6.0 | | |
| F | TMV+p | MT20 | 3.0 | 4.0 | | |
| H | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| I | BMWW-t | MT20 | 4.0 | 6.0 | | |
| J | BS-t | MT20 | 3.0 | 8.0 | | |
| K | BMWW-t | MT20 | 4.0 | 6.0 | | |
| L | BMVW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| L | 1274 | 0 | 1274 | 0 | 0 | 5-8 | 1-8 |
| H | 1274 | 0 | 1274 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| L | 899 | 603 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 296 / 0 | 0 / 0 |
| H | 899 | 603 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 296 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | |
| B- C | 0 / 32 | -91.8 | -91.8 | 0.41 (1) | 10.00 | |
| C- D | -1144 / 0 | -91.8 | -91.8 | 0.34 (1) | 5.54 | |
| D- E | -1144 / 0 | -91.8 | -91.8 | 0.34 (1) | 5.54 | |
| E- F | 0 / 32 | -91.8 | -91.8 | 0.41 (1) | 10.00 | |
| F- G | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | |
| L- B | -305 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | |
| H- F | -305 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | |
| L- K | 0 / 1046 | -18.5 | -18.5 | 0.29 (4) | 10.00 | |
| K- J | 0 / 742 | -18.5 | -18.5 | 0.26 (4) | 10.00 | |
| J- I | 0 / 742 | -18.5 | -18.5 | 0.26 (4) | 10.00 | |
| I- H | 0 / 1046 | -18.5 | -18.5 | 0.29 (4) | 10.00 | |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| D- I | 0 / 470 | 0.11 (1) |
| I- E | -317 / 0 | 0.13 (1) |
| K- D | 0 / 470 | 0.11 (1) |
| C- K | -317 / 0 | 0.13 (1) |
| L- C | -1390 / 0 | 0.86 (1) |
| E- H | -1390 / 0 | 0.86 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.29/1.00 (K-L:4) , WB=0.86/1.00 (C-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

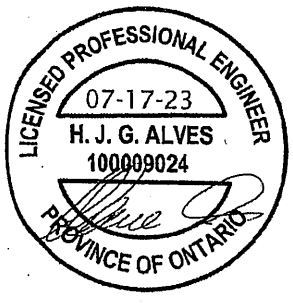
NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90 )
JSI METAL= 0.33 (C) (INPUT = 0.95 )




Structural component only
DWG# T-2311473

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | T34S | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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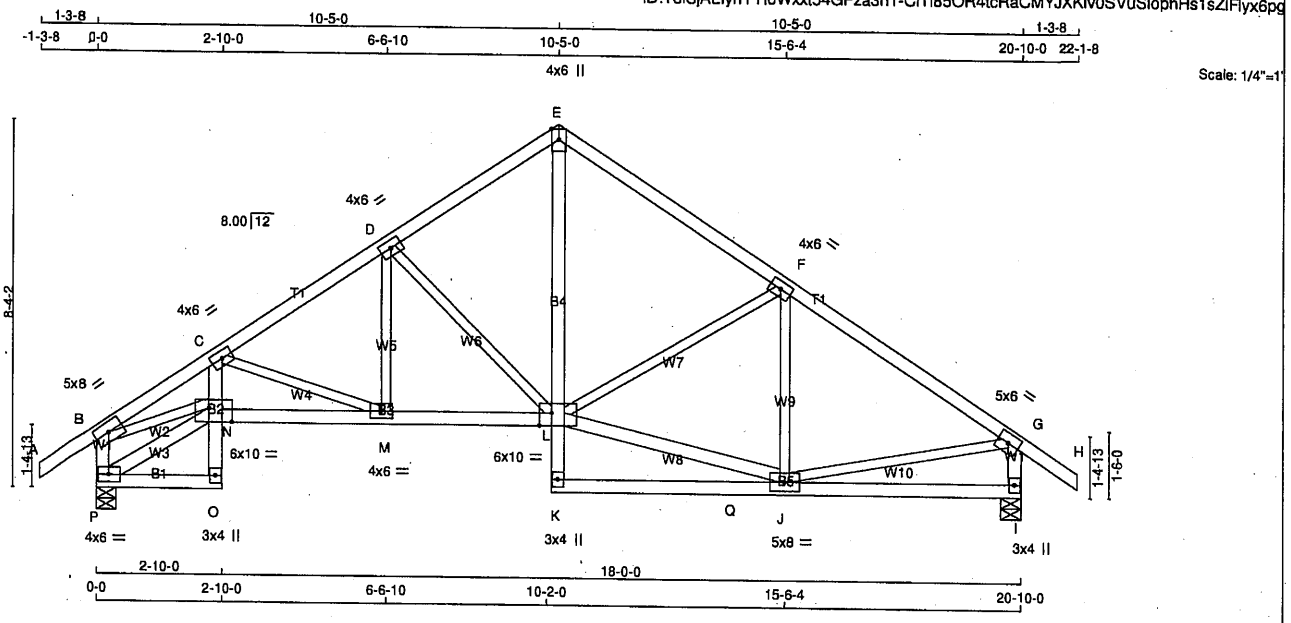


TOTAL WEIGHT = 104 lb

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - E | 2x4 | DRY | No.2 | SPF |
| E - H | 2x4 | DRY | No.2 | SPF |
| P - B | 2x4 | DRY | No.2 | SPF |
| I - G | 2x4 | DRY | No.2 | SPF |
| P - O | 2x4 | DRY | No.2 | SPF |
| O - C | 2x4 | DRY | No.2 | SPF |
| N - L | 2x4 | DRY | No.2 | SPF |
| K - E | 2x4 | DRY | No.2 | SPF |
| K - I | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| P - N | 2x4 | DRY | No.2 | SPF |
| L - J | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES** (table is in inches)

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 8.0 | | |
| C | TMVW-t | MT20 | 4.0 | 6.0 | | |
| D | TMWW-t | MT20 | 4.0 | 6.0 | | |
| E | TTV+p | MT20 | 4.0 | 6.0 | Edge | |
| F | TMWW-t | MT20 | 4.0 | 6.0 | | |
| G | TMVW-t | MT20 | 5.0 | 6.0 | | |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |
| J | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| K | BMV+p | MT20 | 3.0 | 4.0 | | |
| L | BVMWWW-t | MT20 | 6.0 | 10.0 | Edge | 3.25 |
| M | BMWW-t | MT20 | 4.0 | 6.0 | | |
| N | BVMWW-t | MT20 | 6.0 | 10.0 | 3.75 | 6.25 |
| O | BMV+p | MT20 | 3.0 | 4.0 | | |
| P | BMVW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311474

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| I | 1517 | 0 | 1517 | 0 | 0 | 5-8 | 1-10 |
| P | 1675 | 0 | 1675 | 0 | 0 | 5-8 | 1-13 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| I | 1093 | 603 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 490 / 0 | 0 / 0 |
| P | 1219 | 603 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 616 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 | J- F | -505 / 0 | 0.19 (1) |
| B- C | -3171 / 0 | -91.8 | -91.8 | 0.25 (1) | 3.69 | J- G | 0 / 1372 | 0.34 (1) |
| C- D | -2298 / 0 | -91.8 | -91.8 | 0.33 (1) | 4.17 | P- N | -97 / 0 | 0.01 (1) |
| D- E | -1559 / 0 | -91.8 | -91.8 | 0.28 (1) | 4.95 | B- N | 0 / 2673 | 0.66 (1) |
| E- F | -1566 / 0 | -91.8 | -91.8 | 0.55 (1) | 4.55 | C- M | -853 / 0 | 0.25 (1) |
| F- G | -1585 / 0 | -91.8 | -91.8 | 0.56 (1) | 4.53 | M- D | 0 / 668 | 0.22 (4) |
| G- H | 0 / 35 | -91.8 | -91.8 | 0.14 (1) | 10.00 | D- L | -928 / 0 | 0.48 (1) |
| P- B | -1592 / 0 | 0.0 | 0.0 | 0.18 (1) | 6.50 | L- J | 0 / 1386 | 0.25 (1) |
| I- G | -1483 / 0 | 0.0 | 0.0 | 0.17 (1) | 6.69 | L- F | -80 / 44 | 0.05 (1) |
| P- O | 0 / 84 | -18.5 | -44.3 | 0.09 (4) | 10.00 | | | |
| O- N | 0 / 54 | 0.0 | 0.0 | 0.20 (1) | 10.00 | | | |
| N- C | 0 / 489 | 0.0 | 0.0 | 0.28 (1) | 10.00 | | | |
| N- M | 0 / 2733 | -44.3 | -81.4 | 0.77 (4) | 10.00 | | | |
| M- L | 0 / 1929 | -81.4 | -118.5 | 0.66 (4) | 10.00 | | | |
| K- L | 0 / 135 | 0.0 | 0.0 | 0.06 (4) | 10.00 | | | |
| L- E | 0 / 1389 | 0.0 | 0.0 | 0.28 (1) | 10.00 | | | |
| K- Q | 0 / 15 | -81.0 | -18.5 | 0.26 (4) | 10.00 | | | |
| Q- J | 0 / 15 | -18.5 | -18.5 | 0.26 (4) | 10.00 | | | |
| J- I | 0 / 0 | -18.5 | -18.5 | 0.23 (4) | 10.00 | | | |

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.56/1.00 (F-G:1) , BC=0.77/1.00 (M-N:4) , WB=0.66/1.00 (B-N:1) , SSI=0.32/1.00 (L-M:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (N) (INPUT = 0.90 )
JSI METAL= 0.59 (B) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T41S | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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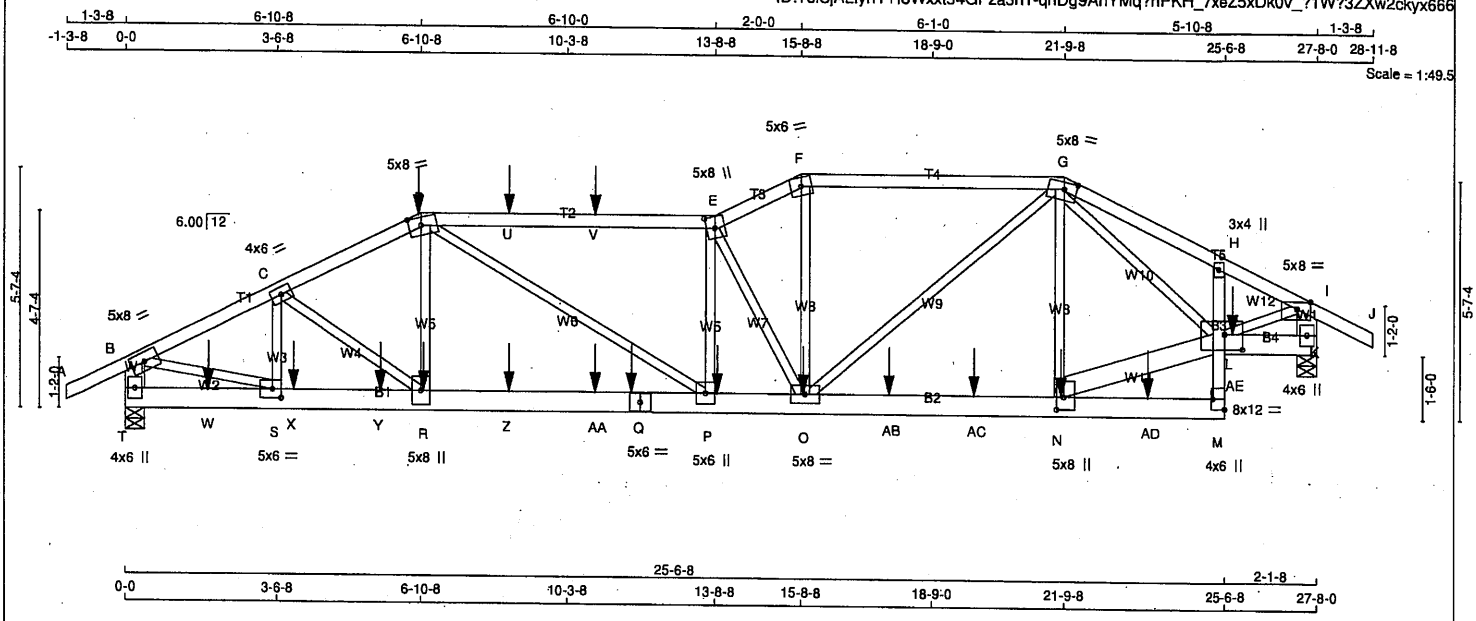





TOTAL WEIGHT = 2 X 142 = 284 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - E | 2x4 | DRY | No.2 | SPF |
| E - F | 2x4 | DRY | No.2 | SPF |
| F - G | 2x4 | DRY | No.2 | SPF |
| G - J | 2x4 | DRY | No.2 | SPF |
| T - B | 2x6 | DRY | No.2 | SPF |
| K - I | 2x6 | DRY | No.2 | SPF |
| T - Q | 2x6 | DRY | No.2 | SPF |
| Q - M | 2x6 | DRY | No.2 | SPF |
| M - H | 2x4 | DRY | No.2 | SPF |
| L - K | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| N - L | 2x6 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- D | 1 | 12 | SIDE(61.0) |
| D- E | 1 | 12 | SIDE(61.0) |
| E- F | 1 | 12 | TOP |
| F- G | 1 | 12 | TOP |
| G- J | 1 | 12 | TOP |
| T- B | 2 | 12 | TOP |
| K- I | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| T- Q | 2 | 12 | SIDE(183.1) |
| Q- M | 2 | 12 | SIDE(183.1) |
| L- K | 2 | 12 | SIDE(0.0) |
| M- H | 1 | 12 | TOP |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| R- D | 1 | 6 | SIDE(5.5) |
| 2x3 | 1 | 6 | |
| 2x6 | 2 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.


Structural component only
DWG# T-2311490

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| T | 3712 | 0 | 3712 | 0 | 0 | 5-8 | 2-0 |
| K | 4034 | 0 | 4034 | 0 | 0 | 5-8 | 2-3 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| T | 2620 | 1746 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 874 / 0 | 0 / 0 |
| K | 2840 | 1932 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 908 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. | FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| FR-TO | | | | | | FR-TO | | |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.07 (1) | 10.00 | S- C | -1128 / 0 | 0.10 (1) |
| B- C | -5155 / 0 | -91.8 | -91.8 | 0.18 (1) | 4.13 | C- R | 0 / 757 | 0.09 (1) |
| C- D | -5861 / 0 | -91.8 | -91.8 | 0.19 (1) | 3.89 | R- D | 0 / 667 | 0.08 (1) |
| D- U | -6823 / 0 | -91.8 | -91.8 | 0.94 (1) | 2.98 | D- P | 0 / 1864 | 0.23 (1) |
| U- V | -6823 / 0 | -91.8 | -91.8 | 0.94 (1) | 2.98 | P- E | -128 / 35 | 0.02 (1) |
| V- E | -6823 / 0 | -91.8 | -91.8 | 0.94 (1) | 2.98 | E- O | -3328 / 0 | 0.61 (1) |
| E- F | -5972 / 0 | -91.8 | -91.8 | 0.15 (1) | 3.90 | O- F | 0 / 2301 | 0.28 (1) |
| F- G | -5388 / 0 | -91.8 | -91.8 | 0.54 (1) | 3.72 | O- G | 0 / 2177 | 0.27 (1) |
| G- H | -4876 / 0 | -91.8 | -91.8 | 0.21 (1) | 4.21 | N- G | -309 / 0 | 0.07 (1) |
| H- I | -4968 / 0 | -91.8 | -91.8 | 0.12 (1) | 4.26 | N- L | 0 / 3940 | 0.26 (1) |
| I- J | 0 / 28 | -91.8 | -91.8 | 0.07 (1) | 10.00 | G- L | 0 / 865 | 0.11 (1) |
| T- B | -3585 / 0 | 0.0 | 0.0 | 0.13 (1) | 7.41 | B- S | 0 / 4728 | 0.59 (1) |
| K- I | -3959 / 0 | 0.0 | 0.0 | 0.14 (1) | 7.13 | L- I | 0 / 4697 | 0.58 (1) |
| T- W | 0 / 0 | -18.5 | -18.5 | 0.10 (1) | 10.00 | | | |
| W- S | 0 / 0 | -18.5 | -18.5 | 0.10 (1) | 10.00 | | | |
| S- X | 0 / 4619 | -18.5 | -18.5 | 0.50 (1) | 10.00 | | | |
| X- Y | 0 / 4619 | -18.5 | -18.5 | 0.50 (1) | 10.00 | | | |
| Y- R | 0 / 4619 | -18.5 | -18.5 | 0.50 (1) | 10.00 | | | |
| R- Z | 0 / 5244 | -18.5 | -18.5 | 0.58 (1) | 10.00 | | | |
| Z- AA | 0 / 5244 | -18.5 | -18.5 | 0.58 (1) | 10.00 | | | |
| AA- Q | 0 / 5244 | -18.5 | -18.5 | 0.58 (1) | 10.00 | | | |
| Q- P | 0 / 5244 | -18.5 | -18.5 | 0.58 (1) | 10.00 | | | |
| P- O | 0 / 6826 | -18.5 | -18.5 | 0.54 (1) | 10.00 | | | |
| O- AB | 0 / 3748 | -18.5 | -18.5 | 0.44 (1) | 10.00 | | | |
| AB- AC | 0 / 3748 | -18.5 | -18.5 | 0.44 (1) | 10.00 | | | |
| AC- N | 0 / 3748 | -18.5 | -18.5 | 0.44 (1) | 10.00 | | | |
| N- AD | 0 / 102 | -18.5 | -18.5 | 0.21 (1) | 10.00 | | | |
| AD- M | 0 / 102 | -18.5 | -18.5 | 0.21 (1) | 10.00 | | | |
| M- L | 0 / 86 | 0.0 | 0.0 | 0.12 (1) | 10.00 | | | |
| L- H | -246 / 0 | 0.0 | 0.0 | 0.11 (1) | 7.81 | | | |
| L- AE | 0 / 0 | -18.5 | -18.5 | 0.04 (1) | 10.00 | | | |
| AE- K | 0 / 0 | -18.5 | -18.5 | 0.04 (1) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| D | 6-10-8 | -337 | -337 | --- | FRONT | VERT | TOTAL | --- | C1 |
| N | 21-8-12 | -253 | -253 | --- | FRONT | VERT | TOTAL | --- | C1 |
| O | 15-9-4 | -253 | -253 | --- | FRONT | VERT | TOTAL | --- | C1 |
| P | 13-9-4 | -253 | -253 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Q | 11-9-4 | -245 | -245 | --- | FRONT | VERT | TOTAL | --- | C1 |
| R | 6-11-4 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.94/1.00 (D-E:1) , BC=0.58/1.00 (P-R:1) , WB=0.61/1.00 (E-O:1) , SSI=0.16/1.00 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90 )
JSI METAL= 0.70 (Q) (INPUT = 0.95 )



CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T41S | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



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**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 8.0 | | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW-m | MT20 | 5.0 | 8.0 | 2.25 | 3.50 |
| E | TTWW+m | MT20 | 5.0 | 6.0 | 3.25 | 2.25 |
| F | TTW-m | MT20 | 5.0 | 6.0 | | |
| G | TTWWW-m | MT20 | 5.0 | 8.0 | 2.00 | 3.50 |
| H | TMV+p | MT20 | 3.0 | 4.0 | | |
| I | TMVW-p | MT20 | 5.0 | 8.0 | Edge | 3.75 |
| K | BMV1+p | MT20 | 4.0 | 6.0 | | |
| L | BVMWWW-l | MT20 | 8.0 | 12.0 | 4.25 | 5.00 |
| M | BMV+p | MT20 | 4.0 | 6.0 | 3.00 | Edge |
| N | BMWW+t | MT20 | 5.0 | 8.0 | 3.50 | 2.00 |
| O | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| P | BMWW+t | MT20 | 5.0 | 6.0 | | |
| Q | BS-t | MT20 | 5.0 | 6.0 | | |
| R | BMWW+t | MT20 | 5.0 | 8.0 | | |
| S | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.50 |
| T | BMV1+p | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| U | 8-11-4 | -23 | -23 | -- | FRONT | VERT | TOTAL | -- | C1 |
| V | 10-11-4 | -23 | -23 | -- | FRONT | VERT | TOTAL | -- | C1 |
| W | 1-11-4 | -36 | -36 | -- | FRONT | VERT | TOTAL | -- | C1 |
| X | 3-11-4 | -36 | -36 | -- | FRONT | VERT | TOTAL | -- | C1 |
| Y | 5-11-4 | -149 | -149 | -- | FRONT | VERT | TOTAL | -- | C1 |
| Z | 8-11-4 | -169 | -169 | -- | FRONT | VERT | TOTAL | -- | C1 |
| AA | 10-11-4 | -169 | -169 | -- | FRONT | VERT | TOTAL | -- | C1 |
| AB | 17-9-4 | -253 | -253 | -- | FRONT | VERT | TOTAL | -- | C1 |
| AC | 19-8-12 | -253 | -253 | -- | FRONT | VERT | TOTAL | -- | C1 |
| AD | 23-8-12 | -253 | -253 | -- | FRONT | VERT | TOTAL | -- | C1 |
| AE | 25-8-12 | -256 | -256 | -- | FRONT | VERT | TOTAL | -- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311490

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T42S | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:40 2023 Page 1
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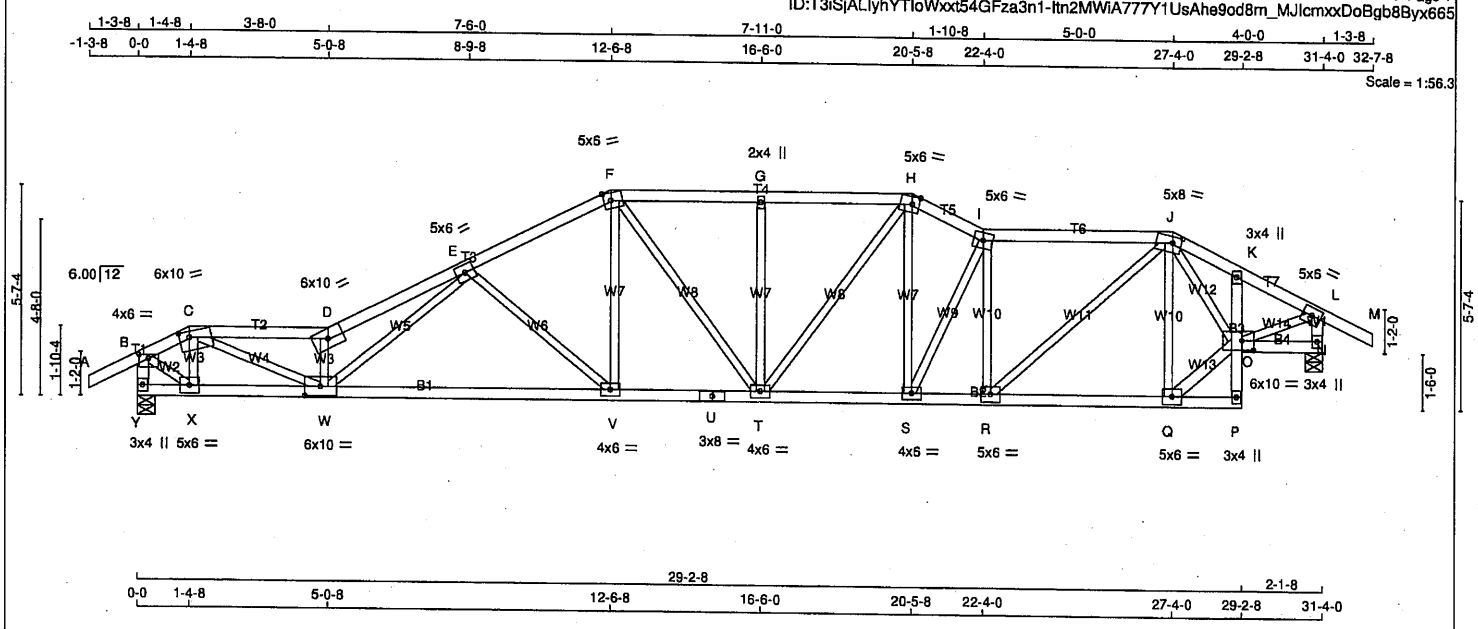


TOTAL WEIGHT = 141 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| H - I | 2x4 | DRY | No.2 | SPF |
| I - J | 2x4 | DRY | No.2 | SPF |
| J - M | 2x4 | DRY | No.2 | SPF |
| Y - B | 2x4 | DRY | No.2 | SPF |
| N - L | 2x4 | DRY | No.2 | SPF |
| Y - U | 2x4 | DRY | No.2 | SPF |
| U - P | 2x4 | DRY | No.2 | SPF |
| P - K | 2x4 | DRY | No.2 | SPF |
| O - N | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| Q - O | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES** (table is in inches)

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-p | MT20 | 4.0 | 6.0 | 1.25 | 3.00 |
| C | TTWW-m | MT20 | 6.0 | 10.0 | 1.75 | 3.25 |
| D | TTW-h | MT20 | 6.0 | 10.0 | | |
| E | TMWW-t | MT20 | 5.0 | 6.0 | | |
| F | TTWW-m | MT20 | 5.0 | 6.0 | 2.50 | 2.25 |
| G | TMW+w | MT20 | 2.0 | 4.0 | | |
| H | TTWW-m | MT20 | 5.0 | 6.0 | 2.50 | 2.25 |
| I | TTWW-m | MT20 | 5.0 | 6.0 | | |
| J | TTWWW-m | MT20 | 5.0 | 8.0 | 2.00 | 2.75 |
| K | TMV+p | MT20 | 3.0 | 4.0 | | |
| L | TMVW-t | MT20 | 5.0 | 6.0 | | |
| N | BMV1+p | MT20 | 3.0 | 4.0 | | |
| O | BVMWWW-l | MT20 | 6.0 | 10.0 | 3.00 | 3.75 |
| P | BMV+p | MT20 | 3.0 | 4.0 | | |
| Q, R, X | | | | | | |
| Q | BMWW-t | MT20 | 5.0 | 6.0 | | |
| S | BMWW-t | MT20 | 4.0 | 6.0 | | |
| T | BMWWW-t | MT20 | 4.0 | 6.0 | | |
| U | BS-t | MT20 | 3.0 | 8.0 | | |
| V | BMWW-t | MT20 | 4.0 | 6.0 | | |
| W | BMWWW-t | MT20 | 6.0 | 10.0 | 3.00 | 4.75 |


Structural component only
DWG# T-2311491

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| Y | 1854 | 0 | 1854 | 0 | 0 | 5-8 | 2-8 |
| N | 1849 | 0 | 1849 | 0 | 0 | 5-8 | 2-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| Y | 1309 | 873 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 437 / 0 | 0 / 0 |
| N | 1305 | 870 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 436 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. | FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A-B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | | 10.00 |
| B-C | -1713 / 0 | -91.8 | -91.8 | 0.15 (1) | | 4.94 |
| C-D | -4533 / 0 | -91.8 | -91.8 | 0.47 (1) | | 2.94 |
| D-E | -5242 / 0 | -91.8 | -91.8 | 0.58 (1) | | 2.64 |
| E-F | -2755 / 0 | -91.8 | -91.8 | 0.33 (1) | | 3.89 |
| F-G | -2562 / 0 | -91.8 | -91.8 | 0.31 (1) | | 4.02 |
| G-H | -2562 / 0 | -91.8 | -91.8 | 0.31 (1) | | 4.02 |
| H-I | -2570 / 0 | -91.8 | -91.8 | 0.12 (1) | | 4.22 |
| I-J | -2506 / 0 | -91.8 | -91.8 | 0.53 (1) | | 3.80 |
| J-K | -1987 / 0 | -91.8 | -91.8 | 0.10 (1) | | 4.72 |
| K-L | -2037 / 0 | -91.8 | -91.8 | 0.13 (1) | | 4.64 |
| L-M | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | | 10.00 |
| Y-B | -1882 / 0 | 0.0 | 0.0 | 0.19 (1) | | 6.12 |
| N-L | -1828 / 0 | 0.0 | 0.0 | 0.18 (1) | | 6.20 |
| Y-X | 0 / 0 | -18.5 | -18.5 | 0.08 (1) | | 10.00 |
| X-W | 0 / 1453 | -18.5 | -18.5 | 0.32 (1) | | 10.00 |
| W-V | 0 / 3212 | -18.5 | -18.5 | 0.66 (1) | | 10.00 |
| V-U | 0 / 2462 | -18.5 | -18.5 | 0.54 (1) | | 10.00 |
| U-T | 0 / 2462 | -18.5 | -18.5 | 0.54 (1) | | 10.00 |
| T-S | 0 / 2310 | -18.5 | -18.5 | 0.41 (1) | | 10.00 |
| S-R | 0 / 2529 | -18.5 | -18.5 | 0.42 (1) | | 10.00 |
| R-Q | 0 / 1394 | -18.5 | -18.5 | 0.31 (1) | | 10.00 |
| Q-P | 0 / 51 | -18.5 | -18.5 | 0.10 (4) | | 10.00 |
| P-O | -19 / 0 | 0.0 | 0.0 | 0.10 (1) | | 7.81 |
| O-K | -161 / 0 | 0.0 | 0.0 | 0.10 (1) | | 7.81 |
| O-N | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | | 10.00 |

| WEBS MEMB. | FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| X-C | -754 / 0 | 0.11 (1) |
| C-W | 0 / 3360 | 0.76 (1) |
| W-D | -2643 / 0 | 0.38 (1) |
| W-E | 0 / 2012 | 0.45 (1) |
| E-V | -1008 / 0 | 0.46 (1) |
| V-F | 0 / 813 | 0.18 (1) |
| F-T | 0 / 167 | 0.04 (1) |
| T-G | -433 / 0 | 0.21 (1) |
| T-H | 0 / 420 | 0.09 (1) |
| S-H | 0 / 580 | 0.13 (1) |
| S-I | -558 / 0 | 0.21 (1) |
| R-I | -957 / 0 | 0.31 (1) |
| R-J | 0 / 1490 | 0.34 (1) |
| Q-J | -1006 / 0 | 0.32 (1) |
| B-X | 0 / 1738 | 0.39 (1) |
| O-L | 0 / 1946 | 0.44 (1) |
| Q-O | 0 / 1767 | 0.28 (1) |
| J-O | 0 / 693 | 0.16 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 712 (0.53")

CSI: TC=0.58/1.00 (D-E:1) , BC=0.68/1.00 (V-W:1) , WB=0.76/1.00 (C-W:1) , SSI=0.18/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90 )
JSI METAL= 0.71 (D) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T42S | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| Y | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311491

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T43S | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:41 2023 Page 1

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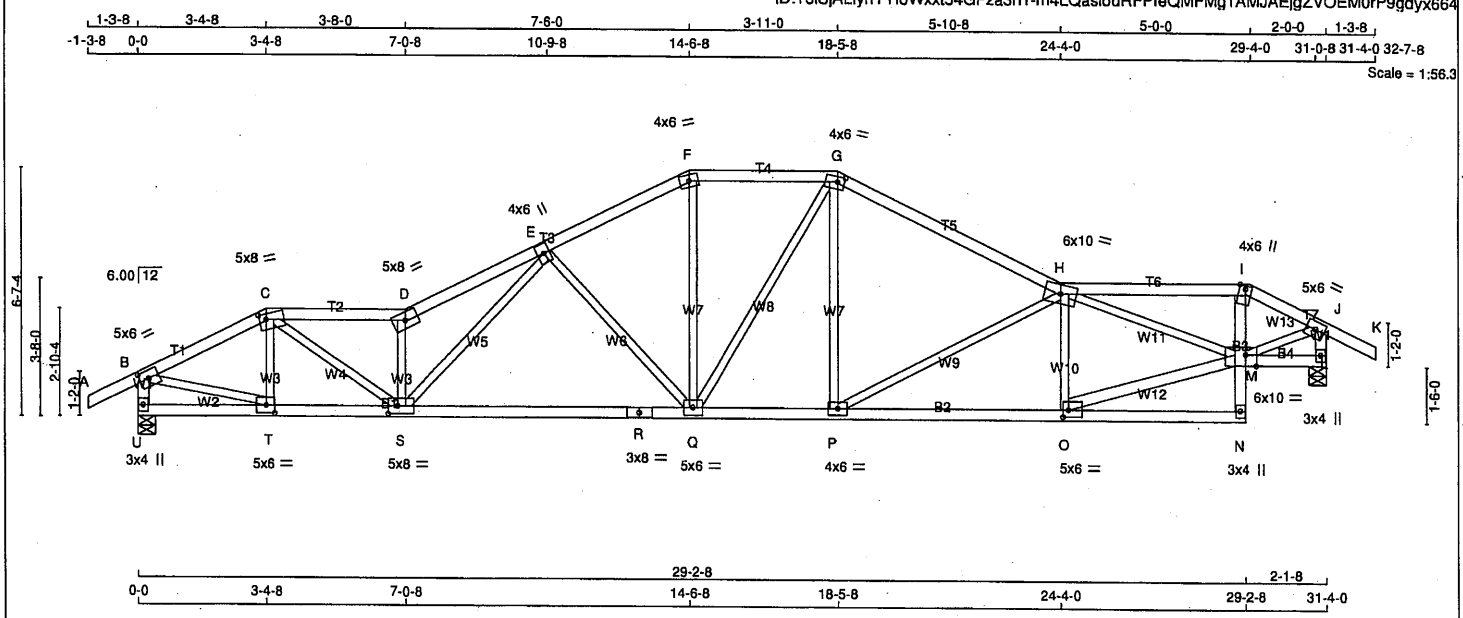


TOTAL WEIGHT = 136 lb [M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - G | 2x4 | DRY | No.2 | SPF |
| G - H | 2x4 | DRY | No.2 | SPF |
| H - I | 2x4 | DRY | No.2 | SPF |
| I - K | 2x4 | DRY | No.2 | SPF |
| U - B | 2x4 | DRY | No.2 | SPF |
| L - J | 2x4 | DRY | No.2 | SPF |
| U - R | 2x4 | DRY | No.2 | SPF |
| R - N | 2x4 | DRY | No.2 | SPF |
| N - I | 2x4 | DRY | No.2 | SPF |
| M - L | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| O - M | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| C | TTWW-m | MT20 | 5.0 | 8.0 | 1.75 | 2.25 |
| D | TTW-h | MT20 | 5.0 | 8.0 | | |
| E | TMWW+t | MT20 | 4.0 | 6.0 | | |
| F | TTW-m | MT20 | 4.0 | 6.0 | | |
| G | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.25 |
| H | TTWWW-m | MT20 | 6.0 | 10.0 | | |
| I | TTV+m | MT20 | 4.0 | 6.0 | Edge | |
| J | TMVW-t | MT20 | 5.0 | 6.0 | | |
| L | BMV1+p | MT20 | 3.0 | 4.0 | | |
| M | BVMWWW-l | MT20 | 6.0 | 10.0 | Edge | 3.50 |
| N | BMV+p | MT20 | 3.0 | 4.0 | | |
| O | BMWW-t | MT20 | 5.0 | 6.0 | 2.25 | 1.75 |
| P | BMWW-t | MT20 | 4.0 | 6.0 | | |
| Q | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| R | BS-t | MT20 | 3.0 | 8.0 | | |
| S | BMWWW-t | MT20 | 5.0 | 8.0 | 2.50 | 3.00 |
| T | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| U | BMV1+p | MT20 | 3.0 | 4.0 | | |


Structural component only
DWG# T-2311492

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| U | 1852 | 0 | 1852 | 0 | 0 | 5-8 | 2-8 |
| L | 1852 | 0 | 1852 | 0 | 0 | 5-8 | 2-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| U | 1307 | 871 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 436 / 0 | 0 / 0 |
| L | 1307 | 871 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 436 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | -2306 / 0 | -91.8 | -91.8 | 0.20 (1) | 4.36 |
| C- D | -3652 / 0 | -91.8 | -91.8 | 0.31 (1) | 3.45 |
| D- E | -4159 / 0 | -91.8 | -91.8 | 0.37 (1) | 3.20 |
| E- F | -2408 / 0 | -91.8 | -91.8 | 0.24 (1) | 4.24 |
| F- G | -2148 / 0 | -91.8 | -91.8 | 0.23 (1) | 4.44 |
| G- H | -2320 / 0 | -91.8 | -91.8 | 0.51 (1) | 3.99 |
| H- I | -1865 / 0 | -91.8 | -91.8 | 0.33 (1) | 4.59 |
| I- J | -2066 / 0 | -91.8 | -91.8 | 0.10 (1) | 4.66 |
| J- K | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| U- B | -1823 / 0 | 0.0 | 0.0 | 0.18 (1) | 6.20 |
| L- J | -1831 / 0 | 0.0 | 0.0 | 0.19 (1) | 6.19 |
| U- T | 0 / 0 | -18.5 | -18.5 | 0.06 (4) | 10.00 |
| T- S | 0 / 2042 | -18.5 | -18.5 | 0.35 (1) | 10.00 |
| S- R | 0 / 2799 | -18.5 | -18.5 | 0.57 (1) | 10.00 |
| R- Q | 0 / 2799 | -18.5 | -18.5 | 0.57 (1) | 10.00 |
| Q- P | 0 / 2083 | -18.5 | -18.5 | 0.46 (1) | 10.00 |
| P- O | 0 / 2798 | -18.5 | -18.5 | 0.52 (1) | 10.00 |
| O- N | 0 / 47 | -18.5 | -18.5 | 0.13 (4) | 10.00 |
| N- M | 0 / 35 | 0.0 | 0.0 | 0.10 (1) | 10.00 |
| M- I | 0 / 598 | 0.0 | 0.0 | 0.19 (1) | 10.00 |
| M- L | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| T- C | -442 / 0 | 0.08 (1) |
| C- S | 0 / 1981 | 0.45 (1) |
| S- D | -2156 / 0 | 0.37 (1) |
| S- E | 0 / 1444 | 0.32 (1) |
| E- Q | -989 / 0 | 0.60 (1) |
| Q- F | 0 / 757 | 0.17 (1) |
| Q- G | 0 / 126 | 0.03 (1) |
| P- G | 0 / 480 | 0.11 (1) |
| P- H | -830 / 0 | 0.75 (1) |
| O- H | -777 / 0 | 0.17 (1) |
| B- T | 0 / 2122 | 0.48 (1) |
| O- M | 0 / 2867 | 0.46 (1) |
| H- M | -978 / 0 | 0.40 (1) |
| M- J | 0 / 1983 | 0.45 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 931 (0.40")

CSI: TC=0.51/1.00 (G-H:1) , BC=0.57/1.00 (Q-S:1) , WB=0.75/1.00 (H-P:1) , SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90 )
JSI METAL= 0.80 (R) (INPUT = 0.95 )



CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T43S | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311492

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T44 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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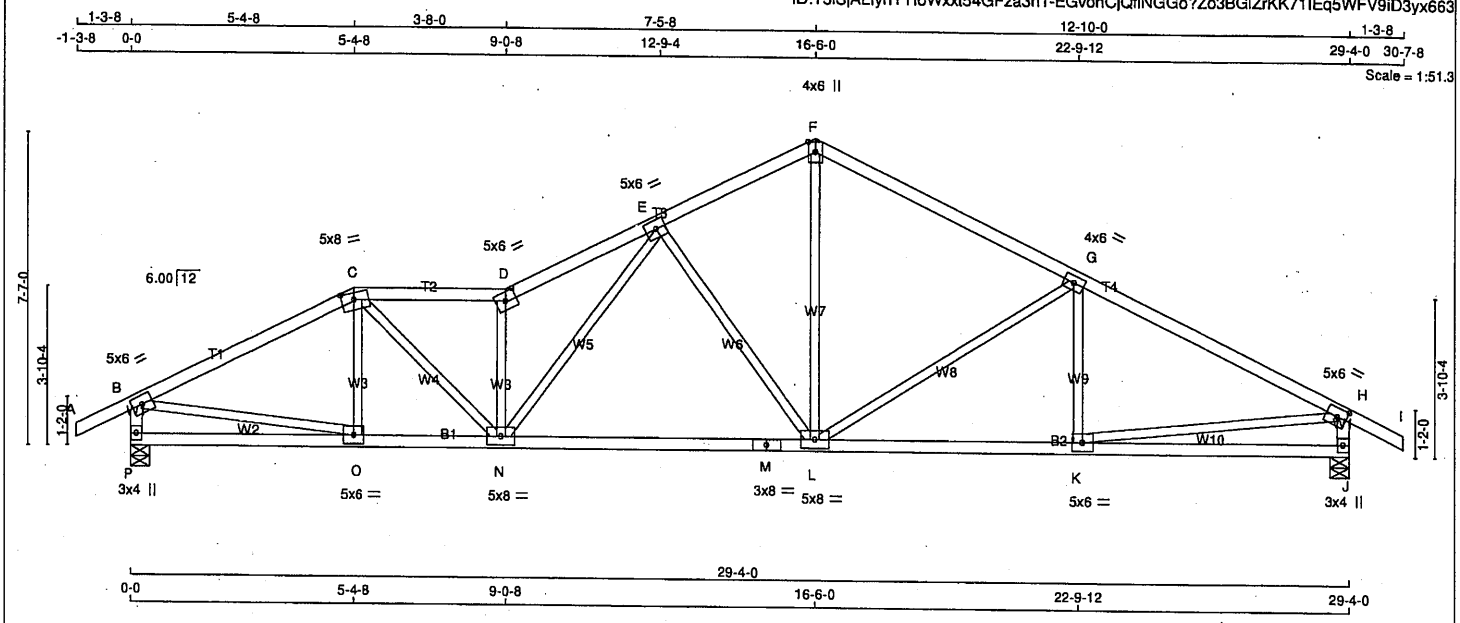


TOTAL WEIGHT = 3 X 119 = 357 lb [M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - I | 2x4 | DRY | No.2 | SPF |
| P - B | 2x4 | DRY | No.2 | SPF |
| J - H | 2x4 | DRY | No.2 | SPF |
| P - M | 2x4 | DRY | No.2 | SPF |
| M - J | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES** (table is in inches)

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | | |
| C | TTWW-m | MT20 | 5.0 | 8.0 | 2.00 | 3.50 |
| D | TTW-h | MT20 | 5.0 | 6.0 | 2.50 | 3.25 |
| E | TMWW-t | MT20 | 5.0 | 6.0 | | |
| F | TTW+p | MT20 | 4.0 | 6.0 | Edge | |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |
| K | BMWW-t | MT20 | 5.0 | 6.0 | | |
| L | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| M | BS-t | MT20 | 3.0 | 8.0 | | |
| N | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| O | BMWW-t | MT20 | 5.0 | 6.0 | | |
| P | BMV1+p | MT20 | 3.0 | 4.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311493

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| P | 1742 | 0 | 1742 | 0 | 0 | 5-8 | 2-3 |
| J | 1742 | 0 | 1742 | 0 | 0 | 5-8 | 2-3 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| P | 1229 | 820 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 409 / 0 | 0 / 0 |
| J | 1229 | 820 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 409 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J .

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | O- C | -226 / 0 | 0.05 (1) |
| B- C | -2253 / 0 | -91.8 | -91.8 | 0.43 (1) | 4.14 | | C- N | 0 / 1163 | 0.26 (1) |
| C- D | -2831 / 0 | -91.8 | -91.8 | 0.25 (1) | 3.94 | | N- D | -1727 / 0 | 0.40 (1) |
| D- E | -3199 / 0 | -91.8 | -91.8 | 0.28 (1) | 3.70 | | N- E | 0 / 1215 | 0.27 (1) |
| E- F | -1826 / 0 | -91.8 | -91.8 | 0.20 (1) | 4.78 | | E- L | -979 / 0 | 0.78 (1) |
| F- G | -1840 / 0 | -91.8 | -91.8 | 0.55 (1) | 4.32 | | L- F | 0 / 1263 | 0.28 (1) |
| G- H | -2331 / 0 | -91.8 | -91.8 | 0.62 (1) | 3.85 | | L- G | -591 / 0 | 0.67 (1) |
| H- I | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | K- G | -158 / 48 | 0.05 (1) |
| P- B | -1697 / 0 | 0.0 | 0.0 | 0.17 (1) | 6.38 | | B- O | 0 / 2038 | 0.46 (1) |
| J- H | -1689 / 0 | 0.0 | 0.0 | 0.17 (1) | 6.39 | | K- H | 0 / 2130 | 0.48 (1) |
| P- O | 0 / 0 | -18.5 | -18.5 | 0.12 (4) | 10.00 | | | | |
| O- N | 0 / 2007 | -18.5 | -18.5 | 0.36 (1) | 10.00 | | | | |
| N- M | 0 / 2187 | -18.5 | -18.5 | 0.47 (1) | 10.00 | | | | |
| M- L | 0 / 2187 | -18.5 | -18.5 | 0.47 (1) | 10.00 | | | | |
| L- K | 0 / 2112 | -18.5 | -18.5 | 0.46 (1) | 10.00 | | | | |
| K- J | 0 / 0 | -18.5 | -18.5 | 0.16 (4) | 10.00 | | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.62/1.00 (G-H:1) , BC=0.47/1.00 (L-N:1) , WB=0.78/1.00 (E-L:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90 )
JSI METAL= 0.65 (M) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T45 | 1 | 3 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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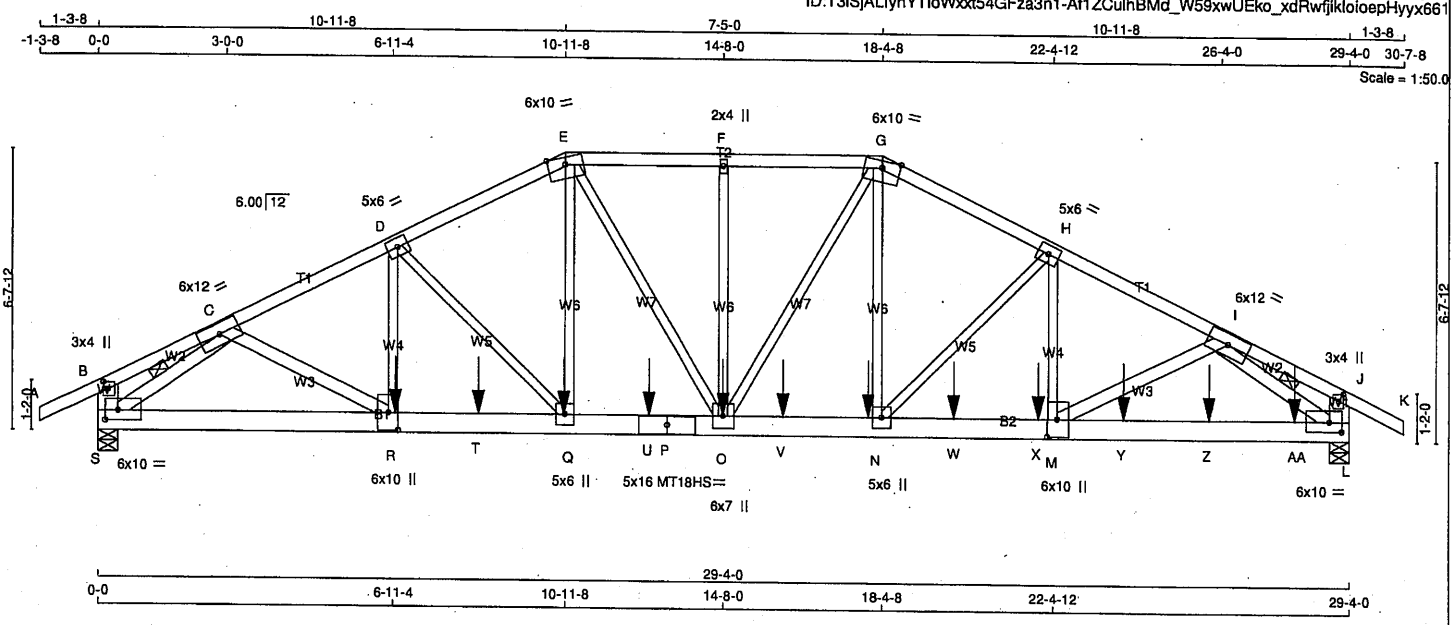


TOTAL WEIGHT = 3 X 153 = 458 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - E | 2x4 | DRY | No.2 | SPF |
| E - G | 2x4 | DRY | No.2 | SPF |
| G - K | 2x4 | DRY | No.2 | SPF |
| S - B | 2x6 | DRY | No.2 | SPF |
| L - J | 2x6 | DRY | No.2 | SPF |
| S - P | 2x6 | DRY | 2100F 1.8E | SPF |
| P - L | 2x6 | DRY | 2100F 1.8E | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| C - R | 2x4 | DRY | No.2 | SPF |
| S - C | 2x4 | DRY | No.2 | SPF |
| M - I | 2x4 | DRY | No.2 | SPF |
| I - L | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- E | 1 | 12 | TOP |
| E- G | 1 | 12 | TOP |
| G- K | 1 | 12 | TOP |
| S- B | 2 | 12 | TOP |
| L- J | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| S- P | 2 | 5 | SIDE(890.1) |
| P- L | 2 | 5 | SIDE(890.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| D- R | 1 | 5 | SIDE(393.8) |
| 2x3 | 1 | 6 | |
| H- M | 1 | 5 | |
| 2x4 | 1 | 6 | |

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311494

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| L | 14408 | 0 | 14408 | 0 | 0 | 5-8 | 5-3 |
| S | 12113 | 0 | 12113 | 0 | 0 | 5-8 | 4-6 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| L | 10173 | 6769 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 3403 / 0 | 0 / 0 |
| S | 8549 | 5706 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 2843 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.04 (1) | 10.00 | R- D | 0 / 3174 | 0.24 (1) |
| B- C | -9 / 0 | -91.8 | -91.8 | 0.06 (1) | 10.00 | D- Q | -3401 / 0 | 0.67 (1) |
| C- D | -21127 / 0 | -91.8 | -91.8 | 0.75 (1) | 2.13 | Q- E | 0 / 6117 | 0.46 (1) |
| D- E | -18513 / 0 | -91.8 | -91.8 | 0.58 (1) | 2.48 | E- O | 0 / 2117 | 0.16 (1) |
| E- F | -17719 / 0 | -91.8 | -91.8 | 0.47 (1) | 2.59 | O- F | -285 / 0 | 0.06 (1) |
| F- G | -17719 / 0 | -91.8 | -91.8 | 0.47 (1) | 2.59 | O- G | 0 / 2659 | 0.20 (1) |
| G- H | -18215 / 0 | -91.8 | -91.8 | 0.56 (1) | 2.51 | N- G | 0 / 5517 | 0.41 (1) |
| H- I | -21348 / 0 | -91.8 | -91.8 | 0.77 (1) | 2.11 | N- H | -4066 / 0 | 0.80 (1) |
| I- J | -10 / 0 | -91.8 | -91.8 | 0.06 (1) | 10.00 | M- H | 0 / 3881 | 0.29 (1) |
| J- K | 0 / 28 | -91.8 | -91.8 | 0.04 (1) | 10.00 | C- R | 0 / 4133 | 0.22 (1) |
| S- B | -278 / 0 | 0.0 | 0.0 | 0.01 (1) | 7.81 | S- C | -19266 / 0 | 0.73 (1) |
| L- J | -279 / 0 | 0.0 | 0.0 | 0.01 (1) | 7.81 | M- I | 0 / 4182 | 0.22 (1) |
| | | | | | | I- L | -19462 / 0 | 0.74 (1) |
| S- R | 0 / 15291 | -18.5 | -18.5 | 0.29 (1) | 10.00 | | | |
| R- T | 0 / 18909 | -18.5 | -18.5 | 0.49 (1) | 10.00 | | | |
| T- Q | 0 / 18909 | -18.5 | -18.5 | 0.49 (1) | 10.00 | | | |
| Q- U | 0 / 16640 | -18.5 | -18.5 | 0.37 (1) | 10.00 | | | |
| U- P | 0 / 16640 | -18.5 | -18.5 | 0.37 (1) | 10.00 | | | |
| P- O | 0 / 16640 | -18.5 | -18.5 | 0.37 (1) | 10.00 | | | |
| O- V | 0 / 16364 | -18.5 | -18.5 | 0.37 (1) | 10.00 | | | |
| V- N | 0 / 16364 | -18.5 | -18.5 | 0.37 (1) | 10.00 | | | |
| N- W | 0 / 19106 | -18.5 | -18.5 | 0.67 (1) | 10.00 | | | |
| W- X | 0 / 19106 | -18.5 | -18.5 | 0.67 (1) | 10.00 | | | |
| X- M | 0 / 19106 | -18.5 | -18.5 | 0.67 (1) | 10.00 | | | |
| M- Y | 0 / 15446 | -18.5 | -18.5 | 0.73 (1) | 10.00 | | | |
| Y- Z | 0 / 15446 | -18.5 | -18.5 | 0.73 (1) | 10.00 | | | |
| Z-AA | 0 / 15446 | -18.5 | -18.5 | 0.73 (1) | 10.00 | | | |
| AA- L | 0 / 15446 | -18.5 | -18.5 | 0.73 (1) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| N | 18-0-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| O | 14-8-0 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| Q | 10-11-4 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| R | 7-0-0 | -3204 | -3204 | --- | BACK | VERT | TOTAL | --- | C1 |
| T | 8-11-4 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| U | 12-11-4 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 866 (0.41")

CSI: TC=0.77/1.00 (H-I:1) , BC=0.73/1.00 (L-M:1) , WB=0.80/1.00 (H-N:1) , SSI=0.61/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |
| MT18HS | 586 | 403 | 2455 | 1382 | 3163 | 3004 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90 )
JSI METAL= 0.85 (I) (INPUT = 0.95 )

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
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Per: joshua.nabua

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T45 | 1 | 3 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



ID:T3iSiALIyhYTloWxxt54GFza3n1-Af1ZCulhBMd_W59xwUEko_xdRwfjikloioepHyyx661

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | 2.00 | 1.25 |
| C | TMWW-t | MT20 | 6.0 | 12.0 | | |
| D | TMWW-t | MT20 | 5.0 | 6.0 | | |
| E | TTWW-m | MT20 | 6.0 | 10.0 | Edge | |
| F | TMW+w | MT20 | 2.0 | 4.0 | | |
| G | TTWW-m | MT20 | 6.0 | 10.0 | Edge | |
| H | TMWW-t | MT20 | 5.0 | 6.0 | | |
| I | TMWW-t | MT20 | 6.0 | 12.0 | | |
| J | TMV+p | MT20 | 3.0 | 4.0 | 2.00 | 1.25 |
| L | BMVW1-t | MT20 | 6.0 | 10.0 | 2.75 | 3.50 |
| M | BMWW+t | MT20 | 6.0 | 10.0 | 5.00 | 2.75 |
| N | BMWW+t | MT20 | 5.0 | 6.0 | | |
| O | BMWWW+t | MT20 | 6.0 | 7.0 | | |
| P | BS-t | MT18HS | 5.0 | 16.0 | | |
| Q | BMWW+t | MT20 | 5.0 | 6.0 | | |
| R | BMWW+t | MT20 | 6.0 | 10.0 | 5.00 | 2.75 |
| S | BMVW1-t | MT20 | 6.0 | 10.0 | 2.75 | 3.50 |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| V | 16-0-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| W | 20-0-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| X | 22-0-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| Y | 24-0-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| Z | 26-0-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |
| AA | 28-0-12 | -1187 | -1187 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311494

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T46 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:45 2023 Page 1
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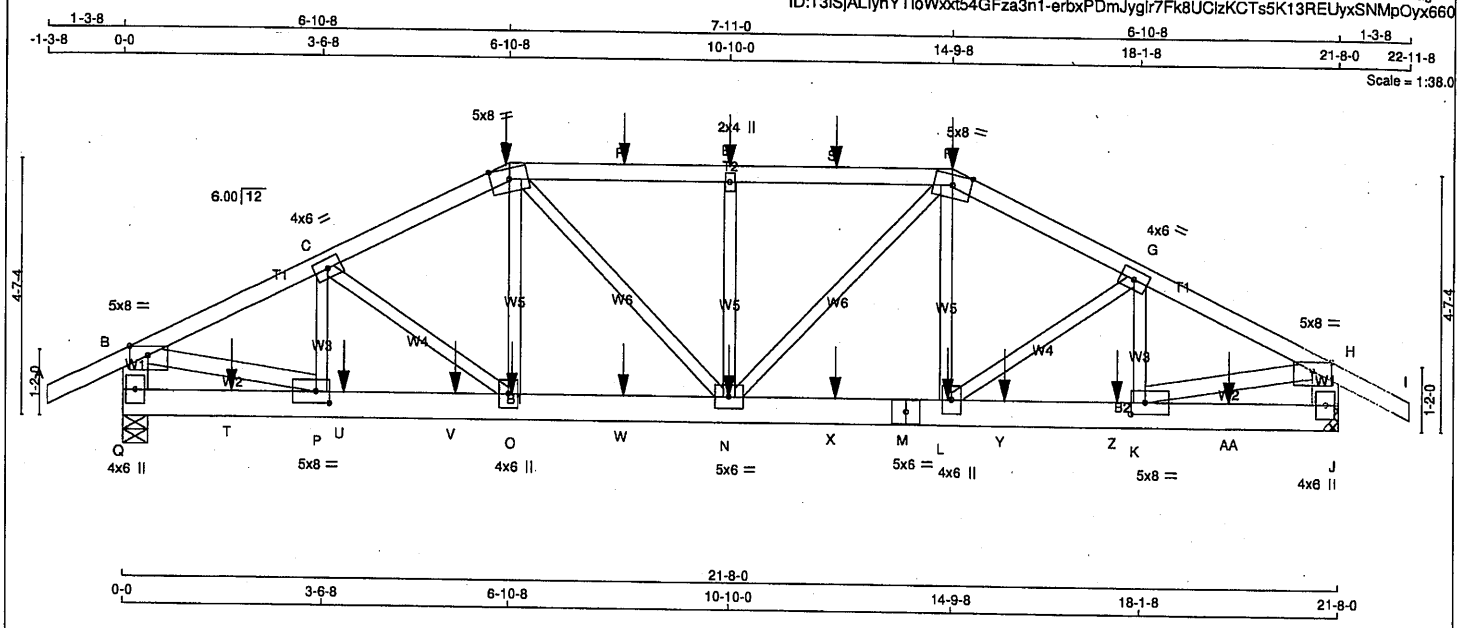





TOTAL WEIGHT = 106 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - I | 2x4 | DRY | No.2 | SPF |
| Q - B | 2x6 | DRY | No.2 | SPF |
| J - H | 2x6 | DRY | No.2 | SPF |
| Q - M | 2x6 | DRY | No.2 | SPF |
| M - J | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| B - P | 2x4 | DRY | No.2 | SPF |
| K - H | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW-m | MT20 | 5.0 | 8.0 | Edge | |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TTWW-m | MT20 | 5.0 | 8.0 | Edge | |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| J | BMV1+p | MT20 | 4.0 | 6.0 | | |
| K | BMWW-t | MT20 | 5.0 | 8.0 | 2.50 | 3.00 |
| L | BMWW+t | MT20 | 4.0 | 6.0 | | |
| M | BS-t | MT20 | 5.0 | 6.0 | | |
| N | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| O | BMWW+t | MT20 | 4.0 | 6.0 | | |
| P | BMWW-t | MT20 | 5.0 | 8.0 | 2.50 | 3.00 |
| Q | BMV1+p | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311495

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| Q | 2755 | 0 | 2755 | 0 | 0 | IN-SX | IN-SX |
| J | 2755 | 0 | 2755 | 0 | 0 | 5-8 | 3-7 |
| | | | | | | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-7.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| Q | 1947 | 1286 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 661 / 0 | 0 / 0 |
| J | 1947 | 1286 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 661 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.13 (1) | 10.00 |
| B- C | -3749 / 0 | -91.8 | -91.8 | 0.37 (1) | 3.31 |
| C- D | -4001 / 0 | -91.8 | -91.8 | 0.38 (1) | 3.19 |
| D- R | -4049 / 0 | -91.8 | -91.8 | 0.52 (1) | 3.00 |
| R- E | -4049 / 0 | -91.8 | -91.8 | 0.52 (1) | 3.00 |
| E- S | -4049 / 0 | -91.8 | -91.8 | 0.52 (1) | 3.00 |
| S- F | -4049 / 0 | -91.8 | -91.8 | 0.52 (1) | 3.00 |
| F- G | -4001 / 0 | -91.8 | -91.8 | 0.38 (1) | 3.19 |
| G- H | -3749 / 0 | -91.8 | -91.8 | 0.37 (1) | 3.31 |
| H- I | 0 / 28 | -91.8 | -91.8 | 0.13 (1) | 10.00 |
| Q- B | -2678 / 0 | 0.0 | 0.0 | 0.19 (1) | 6.32 |
| J- H | -2678 / 0 | 0.0 | 0.0 | 0.19 (1) | 6.32 |
| Q- T | 0 / 0 | -18.5 | -18.5 | 0.08 (1) | 10.00 |
| T- P | 0 / 0 | -18.5 | -18.5 | 0.08 (1) | 10.00 |
| P- U | 0 / 3364 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| U- V | 0 / 3364 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| V- O | 0 / 3364 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| O- W | 0 / 3576 | -18.5 | -18.5 | 0.60 (1) | 10.00 |
| W- N | 0 / 3576 | -18.5 | -18.5 | 0.60 (1) | 10.00 |
| N- X | 0 / 3576 | -18.5 | -18.5 | 0.60 (1) | 10.00 |
| X- M | 0 / 3576 | -18.5 | -18.5 | 0.60 (1) | 10.00 |
| M- L | 0 / 3576 | -18.5 | -18.5 | 0.60 (1) | 10.00 |
| L- Y | 0 / 3364 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| Y- Z | 0 / 3364 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| Z- K | 0 / 3364 | -18.5 | -18.5 | 0.53 (1) | 10.00 |
| K-AA | 0 / 0 | -18.5 | -18.5 | 0.08 (1) | 10.00 |
| AA- J | 0 / 0 | -18.5 | -18.5 | 0.08 (1) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| P- C | -628 / 0 | 0.11 (1) |
| C- O | 0 / 249 | 0.06 (1) |
| O- D | 0 / 510 | 0.13 (1) |
| D- N | 0 / 691 | 0.17 (1) |
| N- E | -500 / 0 | 0.15 (1) |
| N- F | 0 / 691 | 0.17 (1) |
| L- F | 0 / 510 | 0.13 (1) |
| L- G | 0 / 249 | 0.06 (1) |
| K- G | -628 / 0 | 0.11 (1) |
| B- P | 0 / 3443 | 0.61 (1) |
| K- H | 0 / 3443 | 0.61 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| D | 6-10-8 | -337 | -337 | --- | FRONT | VERT | TOTAL | --- | C1 |
| E | 10-10-0 | -23 | -23 | --- | FRONT | VERT | TOTAL | --- | C1 |
| F | 14-9-8 | -337 | -337 | --- | FRONT | VERT | TOTAL | --- | C1 |
| L | 14-8-12 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| N | 10-10-0 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| O | 6-11-4 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| R | 8-11-4 | -23 | -23 | --- | FRONT | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.60/1.00 (L-N:1) , WB=0.61/1.00 (B-P:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90 )
JSI METAL= 0.61 (M) (INPUT = 0.95 )

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024

CONTINUED ON PAGE 2

RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T46 | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



ID:T3lSjALIyhYTloWxxt54GFza3n1-erbxPDmJyqlr7Fk8UClzKCTs5K13REUyxSNMpOyx660

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| S | 12-8-12 | -23 | -23 | --- | FRONT | VERT | TOTAL | --- | C1 |
| T | 1-11-4 | -36 | -36 | --- | FRONT | VERT | TOTAL | --- | C1 |
| U | 3-11-4 | -36 | -36 | --- | FRONT | VERT | TOTAL | --- | C1 |
| V | 5-11-4 | -149 | -149 | --- | FRONT | VERT | TOTAL | --- | C1 |
| W | 8-11-4 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| X | 12-8-12 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Y | 15-8-12 | -149 | -149 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Z | 17-8-12 | -36 | -36 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AA | 19-8-12 | -36 | -36 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311495

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T47 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:46 2023 Page 1
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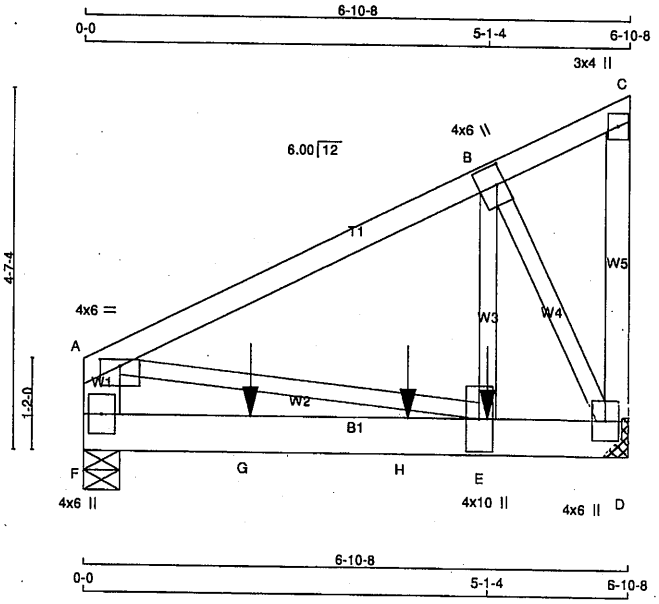


Scale = 1:26.9

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| D - C | 2x4 | DRY | No.2 | SPF |
| F - A | 2x6 | DRY | No.2 | SPF |
| F - D | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- D | 1 | 12 | TOP |
| F- A | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| F- D | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| B- E | 1 | 3 | SIDE(530.7) |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMWW-p | MT20 | 4.0 | 6.0 | 1.00 | 3.00 |
| B | TMWW+t | MT20 | 4.0 | 6.0 | 3.00 | 1.25 |


Structural component only
DWG# T-2311496

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| D | 2734 | 0 | 2734 | 0 | 0 | MECHANICAL | |
| F | 1447 | 0 | 1447 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| D | 1925 | 1311 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 614 / 0 | 0 / 0 |
| F | 1016 | 710 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 306 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | -1653 / 0 | -91.8 | -91.8 | 0.18 (1) | 6.25 | | E- B | 0 / 3094 | 0.38 (1) |
| B- C | -47 / 0 | -91.8 | -91.8 | 0.14 (1) | 6.25 | | B- D | -3177 / 0 | 0.39 (1) |
| D- C | 0 / 24 | 0.0 | 0.0 | 0.00 (1) | 10.00 | | A- E | 0 / 1510 | 0.19 (1) |
| F- A | -1171 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | | | | |
| F- G | 0 / 0 | -18.5 | -18.5 | 0.20 (1) | 10.00 | | | | |
| G- H | 0 / 0 | -18.5 | -18.5 | 0.20 (1) | 10.00 | | | | |
| H- E | 0 / 0 | -18.5 | -18.5 | 0.20 (1) | 10.00 | | | | |
| E- D | 0 / 1493 | -18.5 | -18.5 | 0.14 (1) | 10.00 | | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| E | 5-1-4 | -1932 | -1932 | --- | BACK | VERT | TOTAL | --- | C1 |
| G | 2-1-4 | -205 | -205 | --- | TOP | VERT | TOTAL | --- | C1 |
| H | 4-1-4 | -268 | -268 | --- | TOP | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 36 = 71 lb [M]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.20/1.00 (E-F:1) , WB=0.39/1.00 (B-D:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90 )
JSI METAL= 0.54 (E) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T47 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington


ID:T3lSjALIyhYTloWxxt54GFza3n1-719JdZmxjztilPJK1vGCtP079kSYAk45A67wMqyx66?

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| C | TMV+p | MT20 | 3.0 | 4.0 | | |
| D | BMVW1+p | MT20 | 4.0 | 6.0 | | |
| E | BMWW+t | MT20 | 4.0 | 10.0 | | |
| F | BMV1+p | MT20 | 4.0 | 6.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311496

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T47A | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:46 2023 Page 1
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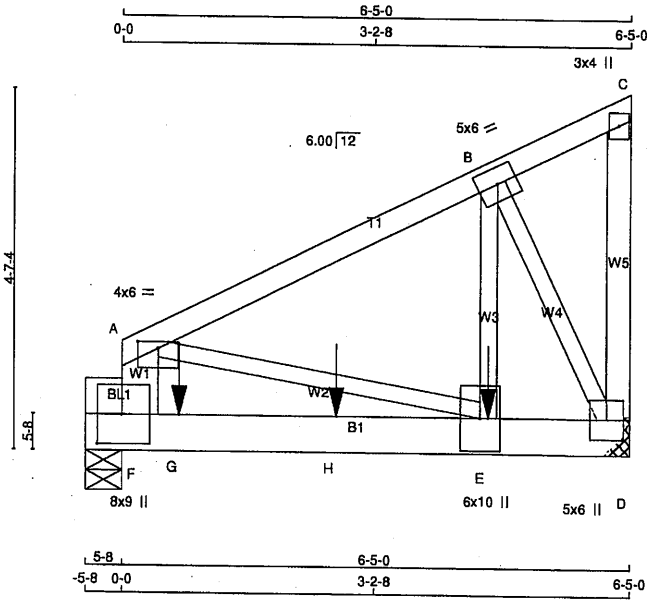


TOTAL WEIGHT = 2 X 36 = 72 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| D - C | 2x4 | DRY | No.2 | SPF |
| F - A | 2x6 | DRY | No.2 | SPF |
| F - D | 2x6 | DRY | No.2 | SPF |
| BEARING BLOCKS | | | | |
| BL1 | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- D | 1 | 12 | TOP |
| F- A | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| F- D | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| B- E | 1 | 2 | SIDE(599.9) |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311497

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| D | 2600 | 0 | 2600 | 0 | 0 | MECHANICAL | |
| F | 1592 | 0 | 1592 | 0 | 0 | 5-8 | 5-0 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| D | 1830 | 1252 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 578 / 0 | 0 / 0 |
| F | 1117 | 784 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 333 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | -1483 / 0 | -91.8 | -91.8 | 0.15 (1) | 6.25 | | E- B | 0 / 2777 | 0.34 (1) |
| B- C | -37 / 0 | -91.8 | -91.8 | 0.11 (1) | 6.25 | | B- D | -2850 / 0 | 0.35 (1) |
| D- C | -3 / 1 | 0.0 | 0.0 | 0.00 (4) | 7.81 | | A- E | 0 / 1650 | 0.20 (1) |
| F- A | -1231 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | | | | |
| F- G | -274 / 0 | -18.5 | -18.5 | 0.16 (1) | 6.25 | | | | |
| G- H | -274 / 0 | -18.5 | -18.5 | 0.16 (1) | 6.25 | | | | |
| H- E | -274 / 0 | -18.5 | -18.5 | 0.16 (1) | 6.25 | | | | |
| E- D | 0 / 1339 | -18.5 | -18.5 | 0.13 (1) | 10.00 | | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| E | 4-7-12 | -2113 | -2113 | --- | FRONT | VERT | TOTAL | --- | C1 |
| G | 8-12 | -170 | -170 | --- | TOP | VERT | TOTAL | --- | C1 |
| H | 2-8-12 | -167 | -167 | --- | TOP | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP | CH. | LL = | 25.6 | PSF |
| | | DL = | 6.0 | PSF |
| BOT | CH. | LL = | 0.0 | PSF |
| | | DL = | 7.4 | PSF |
| TOTAL | LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.16/1.00 (E-F:1) , WB=0.35/1.00 (B-D:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90 )
JSI METAL= 0.36 (E) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T47A | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMVW-p | MT20 | 4.0 | 6.0 | 1.00 | 3.00 |
| B | TMWW-t | MT20 | 5.0 | 6.0 | | |
| C | TMV+p | MT20 | 3.0 | 4.0 | | |
| D | BMVW1+p | MT20 | 5.0 | 6.0 | | |
| E | BMWW+t | MT20 | 6.0 | 10.0 | | |
| F | BMVK1+p | MT20 | 8.0 | 9.0 | 4.50 | 3.75 |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311497

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T48 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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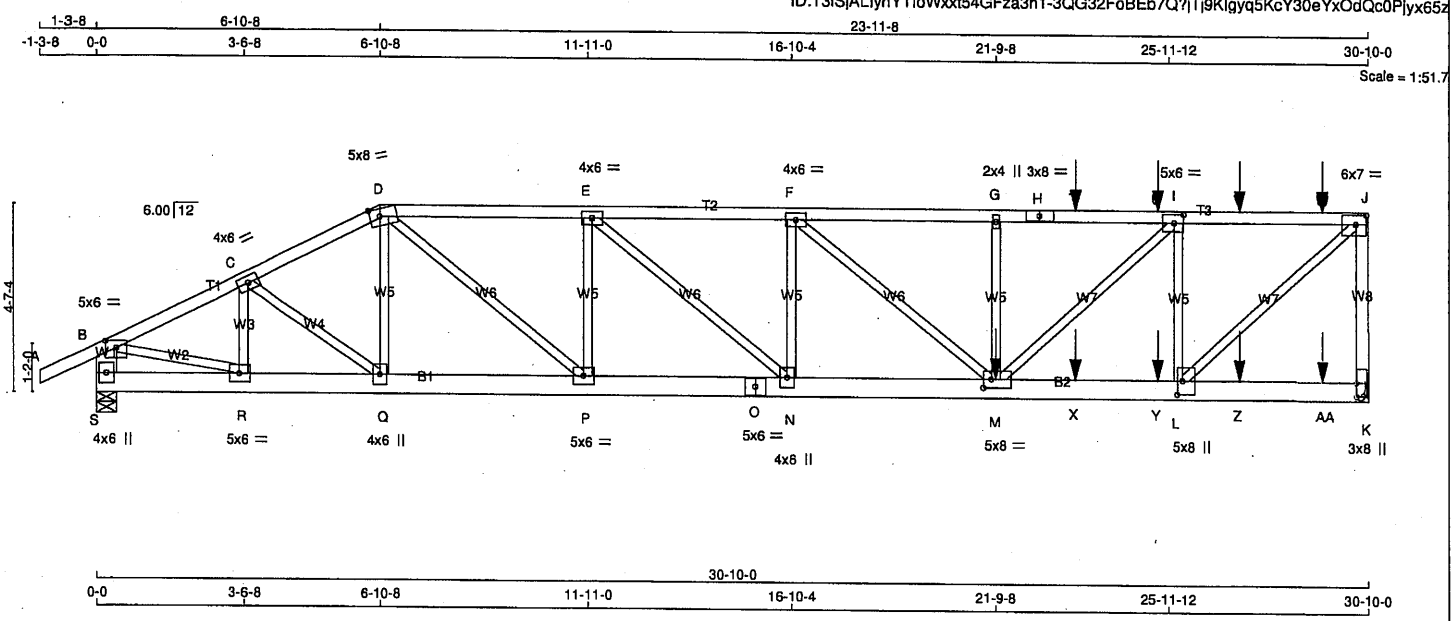


TOTAL WEIGHT = 2 X 146 = 292 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - H | 2x4 | DRY | No.2 | SPF |
| H - J | 2x4 | DRY | No.2 | SPF |
| K - J | 2x4 | DRY | No.2 | SPF |
| S - B | 2x6 | DRY | No.2 | SPF |
| S - O | 2x6 | DRY | No.2 | SPF |
| O - K | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- D | 1 | 12 | TOP |
| D- H | 1 | 12 | TOP |
| H- J | 1 | 12 | SIDE(0.0) |
| J- K | 1 | 12 | TOP |
| S- B | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| S- O | 2 | 12 | TOP |
| O- K | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| G- M | 1 | 4 | SIDE(525.2) |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311498

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| K | 4563 | 0 | 4563 | 0 | 0 | IN-SX | IN-SX |
| S | 2765 | 0 | 2765 | 0 | 0 | MECHANICAL<br>5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-15.

**UNFACTORED REACTIONS**

| | 1ST LCASE | MAX./MIN. COMPONENT REACTIONS | | | | | |
|---|---|---|---|---|---|---|---|
| JT | COMBINED | SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
| K | 3219 | 2158 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 1061 / 0 | 0 / 0 |
| S | 1950 | 1311 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 640 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.07 (1) | 10.00 | |
| B- C | -3763 / 0 | -91.8 | -91.8 | 0.13 (1) | 4.77 | |
| C- D | -4163 / 0 | -91.8 | -91.8 | 0.13 (1) | 4.57 | |
| D- E | -5627 / 0 | -91.8 | -91.8 | 0.33 (1) | 3.86 | |
| E- F | -6857 / 0 | -91.8 | -91.8 | 0.41 (1) | 3.47 | |
| F- G | -7397 / 0 | -91.8 | -91.8 | 0.40 (1) | 3.36 | |
| G- H | -7397 / 0 | -91.8 | -91.8 | 0.42 (1) | 3.33 | |
| H- T | -7397 / 0 | -91.8 | -91.8 | 0.42 (1) | 3.33 | |
| T- U | -7397 / 0 | -91.8 | -91.8 | 0.42 (1) | 3.33 | |
| U- I | -7397 / 0 | -91.8 | -91.8 | 0.42 (1) | 3.33 | |
| I- V | -4494 / 0 | -91.8 | -91.8 | 0.26 (1) | 4.31 | |
| V- W | -4494 / 0 | -91.8 | -91.8 | 0.26 (1) | 4.31 | |
| W- J | -4494 / 0 | -91.8 | -91.8 | 0.26 (1) | 4.31 | |
| K- J | -4321 / 0 | 0.0 | 0.0 | 0.69 (1) | 5.72 | |
| S- B | -2687 / 0 | 0.0 | 0.0 | 0.09 (1) | 7.81 | |
| S- R | 0 / 0 | -18.5 | -18.5 | 0.06 (1) | 10.00 | |
| R- Q | 0 / 3376 | -18.5 | -18.5 | 0.28 (1) | 10.00 | |
| Q- P | 0 / 3706 | -18.5 | -18.5 | 0.27 (1) | 10.00 | |
| P- O | 0 / 5627 | -18.5 | -18.5 | 0.40 (1) | 10.00 | |
| O- N | 0 / 5627 | -18.5 | -18.5 | 0.40 (1) | 10.00 | |
| N- M | 0 / 6857 | -18.5 | -18.5 | 0.52 (1) | 10.00 | |
| M- X | 0 / 4494 | -18.5 | -18.5 | 0.42 (1) | 10.00 | |
| X- Y | 0 / 4494 | -18.5 | -18.5 | 0.42 (1) | 10.00 | |
| Y- L | 0 / 4494 | -18.5 | -18.5 | 0.42 (1) | 10.00 | |
| L- Z | 0 / 0 | -18.5 | -18.5 | 0.10 (1) | 10.00 | |
| Z-AA | 0 / 0 | -18.5 | -18.5 | 0.10 (1) | 10.00 | |
| AA- K | 0 / 0 | -18.5 | -18.5 | 0.10 (1) | 10.00 | |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| R- C | -791 / 0 | 0.07 (1) |
| Q- D | -68 / 36 | 0.01 (1) |
| L- I | -3331 / 0 | 0.51 (1) |
| L- J | 0 / 6096 | 0.75 (1) |
| B- R | 0 / 3456 | 0.43 (1) |
| M- G | -418 / 0 | 0.06 (1) |
| M- I | 0 / 4015 | 0.50 (1) |
| F- M | 0 / 711 | 0.09 (1) |
| D- P | 0 / 2505 | 0.31 (1) |
| N- F | -917 / 0 | 0.14 (1) |
| P- E | -1554 / 0 | 0.24 (1) |
| E- N | 0 / 1619 | 0.20 (1) |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| M | 21-9-8 | -1910 | -1910 | --- | FRONT | VERT | TOTAL | --- | C1 |
| T | 23-8-12 | -23 | -23 | --- | FRONT | VERT | TOTAL | --- | C1 |
| U | 25-8-12 | -23 | -23 | --- | FRONT | VERT | TOTAL | --- | C1 |
| V | 27-8-12 | -23 | -23 | --- | FRONT | VERT | TOTAL | --- | C1 |
| W | 29-8-12 | -23 | -23 | --- | FRONT | VERT | TOTAL | --- | C1 |
| X | 23-8-12 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| Y | 25-8-12 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.69/1.00 (J-K:1) , BC=0.52/1.00 (M-N:1) , WB=0.75/1.00 (J-L:1) , SSI=0.15/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90 )
JSI METAL= 0.81 (L) (INPUT = 0.95 )

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024

CONTINUED ON PAGE 2

RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T48 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



ID:T3iSjALlyhYTloWxxt54GFza3n1-3QG32FoBEb7Q?jTj9KlgyqSKcY30eYxOdQc0Piyx65z

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-p | MT20 | 5.0 | 6.0 | 2.00 | 3.00 |
| C, E, F | | | | | | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW-m | MT20 | 5.0 | 8.0 | 2.25 | 3.00 |
| G | TMW+w | MT20 | 2.0 | 4.0 | | |
| H | TS-t | MT20 | 3.0 | 8.0 | | |
| I | TMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| J | TMVW-t | MT20 | 6.0 | 7.0 | 2.75 | 3.00 |
| K | BMV1+p | MT20 | 3.0 | 8.0 | 4.00 | |
| L | BMWW+t | MT20 | 5.0 | 8.0 | 4.00 | 1.50 |
| M | BMWWW-t | MT20 | 5.0 | 8.0 | 2.50 | 2.25 |
| N | BMWW+t | MT20 | 4.0 | 6.0 | | |
| O | BS-t | MT20 | 5.0 | 6.0 | | |
| P | BMWW-t | MT20 | 5.0 | 6.0 | | |
| Q | BMWW+t | MT20 | 4.0 | 6.0 | | |
| R | BMWW-t | MT20 | 5.0 | 6.0 | | |
| S | BMV1+p | MT20 | 4.0 | 6.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| Z | 27-8-12 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |
| AA | 29-8-12 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.


Structural component only
DWG# T-2311498

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T49 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:49 2023 Page 1
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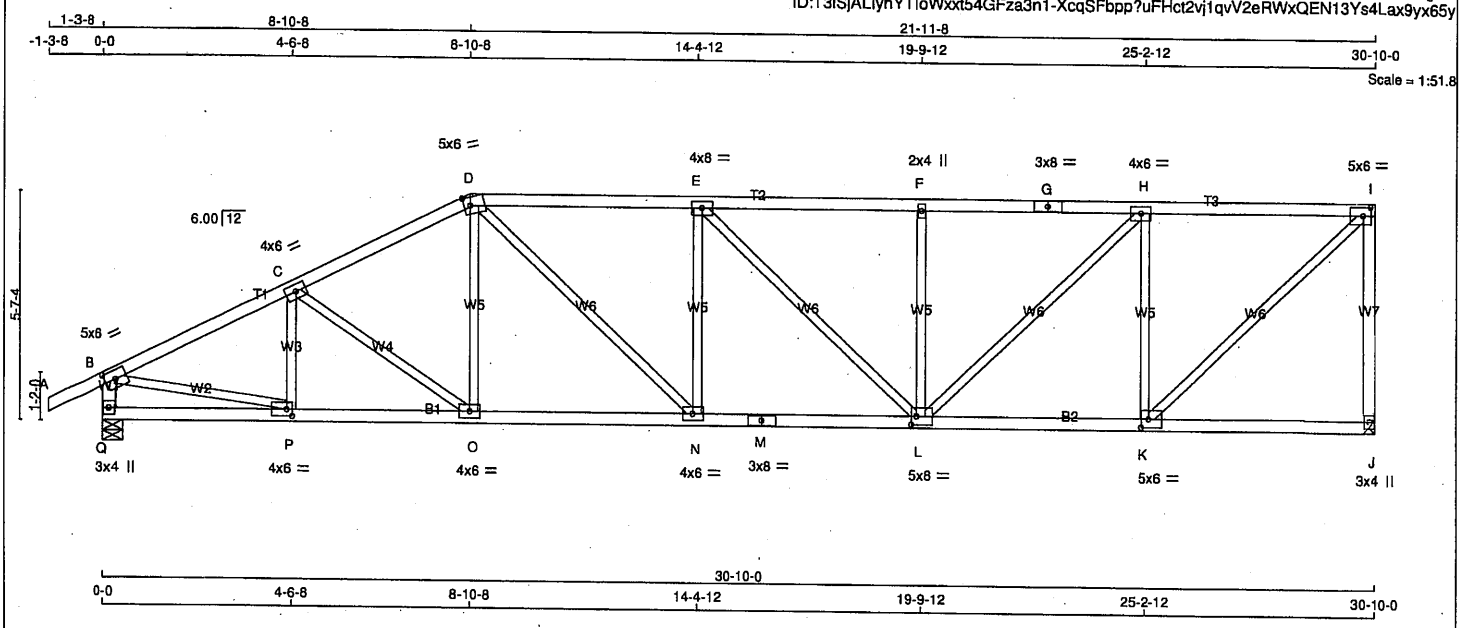


TOTAL WEIGHT = 130 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - G | 2x4 | DRY | No.2 | SPF |
| G - I | 2x4 | DRY | No.2 | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| Q - B | 2x4 | DRY | No.2 | SPF |
| Q - M | 2x4 | DRY | No.2 | SPF |
| M - J | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES** (table is in inches)

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| C, E, H | | | | | | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW-m | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| F | TMW+w | MT20 | 2.0 | 4.0 | | |
| G | TS-t | MT20 | 3.0 | 8.0 | | |
| I | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.25 |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |
| K | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.25 |
| L | BMWWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| M | BS-t | MT20 | 3.0 | 8.0 | | |
| N | BMWW-t | MT20 | 4.0 | 6.0 | | |
| O | BMWW-t | MT20 | 4.0 | 6.0 | | |
| P | BMWW-t | MT20 | 4.0 | 6.0 | 2.00 | 1.50 |
| Q | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311499

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| J | 1700 | 0 | 1700 | 0 | 0 | MECHANICAL | |
| Q | 1824 | 0 | 1824 | 0 | 0 | 5-8 | 2-7 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| J | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |
| Q | 1288 | 858 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 429 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | -2406 / 0 | -91.8 | -91.8 | 0.39 (1) | 4.06 |
| C- D | -2292 / 0 | -91.8 | -91.8 | 0.37 (1) | 4.16 |
| D- E | -2516 / 0 | -91.8 | -91.8 | 0.56 (1) | 3.79 |
| E- F | -2304 / 0 | -91.8 | -91.8 | 0.53 (1) | 3.96 |
| F- G | -2304 / 0 | -91.8 | -91.8 | 0.54 (1) | 3.94 |
| G- H | -2304 / 0 | -91.8 | -91.8 | 0.54 (1) | 3.94 |
| H- I | -1532 / 0 | -91.8 | -91.8 | 0.48 (1) | 4.74 |
| J- I | -1657 / 0 | 0.0 | 0.0 | 0.94 (1) | 6.44 |
| Q- B | -1784 / 0 | 0.0 | 0.0 | 0.18 (1) | 6.26 |
| Q- P | 0 / 0 | -18.5 | -18.5 | 0.08 (4) | 10.00 |
| P- O | 0 / 2170 | -18.5 | -18.5 | 0.40 (1) | 10.00 |
| O- N | 0 / 2034 | -18.5 | -18.5 | 0.38 (1) | 10.00 |
| N- M | 0 / 2516 | -18.5 | -18.5 | 0.46 (1) | 10.00 |
| M- L | 0 / 2516 | -18.5 | -18.5 | 0.46 (1) | 10.00 |
| L- K | 0 / 1532 | -18.5 | -18.5 | 0.32 (1) | 10.00 |
| K- J | 0 / 0 | -18.5 | -18.5 | 0.14 (4) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| P- C | -332 / 0 | 0.07 (1) |
| C- O | -171 / 0 | 0.08 (1) |
| O- D | 0 / 197 | 0.05 (4) |
| B- P | 0 / 2208 | 0.50 (1) |
| K- I | 0 / 2111 | 0.47 (1) |
| D- N | 0 / 668 | 0.15 (1) |
| K- H | -1336 / 0 | 0.63 (1) |
| N- E | -361 / 0 | 0.17 (1) |
| L- H | 0 / 1082 | 0.24 (1) |
| E- L | -297 / 0 | 0.34 (1) |
| L- F | -456 / 0 | 0.22 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.94/1.00 (I-J:1) , BC=0.46/1.00 (L-N:1) , WB=0.63/1.00 (H-K:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90 )
JSI METAL= 0.76 (M) (INPUT = 0.95 )

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T50 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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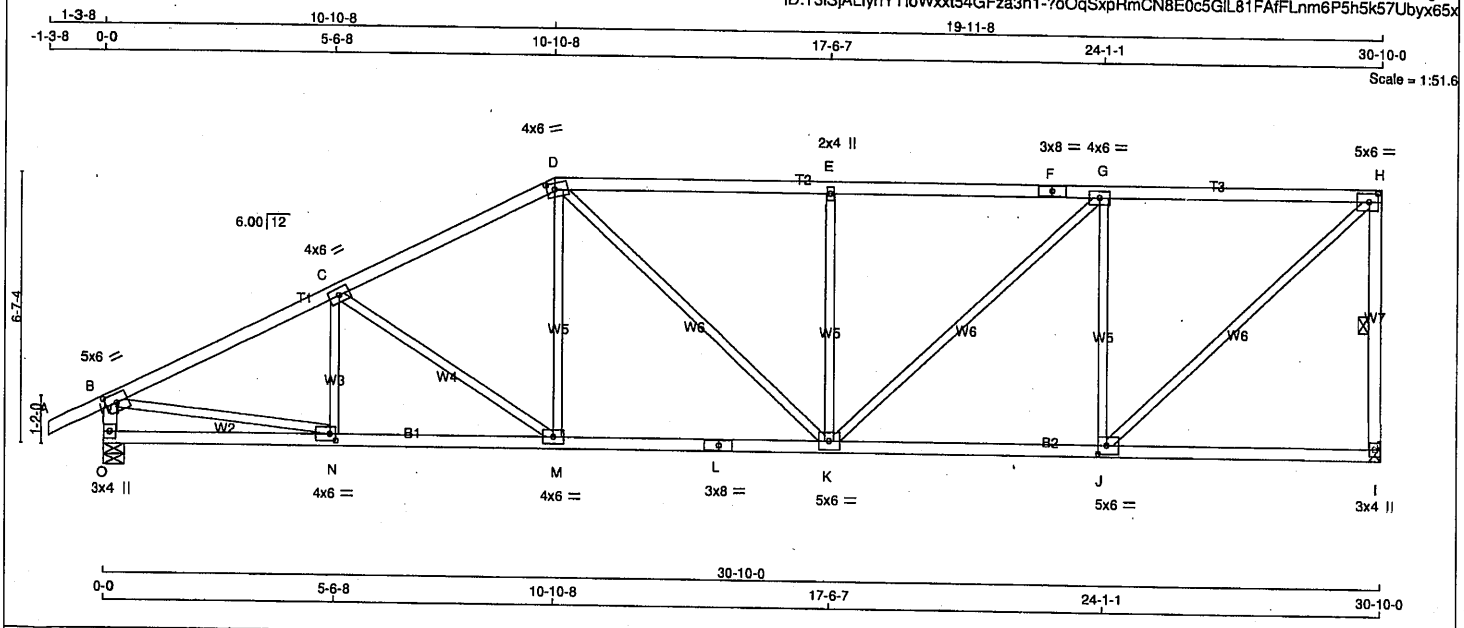


TOTAL WEIGHT = 129 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| O - B | 2x4 | DRY | No.2 | SPF |
| O - L | 2x4 | DRY | No.2 | SPF |
| L - I | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.25 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TS-t | MT20 | 3.0 | 8.0 | | |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TMVW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.50 |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |
| J | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.50 |
| K | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| L | BS-t | MT20 | 3.0 | 8.0 | | |
| M | BMWW-t | MT20 | 4.0 | 6.0 | | |
| N | BMWW-t | MT20 | 4.0 | 6.0 | 2.00 | 1.75 |
| O | BMV1+p | MT20 | 3.0 | 4.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311500

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| I | 1700 | 0 | 1700 | 0 | 0 | IN-SX | IN-SX |
| O | 1824 | 0 | 1824 | 0 | 0 | MECHANICAL<br>5-8 | 2-7 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| I | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |
| O | 1288 | 858 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 429 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | -2461 / 0 | -91.8 | -91.8 | 0.63 (1) | 3.76 |
| C- D | -2155 / 0 | -91.8 | -91.8 | 0.58 (1) | 4.01 |
| D- E | -2106 / 0 | -91.8 | -91.8 | 0.73 (1) | 3.85 |
| E- F | -2106 / 0 | -91.8 | -91.8 | 0.75 (1) | 3.81 |
| F- G | -2106 / 0 | -91.8 | -91.8 | 0.75 (1) | 3.81 |
| G- H | -1495 / 0 | -91.8 | -91.8 | 0.67 (1) | 4.47 |
| I- H | -1649 / 0 | 0.0 | 0.0 | 0.33 (1) | 5.16 |
| O- B | -1778 / 0 | 0.0 | 0.0 | 0.18 (1) | 6.26 |
| O- N | 0 / 0 | -18.5 | -18.5 | 0.12 (4) | 10.00 |
| N- M | 0 / 2224 | -18.5 | -18.5 | 0.44 (1) | 10.00 |
| M- L | 0 / 1908 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| L- K | 0 / 1908 | -18.5 | -18.5 | 0.39 (1) | 10.00 |
| K- J | 0 / 1495 | -18.5 | -18.5 | 0.35 (1) | 10.00 |
| J- I | 0 / 0 | -18.5 | -18.5 | 0.20 (4) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| N- C | -243 / 5 | 0.06 (1) |
| C- M | -386 / 0 | 0.30 (1) |
| M- D | 0 / 332 | 0.07 (1) |
| B- N | 0 / 2250 | 0.51 (1) |
| J- H | 0 / 2047 | 0.46 (1) |
| D- K | 0 / 272 | 0.06 (1) |
| J- G | -1260 / 0 | 0.90 (1) |
| K- E | -659 / 0 | 0.47 (1) |
| K- G | 0 / 848 | 0.19 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.75/1.00 (E-G:1) , BC=0.44/1.00 (M-N:1) , WB=0.90/1.00 (G-J:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90 )
JSI METAL= 0.65 (L) (INPUT = 0.95 )

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T51 | 2 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:51 2023 Page 1
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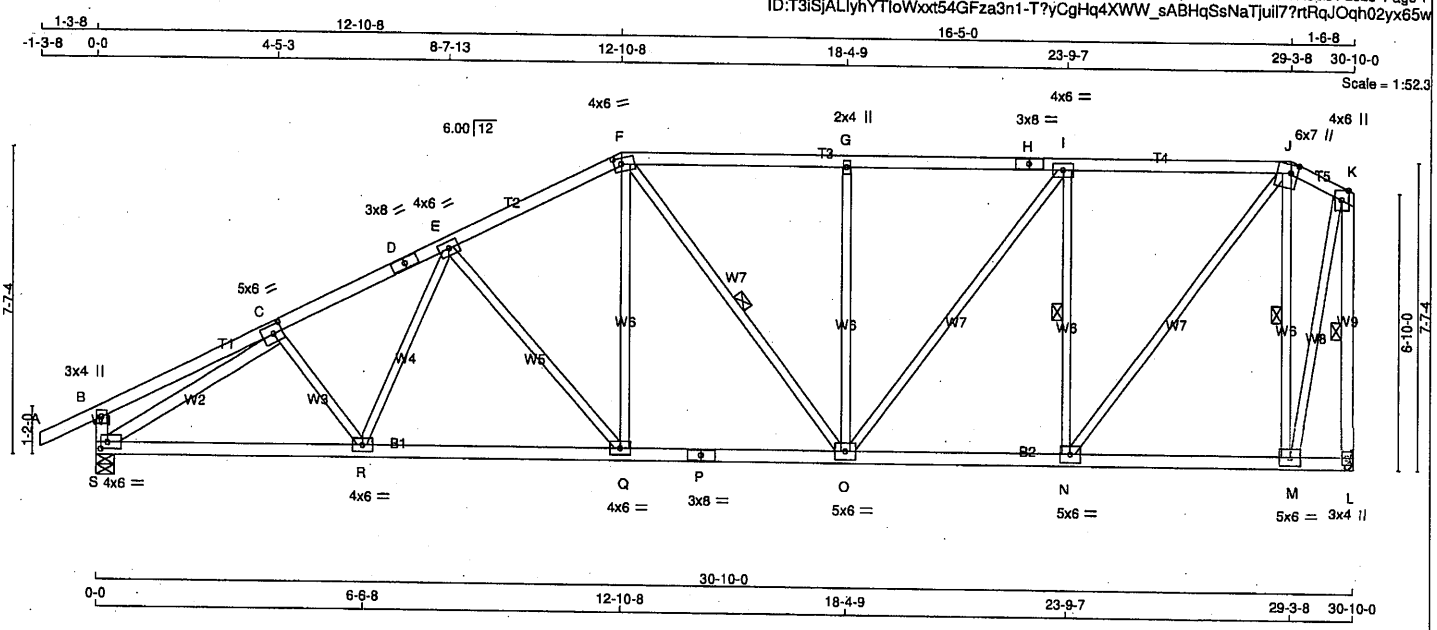


**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| H - J | 2x4 | DRY | No.2 | SPF |
| J - K | 2x4 | DRY | No.2 | SPF |
| S - B | 2x4 | DRY | No.2 | SPF |
| L - K | 2x4 | DRY | No.2 | SPF |
| S - P | 2x4 | DRY | No.2 | SPF |
| P - L | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| S - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.75 |
| D | TS-t | MT20 | 3.0 | 8.0 | | |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.25 |
| G | TMW+w | MT20 | 2.0 | 4.0 | | |
| H | TS-t | MT20 | 3.0 | 8.0 | | |
| I | TMWW-t | MT20 | 4.0 | 6.0 | | |
| J | TTWW+m | MT20 | 6.0 | 7.0 | 2.50 | 2.00 |
| K | TMVW+p | MT20 | 4.0 | 6.0 | Edge | |
| L | BMV1+p | MT20 | 3.0 | 4.0 | | |
| M | BMWW-t | MT20 | 5.0 | 6.0 | | |
| N | BMWW-t | MT20 | 5.0 | 6.0 | | |
| O | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| P | BS-t | MT20 | 3.0 | 8.0 | | |
| Q | BMWW-t | MT20 | 4.0 | 6.0 | | |
| R | BMWW-t | MT20 | 4.0 | 6.0 | | |
| S | BMVW1-t | MT20 | 4.0 | 6.0 | 2.00 | 2.00 |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only.
DWG# T-2311501

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| S | 1824 | 0 | 1824 | 0 | 0 | 5-8 | 2-0 |
| L | 1700 | 0 | 1700 | 0 | 0 | IN-SX<br>MECHANICAL | IN-SX |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| S | 1288 | 858 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 429 / 0 | 0 / 0 |
| L | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-O, I-N, J-M, K-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**
TOTAL LOAD CASES: (4)

CHORDS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 FROM | TO | MAX (PLF) CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | 0 / 15 | -91.8 | -91.8 | 0.20 (1) | 10.00 |
| C- D | -2371 / 0 | -91.8 | -91.8 | 0.32 (1) | 4.17 |
| D- E | -2371 / 0 | -91.8 | -91.8 | 0.32 (1) | 4.17 |
| E- F | -1970 / 0 | -91.8 | -91.8 | 0.29 (1) | 4.53 |
| F- G | -1765 / 0 | -91.8 | -91.8 | 0.45 (1) | 4.52 |
| G- H | -1765 / 0 | -91.8 | -91.8 | 0.45 (1) | 4.52 |
| H- I | -1765 / 0 | -91.8 | -91.8 | 0.45 (1) | 4.52 |
| I- J | -1307 / 0 | -91.8 | -91.8 | 0.42 (1) | 5.12 |
| J- K | -366 / 0 | -91.8 | -91.8 | 0.04 (1) | 6.25 |
| S- B | -289 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 |
| L- K | -1695 / 0 | 0.0 | 0.0 | 0.37 (1) | 5.10 |
| S- R | 0 / 2125 | -18.5 | -18.5 | 0.44 (1) | 10.00 |
| R- Q | 0 / 2067 | -18.5 | -18.5 | 0.42 (1) | 10.00 |
| Q- P | 0 / 1753 | -18.5 | -18.5 | 0.35 (1) | 10.00 |
| P- O | 0 / 1753 | -18.5 | -18.5 | 0.35 (1) | 10.00 |
| O- N | 0 / 1307 | -18.5 | -18.5 | 0.28 (1) | 10.00 |
| N- M | 0 / 342 | -18.5 | -18.5 | 0.15 (4) | 10.00 |
| M- L | 0 / 0 | -18.5 | -18.5 | 0.07 (4) | 10.00 |

WEBS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| C- R | -5 / 58 | 0.02 (4) |
| R- E | 0 / 147 | 0.04 (4) |
| E- Q | -501 / 0 | 0.43 (1) |
| Q- F | 0 / 496 | 0.11 (1) |
| F- O | -5 / 21 | 0.00 (1) |
| O- G | -545 / 0 | 0.58 (1) |
| O- I | 0 / 771 | 0.17 (1) |
| N- I | -1168 / 0 | 0.42 (1) |
| N- J | 0 / 1603 | 0.36 (1) |
| M- J | -1392 / 0 | 0.51 (1) |
| S- C | -2592 / 0 | 0.83 (1) |
| M- K | 0 / 1495 | 0.34 (1) |

TOTAL WEIGHT = 2 X 150 = 300 lb [M]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.45/1.00 (G-I:1) , BC=0.44/1.00 (R-S:1) , WB=0.83/1.00 (C-S:1) , SSI=0.23/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (S) (INPUT = 0.90 )
JSI METAL= 0.58 (C) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T52 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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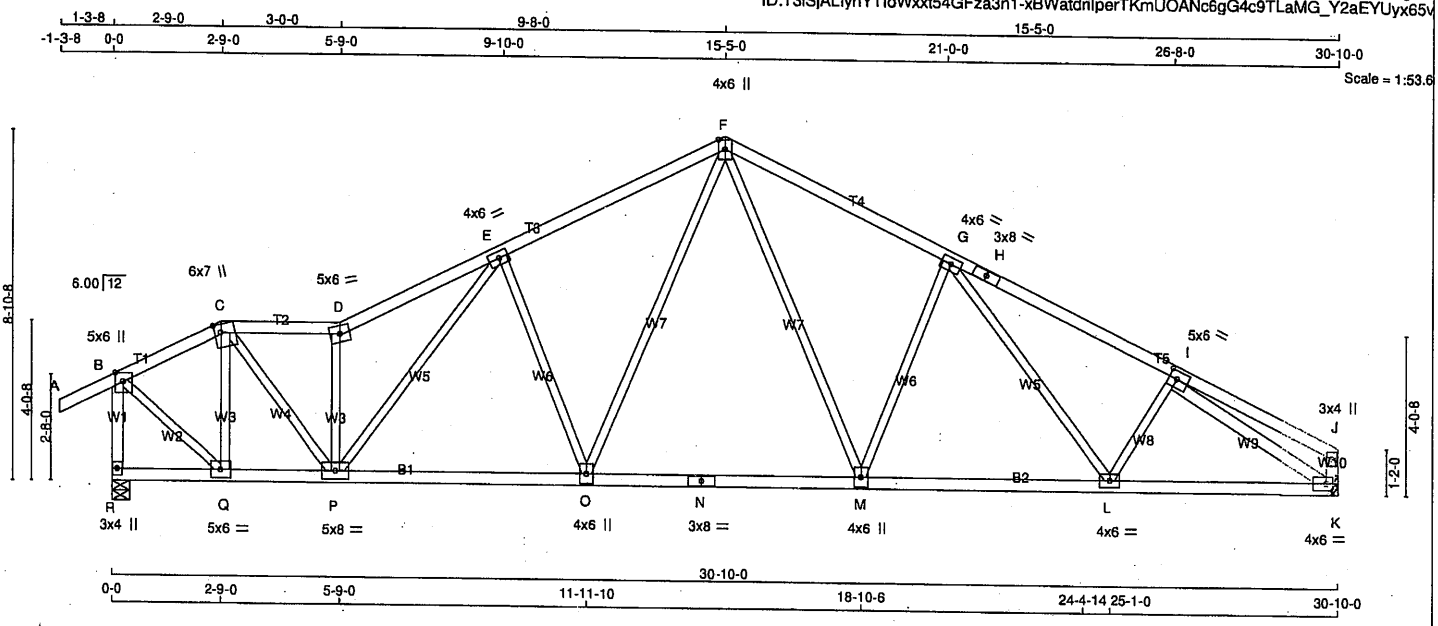


**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| H - J | 2x4 | DRY | No.2 | SPF |
| R - B | 2x4 | DRY | No.2 | SPF |
| K - J | 2x4 | DRY | No.2 | SPF |
| R - N | 2x4 | DRY | No.2 | SPF |
| N - K | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| I - K | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (teble is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW+p | MT20 | 5.0 | 6.0 | Edge | |
| C | TTWW+m | MT20 | 6.0 | 7.0 | 2.50 | 2.00 |
| D | TTW-m | MT20 | 5.0 | 6.0 | | |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TTWW+p | MT20 | 4.0 | 6.0 | Edge | |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TS-t | MT20 | 3.0 | 8.0 | | |
| I | TMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.50 |
| J | TMV+p | MT20 | 3.0 | 4.0 | | |
| K | BMVW1-t | MT20 | 4.0 | 6.0 | 2.00 | 2.00 |
| L | BMWW-t | MT20 | 4.0 | 6.0 | | |
| M | BMWW+t | MT20 | 4.0 | 6.0 | | |
| N | BS-t | MT20 | 3.0 | 8.0 | | |
| O | BMWW+t | MT20 | 4.0 | 6.0 | | |
| P | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| Q | BMWW-t | MT20 | 5.0 | 6.0 | | |
| R | BMV1+p | MT20 | 3.0 | 4.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311502

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| R | 1824 | 0 | 1824 | 0 | 0 | 5-8 | 2-7 |
| K | 1700 | 0 | 1700 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-13.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| R | 1288 | 858 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 429 / 0 | 0 / 0 |
| K | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

CHORDS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 1 | 0.12 (1) | 10.00 |
| B- C | -1309 / 0 | -91.8 | -91.8 | 1 | 0.11 (1) | 5.55 |
| C- D | -2093 / 0 | -91.8 | -91.8 | 1 | 0.15 (1) | 4.57 |
| D- E | -2348 / 0 | -91.8 | -91.8 | 1 | 0.37 (1) | 4.13 |
| E- F | -2040 / 0 | -91.8 | -91.8 | 1 | 0.41 (1) | 4.35 |
| F- G | -2044 / 0 | -91.8 | -91.8 | 1 | 0.42 (1) | 4.33 |
| G- H | -2355 / 0 | -91.8 | -91.8 | 1 | 0.44 (1) | 4.08 |
| H- I | -2355 / 0 | -91.8 | -91.8 | 1 | 0.44 (1) | 4.08 |
| I- J | 0 / 20 | -91.8 | -91.8 | 1 | 0.25 (1) | 10.00 |
| R- B | -1805 / 0 | 0.0 | 0.0 | 1 | 0.25 (1) | 6.23 |
| K- J | -142 / 0 | 0.0 | 0.0 | 1 | 0.01 (1) | 7.81 |
| R- Q | 0 / 0 | -18.5 | -18.5 | 1 | 0.03 (4) | 10.00 |
| Q- P | 0 / 1144 | -18.5 | -18.5 | 1 | 0.21 (1) | 10.00 |
| P- O | 0 / 2018 | -18.5 | -18.5 | 1 | 0.43 (1) | 10.00 |
| O- N | 0 / 1525 | -18.5 | -18.5 | 1 | 0.35 (1) | 10.00 |
| N- M | 0 / 1525 | -18.5 | -18.5 | 1 | 0.35 (1) | 10.00 |
| M- L | 0 / 2025 | -18.5 | -18.5 | 1 | 0.41 (1) | 10.00 |
| L- K | 0 / 2125 | -18.5 | -18.5 | 1 | 0.42 (1) | 10.00 |

WEBS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| Q- C | -902 / 0 | 0.22 (1) |
| C- P | 0 / 1551 | 0.35 (1) |
| P- D | -1314 / 0 | 0.33 (1) |
| P- E | 0 / 191 | 0.04 (1) |
| E- O | -600 / 0 | 0.41 (1) |
| O- F | 0 / 750 | 0.17 (1) |
| F- M | 0 / 752 | 0.17 (1) |
| M- G | -615 / 0 | 0.42 (1) |
| G- L | 0 / 159 | 0.04 (1) |
| L- I | -18 / 59 | 0.02 (4) |
| B- Q | 0 / 1516 | 0.34 (1) |
| I- K | -2608 / 0 | 0.73 (1) |

TOTAL WEIGHT = 3 X 134 = 403 lb [M][F]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.44/1.00 (G-I:1) , BC=0.43/1.00 (O-P:1) , WB=0.73/1.00 (I-K:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90 )
JSI METAL= 0.59 (I) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T53 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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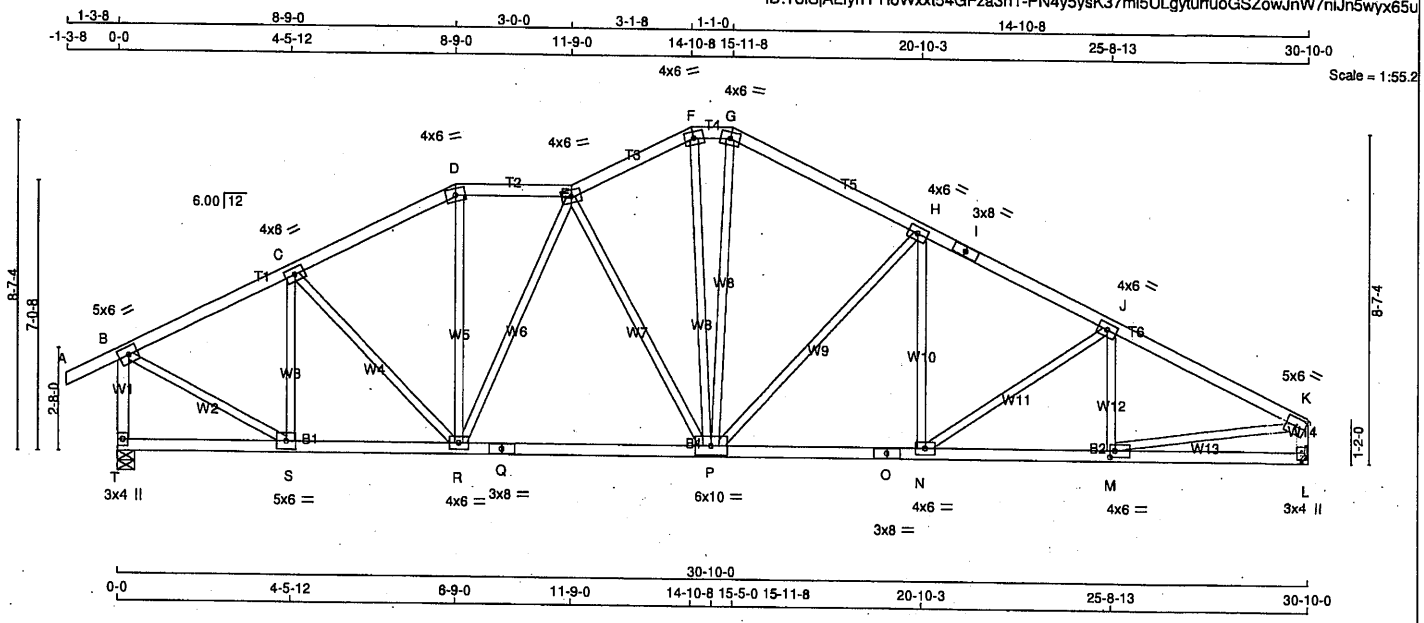


TOTAL WEIGHT = 143 lb [M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - E | 2x4 | DRY | No.2 | SPF |
| E - F | 2x4 | DRY | No.2 | SPF |
| F - G | 2x4 | DRY | No.2 | SPF |
| G - I | 2x4 | DRY | No.2 | SPF |
| I - K | 2x4 | DRY | No.2 | SPF |
| T - B | 2x4 | DRY | No.2 | SPF |
| L - K | 2x4 | DRY | No.2 | SPF |
| T - Q | 2x4 | DRY | No.2 | SPF |
| Q - O | 2x4 | DRY | No.2 | SPF |
| O - L | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X | |
|---|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | | | |
| C, H, J | | | | | | | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | | |
| D, F, G | | | | | | | |
| D | TTW-m | MT20 | 4.0 | 6.0 | | | |
| E | TTWW-m | MT20 | 4.0 | 6.0 | | | |
| I | TS-t | MT20 | 3.0 | 8.0 | | | |
| K | TMVW-t | MT20 | 5.0 | 6.0 | | | Edge |
| L | BMV1+p | MT20 | 3.0 | 4.0 | | | |
| M | BMWW-t | MT20 | 4.0 | 6.0 | 2.00 | 1.50 | |
| N | BMWW-t | MT20 | 4.0 | 6.0 | | | |
| O | BS-t | MT20 | 3.0 | 8.0 | | | |
| P | BMWWW*-I | MT20 | 6.0 | 10.0 | | | |
| Q | BS-t | MT20 | 3.0 | 8.0 | | | |
| R | BMWW-t | MT20 | 4.0 | 6.0 | | | |
| S | BMWW-t | MT20 | 5.0 | 6.0 | | | |
| T | BMV1+p | MT20 | 3.0 | 4.0 | | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311503

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| T | 1824 | 0 | 1824 | 0 | 0 | 5-8 | 2-7 |
| L | 1700 | 0 | 1700 | 0 | 0 | IN-SX<br>MECHANICAL | IN-SX |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| T | 1288 | 858 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 429 / 0 | 0 / 0 |
| L | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A-B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | S- C | -689 / 0 | 0.25 (1) |
| B- C | -1617 / 0 | -91.8 | -91.8 | 0.25 (1) | 4.96 | C- R | 0 / 179 | 0:04 (1) |
| C- D | -1798 / 0 | -91.8 | -91.8 | 0.26 (1) | 4.75 | R- D | 0 / 511 | 0.12 (1) |
| D- E | -1596 / 0 | -91.8 | -91.8 | 0.13 (1) | 5.11 | E- P | -586 / 0 | 0.68 (1) |
| E- F | -1727 / 0 | -91.8 | -91.8 | 0.15 (1) | 4.94 | N- H | 0 / 221 | 0.05 (4) |
| F- G | -1582 / 0 | -91.8 | -91.8 | 0.04 (1) | 5.24 | N- J | -230 / 0 | 0.14 (1) |
| G- H | -1752 / 0 | -91.8 | -91.8 | 0.29 (1) | 4.76 | M- J | -272 / 0 | 0.06 (1) |
| H- I | -2238 / 0 | -91.8 | -91.8 | 0.30 (1) | 4.31 | B- S | 0 / 1655 | 0.37 (1) |
| I- J | -2236 / 0 | -91.8 | -91.8 | 0.30 (1) | 4.31 | M- K | 0 / 2222 | 0.50 (1) |
| J- K | -2431 / 0 | -91.8 | -91.8 | 0.37 (1) | 4.10 | P- G | 0 / 546 | 0.12 (1) |
| T- B | -1788 / 0 | 0.0 | 0.0 | 0.25 (1) | 6.25 | F- P | 0 / 579 | 0.13 (1) |
| L- K | -1657 / 0 | 0.0 | 0.0 | 0.17 (1) | 6.44 | P- H | -666 / 0 | 0.86 (1) |
| | | | | | | R- E | -585 / 0 | 0.62 (1) |
| T- S | 0 / 0 | -18.5 | -18.5 | 0.08 (4) | 10.00 | | | |
| S- R | 0 / 1465 | -18.5 | -18.5 | 0.30 (1) | 10.00 | | | |
| R- Q | 0 / 1827 | -18.5 | -18.5 | 0.38 (1) | 10.00 | | | |
| Q- P | 0 / 1827 | -18.5 | -18.5 | 0.38 (1) | 10.00 | | | |
| P- O | 0 / 2002 | -18.5 | -18.5 | 0.41 (1) | 10.00 | | | |
| O- N | 0 / 2002 | -18.5 | -18.5 | 0.41 (1) | 10.00 | | | |
| N- M | 0 / 2191 | -18.5 | -18.5 | 0.39 (1) | 10.00 | | | |
| M- L | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (J-K:1) , BC=0.41/1.00 (N-P:1) , WB=0.86/1.00 (H-P:1) , SSI=0.19/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90 )
JSI METAL= 0.58 (K) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T54 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:54 2023 Page 1
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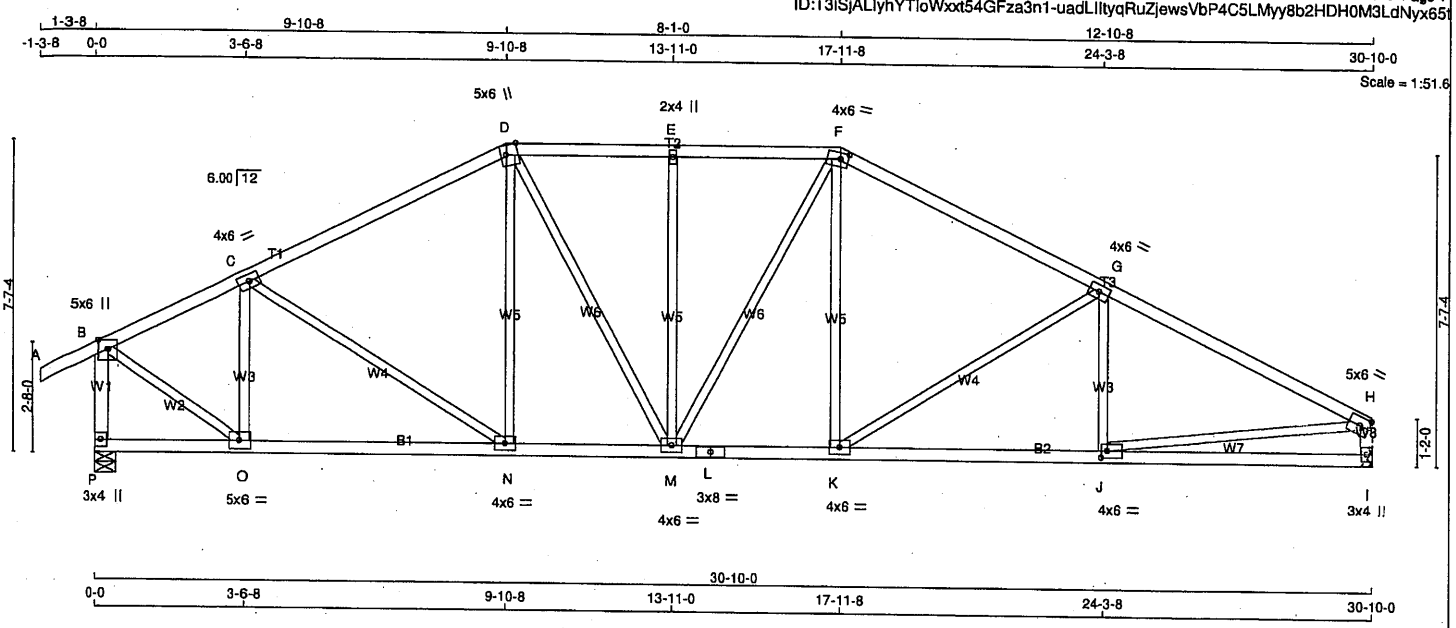





TOTAL WEIGHT = 135 lb

[M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| P - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| P - L | 2x4 | DRY | No.2 | SPF |
| L - I | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW+p | MT20 | 5.0 | 6.0 | Edge | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | TTWW+m | MT20 | 5.0 | 6.0 | Edge | 3.50 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.25 |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TMVW-t | MT20 | 5.0 | 6.0 | | Edge |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |
| J | BMWW-t | MT20 | 4.0 | 6.0 | 2.00 | 1.75 |
| K | BMWW-t | MT20 | 4.0 | 6.0 | | |
| L | BS-t | MT20 | 3.0 | 8.0 | | |
| M | BMWWW-t | MT20 | 4.0 | 6.0 | | |
| N | BMWW-t | MT20 | 4.0 | 6.0 | | |
| O | BMWW-t | MT20 | 5.0 | 6.0 | | |
| P | BMV1+p | MT20 | 3.0 | 4.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| P | 1824 | 0 | 1824 | 0 | 0 | 5-8 | 2-7 |
| I | 1700 | 0 | 1700 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| P | 1288 | 858 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 429 / 0 | 0 / 0 |
| I | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A-B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B-C | -1503 / 0 | -91.8 | -91.8 | 0.38 (1) | 4.92 |
| C-D | -1780 / 0 | -91.8 | -91.8 | 0.50 (1) | 4.47 |
| D-E | -1799 / 0 | -91.8 | -91.8 | 0.22 (1) | 4.79 |
| E-F | -1799 / 0 | -91.8 | -91.8 | 0.22 (1) | 4.79 |
| F-G | -1985 / 0 | -91.8 | -91.8 | 0.57 (1) | 4.17 |
| G-H | -2480 / 0 | -91.8 | -91.8 | 0.64 (1) | 3.73 |
| P-B | -1803 / 0 | 0.0 | 0.0 | 0.25 (1) | 6.23 |
| I-H | -1649 / 0 | 0.0 | 0.0 | 0.17 (1) | 6.45 |
| P-O | 0 / 0 | -18.5 | -18.5 | 0.12 (4) | 10.00 |
| O-N | 0 / 1382 | -18.5 | -18.5 | 0.30 (1) | 10.00 |
| N-M | 0 / 1570 | -18.5 | -18.5 | 0.33 (1) | 10.00 |
| M-L | 0 / 1752 | -18.5 | -18.5 | 0.35 (1) | 10.00 |
| L-K | 0 / 1752 | -18.5 | -18.5 | 0.35 (1) | 10.00 |
| K-J | 0 / 2246 | -18.5 | -18.5 | 0.44 (1) | 10.00 |
| J-I | 0 / 0 | -18.5 | -18.5 | 0.18 (4) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| O-C | -818 / 0 | 0.24 (1) |
| C-N | 0 / 224 | 0.05 (1) |
| N-D | -10 / 74 | 0.03 (4) |
| D-M | 0 / 472 | 0.11 (1) |
| M-E | -448 / 0 | 0.47 (1) |
| M-F | 0 / 96 | 0.02 (1) |
| K-F | 0 / 426 | 0.10 (1) |
| K-G | -594 / 0 | 0.70 (1) |
| J-G | -163 / 54 | 0.05 (1) |
| B-O | 0 / 1660 | 0.37 (1) |
| J-H | 0 / 2265 | 0.51 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.64/1.00 (G-H:1) , BC=0.44/1.00 (J-K:1) , WB=0.70/1.00 (G-K:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90 )
JSI METAL= 0.61 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311504

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T55 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:55 2023 Page 1
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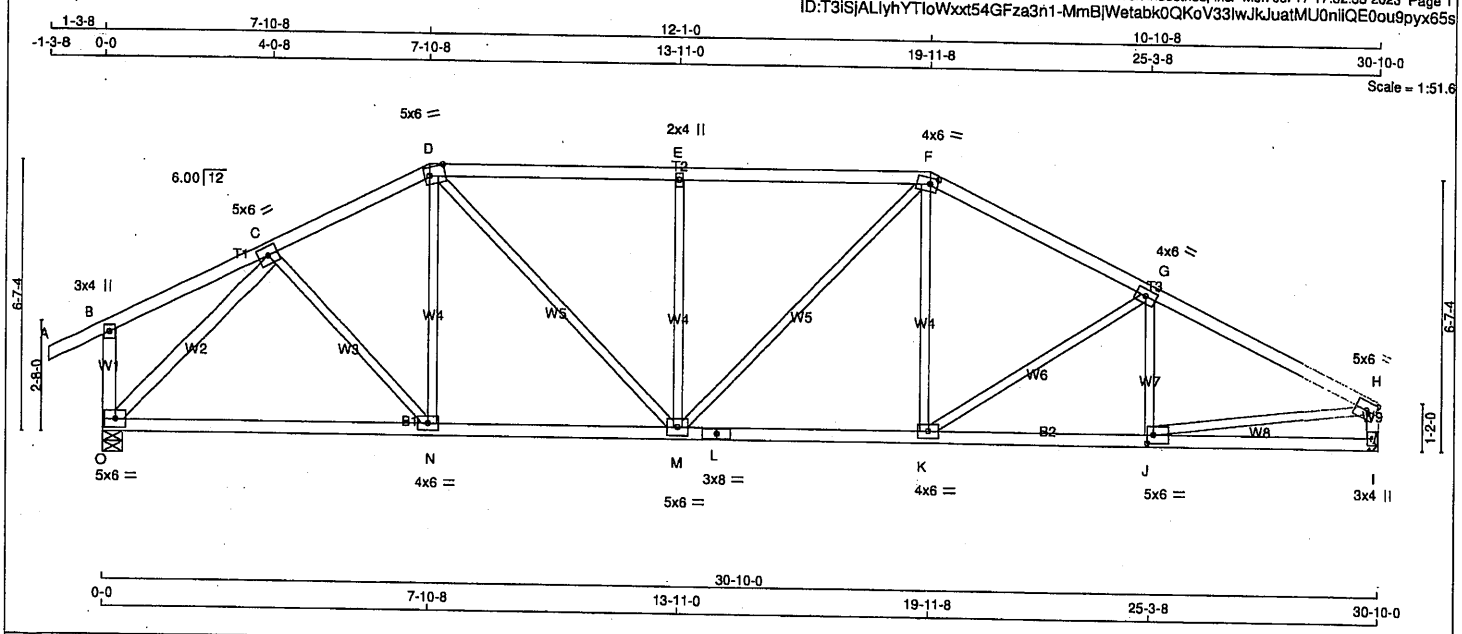


TOTAL WEIGHT = 129 lb [M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - H | 2x4 | DRY | No.2 | SPF |
| O - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| O - L | 2x4 | DRY | No.2 | SPF |
| L - I | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| O - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 5.0 | 6.0 | | |
| D | TTWW-m | MT20 | 5.0 | 6.0 | Edge | 4.50 |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.25 |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TMVW-t | MT20 | 5.0 | 6.0 | | Edge |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |
| J | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| K | BMWW-t | MT20 | 4.0 | 6.0 | | |
| L | BS-t | MT20 | 3.0 | 8.0 | | |
| M | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| N | BMWW-t | MT20 | 4.0 | 6.0 | | |
| O | BMVW1-t | MT20 | 5.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311505

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| O | 1824 | 0 | 1824 | 0 | 0 | IN-SX | IN-SX |
| I | 1700 | 0 | 1700 | 0 | 0 | 5-8 | 2-0 |
| | | | | | | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| O | 1288 | 858 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 429 / 0 | 0 / 0 |
| I | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | | 10.00 |
| B- C | 0 / 19 | -91.8 | -91.8 | 0.22 (1) | | 10.00 |
| C- D | -1810 / 0 | -91.8 | -91.8 | 0.21 (1) | | 4.79 |
| D- E | -2124 / 0 | -91.8 | -91.8 | 0.50 (1) | | 4.13 |
| E- F | -2124 / 0 | -91.8 | -91.8 | 0.50 (1) | | 4.13 |
| F- G | -2154 / 0 | -91.8 | -91.8 | 0.41 (1) | | 4.24 |
| G- H | -2461 / 0 | -91.8 | -91.8 | 0.44 (1) | | 3.99 |
| O- B | -264 / 0 | 0.0 | 0.0 | 0.04 (1) | | 7.81 |
| I- H | -1654 / 0 | 0.0 | 0.0 | 0.17 (1) | | 6.45 |
| O- N | 0 / 1365 | -18.5 | -18.5 | 0.36 (1) | | 10.00 |
| N- M | 0 / 1599 | -18.5 | -18.5 | 0.40 (1) | | 10.00 |
| M- L | 0 / 1908 | -18.5 | -18.5 | 0.38 (1) | | 10.00 |
| L- K | 0 / 1908 | -18.5 | -18.5 | 0.38 (1) | | 10.00 |
| K- J | 0 / 2224 | -18.5 | -18.5 | 0.43 (1) | | 10.00 |
| J- I | 0 / 0 | -18.5 | -18.5 | 0.11 (4) | | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| C- N | 0 / 345 | 0.08 (1) |
| N- D | -97 / 48 | 0.07 (1) |
| D- M | 0 / 759 | 0.17 (1) |
| M- E | -682 / 0 | 0.49 (1) |
| M- F | 0 / 313 | 0.07 (1) |
| K- F | 0 / 329 | 0.07 (1) |
| K- G | -387 / 0 | 0.30 (1) |
| J- G | -242 / 6 | 0.06 (1) |
| O- C | -2028 / 0 | 0.82 (1) |
| J- H | 0 / 2250 | 0.51 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.43/1.00 (J-K:1) , WB=0.82/1.00 (C-O:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90 )
JSI METAL= 0.59 (H) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T56 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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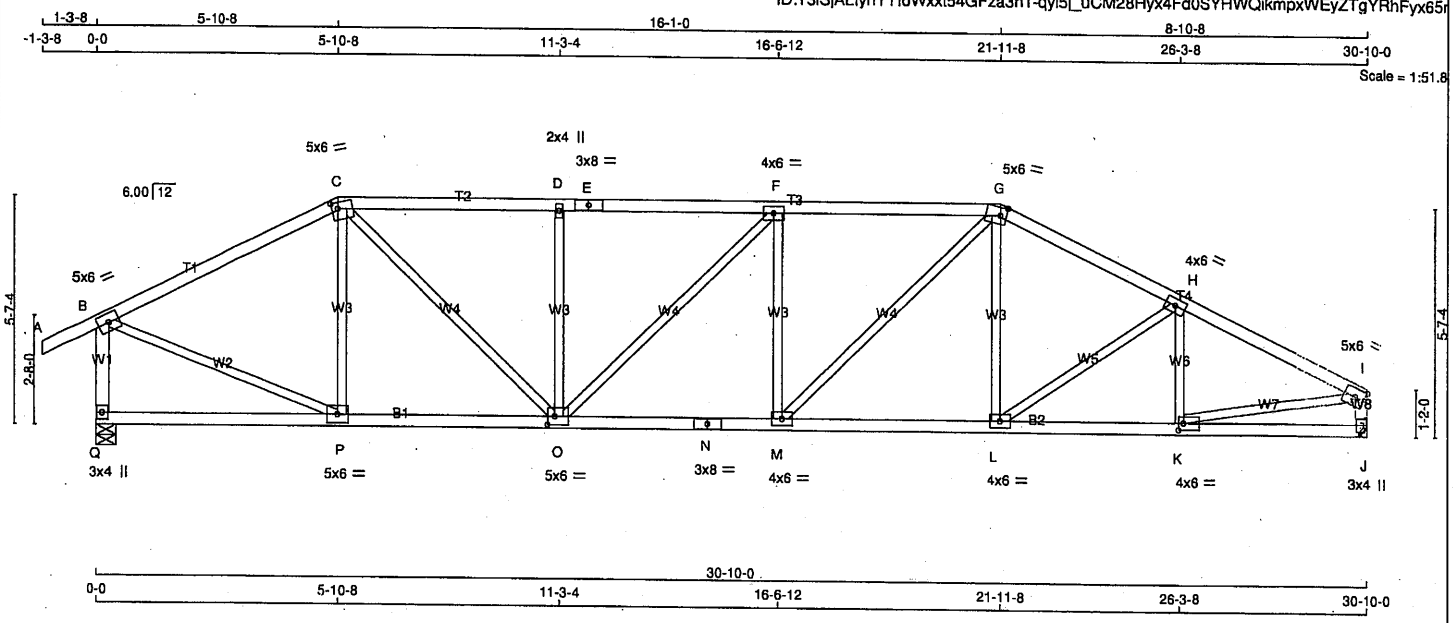





TOTAL WEIGHT = 126 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| E - G | 2x4 | DRY | No.2 | SPF |
| G - I | 2x4 | DRY | No.2 | SPF |
| Q - B | 2x4 | DRY | No.2 | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| Q - N | 2x4 | DRY | No.2 | SPF |
| N - J | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 5.0 | 6.0 | | |
| C | TTWW-m | MT20 | 5.0 | 6.0 | 1.75 | 1.75 |
| D | TMW+w | MT20 | 2.0 | 4.0 | | |
| E | TS-t | MT20 | 3.0 | 8.0 | | |
| F | TMWW-t | MT20 | 4.0 | 6.0 | | |
| G | TTWW-m | MT20 | 5.0 | 6.0 | 2.50 | 1.75 |
| H | TMWW-t | MT20 | 4.0 | 6.0 | | |
| I | TMVW-t | MT20 | 5.0 | 6.0 | | Edge |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |
| K | BMWW-t | MT20 | 4.0 | 6.0 | 2.00 | 1.50 |
| L | BMWW-t | MT20 | 4.0 | 6.0 | | |
| M | BMWW-t | MT20 | 4.0 | 6.0 | | |
| N | BS-t | MT20 | 3.0 | 8.0 | | |
| O | BMWWW-t | MT20 | 5.0 | 6.0 | 2.50 | 2.00 |
| P | BMWW-t | MT20 | 5.0 | 6.0 | | |
| Q | BMV1+p | MT20 | 3.0 | 4.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311506

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| Q | 1824 | 0 | 1824 | 0 | 0 | IN-SX | IN-SX |
| J | 1700 | 0 | 1700 | 0 | 0 | 5-8<br>MECHANICAL | 2-7 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-0.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| Q | 1288 | 858 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 429 / 0 | 0 / 0 |
| J | 1202 | 789 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 413 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A-B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | | 10.00 |
| B-C | -1730 / 0 | -91.8 | -91.8 | 0.69 (1) | | 4.20 |
| C-D | -2339 / 0 | -91.8 | -91.8 | 0.47 (1) | | 4.02 |
| D-E | -2339 / 0 | -91.8 | -91.8 | 0.47 (1) | | 4.00 |
| E-F | -2339 / 0 | -91.8 | -91.8 | 0.47 (1) | | 4.00 |
| F-G | -2507 / 0 | -91.8 | -91.8 | 0.49 (1) | | 3.88 |
| G-H | -2292 / 0 | -91.8 | -91.8 | 0.37 (1) | | 4.16 |
| H-I | -2407 / 0 | -91.8 | -91.8 | 0.39 (1) | | 4.06 |
| Q-B | -1779 / 0 | 0.0 | 0.0 | 0.25 (1) | | 6.26 |
| J-I | -1659 / 0 | 0.0 | 0.0 | 0.17 (1) | | 6.44 |
| Q-P | 0 / 0 | -18.5 | -18.5 | 0.15 (4) | | 10.00 |
| P-O | 0 / 1537 | -18.5 | -18.5 | 0.33 (1) | | 10.00 |
| O-N | 0 / 2507 | -18.5 | -18.5 | 0.45 (1) | | 10.00 |
| N-M | 0 / 2507 | -18.5 | -18.5 | 0.45 (1) | | 10.00 |
| M-L | 0 / 2034 | -18.5 | -18.5 | 0.38 (1) | | 10.00 |
| L-K | 0 / 2170 | -18.5 | -18.5 | 0.40 (1) | | 10.00 |
| K-J | 0 / 0 | -18.5 | -18.5 | 0.08 (4) | | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| P-C | -488 / 0 | 0.23 (1) |
| C-O | 0 / 1125 | 0.25 (1) |
| O-D | -532 / 0 | 0.25 (1) |
| O-F | -238 / 0 | 0.26 (1) |
| M-F | -365 / 0 | 0.17 (1) |
| M-G | 0 / 663 | 0.15 (1) |
| L-G | 0 / 196 | 0.04 (4) |
| L-H | -172 / 0 | 0.08 (1) |
| K-H | -332 / 0 | 0.07 (1) |
| B-P | 0 / 1662 | 0.37 (1) |
| K-I | 0 / 2208 | 0.50 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.45/1.00 (M-O:1) , WB=0.50/1.00 (I-K:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90 )
JSI METAL= 0.77 (N) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T57 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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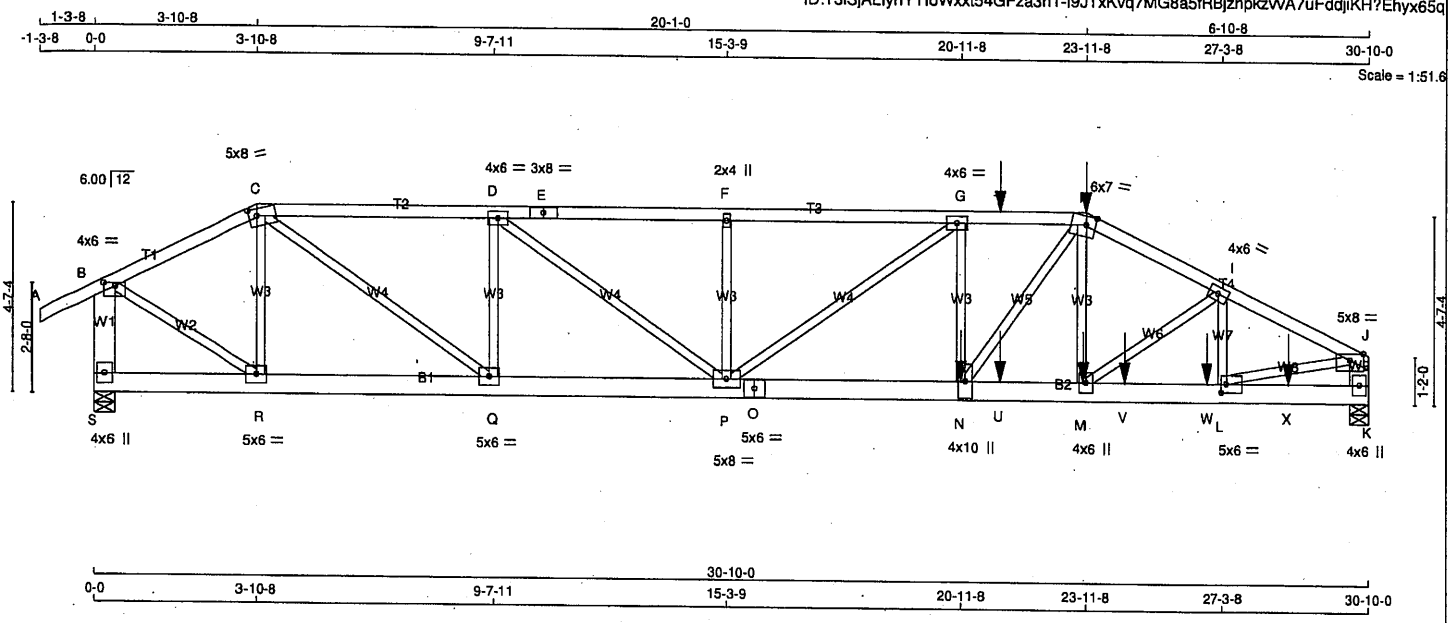


**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| E - H | 2x4 | DRY | No.2 | SPF |
| H - J | 2x4 | DRY | No.2 | SPF |
| S - B | 2x6 | DRY | No.2 | SPF |
| K - J | 2x6 | DRY | No.2 | SPF |
| S - O | 2x6 | DRY | No.2 | SPF |
| O - K | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| A- C | 1 | 12 | TOP |
| C- E | 1 | 12 | TOP |
| E- H | 1 | 12 | SIDE(61.0) |
| H- J | 1 | 12 | SIDE(61.0) |
| S- B | 2 | 12 | TOP |
| K- J | 2 | 12 | TOP |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| S- O | 2 | 12 | TOP |
| O- K | 2 | 12 | SIDE(183.1) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| M- H | 1 | 6 | SIDE(9.0) |
| G- N | 1 | 4 | SIDE(490.5) |
| 2x3 | 1 | 6 | |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.


Structural component only
DWG# T-2311507

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| S | 2918 | 0 | 2918 | 0 | 0 | 5-8 | 1-9 |
| K | 4375 | 0 | 4375 | 0 | 0 | 5-8 | 2-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| S | 2057 | 1388 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 669 / 0 | 0 / 0 |
| K | 3088 | 2062 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 1026 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. | FORCE (LBS) | FACTORED VERT. LOAD LC1 FROM | TO | MAX (PLF) | CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. | FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| FR-TO | | | | | | | FR-TO | | |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.07 (1) | | 10.00 | R- C | -1252 / 0 | 0.19 (1) |
| B- C | -2709 / 0 | -91.8 | -91.8 | 0.16 (1) | | 5.38 | M- H | 0 / 143 | 0.03 (4) |
| C- D | -5215 / 0 | -91.8 | -91.8 | 0.40 (1) | | 3.93 | M- I | 0 / 900 | 0.11 (1) |
| D- E | -7092 / 0 | -91.8 | -91.8 | 0.53 (1) | | 3.33 | L- I | -1266 / 0 | 0.12 (1) |
| E- F | -7092 / 0 | -91.8 | -91.8 | 0.53 (1) | | 3.33 | B- R | 0 / 2789 | 0.35 (1) |
| F- G | -7092 / 0 | -91.8 | -91.8 | 0.46 (1) | | 3.41 | L- J | 0 / 5860 | 0.73 (1) |
| G- T | -8115 / 0 | -91.8 | -91.8 | 0.36 (1) | | 3.20 | N- G | 0 / 323 | 0.04 (1) |
| T- H | -8115 / 0 | -91.8 | -91.8 | 0.36 (1) | | 3.20 | N- H | 0 / 2871 | 0.36 (1) |
| H- I | -7225 / 0 | -91.8 | -91.8 | 0.26 (1) | | 3.49 | P- G | -1277 / 0 | 0.60 (1) |
| I- J | -6392 / 0 | -91.8 | -91.8 | 0.24 (1) | | 3.71 | C- Q | 0 / 3501 | 0.43 (1) |
| S- B | -2902 / 0 | 0.0 | 0.0 | 0.13 (1) | | 7.81 | P- F | -515 / 0 | 0.08 (1) |
| K- J | -4259 / 0 | 0.0 | 0.0 | 0.15 (1) | | 6.93 | Q- D | -1984 / 0 | 0.30 (1) |
| | | | | | | | D- P | 0 / 2344 | 0.29 (1) |
| S- R | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | | 10.00 | | | |
| R- Q | 0 / 2390 | -18.5 | -18.5 | 0.19 (1) | | 10.00 | | | |
| Q- P | 0 / 5215 | -18.5 | -18.5 | 0.38 (1) | | 10.00 | | | |
| P- O | 0 / 8115 | -18.5 | -18.5 | 0.60 (1) | | 10.00 | | | |
| O- N | 0 / 8115 | -18.5 | -18.5 | 0.60 (1) | | 10.00 | | | |
| N- U | 0 / 6451 | -18.5 | -18.5 | 0.49 (1) | | 10.00 | | | |
| U- M | 0 / 6451 | -18.5 | -18.5 | 0.49 (1) | | 10.00 | | | |
| M- V | 0 / 5725 | -18.5 | -18.5 | 0.46 (1) | | 10.00 | | | |
| V- W | 0 / 5725 | -18.5 | -18.5 | 0.46 (1) | | 10.00 | | | |
| W- L | 0 / 5725 | -18.5 | -18.5 | 0.46 (1) | | 10.00 | | | |
| L- X | 0 / 0 | -18.5 | -18.5 | 0.09 (1) | | 10.00 | | | |
| X- K | 0 / 0 | -18.5 | -18.5 | 0.09 (1) | | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| H | 23-11-8 | -346 | -346 | --- | BACK | VERT | TOTAL | --- | C1 |
| M | 23-10-12 | -143 | -143 | --- | BACK | VERT | TOTAL | --- | C1 |
| N | 20-11-8 | -1815 | -1815 | --- | BACK | VERT | TOTAL | --- | C1 |
| T | 21-10-12 | -26 | -26 | --- | BACK | VERT | TOTAL | --- | C1 |
| U | 21-10-12 | -143 | -143 | --- | BACK | VERT | TOTAL | --- | C1 |
| V | 24-10-12 | -122 | -122 | --- | BACK | VERT | TOTAL | --- | C1 |
| W | 26-10-12 | -29 | -29 | --- | BACK | VERT | TOTAL | --- | C1 |
| X | 28-10-12 | -31 | -31 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 145 = 291 lb [M]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.53/1.00 (D-F:1) , BC=0.60/1.00 (N-P:1) , WB=0.73/1.00 (J-L:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90 )
JSI METAL= 0.73 (O) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T57 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



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**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-p | MT20 | 4.0 | 6.0 | 1.00 | 3.00 |
| C | TTWW-m | MT20 | 5.0 | 8.0 | 2.00 | 2.25 |
| D, G, I | | | | | | |
| D | TMWW-t | MT20 | 4.0 | 6.0 | | |
| E | TS-t | MT20 | 3.0 | 8.0 | | |
| F | TMW+w | MT20 | 2.0 | 4.0 | | |
| H | TTWW-m | MT20 | 6.0 | 7.0 | 2.50 | 2.75 |
| J | TMVW-p | MT20 | 5.0 | 8.0 | Edge | |
| K | BMV1+p | MT20 | 4.0 | 6.0 | | |
| L | BMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| M | BMWW+t | MT20 | 4.0 | 6.0 | | |
| N | BMWW+t | MT20 | 4.0 | 10.0 | | |
| O | BS-t | MT20 | 5.0 | 6.0 | | |
| P | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| Q | BMWW-t | MT20 | 5.0 | 6.0 | | |
| R | BMWW-t | MT20 | 5.0 | 6.0 | | |
| S | BMV1+p | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

**CONNECTION REQUIREMENTS**

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.


Structural component only
DWG# T-2311507

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T58 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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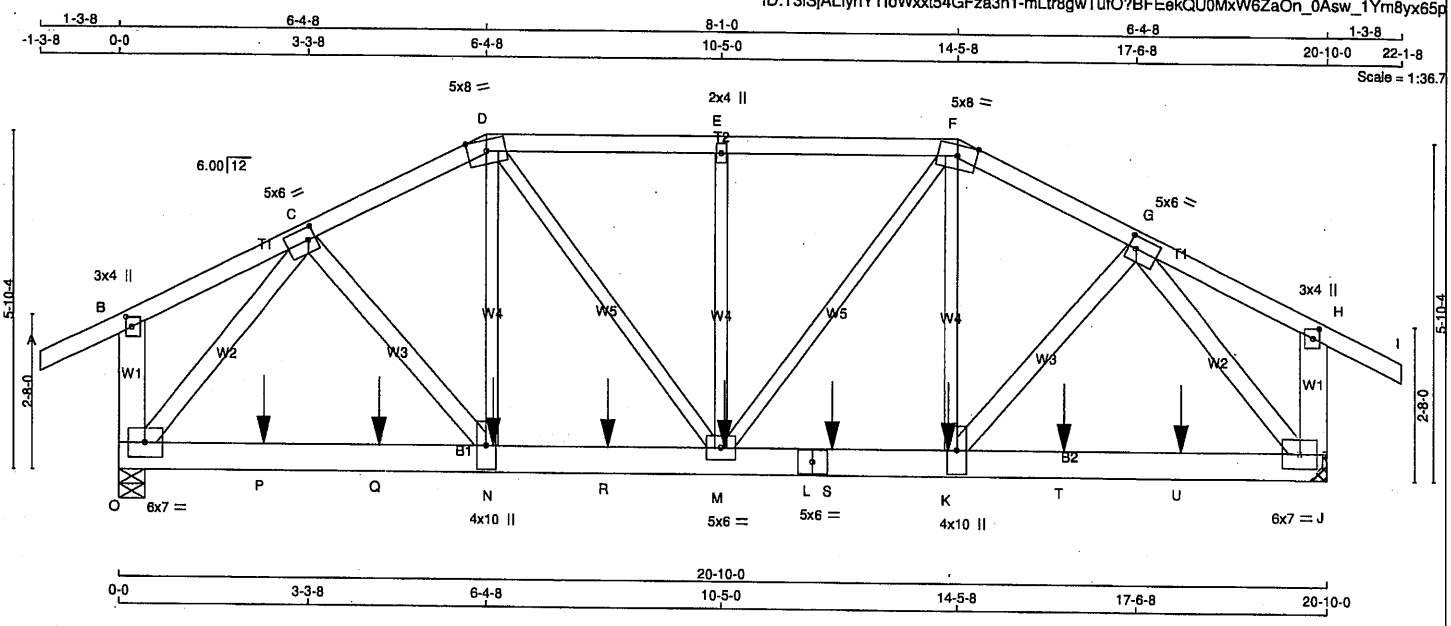


TOTAL WEIGHT = 118 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| F - I | 2x4 | DRY | No.2 | SPF |
| O - B | 2x6 | DRY | No.2 | SPF |
| J - H | 2x6 | DRY | No.2 | SPF |
| O - L | 2x6 | DRY | No.2 | SPF |
| L - J | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| K - G | 2x4 | DRY | No.2 | SPF |
| C - N | 2x4 | DRY | No.2 | SPF |
| O - C | 2x4 | DRY | No.2 | SPF |
| G - J | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | 2.00 | 1.25 |
| C | TMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| D | TTWW-m | MT20 | 5.0 | 8.0 | Edge | |
| E | TMW+w | MT20 | 2.0 | 4.0 | | |
| F | TTWW-m | MT20 | 5.0 | 8.0 | Edge | |
| G | TMWW-t | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| H | TMV+p | MT20 | 3.0 | 4.0 | 2.00 | 1.25 |
| J | BMVW1-t | MT20 | 6.0 | 7.0 | | |
| K | BMWW+t | MT20 | 4.0 | 10.0 | | |
| L | BS-t | MT20 | 5.0 | 6.0 | | |
| M | BMWWW-t | MT20 | 5.0 | 6.0 | | |
| N | BMWW+t | MT20 | 4.0 | 10.0 | | |
| O | BMVW1-t | MT20 | 6.0 | 7.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311508

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| J | 3020 | 0 | 3020 | 0 | 0 | MECHANICAL | |
| O | 3207 | 0 | 3207 | 0 | 0 | 5-8 | 3-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-4.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| J | 2127 | 1442 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 685 / 0 | 0 / 0 |
| O | 2261 | 1526 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 734 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.13 (1) | 10.00 | N- D | 0 / 759 | 0.19 (1) |
| B- C | 0 / 12 | -91.8 | -91.8 | 0.13 (1) | 10.00 | D- M | 0 / 498 | 0.12 (1) |
| C- D | -3185 / 0 | -91.8 | -91.8 | 0.28 (1) | 3.66 | M- E | -437 / 0 | 0.22 (1) |
| D- E | -3145 / 0 | -91.8 | -91.8 | 0.41 (1) | 3.53 | M- F | 0 / 733 | 0.18 (1) |
| E- F | -3145 / 0 | -91.8 | -91.8 | 0.41 (1) | 3.53 | K- F | 0 / 503 | 0.12 (1) |
| F- G | -3035 / 0 | -91.8 | -91.8 | 0.27 (1) | 3.75 | K- G | 0 / 988 | 0.17 (1) |
| G- H | 0 / 12 | -91.8 | -91.8 | 0.14 (1) | 10.00 | C- N | 0 / 1050 | 0.19 (1) |
| H- I | 0 / 28 | -91.8 | -91.8 | 0.13 (1) | 10.00 | O- C | -3378 / 0 | 0.96 (1) |
| O- B | -249 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | G- J | -3229 / 0 | 0.92 (1) |
| J- H | -248 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | | | |
| O- P | 0 / 2172 | -18.5 | -18.5 | 0.93 (1) | 10.00 | | | |
| P- Q | 0 / 2172 | -18.5 | -18.5 | 0.93 (1) | 10.00 | | | |
| Q- N | 0 / 2172 | -18.5 | -18.5 | 0.93 (1) | 10.00 | | | |
| N- R | 0 / 2849 | -18.5 | -18.5 | 0.94 (1) | 10.00 | | | |
| R- M | 0 / 2849 | -18.5 | -18.5 | 0.94 (1) | 10.00 | | | |
| M- L | 0 / 2710 | -18.5 | -18.5 | 0.82 (1) | 10.00 | | | |
| L- S | 0 / 2710 | -18.5 | -18.5 | 0.82 (1) | 10.00 | | | |
| S- K | 0 / 2710 | -18.5 | -18.5 | 0.82 (1) | 10.00 | | | |
| K- T | 0 / 2076 | -18.5 | -18.5 | 0.78 (1) | 10.00 | | | |
| T- U | 0 / 2076 | -18.5 | -18.5 | 0.78 (1) | 10.00 | | | |
| U- J | 0 / 2076 | -18.5 | -18.5 | 0.78 (1) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| K | 14-3-12 | -225 | -225 | --- | BACK | VERT | TOTAL | --- | C1 |
| M | 10-5-12 | -299 | -299 | --- | BACK | VERT | TOTAL | --- | C1 |
| N | 6-6-0 | -308 | -308 | --- | BACK | VERT | TOTAL | --- | C1 |
| P | 2-6-4 | -344 | -344 | --- | BACK | VERT | TOTAL | --- | C1 |
| Q | 4-6-4 | -308 | -308 | --- | BACK | VERT | TOTAL | --- | C1 |
| R | 8-5-12 | -308 | -308 | --- | BACK | VERT | TOTAL | --- | C1 |
| S | 12-3-12 | -299 | -299 | --- | BACK | VERT | TOTAL | --- | C1 |
| T | 16-3-12 | -225 | -225 | --- | BACK | VERT | TOTAL | --- | C1 |
| U | 18-3-12 | -276 | -276 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.94/1.00 (M-N:1) , WB=0.96/1.00 (C-O:1) , SSI=0.45/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90 )
JSI METAL= 0.76 (C) (INPUT = 0.95 )



CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T58 | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.


Structural component only
DWG# T-2311508

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T59 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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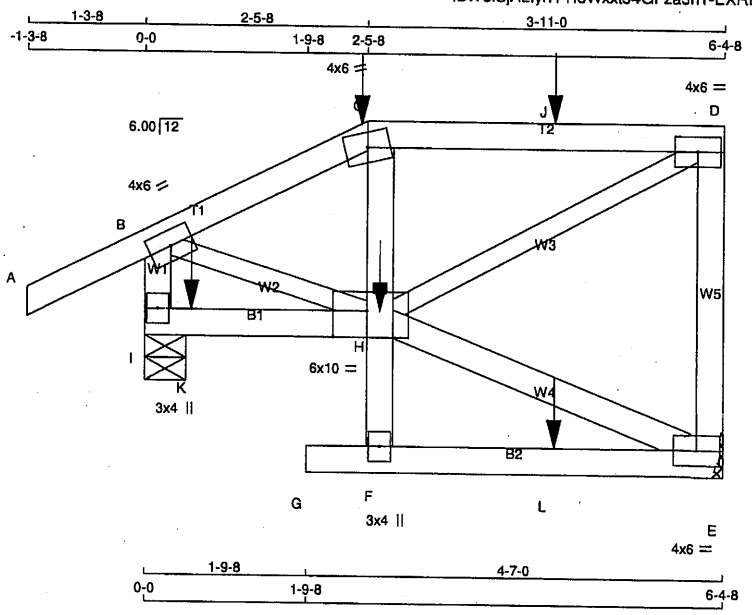


Scale = 1:23.5

TOTAL WEIGHT = 35 lb [M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| I - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| F - C | 2x4 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - E | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TTV-m | MT20 | 4.0 | 6.0 | | |
| D | TMVW-t | MT20 | 4.0 | 6.0 | | |
| E | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| F | BMV+p | MT20 | 3.0 | 4.0 | | |
| H | BVMWWW-I | MT20 | 6.0 | 10.0 | Edge | 4.50 |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.


Structural component only
DWG# T-2311509

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT. | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| E | 411 | 0 | 411 | 0 | 0 | MECHANICAL | |
| I | 556 | 0 | 556 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 291 | 188 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 103 / 0 | 0 / 0 |
| I | 392 | 263 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 129 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.13 (1) | 10.00 | B- H | 0 / 363 | 0.09 (1) |
| B- C | -386 / 0 | -91.8 | -91.8 | 0.11 (1) | 6.25 | H- E | -9 / 0 | 0.00 (1) |
| C- J | -324 / 0 | -91.8 | -91.8 | 0.30 (1) | 6.25 | H- D | 0 / 371 | 0.09 (1) |
| J- D | -324 / 0 | -91.8 | -91.8 | 0.30 (1) | 6.25 | | | |
| E- D | -371 / 0 | 0.0 | 0.0 | 0.09 (1) | 7.81 | | | |
| I- B | -524 / 0 | 0.0 | 0.0 | 0.06 (1) | 7.81 | | | |
| I- K | 0 / 0 | -18.5 | -18.5 | 0.05 (4) | 10.00 | | | |
| K- H | 0 / 0 | -18.5 | -18.5 | 0.05 (4) | 10.00 | | | |
| F- H | 0 / 58 | 0.0 | 0.0 | 0.03 (1) | 10.00 | | | |
| H- C | -216 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | | | |
| G- F | 0 / 0 | -18.5 | -18.5 | 0.02 (4) | 10.00 | | | |
| F- L | 0 / 8 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | |
| L- E | 0 / 8 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| C | 2-5-8 | -8 | -8 | --- | FRONT | VERT | DEAD | --- | C1 |
| C | 2-5-8 | -20 | -20 | --- | BACK | VERT | TOTAL | --- | C1 |
| C | 2-5-8 | -32 | -32 | --- | FRONT | VERT | SNOW | --- | C1 |
| H | 2-7-4 | -3 | -3 | --- | BACK | VERT | TOTAL | --- | C1 |
| J | 4-6-4 | -14 | -14 | --- | BACK | VERT | TOTAL | --- | C1 |
| K | 6-4 | -7 | -7 | --- | BACK | VERT | TOTAL | --- | C1 |
| L | 4-6-4 | -3 | -3 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.10/1.00 (E-F:4) , WB=0.09/1.00 (D-H:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (H) (INPUT = 0.90 )
JSI METAL= 0.11 (B) (INPUT = 0.95 )

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T60 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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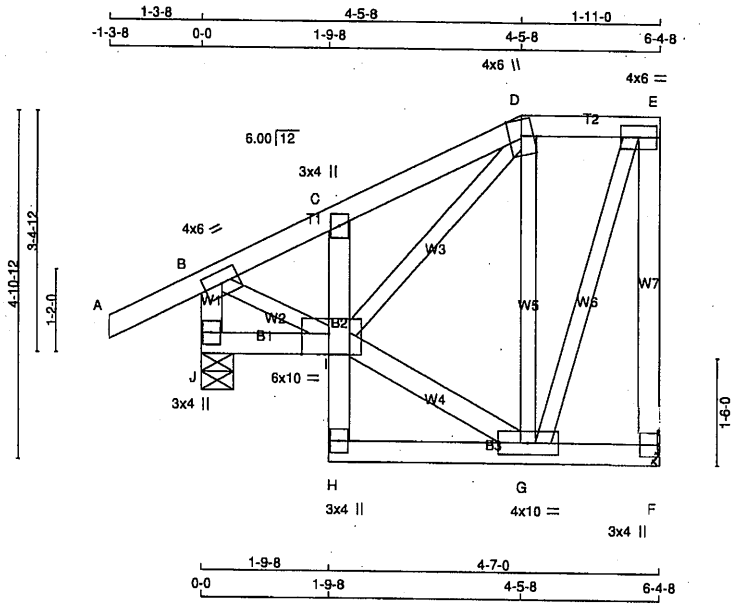


Scale = 1:29.7

TOTAL WEIGHT = 41 lb [M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| D - E | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| J - B | 2x4 | DRY | No.2 | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| H - C | 2x4 | DRY | No.2 | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| I - G | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TMV+p | MT20 | 3.0 | 4.0 | | |
| D | TTWW+m | MT20 | 4.0 | 6.0 | Edge | |
| E | TMVW-t | MT20 | 4.0 | 6.0 | | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | | |
| G | BMWWW-t | MT20 | 4.0 | 10.0 | | |
| H | BMV+p | MT20 | 3.0 | 4.0 | | |
| I | BVMWWW-t | MT20 | 6.0 | 10.0 | Edge | 4.50 |
| J | BMV1+p | MT20 | 3.0 | 4.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 338 | 0 | 338 | 0 | 0 | MECHANICAL | |
| J | 489 | 0 | 489 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 239 | 156 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 84 / 0 | 0 / 0 |
| J | 343 | 240 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 103 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

CHORDS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | -289 / 0 | -91.8 | -91.8 | 0.09 (1) | 6.25 |
| C- D | -265 / 0 | -91.8 | -91.8 | 0.07 (1) | 6.25 |
| D- E | -94 / 0 | -91.8 | -91.8 | 0.04 (1) | 6.25 |
| F- E | -325 / 0 | 0.0 | 0.0 | 0.13 (1) | 7.81 |
| J- B | -471 / 0 | 0.0 | 0.0 | 0.05 (1) | 7.81 |
| J- I | 0 / 0 | -18.5 | -18.5 | 0.02 (4) | 10.00 |
| H- I | 0 / 23 | 0.0 | 0.0 | 0.02 (1) | 10.00 |
| I- C | -183 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 |
| H- G | 0 / 10 | -18.5 | -18.5 | 0.03 (4) | 10.00 |
| G- F | 0 / 0 | -18.5 | -18.5 | 0.02 (4) | 10.00 |

WEBS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| G- D | -241 / 0 | 0.08 (1) |
| G- E | 0 / 255 | 0.06 (1) |
| B- I | 0 / 270 | 0.06 (1) |
| I- G | 0 / 102 | 0.02 (1) |
| I- D | 0 / 211 | 0.05 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (E-F:1) , BC=0.03/1.00 (G-H:4) , WB=0.08/1.00 (D-G:1) , SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90 )
JSI METAL= 0.09 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311510

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T61 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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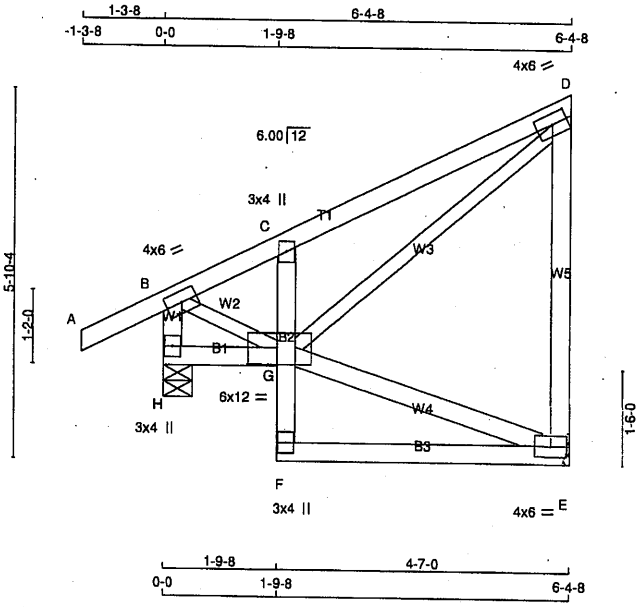


Scale = 1:33.5

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| H - B | 2x4 | DRY | No.2 | SPF |
| H - G | 2x4 | DRY | No.2 | SPF |
| F - C | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES** (table is in inches)

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TMV+p | MT20 | 3.0 | 4.0 | | |
| D | TMVW-t | MT20 | 4.0 | 6.0 | 2.00 | 3.00 |
| E | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| F | BMV+p | MT20 | 3.0 | 4.0 | | |
| G | BVMWWW-t | MT20 | 6.0 | 12.0 | 3.25 | 5.50 |
| H | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 338 | 0 | 338 | 0 | 0 | MECHANICAL | |
| H | 489 | 0 | 489 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 239 | 156 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 84 / 0 | 0 / 0 |
| H | 343 | 240 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 103 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

CHORDS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | -335 / 0 | -91.8 | -91.8 | 0.16 (1) | 6.25 |
| C- D | -335 / 0 | -91.8 | -91.8 | 0.22 (1) | 6.25 |
| E- D | -292 / 0 | 0.0 | 0.0 | 0.18 (1) | 7.81 |
| H- B | -471 / 0 | 0.0 | 0.0 | 0.05 (1) | 7.81 |
| H- G | 0 / 0 | -18.5 | -18.5 | 0.02 (4) | 10.00 |
| F- G | 0 / 46 | 0.0 | 0.0 | 0.04 (1) | 10.00 |
| G- C | -347 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 |
| F- E | 0 / 17 | -18.5 | -18.5 | 0.11 (4) | 10.00 |

WEBS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| B- G | 0 / 343 | 0.08 (1) |
| G- E | -18 / 0 | 0.00 (1) |
| G- D | 0 / 388 | 0.09 (1) |

TOTAL WEIGHT = 38 lb
[M][F]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.11/1.00 (E-F:4) , WB=0.09/1.00 (D-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (D) (INPUT = 0.90 )
JSI METAL= 0.10 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311511

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T62 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:01 2023 Page 1
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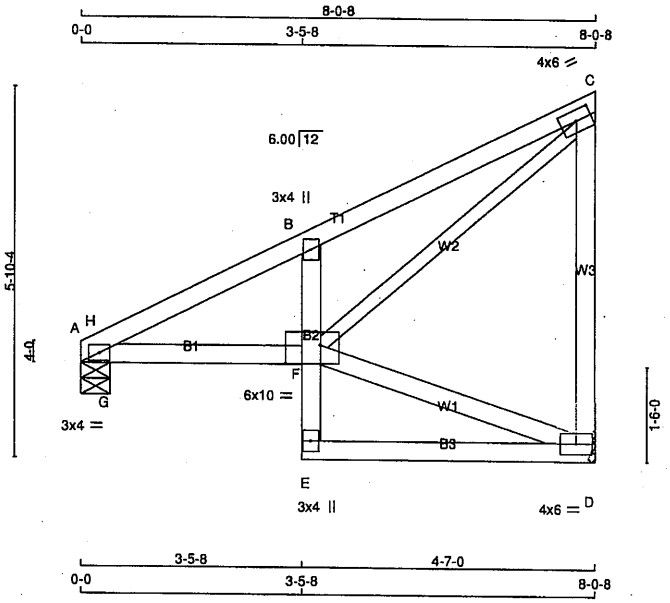


Scale = 1:33.4

TOTAL WEIGHT = 38 lb

[M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| D - C | 2x4 | DRY | No.2 | SPF |
| A - F | 2x4 | DRY | No.2 | SPF |
| E - B | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X | |
|---|---|---|---|---|---|---|---|
| A | TMB1-I | MT20 | 3.0 | 4.0 | | | |
| B | TMV+p | MT20 | 3.0 | 4.0 | | | |
| C | TMVW-t | MT20 | 4.0 | 6.0 | 2.00 | 3.00 | |
| D | BMVW1-t | MT20 | 4.0 | 6.0 | | | |
| E | BMV+p | MT20 | 3.0 | 4.0 | | | |
| F | BVMWW-I | MT20 | 6.0 | 10.0 | 3.50 | 6.50 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| A | 443 | 0 | 443 | 0 | 0 | 5-8 | 1-8 |
| D | 443 | 0 | 443 | 0 | 0 | MECHANICAL | |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| A | 313 | 206 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 108 / 0 | 0 / 0 |
| D | 313 | 206 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 108 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- H | -697 / 0 | -91.8 | -91.8 | 0.03 (4) | 6.25 | F- C | 0 / 744 | 0.17 (1) |
| H- B | -640 / 0 | -91.8 | -91.8 | 0.18 (1) | 6.25 | F- D | -32 / 0 | 0.01 (1) |
| B- C | -635 / 0 | -91.8 | -91.8 | 0.21 (1) | 6.25 | G- H | -41 / 34 | 0.00 (1) |
| D- C | -392 / 0 | 0.0 | 0.0 | 0.25 (1) | 7.81 | | | |
| A- G | 0 / 596 | -18.5 | -18.5 | 0.13 (1) | 10.00 | | | |
| G- F | 0 / 596 | -18.5 | -18.5 | 0.15 (1) | 10.00 | | | |
| E- F | 0 / 46 | 0.0 | 0.0 | 0.07 (1) | 10.00 | | | |
| F- B | -422 / 0 | 0.0 | 0.0 | 0.05 (1) | 7.81 | | | |
| E- D | 0 / 30 | -18.5 | -18.5 | 0.11 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.15/1.00 (F-G:1) , WB=0.17/1.00 (C-F:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90 )
JSI METAL= 0.20 (A) (INPUT = 0.95 )


Structural component only
DWG# T-2311512

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T63 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:02 2023 Page 1
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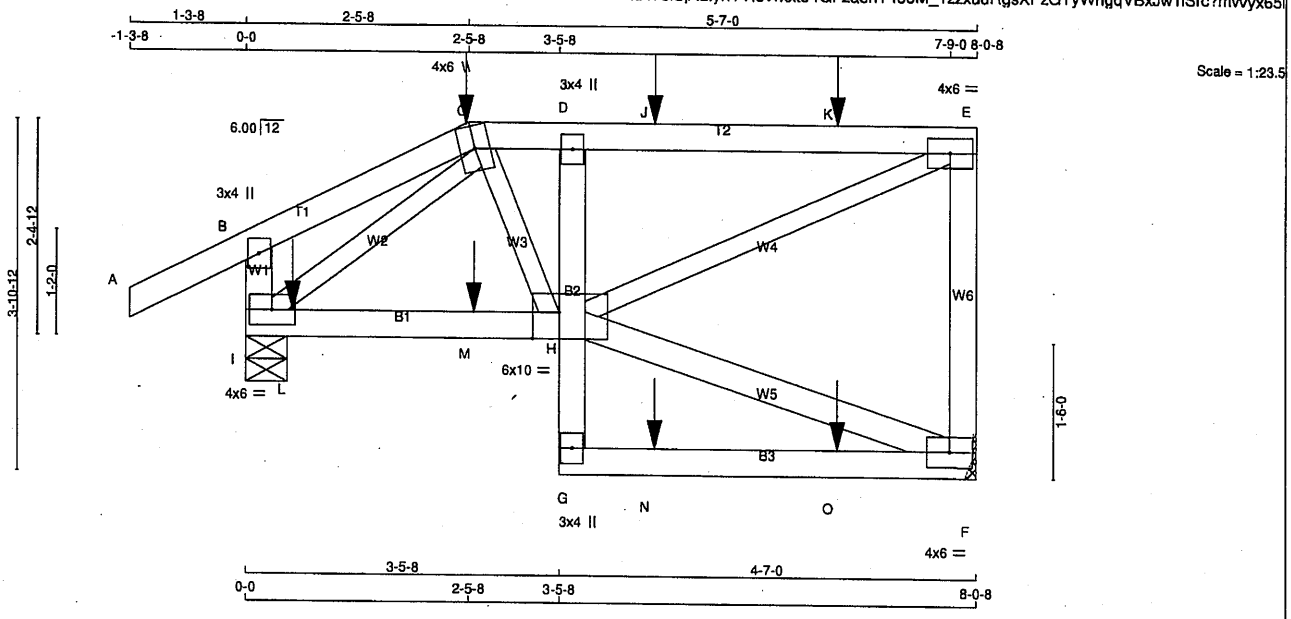


TOTAL WEIGHT = 41 lb

[M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| I - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| G - D | 2x4 | DRY | No.2 | SPF |
| G - F | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TTWW+m | MT20 | 4.0 | 6.0 | | |
| D | TMV+p | MT20 | 3.0 | 4.0 | | |
| E | TMVW-t | MT20 | 4.0 | 6.0 | | |
| F | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| G | BMV+p | MT20 | 3.0 | 4.0 | | |
| H | BVMWWW-I | MT20 | 6.0 | 10.0 | Edge | 3.50 |
| I | BMVW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311513

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| F | 507 | 0 | 507 | 0 | 0 | MECHANICAL | |
| I | 655 | 0 | 655 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 359 | 235 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 123 / 0 | 0 / 0 |
| I | 461 | 313 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 148 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.13 (1) | 10.00 | H- F | -16 / 0 | 0.00 (1) |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.11 (1) | 10.00 | H- E | 0 / 645 | 0.16 (1) |
| C- D | -594 / 0 | -91.8 | -91.8 | 0.28 (1) | 6.25 | C- H | 0 / 323 | 0.08 (1) |
| D- J | -583 / 0 | -91.8 | -91.8 | 0.28 (1) | 6.25 | I- C | -579 / 0 | 0.13 (1) |
| J- K | -583 / 0 | -91.8 | -91.8 | 0.28 (1) | 6.25 | | | |
| K- E | -583 / 0 | -91.8 | -91.8 | 0.28 (1) | 6.25 | | | |
| F- E | -457 / 0 | 0.0 | 0.0 | 0.11 (1) | 7.81 | | | |
| I- B | -239 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | | | |
| I- L | 0 / 443 | -18.5 | -18.5 | 0.13 (1) | 10.00 | | | |
| L- M | 0 / 443 | -18.5 | -18.5 | 0.13 (1) | 10.00 | | | |
| M- H | 0 / 443 | -18.5 | -18.5 | 0.13 (1) | 10.00 | | | |
| G- H | 0 / 52 | 0.0 | 0.0 | 0.04 (1) | 10.00 | | | |
| H- D | -484 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | | | |
| G- N | 0 / 15 | -18.5 | -18.5 | 0.13 (4) | 10.00 | | | |
| N- O | 0 / 15 | -18.5 | -18.5 | 0.13 (4) | 10.00 | | | |
| O- F | 0 / 15 | -18.5 | -18.5 | 0.13 (4) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| C | 2-5-15 | -8 | -8 | --- | FRONT | VERT | DEAD | --- | C1 |
| C | 2-5-15 | -20 | -20 | --- | FRONT | VERT | TOTAL | --- | C1 |
| C | 2-5-15 | -32 | -32 | --- | FRONT | VERT | SNOW | --- | C1 |
| J | 4-6-4 | -14 | -14 | --- | FRONT | VERT | TOTAL | --- | C1 |
| K | 6-6-4 | -14 | -14 | --- | FRONT | VERT | TOTAL | --- | C1 |
| L | 6-4 | -7 | -7 | --- | FRONT | VERT | TOTAL | --- | C1 |
| M | 2-6-4 | -3 | -3 | --- | FRONT | VERT | TOTAL | --- | C1 |
| N | 4-6-4 | -4 | -4 | --- | FRONT | VERT | TOTAL | --- | C1 |
| O | 6-6-4 | -3 | -3 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.28/1.00 (D-E:1) , BC=0.13/1.00 (F-G:4) , WB=0.16/1.00 (E-H:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (C) (INPUT = 0.90 )
JSI METAL= 0.26 (C) (INPUT = 0.95 )

CITY OF RICHMOND HILL
BUILDING DIVISION
08/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T63Z | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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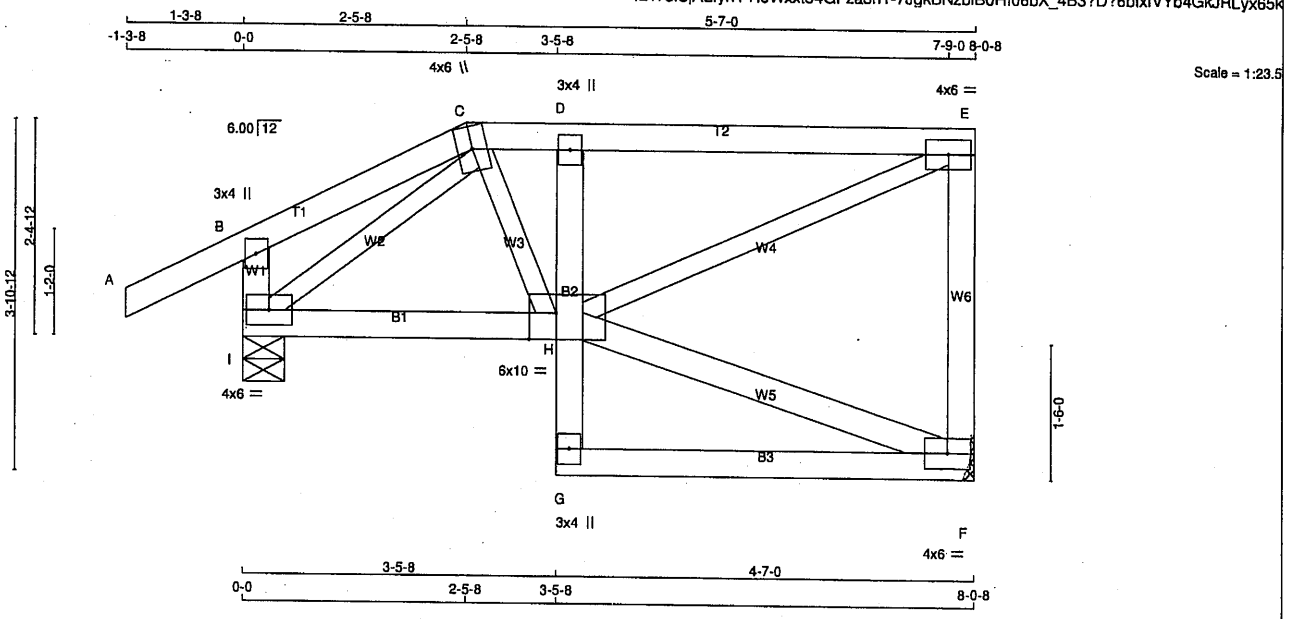


Scale = 1:23.5

TOTAL WEIGHT = 41 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| I - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| G - D | 2x4 | DRY | No.2 | SPF |
| G - F | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TTWW+m | MT20 | 4.0 | 6.0 | | |
| D | TMV+p | MT20 | 3.0 | 4.0 | | |
| E | TMVW-t | MT20 | 4.0 | 6.0 | | |
| F | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| G | BMV+p | MT20 | 3.0 | 4.0 | | |
| H | BVMWWW-I | MT20 | 6.0 | 10.0 | Edge | 3.50 |
| I | BMVW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 443 | 0 | 443 | 0 | 0 | MECHANICAL | |
| I | 568 | 0 | 568 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 313 | 206 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 108 / 0 | 0 / 0 |
| I | 399 | 275 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 124 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | H- F | -13 / 0 | 0.00 (1) |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.10 (1) | 10.00 | H- E | 0 / 539 | 0.12 (1) |
| C- D | -496 / 0 | -91.8 | -91.8 | 0.22 (1) | 6.25 | C- H | 0 / 313 | 0.07 (1) |
| D- E | -487 / 0 | -91.8 | -91.8 | 0.23 (1) | 6.25 | I- C | -458 / 0 | 0.09 (1) |
| F- E | -398 / 0 | 0.0 | 0.0 | 0.09 (1) | 7.81 | | | |
| I- B | -239 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | | | |
| I- H | 0 / 350 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | |
| G- H | 0 / 46 | 0.0 | 0.0 | 0.03 (1) | 10.00 | | | |
| H- D | -438 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | | | |
| G- F | 0 / 13 | -18.5 | -18.5 | 0.11 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.23/1.00 (D-E:1) , BC=0.11/1.00 (F-G:4) , WB=0.12/1.00 (E-H:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.90 )
JSI METAL= 0.21 (C) (INPUT = 0.95 )


Structural component only
DWG# T-2311514

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T64 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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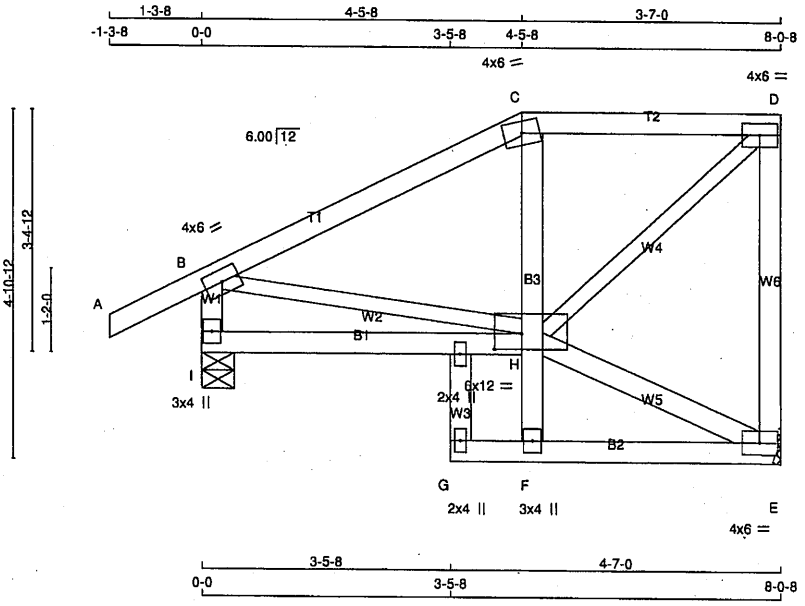


Scale = 1:29.7

TOTAL WEIGHT = 3 X 45 = 134 lb [M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| I - B | 2x4 | DRY | No.2 | SPF |
| I - H | 2x4 | DRY | No.2 | SPF |
| F - C | 2x4 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| H - E | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TTV-m | MT20 | 4.0 | 6.0 | | |
| D | TMVW-t | MT20 | 4.0 | 6.0 | | |
| E | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| F | BMV+p | MT20 | 3.0 | 4.0 | | |
| G | NP+w | MT20 | 2.0 | 4.0 | | |
| H | BVMWWW-I | MT20 | 6.0 | 12.0 | 2.75 | 4.50 |
| I | BMV1+p | MT20 | 3.0 | 4.0 | | |
| J | NP+w | MT20 | 2.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| E | 455 | 0 | 455 | 0 | 0 | MECHANICAL | |
| I | 577 | 0 | 577 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 323 | 206 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 117 / 0 | 0 / 0 |
| I | 406 | 275 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 131 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A-B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | B-H | 0 / 303 | 0.07 (1) | |
| B-C | -334 / 0 | -91.8 | -91.8 | 0.23 (1) | 6.25 | H-E | -7 / 0 | 0.00 (1) | |
| C-D | -285 / 0 | -91.8 | -91.8 | 0.15 (1) | 6.25 | H-D | 0 / 384 | 0.09 (1) | |
| E-D | -421 / 0 | 0.0 | 0.0 | 0.17 (1) | 7.81 | | | | |
| I-B | -534 / 0 | 0.0 | 0.0 | 0.05 (1) | 7.81 | | | | |
| I-H | 0 / 0 | -18.5 | -18.5 | 0.11 (4) | 10.00 | | | | |
| F-H | 0 / 59 | 0.0 | 0.0 | 0.02 (4) | 10.00 | | | | |
| H-C | -220 / 0 | 0.0 | 0.0 | 0.01 (1) | 7.81 | | | | |
| G-F | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | 10.00 | | | | |
| F-E | 0 / 6 | -18.5 | -18.5 | 0.06 (4) | 10.00 | | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.11/1.00 (H-I:4) , WB=0.09/1.00 (D-H:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90 )
JSI METAL= 0.11 (B) (INPUT = 0.95 )


Structural component only
DWG# T-2311515

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T65 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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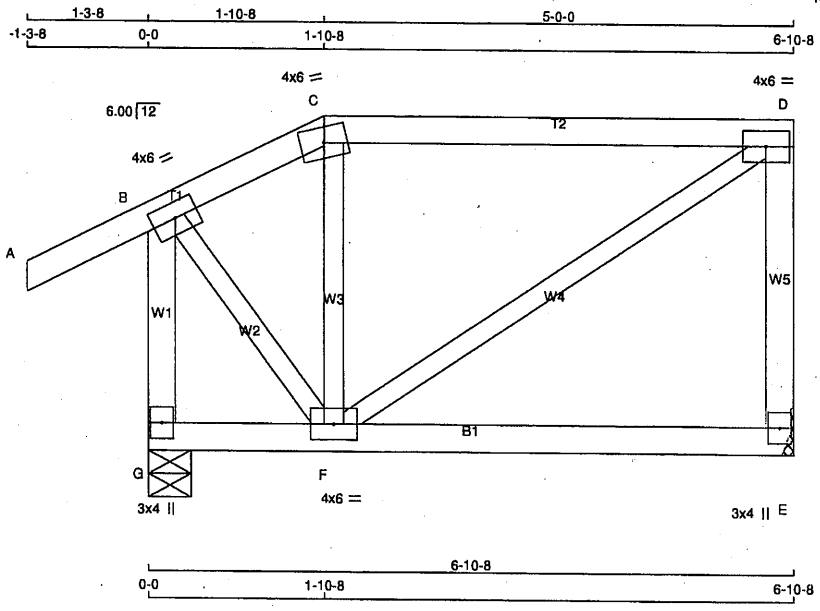


TOTAL WEIGHT = 33 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| G - B | 2x4 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| C | TTW-m | MT20 | 4.0 | 6.0 | |
| D | TMVW-t | MT20 | 4.0 | 6.0 | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | |
| F | BMWWW-t | MT20 | 4.0 | 6.0 | |
| G | BMV1+p | MT20 | 3.0 | 4.0 | |

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| E | 367 | 0 | 367 | 0 | 0 | MECHANICAL | |
| G | 516 | 0 | 516 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 260 | 169 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 91 / 0 | 0 / 0 |
| G | 362 | 252 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 110 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | | 10.00 | F- C | -193 / 0 | 0.04 (1) |
| B- C | -192 / 0 | -91.8 | -91.8 | 0.12 (1) | | 6.25 | F- D | 0 / 178 | 0.04 (1) |
| C- D | -147 / 0 | -91.8 | -91.8 | 0.39 (1) | | 6.25 | B- F | 0 / 238 | 0.05 (1) |
| E- D | -329 / 0 | 0.0 | 0.0 | 0.07 (1) | | 7.81 | | | |
| G- B | -515 / 0 | 0.0 | 0.0 | 0.07 (1) | | 7.81 | | | |
| G- F | 0 / 0 | -18.5 | -18.5 | 0.09 (4) | | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.09 (4) | | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.39/1.00 (C-D:1) , BC=0.09/1.00 (F-G:4) , WB=0.05/1.00 (B-F:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (F) (INPUT = 0.90 )
JSI METAL= 0.08 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311516

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T66 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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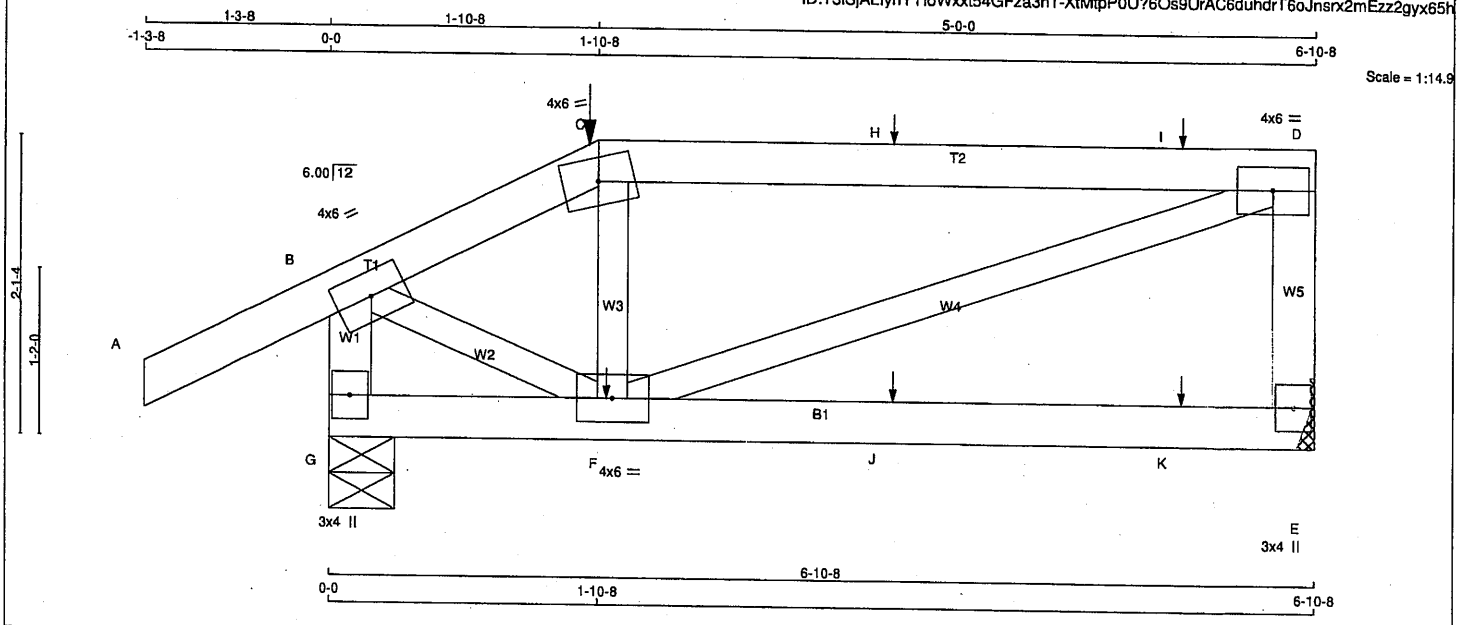



**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| G - B | 2x4 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TTW-m | MT20 | 4.0 | 6.0 | | |
| D | TMVW-t | MT20 | 4.0 | 6.0 | | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | | |
| F | BMWWW-t | MT20 | 4.0 | 6.0 | | |
| G | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 375 | 0 | 375 | 0 | 0 | MECHANICAL | |
| G | 539 | 0 | 539 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 266 | 174 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 92 / 0 | 0 / 0 |
| G | 378 | 265 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 113 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT:
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (7)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.13 (1) | 10.00 | F- C | -152 / 5 | 0.02 (1) | |
| B- C | -356 / 0 | -91.8 | -91.8 | 0.13 (1) | 6.25 | F- D | 0 / 310 | 0.08 (1) | |
| C- H | -291 / 0 | -91.8 | -91.8 | 0.44 (1) | 6.25 | B- F | 0 / 327 | 0.08 (1) | |
| H- I | -291 / 0 | -91.8 | -91.8 | 0.44 (1) | 6.25 | | | | |
| I- D | -291 / 0 | -91.8 | -91.8 | 0.44 (1) | 6.25 | | | | |
| E- D | -337 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | | | | |
| G- B | -537 / 0 | 0.0 | 0.0 | 0.06 (1) | 7.81 | | | | |
| G- F | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | | |
| F- J | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | | |
| J- K | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | | |
| K- E | 0 / 0 | -18.5 | -18.5 | 0.10 (4) | 10.00 | | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| C | 1-10-8 | -4 | -4 | --- | FRONT | VERT | DEAD | --- | C1 |
| C | 1-10-8 | 1 | 1 | 76 | FRONT | VERT | TOTAL | --- | C1 |
| C | 1-10-8 | -18 | -18 | --- | FRONT | VERT | SNOW | --- | C1 |
| F | 1-11-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| H | 3-11-4 | 1 | 1 | 76 | FRONT | VERT | TOTAL | --- | C1 |
| I | 5-11-4 | 1 | 1 | 75 | FRONT | VERT | TOTAL | --- | C1 |
| J | 3-11-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| K | 5-11-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 27 lb [M]

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.44/1.00 (C-D:1) , BC=0.10/1.00 (F-G:4) , WB=0.08/1.00 (B-F:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 987 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311517

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T66 | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:06 2023 Page 2
ID:T3iSjALIyhYTioWxxt54GFza3n1-XtMtpP0U?6Os9UrAC6duhdrT6oJnsrx2mEzz2gyx65h

JSI GRIP= 0.47 (F) (INPUT = 0.90 )
JSI METAL= 0.10 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

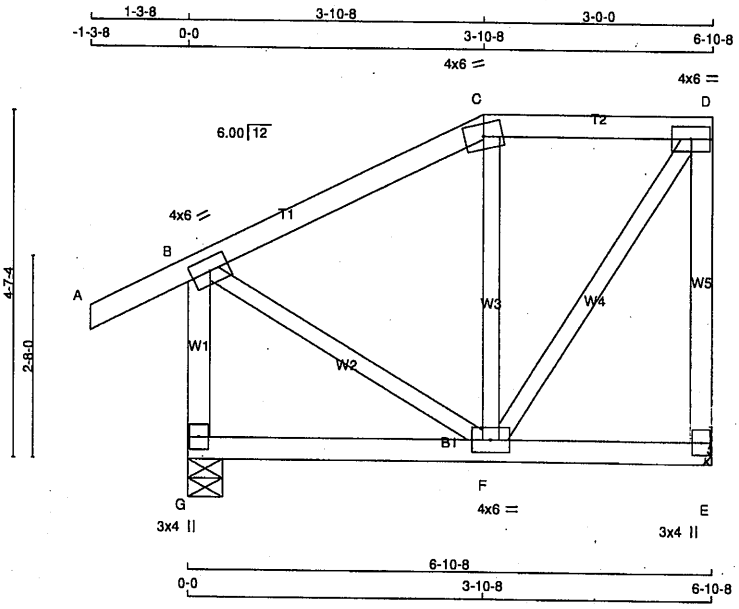
Structural component only
DWG# T-2311517

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T67 | 2 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:07 2023 Page 1
ID:T3iSjALIyhYTloWxxt54GFza3n1-?4wF1l16mQXjmeQMmp87DrNiwCgZbIGB?tlXa6yx65g




Scale = 1:28.1

TOTAL WEIGHT = 2 X 36 = 71 lb

[M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| G - B | 2x4 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TTW-m | MT20 | 4.0 | 6.0 | | |
| D | TMVW-t | MT20 | 4.0 | 6.0 | | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | | |
| F | BMWVW-t | MT20 | 4.0 | 6.0 | | |
| G | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 379 | 0 | 379 | 0 | 0 | MECHANICAL | |
| G | 503 | 0 | 503 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 268 | 176 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 92 / 0 | 0 / 0 |
| G | 354 | 245 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 108 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | F- C | -238 / 0 | 0.07 (1) | |
| B- C | -174 / 0 | -91.8 | -91.8 | 0.18 (1) | 6.25 | F- D | 0 / 269 | 0.06 (1) | |
| C- D | -150 / 0 | -91.8 | -91.8 | 0.11 (1) | 6.25 | B- F | 0 / 181 | 0.04 (1) | |
| E- D | -360 / 0 | 0.0 | 0.0 | 0.12 (1) | 7.81 | | | | |
| G- B | -472 / 0 | 0.0 | 0.0 | 0.07 (1) | 7.81 | | | | |
| G- F | 0 / 0 | -18.5 | -18.5 | 0.06 (4) | 10.00 | | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.06 (4) | 10.00 | | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF. PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.06/1.00 (F-G:4) , WB=0.07/1.00 (C-F:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (F) (INPUT = 0.90 )
JSI METAL= 0.08 (F) (INPUT = 0.95 )


Structural component only
DWG# T-2311518

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T68 | 2 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:07 2023 Page 1
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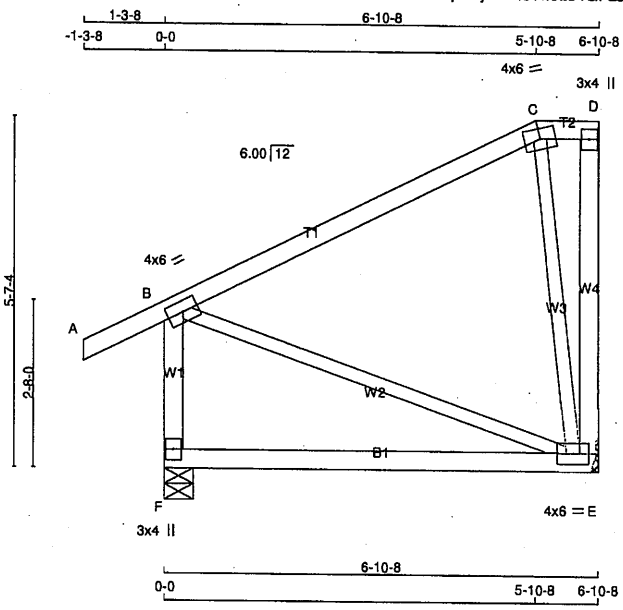


Scale = 1:33.8

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| F - B | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (teble is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| C | TTW-m | MT20 | 4.0 | 6.0 | |
| D | TMV+p | MT20 | 3.0 | 4.0 | |
| E | BMVWW1-t | MT20 | 4.0 | 6.0 | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| E | 379 | 0 | 379 | 0 | 0 | MECHANICAL | |
| F | 503 | 0 | 503 | 0 | 0 | 5-8 | 1-8 |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 268 | 176 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 92 / 0 | 0 / 0 |
| F | 354 | 245 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 108 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | B- E | 0 / 56 | 0.01 (1) |
| B- C | -59 / 0 | -91.8 | -91.8 | 0.41 (1) | 6.25 | C- E | -294 / 0 | 0.14 (1) |
| C- D | 0 / 0 | -91.8 | -91.8 | 0.01 (1) | 10.00 | | | |
| E- D | -44 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | | | |
| F- B | -440 / 0 | 0.0 | 0.0 | 0.06 (1) | 7.81 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.26 (4) | 10.00 | | | |

TOTAL WEIGHT = 2 X 36 = 72 lb
[M][F]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 819 (0.10")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.26/1.00 (E-F:4) , WB=0.14/1.00 (C-E:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90 )
JSI METAL= 0.15 (C) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311519

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T69 | 2 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:08 2023 Page 1
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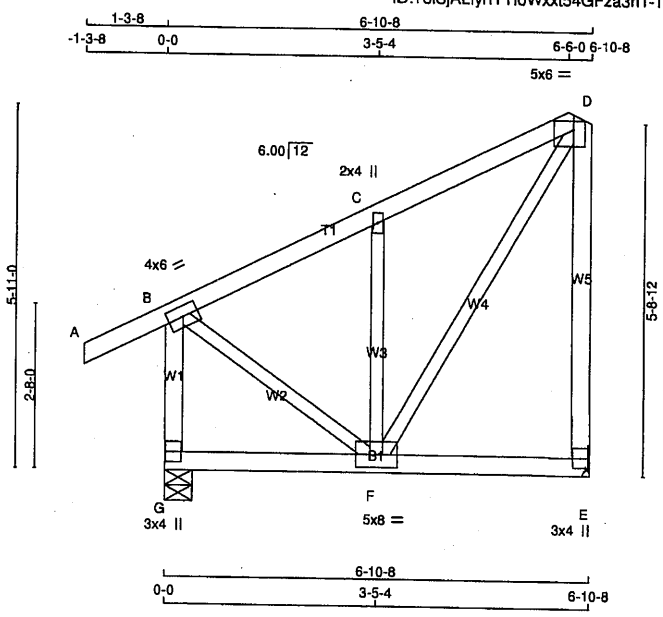


**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| G - B | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| G - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| C | TMW+w | MT20 | 2.0 | 4.0 | | |
| D | TVMW-p | MT20 | 5.0 | 6.0 | 1.75 | 3.75 |
| E | BMV1+p | MT20 | 3.0 | 4.0 | | |
| F | BMWWW-t | MT20 | 5.0 | 8.0 | | |
| G | BMV1+p | MT20 | 3.0 | 4.0 | | |

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| G | 503 | 0 | 503 | 0 | 0 | 5-8 | 1-8 |
| E | 379 | 0 | 379 | 0 | 0 | IN-SX<br>MECHANICAL | IN-SX |

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| G | 354 | 245 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 108 / 0 | 0 / 0 |
| E | 268 | 176 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 92 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | B- F | 0 / 240 | 0.05 (1) |
| B- C | -204 / 0 | -91.8 | -91.8 | 0.13 (1) | 6.25 | F- C | -385 / 0 | 0.11 (1) |
| C- D | -230 / 0 | -91.8 | -91.8 | 0.13 (1) | 6.25 | F- D | 0 / 375 | 0.08 (1) |
| G- B | -476 / 0 | 0.0 | 0.0 | 0.07 (1) | 7.81 | | | |
| E- D | -352 / 0 | 0.0 | 0.0 | 0.26 (1) | 7.81 | | | |
| G- F | 0 / 0 | -18.5 | -18.5 | 0.06 (4) | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.06 (4) | 10.00 | | | |

TOTAL WEIGHT = 2 X 38 = 76 lb
[M][F]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.26/1.00 (D-E:1) , BC=0.06/1.00 (E-F:4) , WB=0.11/1.00 (C-F:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (F) (INPUT = 0.90 )
JSI METAL= 0.16 (C) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311520

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T70 | 1 | 2 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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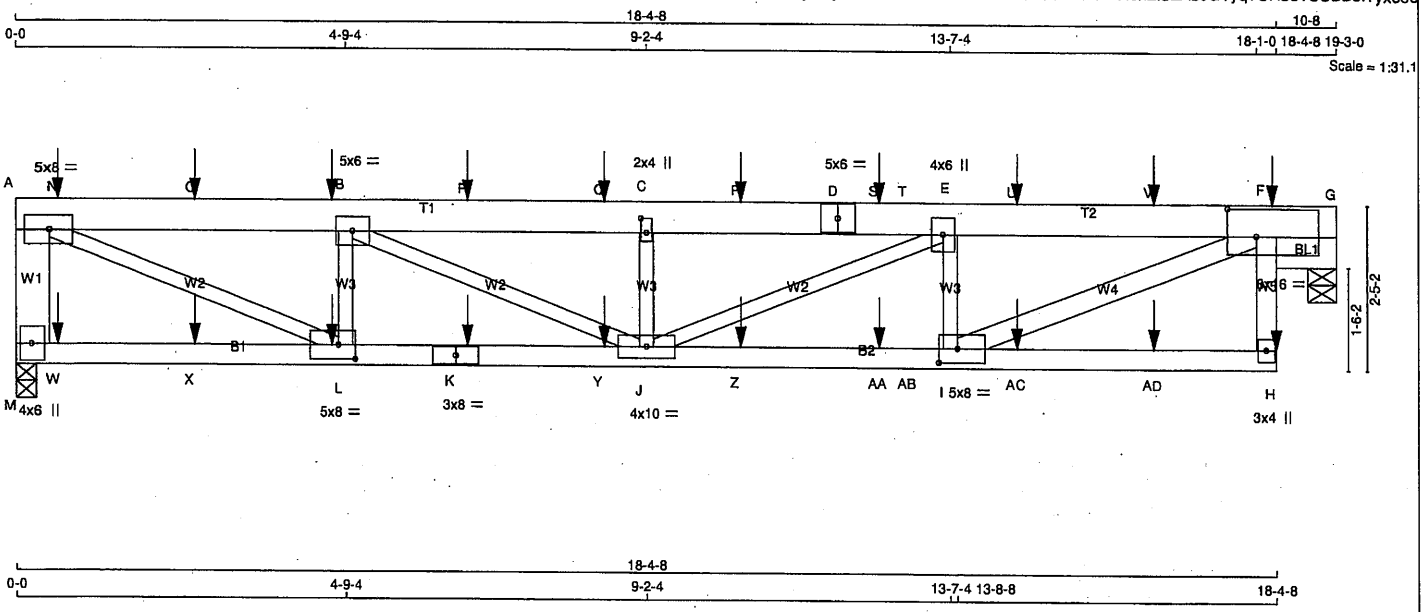





TOTAL WEIGHT = 2 X 82 = 165 lb

[M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| M - A | 2x6 | DRY | No.2 | SPF |
| A - D | 2x6 | DRY | 2100F 1.8E | SPF |
| D - G | 2x6 | DRY | 2100F 1.8E | SPF |
| H - F | 2x4 | DRY | No.2 | SPF |
| M - K | 2x4 | DRY | No.2 | SPF |
| K - H | 2x4 | DRY | No.2 | SPF |
| BEARING BLOCKS | | | | |
| BL1 | 2x6 | DRY | No.2 | SPF |
| ALL WEBS EXCEPT | 2x3 | DRY | No.2 | SPF |
| I - F | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

| CHORDS | #ROWS | SURFACE SPACING (IN) | LOAD(PLF) |
|---|---|---|---|
| TOP CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| M- A | 2 | 12 | SIDE(2.4) |
| A- D | 2 | 12 | SIDE(197.8) |
| D- G | 2 | 12 | SIDE(166.9) |
| F- H | 1 | 12 | SIDE(8.2) |
| BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS | | | |
| M- K | 1 | 12 | SIDE(61.0) |
| K- H | 1 | 12 | SIDE(56.4) |
| WEBS : (0.122"X3") SPIRAL NAILS | | | |
| L- B | 1 | 6 | SIDE(1.3) |
| 2x3 | 1 | 6 | |
| 2x4 | 1 | 6 | |
| F- G | 2 | 6 | SIDE(20.9) |

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.


Structural component only
DWG# T-2311521

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| M | 2386 | 0 | 2386 | 0 | 0 | 3-8 | 1-8 |
| G | 2568 | 0 | 2568 | 0 | 0 | 5-0 | 5-0 |

(** SEE "BEARING NOTE" **)

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): G

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| M | 1687 | 1110 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 577 / 0 | 0 / 0 |
| G | 1815 | 1201 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 614 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| M- A | -2066 / 0 | 0.0 | 0.0 | 0.08 (1) | 7.81 | | I- F | 0 / 5738 | 0.51 (1) |
| A- N | -4224 / 0 | -91.8 | -91.8 | 0.07 (1) | 6.25 | | A- L | 0 / 4598 | 0.57 (1) |
| N- O | -4224 / 0 | -91.8 | -91.8 | 0.07 (1) | 6.25 | | I- E | -1585 / 0 | 0.13 (1) |
| O- B | -4224 / 0 | -91.8 | -91.8 | 0.07 (1) | 6.25 | | L- B | -1087 / 0 | 0.09 (1) |
| B- P | -5559 / 0 | -91.8 | -91.8 | 0.08 (1) | 5.90 | | J- E | 0 / 381 | 0.05 (1) |
| P- Q | -5559 / 0 | -91.8 | -91.8 | 0.08 (1) | 5.90 | | B- J | 0 / 1472 | 0.18 (1) |
| Q- C | -5559 / 0 | -91.8 | -91.8 | 0.08 (1) | 5.90 | | J- C | -228 / 0 | 0.02 (1) |
| C- R | -5559 / 0 | -91.8 | -91.8 | 0.14 (1) | 5.82 | | | | |
| R- D | -5559 / 0 | -91.8 | -91.8 | 0.14 (1) | 5.82 | | | | |
| D- S | -5559 / 0 | -91.8 | -91.8 | 0.14 (1) | 5.82 | | | | |
| S- T | -5559 / 0 | -91.8 | -91.8 | 0.14 (1) | 5.82 | | | | |
| T- E | -5559 / 0 | -137.7 | -137.7 | 0.14 (1) | 5.82 | | | | |
| E- U | -5214 / 0 | -137.7 | -137.7 | 0.53 (1) | 5.31 | | | | |
| U- V | -5214 / 0 | -137.7 | -137.7 | 0.53 (1) | 5.31 | | | | |
| V- F | -5214 / 0 | -137.7 | -137.7 | 0.53 (1) | 5.31 | | | | |
| F- G | -1023 / 0 | -137.7 | -137.7 | 0.21 (1) | 6.25 | | | | |
| H- F | 0 / 451 | 0.0 | 0.0 | 0.04 (1) | 10.00 | | | | |
| M- W | 0 / 0 | -18.5 | -18.5 | 0.21 (1) | 10.00 | | | | |
| W- X | 0 / 0 | -18.5 | -18.5 | 0.21 (1) | 10.00 | | | | |
| X- L | 0 / 0 | -18.5 | -18.5 | 0.21 (1) | 10.00 | | | | |
| L- K | 0 / 4224 | -18.5 | -18.5 | 0.54 (1) | 10.00 | | | | |
| K- Y | 0 / 4224 | -18.5 | -18.5 | 0.54 (1) | 10.00 | | | | |
| Y- J | 0 / 4224 | -18.5 | -18.5 | 0.54 (1) | 10.00 | | | | |
| J- Z | 0 / 5214 | -18.5 | -18.5 | 0.68 (1) | 10.00 | | | | |
| Z-AA | 0 / 5214 | -18.5 | -18.5 | 0.68 (1) | 10.00 | | | | |
| AA-AB | 0 / 5214 | -18.5 | -18.5 | 0.68 (1) | 10.00 | | | | |
| AB- I | 0 / 5214 | -27.7 | -27.7 | 0.68 (1) | 10.00 | | | | |
| I-AC | -32 / 0 | -27.7 | -27.7 | 0.24 (1) | 6.25 | | | | |
| AC-AD | -32 / 0 | -27.7 | -27.7 | 0.24 (1) | 6.25 | | | | |
| AD- H | -32 / 0 | -27.7 | -27.7 | 0.24 (1) | 6.25 | | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| B | 4-6-14 | -6 | -6 | --- | FRONT | VERT | TOTAL | --- | C1 |
| F | 18-3-12 | -8 | -8 | --- | FRONT | VERT | TOTAL | --- | C1 |
| H | 18-4-8 | -176 | -176 | --- | FRONT | VERT | TOTAL | --- | C1 |
| K | 6-6-14 | -169 | -169 | --- | FRONT | VERT | TOTAL | --- | C1 |

**DESIGN CRITERIA**

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 952 (0.23")

CSI: TC=0.53/1.00 (E-F:1) , BC=0.68/1.00 (I-J:1) , WB=0.57/1.00 (A-L:1) , SSI=0.39/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (A) (INPUT = 0.90 )
JSI METAL= 0.77 (K) (INPUT = 0.95 )

CONTINUED ON PAGE 2

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | T70 | 1 | 2 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington



**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMVW-t | MT20 | 5.0 | 8.0 | | |
| B | TMWW-t | MT20 | 5.0 | 6.0 | | |
| C | TMW+w | MT20 | 2.0 | 4.0 | 2.50 | 1.00 |
| D | TS-t | MT20 | 5.0 | 6.0 | | |
| E | TMWW+t | MT20 | 4.0 | 6.0 | | |
| F | TMVWK-t | MT20 | 8.0 | 16.0 | 4.75 | 5.00 |
| H | BMV+p | MT20 | 3.0 | 4.0 | | |
| I | BMWW-t | MT20 | 5.0 | 8.0 | 2.50 | 3.25 |
| J | BMWWW-t | MT20 | 4.0 | 10.0 | | |
| K | BS-t | MT20 | 3.0 | 8.0 | | |
| L | BMWW-t | MT20 | 5.0 | 8.0 | 2.50 | 3.00 |
| M | BMV1+p | MT20 | 4.0 | 6.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| L | 4-6-14 | -169 | -169 | — | FRONT | VERT | TOTAL | — | C1 |
| N | 6-14 | -17 | -17 | — | FRONT | VERT | TOTAL | — | C1 |
| O | 2-6-14 | -6 | -6 | — | FRONT | VERT | TOTAL | — | C1 |
| P | 6-6-14 | -6 | -6 | — | FRONT | VERT | TOTAL | — | C1 |
| Q | 8-6-14 | -6 | -6 | — | FRONT | VERT | TOTAL | — | C1 |
| R | 10-6-14 | -6 | -6 | — | FRONT | VERT | TOTAL | — | C1 |
| S | 12-6-14 | -6 | -6 | — | FRONT | VERT | TOTAL | — | C1 |
| U | 14-6-14 | -6 | -6 | — | FRONT | VERT | TOTAL | — | C1 |
| V | 16-6-14 | -6 | -6 | — | FRONT | VERT | TOTAL | — | C1 |
| W | 6-14 | -172 | -172 | — | FRONT | VERT | TOTAL | — | C1 |
| X | 2-6-14 | -169 | -169 | — | FRONT | VERT | TOTAL | — | C1 |
| Y | 8-6-14 | -169 | -169 | — | FRONT | VERT | TOTAL | — | C1 |
| Z | 10-6-14 | -169 | -169 | — | FRONT | VERT | TOTAL | — | C1 |
| AA | 12-6-14 | -169 | -169 | — | FRONT | VERT | TOTAL | — | C1 |
| AC | 14-6-14 | -169 | -169 | — | FRONT | VERT | TOTAL | — | C1 |
| AD | 16-6-14 | -169 | -169 | — | FRONT | VERT | TOTAL | — | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.


Structural component only
DWG# T-2311521

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | PB1 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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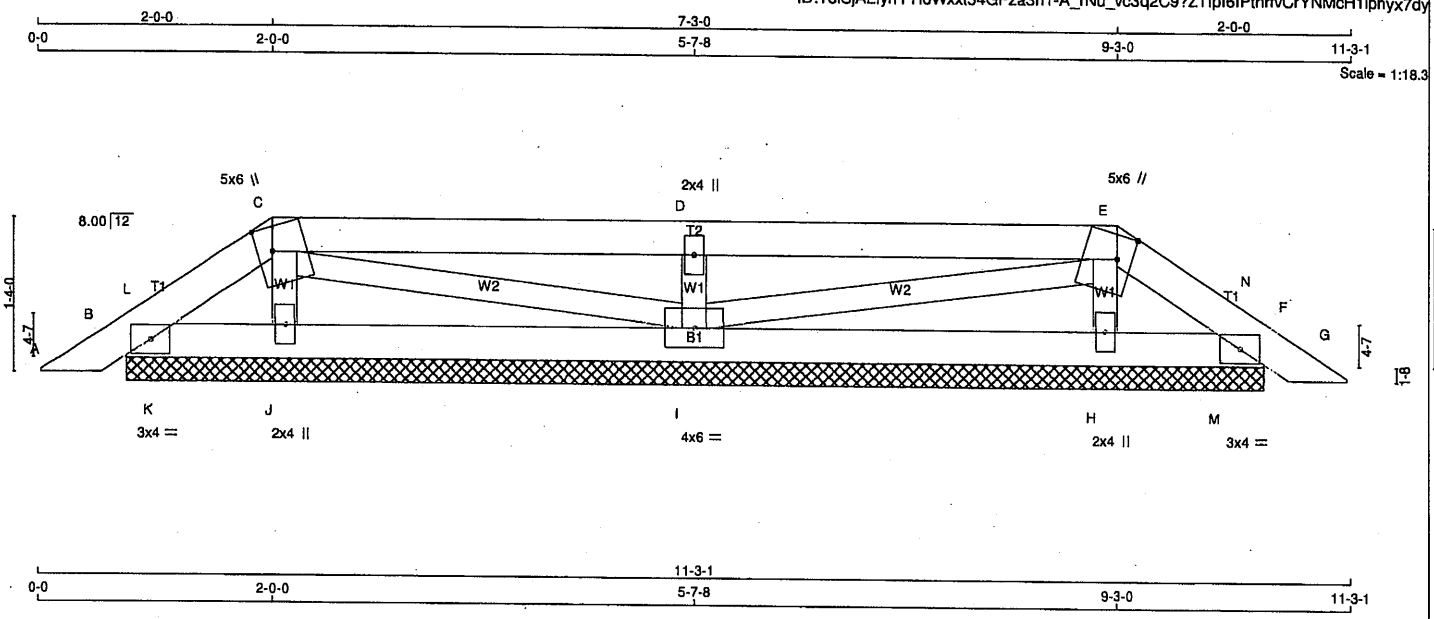


TOTAL WEIGHT = 32 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| E - G | 2x4 | DRY | No.2 | SPF |
| B - F | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMB1-I | MT20 | 3.0 | 4.0 | | |
| C | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| D | TMW+w | MT20 | 2.0 | 4.0 | | |
| E | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| F | TMB1-I | MT20 | 3.0 | 4.0 | | |
| H | BMW1+w | MT20 | 2.0 | 4.0 | | |
| I | BMWWW1-t | MT20 | 4.0 | 6.0 | | |
| J | BMW1+w | MT20 | 2.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG | REQRD BRG |
|---|---|---|---|---|---|---|---|
| B | 138 | 0 | 138 | 0 | 0 | 9-9-3 | 1-8 |
| F | 138 | 0 | 138 | 0 | 0 | 9-9-3 | 1-8 |
| J | 216 | 0 | 216 | 0 | 0 | 9-9-3 | 1-8 |
| H | 216 | 0 | 216 | 0 | 0 | 9-9-3 | 1-8 |
| I | 487 | 0 | 487 | 0 | 0 | 9-9-3 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| B | 95 | 76 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 19 / 0 | 0 / 0 |
| F | 95 | 76 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 19 / 0 | 0 / 0 |
| J | 155 | 90 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 64 / 0 | 0 / 0 |
| H | 155 | 90 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 64 / 0 | 0 / 0 |
| I | 343 | 233 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 110 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 15 | -91.8 | -91.8 | 0.03 (1) | 10.00 | J- C | -147 / 0 | 0.02 (1) |
| B- L | -33 / 0 | -91.8 | -91.8 | 0.00 (4) | 6.25 | H- E | -147 / 0 | 0.02 (1) |
| L- C | -35 / 0 | -91.8 | -91.8 | 0.01 (1) | 6.25 | I- E | -9 / 0 | 0.00 (1) |
| C- D | -2 / 0 | -91.8 | -91.8 | 0.20 (1) | 10.00 | C- I | -9 / 0 | 0.00 (1) |
| D- E | -2 / 0 | -91.8 | -91.8 | 0.20 (1) | 10.00 | I- D | -415 / 0 | 0.06 (1) |
| E- N | -35 / 0 | -91.8 | -91.8 | 0.01 (1) | 6.25 | K- L | -61 / 0 | 0.00 (1) |
| N- F | -33 / 0 | -91.8 | -91.8 | 0.00 (4) | 6.25 | M- N | -61 / 0 | 0.00 (1) |
| F- G | 0 / 15 | -91.8 | -91.8 | 0.03 (1) | 10.00 | | | |
| B- K | 0 / 28 | -18.5 | -18.5 | 0.02 (1) | 10.00 | | | |
| K- J | 0 / 28 | -18.5 | -18.5 | 0.04 (4) | 10.00 | | | |
| J- I | 0 / 11 | -18.5 | -18.5 | 0.05 (4) | 10.00 | | | |
| I- H | 0 / 11 | -18.5 | -18.5 | 0.05 (4) | 10.00 | | | |
| H- M | 0 / 28 | -18.5 | -18.5 | 0.04 (4) | 10.00 | | | |
| M- F | 0 / 28 | -18.5 | -18.5 | 0.02 (1) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (D-E:1) , BC=0.05/1.00 (I-J:4) , WB=0.06/1.00 (D-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90 )
JSI METAL= 0.09 (D) (INPUT = 0.95 )


Structural component only
DWG# T-2311439

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | PB2 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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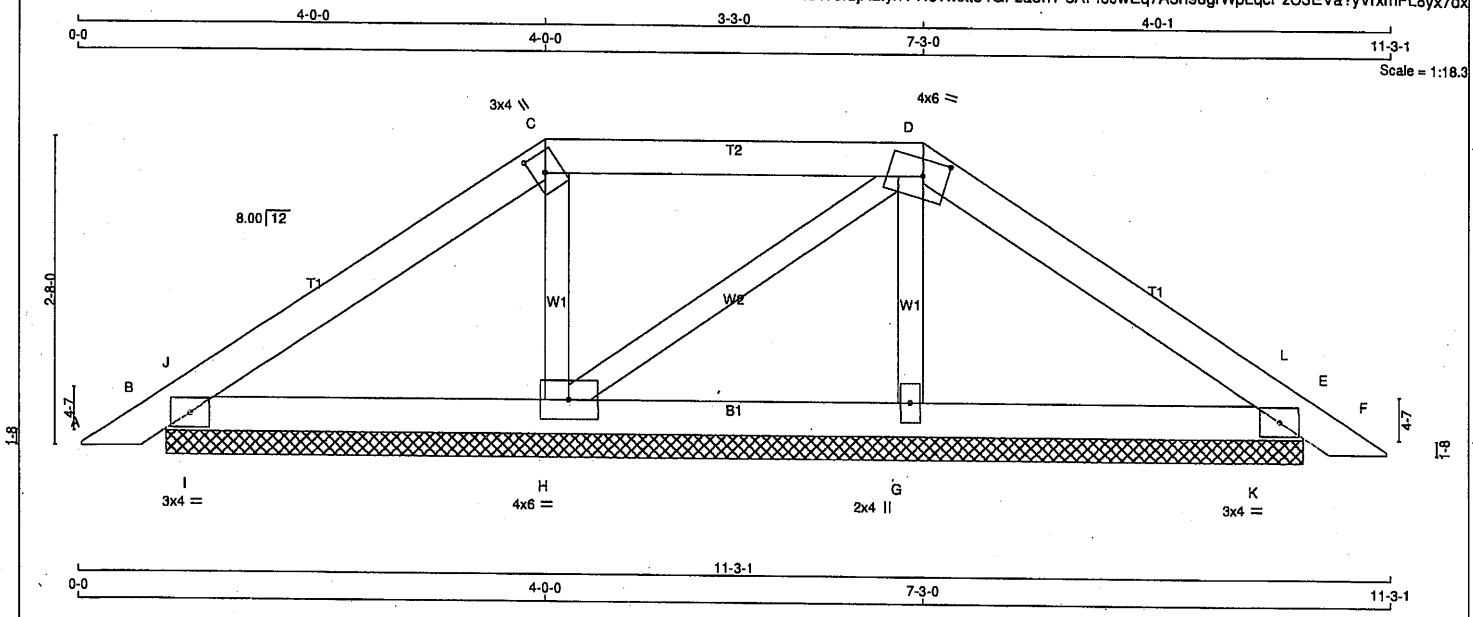


TOTAL WEIGHT = 32 lb

[M]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - D | 2x4 | DRY | No.2 | SPF |
| D - F | 2x4 | DRY | No.2 | SPF |
| B - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMB1-I | MT20 | 3.0 | 4.0 | | |
| C | TTW+h | MT20 | 3.0 | 4.0 | 2.00 | 1.25 |
| D | TTWW-m | MT20 | 4.0 | 6.0 | 1.75 | 2.50 |
| E | TMB1-I | MT20 | 3.0 | 4.0 | | |
| G | BMW1+w | MT20 | 2.0 | 4.0 | | |
| H | BMWW1-t | MT20 | 4.0 | 6.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| B | 277 | 0 | 277 | 0 | 0 | 9-9-3 | 1-8 |
| E | 295 | 0 | 295 | 0 | 0 | 9-9-3 | 1-8 |
| H | 341 | 0 | 341 | 0 | 0 | 9-9-3 | 1-8 |
| G | 282 | 0 | 282 | 0 | 0 | 9-9-3 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| B | 194 | 138 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 56 / 0 | 0 / 0 |
| E | 206 | 148 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 58 / 0 | 0 / 0 |
| H | 241 | 156 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 85 / 0 | 0 / 0 |
| G | 201 | 123 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 78 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A- B | 0 / 15 | -91.8 | -91.8 | 0.03 (1) | 10.00 |
| B- J | -33 / 0 | -91.8 | -91.8 | 0.04 (1) | 6.25 |
| J- C | -87 / 0 | -91.8 | -91.8 | 0.12 (1) | 6.25 |
| C- D | -58 / 0 | -91.8 | -91.8 | 0.17 (1) | 6.25 |
| D- L | -119 / 0 | -91.8 | -91.8 | 0.12 (1) | 6.25 |
| L- E | -67 / 0 | -91.8 | -91.8 | 0.04 (1) | 6.25 |
| E- F | 0 / 15 | -91.8 | -91.8 | 0.03 (1) | 10.00 |
| B- I | 0 / 68 | -18.5 | -18.5 | 0.11 (1) | 10.00 |
| I- H | 0 / 68 | -18.5 | -18.5 | 0.11 (1) | 10.00 |
| H- G | 0 / 86 | -18.5 | -18.5 | 0.06 (1) | 10.00 |
| G- K | 0 / 95 | -18.5 | -18.5 | 0.11 (1) | 10.00 |
| K- E | 0 / 95 | -18.5 | -18.5 | 0.11 (1) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| H- C | -230 / 0 | 0.04 (1) |
| H- D | -35 / 0 | 0.01 (1) |
| G- D | -192 / 0 | 0.03 (1) |
| I- J | -235 / 0 | 0.00 (1) |
| K- L | -232 / 0 | 0.00 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-D:1) , BC=0.11/1.00 (G-K:1) , WB=0.04/1.00 (C-H:1) , SSI=0.18/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (E) (INPUT = 0.90 )
JSI METAL= 0.06 (E) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311440

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | PB3 | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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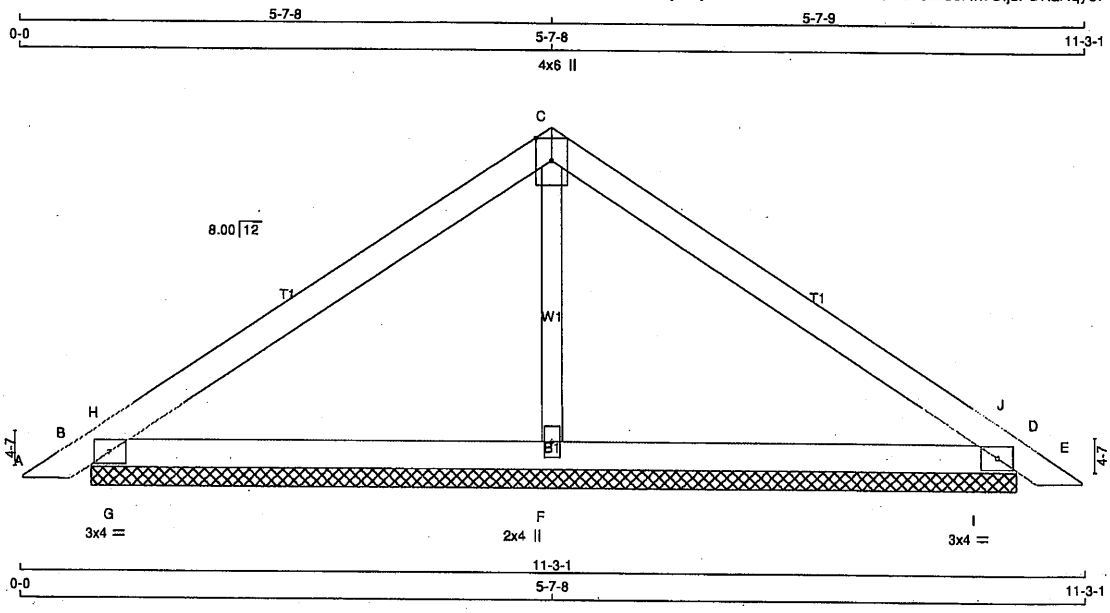


TOTAL WEIGHT = 29 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| B - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMB1-I | MT20 | 3.0 | 4.0 | | |
| C | TTW+p | MT20 | 4.0 | 6.0 | Edge | |
| D | TMB1-I | MT20 | 3.0 | 4.0 | | |
| F | BMW1+w | MT20 | 2.0 | 4.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| B | 406 | 0 | 406 | 0 | 0 | 9-9-3 | 1-8 |
| D | 406 | 0 | 406 | 0 | 0 | 9-9-3 | 1-8 |
| F | 383 | 0 | 383 | 0 | 0 | 9-9-3 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| B | 284 | 202 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 82 / 0 | 0 / 0 |
| D | 284 | 202 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 82 / 0 | 0 / 0 |
| F | 274 | 162 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 113 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 15 | -91.8 | -91.8 | 0.03 (1) | 10.00 | F- C | -165 / 0 | 0.03 (1) |
| B- H | -41 / 13 | -91.8 | -91.8 | 0.14 (1) | 6.25 | G- H | -560 / 0 | 0.00 (1) |
| H- C | -201 / 0 | -91.8 | -91.8 | 0.26 (1) | 6.25 | I- J | -560 / 0 | 0.00 (1) |
| C- J | -201 / 0 | -91.8 | -91.8 | 0.26 (1) | 6.25 | | | |
| J- D | -41 / 13 | -91.8 | -91.8 | 0.14 (1) | 6.25 | | | |
| D- E | 0 / 15 | -91.8 | -91.8 | 0.03 (1) | 10.00 | | | |
| B- G | 0 / 157 | -18.5 | -18.5 | 0.25 (1) | 10.00 | | | |
| G- F | 0 / 157 | -18.5 | -18.5 | 0.25 (1) | 10.00 | | | |
| F- I | 0 / 157 | -18.5 | -18.5 | 0.25 (1) | 10.00 | | | |
| I- D | 0 / 157 | -18.5 | -18.5 | 0.25 (1) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-J:1) , BC=0.25/1.00 (F-I:1) , WB=0.03/1.00 (C-F:1) , SSI=0.42/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90 )
JSI METAL= 0.09 (D) (INPUT = 0.95 )


Structural component only
DWG# T-2311441

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | PB4 | 2 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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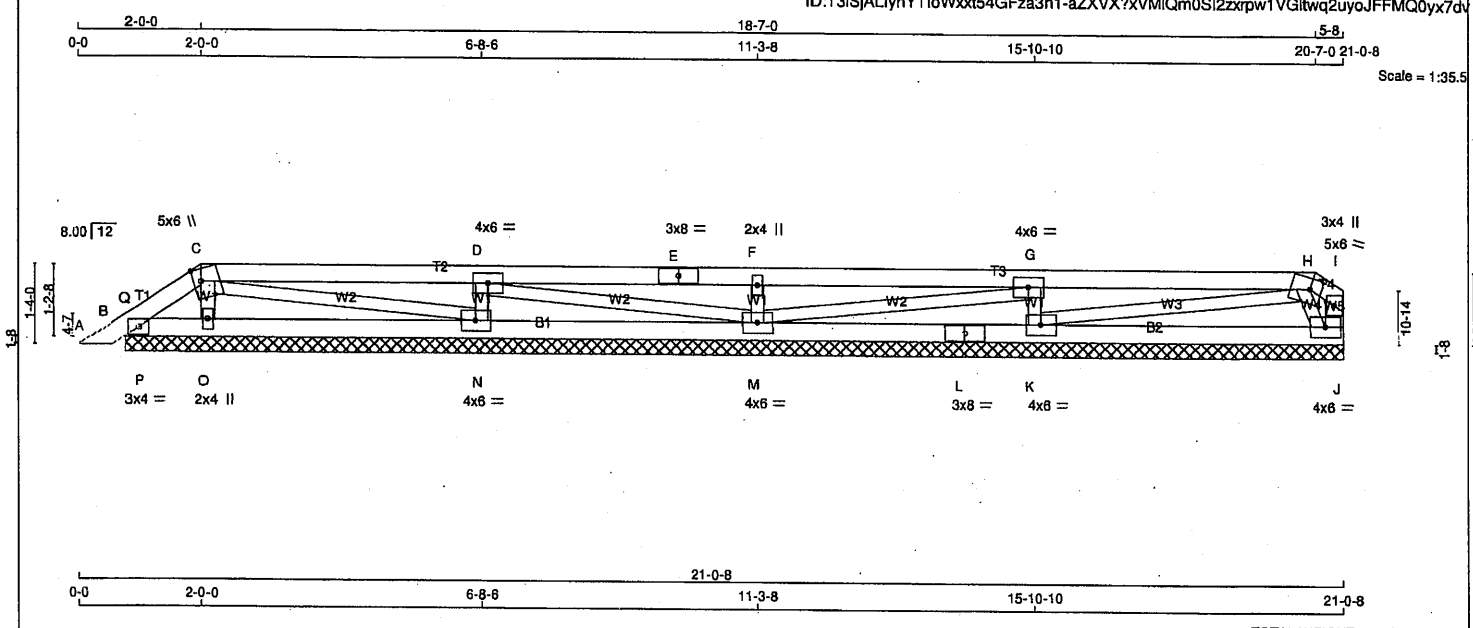


TOTAL WEIGHT = 2 X 65 = 131 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - E | 2x4 | DRY | No.2 | SPF |
| E - H | 2x4 | DRY | No.2 | SPF |
| H - I | 2x4 | DRY | No.2 | SPF |
| J - I | 2x4 | DRY | No.2 | SPF |
| B - L | 2x4 | DRY | No.2 | SPF |
| L - J | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMB1-I | MT20 | 3.0 | 4.0 | | |
| C | TTWW+m | MT20 | 5.0 | 6.0 | 2.50 | 1.50 |
| D | TMWW-t | MT20 | 4.0 | 6.0 | | |
| E | TS-t | MT20 | 3.0 | 8.0 | | |
| F | TMW+w | MT20 | 2.0 | 4.0 | | |
| G | TMWW-t | MT20 | 4.0 | 6.0 | | |
| H | TTWW-m | MT20 | 5.0 | 6.0 | 2.25 | 2.25 |
| I | TMV+p | MT20 | 3.0 | 4.0 | | |
| J | BMVW1-t | MT20 | 4.0 | 6.0 | | |
| K | BMWW1-t | MT20 | 4.0 | 6.0 | | |
| L | BS-I | MT20 | 3.0 | 8.0 | | |
| M | BMWWW1-t | MT20 | 4.0 | 6.0 | | |
| N | BMWW1-t | MT20 | 4.0 | 6.0 | | |
| O | BMW1+w | MT20 | 2.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| J | 239 | 0 | 239 | 0 | 0 | 20-3-10 ( 6-3-1[illegible] | |
| B | 135 | 0 | 135 | 0 | 0 | 20-3-10 ( 6-3-1[illegible] | |
| O | 275 | 0 | 275 | 0 | 0 | 20-3-10 ( 6-3-1[illegible] | |
| N | 576 | 0 | 576 | 0 | 0 | 20-3-10 ( 6-3-1[illegible] | |
| K | 599 | 0 | 599 | 0 | 0 | 20-3-10 ( 6-3-1[illegible] | |
| M | 474 | 0 | 474 | 0 | 0 | 20-3-10 ( 6-3-1[illegible] | |

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| J | 169 | 112 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 57 / 0 | 0 / 0 |
| B | 92 | 79 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 13 / 0 | 0 / 0 |
| O | 197 | 113 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 84 / 0 | 0 / 0 |
| N | 407 | 272 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 135 / 0 | 0 / 0 |
| K | 424 | 276 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 148 / 0 | 0 / 0 |
| M | 335 | 221 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 115 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, B, O, N, K, M

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A-B | 0 / 15 | -91.8 | -91.8 | 0.03 (1) | 10.00 | |
| B-Q | -33 / 5 | -91.8 | -91.8 | 0.01 (4) | 6.25 | |
| Q-C | -44 / 0 | -91.8 | -91.8 | 0.01 (4) | 6.25 | |
| C-D | -7 / 0 | -91.8 | -91.8 | 0.29 (1) | 10.00 | |
| D-E | 0 / 0 | -91.8 | -91.8 | 0.29 (1) | 10.00 | |
| E-F | 0 / 0 | -91.8 | -91.8 | 0.29 (1) | 10.00 | |
| F-G | 0 / 0 | -91.8 | -91.8 | 0.29 (1) | 10.00 | |
| G-H | -28 / 0 | -91.8 | -91.8 | 0.29 (1) | 6.25 | |
| H-I | 0 / 0 | -91.8 | -91.8 | 0.00 (1) | 10.00 | |
| J-I | -23 / 0 | 0.0 | 0.0 | 0.00 (1) | 7.81 | |
| B-P | 0 / 34 | -18.5 | -18.5 | 0.03 (1) | 10.00 | |
| P-O | 0 / 34 | -18.5 | -18.5 | 0.06 (4) | 10.00 | |
| O-N | 0 / 13 | -18.5 | -18.5 | 0.08 (4) | 10.00 | |
| N-M | 0 / 7 | -18.5 | -18.5 | 0.08 (4) | 10.00 | |
| M-L | 0 / 28 | -18.5 | -18.5 | 0.12 (4) | 10.00 | |
| L-K | 0 / 28 | -18.5 | -18.5 | 0.12 (4) | 10.00 | |
| K-J | 0 / 97 | -18.5 | -18.5 | 0.12 (4) | 10.00 | |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| O-C | -186 / 0 | 0.03 (1) |
| C-N | -5 / 0 | 0.00 (1) |
| K-G | -481 / 0 | 0.07 (1) |
| N-D | -487 / 0 | 0.07 (1) |
| M-G | -29 / 0 | 0.01 (1) |
| D-M | -8 / 0 | 0.00 (1) |
| M-F | -388 / 0 | 0.05 (1) |
| H-J | -202 / 0 | 0.03 (1) |
| K-H | -71 / 0 | 0.03 (1) |
| P-Q | -74 / 0 | 0.00 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1) , BC=0.12/1.00 (J-K:4) , WB=0.07/1.00 (D-N:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (M) (INPUT = 0.90 )
JSI METAL= 0.11 (E) (INPUT = 0.95 )


Structural component only
DWG# T-2311442

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | PB5 | 2 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:21 2023 Page 1
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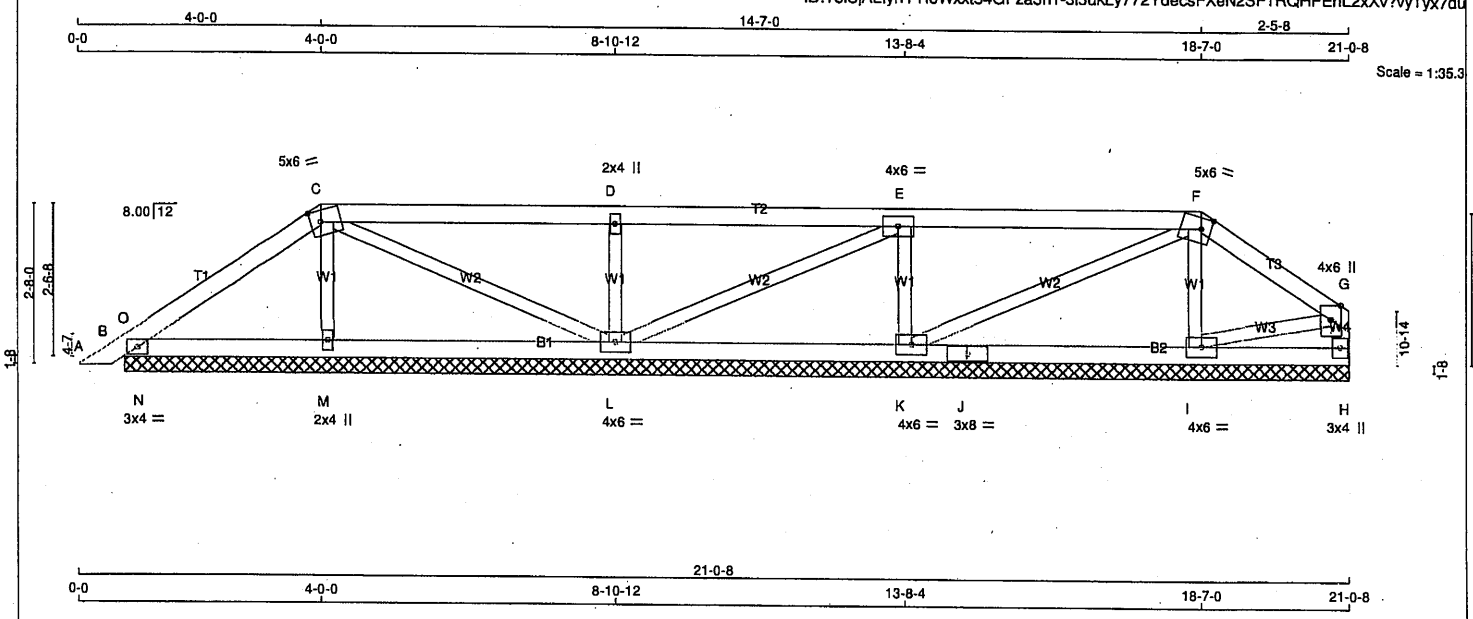


TOTAL WEIGHT = 2 X 70 = 139 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| C - F | 2x4 | DRY | No.2 | SPF |
| F - G | 2x4 | DRY | No.2 | SPF |
| H - G | 2x4 | DRY | No.2 | SPF |
| B - J | 2x4 | DRY | No.2 | SPF |
| J - H | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMB1-I | MT20 | 3.0 | 4.0 | | |
| C | TTWW-m | MT20 | 5.0 | 6.0 | 2.25 | 2.00 |
| D | TMW+w | MT20 | 2.0 | 4.0 | | |
| E | TMWW-t | MT20 | 4.0 | 6.0 | | |
| F | TTWW-m | MT20 | 5.0 | 6.0 | 2.25 | 2.00 |
| G | TMVW+p | MT20 | 4.0 | 6.0 | Edge | |
| H | BMV1+p | MT20 | 3.0 | 4.0 | | |
| I | BMWW1-t | MT20 | 4.0 | 6.0 | | |
| J | BS-t | MT20 | 3.0 | 8.0 | | |
| K | BMWW1-t | MT20 | 4.0 | 6.0 | | |
| L | BMWWW1-t | MT20 | 4.0 | 6.0 | | |
| M | BMW1+w | MT20 | 2.0 | 4.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311443

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-3X | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| H | 173 | 0 | 173 | 0 | 0 | 20-3-10 ( 13-11-82 ) | |
| B | 266 | 0 | 266 | 0 | 0 | 20-3-10 ( 13-11-82 ) | |
| M | 354 | 0 | 354 | 0 | 0 | 20-3-10 ( 13-11-82 ) | |
| I | 314 | 0 | 314 | 0 | 0 | 20-3-10 ( 13-11-82 ) | |
| K | 573 | 0 | 573 | 0 | 0 | 20-3-10 ( 13-11-82 ) | |
| L | 617 | 0 | 617 | 0 | 0 | 20-3-10 ( 13-11-82 ) | |

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| H | 120 | 89 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 31 / 0 | 0 / 0 |
| B | 186 | 135 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 51 / 0 | 0 / 0 |
| M | 253 | 152 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 101 / 0 | 0 / 0 |
| I | 225 | 132 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 93 / 0 | 0 / 0 |
| K | 405 | 269 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 136 / 0 | 0 / 0 |
| L | 435 | 294 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 141 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, B, M, I, K, L

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A-B | 0 / 15 | -91.8 | -91.8 | 0.03 (1) | 10.00 | |
| B-O | -16 / 0 | -91.8 | -91.8 | 0.04 (1) | 6.25 | |
| O-C | -76 / 0 | -91.8 | -91.8 | 0.12 (1) | 6.25 | |
| C-D | 0 / 15 | -91.8 | -91.8 | 0.30 (1) | 10.00 | |
| D-E | 0 / 15 | -91.8 | -91.8 | 0.30 (1) | 10.00 | |
| E-F | -21 / 0 | -91.8 | -91.8 | 0.29 (1) | 6.25 | |
| F-G | -63 / 0 | -91.8 | -91.8 | 0.09 (1) | 6.25 | |
| H-G | -160 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | |
| B-N | 0 / 59 | -18.5 | -18.5 | 0.11 (1) | 10.00 | |
| N-M | 0 / 59 | -18.5 | -18.5 | 0.11 (1) | 10.00 | |
| M-L | 0 / 48 | -18.5 | -18.5 | 0.09 (4) | 10.00 | |
| L-K | 0 / 21 | -18.5 | -18.5 | 0.09 (4) | 10.00 | |
| K-J | 0 / 41 | -18.5 | -18.5 | 0.09 (4) | 10.00 | |
| J-I | 0 / 41 | -18.5 | -18.5 | 0.09 (4) | 10.00 | |
| I-H | 0 / 0 | -18.5 | -18.5 | 0.07 (4) | 10.00 | |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| M-C | -238 / 0 | 0.04 (1) |
| I-F | -250 / 0 | 0.04 (1) |
| I-G | 0 / 54 | 0.01 (1) |
| K-F | -22 / 0 | 0.01 (1) |
| C-L | -68 / 0 | 0.03 (1) |
| K-E | -472 / 0 | 0.08 (1) |
| L-D | -488 / 0 | 0.08 (1) |
| L-E | -39 / 0 | 0.02 (1) |
| N-O | -248 / 0 | 0.00 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (D-E:1) , BC=0.11/1.00 (B-N:1) , WB=0.08/1.00 (D-L:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90 )
JSI METAL= 0.10 (D) (INPUT = 0.95 )

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | V1 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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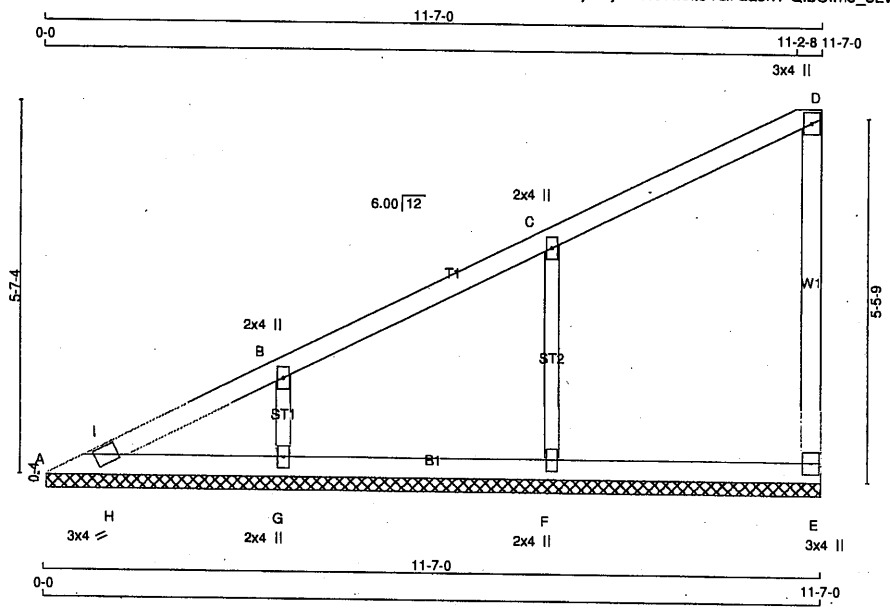


TOTAL WEIGHT = 37 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| A - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TBM1-h | MT20 | 3.0 | 4.0 | | |
| B | TMW+w | MT20 | 2.0 | 4.0 | | |
| C | TMW+w | MT20 | 2.0 | 4.0 | | |
| D | TMV+p | MT20 | 3.0 | 4.0 | | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | | |
| F | BMW1+w | MT20 | 2.0 | 4.0 | | |
| G | BMW1+w | MT20 | 2.0 | 4.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| A | 160 | 0 | 160 | 0 | 0 | 11-6-8 | 1-8 |
| E | 178 | 0 | 178 | 0 | 0 | 11-6-8 | 1-8 |
| F | 495 | 0 | 495 | 0 | 0 | 11-6-8 | 1-8 |
| G | 440 | 0 | 440 | 0 | 0 | 11-6-8 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| A | 113 | 75 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 38 / 0 | 0 / 0 |
| E | 126 | 83 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 43 / 0 | 0 / 0 |
| F | 350 | 230 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 120 / 0 | 0 / 0 |
| G | 311 | 203 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 108 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, F, G

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX: UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- I | -37 / 0 | -91.8 | -91.8 | 0.05 (1) | 6.25 | F- C | -418 / 0 | 0.10 (1) |
| I- B | 0 / 2 | -91.8 | -91.8 | 0.15 (1) | 10.00 | G- B | -335 / 0 | 0.05 (1) |
| B- C | -3 / 0 | -91.8 | -91.8 | 0.22 (1) | 10.00 | H- I | -66 / 3 | 0.00 (1) |
| C- D | -17 / 0 | -91.8 | -91.8 | 0.22 (1) | 6.25 | | | |
| E- D | -147 / 0 | 0.0 | 0.0 | 0.09 (1) | 7.81 | | | |
| A- H | 0 / 15 | -18.5 | -18.5 | 0.09 (1) | 10.00 | | | |
| H- G | 0 / 23 | -18.5 | -18.5 | 0.09 (1) | 10.00 | | | |
| G- F | 0 / 8 | -18.5 | -18.5 | 0.07 (4) | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.07 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-D:1) , BC=0.09/1.00 (G-H:1) , WB=0.10/1.00 (C-F:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (C) (INPUT = 0.90 )
JSI METAL= 0.17 (C) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311522

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | V2 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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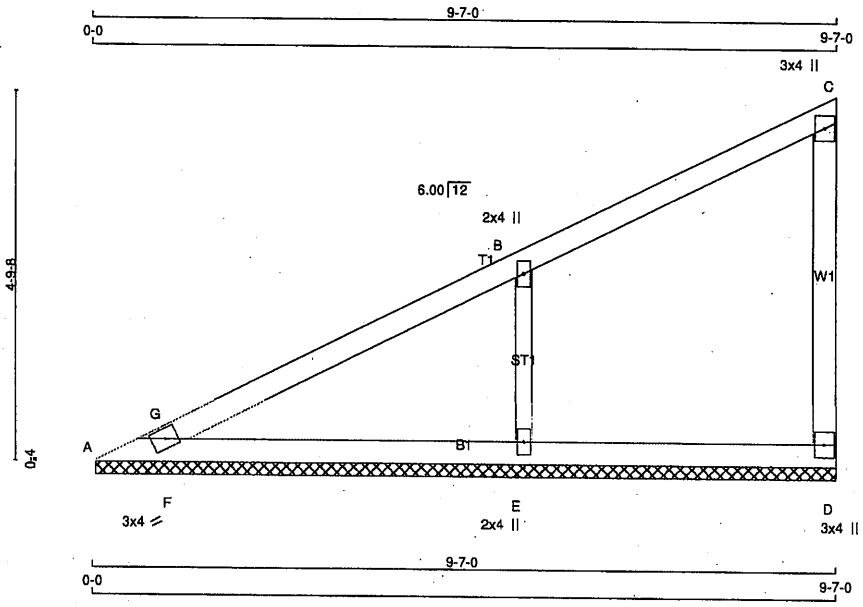


TOTAL WEIGHT = 29 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| D - C | 2x4 | DRY | No.2 | SPF |
| A - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| A | TBM1-h | MT20 | 3.0 | 4.0 | |
| B | TMW+w | MT20 | 2.0 | 4.0 | |
| C | TMV+p | MT20 | 3.0 | 4.0 | |
| D | BMV1+p | MT20 | 3.0 | 4.0 | |
| E | BMW1+w | MT20 | 2.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| A | 248 | 0 | 248 | 0 | 0 | 9-6-8 | 1-8 |
| D | 141 | 0 | 141 | 0 | 0 | 9-6-8 | 1-8 |
| E | 663 | 0 | 663 | 0 | 0 | 9-6-8 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| A | 175 | 115 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 60 / 0 | 0 / 0 |
| D | 99 | 66 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 34 / 0 | 0 / 0 |
| E | 469 | 307 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 162 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D, E

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 FROM (PLF) | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| A-G | -29 / 0 | -91.8 | -91.8 | 0.07 (4) | 6.25 |
| G-B | 0 / 13 | -91.8 | -91.8 | 0.32 (1) | 10.00 |
| B-C | -26 / 0 | -91.8 | -91.8 | 0.32 (1) | 6.25 |
| D-C | -129 / 0 | 0.0 | 0.0 | 0.05 (1) | 7.81 |
| A-F | -7 / 0 | -18.5 | -18.5 | 0.20 (1) | 10.00 |
| F-E | 0 / 13 | -18.5 | -18.5 | 0.21 (1) | 10.00 |
| E-D | 0 / 0 | -18.5 | -18.5 | 0.14 (1) | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| E-B | -504 / 0 | 0.09 (1) |
| F-G | -149 / 5 | 0.00 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (B-G:1) , BC=0.21/1.00 (E-F:1) , WB=0.09/1.00 (B-E:1) , SSI=0.19/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90 )
JSI METAL= 0.21 (B) (INPUT = 0.95 )


Structural component only
DWG# T-2311523

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | V3 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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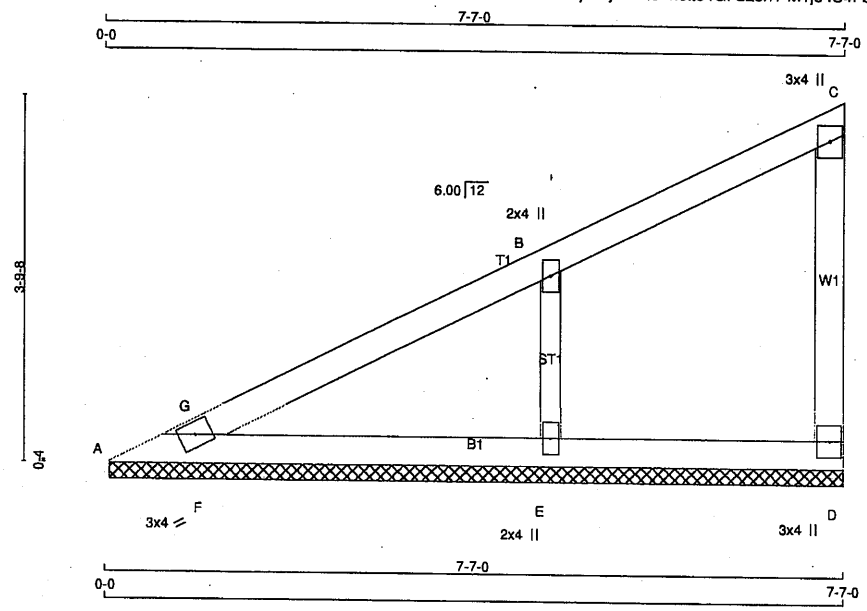


TOTAL WEIGHT = 23 lb

[M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| D - C | 2x4 | DRY | No.2 | SPF |
| A - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| A | TBM1-h | MT20 | 3.0 | 4.0 | |
| B | TMW+w | MT20 | 2.0 | 4.0 | |
| C | TMV+p | MT20 | 3.0 | 4.0 | |
| D | BMV1+p | MT20 | 3.0 | 4.0 | |
| E | BMW1+w | MT20 | 2.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| A | 205 | 0 | 205 | 0 | 0 | 7-6-8 | 1-8 |
| D | 97 | 0 | 97 | 0 | 0 | 7-6-8 | 1-8 |
| E | 529 | 0 | 529 | 0 | 0 | 7-6-8 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| A | 145 | 95 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 50 / 0 | 0 / 0 |
| D | 68 | 45 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 23 / 0 | 0 / 0 |
| E | 374 | 245 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 129 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D, E

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| A- G | -28 / 0 | -91.8 | -91.8 | 0.06 (1) | 6.25 | E- B | -395 / 0 | 0.06 (1) |
| G- B | 0 / 10 | -91.8 | -91.8 | 0.20 (1) | 10.00 | F- G | -111 / 4 | 0.00 (1) |
| B- C | -21 / 0 | -91.8 | -91.8 | 0.20 (1) | 6.25 | | | |
| D- C | -93 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | | | |
| A- F | -1 / 0 | -18.5 | -18.5 | 0.15 (1) | 10.00 | | | |
| F- E | 0 / 13 | -18.5 | -18.5 | 0.15 (1) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.10 (1) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (B-G:1) , BC=0.15/1.00 (E-F:1) , WB=0.06/1.00 (B-E:1) , SSI=0.15/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90 )
JSI METAL= 0.16 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311524

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | V4 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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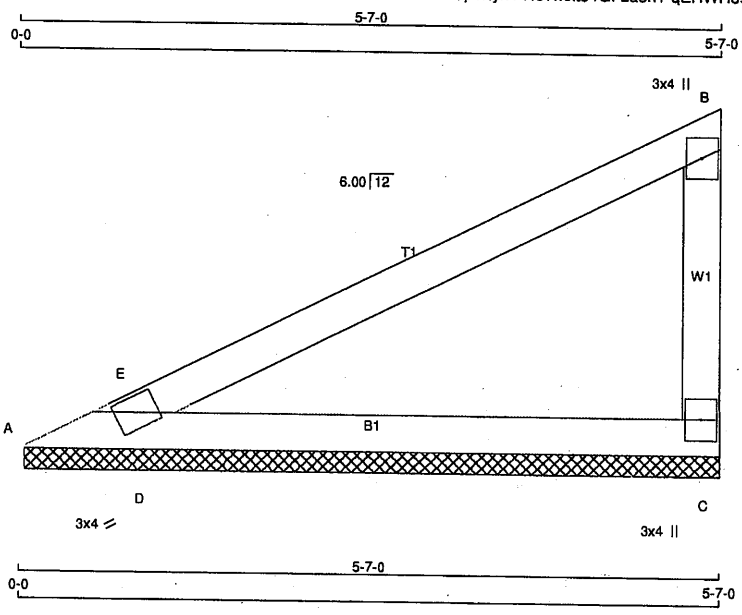


TOTAL WEIGHT = 15 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - B | 2x4 | DRY | No.2 | SPF |
| C - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TBM1-h | MT20 | 3.0 | 4.0 | | |
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| A | 305 | 0 | 305 | 0 | 0 | 5-6-8 | 1-8 |
| C | 305 | 0 | 305 | 0 | 0 | 5-6-8 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| A | 216 | 142 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 74 / 0 | 0 / 0 |
| C | 216 | 142 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 74 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- E | -14 / 0 | -91.8 | -91.8 | 0.09 (4) | | 6.25 | D- E | -209 / 5 | 0.00 (1) |
| E- B | 0 / 4 | -91.8 | -91.8 | 0.34 (1) | | 10.00 | | | |
| C- B | -215 / 0 | 0.0 | 0.0 | 0.03 (1) | | 7.81 | | | |
| A- D | -30 / 0 | -18.5 | -18.5 | 0.30 (1) | | 6.25 | | | |
| D- C | 0 / 0 | -18.5 | -18.5 | 0.30 (1) | | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (B-E:1) , BC=0.30/1.00 (A-D:1) , WB=0.00/1.00 (D-E:1) , SSI=0.17/1.00 (A-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (A) (INPUT = 0.90 )
JSI METAL= 0.06 (B) (INPUT = 0.95 )


Structural component only
DWG# T-2311525

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | V5 | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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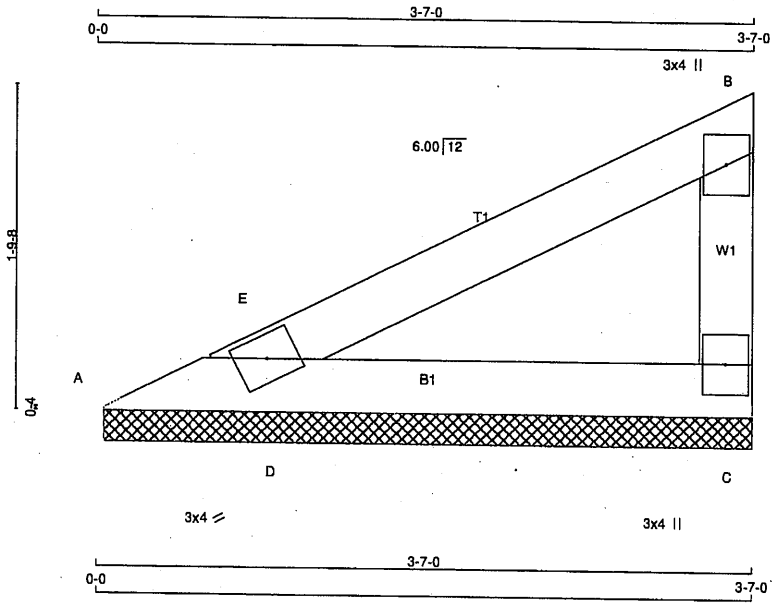


Scale = 1:11.7

TOTAL WEIGHT = 9 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - B | 2x4 | DRY | No.2 | SPF |
| C - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TBM1-h | MT20 | 3.0 | 4.0 | | |
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| A | 195 | 0 | 195 | 0 | 0 | 3-6-8 | 1-8 |
| C | 195 | 0 | 195 | 0 | 0 | 3-6-8 | 1-8 |

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| A | 138 | 91 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 47 / 0 | 0 / 0 |
| C | 138 | 91 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 47 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- E | -15 / 0 | -91.8 | -91.8 | 0.06 (1) | | 6.25 | D- E | -102 / 3 | 0.00 (1) |
| E- B | 0 / 8 | -91.8 | -91.8 | 0.13 (1) | | 10.00 | | | |
| C- B | -133 / 0 | 0.0 | 0.0 | 0.01 (1) | | 7.81 | | | |
| A- D | -13 / 0 | -18.5 | -18.5 | 0.13 (1) | | 6.25 | | | |
| D- C | 0 / 0 | -18.5 | -18.5 | 0.13 (1) | | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (B-E:1) , BC=0.13/1.00 (A-D:1) , WB=0.00/1.00 (D-E:1) , SSI=0.09/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (A) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311526

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | J1 | 21 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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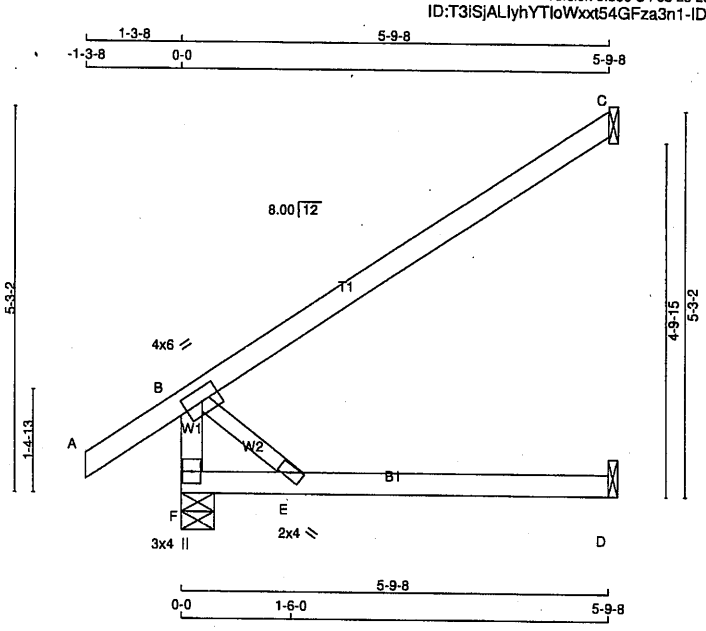


TOTAL WEIGHT = 21 X 19 = 399 lb (M)

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| E | BMW+w | MT20 | 2.0 | 4.0 | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 445 | 0 | 445 | 0 | 0 | 5-8 | 1-8 |
| C | 266 | 0 | 266 | 0 | 0 | 1-8 | 1-8 |
| D | 54 | 0 | 60 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 312 | 218 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 94 / 0 | 0 / 0 |
| C | 183 | 148 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 35 / 0 | 0 / 0 |
| D | 43 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 43 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -392 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.52 (1) | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.14 (4) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.18 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.18/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90 )
JSI METAL= 0.07 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311434

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | J2 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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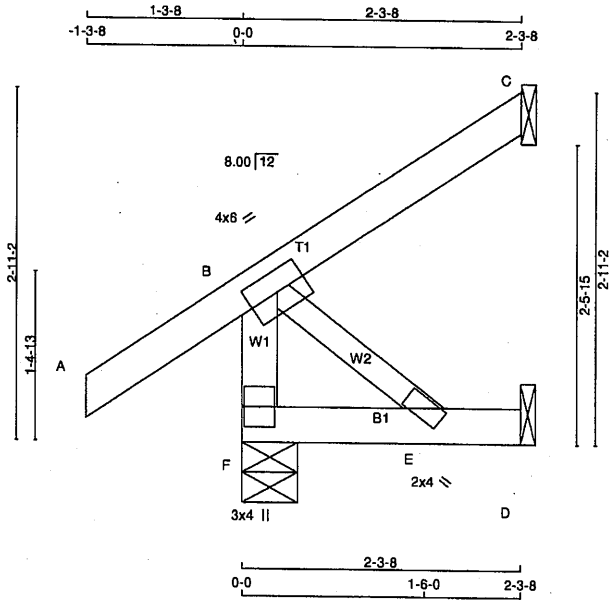


TOTAL WEIGHT = 3 X 10 = 31 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in Inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| E | BMW+w | MT20 | 2.0 | 4.0 | | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 252 | 0 | 252 | 0 | 0 | 5-8 | 1-8 |
| C | 105 | 0 | 105 | 0 | 0 | 1-8 | 1-8 |
| D | 21 | 0 | 24 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 176 | 129 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 47 / 0 | 0 / 0 |
| C | 72 | 59 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 14 / 0 | 0 / 0 |
| D | 17 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 17 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -231 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.13 (5) | 10.00 | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.08 (1) | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311435

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | J3 | 7 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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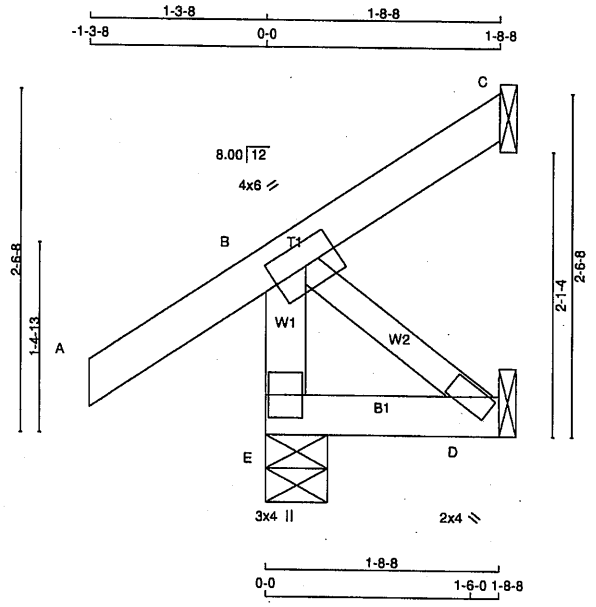


TOTAL WEIGHT = 7 X 9 = 61 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| D | BMW1+w | MT20 | 2.0 | 4.0 | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX. | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 269 | 0 | 269 | 0 | 0 | 5-8 | 1-8 |
| C | 28 | 0 | 28 | 0 | -41 | 1-8 | 1-8 |
| D | 14 | 0 | 16 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 187 | 142 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 44 / 0 | 0 / 0 |
| C | 19 | 15 / -28 | 0 / 0 | 0 / 0 | 0 / 0 | 4 / 0 | 0 / 0 |
| D | 11 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 11 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| E- B | -255 / 0 | 0.0 | 0.0 | 0.03 (1) | | 7.81 | B- D | 0 / 0 | 0.00 (1) |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | | 10.00 | | | |
| B- C | -28 / 0 | -91.8 | -91.8 | 0.12 (1) | | 6.25 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.01 (4) | | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311436

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | J4 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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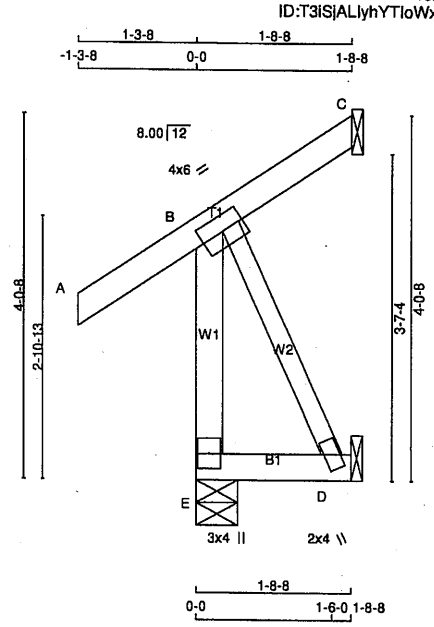


TOTAL WEIGHT = 3 X 11 = 34 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| D | BMW1+w | MT20 | 2.0 | 4.0 | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 269 | 0 | 269 | 0 | 0 | 5-8 | 1-8 |
| C | 28 | 0 | 28 | 0 | -41 | 1-8 | 1-8 |
| D | 14 | 0 | 16 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 187 | 142 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 44 / 0 | 0 / 0 |
| C | 19 | 15 / -28 | 0 / 0 | 0 / 0 | 0 / 0 | 4 / 0 | 0 / 0 |
| D | 11 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 11 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| E- B | -255 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | B- D | 0 / 0 | 0.00 (1) |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | | |
| B- C | -28 / 0 | -91.8 | -91.8 | 0.12 (1) | 6.25 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.01 (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT. CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )


Structural component only
DWG# T-2311437

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | J5 | 10 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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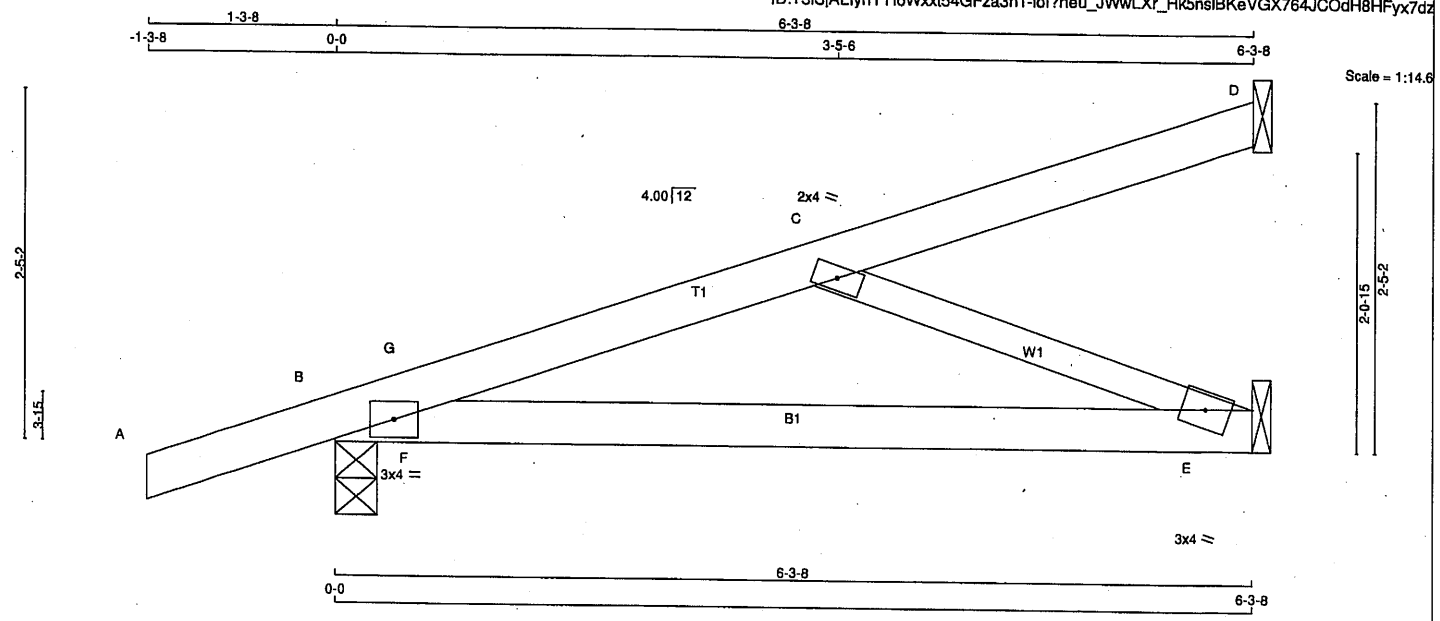


**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - D | 2x4 | DRY | No.2 | SPF |
| B - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMB1-I | MT20 | 3.0 | 4.0 | | |
| C | TMW+w | MT20 | 2.0 | 4.0 | | |
| E | BMW1+w | MT20 | 3.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| D | 97 | 0 | 97 | 0 | 0 | 1-8 | 1-8 |
| B | 456 | 0 | 456 | 0 | 0 | 3-8 | 1-8 |
| E | 257 | 0 | 257 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| D | 66 | 57 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 9 / 0 | 0 / 0 |
| B | 320 | 224 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 96 / 0 | 0 / 0 |
| E | 184 | 109 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 75 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| A- B | 0 / 18 | -91.8 | -91.8 | 0.11 (1) | | 10.00 |
| B- G | -531 / 0 | -91.8 | -91.8 | 0.10 (4) | | 6.25 |
| G- C | -462 / 0 | -91.8 | -91.8 | 0.11 (1) | | 6.25 |
| C- D | -11 / 0 | -91.8 | -91.8 | 0.11 (1) | | 6.25 |
| B- F | 0 / 457 | -18.5 | -18.5 | 0.09 (4) | | 10.00 |
| F- E | 0 / 457 | -18.5 | -18.5 | 0.17 (1) | | 10.00 |

| WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| C- E | -502 / 0 | 0.11 (1) |
| F- G | 0 / 117 | 0.00 (1) |

TOTAL WEIGHT = 10 X 19 = 189 lb
[M][F]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.11/1.00 (C-G:1) , BC=0.17/1.00 (E-F:1) , WB=0.11/1.00 (C-E:1) , SSI=0.14/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (C) (INPUT = 0.90 )
JSI METAL= 0.25 (C) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311438

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 436956 | J6 | 4 | 1 | GREEN PARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.630 S Aug 30 2023 MiTek Industries, Inc. Wed Jan 3 11:00:54 2024 Page 1
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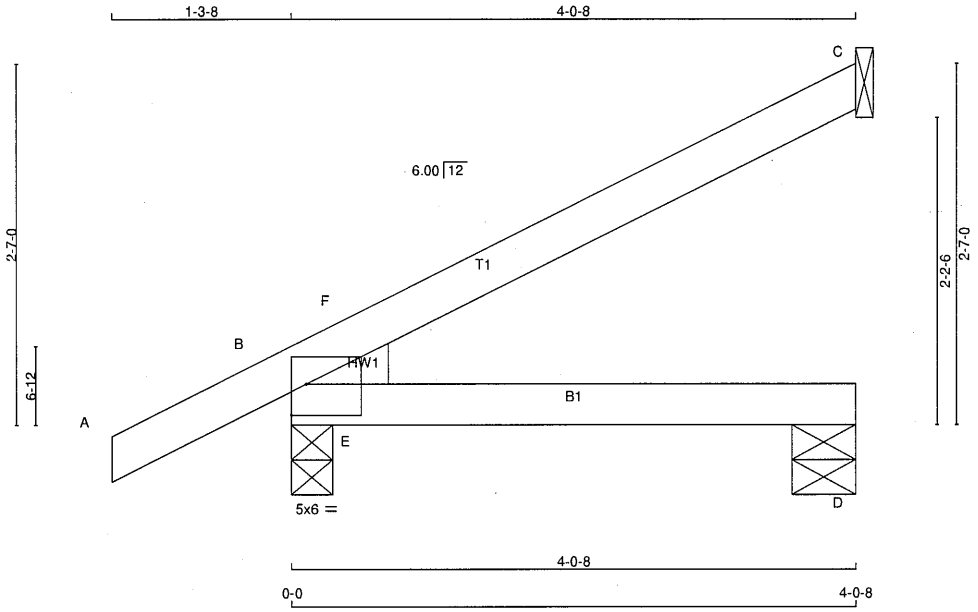


TOTAL WEIGHT = 4 X 12 = 49 lb

[M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| B - D | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMBH1-I | MT20 | 5.0 | 6.0 | 2.75 | Edge |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX | HEEL WEDGE |
|---|---|---|---|---|---|---|---|---|
| C | 160 | 0 | 160 | 0 | 0 | 1-8 | 1-8 | |
| B | 347 | 0 | 347 | 0 | 0 | 3-8 | 1-8 | 2x4 L |
| D | 63 | 0 | 63 | 0 | 0 | 5-8 | 1-8 | |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| C | 111 | 86 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 25 / 0 | 0 / 0 |
| B | 243 | 173 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 70 / 0 | 0 / 0 |
| D | 47 | 17 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 30 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | WEBS MAX. UNBRAC LENGTH | MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 19 | -91.8 | -91.8 | 1 | 0.12 (1) | 10.00 | E- F | -198 / 4 | 0.00 (1) |
| B- F | -23 / 8 | -91.8 | -91.8 | 4 | 0.04 (4) | 6.25 | | | |
| F- C | 0 / 2 | -91.8 | -91.8 | 1 | 0.19 (1) | 10.00 | | | |
| B- E | 0 / 0 | -18.5 | -18.5 | 1 | 0.14 (1) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 1 | 0.14 (1) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.19/1.00 (C-F:1) , BC=0.14/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
01-03-24
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2400313

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432488 | J11 | 9 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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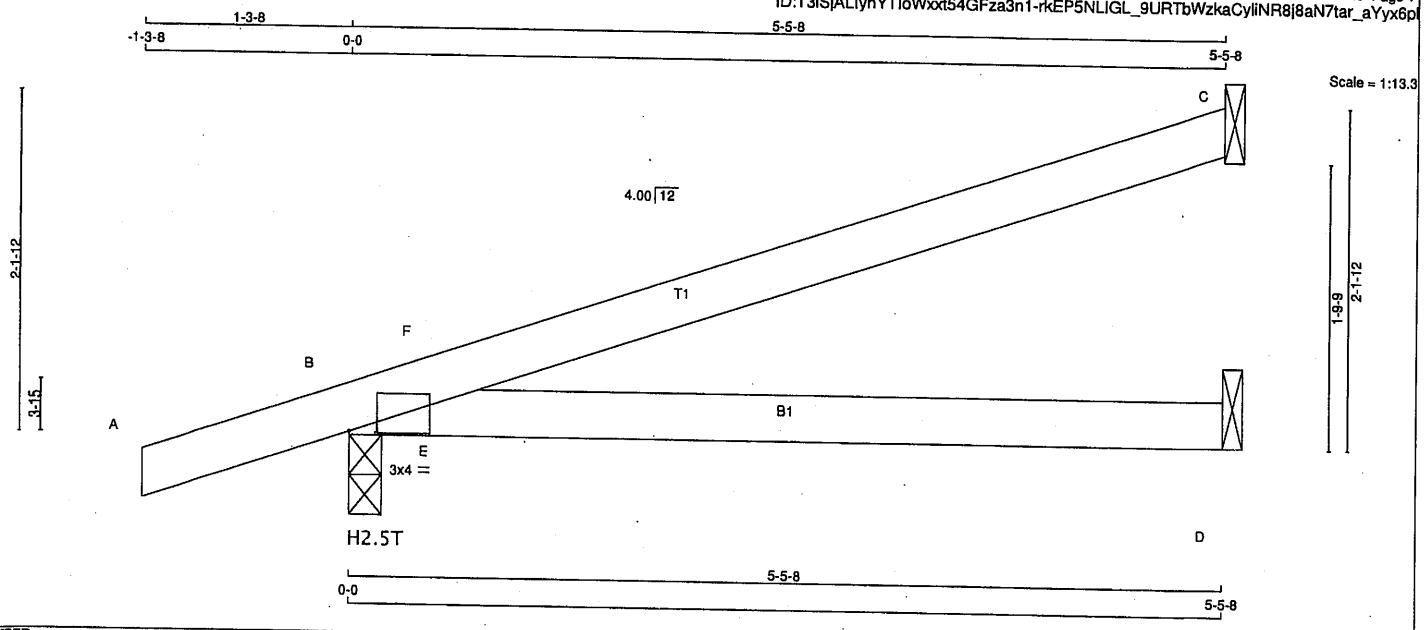


**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| A - C | 2x4 | DRY | No.2 | SPF |
| B - D | 2x4 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMB1-I | MT20 | 3.0 | 4.0 | 2.25 |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| C | 218 | 0 | 218 | 0 | -143 | 1-8 | 1-8 |
| B | 424 | 0 | 424 | 131 | -254 | 2-8 | 1-8 |
| D | 83 | 0 | 83 | 0 | -101 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 254 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 131 LBS FACTORED HORIZONTAL REACTION AT JOINT B

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| C | 151 | 118 / 0 | 0 / 0 | 0 / 0 | 0 / -124 | 34 / 0 | 0 / 0 |
| B | 297 | 208 / 0 | 0 / 0 | 0 / 0 | 0 / -238 | 89 / 0 | 0 / 0 |
| D | 62 | 22 / 0 | 0 / 0 | 0 / 0 | 0 / -98 | 40 / 0 | 0 / 0 |
| HORIZONTAL REACTIONS | | | | | | | |
| B | --- | 0 / 0 | 0 / 0 | 0 / 0 | 93 / 0 | 0 / 0 | 0 /0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (12)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | VERT. LOAD FROM | TO | LC1 MAX (PLF) | CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| A- B | 0 / 18 | -91.8 | -91.8 | 0.11 (1) | | 10.00 | E- F | -304 / 110 | 0.00 (1) |
| B- F | -93 / 14 | -91.8 | -91.8 | 0.06 (12) | | 6.25 | | | |
| F- C | -53 / 2 | -91.8 | -91.8 | 0.35 (1) | | 6.25 | | | |
| B- E | 0 / 0 | -18.5 | -18.5 | 0.25 (1) | | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.25 (1) | | 10.00 | | | |

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF ( 9.2) PSF AT (15-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM).INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2). BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

TOTAL WEIGHT = 9 X 14 = 130 lb [M]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING =** 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 819 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 607 (0.11")

CSI: TC=0.35/1.00 (C-F:1) , BC=0.25/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.24/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90 )
JSI METAL= 0.07 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311467

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | J21 | 12 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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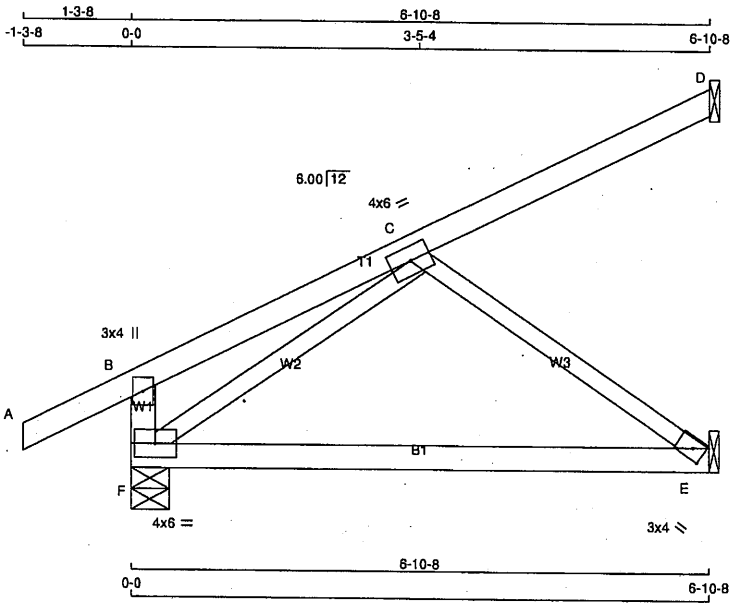


TOTAL WEIGHT = 12 X 26 = 309 lb
[M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - D | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| E | BMW1+w | MT20 | 3.0 | 4.0 | 1.50 | 1.50 |
| F | BMWW1-t | MT20 | 4.0 | 6.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 497 | 0 | 497 | 0 | 0 | 5-8 | 1-8 |
| D | 125 | 0 | 125 | 0 | 0 | 1-8 | 1-8 |
| E | 257 | 0 | 257 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 349 | 242 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 106 / 0 | 0 / 0 |
| D | 86 | 70 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 16 / 0 | 0 / 0 |
| E | 184 | 109 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 75 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

CHORDS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|
| F- B | -239 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 |
| B- C | 0 / 17 | -91.8 | -91.8 | 0.18 (1) | 10.00 |
| C- D | -16 / 0 | -91.8 | -91.8 | 0.14 (1) | 6.25 |
| F- E | 0 / 258 | -18.5 | -18.5 | 0.25 (4) | 10.00 |

WEBS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| F- C | -324 / 0 | 0.09 (1) |
| C- E | -324 / 0 | 0.11 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 888 (0.09")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.25/1.00 (E-F:4) , WB=0.11/1.00 (C-E:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.90 )
JSI METAL= 0.11 (E) (INPUT = 0.95 )


Structural component only
DWG# T-2311485

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | J21A | 2 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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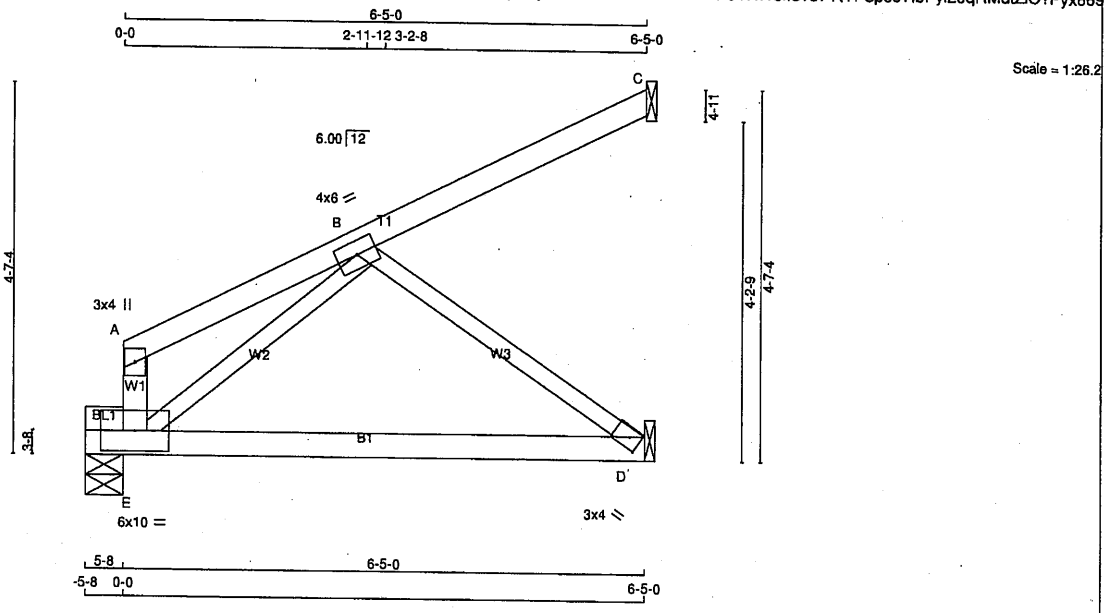


TOTAL WEIGHT = 2 X 24 = 48 lb
[M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - A | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| BEARING BLOCKS | | | | |
| BL1 | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMV+p | MT20 | 3.0 | 4.0 | | |
| B | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | BMW1+w | MT20 | 3.0 | 4.0 | 1.50 | 1.50 |
| E | BMVWK1-t | MT20 | 6.0 | 10.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 353 | 0 | 353 | 0 | 0 | 5-8 | 5-0 |
| C | 129 | 0 | 129 | 0 | 0 | 1-8 | 1-8 |
| D | 220 | 0 | 220 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 249 | 165 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 84 / 0 | 0 / 0 |
| C | 89 | 72 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 17 / 0 | 0 / 0 |
| D | 157 | 91 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 66 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

CHORDS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH |
|---|---|---|---|---|---|---|
| E- A | -87 / 0 | 0.0 | 0.0 | 0.01 (1) | | 7.81 |
| A- B | 0 / 31 | -91.8 | -91.8 | 0.16 (1) | | 10.00 |
| B- C | -15 / 0 | -91.8 | -91.8 | 0.12 (1) | | 6.25 |
| E- D | 0 / 216 | -18.5 | -18.5 | 0.21 (4) | | 10.00 |

WEBS

| MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|
| E- B | -309 / 0 | 0.08 (1) |
| B- D | -270 / 0 | 0.09 (1) |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.21/1.00 (D-E:4) , WB=0.09/1.00 (B-D:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (D) (INPUT = 0.90 )
JSI METAL= 0.09 (D) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311486

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | J22 | 3 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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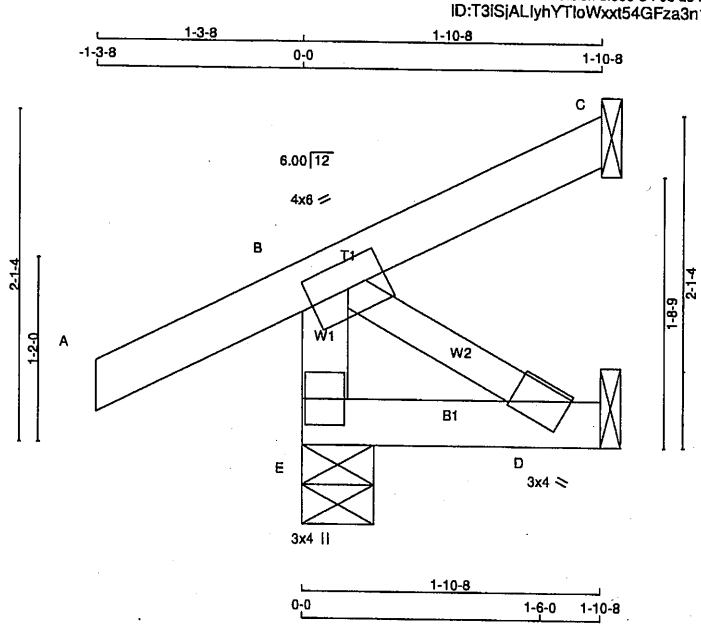


**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| D | BMW1+w | MT20 | 3.0 | 4.0 | | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 269 | 0 | 269 | 0 | 0 | 5-8 | 1-8 |
| C | 41 | 0 | 41 | 0 | -35 | 1-8 | 1-8 |
| D | 14 | 0 | 16 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 187 | 143 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 45 / 0 | 0 / 0 |
| C | 28 | 23 / -25 | 0 / 0 | 0 / 0 | 0 / 0 | 5 / 0 | 0 / 0 |
| D | 11 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 11 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD LC1 (PLF) FROM | TO | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| E-B | -256 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | B- D | 0 / 0 | 0.00 (1) |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | | |
| B- C | -20 / 0 | -91.8 | -91.8 | 0.11 (1) | 6.25 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.01 (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 8 = 25 lb [M]

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311487

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | J23 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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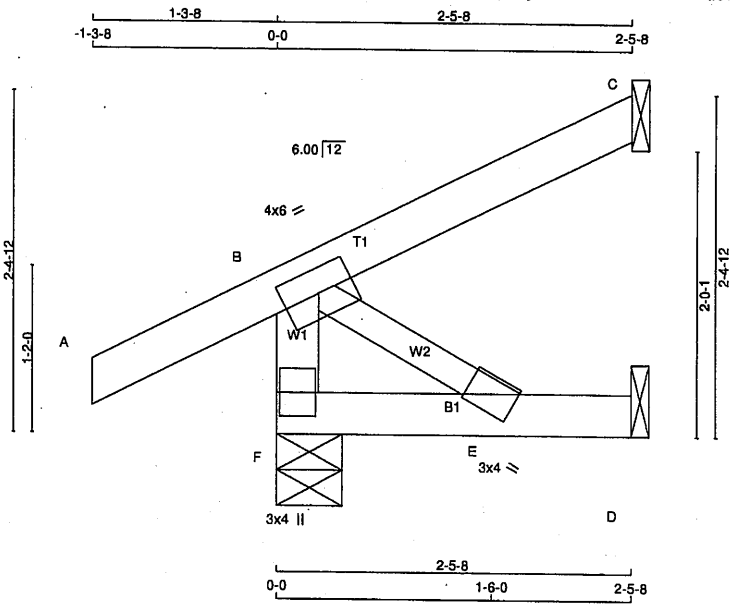


TOTAL WEIGHT = 3 X 10 = 29 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| E | BMW+w | MT20 | 3.0 | 4.0 | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 260 | 0 | 260 | 0 | 0 | 5-8 | 1-8 |
| C | 113 | 0 | 113 | 0 | 0 | 1-8 | 1-8 |
| D | 23 | 0 | 25 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 182 | 132 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 49 / 0 | 0 / 0 |
| C | 78 | 63 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 15 / 0 | 0 / 0 |
| D | 18 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 18 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -237 / 0 | 0.0 | 0.0 | 0.02 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (5) | 10.00 | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.09 (1) | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90 )
JSI METAL= 0.03 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311488

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | J24 | 4 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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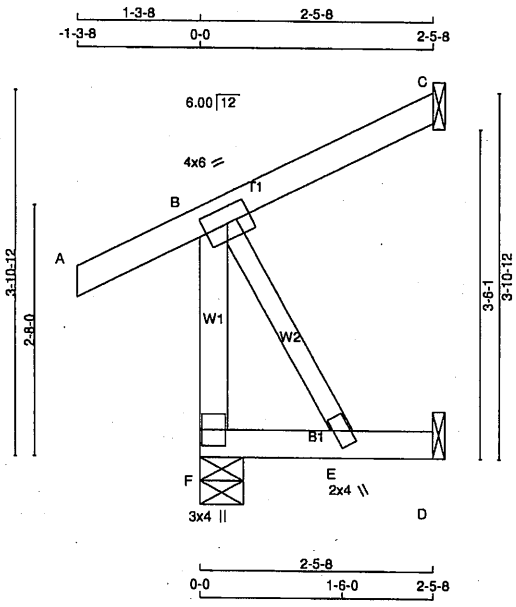


TOTAL WEIGHT = 4 X 12 = 49 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| E | BMW+w | MT20 | 2.0 | 4.0 | | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS, AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 260 | 0 | 260 | 0 | 0 | 5-8 | 1-8 |
| C | 113 | 0 | 113 | 0 | 0 | 1-8 | 1-8 |
| D | 23 | 0 | 25 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 182 | 132 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 49 / 0 | 0 / 0 |
| C | 78 | 63 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 15 / 0 | 0 / 0 |
| D | 18 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 18 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -237 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (5) | 10.00 | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.09 (1) | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.03 (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING =** 24.0 **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90 )
JSI METAL= 0.03 (B) (INPUT = 0.95 )


Structural component only
DWG# T-2311489

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | C1 | 5 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:10 2023 Page 1
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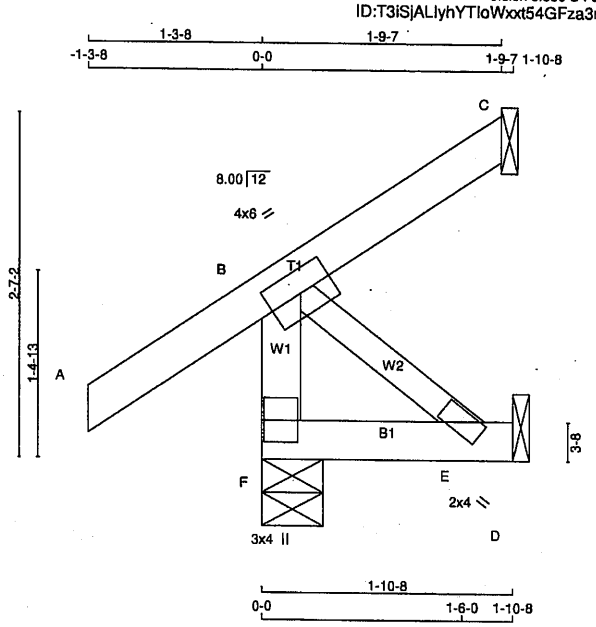


TOTAL WEIGHT = 5 X 9 = 45 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| E | BMW+w | MT20 | 2.0 | 4.0 | | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 274 | 0 | 274 | 0 | 0 | 5-8 | 1-8 |
| C | 34 | 0 | 34 | 0 | -39 | 1-8 | 1-8 |
| D | 17 | 0 | 19 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 190 | 143 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 47 / 0 | 0 / 0 |
| C | 23 | 19 / -27 | 0 / 0 | 0 / 0 | 0 / 0 | 4 / 0 | 0 / 0 |
| D | 14 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 14 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -256 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | | |
| B- C | -27 / 0 | -91.8 | -91.8 | 0.12 (1) | 6.25 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.02 (4) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.01 (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311430

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | C2 | 5 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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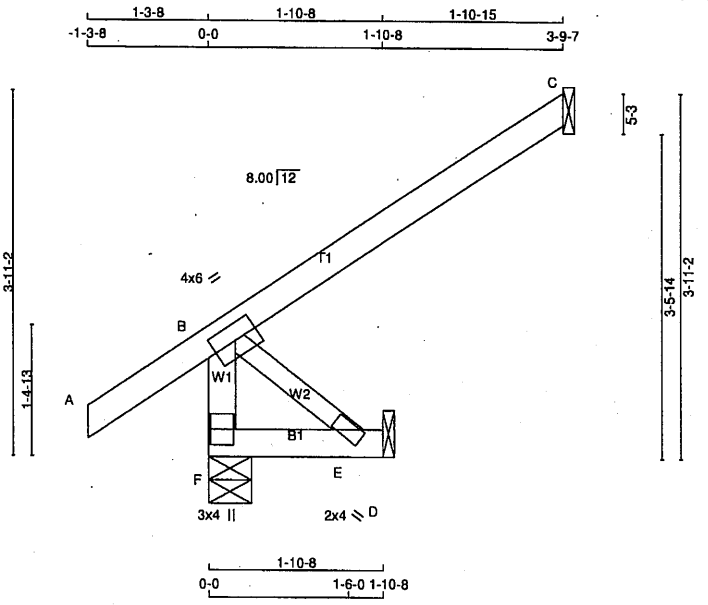


TOTAL WEIGHT = 5 X 12 = 59 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| E | BMW+w | MT20 | 2.0 | 4.0 | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 317 | 0 | 317 | 0 | 0 | 5-8 | 1-8 |
| C | 174 | 0 | 174 | 0 | 0 | 1-8 | 1-8 |
| D | 17 | 0 | 19 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 220 | 167 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 53 / 0 | 0 / 0 |
| C | 120 | 97 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 23 / 0 | 0 / 0 |
| D | 14 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 14 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -300 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.13 (5) | 10.00 | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.22 (1) | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.02 (4) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.01 (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90 )
JSI METAL= 0.05 (B) (INPUT = 0.95 )


Structural component only
DWG# T-2311431

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | C3 | 5 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:11 2023 Page 1
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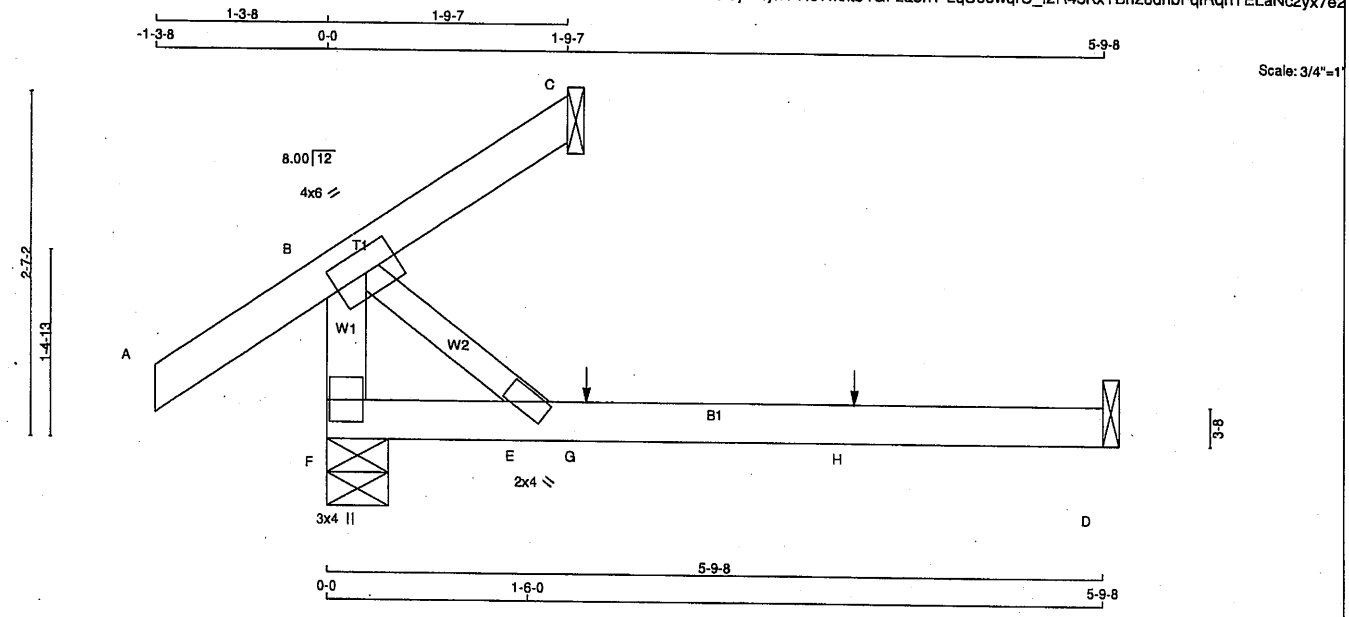


TOTAL WEIGHT = 5 X 13 = 67 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| E | BMW+w | MT20 | 2.0 | 4.0 | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 310 | 0 | 310 | 0 | 0 | 5-8 | 1-8 |
| C | 34 | 0 | 34 | 0 | 0 | 1-8 | 1-8 |
| D | 54 | 0 | 60 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 219 | 143 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 76 / 0 | 0 / 0 |
| C | 23 | 19 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 4 / 0 | 0 / 0 |
| D | 43 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 43 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -256 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.13 (1) | 10.00 | | | |
| B- C | -27 / 0 | -91.8 | -91.8 | 0.12 (1) | 6.25 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.14 (4) | 10.00 | | | |
| E- G | 0 / 0 | -18.5 | -18.5 | 0.18 (4) | 10.00 | | | |
| G- H | 0 / 0 | -18.5 | -18.5 | 0.18 (4) | 10.00 | | | |
| H- D | 0 / 0 | -18.5 | -18.5 | 0.18 (4) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| G | 1-11-4 | 1 | 1 | --- | BACK | VERT | TOTAL | --- | C1 |
| H | 3-11-4 | 1 | 1 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.18/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90 )
JSI METAL= 0.05 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311432

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432478 | C4 | 5 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:12 2023 Page 1
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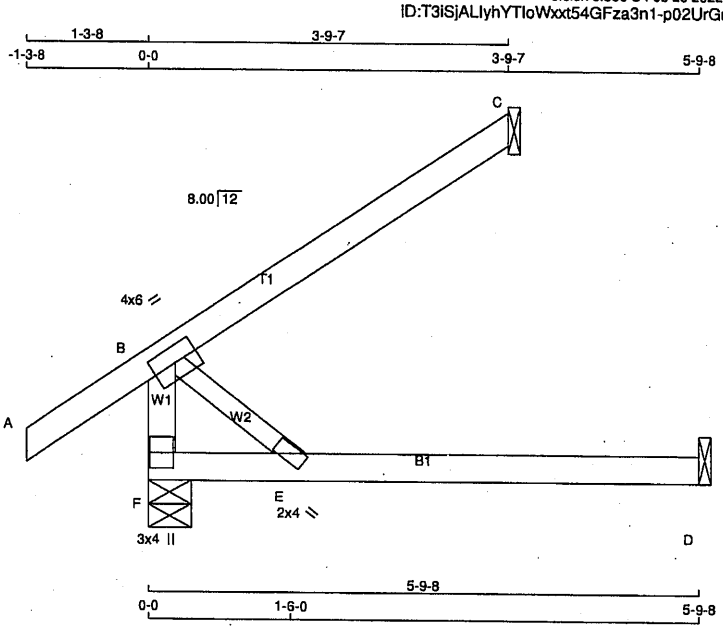


TOTAL WEIGHT = 5 X 16 = 81 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (teble is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| E | BMW+w | MT20 | 2.0 | 4.0 | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 353 | 0 | 353 | 0 | 0 | 5-8 | 1-8 |
| C | 174 | 0 | 174 | 0 | 0 | 1-8 | 1-8 |
| D | 54 | 0 | 60 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 249 | 167 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 82 / 0 | 0 / 0 |
| C | 120 | 97 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 23 / 0 | 0 / 0 |
| D | 43 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 43 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -300 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 35 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.22 (1) | 10.00 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.14 (4) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.18 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.18/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90 )
JSI METAL= 0.05 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311433

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C21 | 4 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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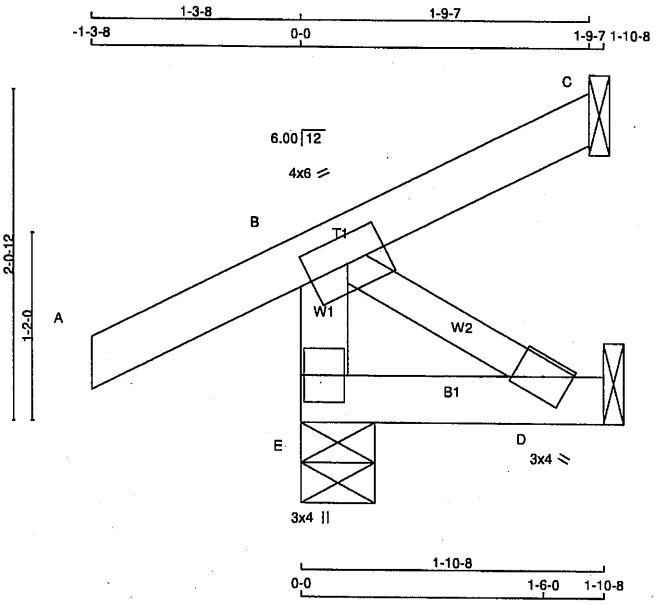


TOTAL WEIGHT = 4 X 8 = 33 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| D | BMW1+w | MT20 | 3.0 | 4.0 | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 268 | 0 | 268 | 0 | 0 | 5-8 | 1-8 |
| C | 35 | 0 | 35 | 0 | -38 | 1-8 | 1-8 |
| D | 14 | 0 | 16 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 186 | 142 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 44 / 0 | 0 / 0 |
| C | 24 | 19 / -26 | 0 / 0 | 0 / 0 | 0 / 0 | 5 / 0 | 0 / 0 |
| D | 11 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 11 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| E- B | -254 / 0 | 0.0 | 0.0 | 0.03 | (1) | 7.81 | B- D | 0 / 0 | 0.00 (1) |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 | (1) | 10.00 | | | |
| B- C | -21 / 0 | -91.8 | -91.8 | 0.11 | (1) | 6.25 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.01 | (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311475

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C22 | 4 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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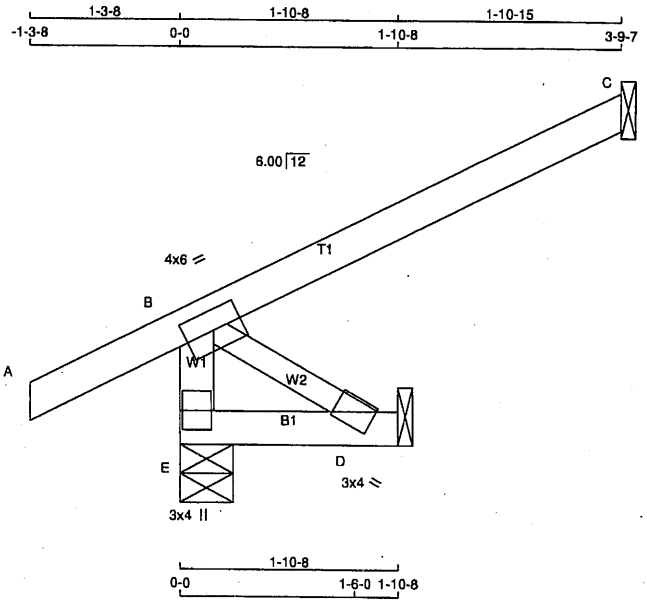


TOTAL WEIGHT = 4 X 11 = 43 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR: |
|---|---|---|---|---|
| E - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| D | BMW1+w | MT20 | 3.0 | 4.0 | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 312 | 0 | 312 | 0 | 0 | 5-8 | 1-8 |
| C | 174 | 0 | 174 | 0 | 0 | 1-8 | 1-8 |
| D | 14 | 0 | 16 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 216 | 166 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 50 / 0 | 0 / 0 |
| C | 120 | 97 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 23 / 0 | 0 / 0 |
| D | 11 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 11 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (5)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| E- B | -298 / 0 | 0.0 | 0.0 | 0.03 | (1) | 7.81 | B- D | 0 / 0 | 0.00 (1) |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 | (5) | 10.00 | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.22 | (1) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.01 | (4) | 10.00 | | | |

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311476

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C23 | 4 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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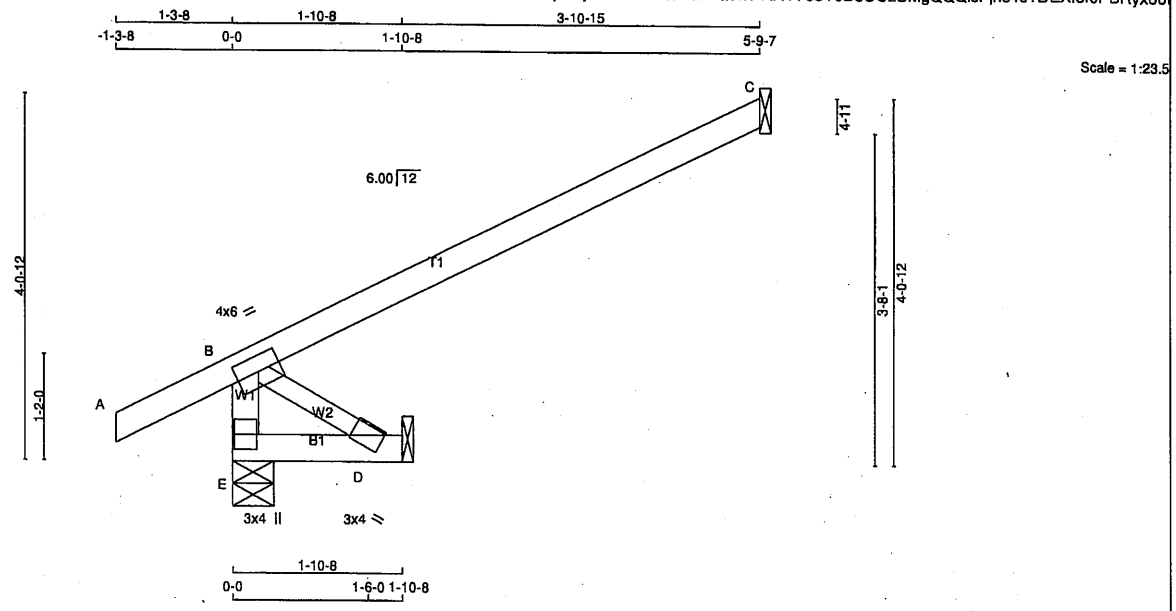


TOTAL WEIGHT = 4 X 13 = 54 lb [M]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF-- |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | | |
| D | BMW1+w | MT20 | 3.0 | 4.0 | | |
| E | BMV1+p | MT20 | 3.0 | 4.0 | | |

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 404 | 0 | 404 | 0 | 0 | 5-8 | 1-8 |
| C | 265 | 0 | 265 | 0 | 0 | 1-8 | 1-8 |
| D | 14 | 0 | 16 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 280 | 217 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 62 / 0 | 0 / 0 |
| C | 183 | 148 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 35 / 0 | 0 / 0 |
| D | 11 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 11 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E .

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| E- B | -390 / 0 | 0.0 | 0.0 | 0.04 (1) | 7.81 | B- D | 0 / 0 | 0.00 (1) |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.52 (1) | 10.00 | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.01 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90 )
JSI METAL= 0.06 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311477

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C24 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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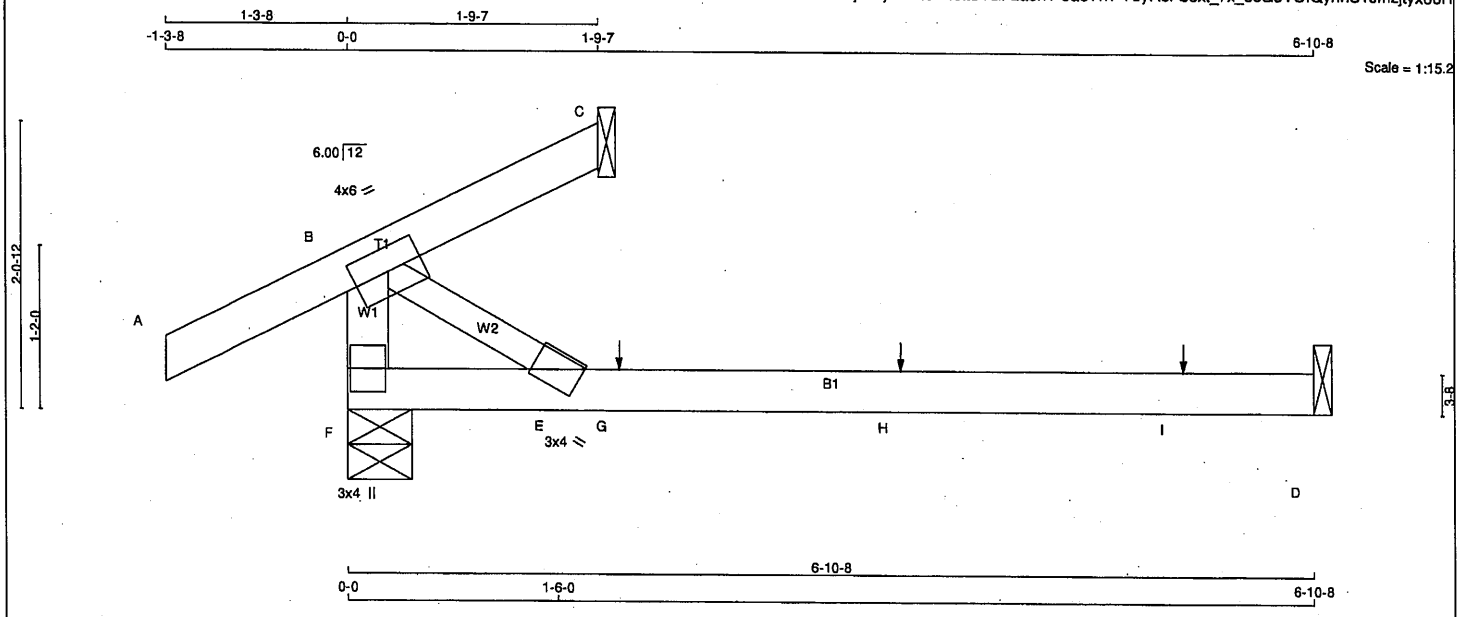


TOTAL WEIGHT = 3 X 14 = 42 lb [M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| E | BMW+w | MT20 | 3.0 | 4.0 | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 317 | 0 | 317 | 0 | 0 | 5-8 | 1-8 |
| C | 35 | 0 | 35 | 0 | 0 | 1-8 | 1-8 |
| D | 64 | 0 | 71 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 226 | 142 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 84 / 0 | 0 / 0 |
| C | 24 | 19 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 5 / 0 | 0 / 0 |
| D | 51 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 51 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -254 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.13 (1) | 10.00 | | | |
| B- C | -21 / 0 | -91.8 | -91.8 | 0.09 (1) | 6.25 | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.17 (4) | 10.00 | | | |
| E- G | 0 / 0 | -18.5 | -18.5 | 0.26 (4) | 10.00 | | | |
| G- H | 0 / 0 | -18.5 | -18.5 | 0.26 (4) | 10.00 | | | |
| H- I | 0 / 0 | -18.5 | -18.5 | 0.26 (4) | 10.00 | | | |
| I- D | 0 / 0 | -18.5 | -18.5 | 0.26 (4) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| G | 1-11-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| H | 3-11-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |
| I | 5-11-4 | 1 | 1 | --- | FRONT | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 819 (0.10")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.26/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.95 NAIL=0.95 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311478

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C24A | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:29 2023 Page 1
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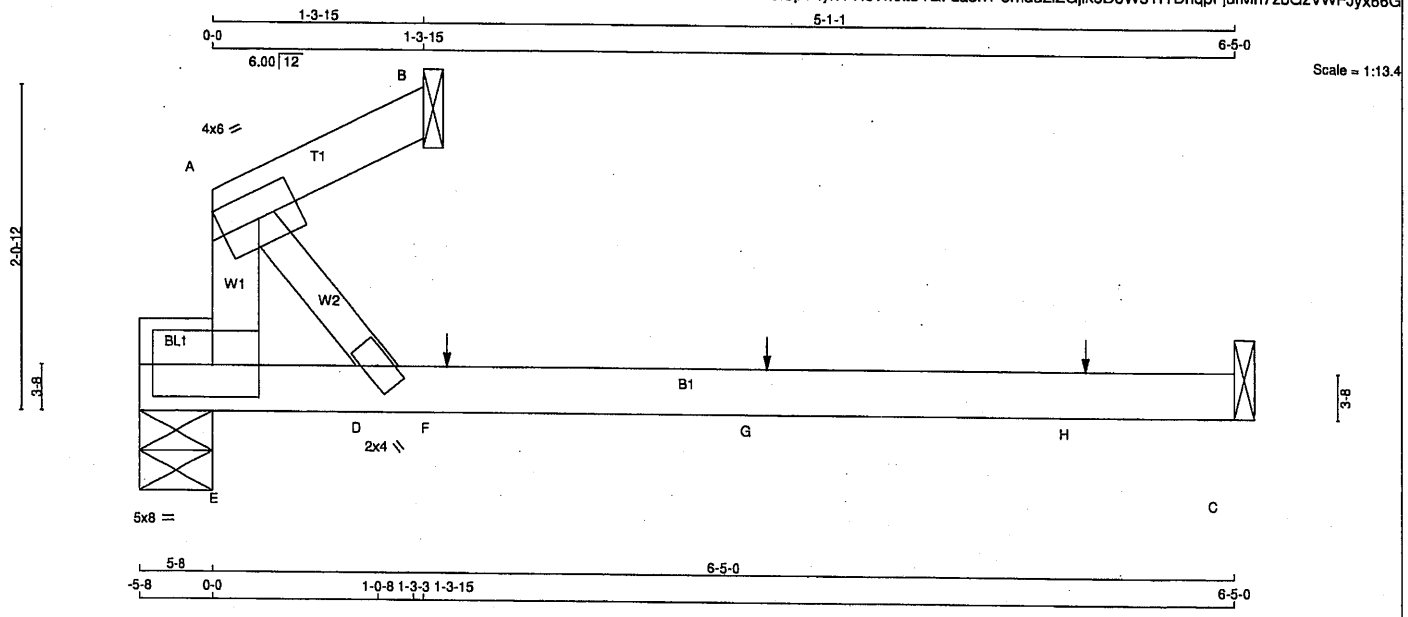


TOTAL WEIGHT = 12 lb [M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - A | 2x4 | DRY | No.2 | SPF |
| A - B | 2x4 | DRY | No.2 | SPF |
| E - C | 2x4 | DRY | No.2 | SPF |
| BEARING BLOCKS | | | | |
| BL1 | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMVW-t | MT20 | 4.0 | 6.0 | | Edge |
| D | BMW+w | MT20 | 2.0 | 4.0 | | |
| E | BMVK1m | MT20 | 5.0 | 8.0 | 2.50 | 3.50 |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 120 | 0 | 120 | 0 | 0 | 5-8 | 5-0 |
| B | 61 | 0 | 61 | 0 | 0 | 1-8 | 1-8 |
| C | 57 | 0 | 65 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 89 | 35 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 55 / 0 | 0 / 0 |
| B | 42 | 34 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 8 / 0 | 0 / 0 |
| C | 45 | 0 / -1 | 0 / 0 | 0 / 0 | 0 / 0 | 46 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| E- A | -71 / 0 | 0.0 | 0.0 | 0.01 (1) | 7.81 | A- D | 0 / 14 | 0.00 (1) |
| A- B | 0 / 0 | -91.8 | -91.8 | 0.03 (1) | 10.00 | | | |
| E- D | -10 / 0 | -18.5 | -18.5 | 0.08 (4) | 10.00 | | | |
| D- F | 0 / 0 | -18.5 | -18.5 | 0.21 (4) | 10.00 | | | |
| F- G | 0 / 0 | -18.5 | -18.5 | 0.21 (4) | 10.00 | | | |
| G- H | 0 / 0 | -18.5 | -18.5 | 0.21 (4) | 10.00 | | | |
| H- C | 0 / 0 | -18.5 | -18.5 | 0.21 (4) | 10.00 | | | |

SPECIFIED CONCENTRATED LOADS (LBS)

| JT | LOC. | LC1 | MAX- | MAX+ | FACE | DIR. | TYPE | HEEL | CONN. |
|---|---|---|---|---|---|---|---|---|---|
| F | 1-5-12 | 1 | 1 | --- | BACK | VERT | TOTAL | --- | C1 |
| G | 3-5-12 | 1 | 1 | --- | BACK | VERT | TOTAL | --- | C1 |
| H | 5-5-12 | 1 | 1 | --- | BACK | VERT | TOTAL | --- | C1 |

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.03/1.00 (A-B:1) , BC=0.21/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.08/1.00 (C-D:4)

DOL LUMBER=0.79 NAIL=0.79 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90 )
JSI METAL= 0.01 (A) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311479

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C25 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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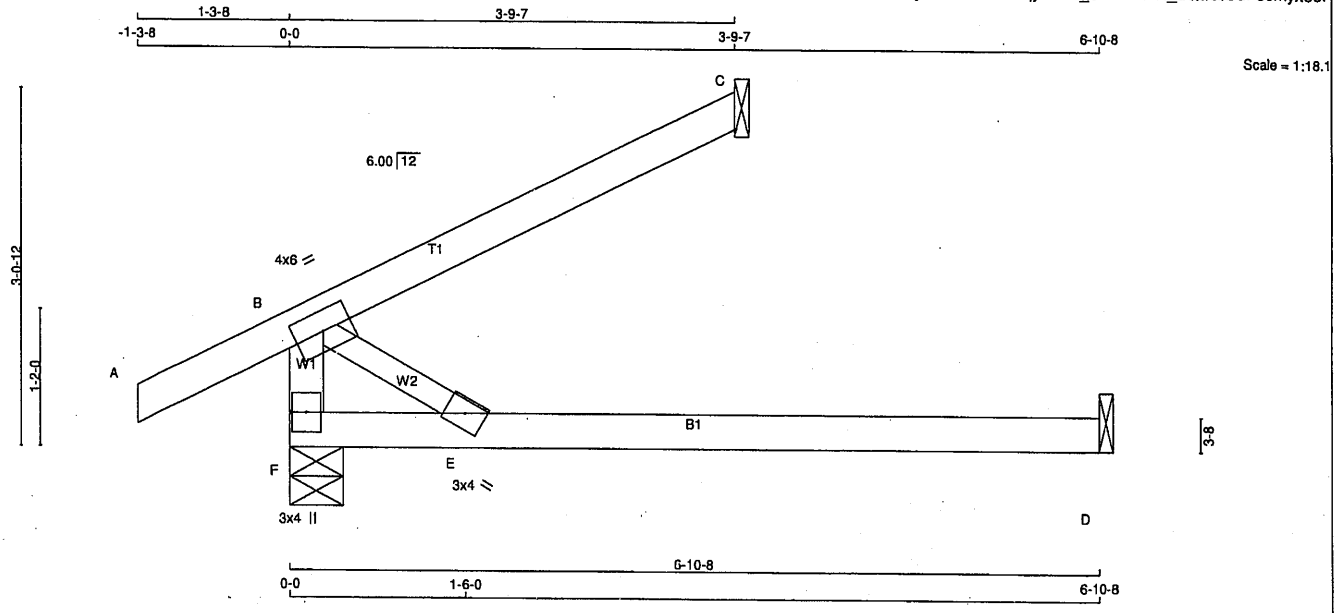


TOTAL WEIGHT = 3 X 17 = 50 lb
[M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| F - D | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in Inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| B | TMVW-t | MT20 | 4.0 | 6.0 | |
| E | BMW+w | MT20 | 3.0 | 4.0 | |
| F | BMV1+p | MT20 | 3.0 | 4.0 | |

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG. IN-SX |
|---|---|---|---|---|---|---|---|
| F | 362 | 0 | 362 | 0 | 0 | 5-8 | 1-8 |
| C | 174 | 0 | 174 | 0 | 0 | 1-8 | 1-8 |
| D | 64 | 0 | 71 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 256 | 166 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 90 / 0 | 0 / 0 |
| C | 120 | 97 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 23 / 0 | 0 / 0 |
| D | 51 | 0 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 51 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| F- B | -298 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | B- E | 0 / 0 | 0.00 (1) | |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | | | | |
| B- C | 0 / 0 | -91.8 | -91.8 | 0.22 (1) | 10.00 | | | | |
| F- E | 0 / 0 | -18.5 | -18.5 | 0.17 (4) | 10.00 | | | | |
| E- D | 0 / 0 | -18.5 | -18.5 | 0.26 (4) | 10.00 | | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 819 (0.10")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.26/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90 )
JSI METAL= 0.04 (B) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311480

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C25A | 1 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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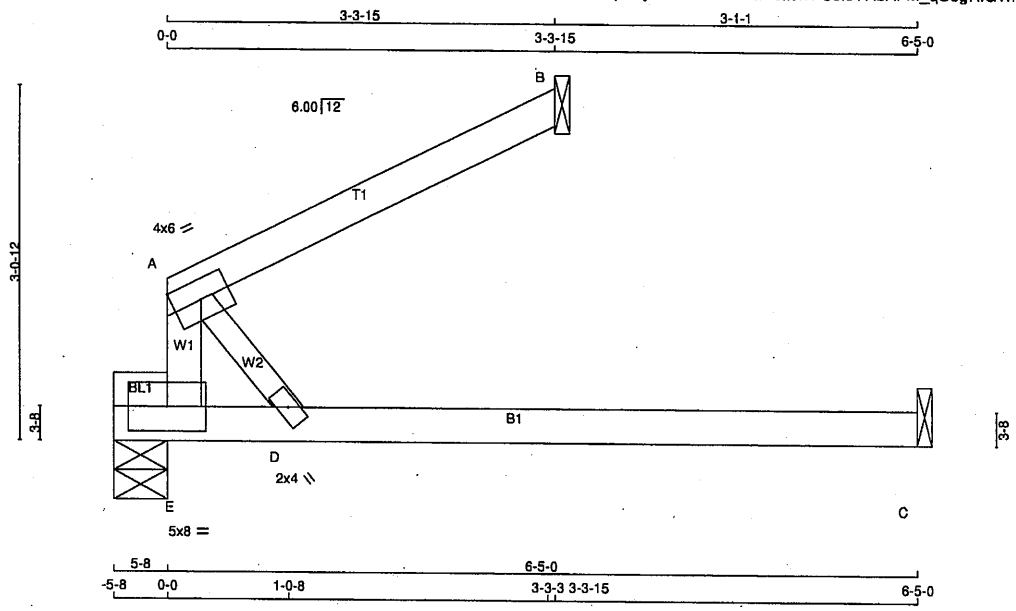


TOTAL WEIGHT = 15 lb

[M][F]

**LUMBER**

N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - A | 2x4 | DRY | No.2 | SPF |
| A - B | 2x4 | DRY | No.2 | SPF |
| E - C | 2x4 | DRY | No.2 | SPF |
| BEARING BLOCKS | | | | |
| BL1 | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN Y | X |
|---|---|---|---|---|---|
| A | TMVW-t | MT20 | 4.0 | 6.0 | Edge |
| D | BMW+w | MT20 | 2.0 | 4.0 | |
| E | BMVK1m | MT20 | 5.0 | 8.0 | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 214 | 0 | 214 | 0 | 0 | 5-8 | 5-0 |
| B | 153 | 0 | 153 | 0 | 0 | 1-8 | 1-8 |
| C | 54 | 0 | 64 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 154 | 87 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 67 / 0 | 0 / 0 |
| B | 105 | 85 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 20 / 0 | 0 / 0 |
| C | 44 | 0 / -2 | 0 / 0 | 0 / 0 | 0 / 0 | 46 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| E- A | -179 / 0 | 0.0 | 0.0 | 0.02 (1) | | 7.81 | A- D | 0 / 34 | 0.01 (1) |
| A- B | 0 / 0 | -91.8 | -91.8 | 0.17 (1) | | 10.00 | | | |
| E- D | -24 / 0 | -18.5 | -18.5 | 0.07 (4) | | 6.25 | | | |
| D- C | 0 / 0 | -18.5 | -18.5 | 0.21 (4) | | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | |
|---|---|---|---|
| TOP CH. | LL = | 25.6 | PSF |
| | DL = | 6.0 | PSF |
| BOT CH. | LL = | 0.0 | PSF |
| | DL = | 7.4 | PSF |
| TOTAL LOAD | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.21/1.00 (C-D:4) , WB=0.01/1.00 (A-D:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (A) (INPUT = 0.90 )
JSI METAL= 0.03 (A) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311481

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C26 | 3 | 1 | GREENPARK HOMES<br>TRUSS DESC. | |

Tamarack Roof Truss, Burlington

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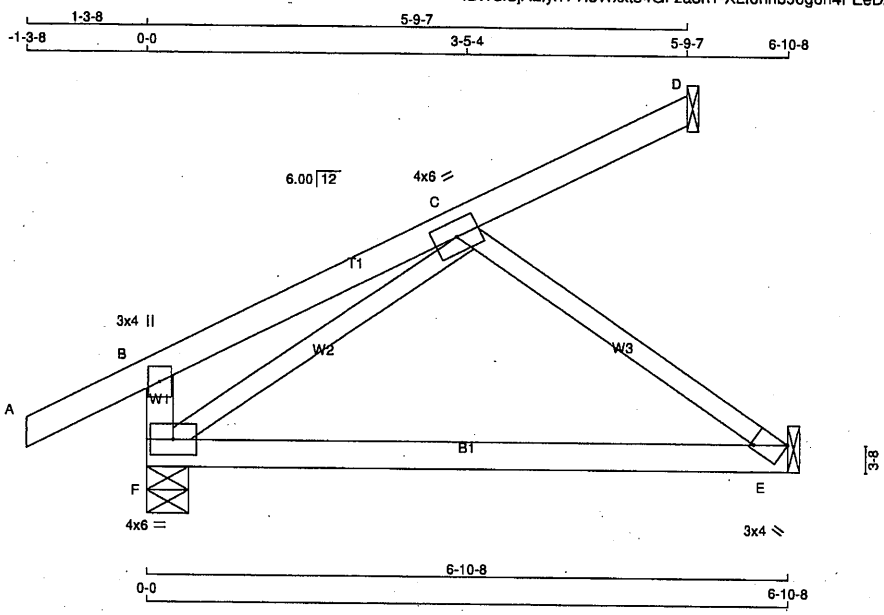


TOTAL WEIGHT = 3 X 24 = 73 lb
[M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| F - B | 2x4 | DRY | No.2 | SPF |
| A - D | 2x4 | DRY | No.2 | SPF |
| F - E | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| B | TMV+p | MT20 | 3.0 | 4.0 | | |
| C | TMWW-t | MT20 | 4.0 | 6.0 | | |
| E | BMW1+w | MT20 | 3.0 | 4.0 | 3.50 | Edge |
| F | BMWW1-t | MT20 | 4.0 | 6.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| F | 478 | 0 | 478 | 0 | 0 | 5-8 | 1-8 |
| D | 73 | 0 | 73 | 0 | 0 | 1-8 | 1-8 |
| E | 228 | 0 | 228 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| F | 336 | 232 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 104 / 0 | 0 / 0 |
| D | 50 | 41 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 10 / 0 | 0 / 0 |
| E | 164 | 93 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 71 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC1 MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|
| F- B | -250 / 0 | 0.0 | 0.0 | 0.03 (1) | 7.81 | F- C | -276 / 0 | 0.08 (1) |
| A- B | 0 / 28 | -91.8 | -91.8 | 0.12 (1) | 10.00 | C- E | -276 / 0 | 0.09 (1) |
| B- C | 0 / 13 | -91.8 | -91.8 | 0.13 (1) | 10.00 | | | |
| C- D | -17 / 0 | -91.8 | -91.8 | 0.10 (1) | 6.25 | | | |
| F- E | 0 / 220 | -18.5 | -18.5 | 0.25 (4) | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 888 (0.09")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.25/1.00 (E-F:4) , WB=0.09/1.00 (C-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90 )
JSI METAL= 0.09 (E) (INPUT = 0.95 )

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311482

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

| JOB NAME | TRUSS NAME | QUANTITY | PLY | JOB DESC. GREENPARK HOMES | DRWG NO. |
|---|---|---|---|---|---|
| 432489 | C26A | 1 | 1 | TRUSS DESC. | |

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:33 2023 Page 1
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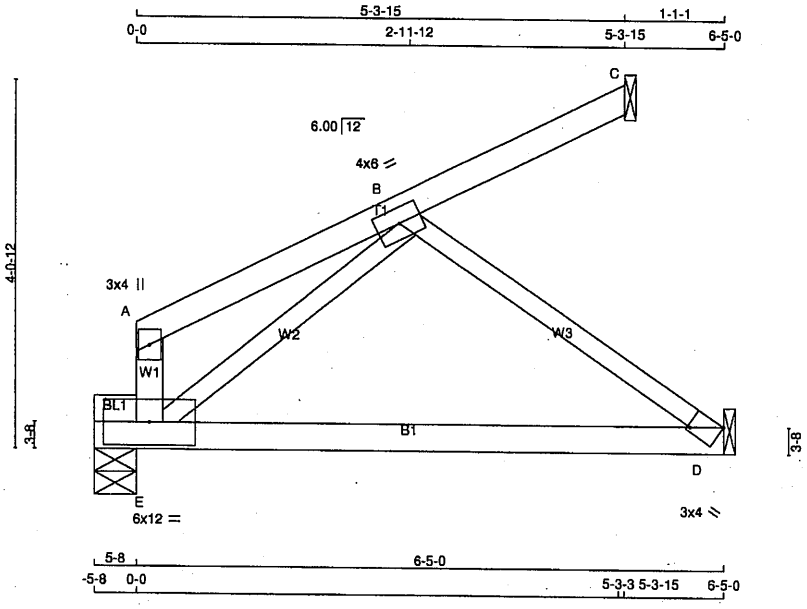



TOTAL WEIGHT = 23 lb
[M][F]

**LUMBER**
N. L. G. A. RULES

| CHORDS | SIZE | | LUMBER | DESCR. |
|---|---|---|---|---|
| E - A | 2x4 | DRY | No.2 | SPF |
| A - C | 2x4 | DRY | No.2 | SPF |
| E - D | 2x4 | DRY | No.2 | SPF |
| BEARING BLOCKS | | | | |
| BL1 | 2x4 | DRY | No.2 | SPF |
| ALL WEBS | 2x3 | DRY | No.2 | SPF |

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

| JT | TYPE | PLATES | W | LEN | Y | X |
|---|---|---|---|---|---|---|
| A | TMV+p | MT20 | 3.0 | 4.0 | | |
| B | TMWW-t | MT20 | 4.0 | 6.0 | | |
| D | BMW1+w | MT20 | 3.0 | 4.0 | 3.50 | Edge |
| E | BMVWK1-t | MT20 | 6.0 | 12.0 | | |

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**NOTES-** (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

| JT | FACTORED GROSS REACTION VERT | HORZ | MAXIMUM FACTORED GROSS REACTION DOWN | HORZ | UPLIFT | INPUT BRG IN-SX | REQRD BRG IN-SX |
|---|---|---|---|---|---|---|---|
| E | 331 | 0 | 331 | 0 | 0 | 5-8 | 5-0 |
| C | 81 | 0 | 81 | 0 | 0 | 1-8 | 1-8 |
| D | 190 | 0 | 190 | 0 | 0 | 1-8 | 1-8 |

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

**UNFACTORED REACTIONS**

| JT | 1ST LCASE COMBINED | MAX./MIN. COMPONENT REACTIONS SNOW | LIVE | PERM.LIVE | WIND | DEAD | SOIL |
|---|---|---|---|---|---|---|---|
| E | 234 | 153 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 81 / 0 | 0 / 0 |
| C | 56 | 45 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 11 / 0 | 0 / 0 |
| D | 137 | 74 / 0 | 0 / 0 | 0 / 0 | 0 / 0 | 63 / 0 | 0 / 0 |

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

**BRACING**
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**
TOTAL LOAD CASES: (4)

| CHORDS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | FACTORED VERT. LOAD (PLF) FROM | TO | LC | MAX CSI (LC) | MAX. UNBRAC LENGTH | WEBS MEMB. FR-TO | MAX. FACTORED FORCE (LBS) | MAX CSI (LC) |
|---|---|---|---|---|---|---|---|---|---|
| E- A | -100 / 0 | 0.0 | 0.0 | 0.01 (1) | | 7.81 | E- B | -258 / 0 | 0.06 (1) |
| A- B | 0 / 27 | -91.8 | -91.8 | 0.11 (1) | | 10.00 | B- D | -221 / 0 | 0.07 (1) |
| B- C | -14 / 0 | -91.8 | -91.8 | 0.08 (1) | | 6.25 | | | |
| E- D | 0 / 176 | -18.5 | -18.5 | 0.21 (4) | | 10.00 | | | |

**DESIGN CRITERIA**

SPECIFIED LOADS:

| | | | | |
|---|---|---|---|---|
| TOP CH. | LL | = | 25.6 | PSF |
| | DL | = | 6.0 | PSF |
| BOT CH. | LL | = | 0.0 | PSF |
| | DL | = | 7.4 | PSF |
| TOTAL LOAD | | = | 39.0 | PSF |

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.21/1.00 (D-E:4) , WB=0.07/1.00 (B-D:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

| PLATE | GRIP(DRY) (PSI) MAX | MIN | SHEAR (PLI) MAX | MIN | SECTION (PLI) MAX | MIN |
|---|---|---|---|---|---|---|
| MT20 | 650 | 371 | 1747 | 788 | 1987 | 1873 |

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (D) (INPUT = 0.90 )
JSI METAL= 0.07 (D) (INPUT = 0.95 )


Structural component only
DWG# T-2311483

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

LICENSED PROFESSIONAL ENGINEER
06/11/18
H.J.G. ALVES
PROVINCE OF ONTARIO

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1800218

Feb 09, 2018

MiTek®

# STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

## TOE-NAIL CAPACITY DETAILS

### LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

| | | | SPF | D. FIR | SPF | D. FIR |
|---|---|---|---|---|---|---|
| COMMON WIRE | 3.00 | 0.144 | 122 | 139 | 30 | 42 |
| | 3.25 | 0.144 | 127 | 144 | 32 | 45 |
| | 3.50 | 0.160 | 152 | 173 | 38 | 52 |
| COMMON SPIRAL | 3.00 | 0.122 | 96 | 108 | 26 | 36 |
| | 3.25 | 0.122 | 97 | 108 | 28 | 40 |
| | 3.50 | 0.152 | 142 | 161 | 36 | 50 |
| 3.25" Gun nail | 3.25 | 0.120 | 94 | 105 | 28 | 39 |

**Note:** If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

| Nail type: | Common wire | Common spiral | Common wire | Common spiral | Gun Nail |
|---|---|---|---|---|---|
| Diameter (in.) | 0.160 | 0.152 | 0.144 | 0.122 | 0.120 |
| Length (in.) | 3.50 | 3.50 | 3.00 | 3.00 | 3.25 |
| 2x4 SPF | 2 | 2 | 3 | 3 | 3 |
| 2x6 SPF | 4 | 4 | 4 | 5 | 5 |
| 2x4 D. FIR | 2 | 2 | 2 | 2 | 2 |
| 2x6 D. FIR | 3 | 3 | 3 | 4 | 4 |

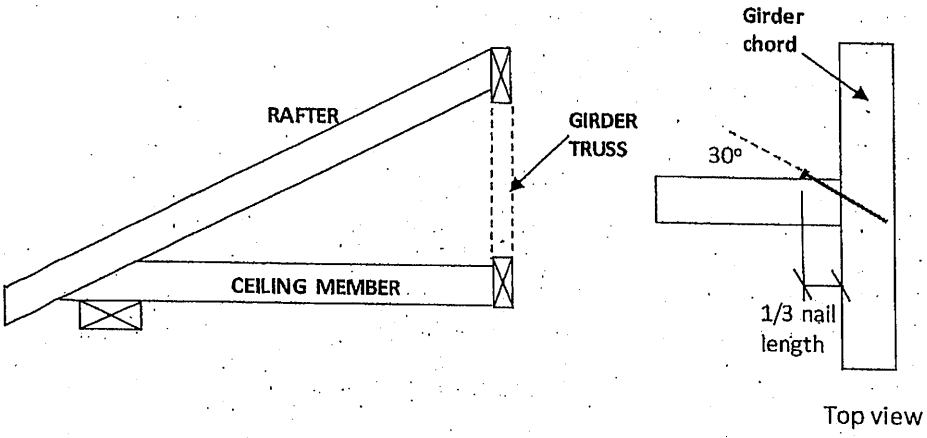



Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

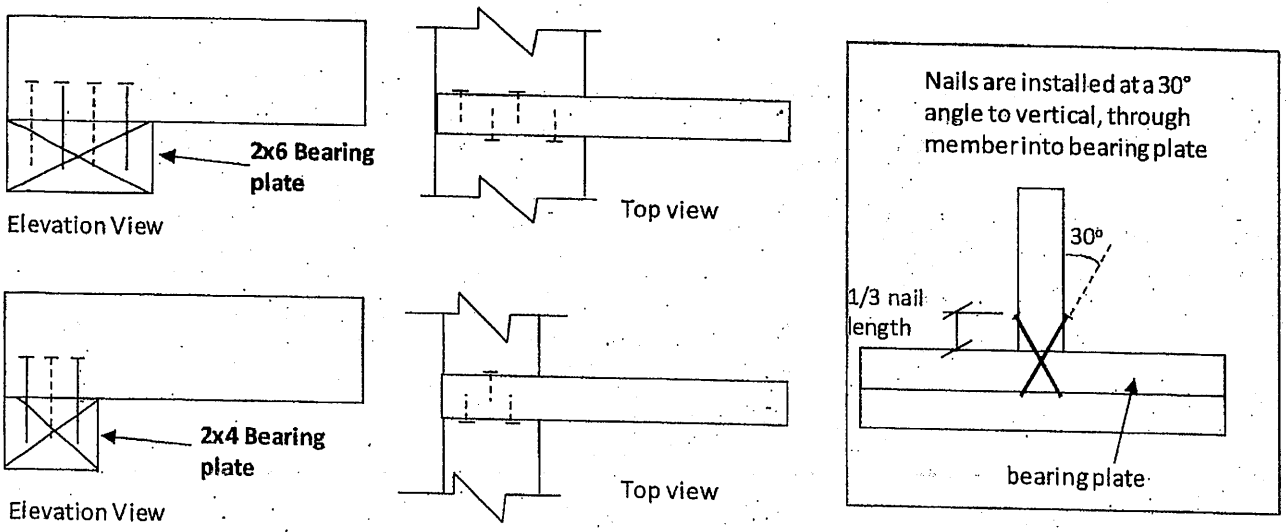
MiTek®

# STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

## TOE-NAIL CAPACITY DETAILS

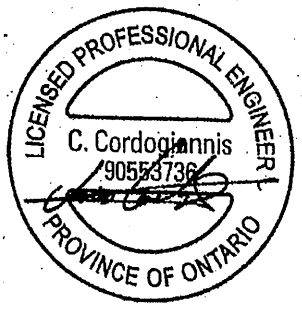
**Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift**




**NOTES:**

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 ($K_D$ factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485







December 21, 2020

# LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie

## Standard and Double-Shear Joist Hangers



VALUE ENGINEERED

***This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.***

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

**Material:** See table on pp. 217–218.

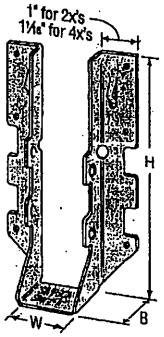
**Finish:** Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

**Installation:**

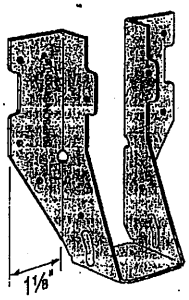
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

**Options:**

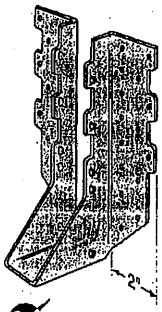
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



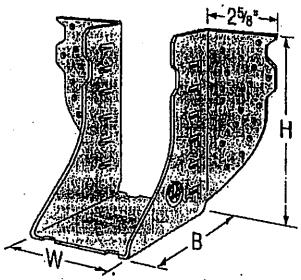

LUS28



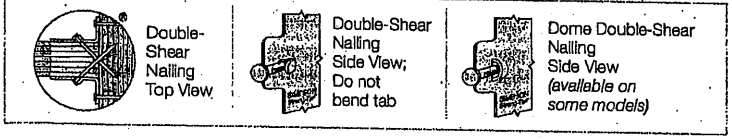

LU26L

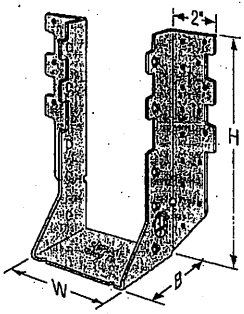



HUS210
*(HUS26, HUS28, and HHUS similar)*

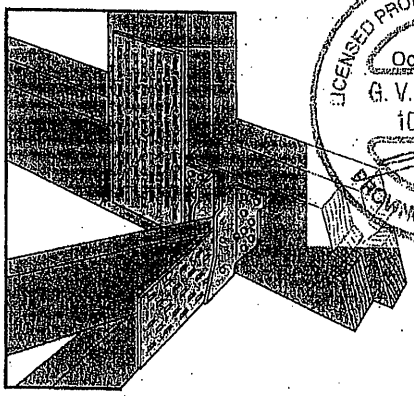



HGUS28-2

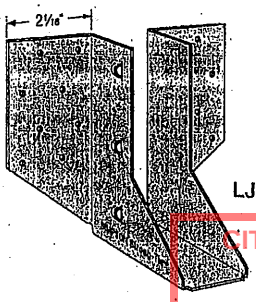



HHUS210-2



**Typical HUS26 Installation with Reduced Heel Height**
*(Truss Designer to provide fastener quantity for connecting multiple members together)*




LJS26DS

TECHNICAL BULLETIN

# LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

**Material:** 18 gauge

**Finish:** G90 galvanized

**Design:**

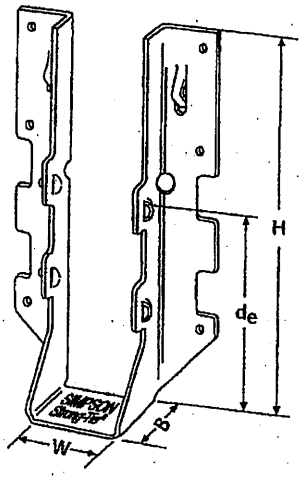
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

**Installation:**

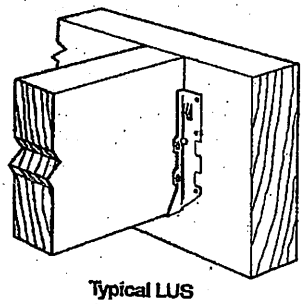
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

**Options:**

- These hangers cannot be modified

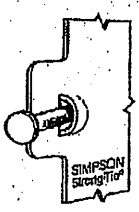


LUS28


Typical LUS Installation

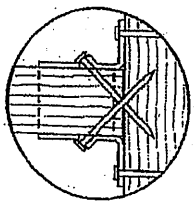
| Model No. | Ga. | Dimensions (in.) | | | | Fasteners | | Factored Resistance (lb.) | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | | | D.Fir-L | | S-P-F | |
| | | W | H | B | $d_e$[1] | Face | Joist | Uplift ($K_D$=1.15) | Normal ($K_D$=1.00) | Uplift ($K_D$=1.15) | Normal ($K_D$=1.00) |
| LUS24 | 18 | 1⁹⁄₁₆ | 3⅛ | 1¾ | 1 ¹⁵⁄₁₆ | (4) 10d | (2) 10d | 710 | 1630 | 645 | 1155 |
| LUS24-2 | 18 | 3⅛ | 3⅛ | 2 | 1 ¹⁵⁄₁₆ | (4) 16d | (2) 16d | 835 | 2020 | 590 | 1435 |
| LUS26 | 18 | 1⁹⁄₁₆ | 4¾ | 1¾ | 3⅝ | (4) 10d | (4) 10d | 1420 | 2170 | 1290 | 1630 |
| LUS26-2 | 18 | 3⅛ | 4⅞ | 2 | 4 | (4) 16d | (4) 16d | 1720 | 2595 | 1545 | 1920 |
| LUS26-3 | 18 | 4⅝ | 4¾₁₆ | 2 | 3¼ | (4) 16d | (4) 16d | 1720 | 2595 | 1545 | 2340 |
| LUS28 | 18 | 1⁹⁄₁₆ | 6⅝ | 1¾ | 3¾ | (6) 10d | (6) 10d | 1420 | 2520 | 1290 | 1790 |
| LUS28-2 | 18 | 3⅛ | 7 | 2 | 4 | (6) 16d | (4) 16d | 1720 | 3325 | 1545 | 2575 |
| LUS28-3 | 18 | 4⅝ | 6¼ | 2 | 3¼ | (6) 16d | (4) 16d | 1720 | 3325 | 1545 | 2375 |
| LUS210 | 18 | 1⁹⁄₁₆ | 7 ¹³⁄₁₆ | 1¾ | 3⅞ | (8) 10d | (4) 10d | 1420 | 2785 | 1290 | 2210 |
| LUS210-2 | 18 | 3⅛ | 9 | 2 | 6 | (8) 16d | (6) 16d | 2580 | 4500 | 2320 | 3195 |
| LUS210-3 | 18 | 4⅝ | 8¾₁₆ | 2 | 5¼ | (8) 16d | (6) 16d | 2580 | 3345 | 2320 | 2375 |

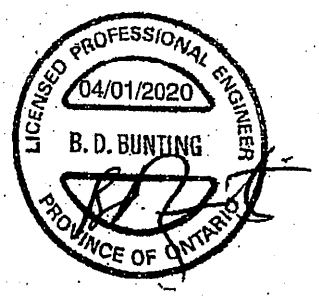
1. $d_e$ is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.




LIMIT STATES DESIGN

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TECHNICAL BULLETIN

# HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

**Material:** See table

**Finish:** G90 galvanized

**Design:**

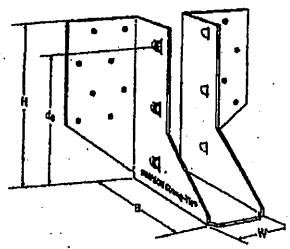
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

**Installation:**

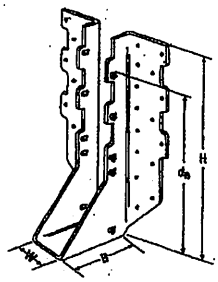
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

**Options:**

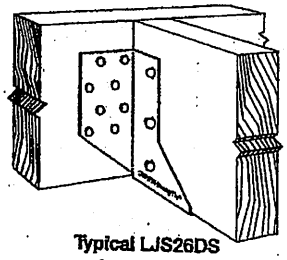
- See current catalogue for options



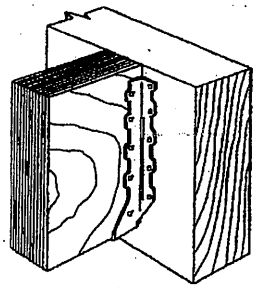
LJS26DS



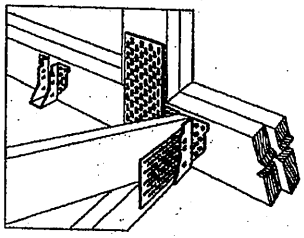
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



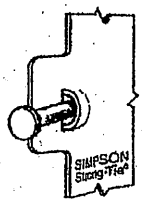
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

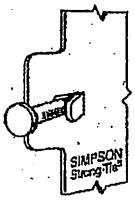
| Model No. | Ga. | Dimensions (in.) | | | | Fasteners | | Factored Resistance (lb.) | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | | | D.Fir-L | | S-P-F | |
| | | W | H | B | $d_e$[1] | Face | Joist | Uplift ($K_D$=1.15) | Normal ($K_D$=1.00) | Uplift ($K_D$=1.15) | Normal ($K_D$=1.00) |
| | | | | | | | | lb. | lb. | lb. | lb. |
| LJS26DS | 18 | 1⁹⁄₁₆ | 5 | 3½ | 4⅝ | (16) 16d | (6) 16d | 2055 | 4285 | 1460 | 4115 |
| HUS26 | 16 | 1⅝ | 5⅜ | 3 | 3¹⁵⁄₁₆ | (14) 16d | (6) 16d | 2705 | 4940 | 2065 | 3875 |
| HUS28 | 16 | 1⅝ | 7³⁄₃₂ | 3 | 6³⁄₃₂ | (22) 16d | (8) 16d | 3605 | 5365 | 2675 | 4345 |
| HUS210 | 16 | 1⅝ | 9³⁄₃₂ | 3 | 7³¹⁄₃₂ | (30) 16d | (10) 16d | 4505 | 5795 | 4010 | 4740 |
| HUS1.81/10 | 16 | 1¹³⁄₁₆ | 9 | 3 | 8 | (30) 16d | (10) 16d | 4505 | 6450 | 4010 | 5200 |

1. $d_e$ is the distance from the seat of the hanger to the highest joist nail.

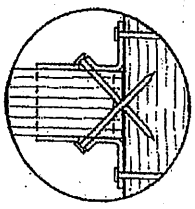


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

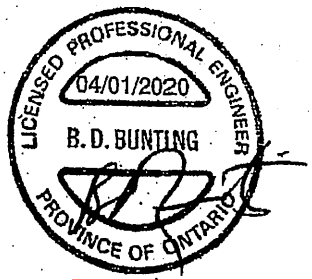
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.




LIMIT STATES DESIGN

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TECHNICAL BULLETIN

# HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

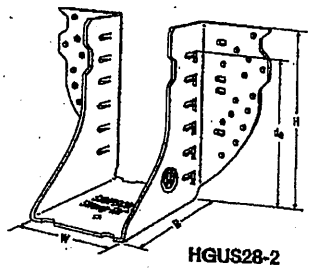
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

**Material:** 12 gauge

**Finish:** G90 galvanized

**Design:**

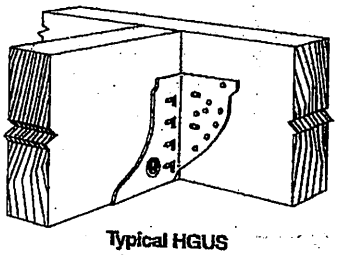
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

**Installation:**

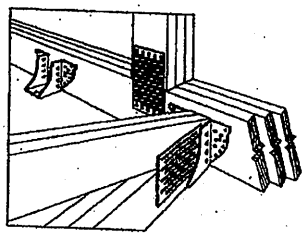
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



**Typical HGUS Installation**

**Options:**

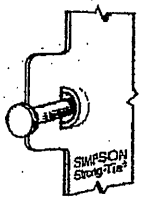
- See current catalogue for options



**Typical HGUS Installation** (Truss Designer to provide fastener quantity for connecting multiple members together)

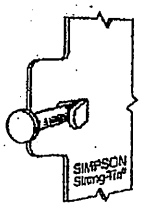
| Model No. | Ga. | Dimensions (in.) | | | | Fasteners | | Factored Resistance (lb.) | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | | | D.Fir-L | | S-P-F | |
| | | W | H | B | $d_s$[1] | Face | Joist | Uplift ($K_D$=1.15) | Normal ($K_D$=1.00) | Uplift ($K_D$=1.15) | Normal ($K_D$=1.00) |
| HGUS26 | 12 | 1⅝ | 5⅜ | 5 | 4⅝₃₂ | (20) 16d | (8) 16d | 2685 | 6625 | 2685 | 5700 |
| HGUS26-2 | 12 | 3⅝₁₆ | 5⅞₁₆ | 4 | 4⅛ | (20) 16d | (8) 16d | 4385 | 8950 | 3100 | 6355 |
| HGUS26-3 | 12 | 4⅝₁₆ | 5½ | 4 | 4⅛ | (20) 16d | (8) 16d | 4385 | 8950 | 3100 | 6355 |
| HGUS26-4 | 12 | 6⅝₁₆ | 5⅞₁₆ | 4 | 4⅛ | (20) 16d | (8) 16d | 4385 | 8950 | 3100 | 6355 |
| HGUS28 | 12 | 1⅝ | 7⅛ | 5 | 6⅛ | (36) 16d | (12) 16d | 3310 | 7675 | 3100 | 6900 |
| HGUS28-2 | 12 | 3⅝₁₆ | 7⅜₁₆ | 4 | 6⅛ | (36) 16d | (12) 16d | 6070 | 12980 | 4310 | 9215 |
| HGUS28-3 | 12 | 4⅝₁₆ | 7¼ | 4 | 6⅛ | (36) 16d | (12) 16d | 6070 | 12980 | 4310 | 9215 |
| HGUS28-4 | 12 | 6⅝₁₆ | 7⅜₁₆ | 4 | 6⅛ | (36) 16d | (12) 16d | 6070 | 12980 | 4310 | 9215 |
| HGUS210 | 12 | 1⅝ | 9⅛ | 5 | 7⅞ | (46) 16d | (16) 16d | 3535 | 11070 | 2510 | 8090 |
| HGUS210-2 | 12 | 3⅝₁₆ | 9⅜₁₆ | 4 | 8⅛ | (46) 16d | (16) 16d | 6840 | 14015 | 4855 | 10270 |
| HGUS210-3 | 12 | 4⅝₁₆ | 9¼ | 4 | 8⅜ | (46) 16d | (16) 16d | 6840 | 14645 | 4855 | 10400 |
| HGUS210-4 | 12 | 6⅝₁₆ | 9⅜₁₆ | 4 | 8⅛ | (46) 16d | (16) 16d | 6840 | 14645 | 4855 | 10400 |
| HGUS212-4 | 12 | 6⅝₁₆ | 10⅝ | 4 | 10⅛ | (56) 16d | (20) 16d | 7640 | 14995 | 5425 | 10645 |
| HGUS214-4 | 12 | 6⅝₁₆ | 12⅝ | 4 | 11⅞ | (66) 16d | (22) 16d | 10130 | 16400 | 7195 | 11645 |

1. $d_s$ is the distance from the seat of the hanger to the highest joist nail.

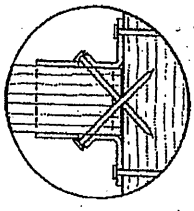



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

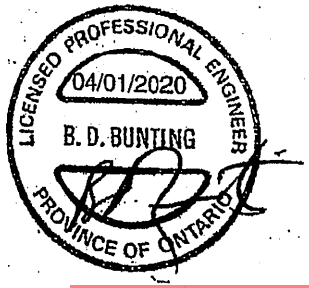
U.S. Patent 5,603,680




Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.




LIMIT STATES DESIGN

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TECHNICAL BULLETIN

# HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

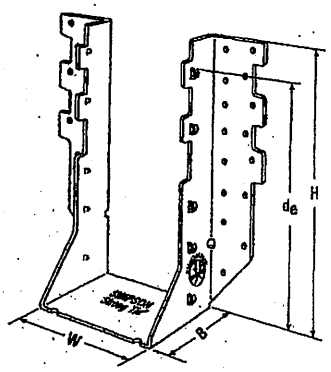
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

**Material:** 14 gauge

**Finish:** G90 galvanized

**Design:**

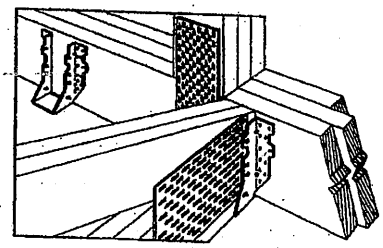
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.




HHUS410

**Installation:**

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



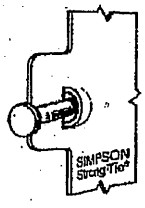
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

**Options:**

- See current catalogue for options

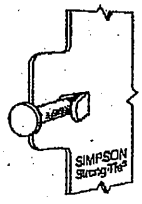
| Model No. | Ga. | Dimensions (in.) | | | | Fasteners | | Factored Resistance (lb.) | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | | | | D.Fir-L | | S-P-F | |
| | | W | H | B | $d_e$[1] | Face | Joist | Uplift ($K_D$=1.15) | Normal ($K_D$=1.00) | Uplift ($K_D$=1.15) | Normal ($K_D$=1.00) |
| HHUS26-2 | 14 | 3 5/16 | 5 13/16 | 3 | 3 15/16 | (14) 16d | (6) 16d | 2850 | 7335 | 2065 | 5205 |
| HHUS28-2 | 14 | 3 5/16 | 7 7/32 | 3 | 6 5/32 | (22) 16d | (8) 16d | 3765 | 8940 | 2675 | 6345 |
| HHUS210-2 | 14 | 3 5/16 | 9 3/32 | 3 | 8 | (30) 16d | (10) 16d | 4670 | 9660 | 4235 | 7000 |
| HHUS210-3 | 14 | 4 11/16 | 9 | 3 | 7 15/16 | (30) 16d | (10) 16d | 4670 | 9670 | 4235 | 6865 |
| HHUS210-4 | 14 | 6 1/8 | 8 29/32 | 3 | 7 27/32 | (30) 16d | (10) 16d | 4670 | 10155 | 4235 | 7210 |
| HHUS46 | 14 | 3 5/8 | 5 13/32 | 3 | 3 15/16 | (14) 16d | (6) 16d | 2540 | 7335 | 2065 | 5205 |
| HHUS48 | 14 | 3 5/8 | 7 1/8 | 3 | 6 1/8 | (22) 16d | (8) 16d | 3765 | 8940 | 2675 | 6345 |
| HHUS410 | 14 | 3 5/8 | 9 | 3 | 8 | (30) 16d | (10) 16d | 4670 | 9855 | 4235 | 7000 |
| HHUS5.50/10 | 14 | 5 1/2 | 9 | 3 | 8 | (30) 16d | (10) 16d | 4670 | 10155 | 4235 | 7210 |
| HHUS7.25/10 | 14 | 7 1/4 | 9 | 3 5/16 | 7 29/32 | (30) 16d | (10) 16d | 4670 | 10155 | 3370 | 7210 |

1. $d_e$ is the distance from the seat of the hanger to the highest joist nail.

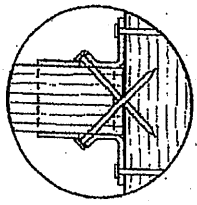



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

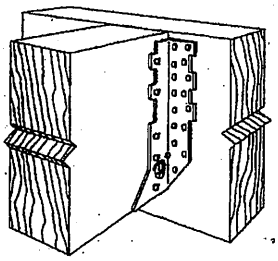
U.S. Patent 5,603,580



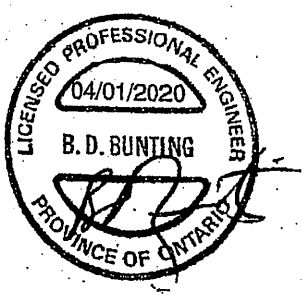

Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HHUS Installation




LIMIT STATES DESIGN

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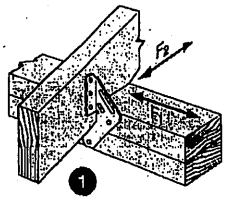
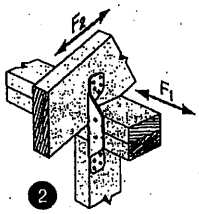
# H/TSP

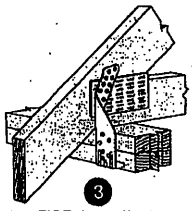
## Seismic and Hurricane Ties (cont.)



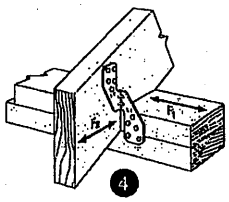

1 H1 Installation



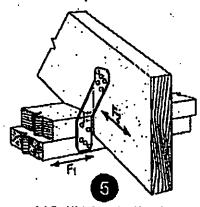

2 H2A Installation



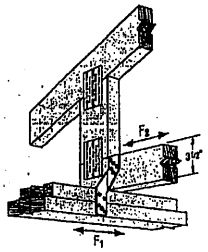

3 TSP Installation



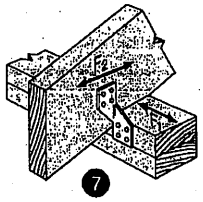

4 H2.5A Installation
*(Nails into both top plates)*



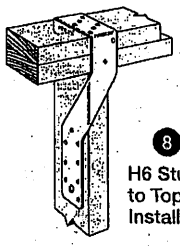

5 H2.5T Installation
*(Nails into both top plates)*



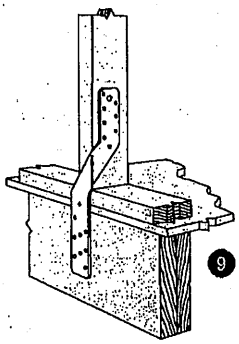

6 H2.5T Installation



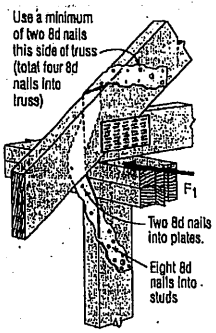

7 H3 Installation
*(Nails into upper top plate)*



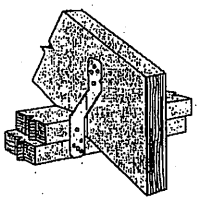
8 H6 Stud to Top Plate Installation



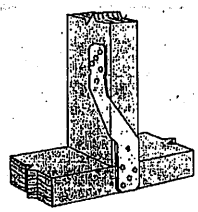
9 H6 Stud to Band Joist Installation



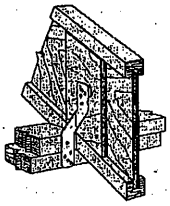

10 H7Z Installation



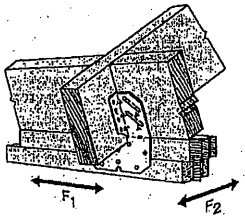
11 H8 Attaching Rafter to Double Top Plates



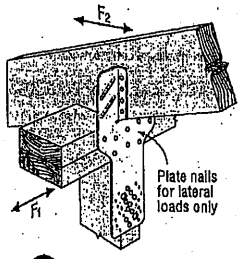
12 H8 attaching Stud to Sill
*((4) 8d into plate, (5) 8d into stud)*



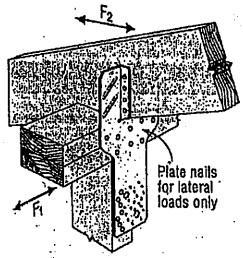
13 H8 attaching I-Joist to Double Top Plates



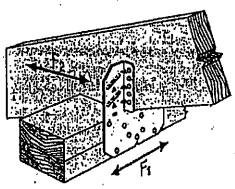

14 H10A Field-Bent Installation




15 H10S Installation



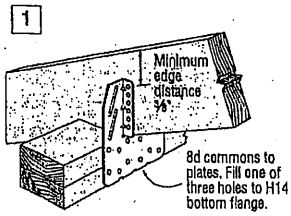

16 H10S Installation with Stud Offset




17 H10A Installation

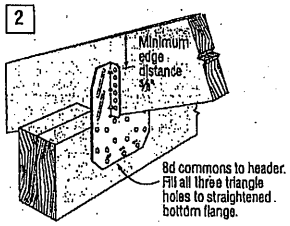
H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

1



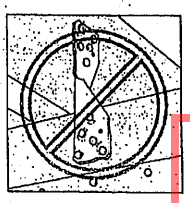

18 H14 Installation to Double Top Plates

2




19 H14 Installation to Double 2x Header

### Avoid a Misinstallation



Do not make new holes or overdrive nails.

SIMPSON Strong-Tie

# H/TSP

## Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

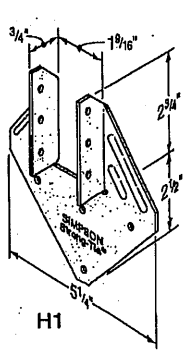
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

**Material:** See table

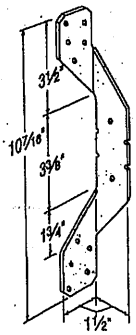
**Finish:** Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp.20–24 or visit **strongtie.com**.

**Installation:**

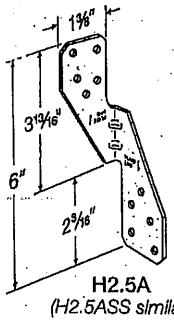
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



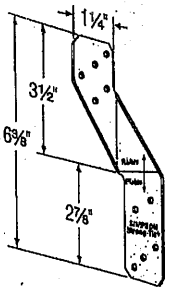
H1



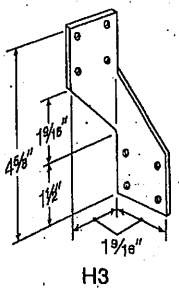
H2A
*(H2ASS similar)*



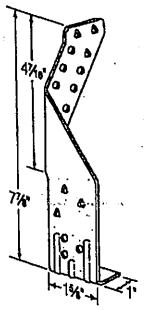
H2.5A
*(H2.5ASS similar)*



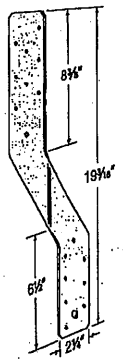
H2.5T



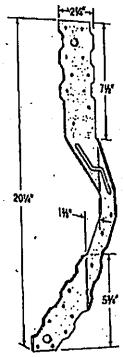
H3



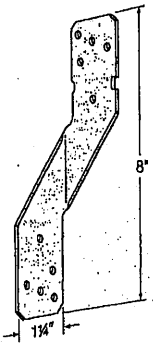
TSP



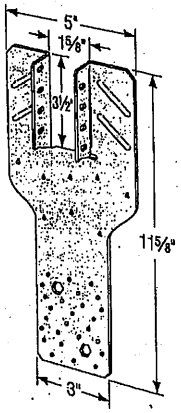
H6



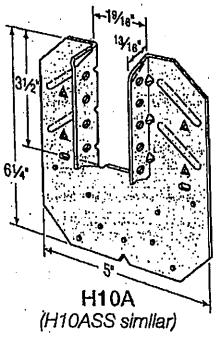
H7Z



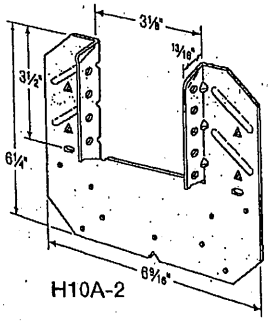
H8



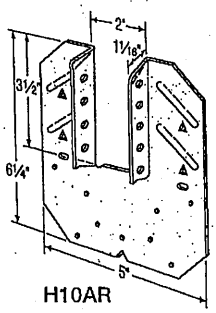
H10S



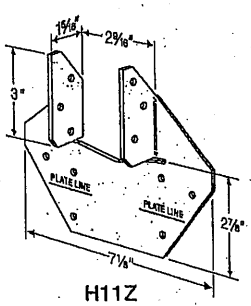
H10A
*(H10ASS similar)*



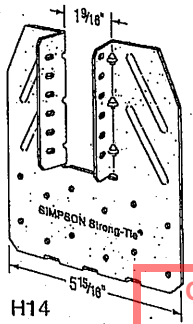
H10A-2



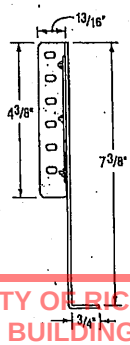
H10AR



H11Z



H14



H14
Profile

TECHNICAL BULLETIN

# H – Seismic and Hurricane Ties

SIMPSON Strong-Tie

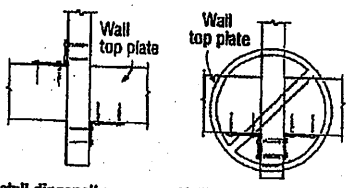
The H connector series provides wind and seismic ties for trusses and rafters.
**Material:** 18 gauge **Finish:** G90 galvanized

**Design:** • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

**Installation:** • Use all specified fasteners
• Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
• H1 can be installed with flanges facing outwards
• Hurricane ties do not replace solid blocking

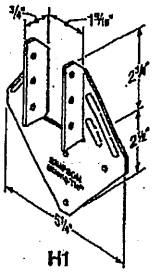
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

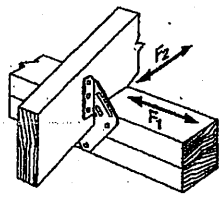
**Hurricane Tie Installations to Achieve Twice the Load (Top View)**

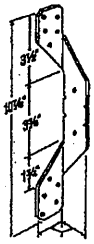


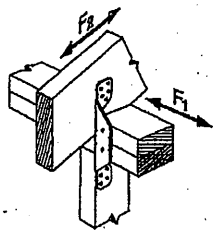

Install diagonally across from each other for minimum 2x truss.

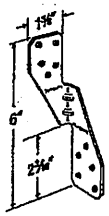
Nailing into both sides of a single ply 2x truss may cause the wood to split.

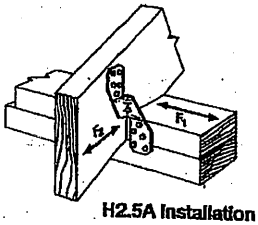

H1

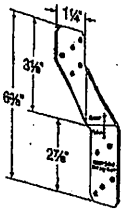

H1 Installation

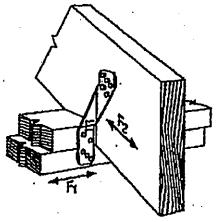

H2A

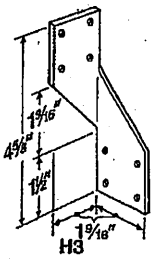

H2A Installation

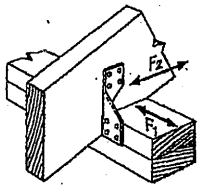

H2.5A

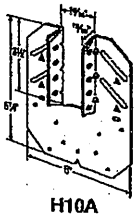

H2.5A Installation

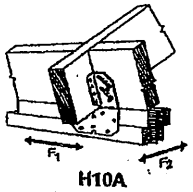

H2.5T


H2.5T Installation (Nails into both top plates)


H3


H3 Installation


H10A


H10A Installation

| Model No. | Ga. | Fasteners | | | Factored Resistance (lb.) | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|
| | | | | | D.Fir-L | | | S-P-F | | |
| | | To Rafter | To Plates | To Studs | Uplift | Normal $F_1$ | Normal $F_2$ | Uplift | Normal $F_1$ | Normal $F_2$ |
| | | | | | ($K_D$=1.15) | | | ($K_D$=1.15) | | |
| H1 | 18 | (6) 8d x 1½" | (4) 8d | — | 740 | 685 | 300 | 680 | 485 | 215 |
| H2A | 18 | (5) 8d x 1½" | (2) 8d x 1½" | (5) 8d x 1½" | 830 | 220 | 75 | 590 | 155 | 55 |
| H2.5A | 18 | (5) 8d | (5) 8d | — | 805 | 160 | 160 | 755 | 160 | 160 |
| H2.5T | 18 | (5) 8d | (5) 8d | — | 835 | 175 | 240 | 740 | 160 | 210 |
| H3 | 18 | (4) 8d | (4) 8d | — | 740 | 180 | 265 | 615 | 125 | 190 |
| H10A | 18 | (9) 10d x 1½" | (9) 10d x 1½" | — | 1735 | 795 | 410 | 1505 | 565 | 290 |

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.



T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

# TC – Truss Connectors

SIMPSON Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

**Material:** 16 gauge

**Finish:** G90 galvanized

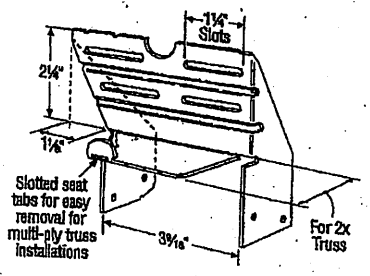
**Design:** Factored resistances are in accordance with CSA 086-14

**Installation:**

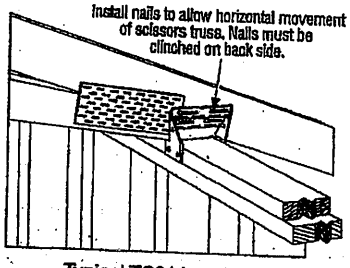
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

**Optional TC Installation:**

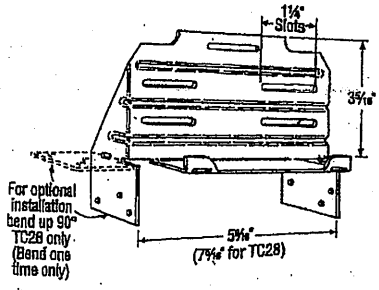
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



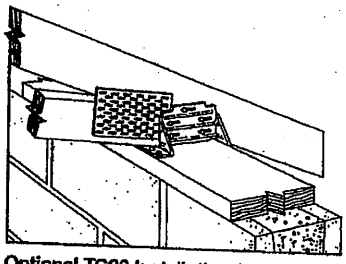

**TC24**
U.S. Patent 4,932,173



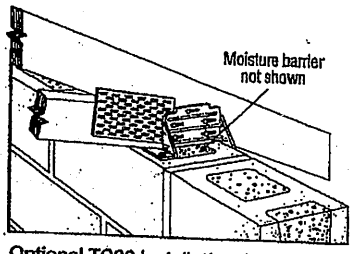

**Typical TC24 Installation**




**TC26**
(TC28 Similar)



**Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer**
(8", 10", 12" Wall Installation Similar)




**Optional TC26 Installation for Grouted Concrete Block using Titen Screws**

| Model No. | Fasteners | | Factored Resistance | |
|---|---|---|---|---|
| | Truss | Wall Plates | D.Fir-L | S-P-F |
| | | | Uplift ($K_D$=1.15) | Uplift ($K_D$=1.15) |
| | | | lb. | lb. |
| TC24 | (4) 10d | (4) 10d | 605 | 430 |
| TC26 | (5) 10d | (6) 10d | 1015 | 720 |
| TC28 | (5) 10d | (6) 10d | 1015 | 720 |

**Optional TC Installation Table**

| Model No. | Fasteners | | Factored Resistance | |
|---|---|---|---|---|
| | Truss | Wall Plates | D.Fir-L | S-P-F |
| | | | Uplift ($K_D$=1.15) | Uplift ($K_D$=1.15) |
| | | | lb. | lb. |
| TC26 | (5) 10d | (6) 10d x 1½" | 810 | 660 |
| | (5) 10d | (6) 10d | 930 | 660 |

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC28 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.



T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

# HTU

SIMPSON Strong-Tie

## Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

### Alternate Installation for (2) 2x4 and (2) 2x6 Headers

| Model No. | Min. Heel Height (in.) | Minimum Header Size | Fasteners | | Factored Resistance | | | |
|---|---|---|---|---|---|---|---|---|
| | | | | | D.Fir-L | | S-P-F | |
| | | | Header | Joist | Uplift ($K_D$ = 1.15) | Normal ($K_D$ = 1.00) | Uplift ($K_D$ = 1.15) | Normal ($K_D$ = 1.00) |
| | | | | | lb. | lb. | lb. | lb. |
| | | | | | kN | kN | kN | kN |
| HTU26 (Min.) | 3⅞ | (2) 2x4 | (10) 16d | (14) 10d x 1½" | 1740 | 3340 | 1235 | 2370 |
| | | | | | 7.74 | 14.86 | 5.49 | 10.54 |
| HTU26 (Max.) | 5½ | (2) 2x4 | (10) 16d | (20) 10d x 1½" | 2470 | 4015 | 1755 | 2850 |
| | | | | | 10.99 | 17.86 | 7.81 | 12.68 |
| HTU28 (Max.) | 3⅞ | (2) 2x6 | (20) 16d | (26) 10d x 1½" | 4150 | 6395 | 2945 | 4540 |
| | | | | | 18.46 | 28.45 | 13.10 | 20.19 |
| HTU210 (Max.) | 7¼ | (2) 2x6 | (20) 16d | (32) 10d x 1½" | 4150 | 6395 | 2946 | 4640 |
| | | | | | 18.46 | 28.45 | 13.10 | 20.19 |

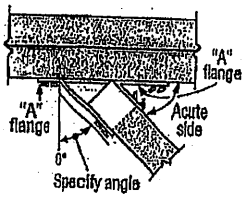
See table footnotes on p. 260.

### Hanger Options

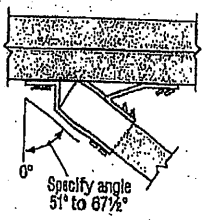
See Hanger Options information on pp. 125–127.

**Skewed Seat**

- Skewable up to 67½°
- Available in single and 2-ply size
- No bevel cut required




Top View HTU Hanger Skewed Right < 51°




Top View HTU Hanger Skewed Right ≥ 51°

### Factored Resistances for Skewed HTU Hangers

| Model No. | Skew Angle (Degrees) | Fasteners | | Factored Resistance | | | |
|---|---|---|---|---|---|---|---|
| | | | | D.Fir-L | | S-P-F | |
| | | Header | Joist | Uplift (KD=1.15) | Normal (KD=1.00) | Uplift (KD=1.15) | Normal (KD=1.00) |
| | | | | lbs | lbs | lbs | lbs |
| | | | | kN | kN | kN | kN |
| HTU26 | < 51 | (20) 16d | (14) 10d x 1½" | 1935 | 4110 | 1300 | 2905 |
| | | | | 8.16 | 18.28 | 5.78 | 12.92 |
| | 51–67½ | (20) 16d | (12) 10d x 1½" | 1350 | 3620 | 955 | 2560 |
| | | | | 6.01 | 16.10 | 4.25 | 11.39 |
| HTU28 | < 51 | (26) 16d | (20) 10d x 1½" | 2810 | 4270 | 1985 | 3030 |
| | | | | 12.50 | 18.99 | 8.83 | 13.48 |
| | 51–67½ | (26) 16d | (17) 10d x 1½" | 2075 | 3930 | 1465 | 2780 |
| | | | | 9.23 | 17.48 | 6.52 | 12.37 |
| HTU210 | < 51 | (32) 16d | (26) 10d x 1½" | 3785 | 4430 | 2675 | 3135 |
| | | | | 16.84 | 19.71 | 11.90 | 13.95 |
| | 51–67½ | (32) 16d | (22) 10d x 1½" | 2795 | 4240 | 1980 | 3000 |
| | | | | 12.43 | 18.86 | 8.81 | 13.35 |
| HTU26-2 | < 51 | (20) 16d | (14) 10d | 2140 | 3715 | 1515 | 2625 |
| | | | | 9.52 | 16.53 | 6.74 | 11.68 |
| | 51–67½ | (20) 16d | (12) 10d | 1610 | 3920 | 1140 | 2785 |
| | | | | 7.18 | 17.44 | 5.07 | 12.39 |
| HTU28-2 | < 51 | (26) 16d | (20) 10d | 3960 | 5425 | 2815 | 3855 |
| | | | | 17.62 | 24.13 | 12.52 | 17.15 |
| | 51–67½ | (26) 16d | (17) 10d | 2385 | 5425 | 1695 | 3855 |
| | | | | 10.61 | 24.13 | 7.54 | 17.15 |
| HTU210-2 | < 51 | (32) 16d | (26) 10d | 5025 | 6890 | 3570 | 4890 |
| | | | | 22.35 | 30.65 | 15.88 | 21.75 |
| | 51–67½ | (36) 16d | (22) 10d | 3145 | 6680 | 2225 | 4745 |
| | | | | 13.99 | 29.72 | 9.90 | 21.10 |

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3½" long, 10d x 1½" = 0.148" dia. x 1½" long, 10d = 0.148" dia. x 3" long. See pp. 27–28 for other nail sizes and information.

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

# TECH-NOTES

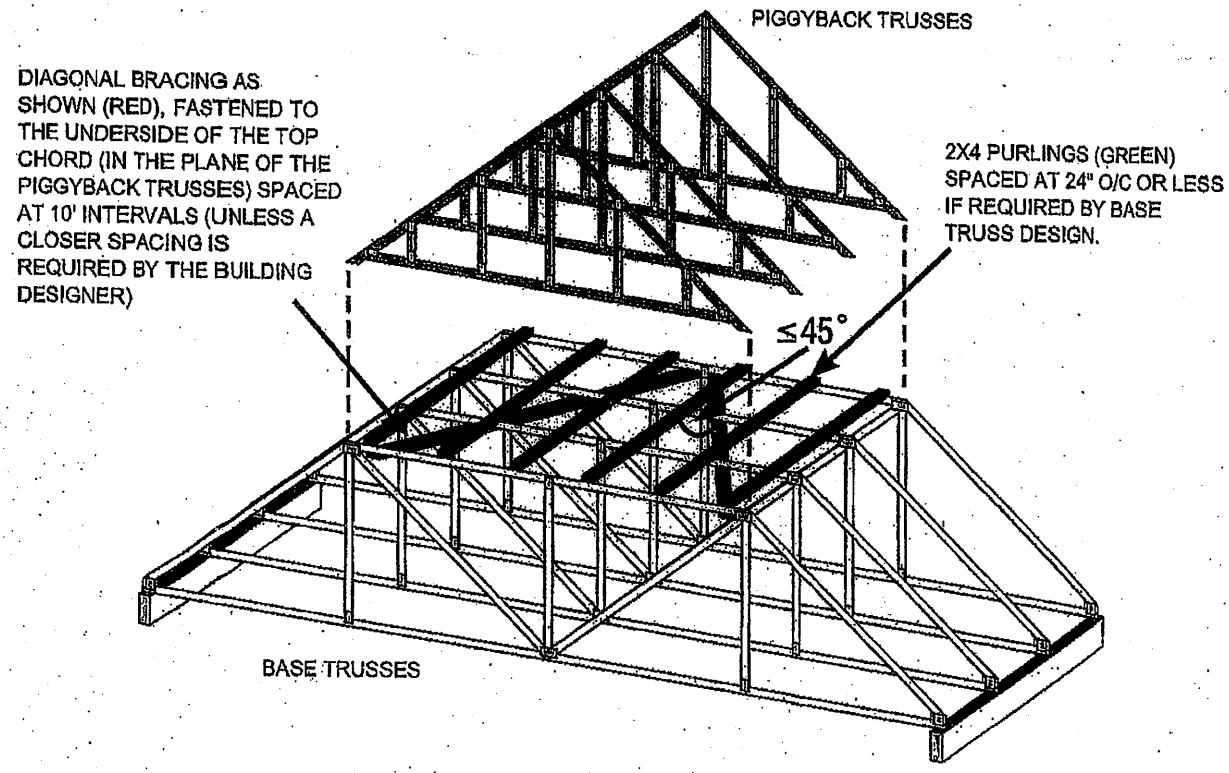
## TN 15-001
## Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:




NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

*SKETCH FROM BCSI-CANADA 2013*

# HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

## Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

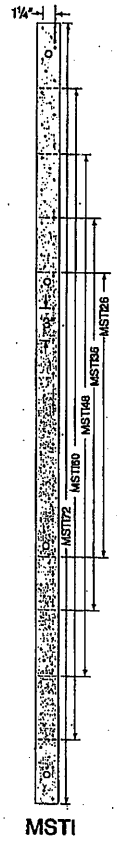
MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

**Finish:** Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

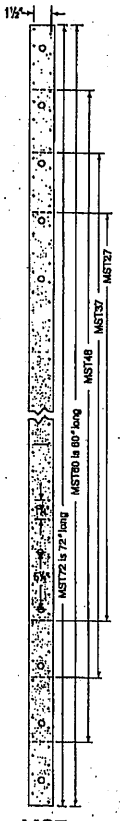
**Installation:** Use all specified fasteners; see General Notes

**Options:** Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

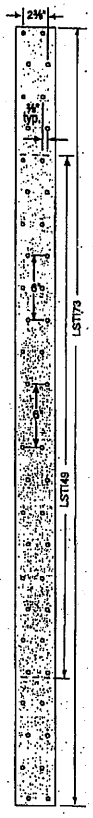




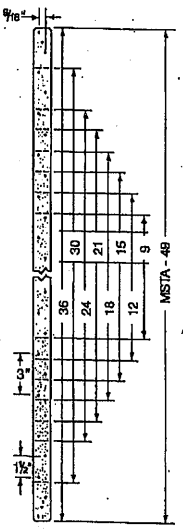
MSTI



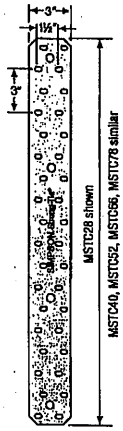
MST



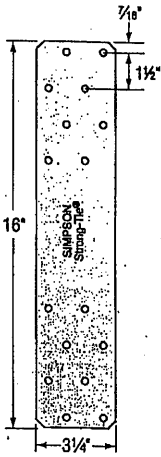
LSTI



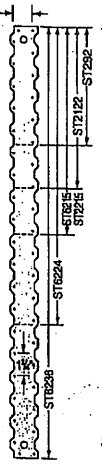
LSTA and MSTA
*(Pilot holes not shown)*



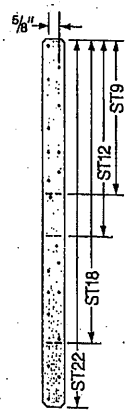
MSTC



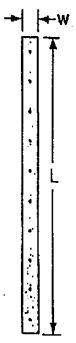
HRS416Z



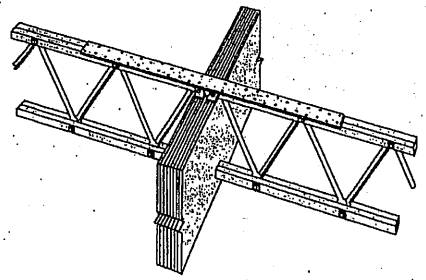
ST

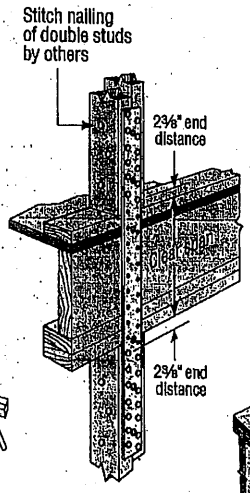


ST9, ST12, ST18, ST22

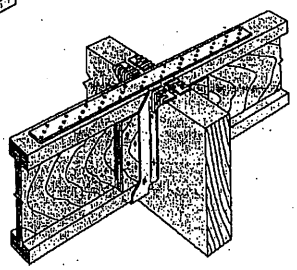


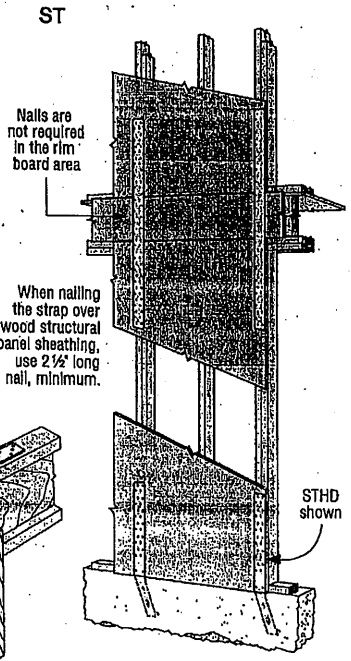
ST2115


Typical LSTI Installation



Floor-to-Floor Tie Installation Showing a Clear Span


Typical MSTI Installation
*(MIT hanger shown)*
LSTI similar



Typical Detail with Strap Installed over Wood Structural Panel Sheathing