ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

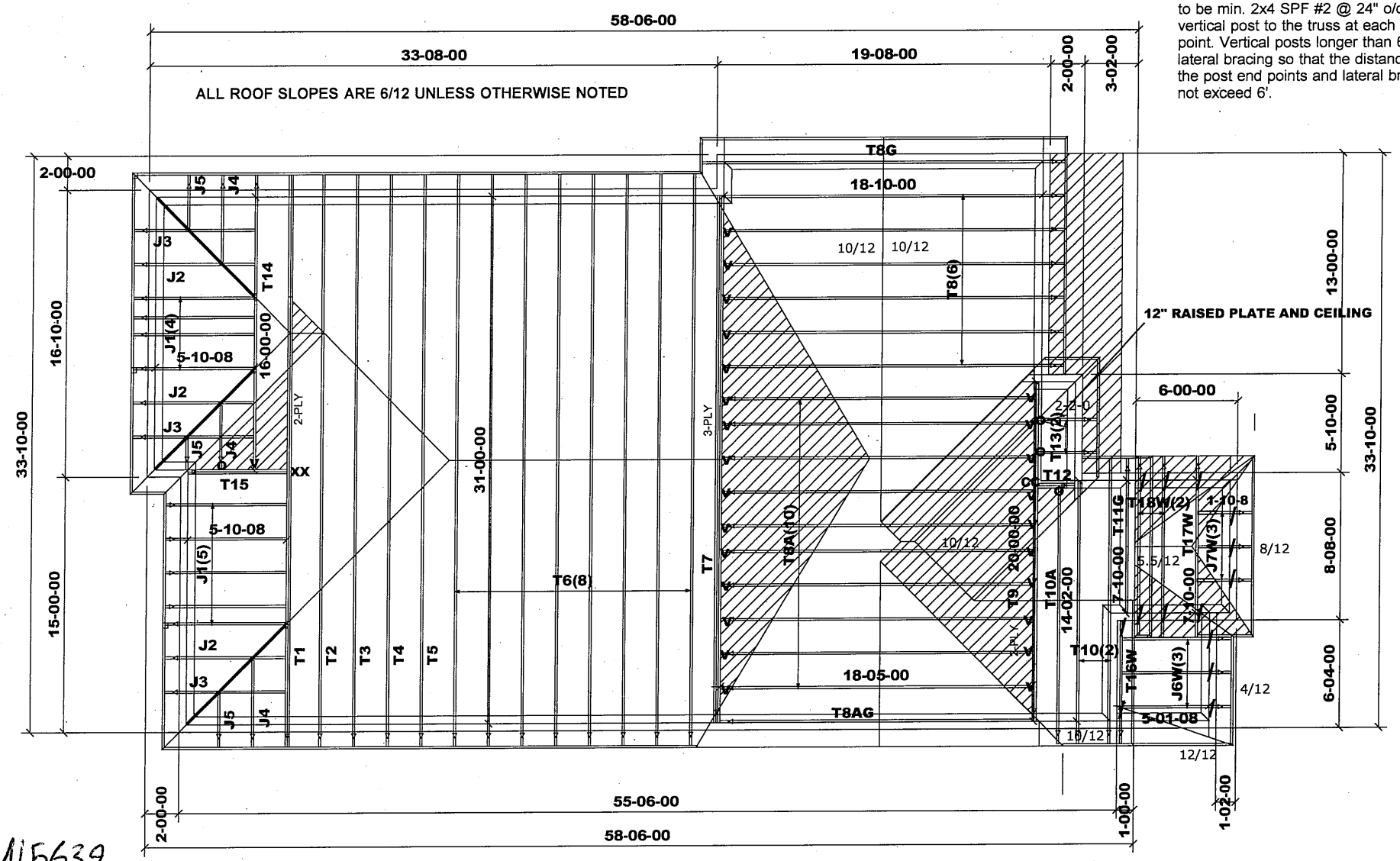
HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HUC26-2- (CC)
HGUS26-2- (XX)
H2.5T- (/)

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.32 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES:
CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.



M15639

TAMARACK ROOF TRUSSES INC.	Job Track: 53256	Builder / Location: GREENPARK HOMES / RICHMOND HILL	Model / Elevation: ROSE 6 / 1- STD.OR OPT.5 BED
	Plan Log: 207464	Project: TRINIGROUP DEVELOPMENTS	
	Layout ID: 434362	Date: 2023-08-25 Sales: Rick DiCiano Designer: YPG	

Mitek ver 8.6.3.353

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
LUS26-2- (VV)
HGUS26-2- (XX)
H2.5T- (/)

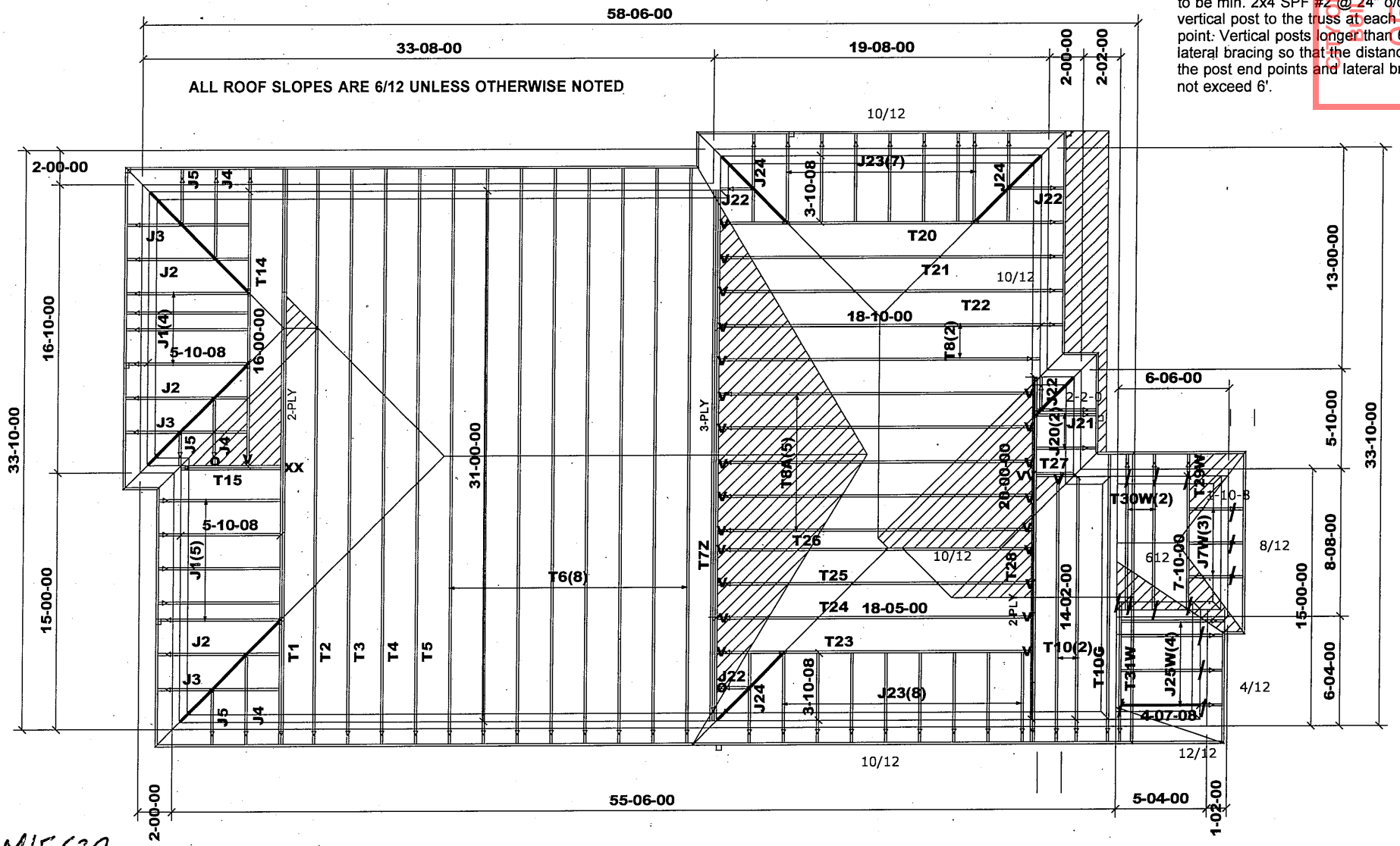
DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 31.32 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES: CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012 - 2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ALL ROOF SLOPES ARE 6/12 UNLESS OTHERWISE NOTED



M15639

TAMARACK ROOF TRUSSES INC.	Job Track: **53256**	Builder / Location: **GREENPARK HOMES / RICHMOND HILL**	Model / Elevation: **ROSE 6 / 2- STD.OR OPT.5 BED**
	Plan Log: **207464**	Project: **TRINIGROUP DEVELOPMENTS**	
	Layout ID: **434363**	Date: 2023-08-25 | Sales: Rick DiCiano | Designer: YPG	

Mitek ver 8.6.3.353

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2- (VV)
HGUS26-2- (XX)
H2.5T- (/)

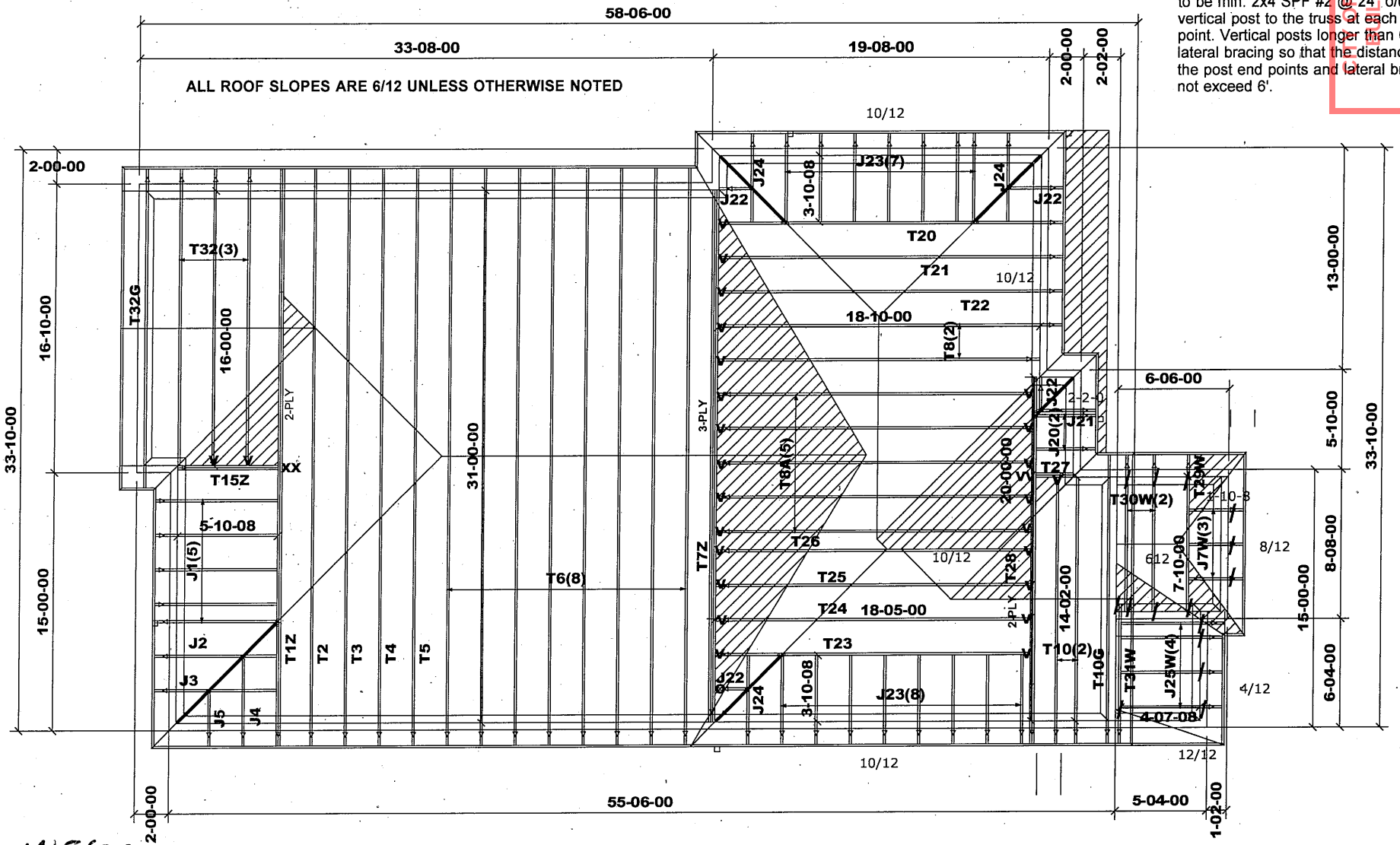
DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.32 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES:
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

ALL ROOF SLOPES ARE 6/12 UNLESS OTHERWISE NOTED



M15639

TAMARACK ROOF TRUSSES INC.	Job Track: 53256	Builder / Location: GREENPARK HOMES / RICHMOND HILL	Model / Elevation: ROSE 6 / 2- REAR UPGRADE
	Plan Log: 207464	Project: TRINIGROUP DEVELOPMENTS	
	Layout ID: 434364	Date: 2023-08-25 Sales: Rick DiCiano Designer: YPG	

Mitek ver 8.6.3.353

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

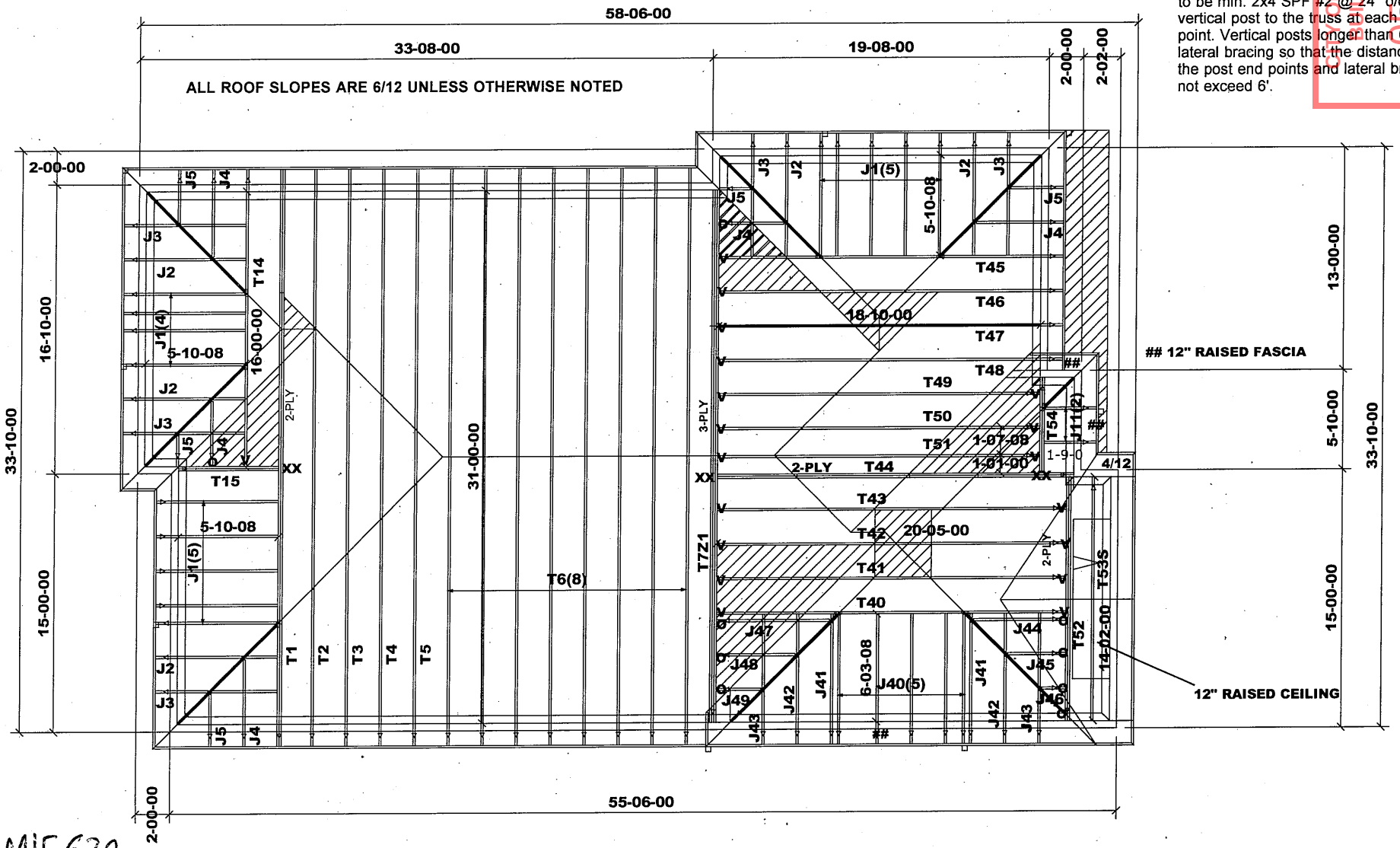
HARDWARE:
LUS24 - (**O**)
LJS26DS - (**V**)
HGUS26-2- (XX)

DESIGN CONFORMS WITH OBC 2012
(2019 amendment) OCCUPANCY:
RESIDENTIAL | PART: 9
Ss = 31.32 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES:
CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.



MI5639

TAMARACK ROOF TRUSSES INC.	Job Track: 53256	Builder / Location: GREENPARK HOMES / RICHMOND HILL		Model / Elevation: ROSE 6 / 3- STD.OR OPT.5 BED
	Plan Log: 207464	Project: TRINIGROUP DEVELOPMENTS		
	Layout ID: 434613	Date: 2023-08-25	Sales: Rick DiCiano	Designer: YPG

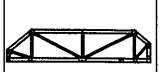
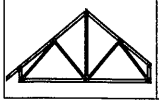
Mitek ver 8.6.3.353

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434362
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	1 of 3
Lot #:		Date:	08-25-2023
Elevation:	1- STD.OR OPT.5 BED	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

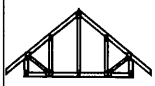
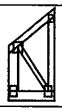
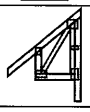
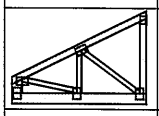
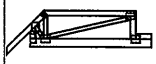
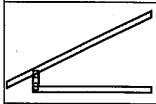
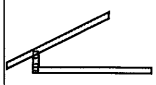
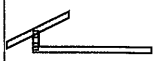
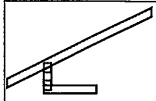
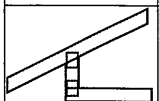
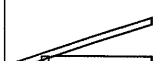
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	**31-00-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	278.51 174.67		
	1	T2 Hip	6 /12	**31-00-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	118.05 74.50		
	1	T3 Hip	6 /12	**31-00-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.51 80.00		
	1	T4 Hip	6 /12	**31-00-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.93 80.00		
	1	T5 Hip	6 /12	**31-00-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	128.89 81.83		
	8	T6 Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1006.02 628.00		
	1 3-ply	T7 Common Girder	6 /12	**31-00-00**	8-11-00	2 x 4 2 x 6		1-02-00 1-02-00	452.7 279.50		
	6	T8 Common	10 /12	**18-10-00**	9-05-13	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	572.51 364.00		
	10	T8A Common	10 /12	**18-05-00**	9-05-13	2 x 4	1-03-08	1-07-11 1-07-11	924.94 586.67		
	1	T8AG GABLE	10 /12	**18-05-00**	9-05-13	2 x 4		1-07-11 1-11-13	92.76 59.50		
	1	T8G GABLE	10 /12	**18-10-00**	9-05-13	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	97.19 62.83		
	1 2-ply	T9 Hip Girder	10 /12	**20-00-00**	4-05-06	2 x 6		1-07-11 2-07-11	214.38 135.00		
	2	T10 Common	10 /12	**14-02-00**	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	133.52 88.33		
	1	T10A Common	10 /12	**13-09-00**	7-06-08	2 x 4	1-03-08	1-07-11 1-11-13	63.94 41.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434362
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	2 of 3
Lot #:		Date:	08-25-2023
Elevation:	1- STD.OR OPT.5 BED	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

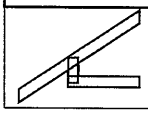
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T11G GABLE	10 /12	7-10-00	4-10-13	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	38.64 26.17		
	1 2-ply	T12 Jack-Closed Girder	10 /12	2-02-00	4-05-06	2 x 4 2 x 6		2-07-11 4-05-06	35.07 25.67		
	2	T13 Jack-Closed	10 /12	2-02-00	4-05-06	2 x 4	1-03-08	1-07-11 3-05-06	35.71 24.67		
	1	T14 Hip Girder	6 /12	16-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	74.95 49.33		
	1 2-ply	T15 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1	T16W Half Hip Girder	12 /12	5-10-08	2-05-10	2 x 4 2 x 6	1-03-08	1-03-04 2-00-07	30.47 20.33		
	1	T17W Hip Girder	5.5 /12	7-10-00	2-06-03	2 x 4	1-05-00 1-05-00	6-01 6-01	26.47 18.00		
	2	T18W Common	5.5 /12	7-10-00	2-09-03	2 x 4	1-05-00 1-05-00	6-01 6-01	50.89 31.33		
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	3	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	42.4 26.00		
	3	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	34.75 22.00		
	3	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	28.71 18.00		
	3	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	21.06 14.00		
	3	J6W Jack-Open	4 /12	5-01-08	2-05-06	2 x 4	1-03-08	3-15 2-00-07	41 26.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434362
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	3 of 3
Lot #:		Date:	08-25-2023
Elevation:	1- STD.OR OPT.5 BED	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	3	J7W Jack-Open	8 /12	1-10-08	2-05-09	2 x 4	1-03-08	9-10 2-00-10	21.31 14.00		

TOTAL # TRUSS= **79** TOTAL BFT OF ALL TRUSSES= **3185** BFT. TOTAL WEIGHT OF ALL TRSSES **5028.83** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	
27	Hardware	LJS26DS	
1	Hardware	HGUS26-2	
1		HUC26-2	
14	Hardware	H2.5T	

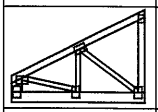
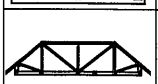
TOTAL NUMBER OF ITEMS= **47**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434363
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	1 of 3
Lot #:		Date:	08-25-2023
Elevation:	2- STD.OR OPT.5 BED	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

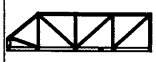
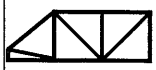
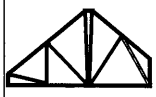
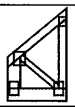
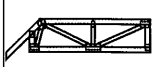
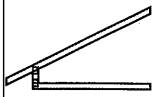
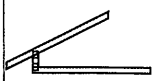
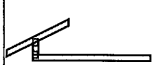
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	**31-00-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	278.51 174.67		
	1	T2 Hip	6 /12	**31-00-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	121.07 76.17		
	1	T3 Hip	6 /12	**31-00-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.51 80.00		
	1	T4 Hip	6 /12	**31-00-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.93 80.00		
	1	T5 Hip	6 /12	**31-00-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	136.93 86.83		
	8	T6 Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1035.62 644.00		
	1 3-ply	T7Z Common Girder	6 /12	**31-00-00**	8-11-00	2 x 4 2 x 6		1-02-00 1-02-00	497.51 305.00		
	2	T8 Common	10 /12	**18-10-00**	9-05-13	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	190.86 121.33		
	5	T8A Common	10 /12	**18-05-00**	9-05-13	2 x 4	1-03-08	1-07-11 1-07-11	462.52 293.33		
	2	T10 Common	10 /12	**14-02-00**	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	141.48 94.33		
	1	T10G GABLE	10 /12	**14-02-00**	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	69.94 46.17		
	1	T14 Hip Girder	6 /12	**16-00-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	74.95 49.33		
	1 2-ply	T15 Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1	T20 Hip Girder	10 /12	**18-10-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	93.31 58.83		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434363
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	2 of 3
Lot #:		Date:	08-25-2023
Elevation:	2- STD.OR OPT.5 BED	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

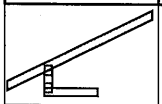
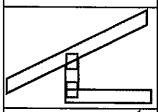
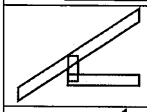
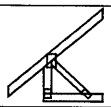
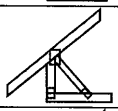
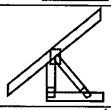
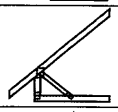
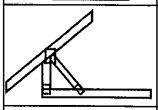
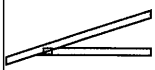
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T21 Hip	10 /12	18-10-00	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	84.26 53.00		
	1	T22 Hip	10 /12	18-10-00	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	93.1 59.83		
	1	T23 Half Hip Girder	10 /12	18-05-00	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	90.41 58.33		
	1	T24 Half Hip	10 /12	18-05-00	6-06-07	2 x 4		1-07-11 6-06-07	80.48 50.00		
	1	T25 Hip	10 /12	18-05-00	8-02-07	2 x 4		1-07-11 3-07-14	86.15 55.33		
	1	T26 Hip	10 /12	18-05-00	9-10-07	2 x 4		1-07-11 3-07-14	96.84 62.83		
	1 2-ply	T27 Jack-Closed Girder	10 /12	2-02-00	3-05-06	2 x 4 2 x 6		1-07-11 3-05-06	27.95 21.33		
	1 2-ply	T28 Hip Girder	10 /12	20-00-00	3-05-06	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	213.86 134.33		
	1	T29W Hip Girder	6 /12	7-10-00	2-05-08	2 x 4	1-03-08 1-03-08	6-12 6-12	26.4 16.67		
	2	T30W Common	6 /12	7-10-00	2-11-03	2 x 4	1-03-08 1-03-08	6-12 6-12	50.85 31.33		
	1	T31W Half Hip Girder	12 /12	6-09-08	2-03-10	2 x 4	1-03-08	1-03-04 1-10-07	27.73 18.83		
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	3	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	42.4 26.00		
	3	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	34.75 22.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434363
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	3 of 3
Lot #:		Date:	08-25-2023
Elevation:	2- STD.OR OPT.5 BED	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	3	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	28.71 18.00		
	3	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	21.06 14.00		
	3	J7W Jack-Open	8 /12	**1-10-08**	2-05-09	2 x 4	1-03-08	9-10 2-00-10	21.31 14.00		
	2	J20 Jack-Open	10 /12	**2-02-00**	3-05-06	2 x 4	1-03-08	1-07-11 3-05-06	21.56 15.33		
	1	J21 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 4-09	1-07-11 3-01-09	10.07 7.67		
	4	J22 Jack-Open	10 /12	**1-10-08**	3-01-08	2 x 4	1-03-08	1-07-11 3-02-07	38.84 28.00		
	15	J23 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	231.73 152.50		
	3	J24 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	36.05 25.00		
	4	J25W Jack-Open	4 /12	**4-07-08**	2-03-06	2 x 4	1-03-08	3-15 1-10-07	49.99 32.00		

TOTAL # TRUSS= **98** TOTAL BFT OF ALL TRUSSES= **3159.97** BFT. TOTAL WEIGHT OF ALL TRSSES **4981.17** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
24	Hardware	LJS26DS	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	
14	Hardware	H2.5T	

TOTAL NUMBER OF ITEMS= **42**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434364
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	1 of 3
Lot #:		Date:	08-25-2023
Elevation:	2- REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

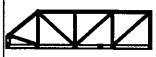
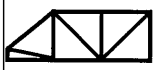
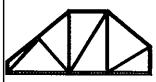
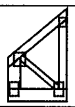
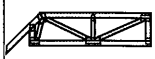
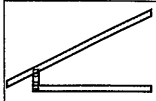
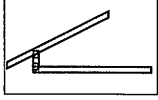
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1Z Hip Girder	6 /12	**31-00-00**	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	278.51 174.67		
	1	T2 Hip	6 /12	**31-00-00**	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	121.07 76.17		
	1	T3 Hip	6 /12	**31-00-00**	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.51 80.00		
	1	T4 Hip	6 /12	**31-00-00**	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.93 80.00		
	1	T5 Hip	6 /12	**31-00-00**	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	136.93 86.83		
	8	T6 Common	6 /12	**31-00-00**	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1035.62 644.00		
	1 3-ply	T7Z Common Girder	6 /12	**31-00-00**	8-11-00	2 x 4 2 x 6		1-02-00 1-02-00	497.51 305.00		
	2	T8 Common	10 /12	**18-10-00**	9-05-13	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	190.86 121.33		
	5	T8A Common	10 /12	**18-05-00**	9-05-13	2 x 4	1-03-08	1-07-11 1-07-11	462.52 293.33		
	2	T10 Common	10 /12	**14-02-00**	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	141.48 94.33		
	1	T10G GABLE	10 /12	**14-02-00**	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	69.94 46.17		
	1 2-ply	T15Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1	T20 Hip Girder	10 /12	**18-10-00**	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	93.31 58.83		
	1	T21 Hip	10 /12	**18-10-00**	6-06-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	84.26 53.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434364
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	2 of 3
Lot #:		Date:	08-25-2023
Elevation:	2- REAR UPGRADE	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T22 Hip	10 /12	**18-10-00**	8-02-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	93.1 59.83		
	1	T23 Half Hip Girder	10 /12	**18-05-00**	4-10-07	2 x 4 2 x 6		1-07-11 4-10-07	90.41 58.33		
	1	T24 Half Hip	10 /12	**18-05-00**	6-06-07	2 x 4		1-07-11 6-06-07	80.48 50.00		
	1	T25 Hip	10 /12	**18-05-00**	8-02-07	2 x 4		1-07-11 3-07-14	86.15 55.33		
	1	T26 Hip	10 /12	**18-05-00**	9-10-07	2 x 4		1-07-11 3-07-14	96.84 62.83		
	1 2-ply	T27 Jack-Closed Girder	10 /12	**2-02-00**	3-05-06	2 x 4 2 x 6		1-07-11 3-05-06	27.95 21.33		
	1 2-ply	T28 Hip Girder	10 /12	**20-00-00**	3-05-06	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	213.86 134.33		
	1	T29W Hip Girder	6 /12	**7-10-00**	2-05-08	2 x 4	1-03-08 1-03-08	6-12 6-12	26.4 16.67		
	2	T30W Common	6 /12	**7-10-00**	2-11-03	2 x 4	1-03-08 1-03-08	6-12 6-12	50.85 31.33		
	1	T31W Half Hip Girder	12 /12	**6-09-08**	2-03-10	2 x 4	1-03-08	1-03-04 1-10-07	27.73 18.83		
	3	T32 Common	6 /12	**16-00-00**	5-02-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	193.47 124.00		
	1	T32G GABLE	6 /12	**16-00-00**	5-02-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	60.27 38.83		
	5	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	1	J2 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	14.13 8.67		

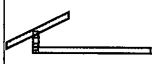
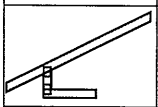
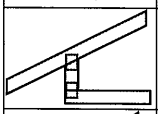
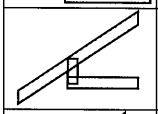
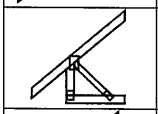
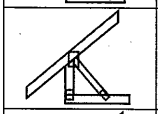
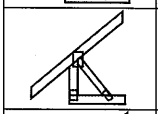
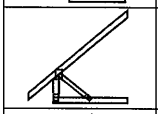
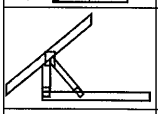
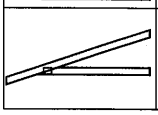
TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	ROSE 6
Lot #:	
Elevation:	2- REAR UPGRADE

Job Track:	53256
PlanLog:	207464
Layout ID:	434364
Ref #	
Page:	3 of 3
Date:	08-25-2023
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	J3 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	11.58 7.33		
	1	J4 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	3	J7W Jack-Open	8 /12	**1-10-08**	2-05-09	2 x 4	1-03-08	9-10 2-00-10	21.31 14.00		
	2	J20 Jack-Open	10 /12	**2-02-00**	3-05-06	2 x 4	1-03-08	1-07-11 3-05-06	21.56 15.33		
	1	J21 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 4-09	1-07-11 3-01-09	10.07 7.67		
	4	J22 Jack-Open	10 /12	**1-10-08**	3-01-08	2 x 4	1-03-08	1-07-11 3-02-07	38.84 28.00		
	15	J23 Jack-Open	10 /12	**3-10-08**	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	231.73 152.50		
	3	J24 Jack-Open	10 /12	**1-09-07**	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	36.05 25.00		
	4	J25W Jack-Open	4 /12	**4-07-08**	2-03-06	2 x 4	1-03-08	3-15 1-10-07	49.99 32.00		

TOTAL # TRUSS= **89** TOTAL BFT OF ALL TRUSSES= **3177.47** BFT. TOTAL WEIGHT OF ALL TRSSES **5008.17** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
26	Hardware	LJS26DS	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	
15	Hardware	H2.5A	

TOTAL NUMBER OF ITEMS= **45**

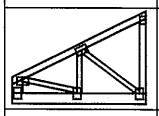
TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	ROSE 6
Lot #:	
Elevation:	3- STD.OR OPT.5 BED

Job Track:	53256
PlanLog:	207464
Layout ID:	434613
Ref #	
Page:	1 of 4
Date:	08-25-2023
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

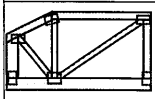
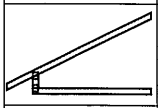
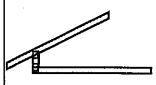
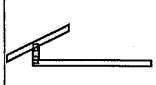
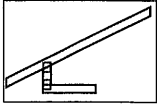
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	31-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	278.51 174.67		
	1	T2 Hip	6 /12	31-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	118.05 74.50		
	1	T3 Hip	6 /12	31-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	126.51 80.00		
	1	T4 Hip	6 /12	31-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	127.93 80.00		
	1	T5 Hip	6 /12	31-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	128.89 81.83		
	8	T6 Common	6 /12	31-00-00	8-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1006.02 628.00		
	1 3-ply	T7Z1 Common Girder	6 /12	31-00-00	8-11-00	2 x 4 2 x 6		1-02-00 1-02-00	452.7 279.50		
	1	T14 Hip Girder	6 /12	16-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	74.95 49.33		
	1 2-ply	T15 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	1	T40 Hip Girder	6 /12	20-05-00	5-03-12	2 x 4 2 x 6		2-04-08 1-09-00	100.15 63.67		
	1	T41 Hip	6 /12	20-05-00	6-03-12	2 x 4		2-04-08 1-09-00	87.13 55.67		
	1	T42 Half Hip	6 /12	20-05-00	6-03-12	2 x 4		2-04-08 6-03-12	87.9 55.00		
	1	T43 Roof Special	6 /12	20-05-00	7-03-12	2 x 4		2-04-08 7-03-12	102.51 65.50		
	1 2-ply	T44 Roof Special Girder	6 /12	20-10-00	8-03-00	2 x 4 2 x 6		2-02-00 8-03-00	241.19 151.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207464
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	434613
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 6	Page:	2 of 4
Lot #:		Date:	08-25-2023
Elevation:	3- STD.OR OPT.5 BED	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T45 Hip Girder	6 /12	18-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	87.28 55.33		
	1	T46 Hip	6 /12	18-10-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	76.89 49.33		
	1	T47 Half Hip	6 /12	18-10-00	5-01-04	2 x 4	1-03-08	1-02-00 5-01-04	77.72 48.33		
	1	T48 Half Hip	6 /12	18-10-00	6-01-04	2 x 4	1-03-08	1-02-00 6-01-04	84.39 52.50		
	1	T49 Half Hip	6 /12	18-10-00	7-01-04	2 x 4		1-02-00 7-01-04	83.21 52.00		
	1	T50 Half Hip	6 /12	18-10-00	8-01-04	2 x 4		1-02-00 8-01-04	82.83 51.17		
	1	T51 Half Hip	6 /12	18-10-00	8-10-04	2 x 4		1-02-00 8-10-04	86.12 54.00		
	1 2-ply	T52 Common Girder	4 /12	14-02-00	4-03-08	2 x 4 2 x 6		1-11-03 1-11-03	132.39 89.33		
	1	T53S Roof Special	4 /12	14-02-00	4-03-08	2 x 4		1-11-03 1-11-03	59.35 40.50		
	1 2-ply	T54 Half Hip Girder	6 /12	5-07-00	3-00-08	2 x 4 2 x 6		2-02-00 3-00-08	58.23 40.33		
	14	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	5	J2 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	70.66 43.33		
	5	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	57.91 36.67		
	5	J4 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	47.85 30.00		

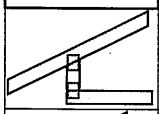
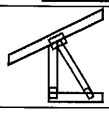
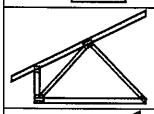
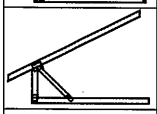
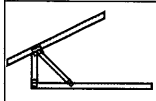
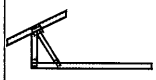
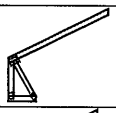
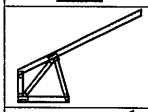
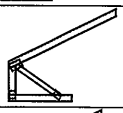
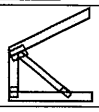
TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	ROSE 6
Lot #:	
Elevation:	3- STD.OR OPT.5 BED

Job Track:	53256
PlanLog:	207464
Layout ID:	434613
Ref #	
Page:	3 of 4
Date:	08-25-2023
Designer:	
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	J5 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	35.1 23.33		
	2	J11 Jack-Open	6 /12	**1-09-00**	3-00-08	2 x 4	1-03-08	2-02-00 3-00-08	19.25 13.67		
	5	J40 Jack-Open	6 /12	**6-03-08**	5-03-12	2 x 4	1-03-08	2-02-00 5-03-12	129.69 85.00		
	2	J41 Jack-Open	6 /12	**5-09-07**	5-00-12	2 x 4	1-03-08 6-01	2-02-00 5-00-12	40.69 27.00		
	2	J42 Jack-Open	6 /12	**3-09-07**	4-00-12	2 x 4	1-03-08 2-06-01	2-02-00 4-00-12	35.59 23.00		
	2	J43 Jack-Open	6 /12	**1-09-07**	3-00-12	2 x 4	1-03-08 4-06-01	2-02-00 3-00-12	29.74 20.33		
	1	J44 Jack-Open	6 /12	**1-05-08**	5-00-12	2 x 4	3-10-15	2-04-08 3-01-04	12.66 8.83		
	1	J45 Jack-Open	6 /12	**1-05-08**	4-00-12	2 x 4	1-10-15	2-04-08 3-01-04	10.11 6.83		
	1	J46 Jack-Open	6 /12	**1-05-08**	3-00-12	2 x 4		2-04-08 3-01-04	7.44 5.50		
	1	J47 Jack-Open	6 /12	**2-08-08**	5-00-12	2 x 4	3-10-15	1-09-00 3-01-04	17.14 11.67		
	1	J48 Jack-Open	6 /12	**2-08-08**	4-00-12	2 x 4	1-10-15	1-09-00 3-01-04	12.5 8.83		
	1	J49 Jack-Open	6 /12	**2-08-08**	3-00-12	2 x 4		1-09-00 3-01-04	9.79 7.50		

TOTAL # TRUSS= **91** TOTAL BFT OF ALL TRUSSES= **2910.65** BFT. TOTAL WEIGHT OF ALL TRSSES **4590.5** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	LUS24	
20	Hardware	LJS26DS	

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	ROSE 6
Lot #:	
Elevation:	3- STD.OR OPT.5 BED

Job Track:	53256
PlanLog:	207464
Layout ID:	434613
Ref #	
Page:	4 of 4
Date:	08-25-2023
Designer:	
Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	

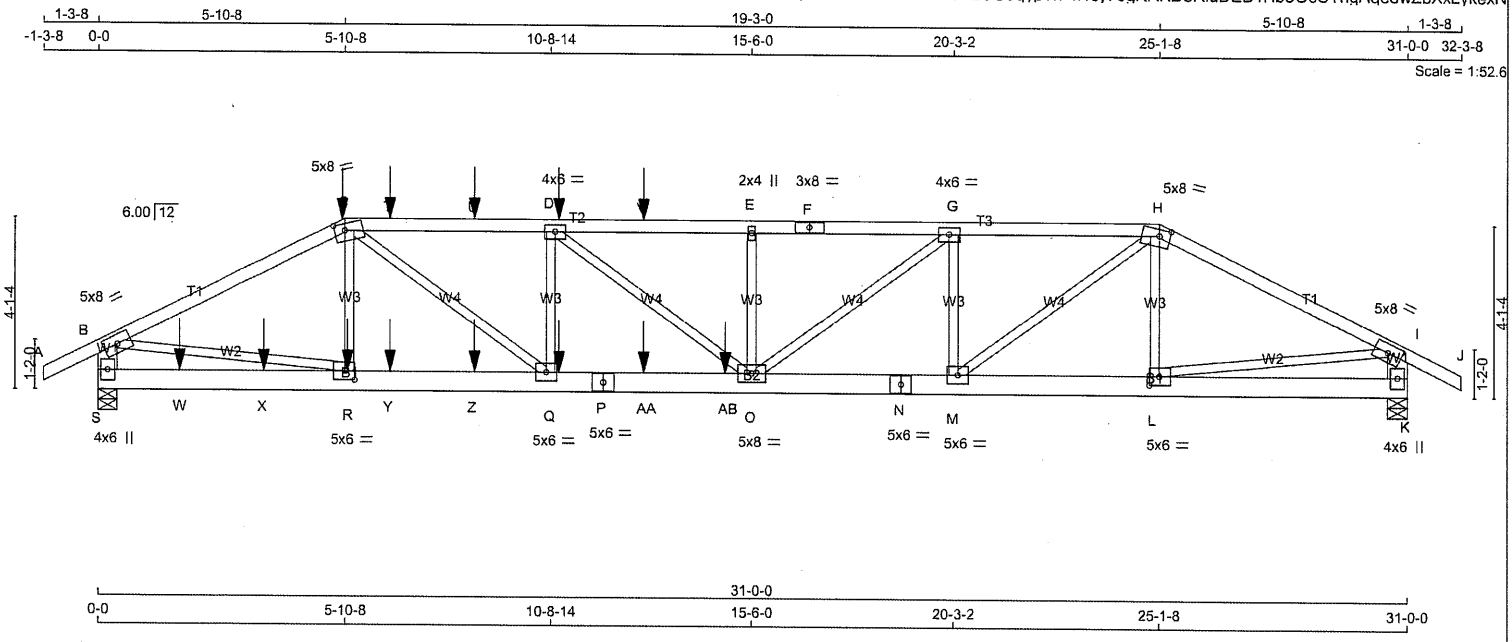
TOTAL NUMBER OF ITEMS= **32**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434362	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:32:38 2023 Page 1

ID:LpDGmu0HxWz8EPf6?EUU9qyp?l7-k4cy7oqKRKDbRiuDED1Hb8O6SThqAqeowZbXxLykexN



TOTAL WEIGHT = 2 X 139 = 279 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(183.1)
P- N	2	12	SIDE(0.0)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY
DWG # TR23080929

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3334	0	3334	0	0	5-8	1-13
K	2791	0	2791	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2356	1557 / 0	0 / 0	0 / 0	0 / 0	800 / 0	0 / 0
K	1970	1317 / 0	0 / 0	0 / 0	0 / 0	653 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	R- C	-435 / 20		0.06 (1)
B- C	-5062 / 0	-91.8	-91.8	0.52 (1)	3.87	C- Q	0 / 2915		0.36 (1)
C- T	-6828 / 0	-91.8	-91.8	0.48 (1)	3.37	Q- D	-1412 / 0		0.18 (1)
T- U	-6828 / 0	-91.8	-91.8	0.48 (1)	3.37	D- O	0 / 988		0.12 (1)
U- D	-6828 / 0	-91.8	-91.8	0.48 (1)	3.37	O- E	-425 / 0		0.05 (1)
D- V	-7606 / 0	-91.8	-91.8	0.53 (1)	3.16	O- G	0 / 2041		0.25 (1)
V- E	-7606 / 0	-91.8	-91.8	0.53 (1)	3.16	M- G	-1750 / 0		0.22 (1)
E- F	-7606 / 0	-91.8	-91.8	0.43 (1)	3.28	M- H	0 / 2889		0.36 (1)
F- G	-7606 / 0	-91.8	-91.8	0.43 (1)	3.28	L- H	-354 / 0		0.04 (1)
G- H	-5999 / 0	-91.8	-91.8	0.33 (1)	3.75	B- R	0 / 4563		0.56 (1)
H- I	-4155 / 0	-91.8	-91.8	0.45 (1)	4.26	L- I	0 / 3746		0.46 (1)
I- J	0 / 28	-91.8	-91.8	0.07 (1)	10.00				
S- B	-3249 / 0	0.0	0.0	0.11 (1)	7.69				
K- I	-2738 / 0	0.0	0.0	0.10 (1)	7.81				
S- W	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
W- X	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
X- R	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
R- Y	0 / 4513	-18.5	-18.5	0.33 (1)	10.00				
Y- Z	0 / 4513	-18.5	-18.5	0.33 (1)	10.00				
Z- Q	0 / 4513	-18.5	-18.5	0.33 (1)	10.00				
Q- P	0 / 6828	-18.5	-18.5	0.69 (1)	10.00				
P-AA	0 / 6828	-18.5	-18.5	0.69 (1)	10.00				
AA-AB	0 / 6828	-18.5	-18.5	0.69 (1)	10.00				
AB- O	0 / 6828	-18.5	-18.5	0.69 (1)	10.00				
O- N	0 / 5999	-18.5	-18.5	0.45 (1)	10.00				
N- M	0 / 5999	-18.5	-18.5	0.45 (1)	10.00				
M- L	0 / 3705	-18.5	-18.5	0.28 (1)	10.00				
L- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-254	-254	---	BACK	VERT	TOTAL	---	C1
D	10-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Q	10-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
R	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
T	6-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
U	8-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
V	12-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	1-11-4	-20	-20	---	BACK	VERT	TOTAL	---	C1
X	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	6-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.53/1.00 (D-E:1) , BC=0.69/1.00 (O-Q:1) , WB=0.56/1.00 (B-R:1) , SSI=0.44/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.66 (P) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T1	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-DGALK8hyCeLS3rTPoxZW8MwHCt1vvHux8DL4ToykexM

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.00
I	TMVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	4.0	6.0		

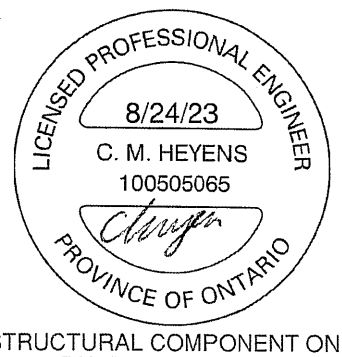
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Z	8-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AA	12-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	14-10-8	-1032	-1032	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR23080929

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434364	T1Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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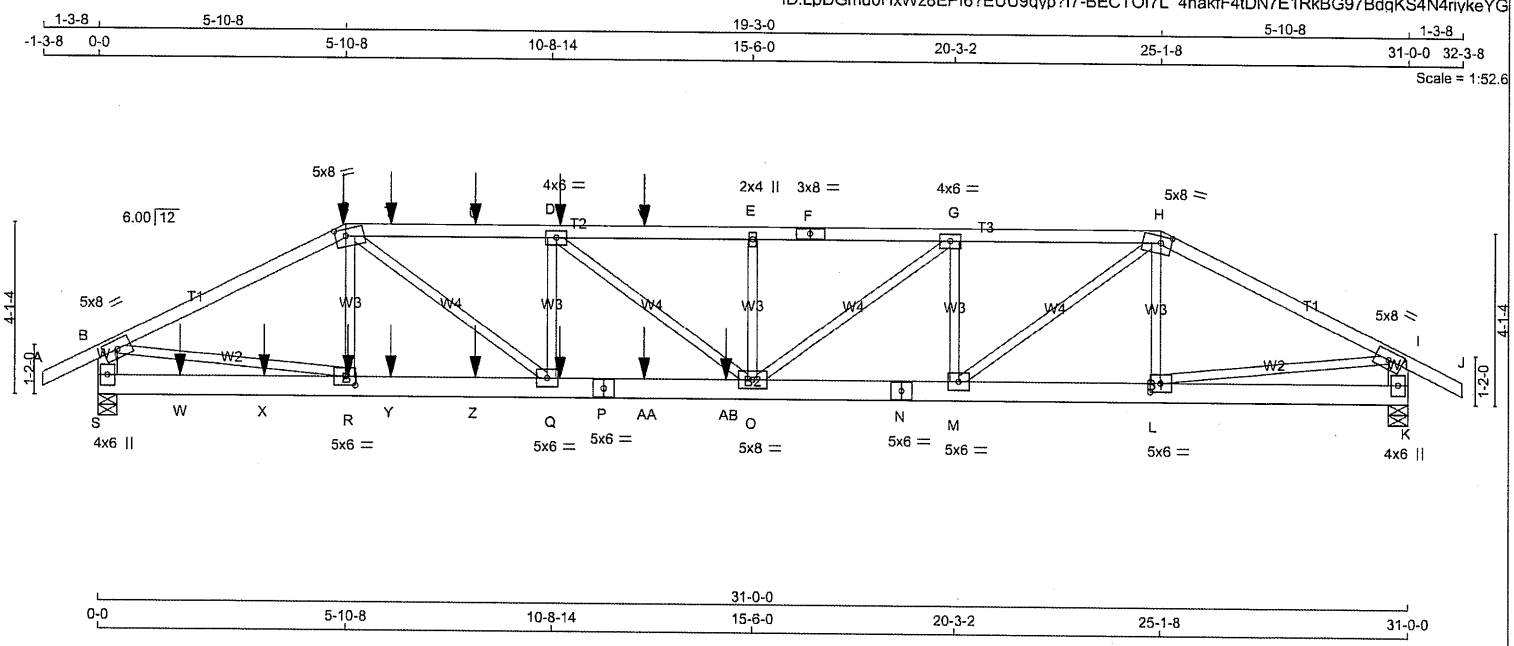

TOTAL WEIGHT = 2 X 139 = 279 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(183.1)
P- N	2	12	SIDE(0.0)
N- K	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY
DWG # TR23080971

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3266	0	3266	0	0	5-8	1-12
K	2728	0	2728	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2308	1528 / 0	0 / 0	0 / 0	0 / 0	780 / 0	0 / 0
K	1925	1290 / 0	0 / 0	0 / 0	0 / 0	634 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00
B- C	-4943 / 0	-91.8	-91.8	0.51 (1)	3.92
C- T	-6629 / 0	-91.8	-91.8	0.47 (1)	3.43
T- U	-6629 / 0	-91.8	-91.8	0.47 (1)	3.43
U- D	-6629 / 0	-91.8	-91.8	0.47 (1)	3.43
D- V	-7340 / 0	-91.8	-91.8	0.51 (1)	3.22
V- E	-7340 / 0	-91.8	-91.8	0.51 (1)	3.22
E- F	-7340 / 0	-91.8	-91.8	0.41 (1)	3.35
F- G	-7340 / 0	-91.8	-91.8	0.41 (1)	3.35
G- H	-5820 / 0	-91.8	-91.8	0.32 (1)	3.81
H- I	-4044 / 0	-91.8	-91.8	0.45 (1)	4.32
I- J	0 / 28	-91.8	-91.8	0.07 (1)	10.00
S- B	-3182 / 0	0.0	0.0	0.11 (1)	7.75
K- I	-2675 / 0	0.0	0.0	0.09 (1)	7.81
S- W	0 / 0	-18.5	-18.5	0.07 (4)	10.00
W- X	0 / 0	-18.5	-18.5	0.07 (4)	10.00
X- R	0 / 0	-18.5	-18.5	0.07 (4)	10.00
R- Y	0 / 4407	-18.5	-18.5	0.32 (1)	10.00
Y- Z	0 / 4407	-18.5	-18.5	0.32 (1)	10.00
Z- Q	0 / 4407	-18.5	-18.5	0.32 (1)	10.00
Q- P	0 / 6629	-18.5	-18.5	0.66 (1)	10.00
P-AA	0 / 6629	-18.5	-18.5	0.66 (1)	10.00
AA-AB	0 / 6629	-18.5	-18.5	0.66 (1)	10.00
AB- O	0 / 6629	-18.5	-18.5	0.66 (1)	10.00
O- N	0 / 5820	-18.5	-18.5	0.43 (1)	10.00
N- M	0 / 5820	-18.5	-18.5	0.43 (1)	10.00
M- L	0 / 3605	-18.5	-18.5	0.28 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-417 / 27	0.05 (1)
Q- D	-1360 / 0	0.17 (1)
Q- O	0 / 903	0.11 (1)
O- E	-426 / 0	0.05 (1)
O- G	0 / 1931	0.24 (1)
M- G	-1682 / 0	0.21 (1)
M- H	0 / 2789	0.35 (1)
L- H	-343 / 0	0.04 (1)
B- R	0 / 4455	0.55 (1)
L- I	0 / 3645	0.45 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-254	-254	---	BACK	VERT	TOTAL	---	C1
D	10-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
Q	10-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
R	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
T	6-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
U	8-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
V	12-11-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	1-11-4	-20	-20	---	BACK	VERT	TOTAL	---	C1
X	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	6-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.51/1.00 (D-E:1) , BC=0.66/1.00 (O-Q:1) , WB=0.55/1.00 (B-R:1) , SSI=0.41/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.64 (P) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434364	T1Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-fQmrce7zlOpRMppGRxuMmE_vxfVMw4wTgk6dN8ykeYF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	5.0	8.0	2.00	3.00
I	TMVW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Z	8-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AA	12-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AB	14-10-8	-938	-938	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080971

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T2	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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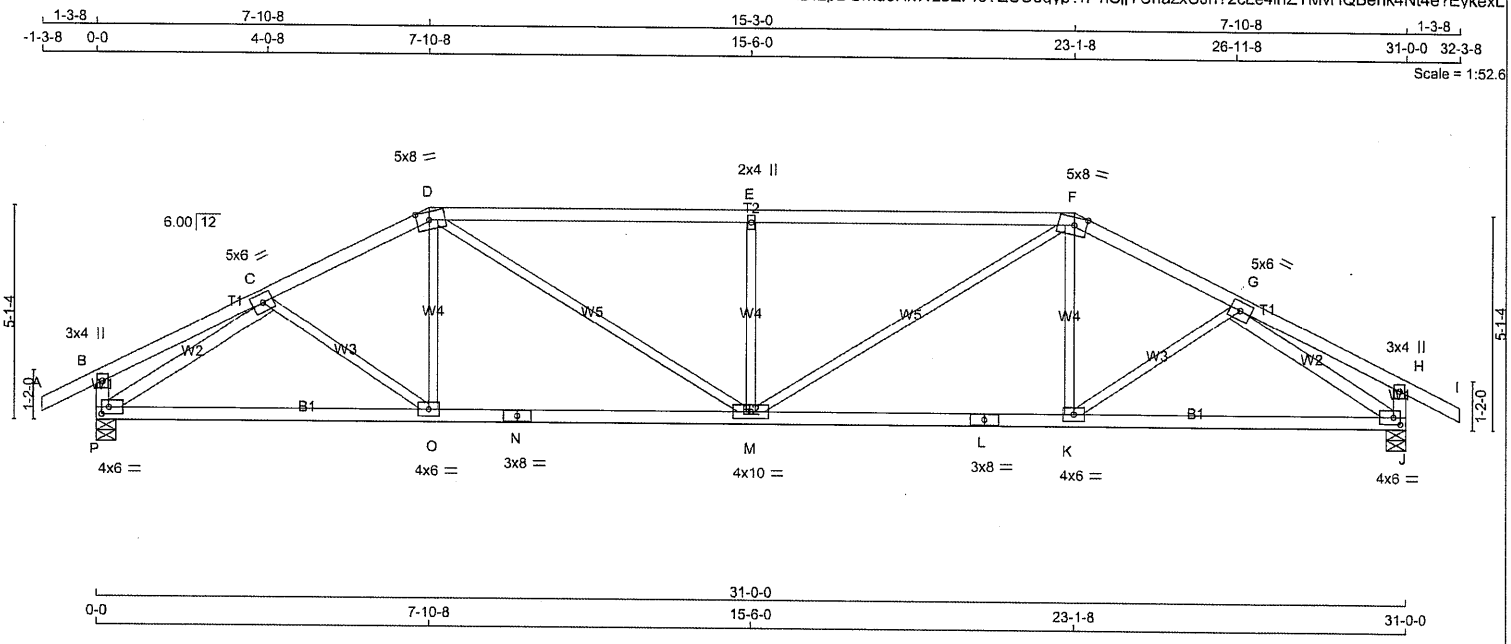

TOTAL WEIGHT = 121 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	3.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0	2.00	2.00
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	10.0		
N	BS-t	MT20	3.0	8.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMVW1-t	MT20	4.0	6.0	2.00	2.00

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1833	0	1833	0	0	5-8	2-0
J	1833	0	1833	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- O	0 / 69	0.02 (4)
B- C	0 / 17	-91.8	-91.8	0.20 (1)	10.00	O- D	0 / 158	0.05 (4)
C- D	-2380 / 0	-91.8	-91.8	0.23 (1)	4.27	D- M	0 / 924	0.21 (1)
D- E	-2897 / 0	-91.8	-91.8	0.92 (1)	2.97	M- E	-862 / 0	0.33 (1)
E- F	-2897 / 0	-91.8	-91.8	0.92 (1)	2.97	M- F	0 / 924	0.21 (1)
F- G	-2380 / 0	-91.8	-91.8	0.23 (1)	4.27	K- F	0 / 158	0.05 (4)
G- H	0 / 17	-91.8	-91.8	0.20 (1)	10.00	K- G	0 / 69	0.02 (4)
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P- C	-2584 / 0	0.72 (1)
P- B	-269 / 0	0.0	0.0	0.03 (1)	7.81	G- J	-2584 / 0	0.72 (1)
J- H	-269 / 0	0.0	0.0	0.03 (1)	7.81			
P- O	0 / 2100	-18.5	-18.5	0.49 (1)	10.00			
O- N	0 / 2115	-18.5	-18.5	0.50 (1)	10.00			
N- M	0 / 2115	-18.5	-18.5	0.50 (1)	10.00			
M- L	0 / 2115	-18.5	-18.5	0.50 (1)	10.00			
L- K	0 / 2115	-18.5	-18.5	0.50 (1)	10.00			
K- J	0 / 2100	-18.5	-18.5	0.49 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.92/1.00 (D-E:1) , BC=0.50/1.00 (K-M:1) , WB=0.72/1.00 (G-J:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.60 (L) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080930

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T3	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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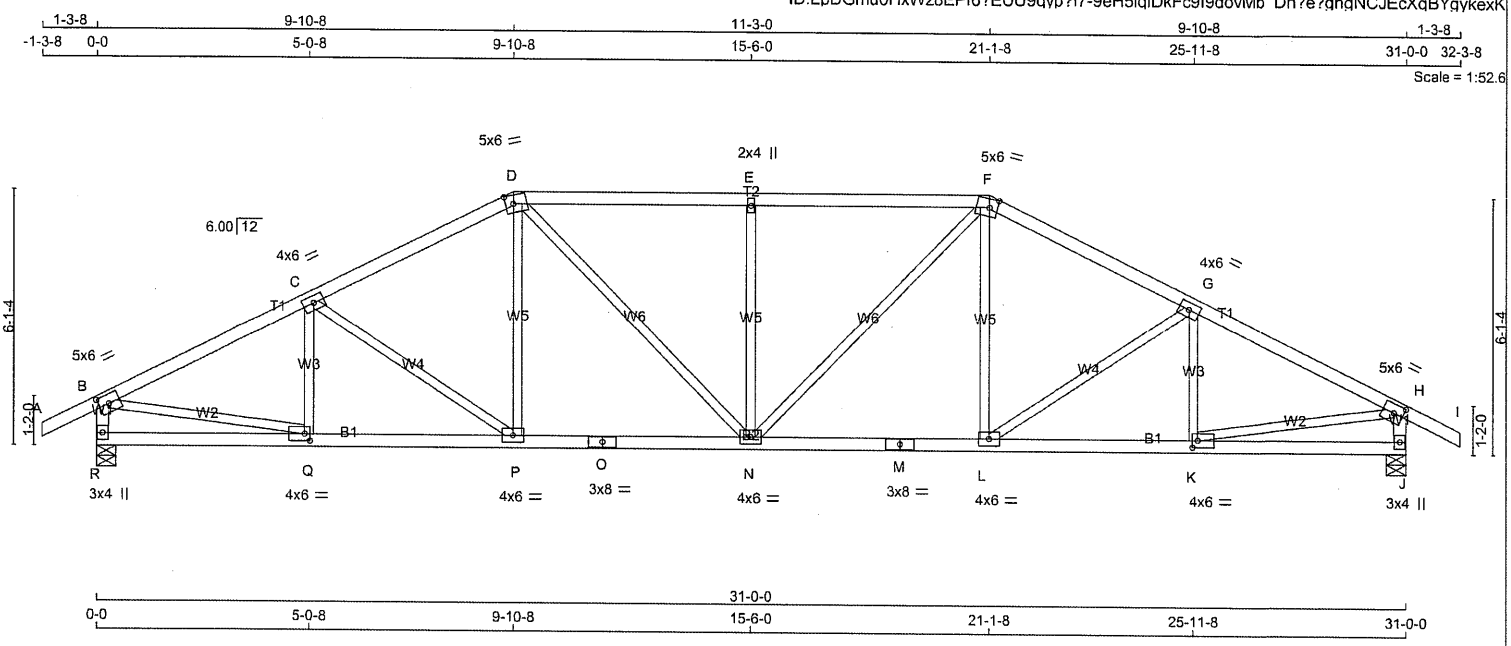

TOTAL WEIGHT = 127 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.50	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.25
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	1.50
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1833	0	1833	0	0	5-8	2-7
J	1833	0	1833	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	Q- C	-286 / 0	0.06 (1)
B- C	-2455 / 0	-91.8	-91.8	0.36 (1)	4.08	C- P	-279 / 0	0.17 (1)
C- D	-2245 / 0	-91.8	-91.8	0.34 (1)	4.25	P- D	0 / 261	0.06 (1)
D- E	-2344 / 0	-91.8	-91.8	0.45 (1)	4.03	D- N	0 / 509	0.11 (1)
E- F	-2344 / 0	-91.8	-91.8	0.45 (1)	4.03	N- E	-632 / 0	0.37 (1)
F- G	-2245 / 0	-91.8	-91.8	0.34 (1)	4.25	N- F	0 / 509	0.11 (1)
G- H	-2455 / 0	-91.8	-91.8	0.36 (1)	4.08	L- F	0 / 261	0.06 (1)
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	L- G	-279 / 0	0.17 (1)
R- B	-1790 / 0	0.0	0.0	0.18 (1)	6.24	K- G	-286 / 0	0.06 (1)
J- H	-1790 / 0	0.0	0.0	0.18 (1)	6.24	B- Q	0 / 2248	0.51 (1)
						K- H	0 / 2248	0.51 (1)
R- Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
Q- P	0 / 2216	-18.5	-18.5	0.42 (1)	10.00			
P- O	0 / 1990	-18.5	-18.5	0.38 (1)	10.00			
O- N	0 / 1990	-18.5	-18.5	0.38 (1)	10.00			
N- M	0 / 1990	-18.5	-18.5	0.38 (1)	10.00			
M- L	0 / 1990	-18.5	-18.5	0.38 (1)	10.00			
L- K	0 / 2216	-18.5	-18.5	0.42 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.42/1.00 (K-L:1) , WB=0.51/1.00 (H-K:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.62 (O) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080931

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T4	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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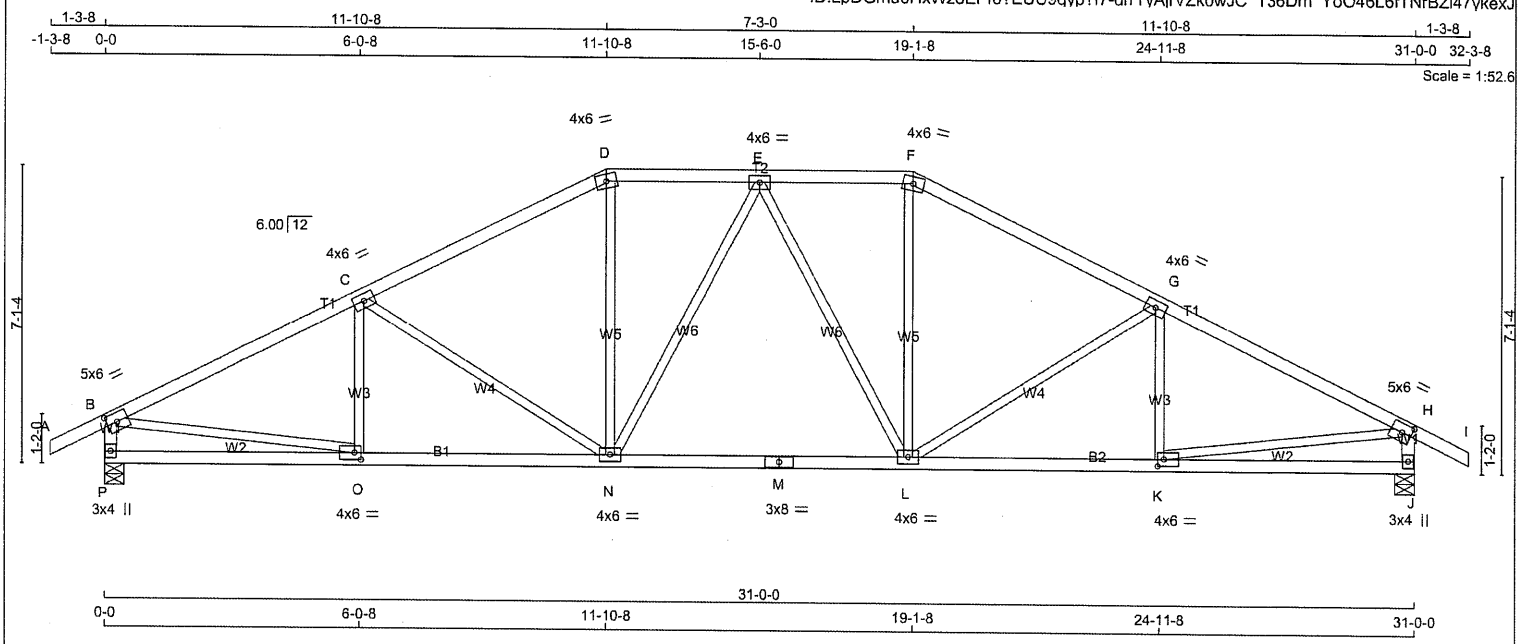

TOTAL WEIGHT = 128 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C, E, G						
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.75
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0	2.00	1.75
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1833	0	1833	0	0	5-8	2-7
J	1833	0	1833	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
J	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	O- C	-206 / 25	0.05 (1)
B- C	-2489 / 0	-91.8	-91.8	0.54 (1)	3.86	C- N	-486 / 0	0.47 (1)
C- D	-2093 / 0	-91.8	-91.8	0.48 (1)	4.20	N- D	0 / 588	0.13 (1)
D- E	-1854 / 0	-91.8	-91.8	0.18 (1)	4.78	N- E	-223 / 0	0.27 (1)
E- F	-1854 / 0	-91.8	-91.8	0.18 (1)	4.78	E- L	-223 / 0	0.27 (1)
F- G	-2093 / 0	-91.8	-91.8	0.48 (1)	4.20	L- F	0 / 588	0.13 (1)
G- H	-2489 / 0	-91.8	-91.8	0.54 (1)	3.86	L- G	-486 / 0	0.47 (1)
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	K- G	-206 / 25	0.05 (1)
P- B	-1784 / 0	0.0	0.0	0.18 (1)	6.26	B- O	0 / 2274	0.51 (1)
J- H	-1784 / 0	0.0	0.0	0.18 (1)	6.26	K- H	0 / 2274	0.51 (1)
P- O	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
O- N	0 / 2251	-18.5	-18.5	0.45 (1)	10.00			
N- M	0 / 1956	-18.5	-18.5	0.40 (1)	10.00			
M- L	0 / 1956	-18.5	-18.5	0.40 (1)	10.00			
L- K	0 / 2251	-18.5	-18.5	0.45 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.45/1.00 (N-O:1) , WB=0.51/1.00 (B-O:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.67 (M) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080932

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T5	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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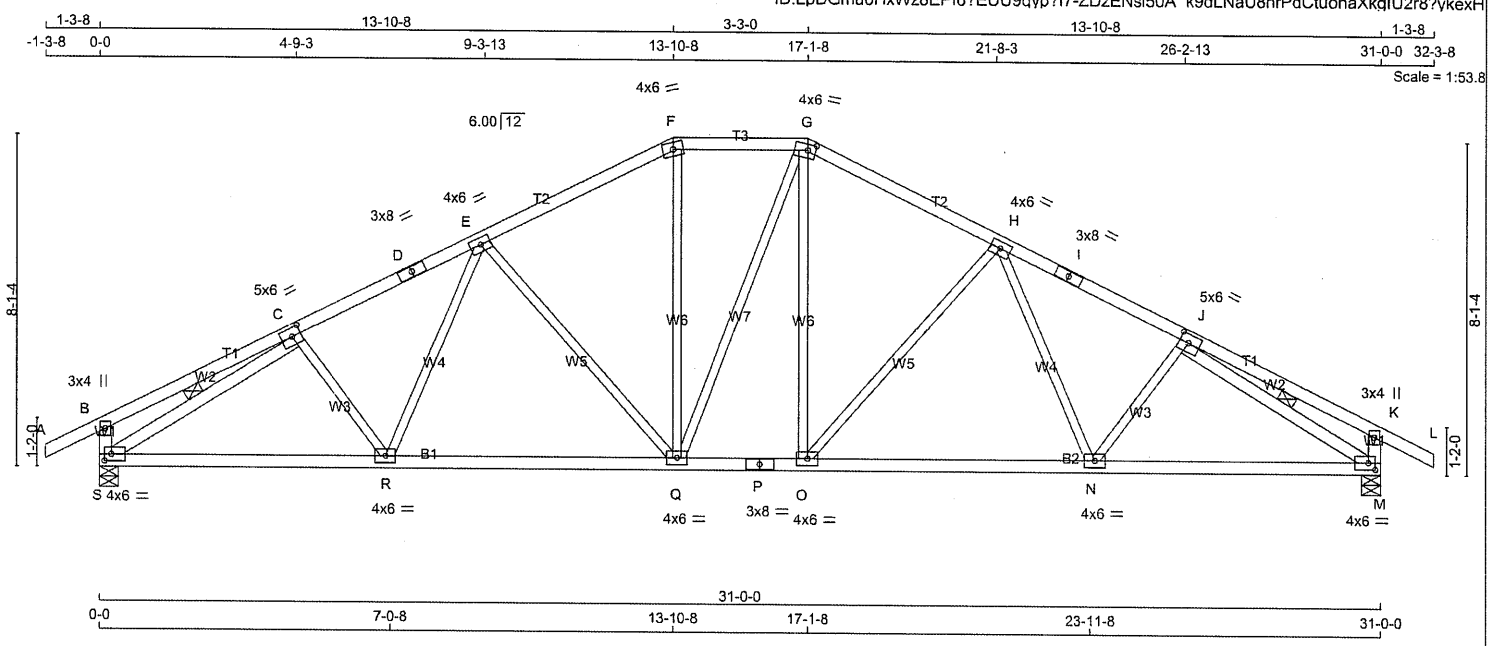

TOTAL WEIGHT = 137 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	4.0	6.0	1.75	2.25
H	TMWW-t	MT20	4.0	6.0		
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	6.0	2.00	2.00
N, O, R						
N	BMWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWWW-t	MT20	4.0	6.0		
S	BMVW1-t	MT20	4.0	6.0	2.00	2.00

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080933

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	1833	0	1833	0	0	5-8	2-0
M	1833	0	1833	0	0	5-8	2-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
M	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-S, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- R	-51 / 46	0.02 (4)
B- C	0 / 16	-91.8	-91.8	0.24 (1)	10.00	R- E	0 / 201	0.05 (4)
C- D	-2380 / 0	-91.8	-91.8	0.28 (1)	4.22	E- Q	-572 / 0	0.59 (1)
D- E	-2380 / 0	-91.8	-91.8	0.28 (1)	4.22	Q- F	0 / 520	0.12 (1)
E- F	-1896 / 0	-91.8	-91.8	0.27 (1)	4.64	Q- G	0 / 4	0.00 (1)
F- G	-1685 / 0	-91.8	-91.8	0.15 (1)	4.99	O- G	0 / 516	0.12 (1)
G- H	-1895 / 0	-91.8	-91.8	0.27 (1)	4.64	O- H	-574 / 0	0.59 (1)
H- I	-2381 / 0	-91.8	-91.8	0.28 (1)	4.22	H- N	0 / 204	0.05 (4)
I- J	-2381 / 0	-91.8	-91.8	0.28 (1)	4.22	N- J	-51 / 46	0.02 (4)
J- K	0 / 16	-91.8	-91.8	0.24 (1)	10.00	S- C	-2618 / 0	0.44 (1)
K- L	0 / 28	-91.8	-91.8	0.12 (1)	10.00	J- M	-2619 / 0	0.44 (1)
S- B	-302 / 0	0.0	0.0	0.03 (1)	7.81			
M- K	-302 / 0	0.0	0.0	0.03 (1)	7.81			
S- R	0 / 2161	-18.5	-18.5	0.46 (1)	10.00			
R- Q	0 / 2054	-18.5	-18.5	0.44 (1)	10.00			
Q- P	0 / 1684	-18.5	-18.5	0.34 (1)	10.00			
P- O	0 / 1684	-18.5	-18.5	0.34 (1)	10.00			
O- N	0 / 2054	-18.5	-18.5	0.44 (1)	10.00			
N- M	0 / 2161	-18.5	-18.5	0.46 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.28/1.00 (H-J:1) , BC=0.46/1.00 (M-N:1) , WB=0.59/0.90 (H-O:1) , SSI=0.18/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.59 (J) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T6	8	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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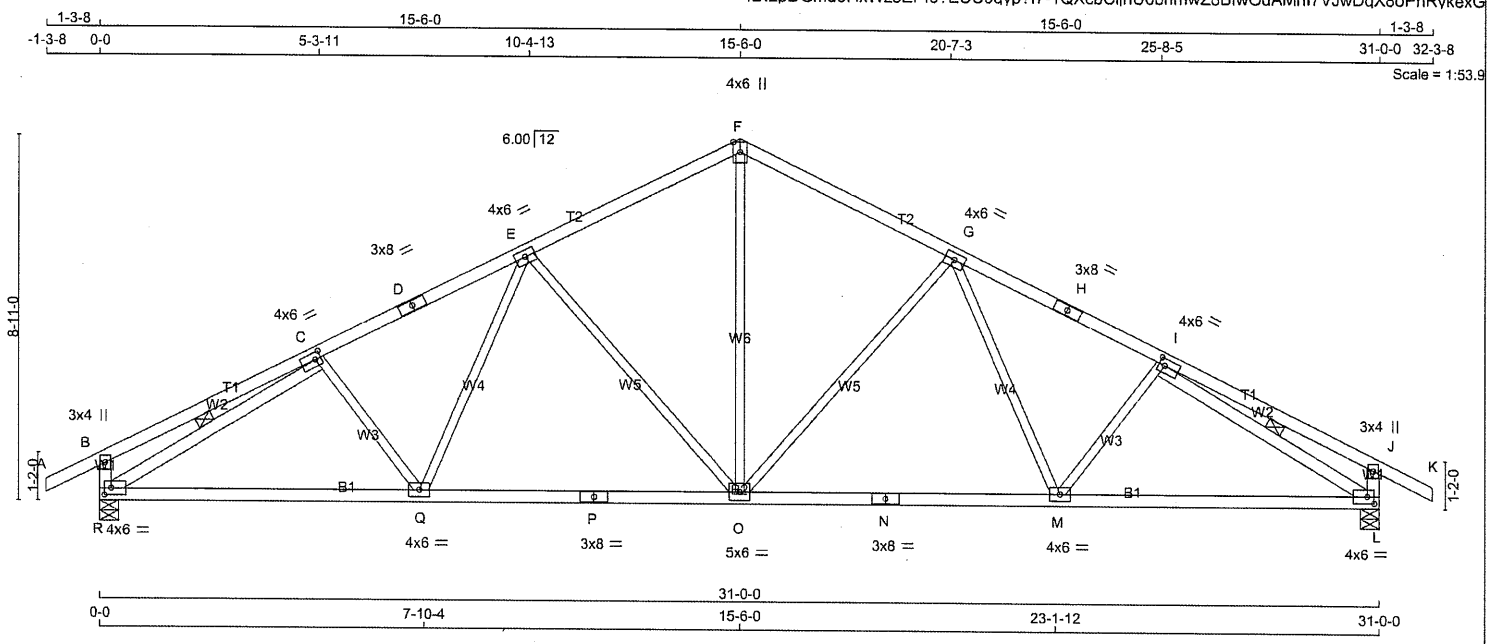

TOTAL WEIGHT = 8 X 129 = 1036 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	6.0	2.00	1.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	6.0	2.00	2.00
M	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BMVW1-t	MT20	4.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080934

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1833	0	1833	0	0	5-8	2-0
L	1833	0	1833	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
L	1294	863 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	1	0.12 (1)	10.00	O- F	0 / 1162	0.26 (1)
B- C	0 / 19	-91.8	-91.8	1	0.30 (1)	10.00	O- G	-677 / 0	0.89 (1)
C- D	-2350 / 0	-91.8	-91.8	1	0.34 (1)	4.19	G- M	0 / 291	0.07 (1)
D- E	-2350 / 0	-91.8	-91.8	1	0.34 (1)	4.19	M- I	-137 / 24	0.04 (1)
E- F	-1751 / 0	-91.8	-91.8	1	0.33 (1)	4.72	E- O	-677 / 0	0.89 (1)
F- G	-1751 / 0	-91.8	-91.8	1	0.33 (1)	4.72	Q- E	0 / 291	0.07 (1)
G- H	-2350 / 0	-91.8	-91.8	1	0.34 (1)	4.19	C- Q	-137 / 24	0.04 (1)
H- I	-2350 / 0	-91.8	-91.8	1	0.34 (1)	4.19	R- C	-2625 / 0	0.51 (1)
I- J	0 / 19	-91.8	-91.8	1	0.30 (1)	10.00	I- L	-2625 / 0	0.51 (1)
J- K	0 / 28	-91.8	-91.8	1	0.12 (1)	10.00			
R- B	-320 / 0	0.0	0.0	3	0.03 (1)	7.81			
L- J	-320 / 0	0.0	0.0	3	0.03 (1)	7.81			
R- Q	0 / 2185	-18.5	-18.5	1	0.49 (1)	10.00			
Q- P	0 / 1991	-18.5	-18.5	1	0.46 (1)	10.00			
P- O	0 / 1991	-18.5	-18.5	1	0.46 (1)	10.00			
O- N	0 / 1991	-18.5	-18.5	1	0.46 (1)	10.00			
N- M	0 / 1991	-18.5	-18.5	1	0.46 (1)	10.00			
M- L	0 / 2185	-18.5	-18.5	1	0.49 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.34/1.00 (G-I:1) , BC=0.49/1.00 (L-M:1) , WB=0.89/1.00 (G-O:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.68 (P) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T7	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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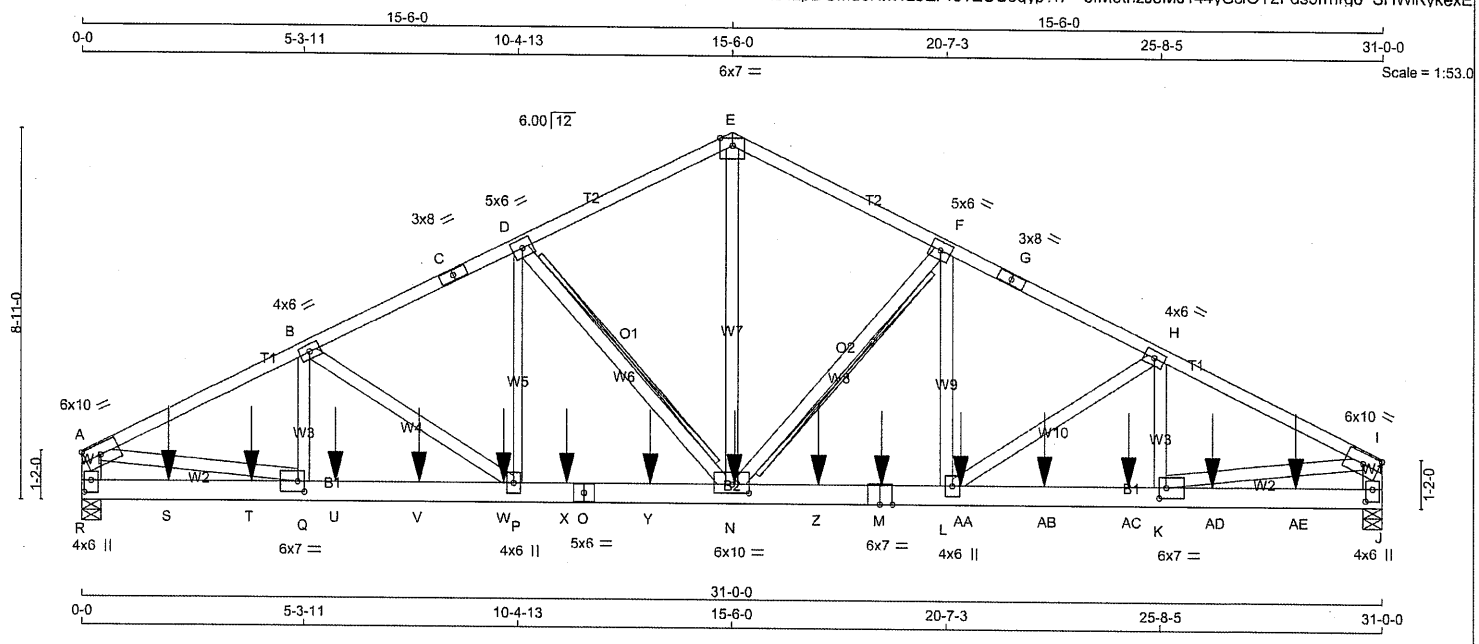


TOTAL WEIGHT = 3 X 166 = 498 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - A	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
R - O	2x6	DRY	2100F 1.8E	SPF
O - M	2x6	DRY	2100F 1.8E	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
P - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

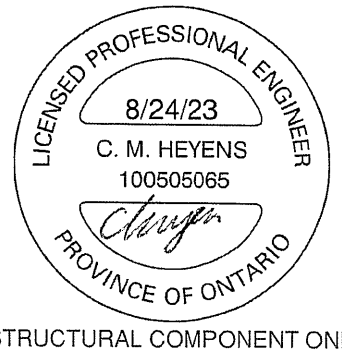
DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
G- I	1	12	TOP
R- A	2	12	TOP
J- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	12	SIDE(296.7)
O- M	2	11	SIDE(439.8)
M- J	2	11	SIDE(395.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



STRUCTURAL COMPONENT ONLY
DWG # TR23080935

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	10171	0	10171	0	0	5-8	3-8
J	10220	0	10220	0	0	5-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	7162	4874 / 0	0 / 0	0 / 0	0 / 0	2288 / 0	0 / 0
J	7196	4896 / 0	0 / 0	0 / 0	0 / 0	2300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J
BEARING SIZE FACTOR = 1.15 AT JNT(S) R, J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT F-N, D-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-15820 / 0	-91.8	-91.8	0.68 (1)	2.75
B- C	-14321 / 0	-91.8	-91.8	0.51 (1)	3.00
C- D	-14321 / 0	-91.8	-91.8	0.51 (1)	3.00
D- E	-11310 / 0	-91.8	-91.8	0.36 (1)	3.45
E- F	-11310 / 0	-91.8	-91.8	0.36 (1)	3.45
F- G	-14348 / 0	-91.8	-91.8	0.51 (1)	3.00
G- H	-14348 / 0	-91.8	-91.8	0.51 (1)	3.00
H- I	-15874 / 0	-91.8	-91.8	0.68 (1)	2.75
R- A	-9366 / 0	0.0	0.0	0.20 (1)	5.98
J- I	-9396 / 0	0.0	0.0	0.20 (1)	5.97
R- S	0 / 0	-18.5	-18.5	0.20 (1)	10.00
S- T	0 / 0	-18.5	-18.5	0.20 (1)	10.00
T- Q	0 / 0	-18.5	-18.5	0.20 (1)	10.00
Q- U	0 / 14160	-18.5	-18.5	0.36 (1)	10.00
U- V	0 / 14160	-18.5	-18.5	0.36 (1)	10.00
V- W	0 / 14160	-18.5	-18.5	0.36 (1)	10.00
W- P	0 / 14160	-18.5	-18.5	0.36 (1)	10.00
P- X	0 / 12814	-18.5	-18.5	0.37 (1)	10.00
X- O	0 / 12814	-18.5	-18.5	0.37 (1)	10.00
O- Y	0 / 12814	-18.5	-18.5	0.37 (1)	10.00
Y- N	0 / 12814	-18.5	-18.5	0.37 (1)	10.00
N- Z	0 / 12838	-18.5	-18.5	0.37 (1)	10.00
Z- M	0 / 12838	-18.5	-18.5	0.37 (1)	10.00
M- L	0 / 12838	-18.5	-18.5	0.37 (1)	10.00
L-AA	0 / 14208	-18.5	-18.5	0.36 (1)	10.00
AA-AB	0 / 14208	-18.5	-18.5	0.36 (1)	10.00
AB-AC	0 / 14208	-18.5	-18.5	0.36 (1)	10.00
AC- K	0 / 14208	-18.5	-18.5	0.36 (1)	10.00
K-AD	0 / 0	-18.5	-18.5	0.20 (1)	10.00
AD-AE	0 / 0	-18.5	-18.5	0.20 (1)	10.00
AE- J	0 / 0	-18.5	-18.5	0.20 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- E	0 / 9709	0.52 (1)
N- F	-4218 / 0	0.32 (1)
L- F	0 / 4062	0.22 (1)
L- H	-1651 / 0	0.25 (1)
K- H	0 / 1115	0.06 (1)
D- N	-4182 / 0	0.31 (1)
P- D	0 / 4020	0.30 (1)
B- P	-1621 / 0	0.25 (1)
Q- B	0 / 1088	0.06 (1)
A- Q	0 / 14310	0.77 (1)
K- I	0 / 14358	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 982 (0.38")

CSI: TC=0.68/1.00 (H-I:1) , BC=0.37/1.00 (L-N:1) , WB=0.77/1.00 (I-K:1) , SSI=0.38/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.92 (K) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T7	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	10.0		Edge
B	TMWW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	8.0		
D	TMWW-t	MT20	5.0	6.0		
E	TTW-p	MT20	6.0	7.0	Edge	
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	6.0	10.0		Edge
J	BMV1+p	MT20	4.0	6.0	3.50	2.00
K	BMWW-t	MT20	6.0	7.0	3.00	2.00
L	BMWW+t	MT20	4.0	6.0		
M	BS-t	MT20	6.0	7.0		
N	BMWWW-t	MT20	6.0	10.0	2.50	5.00
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	6.0	7.0	3.00	2.00
R	BMV1+p	MT20	4.0	6.0	3.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	19-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
N	15-6-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
S	2-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
T	4-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
U	6-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
V	8-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
W	10-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
X	11-6-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
Y	13-6-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
Z	17-6-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
AA	20-11-4	-807	-807	---	FRONT	VERT	TOTAL	---	C1
AB	22-11-4	-807	-807	---	FRONT	VERT	TOTAL	---	C1
AC	24-11-4	-807	-807	---	FRONT	VERT	TOTAL	---	C1
AD	26-11-4	-807	-807	---	FRONT	VERT	TOTAL	---	C1
AE	28-11-4	-807	-807	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080935

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T7Z	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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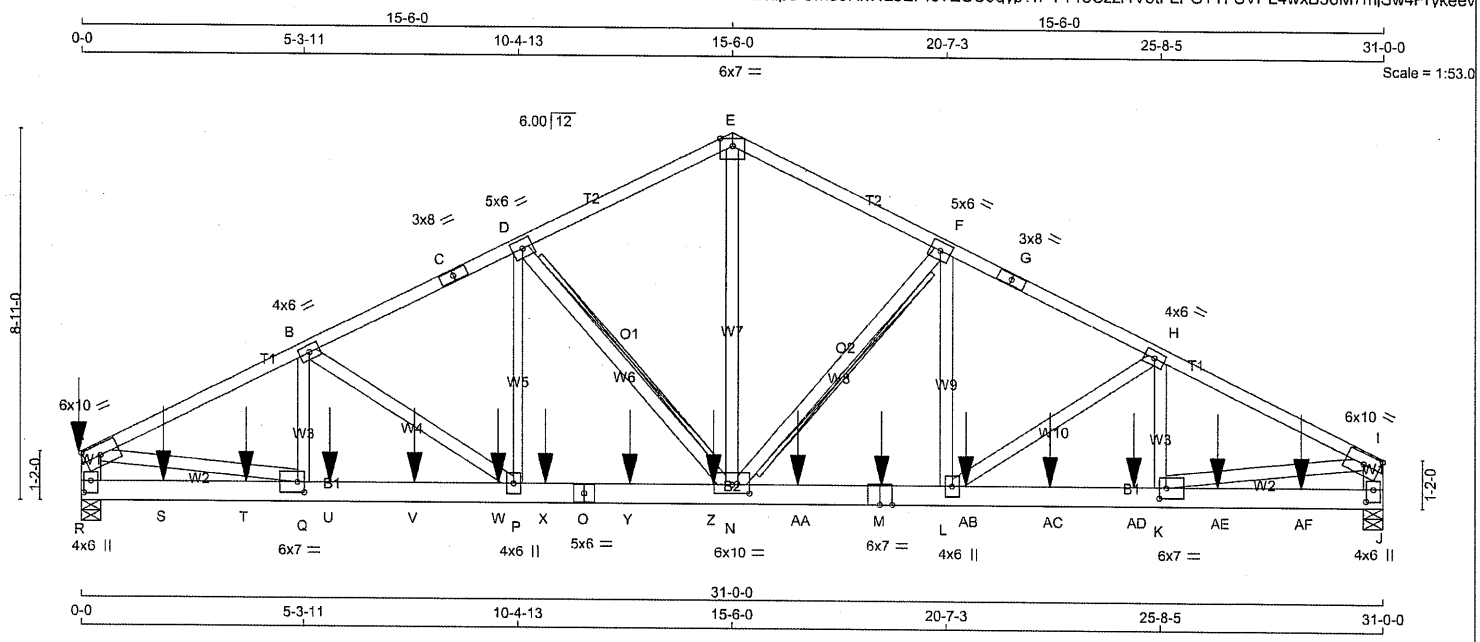


TOTAL WEIGHT = 3 X 166 = 498 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - A	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
R - O	2x6	DRY	2100F 1.8E	SPF
O - M	2x6	DRY	2100F 1.8E	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
P - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(91.5)
C- E	1	12	TOP
E- G	1	12	TOP
G- I	1	12	TOP
R- A	2	12	TOP
J- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	11	SIDE(553.2)
O- M	2	11	SIDE(439.8)
M- J	2	9	SIDE(426.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY
DWG # TR23080957

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	9640	0	9640	0	0	5-8	3-0
J	10496	0	10496	0	0	5-8	3-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	6790	4610 / 0	0 / 0	0 / 0	0 / 0	2180 / 0	0 / 0
J	7395	5012 / 0	0 / 0	0 / 0	0 / 0	2382 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J
BEARING SIZE FACTOR = 1.15 AT JNT(S) R, J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT F-N, D-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-15210 / 0	-91.8	-91.8	0.64 (1)	2.84
B- C	-13960 / 0	-91.8	-91.8	0.49 (1)	3.05
C- D	-13960 / 0	-91.8	-91.8	0.49 (1)	3.05
D- E	-10996 / 0	-91.8	-91.8	0.35 (1)	3.50
E- F	-10996 / 0	-91.8	-91.8	0.35 (1)	3.50
F- G	-13956 / 0	-91.8	-91.8	0.49 (1)	3.05
G- H	-13956 / 0	-91.8	-91.8	0.49 (1)	3.05
H- I	-15801 / 0	-91.8	-91.8	0.68 (1)	2.76
R- A	-9193 / 0	0.0	0.0	0.20 (1)	6.03
J- I	-9354 / 0	0.0	0.0	0.20 (1)	5.98
R- S	0 / 0	-18.5	-18.5	0.13 (1)	10.00
S- T	0 / 0	-18.5	-18.5	0.13 (1)	10.00
T- Q	0 / 0	-18.5	-18.5	0.13 (1)	10.00
Q- U	0 / 13614	-18.5	-18.5	0.34 (1)	10.00
U- V	0 / 13614	-18.5	-18.5	0.34 (1)	10.00
V- W	0 / 13614	-18.5	-18.5	0.34 (1)	10.00
W- P	0 / 13614	-18.5	-18.5	0.34 (1)	10.00
P- X	0 / 12491	-18.5	-18.5	0.37 (1)	10.00
X- O	0 / 12491	-18.5	-18.5	0.37 (1)	10.00
O- Y	0 / 12491	-18.5	-18.5	0.37 (1)	10.00
Y- Z	0 / 12491	-18.5	-18.5	0.37 (1)	10.00
Z- N	0 / 12491	-18.5	-18.5	0.37 (1)	10.00
N-AA	0 / 12487	-18.5	-18.5	0.37 (1)	10.00
AA- M	0 / 12487	-18.5	-18.5	0.37 (1)	10.00
M- L	0 / 12487	-18.5	-18.5	0.37 (1)	10.00
L-AB	0 / 14143	-18.5	-18.5	0.38 (1)	10.00
AB-AC	0 / 14143	-18.5	-18.5	0.38 (1)	10.00
AC-AD	0 / 14143	-18.5	-18.5	0.38 (1)	10.00
AD- K	0 / 14143	-18.5	-18.5	0.38 (1)	10.00
K-AE	0 / 0	-18.5	-18.5	0.26 (1)	10.00
AE-AF	0 / 0	-18.5	-18.5	0.26 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- E	0 / 9429	0.51 (1)
N- F	-4110 / 0	0.31 (1)
L- F	0 / 3942	0.21 (1)
L- H	-1995 / 0	0.30 (1)
K- H	0 / 1451	0.08 (1)
D- N	-4115 / 0	0.31 (1)
P- D	0 / 3949	0.30 (1)
B- P	-1354 / 0	0.21 (1)
Q- B	0 / 826	0.04 (1)
A- Q	0 / 13759	0.74 (1)
K- I	0 / 14293	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.68/1.00 (H-I:1) , BC=0.38/1.00 (K-L:1) , WB=0.77/1.00 (I-K:1) , SSI=0.38/1.00 (N-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.92 (K) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T7Z	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-7T1oCzzl4V8tFLFOTTFSVPL4wxB36M7mjSw4Prykeev

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	10.0		Edge
B	TMWW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	8.0		
D	TMWW-t	MT20	5.0	6.0		
E	TTW-p	MT20	6.0	7.0	Edge	
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	6.0	10.0		Edge
J	BMV1+p	MT20	4.0	6.0	3.50	2.00
K	BMWW-t	MT20	6.0	7.0	3.00	2.00
L	BMWW+t	MT20	4.0	6.0		
M	BS-t	MT20	6.0	7.0		
N	BMWWW-t	MT20	6.0	10.0	2.50	5.00
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	6.0	7.0	3.00	2.00
R	BMV1+p	MT20	4.0	6.0	3.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

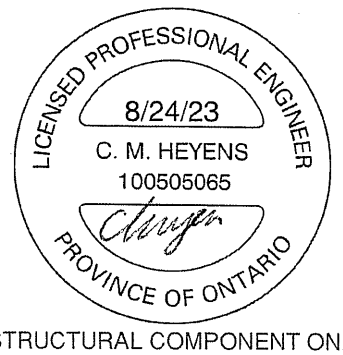
CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	FACTORED FORCE (LBS)	MAX CSI (LC)
AF- J	0 / 0		-18.5	-18.5	0.26 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-124	-124	---	FRONT	VERT	TOTAL	---	C1
M	19-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
S	1-11-4	-180	-180	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-1021	-1021	---	FRONT	VERT	TOTAL	---	C1
U	5-11-4	-703	-703	---	FRONT	VERT	TOTAL	---	C1
V	7-11-4	-703	-703	---	FRONT	VERT	TOTAL	---	C1
W	9-11-4	-703	-703	---	FRONT	VERT	TOTAL	---	C1
X	11-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
Y	13-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
Z	15-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
AA	17-0-12	-791	-791	---	FRONT	VERT	TOTAL	---	C1
AB	21-0-12	-807	-807	---	FRONT	VERT	TOTAL	---	C1
AC	23-0-12	-807	-807	---	FRONT	VERT	TOTAL	---	C1
AD	25-0-12	-807	-807	---	FRONT	VERT	TOTAL	---	C1
AE	27-0-12	-807	-807	---	FRONT	VERT	TOTAL	---	C1
AF	29-0-12	-1152	-1152	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR23080957

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T7Z1	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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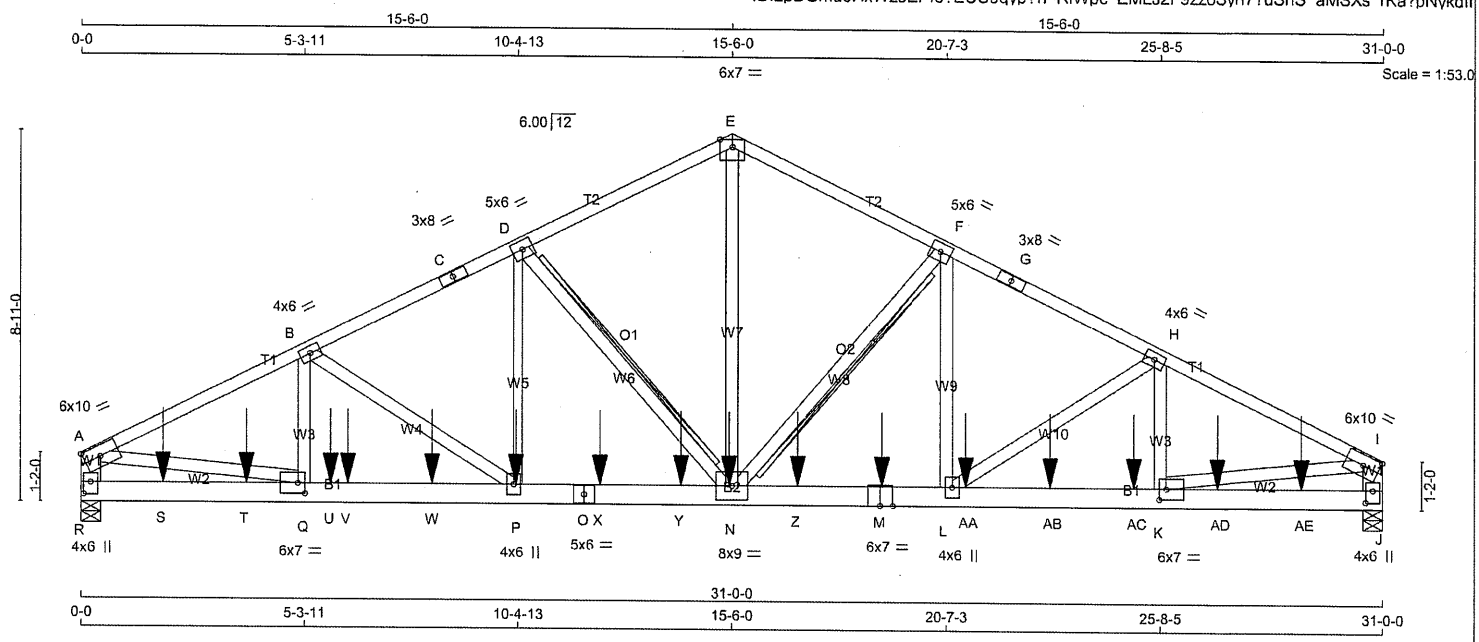

TOTAL WEIGHT = 3 X 166 = 498 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - A	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
R - O	2x6	DRY	2100F 1.8E	SPF
O - M	2x6	DRY	2100F 1.8E	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
P - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
G- I	1	12	TOP
R- A	2	12	TOP
J- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	6	SIDE(692.3)
O- M	2	11	SIDE(441.5)
M- J	2	9	SIDE(471.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

STRUCTURAL COMPONENT ONLY
DWG # TR23080986

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	9445	0	9445	0	0	5-8	2-15
J	9962	0	9962	0	0	5-8	3-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	6660	4477 / 0	0 / 0	0 / 0	0 / 0	2183 / 0	0 / 0
J	7025	4723 / 0	0 / 0	0 / 0	0 / 0	2302 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J
BEARING SIZE FACTOR = 1.15 AT JNT(S) R, J (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT F-N, D-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO
A- B	-15829 / 0	-91.8	-91.8	0.68 (1)	2.75
B- C	-14626 / 0	-91.8	-91.8	0.53 (1)	2.95
C- D	-14626 / 0	-91.8	-91.8	0.53 (1)	2.95
D- E	-11478 / 0	-91.8	-91.8	0.37 (1)	3.42
E- F	-11478 / 0	-91.8	-91.8	0.37 (1)	3.42
F- G	-14220 / 0	-91.8	-91.8	0.50 (1)	3.02
G- H	-14220 / 0	-91.8	-91.8	0.50 (1)	3.02
H- I	-15898 / 0	-91.8	-91.8	0.68 (1)	2.75
R- A	-9371 / 0	0.0	0.0	0.20 (1)	5.98
J- I	-9410 / 0	0.0	0.0	0.20 (1)	5.97
R- S	0 / 0	-18.5	-18.5	0.07 (1)	10.00
S- T	0 / 0	-18.5	-18.5	0.07 (1)	10.00
T- Q	0 / 0	-18.5	-18.5	0.07 (1)	10.00
Q- U	0 / 14168	-18.5	-18.5	0.45 (1)	10.00
U- V	0 / 14168	-18.5	-18.5	0.45 (1)	10.00
V- W	0 / 14168	-18.5	-18.5	0.45 (1)	10.00
W- P	0 / 14168	-18.5	-18.5	0.45 (1)	10.00
P- O	0 / 13087	-18.5	-18.5	0.37 (1)	10.00
O- X	0 / 13087	-18.5	-18.5	0.37 (1)	10.00
X- Y	0 / 13087	-18.5	-18.5	0.37 (1)	10.00
Y- N	0 / 13087	-18.5	-18.5	0.37 (1)	10.00
N- Z	0 / 12723	-18.5	-18.5	0.36 (1)	10.00
Z- M	0 / 12723	-18.5	-18.5	0.36 (1)	10.00
M- L	0 / 12723	-18.5	-18.5	0.36 (1)	10.00
L-AA	0 / 14229	-18.5	-18.5	0.35 (1)	10.00
AA-AB	0 / 14229	-18.5	-18.5	0.35 (1)	10.00
AB-AC	0 / 14229	-18.5	-18.5	0.35 (1)	10.00
AC- K	0 / 14229	-18.5	-18.5	0.35 (1)	10.00
K-AD	0 / 0	-18.5	-18.5	0.16 (1)	10.00
AD-AE	0 / 0	-18.5	-18.5	0.16 (1)	10.00
AE- J	0 / 0	-18.5	-18.5	0.16 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- E	0 / 9859	0.53 (1)
N- F	-3809 / 0	0.29 (1)
L- F	0 / 3617	0.19 (1)
L- H	-1815 / 0	0.27 (1)
K- H	0 / 1275	0.07 (1)
D- N	-4369 / 0	0.33 (1)
P- D	0 / 4224	0.32 (1)
B- P	-1303 / 0	0.20 (1)
Q- B	0 / 778	0.04 (1)
A- Q	0 / 14318	0.77 (1)
K- I	0 / 14380	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 988 (0.38")

CSI: TC=0.68/1.00 (H-I:1) , BC=0.45/1.00 (P-Q:1) , WB=0.77/1.00 (I-K:1) , SSI=0.50/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.92 (K) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T7Z1	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	10.0		Edge
B	TMWW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	8.0		
D	TMWW-t	MT20	5.0	6.0		
E	TTW-p	MT20	6.0	7.0	Edge	
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	6.0	10.0		Edge
J	BMV1+p	MT20	4.0	6.0	3.50	2.00
K	BMWW-t	MT20	6.0	7.0	3.00	2.00
L	BMWW+t	MT20	4.0	6.0		
M	BS-t	MT20	6.0	7.0		
N	BMWWW-t	MT20	8.0	9.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	6.0	7.0	3.00	2.00
R	BMV1+p	MT20	4.0	6.0	3.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	19-0-12	-719	-719	—	FRONT	VERT	TOTAL	---	C1
N	15-5-4	-719	-719	---	FRONT	VERT	TOTAL	---	C1
P	10-4-4	-781	-781	---	FRONT	VERT	TOTAL	---	C1
S	1-11-4	-88	-88	---	FRONT	VERT	TOTAL	---	C1
T	3-11-4	-151	-151	---	FRONT	VERT	TOTAL	---	C1
U	5-11-4	-214	-214	---	FRONT	VERT	TOTAL	---	C1
V	6-4-4	-1642	-1642	---	FRONT	VERT	TOTAL	---	C1
W	8-4-4	-781	-781	---	FRONT	VERT	TOTAL	---	C1
X	12-4-4	-781	-781	---	FRONT	VERT	TOTAL	---	C1
Y	14-3-8	-914	-914	---	FRONT	VERT	TOTAL	---	C1
Z	17-0-12	-719	-719	---	FRONT	VERT	TOTAL	---	C1
AA	21-0-12	-719	-719	---	FRONT	VERT	TOTAL	---	C1
AB	23-0-12	-719	-719	---	FRONT	VERT	TOTAL	---	C1
AC	25-0-12	-805	-805	---	FRONT	VERT	TOTAL	---	C1
AD	27-0-12	-1276	-1276	---	FRONT	VERT	TOTAL	---	C1
AE	29-0-12	-239	-239	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080986

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T8	6	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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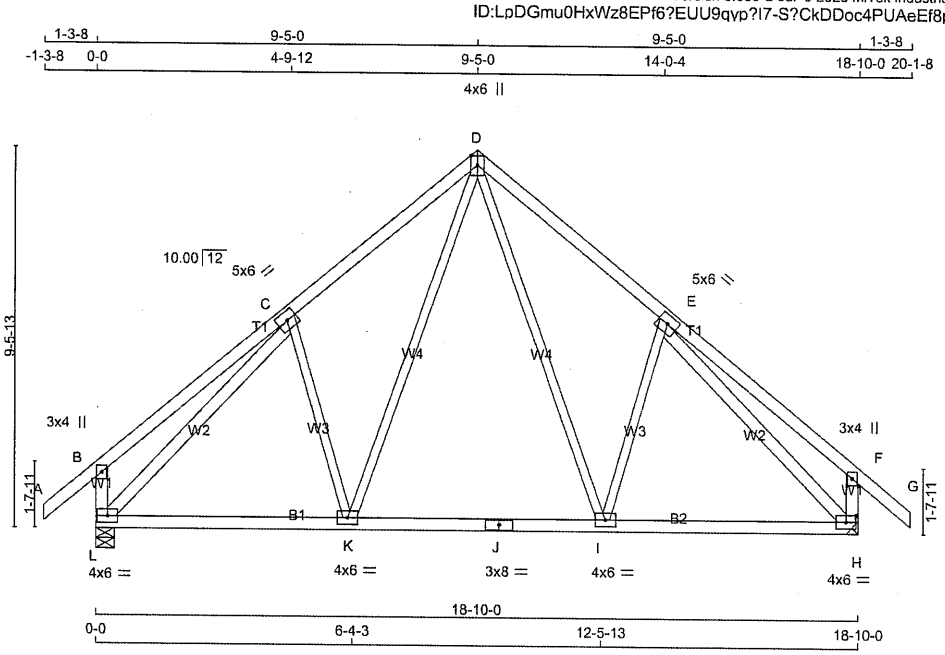

TOTAL WEIGHT = 6 X 95 = 573 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1165	0	1165	0	0	IN-SX	IN-SX
H	1165	0	1165	0	0	5-8 MECHANICAL	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	822	553 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
H	822	553 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00
B- C	0 / 33	-91.8	-91.8	0.34 (1)	10.00
C- D	-895 / 0	-91.8	-91.8	0.27 (1)	6.18
D- E	-895 / 0	-91.8	-91.8	0.27 (1)	6.18
E- F	0 / 33	-91.8	-91.8	0.34 (1)	10.00
F- G	0 / 41	-91.8	-91.8	0.13 (1)	10.00
L- B	-288 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-288 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 745	-18.5	-18.5	0.23 (4)	10.00
K- J	0 / 532	-18.5	-18.5	0.21 (4)	10.00
J- I	0 / 532	-18.5	-18.5	0.21 (4)	10.00
I- H	0 / 745	-18.5	-18.5	0.23 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 405	0.09 (1)
I- E	-270 / 0	0.14 (1)
K- D	0 / 405	0.09 (1)
C- K	-270 / 0	0.14 (1)
L- C	-1115 / 0	0.72 (1)
E- H	-1115 / 0	0.72 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.23/1.00 (K-L:4) , WB=0.72/1.00 (E-H:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (C) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080936

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T8A	10	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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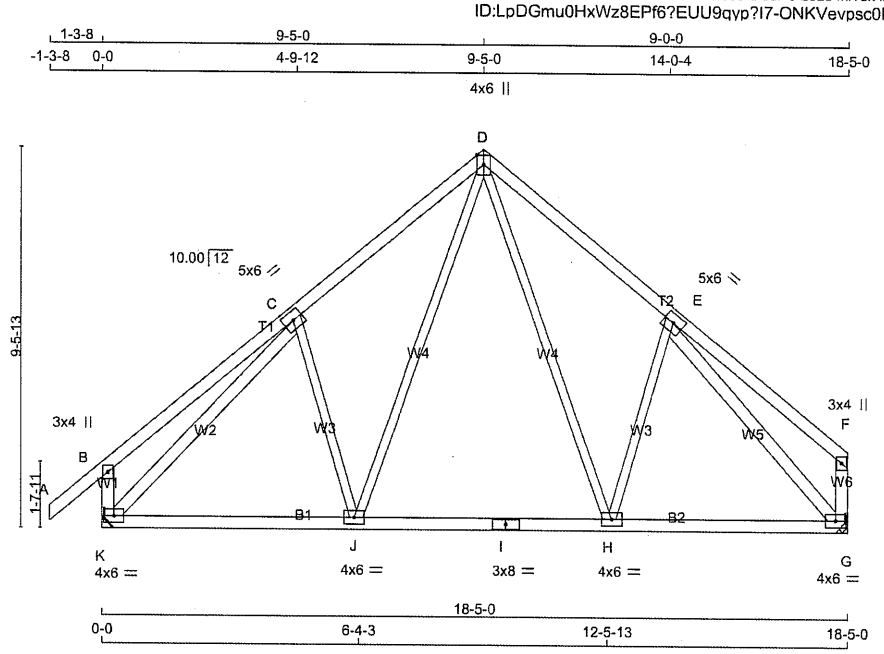

TOTAL WEIGHT = 10 X 93 = 925 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1142	0	1142	0	0	MECHANICAL	
G	1015	0	1015	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, G. MINIMUM BEARING LENGTH AT JOINT K = 1-8, JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	805	542 / 0	0 / 0	0 / 0	0 / 0	263 / 0	0 / 0
G	718	471 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	
B- C	0 / 33	-91.8	-91.8	0.34 (1)	10.00	
C- D	-866 / 0	-91.8	-91.8	0.27 (1)	6.25	
D- E	-831 / 0	-91.8	-91.8	0.26 (1)	6.25	
E- F	0 / 34	-91.8	-91.8	0.31 (1)	10.00	
K- B	-288 / 0	0.0	0.0	0.03 (1)	7.81	
G- F	-141 / 0	0.0	0.0	0.02 (1)	7.81	
K- J	0 / 725	-18.5	-18.5	0.23 (4)	10.00	
J- I	0 / 508	-18.5	-18.5	0.21 (4)	10.00	
I- H	0 / 508	-18.5	-18.5	0.21 (4)	10.00	
H- G	0 / 678	-18.5	-18.5	0.21 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- H	0 / 334	0.08 (1)
H- E	-205 / 0	0.11 (1)
J- D	0 / 412	0.09 (1)
C- J	-275 / 0	0.14 (1)
K- C	-1085 / 0	0.70 (1)
E- G	-1073 / 0	0.62 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.23/1.00 (J-K:4) , WB=0.70/1.00 (C-K:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (E) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080937

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T8AG	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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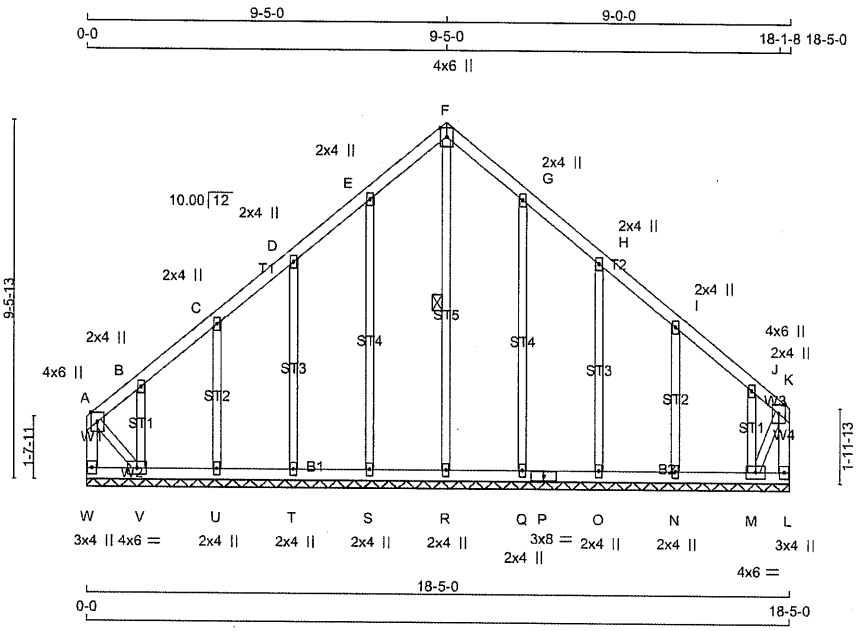

TOTAL WEIGHT = 93 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
W - A	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
W - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B, C, D, E, G, H, I, J						
B	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
K	TMVW+p	MT20	4.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	6.0		
N, O, Q, R, S, T, U						
N	BMW1+w	MT20	2.0	4.0		
P	BS-t	MT20	3.0	8.0		
V	BMWW1-t	MT20	4.0	6.0		
W	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- A	-104 / 0	0.0	0.0	0.01 (1)	7.81	R- F	-123 / 0	0.07 (1)
A- B	-34 / 0	-91.8	-91.8	0.03 (1)	6.25	S- E	-209 / 0	0.24 (1)
B- C	-36 / 0	-91.8	-91.8	0.04 (1)	6.25	T- D	-175 / 0	0.10 (1)
C- D	-34 / 0	-91.8	-91.8	0.04 (1)	6.25	U- C	-185 / 0	0.05 (1)
D- E	-26 / 0	-91.8	-91.8	0.05 (1)	6.25	V- B	-164 / 0	0.03 (1)
E- F	-37 / 0	-91.8	-91.8	0.05 (1)	6.25	Q- G	-209 / 0	0.24 (1)
F- G	-37 / 0	-91.8	-91.8	0.05 (1)	6.25	O- H	-175 / 0	0.10 (1)
G- H	-26 / 0	-91.8	-91.8	0.05 (1)	6.25	N- I	-187 / 0	0.06 (1)
H- I	-34 / 0	-91.8	-91.8	0.04 (1)	6.25	M- J	-149 / 0	0.03 (1)
I- J	-35 / 0	-91.8	-91.8	0.04 (1)	6.25	A- V	0 / 49	0.01 (1)
J- K	-30 / 0	-91.8	-91.8	0.03 (1)	6.25	M- K	0 / 68	0.02 (1)
L- K	-104 / 0	0.0	0.0	0.01 (1)	7.81			
W- V	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
V- U	0 / 29	-18.5	-18.5	0.02 (4)	10.00			
U- T	0 / 25	-18.5	-18.5	0.02 (4)	10.00			
T- S	0 / 22	-18.5	-18.5	0.02 (4)	10.00			
S- R	0 / 20	-18.5	-18.5	0.02 (4)	10.00			
R- Q	0 / 20	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 22	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 22	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 25	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 29	-18.5	-18.5	0.02 (4)	10.00			
M- L	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (F-G:1) , BC=0.02/1.00 (M-N:4) , WB=0.24/1.00 (G-Q:1) , SSI=0.07/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (M) (INPUT = 0.90)
JSI METAL= 0.11 (G) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080938

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T8G	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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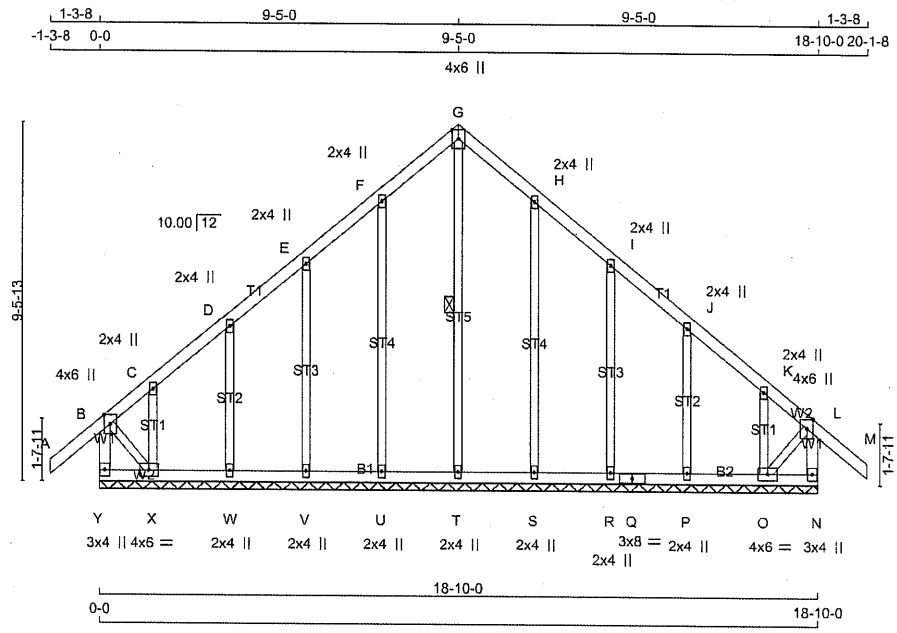

TOTAL WEIGHT = 97 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	6.0	Edge	
L	TMVW+p	MT20	4.0	6.0	Edge	
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P, R, S, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
X	BMWW1-t	MT20	4.0	6.0		
Y	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- B	-292 / 0	0.0	0.0	0.03 (1)	7.81	T- G	-133 / 0	0.08 (1)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	U- F	-209 / 0	0.24 (1)
B- C	-72 / 0	-91.8	-91.8	0.12 (1)	6.25	V- E	-173 / 0	0.10 (1)
C- D	-22 / 0	-91.8	-91.8	0.05 (1)	6.25	W- D	-196 / 0	0.06 (1)
D- E	-26 / 0	-91.8	-91.8	0.05 (1)	6.25	X- C	-79 / 0	0.01 (1)
E- F	-17 / 0	-91.8	-91.8	0.05 (1)	6.25	S- H	-209 / 0	0.24 (1)
F- G	-29 / 0	-91.8	-91.8	0.05 (1)	6.25	R- I	-173 / 0	0.10 (1)
G- H	-29 / 0	-91.8	-91.8	0.05 (1)	6.25	P- J	-196 / 0	0.06 (1)
H- I	-17 / 0	-91.8	-91.8	0.05 (1)	6.25	O- K	-79 / 0	0.01 (1)
I- J	-26 / 0	-91.8	-91.8	0.05 (1)	6.25	B- X	0 / 35	0.01 (1)
J- K	-22 / 0	-91.8	-91.8	0.05 (1)	6.25	O- L	0 / 35	0.01 (1)
K- L	-72 / 0	-91.8	-91.8	0.12 (1)	6.25			
L- M	0 / 41	-91.8	-91.8	0.13 (1)	10.00			
N- L	-292 / 0	0.0	0.0	0.03 (1)	7.81			
Y- X	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
X- W	0 / 23	-18.5	-18.5	0.02 (4)	10.00			
W- V	0 / 19	-18.5	-18.5	0.02 (4)	10.00			
V- U	0 / 16	-18.5	-18.5	0.02 (4)	10.00			
U- T	0 / 13	-18.5	-18.5	0.02 (4)	10.00			
T- S	0 / 13	-18.5	-18.5	0.02 (4)	10.00			
S- R	0 / 16	-18.5	-18.5	0.02 (4)	10.00			
R- Q	0 / 19	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 19	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 23	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1) , BC=0.02/1.00 (W-X:4) , WB=0.24/1.00 (F-U:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (D) (INPUT = 0.90)
JSI METAL= 0.11 (H) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080939

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T9	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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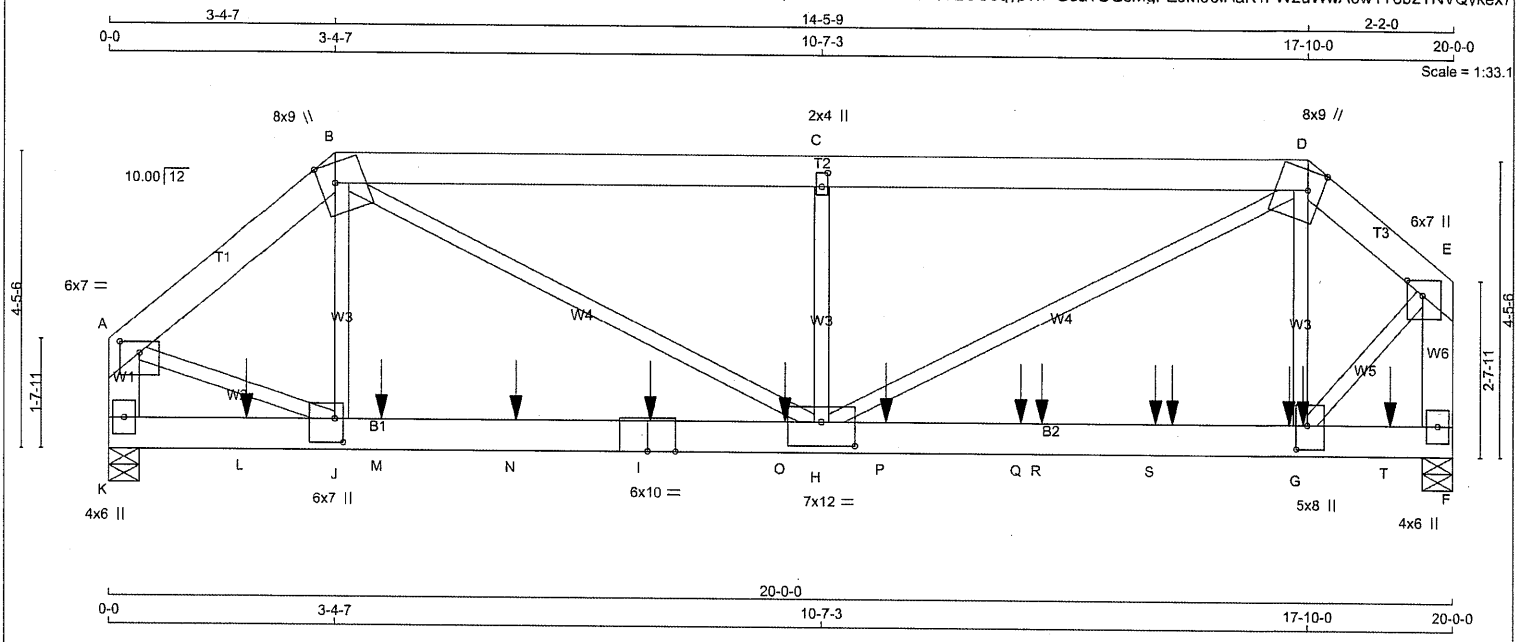

TOTAL WEIGHT = 2 X 107 = 214 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - I	2x6	DRY	2100F 1.8E	SPF
I - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	2	12	TOP
B- D	2	12	TOP
D- E	2	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- I	2	12	SIDE(0.0)
I- F	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
G- D	1	6	SIDE(62.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080940

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	5859	0	5859	0	0	5-8	3-3
F	6924	0	6924	0	0	5-8	3-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	4134	2765 / 0	0 / 0	0 / 0	0 / 0	1369 / 0	0 / 0
F	4883	3282 / 0	0 / 0	0 / 0	0 / 0	1601 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-6460 / 0	-91.8	-91.8	0.10 (1)	4.63	J- B	0 / 1485	0.18 (1)
B- C	-8984 / 0	-91.8	-91.8	0.33 (1)	3.83	H- B	0 / 4567	0.57 (1)
C- D	-8984 / 0	-91.8	-91.8	0.33 (1)	3.83	H- C	-656 / 0	0.09 (1)
D- E	-5253 / 0	-91.8	-91.8	0.05 (1)	5.08	H- D	0 / 5668	0.70 (1)
K- A	-5900 / 0	0.0	0.0	0.21 (1)	6.07	G- D	0 / 247	0.03 (1)
F- E	-7226 / 0	0.0	0.0	0.32 (1)	5.57	A- J	0 / 5184	0.64 (1)
						G- E	0 / 5469	0.68 (1)
K- L	0 / 0	-18.5	-18.5	0.33 (1)	10.00			
L- J	0 / 0	-18.5	-18.5	0.33 (1)	10.00			
J- M	0 / 4977	-18.5	-18.5	0.59 (1)	10.00			
M- N	0 / 4977	-18.5	-18.5	0.59 (1)	10.00			
N- I	0 / 4977	-18.5	-18.5	0.59 (1)	10.00			
I- O	0 / 4977	-18.5	-18.5	0.59 (1)	10.00			
O- H	0 / 4977	-18.5	-18.5	0.59 (1)	10.00			
H- P	0 / 4010	-18.5	-18.5	0.56 (1)	10.00			
P- Q	0 / 4010	-18.5	-18.5	0.56 (1)	10.00			
Q- R	0 / 4010	-18.5	-18.5	0.56 (1)	10.00			
R- S	0 / 4010	-18.5	-18.5	0.56 (1)	10.00			
S- G	0 / 4010	-18.5	-18.5	0.56 (1)	10.00			
G- T	0 / 0	-18.5	-18.5	0.41 (1)	10.00			
T- F	0 / 0	-18.5	-18.5	0.41 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	17-6-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
G	17-9-4	-70	-70	---	FRONT	VERT	TOTAL	---	C1
I	8-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
L	2-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
M	4-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
N	6-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
O	10-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
P	11-6-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
Q	13-6-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
R	13-10-8	-286	-286	---	FRONT	VERT	TOTAL	---	C1
S	15-6-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
S	15-9-12	-70	-70	---	FRONT	VERT	TOTAL	---	C1
T	19-0-12	-704	-704	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 912 (0.26")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.59/1.00 (H-J:1) , WB=0.70/1.00 (D-H:1) , SSI=0.72/1.00 (H-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.78 (I) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434362	T9	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	7.0	2.00	3.50
B	TTWW+m	MT20	8.0	9.0	Edge	2.75
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TTWW+m	MT20	8.0	9.0	3.50	2.50
E	TMVW+p	MT20	6.0	7.0	2.75	2.75
F	BMV1+p	MT20	4.0	6.0		
G	BMWW+t	MT20	5.0	8.0	4.25	2.00
H	BMWWW-t	MT20	7.0	12.0	4.25	6.00
I	BS-t	MT20	6.0	10.0		
J	BMWW+t	MT20	6.0	7.0	4.25	1.50
K	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

STRUCTURAL COMPONENT ONLY
DWG # TR23080940

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T10	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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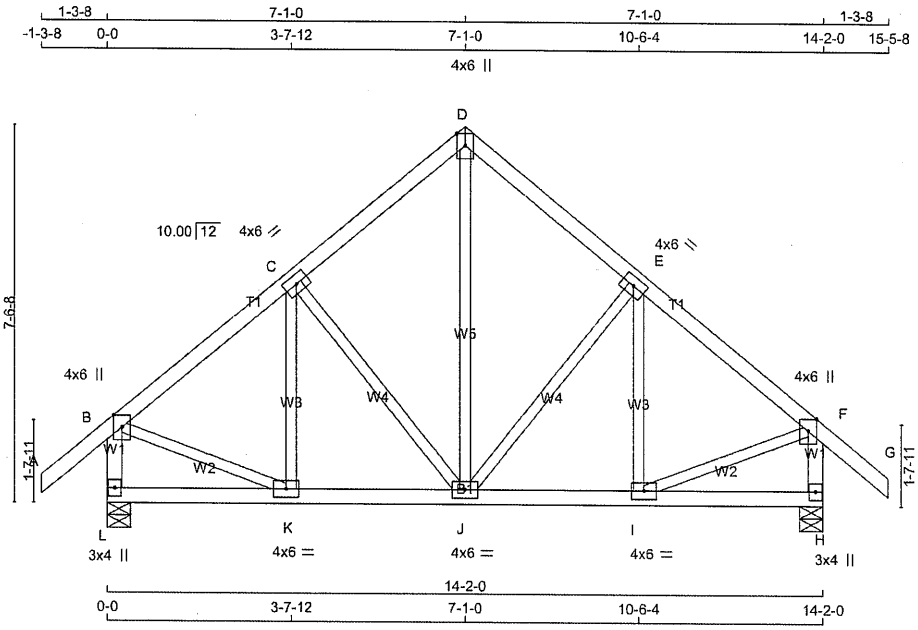

TOTAL WEIGHT = 2 X 71 = 141 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1320	0	1320	0	0	5-8	1-8
H	1320	0	1320	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	969	433 / 0	0 / 0	0 / 0	0 / 0	536 / 0	0 / 0
H	969	433 / 0	0 / 0	0 / 0	0 / 0	536 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.15 (1)	10.00	J- D	0 / 830	0.25 (4)	
B- C	-1057 / 0	-91.8	-91.8	0.23 (1)	5.80	J- E	-302 / 0	0.16 (1)	
C- D	-860 / 0	-91.8	-91.8	0.22 (1)	6.25	I- E	-5 / 143	0.05 (4)	
D- E	-860 / 0	-91.8	-91.8	0.22 (1)	6.25	C- J	-302 / 0	0.16 (1)	
E- F	-1057 / 0	-91.8	-91.8	0.23 (1)	5.80	K- C	-5 / 143	0.05 (4)	
F- G	0 / 41	-91.8	-91.8	0.15 (1)	10.00	B- K	0 / 881	0.23 (1)	
L- B	-1247 / 0	0.0	0.0	0.15 (1)	7.14	I- F	0 / 881	0.23 (1)	
H- F	-1247 / 0	0.0	0.0	0.15 (1)	7.14				
L- K	0 / 0	-39.8	-77.7	0.22 (4)	10.00				
K- J	0 / 830	-77.7	-113.5	0.39 (4)	10.00				
J- I	0 / 830	-113.5	-77.7	0.39 (4)	10.00				
I- H	0 / 0	-77.7	-39.8	0.22 (4)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.39/1.00 (J-K:4) , WB=0.25/1.00 (D-J:4) , SSI=0.26/1.00 (J-K:4)

DOL LUMBER=0.95 NAIL=0.95 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080941

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T10A	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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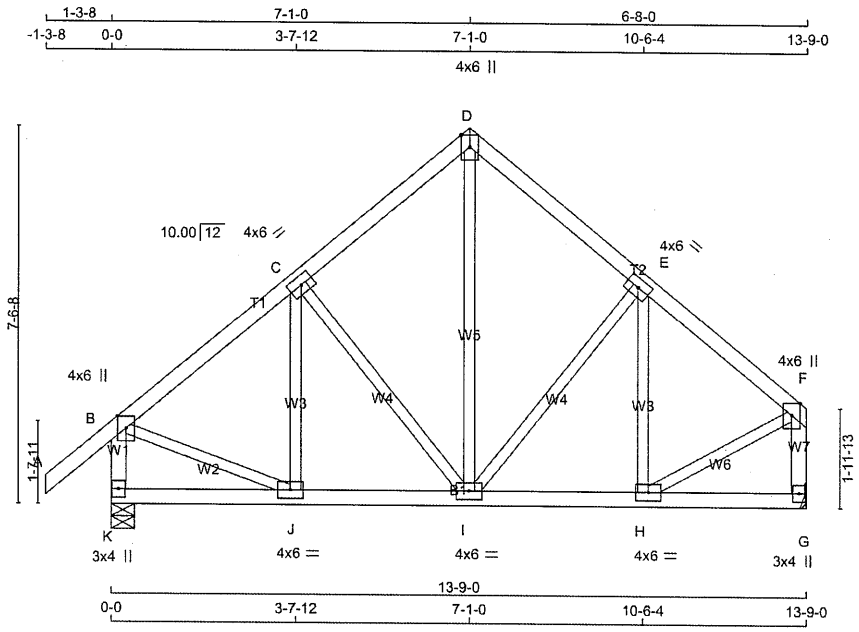

TOTAL WEIGHT = 68 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	885	0	885	0	0	IN-SX	IN-SX
G	758	0	758	0	0	5-8	1-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	624	423 / 0	0 / 0	0 / 0	0 / 0	201 / 0	0 / 0
G	536	352 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13 (1)		10.00	J- C	-109 / 22	0.03 (1)
B- C	-631 / 0	-91.8	-91.8	0.15 (1)		6.25	C- I	-225 / 0	0.12 (1)
C- D	-498 / 0	-91.8	-91.8	0.15 (1)		6.25	I- D	0 / 364	0.08 (1)
D- E	-495 / 0	-91.8	-91.8	0.13 (1)		6.25	I- E	-160 / 0	0.08 (1)
E- F	-578 / 0	-91.8	-91.8	0.13 (1)		6.25	H- E	-170 / 0	0.05 (1)
K- B	-856 / 0	0.0	0.0	0.09 (1)		7.81	B- J	0 / 534	0.12 (1)
G- F	-733 / 0	0.0	0.0	0.08 (1)		7.81	H- F	0 / 519	0.12 (1)
K- J	0 / 0	-18.5	-18.5	0.05 (4)		10.00			
J- I	0 / 503	-18.5	-18.5	0.10 (1)		10.00			
I- H	0 / 462	-18.5	-18.5	0.10 (1)		10.00			
H- G	0 / 0	-18.5	-18.5	0.05 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.10/1.00 (I-J:1) , WB=0.12/1.00 (B-J:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (B) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR23080942

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T10G	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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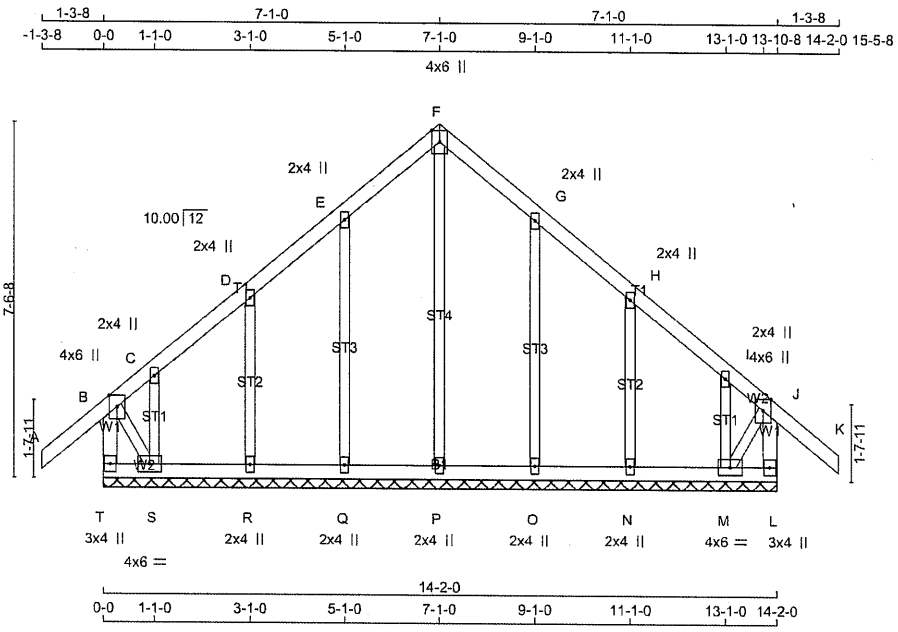

TOTAL WEIGHT = 70 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
J	TMVW+p	MT20	4.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	6.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	P- F	-143 / 0	0.14 (1)
B- C	-72 / 0	-91.8	-91.8	0.12 (1)	6.25	Q- E	-206 / 0	0.11 (1)
C- D	-13 / 0	-91.8	-91.8	0.04 (1)	6.25	R- D	-187 / 0	0.05 (1)
D- E	-12 / 0	-91.8	-91.8	0.05 (1)	6.25	S- C	-49 / 0	0.01 (1)
E- F	-21 / 0	-91.8	-91.8	0.05 (1)	6.25	O- G	-206 / 0	0.11 (1)
F- G	-21 / 0	-91.8	-91.8	0.05 (1)	6.25	N- H	-187 / 0	0.05 (1)
G- H	-12 / 0	-91.8	-91.8	0.05 (1)	6.25	M- I	-49 / 0	0.01 (1)
H- I	-13 / 0	-91.8	-91.8	0.04 (1)	6.25	B- S	0 / 27	0.01 (1)
I- J	-72 / 0	-91.8	-91.8	0.12 (1)	6.25	M- J	0 / 27	0.01 (1)
J- K	0 / 41	-91.8	-91.8	0.13 (1)	10.00			
T- B	-284 / 0	0.0	0.0	0.03 (1)	7.81			
L- J	-284 / 0	0.0	0.0	0.03 (1)	7.81			
T- S	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
S- R	0 / 15	-18.5	-18.5	0.02 (4)	10.00			
R- Q	0 / 11	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 7	-18.5	-18.5	0.01 (4)	10.00			
P- O	0 / 7	-18.5	-18.5	0.01 (4)	10.00			
O- N	0 / 11	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 15	-18.5	-18.5	0.02 (4)	10.00			
M- L	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1) , BC=0.02/1.00 (R-S:4) , WB=0.14/1.00 (F-P:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (J) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080958

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T11G	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:32:58 2023 Page 1
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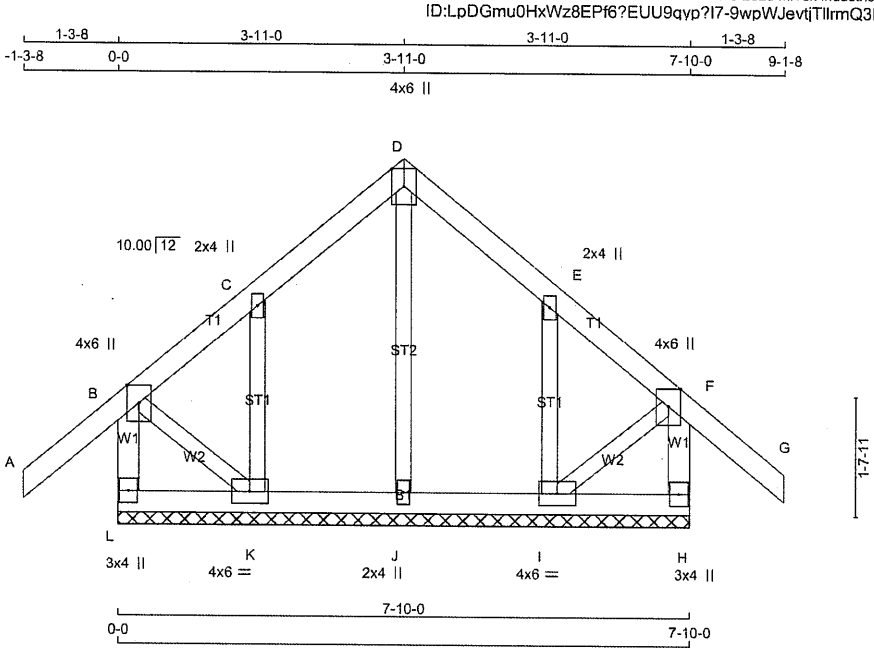

TOTAL WEIGHT = 39 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-268 / 0	0.0	0.0	0.03 (1)	7.81	J- D	-153 / 0	0.05 (1)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	K- C	-152 / 0	0.03 (1)
B- C	-41 / 0	-91.8	-91.8	0.12 (1)	6.25	I- E	-152 / 0	0.03 (1)
C- D	-20 / 0	-91.8	-91.8	0.05 (1)	6.25	B- K	0 / 18	0.00 (1)
D- E	-20 / 0	-91.8	-91.8	0.05 (1)	6.25	I- F	0 / 18	0.00 (1)
E- F	-41 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- G	0 / 41	-91.8	-91.8	0.13 (1)	10.00			
H- F	-268 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 10	-18.5	-18.5	0.02 (4)	10.00			
J- I	0 / 10	-18.5	-18.5	0.02 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1) , BC=0.02/1.00 (J-K:4) , WB=0.05/1.00 (D-J:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080943

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434362	T12	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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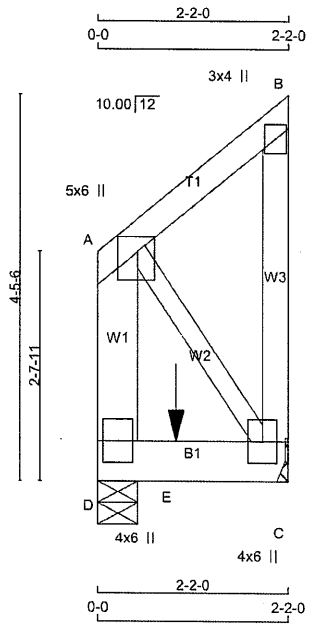


Scale = 1:25.4

TOTAL WEIGHT = 2 X 18 = 35 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080944

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	554	0	554	0	0	IN-SX 5-8	IN-SX 1-8
C	426	0	426	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	391	262 / 0	0 / 0	0 / 0	0 / 0	129 / 0	0 / 0
C	300	201 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-99 / 0	0.0	0.0	0.00 (1)		7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.04 (1)		10.00			
C- B	-99 / 0	0.0	0.0	0.01 (1)		7.81			
D- E	0 / 0	-18.5	-18.5	0.15 (1)		10.00			
E- C	0 / 0	-18.5	-18.5	0.15 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	10-12	-522	-522	—	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.04/1.00 (A-B:1) , BC=0.15/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.02 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T12	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-d6NuX_wVUntcTw?Fz8vCyalqrx_3bSftlKB8Adykex2

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

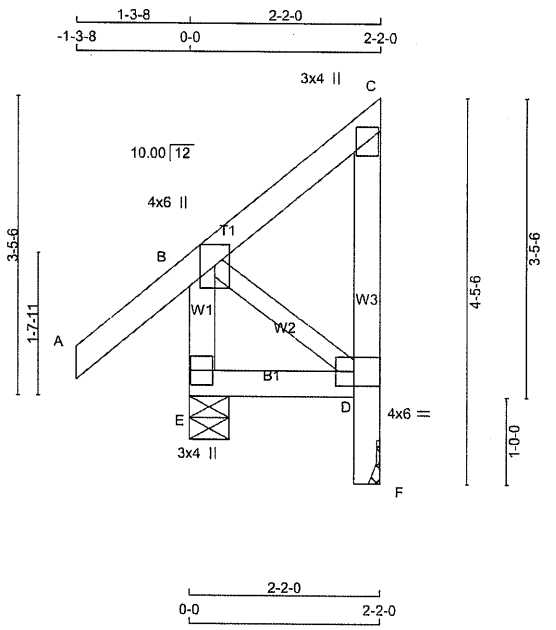
STRUCTURAL COMPONENT ONLY
DWG # TR23080944

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T13	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-5IxHkKx7F5?T44aSWrRRVnI ELMCKvv1z wij4ykex1



Scale = 1:25.4

TOTAL WEIGHT = 2 X 15 = 30 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMV+p	MT20	3.0	4.0		
D	BVMW-!	MT20	4.0	6.0	2.00	2.50
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	247	0	247	0	0	5-8	1-8
F	119	0	119	0	0	IN-SX MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	172	126 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
F	84	55 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-226 / 0	0.0	0.0	0.02 (1)	7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 41	-91.8	-91.8	0.13 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.07 (1)	10.00			
F- D	-119 / 0	0.0	0.0	0.01 (1)	7.81			
D- C	-99 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080945

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T14	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:33:02 2023 Page 1
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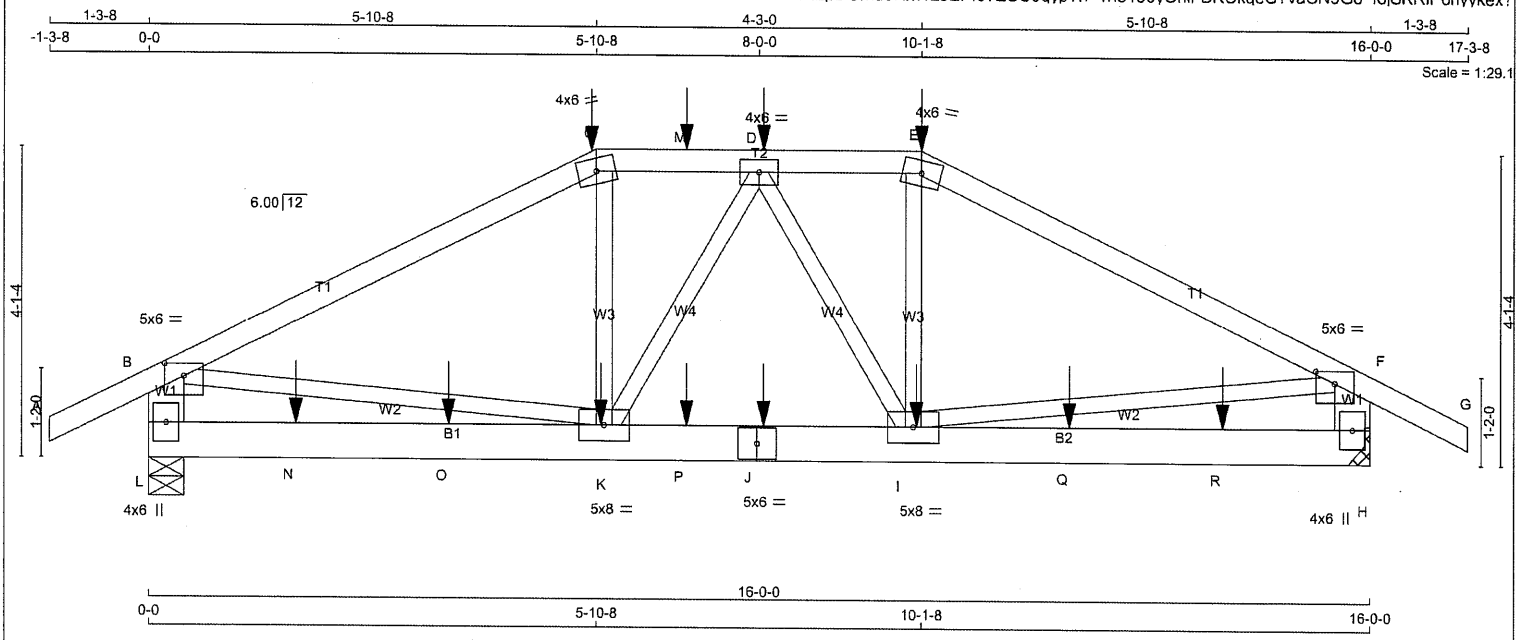

TOTAL WEIGHT = 75 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TTW-m	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMVW-p	MT20	5.0	6.0	2.00	3.00
H	BMV1+p	MT20	4.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080946

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1597	0	1597	0	0	IN-SX	IN-SX
H	1580	0	1580	0	0	5-8 MECHANICAL	1-12

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-11.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1129	741 / 0	0 / 0	0 / 0	0 / 0	388 / 0	0 / 0
H	1118	734 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	K- C	0 / 158	0.06 (4)
B- C	-1998 / 0	-91.8	-91.8	0.80 (1)	3.73	K- D	-204 / 0	0.06 (1)
C- M	-1791 / 0	-91.8	-91.8	0.16 (1)	4.82	D- I	-255 / 0	0.08 (1)
M- D	-1791 / 0	-91.8	-91.8	0.16 (1)	4.82	I- E	0 / 168	0.06 (4)
D- E	-1768 / 0	-91.8	-91.8	0.12 (1)	4.88	B- K	0 / 1800	0.45 (1)
E- F	-1969 / 0	-91.8	-91.8	0.79 (1)	3.76	I- F	0 / 1775	0.44 (1)
F- G	0 / 28	-91.8	-91.8	0.13 (1)	10.00			
L- B	-1519 / 0	0.0	0.0	0.11 (1)	7.81			
H- F	-1502 / 0	0.0	0.0	0.11 (1)	7.81			
L- N	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
N- O	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
O- K	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
K- P	0 / 1886	-18.5	-18.5	0.28 (1)	10.00			
P- J	0 / 1886	-18.5	-18.5	0.28 (1)	10.00			
J- I	0 / 1886	-18.5	-18.5	0.28 (1)	10.00			
I- Q	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
Q- R	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
R- H	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-254	-254	---	FRONT	VERT	TOTAL	---	C1
D	8-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
E	10-1-8	-254	-254	---	FRONT	VERT	TOTAL	---	C1
I	10-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
J	8-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
K	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
M	7-0-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
N	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	7-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Q	12-0-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	14-0-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.80/1.00 (B-C:1) , BC=0.28/1.00 (I-K:1) , WB=0.45/1.00 (B-K:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434362	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

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8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080946

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T15	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:33:03 2023 Page 1
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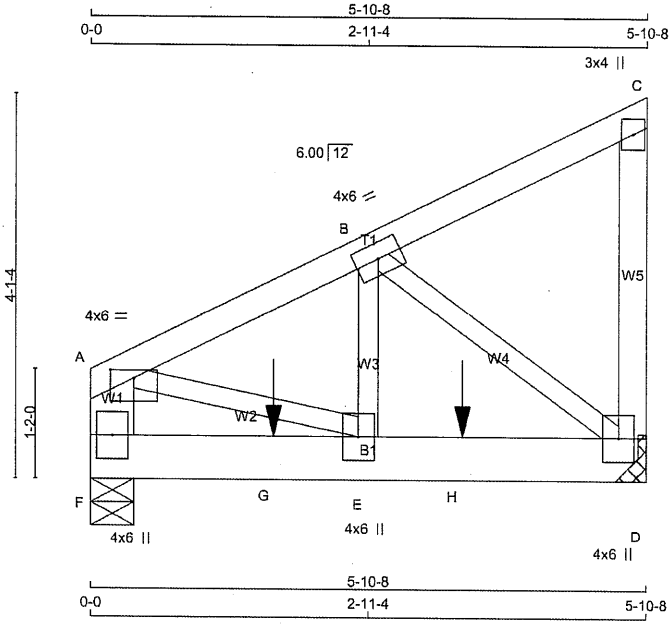

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080947

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1071	0	1071	0	0	5-8	IN-SX
D	1485	0	1485	0	0	MECHANICAL	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	753	520 / 0	0 / 0	0 / 0	0 / 0	233 / 0	0 / 0
D	1047	705 / 0	0 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1050 / 0	0.0	0.0	0.04 (1)		7.81	A- E	0 / 1272	0.16 (1)
A- B	-1365 / 0	-91.8	-91.8	0.06 (1)		6.25	E- B	0 / 1237	0.15 (1)
B- C	-11 / 0	-91.8	-91.8	0.05 (1)		6.25	B- D	-1548 / 0	0.18 (1)
D- C	-111 / 0	0.0	0.0	0.01 (1)		7.81			
F- G	0 / 0	-18.5	-18.5	0.13 (1)		10.00			
G- E	0 / 0	-18.5	-18.5	0.13 (1)		10.00			
E- H	0 / 1230	-18.5	-18.5	0.38 (1)		10.00			
H- D	0 / 1230	-18.5	-18.5	0.38 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-239	-239	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	-1103	-1103	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.38/1.00 (D-E:1) , WB=0.18/1.00 (B-D:1) , SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (D) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434362	T15	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080947

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434364	T15Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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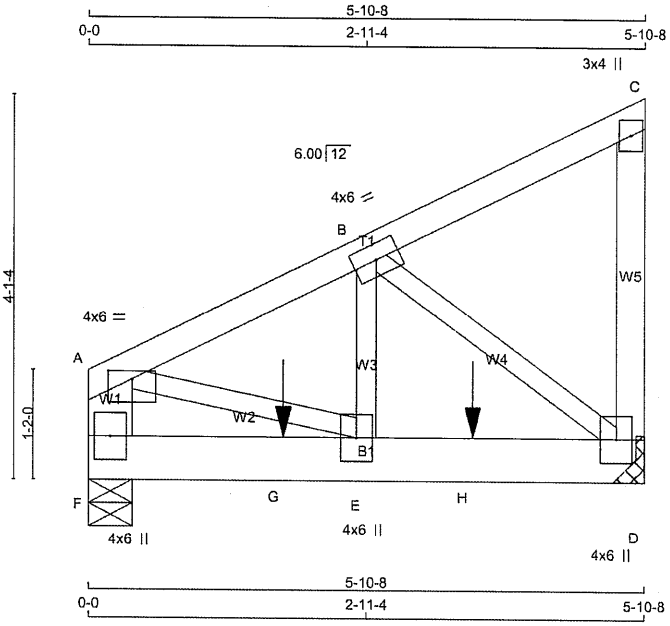

Scale = 1:23.5

TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F-A	2	12	TOP
A-C	1	12	TOP
C-D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080972

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1270	0	1270	0	0	5-8	1-8
D	1354	0	1354	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	894	609 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
D	953	649 / 0	0 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1076 / 0	0.0	0.0	0.04 (1)		7.81	A- E	0 / 1306	0.16 (1)
A- B	-1401 / 0	-91.8	-91.8	0.06 (1)		6.25	E- B	0 / 1279	0.16 (1)
B- C	-11 / 0	-91.8	-91.8	0.05 (1)		6.25	B- D	-1589 / 0	0.19 (1)
D- C	-111 / 0	0.0	0.0	0.01 (1)		7.81			
F- G	0 / 0	-18.5	-18.5	0.14 (1)		10.00			
G- E	0 / 0	-18.5	-18.5	0.14 (1)		10.00			
E- H	0 / 1263	-18.5	-18.5	0.26 (1)		10.00			
H- D	0 / 1263	-18.5	-18.5	0.26 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-695	-695	---	BACK	VERT	TOTAL	---	C1
H	4-0-12	-695	-695	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.26/1.00 (D-E:1) , WB=0.19/1.00 (B-D:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434364	T15Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080972

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T16W	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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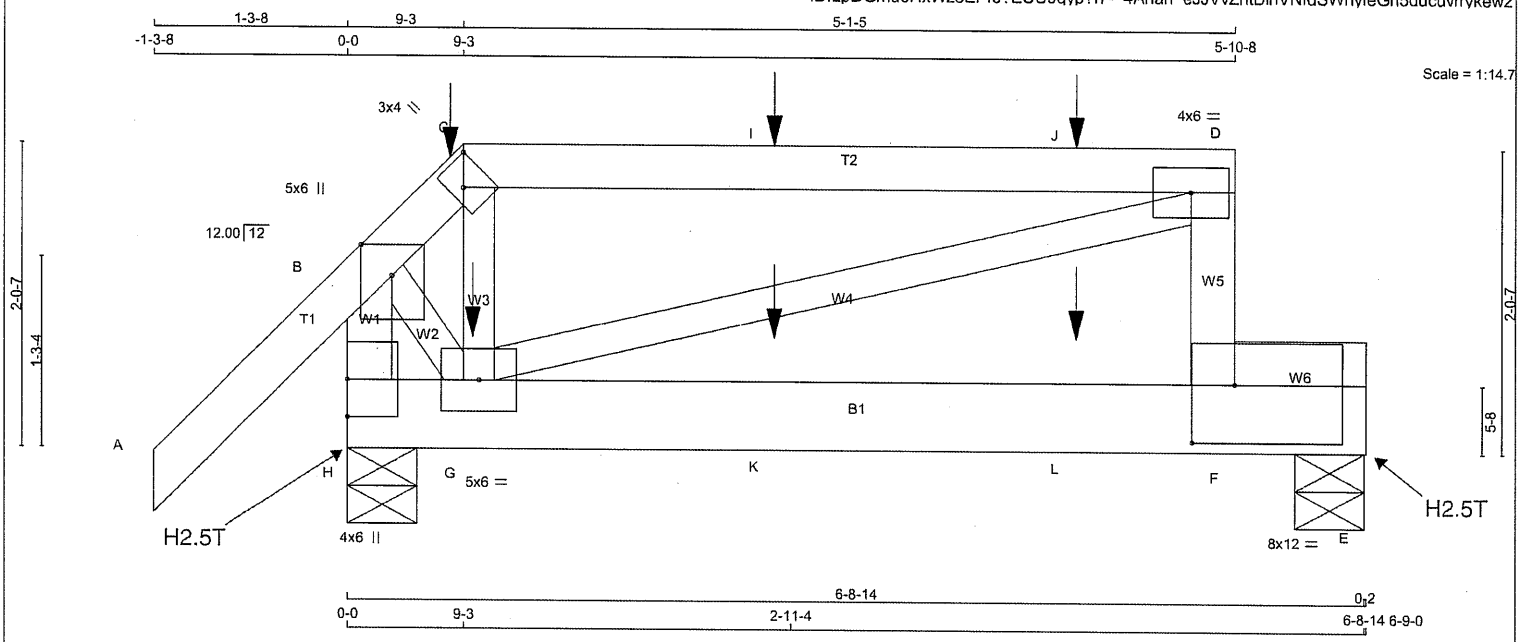

TOTAL WEIGHT = 30 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TTW+h	MT20	3.0	4.0	Edge	2.00
D	TMVW-t	MT20	4.0	6.0		
F	BMVW-t	MT20	8.0	12.0	4.75	3.50
G	BMWWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080948

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
END VERTICALS ARE NOT EXPOSED TO WIND.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	822	0	822	114	-553	5-8	1-8
E	501	0	501	0	-389	5-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT H FOR 553 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 389 LBS FACTORED UPLIFT

PROVIDE FOR 114 LBS FACTORED HORIZONTAL REACTION AT JOINT H

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	579	392 / 0	0 / 0	0 / 0	0 / -515	187 / 0	0 / 0
E	356	224 / 0	0 / 0	0 / 0	0 / -363	132 / 0	0 / 0
HORIZONTAL REACTIONS							
H	---	0 / 0	0 / 0	0 / 0	82 / -30	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 45	-91.8	-91.8	0.14 (1)	10.00	G- D	-423 / 414	0.18 (7)
B- C	-651 / 611	-91.8	-91.8	0.20 (7)	6.25	G- C	-194 / 93	0.04 (12)
C- I	-393 / 401	-91.8	-91.8	0.73 (1)	6.25	B- G	-546 / 567	0.14 (1)
I- J	-393 / 401	-91.8	-91.8	0.73 (1)	6.25			
J- D	-393 / 401	-91.8	-91.8	0.73 (1)	6.25			
F- D	-496 / 357	0.0	0.0	0.06 (1)	7.81			
H- B	-1096 / 853	0.0	0.0	0.13 (7)	7.52			
H- G	-114 / 14	-18.5	-18.5	0.17 (1)	6.25			
G- K	0 / 0	-18.5	-18.5	0.31 (1)	10.00			
K- L	0 / 0	-18.5	-18.5	0.31 (1)	10.00			
L- F	0 / 0	-18.5	-18.5	0.31 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.19 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	9-3	-89	-89	82	FRONT	VERT	TOTAL	---	C1
G	9-15	-45	-45	60	FRONT	VERT	TOTAL	---	C1
I	2-9-15	-78	-78	80	FRONT	VERT	TOTAL	---	C1
J	4-9-15	-78	-78	80	FRONT	VERT	TOTAL	---	C1
K	2-9-15	-43	-43	53	FRONT	VERT	TOTAL	---	C1
L	4-9-15	-43	-43	53	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.73/1.00 (C-D:1) , BC=0.31/1.00 (F-G:1) , WB=0.18/1.00 (D-G:7) , SSI=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.23 (C) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T17W	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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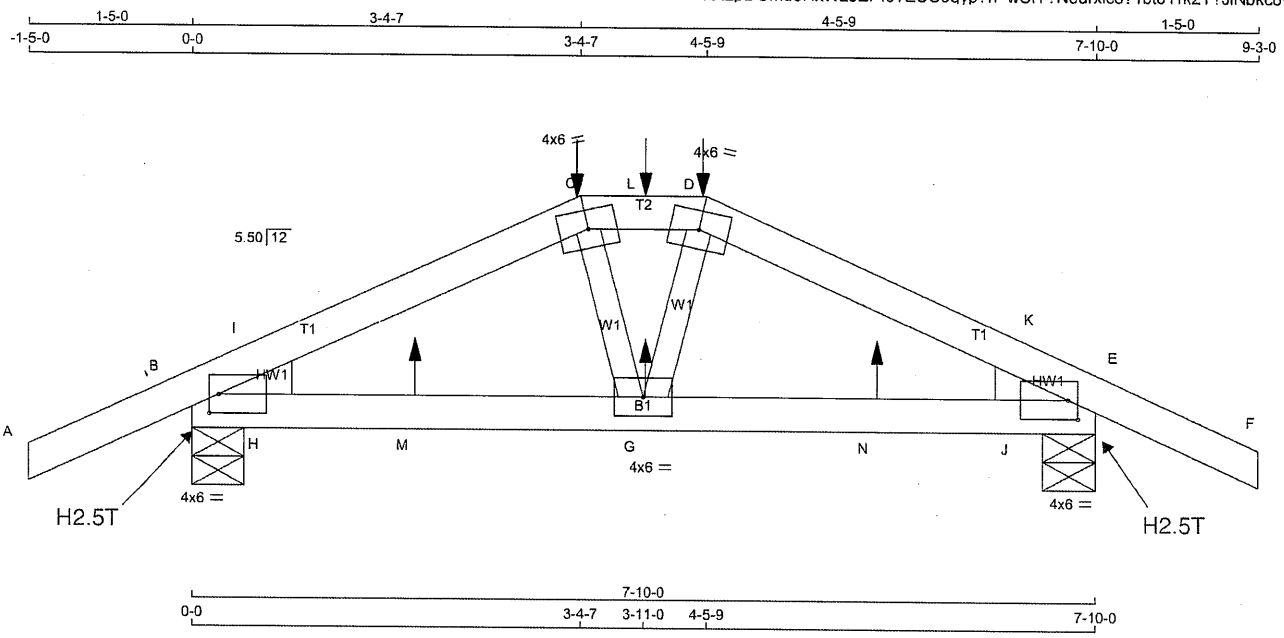

TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0	2.00	1.00
C	TTW-m	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	6.0		
E	TMBH1-I	MT20	4.0	6.0	2.00	1.00
G	BMWW-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	613	0	613	52	-372	5-8	1-8	2x4 L
E	613	0	613	0	-316	5-8	1-8	2x4 R

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 372 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 316 LBS FACTORED UPLIFT

PROVIDE FOR 52 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	431	299 / 0	0 / 0	0 / 0	0 / -350	131 / 0	0 / 0
E	431	299 / 0	0 / 0	0 / 0	0 / -310	131 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	37 / -37	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (19)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 21	-91.8	-91.8	0.15 (1)	10.00	C- G	-87 / 45	0.02 (12)
B- I	-692 / 399	-91.8	-91.8	0.07 (7)	6.25	G- D	-102 / 44	0.02 (12)
I- C	-636 / 405	-91.8	-91.8	0.14 (7)	6.25	H- I	-86 / 30	0.00 (1)
C- L	-586 / 425	-91.8	-91.8	0.09 (7)	6.25	J- K	-82 / 35	0.00 (1)
L- D	-586 / 425	-91.8	-91.8	0.09 (7)	6.25			
D- K	-636 / 411	-91.8	-91.8	0.13 (8)	6.25			
K- E	-692 / 425	-91.8	-91.8	0.07 (7)	6.25			
E- F	0 / 21	-91.8	-91.8	0.15 (1)	10.00			
B- H	-330 / 574	-18.5	-18.5	0.15 (1)	6.25			
H- M	-330 / 574	-18.5	-18.5	0.15 (1)	6.25			
M- G	-330 / 574	-18.5	-18.5	0.15 (1)	6.25			
G- N	-320 / 574	-18.5	-18.5	0.15 (1)	6.25			
N- J	-320 / 574	-18.5	-18.5	0.15 (1)	6.25			
J- E	-320 / 574	-18.5	-18.5	0.15 (1)	6.25			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-4-13	-8	-8	---	FRONT	VERT	DEAD	---	C1
C	3-4-13	-31	-31	---	FRONT	VERT	SNOW	---	C1
D	4-5-3	-8	-8	---	FRONT	VERT	DEAD	---	C1
D	4-5-3	-31	-31	---	FRONT	VERT	SNOW	---	C1
G	3-11-4	6	-5	20	BACK	VERT	TOTAL	---	C1
L	3-11-4	-3	-31	41	BACK	VERT	TOTAL	---	C1
M	1-11-4	6	-5	20	BACK	VERT	TOTAL	---	C1
N	5-11-4	6	-5	20	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.15/1.00 (G-H:1) , WB=0.02/1.00 (C-G:12) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (E) (INPUT = 0.90)
JSI METAL= 0.15 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080949

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T17W	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080949

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	T18W	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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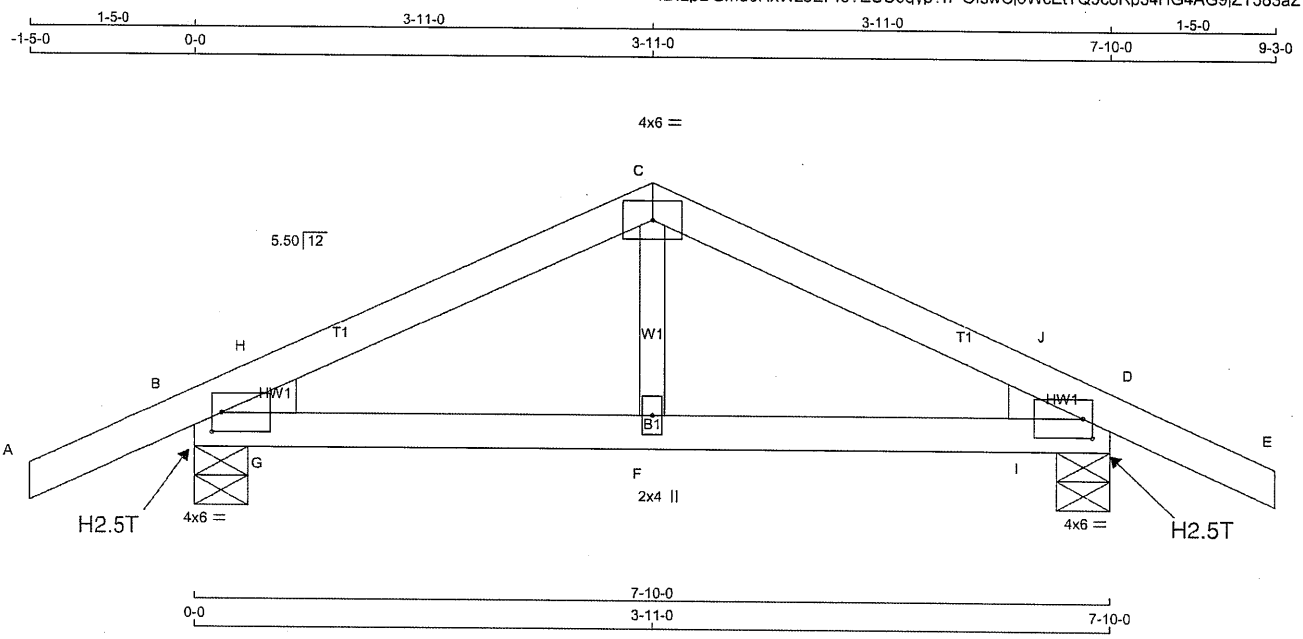

TOTAL WEIGHT = 2 X 25 = 51 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0	2.00	1.00
C	TTW-p	MT20	4.0	6.0		
D	TMBH1-I	MT20	4.0	6.0	2.00	1.00
F	BMW+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	567	0	567	57	-347	5-8	1-8	2x4 L
D	567	0	567	0	-279	5-8	1-8	2x4 R

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 347 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 279 LBS FACTORED UPLIFT

PROVIDE FOR 57 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	399	276 / 0	0 / 0	0 / 0	0 / -327	123 / 0	0 / 0
D	399	276 / 0	0 / 0	0 / 0	0 / -278	123 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	41 / -41	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 21	-91.8	-91.8	0.14 (1)	10.00	F- C	-175 / 145	0.03 (1)
B- H	-536 / 307	-91.8	-91.8	0.05 (7)	6.25	G- H	-145 / 64	0.00 (1)
H- C	-508 / 343	-91.8	-91.8	0.14 (7)	6.25	I- J	-139 / 110	0.00 (1)
C- J	-508 / 362	-91.8	-91.8	0.13 (1)	6.25			
J- D	-536 / 357	-91.8	-91.8	0.06 (7)	6.25			
D- E	0 / 21	-91.8	-91.8	0.14 (1)	10.00			
B- G	-270 / 457	-18.5	-18.5	0.17 (1)	6.25			
G- F	-270 / 457	-18.5	-18.5	0.17 (1)	6.25			
F- I	-270 / 457	-18.5	-18.5	0.17 (1)	6.25			
I- D	-270 / 457	-18.5	-18.5	0.17 (1)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.14/1.00 (C-H:7) , BC=0.17/1.00 (F-I:1) , WB=0.03/1.00 (C-F:1) , SSI=0.12/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080950

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T20	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:52:23 2023 Page 1
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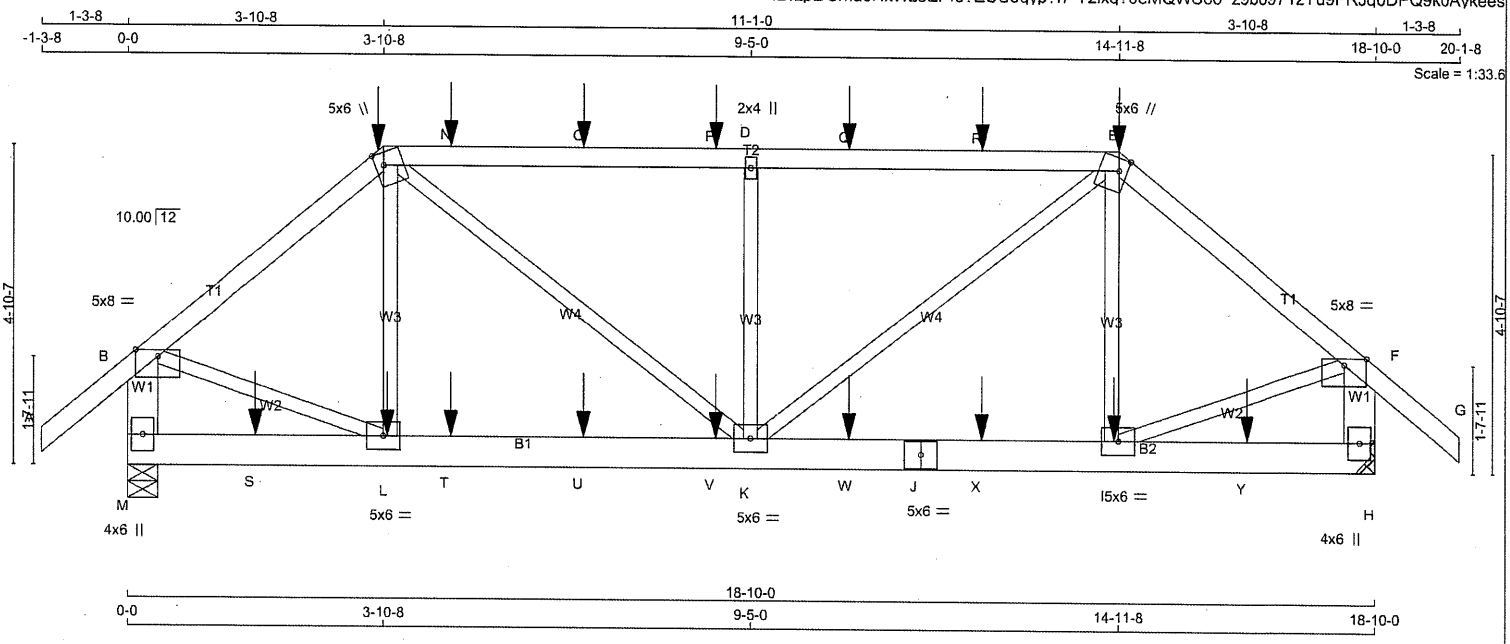


TOTAL WEIGHT = 93 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

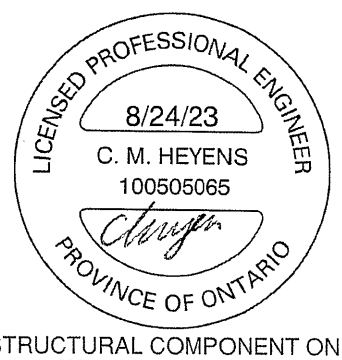
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	4.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR23080959

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1682	0	1682	0	0	IN-SX 5-8	IN-SX 1-13
H	1653	0	1653	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-13.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1187	792 / 0	0 / 0	0 / 0	0 / 0	395 / 0	0 / 0
H	1167	779 / 0	0 / 0	0 / 0	0 / 0	388 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 41	-91.8	-91.8	0.14 (1)	10.00
B- C	-1534 / 0	-91.8	-91.8	0.32 (1)	4.93
C- N	-1798 / 0	-91.8	-91.8	0.89 (1)	3.43
N- O	-1798 / 0	-91.8	-91.8	0.89 (1)	3.43
O- P	-1798 / 0	-91.8	-91.8	0.89 (1)	3.43
P- D	-1798 / 0	-91.8	-91.8	0.89 (1)	3.43
D- Q	-1798 / 0	-91.8	-91.8	0.89 (1)	3.43
Q- R	-1798 / 0	-91.8	-91.8	0.89 (1)	3.43
R- E	-1798 / 0	-91.8	-91.8	0.89 (1)	3.43
E- F	-1501 / 0	-91.8	-91.8	0.31 (1)	4.97
F- G	0 / 41	-91.8	-91.8	0.14 (1)	10.00
M- B	-1651 / 0	0.0	0.0	0.12 (1)	7.64
H- F	-1622 / 0	0.0	0.0	0.12 (1)	7.69
M- S	0 / 0	-18.5	-18.5	0.07 (4)	10.00
S- L	0 / 0	-18.5	-18.5	0.07 (4)	10.00
L- T	0 / 1171	-18.5	-18.5	0.21 (1)	10.00
T- U	0 / 1171	-18.5	-18.5	0.21 (1)	10.00
U- V	0 / 1171	-18.5	-18.5	0.21 (1)	10.00
V- K	0 / 1171	-18.5	-18.5	0.21 (1)	10.00
K- W	0 / 1146	-18.5	-18.5	0.20 (1)	10.00
W- J	0 / 1146	-18.5	-18.5	0.20 (1)	10.00
J- X	0 / 1146	-18.5	-18.5	0.20 (1)	10.00
X- I	0 / 1146	-18.5	-18.5	0.20 (1)	10.00
I- Y	0 / 0	-18.5	-18.5	0.07 (4)	10.00
Y- H	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-197 / 46	0.07 (1)
K- D	-909 / 0	0.31 (1)
K- E	0 / 839	0.21 (1)
I- E	-197 / 39	0.07 (1)
B- L	0 / 1229	0.30 (1)
I- F	0 / 1203	0.30 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-145	-145	---	FRONT	VERT	TOTAL	---	C1
E	14-11-8	-145	-145	---	FRONT	VERT	TOTAL	---	C1
I	14-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
L	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
N	4-10-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1
O	6-10-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1
P	8-10-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1
Q	10-10-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1
R	12-10-12	-59	-59	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.89/1.00 (D-E:1) , BC=0.21/1.00 (K-L:1) , WB=0.31/1.00 (D-K:1) , SSI=0.42/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.27 (J) (INPUT = 0.95)

CONTINUED ON PAGE 2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	1-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
T	4-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
U	6-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
V	8-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
W	10-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
X	12-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Y	16-10-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080959

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T21	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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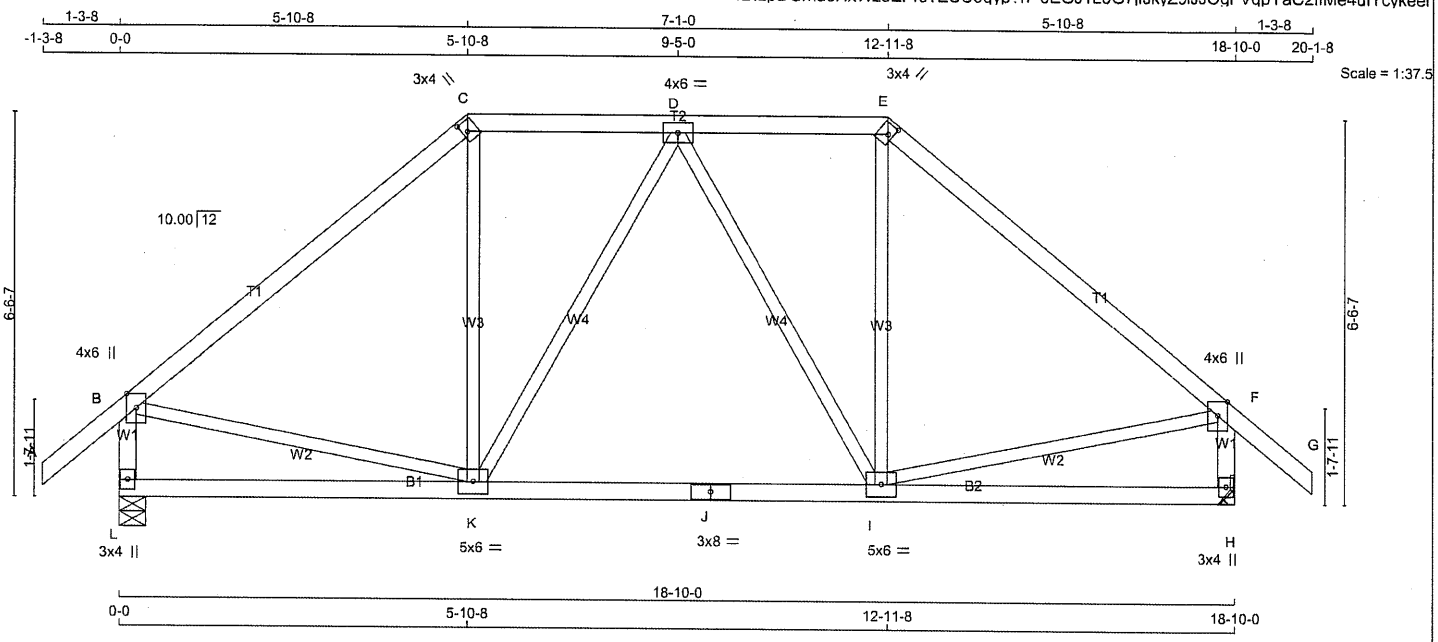

TOTAL WEIGHT = 84 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+h	MT20	3.0	4.0	2.00	1.00
D	TMWW-t	MT20	4.0	6.0		
E	TTW+h	MT20	3.0	4.0	2.00	1.00
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1165	0	1165	0	0	5-8	1-8
H	1165	0	1165	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	822	553 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
H	822	553 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	K- C	0 / 178	0.04 (4)
B- C	-897 / 0	-91.8	-91.8	0.43 (1)	5.92	K- D	-223 / 0	0.22 (1)
C- D	-693 / 0	-91.8	-91.8	0.15 (1)	6.25	D- I	-223 / 0	0.22 (1)
D- E	-693 / 0	-91.8	-91.8	0.15 (1)	6.25	I- E	0 / 178	0.04 (4)
E- F	-897 / 0	-91.8	-91.8	0.43 (1)	5.92	B- K	0 / 704	0.16 (1)
F- G	0 / 41	-91.8	-91.8	0.13 (1)	10.00	I- F	0 / 704	0.16 (1)
L- B	-1121 / 0	0.0	0.0	0.12 (1)	7.49			
H- F	-1121 / 0	0.0	0.0	0.12 (1)	7.49			
L- K	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
K- J	0 / 799	-18.5	-18.5	0.24 (4)	10.00			
J- I	0 / 799	-18.5	-18.5	0.24 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.24/1.00 (I-K:4) , WB=0.22/1.00 (D-K:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR23080960

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T22	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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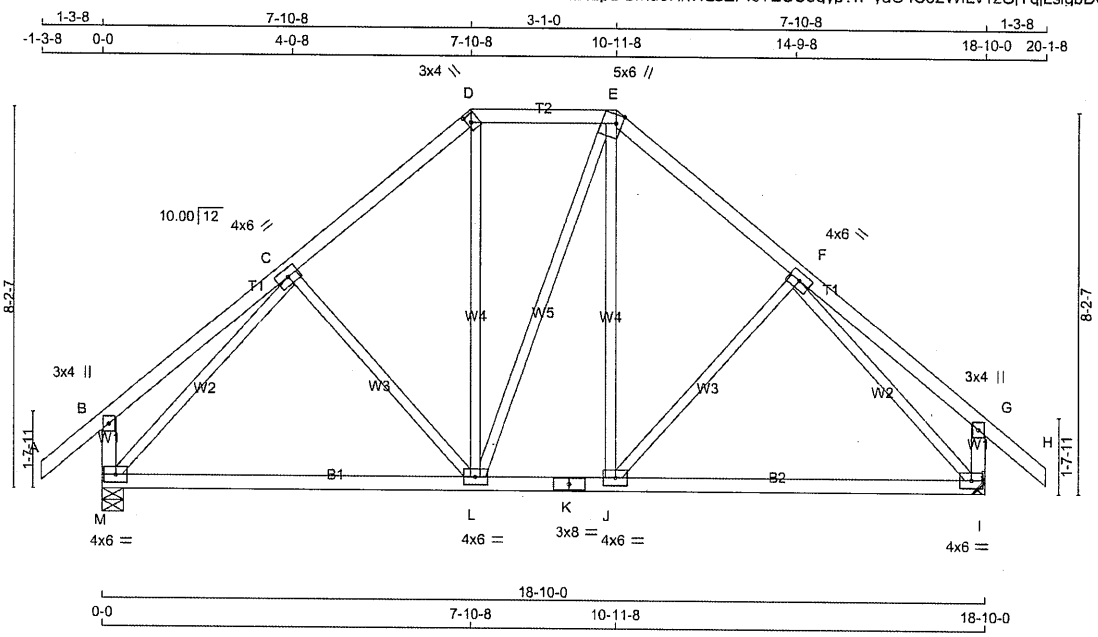

Scale = 1:47.4

TOTAL WEIGHT = 93 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.00
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1165	0	1165	0	0	IN-SX	IN-SX
I	1165	0	1165	0	0	5-8	1-8
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	822	553 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
I	822	553 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	C- L	-160 / 0	0.10 (1)
B- C	0 / 27	-91.8	-91.8	0.23 (1)	10.00	L- D	0 / 232	0.05 (1)
C- D	-826 / 0	-91.8	-91.8	0.18 (1)	6.25	L- E	0 / 1	0.00 (1)
D- E	-616 / 0	-91.8	-91.8	0.12 (1)	6.25	J- E	0 / 230	0.05 (1)
E- F	-826 / 0	-91.8	-91.8	0.18 (1)	6.25	J- F	-161 / 0	0.11 (1)
F- G	0 / 27	-91.8	-91.8	0.23 (1)	10.00	M- C	-1106 / 0	0.68 (1)
G- H	0 / 41	-91.8	-91.8	0.13 (1)	10.00	F- I	-1105 / 0	0.68 (1)
M- B	-264 / 0	0.0	0.0	0.03 (1)	7.81			
I- G	-264 / 0	0.0	0.0	0.03 (1)	7.81			
M- L	0 / 719	-18.5	-18.5	0.29 (4)	10.00			
L- K	0 / 615	-18.5	-18.5	0.29 (4)	10.00			
K- J	0 / 615	-18.5	-18.5	0.29 (4)	10.00			
J- I	0 / 719	-18.5	-18.5	0.30 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.23/1.00 (F-G:1) , BC=0.30/1.00 (I-J:4) , WB=0.68/1.00 (C-M:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.47 (K) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080961

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T23	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:52:27 2023 Page 1
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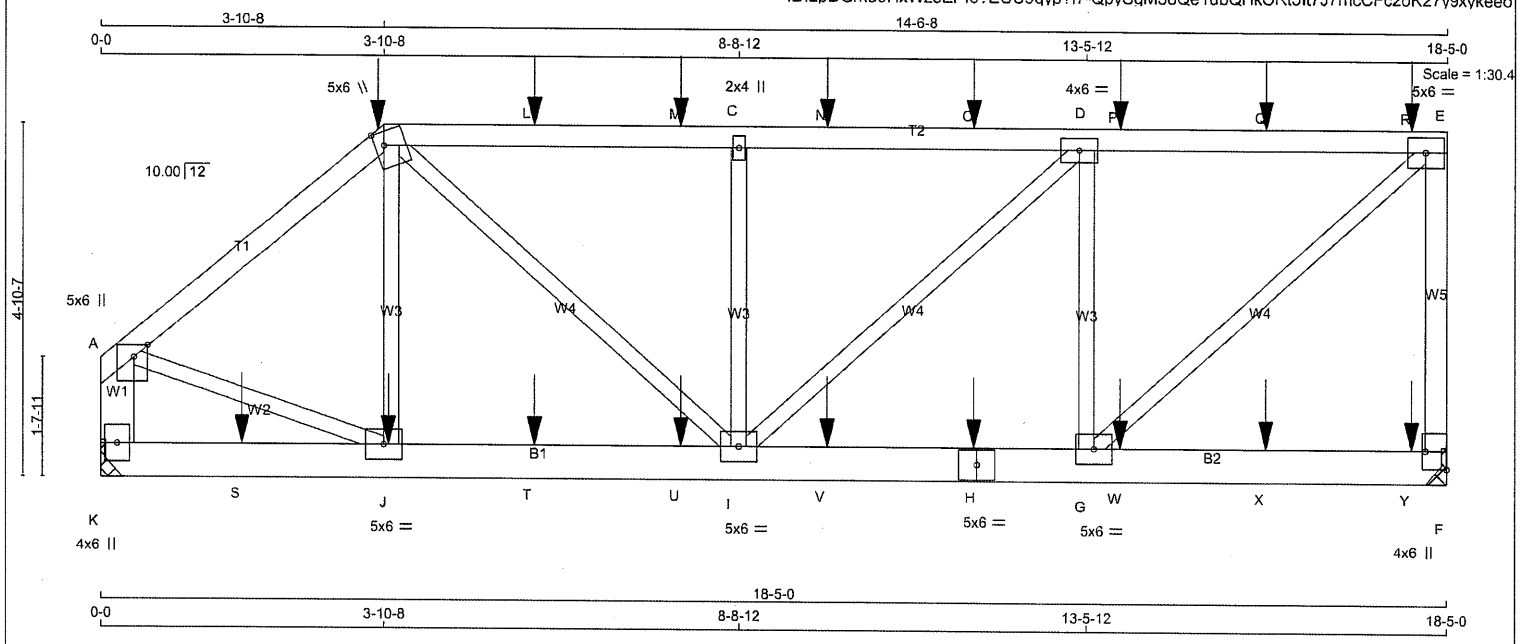

TOTAL WEIGHT = 90 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	4.0	6.0	3.00	Edge
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080962

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1562	0	1562	0	0	MECHANICAL	
K	1465	0	1465	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, K. MINIMUM BEARING LENGTH AT JOINT F = 1-11, JOINT K = 1-9.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1104	726 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
K	1036	679 / 0	0 / 0	0 / 0	0 / 0	357 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1428 / 0	-91.8	-91.8	0.31 (1)	5.08	J- B	-190 / 35	0.06 (1)	
B- L	-1619 / 0	-91.8	-91.8	0.52 (1)	4.50	A- J	0 / 1144	0.28 (1)	
L- M	-1619 / 0	-91.8	-91.8	0.52 (1)	4.50	G- E	0 / 1753	0.43 (1)	
M- C	-1619 / 0	-91.8	-91.8	0.52 (1)	4.50	B- I	0 / 721	0.18 (1)	
C- N	-1620 / 0	-91.8	-91.8	0.55 (1)	4.43	G- D	-1030 / 0	0.35 (1)	
N- O	-1620 / 0	-91.8	-91.8	0.55 (1)	4.43	I- C	-690 / 0	0.23 (1)	
O- D	-1620 / 0	-91.8	-91.8	0.55 (1)	4.43	I- D	0 / 445	0.11 (1)	
D- P	-1297 / 0	-91.8	-91.8	0.53 (1)	4.88				
P- Q	-1297 / 0	-91.8	-91.8	0.53 (1)	4.88				
Q- R	-1297 / 0	-91.8	-91.8	0.53 (1)	4.88				
R- E	-1297 / 0	-91.8	-91.8	0.53 (1)	4.88				
F- E	-1493 / 0	0.0	0.0	0.55 (1)	6.67				
K- A	-1431 / 0	0.0	0.0	0.11 (1)	7.81				
K- S	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
S- J	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
J- T	0 / 1089	-18.5	-18.5	0.19 (1)	10.00				
T- U	0 / 1089	-18.5	-18.5	0.19 (1)	10.00				
U- I	0 / 1089	-18.5	-18.5	0.19 (1)	10.00				
I- V	0 / 1297	-18.5	-18.5	0.22 (1)	10.00				
V- H	0 / 1297	-18.5	-18.5	0.22 (1)	10.00				
H- G	0 / 1297	-18.5	-18.5	0.22 (1)	10.00				
G- W	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
W- X	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
X- Y	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
Y- F	0 / 0	-18.5	-18.5	0.07 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-10-8	-145	-145	---	FRONT	VERT	TOTAL	---	C1
H	11-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
J	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
L	5-11-4	-59	-59	---	FRONT	VERT	TOTAL	---	C1
M	7-11-4	-59	-59	---	FRONT	VERT	TOTAL	---	C1
N	9-11-4	-59	-59	---	FRONT	VERT	TOTAL	---	C1
O	11-11-4	-59	-59	---	FRONT	VERT	TOTAL	---	C1
P	13-11-4	-59	-59	---	FRONT	VERT	TOTAL	---	C1
Q	15-11-4	-59	-59	---	FRONT	VERT	TOTAL	---	C1
R	17-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
S	1-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.55/1.00 (E-F:1) , BC=0.22/1.00 (G-I:1) , WB=0.43/1.00 (E-G:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (A) (INPUT = 0.90)
JSI METAL= 0.35 (G) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T23	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	5-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
U	7-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
V	9-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
X	15-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Y	17-11-4	-18	-18	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080962

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T24	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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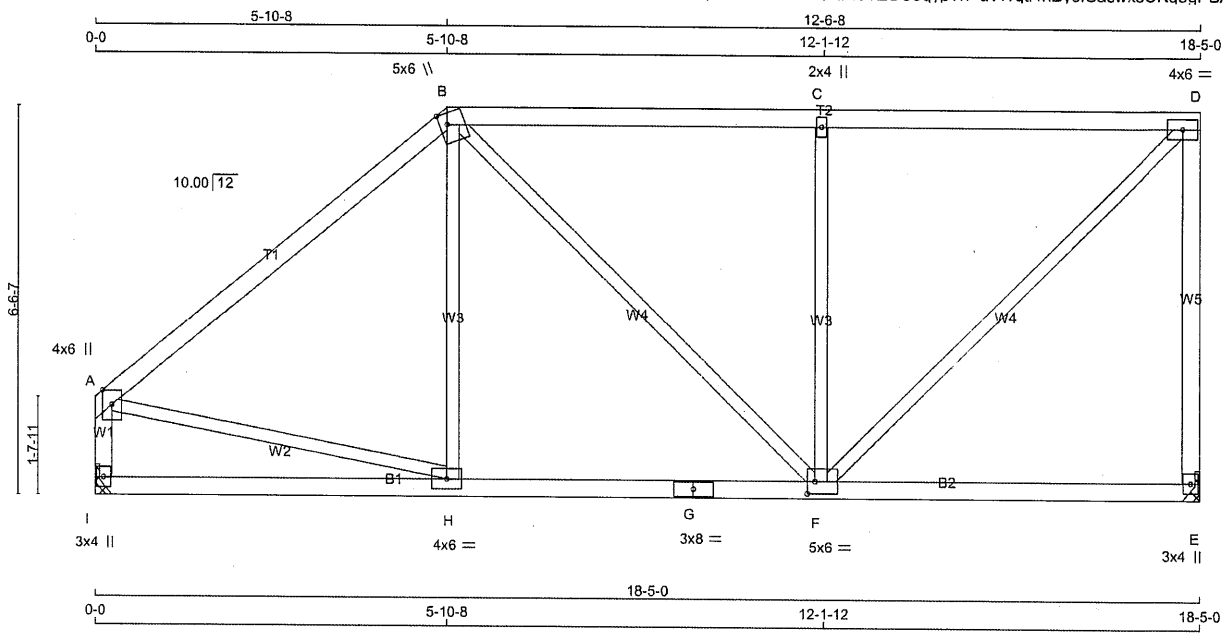

TOTAL WEIGHT = 80 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	5.0	6.0	2.50	1.50
G	BS-t	MT20	3.0	8.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1015	0	1015	0	0	MECHANICAL	
I	1015	0	1015	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, I. MINIMUM BEARING LENGTH AT JOINT E = 1-8, JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	718	471 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0
I	718	471 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-867 / 0	-91.8	-91.8	0.62 (1)	5.60	H- B	-22 / 86	0.03 (4)
B- C	-755 / 0	-91.8	-91.8	0.64 (1)	5.74	B- F	0 / 129	0.03 (1)
C- D	-755 / 0	-91.8	-91.8	0.64 (1)	5.74	F- C	-713 / 0	0.50 (1)
E- D	-967 / 0	0.0	0.0	0.85 (1)	7.81	F- D	0 / 1062	0.24 (1)
I- A	-971 / 0	0.0	0.0	0.10 (1)	7.81	A- H	0 / 680	0.15 (1)
I- H	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
H- G	0 / 664	-18.5	-18.5	0.22 (4)	10.00			
G- F	0 / 664	-18.5	-18.5	0.22 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.85/1.00 (D-E:1) , BC=0.22/1.00 (F-H:4) , WB=0.50/1.00 (C-F:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (F) (INPUT = 0.90)
JSI METAL= 0.43 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080963

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T25	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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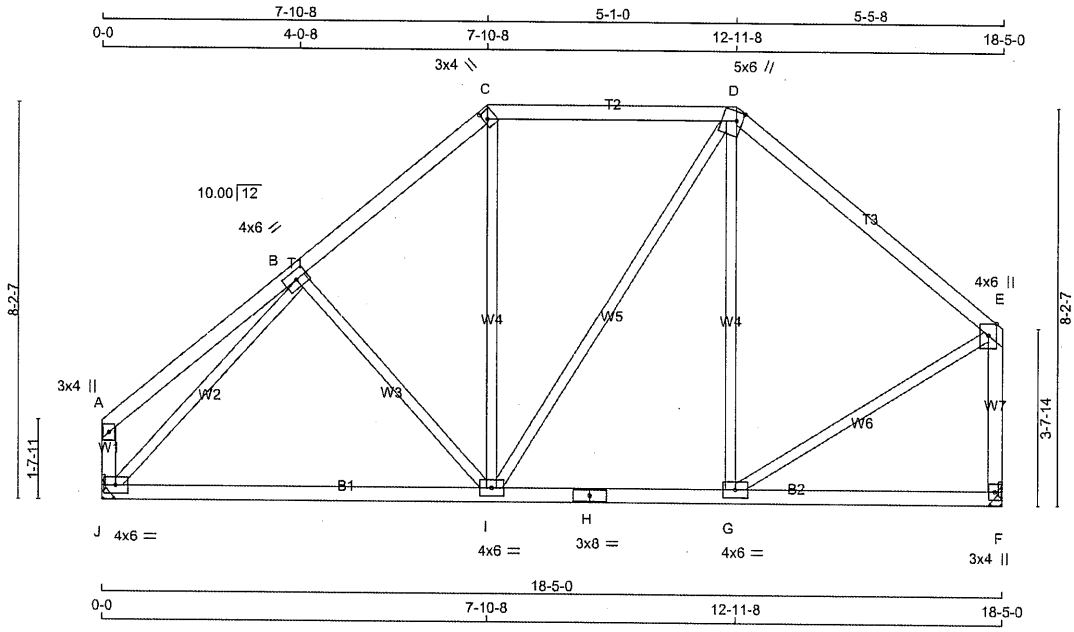

TOTAL WEIGHT = 86 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1015	0	1015	0	0	MECHANICAL	
F	1015	0	1015	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	718	471 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0
F	718	471 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-91.8	-91.8	0.23	(1)	10.00	B- I	-166 / 0	0.11 (1)
B- C	-797 / 0	-91.8	-91.8	0.18	(1)	6.25	I- C	0 / 121	0.04 (4)
C- D	-591 / 0	-91.8	-91.8	0.31	(1)	6.25	I- D	0 / 172	0.04 (1)
D- E	-655 / 0	-91.8	-91.8	0.36	(1)	6.25	G- D	-209 / 0	0.27 (1)
J- A	-137 / 0	0.0	0.0	0.01	(1)	7.81	J- B	-1076 / 0	0.66 (1)
F- E	-971 / 0	0.0	0.0	0.22	(1)	7.81	G- E	0 / 585	0.13 (1)
J- I	0 / 700	-18.5	-18.5	0.29	(4)	10.00			
I- H	0 / 500	-18.5	-18.5	0.28	(4)	10.00			
H- G	0 / 500	-18.5	-18.5	0.28	(4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.13	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.29/1.00 (I-J:4) , WB=0.66/1.00 (B-J:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR23080964

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T26	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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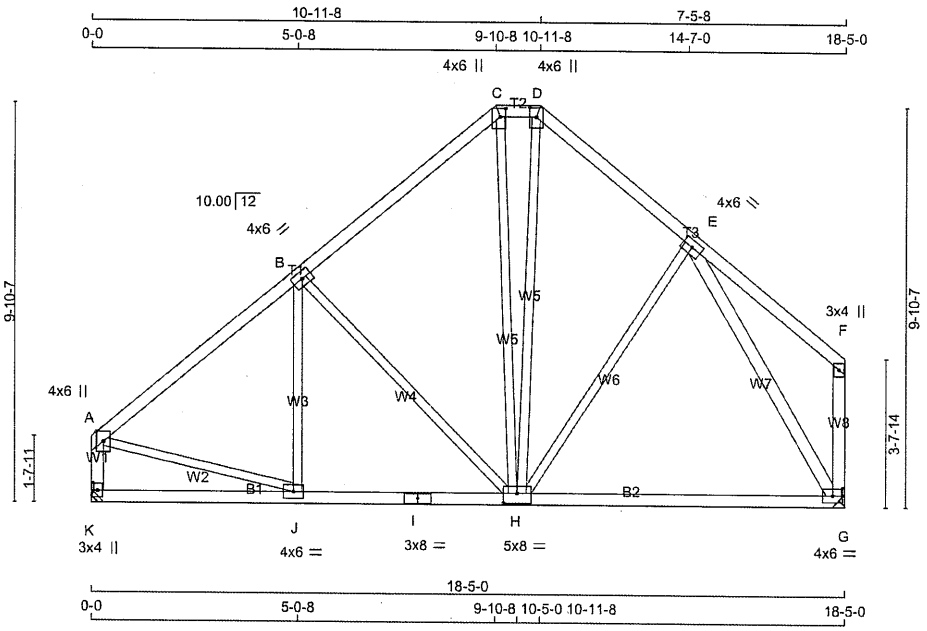

TOTAL WEIGHT = 97 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	2.50	1.50
D	TTW+p	MT20	4.0	6.0	2.75	2.00
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWWWW*-I	MT20	5.0	8.0	3.00	4.00
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1015	0	1015	0	0	MECHANICAL	
G	1015	0	1015	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, G. MINIMUM BEARING LENGTH AT JOINT K = 1-8, JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	718	471 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0
G	718	471 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-916 / 0	-91.8	-91.8	0.31 (1)	6.07	J- B	-102 / 36	0.05 (1)
B- C	-661 / 0	-91.8	-91.8	0.30 (1)	6.25	B- H	-358 / 0	0.42 (1)
C- D	-490 / 0	-91.8	-91.8	0.01 (1)	6.25	H- E	-2 / 41	0.01 (4)
D- E	-649 / 0	-91.8	-91.8	0.17 (1)	6.25	A- J	0 / 754	0.17 (1)
E- F	0 / 26	-91.8	-91.8	0.21 (1)	10.00	E- G	-952 / 0	0.72 (1)
K- A	-974 / 0	0.0	0.0	0.10 (1)	7.81	C- H	0 / 188	0.04 (1)
G- F	-131 / 0	0.0	0.0	0.03 (1)	7.81	H- D	0 / 222	0.05 (1)
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
J- I	0 / 730	-18.5	-18.5	0.31 (4)	10.00			
I- H	0 / 730	-18.5	-18.5	0.31 (4)	10.00			
H- G	0 / 479	-18.5	-18.5	0.29 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.31/1.00 (A-B:1) , BC=0.31/1.00 (H-J:4) , WB=0.72/1.00 (E-G:1) , SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (H) (INPUT = 0.90)
JSI METAL= 0.44 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080965

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T27	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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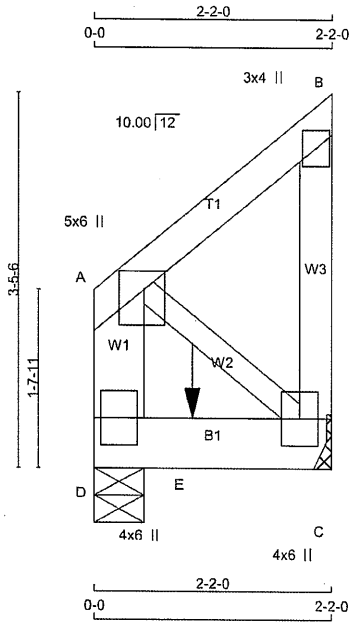

Scale = 1:20.3

TOTAL WEIGHT = 2 X 14 = 28 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		
D	BMV1+p	MT20	4.0	6.0		

STRUCTURAL COMPONENT ONLY
DWG # TR23080966

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	883	0	883	0	0	5-8	1-8
C	658	0	658	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	645	310 / 0	0 / 0	0 / 0	0 / 0	335 / 0	0 / 0
C	479	235 / 0	0 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-99 / 0	0.0	0.0	0.00 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.04 (1)	10.00			
C- B	-99 / 0	0.0	0.0	0.01 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.26 (1)	10.00			
E- C	0 / 0	-18.5	-18.5	0.26 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	10-12	-955	-955	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.04/1.00 (A-B:1) , BC=0.26/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.02 (B) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434363	T27	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080966

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T28	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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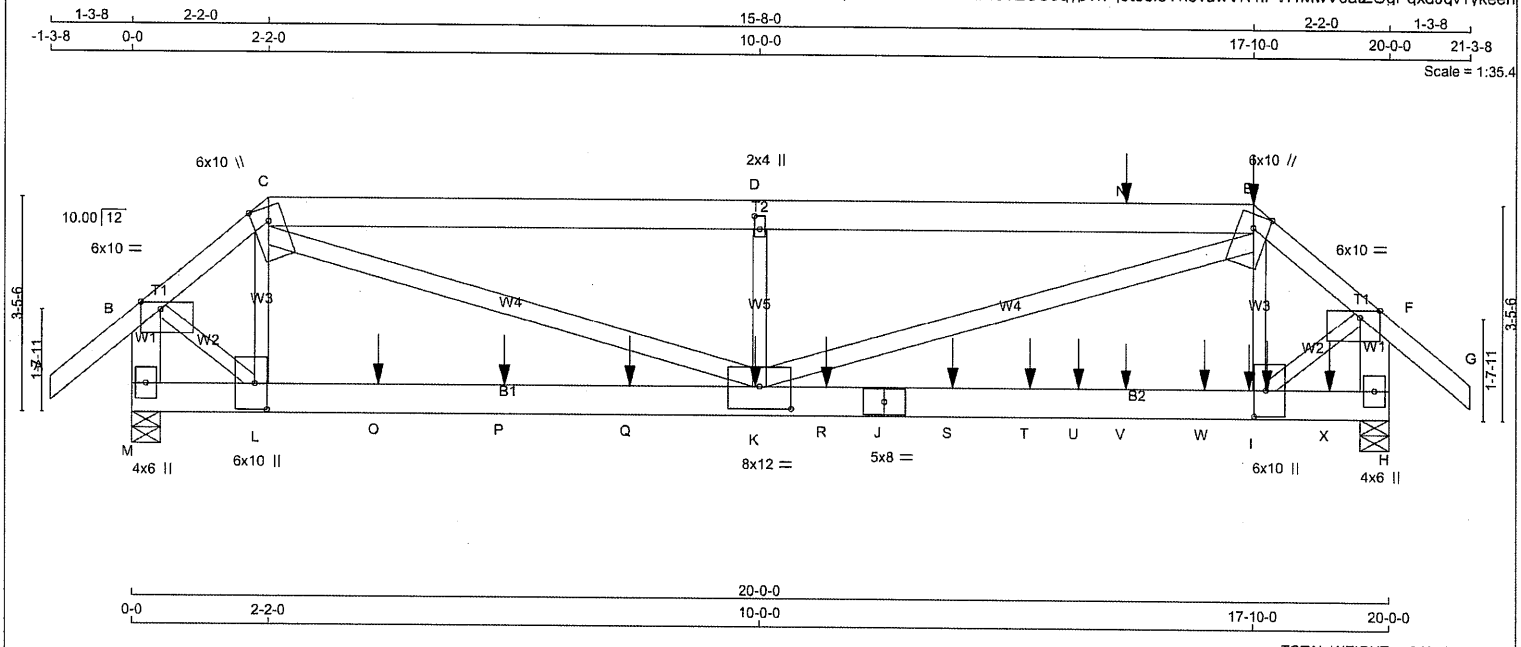

TOTAL WEIGHT = 2 X 107 = 214 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
E- G	1	12	SIDE(61.0)
C- E	2	12	SIDE(61.0)
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(197.8)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
D- K	1	6	SIDE(62.3)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080967

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	5688	0	5688	0	0	5-8	3-1
H	6968	0	6968	0	0	5-8	3-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	4017	2666 / 0	0 / 0	0 / 0	0 / 0	1350 / 0	0 / 0
H	4927	3240 / 0	0 / 0	0 / 0	0 / 0	1687 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 41	-91.8	-91.8	0.07 (1)	10.00	L- C	-62 / 465	0.06 (1)
B- C	-6111 / 0	-91.8	-91.8	0.16 (1)	3.85	C- K	0 / 8434	0.75 (1)
C- D	-12573 / 0	-91.8	-91.8	0.57 (1)	3.11	K- D	-873 / 0	0.09 (5)
D- N	-12573 / 0	-91.8	-91.8	0.57 (1)	3.10	K- E	0 / 7729	0.68 (1)
N- E	-12573 / 0	-91.8	-91.8	0.57 (1)	3.10	I- E	0 / 1259	0.16 (1)
E- F	-7000 / 0	-91.8	-91.8	0.20 (1)	3.58	B- L	0 / 5477	0.68 (1)
F- G	0 / 41	-91.8	-91.8	0.07 (1)	10.00	I- F	0 / 6278	0.78 (1)
M- B	-6900 / 0	0.0	0.0	0.25 (1)	5.68			
H- F	-7878 / 0	0.0	0.0	0.29 (1)	5.36			
M- L	0 / 0	-18.5	-18.5	0.51 (1)	10.00			
L- O	0 / 4699	-18.5	-18.5	0.66 (1)	10.00			
O- P	0 / 4699	-18.5	-18.5	0.66 (1)	10.00			
P- Q	0 / 4699	-18.5	-18.5	0.66 (1)	10.00			
Q- K	0 / 4699	-18.5	-18.5	0.66 (1)	10.00			
K- R	0 / 5358	-18.5	-18.5	0.75 (1)	10.00			
R- J	0 / 5358	-18.5	-18.5	0.75 (1)	10.00			
J- S	0 / 5358	-18.5	-18.5	0.75 (1)	10.00			
S- T	0 / 5358	-18.5	-18.5	0.75 (1)	10.00			
T- U	0 / 5358	-18.5	-18.5	0.75 (1)	10.00			
U- V	0 / 5358	-18.5	-18.5	0.75 (1)	10.00			
V- W	0 / 5358	-18.5	-18.5	0.75 (1)	10.00			
W- I	0 / 5358	-18.5	-18.5	0.75 (1)	10.00			
I- X	0 / 0	-18.5	-18.5	0.61 (1)	10.00			
X- H	0 / 0	-18.5	-18.5	0.61 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	17-10-0	-14	-14	125	FRONT	VERT	TOTAL	---	C1
I	17-9-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
I	18-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
K	9-11-4	-703	-703	---	BACK	VERT	TOTAL	---	C1
N	15-9-12	-5	-5	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-1090	-1090	---	BACK	VERT	TOTAL	---	C1
P	5-11-4	-703	-703	---	BACK	VERT	TOTAL	---	C1
Q	7-11-4	-703	-703	---	BACK	VERT	TOTAL	---	C1
R	11-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
S	13-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
T	14-3-8	-466	-466	---	FRONT	VERT	TOTAL	---	C1
U	15-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1
V	15-9-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
W	17-0-12	-703	-703	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 553 (0.43")

CSI: TC=0.57/1.00 (D-E:1) , BC=0.75/1.00 (I-K:1) , WB=0.78/1.00 (F-I:1) , SSI=0.83/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.94 (E) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434363	T28	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-j9t58I8YnovuwVK4IPVl4MwV0atZOgPqxdJqv1ykeeh

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	10.0	Edge	3.75
C	TTWW+m	MT20	6.0	10.0	Edge	
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TTWW+m	MT20	6.0	10.0	Edge	
F	TMVW-p	MT20	6.0	10.0	Edge	3.75
H	BMV1+p	MT20	4.0	6.0		
I	BMWW+t	MT20	6.0	10.0	5.00	2.25
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	8.0	12.0	4.25	6.00
L	BMWW+t	MT20	6.0	10.0	5.00	2.25
M	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	19-0-12	-704	-704	—	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080967

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T29W	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:52:36 2023 Page 1
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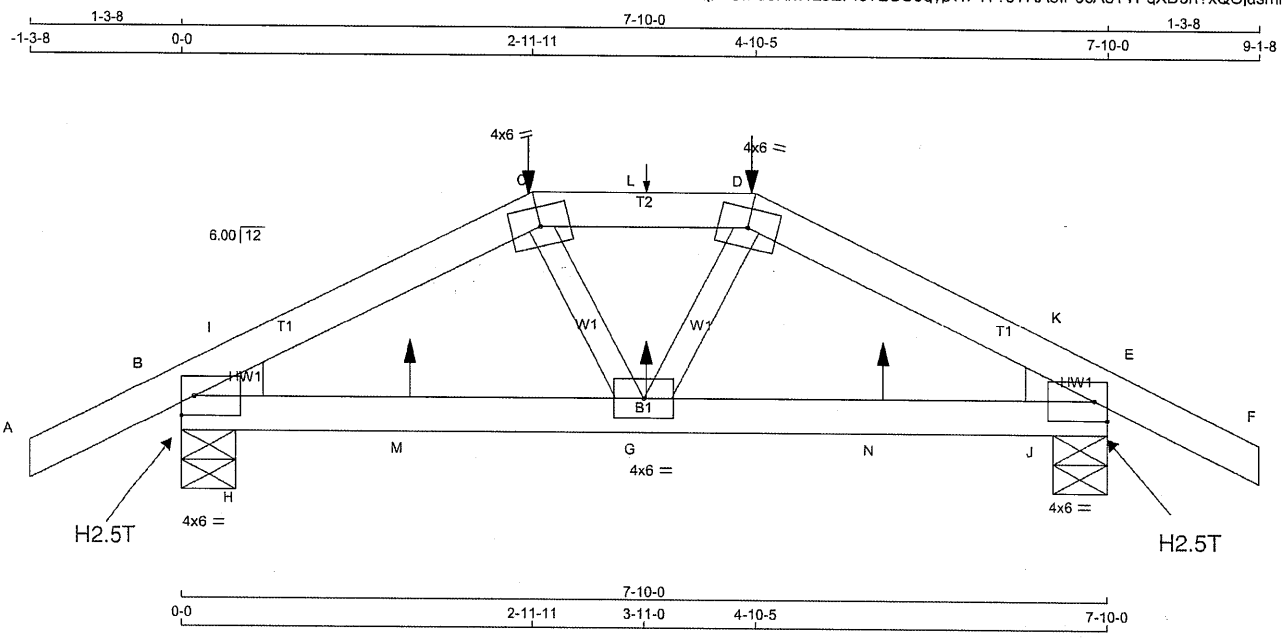

TOTAL WEIGHT = 26 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		Edge
C	TTW-m	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	6.0		
E	TMBH1-I	MT20	4.0	6.0		Edge
G	BMWW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080968

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	600	0	600	51	-360	5-8	1-8	2x4 L
E	600	0	600	0	-304	5-8	1-8	2x4 R

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 360 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 304 LBS FACTORED UPLIFT

PROVIDE FOR 51 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	422	292 / 0	0 / 0	0 / 0	0 / -340	129 / 0	0 / 0
E	422	292 / 0	0 / 0	0 / 0	0 / -300	129 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	36 / -36	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (19)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-91.8	-91.8	0.13 (1)	10.00	C- G	-93 / 45	0.02 (12)
B- I	-686 / 387	-91.8	-91.8	0.07 (1)	6.25	G- D	-101 / 45	0.02 (12)
I- C	-600 / 370	-91.8	-91.8	0.12 (7)	6.25	H- I	-85 / 52	0.00 (1)
C- L	-544 / 399	-91.8	-91.8	0.10 (7)	6.25	J- K	-72 / 52	0.00 (1)
L- D	-544 / 399	-91.8	-91.8	0.10 (7)	6.25			
D- K	-600 / 370	-91.8	-91.8	0.11 (8)	6.25			
K- E	-686 / 408	-91.8	-91.8	0.07 (7)	6.25			
E- F	0 / 19	-91.8	-91.8	0.13 (1)	10.00			
B- H	-293 / 530	-18.5	-18.5	0.10 (1)	6.25			
H- M	-293 / 530	-18.5	-18.5	0.12 (1)	6.25			
M- G	-293 / 530	-18.5	-18.5	0.12 (1)	6.25			
G- N	-278 / 530	-18.5	-18.5	0.12 (1)	6.25			
N- J	-278 / 530	-18.5	-18.5	0.12 (1)	6.25			
J- E	-278 / 530	-18.5	-18.5	0.10 (1)	6.25			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-0-1	-8	-8	---	FRONT	VERT	DEAD	---	C1
C	3-0-1	-31	-31	---	FRONT	VERT	SNOW	---	C1
D	4-9-15	-8	-8	---	FRONT	VERT	DEAD	---	C1
D	4-9-15	-31	-31	---	FRONT	VERT	SNOW	---	C1
G	3-11-4	6	-5	20	BACK	VERT	TOTAL	---	C1
L	3-11-4	1	-29	62	BACK	VERT	TOTAL	---	C1
M	1-11-4	6	-5	20	BACK	VERT	TOTAL	---	C1
N	5-11-4	6	-5	20	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.12/1.00 (G-J:1) , WB=0.02/1.00 (C-G:12) , SSI=0.10/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL = 0.14 (B) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434363	T29W	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-fY?sYRAoIP9cAoTTPqXD9n?xQOjusmn7Oxow_vykeef

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

STRUCTURAL COMPONENT ONLY
DWG # TR23080968

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T30W	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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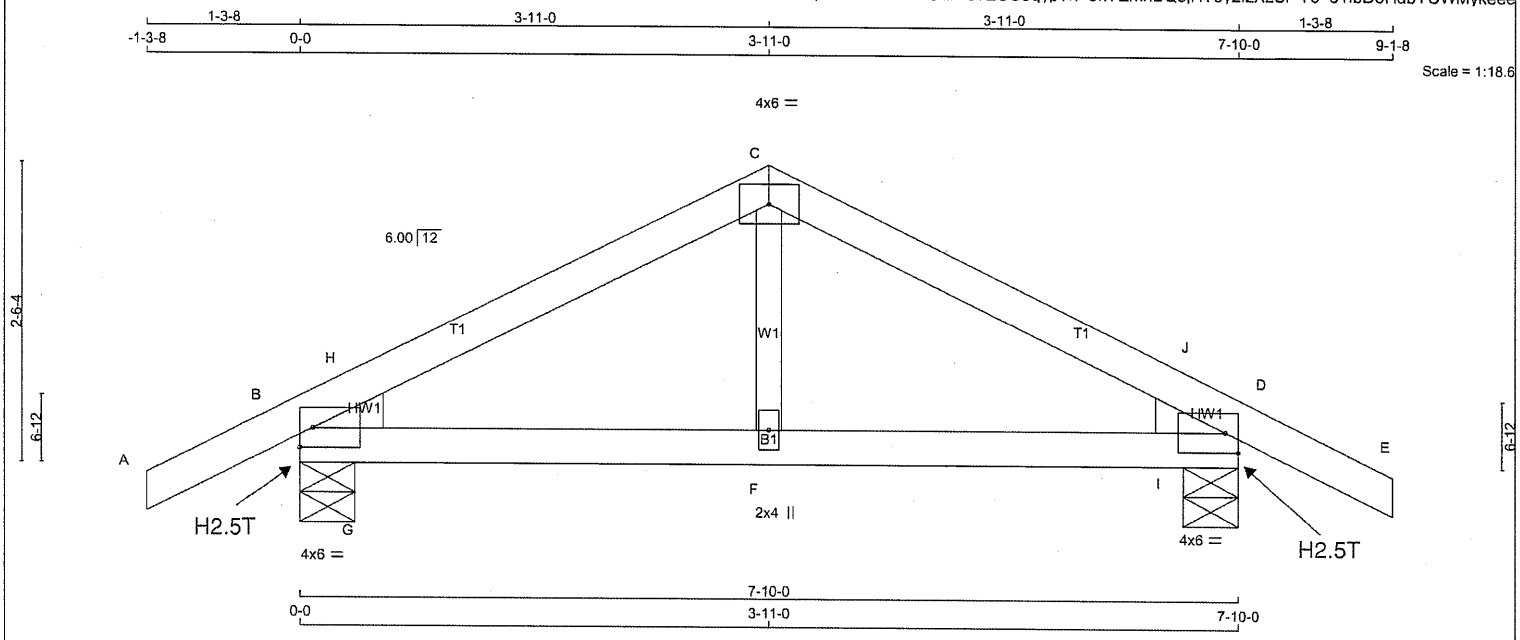

TOTAL WEIGHT = 2 X 25 = 51 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMBH1-I	MT20	4.0	6.0	Edge
C	TTW-p	MT20	4.0	6.0	
D	TMBH1-I	MT20	4.0	6.0	Edge
F	BMW+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	556	0	556	61	-341	5-8	1-8	2x4 L
D	556	0	556	0	-279	5-8	1-8	2x4 R

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 341 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 279 LBS FACTORED UPLIFT

PROVIDE FOR 61 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	391	270 / 0	0 / 0	0 / 0	0 / -322	121 / 0	0 / 0
D	391	270 / 0	0 / 0	0 / 0	0 / -277	121 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	44 / -44	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-91.8	-91.8	0.12 (1)	10.00	F- C	-173 / 134	0.03 (12)
B- H	-492 / 273	-91.8	-91.8	0.07 (1)	6.25	G- H	-162 / 74	0.00 (1)
H- C	-466 / 315	-91.8	-91.8	0.14 (7)	6.25	I- J	-145 / 134	0.00 (1)
C- J	-466 / 337	-91.8	-91.8	0.13 (1)	6.25			
J- D	-492 / 336	-91.8	-91.8	0.07 (7)	6.25			
D- E	0 / 19	-91.8	-91.8	0.12 (1)	10.00			
B- G	-239 / 409	-18.5	-18.5	0.15 (1)	6.25			
G- F	-239 / 409	-18.5	-18.5	0.15 (1)	6.25			
F- I	-239 / 409	-18.5	-18.5	0.15 (1)	6.25			
I- D	-239 / 409	-18.5	-18.5	0.15 (1)	6.25			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD.

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (C-H:7) , BC=0.15/1.00 (F-I:1) , WB=0.03/1.00 (C-F:12) , SSI=0.12/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080969

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	T31W	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 12:52:38 2023 Page 1
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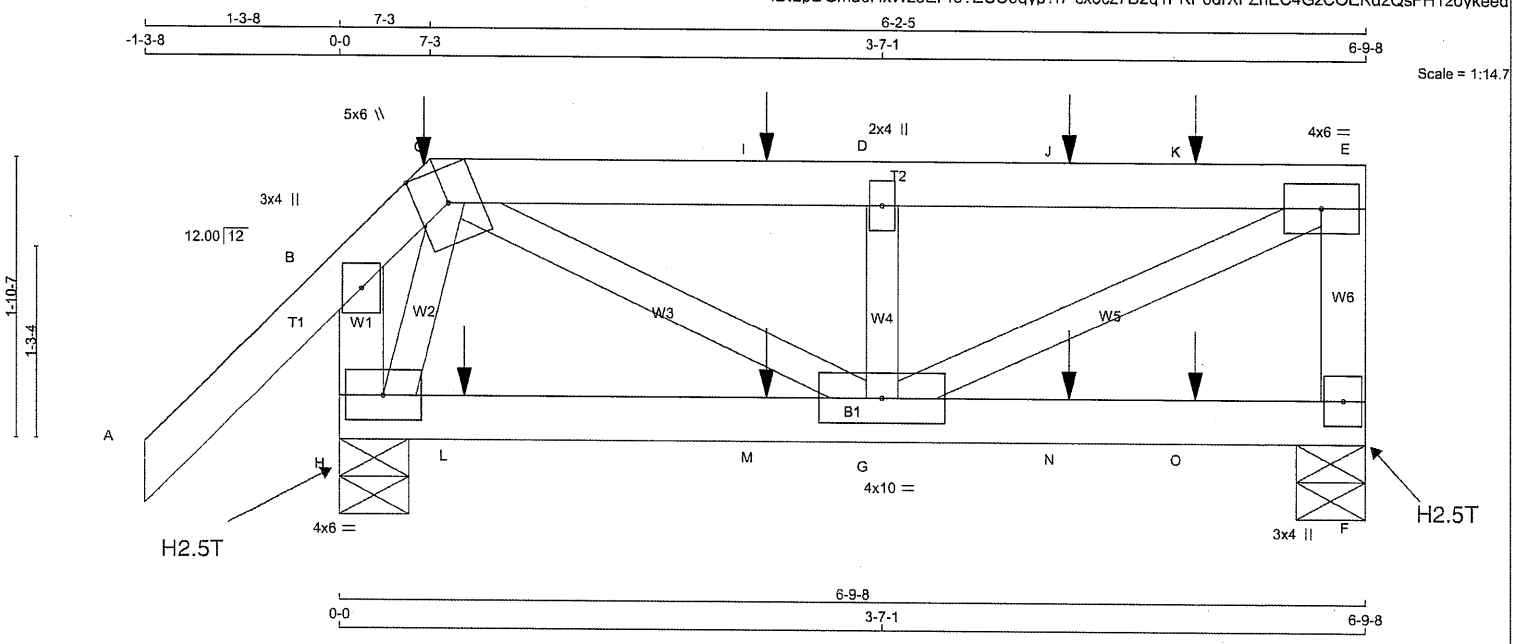

TOTAL WEIGHT = 28 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	10.0		
H	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080970

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
END VERTICALS ARE NOT EXPOSED TO WIND.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	662	0	662	0	-541	5-8	1-8
H	808	0	808	104	-565	5-8	1-8

PROVIDE ANCHORAGE AT BEARING JOINT F FOR 541 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 565 LBS FACTORED UPLIFT

PROVIDE FOR 104 LBS FACTORED HORIZONTAL REACTION AT JOINT H

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	468	305 / 0	0 / 0	0 / 0	0 / -491	163 / 0	0 / 0
H	569	386 / 0	0 / 0	0 / 0	0 / -521	183 / 0	0 / 0
HORIZONTAL REACTIONS							
H	---	0 / 0	0 / 0	0 / 0	74 / -27	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 45	-91.8	-91.8	0.14 (1)	10.00	C- G	-555 / 710	0.18 (1)
B- C	-114 / 205	-91.8	-91.8	0.14 (7)	6.25	G- D	-537 / 333	0.09 (1)
C- I	-810 / 677	-91.8	-91.8	0.24 (1)	6.25	G- E	-755 / 903	0.22 (1)
I- D	-810 / 677	-91.8	-91.8	0.24 (1)	6.25	H- C	-480 / 470	0.10 (5)
D- J	-810 / 677	-91.8	-91.8	0.25 (1)	6.25			
J- K	-810 / 677	-91.8	-91.8	0.25 (1)	6.25			
K- E	-810 / 677	-91.8	-91.8	0.25 (1)	6.25			
F- E	-594 / 458	0.0	0.0	0.07 (7)	7.81			
H- B	-319 / 271	0.0	0.0	0.04 (7)	7.81			
H- L	-188 / 185	-18.5	-18.5	0.13 (12)	6.25			
L- M	-188 / 185	-18.5	-18.5	0.13 (12)	6.25			
M- G	-188 / 185	-18.5	-18.5	0.13 (12)	6.25			
G- N	0 / 0	-18.5	-18.5	0.11 (12)	10.00			
N- O	0 / 0	-18.5	-18.5	0.11 (12)	10.00			
O- F	0 / 0	-18.5	-18.5	0.11 (12)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	7-14	-74	-74	74	FRONT	VERT	TOTAL	---	C1
I	2-9-15	-64	-64	72	FRONT	VERT	TOTAL	---	C1
J	4-9-15	-64	-64	72	FRONT	VERT	TOTAL	---	C1
K	5-7-15	-64	-64	72	FRONT	VERT	TOTAL	---	C1
L	9-15	-39	-39	48	FRONT	VERT	TOTAL	---	C1
M	2-9-15	-38	-38	48	FRONT	VERT	TOTAL	---	C1
N	4-9-15	-38	-38	48	FRONT	VERT	TOTAL	---	C1
O	5-7-15	-38	-38	48	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.13/1.00 (G-H:12) , WB=0.22/1.00 (E-G:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434363	T31W	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

STRUCTURAL COMPONENT ONLY
DWG # TR23080970

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434364	T32	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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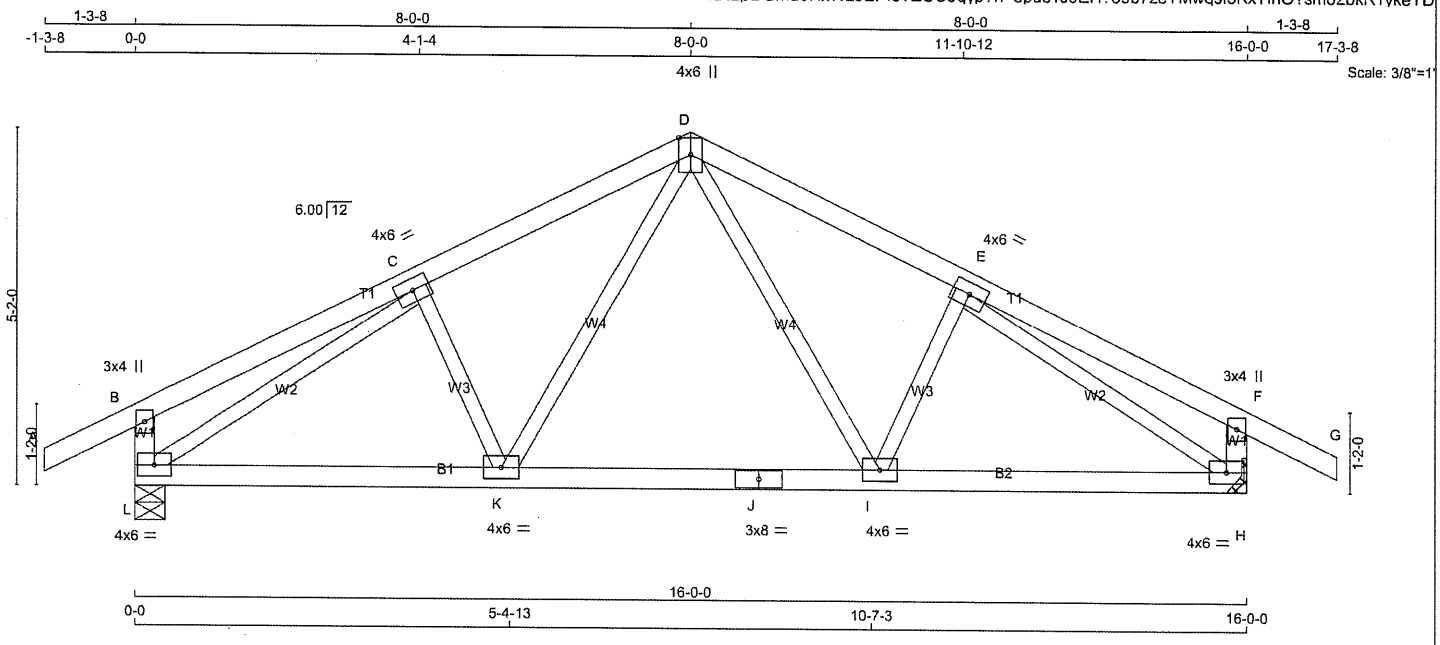

TOTAL WEIGHT = 3 X 64 = 193 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1007	0	1007	0	0	5-8	1-8
H	1007	0	1007	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	709	479 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0
H	709	479 / 0	0 / 0	0 / 0	0 / 0	231 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	D- I	0 / 344	0.08 (1)
B- C	0 / 18	-91.8	-91.8	0.23 (1)	10.00	I- E	-214 / 0	0.04 (1)
C- D	-1011 / 0	-91.8	-91.8	0.18 (1)	6.02	K- D	0 / 344	0.08 (1)
D- E	-1011 / 0	-91.8	-91.8	0.18 (1)	6.02	C- K	-214 / 0	0.04 (1)
E- F	0 / 18	-91.8	-91.8	0.23 (1)	10.00	L- C	-1201 / 0	0.46 (1)
F- G	0 / 28	-91.8	-91.8	0.12 (1)	10.00	E- H	-1201 / 0	0.46 (1)
L- B	-267 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-267 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 977	-18.5	-18.5	0.22 (1)	10.00			
K- J	0 / 721	-18.5	-18.5	0.18 (4)	10.00			
J- I	0 / 721	-18.5	-18.5	0.18 (4)	10.00			
I- H	0 / 977	-18.5	-18.5	0.22 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.22/1.00 (K-L:1) , WB=0.46/1.00 (C-L:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (E) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080973

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434364	T32G	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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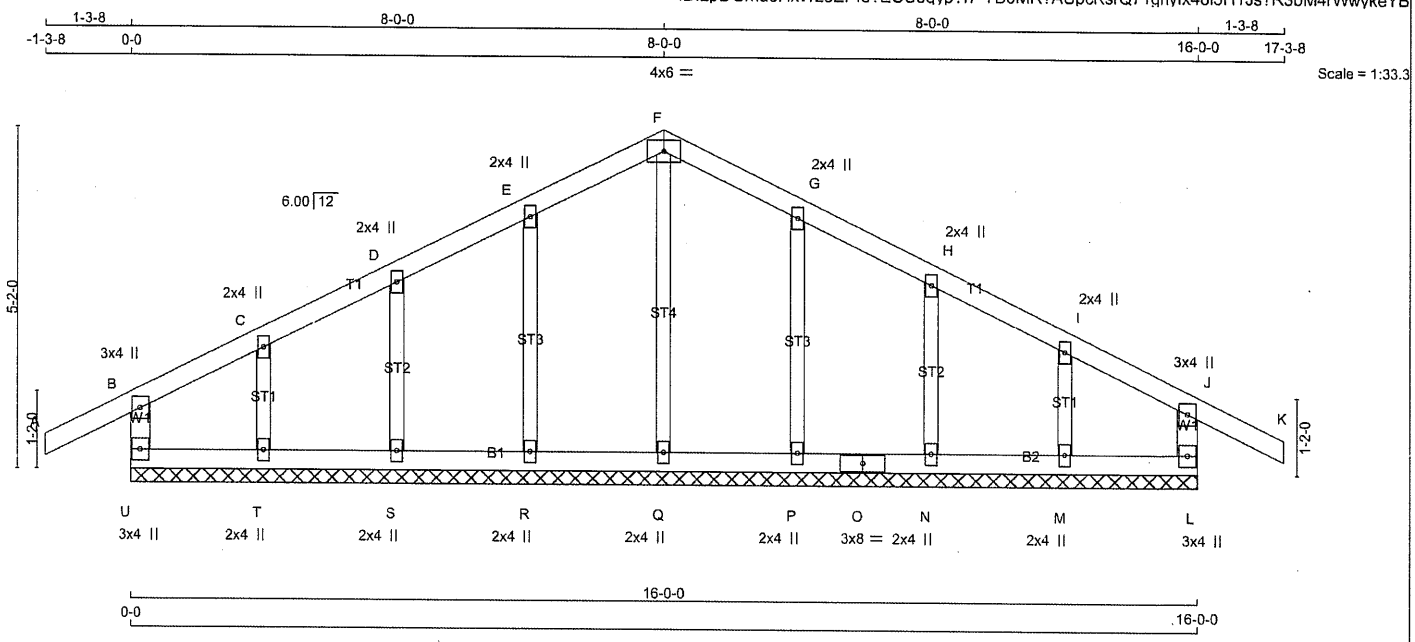

TOTAL WEIGHT = 60 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
U - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, G, H, I					
C	TMW+w	MT20	2.0	4.0	
F	TTW-p	MT20	4.0	6.0	
J	TMV+p	MT20	3.0	4.0	
L	BMV1+p	MT20	3.0	4.0	
M, N, P, Q, R, S, T					
M	BMW1+w	MT20	2.0	4.0	
O	BS-t	MT20	3.0	8.0	
U	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- B	-225 / 0	0.0	0.0	0.02 (1)	7.81	Q- F	-181 / 0	0.07 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	R- E	-185 / 0	0.05 (1)
B- C	-25 / 0	-91.8	-91.8	0.04 (1)	6.25	S- D	-182 / 0	0.03 (1)
C- D	-18 / 0	-91.8	-91.8	0.04 (1)	6.25	T- C	-177 / 0	0.03 (1)
D- E	-13 / 0	-91.8	-91.8	0.04 (1)	6.25	P- G	-185 / 0	0.05 (1)
E- F	-10 / 0	-91.8	-91.8	0.04 (1)	10.00	N- H	-182 / 0	0.03 (1)
F- G	-10 / 0	-91.8	-91.8	0.04 (1)	10.00	M- I	-177 / 0	0.03 (1)
G- H	-13 / 0	-91.8	-91.8	0.04 (1)	6.25			
H- I	-18 / 0	-91.8	-91.8	0.04 (1)	6.25			
I- J	-25 / 0	-91.8	-91.8	0.04 (1)	6.25			
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
L- J	-225 / 0	0.0	0.0	0.02 (1)	7.81			
U- T	0 / 22	-18.5	-18.5	0.02 (4)	10.00			
T- S	0 / 16	-18.5	-18.5	0.02 (4)	10.00			
S- R	0 / 12	-18.5	-18.5	0.02 (4)	10.00			
R- Q	0 / 9	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 9	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 12	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 12	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 16	-18.5	-18.5	0.02 (4)	10.00			
M- L	0 / 22	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (J-K:1) , BC=0.02/1.00 (L-M:4) , WB=0.07/1.00 (F-Q:1) , SSI=0.09/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.08 (G) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080974

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T40	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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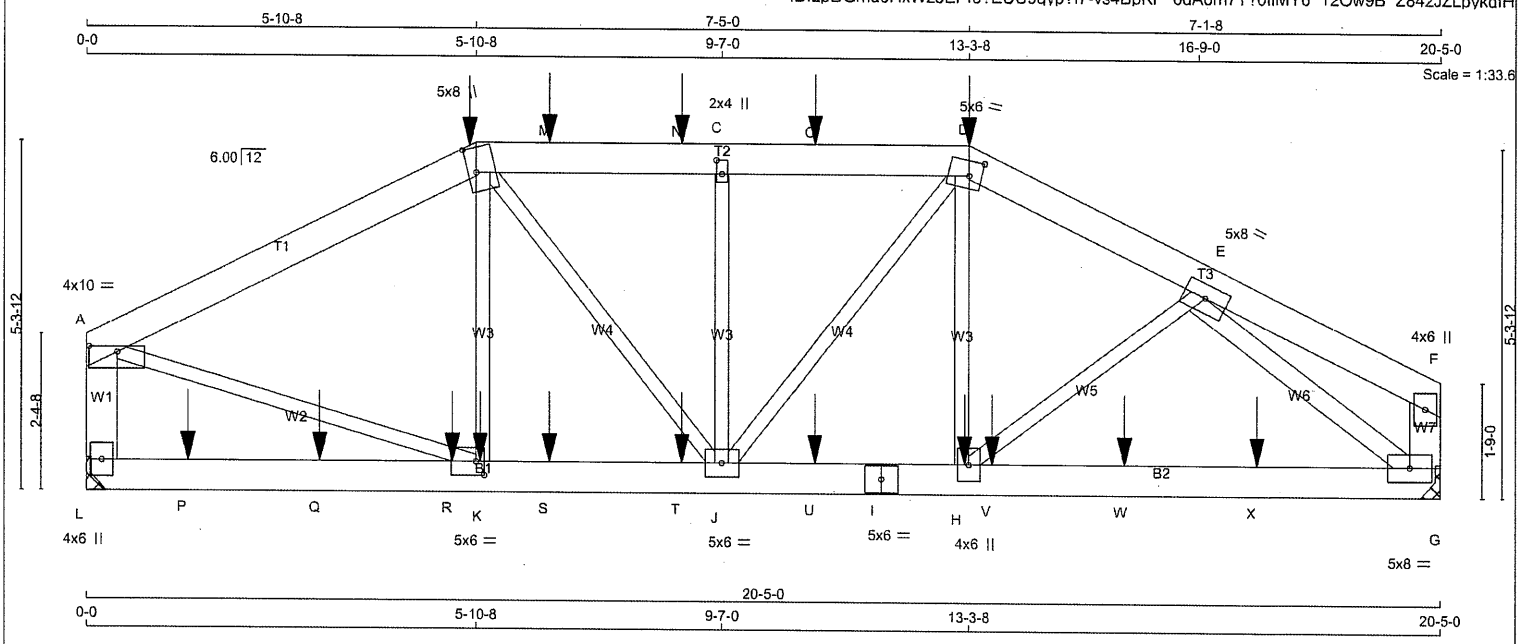


TOTAL WEIGHT = 114 lb [M]

LUMBER
N. L. G. A. RULES

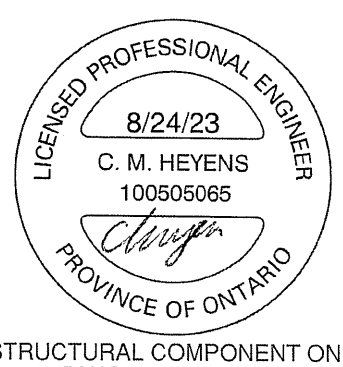
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
L - A	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	10.0	1.00	5.00
B	TTWW+m	MT20	5.0	8.0	4.50	1.50
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TTWW-m	MT20	5.0	6.0	2.75	2.25
E	TMWW-t	MT20	5.0	8.0		
F	TMV+p	MT20	4.0	6.0		
G	BMVW1-t	MT20	5.0	8.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	1.50
L	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR23080987

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	2589	0	2589	0	0	IN-SX	IN-SX
G	2421	0	2421	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L, G. MINIMUM BEARING LENGTH AT JOINT L = 2-15, JOINT G = 2-10.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1830	1203 / 0	0 / 0	0 / 0	0 / 0	627 / 0	0 / 0
G	1711	1129 / 0	0 / 0	0 / 0	0 / 0	582 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3050 / 0	-91.8	-91.8	0.37 (1)		4.43
B- M	-3148 / 0	-91.8	-91.8	0.24 (1)		4.51
M- N	-3148 / 0	-91.8	-91.8	0.24 (1)		4.51
N- C	-3148 / 0	-91.8	-91.8	0.24 (1)		4.51
C- O	-3148 / 0	-91.8	-91.8	0.22 (1)		4.54
O- D	-3148 / 0	-91.8	-91.8	0.22 (1)		4.54
D- E	-3318 / 0	-91.8	-91.8	0.15 (1)		4.52
E- F	-1 / 5	-91.8	-91.8	0.09 (1)		10.00
L- A	-2500 / 0	0.0	0.0	0.20 (1)		6.50
G- F	-151 / 0	0.0	0.0	0.01 (1)		7.81
L- P	0 / 0	-18.5	-18.5	0.15 (4)		10.00
P- Q	0 / 0	-18.5	-18.5	0.15 (4)		10.00
Q- R	0 / 0	-18.5	-18.5	0.15 (4)		10.00
R- K	0 / 0	-18.5	-18.5	0.15 (4)		10.00
K- S	0 / 2713	-18.5	-18.5	0.42 (1)		10.00
S- T	0 / 2713	-18.5	-18.5	0.42 (1)		10.00
T- J	0 / 2713	-18.5	-18.5	0.42 (1)		10.00
J- U	0 / 2958	-18.5	-18.5	0.48 (1)		10.00
U- I	0 / 2958	-18.5	-18.5	0.48 (1)		10.00
I- H	0 / 2958	-18.5	-18.5	0.48 (1)		10.00
H- V	0 / 2545	-18.5	-18.5	0.46 (1)		10.00
V- W	0 / 2545	-18.5	-18.5	0.46 (1)		10.00
W- X	0 / 2545	-18.5	-18.5	0.46 (1)		10.00
X- G	0 / 2545	-18.5	-18.5	0.46 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-566 / 41	0.21 (1)
B- J	0 / 716	0.18 (1)
J- C	-784 / 0	0.30 (1)
J- D	0 / 313	0.08 (1)
H- D	-32 / 200	0.08 (4)
H- E	0 / 536	0.13 (1)
A- K	0 / 2858	0.71 (1)
E- G	-3352 / 0	0.81 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-10-8	-556	-556	---	BACK	VERT	TOTAL	---	C1
D	13-3-8	-638	-638	---	BACK	VERT	TOTAL	---	C1
H	13-2-12	-32	-32	---	BACK	VERT	TOTAL	---	C1
K	5-11-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
M	6-11-12	-136	-136	---	BACK	VERT	TOTAL	---	C1
N	8-11-12	-136	-136	---	BACK	VERT	TOTAL	---	C1
O	10-11-12	-136	-136	---	BACK	VERT	TOTAL	---	C1
P	1-6-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
Q	3-6-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
R	5-6-4	-32	-32	---	BACK	VERT	TOTAL	---	C1
S	6-11-12	-32	-32	---	BACK	VERT	TOTAL	---	C1
T	8-11-12	-32	-32	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.37/1.00 (A-B:1) , BC=0.48/1.00 (H-J:1) , WB=0.81/1.00 (E-G:1) , SSI=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.62 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	10-11-12	-32	-32	---	BACK	VERT	TOTAL	---	C1
V	13-7-12	-32	-32	---	BACK	VERT	TOTAL	---	C1
W	15-7-12	-32	-32	---	BACK	VERT	TOTAL	---	C1
X	17-7-12	-32	-32	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080987

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T41	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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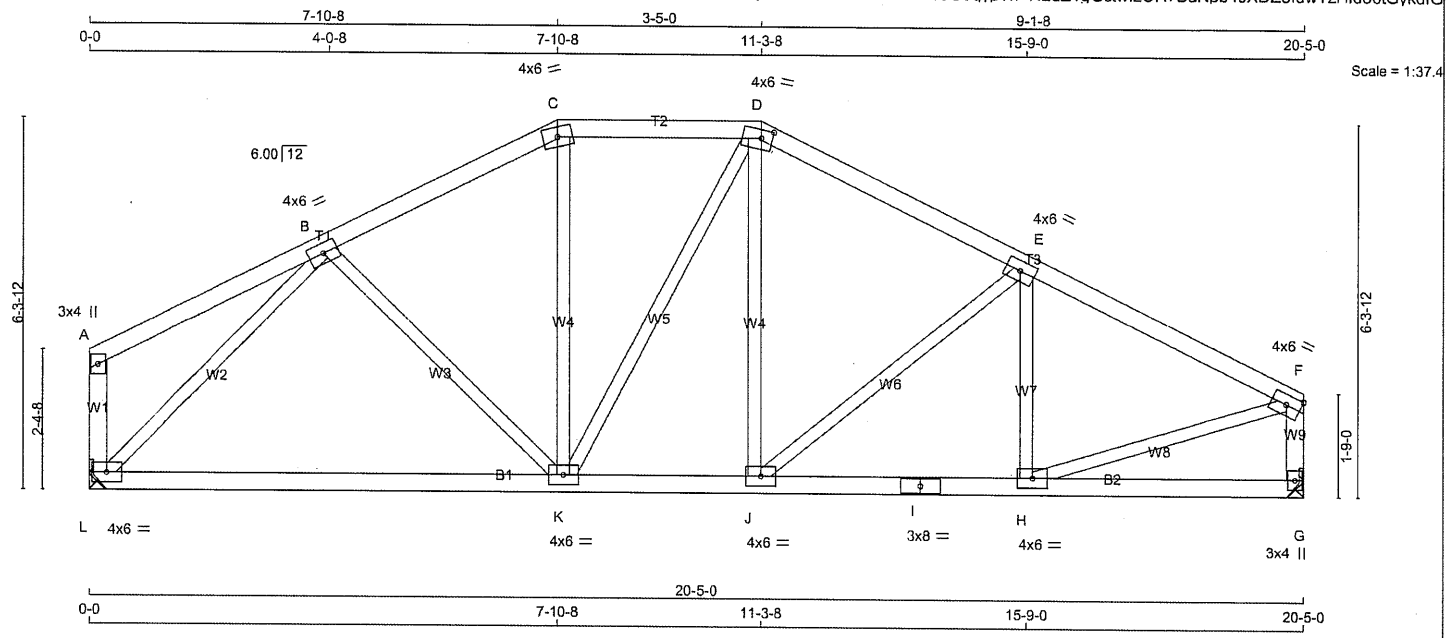

TOTAL WEIGHT = 87 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	6.0		Edge
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1126	0	1126	0	0	MECHANICAL	
G	1126	0	1126	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L, G. MINIMUM BEARING LENGTH AT JOINT L = 1-8, JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	796	522 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
G	796	522 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-91.8	-91.8	0.22 (1)	10.00
B- C	-1050 / 0	-91.8	-91.8	0.18 (1)	5.93
C- D	-925 / 0	-91.8	-91.8	0.15 (1)	6.25
D- E	-1076 / 0	-91.8	-91.8	0.25 (1)	5.79
E- F	-1256 / 0	-91.8	-91.8	0.26 (1)	5.46
L- A	-139 / 0	0.0	0.0	0.02 (1)	7.81
G- F	-1090 / 0	0.0	0.0	0.12 (1)	7.57
L- K	0 / 903	-18.5	-18.5	0.33 (4)	10.00
K- J	0 / 945	-18.5	-18.5	0.34 (4)	10.00
J- I	0 / 1142	-18.5	-18.5	0.23 (1)	10.00
I- H	0 / 1142	-18.5	-18.5	0.23 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	0 / 64	0.02 (4)
K- C	0 / 167	0.04 (4)
K- D	-41 / 17	0.04 (1)
J- D	0 / 198	0.04 (1)
J- E	-261 / 0	0.16 (1)
H- E	-252 / 0	0.06 (1)
L- B	-1293 / 0	0.67 (1)
H- F	0 / 1196	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.26/1.00 (E-F:1) , BC=0.34/1.00 (J-K:4) , WB=0.67/1.00 (B-L:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (H) (INPUT = 0.90)
JSI METAL= 0.38 (I) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080988

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T42	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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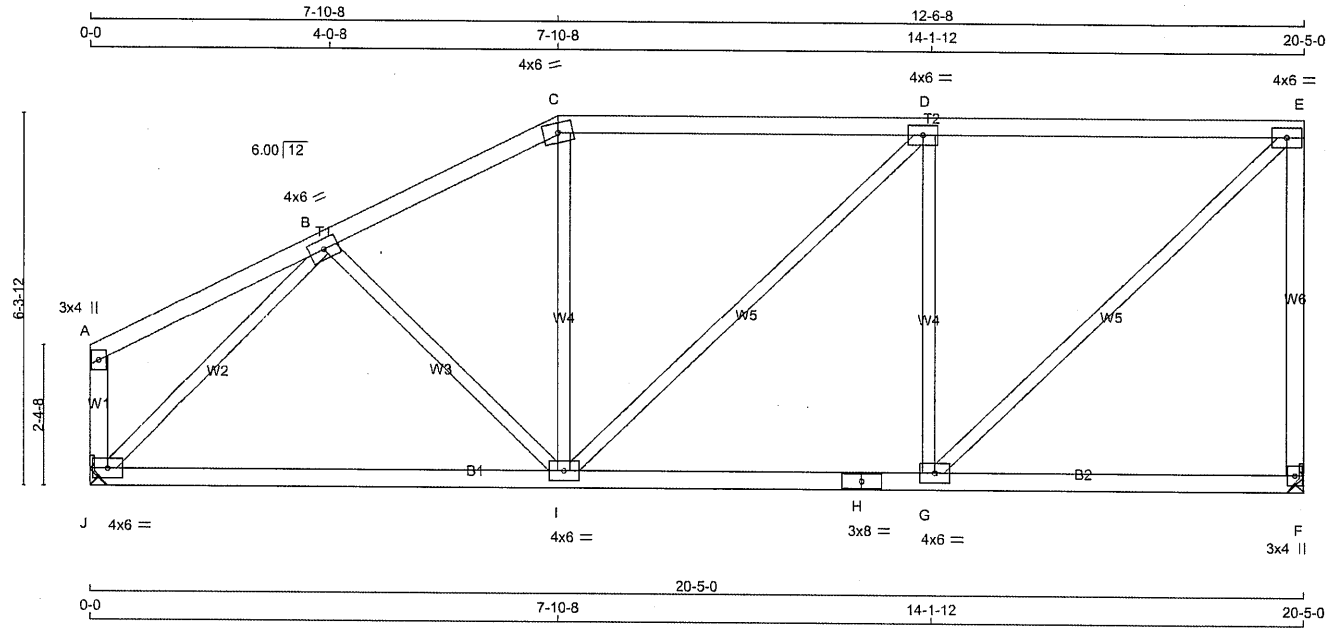

TOTAL WEIGHT = 88 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1126	0	1126	0	0	IN-SX	IN-SX
J	1126	0	1126	0	0	MECHANICAL	
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, J. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	796	522 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
J	796	522 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-91.8	-91.8	0.23	(1)	10.00	B- I	0 / 63	0.02 (4)
B- C	-1049 / 0	-91.8	-91.8	0.24	(1)	5.82	I- C	0 / 107	0.03 (4)
C- D	-923 / 0	-91.8	-91.8	0.66	(1)	5.30	I- D	0 / 37	0.01 (4)
D- E	-898 / 0	-91.8	-91.8	0.66	(1)	5.36	G- D	-739 / 0	0.47 (1)
F- E	-1076 / 0	0.0	0.0	0.85	(1)	7.61	G- E	0 / 1240	0.28 (1)
J- A	-139 / 0	0.0	0.0	0.02	(1)	7.81	J- B	-1293 / 0	0.67 (1)
J- I	0 / 902	-18.5	-18.5	0.32	(4)	10.00			
I- H	0 / 898	-18.5	-18.5	0.32	(4)	10.00			
H- G	0 / 898	-18.5	-18.5	0.32	(4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.15	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.85/1.00 (E-F:1) , BC=0.32/1.00 (G-I:4) , WB=0.67/1.00 (B-J:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSI METAL= 0.28 (H) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080989

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T43	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 14:00:15 2023 Page 1
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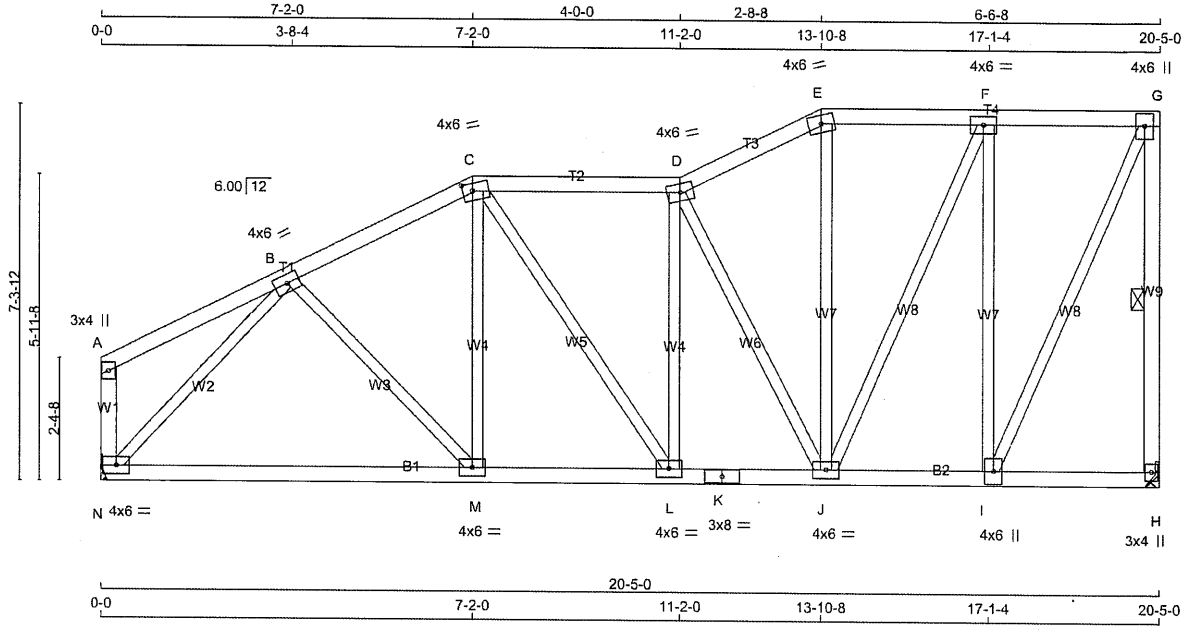

TOTAL WEIGHT = 108 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTWW-m	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080990

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1126	0	1126	0	0	MECHANICAL	
N	1126	0	1126	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, N. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT N = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	796	522 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
N	796	522 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 17	-91.8	-91.8	0.18 (1)		10.00
B- C	-1069 / 0	-91.8	-91.8	0.20 (1)		5.85
C- D	-1003 / 0	-91.8	-91.8	0.27 (1)		5.88
D- E	-788 / 0	-91.8	-91.8	0.12 (1)		6.25
E- F	-706 / 0	-91.8	-91.8	0.15 (1)		6.25
F- G	-462 / 0	-91.8	-91.8	0.15 (1)		6.25
H- G	-1100 / 0	0.0	0.0	0.26 (1)		6.03
N- A	-127 / 0	0.0	0.0	0.02 (1)		7.81
N- M	0 / 875	-18.5	-18.5	0.28 (4)		10.00
M- L	0 / 941	-18.5	-18.5	0.29 (4)		10.00
L- K	0 / 1004	-18.5	-18.5	0.19 (1)		10.00
K- J	0 / 1004	-18.5	-18.5	0.19 (1)		10.00
J- I	0 / 462	-18.5	-18.5	0.10 (1)		10.00
I- H	0 / 0	-18.5	-18.5	0.04 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- M	0 / 94	0.03 (4)
M- C	0 / 98	0.03 (4)
C- L	0 / 109	0.02 (1)
L- D	-57 / 24	0.03 (1)
D- J	-673 / 0	0.48 (1)
J- E	0 / 111	0.03 (4)
N- B	-1286 / 0	0.57 (1)
J- F	0 / 600	0.13 (1)
I- F	-911 / 0	0.86 (1)
I- G	0 / 1084	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.29/1.00 (L-M:4) , WB=0.86/1.00 (F-I:1) , SSI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (J) (INPUT = 0.90)
JSI METAL= 0.34 (K) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T44	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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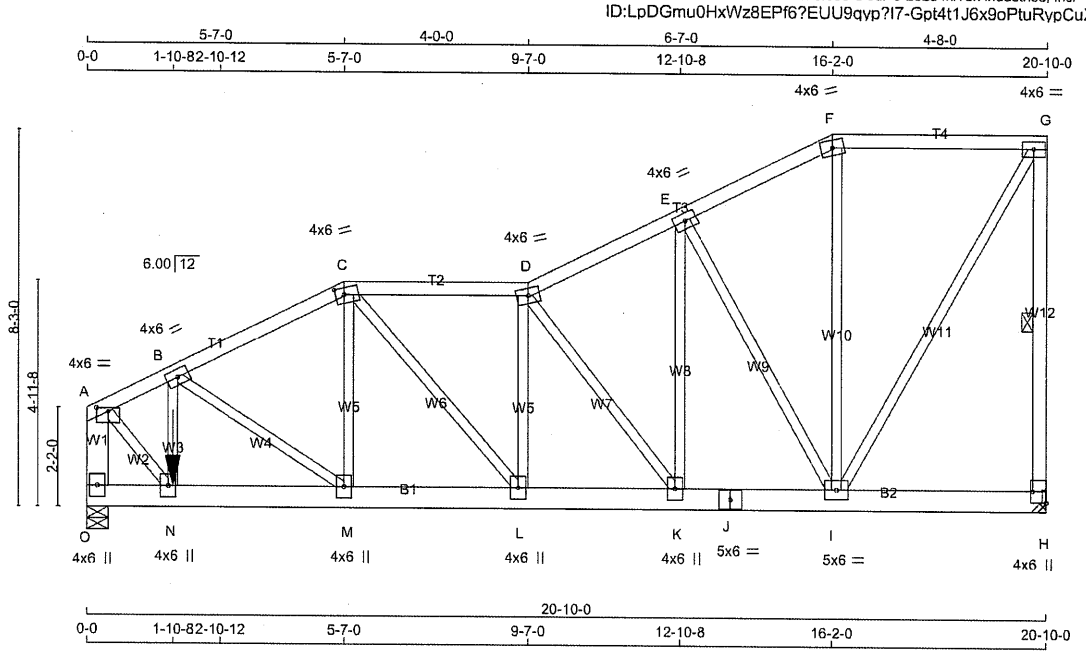

TOTAL WEIGHT = 2 X 121 = 241 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
G- H	1	12	TOP
O- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- J	2	12	SIDE(183.1)
J- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
B- N	1	4	SIDE(294.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080991

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1313	0	1313	0	0	MECHANICAL	
O	2814	0	2814	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	928	611 / 0	0 / 0	0 / 0	0 / 0	317 / 0	0 / 0
O	1986	1326 / 0	0 / 0	0 / 0	0 / 0	661 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2050 / 0	-91.8	-91.8	0.08 (1)	6.09
B- C	-1779 / 0	-91.8	-91.8	0.10 (1)	6.25
C- D	-1696 / 0	-91.8	-91.8	0.15 (1)	6.25
D- E	-1249 / 0	-91.8	-91.8	0.10 (1)	6.25
E- F	-707 / 0	-91.8	-91.8	0.09 (1)	6.25
F- G	-616 / 0	-91.8	-91.8	0.19 (1)	6.25
H- G	-1277 / 0	0.0	0.0	0.19 (1)	6.25
O- A	-2718 / 0	0.0	0.0	0.11 (1)	7.81
O- N	0 / 0	-18.5	-18.5	0.05 (1)	10.00
N- M	0 / 1852	-18.5	-18.5	0.17 (1)	10.00
M- L	0 / 1588	-18.5	-18.5	0.12 (1)	10.00
L- K	0 / 1697	-18.5	-18.5	0.13 (1)	10.00
K- J	0 / 1133	-18.5	-18.5	0.09 (1)	10.00
J- I	0 / 1133	-18.5	-18.5	0.09 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- M	-333 / 0	0.06 (1)
M- C	0 / 337	0.04 (1)
C- L	0 / 166	0.02 (1)
L- D	-77 / 24	0.01 (1)
D- K	-966 / 0	0.26 (1)
K- E	0 / 823	0.10 (1)
E- I	-1077 / 0	0.51 (1)
I- F	-19 / 46	0.01 (1)
I- G	0 / 1230	0.15 (1)
N- B	0 / 35	0.01 (4)
A- N	0 / 2549	0.32 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	1-10-8	-1290	-1290	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.19/1.00 (G-H:1) , BC=0.17/1.00 (M-N:1) , WB=0.51/1.00 (E-I:1) , SSI=0.09/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (N) (INPUT = 0.90)
JSI METAL= 0.42 (N) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434613	T44	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-Gpt4t1J6x9oPtuRypCuXF9hwZPi5sKRtDF1J01ykdfC

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTWW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TMVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	4.0	6.0	3.00	Edge
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K, L, M, N						
K	BMWW+t	MT20	4.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080991

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T45	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 14:00:18 2023 Page 1
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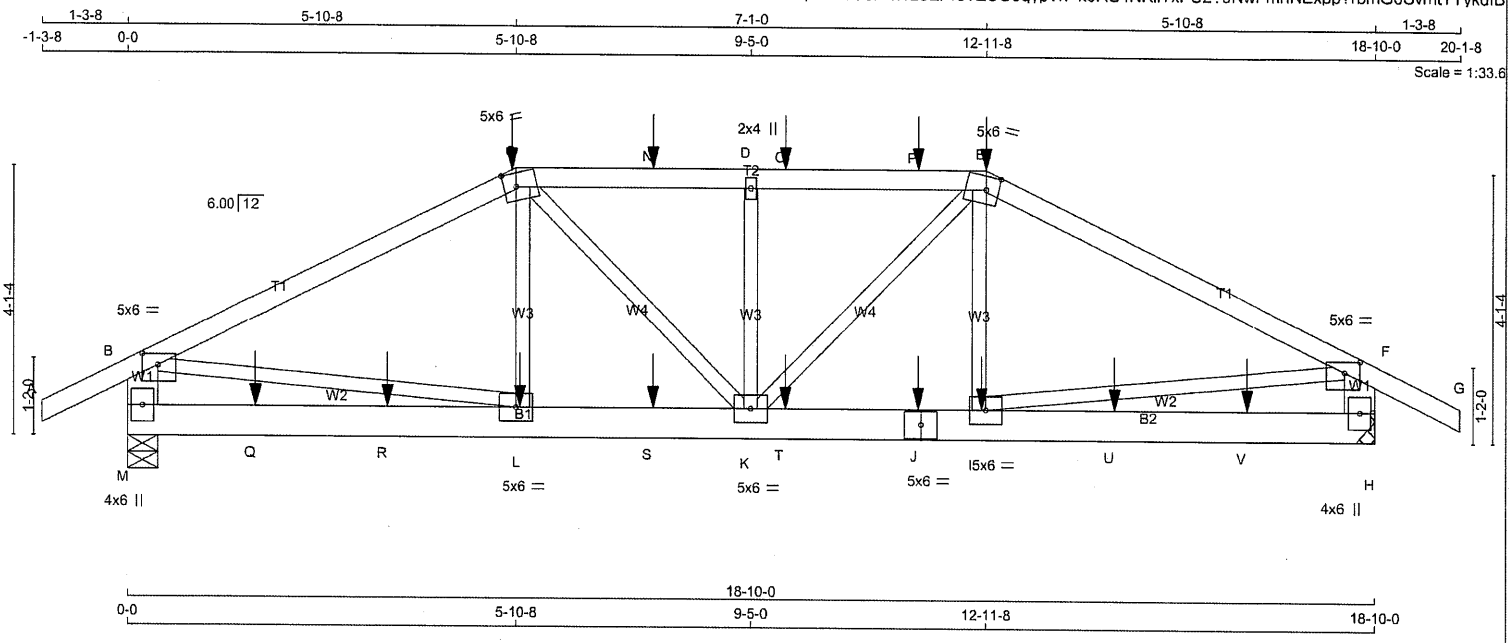

TOTAL WEIGHT = 87 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TTWW-m	MT20	5.0	6.0	2.50	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.25
F	TMVW-p	MT20	5.0	6.0	2.00	2.75
H	BMV1+p	MT20	4.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY
DWG # TR23080992

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1802	0	1802	0	0	5-8	1-15
H	1825	0	1825	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1274	836 / 0	0 / 0	0 / 0	0 / 0	439 / 0	0 / 0
H	1290	846 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	L- C	-92 / 117	0.04 (4)
B- C	-2361 / 0	-91.8	-91.8	0.86 (1)	3.41	C- K	0 / 491	0.12 (1)
C- N	-2446 / 0	-91.8	-91.8	0.40 (1)	3.92	K- D	-618 / 0	0.16 (1)
N- D	-2446 / 0	-91.8	-91.8	0.40 (1)	3.92	K- E	0 / 439	0.11 (1)
D- O	-2446 / 0	-91.8	-91.8	0.40 (1)	3.92	I- E	-85 / 130	0.05 (4)
O- P	-2446 / 0	-91.8	-91.8	0.40 (1)	3.92	B- L	0 / 2128	0.53 (1)
P- E	-2446 / 0	-91.8	-91.8	0.40 (1)	3.92	I- F	0 / 2164	0.54 (1)
E- F	-2401 / 0	-91.8	-91.8	0.87 (1)	3.37			
F- G	0 / 28	-91.8	-91.8	0.13 (1)	10.00			
M- B	-1726 / 0	0.0	0.0	0.12 (1)	7.52			
H- F	-1748 / 0	0.0	0.0	0.12 (1)	7.48			
M- Q	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
Q- R	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
R- L	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
L- S	0 / 2108	-18.5	-18.5	0.33 (1)	10.00			
S- K	0 / 2108	-18.5	-18.5	0.33 (1)	10.00			
K- T	0 / 2144	-18.5	-18.5	0.33 (1)	10.00			
T- J	0 / 2144	-18.5	-18.5	0.33 (1)	10.00			
J- I	0 / 2144	-18.5	-18.5	0.33 (1)	10.00			
I- U	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
U- V	0 / 0	-18.5	-18.5	0.13 (4)	10.00			
V- H	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-254	-254	---	FRONT	VERT	TOTAL	---	C1
E	12-11-8	-254	-254	---	FRONT	VERT	TOTAL	---	C1
I	12-10-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
J	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
L	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	7-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
O	9-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
P	11-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Q	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
R	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.87/1.00 (E-F:1) , BC=0.33/1.00 (I-K:1) , WB=0.54/1.00 (F-I:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.49 (L) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434613	T45	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	14-10-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
V	16-10-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080992

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T46	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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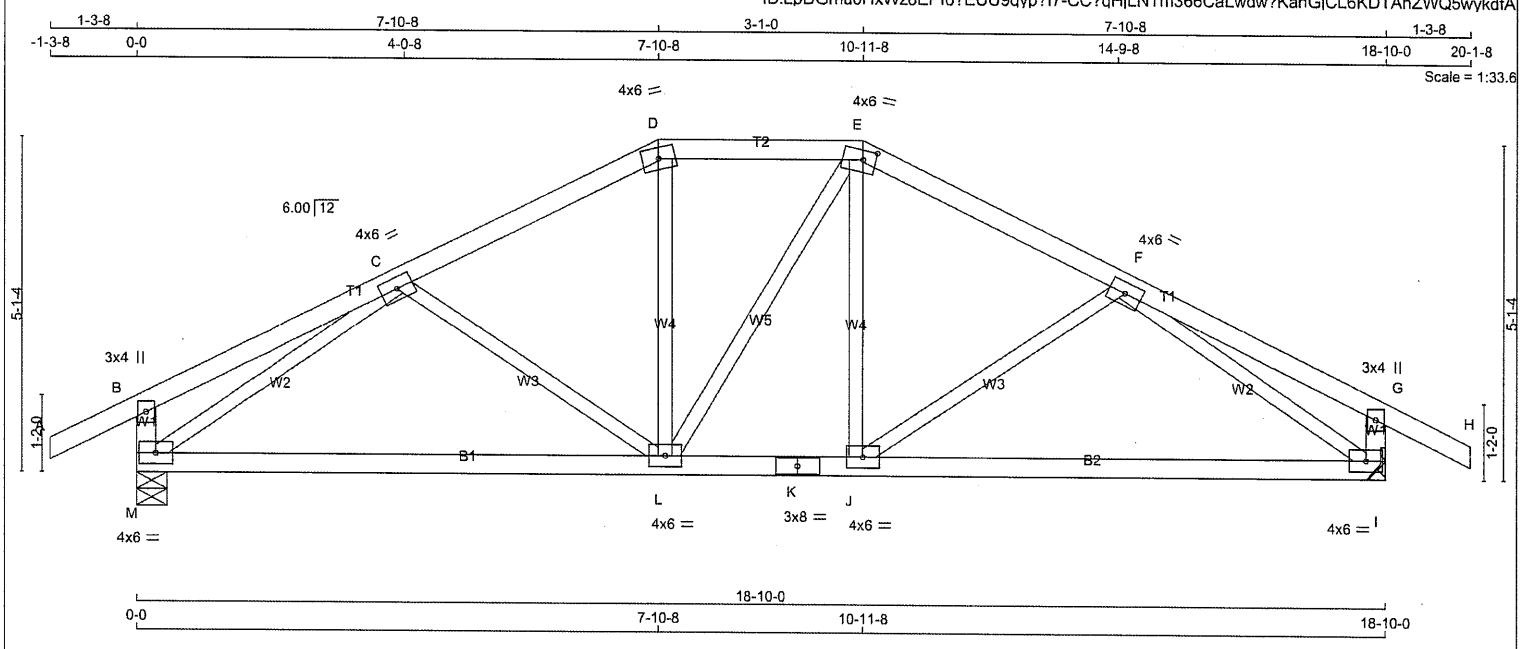

TOTAL WEIGHT = 77 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	6.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080993

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1163	0	1163	0	0	5-8	IN-SX
I	1163	0	1163	0	0	MECHANICAL	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	820	551 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
I	820	551 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 18	-91.8	-91.8	0.22 (1)	10.00
C- D	-1142 / 0	-91.8	-91.8	0.18 (1)	5.76
D- E	-1010 / 0	-91.8	-91.8	0.12 (1)	6.10
E- F	-1142 / 0	-91.8	-91.8	0.18 (1)	5.76
F- G	0 / 18	-91.8	-91.8	0.22 (1)	10.00
G- H	0 / 28	-91.8	-91.8	0.12 (1)	10.00
M- B	-264 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-263 / 0	0.0	0.0	0.03 (1)	7.81
M- L	0 / 1163	-18.5	-18.5	0.33 (1)	10.00
L- K	0 / 1009	-18.5	-18.5	0.32 (4)	10.00
K- J	0 / 1009	-18.5	-18.5	0.32 (4)	10.00
J- I	0 / 1162	-18.5	-18.5	0.33 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-191 / 0	0.08 (1)
L- D	0 / 220	0.05 (1)
L- E	0 / 0	0.00 (1)
J- E	0 / 219	0.05 (1)
J- F	-191 / 0	0.08 (1)
M- C	-1433 / 0	0.54 (1)
F- I	-1432 / 0	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.22/1.00 (F-G:1) , BC=0.33/1.00 (I-J:1) , WB=0.54/1.00 (C-M:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.53 (K) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T47	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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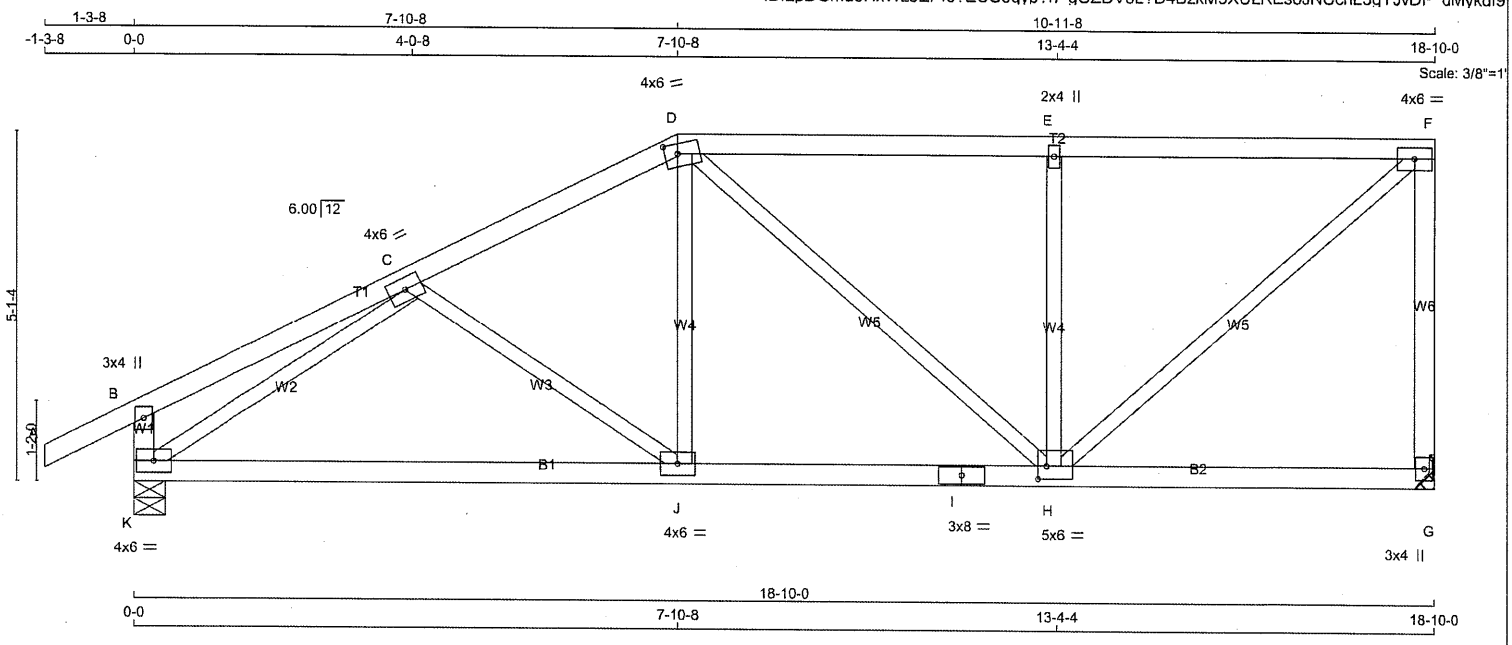

TOTAL WEIGHT = 78 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1038	0	1038	0	0	MECHANICAL	
K	1163	0	1163	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	734	482 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	820	551 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B- C	0 / 18	-91.8	-91.8	0.22 (1)	10.00	
C- D	-1144 / 0	-91.8	-91.8	0.24 (1)	5.65	
D- E	-918 / 0	-91.8	-91.8	0.49 (1)	5.69	
E- F	-918 / 0	-91.8	-91.8	0.49 (1)	5.69	
G- F	-993 / 0	0.0	0.0	0.44 (1)	7.81	
K- B	-266 / 0	0.0	0.0	0.03 (1)	7.81	
K- J	0 / 1164	-18.5	-18.5	0.33 (4)	10.00	
J- I	0 / 1012	-18.5	-18.5	0.33 (4)	10.00	
I- H	0 / 1012	-18.5	-18.5	0.33 (4)	10.00	
H- G	0 / 0	-18.5	-18.5	0.12 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-193 / 0	0.08 (1)
J- D	0 / 258	0.06 (1)
D- H	-126 / 0	0.12 (1)
H- E	-622 / 0	0.24 (1)
H- F	0 / 1217	0.27 (1)
K- C	-1433 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.49/1.00 (E-F:1) , BC=0.33/1.00 (H-J:4) , WB=0.55/1.00 (C-K:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080994

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T48	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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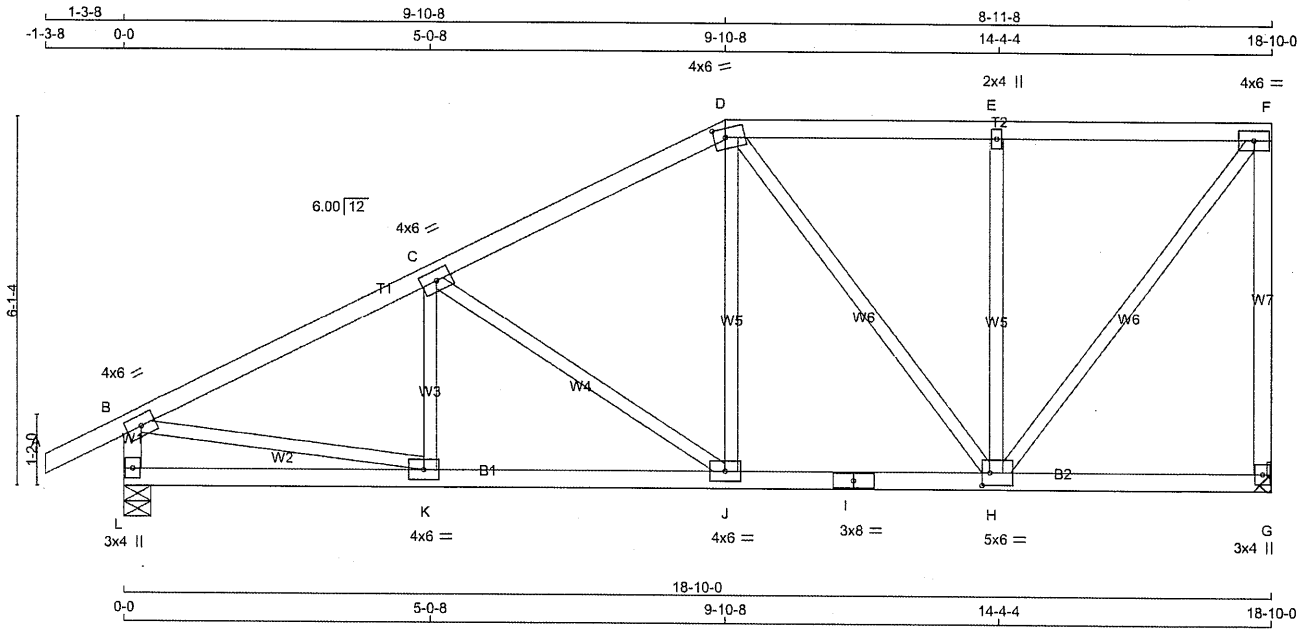

TOTAL WEIGHT = 84 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1038	0	1038	0	0	MECHANICAL	
L	1163	0	1163	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	734	482 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
L	820	551 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B- C	-1343 / 0	-91.8	-91.8	0.30 (1)	5.26	
C- D	-964 / 0	-91.8	-91.8	0.29 (1)	5.98	
D- E	-656 / 0	-91.8	-91.8	0.23 (1)	6.25	
E- F	-656 / 0	-91.8	-91.8	0.23 (1)	6.25	
G- F	-1003 / 0	0.0	0.0	0.72 (1)	7.81	
L- B	-1123 / 0	0.0	0.0	0.11 (1)	7.48	
L- K	0 / 0	-18.5	-18.5	0.11 (4)	10.00	
K- J	0 / 1222	-18.5	-18.5	0.24 (1)	10.00	
J- I	0 / 846	-18.5	-18.5	0.18 (1)	10.00	
I- H	0 / 846	-18.5	-18.5	0.18 (1)	10.00	
H- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-108 / 47	0.02 (1)
C- J	-463 / 0	0.29 (1)
J- D	0 / 346	0.08 (1)
D- H	-312 / 0	0.31 (1)
H- E	-507 / 0	0.30 (1)
H- F	0 / 1069	0.24 (1)
B- K	0 / 1240	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.72/1.00 (F-G:1) , BC=0.24/1.00 (J-K:1) , WB=0.31/1.00 (D-H:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080995

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T49	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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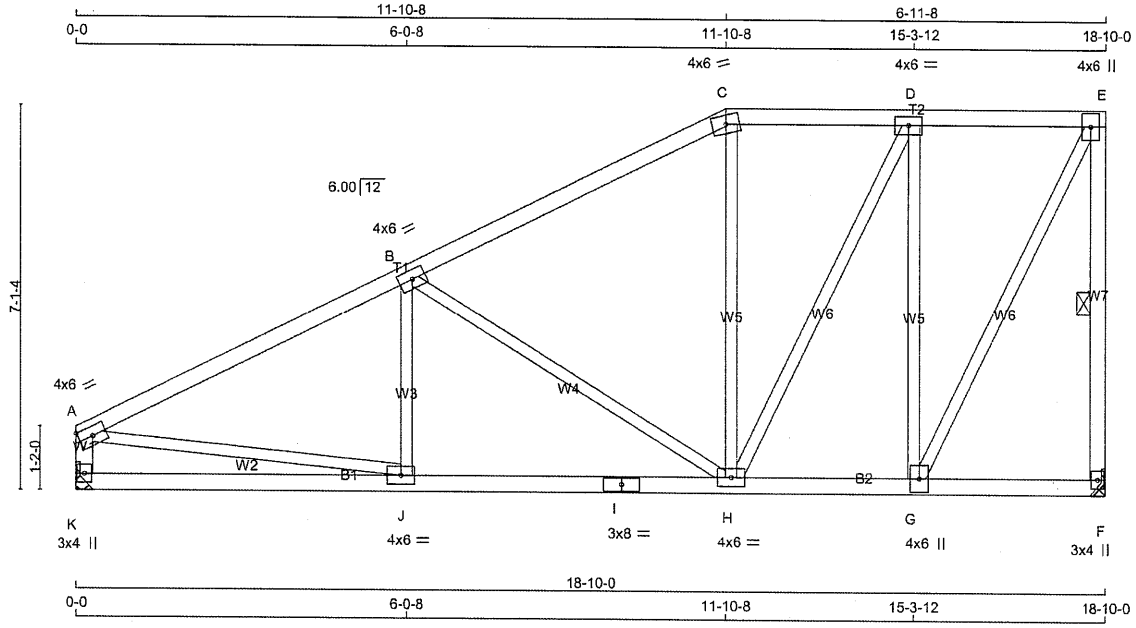

TOTAL WEIGHT = 88 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TMWW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW+t	MT20	4.0	6.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080996

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1038	0	1038	0	0	IN-SX	IN-SX
K	1038	0	1038	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, K. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	734	482 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	734	482 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1329 / 0	-91.8	-91.8	0.45 (1)	5.07
B- C	-777 / 0	-91.8	-91.8	0.42 (1)	6.24
C- D	-668 / 0	-91.8	-91.8	0.14 (1)	6.25
D- E	-457 / 0	-91.8	-91.8	0.13 (1)	6.25
F- E	-1010 / 0	0.0	0.0	0.23 (1)	6.23
K- A	-992 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00
J- I	0 / 1214	-18.5	-18.5	0.28 (1)	10.00
I- H	0 / 1214	-18.5	-18.5	0.28 (1)	10.00
H- G	0 / 457	-18.5	-18.5	0.12 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.05 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-47 / 85	0.03 (4)
B- H	-653 / 0	0.63 (1)
H- C	0 / 55	0.02 (4)
A- J	0 / 1226	0.28 (1)
H- D	0 / 481	0.11 (1)
G- D	-821 / 0	0.72 (1)
G- E	0 / 995	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.45/1.00 (A-B:1) , BC=0.28/1.00 (H-J:1) , WB=0.72/1.00 (D-G:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (J) (INPUT = 0.90)
JSI METAL= 0.43 (I) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T50	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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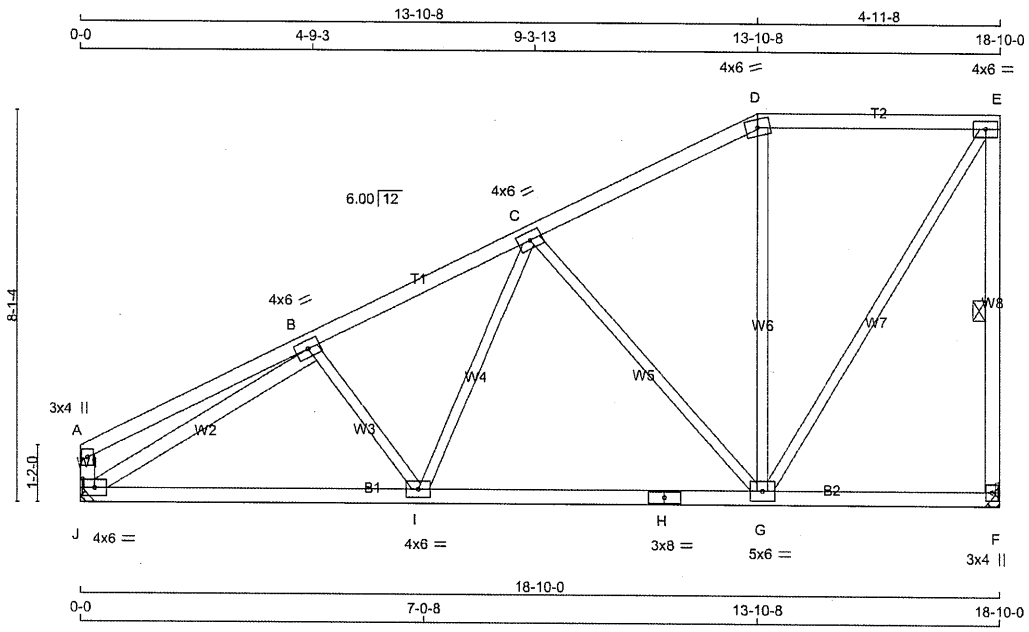

TOTAL WEIGHT = 87 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080997

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1038	0	1038	0	0	MECHANICAL	
J	1038	0	1038	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, J. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	734	482 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
J	734	482 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-91.8	-91.8	0.25 (1)	10.00
B- C	-1180 / 0	-91.8	-91.8	0.22 (1)	5.63
C- D	-562 / 0	-91.8	-91.8	0.22 (1)	6.25
D- E	-484 / 0	-91.8	-91.8	0.29 (1)	6.25
F- E	-1005 / 0	0.0	0.0	0.30 (1)	6.24
J- A	-175 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 1185	-18.5	-18.5	0.32 (4)	10.00
I- H	0 / 925	-18.5	-18.5	0.30 (4)	10.00
H- G	0 / 925	-18.5	-18.5	0.30 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- I	-217 / 0	0.06 (1)
I- C	0 / 347	0.08 (1)
C- G	-670 / 0	0.69 (1)
G- D	-156 / 12	0.20 (1)
G- E	0 / 916	0.21 (1)
J- B	-1436 / 0	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.30/1.00 (E-F:1) , BC=0.32/1.00 (I-J:4) , WB=0.69/1.00 (C-G:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T51	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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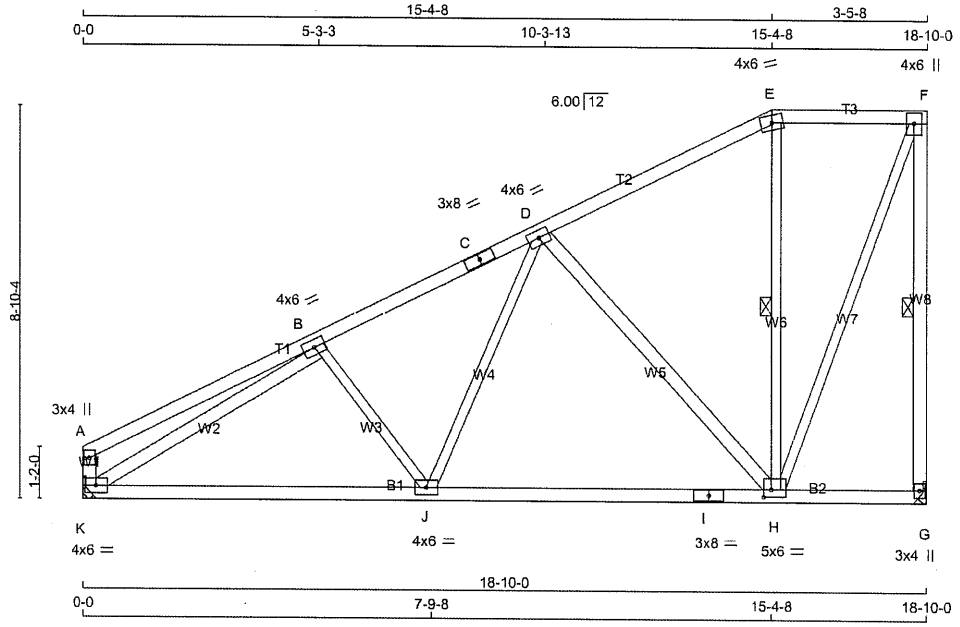

Scale = 1:49.7

TOTAL WEIGHT = 93 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	8.0		
D	TMWW-t	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.00	2.00
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1038	0	1038	0	0	IN-SX	IN-SX
K	1038	0	1038	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G, K. MINIMUM BEARING LENGTH AT JOINT G = 1-8, JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	734	482 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	734	482 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-91.8	-91.8	0.31 (1)	10.00
B- C	-1131 / 0	-91.8	-91.8	0.27 (1)	5.66
C- D	-1131 / 0	-91.8	-91.8	0.27 (1)	5.66
D- E	-403 / 0	-91.8	-91.8	0.27 (1)	6.25
E- F	-339 / 0	-91.8	-91.8	0.14 (1)	6.25
G- F	-1023 / 0	0.0	0.0	0.37 (1)	6.20
K- A	-193 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 1183	-18.5	-18.5	0.38 (4)	10.00
J- I	0 / 849	-18.5	-18.5	0.35 (4)	10.00
I- H	0 / 849	-18.5	-18.5	0.35 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- J	-284 / 0	0.09 (1)
J- D	0 / 425	0.10 (1)
D- H	-768 / 0	0.73 (1)
H- E	-178 / 0	0.09 (1)
H- F	0 / 929	0.21 (1)
K- B	-1422 / 0	0.67 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.37/1.00 (F-G:1) , BC=0.38/1.00 (J-K:4) , WB=0.73/1.00 (D-H:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080998

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T52	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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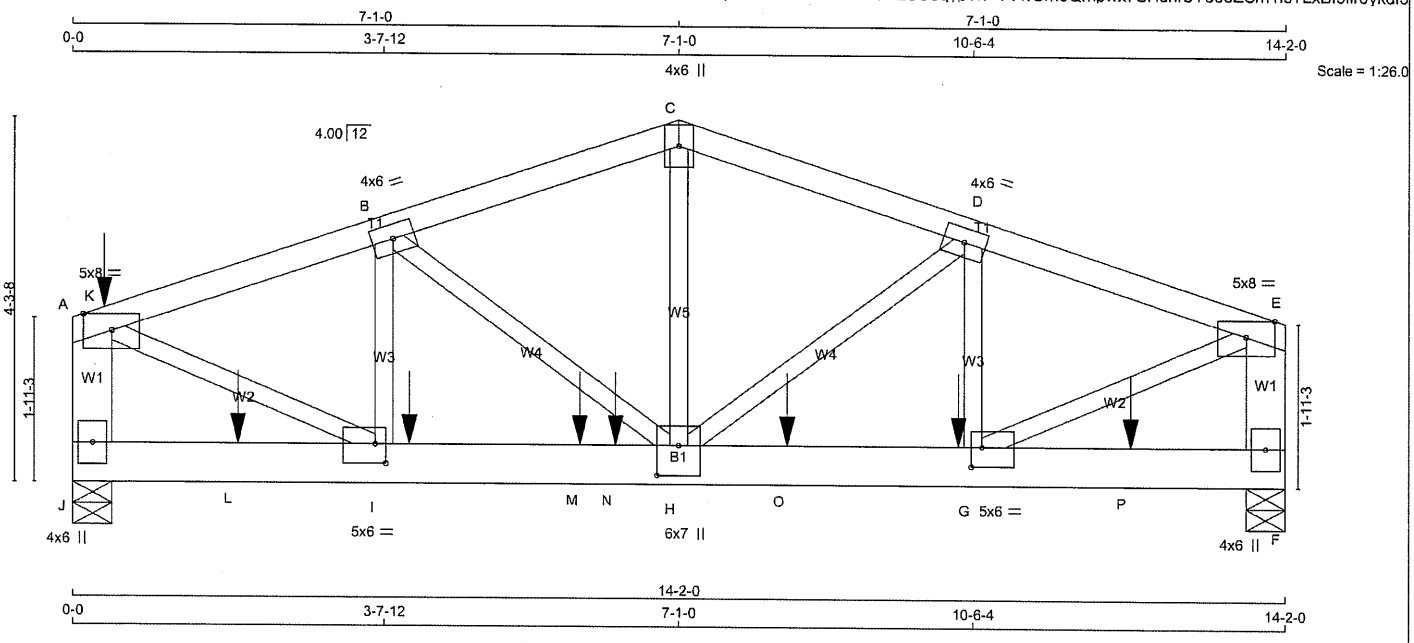

TOTAL WEIGHT = 2 X 66 = 132 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	TOP
J- A	2	12	SIDE(27.7)
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
D- G	1	6	SIDE(105.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080999

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	3721	0	3721	0	0	5-8	2-0
F	4536	0	4536	0	0	5-8	2-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	2617	1800 / 0	0 / 0	0 / 0	0 / 0	818 / 0	0 / 0
F	3200	2144 / 0	0 / 0	0 / 0	0 / 0	1056 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- K	-4564 / 0	-91.8	-91.8	0.23 (1)	4.29		H- C	0 / 3005	0.37 (1)
K- B	-4564 / 0	-91.8	-91.8	0.23 (1)	4.29		H- D	-229 / 0	0.04 (1)
B- C	-5180 / 0	-91.8	-91.8	0.18 (1)	4.12		G- D	-156 / 15	0.01 (1)
C- D	-5175 / 0	-91.8	-91.8	0.18 (1)	4.13		B- H	0 / 775	0.10 (1)
D- E	-5353 / 0	-91.8	-91.8	0.20 (1)	4.05		I- B	-1115 / 0	0.11 (1)
J- A	-3696 / 0	0.0	0.0	0.14 (1)	7.32		A- I	0 / 4672	0.58 (1)
F- E	-4003 / 0	0.0	0.0	0.15 (1)	7.10		G- E	0 / 5522	0.68 (1)
J- L	0 / 0	-18.5	-18.5	0.05 (1)	10.00				
L- I	0 / 0	-18.5	-18.5	0.05 (1)	10.00				
I- M	0 / 4300	-18.5	-18.5	0.61 (1)	10.00				
M- N	0 / 4300	-18.5	-18.5	0.61 (1)	10.00				
N- H	0 / 4300	-18.5	-18.5	0.61 (1)	10.00				
H- O	0 / 5083	-18.5	-18.5	0.64 (1)	10.00				
O- G	0 / 5083	-18.5	-18.5	0.64 (1)	10.00				
G- P	0 / 0	-18.5	-18.5	0.34 (1)	10.00				
P- F	0 / 0	-18.5	-18.5	0.34 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	10-4-4	-781	-781	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-106	-106	---	BACK	VERT	TOTAL	---	C1
K	5-0	-234	-234	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-43	-43	---	BACK	VERT	TOTAL	---	C1
M	5-11-4	-170	-170	---	BACK	VERT	TOTAL	---	C1
N	6-4-4	-1816	-1816	---	BACK	VERT	TOTAL	---	C1
O	8-4-4	-781	-781	---	BACK	VERT	TOTAL	---	C1
P	12-4-4	-781	-781	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.64/1.00 (G-H:1) , WB=0.68/1.00 (E-G:1) , SSI=0.81/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.58 (I) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434613	T52	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:LpDGmu0HxWz8EPf6?EUU9qyp?I7-VYwUm6Qmpwx7SHdhrbYe63ZSm1fsTLxBI9ilr0ykdf3

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1+p	MT20	4.0	6.0		
G	BMWW-t	MT20	5.0	6.0	2.75	1.50
H	BMWWW+t	MT20	6.0	7.0	4.25	3.00
I	BMWW-t	MT20	5.0	6.0	2.75	1.50
J	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

STRUCTURAL COMPONENT ONLY
DWG # TR23080999

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T53S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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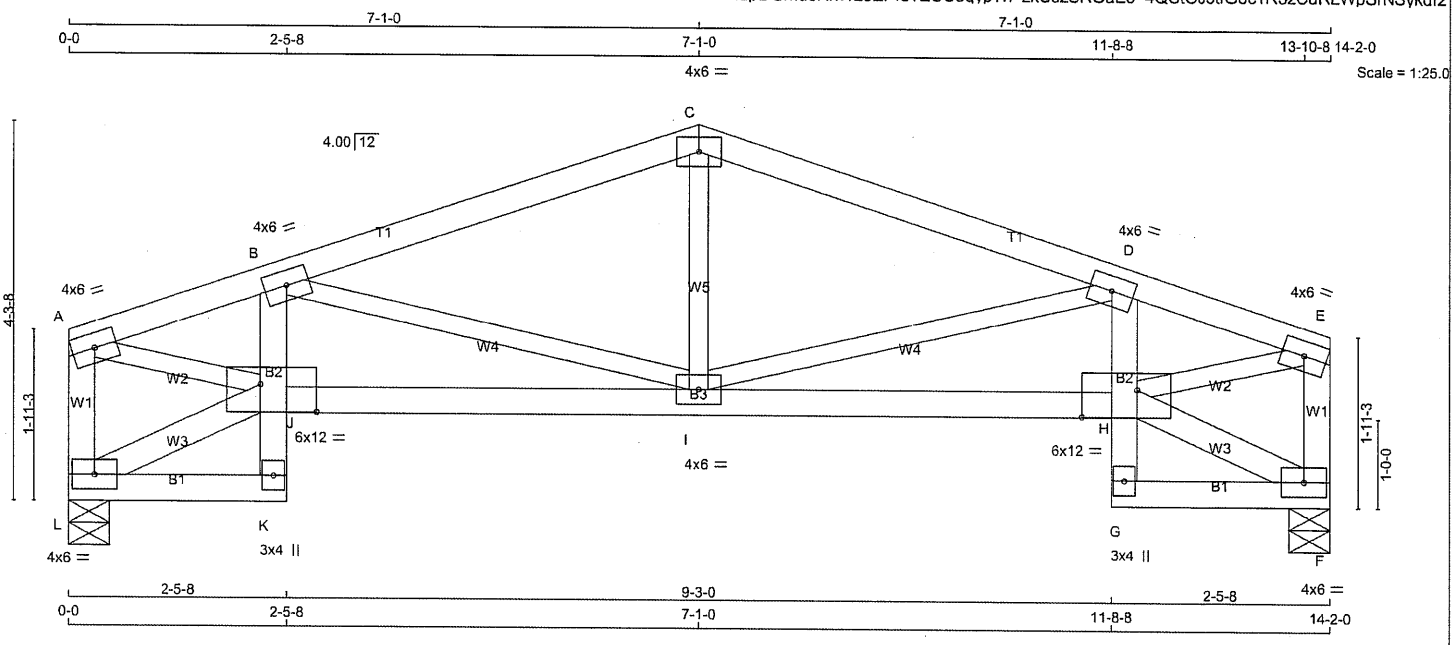

TOTAL WEIGHT = 59 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, B, D, E						
A	TMVW-t	MT20	4.0	6.0		
C	TTW-p	MT20	4.0	6.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWW-l	MT20	6.0	12.0	3.75	7.50
I	BMWWW-t	MT20	4.0	6.0		
J	BVMWW-l	MT20	6.0	12.0	3.75	7.50
K	BMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	781	0	781	0	0	5-8	1-8
F	781	0	781	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	552	362 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0
F	552	362 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1272 / 0	-91.8	-91.8	0.18 (1)		5.52
B- C	-1002 / 0	-91.8	-91.8	0.26 (1)		5.93
C- D	-1002 / 0	-91.8	-91.8	0.26 (1)		5.93
D- E	-1272 / 0	-91.8	-91.8	0.18 (1)		5.52
L- A	-737 / 0	0.0	0.0	0.08 (1)		7.81
F- E	-737 / 0	0.0	0.0	0.08 (1)		7.81
L- K	0 / 49	-18.5	-18.5	0.03 (4)		10.00
K- J	0 / 24	0.0	0.0	0.08 (1)		10.00
J- B	-236 / 0	0.0	0.0	0.06 (1)		7.81
J- I	0 / 1267	-18.5	-18.5	0.27 (1)		10.00
I- H	0 / 1267	-18.5	-18.5	0.27 (1)		10.00
G- H	0 / 24	0.0	0.0	0.08 (1)		10.00
H- D	-236 / 0	0.0	0.0	0.06 (1)		7.81
G- F	0 / 49	-18.5	-18.5	0.03 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 256	0.06 (1)
I- D	-341 / 0	0.14 (1)
B- I	-341 / 0	0.14 (1)
L- J	-54 / 0	0.01 (1)
A- J	0 / 1261	0.28 (1)
H- F	-54 / 0	0.01 (1)
H- E	0 / 1261	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.26/1.00 (B-C:1) , BC=0.27/1.00 (I-J:1) , WB=0.28/1.00 (A-J:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (A) (INPUT = 0.90)
JSI METAL= 0.31 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23081000

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	T54	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 14:00:28 2023 Page 1
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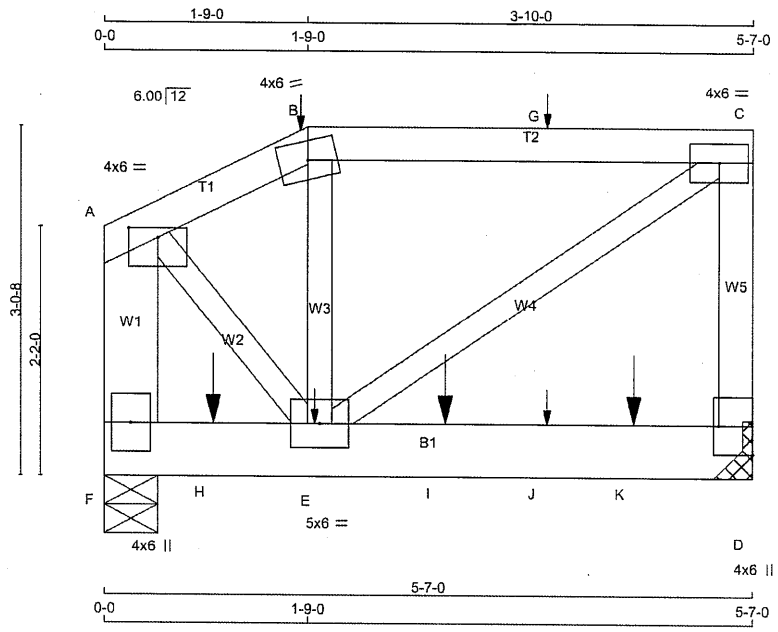


TOTAL WEIGHT = 2 X 29 = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- C	1	12	SIDE(61.0)
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

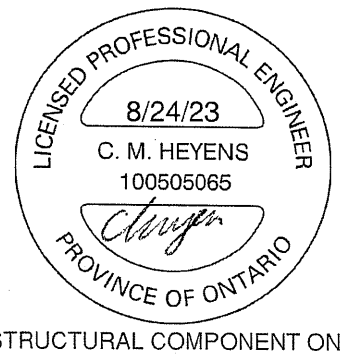
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00



STRUCTURAL COMPONENT ONLY
DWG # TR23081001

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1849	0	1849	0	0	IN-SX	IN-SX
F	1826	0	1826	0	0	MECHANICAL 5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1305	871 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
F	1289	860 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1245 / 0	-91.8	-91.8	0.03 (1)	6.25	E- B	0 / 303	0.04 (1)
B- G	-1123 / 0	-91.8	-91.8	0.13 (1)	6.25	E- C	0 / 1381	0.17 (1)
G- C	-1123 / 0	-91.8	-91.8	0.13 (1)	6.25	A- E	0 / 1538	0.19 (1)
D- C	-976 / 0	0.0	0.0	0.08 (1)	7.81			
F- A	-1705 / 0	0.0	0.0	0.07 (1)	7.81			
F- H	0 / 0	-18.5	-18.5	0.27 (1)	10.00			
H- E	0 / 0	-18.5	-18.5	0.27 (1)	10.00			
E- I	0 / 0	-18.5	-18.5	0.32 (1)	10.00			
I- J	0 / 0	-18.5	-18.5	0.32 (1)	10.00			
J- K	0 / 0	-18.5	-18.5	0.32 (1)	10.00			
K- D	0 / 0	-18.5	-18.5	0.32 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	1-9-0	1	1	78	BACK	VERT	TOTAL	---	C1
E	1-9-12	1	1	---	BACK	VERT	TOTAL	---	C1
G	3-9-12	1	1	78	BACK	VERT	TOTAL	---	C1
H	11-4	-720	-720	---	FRONT	VERT	TOTAL	---	C1
I	2-11-4	-719	-719	---	FRONT	VERT	TOTAL	---	C1
J	3-9-12	1	1	---	BACK	VERT	TOTAL	---	C1
K	4-6-12	-719	-719	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.32/1.00 (D-E:1) , WB=0.19/1.00 (A-E:1) , SSI=0.41/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (E) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
434613	T54	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TTW-m	MT20	4.0	6.0		
C	TMVW-t	MT20	4.0	6.0		
D	BMV1+p	MT20	4.0	6.0	3.00	Edge
E	BMWWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23081001

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	J1	9	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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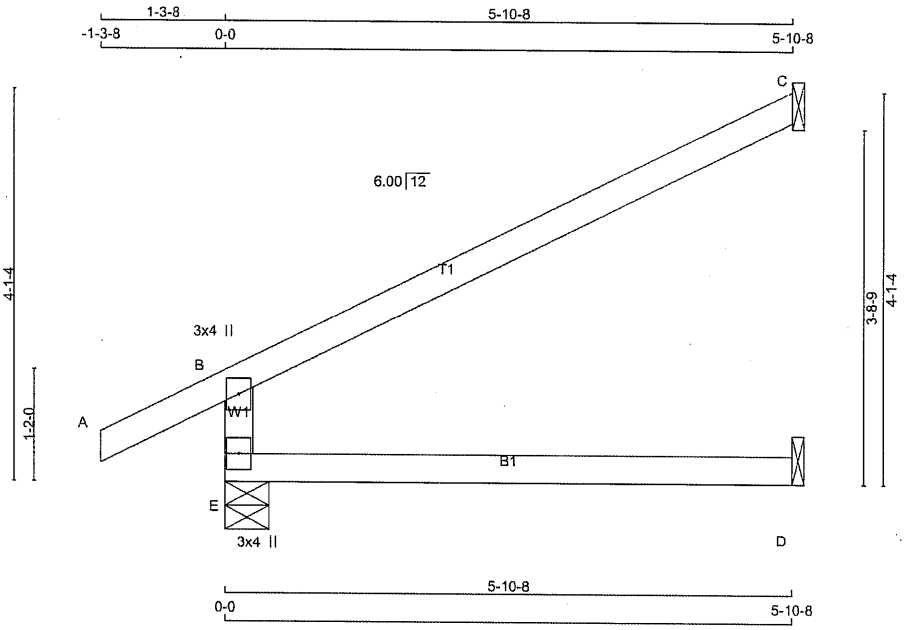

TOTAL WEIGHT = 9 X 17 = 151 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	525	0	525	0	0	5-8	1-8
C	202	0	202	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-461 / 0	0.0	0.0	0.13 (4)		7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)		10.00			
B- C	-30 / 0	-91.8	-91.8	0.54 (1)		6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080922

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	J2	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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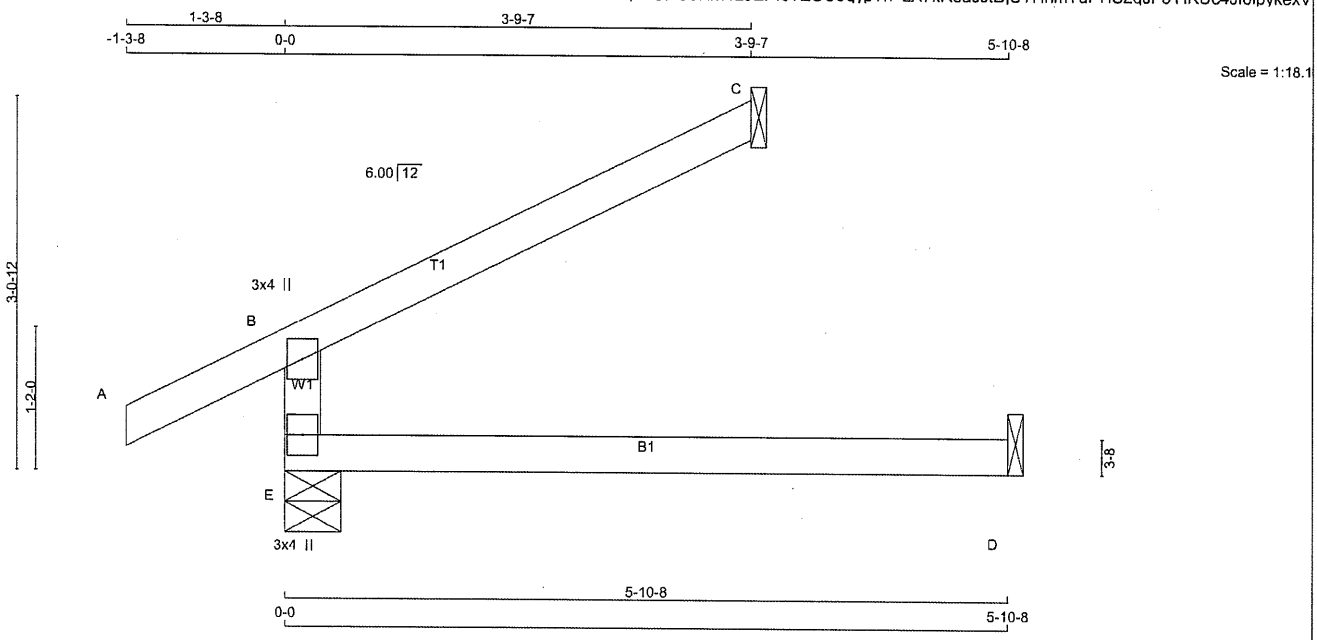

TOTAL WEIGHT = 3 X 14 = 42 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	405	0	405	0	0	5-8	1-8
C	130	0	130	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	286	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-342 / 0	0.0	0.0	0.13 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-19 / 0	-91.8	-91.8	0.22 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080923

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	J3	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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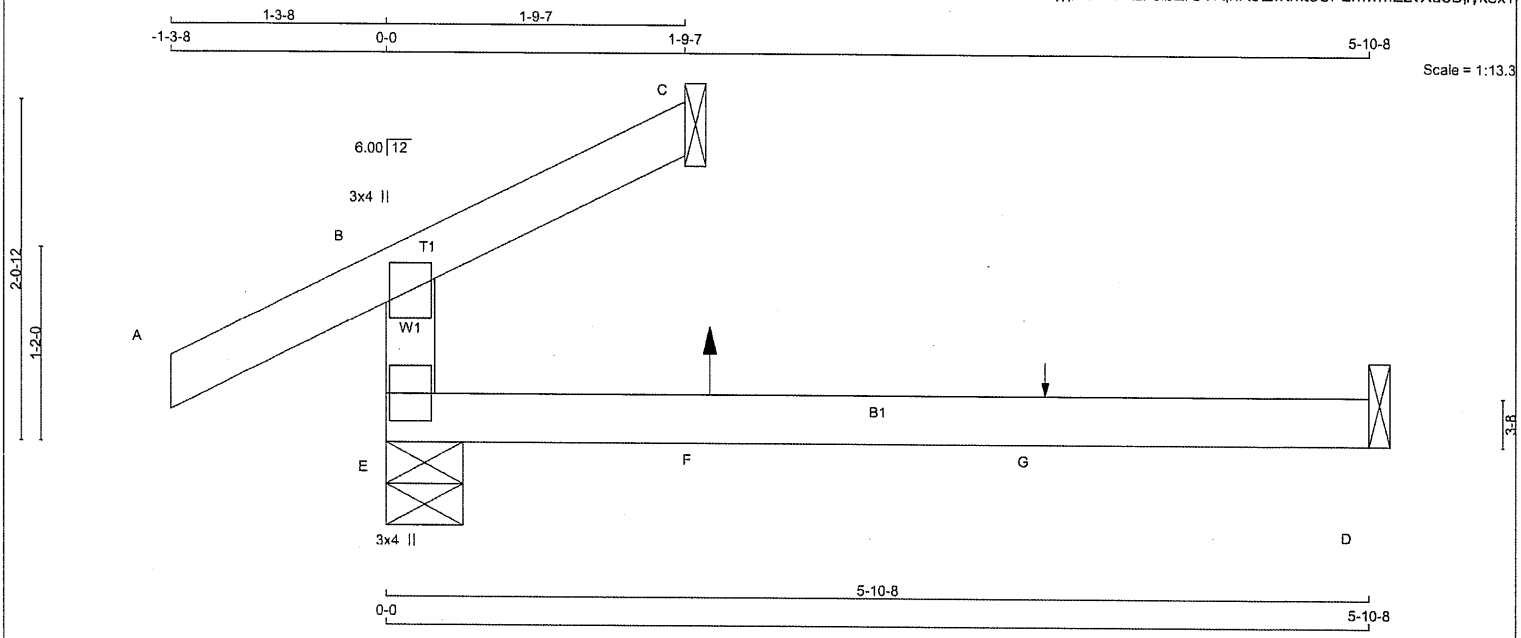

TOTAL WEIGHT = 3 X 12 = 35 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	284	0	284	0	0	5-8	1-8
C	63	0	63	0	0	1-8	1-8
D	44	0	52	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	137 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	46	21 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-227 / 0	0.0	0.0	0.11 (4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-9 / 9	-91.8	-91.8	0.08 (4)	10.00			
E- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
F- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
G- D	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	FRONT	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080924

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	J4	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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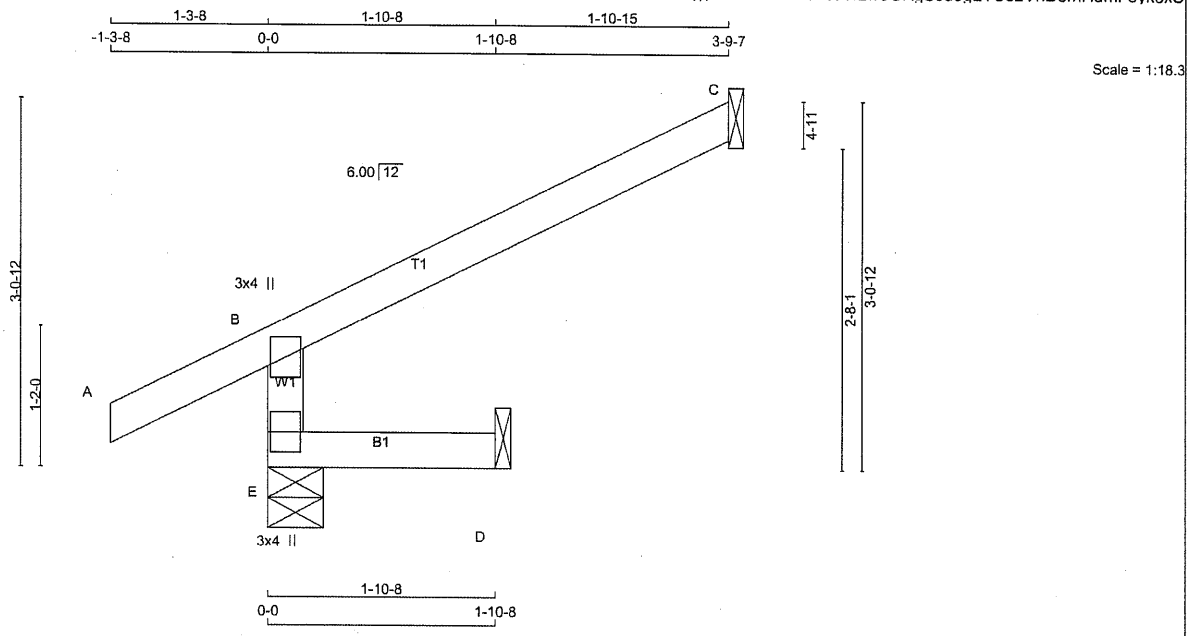

TOTAL WEIGHT = 3 X 10 = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	361	0	361	0	0	5-8	1-8
C	130	0	130	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-342 / 0	0.0	0.0	0.01	(4)	7.81			
A- B	0 / 28	-91.8	-91.8	0.13	(5)	10.00			
B- C	-19 / 0	-91.8	-91.8	0.22	(1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02	(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080925

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	J5	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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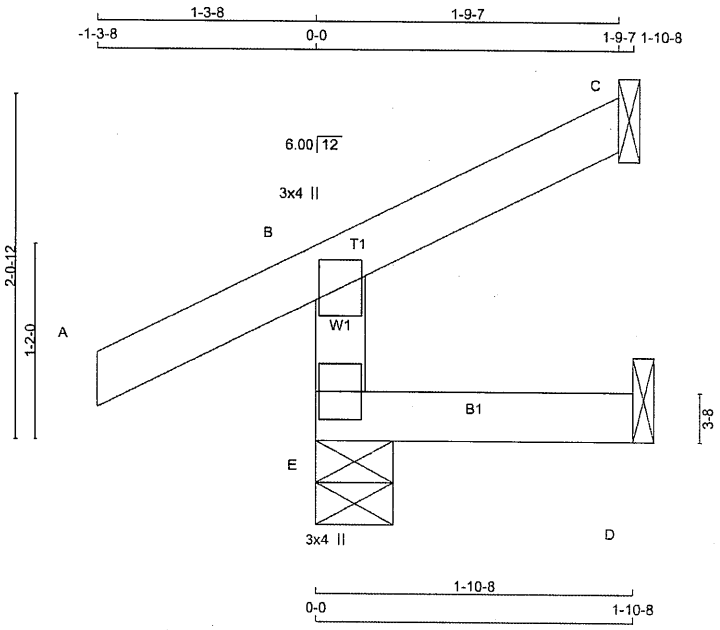

TOTAL WEIGHT = 3 X 7 = 21 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	271	0	271	0	0	5-8	1-8
C	45	0	45	0	-23	1-8	1-8
D	8	0	17	0	-2	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-244 / 0	0.0	0.0	0.04 (5)	7.81			
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-17 / 0	-91.8	-91.8	0.09 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080926

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	J6W	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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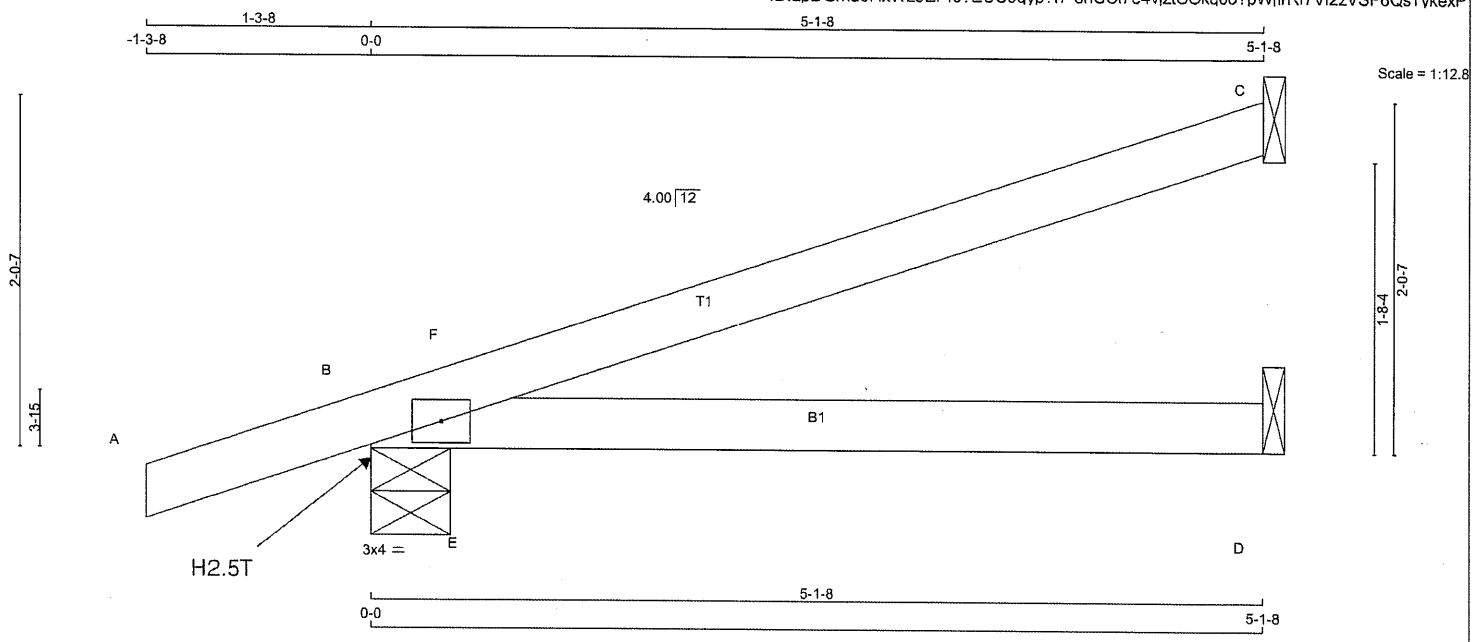

TOTAL WEIGHT = 3 X 14 = 41 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	205	0	205	0	-134	1-8	1-8
B	405	0	405	124	-241	5-8	1-8
D	78	0	78	0	-95	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 241 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 124 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	142	110 / 0	0 / 0	0 / 0	0 / -116	32 / 0	0 / 0
B	284	200 / 0	0 / 0	0 / 0	0 / -227	85 / 0	0 / 0
D	58	21 / 0	0 / 0	0 / 0	0 / -92	37 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 18	-91.8	-91.8	0.11 (1)	10.00	E- F	-272 / 98	0.00 (1)
B- F	-86 / 9	-91.8	-91.8	0.06 (12)	6.25			
F- C	-50 / 2	-91.8	-91.8	0.31 (1)	6.25			
B- E	0 / 0	-18.5	-18.5	0.22 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.22 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 987 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 728 (0.08")

CSI: TC=0.31/1.00 (C-F:1) , BC=0.22/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.21/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080927

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434362	J7W	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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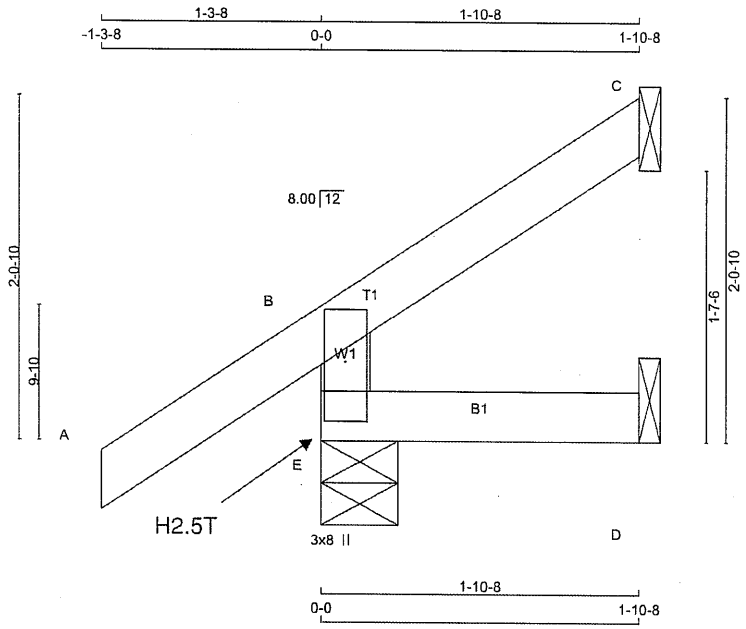

TOTAL WEIGHT = 3 X 7 = 21 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
E						
E	TMBMV1+p	MT20	3.0	8.0	4.25	1.50

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
END VERTICALS ARE NOT EXPOSED TO WIND.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	275	0	275	118	-99	5-8	1-8
C	51	0	62	0	-64	1-8	1-8
D	6	0	16	0	-40	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 118 LBS FACTORED HORIZONTAL REACTION AT JOINT E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	192	144 / 0	0 / 0	0 / 0	0 / -101	48 / 0	0 / 0
C	36	28 / -15	0 / 0	0 / 0	26 / -51	8 / 0	0 / 0
D	6	0 / -10	0 / 0	0 / 0	0 / -36	11 / 0	0 / 0
HORIZONTAL REACTIONS							
E	---	0 / 0	0 / 0	0 / 0	84 / -37	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (13)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-258 / 77	0.0	0.0	0.06 (7)	7.81			
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	-41 / 3	-91.8	-91.8	0.09 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.05 (13)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.05/1.00 (D-E:13) , WB=0.00/1.00 (n/a:0) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080928

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J11	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 13:59:57 2023 Page 1
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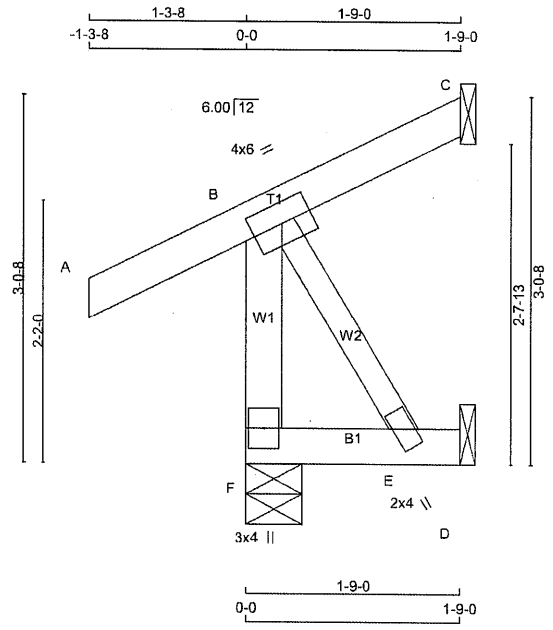

Scale = 1:18.2

TOTAL WEIGHT = 2 X 10 = 19 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	269	0	269	0	0	5-8	1-8
C	32	0	32	0	-39	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	187	141 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	22	18 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-253 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-22 / 0	-91.8	-91.8	0.11 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080975

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	J20	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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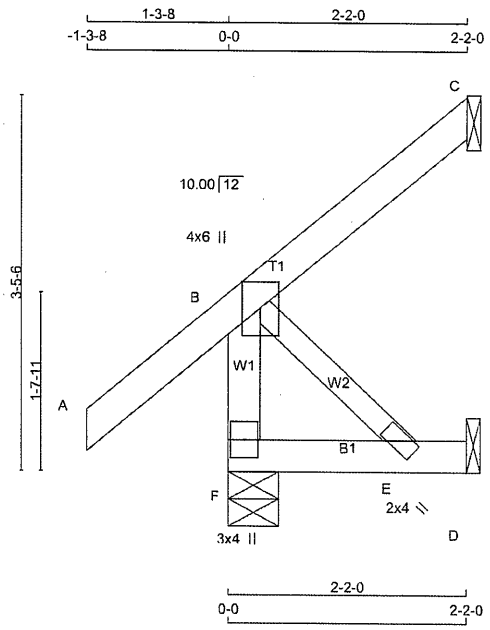

TOTAL WEIGHT = 2 X 11 = 22 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	247	0	247	0	0	5-8	1-8
C	99	0	99	0	0	1-8	1-8
D	20	0	22	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	172	126 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	68	55 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	16	0 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-226 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 41	-91.8	-91.8	0.13 (5)	10.00
B- C	0 / 0	-91.8	-91.8	0.07 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.03 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080951

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	J21	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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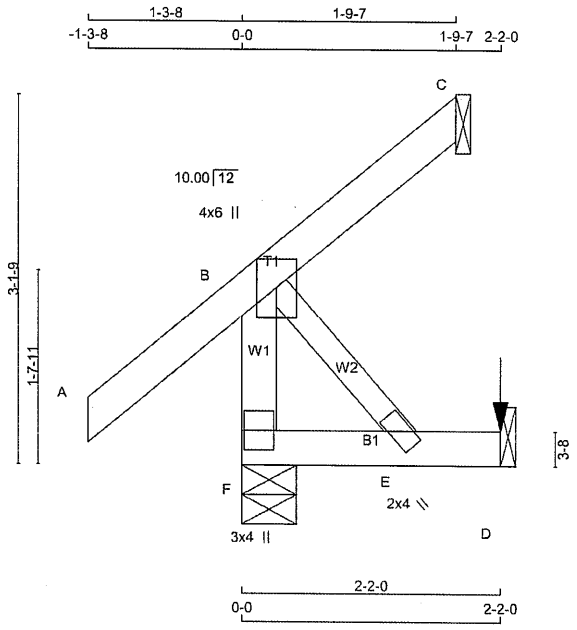

TOTAL WEIGHT = 10 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	278	0	278	0	0	5-8	1-8
C	33	0	33	0	-40	1-8	1-8
D	26	0	29	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	194	144 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
C	23	18 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO					
F- B	-258 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 41	-91.8	-91.8	0.13 (5)	10.00			
B- C	-32 / 0	-91.8	-91.8	0.13 (5)	6.25			
F- E	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	2-2-0	-5	-5	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080952

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	J22	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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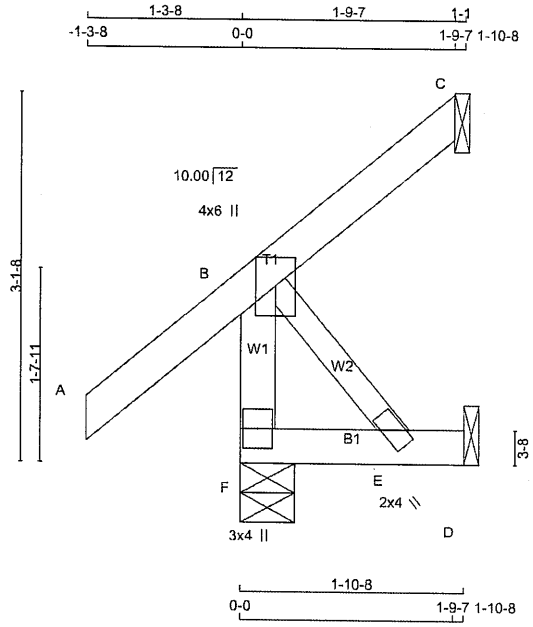

TOTAL WEIGHT = 4 X 10 = 39 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	276	0	276	0	0	5-8	1-8
C	33	0	33	0	-40	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	192	144 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	23	18 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-258 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 41	-91.8	-91.8	0.13 (1)	10.00			
B- C	-32 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

STRUCTURAL COMPONENT ONLY
DWG # TR23080953

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	J23	15	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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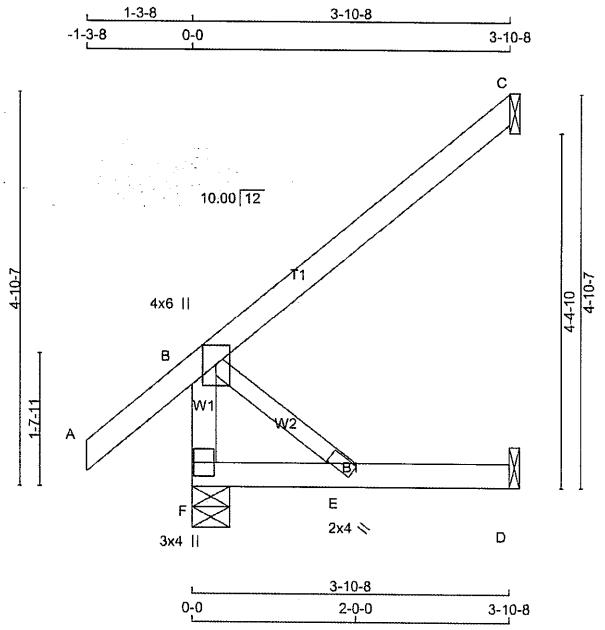

TOTAL WEIGHT = 15 X 15 = 232 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	341	0	341	0	0	5-8	1-8
C	178	0	178	0	0	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	239	170 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
C	122	99 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-305 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 41	-91.8	-91.8	0.14 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.23 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080954

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	J24	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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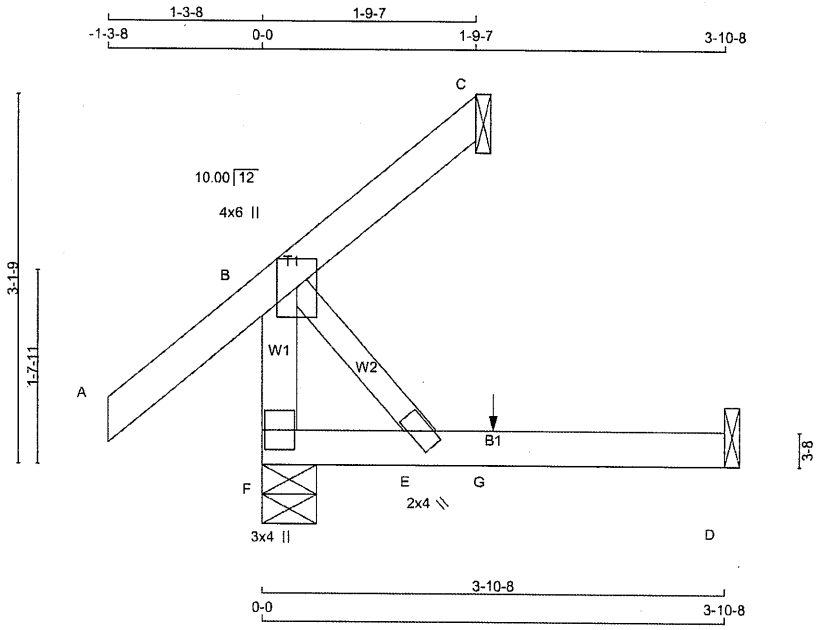

TOTAL WEIGHT = 3 X 12 = 36 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	294	0	294	0	0	5-8	1-8
C	33	0	33	0	-40	1-8	1-8
D	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	207	144 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	23	18 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-258 / 0	0.0	0.0	0.03 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 41	-91.8	-91.8	0.14 (5)		10.00			
B- C	-32 / 0	-91.8	-91.8	0.14 (5)		6.25			
F- E	0 / 0	-18.5	-18.5	0.07 (4)		10.00			
E- G	0 / 0	-18.5	-18.5	0.08 (4)		10.00			
G- D	0 / 0	-18.5	-18.5	0.08 (4)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (A-B:5) , BC=0.08/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080955

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434363	J25W	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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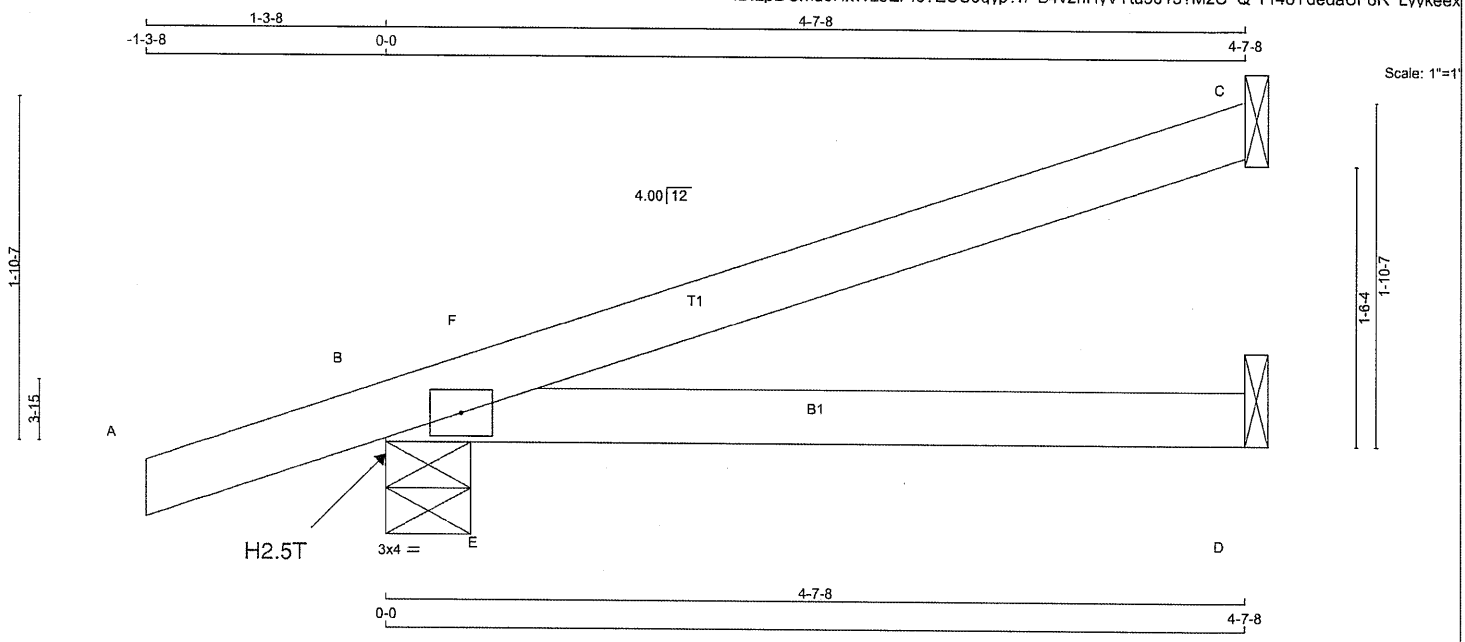

TOTAL WEIGHT = 4 X 12 = 50 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	184	0	184	0	-121	1-8	1-8
B	378	0	378	114	-222	5-8	1-8
D	71	0	71	0	-86	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 222 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 114 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	127	99 / 0	0 / 0	0 / 0	0 / -104	29 / 0	0 / 0
B	265	187 / 0	0 / 0	0 / 0	0 / -209	78 / 0	0 / 0
D	53	19 / 0	0 / 0	0 / 0	0 / -83	33 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 18	-91.8	-91.8	0.11 (1)	10.00	E- F	-227 / 82	0.00 (1)
B- F	-75 / 2	-91.8	-91.8	0.05 (12)	6.25			
F- C	-45 / 2	-91.8	-91.8	0.25 (1)	6.25			
B- E	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.19 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 979 (0.06")

CSI: TC=0.25/1.00 (C-F:1) , BC=0.19/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080956

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J40	5	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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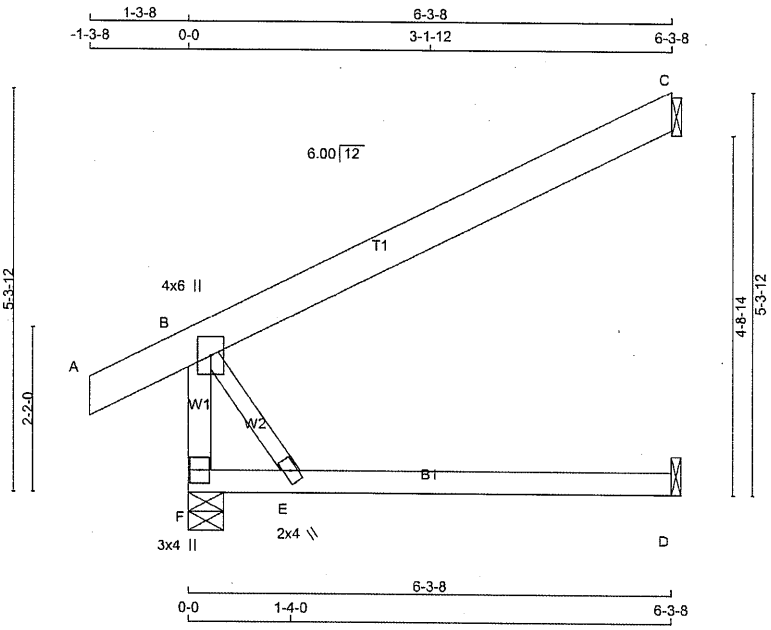

TOTAL WEIGHT = 5 X 26 = 130 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW+p	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	475	0	475	0	0	5-8	1-8
C	289	0	289	0	0	1-8	1-8
D	58	0	65	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	333	232 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
C	199	161 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
D	47	0 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-417 / 0	0.0	0.0	0.05 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 29	-91.8	-91.8	0.06 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.30 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.30/1.00 (B-C:1) , BC=0.21/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080976

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J41	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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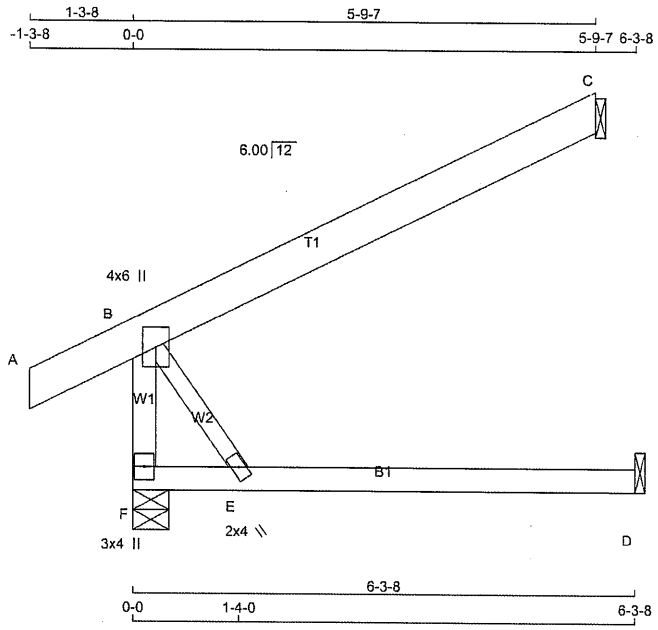

TOTAL WEIGHT = 2 X 25 = 50 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW+p	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	452	0	452	0	0	5-8	1-8
C	265	0	265	0	0	1-8	1-8
D	58	0	65	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	317	219 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
C	183	148 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	47	0 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-393 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 29	-91.8	-91.8	0.06 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.26 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.26/1.00 (B-C:1) , BC=0.21/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080977

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J42	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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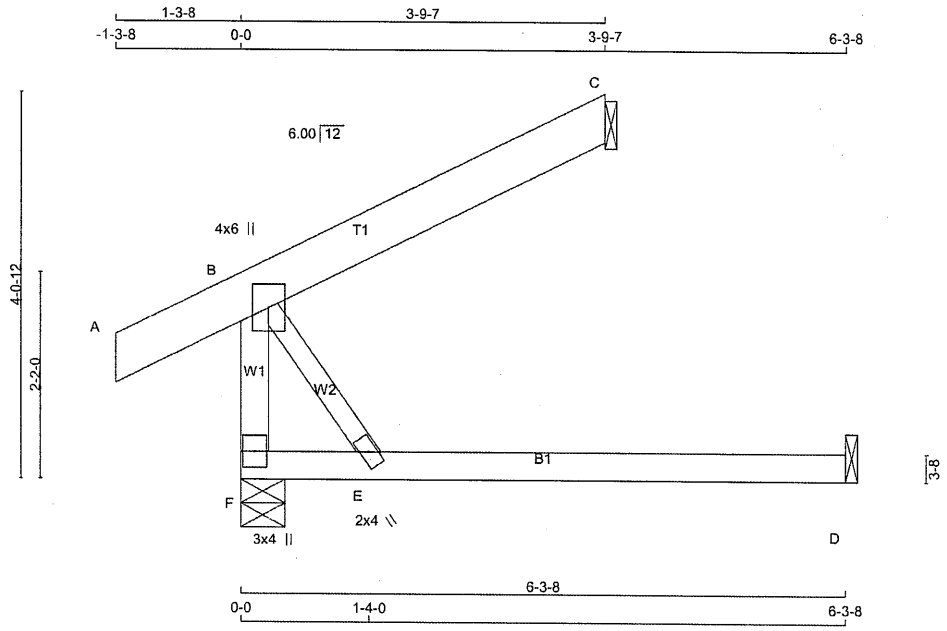

TOTAL WEIGHT = 2 X 21 = 42 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW+p	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	360	0	360	0	0	5-8	1-8
C	174	0	174	0	0	1-8	1-8
D	58	0	65	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	254	168 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	47	0 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-302 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 29	-91.8	-91.8	0.06 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.11 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.11/1.00 (B-C:1) , BC=0.21/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080978

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J43	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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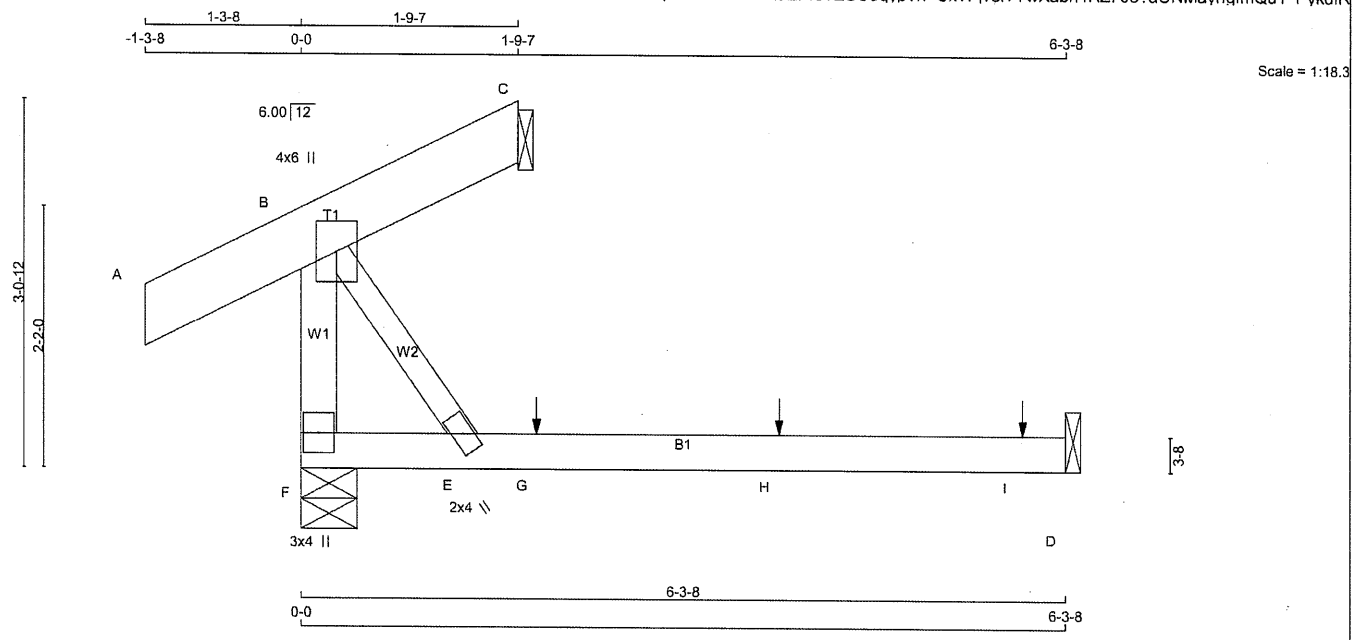

TOTAL WEIGHT = 2 X 17 = 34 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW+p	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	318	0	318	0	0	5-8	1-8
C	32	0	32	0	0	1-8	1-8
D	58	0	65	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	225	145 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	22	18 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	47	0 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-260 / 0	0.0	0.0	0.03 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 29	-91.8	-91.8	0.06 (1)		10.00			
B- C	-22 / 0	-91.8	-91.8	0.05 (1)		6.25			
F- E	0 / 0	-18.5	-18.5	0.14 (4)		10.00			
E- G	0 / 0	-18.5	-18.5	0.21 (4)		10.00			
G- H	0 / 0	-18.5	-18.5	0.21 (4)		10.00			
H- I	0 / 0	-18.5	-18.5	0.21 (4)		10.00			
I- D	0 / 0	-18.5	-18.5	0.21 (4)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
I	5-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.21/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (D-E:4)

DOL LUMBER=0.97 NAIL=0.97 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080979

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J44	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 14:00:03 2023 Page 1
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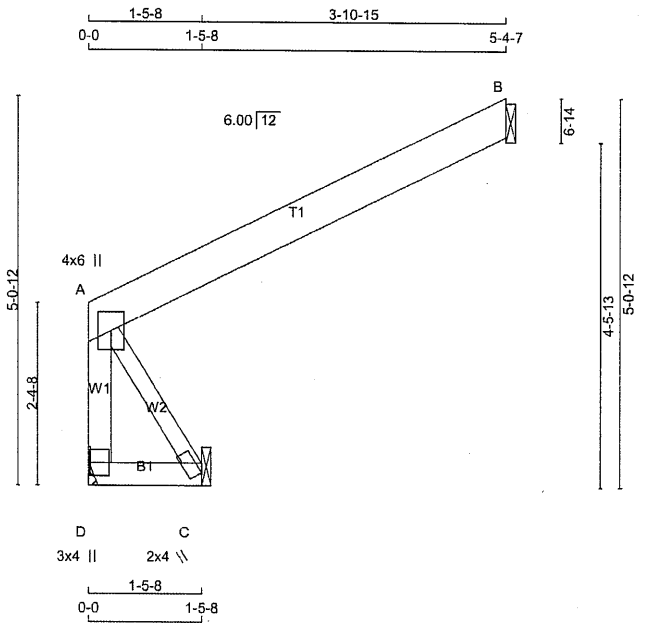

Scale = 1:28.7

TOTAL WEIGHT = 16 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
C	BMW1+w	MT20	2.0	4.0		Edge
D	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	259	0	259	0	0	MECHANICAL	
B	246	0	246	0	0	1-8	1-8
C	12	0	14	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	179	137 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0
B	170	137 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
C	10	0 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-246 / 0	0.0	0.0	0.03 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
D- C	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (A-B:1) , BC=0.01/1.00 (C-D:4) , WB=0.00/1.00 (A-C:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080980

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J45	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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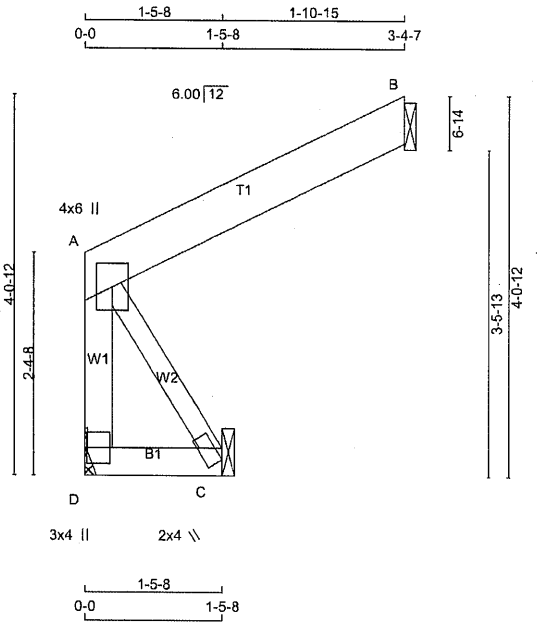

TOTAL WEIGHT = 12 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
C	BMW1+w	MT20	2.0	4.0		Edge
D	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	167	0	167	0	0	MECHANICAL	
B	155	0	155	0	0	1-8	1-8
C	12	0	14	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	116	86 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
B	106	86 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
C	10	0 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-155 / 0	0.0	0.0	0.02 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.09 (1)	10.00			
D- C	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.09/1.00 (A-B:1) , BC=0.01/1.00 (C-D:4) , WB=0.00/1.00 (A-C:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.03 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080981

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J46	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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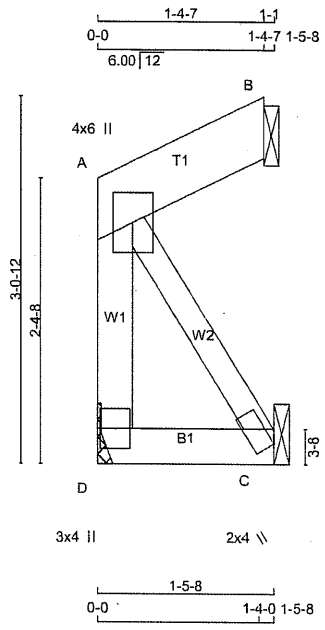

Scale = 1:18.4

TOTAL WEIGHT = 8 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
C	BMW1+w	MT20	2.0	4.0		Edge
D	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	75	0	75	0	0	MECHANICAL	
B	63	0	63	0	0	1-8	1-8
C	12	0	14	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	53	35 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
B	43	35 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
C	10	0 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-63 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.01 (1)	10.00			
D- C	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.01/1.00 (A-B:1) , BC=0.01/1.00 (C-D:4) , WB=0.00/1.00 (A-C:1) , SSI=0.03/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.03 (A) (INPUT = 0.90)
JSI METAL= 0.01 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080982

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J47	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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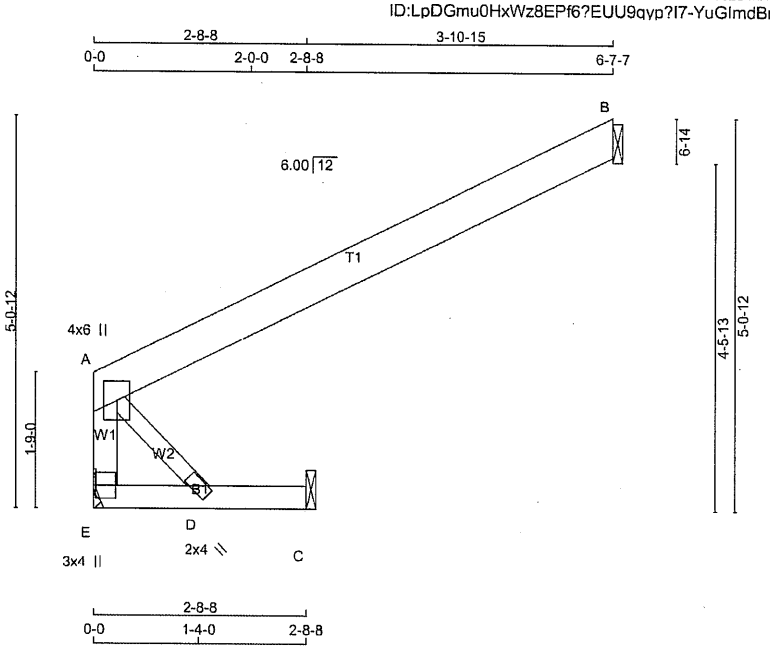

TOTAL WEIGHT = 19 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x6	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	329	0	329	0	0	MECHANICAL	
B	304	0	304	0	0	1-8	1-8
C	25	0	28	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	229	169 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
B	209	169 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0
C	20	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-304 / 0	0.0	0.0	0.03 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.34 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.34/1.00 (A-B:1) , BC=0.04/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (A) (INPUT = 0.90)
JSI METAL= 0.06 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080983

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J48	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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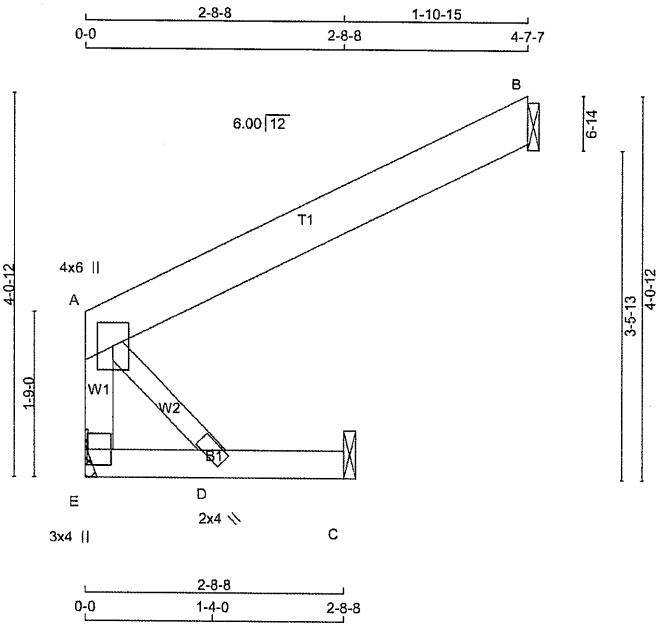

TOTAL WEIGHT = 15 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x6	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW+p	MT20	4.0	6.0	
D	BMW+w	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	237	0	237	0	0	MECHANICAL	
B	212	0	212	0	0	1-8	1-8
C	25	0	28	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	166	118 / 0	0 / 0	0 / 0	0 / 0	48 / 0	0 / 0
B	146	118 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
C	20	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-212 / 0	0.0	0.0	0.02 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.16 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.04/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080984

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
434613	J49	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.630 S Jul 5 2023 MiTek Industries, Inc. Thu Aug 24 14:00:09 2023 Page 1
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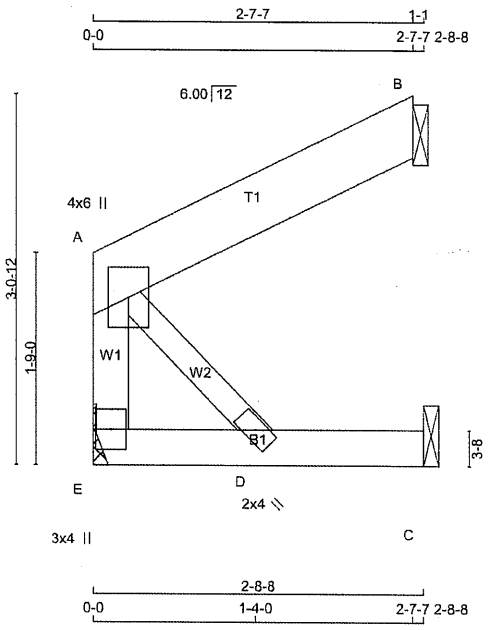


Scale = 1:18.2

TOTAL WEIGHT = 11 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x6	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW+p	MT20	4.0	6.0	
D	BMW+w	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	145	0	145	0	0	MECHANICAL	
B	120	0	120	0	0	1-8	1-8
C	25	0	28	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	103	67 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0
B	83	67 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
C	20	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-120 / 0	0.0	0.0	0.01 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.05 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.05/1.00 (A-B:1) , BC=0.04/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (A) (INPUT = 0.90)
JSI METAL= 0.03 (A) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
8/24/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23080985

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

EWP DESIGN INC.

(905) 832-2250 FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

February 1, 2019

MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	NAIL DIA.	NAIL LENGTH	LATERAL RESISTANCE (lbs)		WITHDRAWAL RESISTANCE (lbs)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

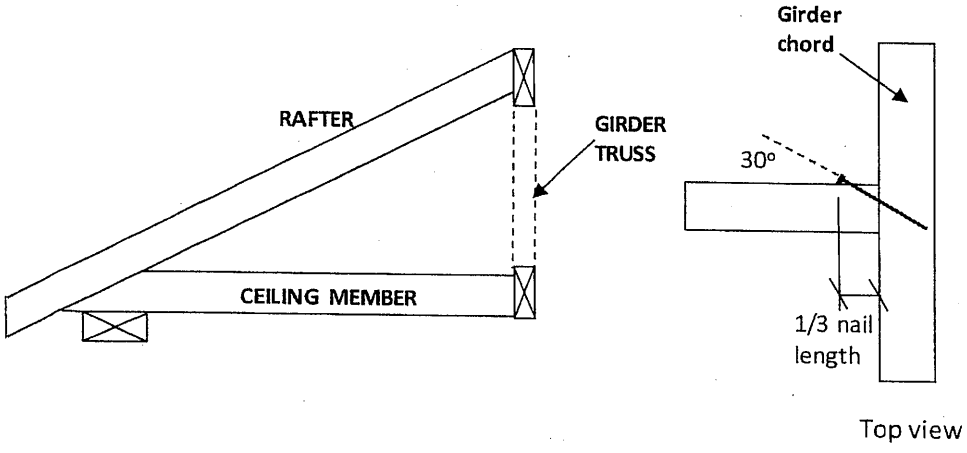


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

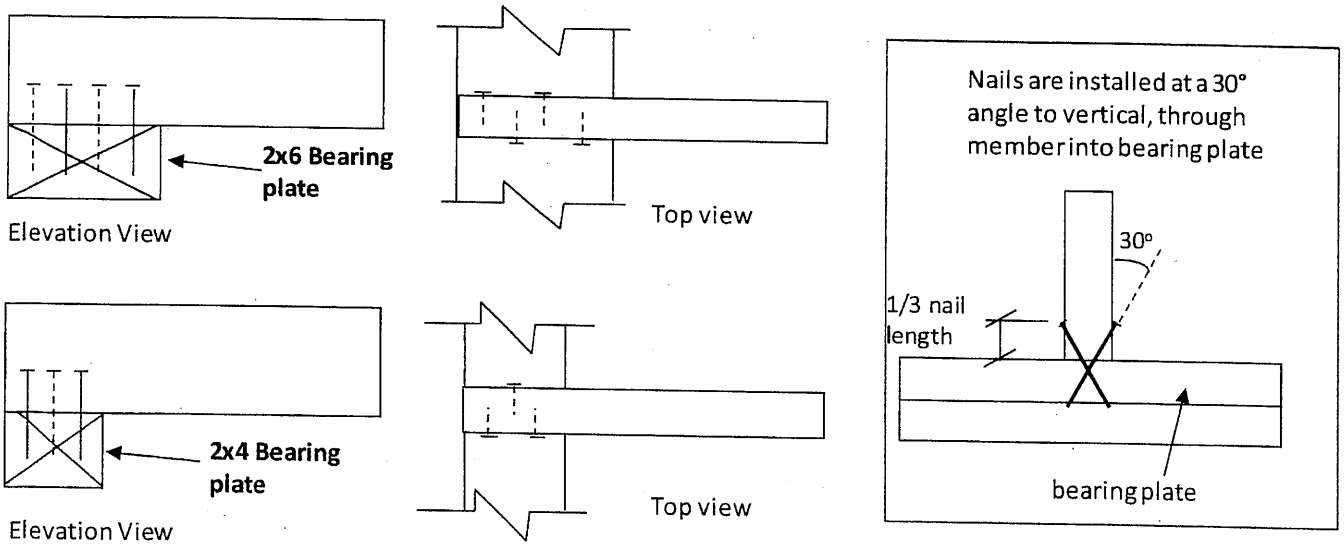
MiTek®

STANDARD DETAIL MSD2015-H

Issued: **SEPTEMBER 22, 2020**
Expiry: **APRIL 30, 2022**

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

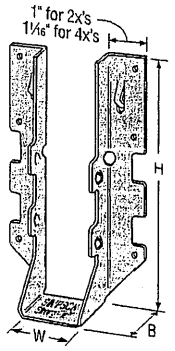
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

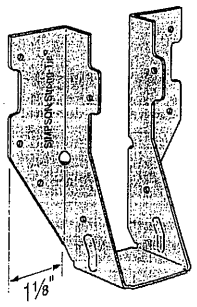
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

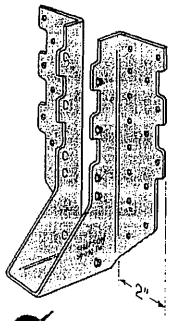
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



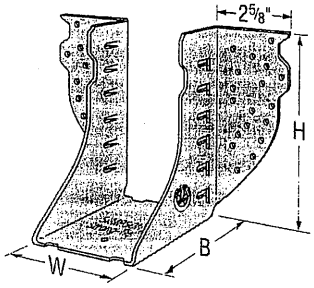
LUS28



LU26L



HUS210
(HUS26, HUS28, and HHUS similar)



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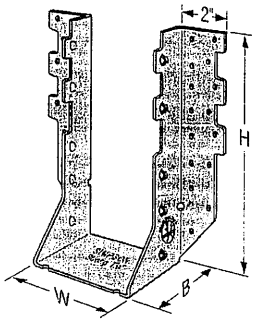
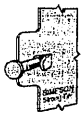
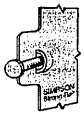


HHUS210-2

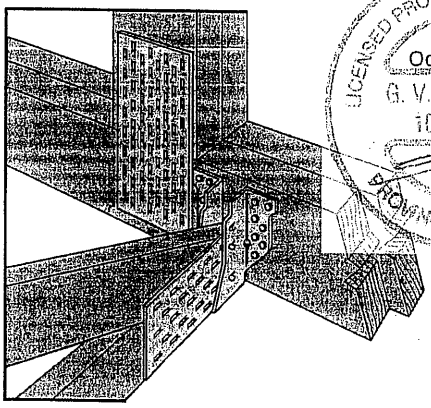
Double-Shear Nailing Top View



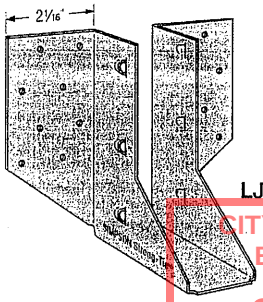
Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View *(available on some models)*



Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

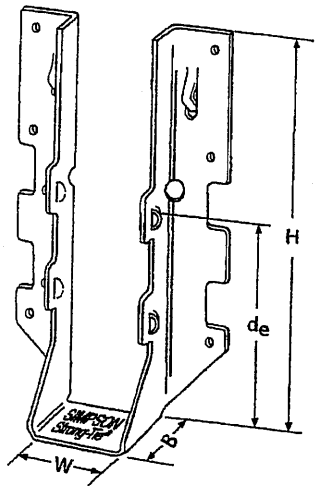
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

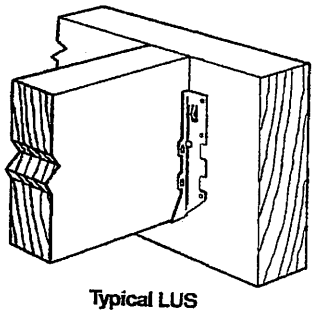
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



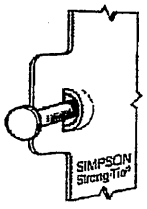
LUS28



Typical LUS installation

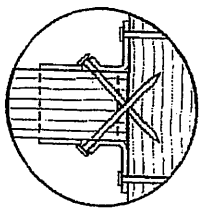
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

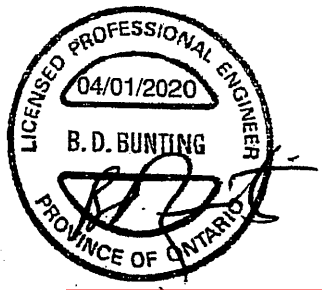


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

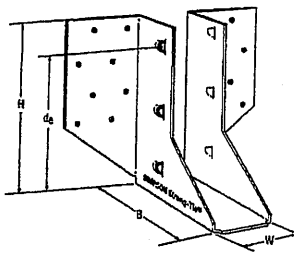
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

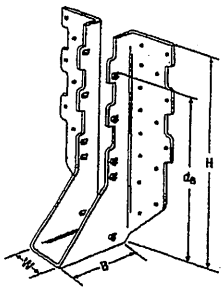
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

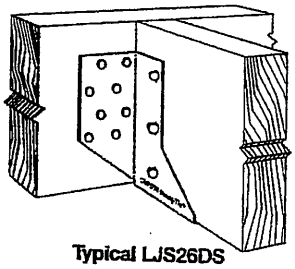
- See current catalogue for options



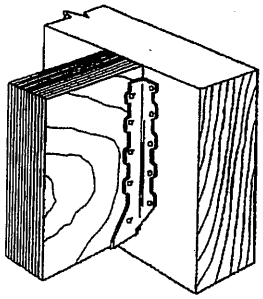
LJS26DS



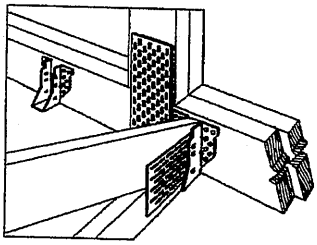
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



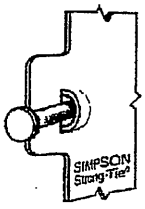
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

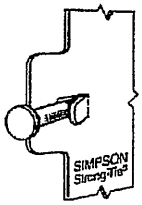
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

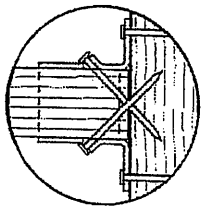


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

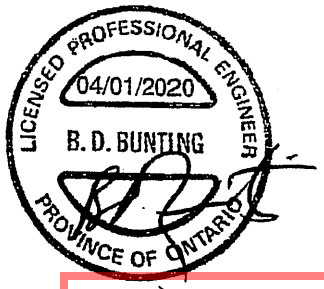
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

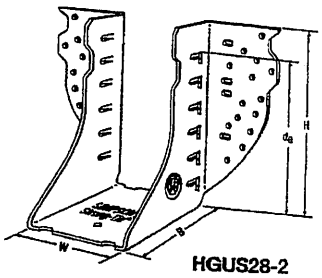
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

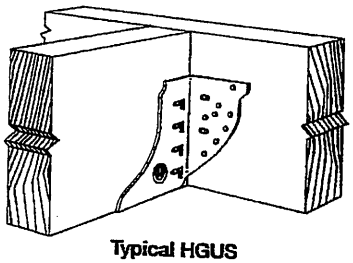
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

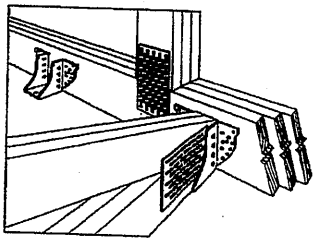
Options:

- See current catalogue for options



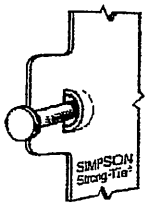
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

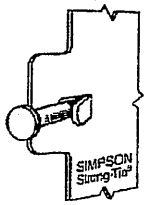
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⁹⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⁹⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

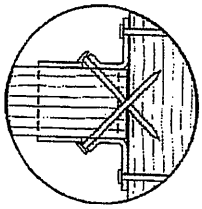


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LIMIT STATES DESIGN

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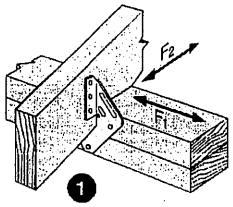
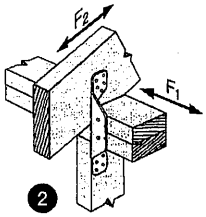
H/TSP

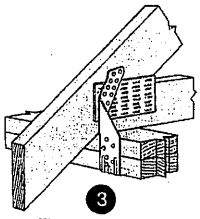
Seismic and Hurricane Ties (cont.)



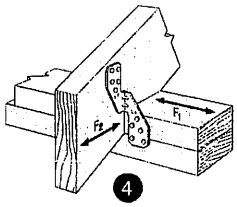
1 H1 Installation



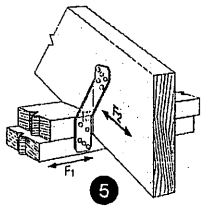
2 H2A Installation



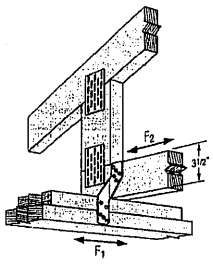
3 TSP Installation



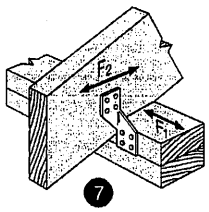
4 H2.5A Installation
(Nails into both top plates)



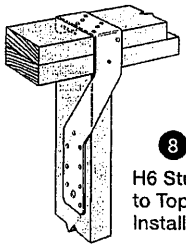
5 H2.5T Installation
(Nails into both top plates)



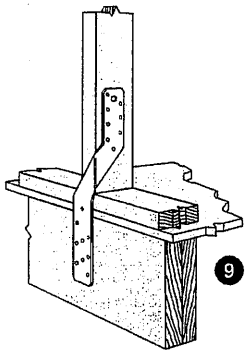
6 H2.5T Installation



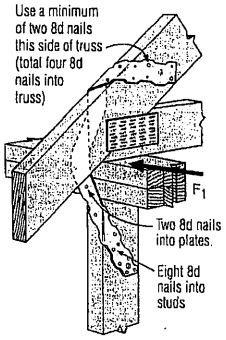
7 H3 Installation
(Nails into upper top plate)



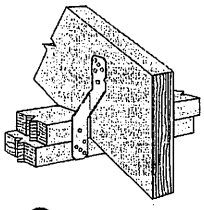
8 H6 Stud to Top Plate Installation



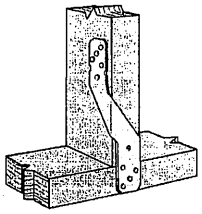
9 H6 Stud to Band Joist Installation



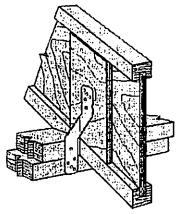
10 H7Z Installation



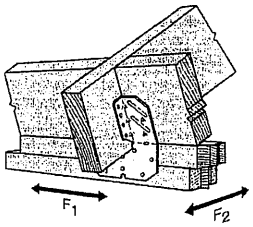
11 H8 Attaching Rafter to Double Top Plates



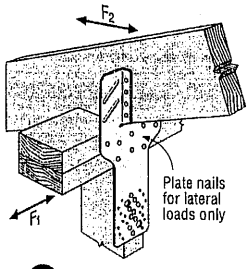
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



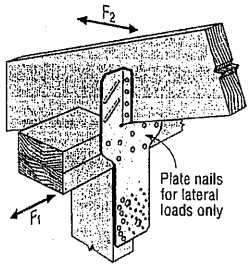
13 H8 attaching I-Joist to Double Top Plates



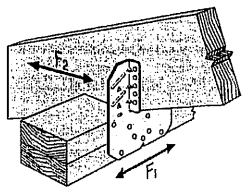
14 H10A Field-Bent Installation



15 H10S Installation



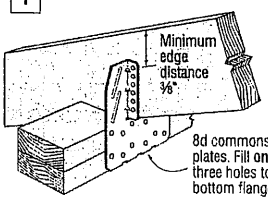
16 H10S Installation with Stud Offset



17 H10A Installation

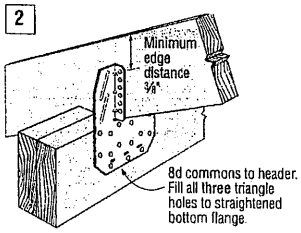
H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

1



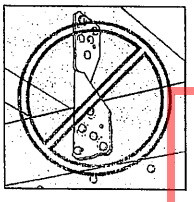
18 H14 Installation to Double Top Plates

2



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

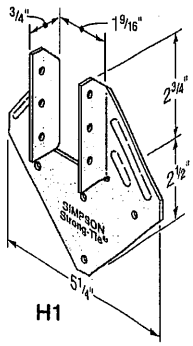
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

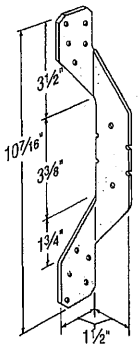
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit **strongtie.com**.

Installation:

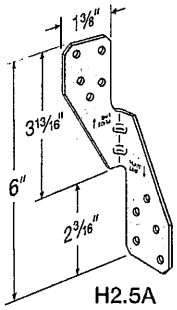
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



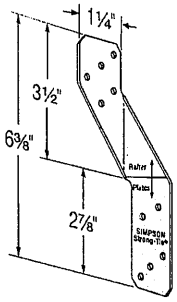
H1



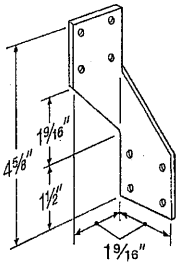
H2A
(H2ASS similar)



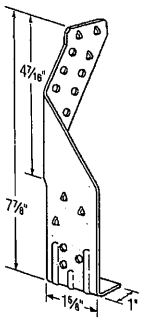
H2.5A
(H2.5ASS similar)



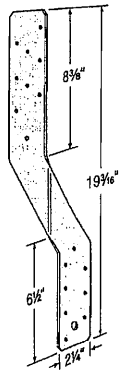
H2.5T



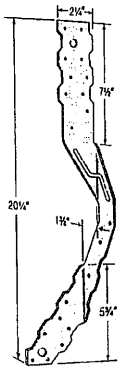
H3



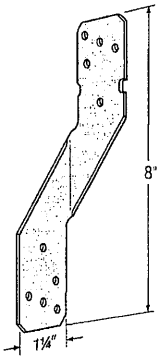
TSP



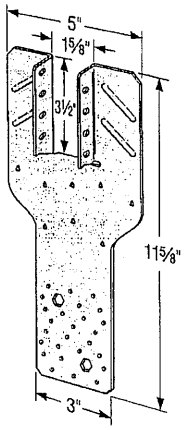
H6



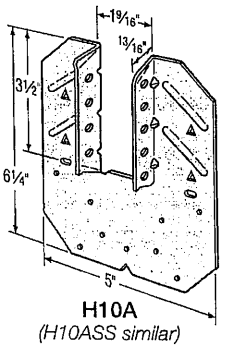
H7Z



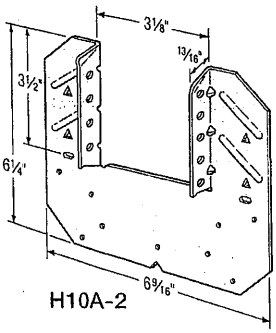
H8



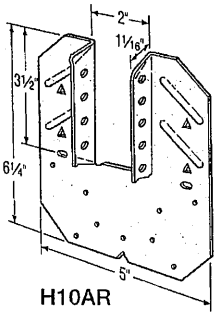
H10S



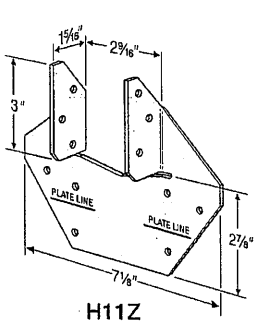
H10A
(H10ASS similar)



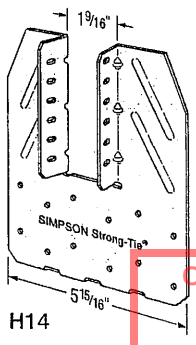
H10A-2



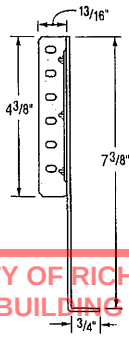
H10AR



H11Z



H14



H14
Profile

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON Strong-Tie

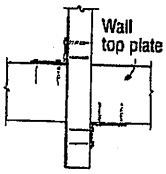
The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

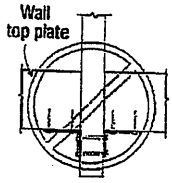
Installation: • Use all specified fasteners
• Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
• H1 can be installed with flanges facing outwards
• Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

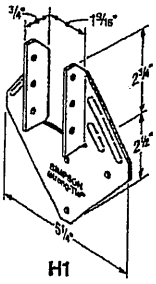
Hurricane Tie Installations to Achieve Twice the Load (Top View)

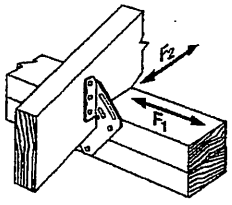


Install diagonally across from each other for minimum 2x truss.

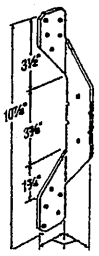


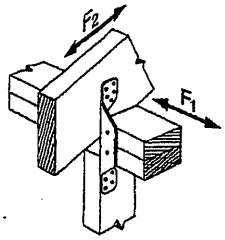
Nailing into both sides of a single ply 2x truss may cause the wood to split.


H1

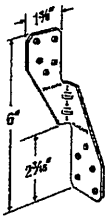


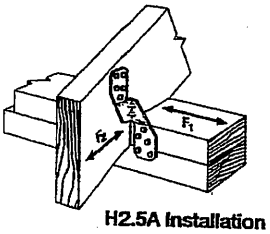
H1 Installation


H2A

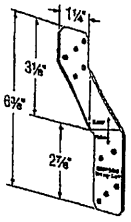


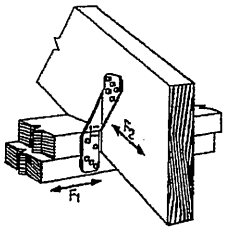
H2A Installation


H2.5A

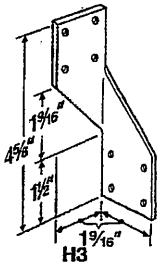


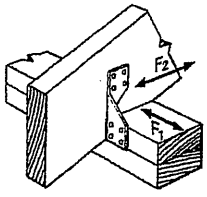
H2.5A Installation


H2.5T

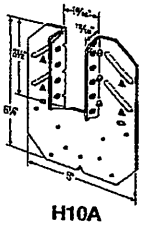


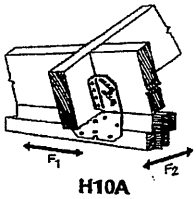
H2.5T Installation
(Nails into both top plates)


H3



H3 Installation


H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal F_1	Normal F_2	Uplift	Normal F_1	Normal F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

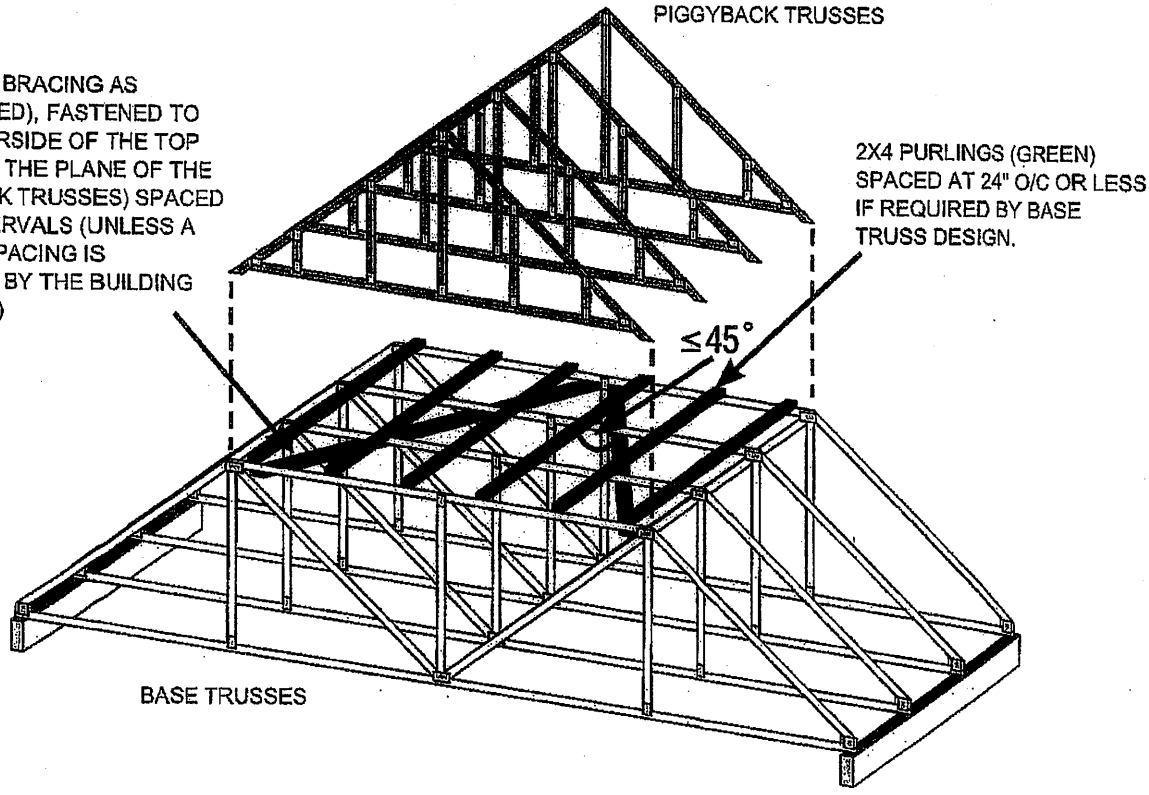
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

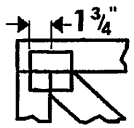


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

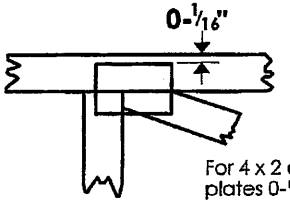
SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

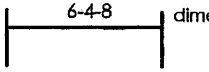
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

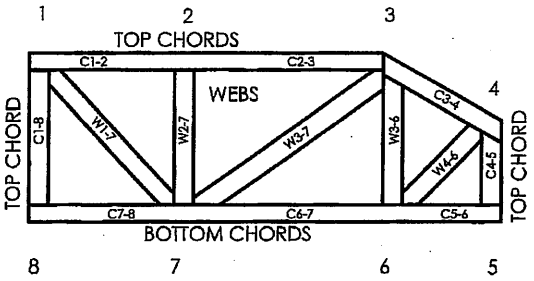
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.