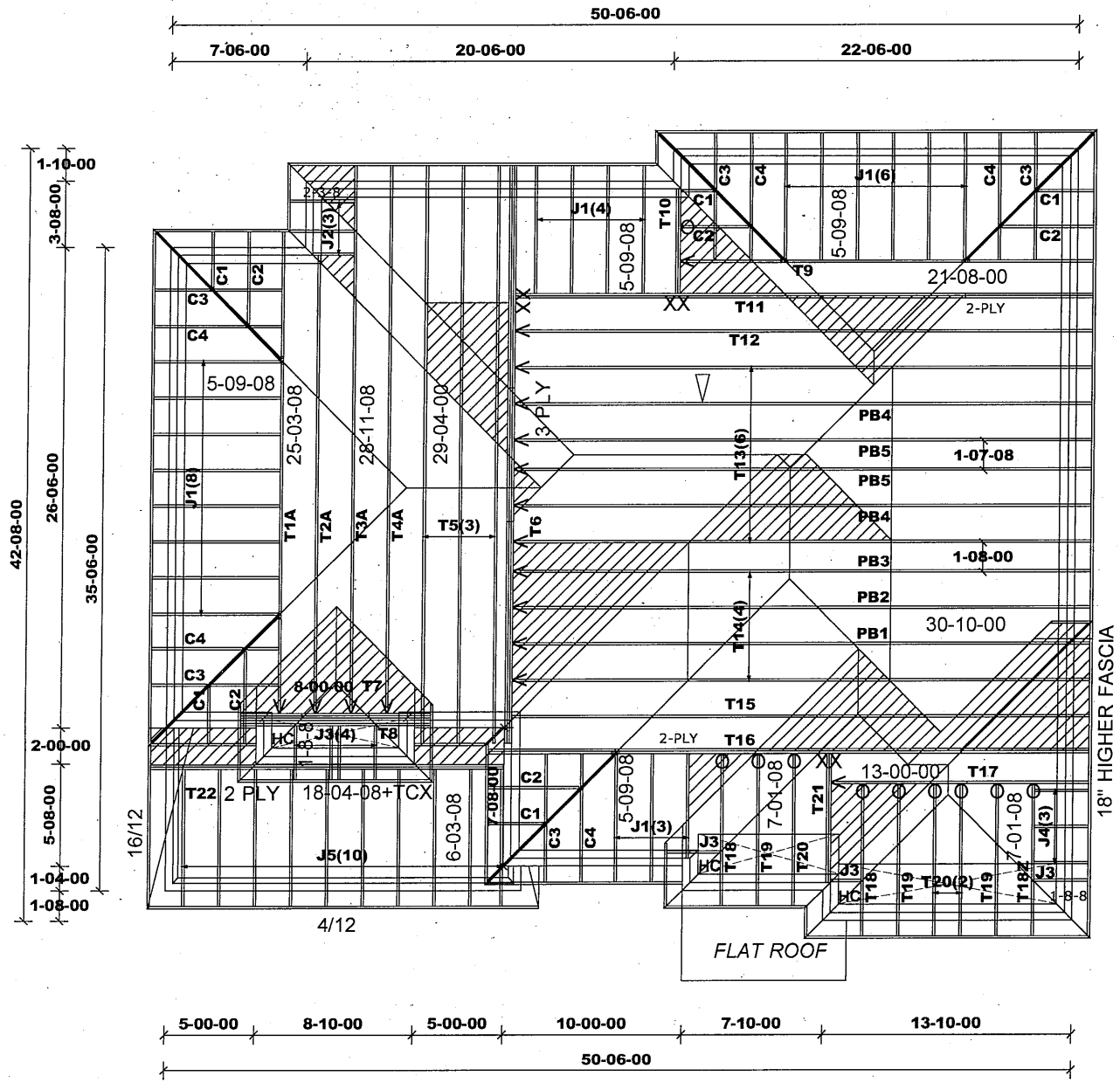


ALL ROOF PITCHES ARE 8/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

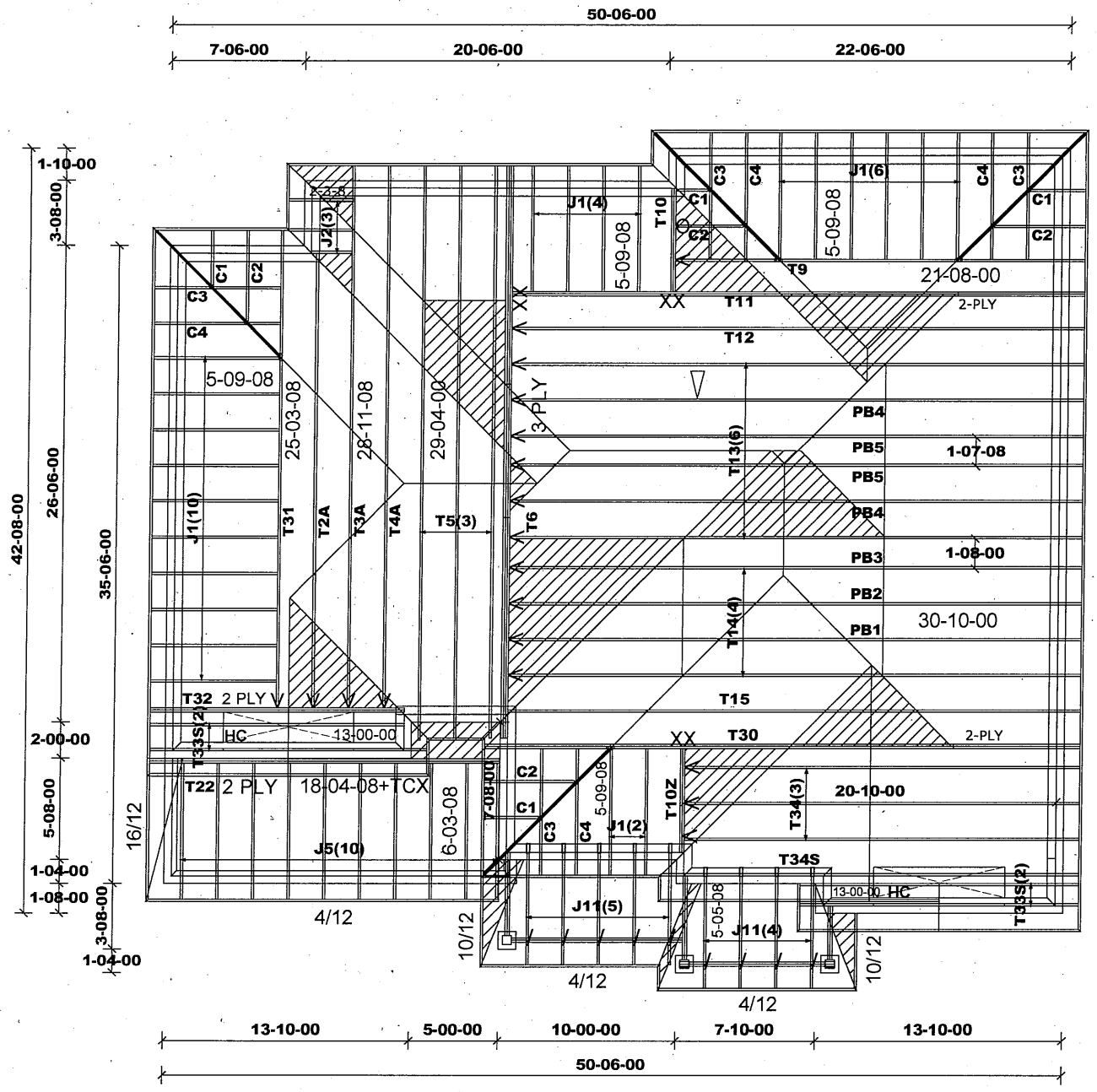
All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

HC: 18" RAISED CEILING & PLATE

M15568

TAMARACK ROOF TRUSSES INC.	Job Track: 53256	Builder / Location: GREENPARK HOMES / RICHMOND HILL	Model / Elevation: ROSE 9 / 1
	Plan Log: 207149	Project: TRINIGROUP DEVELOPMENTS	
	Layout ID: 432478	Date: 2023-07-19 Sales: Rick DiCiano Designer: LC/YPG	

Mitek ver 8.5.3.233



ALL ROOF PITCHES ARE 8/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (/)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

HC: 18" RAISED CEILING & PLATE

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.mabua

M15568

TAMARACK ROOF TRUSSES INC.

Job Track:	53256
Plan Log:	207149
Layout ID:	432488

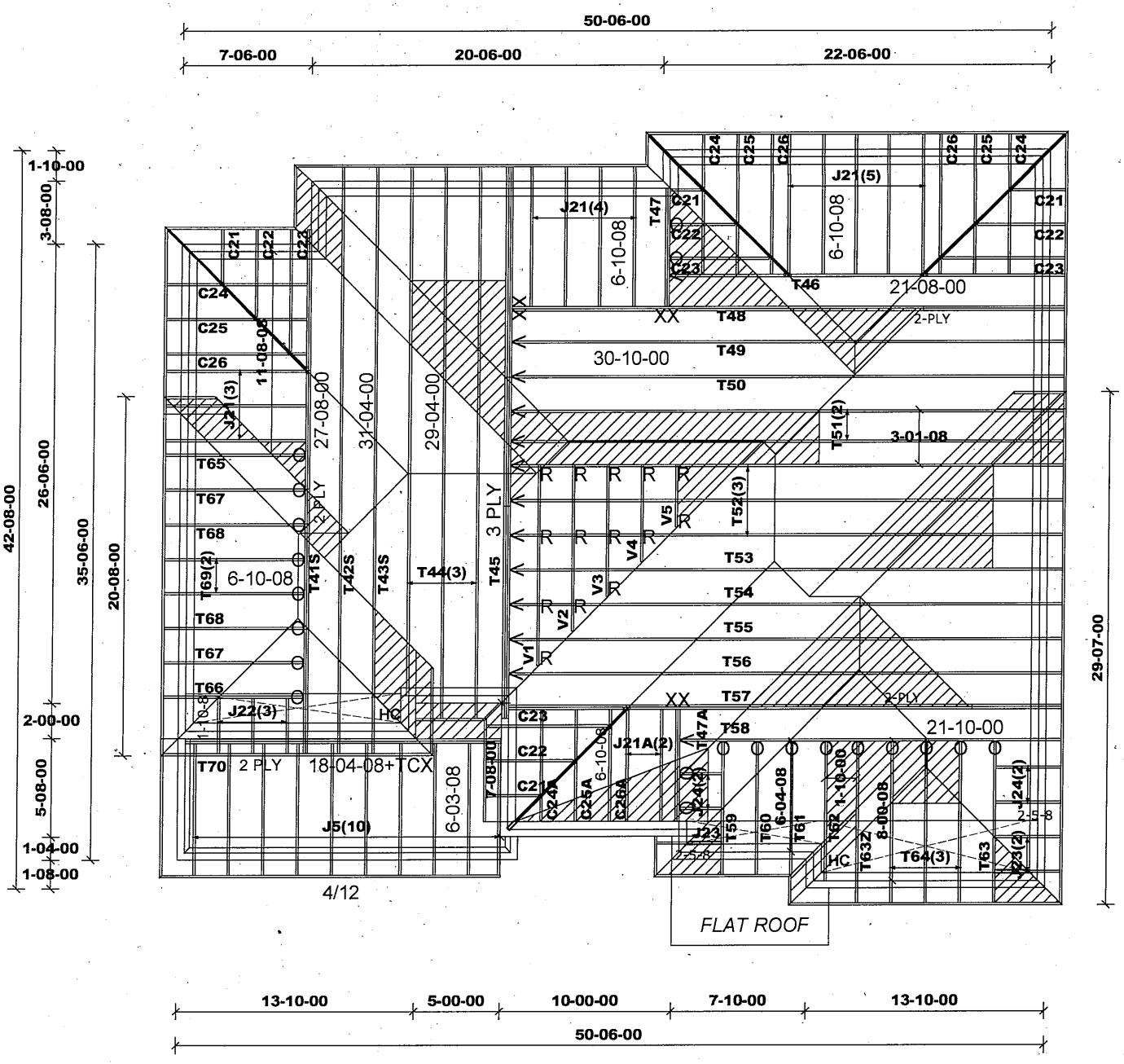
Builder / Location: **GREENPARK HOMES / RICHMOND HILL**

Model / Elevation: **ROSE 9 / 2**

Project: **TRINIGROUP DEVELOPMENTS**

Date: 2023-07-19 Sales: Rick DiCiano Designer: LC/YPG

Mitek ver 8.5.3.233



ALL ROOF PITCHES ARE 6/12
UNLESS OTHERWISE NOTED

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)
VTCR - (R)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH OBC 2012 (2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

HC: 18" RAISED CEILING & PLATE

M15568

TAMARACK ROOF TRUSSES INC.

Job Track:	53256
Plan Log:	207149
Layout ID:	432489

Builder / Location: **GREENPARK HOMES / RICHMOND HILL**

Model / Elevation: **ROSE 9 / 3**

Project: **TRINIGROUP DEVELOPMENTS**

Date: 2023-07-19 | Sales: Rick DiCiano | Designer:LC/YPG

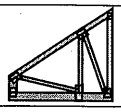
Mitek ver 8.5.3.233

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207149
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432478
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 9	Page:	1 of 3
Lot #:		Date:	07-19-2023
Elevation:	1	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T1A Hip Girder	8 /12	25-03-08	5-03-02	2 x 4 2 x 6	1-03-08	1-04-13 1-07-13	126.76 79.50		
	1	T2A Hip	8 /12	25-03-08	6-07-02	2 x 4	1-03-08	1-04-13 1-07-13	112.39 72.50		
	1	T3A Roof Special Girder	8 /12	28-11-08	7-11-02	2 x 4	1-03-08	1-04-13 1-07-13	127.54 83.17		
	1	T4A Roof Special	8 /12	28-11-08	9-03-02	2 x 4	1-03-08	1-04-13 1-07-13	140.85 88.83		
	3	T5 Roof Special	8 /12	29-04-00	9-11-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	409.09 262.50		
	1 3-ply	T6 Hip Girder	8 /12	29-04-00	8-08-08	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	514.01 310.00		
	1 2-ply	T7 Hip Girder	8 /12	8-00-00	4-03-13	2 x 4 2 x 6	1-03-08 1-03-08	2-10-13 2-10-13	107.99 68.67		
	1	T8 Hip Girder	8 /12	8-00-00	2-06-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	34.91 21.83		
	1	T9 Hip Girder	8 /12	21-08-00	5-03-02	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	109.39 67.83		
	1 2-ply	T10 Monopitch Girder	8 /12	5-09-08	5-03-02	2 x 4 2 x 6		1-04-13 5-03-02	67.68 43.33		
	1 2-ply	T11 Half Hip Girder	8 /12	30-10-00	5-03-02	2 x 4 2 x 6	1-03-08	1-04-13 5-03-02	299.9 185.33		
	1	T12 Half Hip	8 /12	30-10-00	6-07-02	2 x 4	1-03-08	1-04-13 6-07-02	137.99 88.33		
	6	T13 Half Hip	8 /12	30-10-00	7-11-02	2 x 4	1-03-08	1-04-13 7-11-02	831.64 517.00		
	4	T14 Hip	8 /12	30-10-00	7-11-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	553.34 350.67		

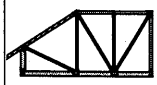
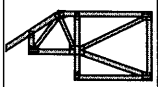
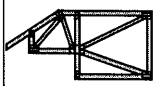
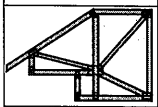
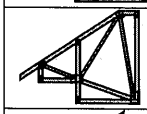
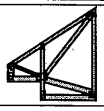
TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	ROSE 9
Lot #:	
Elevation:	1

Job Track:	53256
PlanLog:	207149
Layout ID:	432478
Ref #	
Page:	2 of 3
Date:	07-19-2023
Designer:	Leo Chen
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T15 Hip	8 /12	30-10-00	6-07-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	138.17 87.33		
	1 2-ply	T16 Hip Girder	8 /12	30-10-00	5-03-02	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	301.8 184.67		
	1	T17 Half Hip Girder	8 /12	13-00-00	6-06-08	2 x 4 2 x 6	1-03-08	2-10-13 6-06-08	77.17 48.33		
	2	T18 Half Hip Girder	8 /12	7-01-08	4-00-08	2 x 4	1-03-08	1-04-13 4-00-08	79.1 53.67		
	1	T18Z Half Hip Girder	8 /12	7-01-08	4-00-08	2 x 4	1-03-08	1-04-13 4-00-08	39.62 26.83		
	3	T19 Half Hip	8 /12	7-01-08	5-04-08	2 x 4	1-03-08	1-04-13 5-04-08	131.24 87.50		
	3	T20 Half Hip	8 /12	7-01-08	6-08-08	2 x 4	1-03-08	1-04-13 6-08-08	145.6 98.00		
	1 2-ply	T21 Monopitch Girder	8 /12	7-01-08	7-07-13	2 x 4 2 x 6		1-04-13 7-07-13	106.97 68.33		
	1 2-ply	T22 Half Hip Girder	16 /12	18-04-08	3-00-06	2 x 6 2 x 4	1-03-08	1-08-15 2-05-02	174.18 110.33		
	1	PB1 Piggyback	8 /12	11-03-01	1-04-00	2 x 4			31.85 21.50		
	1	PB2 Piggyback	8 /12	11-03-01	2-08-00	2 x 4			31.75 20.00		
	1	PB3 Piggyback	8 /12	11-03-01	3-09-00	2 x 4			28.93 17.50		
	2	PB4 Piggyback	8 /12	21-00-08	1-04-00	2 x 4		1-00-06	130.94 85.00		
	2	PB5 Piggyback	8 /12	21-00-08	2-08-00	2 x 4		1-00-06	139.43 87.33		

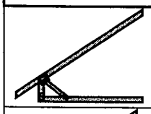
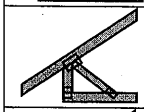
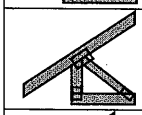
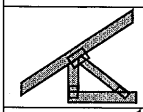
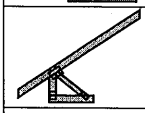
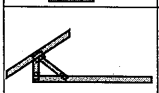
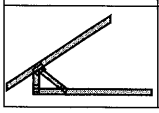
TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	ROSE 9
Lot #:	
Elevation:	1

Job Track:	53256
PlanLog:	207149
Layout ID:	432478
Ref #	
Page:	3 of 3
Date:	07-19-2023
Designer:	Leo Chen
Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	21	J1 Jack-Open	8 /12	**5-09-08**	5-03-02	2 x 4	1-03-08	1-04-13 5-03-02	398.61 245.00		
	3	J2 Jack-Open	8 /12	**2-03-08**	2-11-02	2 x 4	1-03-08	1-04-13 2-11-02	30.58 21.00		
	7	J3 Jack-Open	8 /12	**1-08-08**	2-06-08	2 x 4	1-03-08	1-04-13 2-06-08	61.1 39.67		
	3	J4 Jack-Open	8 /12	**1-08-08**	4-00-08	2 x 4	1-03-08	2-10-13 4-00-08	34.11 22.50		
	10	J5 Jack-Partial	4 /12	**6-03-08**	2-10-00	2 x 4	1-03-08	3-15 2-05-02	189.33 126.67		
	5	C1 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	45.13 28.33		
	5	C2 Jack-Open	8 /12	**1-10-08**	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	58.84 38.33		
	5	C3 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 4-00-01	1-04-13 2-07-02	67.46 41.67		
	5	C4 Jack-Open	8 /12	**3-09-07**	3-11-02	2 x 4	1-03-08 2-00-01	1-04-13 3-11-02	81.17 51.67		

TOTAL # TRUSS= **117** TOTAL BFT OF ALL TRUSSES= **3850.65** BFT. TOTAL WEIGHT OF ALL TRSSES **6096.6** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
17	Hardware	LJS26DS	
10	Hardware	LUS24	

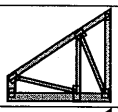
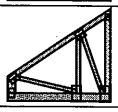
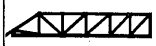
TOTAL NUMBER OF ITEMS= **30**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207149
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432488
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 9	Page:	1 of 3
Lot #:		Date:	07-19-2023
Elevation:	2	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

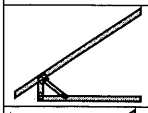
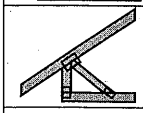
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T2A Hip	8 /12	25-03-08	6-07-02	2 x 4	1-03-08	1-04-13 1-07-13	112.39 72.50		
	1	T3A Roof Special Girder	8 /12	28-11-08	7-11-02	2 x 4	1-03-08	1-04-13 1-07-13	127.54 83.17		
	1	T4A Roof Special	8 /12	28-11-08	9-03-02	2 x 4	1-03-08	1-04-13 1-07-13	136.03 86.17		
	3	T5 Roof Special	8 /12	29-04-00	9-11-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	409.09 262.50		
	1 3-ply	T6 Hip Girder	8 /12	29-04-00	8-08-08	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	514.01 310.00		
	1	T9 Hip Girder	8 /12	21-08-00	5-03-02	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	109.39 67.83		
	1 2-ply	T10 Monopitch Girder	8 /12	5-09-08	5-03-02	2 x 4 2 x 6		1-04-13 5-03-02	67.63 43.33		
	1 2-ply	T10Z Monopitch Girder	8 /12	5-09-08	5-03-02	2 x 4 2 x 6		1-04-13 5-03-02	67.68 43.33		
	1 2-ply	T11 Half Hip Girder	8 /12	30-10-00	5-03-02	2 x 4 2 x 6	1-03-08	1-04-13 5-03-02	299.9 185.33		
	1	T12 Half Hip	8 /12	30-10-00	6-07-02	2 x 4	1-03-08	1-04-13 6-07-02	137.99 88.33		
	6	T13 Half Hip	8 /12	30-10-00	7-11-02	2 x 4	1-03-08	1-04-13 7-11-02	831.64 517.00		
	4	T14 Hip	8 /12	30-10-00	7-11-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	553.34 350.67		
	1	T15 Hip	8 /12	30-10-00	6-07-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	138.17 87.33		
	1 2-ply	T22 Half Hip Girder	16 /12	18-04-08	3-00-06	2 x 6 2 x 4	1-03-08	1-08-15 2-05-02	174.18 110.33		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207149
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432488
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 9	Page:	2 of 3
Lot #:		Date:	07-19-2023
Elevation:	2	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

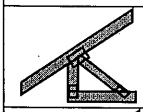
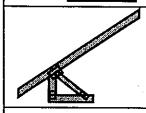
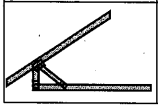
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T30 Hip Girder	8 /12	30-10-00	5-03-02	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	296.84 179.67		
	1	T31 Half Hip Girder	8 /12	25-03-08	5-03-02	2 x 4 2 x 6	1-03-08	1-04-13 5-03-02	127.08 78.33		
	1 2-ply	T32 Common Girder	8 /12	13-00-00	5-08-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	162.89 98.67		
	4	T33S Roof Special	8 /12	13-00-00	5-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	256.21 164.00		
	3	T34 Common	8 /12	20-10-00	8-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	283.76 176.00		
	1	T34S Roof Special	8 /12	20-10-00	8-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	103.69 66.33		
	1	PB1 Piggyback	8 /12	11-03-01	1-04-00	2 x 4			31.85 21.50		
	1	PB2 Piggyback	8 /12	11-03-01	2-08-00	2 x 4			31.75 20.00		
	1	PB3 Piggyback	8 /12	11-03-01	3-09-00	2 x 4			28.93 17.50		
	2	PB4 Piggyback	8 /12	21-00-08	1-04-00	2 x 4		1-00-06	130.94 85.00		
	2	PB5 Piggyback	8 /12	21-00-08	2-08-00	2 x 4		1-00-06	139.43 87.33		
	22	J1 Jack-Open	8 /12	5-09-08	5-03-02	2 x 4	1-03-08	1-04-13 5-03-02	417.6 256.67		
	3	J2 Jack-Open	8 /12	2-03-08	2-11-02	2 x 4	1-03-08	1-04-13 2-11-02	30.58 21.00		
	10	J5 Jack-Partial	4 /12	6-03-08	2-10-00	2 x 4	1-03-08	3-15 2-05-02	189.33 126.67		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207149
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432488
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 9	Page:	3 of 3
Lot #:		Date:	07-19-2023
Elevation:	2	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	9	J11 Jack-Open	4 /12	**5-05-08**	2-06-11	2 x 4	1-03-08	3-15 2-01-12	130.04 84.00		
	4	C1 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	36.11 22.67		
	4	C2 Jack-Open	8 /12	**1-10-08**	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	47.07 30.67		
	4	C3 Jack-Open	8 /12	**1-09-07**	2-07-02	2 x 4	1-03-08 4-00-01	1-04-13 2-07-02	53.97 33.33		
	4	C4 Jack-Open	8 /12	**3-09-07**	3-11-02	2 x 4	1-03-08 2-00-01	1-04-13 3-11-02	64.93 41.33		

TOTAL # TRUSS= **110** TOTAL BFT OF ALL TRUSSES= **3918.49** BFT. TOTAL WEIGHT OF ALL TRSSES **6242** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	H2.5T	
2	Hardware	HGUS26-2	
20	Hardware	LJS26DS	
1	Hardware	LUS24	

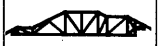
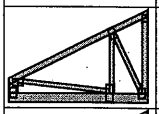
TOTAL NUMBER OF ITEMS= **32**

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207149
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432489
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 9	Page:	1 of 4
Lot #:		Date:	07-19-2023
Elevation:	3	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T41S Roof Special Girder	6 /12	27-08-00	5-07-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	284.01 180.33		
	1	T42S Roof Special	6 /12	31-04-00	5-07-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	140.78 94.17		
	1	T43S Roof Special	6 /12	31-04-00	6-07-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	135.57 87.83		
	3	T44 Roof Special	6 /12	29-04-00	7-07-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	356.68 226.50		
	1 3-ply	T45 Hip Girder	6 /12	29-04-00	6-07-12	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	458.39 282.00		
	1	T46 Hip Girder	6 /12	21-08-00	4-07-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	106.15 67.00		
	1 2-ply	T47 Monopitch Girder	6 /12	6-10-08	4-07-04	2 x 4 2 x 6		1-02-00 4-07-04	71.09 44.00		
	1 2-ply	T47A Monopitch Girder	6 /12	6-05-00	4-07-04	2 x 4 2 x 6		1-04-12 4-07-04	71.71 46.00		
	1 2-ply	T48 Half Hip Girder	6 /12	30-10-00	4-07-04	2 x 4 2 x 6	1-03-08	1-02-00 4-07-04	292.01 180.67		
	1	T49 Half Hip	6 /12	30-10-00	5-07-04	2 x 4	1-03-08	1-02-00 5-07-04	129.7 83.00		
	1	T50 Half Hip	6 /12	30-10-00	6-07-04	2 x 4	1-03-08	1-02-00 6-07-04	129.38 81.17		
	2	T51 Hip	6 /12	30-10-00	7-07-04	2 x 4	1-03-08	1-02-00 6-10-00	299.68 189.67		
	3	T52 Roof Special	6 /12	30-10-00	8-10-08	2 x 4	1-03-08	2-08-00 1-02-00	402.54 251.50		
	1	T53 Roof Special	6 /12	30-10-00	8-07-04	2 x 4	1-03-08	2-08-00 1-02-00	143.35 92.17		

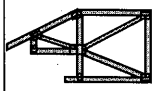
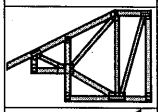
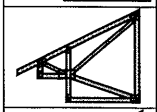
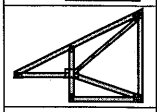
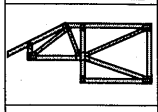
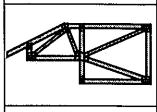
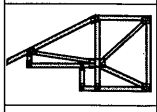
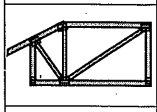
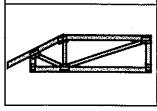
TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	GREENPARK HOMES
Project:	TRINIGROUP DEVELOPMENTS
Location:	RICHMOND HILL
Model:	ROSE 9
Lot #:	
Elevation:	3

Job Track:	53256
PlanLog:	207149
Layout ID:	432489
Ref #	
Page:	2 of 4
Date:	07-19-2023
Designer:	Leo Chen
Sales Rep:	Rick DiCiano

Roof Trusses

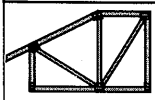
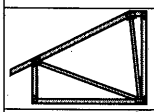
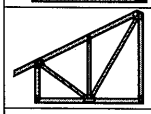
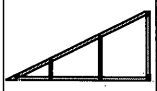
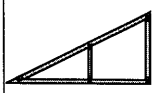
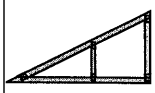
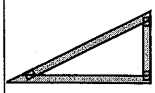
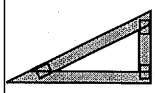
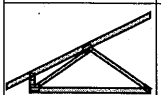
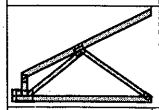
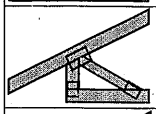
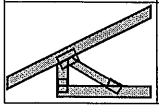
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T54 Hip	6 /12	30-10-00	7-07-04	2 x 4	1-03-08	2-08-00 1-02-00	135.18 85.50		
	1	T55 Hip	6 /12	30-10-00	6-07-04	2 x 4	1-03-08	2-08-00 1-02-00	129.18 83.00		
	1	T56 Hip	6 /12	30-10-00	5-07-04	2 x 4	1-03-08	2-08-00 1-02-00	126.21 82.00		
	1 2-ply	T57 Hip Girder	6 /12	30-10-00	4-07-04	2 x 4 2 x 6	1-03-08	2-08-00 1-02-00	290.56 180.67		
	1	T58 Hip Girder	6 /12	20-10-00	5-10-04	2 x 4 2 x 6	1-03-08 1-03-08	2-08-00 2-08-00	117.77 75.33		
	1	T59 Half Hip Girder	6 /12	6-04-08	3-10-12	2 x 4	1-03-08	1-02-00 3-10-12	34.9 23.50		
	1	T60 Half Hip	6 /12	6-04-08	4-10-12	2 x 4	1-03-08	1-02-00 4-10-12	40.91 27.33		
	1	T61 Monopitch	6 /12	6-04-08	5-10-04	2 x 4	1-03-08	1-02-00 5-10-04	38.09 25.33		
	1	T62 Monopitch	6 /12	8-00-08	5-10-04	2 x 4		4-00 5-10-04	38.33 25.67		
	1	T63 Half Hip Girder	6 /12	8-00-08	3-10-12	2 x 4	1-03-08	1-02-00 3-10-12	41.27 28.17		
	1	T63Z Half Hip	6 /12	8-00-08	3-10-12	2 x 4	1-03-08	1-02-00 3-10-12	41.27 28.17		
	3	T64 Half Hip	6 /12	8-00-08	4-10-12	2 x 4	1-03-08	1-02-00 4-10-12	133.52 91.00		
	1	T65 Half Hip	6 /12	6-10-08	3-07-04	2 x 4	1-03-08	2-08-00 3-07-04	32.77 21.83		
	1	T66 Half Hip Girder	6 /12	6-10-08	2-01-04	2 x 4	1-03-08	1-02-00 2-01-04	26.79 17.17		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207149
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432489
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 9	Page:	3 of 4
Lot #:		Date:	07-19-2023
Elevation:	3	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

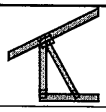
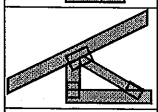
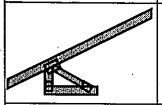
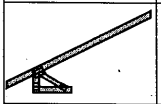
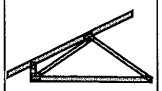
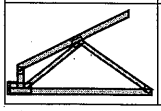
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T67 Half Hip	6 /12	6-10-08	4-07-04	2 x 4	1-03-08	2-08-00 4-07-04	71.31 47.00		
	2	T68 Half Hip	6 /12	6-10-08	5-07-04	2 x 4	1-03-08	2-08-00 5-07-04	71.8 47.67		
	2	T69 Common	6 /12	6-10-08	5-11-00	2 x 4	1-03-08	2-08-00 5-08-12	76.09 48.33		
	1 2-ply	T70 Flat Girder	0 /12	18-04-08	2-05-02	2 x 6 2 x 4		2-05-02 2-05-02	164.86 101.67		
	1	V1 Valley	6 /12	11-07-00	5-07-04	2 x 4		5-07-04	36.57 23.67		
	1	V2 Valley	6 /12	9-07-00	4-09-08	2 x 4		4-09-08	28.88 18.83		
	1	V3 Valley	6 /12	7-07-00	3-09-08	2 x 4		3-09-08	22.5 14.33		
	1	V4 Valley	6 /12	5-07-00	2-09-08	2 x 4		2-09-08	15.12 10.00		
	1	V5 Valley	6 /12	3-07-00	1-09-08	2 x 4		1-09-08	9.15 6.67		
	10	J5 Jack-Partial	4 /12	6-03-08	2-10-00	2 x 4	1-03-08	3-15 2-05-02	189.33 126.67		
	12	J21 Jack-Partial	6 /12	6-10-08	4-07-04	2 x 4	1-03-08	1-02-00 4-07-04	308.82 198.00		
	2	J21A Jack-Partial	6 /12	6-05-00	4-07-04	2 x 4		1-04-12 4-07-04	47.96 31.67		
	3	J22 Jack-Open	6 /12	1-10-08	2-01-04	2 x 4	1-03-08	1-02-00 2-01-04	25.25 17.00		
	3	J23 Jack-Open	6 /12	2-05-08	2-04-12	2 x 4	1-03-08	1-02-00 2-04-12	29.48 21.00		

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	53256
Builder:	GREENPARK HOMES	PlanLog:	207149
Project:	TRINIGROUP DEVELOPMENTS	Layout ID:	432489
Location:	RICHMOND HILL	Ref #	
Model:	ROSE 9	Page:	4 of 4
Lot #:		Date:	07-19-2023
Elevation:	3	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	J24 Jack-Open	6 /12	**2-05-08**	3-10-12	2 x 4	1-03-08	2-08-00 3-10-12	49.36 35.33		
	4	C21 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	33.22 22.67		
	4	C22 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	43.41 28.00		
	4	C23 Jack-Open	6 /12	**1-10-08**	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	53.61 36.00		
	3	C24 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 5-01-01	1-02-00 2-00-12	42.02 27.00		
	1	C24A Jack-Open	6 /12	**6-05-00**	2-00-12	2 x 4		1-04-12 4-07-04	12.31 8.33		
	3	C25 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 3-01-01	1-02-00 3-00-12	49.67 31.00		
	1	C25A Jack-Open	6 /12	**6-05-00**	3-00-12	2 x 4		1-04-12 4-07-04	14.86 9.67		
	3	C26 Jack-Open	6 /12	**5-09-07**	4-00-12	2 x 4	1-03-08 1-01-01	1-02-00 4-00-12	73.04 47.50		
	1	C26A Jack-Open	6 /12	**6-05-00**	4-00-12	2 x 4		1-04-12 4-07-04	22.59 15.17		

TOTAL # TRUSS= **113** TOTAL BFT OF ALL TRUSSES= **3673.86** BFT. TOTAL WEIGHT OF ALL TRSSES **5739.03** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
14		VTCR	
3	Hardware	HGUS26-2	
14	Hardware	LJS26DS	
21	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= **52**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T1A	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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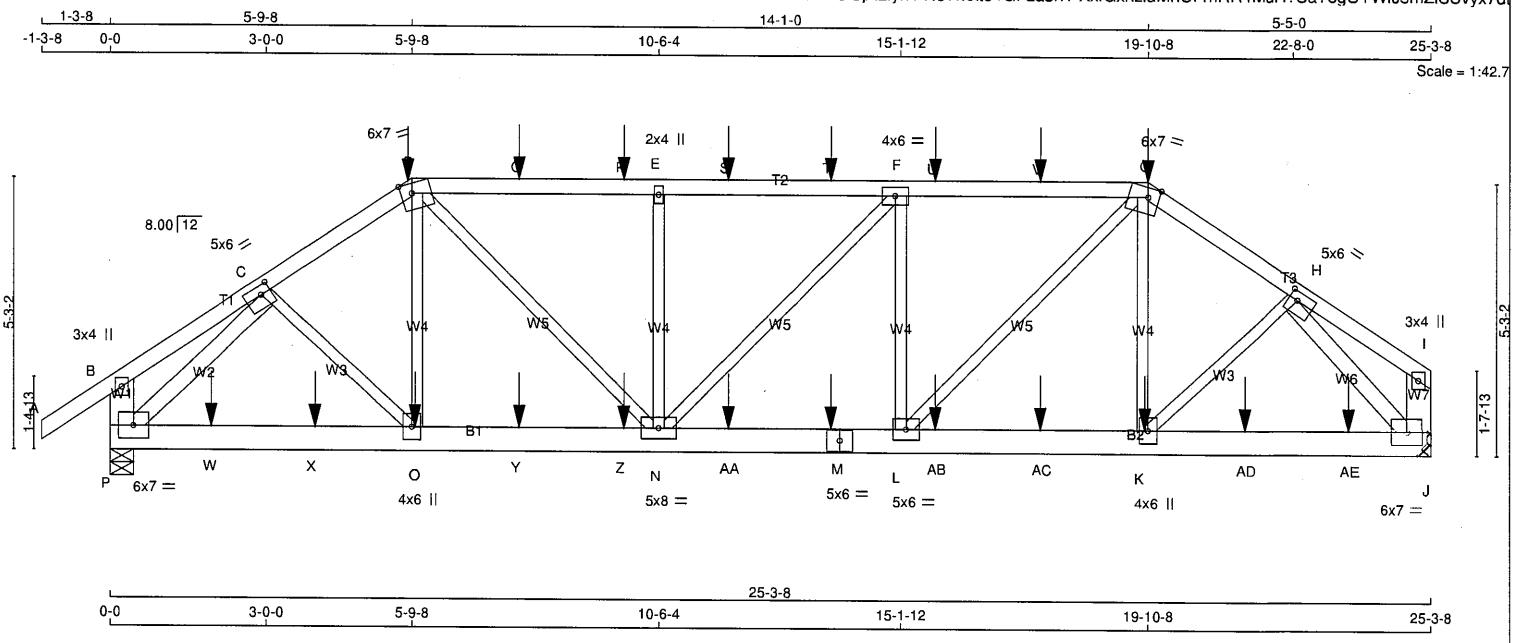


TOTAL WEIGHT = 127 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	2.25
D	TTWW-m	MT20	6.0	7.0	Edge	2.50
E	TMV+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTWW-m	MT20	6.0	7.0	Edge	2.50
H	TMWW-t	MT20	5.0	6.0	2.00	2.00
I	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	6.0	7.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMVW1-t	MT20	6.0	7.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311444

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	2618	0	2618	0	0	MECHANICAL	
P	2708	0	2708	0	0	5-8	2-15

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-13.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1853	1206 / 0	0 / 0	0 / 0	0 / 0	647 / 0	0 / 0
P	1914	1260 / 0	0 / 0	0 / 0	0 / 0	654 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	O- D	-57 / 127	0.05 (4)	
B- C	0 / 12	-91.8	-91.8	0.10 (1)	10.00	K- G	-223 / 68	0.09 (1)	
C- D	-3230 / 0	-91.8	-91.8	0.25 (1)	3.67	K- H	0 / 632	0.16 (1)	
D- Q	-3634 / 0	-91.8	-91.8	0.86 (1)	2.62	H- J	-3233 / 0	0.57 (1)	
Q- R	-3634 / 0	-91.8	-91.8	0.86 (1)	2.62	C- O	0 / 421	0.10 (1)	
R- E	-3634 / 0	-91.8	-91.8	0.86 (1)	2.62	P- C	-3370 / 0	0.65 (1)	
E- S	-3634 / 0	-91.8	-91.8	0.87 (1)	2.58	L- G	0 / 1513	0.37 (1)	
S- T	-3634 / 0	-91.8	-91.8	0.87 (1)	2.58	D- N	0 / 1385	0.34 (1)	
T- F	-3634 / 0	-91.8	-91.8	0.87 (1)	2.58	L- F	-925 / 0	0.37 (1)	
F- U	-3610 / 0	-91.8	-91.8	0.88 (1)	2.59	N- E	-893 / 0	0.35 (1)	
U- V	-3610 / 0	-91.8	-91.8	0.88 (1)	2.59	N- F	0 / 34	0.01 (1)	
V- G	-3610 / 0	-91.8	-91.8	0.88 (1)	2.59				
G- H	-3097 / 0	-91.8	-91.8	0.24 (1)	3.75				
H- I	0 / 13	-91.8	-91.8	0.09 (1)	10.00				
P- B	-236 / 0	0.0	0.0	0.02 (1)	7.81				
J- I	-92 / 0	0.0	0.0	0.01 (1)	7.81				
P- W	0 / 2369	-18.5	-18.5	0.39 (1)	10.00				
W- X	0 / 2369	-18.5	-18.5	0.39 (1)	10.00				
X- O	0 / 2369	-18.5	-18.5	0.39 (1)	10.00				
O- Y	0 / 2670	-18.5	-18.5	0.41 (1)	10.00				
Y- Z	0 / 2670	-18.5	-18.5	0.41 (1)	10.00				
Z- N	0 / 2670	-18.5	-18.5	0.41 (1)	10.00				
N-AA	0 / 3611	-18.5	-18.5	0.55 (1)	10.00				
AA- M	0 / 3611	-18.5	-18.5	0.55 (1)	10.00				
M- L	0 / 3611	-18.5	-18.5	0.55 (1)	10.00				
L-AB	0 / 2557	-18.5	-18.5	0.39 (1)	10.00				
AB-AC	0 / 2557	-18.5	-18.5	0.39 (1)	10.00				
AC- K	0 / 2557	-18.5	-18.5	0.39 (1)	10.00				
K-AD	0 / 2108	-18.5	-18.5	0.34 (1)	10.00				
AD-AE	0 / 2108	-18.5	-18.5	0.34 (1)	10.00				
AE- J	0 / 2108	-18.5	-18.5	0.34 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-9-8	-327	-327	---	FRONT	VERT	TOTAL	---	C1
G	19-10-8	-327	-327	---	FRONT	VERT	TOTAL	---	C1
K	19-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.88/1.00 (F-G:1) , BC=0.55/1.00 (L-N:1) , WB=0.65/1.00 (C-P:1) , SSI=0.45/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (G) (INPUT = 0.90)
JSI METAL= 0.77 (C) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

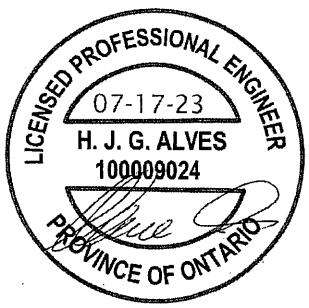
CONTINUED ON PAGE 2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	13-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
O	5-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Q	7-10-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
R	9-10-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
S	11-10-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
T	13-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
U	15-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
V	17-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
W	1-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
X	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Y	7-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Z	9-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AA	11-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AB	15-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AC	17-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AD	21-8-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AE	23-8-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2311444

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T2A	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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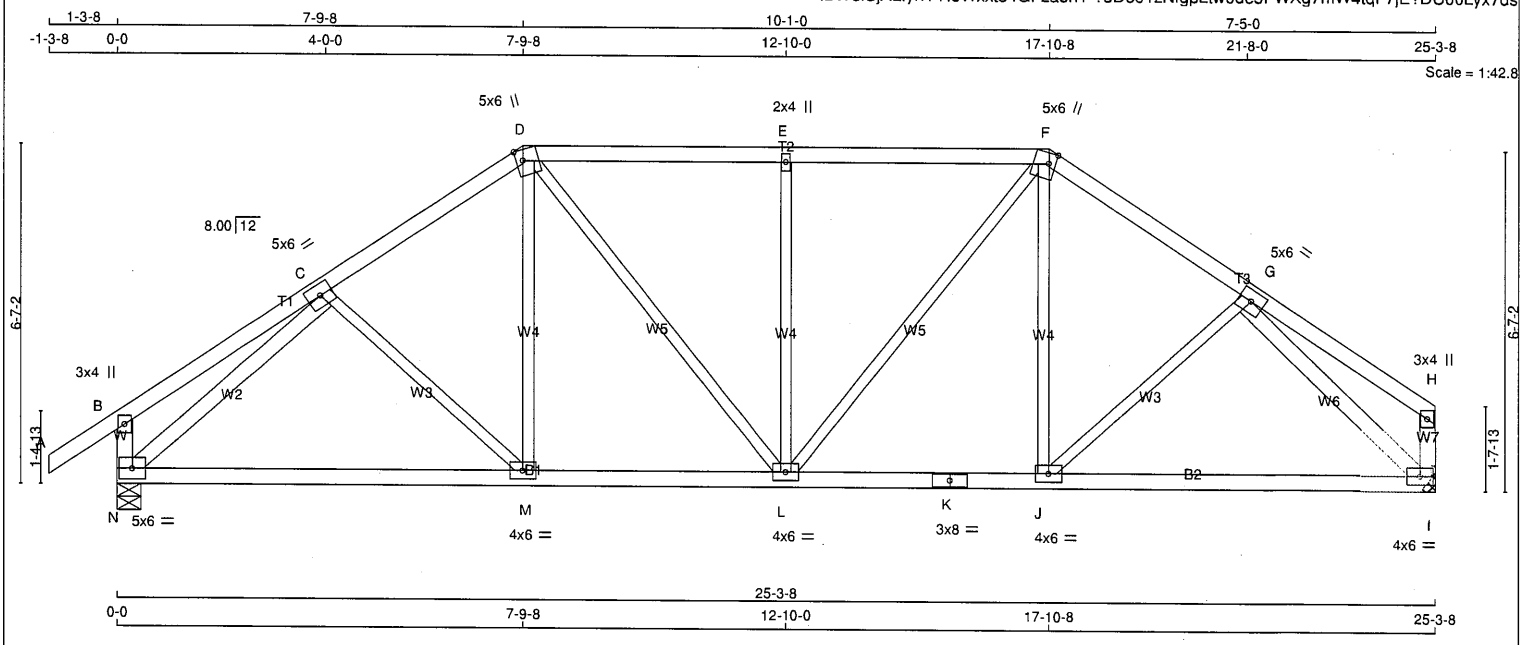

TOTAL WEIGHT = 112 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1520	0	1520	0	0	5-8	1-10
I	1394	0	1394	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1073	717 / 0	0 / 0	0 / 0	0 / 0	355 / 0	0 / 0
I	986	647 / 0	0 / 0	0 / 0	0 / 0	339 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C- M	-78 / 27	0.04 (1)
B- C	0 / 22	-91.8	-91.8	0.22 (1)	10.00	M- D	0 / 205	0.05 (4)
C- D	-1478 / 0	-91.8	-91.8	0.19 (1)	5.21	D- L	0 / 367	0.08 (1)
D- E	-1442 / 0	-91.8	-91.8	0.32 (1)	5.09	L- E	-564 / 0	0.40 (1)
E- F	-1442 / 0	-91.8	-91.8	0.32 (1)	5.09	L- F	0 / 422	0.09 (1)
F- G	-1436 / 0	-91.8	-91.8	0.18 (1)	5.28	J- F	0 / 136	0.04 (4)
G- H	0 / 23	-91.8	-91.8	0.20 (1)	10.00	J- G	0 / 55	0.02 (4)
N- B	-264 / 0	0.0	0.0	0.03 (1)	7.81	N- C	-1742 / 0	0.58 (1)
I- H	-121 / 0	0.0	0.0	0.01 (1)	7.81	G- I	-1684 / 0	0.50 (1)
N- M	0 / 1266	-18.5	-18.5	0.35 (4)	10.00			
M- L	0 / 1213	-18.5	-18.5	0.36 (4)	10.00			
L- K	0 / 1178	-18.5	-18.5	0.33 (4)	10.00			
K- J	0 / 1178	-18.5	-18.5	0.33 (4)	10.00			
J- I	0 / 1164	-18.5	-18.5	0.32 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.36/1.00 (L-M:4) , WB=0.58/1.00 (C-N:1) , SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (I) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 0.95)

Structural component only
DWG# T-2311445

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T3A	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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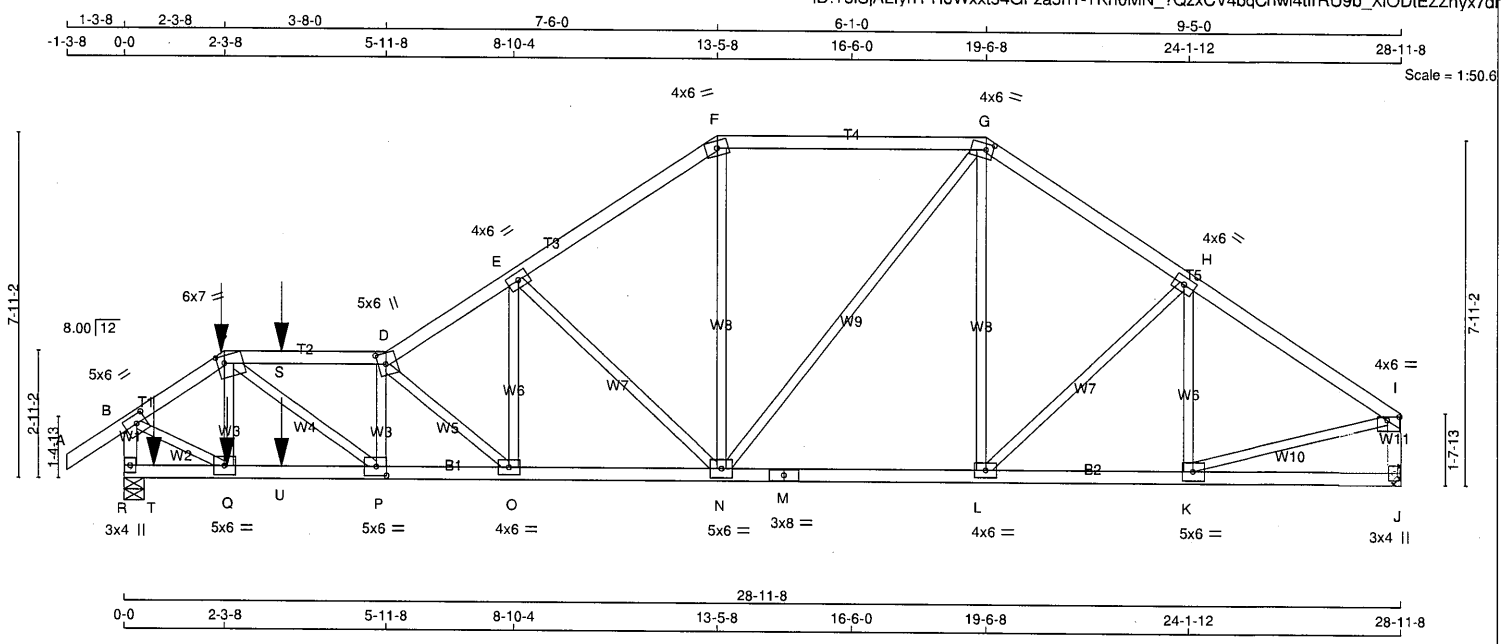

TOTAL WEIGHT = 128 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	2.75
C	TTWW-m	MT20	6.0	7.0	2.00	2.00
D	TTWW+m	MT20	5.0	6.0	3.00	2.00
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	4.0	6.0	1.75	2.00
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-p	MT20	4.0	6.0	1.00	3.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311446

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1811	0	1811	0	0	5-8	2-6
J	1604	0	1604	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-12.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1279	851 / 0	0 / 0	0 / 0	0 / 0	428 / 0	0 / 0
J	1135	745 / 0	0 / 0	0 / 0	0 / 0	390 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	Q- C	-537 / 0	0.10 (1)
B- C	-1681 / 0	-91.8	-91.8	0.13 (1)	4.98	C- P	0 / 1890	0.47 (1)
C- S	-2895 / 0	-91.8	-91.8	0.38 (1)	3.69	P- D	-1092 / 0	0.21 (1)
S- D	-2895 / 0	-91.8	-91.8	0.38 (1)	3.69	D- O	-946 / 0	0.26 (1)
D- E	-2650 / 0	-91.8	-91.8	0.37 (1)	3.84	O- E	0 / 709	0.18 (1)
E- F	-1853 / 0	-91.8	-91.8	0.35 (1)	4.55	E- N	-994 / 0	0.78 (1)
F- G	-1530 / 0	-91.8	-91.8	0.76 (1)	4.13	N- F	0 / 567	0.14 (1)
G- H	-1647 / 0	-91.8	-91.8	0.45 (1)	4.61	L- G	0 / 230	0.06 (1)
H- I	-1750 / 0	-91.8	-91.8	0.46 (1)	4.49	L- H	-187 / 0	0.15 (1)
R- B	-1799 / 0	0.0	0.0	0.20 (1)	6.19	K- H	-318 / 0	0.12 (1)
J- I	-1566 / 0	0.0	0.0	0.18 (1)	6.55	B- Q	0 / 1530	0.38 (1)
						K- I	0 / 1534	0.38 (1)
R- T	0 / 0	-18.5	-18.5	0.06 (4)	10.00	N- G	0 / 296	0.07 (1)
T- Q	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
Q- U	0 / 1373	-18.5	-18.5	0.30 (1)	10.00			
U- P	0 / 1373	-18.5	-18.5	0.30 (1)	10.00			
P- O	0 / 2939	-18.5	-18.5	0.58 (1)	10.00			
O- N	0 / 2237	-18.5	-18.5	0.47 (1)	10.00			
N- M	0 / 1347	-18.5	-18.5	0.32 (1)	10.00			
M- L	0 / 1347	-18.5	-18.5	0.32 (1)	10.00			
L- K	0 / 1479	-18.5	-18.5	0.32 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-3-8	-7	-7	---	FRONT	VERT	DEAD	---	C1
C	2-3-8	-15	-15	---	FRONT	VERT	TOTAL	---	C1
C	2-3-8	-29	-29	---	FRONT	VERT	SNOW	---	C1
Q	2-4-4	-2	-2	---	FRONT	VERT	TOTAL	---	C1
S	3-7-4	-9	-9	---	FRONT	VERT	TOTAL	---	C1
T	8-4	-4	-4	---	FRONT	VERT	TOTAL	---	C1
U	3-7-4	-2	-2	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.76/1.00 (F-G:1) , BC=0.58/1.00 (O-P:1) , WB=0.78/1.00 (E-N:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (D) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T3A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2311446

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T4A	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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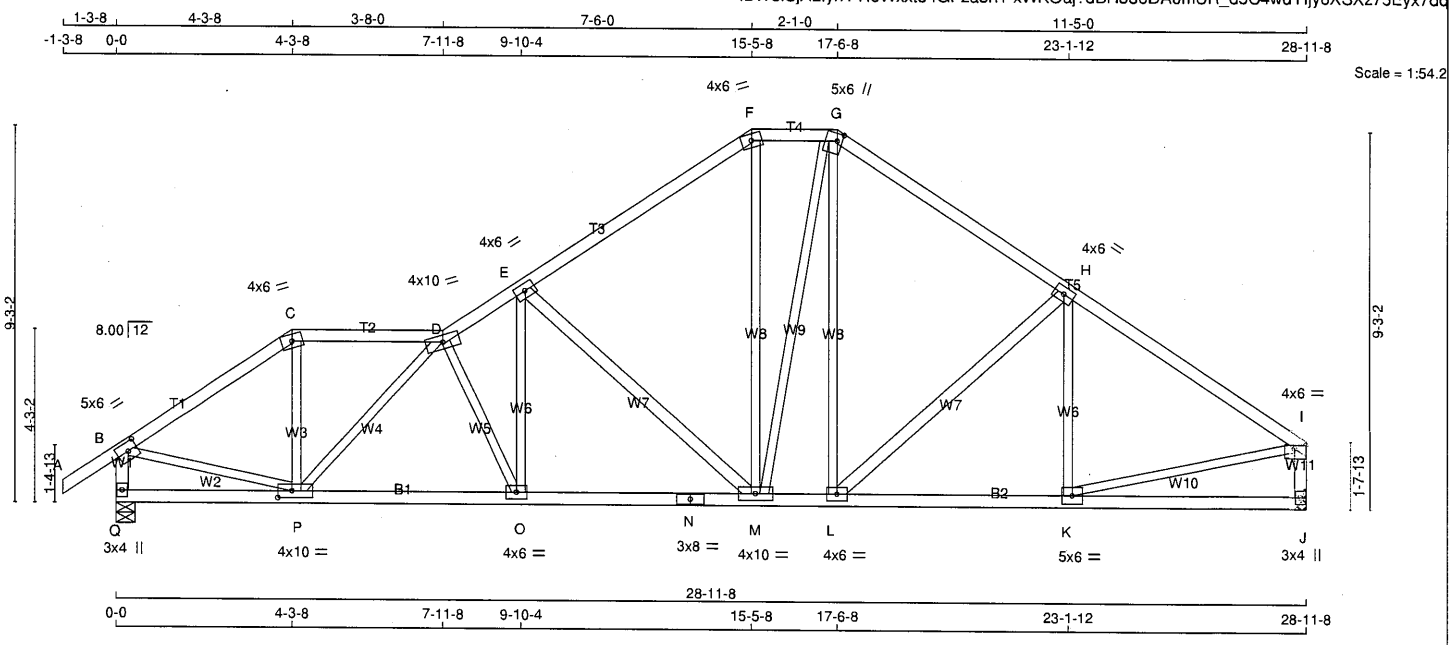

TOTAL WEIGHT = 141 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TTW-m	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	10.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-p	MT20	4.0	6.0	1.00	3.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMWWW-t	MT20	4.0	10.0		
N	BS-t	MT20	3.0	8.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWWW-t	MT20	4.0	10.0	2.00	4.00
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311447

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1722	0	1722	0	0	5-8	2-2
J	1596	0	1596	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-12.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1216	811 / 0	0 / 0	0 / 0	0 / 0	405 / 0	0 / 0
J	1129	741 / 0	0 / 0	0 / 0	0 / 0	388 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1803 / 0	-91.8	-91.8	0.26 (1)	4.74
C- D	-1526 / 0	-91.8	-91.8	0.18 (1)	5.15
D- E	-2378 / 0	-91.8	-91.8	0.34 (1)	4.11
E- F	-1568 / 0	-91.8	-91.8	0.38 (1)	4.86
F- G	-1287 / 0	-91.8	-91.8	0.07 (1)	5.63
G- H	-1519 / 0	-91.8	-91.8	0.43 (1)	4.85
H- I	-1774 / 0	-91.8	-91.8	0.45 (1)	4.53
Q- B	-1688 / 0	0.0	0.0	0.17 (1)	6.39
J- I	-1552 / 0	0.0	0.0	0.16 (1)	6.61
Q- P	0 / 0	-18.5	-18.5	0.09 (4)	10.00
P- O	0 / 2322	-18.5	-18.5	0.42 (1)	10.00
O- N	0 / 2052	-18.5	-18.5	0.39 (1)	10.00
N- M	0 / 2052	-18.5	-18.5	0.39 (1)	10.00
M- L	0 / 1236	-18.5	-18.5	0.26 (1)	10.00
L- K	0 / 1505	-18.5	-18.5	0.32 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	0 / 639	0.14 (1)
M- F	0 / 553	0.12 (1)
M- G	0 / 238	0.05 (1)
L- G	0 / 304	0.07 (1)
L- H	-369 / 0	0.32 (1)
K- H	-223 / 23	0.10 (1)
B- P	0 / 1541	0.35 (1)
K- I	0 / 1544	0.35 (1)
E- M	-1044 / 0	0.91 (1)
O- E	0 / 661	0.15 (1)
D- O	-623 / 0	0.20 (1)
P- D	-1202 / 0	0.58 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.45/1.00 (H-I:1) , BC=0.42/1.00 (O-P:1) , WB=0.91/1.00 (E-M:1) , SSI=0.21/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.90)
JSI METAL= 0.63 (N) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T5	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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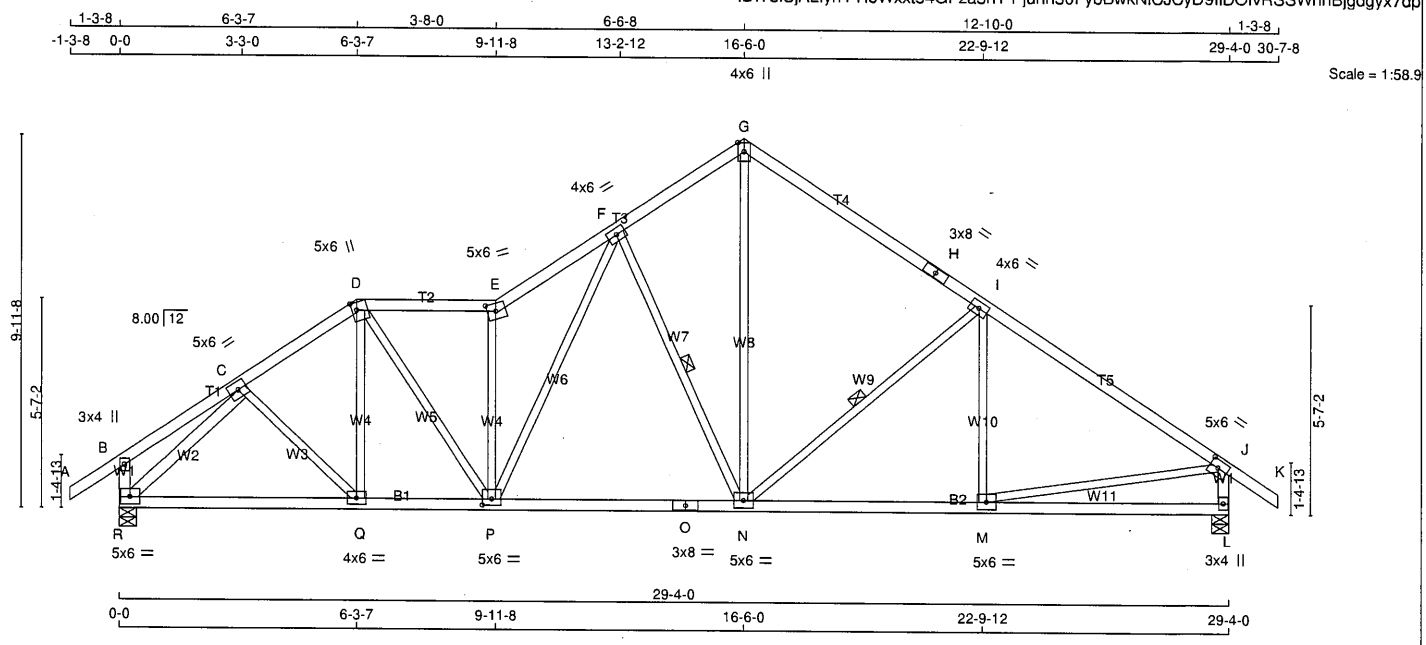

TOTAL WEIGHT = 3 X 136 = 409 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TTW-m	MT20	5.0	6.0	2.50	2.75
F	TMWW-t	MT20	4.0	6.0		
G	TTW+p	MT20	4.0	6.0	Edge	
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	5.0	6.0	2.00	3.00
Q	BMWW-t	MT20	4.0	6.0		
R	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1743	0	1743	0	0	5-8	IN-SX 1-14
L	1743	0	1743	0	0	5-8	IN-SX 2-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1230	821 / 0	0 / 0	0 / 0	0 / 0	410 / 0	0 / 0
L	1230	821 / 0	0 / 0	0 / 0	0 / 0	410 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 16	-91.8	-91.8	0.13 (1)	10.00
C- D	-1850 / 0	-91.8	-91.8	0.14 (1)	4.83
D- E	-2000 / 0	-91.8	-91.8	0.20 (1)	4.61
E- F	-2417 / 0	-91.8	-91.8	0.19 (1)	4.27
F- G	-1485 / 0	-91.8	-91.8	0.15 (1)	5.24
G- H	-1506 / 0	-91.8	-91.8	0.54 (1)	4.69
H- I	-1506 / 0	-91.8	-91.8	0.54 (1)	4.69
I- J	-1892 / 0	-91.8	-91.8	0.60 (1)	4.23
J- K	0 / 35	-91.8	-91.8	0.12 (1)	10.00
R- B	-240 / 0	0.0	0.0	0.02 (1)	7.81
L- J	-1692 / 0	0.0	0.0	0.17 (1)	6.39
R- Q	0 / 1437	-18.5	-18.5	0.32 (1)	10.00
Q- P	0 / 1524	-18.5	-18.5	0.33 (1)	10.00
P- O	0 / 1591	-18.5	-18.5	0.36 (1)	10.00
O- N	0 / 1591	-18.5	-18.5	0.36 (1)	10.00
N- M	0 / 1606	-18.5	-18.5	0.36 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	0 / 120	0.03 (4)
Q- D	0 / 64	0.02 (4)
D- P	0 / 853	0.19 (1)
P- E	-1624 / 0	0.77 (1)
P- F	0 / 1080	0.24 (1)
F- N	-906 / 0	0.41 (1)
N- G	0 / 1291	0.29 (1)
N- I	-510 / 0	0.24 (1)
M- I	-140 / 56	0.07 (1)
R- C	-2043 / 0	0.46 (1)
M- J	0 / 1628	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.60/1.00 (I-J:1) , BC=0.36/1.00 (M-N:1) , WB=0.77/1.00 (E-P:1) , SSI=0.24/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (N) (INPUT = 0.90)
JSI METAL= 0.51 (E) (INPUT = 0.95)

Structural component only
DWG# T-2311448

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T6	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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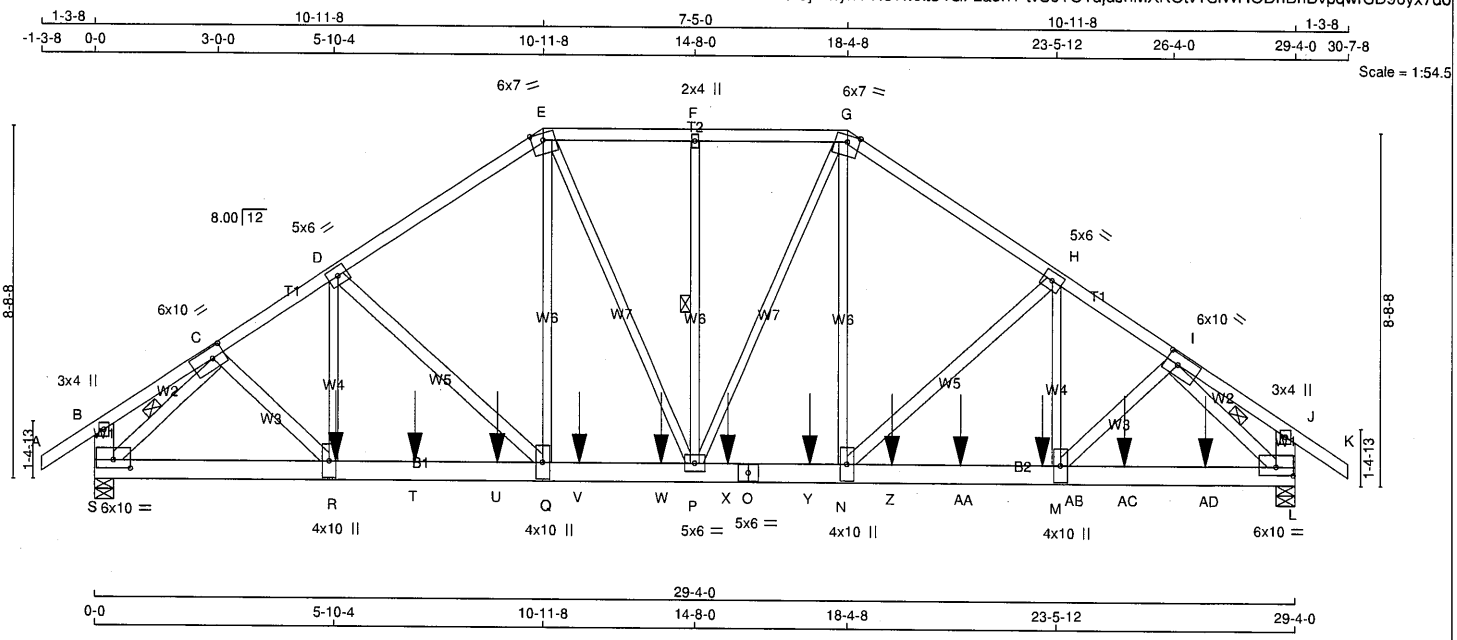

TOTAL WEIGHT = 3 X 171 = 514 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
O - L	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
C - R	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- G	1	12	TOP
G- K	1	12	TOP
S- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- O	2	8	SIDE(590.6)
O- L	2	8	SIDE(910.0)
WEBS : (0.122"X3") SPIRAL NAILS			
D- R	1	4	SIDE(670.8)
2x3	1	4	
Q- E	1	6	
P- E	1	6	
F- P	1	6	
P- G	1	6	
N- G	1	6	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

Structural component only
DWG# T-2311449

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	13950	0	13950	0	0	5-8	5-1
S	12357	0	12357	0	0	5-8	4-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	9843	6587 / 0	0 / 0	0 / 0	0 / 0	3256 / 0	0 / 0
S	8723	5815 / 0	0 / 0	0 / 0	0 / 0	2908 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P, C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.04 (1)	10.00	
B- C	-22 / 0	-91.8	-91.8	0.07 (1)	6.25	
C- D	-17175 / 0	-91.8	-91.8	0.46 (1)	2.64	
D- E	-14791 / 0	-91.8	-91.8	0.59 (1)	2.92	
E- F	-13119 / 0	-91.8	-91.8	0.28 (1)	3.21	
F- G	-13119 / 0	-91.8	-91.8	0.28 (1)	3.21	
G- H	-14811 / 0	-91.8	-91.8	0.60 (1)	2.91	
H- I	-17646 / 0	-91.8	-91.8	0.48 (1)	2.58	
I- J	-23 / 0	-91.8	-91.8	0.07 (1)	6.25	
J- K	0 / 35	-91.8	-91.8	0.04 (1)	10.00	
S- B	-297 / 0	0.0	0.0	0.01 (1)	7.81	
L- J	-300 / 0	0.0	0.0	0.01 (1)	7.81	
S- R	0 / 11904	-18.5	-18.5	0.23 (1)	10.00	
R- T	0 / 14333	-18.5	-18.5	0.47 (1)	10.00	
T- U	0 / 14333	-18.5	-18.5	0.47 (1)	10.00	
U- Q	0 / 14333	-18.5	-18.5	0.47 (1)	10.00	
Q- V	0 / 12352	-18.5	-18.5	0.40 (1)	10.00	
V- W	0 / 12352	-18.5	-18.5	0.40 (1)	10.00	
W- P	0 / 12352	-18.5	-18.5	0.40 (1)	10.00	
P- X	0 / 12370	-18.5	-18.5	0.37 (1)	10.00	
X- O	0 / 12370	-18.5	-18.5	0.37 (1)	10.00	
O- Y	0 / 12370	-18.5	-18.5	0.37 (1)	10.00	
Y- N	0 / 12370	-18.5	-18.5	0.37 (1)	10.00	
N- Z	0 / 14726	-18.5	-18.5	0.55 (1)	10.00	
Z- AA	0 / 14726	-18.5	-18.5	0.55 (1)	10.00	
AA- AB	0 / 14726	-18.5	-18.5	0.55 (1)	10.00	
AB- M	0 / 14726	-18.5	-18.5	0.55 (1)	10.00	
M- AC	0 / 12226	-18.5	-18.5	0.52 (1)	10.00	
AC- AD	0 / 12226	-18.5	-18.5	0.52 (1)	10.00	
AD- L	0 / 12226	-18.5	-18.5	0.52 (1)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- D	0 / 2778	0.21 (1)
D- Q	-2824 / 0	0.65 (1)
Q- E	0 / 6096	0.46 (1)
E- P	0 / 1883	0.14 (1)
P- F	-279 / 0	0.04 (1)
P- G	0 / 1839	0.14 (1)
N- G	0 / 6148	0.46 (1)
N- H	-3340 / 0	0.76 (1)
M- H	0 / 3379	0.25 (1)
C- R	0 / 3395	0.18 (1)
S- C	-16949 / 0	0.68 (1)
M- I	0 / 3495	0.19 (1)
I- L	-17408 / 0	0.69 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	5-11-0	-2699	-2699	---	BACK	VERT	TOTAL	---	C1
T	7-10-4	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
U	9-10-4	-1187	-1187	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.60/1.00 (G-H:1) , BC=0.55/1.00 (M-N:1) , WB=0.76/1.00 (H-N:1) , SSI=0.67/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.84 (I) (INPUT = 0.95)

CONTINUED ON PAGE 2

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	6.0	10.0	3.00	3.75
D	TMWW-t	MT20	5.0	6.0		
E	TTWW-m	MT20	6.0	7.0	Edge	
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	6.0	7.0	Edge	
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	6.0	10.0	3.00	3.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	6.0	10.0	2.50	5.00
M, N, Q, R						
M	BMWW+t	MT20	4.0	10.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
S	BMVW1-t	MT20	6.0	10.0	2.50	5.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	11-10-4	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
W	13-10-4	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
X	15-5-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
Y	17-5-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
Z	19-5-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
AA	21-1-12	-1274	-1274	---	BACK	VERT	TOTAL	---	C1
AB	23-1-12	-1274	-1274	---	BACK	VERT	TOTAL	---	C1
AC	25-1-12	-1274	-1274	---	BACK	VERT	TOTAL	---	C1
AD	27-1-12	-1274	-1274	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311449

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T7	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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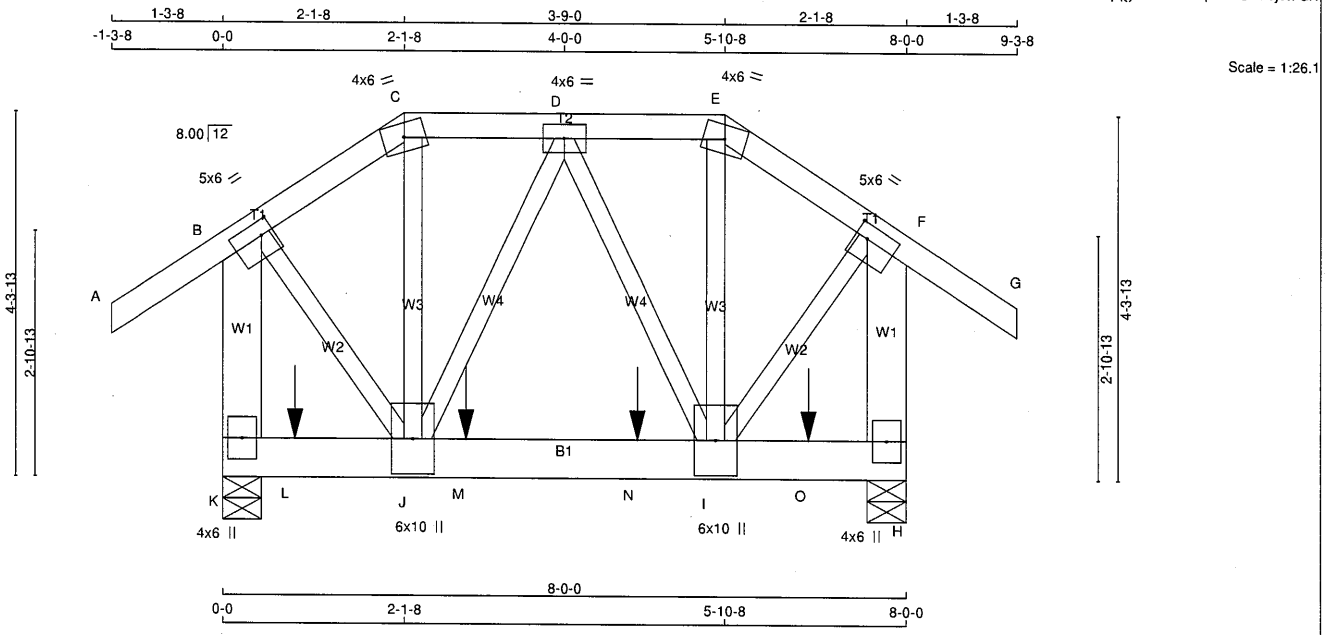

TOTAL WEIGHT = 2 X 54 = 108 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
K- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(10.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2311450

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	4700	0	4700	0	0	5-8	2-9
H	3725	0	3725	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	3326	2168 / 0	0 / 0	0 / 0	0 / 0	1159 / 0	0 / 0
H	2635	1721 / 0	0 / 0	0 / 0	0 / 0	914 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00		J- C	0 / 1005	0.12 (1)
B- C	-2154 / 0	-91.8	-91.8	0.05 (1)	6.01		I- E	0 / 938	0.12 (1)
C- D	-1813 / 0	-91.8	-91.8	0.04 (1)	6.25		B- J	0 / 2682	0.33 (1)
D- E	-1710 / 0	-91.8	-91.8	0.04 (1)	6.25		I- F	0 / 2530	0.31 (1)
E- F	-2032 / 0	-91.8	-91.8	0.05 (1)	6.14		J- D	0 / 47	0.01 (4)
F- G	0 / 35	-91.8	-91.8	0.07 (1)	10.00		D- I	-204 / 0	0.03 (1)
K- B	-3427 / 0	0.0	0.0	0.18 (1)	7.54				
H- F	-3246 / 0	0.0	0.0	0.17 (1)	7.69				
K- L	0 / 0	-37.3	-37.3	0.39 (1)	10.00				
L- J	0 / 0	-37.3	-37.3	0.39 (1)	10.00				
J- M	0 / 1793	-37.3	-37.3	0.42 (1)	10.00				
M- N	0 / 1793	-37.3	-37.3	0.42 (1)	10.00				
N- I	0 / 1793	-37.3	-37.3	0.42 (1)	10.00				
I- O	0 / 0	-37.3	-37.3	0.27 (1)	10.00				
O- H	0 / 0	-37.3	-37.3	0.27 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	10-4	-1839	-1839	---	BACK	VERT	TOTAL	---	C1
M	2-10-4	-971	-971	---	BACK	VERT	TOTAL	---	C1
N	4-10-4	-1120	-1120	---	BACK	VERT	TOTAL	---	C1
O	6-10-4	-1114	-1114	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.18/1.00 (B-K:1) , BC=0.42/1.00 (I-J:1) , WB=0.33/1.00 (B-J:1) , SSI=0.54/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (I) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T7	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTIoWxxt54GFza3n1-L50XCk1WUCRezhvbRc?hEjqgW5ZzvRqz8VCniZyx7dn

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	1.75
C	TTW-m	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TTW-m	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0	2.00	1.75
H	BMV1+p	MT20	4.0	6.0		
I	BMWWW+t	MT20	6.0	10.0		
J	BMWWW+t	MT20	6.0	10.0		
K	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311450

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T8	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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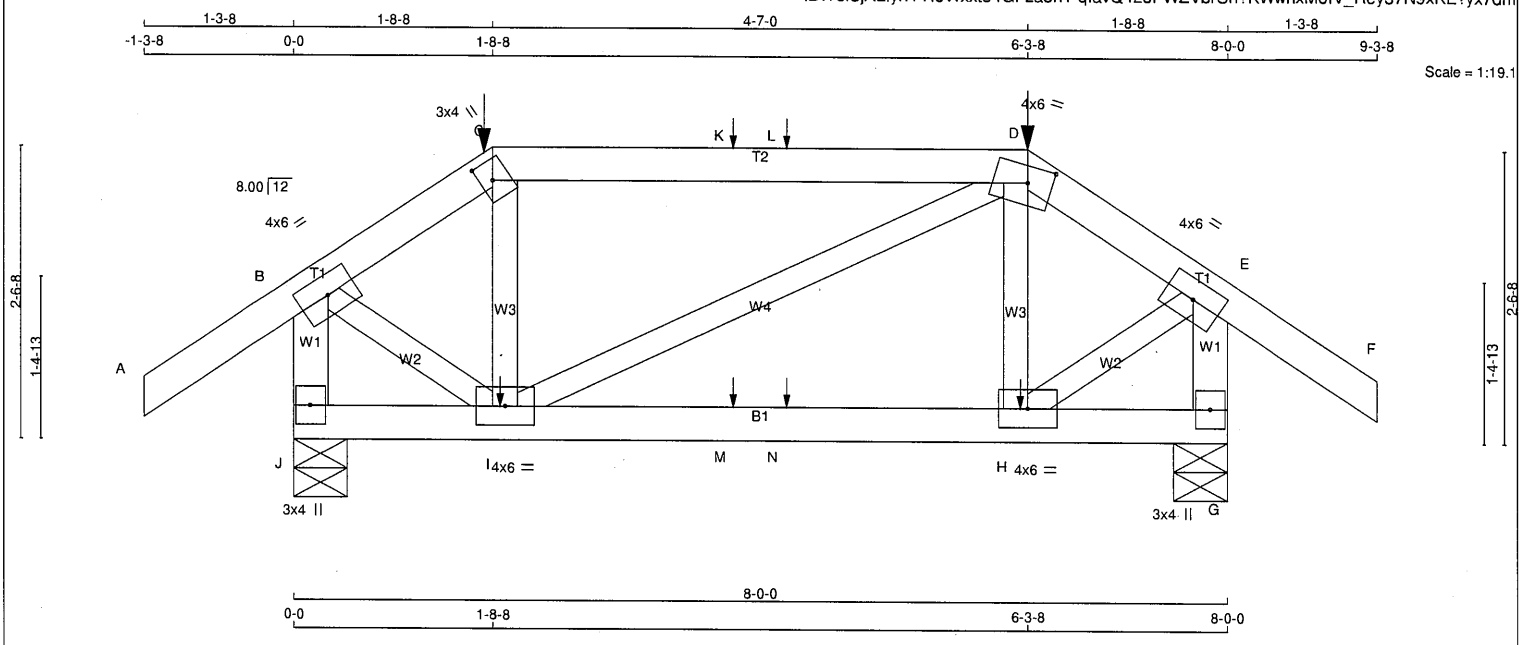
ID:T3iSjALIyhYTIoWxxt54GFza3n1-qIavQ428FWZVbrUn?KWwnxMoIV_Rey37N9xKE?yx7dm



TOTAL WEIGHT = 35 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REORD BRG IN-SX
J	596	0	596	0	0	5-8	1-8
G	596	0	596	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	419	291 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
G	419	291 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14	(1)	10.00	I- C	-89 / 24	0.02 (1)
B- C	-347 / 0	-91.8	-91.8	0.13	(1)	6.25	I- D	0 / 0	0.00 (1)
C- K	-260 / 0	-91.8	-91.8	0.37	(1)	6.25	H- D	-89 / 24	0.02 (1)
K- L	-260 / 0	-91.8	-91.8	0.37	(1)	6.25	B- I	0 / 307	0.08 (1)
L- D	-260 / 0	-91.8	-91.8	0.37	(1)	6.25	H- E	0 / 308	0.08 (1)
D- E	-347 / 0	-91.8	-91.8	0.13	(1)	6.25			
E- F	0 / 35	-91.8	-91.8	0.14	(1)	10.00			
J- B	-590 / 0	0.0	0.0	0.07	(1)	7.81			
G- E	-591 / 0	0.0	0.0	0.07	(1)	7.81			
J- I	0 / 0	-18.5	-18.5	0.06	(4)	10.00			
I- M	0 / 260	-18.5	-18.5	0.09	(4)	10.00			
M- N	0 / 260	-18.5	-18.5	0.09	(4)	10.00			
N- H	0 / 260	-18.5	-18.5	0.09	(4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.06	(4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-8-8	-4	-4	---	FRONT	VERT	DEAD	---	C1
C	1-8-8	1	1	79	FRONT	VERT	TOTAL	---	C1
C	1-8-8	-16	-16	---	FRONT	VERT	SNOW	---	C1
D	6-3-8	-4	-4	---	FRONT	VERT	DEAD	---	C1
D	6-3-8	1	1	79	FRONT	VERT	TOTAL	---	C1
D	6-3-8	-16	-16	---	FRONT	VERT	SNOW	---	C1
H	6-2-12	1	1	---	FRONT	VERT	TOTAL	---	C1
I	1-9-4	1	1	---	FRONT	VERT	TOTAL	---	C1
K	3-9-4	1	1	79	FRONT	VERT	TOTAL	---	C1
L	4-2-12	1	1	79	FRONT	VERT	TOTAL	---	C1
M	3-9-4	1	1	---	FRONT	VERT	TOTAL	---	C1
N	4-2-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.09/1.00 (H-I:4) , WB=0.08/1.00 (E-H:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	374	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311451

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:T3iSjALIyhYTIoWxxt54GFza3n1-qIavQ428FWZVbrUn?KWwnxMoIV_Rey37N9xKE?yx7dm

JSI GRIP= 0.48 (C) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311451

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T9	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:30 2023 Page 1
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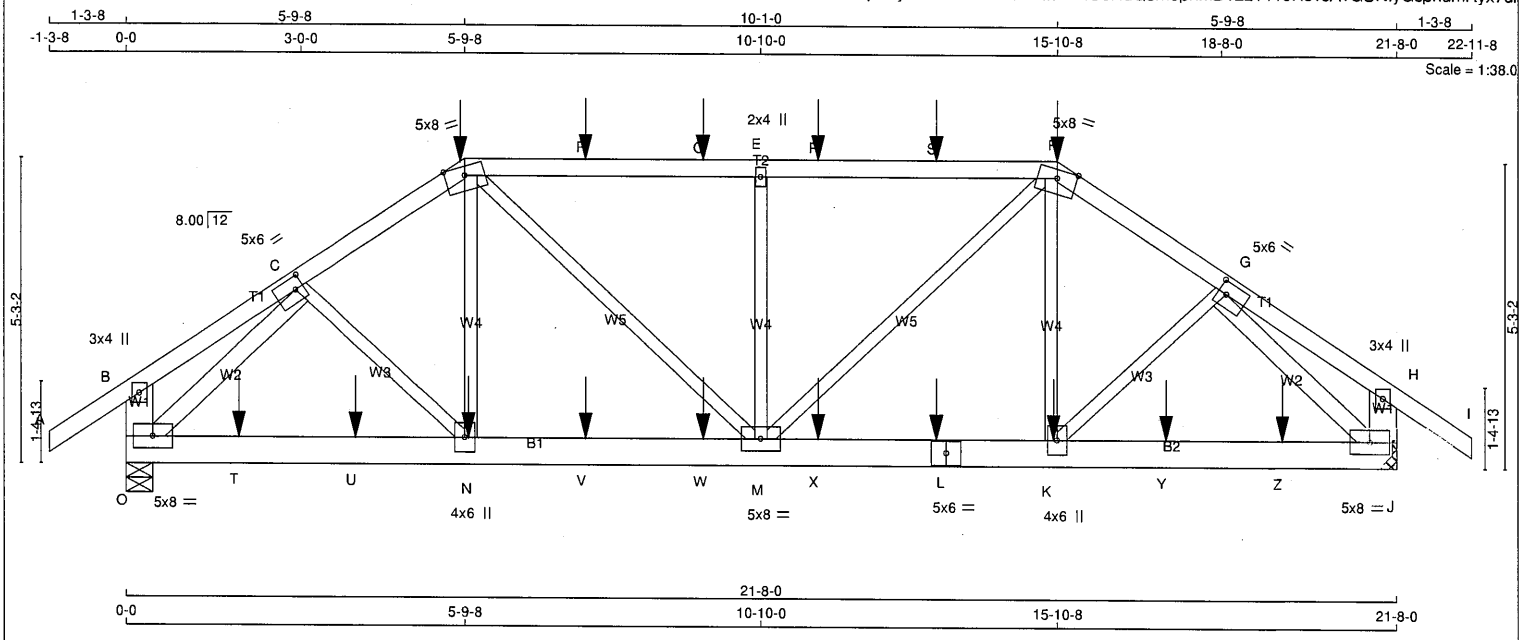

TOTAL WEIGHT = 109 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMWW-t	MT20	5.0	6.0	2.50	1.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	8.0		
K	BMWW+t	MT20	4.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW+t	MT20	4.0	6.0		
O	BMVW1-t	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311452

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2317	0	2317	0	0	5-8	2-8
J	2317	0	2317	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1638	1079 / 0	0 / 0	0 / 0	0 / 0	559 / 0	0 / 0
J	1638	1079 / 0	0 / 0	0 / 0	0 / 0	559 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	
B- C	0 / 13	-91.8	-91.8	0.10 (1)	10.00	
C- D	-2669 / 0	-91.8	-91.8	0.21 (1)	4.04	
D- P	-2830 / 0	-91.8	-91.8	0.81 (1)	3.45	
P- Q	-2830 / 0	-91.8	-91.8	0.81 (1)	3.45	
Q- E	-2830 / 0	-91.8	-91.8	0.81 (1)	3.45	
E- R	-2830 / 0	-91.8	-91.8	0.81 (1)	3.45	
R- S	-2830 / 0	-91.8	-91.8	0.81 (1)	3.45	
S- F	-2830 / 0	-91.8	-91.8	0.81 (1)	3.45	
F- G	-2670 / 0	-91.8	-91.8	0.21 (1)	4.04	
G- H	0 / 13	-91.8	-91.8	0.10 (1)	10.00	
H- I	0 / 35	-91.8	-91.8	0.14 (1)	10.00	
O- B	-235 / 0	0.0	0.0	0.02 (1)	7.81	
J- H	-235 / 0	0.0	0.0	0.02 (1)	7.81	
O- T	0 / 1984	-18.5	-18.5	0.34 (1)	10.00	
T- U	0 / 1984	-18.5	-18.5	0.34 (1)	10.00	
U- N	0 / 1984	-18.5	-18.5	0.34 (1)	10.00	
N- V	0 / 2206	-18.5	-18.5	0.36 (1)	10.00	
V- W	0 / 2206	-18.5	-18.5	0.36 (1)	10.00	
W- M	0 / 2206	-18.5	-18.5	0.36 (1)	10.00	
M- X	0 / 2206	-18.5	-18.5	0.36 (1)	10.00	
X- L	0 / 2206	-18.5	-18.5	0.36 (1)	10.00	
L- K	0 / 2206	-18.5	-18.5	0.36 (1)	10.00	
K- Y	0 / 1985	-18.5	-18.5	0.34 (1)	10.00	
Y- Z	0 / 1985	-18.5	-18.5	0.34 (1)	10.00	
Z- J	0 / 1985	-18.5	-18.5	0.34 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 307	0.08 (1)
N- D	0 / 160	0.06 (4)
K- F	0 / 161	0.06 (4)
K- G	0 / 308	0.08 (1)
O- C	-2823 / 0	0.55 (1)
G- J	-2824 / 0	0.55 (1)
M- F	0 / 869	0.22 (1)
D- M	0 / 870	0.22 (1)
M- E	-1081 / 0	0.43 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-9-8	-327	-327	---	FRONT	VERT	TOTAL	---	C1
F	15-10-8	-327	-327	---	FRONT	VERT	TOTAL	---	C1
K	15-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
L	13-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
N	5-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
P	7-10-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
Q	9-10-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
R	11-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.81/1.00 (D-E:1) , BC=0.36/1.00 (M-N:1) , WB=0.55/1.00 (G-J:1) , SSI=0.47/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.64 (C) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T9	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTIoWxxt54GFza3n1-lU8HdQ3m0phMD?2zY119K8vsAvGUNlyGcphumRyx7dl

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	13-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
T	1-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
U	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
V	7-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
W	9-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
X	11-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Y	17-8-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Z	19-8-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311452

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T10	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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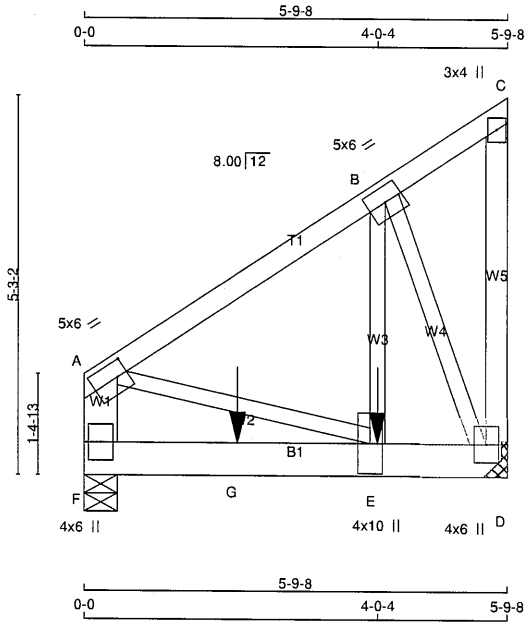

Scale = 1:30.6

TOTAL WEIGHT = 2 X 34 = 68 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	4	SIDE(416.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMWW-t	MT20	5.0	6.0		

Structural component only
DWG# T-2311453

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	2025	0	2025	0	0	MECHANICAL	
F	1215	0	1215	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1428	958 / 0	0 / 0	0 / 0	0 / 0	471 / 0	0 / 0
F	855	584 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1105 / 0	-91.8	-91.8	0.11 (1)	6.25	E- B	0 / 2202	0.27 (1)
B- C	-34 / 0	-91.8	-91.8	0.08 (1)	6.25	B- D	-2149 / 0	0.30 (1)
D- C	-19 / 0	0.0	0.0	0.00 (1)	7.81	A- E	0 / 960	0.12 (1)
F- A	-1010 / 0	0.0	0.0	0.04 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
E- D	0 / 932	-18.5	-18.5	0.10 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	4-0-4	-1623	-1623	---	BACK	VERT	TOTAL	---	C1
G	2-1-4	-208	-208	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT. CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.14/1.00 (E-F:1) , WB=0.30/1.00 (B-D:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL= 0.37 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T10	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTIoWxxt54GFza3n1-lU8HdQ3m0phMD?2zY119K8v13vJoNMlGcphumRyx7dl

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	10.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311453

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432488	T10Z	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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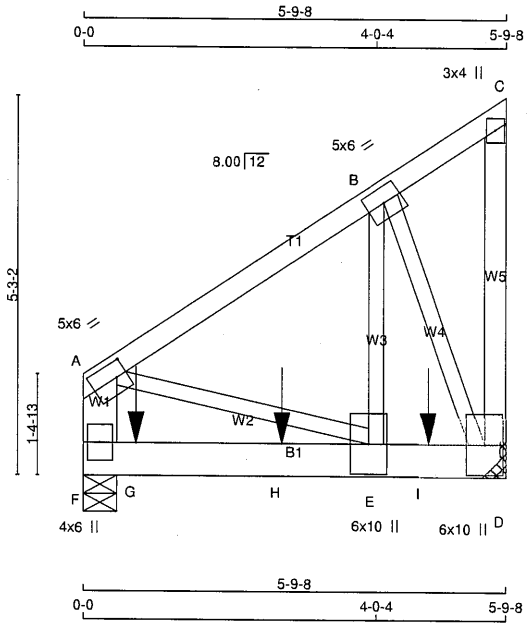

TOTAL WEIGHT = 2 X 34 = 68 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311468

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	2095	0	2095	0	0	MECHANICAL	
F	2314	0	2314	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1476	1001 / 0	0 / 0	0 / 0	0 / 0	475 / 0	0 / 0
F	1630	1105 / 0	0 / 0	0 / 0	0 / 0	525 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
A- B	-1151 / 0	-91.8	-91.8	0.11 (1)	6.25	E- B	0 / 2308	0.29 (1)
B- C	-34 / 0	-91.8	-91.8	0.08 (1)	6.25	B- D	-2237 / 0	0.32 (1)
D- C	-20 / 0	0.0	0.0	0.00 (1)	7.81	A- E	0 / 1000	0.12 (1)
F- A	-1045 / 0	0.0	0.0	0.04 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.34 (1)	10.00			
G- H	0 / 0	-18.5	-18.5	0.34 (1)	10.00			
H- E	0 / 0	-18.5	-18.5	0.34 (1)	10.00			
E- I	0 / 970	-18.5	-18.5	0.36 (1)	10.00			
I- D	0 / 970	-18.5	-18.5	0.36 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	8-12	-886	-886	---	FRONT	VERT	TOTAL	---	C1
H	2-8-12	-884	-884	---	FRONT	VERT	TOTAL	---	C1
I	4-8-12	-884	-884	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.36/1.00 (D-E:1) , WB=0.32/1.00 (B-D:1) , SSI=0.44/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
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Per: joshua.nabua

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432488	T10Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTloWxxt54GFza3n1-rkEP5NLIGL_9URTbWzkaCylmCR6_8WQ7tar_aYyx6pl

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	6.0	10.0		
E	BMWW+t	MT20	6.0	10.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311468

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T11	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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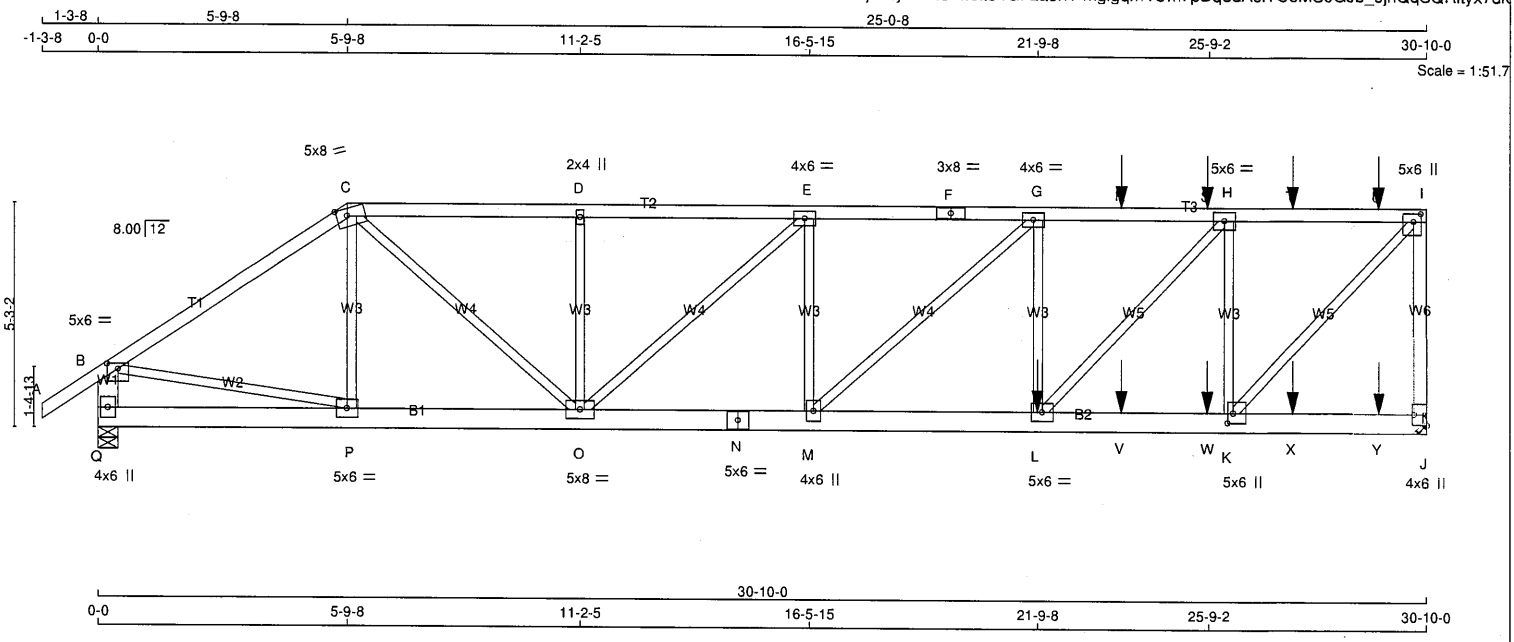

TOTAL WEIGHT = 2 X 150 = 300 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- I	1	12	SIDE(0.0)
I- J	1	12	TOP
Q- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	TOP
N- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
G- L	1	6	SIDE(340.4)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2311454

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	3843	0	3843	0	0	MECHANICAL	
Q	2525	0	2525	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-12.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	2714	1802 / 0	0 / 0	0 / 0	0 / 0	912 / 0	0 / 0
Q	1782	1192 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	P- C	-292 / 0	0.06 (1)
B- C	-2997 / 0	-91.8	-91.8	0.39 (1)	4.93	K- H	-3239 / 0	0.64 (1)
C- D	-4126 / 0	-91.8	-91.8	0.30 (1)	4.43	B- P	0 / 2526	0.31 (1)
D- E	-4126 / 0	-91.8	-91.8	0.30 (1)	4.43	L- G	-258 / 0	0.05 (1)
E- F	-5071 / 0	-91.8	-91.8	0.29 (1)	4.09	L- H	0 / 3114	0.39 (1)
F- G	-5071 / 0	-91.8	-91.8	0.29 (1)	4.09	K- I	0 / 4750	0.59 (1)
G- R	-5352 / 0	-91.8	-91.8	0.44 (1)	3.78	M- G	-382 / 0	0.19 (1)
R- S	-5352 / 0	-91.8	-91.8	0.44 (1)	3.78	C- O	0 / 2211	0.27 (1)
S- H	-5352 / 0	-91.8	-91.8	0.44 (1)	3.78	M- E	0 / 396	0.05 (1)
H- T	-3261 / 0	-91.8	-91.8	0.35 (1)	4.76	O- D	-539 / 0	0.11 (1)
T- U	-3261 / 0	-91.8	-91.8	0.35 (1)	4.76	O- E	-1285 / 0	0.63 (1)
U- I	-3261 / 0	-91.8	-91.8	0.35 (1)	4.76			
J- I	-3772 / 0	0.0	0.0	0.85 (1)	6.07			
Q- B	-2479 / 0	0.0	0.0	0.09 (1)	7.81			
Q- P	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
P- O	0 / 2484	-18.5	-18.5	0.19 (1)	10.00			
O- N	0 / 5071	-18.5	-18.5	0.36 (1)	10.00			
N- M	0 / 5071	-18.5	-18.5	0.36 (1)	10.00			
M- L	0 / 5352	-18.5	-18.5	0.40 (1)	10.00			
L- V	0 / 3261	-18.5	-18.5	0.27 (1)	10.00			
V- W	0 / 3261	-18.5	-18.5	0.27 (1)	10.00			
W- K	0 / 3261	-18.5	-18.5	0.27 (1)	10.00			
K- X	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
X- Y	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
Y- J	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	21-9-8	-1414	-1414	---	FRONT	VERT	TOTAL	---	C1
R	23-8-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
S	25-8-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
T	27-8-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
U	29-8-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
V	23-8-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
W	25-8-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
X	27-8-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Y	29-8-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.85/1.00 (I-J:1) , BC=0.40/1.00 (L-M:1) , WB=0.64/1.00 (H-K:1) , SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (I) (INPUT = 0.90)
JSI METAL= 0.57 (I) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T11	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	8.0	2.00	3.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMVW+p	MT20	5.0	6.0	2.25	2.00
J	BMV1+p	MT20	4.0	6.0	3.00	Edge
K	BMWW+t	MT20	5.0	6.0	2.75	1.50
L	BMWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311454

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T12	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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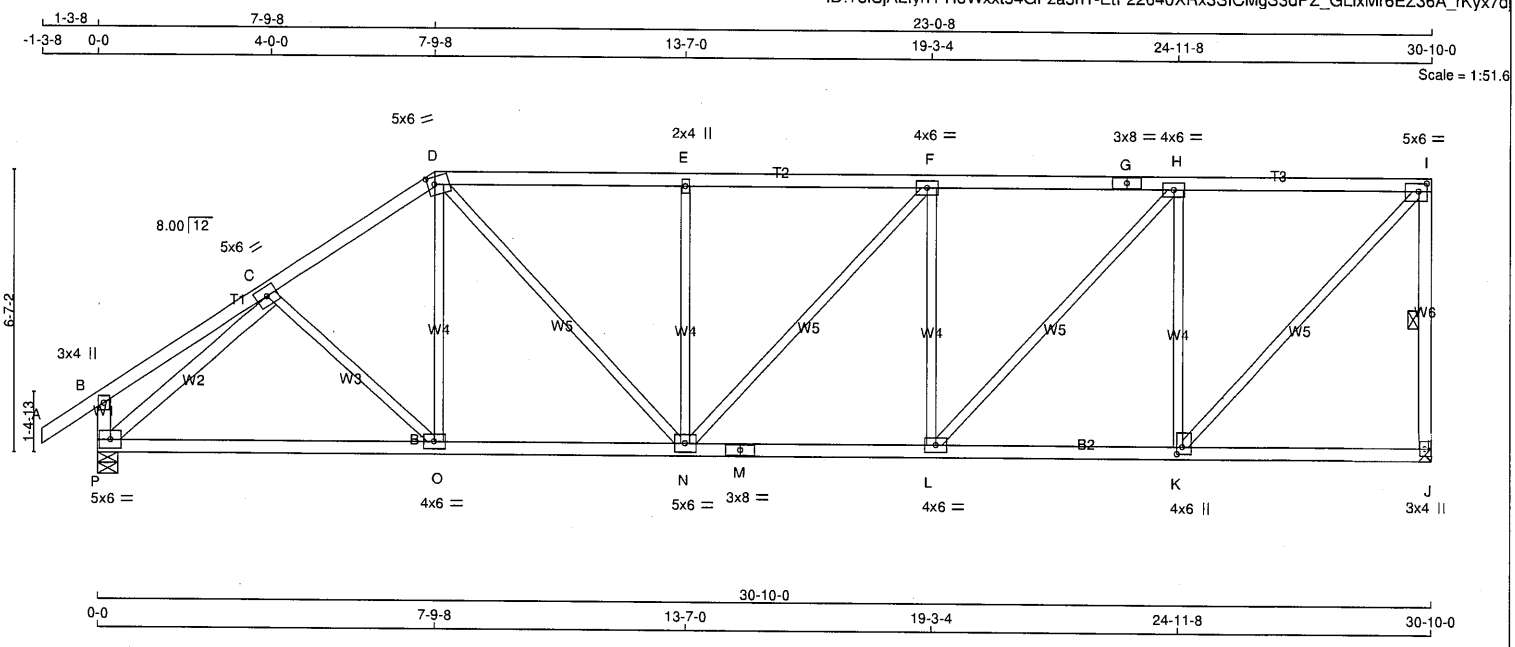

TOTAL WEIGHT = 138 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMV+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW+t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1700	0	1700	0	0	IN-SX	IN-SX
P	1826	0	1826	0	0	MECHANICAL 5-8	2-0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0
P	1289	859 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C- O	0 / 54	0.02 (4)
B- C	0 / 22	-91.8	-91.8	0.21 (1)	10.00	O- D	0 / 152	0.05 (4)
C- D	-1934 / 0	-91.8	-91.8	0.28 (1)	4.58	P- C	-2185 / 0	0.73 (1)
D- E	-2103 / 0	-91.8	-91.8	0.57 (1)	4.06	K- I	0 / 1969	0.44 (1)
E- F	-2103 / 0	-91.8	-91.8	0.57 (1)	4.06	D- N	0 / 756	0.17 (1)
F- G	-1987 / 0	-91.8	-91.8	0.57 (1)	4.14	K- H	-1321 / 0	0.94 (1)
G- H	-1987 / 0	-91.8	-91.8	0.57 (1)	4.14	N- E	-595 / 0	0.43 (1)
H- I	-1343 / 0	-91.8	-91.8	0.51 (1)	4.91	L- H	0 / 962	0.22 (1)
J- I	-1655 / 0	0.0	0.0	0.33 (1)	5.15	N- F	0 / 173	0.04 (1)
P- B	-265 / 0	0.0	0.0	0.03 (1)	7.81	L- F	-609 / 0	0.44 (1)
P- O	0 / 1589	-18.5	-18.5	0.39 (1)	10.00			
O- N	0 / 1591	-18.5	-18.5	0.39 (1)	10.00			
N- M	0 / 1987	-18.5	-18.5	0.38 (1)	10.00			
M- L	0 / 1987	-18.5	-18.5	0.38 (1)	10.00			
L- K	0 / 1343	-18.5	-18.5	0.29 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.57/1.00 (D-E:1) , BC=0.39/1.00 (N-O:1) , WB=0.94/1.00 (H-K:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90)
JSI METAL= 0.62 (M) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311455

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T13	6	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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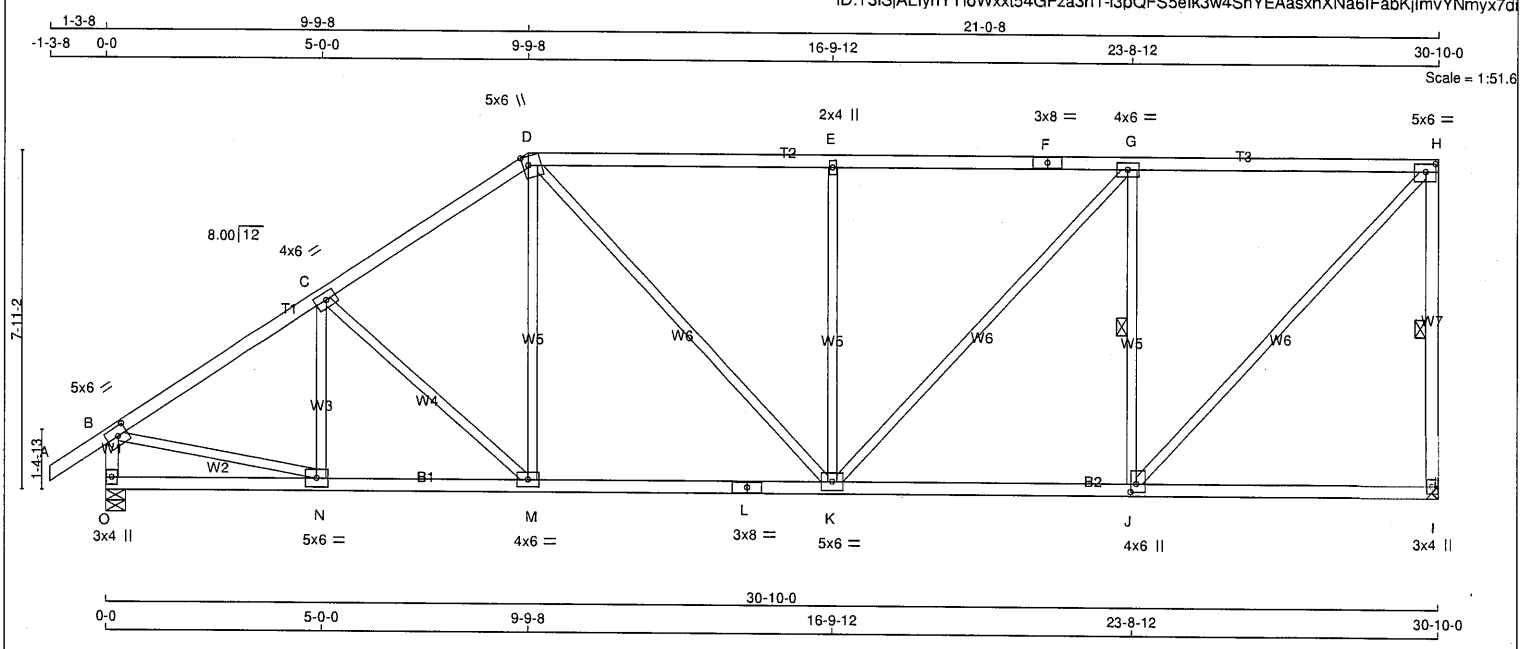

TOTAL WEIGHT = 6 X 139 = 832 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.25	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW+t	MT20	4.0	6.0	2.25	1.50
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1700	0	1700	0	0	MECHANICAL	
O	1826	0	1826	0	0	5-8	2-7

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0
O	1289	859 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	N- C	-274 / 0	0.09 (1)
B- C	-1987 / 0	-91.8	-91.8	0.47 (1)	4.30	C- M	-245 / 0	0.19 (1)
C- D	-1829 / 0	-91.8	-91.8	0.45 (1)	4.46	M- D	0 / 281	0.06 (1)
D- E	-1766 / 0	-91.8	-91.8	0.78 (1)	4.03	B- N	0 / 1716	0.39 (1)
E- F	-1766 / 0	-91.8	-91.8	0.80 (1)	3.98	J- H	0 / 1890	0.43 (1)
F- G	-1766 / 0	-91.8	-91.8	0.80 (1)	3.98	D- K	0 / 395	0.09 (1)
G- H	-1287 / 0	-91.8	-91.8	0.73 (1)	4.59	J- G	-1237 / 0	0.49 (1)
I- H	-1647 / 0	0.0	0.0	0.47 (1)	5.16	K- E	-695 / 0	0.83 (1)
O- B	-1784 / 0	0.0	0.0	0.18 (1)	6.26	K- G	0 / 714	0.16 (1)
O- N	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
N- M	0 / 1677	-18.5	-18.5	0.35 (1)	10.00			
M- L	0 / 1499	-18.5	-18.5	0.32 (1)	10.00			
L- K	0 / 1499	-18.5	-18.5	0.32 (1)	10.00			
K- J	0 / 1287	-18.5	-18.5	0.33 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.80/1.00 (E-G:1) , BC=0.35/1.00 (M-N:1) , WB=0.83/1.00 (E-K:1) , SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.57 (J) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311456

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T14	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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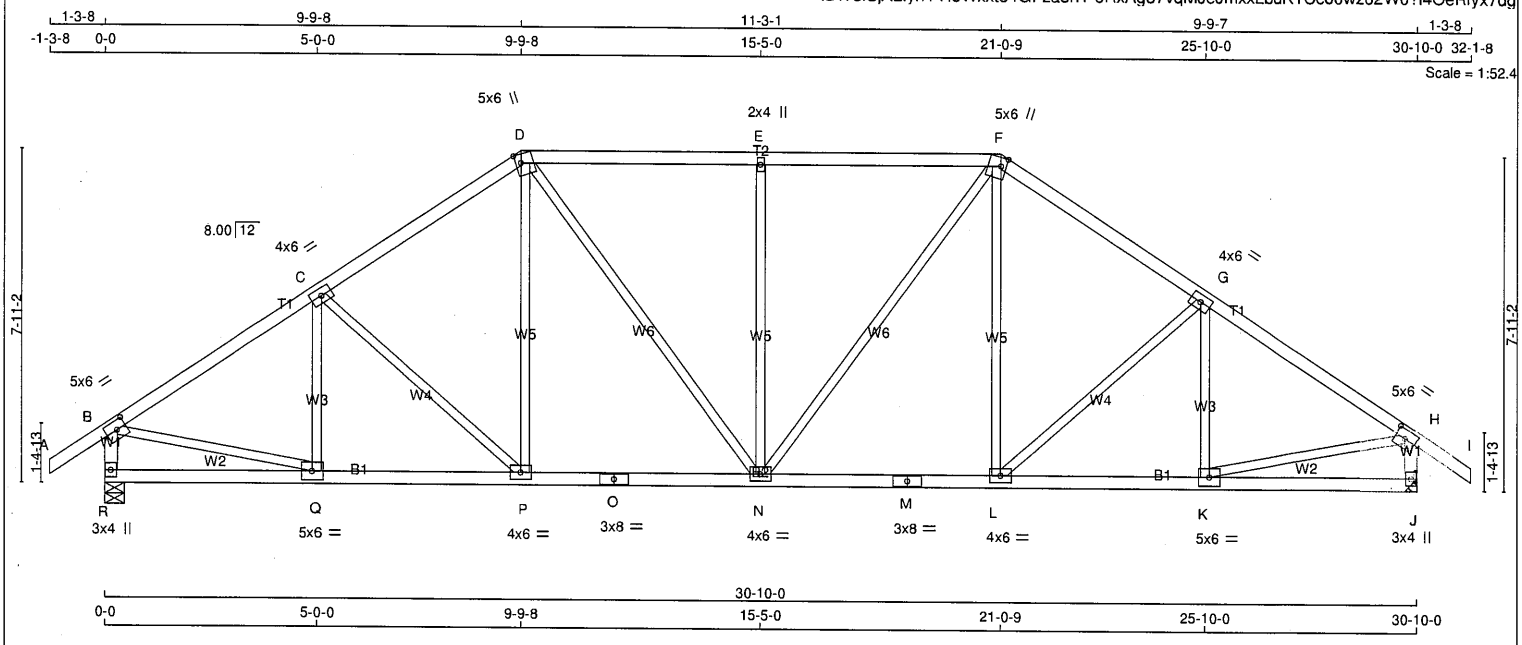

TOTAL WEIGHT = 4 X 138 = 553 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1826	0	1826	0	0	5-8	2-7
J	1826	0	1826	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-7.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1289	859 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0
J	1289	859 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00
B- C	-1988 / 0	-91.8	-91.8	0.34 (1)	4.47
C- D	-1827 / 0	-91.8	-91.8	0.33 (1)	4.64
D- E	-1767 / 0	-91.8	-91.8	0.41 (1)	4.57
E- F	-1767 / 0	-91.8	-91.8	0.41 (1)	4.57
F- G	-1827 / 0	-91.8	-91.8	0.33 (1)	4.64
G- H	-1988 / 0	-91.8	-91.8	0.34 (1)	4.47
H- I	0 / 35	-91.8	-91.8	0.12 (1)	10.00
R- B	-1785 / 0	0.0	0.0	0.18 (1)	6.25
J- H	-1785 / 0	0.0	0.0	0.18 (1)	6.25
R- Q	0 / 0	-18.5	-18.5	0.10 (4)	10.00
Q- P	0 / 1678	-18.5	-18.5	0.33 (1)	10.00
P- O	0 / 1496	-18.5	-18.5	0.30 (1)	10.00
O- N	0 / 1496	-18.5	-18.5	0.30 (1)	10.00
N- M	0 / 1496	-18.5	-18.5	0.30 (1)	10.00
M- L	0 / 1496	-18.5	-18.5	0.30 (1)	10.00
L- K	0 / 1678	-18.5	-18.5	0.33 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-268 / 0	0.09 (1)
C- P	-250 / 0	0.20 (1)
P- D	0 / 266	0.06 (1)
D- N	0 / 456	0.10 (1)
N- E	-631 / 0	0.75 (1)
N- F	0 / 456	0.10 (1)
L- F	0 / 266	0.06 (1)
L- G	-250 / 0	0.20 (1)
K- G	-268 / 0	0.09 (1)
B- Q	0 / 1717	0.39 (1)
K- H	0 / 1717	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.33/1.00 (P-Q:1) , WB=0.75/1.00 (E-N:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.50 (H) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311457

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T15	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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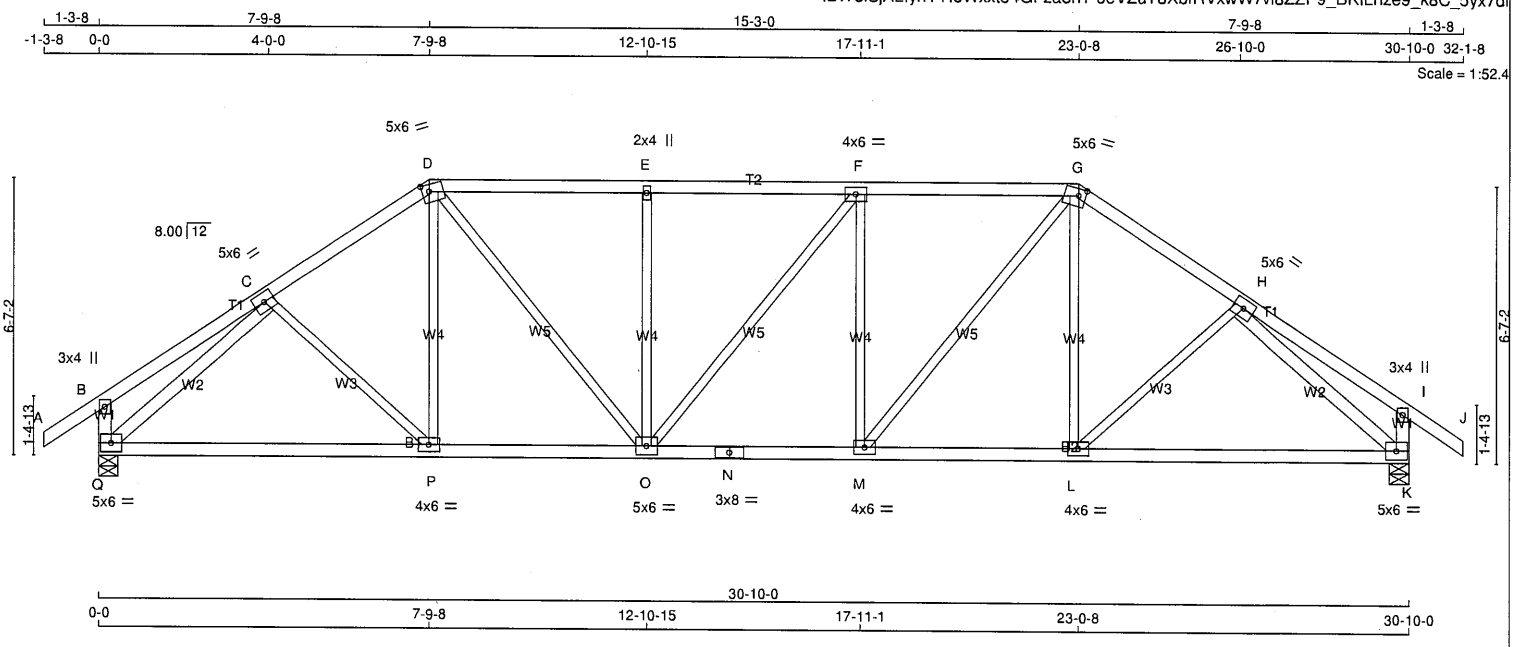

TOTAL WEIGHT = 138 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.00	2.00
H	TMWW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L, M, P						
L	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	6.0		
Q	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1826	0	1826	0	0	5-8	2-0
K	1826	0	1826	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1289	859 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0
K	1289	859 / 0	0 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	C- P	0 / 53	0.02 (4)
B- C	0 / 22	-91.8	-91.8	0.21 (1)	10.00	P- D	0 / 149	0.05 (4)
C- D	-1934 / 0	-91.8	-91.8	0.21 (1)	4.67	L- G	0 / 148	0.05 (4)
D- E	-2061 / 0	-91.8	-91.8	0.32 (1)	4.42	L- H	0 / 53	0.02 (4)
E- F	-2061 / 0	-91.8	-91.8	0.30 (1)	4.45	Q- C	-2185 / 0	0.73 (1)
F- G	-2063 / 0	-91.8	-91.8	0.32 (1)	4.42	H- K	-2185 / 0	0.73 (1)
G- H	-1934 / 0	-91.8	-91.8	0.21 (1)	4.67	M- G	0 / 749	0.17 (1)
H- I	0 / 22	-91.8	-91.8	0.21 (1)	10.00	D- O	0 / 746	0.17 (1)
I- J	0 / 35	-91.8	-91.8	0.12 (1)	10.00	M- F	-502 / 0	0.36 (1)
Q- B	-265 / 0	0.0	0.0	0.03 (1)	7.81	O- E	-501 / 0	0.36 (1)
K- I	-265 / 0	0.0	0.0	0.03 (1)	7.81	O- F	-3 / 0	0.00 (1)
Q- P	0 / 1588	-18.5	-18.5	0.39 (1)	10.00			
P- O	0 / 1591	-18.5	-18.5	0.39 (1)	10.00			
O- N	0 / 2063	-18.5	-18.5	0.39 (1)	10.00			
N- M	0 / 2063	-18.5	-18.5	0.39 (1)	10.00			
M- L	0 / 1590	-18.5	-18.5	0.39 (1)	10.00			
L- K	0 / 1588	-18.5	-18.5	0.39 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.39/1.00 (O-P:1) , WB=0.73/1.00 (C-Q:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.68 (N) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311458

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T16	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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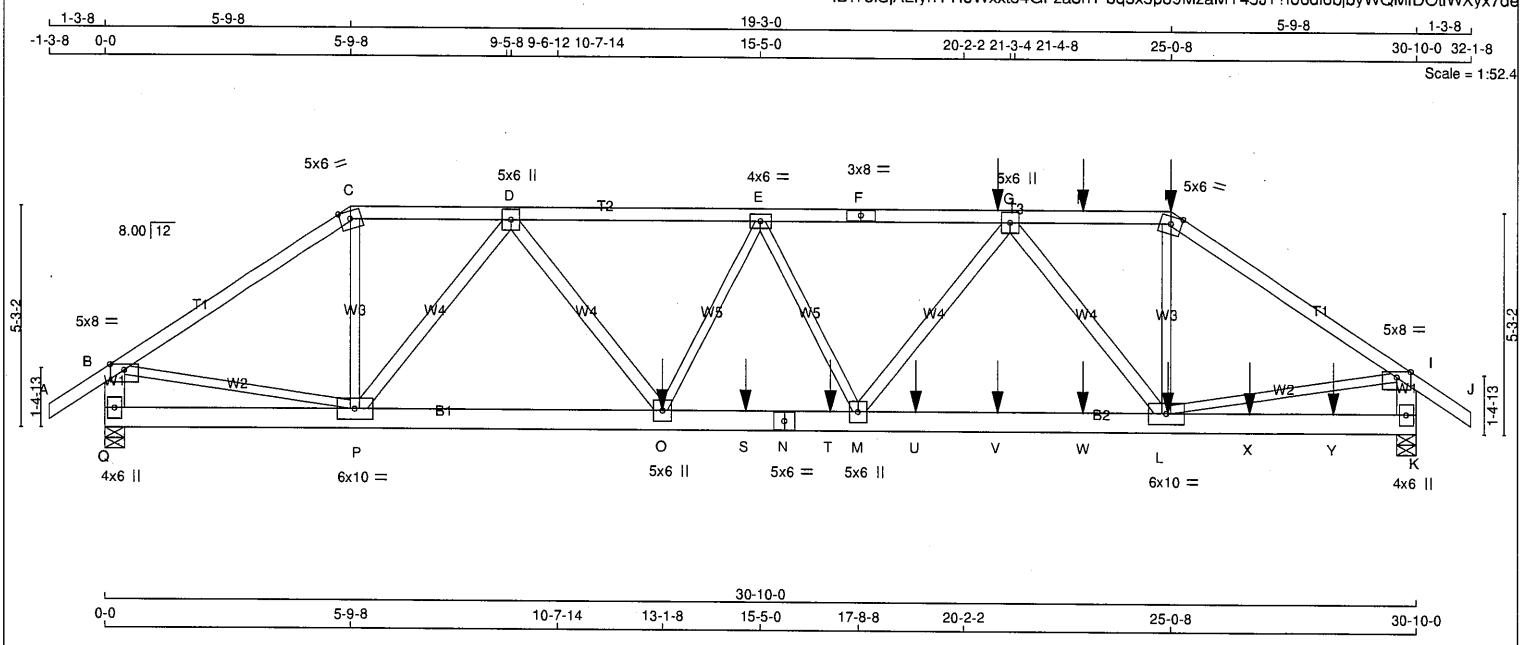

TOTAL WEIGHT = 2 X 151 = 302 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - D	2x4	DRY	No.2	SPF
D - O	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
Q- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	6	SIDE(2.2)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311459

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3577	0	3577	0	0	5-8	1-15
K	3989	0	3989	0	0	5-8	2-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2522	1699 / 0	0 / 0	0 / 0	0 / 0	822 / 0	0 / 0
K	2815	1877 / 0	0 / 0	0 / 0	0 / 0	938 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-91.8	-91.8	0.07 (1)		10.00
B- C	-4505 / 0	-91.8	-91.8	0.49 (1)		4.11
C- D	-3829 / 0	-91.8	-91.8	0.21 (1)		4.64
D- E	-7305 / 0	-91.8	-91.8	0.52 (1)		3.32
E- F	-7095 / 0	-91.8	-91.8	0.50 (1)		3.38
F- G	-7095 / 0	-91.8	-91.8	0.50 (1)		3.38
G- R	-4288 / 0	-91.8	-91.8	0.26 (1)		4.36
R- H	-4288 / 0	-91.8	-91.8	0.26 (1)		4.36
H- I	-5069 / 0	-91.8	-91.8	0.54 (1)		3.88
I- J	0 / 35	-91.8	-91.8	0.07 (1)		10.00
Q- B	-3520 / 0	0.0	0.0	0.13 (1)		7.46
K- I	-3912 / 0	0.0	0.0	0.14 (1)		7.16
Q- P	0 / 0	-18.5	-18.5	0.04 (4)		10.00
P- O	0 / 5741	-18.5	-18.5	0.41 (1)		10.00
O- S	0 / 7328	-18.5	-18.5	0.62 (1)		10.00
S- N	0 / 7328	-18.5	-18.5	0.62 (1)		10.00
N- T	0 / 7328	-18.5	-18.5	0.62 (1)		10.00
T- M	0 / 7328	-18.5	-18.5	0.62 (1)		10.00
M- U	0 / 5971	-18.5	-18.5	0.53 (1)		10.00
U- V	0 / 5971	-18.5	-18.5	0.53 (1)		10.00
V- W	0 / 5971	-18.5	-18.5	0.53 (1)		10.00
W- L	0 / 5971	-18.5	-18.5	0.53 (1)		10.00
L- X	0 / 0	-18.5	-18.5	0.09 (4)		10.00
X- Y	0 / 0	-18.5	-18.5	0.09 (4)		10.00
Y- K	0 / 0	-18.5	-18.5	0.09 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	0 / 2105	0.26 (1)
L- H	0 / 1875	0.23 (1)
B- P	0 / 3795	0.47 (1)
L- I	0 / 4270	0.53 (1)
O- E	-55 / 11	0.01 (1)
E- M	-548 / 0	0.14 (1)
P- D	-3239 / 0	0.76 (1)
D- O	0 / 2655	0.23 (1)
G- L	-2851 / 0	0.67 (1)
M- G	0 / 1908	0.17 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	20-11-12	-120	-120	---	BACK	VERT	TOTAL	---	C1
H	25-0-8	-329	-329	---	BACK	VERT	TOTAL	---	C1
L	24-11-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
O	13-1-8	-1256	-1256	---	BACK	VERT	TOTAL	---	C1
R	22-11-12	-120	-120	---	BACK	VERT	TOTAL	---	C1
S	15-0-12	-263	-263	---	BACK	VERT	TOTAL	---	C1
T	17-0-12	-272	-272	---	BACK	VERT	TOTAL	---	C1
U	19-0-12	-260	-260	---	BACK	VERT	TOTAL	---	C1
V	20-11-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
W	22-11-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
X	26-10-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
Y	28-10-12	-28	-28	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.54/1.00 (H-I:1) , BC=0.62/1.00 (M-O:1) , WB=0.76/1.00 (D-P:1) , SSI=0.20/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (N) (INPUT = 0.90)
JSI METAL= 0.77 (N) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T16	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTIoWxxt54GFza3n1-bq3x5p89MzaMY45JT?fo6di6bjbyWQMIDOtlWXyx7de

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTW-m	MT20	5.0	6.0	Edge	
D	TMWW+t	MT20	5.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW+t	MT20	5.0	6.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	4.0	6.0		
L	BMWWW-t	MT20	6.0	10.0		
M	BMWW+t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW+t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	10.0		
Q	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2311459

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T17	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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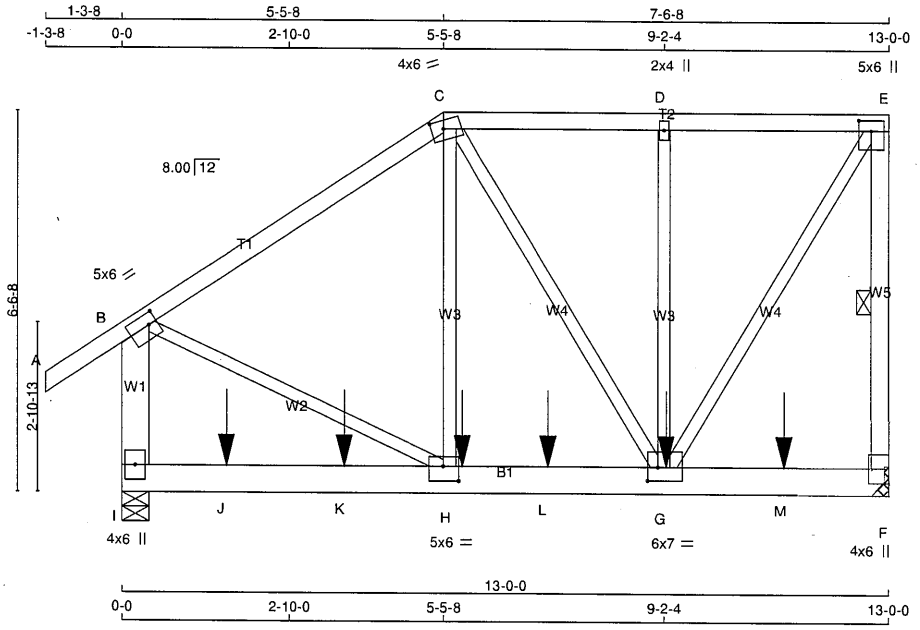

Scale = 1:37.9

TOTAL WEIGHT = 77 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x6	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	1.75
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	5.0	6.0	2.25	2.50
F	BMV1+p	MT20	4.0	6.0	3.00	Edge
G	BMWWW-t	MT20	6.0	7.0	2.75	2.00
H	BMWW-t	MT20	5.0	6.0	3.00	3.00
I	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1839	0	1839	0	0	MECHANICAL	
I	1982	0	1982	0	0	5-8	2-2

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 2-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1297	873 / 0	0 / 0	0 / 0	0 / 0	424 / 0	0 / 0
I	1395	950 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	H- C	0 / 727	0.18 (1)	
B- C	-1359 / 0	-91.8	-91.8	0.62 (1)	4.69	C- G	-419 / 0	0.40 (1)	
C- D	-924 / 0	-91.8	-91.8	0.23 (1)	6.10	G- D	-418 / 0	0.28 (1)	
D- E	-924 / 0	-91.8	-91.8	0.23 (1)	6.10	G- E	0 / 1757	0.43 (1)	
F- E	-1632 / 0	0.0	0.0	0.33 (1)	5.15	B- H	0 / 1238	0.31 (1)	
I- B	-1639 / 0	0.0	0.0	0.17 (1)	7.66				
I- J	0 / 0	-18.5	-18.5	0.43 (1)	10.00				
J- K	0 / 0	-18.5	-18.5	0.43 (1)	10.00				
K- H	0 / 0	-18.5	-18.5	0.43 (1)	10.00				
H- L	0 / 1142	-18.5	-18.5	0.50 (1)	10.00				
L- G	0 / 1142	-18.5	-18.5	0.50 (1)	10.00				
G- M	0 / 0	-18.5	-18.5	0.25 (1)	10.00				
M- F	0 / 0	-18.5	-18.5	0.25 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	9-2-12	-272	-272	---	BACK	VERT	TOTAL	---	C1
H	5-9-4	-263	-263	---	BACK	VERT	TOTAL	---	C1
J	1-9-4	-261	-261	---	BACK	VERT	TOTAL	---	C1
K	3-9-4	-272	-272	---	BACK	VERT	TOTAL	---	C1
L	7-2-12	-263	-263	---	BACK	VERT	TOTAL	---	C1
M	11-2-12	-260	-260	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	25.6 PSF
		DL	=	6.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.4 PSF
TOTAL	LOAD		=	39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.50/1.00 (G-H:1) , WB=0.43/1.00 (E-G:1) , SSI=0.34/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311460

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T18	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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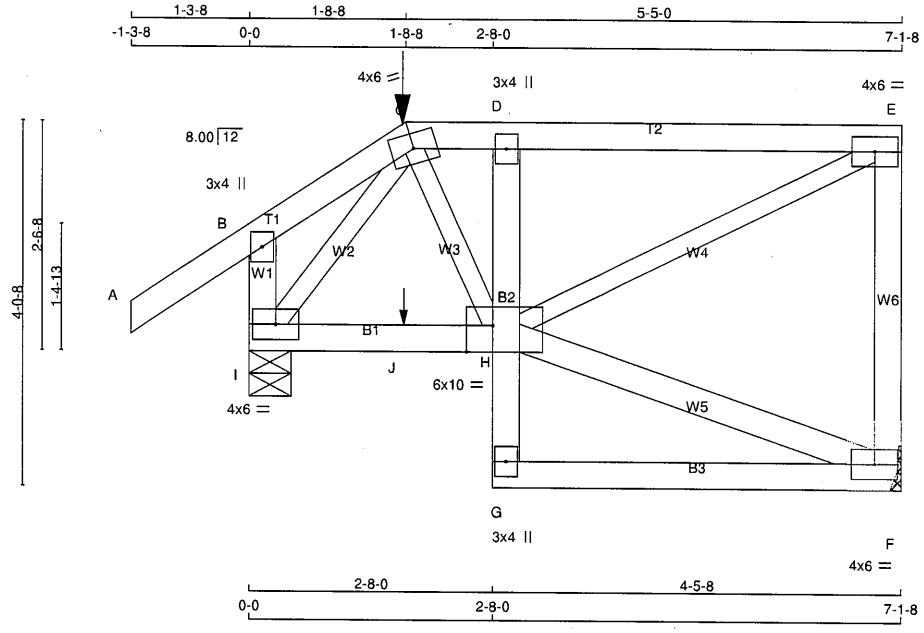

TOTAL WEIGHT = 2 X 40 = 79 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	388	0	388	0	0	MECHANICAL	
I	553	0	553	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	274	179 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
I	388	271 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	C- H	0 / 375	0.09 (5)
B- C	-27 / 0	-91.8	-91.8	0.13 (1)	6.25	H- F	-9 / 0	0.00 (1)
C- D	-357 / 0	-91.8	-91.8	0.24 (5)	6.25	H- E	0 / 396	0.10 (1)
D- E	-351 / 0	-91.8	-91.8	0.24 (5)	6.25	I- C	-344 / 0	0.06 (1)
F- E	-345 / 0	0.0	0.0	0.09 (1)	7.81			
I- B	-256 / 0	0.0	0.0	0.03 (1)	7.81			
I- J	0 / 211	-18.5	-18.5	0.07 (4)	10.00			
J- H	0 / 211	-18.5	-18.5	0.07 (4)	10.00			
G- H	0 / 45	0.0	0.0	0.03 (1)	10.00			
H- D	-430 / 0	0.0	0.0	0.02 (1)	7.81			
G- F	0 / 9	-18.5	-18.5	0.11 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-0	-4	-4	---	FRONT	VERT	DEAD	---	C1
C	1-9-0	1	1	79	BACK	VERT	TOTAL	---	C1
C	1-9-0	-16	-16	---	FRONT	VERT	SNOW	---	C1
J	1-8-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.24/1.00 (D-E:5) , BC=0.11/1.00 (F-G:4) , WB=0.10/1.00 (E-H:1) , SSI=0.20/1.00 (D-E:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311461

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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JSI GRIP= 0.60 (C) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311461

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T18Z	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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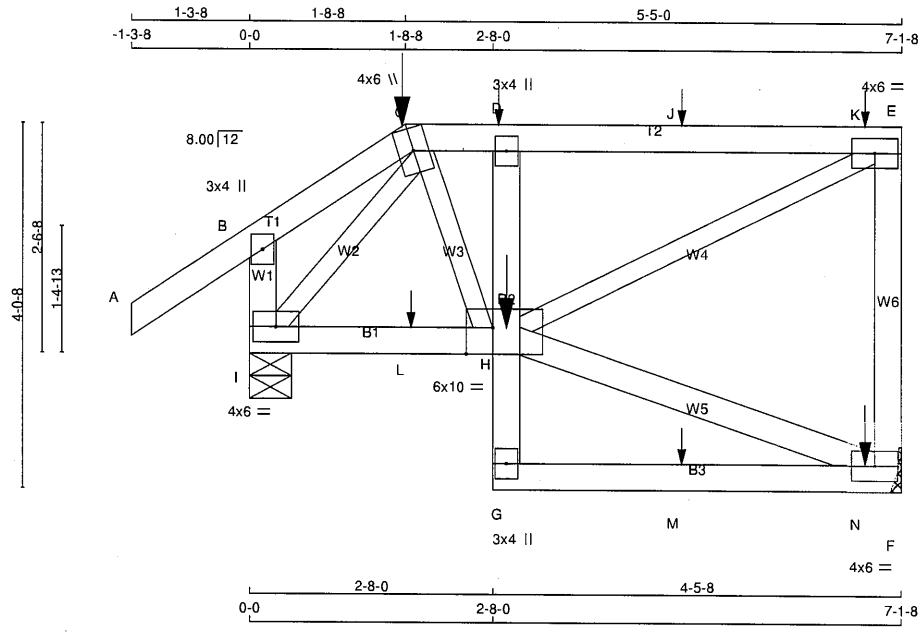

Scale = 1:24.4

TOTAL WEIGHT = 40 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	390	0	390	0	0	MECHANICAL	
I	555	0	555	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	276	179 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
I	390	271 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	C- H	0 / 344	0.09 (1)
B- C	-27 / 0	-91.8	-91.8	0.13 (1)	6.25	H- F	-9 / 0	0.00 (1)
C- D	-360 / 0	-91.8	-91.8	0.23 (1)	6.25	H- E	0 / 399	0.10 (1)
D- J	-354 / 0	-91.8	-91.8	0.24 (1)	6.25	I- C	-347 / 0	0.07 (1)
J- K	-354 / 0	-91.8	-91.8	0.24 (1)	6.25			
K- E	-354 / 0	-91.8	-91.8	0.24 (1)	6.25			
F- E	-346 / 0	0.0	0.0	0.09 (1)	7.81			
I- B	-256 / 0	0.0	0.0	0.03 (1)	7.81			
I- L	0 / 213	-18.5	-18.5	0.07 (4)	10.00			
L- H	0 / 213	-18.5	-18.5	0.07 (4)	10.00			
G- H	0 / 45	0.0	0.0	0.03 (1)	10.00			
H- D	-429 / 0	0.0	0.0	0.02 (1)	7.81			
G- M	0 / 9	-18.5	-18.5	0.11 (4)	10.00			
M- N	0 / 9	-18.5	-18.5	0.11 (4)	10.00			
N- F	0 / 9	-18.5	-18.5	0.11 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-0	-4	-4	---	FRONT	VERT	DEAD	---	C1
C	1-9-0	1	1	79	FRONT	VERT	TOTAL	---	C1
C	1-9-0	-16	-16	---	FRONT	VERT	SNOW	---	C1
D	2-8-12	1	1	79	FRONT	VERT	TOTAL	---	C1
H	2-9-12	-3	-3	---	FRONT	VERT	TOTAL	---	C1
J	4-8-12	1	1	79	FRONT	VERT	TOTAL	---	C1
K	6-8-12	1	1	64	FRONT	VERT	TOTAL	---	C1
L	1-9-4	1	1	---	FRONT	VERT	TOTAL	---	C1
M	4-8-12	1	1	---	FRONT	VERT	TOTAL	---	C1
N	6-8-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.24/1.00 (D-E:1) , BC=0.11/1.00 (F-G:4) , WB=0.10/1.00 (E-H:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311462

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T18Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTloWxxt54GFza3n1-XDBhWVAPuaq4oNEiaQhGB2nXnXPN_UBbgiMsbQyx7dc

JSI GRIP= 0.28 (C) (INPUT = 0.90)
JSI METAL= 0.15 (C) (INPUT = 0.95)

Structural component only
DWG# T-2311462

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T19	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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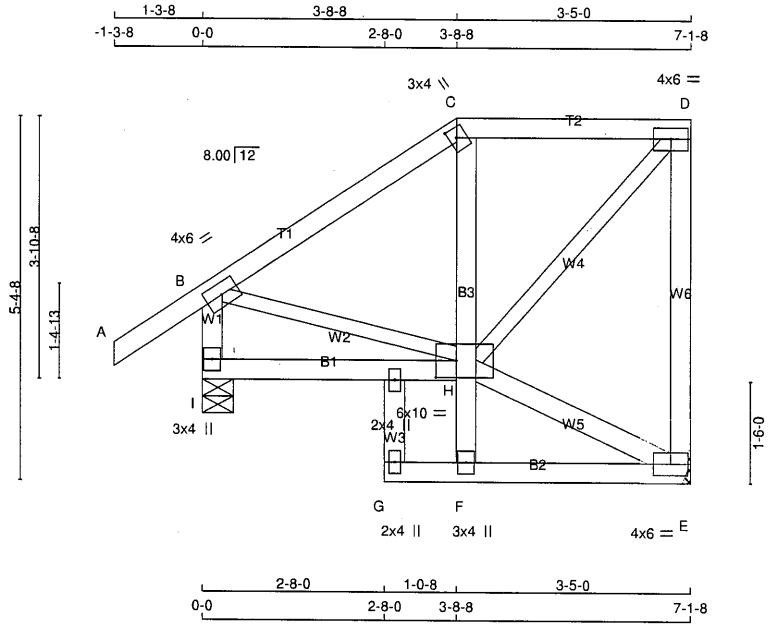

TOTAL WEIGHT = 3 X 44 = 131 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTV+h	MT20	3.0	4.0	1.75	1.25
D	TMVW-t	MT20	4.0	6.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMV+p	MT20	3.0	4.0		
G	NP+w	MT20	2.0	4.0		
H	BVMWWW-I	MT20	6.0	10.0	3.00	3.50
I	BMV1+p	MT20	3.0	4.0		
J	NP+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	405	0	405	0	0	MECHANICAL	
I	529	0	529	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	287	182 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
I	373	253 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12	(1)	10.00	B- H	0 / 215	0.05 (1)
B- C	-250 / 0	-91.8	-91.8	0.16	(1)	6.25	H- E	-5 / 0	0.00 (1)
C- D	-198 / 0	-91.8	-91.8	0.14	(1)	6.25	H- D	0 / 293	0.07 (1)
E- D	-372 / 0	0.0	0.0	0.19	(1)	7.81			
I- B	-493 / 0	0.0	0.0	0.05	(1)	7.81			
I- H	0 / 0	-18.5	-18.5	0.08	(4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.03	(4)	10.00			
F- E	0 / 4	-18.5	-18.5	0.06	(4)	10.00			
F- H	0 / 58	0.0	0.0	0.02	(4)	10.00			
H- C	-189 / 0	0.0	0.0	0.01	(1)	7.81			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.19/1.00 (D-E:1) , BC=0.08/1.00 (H-I:4) , WB=0.07/1.00 (D-H:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 0.95)

Structural component only
DWG# T-2311463

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T20	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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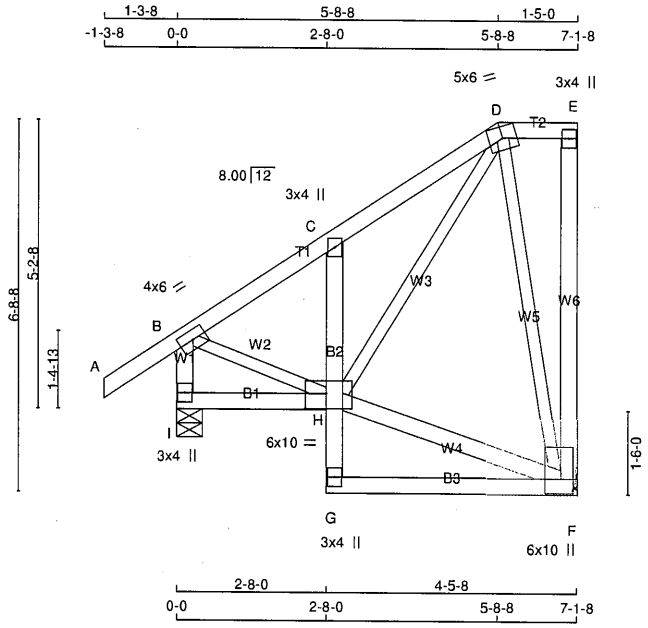

Scale = 1:39.7

TOTAL WEIGHT = 3 X 49 = 146 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	TTWW-m	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVWW1+p	MT20	6.0	10.0	3.25	2.50
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	6.0	10.0	3.25	4.50
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	393	0	393	0	0	MECHANICAL	
I	519	0	519	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	278	182 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
I	364	253 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	B- H	0 / 278	0.06 (1)
B- C	-294 / 0	-91.8	-91.8	0.09 (1)	6.25	H- F	0 / 59	0.01 (1)
C- D	-310 / 0	-91.8	-91.8	0.09 (1)	6.25	D- F	-316 / 0	0.23 (1)
D- E	0 / 0	-91.8	-91.8	0.03 (1)	10.00	H- D	0 / 349	0.08 (1)
F- E	-63 / 0	0.0	0.0	0.06 (1)	7.81			
I- B	-493 / 0	0.0	0.0	0.05 (1)	7.81			
I- H	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
G- H	0 / 45	0.0	0.0	0.03 (1)	10.00			
H- C	-314 / 0	0.0	0.0	0.02 (1)	7.81			
G- F	0 / 10	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.10/1.00 (F-G:4) , WB=0.23/1.00 (D-F:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (D) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311464

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T21	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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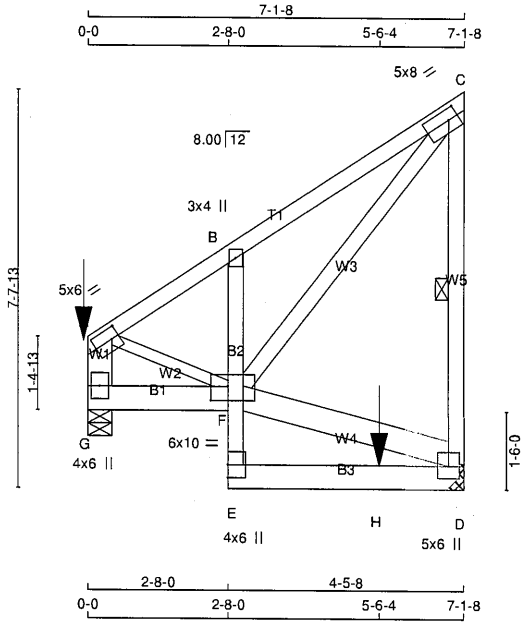

TOTAL WEIGHT = 2 X 53 = 107 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
G- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
G- F	2	12	TOP
E- D	2	12	SIDE(0.0)
B- E	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2311465

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1804	0	1804	0	0	IN-SX	IN-SX
G	1036	0	1036	0	0	MECHANICAL	
						5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1271	859 / 0	0 / 0	0 / 0	0 / 0	412 / 0	0 / 0
G	728	501 / 0	0 / 0	0 / 0	0 / 0	227 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-800 / 0	-91.8	-91.8	0.11 (1)	6.25	A- F	0 / 732	0.09 (1)
B- C	-819 / 0	-91.8	-91.8	0.12 (1)	6.25	F- D	-26 / 0	0.00 (1)
D- C	-612 / 0	0.0	0.0	0.08 (1)	6.25	F- C	0 / 1109	0.10 (1)
G- A	-1010 / 0	0.0	0.0	0.04 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
E- F	0 / 717	0.0	0.0	0.09 (1)	10.00			
F- B	-398 / 0	0.0	0.0	0.03 (1)	7.81			
E- H	0 / 25	-18.5	-18.5	0.69 (1)	10.00			
H- D	0 / 25	-18.5	-18.5	0.69 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
A	0-0	-162	-162	---	TOP	VERT	TOTAL	---	C1
H	5-6-4	-1282	-1282	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.12/1.00 (B-C:1) , BC=0.69/1.00 (D-E:1) , WB=0.10/1.00 (C-F:1) , SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (A) (INPUT = 0.90)
JSI METAL= 0.11 (A) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432478	T21	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	8.0	2.50	2.25
D	BMVW1+p	MT20	5.0	6.0	2.75	2.50
E	BMV+p	MT20	4.0	6.0		
F	BVMWWW-I	MT20	6.0	10.0	3.25	4.00
G	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311465

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T22	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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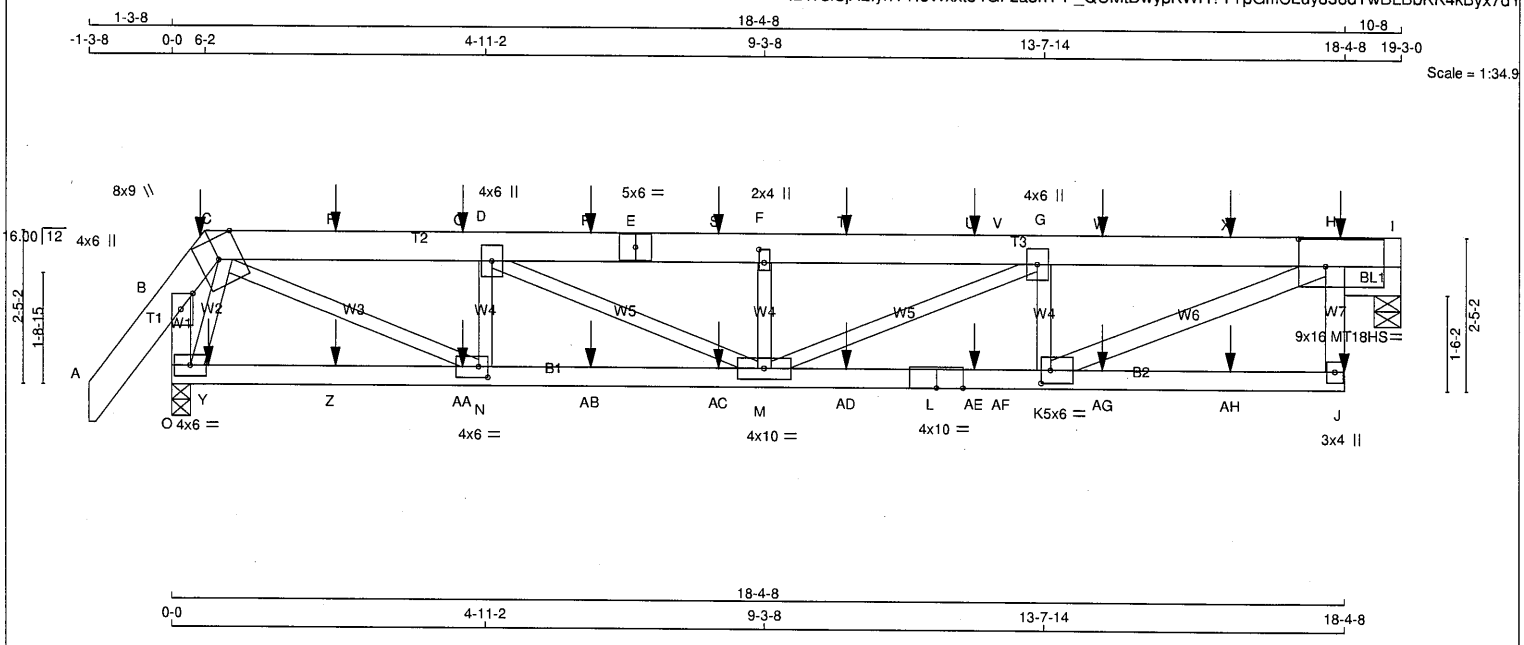

TOTAL WEIGHT = 2 X 87 = 174 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	2100F 1.8E	SPF
E - I	2x6	DRY	2100F 1.8E	SPF
J - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(13.8)
C- E	2	12	SIDE(175.2)
E- I	2	12	SIDE(166.9)
H- J	1	12	SIDE(8.2)
O- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	1	12	SIDE(54.7)
L- J	1	12	SIDE(56.4)
WEBS : (0.122"X3") SPIRAL NAILS			
D- N	1	6	SIDE(7.6)
2x3	1	6	
2x4	1	6	
H- I	2	6	SIDE(20.9)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2311466

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2827	0	2827	0	0	3-8	1-9
I	2596	0	2596	0	0	5-0	5-0

(** SEE "BEARING NOTE" **)

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): I

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1995	1332 / 0	0 / 0	0 / 0	0 / 0	663 / 0	0 / 0
I	1834	1213 / 0	0 / 0	0 / 0	0 / 0	621 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 78	-137.7	-137.7	0.06 (1)	10.00	N- D	-1096 / 0	0.09 (1)
B- C	-175 / 0	-137.7	-137.7	0.05 (1)	6.25	K- H	0 / 5804	0.51 (1)
C- P	-4522 / 0	-137.7	-137.7	0.08 (1)	6.25	D- M	0 / 1288	0.16 (1)
P- Q	-4522 / 0	-137.7	-137.7	0.08 (1)	6.25	K- G	-1638 / 0	0.14 (1)
Q- D	-4522 / 0	-137.7	-137.7	0.08 (1)	6.25	M- F	-212 / 0	0.02 (1)
D- R	-5688 / 0	-91.8	-91.8	0.08 (1)	5.85	M- G	0 / 468	0.06 (1)
R- E	-5688 / 0	-91.8	-91.8	0.08 (1)	5.85	O- C	-2110 / 0	0.18 (1)
E- S	-5688 / 0	-91.8	-91.8	0.08 (1)	5.85	C- N	0 / 4328	0.54 (1)
S- F	-5688 / 0	-91.8	-91.8	0.08 (1)	5.85			
F- T	-5688 / 0	-91.8	-91.8	0.14 (1)	5.77			
T- U	-5688 / 0	-91.8	-91.8	0.14 (1)	5.77			
U- V	-5688 / 0	-91.8	-91.8	0.14 (1)	5.77			
V- G	-5688 / 0	-137.7	-137.7	0.14 (1)	5.77			
G- W	-5264 / 0	-137.7	-137.7	0.54 (1)	5.27			
W- X	-5264 / 0	-137.7	-137.7	0.54 (1)	5.27			
X- H	-5264 / 0	-137.7	-137.7	0.54 (1)	5.27			
H- I	-1034 / 0	-137.7	-137.7	0.21 (1)	6.25			
J- H	0 / 448	0.0	0.0	0.04 (1)	10.00			
O- B	-456 / 0	0.0	0.0	0.03 (1)	7.81			
O- Y	0 / 615	-27.7	-27.7	0.30 (1)	10.00			
Y- Z	0 / 615	-27.7	-27.7	0.30 (1)	10.00			
Z-AA	0 / 615	-27.7	-27.7	0.30 (1)	10.00			
AA- N	0 / 615	-27.7	-27.7	0.30 (1)	10.00			
N-AB	0 / 4522	-18.5	-18.5	0.63 (1)	10.00			
AB-AC	0 / 4522	-18.5	-18.5	0.63 (1)	10.00			
AC- M	0 / 4522	-18.5	-18.5	0.63 (1)	10.00			
M-AD	0 / 5264	-18.5	-18.5	0.68 (1)	10.00			
AD- L	0 / 5264	-18.5	-18.5	0.68 (1)	10.00			
L-AE	0 / 5264	-18.5	-18.5	0.68 (1)	10.00			
AE-AF	0 / 5264	-18.5	-18.5	0.68 (1)	10.00			
AF- K	0 / 5264	-27.7	-27.7	0.68 (1)	10.00			
K-AG	-32 / 0	-27.7	-27.7	0.24 (1)	6.25			
AG-AH	-32 / 0	-27.7	-27.7	0.24 (1)	6.25			
AH- J	-32 / 0	-27.7	-27.7	0.24 (1)	6.25			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 964 (0.23")

CSI: TC=0.54/1.00 (G-H:1) , BC=0.68/1.00 (K-M:1) , WB=0.54/1.00 (C-N:1) , SSI=0.39/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	T22	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTloWxxt54GFza3n1-P_QCMtDwypKWH?YTpGmCLuy838dTwBLBbKK4kByx7dY

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	6.0	3.00	Edge
C	TTWW+m	MT20	8.0	9.0	Edge	4.25
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TMWW+t	MT20	4.0	6.0		
H	TMVWK-t	MT18HS	9.0	16.0	5.25	5.25
J	BMV+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	1.75
L	BS-t	MT20	4.0	10.0		
M	BMWWW-t	MT20	4.0	10.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75
O	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	7-8	-19	-19	---	FRONT	VERT	TOTAL	---	C1
H	18-3-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
J	18-4-8	-176	-176	---	FRONT	VERT	TOTAL	---	C1
P	2-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
Q	4-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
R	6-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
S	8-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
T	10-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
U	12-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
W	14-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
X	16-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
Y	6-14	-172	-172	---	FRONT	VERT	TOTAL	---	C1
Z	2-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AA	4-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AB	6-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AC	8-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AD	10-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AE	12-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AG	14-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AH	16-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.79 (N) (INPUT = 0.90)
JSI METAL= 0.63 (L) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311466

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432488	T30	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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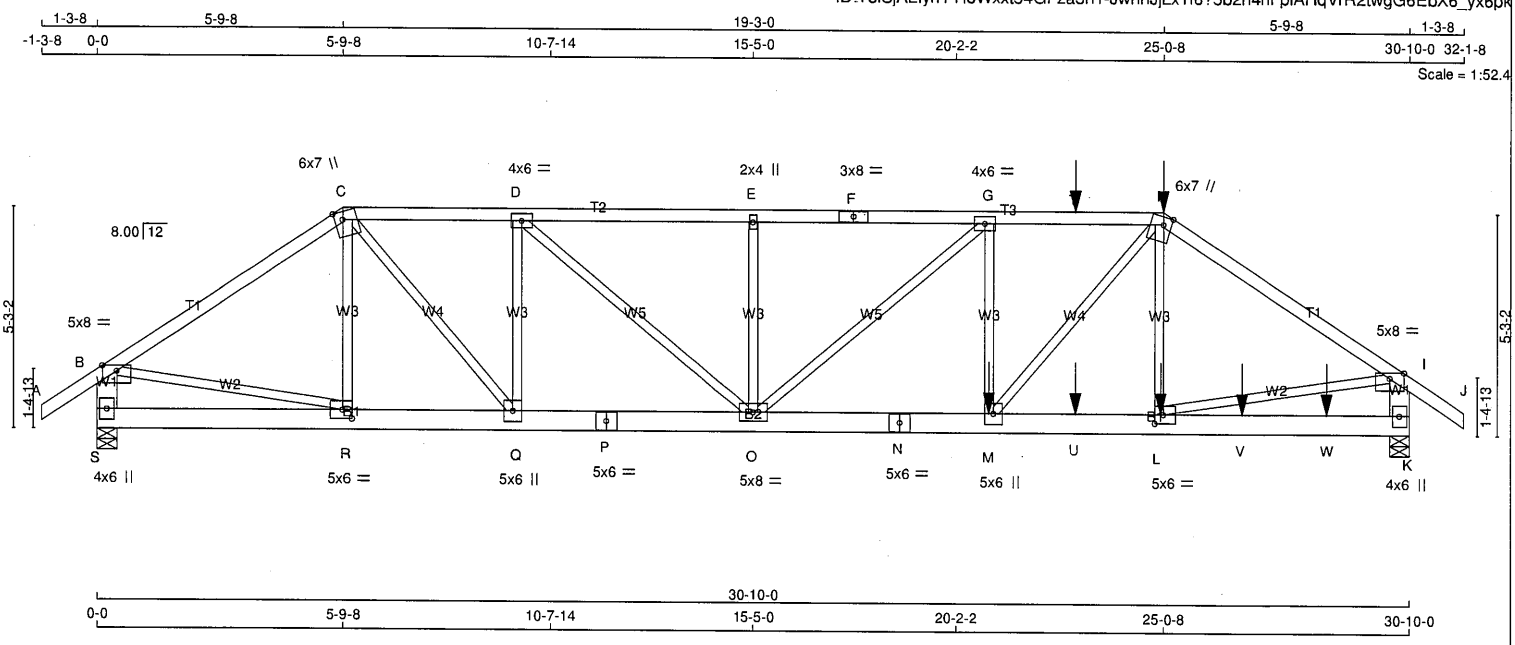

TOTAL WEIGHT = 2 X 148 = 297 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	TOP
P- N	2	12	TOP
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
L- H	1	6	SIDE(2.2)
G- M	1	6	SIDE(358.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2311469

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2647	0	2647	0	0	5-8	1-8
K	3871	0	3871	0	0	5-8	2-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1867	1254 / 0	0 / 0	0 / 0	0 / 0	613 / 0	0 / 0
K	2732	1824 / 0	0 / 0	0 / 0	0 / 0	908 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.07	(1)	10.00	R- C	-343 / 0	0.07 (1)
B- C	-3165 / 0	-91.8	-91.8	0.40	(1)	4.82	C- Q	0 / 2199	0.27 (1)
C- D	-4035 / 0	-91.8	-91.8	0.22	(1)	4.53	Q- D	-1598 / 0	0.32 (1)
D- E	-5309 / 0	-91.8	-91.8	0.32	(1)	3.98	D- O	0 / 1698	0.21 (1)
E- F	-5309 / 0	-91.8	-91.8	0.35	(1)	3.95	O- E	-506 / 0	0.10 (1)
F- G	-5309 / 0	-91.8	-91.8	0.35	(1)	3.95	O- G	-762 / 0	0.40 (1)
G- T	-5881 / 0	-91.8	-91.8	0.34	(1)	3.74	M- G	-67 / 45	0.01 (1)
T- H	-5881 / 0	-91.8	-91.8	0.34	(1)	3.74	M- H	0 / 2853	0.35 (1)
H- I	-4883 / 0	-91.8	-91.8	0.52	(1)	3.96	L- H	-450 / 21	0.09 (1)
I- J	0 / 35	-91.8	-91.8	0.07	(1)	10.00	B- R	0 / 2668	0.33 (1)
S- B	-2597 / 0	0.0	0.0	0.09	(1)	7.81	L- I	0 / 4115	0.51 (1)
K- I	-3793 / 0	0.0	0.0	0.14	(1)	7.25			
S- R	0 / 0	-18.5	-18.5	0.04	(4)	10.00			
R- Q	0 / 2623	-18.5	-18.5	0.18	(1)	10.00			
Q- P	0 / 4035	-18.5	-18.5	0.29	(1)	10.00			
P- O	0 / 4035	-18.5	-18.5	0.29	(1)	10.00			
O- N	0 / 5881	-18.5	-18.5	0.44	(1)	10.00			
N- M	0 / 5881	-18.5	-18.5	0.44	(1)	10.00			
M- U	0 / 4048	-18.5	-18.5	0.31	(1)	10.00			
U- L	0 / 4048	-18.5	-18.5	0.31	(1)	10.00			
L- V	0 / 0	-18.5	-18.5	0.07	(4)	10.00			
V- W	0 / 0	-18.5	-18.5	0.07	(4)	10.00			
W- K	0 / 0	-18.5	-18.5	0.07	(4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	25-0-8	-329	-329	---	BACK	VERT	TOTAL	---	C1
L	24-11-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
M	20-11-8	-1461	-1461	---	BACK	VERT	TOTAL	---	C1
T	22-11-12	-120	-120	---	BACK	VERT	TOTAL	---	C1
U	22-11-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
V	26-10-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
W	28-10-12	-28	-28	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.52/1.00 (H-I:1) , BC=0.44/1.00 (M-O:1) , WB=0.51/1.00 (I-L:1) , SSI=0.14/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.56 (P) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432488	T30	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	7.0	2.25	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	7.0	2.25	2.25
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW+t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311469

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432488	T31	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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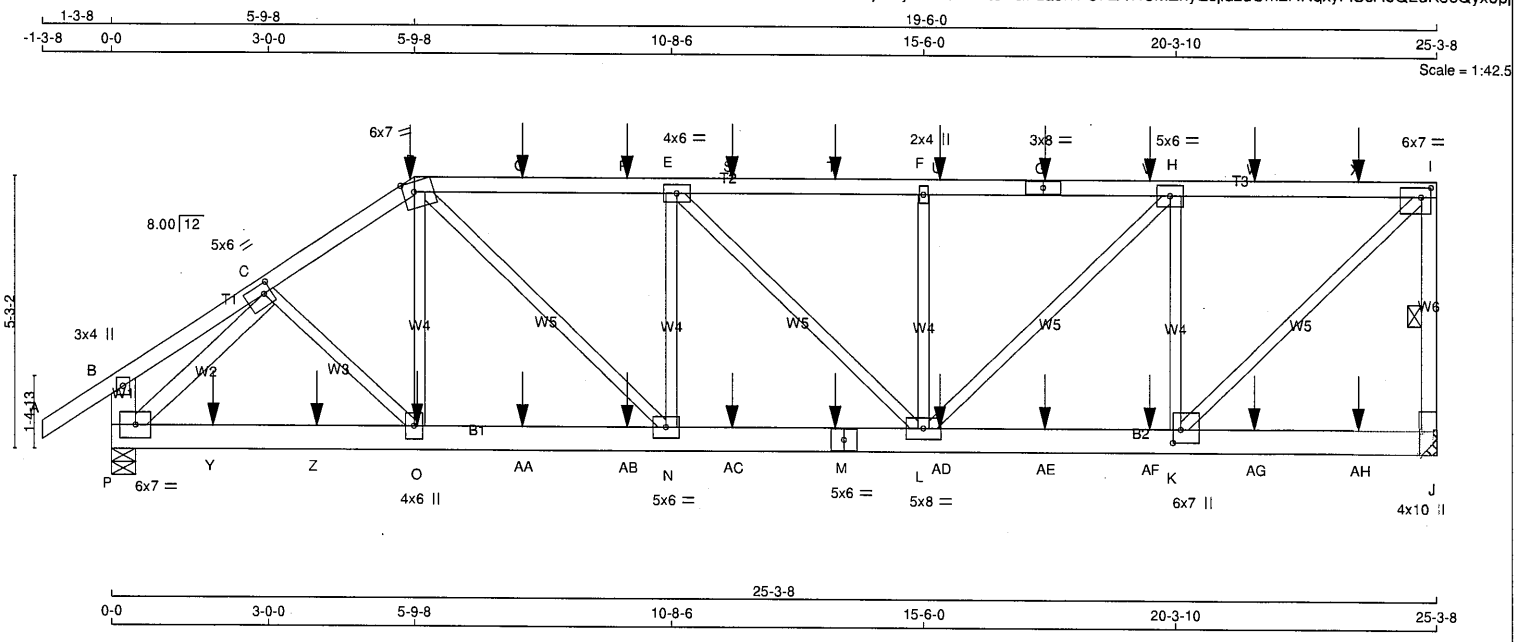

TOTAL WEIGHT = 127 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW-m	MT20	6.0	7.0	Edge	2.50
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMVW-t	MT20	6.0	7.0	2.25	2.25
J	BMV1+t	MT20	4.0	10.0	Edge	0.50
K	BMWW+t	MT20	6.0	7.0	3.00	1.75
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMVW1-t	MT20	6.0	7.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311470

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	2699	0	2699	0	0	MECHANICAL	
P	2687	0	2687	0	0	5-8	2-15

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1908	1254 / 0	0 / 0	0 / 0	0 / 0	654 / 0	0 / 0
P	1900	1250 / 0	0 / 0	0 / 0	0 / 0	650 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	O- D	-64 / 127	0.05 (4)
B- C	0 / 12	-91.8	-91.8	0.10 (1)	10.00	C- O	0 / 415	0.10 (1)
C- D	-3199 / 0	-91.8	-91.8	0.25 (1)	3.69	P- C	-3340 / 0	0.65 (1)
D- Q	-3618 / 0	-91.8	-91.8	0.73 (1)	3.33	K- I	0 / 3306	0.82 (1)
Q- R	-3618 / 0	-91.8	-91.8	0.73 (1)	3.33	D- N	0 / 1373	0.34 (1)
R- E	-3618 / 0	-91.8	-91.8	0.73 (1)	3.33	K- H	-2110 / 0	0.84 (1)
E- S	-3452 / 0	-91.8	-91.8	0.71 (1)	3.41	N- E	-803 / 0	0.32 (1)
S- T	-3452 / 0	-91.8	-91.8	0.71 (1)	3.41	L- H	0 / 1556	0.39 (1)
T- F	-3452 / 0	-91.8	-91.8	0.71 (1)	3.41	E- L	-237 / 0	0.20 (1)
F- U	-3452 / 0	-91.8	-91.8	0.72 (1)	3.39	L- F	-791 / 0	0.31 (1)
U- G	-3452 / 0	-91.8	-91.8	0.72 (1)	3.39			
G- V	-3452 / 0	-91.8	-91.8	0.72 (1)	3.39			
V- H	-3452 / 0	-91.8	-91.8	0.72 (1)	3.39			
H- W	-2361 / 0	-91.8	-91.8	0.66 (1)	4.09			
W- X	-2361 / 0	-91.8	-91.8	0.66 (1)	4.09			
X- I	-2361 / 0	-91.8	-91.8	0.66 (1)	4.09			
J- I	-2627 / 0	0.0	0.0	0.40 (1)	4.19			
P- B	-236 / 0	0.0	0.0	0.02 (1)	7.81			
P- Y	0 / 2347	-18.5	-18.5	0.39 (1)	10.00			
Y- Z	0 / 2347	-18.5	-18.5	0.39 (1)	10.00			
Z- O	0 / 2347	-18.5	-18.5	0.39 (1)	10.00			
O-AA	0 / 2644	-18.5	-18.5	0.41 (1)	10.00			
AA-AB	0 / 2644	-18.5	-18.5	0.41 (1)	10.00			
AB- N	0 / 2644	-18.5	-18.5	0.41 (1)	10.00			
N-AC	0 / 3618	-18.5	-18.5	0.55 (1)	10.00			
AC- M	0 / 3618	-18.5	-18.5	0.55 (1)	10.00			
M- L	0 / 3618	-18.5	-18.5	0.55 (1)	10.00			
L-AD	0 / 2361	-18.5	-18.5	0.38 (1)	10.00			
AD-AE	0 / 2361	-18.5	-18.5	0.38 (1)	10.00			
AE-AF	0 / 2361	-18.5	-18.5	0.38 (1)	10.00			
AF- K	0 / 2361	-18.5	-18.5	0.38 (1)	10.00			
K-AG	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.73/1.00 (D-E:1) , BC=0.55/1.00 (L-N:1) , WB=0.84/1.00 (H-K:1) , SSI=0.46/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (L) (INPUT = 0.90)
JSI METAL= 0.76 (C) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
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CONTINUED ON PAGE 2

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AG-AH	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
AH- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-9-8	-335	-335	---	FRONT	VERT	TOTAL	---	C1
G	17-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
M	13-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
O	5-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Q	7-10-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
R	9-10-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
S	11-10-4	-120	-120	---	FRONT	VERT	TOTAL	---	C1
T	13-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
U	15-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
V	19-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
W	21-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
X	23-9-12	-120	-120	---	FRONT	VERT	TOTAL	---	C1
Y	1-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
Z	3-11-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AA	7-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AB	9-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AC	11-10-4	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AD	15-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AE	17-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AF	19-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AG	21-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
AH	23-9-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2311470

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432488	T32	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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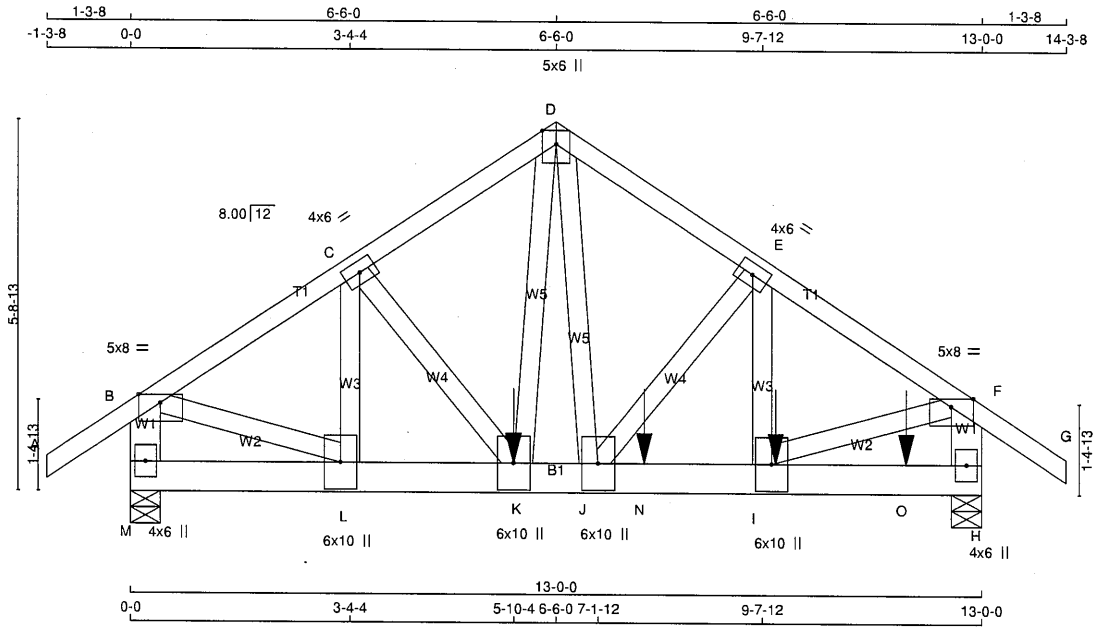

TOTAL WEIGHT = 2 X 81 = 163 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
D- K	1	5	SIDE(516.1)
I- E	1	6	SIDE(230.7)
2x4	1	5	
B- L	1	6	
L- C	1	6	
C- K	1	6	
F- I	1	6	
E- J	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2311471

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	3382	0	3382	0	0	5-8	1-13
H	5523	0	5523	0	0	5-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2387	1593 / 0	0 / 0	0 / 0	0 / 0	794 / 0	0 / 0
H	3899	2599 / 0	0 / 0	0 / 0	0 / 0	1299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	K- D	0 / 1984	0.18 (1)
B- C	-3816 / 0	-91.8	-91.8	0.12 (1)	4.75	D- J	0 / 2465	0.22 (1)
C- D	-4198 / 0	-91.8	-91.8	0.13 (1)	4.57	B- L	0 / 3324	0.29 (1)
D- E	-4251 / 0	-91.8	-91.8	0.13 (1)	4.53	L- C	-907 / 0	0.07 (1)
E- F	-5340 / 0	-91.8	-91.8	0.18 (1)	4.07	C- K	0 / 467	0.04 (1)
F- G	0 / 35	-91.8	-91.8	0.07 (1)	10.00	I- F	0 / 4644	0.41 (1)
M- B	-3327 / 0	0.0	0.0	0.12 (1)	7.62	I- E	0 / 1416	0.13 (1)
H- F	-4552 / 0	0.0	0.0	0.16 (1)	6.75	J- E	-1464 / 0	0.15 (1)
M- L	0 / 0	-18.5	-18.5	0.03 (1)	10.00			
L- K	0 / 3186	-18.5	-18.5	0.25 (1)	10.00			
K- J	0 / 3290	-18.5	-18.5	0.32 (1)	10.00			
J- N	0 / 4452	-18.5	-18.5	0.48 (1)	10.00			
N- I	0 / 4452	-18.5	-18.5	0.48 (1)	10.00			
I- O	0 / 0	-18.5	-18.5	0.40 (1)	10.00			
O- H	0 / 0	-18.5	-18.5	0.40 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	9-10-4	-1120	-1120	---	BACK	VERT	TOTAL	---	C1
K	5-10-4	-1893	-1893	---	BACK	VERT	TOTAL	---	C1
N	7-10-4	-971	-971	---	BACK	VERT	TOTAL	---	C1
O	11-10-4	-1114	-1114	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.18/1.00 (E-F:1) , BC=0.48/1.00 (I-J:1) , WB=0.41/1.00 (F-I:1) , SSI=0.32/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.90)
JSI METAL= 0.67 (L) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432488	T32	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	4.0	6.0		
I, J, K, L						
I	BMWW+t	MT20	6.0	10.0		
M	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311471

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432488	T33S	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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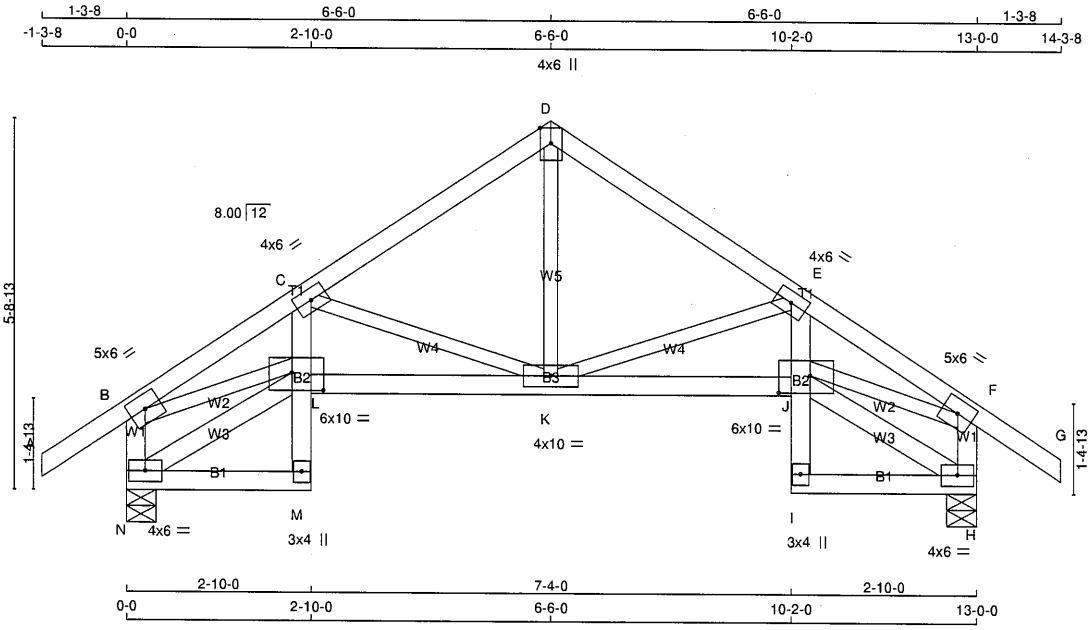

TOTAL WEIGHT = 4 X 64 = 256 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-t	MT20	6.0	10.0	3.25	5.75
K	BMWWW-t	MT20	4.0	10.0		
L	BVMWW-t	MT20	6.0	10.0	3.25	5.75
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1046	0	1046	0	0	5-8	1-8
H	1046	0	1046	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	756	403 / 0	0 / 0	0 / 0	0 / 0	353 / 0	0 / 0
H	756	403 / 0	0 / 0	0 / 0	0 / 0	353 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	K- D	0 / 825	0.22 (4)
B- C	-1783 / 0	-91.8	-91.8	0.17 (1)	4.81	C- K	-746 / 0	0.22 (1)
C- D	-1032 / 0	-91.8	-91.8	0.21 (1)	5.89	K- E	-746 / 0	0.22 (1)
D- E	-1032 / 0	-91.8	-91.8	0.21 (1)	5.89	N- L	-55 / 0	0.01 (1)
E- F	-1783 / 0	-91.8	-91.8	0.17 (1)	4.81	B- L	0 / 1516	0.38 (1)
F- G	0 / 35	-91.8	-91.8	0.14 (1)	10.00	J- H	-55 / 0	0.01 (1)
N- B	-983 / 0	0.0	0.0	0.11 (1)	7.81	J- F	0 / 1516	0.38 (1)
H- F	-983 / 0	0.0	0.0	0.11 (1)	7.81			
N- M	0 / 48	-18.5	-44.3	0.08 (4)	10.00			
M- L	0 / 54	0.0	0.0	0.12 (1)	10.00			
L- C	0 / 334	0.0	0.0	0.17 (1)	10.00			
L- K	0 / 1550	-44.3	-81.0	0.51 (4)	10.00			
K- J	0 / 1550	-81.0	-44.3	0.51 (4)	10.00			
I- J	0 / 54	0.0	0.0	0.12 (1)	10.00			
J- E	0 / 334	0.0	0.0	0.17 (1)	10.00			
I- H	0 / 48	-44.3	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.51/1.00 (J-K:4) , WB=0.38/1.00 (F-J:1) , SSI=0.24/1.00 (K-L:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.77 (K) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311472

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432488	T34	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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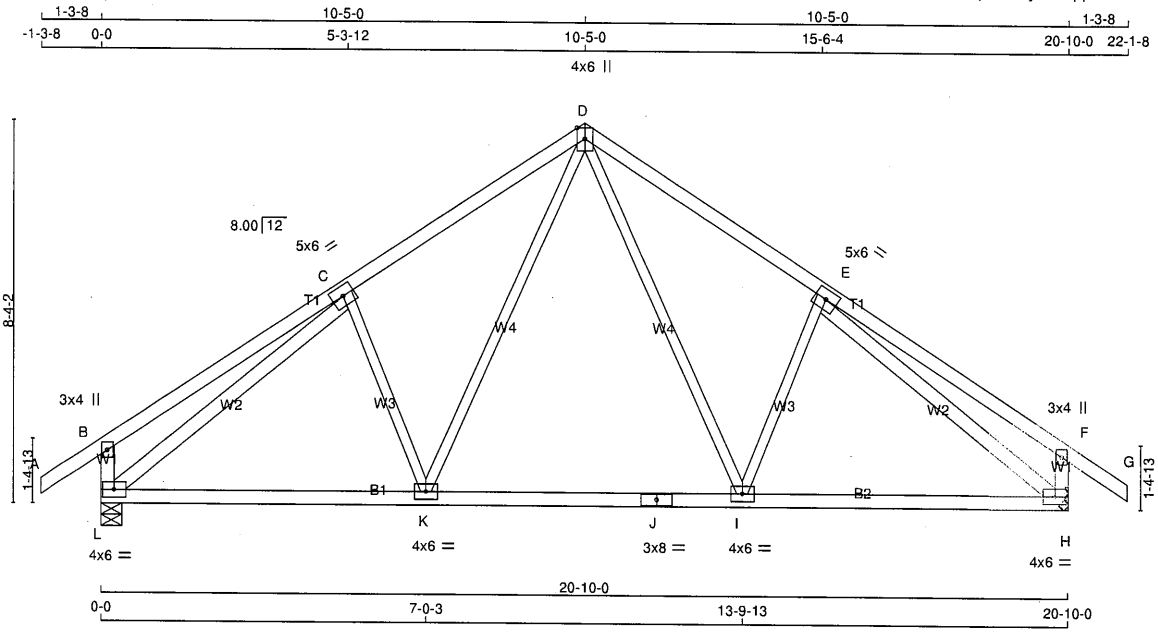

TOTAL WEIGHT = 3 X 95 = 284 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1274	0	1274	0	0	5-8	1-8
H	1274	0	1274	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	899	603 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0
H	899	603 / 0	0 / 0	0 / 0	0 / 0	296 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	D- I	0 / 470	0.11 (1)
B- C	0 / 32	-91.8	-91.8	0.41 (1)	10.00	I- E	-317 / 0	0.13 (1)
C- D	-1144 / 0	-91.8	-91.8	0.34 (1)	5.54	K- D	0 / 470	0.11 (1)
D- E	-1144 / 0	-91.8	-91.8	0.34 (1)	5.54	C- K	-317 / 0	0.13 (1)
E- F	0 / 32	-91.8	-91.8	0.41 (1)	10.00	L- C	-1390 / 0	0.86 (1)
F- G	0 / 35	-91.8	-91.8	0.12 (1)	10.00	E- H	-1390 / 0	0.86 (1)
L- B	-305 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-305 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 1046	-18.5	-18.5	0.29 (4)	10.00			
K- J	0 / 742	-18.5	-18.5	0.26 (4)	10.00			
J- I	0 / 742	-18.5	-18.5	0.26 (4)	10.00			
I- H	0 / 1046	-18.5	-18.5	0.29 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.29/1.00 (K-L:4) , WB=0.86/1.00 (C-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (C) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311473

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432488	T34S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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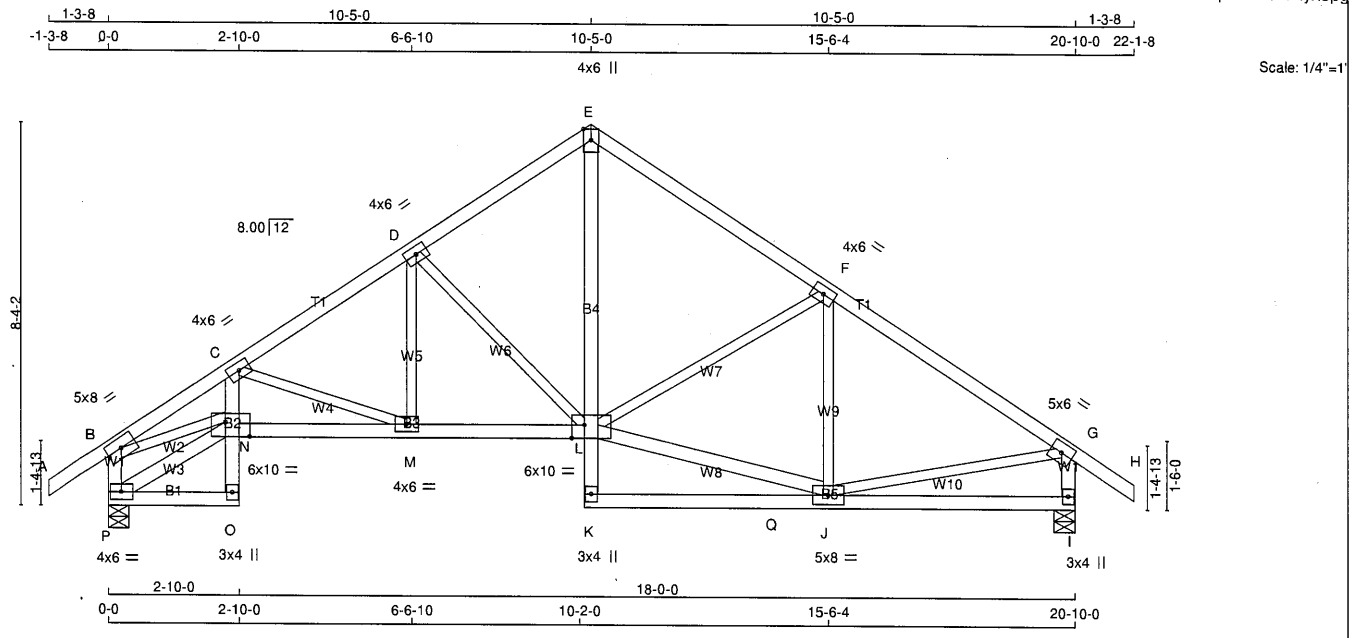

TOTAL WEIGHT = 104 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMVW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TTV+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWWW-t	MT20	6.0	10.0	Edge	3.25
M	BMWW-t	MT20	4.0	6.0		
N	BVMWW-t	MT20	6.0	10.0	3.75	6.25
O	BMV+p	MT20	3.0	4.0		
P	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311474

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1517	0	1517	0	0	5-8	1-10
P	1675	0	1675	0	0	5-8	1-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1093	603 / 0	0 / 0	0 / 0	0 / 0	490 / 0	0 / 0
P	1219	603 / 0	0 / 0	0 / 0	0 / 0	616 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 35	-91.8	-91.8	0.14 (1)	10.00	J- F	-505 / 0	0.19 (1)
B- C	-3171 / 0	-91.8	-91.8	0.25 (1)	3.69	J- G	0 / 1372	0.34 (1)
C- D	-2298 / 0	-91.8	-91.8	0.33 (1)	4.17	P- N	-97 / 0	0.01 (1)
D- E	-1559 / 0	-91.8	-91.8	0.28 (1)	4.95	B- N	0 / 2673	0.66 (1)
E- F	-1566 / 0	-91.8	-91.8	0.55 (1)	4.55	C- M	-853 / 0	0.25 (1)
F- G	-1585 / 0	-91.8	-91.8	0.56 (1)	4.53	M- D	0 / 668	0.22 (4)
G- H	0 / 35	-91.8	-91.8	0.14 (1)	10.00	D- L	-928 / 0	0.48 (1)
P- B	-1592 / 0	0.0	0.0	0.18 (1)	6.50	L- J	0 / 1386	0.25 (1)
I- G	-1483 / 0	0.0	0.0	0.17 (1)	6.69	L- F	-80 / 44	0.05 (1)
P- O	0 / 84	-18.5	-44.3	0.09 (4)	10.00			
O- N	0 / 54	0.0	0.0	0.20 (1)	10.00			
N- C	0 / 489	0.0	0.0	0.28 (1)	10.00			
N- M	0 / 2733	-44.3	-81.4	0.77 (4)	10.00			
M- L	0 / 1929	-81.4	-118.5	0.66 (4)	10.00			
K- L	0 / 135	0.0	0.0	0.06 (4)	10.00			
L- E	0 / 1389	0.0	0.0	0.28 (1)	10.00			
K- Q	0 / 15	-81.0	-18.5	0.26 (4)	10.00			
Q- J	0 / 15	-18.5	-18.5	0.26 (4)	10.00			
J- I	0 / 0	-18.5	-18.5	0.23 (4)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.56/1.00 (F-G:1) , BC=0.77/1.00 (M-N:4) , WB=0.66/1.00 (B-N:1) , SSI=0.32/1.00 (L-M:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (N) (INPUT = 0.90)
JSI METAL= 0.59 (B) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T41S	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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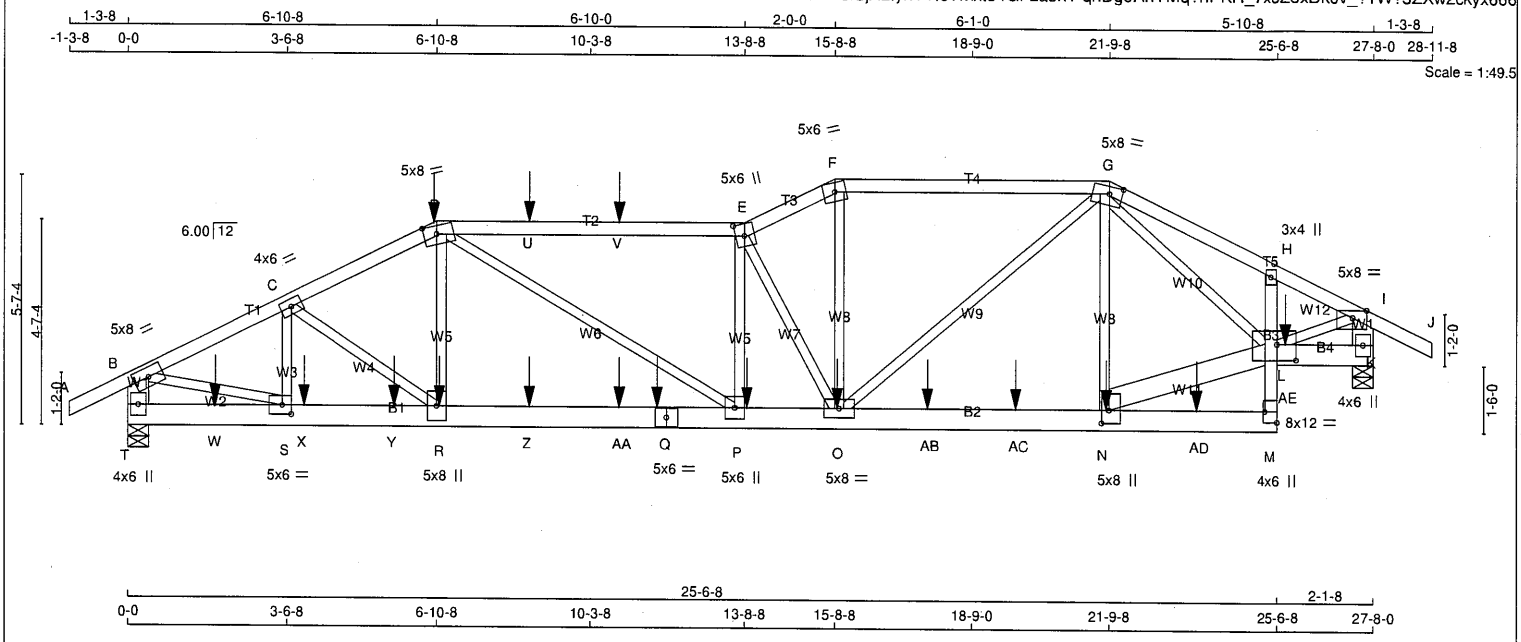

TOTAL WEIGHT = 2 X 142 = 284 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- E	1	12	SIDE(61.0)
E- F	1	12	TOP
F- G	1	12	TOP
G- J	1	12	TOP
T- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	2	12	SIDE(183.1)
Q- M	2	12	SIDE(183.1)
L- K	2	12	SIDE(0.0)
M- H	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
R- D	1	6	SIDE(5.5)
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311490

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	3712	0	3712	0	0	5-8	2-0
K	4034	0	4034	0	0	5-8	2-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2620	1746 / 0	0 / 0	0 / 0	0 / 0	874 / 0	0 / 0
K	2840	1932 / 0	0 / 0	0 / 0	0 / 0	908 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	S- C	-1128 / 0	0.10 (1)
B- C	-5155 / 0	-91.8	-91.8	0.18 (1)	4.13	C- R	0 / 757	0.09 (1)
C- D	-5861 / 0	-91.8	-91.8	0.19 (1)	3.89	R- D	0 / 667	0.08 (1)
D- U	-6823 / 0	-91.8	-91.8	0.94 (1)	2.98	D- P	0 / 1864	0.23 (1)
U- V	-6823 / 0	-91.8	-91.8	0.94 (1)	2.98	P- E	-128 / 35	0.02 (1)
V- E	-6823 / 0	-91.8	-91.8	0.94 (1)	2.98	E- O	-3328 / 0	0.61 (1)
E- F	-5972 / 0	-91.8	-91.8	0.15 (1)	3.90	O- F	0 / 2301	0.28 (1)
F- G	-5388 / 0	-91.8	-91.8	0.54 (1)	3.72	O- G	0 / 2177	0.27 (1)
G- H	-4876 / 0	-91.8	-91.8	0.21 (1)	4.21	N- G	-309 / 0	0.07 (1)
H- I	-4968 / 0	-91.8	-91.8	0.12 (1)	4.26	N- L	0 / 3940	0.26 (1)
I- J	0 / 28	-91.8	-91.8	0.07 (1)	10.00	G- L	0 / 865	0.11 (1)
T- B	-3585 / 0	0.0	0.0	0.13 (1)	7.41	B- S	0 / 4728	0.59 (1)
K- I	-3959 / 0	0.0	0.0	0.14 (1)	7.13	L- I	0 / 4697	0.58 (1)
T- W	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
W- S	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
S- X	0 / 4619	-18.5	-18.5	0.50 (1)	10.00			
X- Y	0 / 4619	-18.5	-18.5	0.50 (1)	10.00			
Y- R	0 / 4619	-18.5	-18.5	0.50 (1)	10.00			
R- Z	0 / 5244	-18.5	-18.5	0.58 (1)	10.00			
Z-AA	0 / 5244	-18.5	-18.5	0.58 (1)	10.00			
AA- Q	0 / 5244	-18.5	-18.5	0.58 (1)	10.00			
Q- P	0 / 5244	-18.5	-18.5	0.58 (1)	10.00			
P- O	0 / 6826	-18.5	-18.5	0.54 (1)	10.00			
O-AB	0 / 3748	-18.5	-18.5	0.44 (1)	10.00			
AB-AC	0 / 3748	-18.5	-18.5	0.44 (1)	10.00			
AC- N	0 / 3748	-18.5	-18.5	0.44 (1)	10.00			
N-AD	0 / 102	-18.5	-18.5	0.21 (1)	10.00			
AD- M	0 / 102	-18.5	-18.5	0.21 (1)	10.00			
M- L	0 / 86	0.0	0.0	0.12 (1)	10.00			
L- H	-246 / 0	0.0	0.0	0.11 (1)	7.81			
L-AE	0 / 0	-18.5	-18.5	0.04 (1)	10.00			
AE- K	0 / 0	-18.5	-18.5	0.04 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-10-8	-337	-337	---	FRONT	VERT	TOTAL	---	C1
N	21-8-12	-253	-253	---	FRONT	VERT	TOTAL	---	C1
O	15-9-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
P	13-9-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
Q	11-9-4	-245	-245	---	FRONT	VERT	TOTAL	---	C1
R	6-11-4	-169	-169	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.94/1.00 (D-E:1) , BC=0.58/1.00 (P-R:1) , WB=0.61/1.00 (E-O:1) , SSI=0.16/1.00 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.70 (Q) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T41S	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TTWW+m	MT20	5.0	6.0	3.25	2.25
F	TTW-m	MT20	5.0	6.0		
G	TTWWW-m	MT20	5.0	8.0	2.00	3.50
H	TMV+p	MT20	3.0	4.0		
I	TMVW-p	MT20	5.0	8.0	Edge	3.75
K	BMV1+p	MT20	4.0	6.0		
L	BVMWWW-I	MT20	8.0	12.0	4.25	5.00
M	BMV+p	MT20	4.0	6.0	3.00	Edge
N	BMWW+t	MT20	5.0	8.0	3.50	2.00
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	5.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	8-11-4	-23	-23	---	FRONT	VERT	TOTAL	---	C1
V	10-11-4	-23	-23	---	FRONT	VERT	TOTAL	---	C1
W	1-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
X	3-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
Y	5-11-4	-149	-149	---	FRONT	VERT	TOTAL	---	C1
Z	8-11-4	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AA	10-11-4	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AB	17-9-4	-253	-253	---	FRONT	VERT	TOTAL	---	C1
AC	19-8-12	-253	-253	---	FRONT	VERT	TOTAL	---	C1
AD	23-8-12	-253	-253	---	FRONT	VERT	TOTAL	---	C1
AE	25-8-12	-256	-256	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2311490

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T42S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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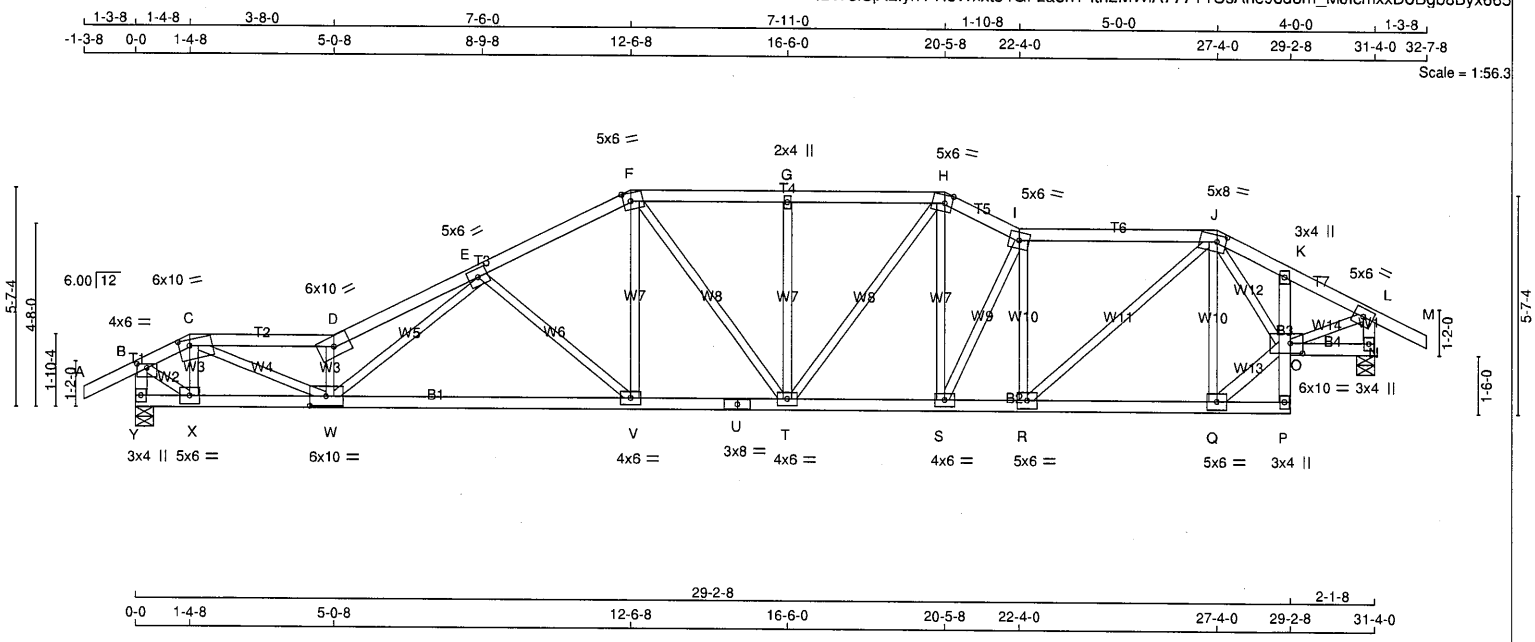

TOTAL WEIGHT = 141 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - U	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.25	3.00
C	TTWW-m	MT20	6.0	10.0	1.75	3.25
D	TTW-h	MT20	6.0	10.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.50	2.25
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	6.0	2.50	2.25
I	TTWW-m	MT20	5.0	6.0		
J	TTWWW-m	MT20	5.0	8.0	2.00	2.75
K	TMV+p	MT20	3.0	4.0		
L	TMVW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		
O	BVMWWW-t	MT20	6.0	10.0	3.00	3.75
P	BMV+p	MT20	3.0	4.0		
Q, R, X						
Q	BMWW-t	MT20	5.0	6.0		
S	BMWW-t	MT20	4.0	6.0		
T	BMWWW-t	MT20	4.0	6.0		
U	BS-t	MT20	3.0	8.0		
V	BMWW-t	MT20	4.0	6.0		
W	BMWWW-t	MT20	6.0	10.0	3.00	4.75

Structural component only
DWG# T-2311491

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	1854	0	1854	0	0	5-8	2-8
N	1849	0	1849	0	0	5-8	2-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1309	873 / 0	0 / 0	0 / 0	0 / 0	437 / 0	0 / 0
N	1305	870 / 0	0 / 0	0 / 0	0 / 0	436 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	1	0.12 (1)	10.00
B- C	-1713 / 0	-91.8	-91.8	1	0.15 (1)	4.94
C- D	-4533 / 0	-91.8	-91.8	1	0.47 (1)	2.94
D- E	-5242 / 0	-91.8	-91.8	1	0.58 (1)	2.64
E- F	-2755 / 0	-91.8	-91.8	1	0.33 (1)	3.89
F- G	-2562 / 0	-91.8	-91.8	1	0.31 (1)	4.02
G- H	-2562 / 0	-91.8	-91.8	1	0.31 (1)	4.02
H- I	-2570 / 0	-91.8	-91.8	1	0.12 (1)	4.22
I- J	-2506 / 0	-91.8	-91.8	1	0.53 (1)	3.80
J- K	-1987 / 0	-91.8	-91.8	1	0.10 (1)	4.72
K- L	-2037 / 0	-91.8	-91.8	1	0.13 (1)	4.64
L- M	0 / 28	-91.8	-91.8	1	0.12 (1)	10.00
Y- B	-1882 / 0	0.0	0.0	1	0.19 (1)	6.12
N- L	-1828 / 0	0.0	0.0	1	0.18 (1)	6.20
Y- X	0 / 0	-18.5	-18.5	1	0.08 (1)	10.00
X- W	0 / 1453	-18.5	-18.5	1	0.32 (1)	10.00
W- V	0 / 3212	-18.5	-18.5	1	0.68 (1)	10.00
V- U	0 / 2462	-18.5	-18.5	1	0.54 (1)	10.00
U- T	0 / 2462	-18.5	-18.5	1	0.54 (1)	10.00
T- S	0 / 2310	-18.5	-18.5	1	0.41 (1)	10.00
S- R	0 / 2529	-18.5	-18.5	1	0.42 (1)	10.00
R- Q	0 / 1394	-18.5	-18.5	1	0.31 (1)	10.00
Q- P	0 / 51	-18.5	-18.5	1	0.10 (4)	10.00
P- O	-19 / 0	0.0	0.0	1	0.10 (1)	7.81
O- K	-161 / 0	0.0	0.0	1	0.10 (1)	7.81
O- N	0 / 0	-18.5	-18.5	1	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- C	-754 / 0	0.11 (1)
W- D	0 / 3360	0.76 (1)
W- E	-2643 / 0	0.38 (1)
V- E	0 / 2012	0.45 (1)
V- F	-1008 / 0	0.46 (1)
F- T	0 / 813	0.18 (1)
T- G	0 / 167	0.04 (1)
T- H	-433 / 0	0.21 (1)
S- H	0 / 420	0.09 (1)
S- I	0 / 580	0.13 (1)
R- I	-558 / 0	0.21 (1)
R- J	-957 / 0	0.31 (1)
Q- J	0 / 1490	0.34 (1)
Q- P	-1006 / 0	0.32 (1)
B- X	0 / 1738	0.39 (1)
O- L	0 / 1946	0.44 (1)
Q- O	0 / 1767	0.28 (1)
J- O	0 / 693	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 712 (0.53")

CSI: TC=0.58/1.00 (D-E:1) , BC=0.68/1.00 (V-W:1) , WB=0.76/1.00 (C-W:1) , SSI=0.18/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (D) (INPUT = 0.90)
JSI METAL = 0.71 (D) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	T42S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTIoWxxt54GFza3n1-ltn2MWiA777Y1UsAhe9od8m_MJIcmxxDoBgb8Byx665

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Y	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311491

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T43S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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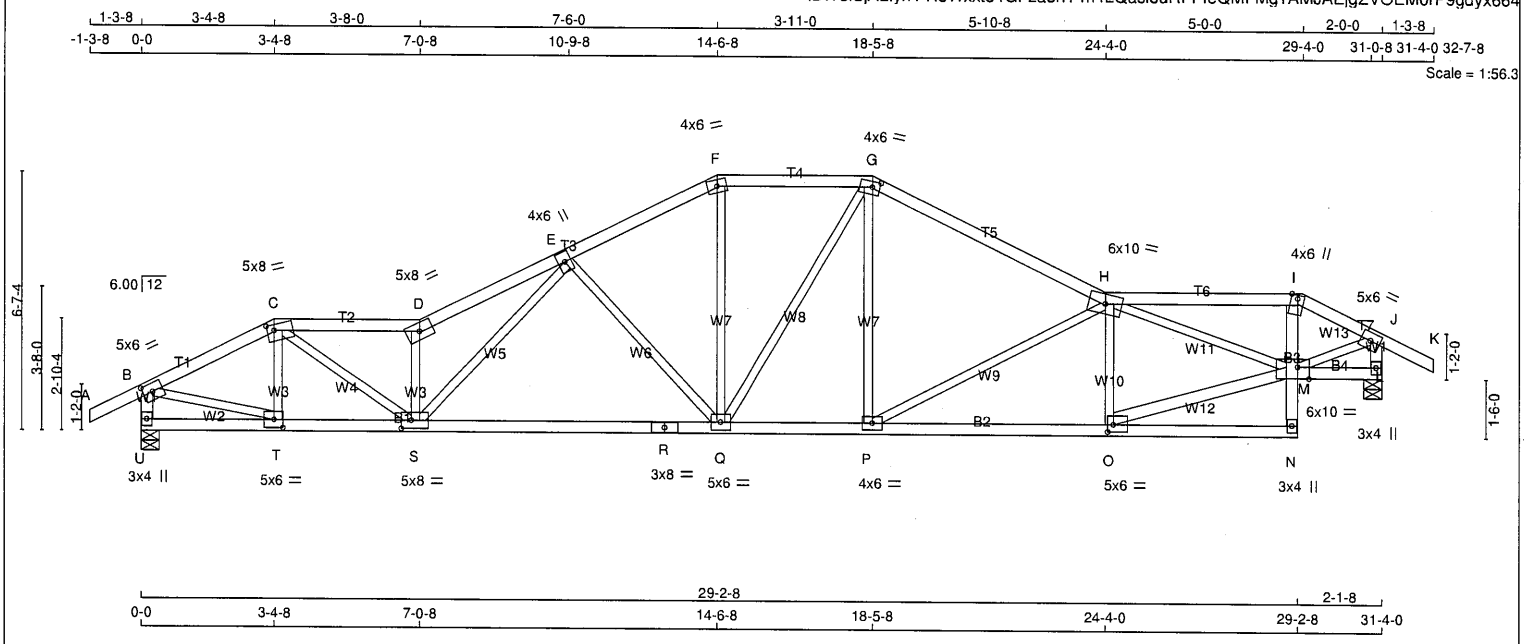

TOTAL WEIGHT = 136 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TTWW-m	MT20	5.0	8.0	1.75	2.25
D	TTW-h	MT20	5.0	8.0		
E	TMWW+t	MT20	4.0	6.0		
F	TTW-m	MT20	4.0	6.0		
G	TTWW-m	MT20	4.0	6.0	1.75	2.25
H	TTWWW-m	MT20	6.0	10.0		
I	TTV+m	MT20	4.0	6.0	Edge	
J	TMVW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		
M	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
N	BMV+p	MT20	3.0	4.0		
O	BMWW-t	MT20	5.0	6.0	2.25	1.75
P	BMWW-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	3.0	8.0		
S	BMWWW-t	MT20	5.0	8.0	2.50	3.00
T	BMWW-t	MT20	5.0	6.0	2.50	2.75
U	BMV1+p	MT20	3.0	4.0		

Structural component only
DWG# T-2311492

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	1852	0	1852	0	0	5-8	2-8
L	1852	0	1852	0	0	5-8	2-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1307	871 / 0	0 / 0	0 / 0	0 / 0	436 / 0	0 / 0
L	1307	871 / 0	0 / 0	0 / 0	0 / 0	436 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-2306 / 0	-91.8	-91.8	0.20 (1)	4.36
C- D	-3652 / 0	-91.8	-91.8	0.31 (1)	3.45
D- E	-4159 / 0	-91.8	-91.8	0.37 (1)	3.20
E- F	-2408 / 0	-91.8	-91.8	0.24 (1)	4.24
F- G	-2148 / 0	-91.8	-91.8	0.23 (1)	4.44
G- H	-2320 / 0	-91.8	-91.8	0.51 (1)	3.99
H- I	-1865 / 0	-91.8	-91.8	0.33 (1)	4.59
I- J	-2066 / 0	-91.8	-91.8	0.10 (1)	4.66
J- K	0 / 28	-91.8	-91.8	0.12 (1)	10.00
U- B	-1823 / 0	0.0	0.0	0.18 (1)	6.20
L- J	-1831 / 0	0.0	0.0	0.19 (1)	6.19
U- T	0 / 0	-18.5	-18.5	0.06 (4)	10.00
T- S	0 / 2042	-18.5	-18.5	0.35 (1)	10.00
S- R	0 / 2799	-18.5	-18.5	0.57 (1)	10.00
R- Q	0 / 2799	-18.5	-18.5	0.57 (1)	10.00
Q- P	0 / 2083	-18.5	-18.5	0.46 (1)	10.00
P- O	0 / 2798	-18.5	-18.5	0.52 (1)	10.00
O- N	0 / 47	-18.5	-18.5	0.13 (4)	10.00
N- M	0 / 35	0.0	0.0	0.10 (1)	10.00
M- I	0 / 598	0.0	0.0	0.19 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-442 / 0	0.08 (1)
C- S	0 / 1981	0.45 (1)
S- D	-2156 / 0	0.37 (1)
S- E	0 / 1444	0.32 (1)
E- Q	-989 / 0	0.60 (1)
Q- F	0 / 757	0.17 (1)
Q- G	0 / 126	0.03 (1)
P- G	0 / 480	0.11 (1)
P- H	-830 / 0	0.75 (1)
O- H	-777 / 0	0.17 (1)
B- T	0 / 2122	0.48 (1)
O- M	0 / 2867	0.46 (1)
H- M	-978 / 0	0.40 (1)
M- J	0 / 1983	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.04")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.04")
CALCULATED VERT. DEFL.(TL) = L/ 931 (0.40")

CSI: TC=0.51/1.00 (G-H:1) , BC=0.57/1.00 (Q-S:1) , WB=0.75/1.00 (H-P:1) , SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.80 (R) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T43S	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311492

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T44	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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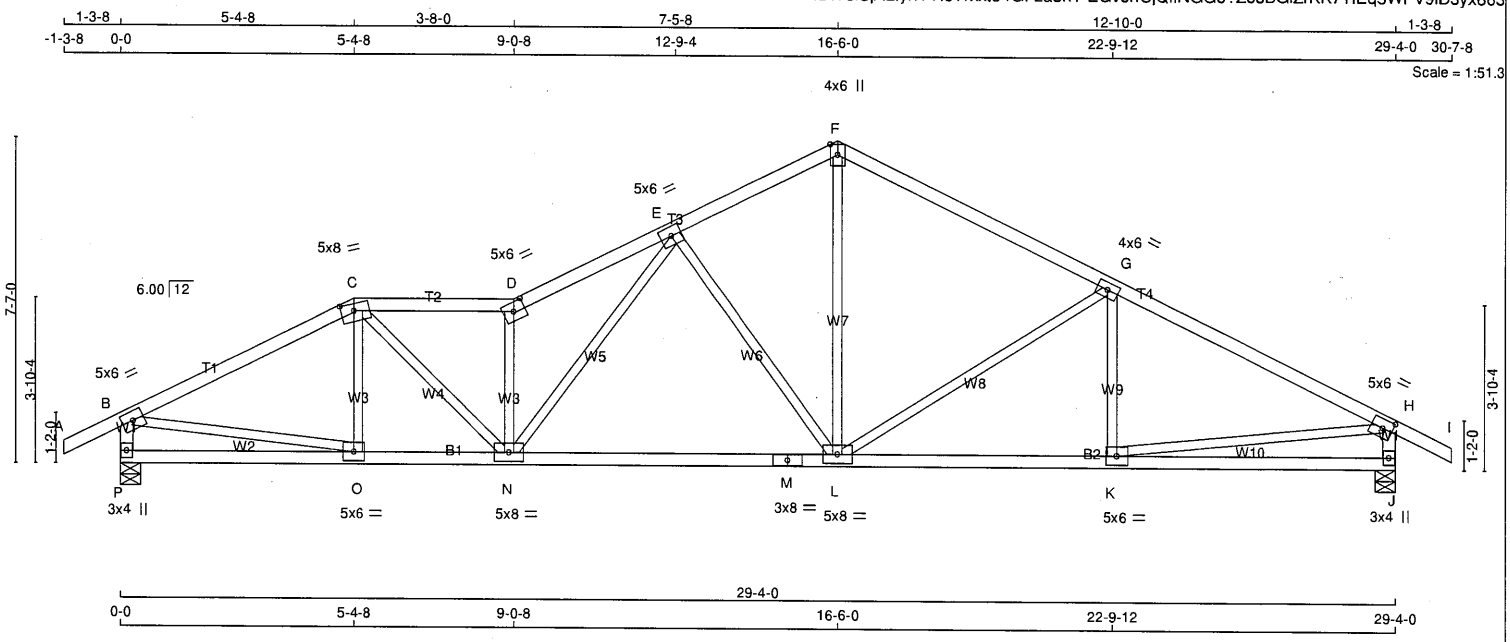

TOTAL WEIGHT = 3 X 119 = 357 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.50
D	TTW-h	MT20	5.0	6.0	2.50	3.25
E	TMWW-t	MT20	5.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1742	0	1742	0	0	5-8	2-3
J	1742	0	1742	0	0	5-8	2-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1229	820 / 0	0 / 0	0 / 0	0 / 0	409 / 0	0 / 0
J	1229	820 / 0	0 / 0	0 / 0	0 / 0	409 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	O- C	-226 / 0	0.05 (1)
B- C	-2253 / 0	-91.8	-91.8	0.43 (1)	4.14	C- N	0 / 1163	0.26 (1)
C- D	-2831 / 0	-91.8	-91.8	0.25 (1)	3.94	N- D	-1727 / 0	0.40 (1)
D- E	-3199 / 0	-91.8	-91.8	0.28 (1)	3.70	N- E	0 / 1215	0.27 (1)
E- F	-1826 / 0	-91.8	-91.8	0.20 (1)	4.78	E- L	-979 / 0	0.78 (1)
F- G	-1840 / 0	-91.8	-91.8	0.55 (1)	4.32	L- F	0 / 1263	0.28 (1)
G- H	-2331 / 0	-91.8	-91.8	0.62 (1)	3.85	L- G	-591 / 0	0.67 (1)
H- I	0 / 28	-91.8	-91.8	0.12 (1)	10.00	K- G	-158 / 48	0.05 (1)
P- B	-1697 / 0	0.0	0.0	0.17 (1)	6.38	B- O	0 / 2038	0.46 (1)
J- H	-1689 / 0	0.0	0.0	0.17 (1)	6.39	K- H	0 / 2130	0.48 (1)
P- O	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
O- N	0 / 2007	-18.5	-18.5	0.36 (1)	10.00			
N- M	0 / 2187	-18.5	-18.5	0.47 (1)	10.00			
M- L	0 / 2187	-18.5	-18.5	0.47 (1)	10.00			
L- K	0 / 2112	-18.5	-18.5	0.46 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.62/1.00 (G-H:1) , BC=0.47/1.00 (L-N:1) , WB=0.78/1.00 (E-L:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 0.95)

Structural component only
DWG# T-2311493

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T45	1	3	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:44 2023 Page 1
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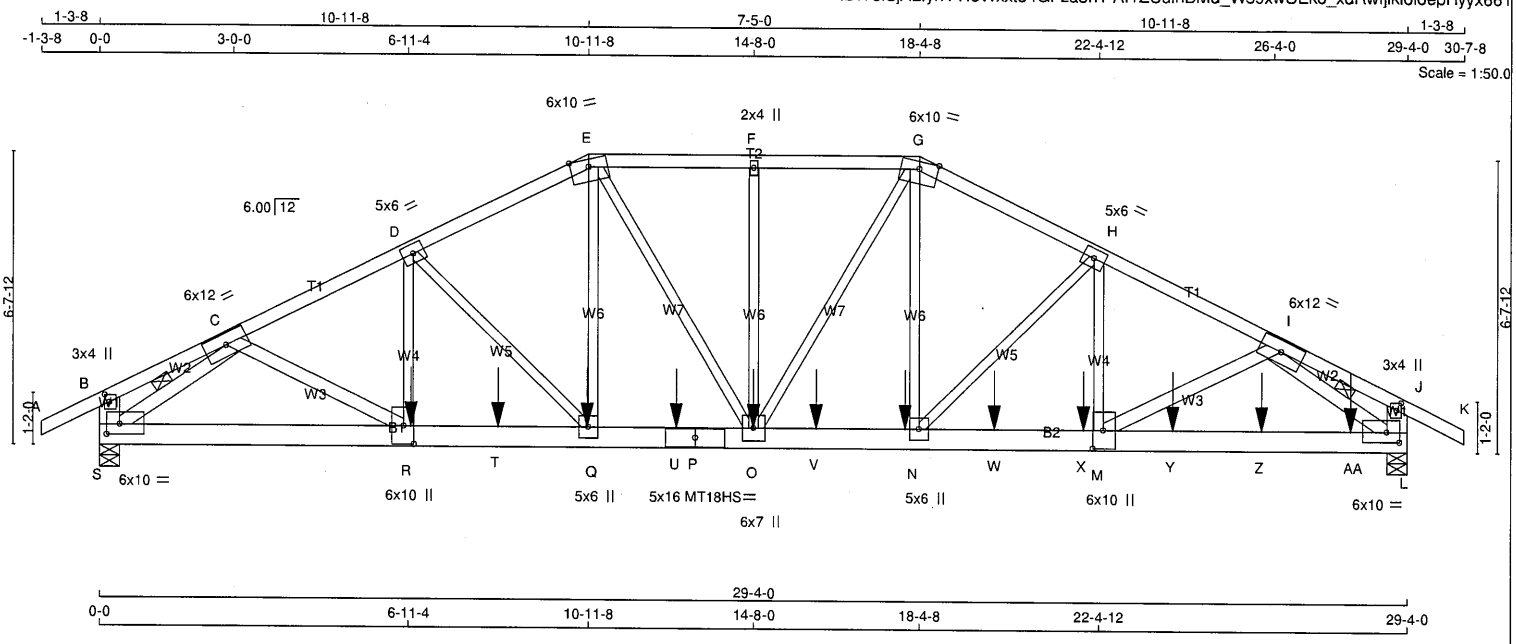

TOTAL WEIGHT = 3 X 153 = 458 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - R	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- G	1	12	TOP
G- K	1	12	TOP
S- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	5	SIDE(890.1)
P- L	2	5	SIDE(890.1)
WEBS : (0.122"X3") SPIRAL NAILS			
D- R	1	5	SIDE(393.8)
2x3	1	6	
H- M	1	5	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2311494

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	14408	0	14408	0	0	5-8	5-3
S	12113	0	12113	0	0	5-8	4-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	10173	6769 / 0	0 / 0	0 / 0	0 / 0	3403 / 0	0 / 0
S	8549	5706 / 0	0 / 0	0 / 0	0 / 0	2843 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-S, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.04 (1)	10.00
B- C	-9 / 0	-91.8	-91.8	0.06 (1)	10.00
C- D	-21127 / 0	-91.8	-91.8	0.75 (1)	2.13
D- E	-18513 / 0	-91.8	-91.8	0.58 (1)	2.48
E- F	-17719 / 0	-91.8	-91.8	0.47 (1)	2.59
F- G	-17719 / 0	-91.8	-91.8	0.47 (1)	2.59
G- H	-18215 / 0	-91.8	-91.8	0.56 (1)	2.51
H- I	-21348 / 0	-91.8	-91.8	0.77 (1)	2.11
I- J	-10 / 0	-91.8	-91.8	0.06 (1)	10.00
J- K	0 / 28	-91.8	-91.8	0.04 (1)	10.00
S- B	-278 / 0	0.0	0.0	0.01 (1)	7.81
L- J	-279 / 0	0.0	0.0	0.01 (1)	7.81
S- R	0 / 15291	-18.5	-18.5	0.29 (1)	10.00
R- T	0 / 18909	-18.5	-18.5	0.49 (1)	10.00
T- Q	0 / 18909	-18.5	-18.5	0.49 (1)	10.00
Q- U	0 / 16640	-18.5	-18.5	0.37 (1)	10.00
U- P	0 / 16640	-18.5	-18.5	0.37 (1)	10.00
P- O	0 / 16640	-18.5	-18.5	0.37 (1)	10.00
O- V	0 / 16364	-18.5	-18.5	0.37 (1)	10.00
V- N	0 / 16364	-18.5	-18.5	0.37 (1)	10.00
N- W	0 / 19106	-18.5	-18.5	0.67 (1)	10.00
W- X	0 / 19106	-18.5	-18.5	0.67 (1)	10.00
X- M	0 / 19106	-18.5	-18.5	0.67 (1)	10.00
M- Y	0 / 15446	-18.5	-18.5	0.73 (1)	10.00
Y- Z	0 / 15446	-18.5	-18.5	0.73 (1)	10.00
Z-AA	0 / 15446	-18.5	-18.5	0.73 (1)	10.00
AA- L	0 / 15446	-18.5	-18.5	0.73 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- D	0 / 3174	0.24 (1)
D- Q	-3401 / 0	0.67 (1)
Q- E	0 / 6117	0.46 (1)
E- O	0 / 2117	0.16 (1)
O- F	-285 / 0	0.06 (1)
O- G	0 / 2659	0.20 (1)
N- G	0 / 5517	0.41 (1)
N- H	-4066 / 0	0.80 (1)
M- H	0 / 3881	0.29 (1)
C- R	0 / 4133	0.22 (1)
S- C	-19266 / 0	0.73 (1)
M- I	0 / 4182	0.22 (1)
I- L	-19462 / 0	0.74 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	18-0-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
O	14-8-0	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
Q	10-11-4	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
R	7-0-0	-3204	-3204	---	BACK	VERT	TOTAL	---	C1
T	8-11-4	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
U	12-11-4	-1187	-1187	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 866 (0.41")

CSI: TC=0.77/1.00 (H-I:1) , BC=0.73/1.00 (L-M:1) , WB=0.80/1.00 (H-N:1) , SSI=0.61/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.85 (I) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	T45	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C	TMWW-t	MT20	6.0	12.0		
D	TMWW-t	MT20	5.0	6.0		
E	TTWW-m	MT20	6.0	10.0	Edge	
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	6.0	10.0	Edge	
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	6.0	12.0		
J	TMV+p	MT20	3.0	4.0	2.00	1.25
L	BMVW1-t	MT20	6.0	10.0	2.75	3.50
M	BMWW+t	MT20	6.0	10.0	5.00	2.75
N	BMWW+t	MT20	5.0	6.0		
O	BMWWW+t	MT20	6.0	7.0		
P	BS-t	MT18HS	5.0	16.0		
Q	BMWW+t	MT20	5.0	6.0		
R	BMWW+t	MT20	6.0	10.0	5.00	2.75
S	BMVW1-t	MT20	6.0	10.0	2.75	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	16-0-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
W	20-0-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
X	22-0-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
Y	24-0-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
Z	26-0-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1
AA	28-0-12	-1187	-1187	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311494

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T46	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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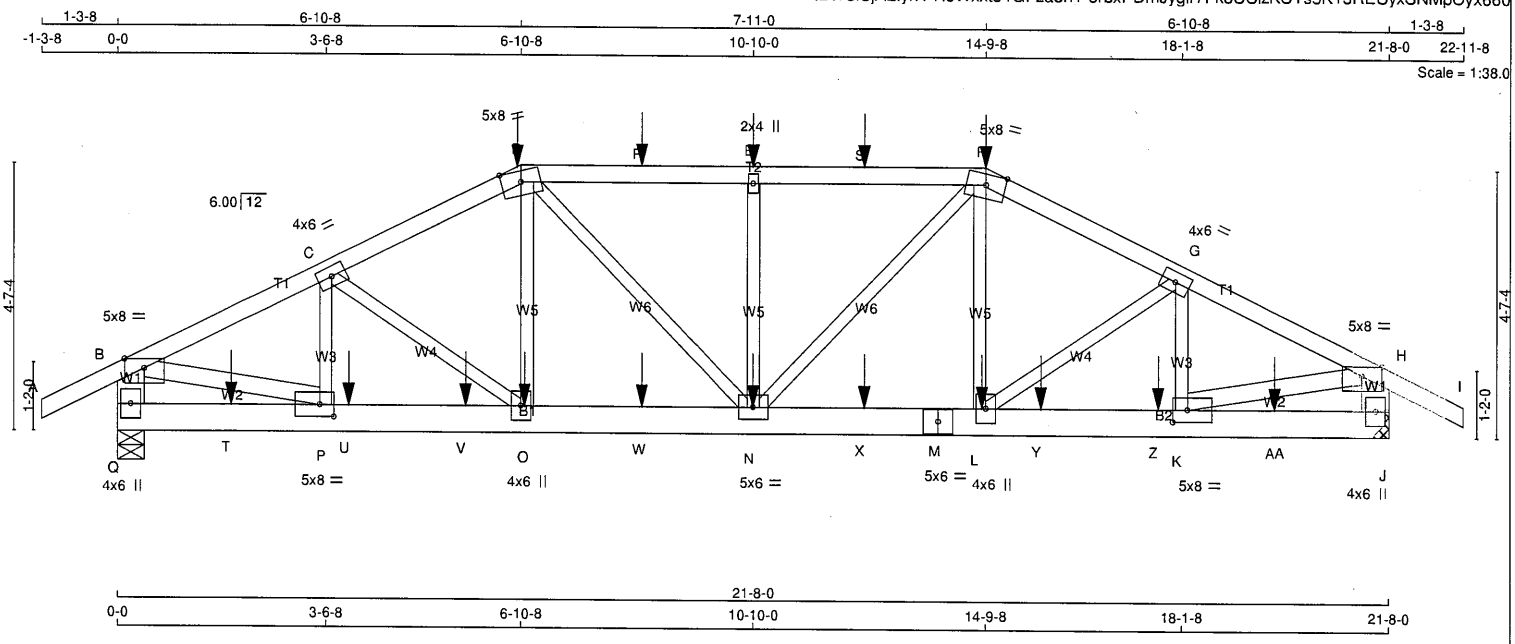

TOTAL WEIGHT = 106 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	8.0	2.50	3.00
L	BMWW+t	MT20	4.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.50	3.00
Q	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311495

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2755	0	2755	0	0	5-8	3-7
J	2755	0	2755	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-7.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1947	1286 / 0	0 / 0	0 / 0	0 / 0	661 / 0	0 / 0
J	1947	1286 / 0	0 / 0	0 / 0	0 / 0	661 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	P- C	-628 / 0	0.11 (1)
B- C	-3749 / 0	-91.8	-91.8	0.37 (1)	3.31	O- C	0 / 249	0.06 (1)
C- D	-4001 / 0	-91.8	-91.8	0.38 (1)	3.19	O- D	0 / 510	0.13 (1)
D- R	-4049 / 0	-91.8	-91.8	0.52 (1)	3.00	D- N	0 / 691	0.17 (1)
R- E	-4049 / 0	-91.8	-91.8	0.52 (1)	3.00	N- E	-500 / 0	0.15 (1)
E- S	-4049 / 0	-91.8	-91.8	0.52 (1)	3.00	N- F	0 / 691	0.17 (1)
S- F	-4049 / 0	-91.8	-91.8	0.52 (1)	3.00	L- F	0 / 510	0.13 (1)
F- G	-4001 / 0	-91.8	-91.8	0.38 (1)	3.19	L- G	0 / 249	0.06 (1)
G- H	-3749 / 0	-91.8	-91.8	0.37 (1)	3.31	K- G	-628 / 0	0.11 (1)
H- I	0 / 28	-91.8	-91.8	0.13 (1)	10.00	B- P	0 / 3443	0.61 (1)
Q- B	-2678 / 0	0.0	0.0	0.19 (1)	6.32	K- H	0 / 3443	0.61 (1)
J- H	-2678 / 0	0.0	0.0	0.19 (1)	6.32			
Q- T	0 / 0	-18.5	-18.5	0.08 (1)	10.00			
T- P	0 / 0	-18.5	-18.5	0.08 (1)	10.00			
P- U	0 / 3364	-18.5	-18.5	0.53 (1)	10.00			
U- V	0 / 3364	-18.5	-18.5	0.53 (1)	10.00			
V- O	0 / 3364	-18.5	-18.5	0.53 (1)	10.00			
O- W	0 / 3576	-18.5	-18.5	0.60 (1)	10.00			
W- N	0 / 3576	-18.5	-18.5	0.60 (1)	10.00			
N- X	0 / 3576	-18.5	-18.5	0.60 (1)	10.00			
X- M	0 / 3576	-18.5	-18.5	0.60 (1)	10.00			
M- L	0 / 3576	-18.5	-18.5	0.60 (1)	10.00			
L- Y	0 / 3364	-18.5	-18.5	0.53 (1)	10.00			
Y- Z	0 / 3364	-18.5	-18.5	0.53 (1)	10.00			
Z- K	0 / 3364	-18.5	-18.5	0.53 (1)	10.00			
K-AA	0 / 0	-18.5	-18.5	0.08 (1)	10.00			
AA- J	0 / 0	-18.5	-18.5	0.08 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-10-8	-337	-337	---	FRONT	VERT	TOTAL	---	C1
E	10-10-0	-23	-23	---	FRONT	VERT	TOTAL	---	C1
F	14-9-8	-337	-337	---	FRONT	VERT	TOTAL	---	C1
L	14-8-12	-169	-169	---	FRONT	VERT	TOTAL	---	C1
N	10-10-0	-169	-169	---	FRONT	VERT	TOTAL	---	C1
O	6-11-4	-169	-169	---	FRONT	VERT	TOTAL	---	C1
R	8-11-4	-23	-23	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.60/1.00 (L-N:1) , WB=0.61/1.00 (B-P:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.61 (M) (INPUT = 0.95)

CONTINUED ON PAGE 2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	12-8-12	-23	-23	---	FRONT	VERT	TOTAL	---	C1
T	1-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
U	3-11-4	-36	-36	---	FRONT	VERT	TOTAL	---	C1
V	5-11-4	-149	-149	---	FRONT	VERT	TOTAL	---	C1
W	8-11-4	-169	-169	---	FRONT	VERT	TOTAL	---	C1
X	12-8-12	-169	-169	---	FRONT	VERT	TOTAL	---	C1
Y	15-8-12	-149	-149	---	FRONT	VERT	TOTAL	---	C1
Z	17-8-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
AA	19-8-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311495

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T47	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:46 2023 Page 1
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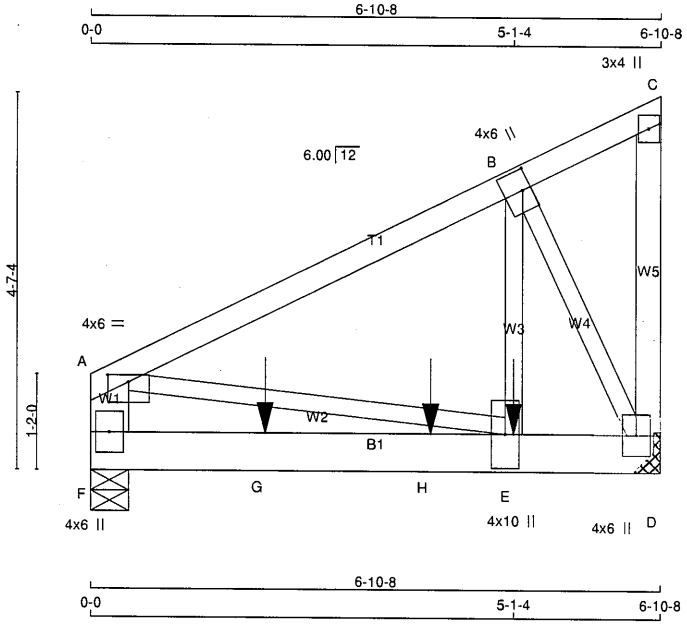

Scale = 1:26.9

TOTAL WEIGHT = 2 X 36 = 71 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	3	SIDE(530.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW+t	MT20	4.0	6.0	3.00	1.25

Structural component only
DWG# T-2311496

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	2734	0	2734	0	0	IN-SX	IN-SX
F	1447	0	1447	0	0	MECHANICAL 5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1925	1311 / 0	0 / 0	0 / 0	0 / 0	614 / 0	0 / 0
F	1016	710 / 0	0 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1653 / 0	-91.8	-91.8	0.18 (1)	6.25	E- B	0 / 3094	0.38 (1)
B- C	-47 / 0	-91.8	-91.8	0.14 (1)	6.25	B- D	-3177 / 0	0.39 (1)
D- C	0 / 24	0.0	0.0	0.00 (1)	10.00	A- E	0 / 1510	0.19 (1)
F- A	-1171 / 0	0.0	0.0	0.04 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
G- H	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
H- E	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
E- D	0 / 1493	-18.5	-18.5	0.14 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	5-1-4	-1932	-1932	---	BACK	VERT	TOTAL	---	C1
G	2-1-4	-205	-205	---	TOP	VERT	TOTAL	---	C1
H	4-1-4	-268	-268	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.20/1.00 (E-F:1) , WB=0.39/1.00 (B-D:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.54 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	T47	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALlyhYTloWxxt54GFza3n1-719JdZmxjztilPJK1vGCtP079kSYAk45A67wMqyx66?

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	10.0		
F	BMV1+p	MT20	4.0	6.0		

NOTES (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311496

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T47A	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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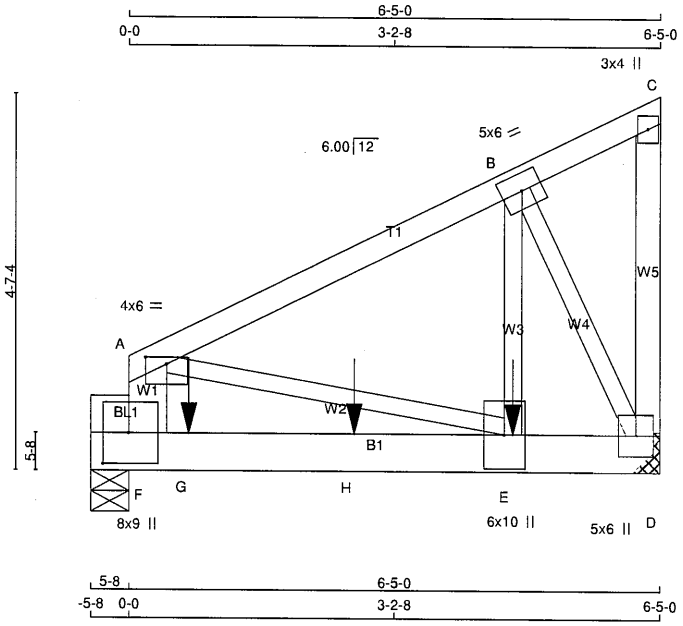

Scale = 1:27.0

TOTAL WEIGHT = 2 X 36 = 72 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(599.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2311497

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	2600	0	2600	0	0	MECHANICAL	
F	1592	0	1592	0	0	5-8	5-0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1830	1252 / 0	0 / 0	0 / 0	0 / 0	578 / 0	0 / 0
F	1117	784 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1483 / 0	-91.8	-91.8	0.15 (1)	6.25	E- B	0 / 2777	0.34 (1)
B- C	-37 / 0	-91.8	-91.8	0.11 (1)	6.25	B- D	-2850 / 0	0.35 (1)
D- C	-3 / 1	0.0	0.0	0.00 (4)	7.81	A- E	0 / 1650	0.20 (1)
F- A	-1231 / 0	0.0	0.0	0.04 (1)	7.81			
F- G	-274 / 0	-18.5	-18.5	0.16 (1)	6.25			
G- H	-274 / 0	-18.5	-18.5	0.16 (1)	6.25			
H- E	-274 / 0	-18.5	-18.5	0.16 (1)	6.25			
E- D	0 / 1339	-18.5	-18.5	0.13 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	4-7-12	-2113	-2113	---	FRONT	VERT	TOTAL	---	C1
G	8-12	-170	-170	---	TOP	VERT	TOTAL	---	C1
H	2-8-12	-167	-167	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.16/1.00 (E-F:1) , WB=0.35/1.00 (B-D:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	T47A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTIoWxxt54GFza3n1-719JdZmxjztilPJK1vGCtP07ekTAAlj5A67wMqyx66?

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	5.0	6.0		
E	BMWW+t	MT20	6.0	10.0		
F	BMVK1+p	MT20	8.0	9.0	4.50	3.75

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311497

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T48	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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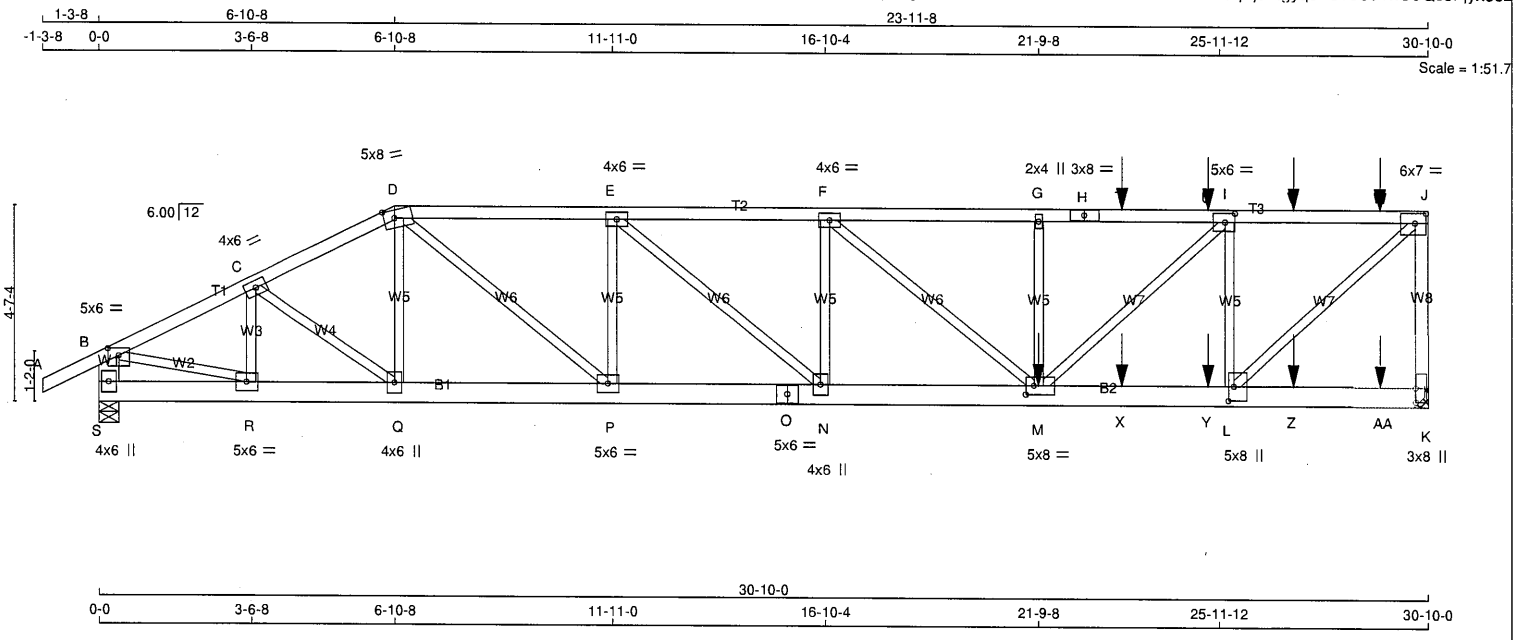

TOTAL WEIGHT = 2 X 146 = 292 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- H	1	12	TOP
H- J	1	12	SIDE(0.0)
J- K	1	12	TOP
S- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- O	2	12	TOP
O- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
G- M	1	4	SIDE(525.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Structural component only
DWG# T-2311498

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	4563	0	4563	0	0	MECHANICAL	
S	2765	0	2765	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-15.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	3219	2158 / 0	0 / 0	0 / 0	0 / 0	1061 / 0	0 / 0
S	1950	1311 / 0	0 / 0	0 / 0	0 / 0	640 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	R- C	-791 / 0	0.07 (1)
B- C	-3763 / 0	-91.8	-91.8	0.13 (1)	4.77	C- Q	0 / 413	0.05 (1)
C- D	-4163 / 0	-91.8	-91.8	0.13 (1)	4.57	Q- D	-68 / 36	0.01 (1)
D- E	-5627 / 0	-91.8	-91.8	0.33 (1)	3.86	L- I	-3331 / 0	0.51 (1)
E- F	-6857 / 0	-91.8	-91.8	0.41 (1)	3.47	L- J	0 / 6096	0.75 (1)
F- G	-7397 / 0	-91.8	-91.8	0.40 (1)	3.38	B- R	0 / 3456	0.43 (1)
G- H	-7397 / 0	-91.8	-91.8	0.42 (1)	3.33	M- G	-418 / 0	0.06 (1)
H- T	-7397 / 0	-91.8	-91.8	0.42 (1)	3.33	M- I	0 / 4015	0.50 (1)
T- U	-7397 / 0	-91.8	-91.8	0.42 (1)	3.33	F- M	0 / 711	0.09 (1)
U- I	-7397 / 0	-91.8	-91.8	0.42 (1)	3.33	D- P	0 / 2505	0.31 (1)
I- V	-4494 / 0	-91.8	-91.8	0.26 (1)	4.31	N- F	-917 / 0	0.14 (1)
V- W	-4494 / 0	-91.8	-91.8	0.26 (1)	4.31	P- E	-1554 / 0	0.24 (1)
W- J	-4494 / 0	-91.8	-91.8	0.26 (1)	4.31	E- N	0 / 1619	0.20 (1)
K- J	-4321 / 0	0.0	0.0	0.69 (1)	5.72			
S- B	-2687 / 0	0.0	0.0	0.09 (1)	7.81			
S- R	0 / 0	-18.5	-18.5	0.06 (1)	10.00			
R- Q	0 / 3376	-18.5	-18.5	0.28 (1)	10.00			
Q- P	0 / 3706	-18.5	-18.5	0.27 (1)	10.00			
P- O	0 / 5627	-18.5	-18.5	0.40 (1)	10.00			
O- N	0 / 5627	-18.5	-18.5	0.40 (1)	10.00			
N- M	0 / 6857	-18.5	-18.5	0.52 (1)	10.00			
M- X	0 / 4494	-18.5	-18.5	0.42 (1)	10.00			
X- Y	0 / 4494	-18.5	-18.5	0.42 (1)	10.00			
Y- L	0 / 4494	-18.5	-18.5	0.42 (1)	10.00			
L- Z	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
Z-AA	0 / 0	-18.5	-18.5	0.10 (1)	10.00			
AA- K	0 / 0	-18.5	-18.5	0.10 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	21-9-8	-1910	-1910	---	FRONT	VERT	TOTAL	---	C1
T	23-8-12	-23	-23	---	FRONT	VERT	TOTAL	---	C1
U	25-8-12	-23	-23	---	FRONT	VERT	TOTAL	---	C1
V	27-8-12	-23	-23	---	FRONT	VERT	TOTAL	---	C1
W	29-8-12	-23	-23	---	FRONT	VERT	TOTAL	---	C1
X	23-8-12	-169	-169	---	FRONT	VERT	TOTAL	---	C1
Y	25-8-12	-169	-169	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.69/1.00 (J-K:1) , BC=0.52/1.00 (M-N:1) , WB=0.75/1.00 (J-L:1) , SSI=0.15/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.81 (L) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T48	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C, E, F						
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.25	3.00
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TMVW-t	MT20	6.0	7.0	2.75	3.00
K	BMV1+p	MT20	3.0	8.0	4.00	
L	BMWW+t	MT20	5.0	8.0	4.00	1.50
M	BMWWW-t	MT20	5.0	8.0	2.50	2.25
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Z	27-8-12	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AA	29-8-12	-169	-169	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2311498

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T49	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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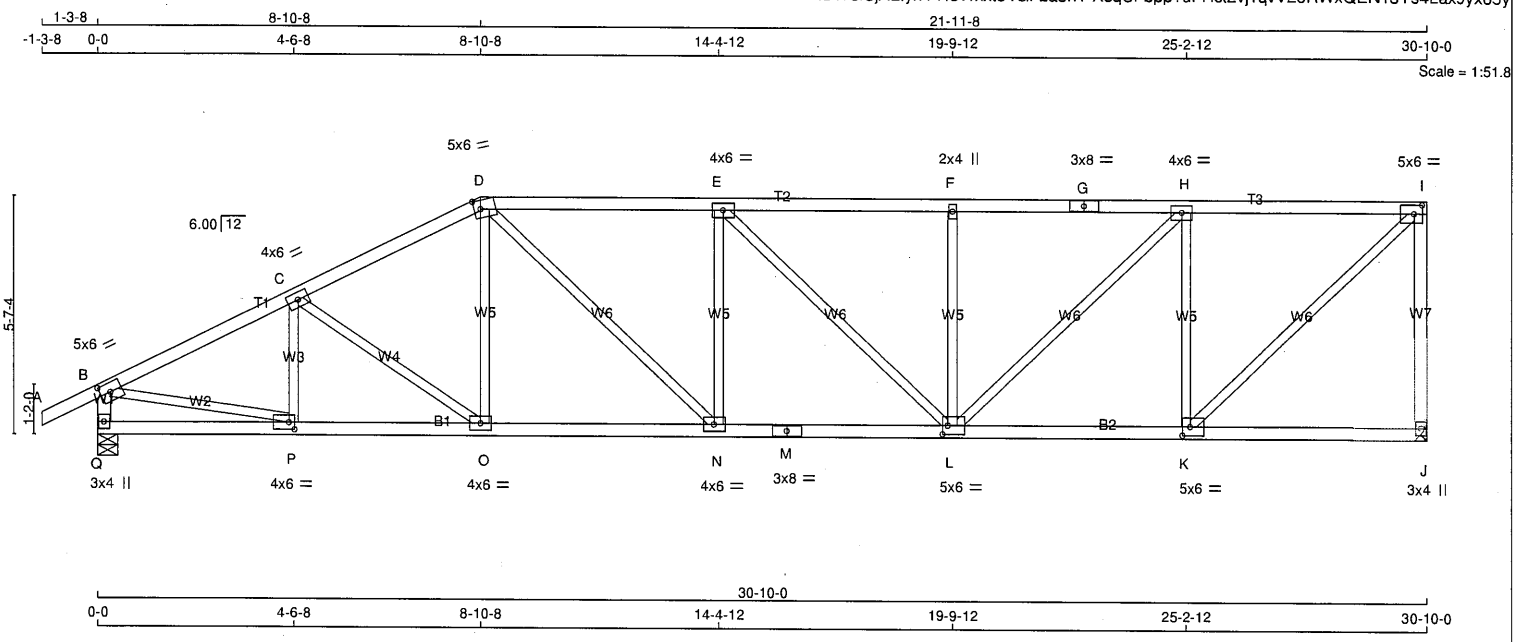

TOTAL WEIGHT = 130 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C, E, H						
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.50	1.75
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
I	TMVW-t	MT20	5.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWWW-t	MT20	5.0	6.0	2.50	1.50
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0	2.00	1.50
Q	BMV1+p	MT20	3.0	4.0		

NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1700	0	1700	0	0	MECHANICAL	
Q	1824	0	1824	0	0	5-8	2-7

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0
Q	1288	858 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P- C	-332 / 0	0.07 (1)
B- C	-2406 / 0	-91.8	-91.8	0.39 (1)	4.06	C- O	-171 / 0	0.08 (1)
C- D	-2292 / 0	-91.8	-91.8	0.37 (1)	4.16	O- D	0 / 197	0.05 (4)
D- E	-2516 / 0	-91.8	-91.8	0.56 (1)	3.79	B- P	0 / 2208	0.50 (1)
E- F	-2304 / 0	-91.8	-91.8	0.53 (1)	3.96	K- I	0 / 2111	0.47 (1)
F- G	-2304 / 0	-91.8	-91.8	0.54 (1)	3.94	D- N	0 / 668	0.15 (1)
G- H	-2304 / 0	-91.8	-91.8	0.54 (1)	3.94	K- H	-1336 / 0	0.63 (1)
H- I	-1532 / 0	-91.8	-91.8	0.48 (1)	4.74	N- E	-361 / 0	0.17 (1)
J- I	-1657 / 0	0.0	0.0	0.94 (1)	6.44	L- H	0 / 1082	0.24 (1)
Q- B	-1784 / 0	0.0	0.0	0.18 (1)	6.26	E- L	-297 / 0	0.34 (1)
						L- F	-456 / 0	0.22 (1)
Q- P	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
P- O	0 / 2170	-18.5	-18.5	0.40 (1)	10.00			
O- N	0 / 2034	-18.5	-18.5	0.38 (1)	10.00			
N- M	0 / 2516	-18.5	-18.5	0.46 (1)	10.00			
M- L	0 / 2516	-18.5	-18.5	0.46 (1)	10.00			
L- K	0 / 1532	-18.5	-18.5	0.32 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.94/1.00 (I-J:1) , BC=0.46/1.00 (L-N:1) , WB=0.63/1.00 (H-K:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.76 (M) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311499

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T50	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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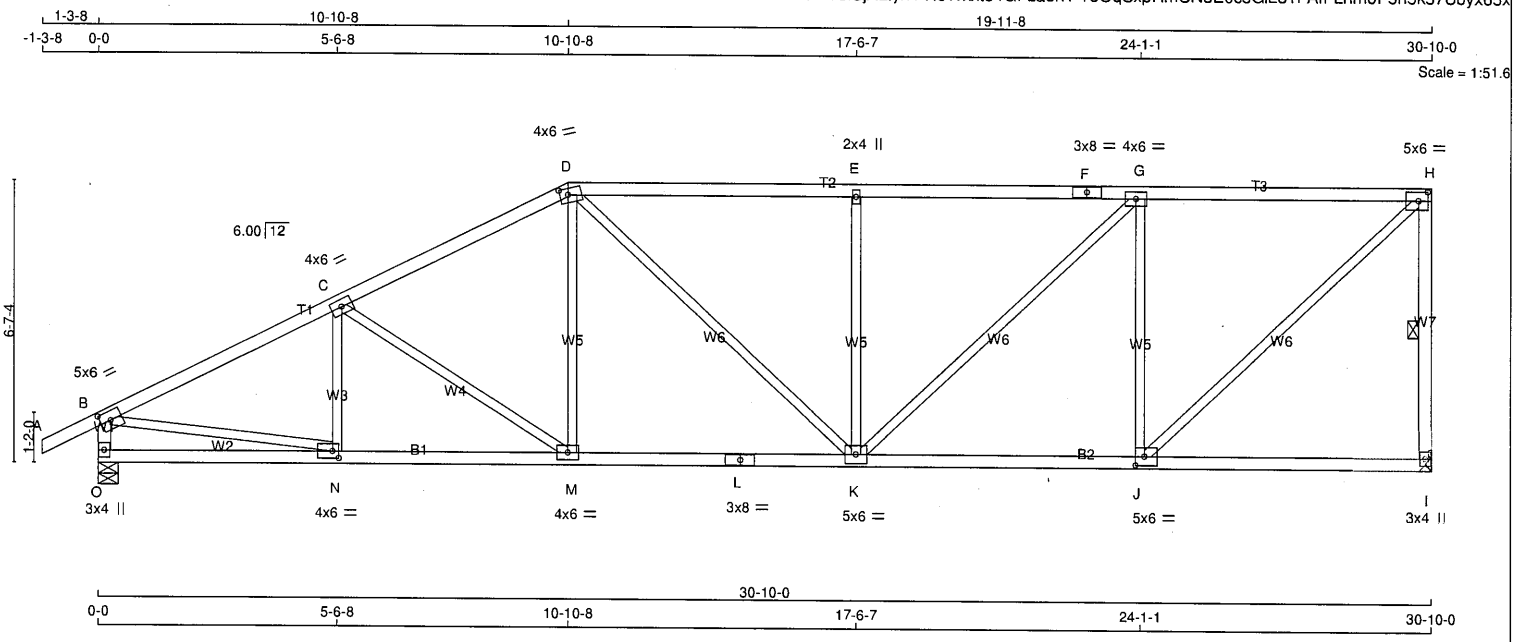

TOTAL WEIGHT = 129 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.50
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.50
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0	2.00	1.75
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311500

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1700	0	1700	0	0	MECHANICAL	
O	1824	0	1824	0	0	5-8	2-7

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0
O	1288	858 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	N- C	-243 / 5	0.06 (1)
B- C	-2461 / 0	-91.8	-91.8	0.63 (1)	3.76	C- M	-386 / 0	0.30 (1)
C- D	-2155 / 0	-91.8	-91.8	0.58 (1)	4.01	M- D	0 / 332	0.07 (1)
D- E	-2106 / 0	-91.8	-91.8	0.73 (1)	3.85	B- N	0 / 2250	0.51 (1)
E- F	-2106 / 0	-91.8	-91.8	0.75 (1)	3.81	J- H	0 / 2047	0.46 (1)
F- G	-2106 / 0	-91.8	-91.8	0.75 (1)	3.81	D- K	0 / 272	0.06 (1)
G- H	-1495 / 0	-91.8	-91.8	0.67 (1)	4.47	J- G	-1260 / 0	0.90 (1)
I- H	-1649 / 0	0.0	0.0	0.33 (1)	5.16	K- E	-659 / 0	0.47 (1)
O- B	-1778 / 0	0.0	0.0	0.18 (1)	6.26	K- G	0 / 848	0.19 (1)
O- N	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
N- M	0 / 2224	-18.5	-18.5	0.44 (1)	10.00			
M- L	0 / 1908	-18.5	-18.5	0.39 (1)	10.00			
L- K	0 / 1908	-18.5	-18.5	0.39 (1)	10.00			
K- J	0 / 1495	-18.5	-18.5	0.35 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.75/1.00 (E-G:1) , BC=0.44/1.00 (M-N:1) , WB=0.90/1.00 (G-J:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.65 (L) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T51	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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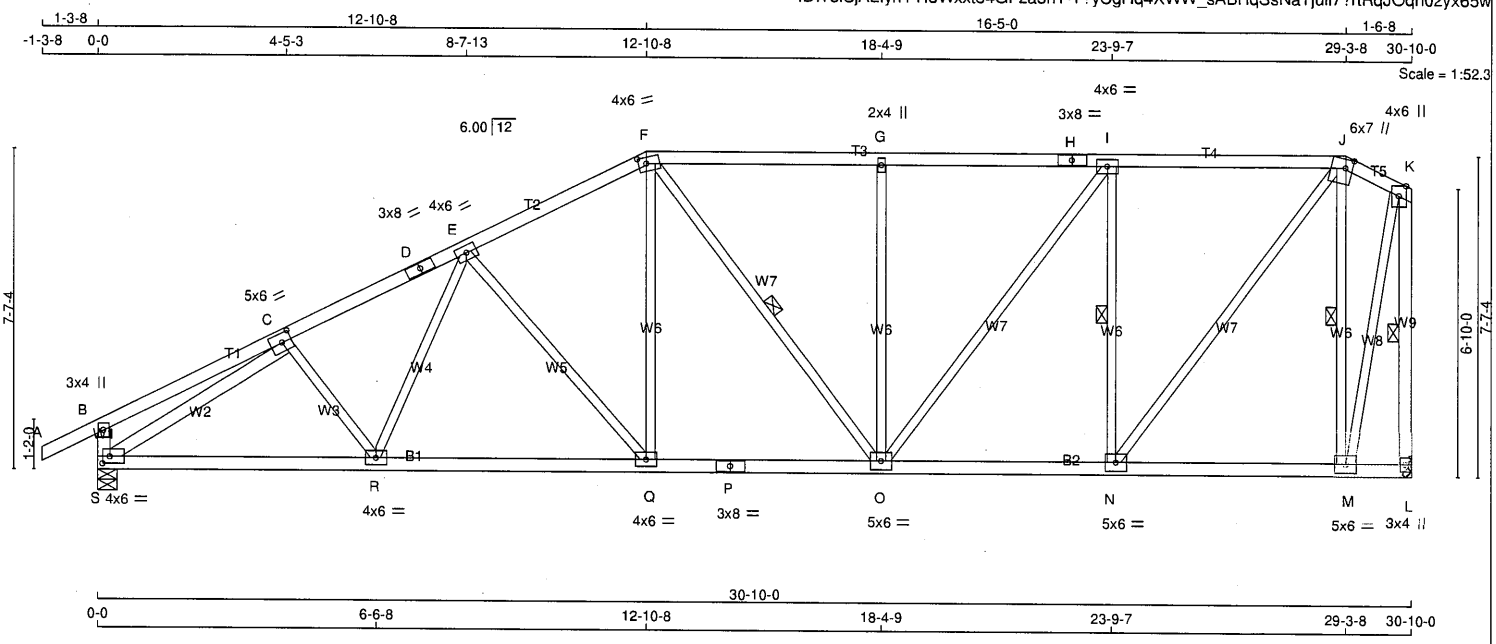

TOTAL WEIGHT = 2 X 150 = 300 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.25
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	7.0	2.50	2.00
K	TMVW+p	MT20	4.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	6.0		
S	BMVW1-t	MT20	4.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311501

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	1824	0	1824	0	0	5-8	2-0
L	1700	0	1700	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1288	858 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0
L	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-O, I-N, J-M, K-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- R	-5 / 58	0.02 (4)
B- C	0 / 15	-91.8	-91.8	0.20 (1)	10.00	R- E	0 / 147	0.04 (4)
C- D	-2371 / 0	-91.8	-91.8	0.32 (1)	4.17	E- Q	-501 / 0	0.43 (1)
D- E	-2371 / 0	-91.8	-91.8	0.32 (1)	4.17	Q- F	0 / 496	0.11 (1)
E- F	-1970 / 0	-91.8	-91.8	0.29 (1)	4.53	F- O	-5 / 21	0.00 (1)
F- G	-1765 / 0	-91.8	-91.8	0.45 (1)	4.52	O- G	-545 / 0	0.58 (1)
G- H	-1765 / 0	-91.8	-91.8	0.45 (1)	4.52	O- I	0 / 771	0.17 (1)
H- I	-1765 / 0	-91.8	-91.8	0.45 (1)	4.52	N- I	-1168 / 0	0.42 (1)
I- J	-1307 / 0	-91.8	-91.8	0.42 (1)	5.12	N- J	0 / 1603	0.36 (1)
J- K	-366 / 0	-91.8	-91.8	0.04 (1)	6.25	M- J	-1392 / 0	0.51 (1)
S- B	-289 / 0	0.0	0.0	0.03 (1)	7.81	S- C	-2592 / 0	0.83 (1)
L- K	-1695 / 0	0.0	0.0	0.37 (1)	5.10	M- K	0 / 1495	0.34 (1)
S- R	0 / 2125	-18.5	-18.5	0.44 (1)	10.00			
R- Q	0 / 2067	-18.5	-18.5	0.42 (1)	10.00			
Q- P	0 / 1753	-18.5	-18.5	0.35 (1)	10.00			
P- O	0 / 1753	-18.5	-18.5	0.35 (1)	10.00			
O- N	0 / 1307	-18.5	-18.5	0.28 (1)	10.00			
N- M	0 / 342	-18.5	-18.5	0.15 (4)	10.00			
M- L	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.45/1.00 (G-I:1) , BC=0.44/1.00 (R-S:1) , WB=0.83/1.00 (C-S:1) , SSI=0.23/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (S) (INPUT = 0.90)
JSI METAL= 0.58 (C) (INPUT = 0.95)

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T52	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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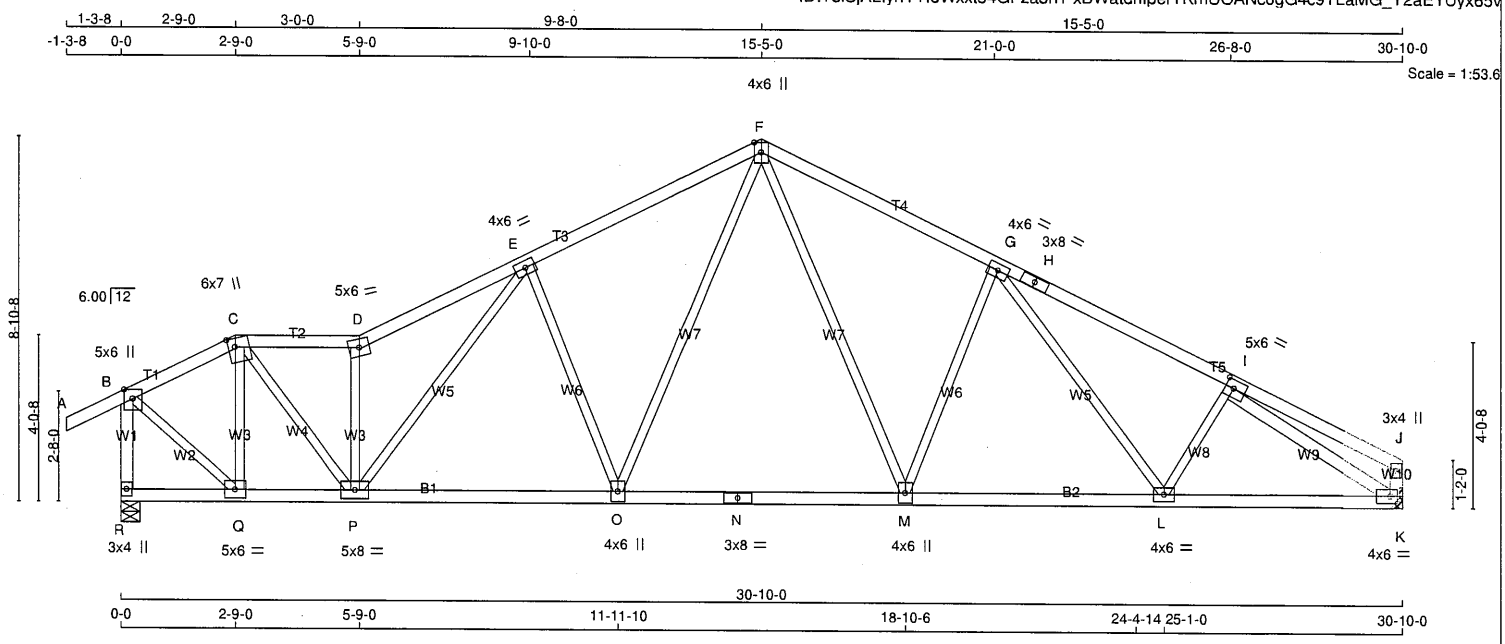

TOTAL WEIGHT = 3 X 134 = 403 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	6.0	7.0	2.50	2.00
D	TTW-m	MT20	5.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTWW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	6.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	6.0	2.00	2.00
L	BMWW-t	MT20	4.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWW+t	MT20	4.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311502

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1824	0	1824	0	0	5-8	2-7
K	1700	0	1700	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-13.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1288	858 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0
K	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	Q- C	-902 / 0	0.22 (1)
B- C	-1309 / 0	-91.8	-91.8	0.11 (1)	5.55	C- P	0 / 1551	0.35 (1)
C- D	-2093 / 0	-91.8	-91.8	0.15 (1)	4.57	P- D	-1314 / 0	0.33 (1)
D- E	-2348 / 0	-91.8	-91.8	0.37 (1)	4.13	P- E	0 / 191	0.04 (1)
E- F	-2040 / 0	-91.8	-91.8	0.41 (1)	4.35	E- O	-600 / 0	0.41 (1)
F- G	-2044 / 0	-91.8	-91.8	0.42 (1)	4.33	O- F	0 / 750	0.17 (1)
G- H	-2355 / 0	-91.8	-91.8	0.44 (1)	4.08	F- M	0 / 752	0.17 (1)
H- I	-2355 / 0	-91.8	-91.8	0.44 (1)	4.08	M- G	-615 / 0	0.42 (1)
I- J	0 / 20	-91.8	-91.8	0.25 (1)	10.00	G- L	0 / 159	0.04 (1)
R- B	-1805 / 0	0.0	0.0	0.25 (1)	6.23	L- I	-18 / 59	0.02 (4)
K- J	-142 / 0	0.0	0.0	0.01 (1)	7.81	B- Q	0 / 1516	0.34 (1)
						I- K	-2608 / 0	0.73 (1)
R- Q	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
Q- P	0 / 1144	-18.5	-18.5	0.21 (1)	10.00			
P- O	0 / 2018	-18.5	-18.5	0.43 (1)	10.00			
O- N	0 / 1525	-18.5	-18.5	0.35 (1)	10.00			
N- M	0 / 1525	-18.5	-18.5	0.35 (1)	10.00			
M- L	0 / 2025	-18.5	-18.5	0.41 (1)	10.00			
L- K	0 / 2125	-18.5	-18.5	0.42 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.44/1.00 (G-I:1) , BC=0.43/1.00 (O-P:1) , WB=0.73/1.00 (I-K:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.59 (I) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T53	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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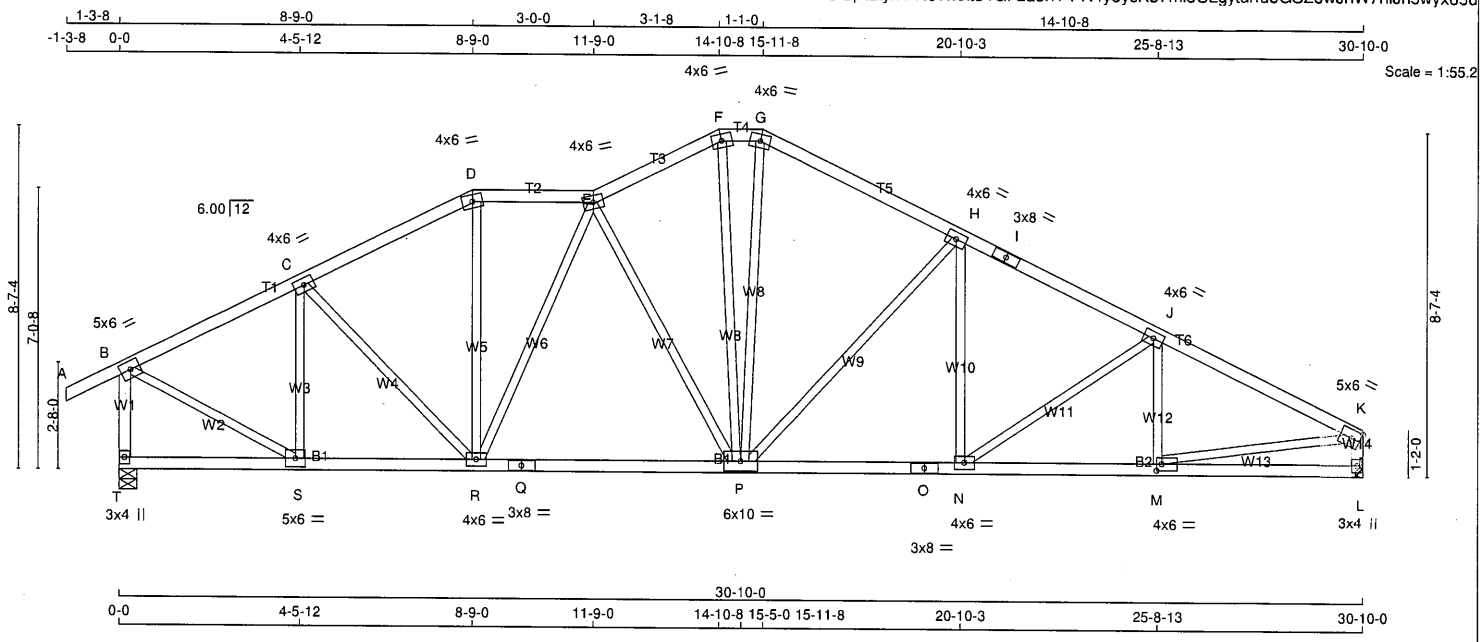

TOTAL WEIGHT = 143 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C, H, J						
C	TMWW-t	MT20	4.0	6.0		
D, F, G						
D	TTW-m	MT20	4.0	6.0		
E	TTWW-m	MT20	4.0	6.0		
I	TS-t	MT20	3.0	8.0		
K	TMVW-t	MT20	5.0	6.0		Edge
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	1.50
N	BMWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMWWWW*-I	MT20	6.0	10.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWWW-t	MT20	4.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311503

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	1824	0	1824	0	0	5-8	2-7
L	1700	0	1700	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1288	858 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0
L	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-1617 / 0	-91.8	-91.8	0.25 (1)	4.96
C- D	-1798 / 0	-91.8	-91.8	0.26 (1)	4.75
D- E	-1596 / 0	-91.8	-91.8	0.13 (1)	5.11
E- F	-1727 / 0	-91.8	-91.8	0.15 (1)	4.94
F- G	-1582 / 0	-91.8	-91.8	0.04 (1)	5.24
G- H	-1752 / 0	-91.8	-91.8	0.29 (1)	4.76
H- I	-2238 / 0	-91.8	-91.8	0.30 (1)	4.31
I- J	-2238 / 0	-91.8	-91.8	0.30 (1)	4.31
J- K	-2431 / 0	-91.8	-91.8	0.37 (1)	4.10
T- B	-1788 / 0	0.0	0.0	0.25 (1)	6.25
L- K	-1657 / 0	0.0	0.0	0.17 (1)	6.44
T- S	0 / 0	-18.5	-18.5	0.08 (4)	10.00
S- R	0 / 1465	-18.5	-18.5	0.30 (1)	10.00
R- Q	0 / 1827	-18.5	-18.5	0.38 (1)	10.00
Q- P	0 / 1827	-18.5	-18.5	0.38 (1)	10.00
P- O	0 / 2002	-18.5	-18.5	0.41 (1)	10.00
O- N	0 / 2002	-18.5	-18.5	0.41 (1)	10.00
N- M	0 / 2191	-18.5	-18.5	0.39 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-689 / 0	0.25 (1)
C- R	0 / 179	0.04 (1)
R- D	0 / 511	0.12 (1)
E- P	-586 / 0	0.68 (1)
N- H	0 / 221	0.05 (4)
N- J	-230 / 0	0.14 (1)
M- J	-272 / 0	0.06 (1)
B- S	0 / 1655	0.37 (1)
M- K	0 / 2222	0.50 (1)
P- G	0 / 546	0.12 (1)
F- P	0 / 579	0.13 (1)
P- H	-666 / 0	0.86 (1)
R- E	-585 / 0	0.62 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (J-K:1) , BC=0.41/1.00 (N-P:1) , WB=0.86/1.00 (H-P:1) , SSI=0.19/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.58 (K) (INPUT = 0.95)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T54	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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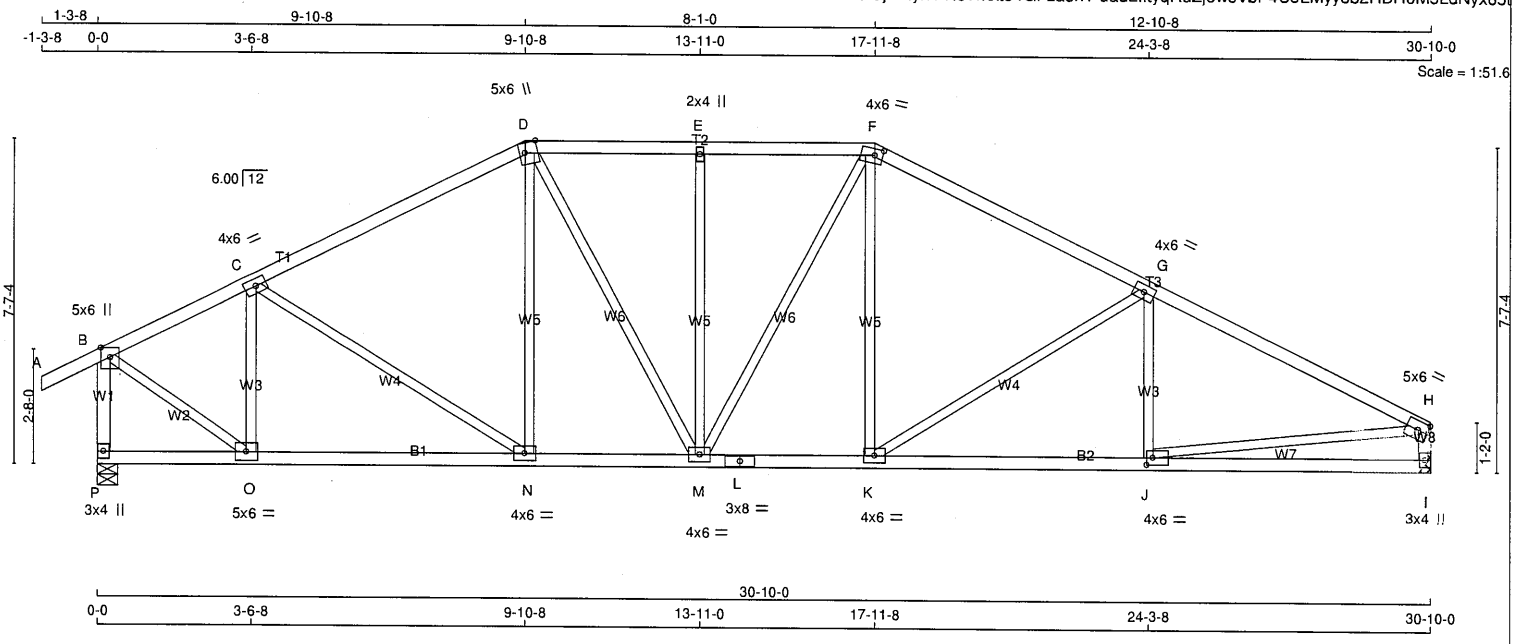

TOTAL WEIGHT = 135 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	Edge	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.25
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		Edge
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	1.75
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1824	0	1824	0	0	5-8	2-7
I	1700	0	1700	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1288	858 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0
I	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	-1503 / 0	-91.8	-91.8	0.38 (1)	4.92
C- D	-1780 / 0	-91.8	-91.8	0.50 (1)	4.47
D- E	-1799 / 0	-91.8	-91.8	0.22 (1)	4.79
E- F	-1799 / 0	-91.8	-91.8	0.22 (1)	4.79
F- G	-1985 / 0	-91.8	-91.8	0.57 (1)	4.17
G- H	-2480 / 0	-91.8	-91.8	0.64 (1)	3.73
P- B	-1803 / 0	0.0	0.0	0.25 (1)	6.23
I- H	-1649 / 0	0.0	0.0	0.17 (1)	6.45
P- O	0 / 0	-18.5	-18.5	0.12 (4)	10.00
O- N	0 / 1382	-18.5	-18.5	0.30 (1)	10.00
N- M	0 / 1570	-18.5	-18.5	0.33 (1)	10.00
M- L	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
L- K	0 / 1752	-18.5	-18.5	0.35 (1)	10.00
K- J	0 / 2246	-18.5	-18.5	0.44 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.18 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-818 / 0	0.24 (1)
C- N	0 / 224	0.05 (1)
N- D	-10 / 74	0.03 (4)
D- M	0 / 472	0.11 (1)
M- E	-448 / 0	0.47 (1)
M- F	0 / 96	0.02 (1)
K- F	0 / 426	0.10 (1)
K- G	-594 / 0	0.70 (1)
J- G	-163 / 54	0.05 (1)
B- O	0 / 1660	0.37 (1)
J- H	0 / 2265	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.64/1.00 (G-H:1) , BC=0.44/1.00 (J-K:1) , WB=0.70/1.00 (G-K:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311504

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T55	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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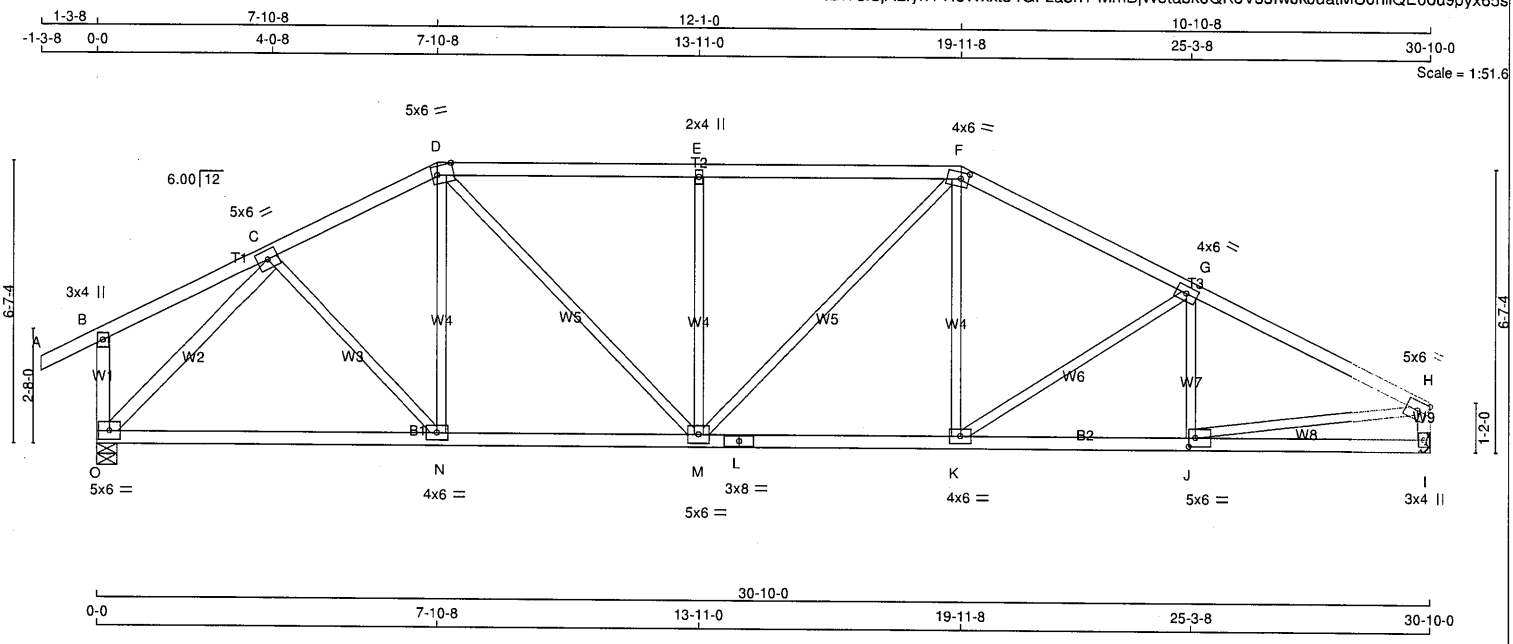

TOTAL WEIGHT = 129 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	Edge	4.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.25
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0		Edge
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0	2.50	1.75
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1824	0	1824	0	0	5-8	2-0
I	1700	0	1700	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1288	858 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0
I	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00
B- C	0 / 19	-91.8	-91.8	0.22 (1)	10.00
C- D	-1810 / 0	-91.8	-91.8	0.21 (1)	4.79
D- E	-2124 / 0	-91.8	-91.8	0.50 (1)	4.13
E- F	-2124 / 0	-91.8	-91.8	0.50 (1)	4.13
F- G	-2154 / 0	-91.8	-91.8	0.41 (1)	4.24
G- H	-2461 / 0	-91.8	-91.8	0.44 (1)	3.99
O- B	-264 / 0	0.0	0.0	0.04 (1)	7.81
I- H	-1654 / 0	0.0	0.0	0.17 (1)	6.45
O- N	0 / 1365	-18.5	-18.5	0.36 (1)	10.00
N- M	0 / 1599	-18.5	-18.5	0.40 (1)	10.00
M- L	0 / 1908	-18.5	-18.5	0.38 (1)	10.00
L- K	0 / 1908	-18.5	-18.5	0.38 (1)	10.00
K- J	0 / 2224	-18.5	-18.5	0.43 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 345	0.08 (1)
N- D	-97 / 48	0.07 (1)
D- M	0 / 759	0.17 (1)
M- E	-682 / 0	0.49 (1)
M- F	0 / 313	0.07 (1)
K- F	0 / 329	0.07 (1)
K- G	-387 / 0	0.30 (1)
J- G	-242 / 6	0.06 (1)
O- C	-2028 / 0	0.82 (1)
J- H	0 / 2250	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.43/1.00 (J-K:1) , WB=0.82/1.00 (C-O:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.59 (H) (INPUT = 0.95)

Structural component only
DWG# T-2311505

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T56	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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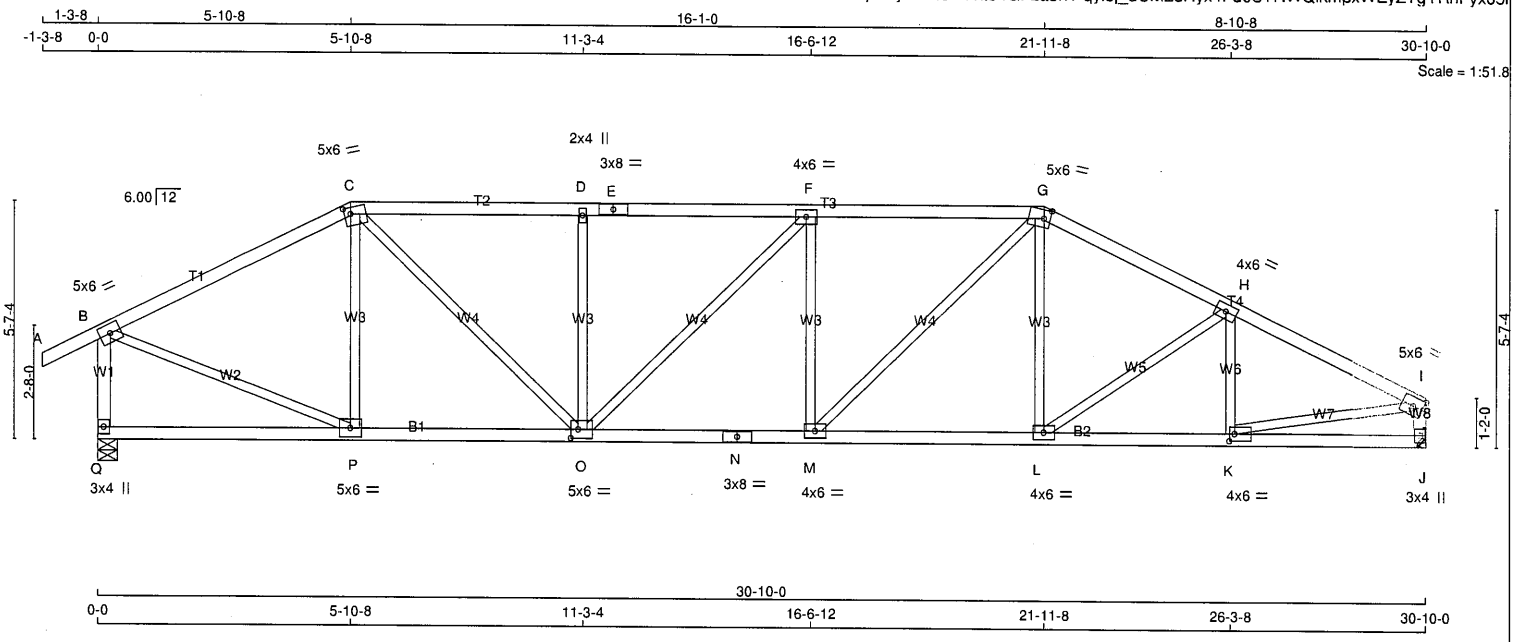

TOTAL WEIGHT = 126 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0		
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.50	1.75
H	TMWW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0		Edge
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	2.00
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1824	0	1824	0	0	5-8	2-7
J	1700	0	1700	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 2-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1288	858 / 0	0 / 0	0 / 0	0 / 0	429 / 0	0 / 0
J	1202	789 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P- C	-488 / 0	0.23 (1)
B- C	-1730 / 0	-91.8	-91.8	0.69 (1)	4.20	C- O	0 / 1125	0.25 (1)
C- D	-2339 / 0	-91.8	-91.8	0.47 (1)	4.02	O- D	-532 / 0	0.25 (1)
D- E	-2339 / 0	-91.8	-91.8	0.47 (1)	4.00	O- F	-238 / 0	0.26 (1)
E- F	-2339 / 0	-91.8	-91.8	0.47 (1)	4.00	M- F	-365 / 0	0.17 (1)
F- G	-2507 / 0	-91.8	-91.8	0.49 (1)	3.88	M- G	0 / 663	0.15 (1)
G- H	-2292 / 0	-91.8	-91.8	0.37 (1)	4.16	L- G	0 / 196	0.04 (4)
H- I	-2407 / 0	-91.8	-91.8	0.39 (1)	4.06	L- H	-172 / 0	0.08 (1)
Q- B	-1779 / 0	0.0	0.0	0.25 (1)	6.26	K- H	-332 / 0	0.07 (1)
J- I	-1659 / 0	0.0	0.0	0.17 (1)	6.44	B- P	0 / 1662	0.37 (1)
						K- I	0 / 2208	0.50 (1)
Q- P	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
P- O	0 / 1537	-18.5	-18.5	0.33 (1)	10.00			
O- N	0 / 2507	-18.5	-18.5	0.45 (1)	10.00			
N- M	0 / 2507	-18.5	-18.5	0.45 (1)	10.00			
M- L	0 / 2034	-18.5	-18.5	0.38 (1)	10.00			
L- K	0 / 2170	-18.5	-18.5	0.40 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.45/1.00 (M-O:1) , WB=0.50/1.00 (I-K:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.77 (N) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311506

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T57	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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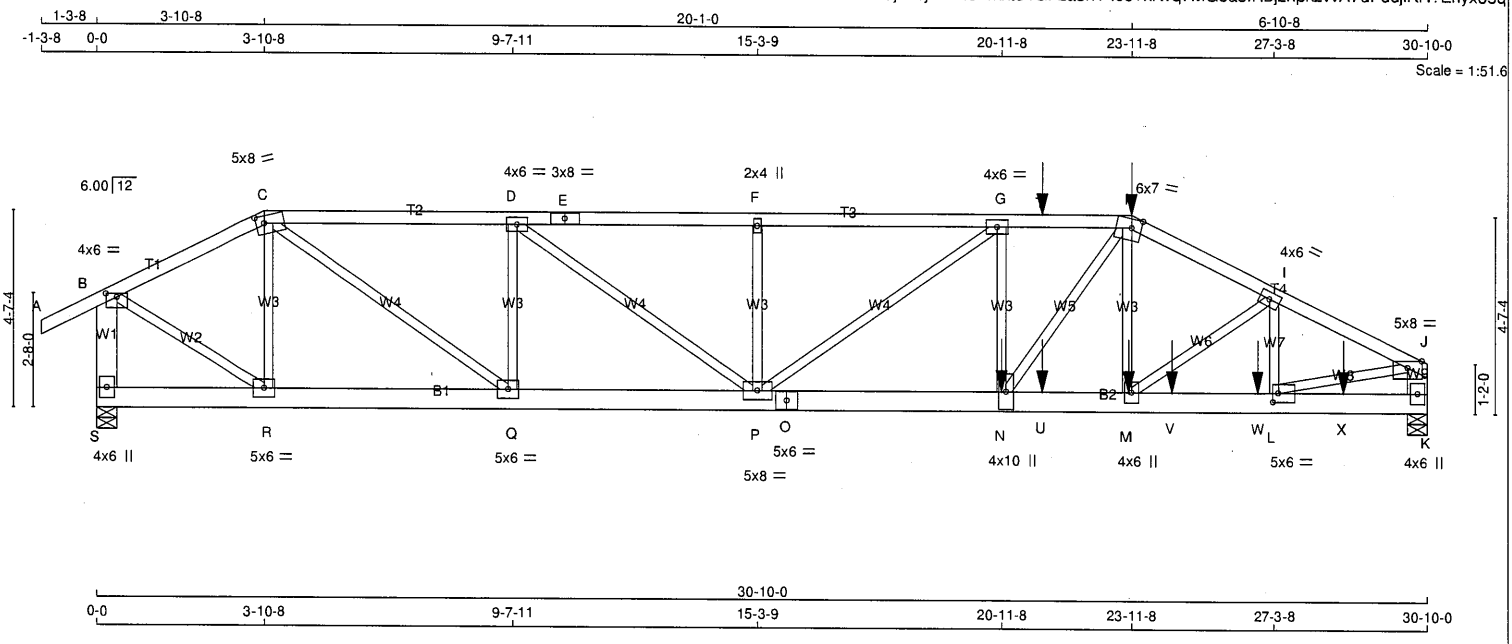

TOTAL WEIGHT = 2 X 145 = 291 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
S- B	2	12	TOP
K- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- O	2	12	TOP
O- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
M- H	1	6	SIDE(9.0)
G- N	1	4	SIDE(490.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

Structural component only
DWG# T-2311507

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2918	0	2918	0	0	5-8	1-9
K	4375	0	4375	0	0	5-8	2-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	MAX./MIN. COMPONENT REACTIONS						
S	2057	1388 / 0	0 / 0	0 / 0	0 / 0	669 / 0	0 / 0
K	3088	2062 / 0	0 / 0	0 / 0	0 / 0	1026 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	
B- C	-2709 / 0	-91.8	-91.8	0.16 (1)	5.38	
C- D	-5215 / 0	-91.8	-91.8	0.40 (1)	3.93	
D- E	-7092 / 0	-91.8	-91.8	0.53 (1)	3.33	
E- F	-7092 / 0	-91.8	-91.8	0.53 (1)	3.33	
F- G	-7092 / 0	-91.8	-91.8	0.46 (1)	3.41	
G- T	-8115 / 0	-91.8	-91.8	0.36 (1)	3.20	
T- H	-8115 / 0	-91.8	-91.8	0.36 (1)	3.20	
H- I	-7225 / 0	-91.8	-91.8	0.26 (1)	3.49	
I- J	-6392 / 0	-91.8	-91.8	0.24 (1)	3.71	
S- B	-2902 / 0	0.0	0.0	0.13 (1)	7.81	
K- J	-4259 / 0	0.0	0.0	0.15 (1)	6.93	
S- R	0 / 0	-18.5	-18.5	0.03 (4)	10.00	
R- Q	0 / 2390	-18.5	-18.5	0.19 (1)	10.00	
Q- P	0 / 5215	-18.5	-18.5	0.38 (1)	10.00	
P- O	0 / 8115	-18.5	-18.5	0.60 (1)	10.00	
O- N	0 / 8115	-18.5	-18.5	0.60 (1)	10.00	
N- U	0 / 6451	-18.5	-18.5	0.49 (1)	10.00	
U- M	0 / 6451	-18.5	-18.5	0.49 (1)	10.00	
M- V	0 / 5725	-18.5	-18.5	0.46 (1)	10.00	
V- W	0 / 5725	-18.5	-18.5	0.46 (1)	10.00	
W- L	0 / 5725	-18.5	-18.5	0.46 (1)	10.00	
L- X	0 / 0	-18.5	-18.5	0.09 (1)	10.00	
X- K	0 / 0	-18.5	-18.5	0.09 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-1252 / 0	0.19 (1)
M- H	0 / 143	0.03 (4)
M- I	0 / 900	0.11 (1)
L- I	-1266 / 0	0.12 (1)
B- R	0 / 2789	0.35 (1)
L- J	0 / 5860	0.73 (1)
N- G	0 / 323	0.04 (1)
N- H	0 / 2871	0.36 (1)
P- G	-1277 / 0	0.60 (1)
C- Q	0 / 3501	0.43 (1)
P- F	-515 / 0	0.08 (1)
Q- D	-1984 / 0	0.30 (1)
D- P	0 / 2344	0.29 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	23-11-8	-346	-346	---	BACK	VERT	TOTAL	---	C1
M	23-10-12	-143	-143	---	BACK	VERT	TOTAL	---	C1
N	20-11-8	-1815	-1815	---	BACK	VERT	TOTAL	---	C1
T	21-10-12	-26	-26	---	BACK	VERT	TOTAL	---	C1
U	21-10-12	-143	-143	---	BACK	VERT	TOTAL	---	C1
V	24-10-12	-122	-122	---	BACK	VERT	TOTAL	---	C1
W	26-10-12	-29	-29	---	BACK	VERT	TOTAL	---	C1
X	28-10-12	-31	-31	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.53/1.00 (D-F:1) , BC=0.60/1.00 (N-P:1) , WB=0.73/1.00 (J-L:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.73 (O) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	T57	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTIoWxxt54GFza3n1-I9JTxKvq7MG8a5fRBjznpkzvvA7uFddjiKH?Ehyx65q

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TTWW-m	MT20	5.0	8.0	2.00	2.25
D, G, I						
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	6.0	7.0	2.50	2.75
J	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	1.50
M	BMWW+t	MT20	4.0	6.0		
N	BMWW+t	MT20	4.0	10.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2311507

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T58	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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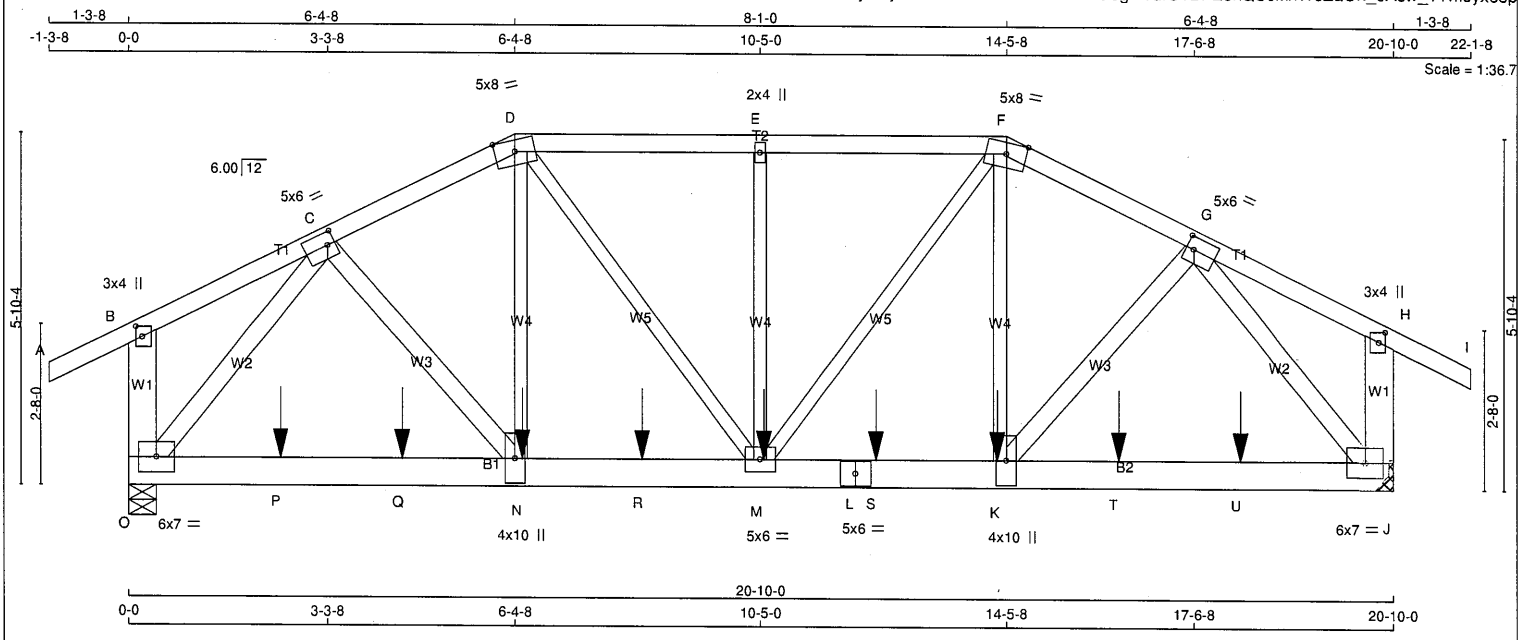

TOTAL WEIGHT = 118 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	2.00	1.25
C	TMWW-t	MT20	5.0	6.0	2.50	1.50
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMWW-t	MT20	5.0	6.0	2.50	1.50
H	TMV+p	MT20	3.0	4.0	2.00	1.25
J	BMVW1-t	MT20	6.0	7.0		
K	BMWW+t	MT20	4.0	10.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	4.0	10.0		
O	BMVW1-t	MT20	6.0	7.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

Structural component only
DWG# T-2311508

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	3020	0	3020	0	0	MECHANICAL	
O	3207	0	3207	0	0	5-8	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-4.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	2127	1442 / 0	0 / 0	0 / 0	0 / 0	685 / 0	0 / 0
O	2261	1526 / 0	0 / 0	0 / 0	0 / 0	734 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	N- D	0 / 759	0.19 (1)
B- C	0 / 12	-91.8	-91.8	0.13 (1)	10.00	D- M	0 / 498	0.12 (1)
C- D	-3185 / 0	-91.8	-91.8	0.28 (1)	3.66	M- E	-437 / 0	0.22 (1)
D- E	-3145 / 0	-91.8	-91.8	0.41 (1)	3.53	M- F	0 / 733	0.18 (1)
E- F	-3145 / 0	-91.8	-91.8	0.41 (1)	3.53	K- F	0 / 503	0.12 (1)
F- G	-3035 / 0	-91.8	-91.8	0.27 (1)	3.75	K- G	0 / 988	0.17 (1)
G- H	0 / 12	-91.8	-91.8	0.14 (1)	10.00	C- N	0 / 1050	0.19 (1)
H- I	0 / 28	-91.8	-91.8	0.13 (1)	10.00	O- C	-3378 / 0	0.96 (1)
O- B	-249 / 0	0.0	0.0	0.02 (1)	7.81	G- J	-3229 / 0	0.92 (1)
J- H	-248 / 0	0.0	0.0	0.02 (1)	7.81			
O- P	0 / 2172	-18.5	-18.5	0.93 (1)	10.00			
P- Q	0 / 2172	-18.5	-18.5	0.93 (1)	10.00			
Q- N	0 / 2172	-18.5	-18.5	0.93 (1)	10.00			
N- R	0 / 2849	-18.5	-18.5	0.94 (1)	10.00			
R- M	0 / 2849	-18.5	-18.5	0.94 (1)	10.00			
M- L	0 / 2710	-18.5	-18.5	0.82 (1)	10.00			
L- S	0 / 2710	-18.5	-18.5	0.82 (1)	10.00			
S- K	0 / 2710	-18.5	-18.5	0.82 (1)	10.00			
K- T	0 / 2076	-18.5	-18.5	0.78 (1)	10.00			
T- U	0 / 2076	-18.5	-18.5	0.78 (1)	10.00			
U- J	0 / 2076	-18.5	-18.5	0.78 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	14-3-12	-225	-225	---	BACK	VERT	TOTAL	---	C1
M	10-5-12	-299	-299	---	BACK	VERT	TOTAL	---	C1
N	6-6-0	-308	-308	---	BACK	VERT	TOTAL	---	C1
P	2-6-4	-344	-344	---	BACK	VERT	TOTAL	---	C1
Q	4-6-4	-308	-308	---	BACK	VERT	TOTAL	---	C1
R	8-5-12	-308	-308	---	BACK	VERT	TOTAL	---	C1
S	12-3-12	-299	-299	---	BACK	VERT	TOTAL	---	C1
T	16-3-12	-225	-225	---	BACK	VERT	TOTAL	---	C1
U	18-3-12	-276	-276	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.94/1.00 (M-N:1) , WB=0.96/1.00 (C-O:1) , SSI=0.45/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.76 (C) (INPUT = 0.95)

CONTINUED ON PAGE 2

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-2311508

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T59	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:59 2023 Page 1
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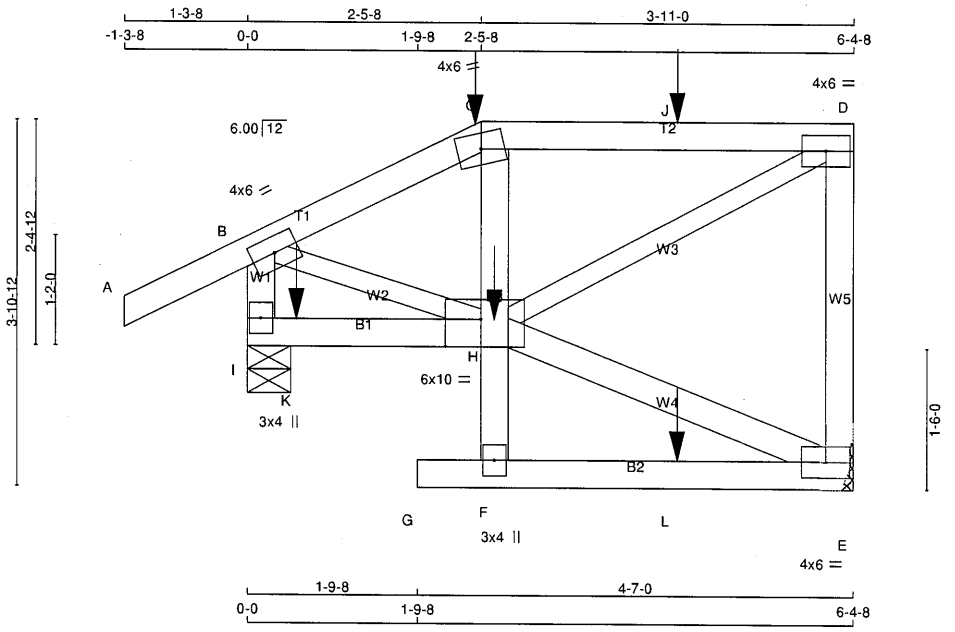

Scale = 1:23.5

TOTAL WEIGHT = 35 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTV-m	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	6.0	10.0	Edge	4.50
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	411	0	411	0	0	MECHANICAL	
I	556	0	556	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	291	188 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
I	392	263 / 0	0 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	B- H	0 / 363	0.09 (1)
B- C	-386 / 0	-91.8	-91.8	0.11 (1)	6.25	H- E	-9 / 0	0.00 (1)
C- J	-324 / 0	-91.8	-91.8	0.30 (1)	6.25	H- D	0 / 371	0.09 (1)
J- D	-324 / 0	-91.8	-91.8	0.30 (1)	6.25			
E- D	-371 / 0	0.0	0.0	0.09 (1)	7.81			
I- B	-524 / 0	0.0	0.0	0.06 (1)	7.81			
I- K	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
K- H	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
F- H	0 / 58	0.0	0.0	0.03 (1)	10.00			
H- C	-216 / 0	0.0	0.0	0.02 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
F- L	0 / 8	-18.5	-18.5	0.10 (4)	10.00			
L- E	0 / 8	-18.5	-18.5	0.10 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-8	-8	-8	---	FRONT	VERT	DEAD	---	C1
C	2-5-8	-20	-20	---	BACK	VERT	TOTAL	---	C1
C	2-5-8	-32	-32	---	FRONT	VERT	SNOW	---	C1
H	2-7-4	-3	-3	---	BACK	VERT	TOTAL	---	C1
J	4-6-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
K	6-4	-7	-7	---	BACK	VERT	TOTAL	---	C1
L	4-6-4	-3	-3	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.10/1.00 (E-F:4) , WB=0.09/1.00 (D-H:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (H) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311509

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T60	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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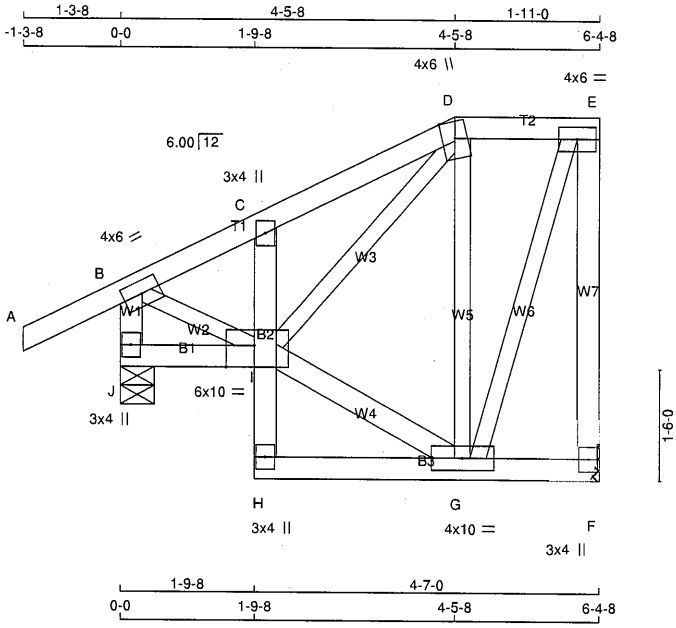

Scale = 1:29.7

TOTAL WEIGHT = 41 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	10.0		
H	BMV+p	MT20	3.0	4.0		
I	BVMWWW-I	MT20	6.0	10.0	Edge	4.50
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	338	0	338	0	0	MECHANICAL	
J	489	0	489	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	239	156 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
J	343	240 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	G- D	-241 / 0	0.08 (1)
B- C	-289 / 0	-91.8	-91.8	0.09 (1)	6.25	G- E	0 / 255	0.06 (1)
C- D	-265 / 0	-91.8	-91.8	0.07 (1)	6.25	B- I	0 / 270	0.06 (1)
D- E	-94 / 0	-91.8	-91.8	0.04 (1)	6.25	I- G	0 / 102	0.02 (1)
F- E	-325 / 0	0.0	0.0	0.13 (1)	7.81	I- D	0 / 211	0.05 (1)
J- B	-471 / 0	0.0	0.0	0.05 (1)	7.81			
J- I	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
H- I	0 / 23	0.0	0.0	0.02 (1)	10.00			
I- C	-183 / 0	0.0	0.0	0.02 (1)	7.81			
H- G	0 / 10	-18.5	-18.5	0.03 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (E-F:1) , BC=0.03/1.00 (G-H:4) , WB=0.08/1.00 (D-G:1) , SSI=0.09/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 0.95)

Structural component only
DWG# T-2311510

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T61	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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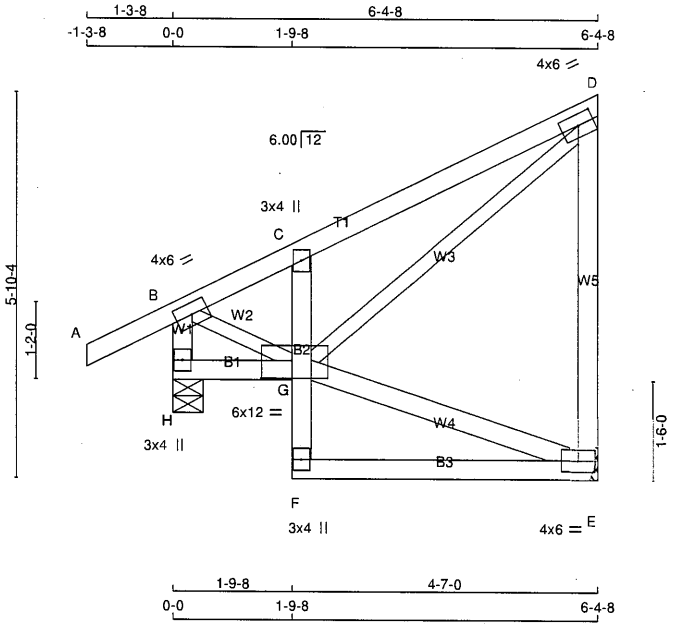

TOTAL WEIGHT = 38 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	TMVW-t	MT20	4.0	6.0	2.00	3.00
E	BMVW1-t	MT20	4.0	6.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWWW-t	MT20	6.0	12.0	3.25	5.50
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	338	0	338	0	0	MECHANICAL	
H	489	0	489	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	239	156 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
H	343	240 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	B- G	0 / 343	0.08 (1)
B- C	-335 / 0	-91.8	-91.8	0.16 (1)	6.25	G- E	-18 / 0	0.00 (1)
C- D	-335 / 0	-91.8	-91.8	0.22 (1)	6.25	G- D	0 / 388	0.09 (1)
E- D	-292 / 0	0.0	0.0	0.18 (1)	7.81			
H- B	-471 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
F- G	0 / 46	0.0	0.0	0.04 (1)	10.00			
G- C	-347 / 0	0.0	0.0	0.03 (1)	7.81			
F- E	0 / 17	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.11/1.00 (E-F:4) , WB=0.09/1.00 (D-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (D) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311511

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T62	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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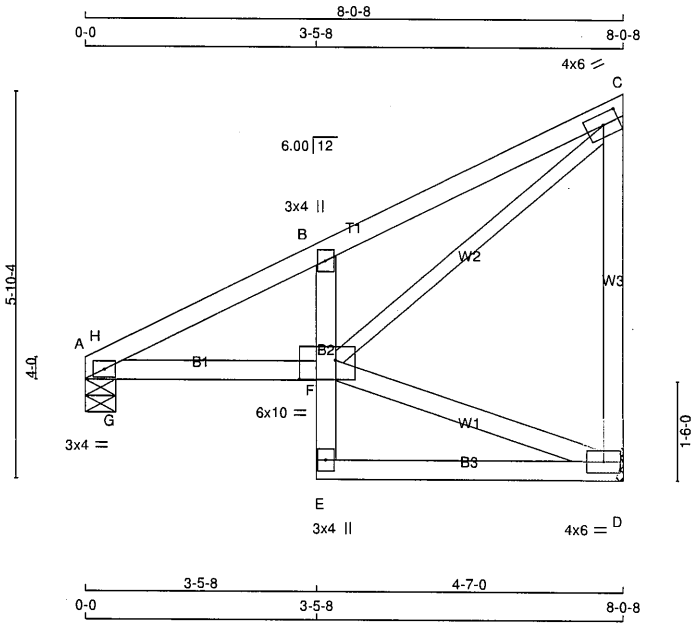

Scale = 1:33.4

TOTAL WEIGHT = 38 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	3.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	3.00
D	BMVW1-t	MT20	4.0	6.0		
E	BMV+p	MT20	3.0	4.0		
F	BVMWW-I	MT20	6.0	10.0	3.50	6.50

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	443	0	443	0	0	5-8	1-8
D	443	0	443	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	313	206 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
D	313	206 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- H	-697 / 0	-91.8	-91.8	0.03 (4)	6.25	F- C	0 / 744	0.17 (1)
H- B	-640 / 0	-91.8	-91.8	0.18 (1)	6.25	F- D	-32 / 0	0.01 (1)
B- C	-635 / 0	-91.8	-91.8	0.21 (1)	6.25	G- H	-41 / 34	0.00 (1)
D- C	-392 / 0	0.0	0.0	0.25 (1)	7.81			
A- G	0 / 596	-18.5	-18.5	0.13 (1)	10.00			
G- F	0 / 596	-18.5	-18.5	0.15 (1)	10.00			
E- F	0 / 46	0.0	0.0	0.07 (1)	10.00			
F- B	-422 / 0	0.0	0.0	0.05 (1)	7.81			
E- D	0 / 30	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.15/1.00 (F-G:1) , WB=0.17/1.00 (C-F:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.20 (A) (INPUT = 0.95)

Structural component only
DWG# T-2311512

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T63	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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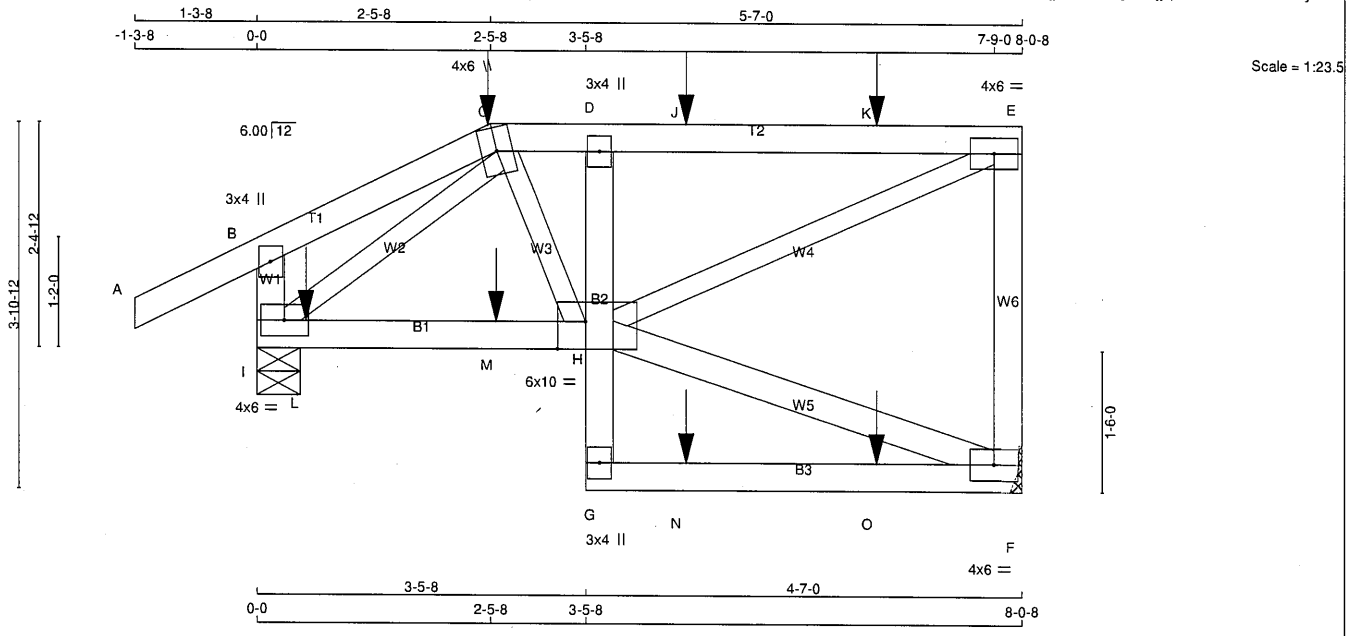

TOTAL WEIGHT = 41 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	507	0	507	0	0	MECHANICAL	
I	655	0	655	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	359	235 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
I	461	313 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	H- F	-16 / 0	0.00 (1)
B- C	0 / 0	-91.8	-91.8	0.11 (1)	10.00	H- E	0 / 645	0.16 (1)
C- D	-594 / 0	-91.8	-91.8	0.28 (1)	6.25	C- H	0 / 323	0.08 (1)
D- J	-583 / 0	-91.8	-91.8	0.28 (1)	6.25	I- C	-579 / 0	0.13 (1)
J- K	-583 / 0	-91.8	-91.8	0.28 (1)	6.25			
K- E	-583 / 0	-91.8	-91.8	0.28 (1)	6.25			
F- E	-457 / 0	0.0	0.0	0.11 (1)	7.81			
I- B	-239 / 0	0.0	0.0	0.03 (1)	7.81			
I- L	0 / 443	-18.5	-18.5	0.13 (1)	10.00			
L- M	0 / 443	-18.5	-18.5	0.13 (1)	10.00			
M- H	0 / 443	-18.5	-18.5	0.13 (1)	10.00			
G- H	0 / 52	0.0	0.0	0.04 (1)	10.00			
H- D	-484 / 0	0.0	0.0	0.04 (1)	7.81			
G- N	0 / 15	-18.5	-18.5	0.13 (4)	10.00			
N- O	0 / 15	-18.5	-18.5	0.13 (4)	10.00			
O- F	0 / 15	-18.5	-18.5	0.13 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-5-15	-8	-8	---	FRONT	VERT	DEAD	---	C1
C	2-5-15	-20	-20	---	FRONT	VERT	TOTAL	---	C1
C	2-5-15	-32	-32	---	FRONT	VERT	SNOW	---	C1
J	4-6-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
K	6-6-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
L	6-4	-7	-7	---	FRONT	VERT	TOTAL	---	C1
M	2-6-4	-3	-3	---	FRONT	VERT	TOTAL	---	C1
N	4-6-4	-4	-4	---	FRONT	VERT	TOTAL	---	C1
O	6-6-4	-3	-3	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.28/1.00 (D-E:1) , BC=0.13/1.00 (F-G:4) , WB=0.16/1.00 (E-H:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (C) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311513

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T63Z	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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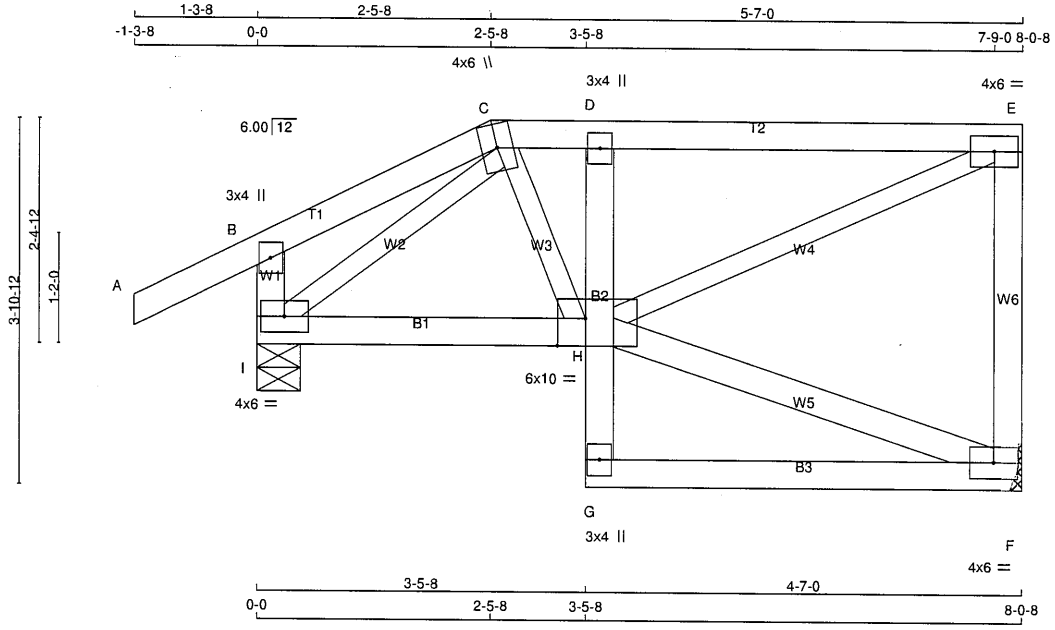

Scale = 1:23.5

TOTAL WEIGHT = 41 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW+m	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-I	MT20	6.0	10.0	Edge	3.50
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	443	0	443	0	0	MECHANICAL	
I	568	0	568	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	313	206 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
I	399	275 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	H- F	-13 / 0	0.00 (1)
B- C	0 / 0	-91.8	-91.8	0.10 (1)	10.00	H- E	0 / 539	0.12 (1)
C- D	-496 / 0	-91.8	-91.8	0.22 (1)	6.25	C- H	0 / 313	0.07 (1)
D- E	-487 / 0	-91.8	-91.8	0.23 (1)	6.25	I- C	-458 / 0	0.09 (1)
F- E	-398 / 0	0.0	0.0	0.09 (1)	7.81			
I- B	-239 / 0	0.0	0.0	0.02 (1)	7.81			
I- H	0 / 350	-18.5	-18.5	0.10 (4)	10.00			
G- H	0 / 46	0.0	0.0	0.03 (1)	10.00			
H- D	-438 / 0	0.0	0.0	0.03 (1)	7.81			
G- F	0 / 13	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.23/1.00 (D-E:1) , BC=0.11/1.00 (F-G:4) , WB=0.12/1.00 (E-H:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (C) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 0.95)

Structural component only
DWG# T-2311514

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T64	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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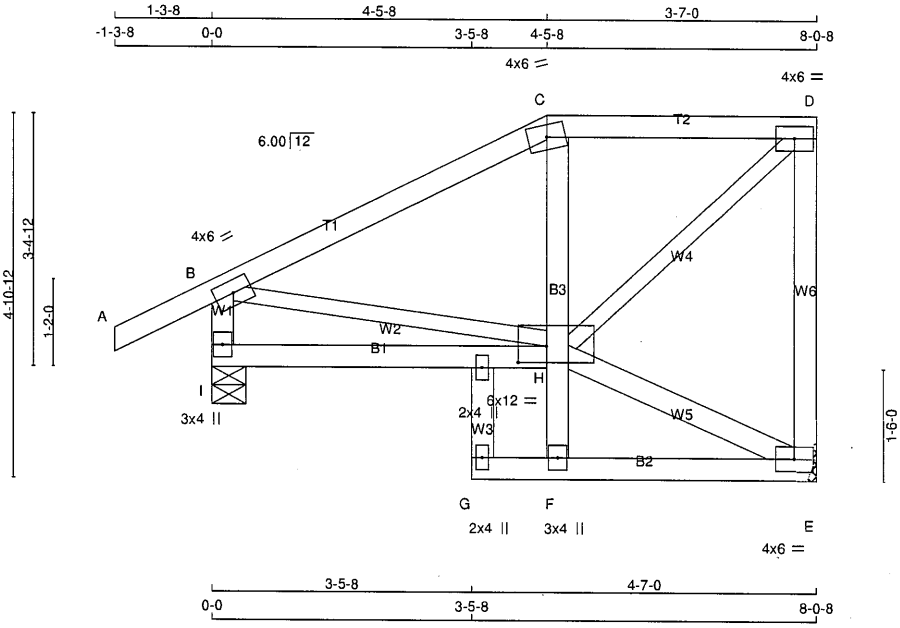


Scale = 1:29.7

TOTAL WEIGHT = 3 X 45 = 134 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTV-m	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMVW1-t	MT20	4.0	6.0		
F	BMV+p	MT20	3.0	4.0		
G	NP+w	MT20	2.0	4.0		
H	BVMWWW-I	MT20	6.0	12.0	2.75	4.50
I	BMV1+p	MT20	3.0	4.0		
J	NP+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	455	0	455	0	0	MECHANICAL	
I	577	0	577	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	323	206 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0
I	406	275 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	B- H	0 / 303	0.07 (1)
B- C	-334 / 0	-91.8	-91.8	0.23 (1)	6.25	H- E	-7 / 0	0.00 (1)
C- D	-285 / 0	-91.8	-91.8	0.15 (1)	6.25	H- D	0 / 384	0.09 (1)
E- D	-421 / 0	0.0	0.0	0.17 (1)	7.81			
I- B	-534 / 0	0.0	0.0	0.05 (1)	7.81			
I- H	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
F- H	0 / 59	0.0	0.0	0.02 (4)	10.00			
H- C	-220 / 0	0.0	0.0	0.01 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
F- E	0 / 6	-18.5	-18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.11/1.00 (H-I:4) , WB=0.09/1.00 (D-H:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 0.95)

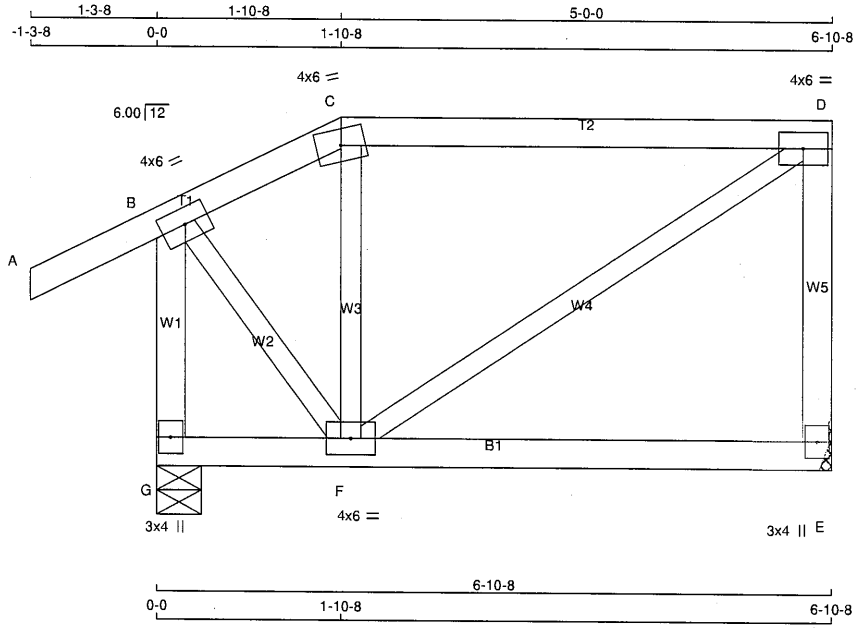
Structural component only
DWG# T-2311515

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T65	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:05 2023 Page 1
ID:T3iSjALIyhYTIoWxxt54GFza3n1-3hoVc3?sEpG?XKG_fP6f8QII1Ozd7P6uXaDQWEyx65i



Scale = 1:22.7

TOTAL WEIGHT = 33 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	367	0	367	0	0	MECHANICAL	
G	516	0	516	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	260	169 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
G	362	252 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	F- C	-193 / 0	0.04 (1)	
B- C	-192 / 0	-91.8	-91.8	0.12 (1)	6.25	F- D	0 / 178	0.04 (1)	
C- D	-147 / 0	-91.8	-91.8	0.39 (1)	6.25	B- F	0 / 238	0.05 (1)	
E- D	-329 / 0	0.0	0.0	0.07 (1)	7.81				
G- B	-515 / 0	0.0	0.0	0.07 (1)	7.81				
G- F	0 / 0	-18.5	-18.5	0.09 (4)	10.00				
F- E	0 / 0	-18.5	-18.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.39/1.00 (C-D:1) , BC=0.09/1.00 (F-G:4) , WB=0.05/1.00 (B-F:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (F) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311516

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T66	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:06 2023 Page 1
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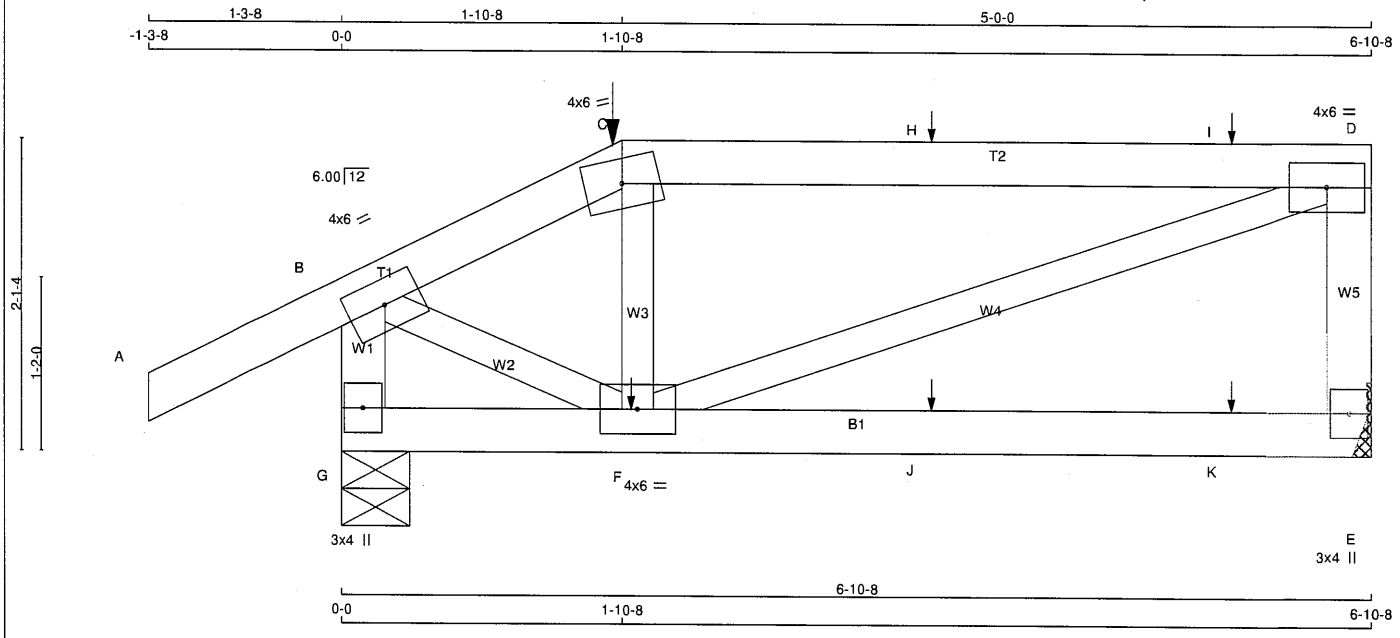

TOTAL WEIGHT = 27 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	375	0	375	0	0	MECHANICAL	
G	539	0	539	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	266	174 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
G	378	265 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00	F- C	-152 / 5	0.02 (1)	
B- C	-356 / 0	-91.8	-91.8	0.13 (1)	6.25	F- D	0 / 310	0.08 (1)	
C- H	-291 / 0	-91.8	-91.8	0.44 (1)	6.25	B- F	0 / 327	0.08 (1)	
H- I	-291 / 0	-91.8	-91.8	0.44 (1)	6.25				
I- D	-291 / 0	-91.8	-91.8	0.44 (1)	6.25				
E- D	-337 / 0	0.0	0.0	0.04 (1)	7.81				
G- B	-537 / 0	0.0	0.0	0.06 (1)	7.81				
G- F	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
F- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
J- K	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
K- E	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-10-8	-4	-4	---	FRONT	VERT	DEAD	---	C1
C	1-10-8	1	1	76	FRONT	VERT	TOTAL	---	C1
C	1-10-8	-18	-18	---	FRONT	VERT	SNOW	---	C1
F	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	76	FRONT	VERT	TOTAL	---	C1
I	5-11-4	1	1	75	FRONT	VERT	TOTAL	---	C1
J	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
K	5-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.44/1.00 (C-D:1) , BC=0.10/1.00 (F-G:4) , WB=0.08/1.00 (B-F:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311517

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	T66	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTIoWxxt54GFza3n1-XtMtpP0U?6Os9UrAC6duhdrT6oJnsrx2mEzz2gyx65h

JSI GRIP= 0.47 (F) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311517

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T67	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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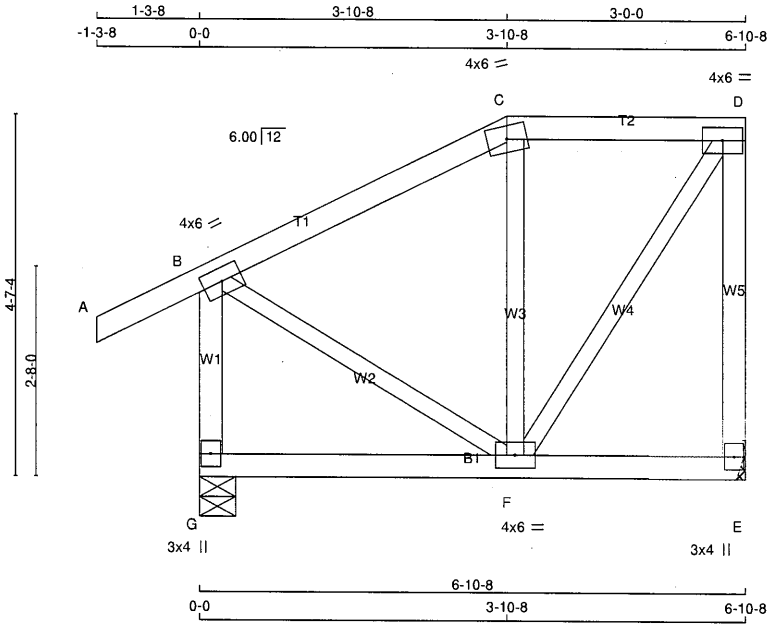

Scale = 1:28.1

TOTAL WEIGHT = 2 X 36 = 71 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	6.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	379	0	379	0	0	MECHANICAL	
G	503	0	503	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	268	176 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
G	354	245 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12	(1)	10.00	F- C	-238 / 0	0.07 (1)
B- C	-174 / 0	-91.8	-91.8	0.18	(1)	6.25	F- D	0 / 269	0.06 (1)
C- D	-150 / 0	-91.8	-91.8	0.11	(1)	6.25	B- F	0 / 181	0.04 (1)
E- D	-360 / 0	0.0	0.0	0.12	(1)	7.81			
G- B	-472 / 0	0.0	0.0	0.07	(1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.06	(4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.06	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.06/1.00 (F-G:4) , WB=0.07/1.00 (C-F:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (F) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311518

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T68	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:07 2023 Page 1
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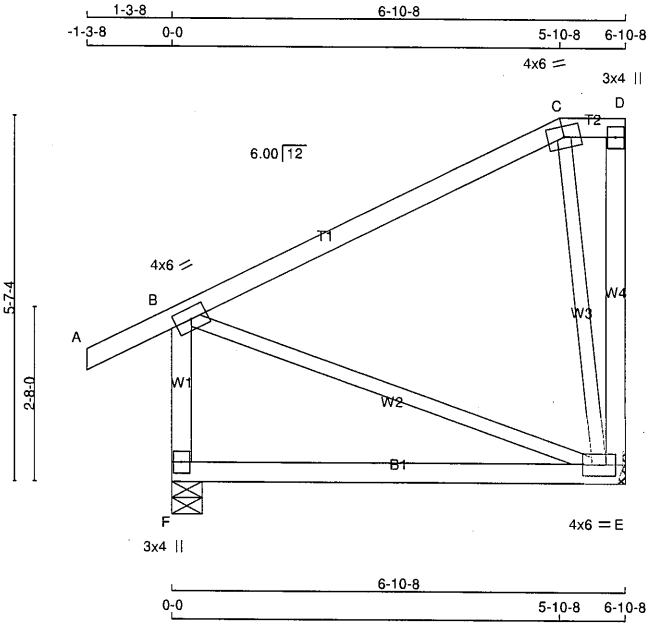

Scale = 1:33.8

TOTAL WEIGHT = 2 X 36 = 72 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	379	0	379	0	0	MECHANICAL	
F	503	0	503	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	268	176 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
F	354	245 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	B- E	0 / 56	0.01 (1)
B- C	-59 / 0	-91.8	-91.8	0.41 (1)	6.25	C- E	-294 / 0	0.14 (1)
C- D	0 / 0	-91.8	-91.8	0.01 (1)	10.00			
E- D	-44 / 0	0.0	0.0	0.03 (1)	7.81			
F- B	-440 / 0	0.0	0.0	0.06 (1)	7.81			
F- E	0 / 0	-18.5	-18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 819 (0.10")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.26/1.00 (E-F:4) , WB=0.14/1.00 (C-E:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.15 (C) (INPUT = 0.95)

Structural component only
DWG# T-2311519

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T69	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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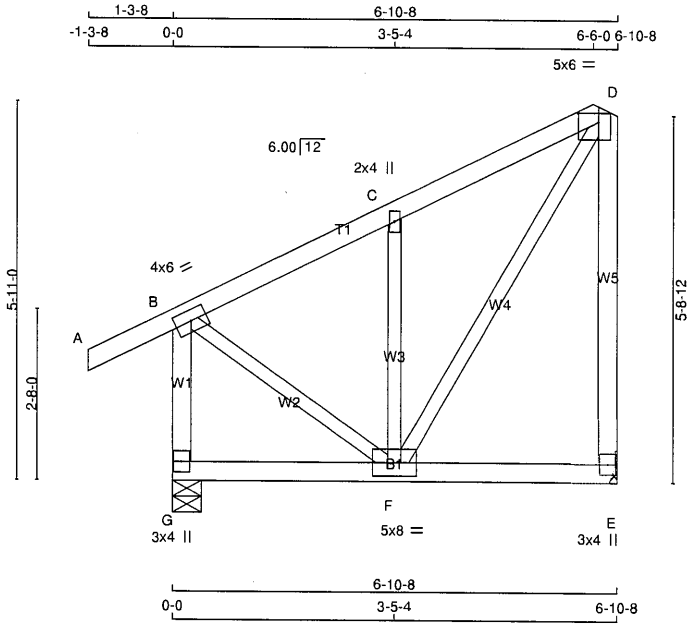

TOTAL WEIGHT = 2 X 38 = 76 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TVMW-p	MT20	5.0	6.0	1.75	3.75
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	5.0	8.0		
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	503	0	503	0	0	5-8	1-8
E	379	0	379	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	354	245 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
E	268	176 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	B- F	0 / 240	0.05 (1)
B- C	-204 / 0	-91.8	-91.8	0.13 (1)	6.25	F- C	-385 / 0	0.11 (1)
C- D	-230 / 0	-91.8	-91.8	0.13 (1)	6.25	F- D	0 / 375	0.08 (1)
G- B	-476 / 0	0.0	0.0	0.07 (1)	7.81			
E- D	-352 / 0	0.0	0.0	0.26 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.26/1.00 (D-E:1) , BC=0.06/1.00 (E-F:4) , WB=0.11/1.00 (C-F:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (F) (INPUT = 0.90)
JSI METAL= 0.16 (C) (INPUT = 0.95)

Structural component only
DWG# T-2311520

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	T70	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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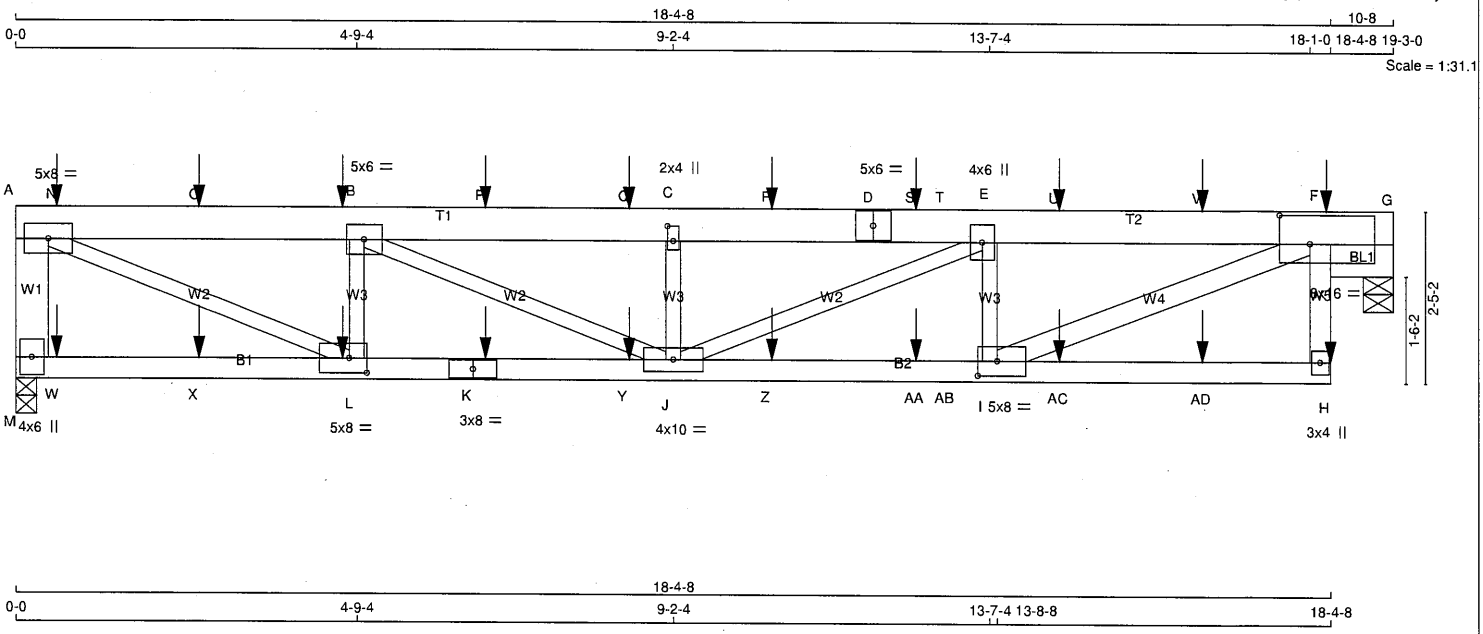

TOTAL WEIGHT = 2 X 82 = 165 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
M - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	2100F 1.8E	SPF
D - G	2x6	DRY	2100F 1.8E	SPF
H - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
M- A	2	12	SIDE(2.4)
A- D	2	12	SIDE(197.8)
D- G	2	12	SIDE(166.9)
F- H	1	12	SIDE(8.2)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- K	1	12	SIDE(61.0)
K- H	1	12	SIDE(56.4)
WEBS : (0.122"X3") SPIRAL NAILS			
L- B	1	6	SIDE(1.3)
2x3	1	6	
2x4	1	6	
F- G	2	6	SIDE(20.9)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

Structural component only
DWG# T-2311521

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	2386	0	2386	0	0	3-8	1-8
G	2568	0	2568	0	0	5-0	5-0

(** SEE "BEARING NOTE" **)

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): G

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1687	1110 / 0	0 / 0	0 / 0	0 / 0	577 / 0	0 / 0
G	1815	1201 / 0	0 / 0	0 / 0	0 / 0	614 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO									
M- A	-2066 / 0	0.0	0.0	0.08 (1)		7.81	I- F	0 / 5738	0.51 (1)
A- N	-4224 / 0	-91.8	-91.8	0.07 (1)		6.25	A- L	0 / 4598	0.57 (1)
N- O	-4224 / 0	-91.8	-91.8	0.07 (1)		6.25	I- E	-1585 / 0	0.13 (1)
O- B	-4224 / 0	-91.8	-91.8	0.07 (1)		6.25	L- B	-1087 / 0	0.09 (1)
B- P	-5559 / 0	-91.8	-91.8	0.08 (1)		5.90	J- E	0 / 381	0.05 (1)
P- Q	-5559 / 0	-91.8	-91.8	0.08 (1)		5.90	B- J	0 / 1472	0.18 (1)
Q- C	-5559 / 0	-91.8	-91.8	0.08 (1)		5.90	J- C	-228 / 0	0.02 (1)
C- R	-5559 / 0	-91.8	-91.8	0.14 (1)		5.82			
R- D	-5559 / 0	-91.8	-91.8	0.14 (1)		5.82			
D- S	-5559 / 0	-91.8	-91.8	0.14 (1)		5.82			
S- T	-5559 / 0	-91.8	-91.8	0.14 (1)		5.82			
T- E	-5559 / 0	-137.7	-137.7	0.14 (1)		5.82			
E- U	-5214 / 0	-137.7	-137.7	0.53 (1)		5.31			
U- V	-5214 / 0	-137.7	-137.7	0.53 (1)		5.31			
V- F	-5214 / 0	-137.7	-137.7	0.53 (1)		5.31			
F- G	-1023 / 0	-137.7	-137.7	0.21 (1)		6.25			
H- F	0 / 451	0.0	0.0	0.04 (1)		10.00			
M- W	0 / 0	-18.5	-18.5	0.21 (1)		10.00			
W- X	0 / 0	-18.5	-18.5	0.21 (1)		10.00			
X- L	0 / 0	-18.5	-18.5	0.21 (1)		10.00			
L- K	0 / 4224	-18.5	-18.5	0.54 (1)		10.00			
K- Y	0 / 4224	-18.5	-18.5	0.54 (1)		10.00			
Y- J	0 / 4224	-18.5	-18.5	0.54 (1)		10.00			
J- Z	0 / 5214	-18.5	-18.5	0.68 (1)		10.00			
Z-AA	0 / 5214	-18.5	-18.5	0.68 (1)		10.00			
AA-AB	0 / 5214	-18.5	-18.5	0.68 (1)		10.00			
AB- I	0 / 5214	-27.7	-27.7	0.68 (1)		10.00			
I-AC	-32 / 0	-27.7	-27.7	0.24 (1)		6.25			
AC-AD	-32 / 0	-27.7	-27.7	0.24 (1)		6.25			
AD- H	-32 / 0	-27.7	-27.7	0.24 (1)		6.25			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	4-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
F	18-3-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
H	18-4-8	-176	-176	---	FRONT	VERT	TOTAL	---	C1
K	6-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 952 (0.23")

CSI: TC=0.53/1.00 (E-F:1) , BC=0.68/1.00 (I-J:1) , WB=0.57/1.00 (A-L:1) , SSI=0.39/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (A) (INPUT = 0.90)
JSI METAL= 0.77 (K) (INPUT = 0.95)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	T70	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:T3iSjALIyhYTloWxxt54GFza3n1-xS10SR2MI1nR0xZluEAbJGTyq?CR351USBBef?yx65e

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TMWW-t	MT20	5.0	6.0		
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TS-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMVWK-t	MT20	8.0	16.0	4.75	5.00
H	BMV+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	8.0	2.50	3.25
J	BMWWW-t	MT20	4.0	10.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.00
M	BMV1+p	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	4-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
N	6-14	-17	-17	---	FRONT	VERT	TOTAL	---	C1
O	2-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
P	6-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
Q	8-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
R	10-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
S	12-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
U	14-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
V	16-6-14	-6	-6	---	FRONT	VERT	TOTAL	---	C1
W	6-14	-172	-172	---	FRONT	VERT	TOTAL	---	C1
X	2-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
Y	8-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
Z	10-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AA	12-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AC	14-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1
AD	16-6-14	-169	-169	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311521

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	PB1	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 15:48:17 2023 Page 1
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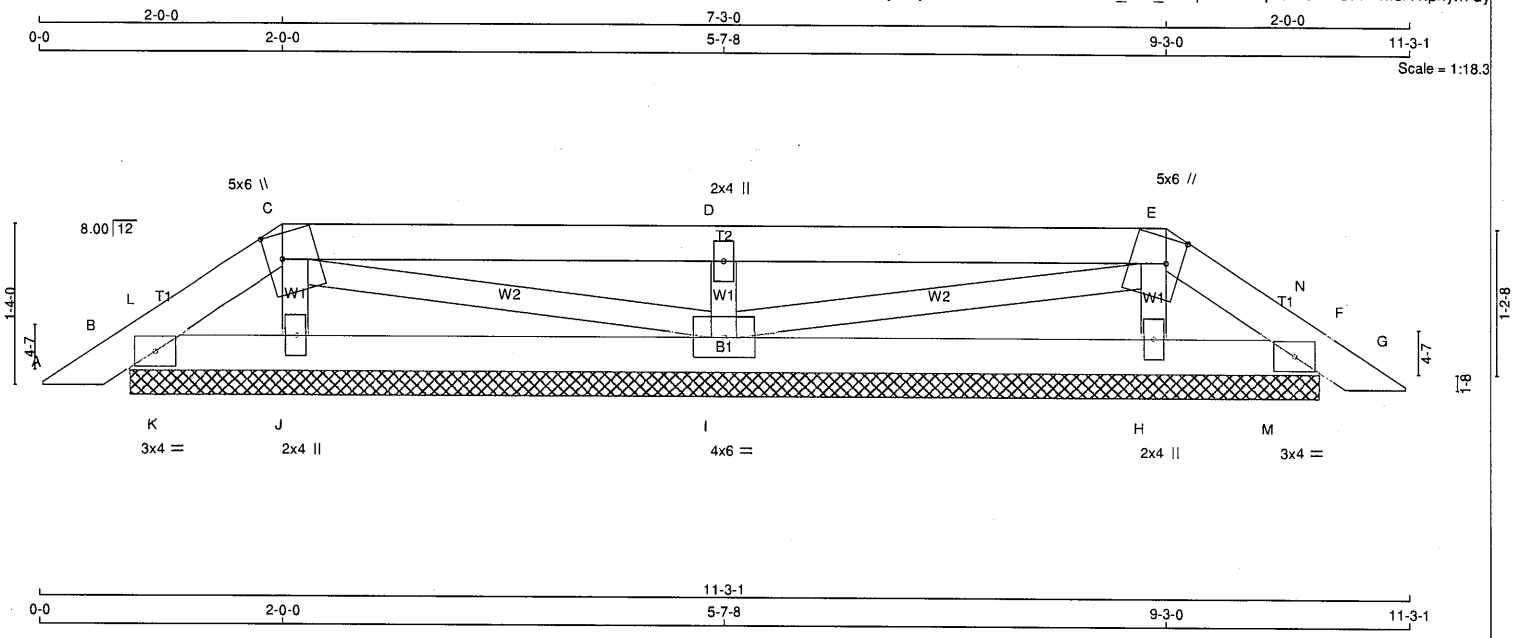

TOTAL WEIGHT = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	138	0	138	0	0	9-9-3	1-8
F	138	0	138	0	0	9-9-3	1-8
J	216	0	216	0	0	9-9-3	1-8
H	216	0	216	0	0	9-9-3	1-8
I	487	0	487	0	0	9-9-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	95	76 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
F	95	76 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
J	155	90 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
H	155	90 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
I	343	233 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	J- C	-147 / 0	0.02 (1)
B- L	-33 / 0	-91.8	-91.8	0.00 (4)	6.25	H- E	-147 / 0	0.02 (1)
L- C	-35 / 0	-91.8	-91.8	0.01 (1)	6.25	I- E	-9 / 0	0.00 (1)
C- D	-2 / 0	-91.8	-91.8	0.20 (1)	10.00	C- I	-9 / 0	0.00 (1)
D- E	-2 / 0	-91.8	-91.8	0.20 (1)	10.00	I- D	-415 / 0	0.06 (1)
E- N	-35 / 0	-91.8	-91.8	0.01 (1)	6.25	K- L	-61 / 0	0.00 (1)
N- F	-33 / 0	-91.8	-91.8	0.00 (4)	6.25	M- N	-61 / 0	0.00 (1)
F- G	0 / 15	-91.8	-91.8	0.03 (1)	10.00			
B- K	0 / 28	-18.5	-18.5	0.02 (1)	10.00			
K- J	0 / 28	-18.5	-18.5	0.04 (4)	10.00			
J- I	0 / 11	-18.5	-18.5	0.05 (4)	10.00			
I- H	0 / 11	-18.5	-18.5	0.05 (4)	10.00			
H- M	0 / 28	-18.5	-18.5	0.04 (4)	10.00			
M- F	0 / 28	-18.5	-18.5	0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (D-E:1) , BC=0.05/1.00 (I-J:4) , WB=0.06/1.00 (D-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 0.95)

Structural component only
DWG# T-2311439

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	PB2	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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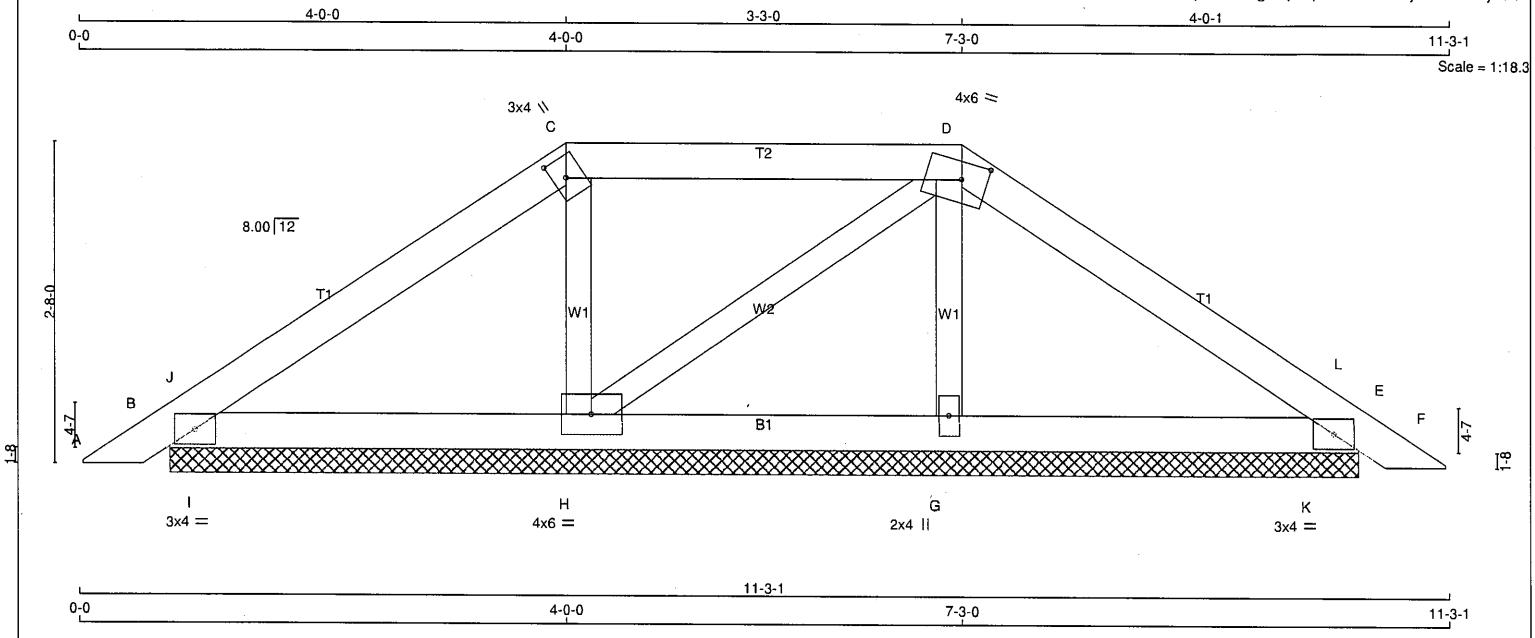

TOTAL WEIGHT = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMB1-I	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	277	0	277	0	0	9-9-3	1-8
E	295	0	295	0	0	9-9-3	1-8
H	341	0	341	0	0	9-9-3	1-8
G	282	0	282	0	0	9-9-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	194	138 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
E	206	148 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
H	241	156 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
G	201	123 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	H- C	-230 / 0	0.04 (1)
B- J	-33 / 0	-91.8	-91.8	0.04 (1)	6.25	H- D	-35 / 0	0.01 (1)
J- C	-87 / 0	-91.8	-91.8	0.12 (1)	6.25	G- D	-192 / 0	0.03 (1)
C- D	-58 / 0	-91.8	-91.8	0.17 (1)	6.25	I- J	-235 / 0	0.00 (1)
D- L	-119 / 0	-91.8	-91.8	0.12 (1)	6.25	K- L	-232 / 0	0.00 (1)
L- E	-67 / 0	-91.8	-91.8	0.04 (1)	6.25			
E- F	0 / 15	-91.8	-91.8	0.03 (1)	10.00			
B- I	0 / 68	-18.5	-18.5	0.11 (1)	10.00			
I- H	0 / 68	-18.5	-18.5	0.11 (1)	10.00			
H- G	0 / 86	-18.5	-18.5	0.06 (1)	10.00			
G- K	0 / 95	-18.5	-18.5	0.11 (1)	10.00			
K- E	0 / 95	-18.5	-18.5	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-D:1) , BC=0.11/1.00 (G-K:1) , WB=0.04/1.00 (C-H:1) , SSI=0.18/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (E) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311440

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	PB3	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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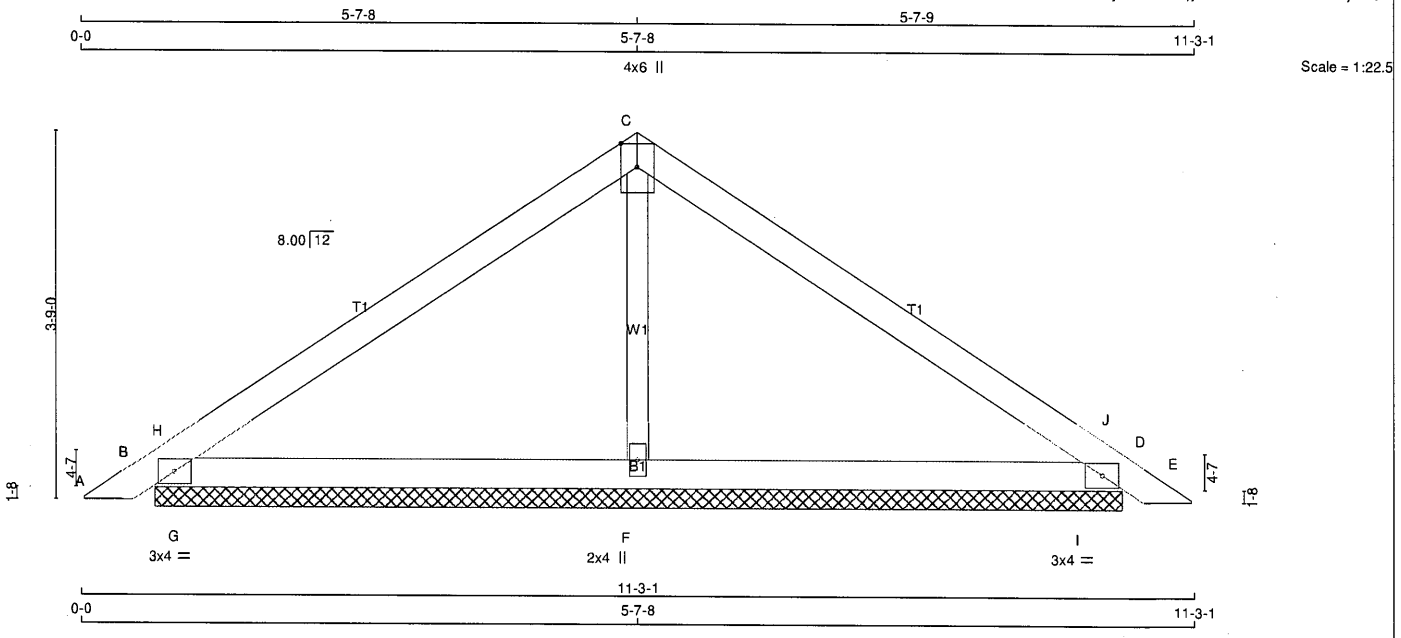

TOTAL WEIGHT = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	406	0	406	0	0	9-9-3	1-8
D	406	0	406	0	0	9-9-3	1-8
F	383	0	383	0	0	9-9-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	284	202 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
D	284	202 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
F	274	162 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	F- C	-165 / 0	0.03 (1)
B- H	-41 / 13	-91.8	-91.8	0.14 (1)	6.25	G- H	-560 / 0	0.00 (1)
H- C	-201 / 0	-91.8	-91.8	0.26 (1)	6.25	I- J	-560 / 0	0.00 (1)
C- J	-201 / 0	-91.8	-91.8	0.26 (1)	6.25			
J- D	-41 / 13	-91.8	-91.8	0.14 (1)	6.25			
D- E	0 / 15	-91.8	-91.8	0.03 (1)	10.00			
B- G	0 / 157	-18.5	-18.5	0.25 (1)	10.00			
G- F	0 / 157	-18.5	-18.5	0.25 (1)	10.00			
F- I	0 / 157	-18.5	-18.5	0.25 (1)	10.00			
I- D	0 / 157	-18.5	-18.5	0.25 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (C-J:1) , BC=0.25/1.00 (F-I:1) , WB=0.03/1.00 (C-F:1) , SSI=0.42/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 0.95)

Structural component only
DWG# T-2311441

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	PB4	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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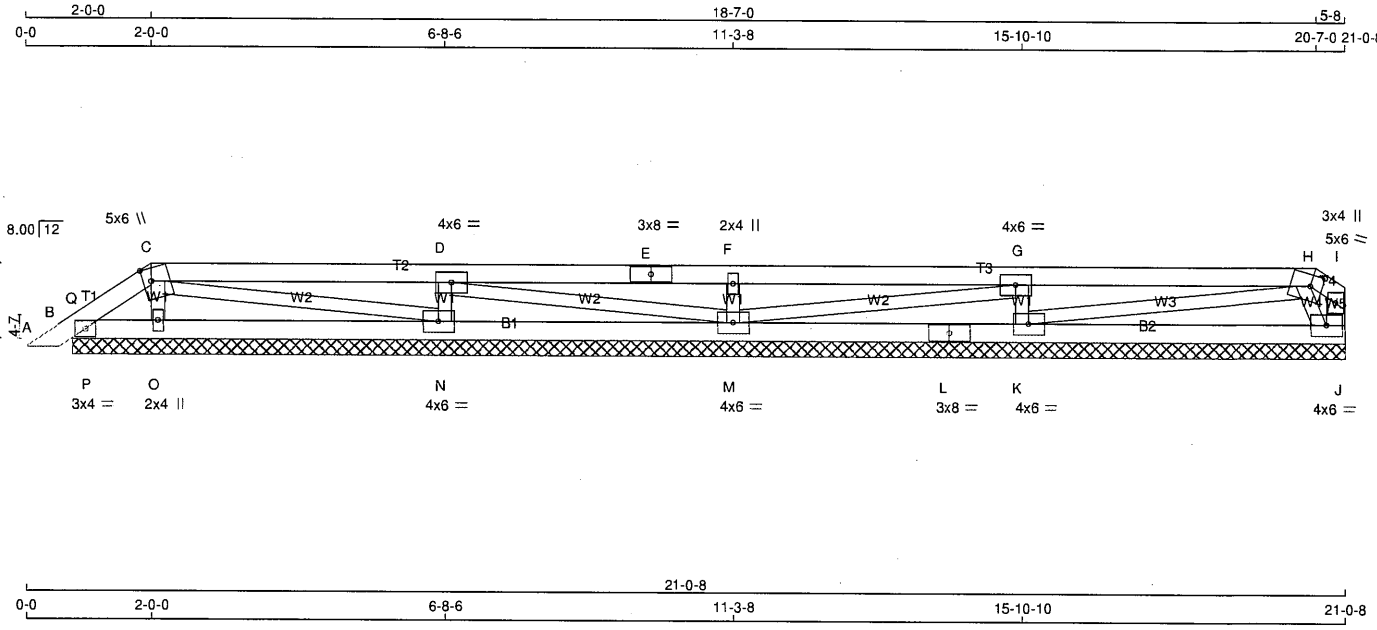

TOTAL WEIGHT = 2 X 65 = 131 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMWW-t	MT20	4.0	6.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TTWW-m	MT20	5.0	6.0	2.25	2.25
I	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW1-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW1-t	MT20	4.0	6.0		
N	BMWW1-t	MT20	4.0	6.0		
O	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	239	0	239	0	0	20-3-10 (6-3-14)	
B	135	0	135	0	0	20-3-10 (6-3-14)	
O	275	0	275	0	0	20-3-10 (6-3-14)	
N	576	0	576	0	0	20-3-10 (6-3-14)	
K	599	0	599	0	0	20-3-10 (6-3-14)	
M	474	0	474	0	0	20-3-10 (6-3-14)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	169	112 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0
B	92	79 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
O	197	113 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
N	407	272 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
K	424	276 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
M	335	221 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, B, O, N, K, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00	O- C	-186 / 0	0.03 (1)
B- Q	-33 / 5	-91.8	-91.8	0.01 (4)	6.25	C- N	-5 / 0	0.00 (1)
Q- C	-44 / 0	-91.8	-91.8	0.01 (4)	6.25	K- G	-481 / 0	0.07 (1)
C- D	-7 / 0	-91.8	-91.8	0.29 (1)	10.00	N- D	-487 / 0	0.07 (1)
D- E	0 / 0	-91.8	-91.8	0.29 (1)	10.00	M- G	-29 / 0	0.01 (1)
E- F	0 / 0	-91.8	-91.8	0.29 (1)	10.00	D- M	-8 / 0	0.00 (1)
F- G	0 / 0	-91.8	-91.8	0.29 (1)	10.00	M- F	-388 / 0	0.05 (1)
G- H	-28 / 0	-91.8	-91.8	0.29 (1)	6.25	H- J	-202 / 0	0.03 (1)
H- I	0 / 0	-91.8	-91.8	0.00 (1)	10.00	K- H	-71 / 0	0.03 (1)
J- I	-23 / 0	0.0	0.0	0.00 (1)	7.81	P- Q	-74 / 0	0.00 (1)
B- P	0 / 34	-18.5	-18.5	0.03 (1)	10.00			
P- O	0 / 34	-18.5	-18.5	0.06 (4)	10.00			
O- N	0 / 13	-18.5	-18.5	0.08 (4)	10.00			
N- M	0 / 7	-18.5	-18.5	0.08 (4)	10.00			
M- L	0 / 28	-18.5	-18.5	0.12 (4)	10.00			
L- K	0 / 28	-18.5	-18.5	0.12 (4)	10.00			
K- J	0 / 97	-18.5	-18.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1) , BC=0.12/1.00 (J-K:4) , WB=0.07/1.00 (D-N:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (M) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311442

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	PB5	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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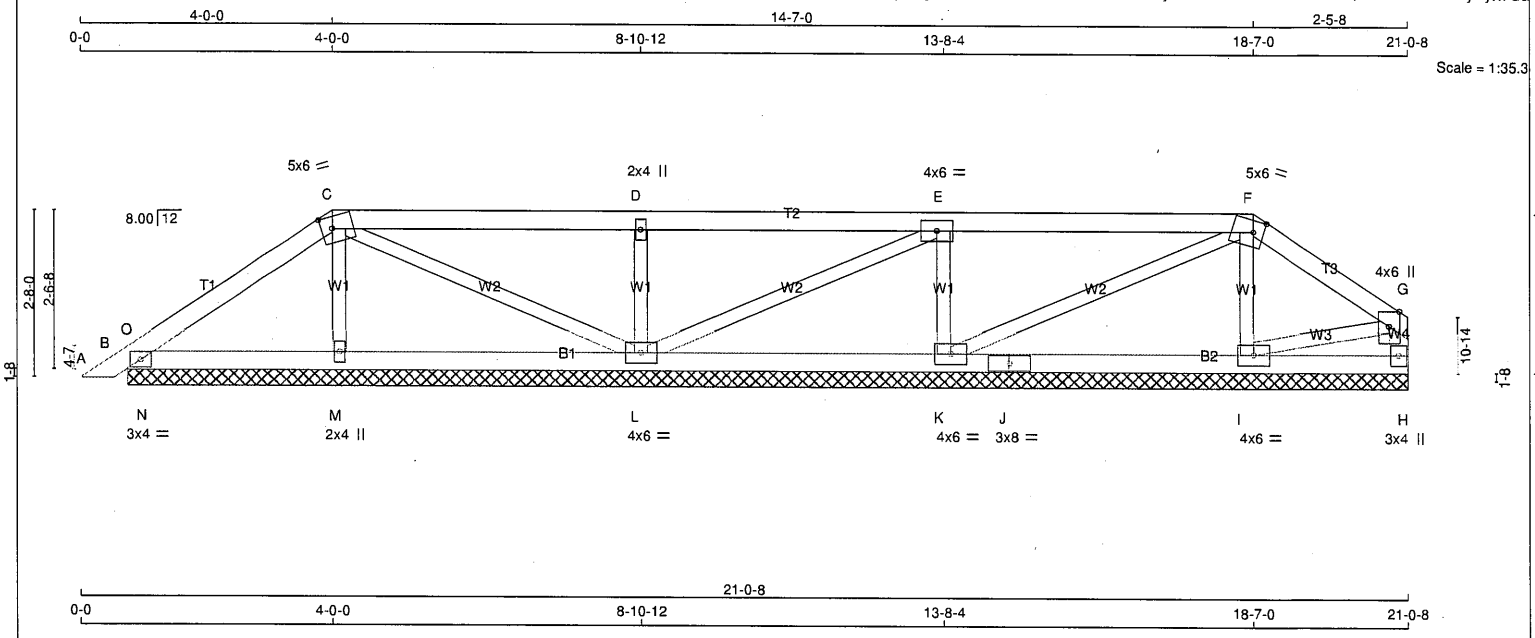

TOTAL WEIGHT = 2 X 70 = 139 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW1-t	MT20	4.0	6.0		
L	BMWWW1-t	MT20	4.0	6.0		
M	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	173	0	173	0	0	20-3-10 (13-11-02)	
B	266	0	266	0	0	20-3-10 (13-11-02)	
M	354	0	354	0	0	20-3-10 (13-11-02)	
I	314	0	314	0	0	20-3-10 (13-11-02)	
K	573	0	573	0	0	20-3-10 (13-11-02)	
L	617	0	617	0	0	20-3-10 (13-11-02)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	120	89 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
B	186	135 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
M	253	152 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
I	225	132 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
K	405	269 / 0	0 / 0	0 / 0	0 / 0	136 / 0	0 / 0
L	435	294 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, B, M, I, K, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-91.8	-91.8	0.03 (1)	10.00
B- O	-16 / 0	-91.8	-91.8	0.04 (1)	6.25
O- C	-76 / 0	-91.8	-91.8	0.12 (1)	6.25
C- D	0 / 15	-91.8	-91.8	0.30 (1)	10.00
D- E	0 / 15	-91.8	-91.8	0.30 (1)	10.00
E- F	-21 / 0	-91.8	-91.8	0.29 (1)	6.25
F- G	-63 / 0	-91.8	-91.8	0.09 (1)	6.25
H- G	-160 / 0	0.0	0.0	0.02 (1)	7.81
B- N	0 / 59	-18.5	-18.5	0.11 (1)	10.00
N- M	0 / 59	-18.5	-18.5	0.11 (1)	10.00
M- L	0 / 48	-18.5	-18.5	0.09 (4)	10.00
L- K	0 / 21	-18.5	-18.5	0.09 (4)	10.00
K- J	0 / 41	-18.5	-18.5	0.09 (4)	10.00
J- I	0 / 41	-18.5	-18.5	0.09 (4)	10.00
I- H	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-238 / 0	0.04 (1)
I- F	-250 / 0	0.04 (1)
I- G	0 / 54	0.01 (1)
K- F	-22 / 0	0.01 (1)
C- L	-68 / 0	0.03 (1)
K- E	-472 / 0	0.08 (1)
L- D	-488 / 0	0.08 (1)
L- E	-39 / 0	0.02 (1)
N- O	-248 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (D-E:1) , BC=0.11/1.00 (B-N:1) , WB=0.08/1.00 (D-L:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 0.95)

Structural component only
DWG# T-2311443

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	V1	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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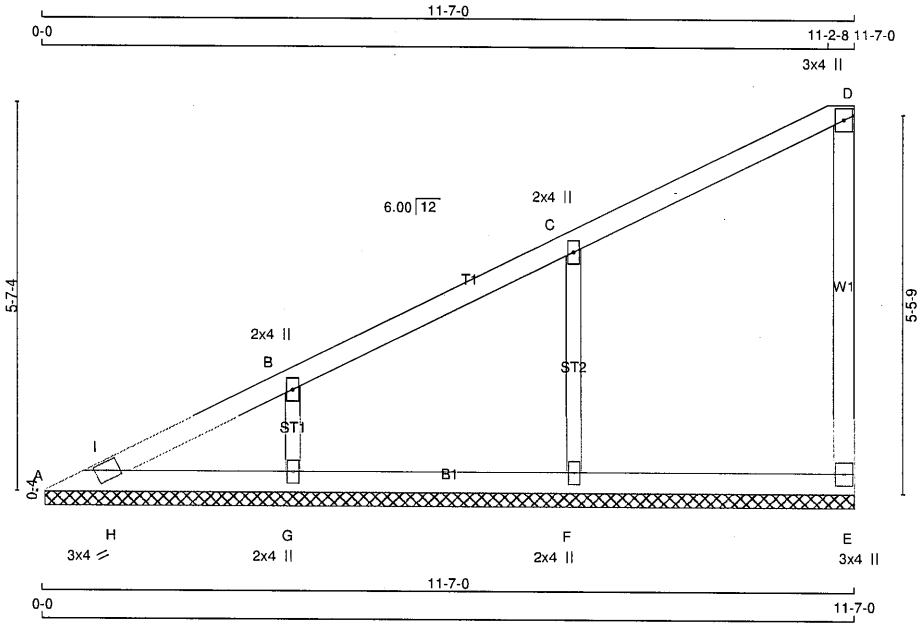

TOTAL WEIGHT = 37 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		
G	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	160	0	160	0	0	11-6-8	1-8
E	178	0	178	0	0	11-6-8	1-8
F	495	0	495	0	0	11-6-8	1-8
G	440	0	440	0	0	11-6-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	113	75 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
E	126	83 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
F	350	230 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0
G	311	203 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, F, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- I	-37 / 0	-91.8	-91.8	0.05 (1)	6.25	F- C	-418 / 0	0.10 (1)
I- B	0 / 2	-91.8	-91.8	0.15 (1)	10.00	G- B	-335 / 0	0.05 (1)
B- C	-3 / 0	-91.8	-91.8	0.22 (1)	10.00	H- I	-66 / 3	0.00 (1)
C- D	-17 / 0	-91.8	-91.8	0.22 (1)	6.25			
E- D	-147 / 0	0.0	0.0	0.09 (1)	7.81			
A- H	0 / 15	-18.5	-18.5	0.09 (1)	10.00			
H- G	0 / 23	-18.5	-18.5	0.09 (1)	10.00			
G- F	0 / 8	-18.5	-18.5	0.07 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.22/1.00 (C-D:1) , BC=0.09/1.00 (G-H:1) , WB=0.10/1.00 (C-F:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (C) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 0.95)

Structural component only
DWG# T-2311522

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	V2	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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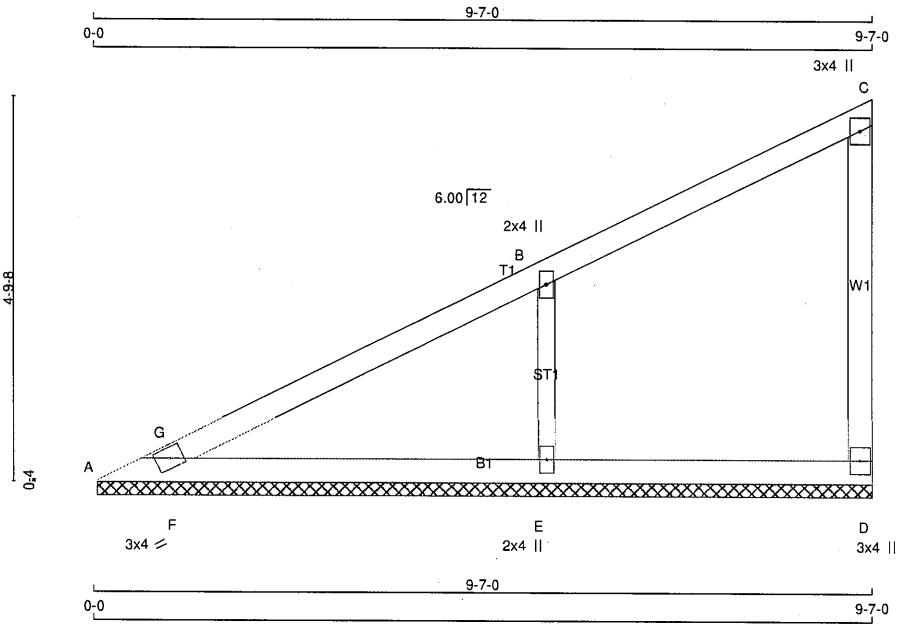

Scale = 1:27.5

TOTAL WEIGHT = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	248	0	248	0	0	9-6-8	1-8
D	141	0	141	0	0	9-6-8	1-8
E	663	0	663	0	0	9-6-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	175	115 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
D	99	66 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
E	469	307 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- G	-29 / 0	-91.8	-91.8	0.07 (4)	6.25	E- B	-504 / 0	0.09 (1)
G- B	0 / 13	-91.8	-91.8	0.32 (1)	10.00	F- G	-149 / 5	0.00 (1)
B- C	-26 / 0	-91.8	-91.8	0.32 (1)	6.25			
D- C	-129 / 0	0.0	0.0	0.05 (1)	7.81			
A- F	-7 / 0	-18.5	-18.5	0.20 (1)	10.00			
F- E	0 / 13	-18.5	-18.5	0.21 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (B-G:1) , BC=0.21/1.00 (E-F:1) , WB=0.09/1.00 (B-E:1) , SSI=0.19/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311523

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	V3	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:33:12 2023 Page 1
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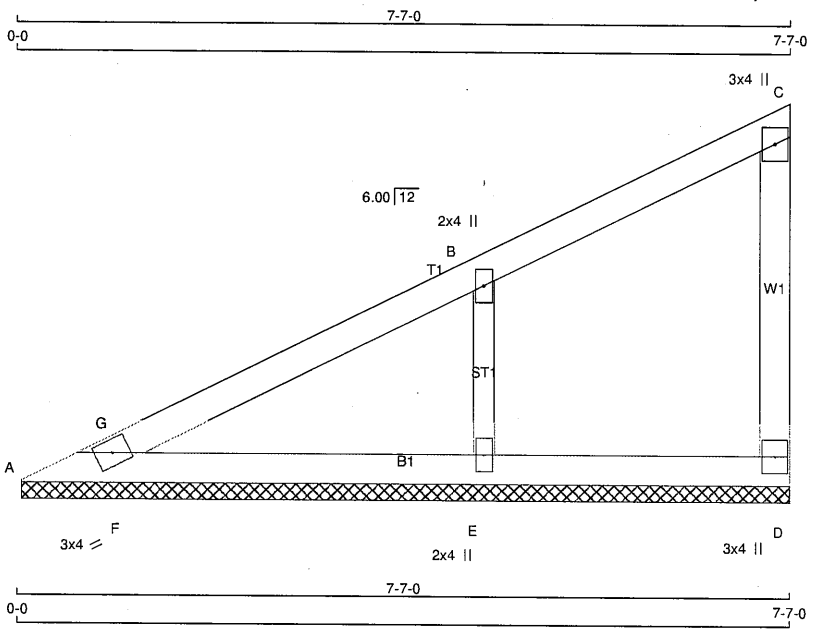

TOTAL WEIGHT = 23 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		
E	BMW1+w	MT20	2.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	205	0	205	0	0	7-6-8	1-8
D	97	0	97	0	0	7-6-8	1-8
E	529	0	529	0	0	7-6-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	145	95 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
D	68	45 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
E	374	245 / 0	0 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- G	-28 / 0	-91.8	-91.8	0.06 (1)	6.25	E- B	-395 / 0	0.06 (1)
G- B	0 / 10	-91.8	-91.8	0.20 (1)	10.00	F- G	-111 / 4	0.00 (1)
B- C	-21 / 0	-91.8	-91.8	0.20 (1)	6.25			
D- C	-93 / 0	0.0	0.0	0.02 (1)	7.81			
A- F	-1 / 0	-18.5	-18.5	0.15 (1)	10.00			
F- E	0 / 13	-18.5	-18.5	0.15 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (B-G:1) , BC=0.15/1.00 (E-F:1) , WB=0.06/1.00 (B-E:1) , SSI=0.15/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 0.95)

Structural component only
DWG# T-2311524

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	V4	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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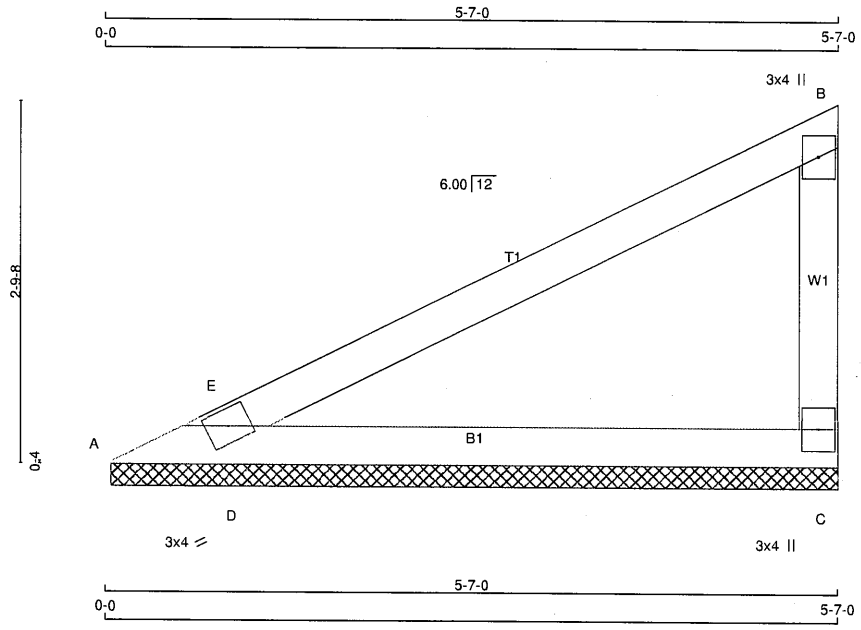

TOTAL WEIGHT = 15 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TBM1-h	MT20	3.0	4.0	
B	TMV+p	MT20	3.0	4.0	
C	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	305	0	305	0	0	5-6-8	1-8
C	305	0	305	0	0	5-6-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	216	142 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0
C	216	142 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	-14 / 0	-91.8	-91.8	0.09 (4)	6.25	D- E	-209 / 5	0.00 (1)
E- B	0 / 4	-91.8	-91.8	0.34 (1)	10.00			
C- B	-215 / 0	0.0	0.0	0.03 (1)	7.81			
A- D	-30 / 0	-18.5	-18.5	0.30 (1)	6.25			
D- C	0 / 0	-18.5	-18.5	0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (B-E:1) , BC=0.30/1.00 (A-D:1) , WB=0.00/1.00 (D-E:1) , SSI=0.17/1.00 (A-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (A) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311525

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	V5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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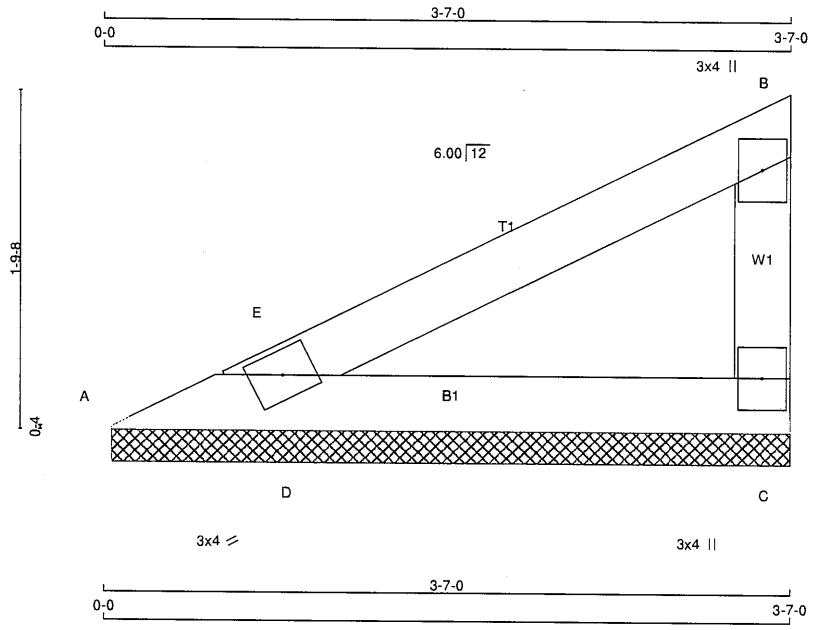

Scale = 1:11.7

TOTAL WEIGHT = 9 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	195	0	195	0	0	3-6-8	1-8
C	195	0	195	0	0	3-6-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	138	91 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	138	91 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	-15 / 0	-91.8	-91.8	0.06 (1)	6.25	D- E	-102 / 3	0.00 (1)	
E- B	0 / 8	-91.8	-91.8	0.13 (1)	10.00				
C- B	-133 / 0	0.0	0.0	0.01 (1)	7.81				
A- D	-13 / 0	-18.5	-18.5	0.13 (1)	6.25				
D- C	0 / 0	-18.5	-18.5	0.13 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (B-E:1) , BC=0.13/1.00 (A-D:1) , WB=0.00/1.00 (D-E:1) , SSI=0.09/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311526

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	J1	21	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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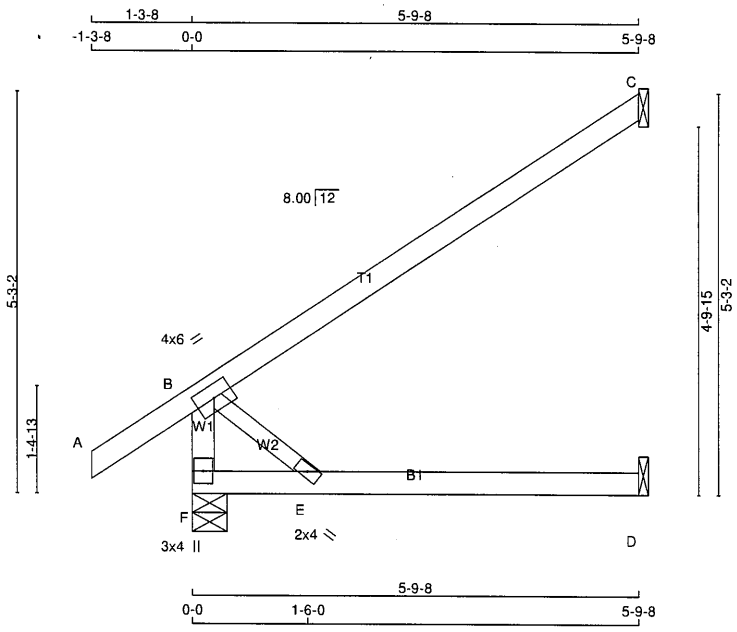

TOTAL WEIGHT = 21 X 19 = 399 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	445	0	445	0	0	5-8	1-8
C	266	0	266	0	0	1-8	1-8
D	54	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	312	218 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
C	183	148 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-392 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.52 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.18/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311434

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	J2	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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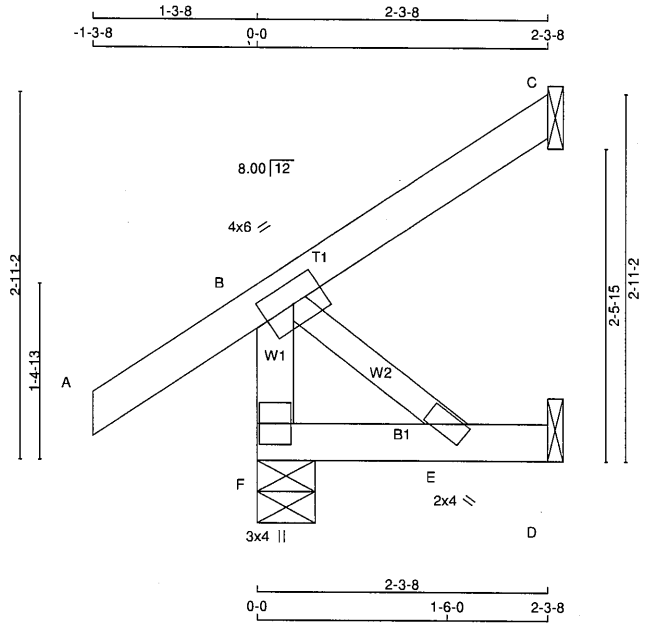

TOTAL WEIGHT = 3 X 10 = 31 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	252	0	252	0	0	5-8	1-8
C	105	0	105	0	0	1-8	1-8
D	21	0	24	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	176	129 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	72	59 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	17	0 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-231 / 0	0.0	0.0	0.02 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.13 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.08 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311435

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	J3	7	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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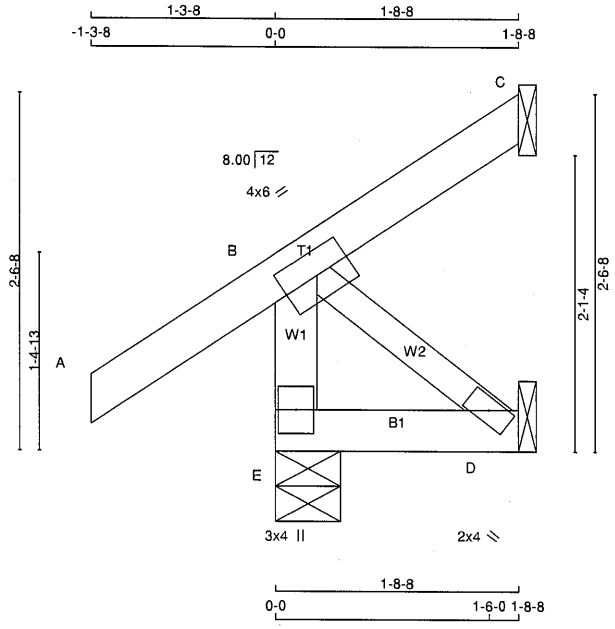

Scale = 1:15.7

TOTAL WEIGHT = 7 X 9 = 61 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
D	BMW1+w	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	269	0	269	0	0	5-8	1-8
C	28	0	28	0	-41	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	187	142 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
C	19	15 / -28	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-255 / 0	0.0	0.0	0.03 (1)	B- D	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)			
B- C	-28 / 0	-91.8	-91.8	0.12 (1)			
E- D	0 / 0	-18.5	-18.5	0.01 (4)			

MAX. UNBRAC LENGTH
7.81
10.00
6.25
10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311436

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	J4	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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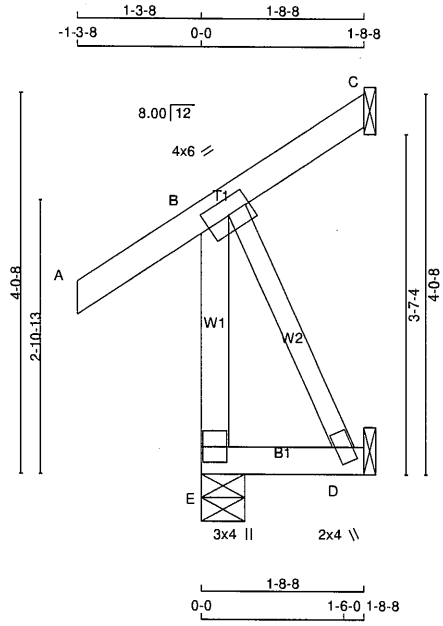

TOTAL WEIGHT = 3 X 11 = 34 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
D	BMW1+w	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	269	0	269	0	0	5-8	1-8
C	28	0	28	0	-41	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	187	142 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
C	19	15 / -28	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-255 / 0	0.0	0.0	0.04 (1)	7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	-28 / 0	-91.8	-91.8	0.12 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311437

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	J5	10	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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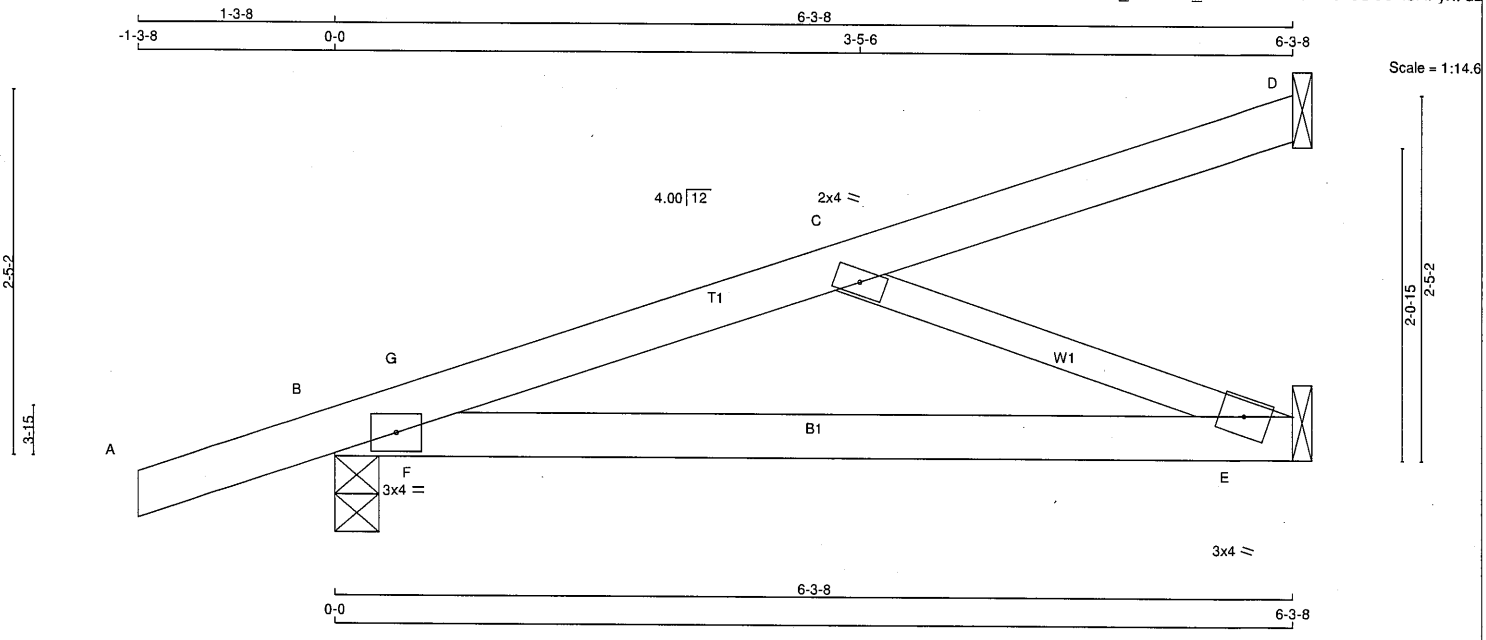

TOTAL WEIGHT = 10 X 19 = 189 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
E	BMW1+w	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	97	0	97	0	0	1-8	1-8
B	456	0	456	0	0	3-8	1-8
E	257	0	257	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	66	57 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
B	320	224 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
E	184	109 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-91.8	-91.8	0.11 (1)	10.00	
B- G	-531 / 0	-91.8	-91.8	0.10 (4)	6.25	
G- C	-462 / 0	-91.8	-91.8	0.11 (1)	6.25	
C- D	-11 / 0	-91.8	-91.8	0.11 (1)	6.25	
B- F	0 / 457	-18.5	-18.5	0.09 (4)	10.00	
F- E	0 / 457	-18.5	-18.5	0.17 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- E	-502 / 0	0.11 (1)
F- G	0 / 117	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.11/1.00 (C-G:1) , BC=0.17/1.00 (E-F:1) , WB=0.11/1.00 (C-E:1) , SSI=0.14/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 0.95)

Structural component only
DWG# T-2311438

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432488	J11	9	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 16:43:58 2023 Page 1
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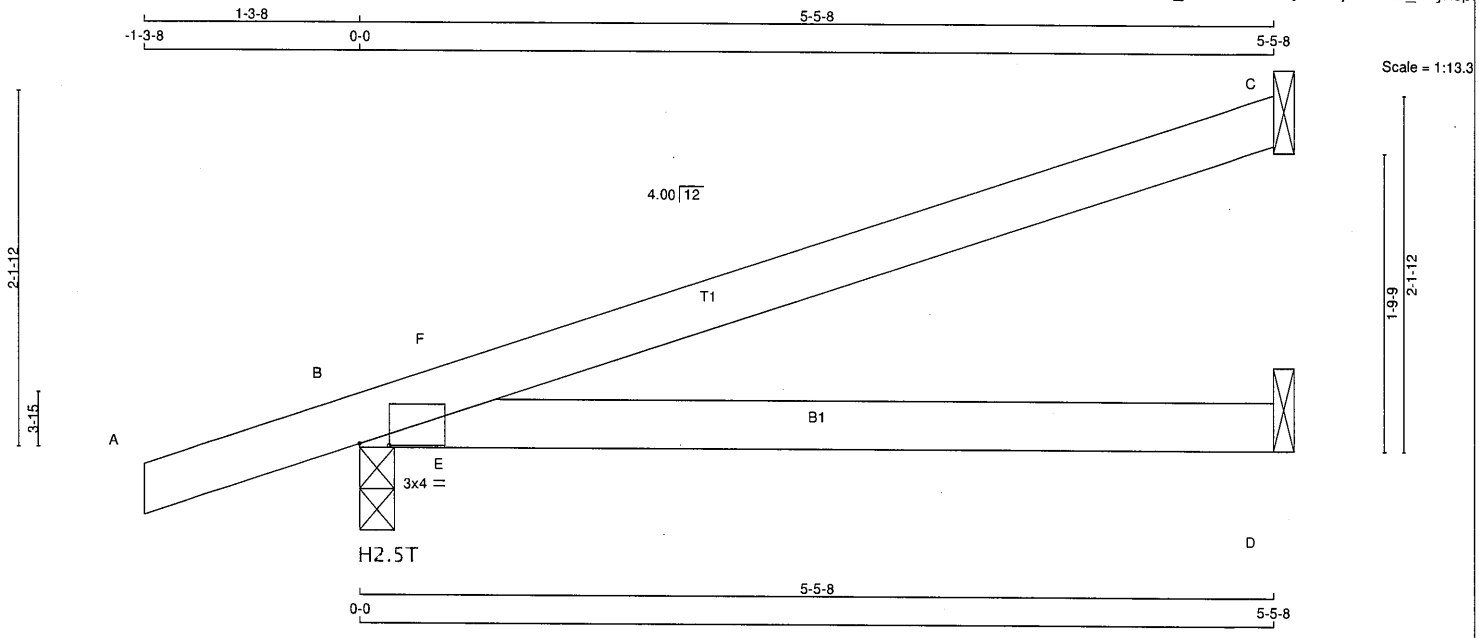

TOTAL WEIGHT = 9 X 14 = 130 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	2.25

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	218	0	218	0	-143	1-8	1-8
B	424	0	424	131	-254	2-8	1-8
D	83	0	83	0	-101	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 254 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 131 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	151	118 / 0	0 / 0	0 / 0	0 / -124	34 / 0	0 / 0
B	297	208 / 0	0 / 0	0 / 0	0 / -238	89 / 0	0 / 0
D	62	22 / 0	0 / 0	0 / 0	0 / -98	40 / 0	0 / 0
HORIZONTAL REACTIONS							
B	---	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (12)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 18	-91.8	-91.8	0.11 (1)	10.00	E- F	-304 / 110	0.00 (1)
B- F	-93 / 14	-91.8	-91.8	0.06 (12)	6.25			
F- C	-53 / 2	-91.8	-91.8	0.35 (1)	6.25			
B- E	0 / 0	-18.5	-18.5	0.25 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.25 (1)	10.00			

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 9.2} PSF AT {15-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 819 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 607 (0.11")

CSI: TC=0.35/1.00 (C-F:1) , BC=0.25/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.24/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 0.95)

Structural component only
DWG# T-2311467

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	J21	12	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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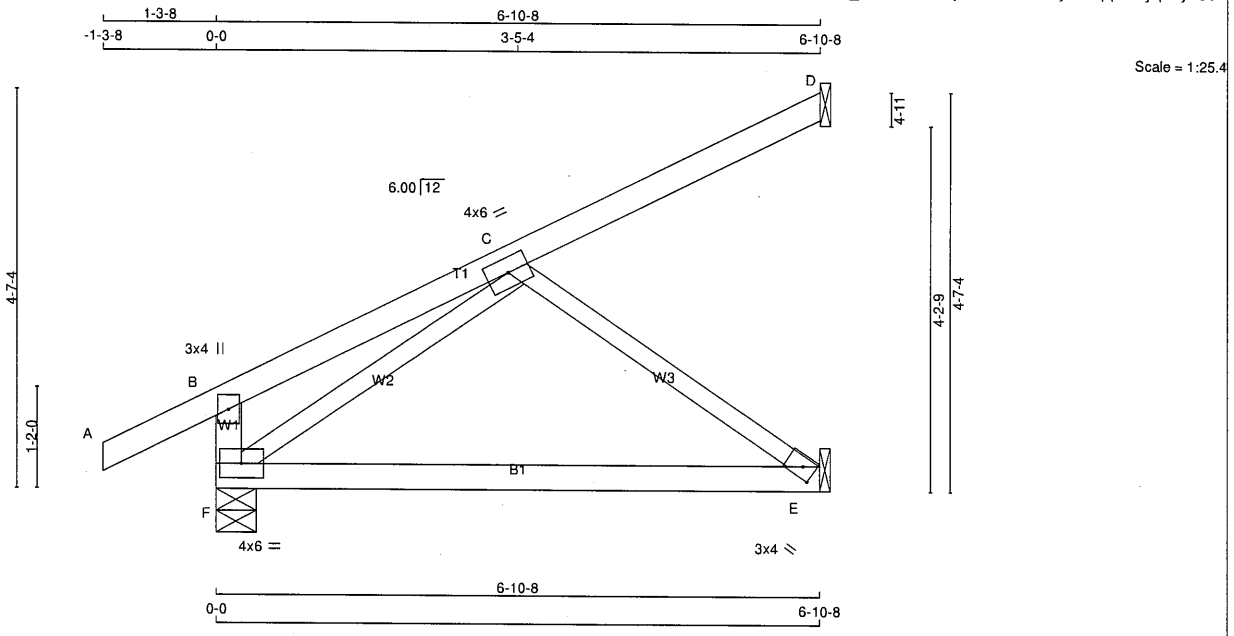

TOTAL WEIGHT = 12 X 26 = 309 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
E	BMW1+w	MT20	3.0	4.0	1.50	1.50
F	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	497	0	497	0	0	5-8	1-8
D	125	0	125	0	0	1-8	1-8
E	257	0	257	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	349	242 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0
D	86	70 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	184	109 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-239 / 0	0.0	0.0	0.02 (1)	7.81	F- C	-324 / 0	0.09 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- E	-324 / 0	0.11 (1)
B- C	0 / 17	-91.8	-91.8	0.18 (1)	10.00			
C- D	-16 / 0	-91.8	-91.8	0.14 (1)	6.25			
F- E	0 / 258	-18.5	-18.5	0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 888 (0.09")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.25/1.00 (E-F:4) , WB=0.11/1.00 (C-E:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (E) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311485

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	J21A	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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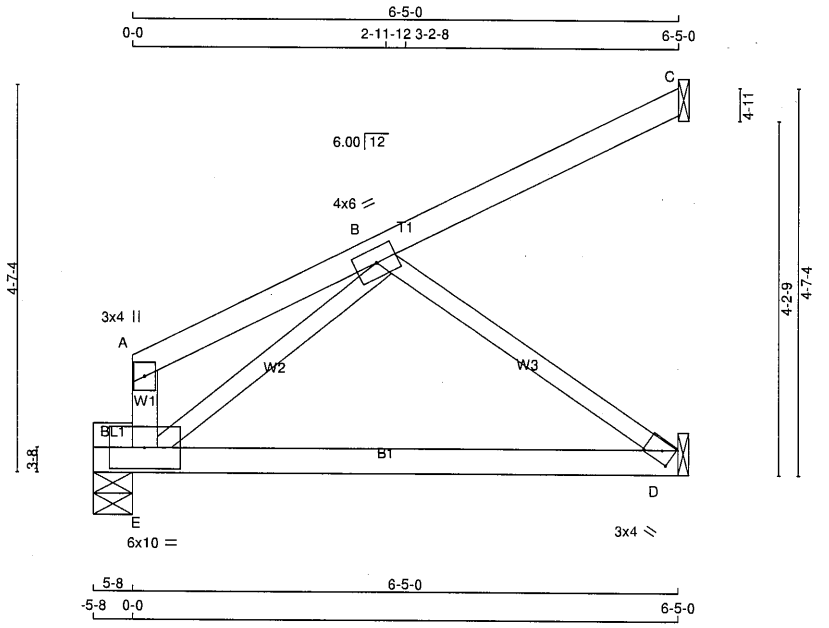

TOTAL WEIGHT = 2 X 24 = 48 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
D	BMW1+w	MT20	3.0	4.0	1.50	1.50
E	BMVWK1-t	MT20	6.0	10.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	353	0	353	0	0	5-8	5-0
C	129	0	129	0	0	1-8	1-8
D	220	0	220	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	249	165 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
C	89	72 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	157	91 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-87 / 0	0.0	0.0	0.01 (1)	7.81	E- B	-309 / 0	0.08 (1)
A- B	0 / 31	-91.8	-91.8	0.16 (1)	10.00	B- D	-270 / 0	0.09 (1)
B- C	-15 / 0	-91.8	-91.8	0.12 (1)	6.25			
E- D	0 / 216	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.21/1.00 (D-E:4) , WB=0.09/1.00 (B-D:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (D) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 0.95)

Structural component only
DWG# T-2311486

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	J22	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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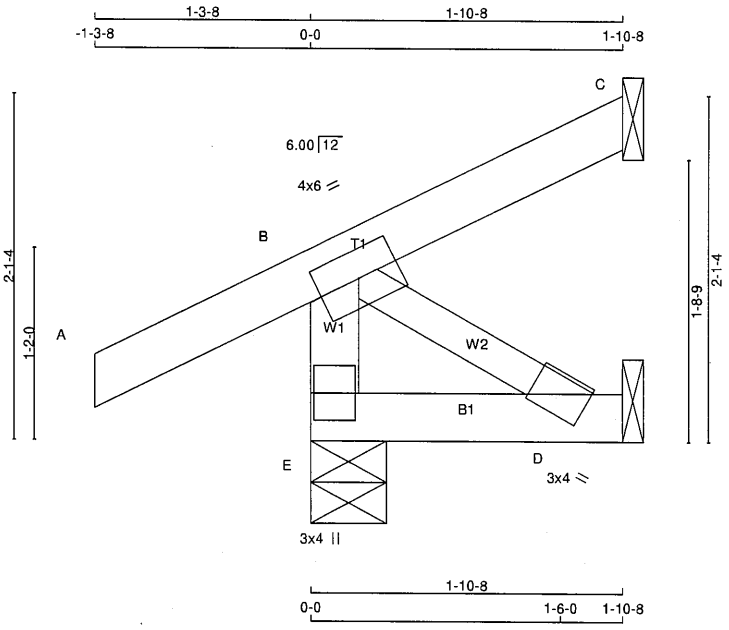

Scale = 1:13.4

TOTAL WEIGHT = 3 X 8 = 25 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
D	BMW1+w	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	269	0	269	0	0	5-8	1-8
C	41	0	41	0	-35	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	187	143 / 0	0 / 0	0 / 0	0 / 0	45 / 0	0 / 0
C	28	23 / -25	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-20 / 0	-91.8	-91.8	0.11 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311487

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
432489	J23	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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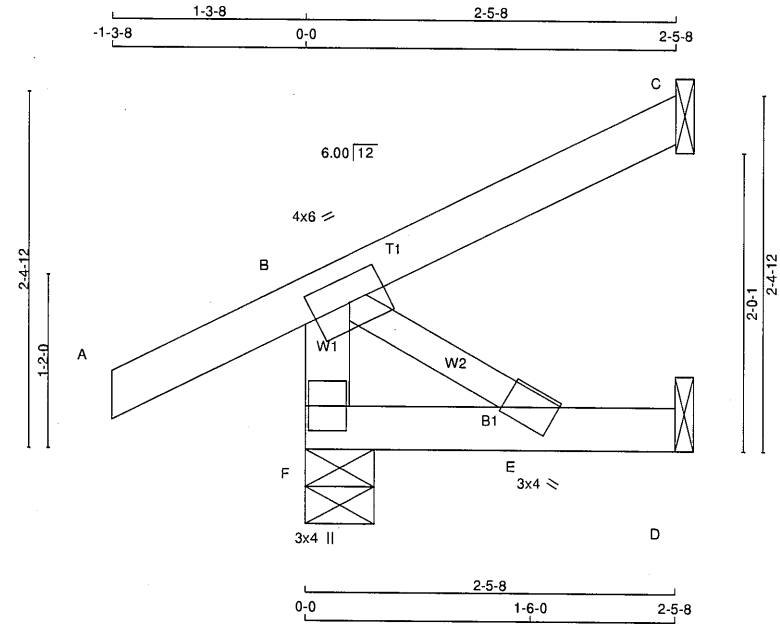

TOTAL WEIGHT = 3 X 10 = 29 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	260	0	260	0	0	5-8	1-8
C	113	0	113	0	0	1-8	1-8
D	23	0	25	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	182	132 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
C	78	63 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-237 / 0	0.0	0.0	0.02 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.09 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311488

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	J24	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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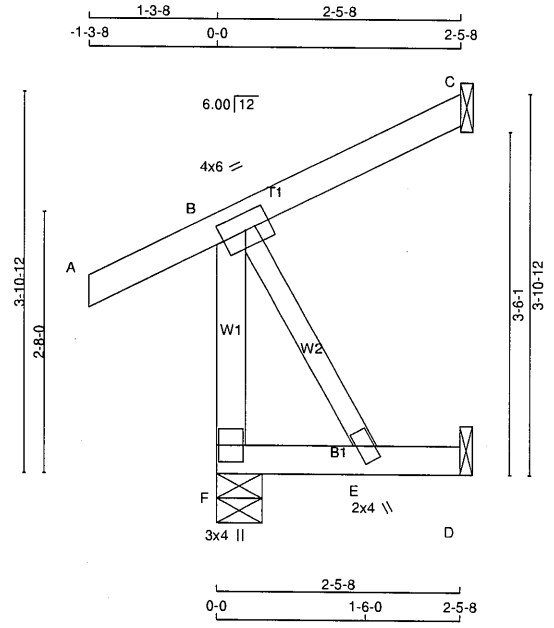

TOTAL WEIGHT = 4 X 12 = 49 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	260	0	260	0	0	5-8	1-8
C	113	0	113	0	0	1-8	1-8
D	23	0	25	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	182	132 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
C	78	63 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-237 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.09 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.03 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:5) , BC=0.03/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311489

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	C1	5	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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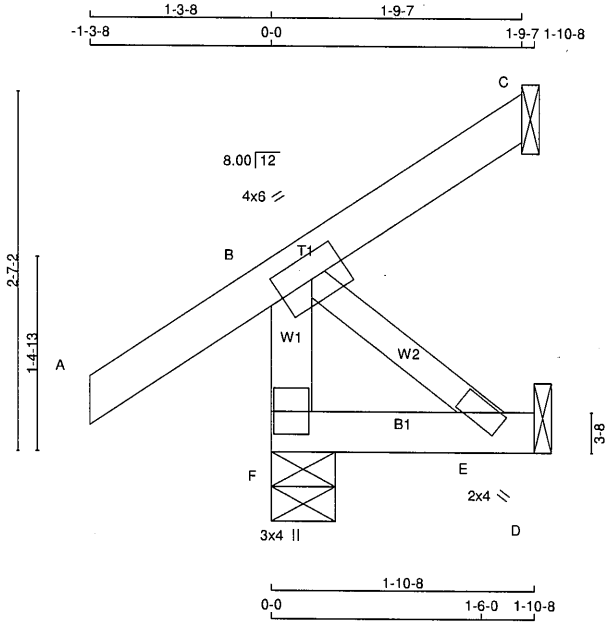

Scale: 3/4"=1'

TOTAL WEIGHT = 5 X 9 = 45 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMVW-t	MT20	4.0	6.0	
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	274	0	274	0	0	5-8	1-8
C	34	0	34	0	-39	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311430

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	C2	5	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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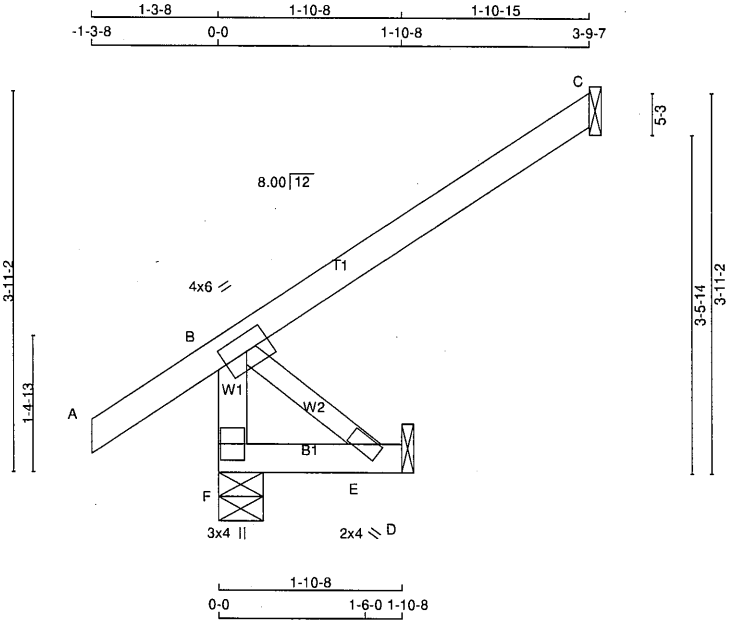

TOTAL WEIGHT = 5 X 12 = 59 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	317	0	317	0	0	5-8	1-8
C	174	0	174	0	0	1-8	1-8
D	17	0	19	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	220	167 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-300 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.13 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.02/1.00 (E-F:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311431

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	C3	5	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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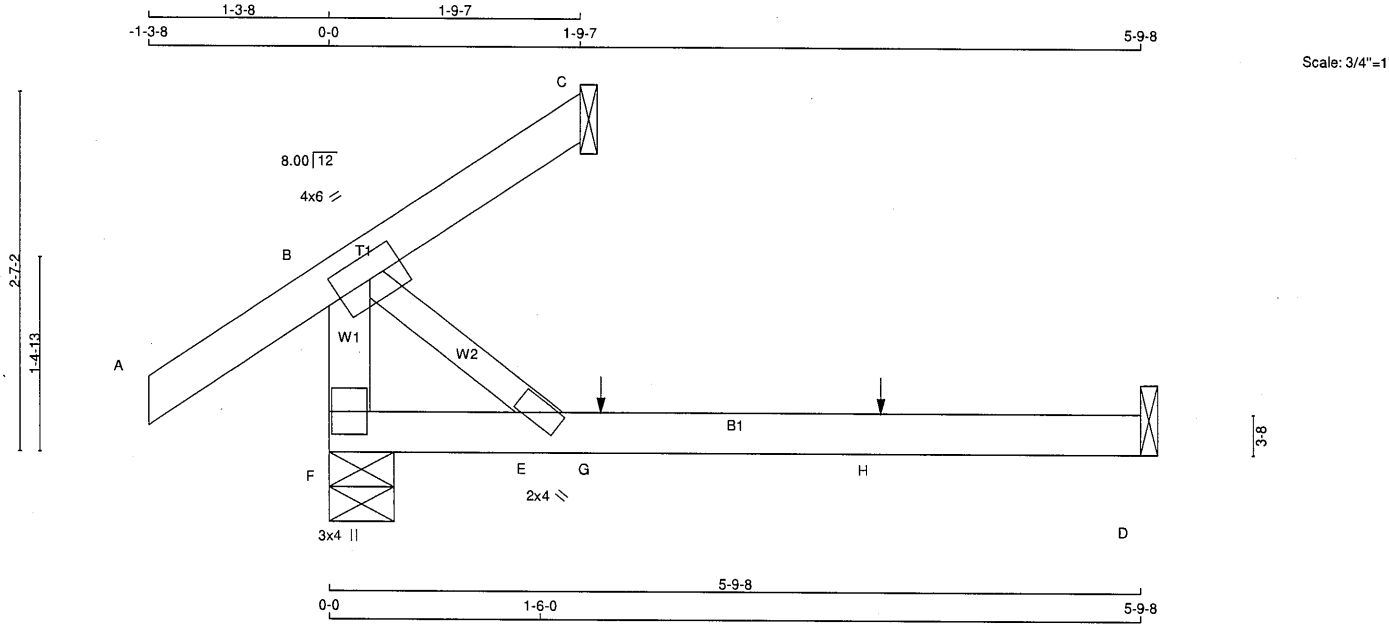

TOTAL WEIGHT = 5 X 13 = 67 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	310	0	310	0	0	5-8	1-8
C	34	0	34	0	0	1-8	1-8
D	54	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	219	143 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
C	23	19 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.13 (1)	10.00			
B- C	-27 / 0	-91.8	-91.8	0.12 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
H- D	0 / 0	-18.5	-18.5	0.18 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.18/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311432

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432478	C4	5	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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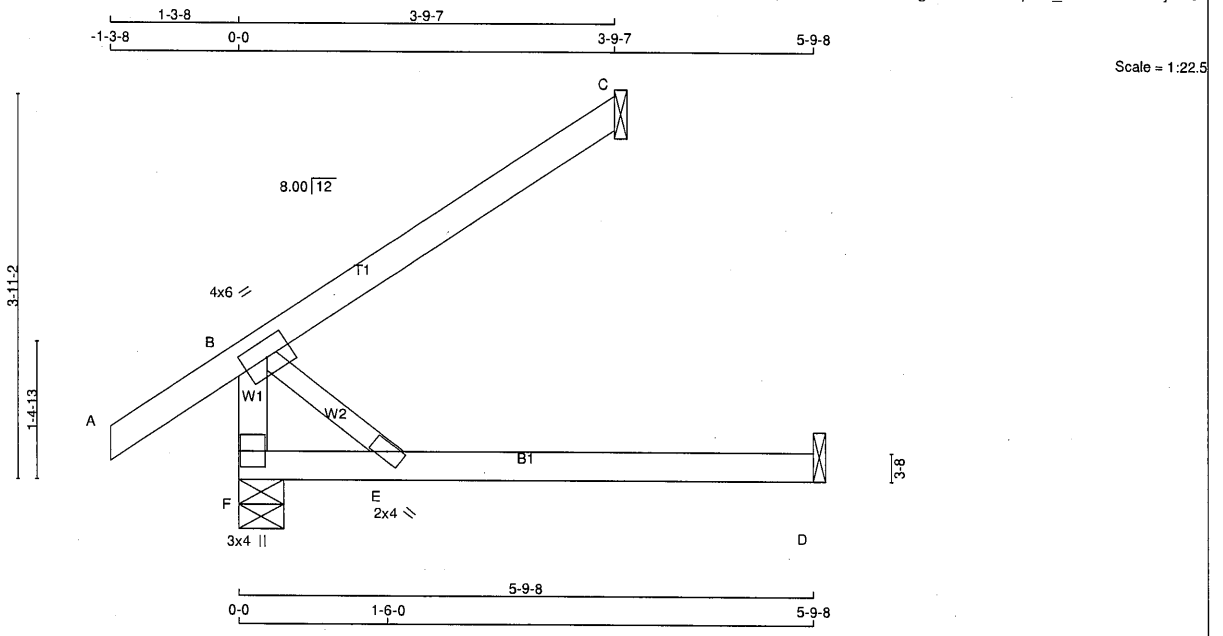

TOTAL WEIGHT = 5 X 16 = 81 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	353	0	353	0	0	5-8	1-8
C	174	0	174	0	0	1-8	1-8
D	54	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	249	167 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-300 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 35	-91.8	-91.8	0.12 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.18/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311433

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C21	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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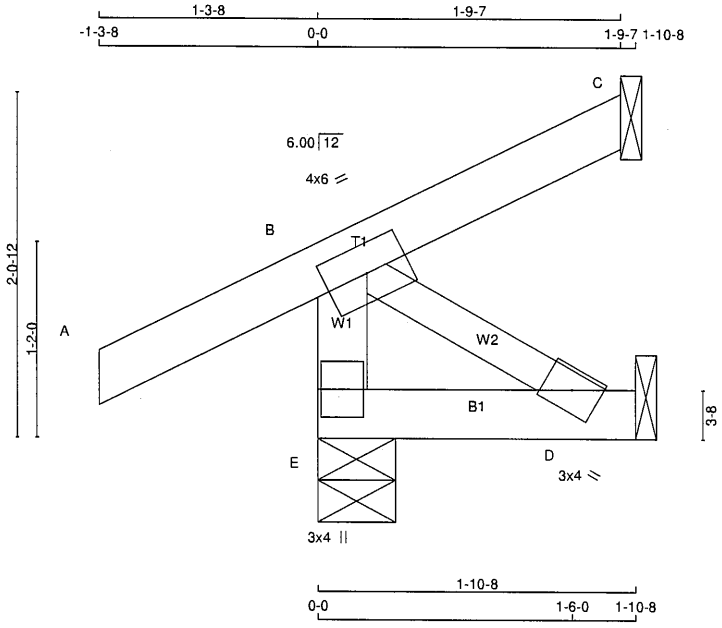

Scale = 1:13.2

TOTAL WEIGHT = 4 X 8 = 33 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
D	BMW1+w	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	268	0	268	0	0	5-8	1-8
C	35	0	35	0	-38	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	186	142 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
C	24	19 / -26	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-254 / 0	0.0	0.0	0.03 (1)	7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	-21 / 0	-91.8	-91.8	0.11 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.12/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311475

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C22	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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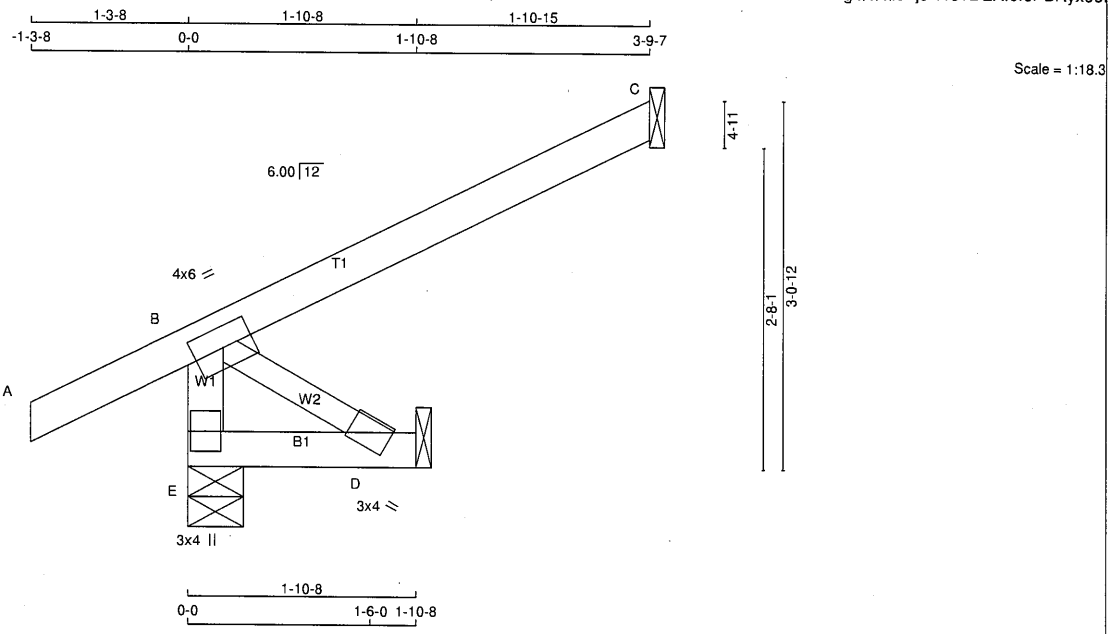

TOTAL WEIGHT = 4 X 11 = 43 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
D	BMW1+w	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	312	0	312	0	0	5-8	1-8
C	174	0	174	0	0	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	216	166 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD FROM	LC1 TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-298 / 0	0.0	0.0	0.03 (1)	7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (5)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

Structural component only
DWG# T-2311476

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C23	4	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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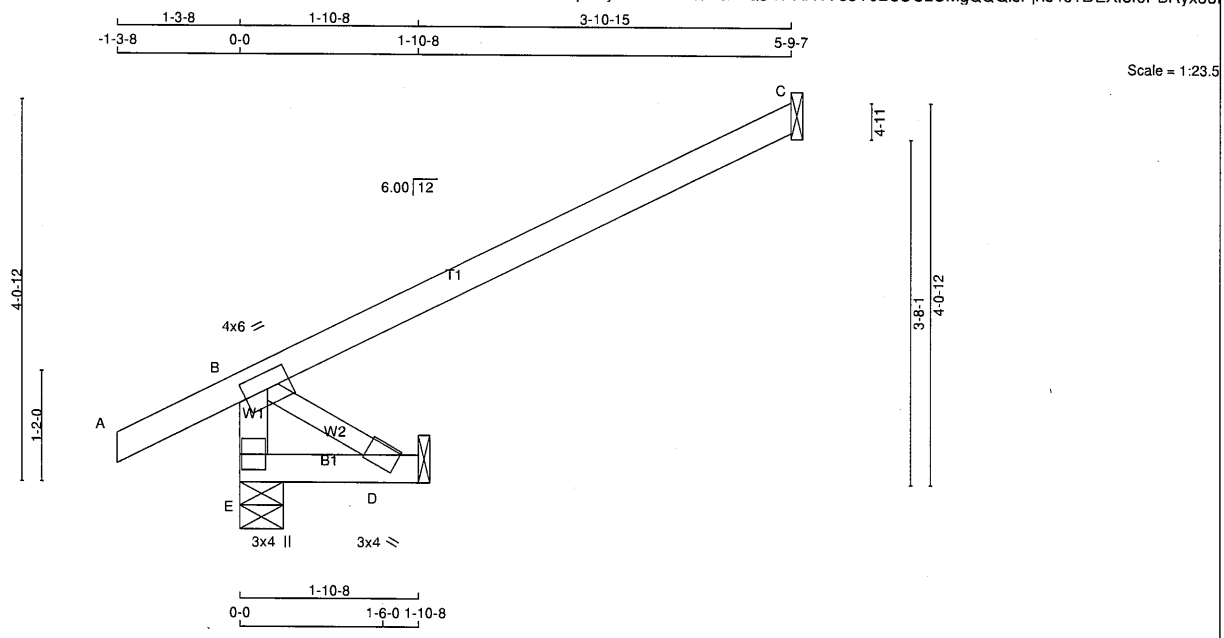

TOTAL WEIGHT = 4 X 13 = 54 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
D	BMW1+w	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	404	0	404	0	0	5-8	1-8
C	265	0	265	0	0	1-8	1-8
D	14	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	280	217 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	183	148 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E .

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO						
E- B	-390 / 0	0.0 0.0	0.04 (1)		7.81	B- D	0 / 0	0.00 (1)
A- B	0 / 28	-91.8 -91.8	0.12 (1)		10.00			
B- C	0 / 0	-91.8 -91.8	0.52 (1)		10.00			
E- D	0 / 0	-18.5 -18.5	0.01 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.52/1.00 (B-C:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (B-D:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 0.95)

Structural component only
DWG# T-2311477

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C24	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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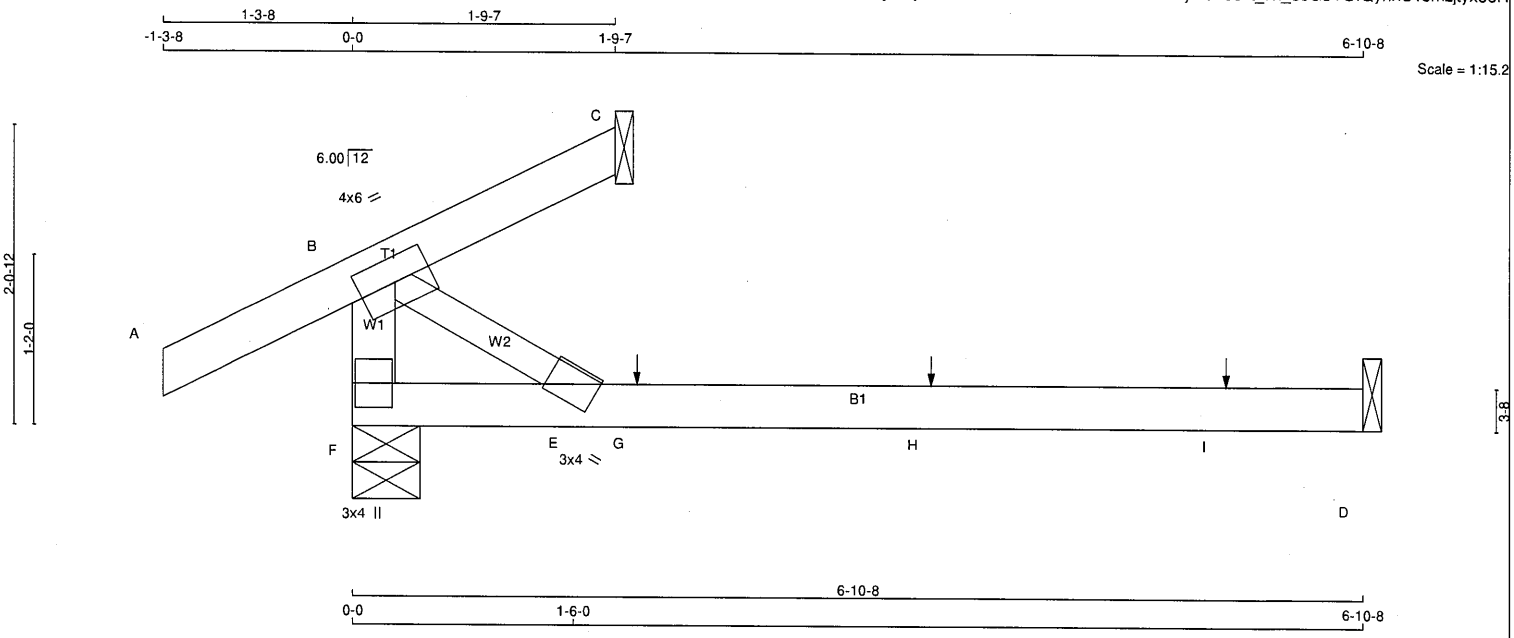

TOTAL WEIGHT = 3 X 14 = 42 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	317	0	317	0	0	5-8	1-8
C	35	0	35	0	0	1-8	1-8
D	64	0	71	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	226	142 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
C	24	19 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	51	0 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-254 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.13 (1)	10.00			
B- C	-21 / 0	-91.8	-91.8	0.09 (1)	6.25			
F- E	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
E- G	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
H- I	0 / 0	-18.5	-18.5	0.26 (4)	10.00			
I- D	0 / 0	-18.5	-18.5	0.26 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
I	5-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	25.6	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 819 (0.10")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.26/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=0.95 NAIL=0.95 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

Structural component only
DWG# T-2311478

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C24A	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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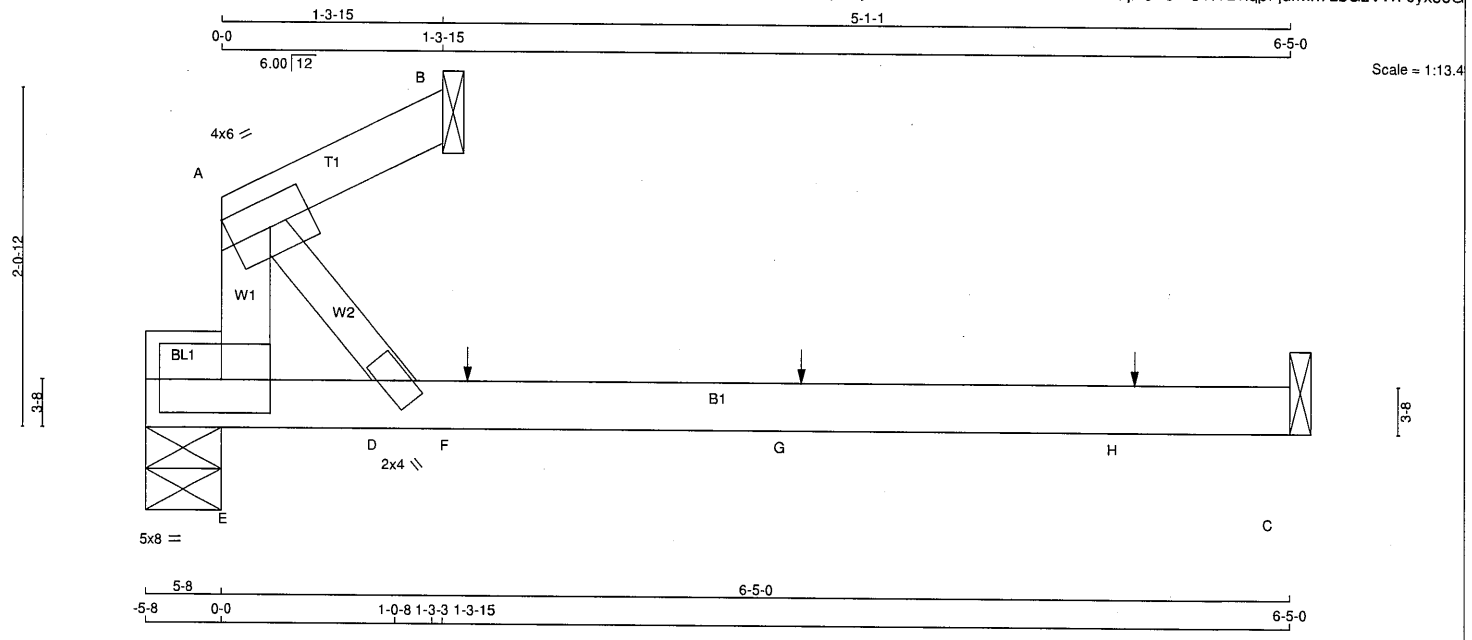

TOTAL WEIGHT = 12 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
D	BMW+w	MT20	2.0	4.0		
E	BMVK1m	MT20	5.0	8.0	2.50	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	120	0	120	0	0	5-8	5-0
B	61	0	61	0	0	1-8	1-8
C	57	0	65	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	89	35 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
B	42	34 / 0	0 / 0	0 / 0	0 / 0	8 / 0	0 / 0
C	45	0 / -1	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED MAX CSI (LC)
E- A	-71 / 0	0.0	0.0	0.01 (1)	7.81	A- D	0 / 14	0.00 (1)
A- B	0 / 0	-91.8	-91.8	0.03 (1)	10.00			
E- D	-10 / 0	-18.5	-18.5	0.08 (4)	10.00			
D- F	0 / 0	-18.5	-18.5	0.21 (4)	10.00			
F- G	0 / 0	-18.5	-18.5	0.21 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.21 (4)	10.00			
H- C	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-5-12	1	1	---	BACK	VERT	TOTAL	---	C1
G	3-5-12	1	1	---	BACK	VERT	TOTAL	---	C1
H	5-5-12	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.03/1.00 (A-B:1) , BC=0.21/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.08/1.00 (C-D:4)

DOL LUMBER=0.79 NAIL=0.79 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.01 (A) (INPUT = 0.95)

Structural component only
DWG# T-2311479

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C25	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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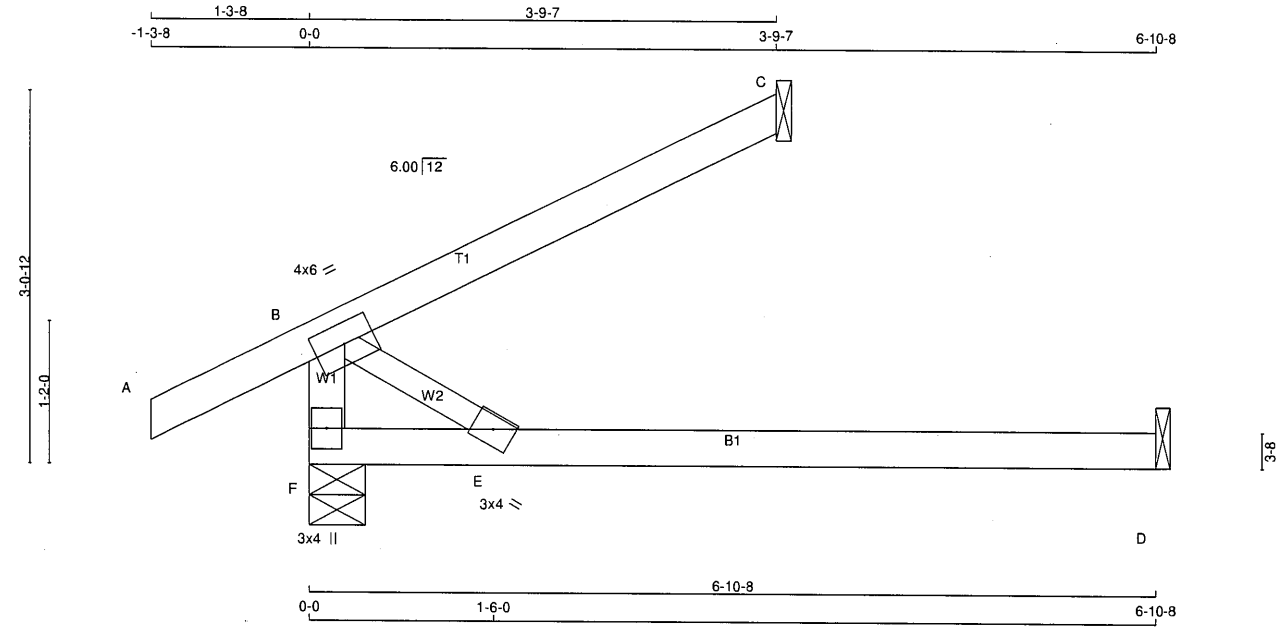

TOTAL WEIGHT = 3 X 17 = 50 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0		
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	362	0	362	0	0	5-8	1-8
C	174	0	174	0	0	1-8	1-8
D	64	0	71	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	256	166 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
C	120	97 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	51	0 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-298 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00			
B- C	0 / 0	-91.8	-91.8	0.22 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 819 (0.10")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.26/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311480

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C25A	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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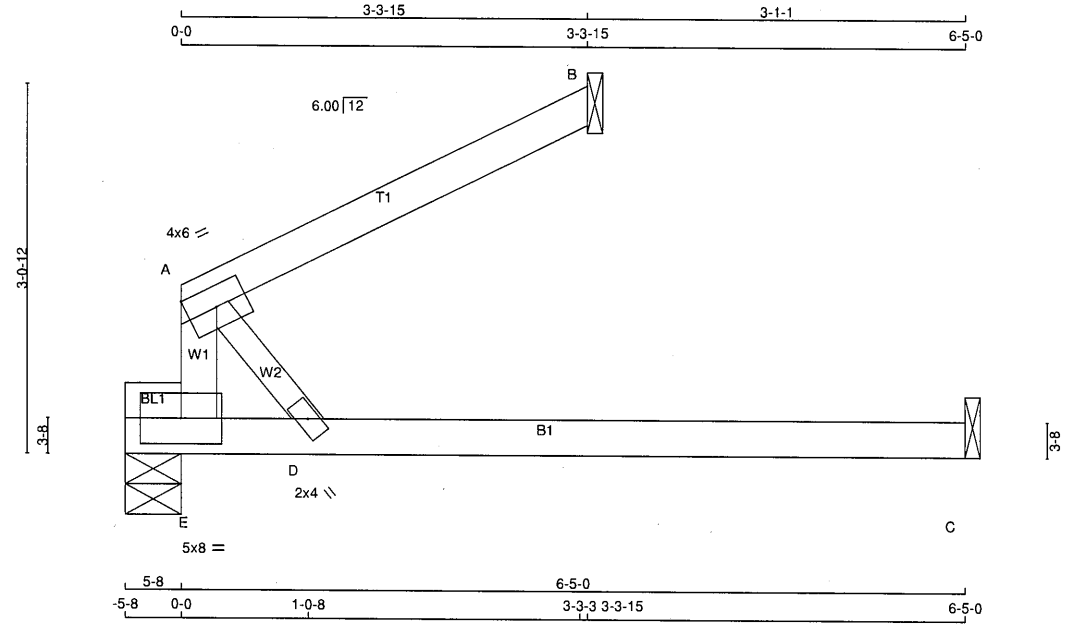

TOTAL WEIGHT = 15 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
D	BMW+w	MT20	2.0	4.0		
E	BMVK1m	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	214	0	214	0	0	5-8	5-0
B	153	0	153	0	0	1-8	1-8
C	54	0	64	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	154	87 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
B	105	85 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
C	44	0 / -2	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-179 / 0	0.0	0.0	0.02 (1)	7.81	A- D	0 / 34	0.01 (1)
A- B	0 / 0	-91.8	-91.8	0.17 (1)	10.00			
E- D	-24 / 0	-18.5	-18.5	0.07 (4)	6.25			
D- C	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.21/1.00 (C-D:4) , WB=0.01/1.00 (A-D:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.09 (A) (INPUT = 0.90)
JSI METAL= 0.03 (A) (INPUT = 0.95)

Structural component only
DWG# T-2311481

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C26	3	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:32 2023 Page 1
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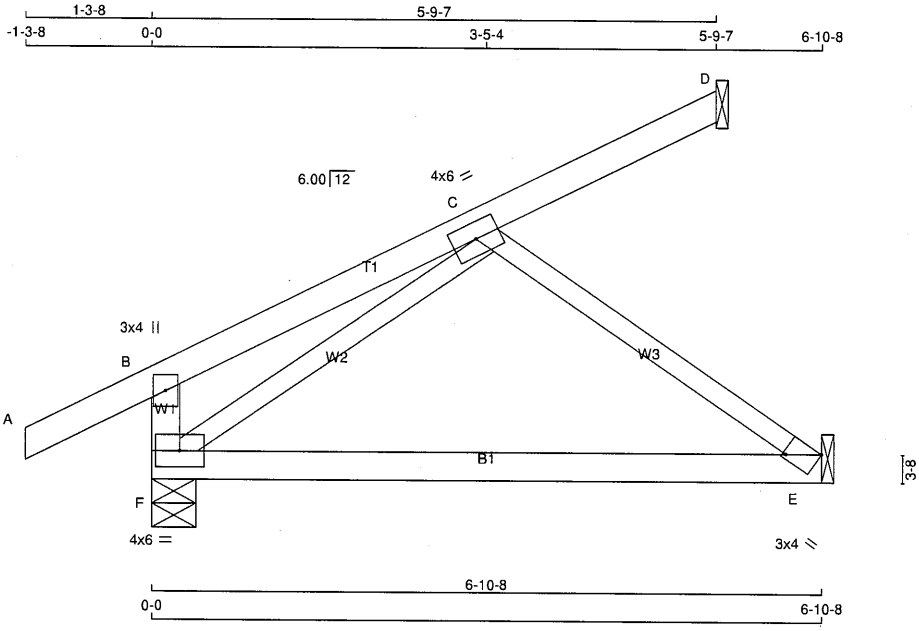

TOTAL WEIGHT = 3 X 24 = 73 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
E	BMW1+w	MT20	3.0	4.0	3.50	Edge
F	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	478	0	478	0	0	5-8	1-8
D	73	0	73	0	0	1-8	1-8
E	228	0	228	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	336	232 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0
D	50	41 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
E	164	93 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-250 / 0	0.0	0.0	0.03 (1)	7.81	F- C	-276 / 0	0.08 (1)
A- B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	C- E	-276 / 0	0.09 (1)
B- C	0 / 13	-91.8	-91.8	0.13 (1)	10.00			
C- D	-17 / 0	-91.8	-91.8	0.10 (1)	6.25			
F- E	0 / 220	-18.5	-18.5	0.25 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 888 (0.09")

CSI: TC=0.13/1.00 (B-C:1) , BC=0.25/1.00 (E-F:4) , WB=0.09/1.00 (C-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 0.95)

LICENSED PROFESSIONAL ENGINEER
07-17-23
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-2311482

CITY OF RICHMOND HILL
BUILDING DIVISION
05/01/2024
RECEIVED
Per: joshua.nabua

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
432489	C26A	1	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Jul 17 17:32:33 2023 Page 1
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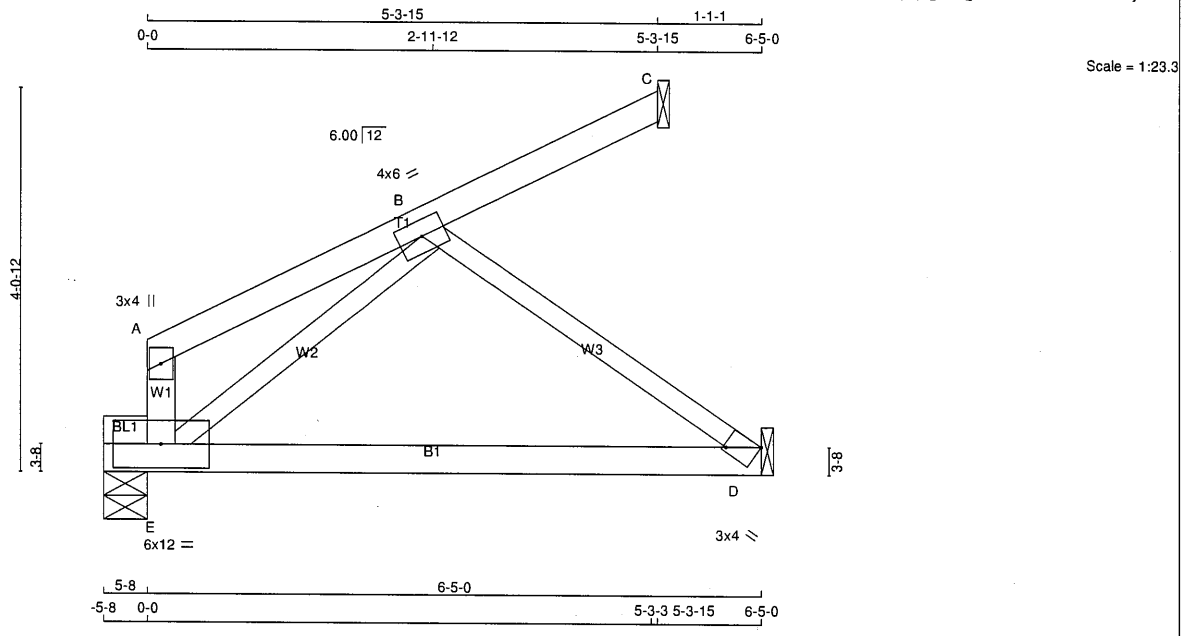


TOTAL WEIGHT = 23 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
D	BMW1+w	MT20	3.0	4.0	3.50	Edge
E	BMVWK1-t	MT20	6.0	12.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	331	0	331	0	0	5-8	5-0
C	81	0	81	0	0	1-8	1-8
D	190	0	190	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	234	153 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	56	45 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	137	74 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-100 / 0	0.0	0.0	0.01 (1)	7.81	E- B	-258 / 0	0.06 (1)
A- B	0 / 27	-91.8	-91.8	0.11 (1)	10.00	B- D	-221 / 0	0.07 (1)
B- C	-14 / 0	-91.8	-91.8	0.08 (1)	6.25			
E- D	0 / 176	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.21/1.00 (D-E:4) , WB=0.07/1.00 (B-D:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

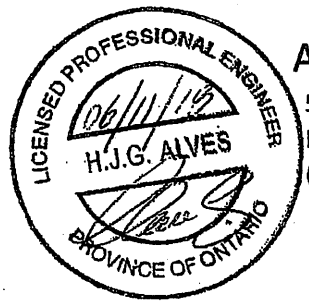
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (D) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 0.95)

Structural component only
DWG# T-2311483



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering tem and General Safety notes.

T-1800218

Feb 09, 2018

MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

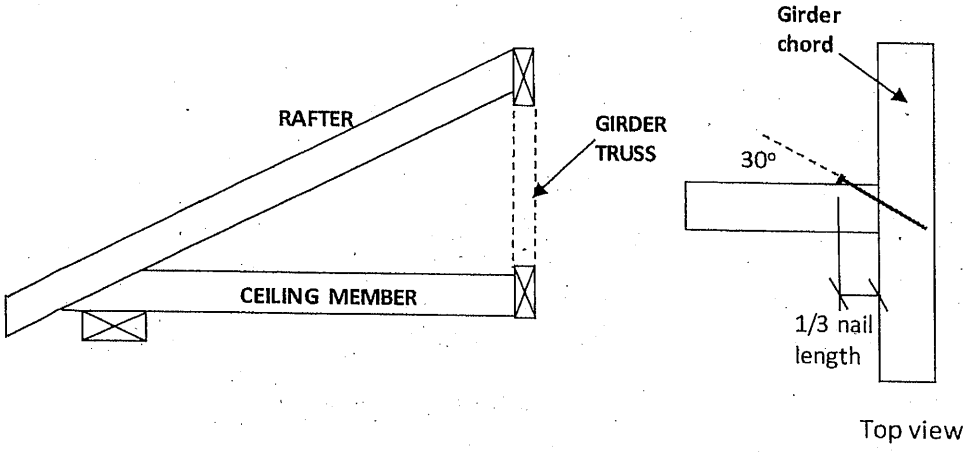


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

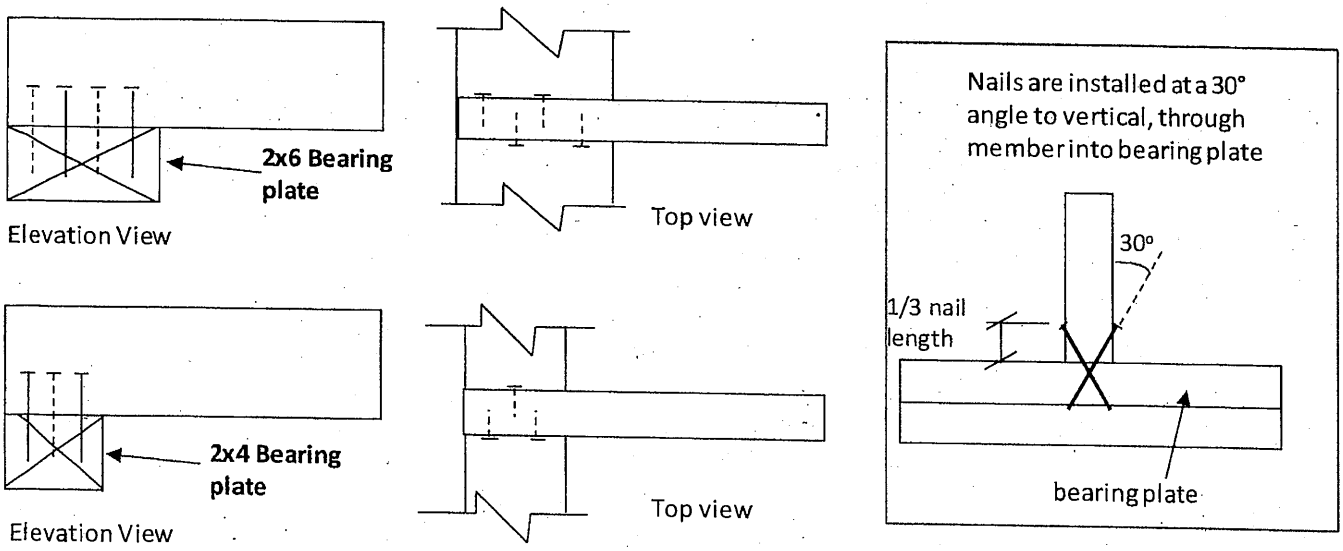
MiTek®

STANDARD DETAIL MSD2015-H

Issued: **SEPTEMBER 22, 2020**
Expiry: **APRIL 30, 2022**

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

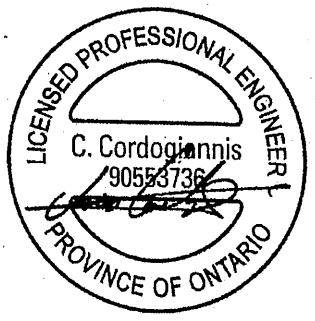
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



December 21, 2020

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

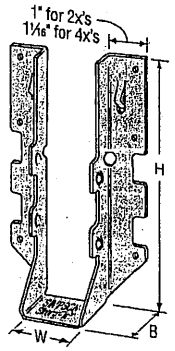
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

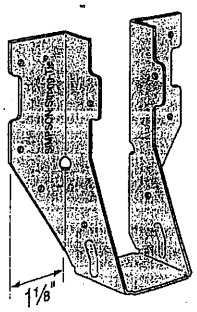
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

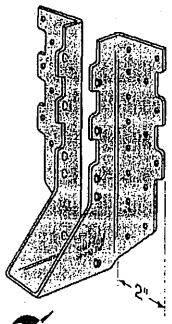
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



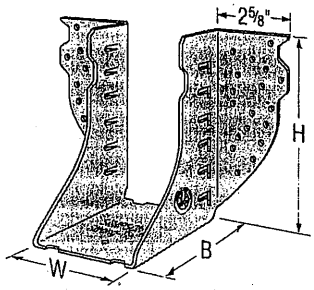
LUS28



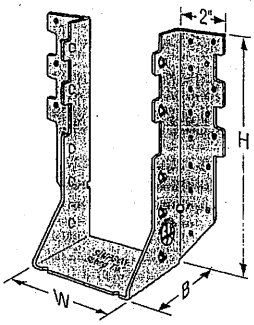
LU26L



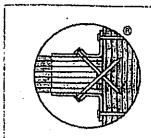
HUS210
(HUS26, HUS28, and HHUS similar)



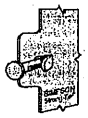
HGUS28-2



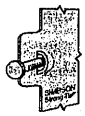
HHUS210-2



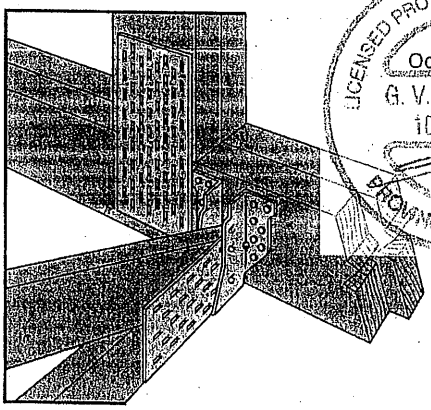
Double-Shear Nailing Top View



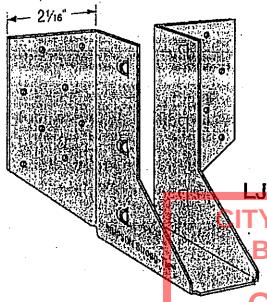
Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View *(available on some models)*



Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

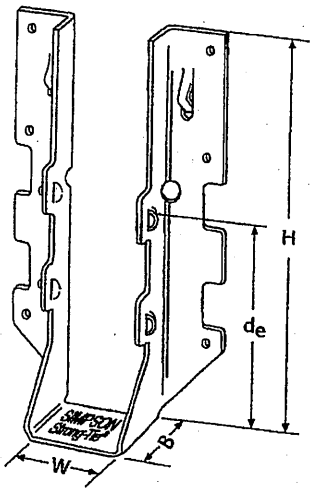
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

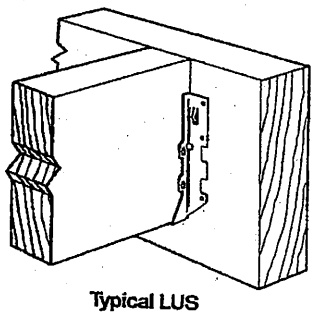
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

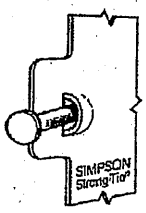


LUS28


Typical LUS Installation

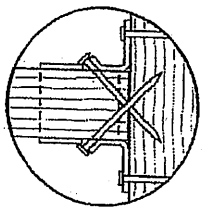
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

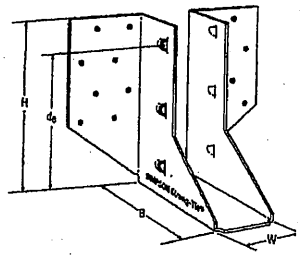
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

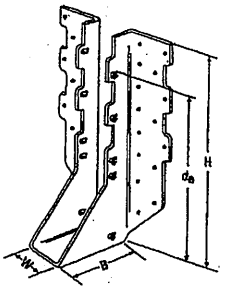
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

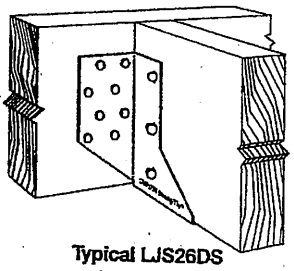
- See current catalogue for options



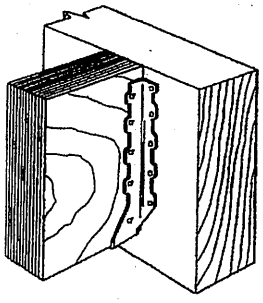
LJS26DS



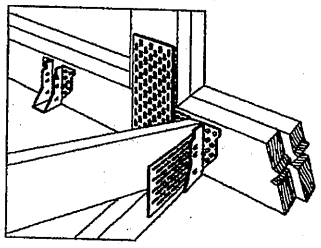
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



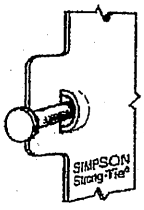
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

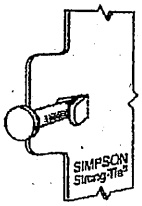
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6³⁄₃₂	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

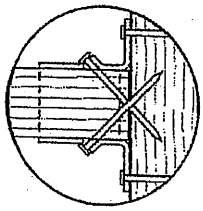


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

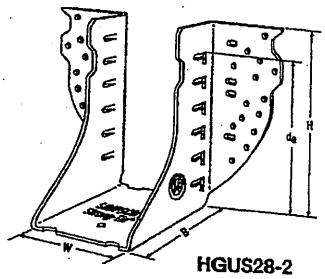
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

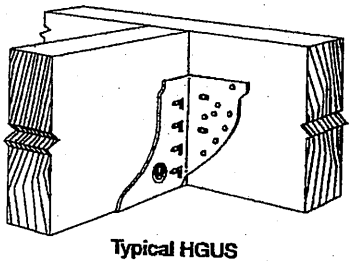
Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

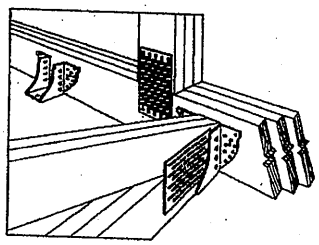

Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

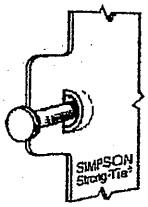
Options:

- See current catalogue for options


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

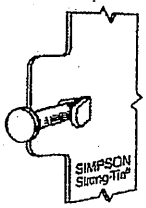
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⅜₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⅜₁₆	5⅞₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⅜₁₆	7¾₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⅜₁₆	7¾₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅛	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3⅜₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⅜₁₆	9⅜₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⅜₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⅜₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

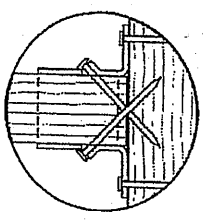


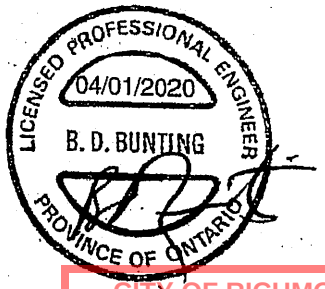
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

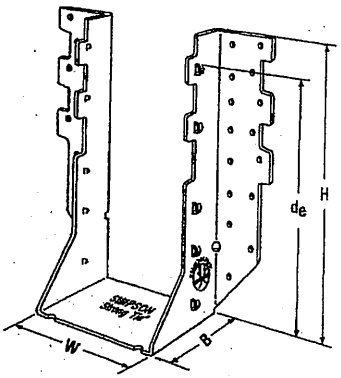
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

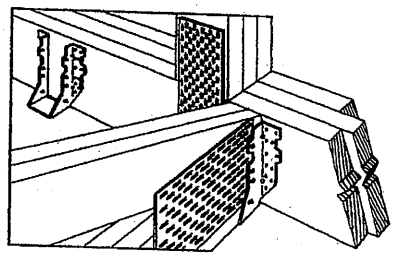
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HHUS410

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



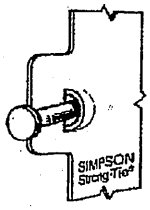
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Options:

- See current catalogue for options

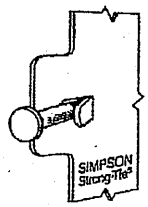
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L.		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

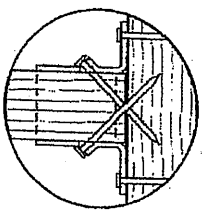


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

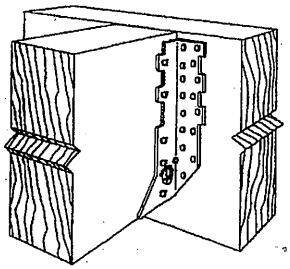
U.S. Patent 5,603,580



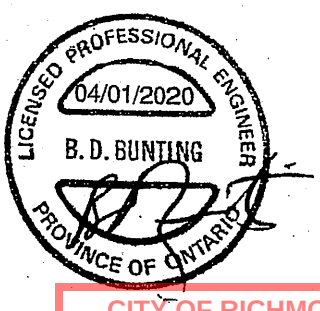
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



Typical HHUS Installation



LIMIT STATES DESIGN

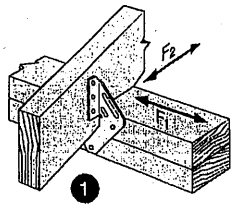
This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHHUS20 3/20 exp. 6/22

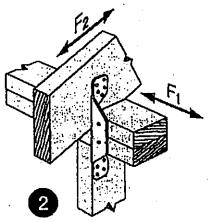
(800) 999-5099
strongtie.com

H/TSP

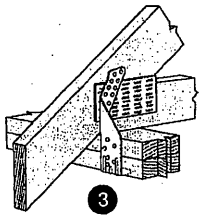
Seismic and Hurricane Ties (cont.)



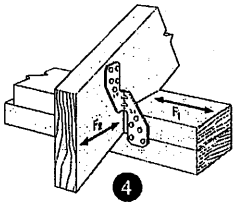
1 H1 Installation



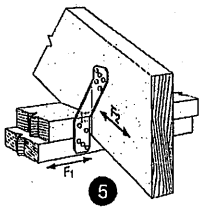
2 H2A Installation



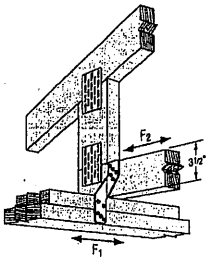
3 TSP Installation



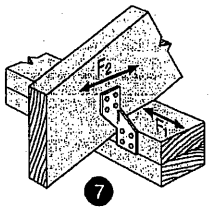
4 H2.5A Installation
(Nails into both top plates)



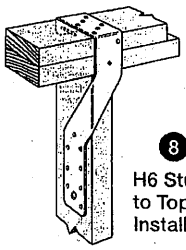
5 H2.5T Installation
(Nails into both top plates)



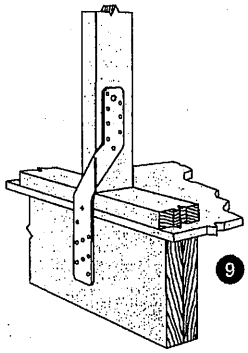
6 H2.5T Installation



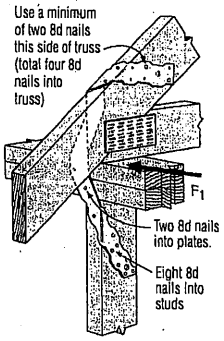
7 H3 Installation
(Nails into upper top plate)



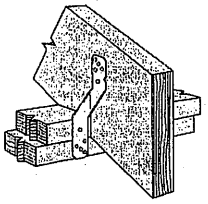
8 H6 Stud to Top Plate Installation



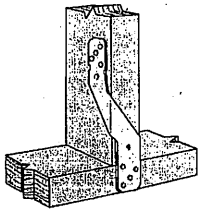
9 H6 Stud to Band Joist Installation



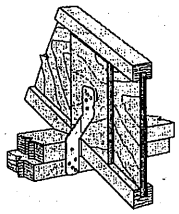
10 H7Z Installation



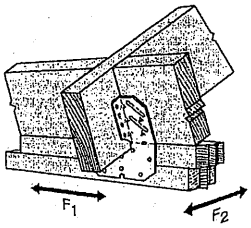
11 H8 Attaching Rafter to Double Top Plates



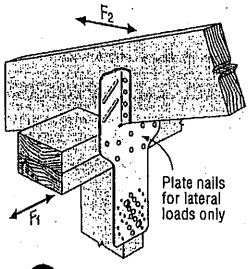
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



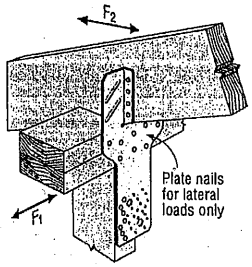
13 H8 attaching I-Joist to Double Top Plates



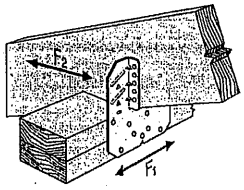
14 H10A Field-Bent Installation



15 H10S Installation



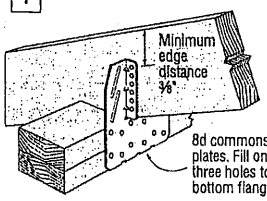
16 H10S Installation with Stud Offset



17 H10A Installation

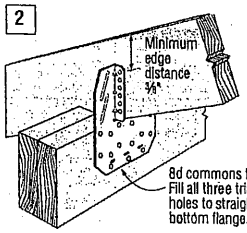
H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

1



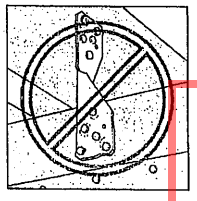
18 H14 Installation to Double Top Plates

2



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

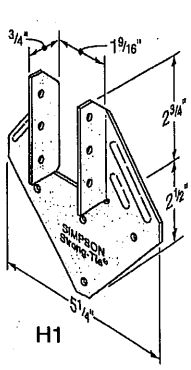
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

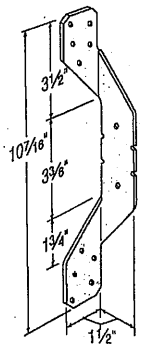
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit **strongtie.com**.

Installation:

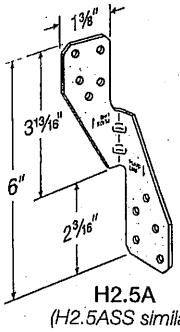
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



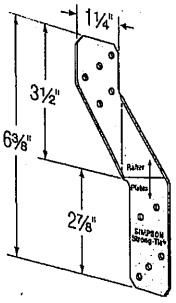
H1



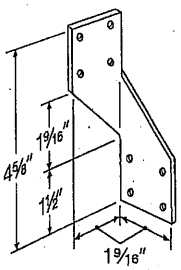
H2A
(H2ASS similar)



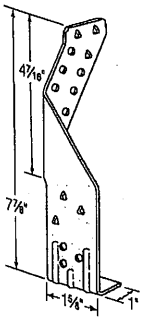
H2.5A
(H2.5ASS similar)



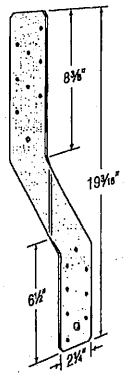
H2.5T



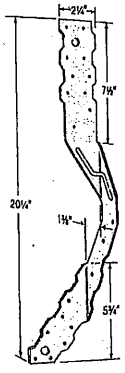
H3



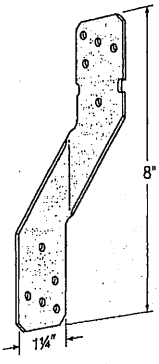
TSP



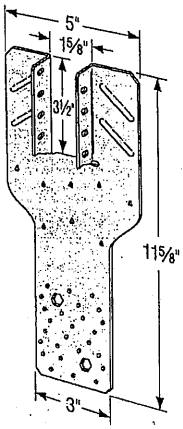
H6



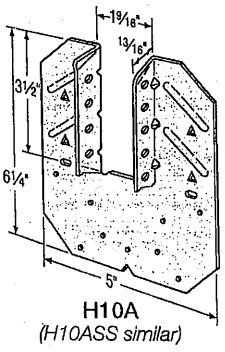
H7Z



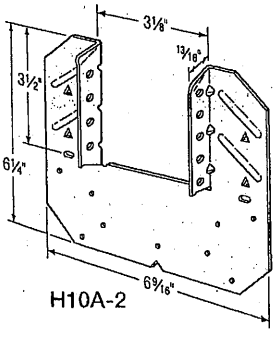
H8



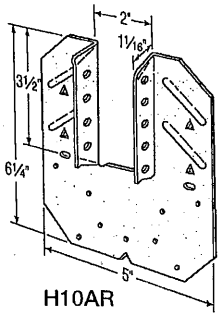
H10S



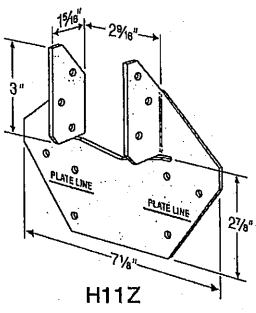
H10A
(H10ASS similar)



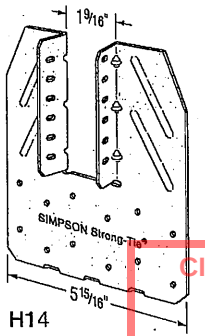
H10A-2



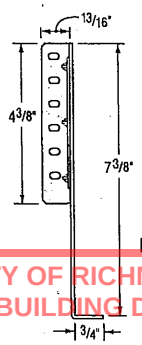
H10AR



H11Z



H14



H14
Profile

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON Strong-Tie

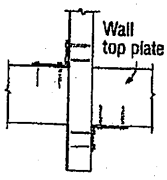
The H connector series provides wind and seismic ties for trusses and rafters.
Material: 18 gauge **Finish:** G90 galvanized

Design:
- Factored resistances are in accordance with CSA O86-14
- Factored resistances have been increased 15%. No further increase is permitted.

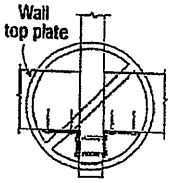
Installation:
- Use all specified fasteners
- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

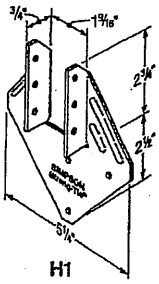
Hurricane Tie Installations to Achieve Twice the Load (Top View)



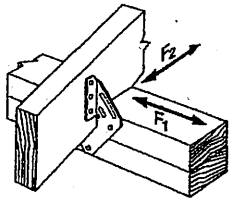
Install diagonally across from each other for minimum 2x truss.



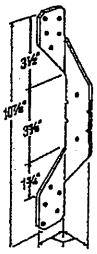
Nailing into both sides of a single ply 2x truss may cause the wood to split.



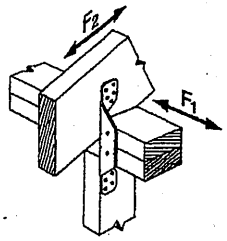
H1



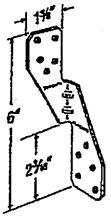
H1 Installation



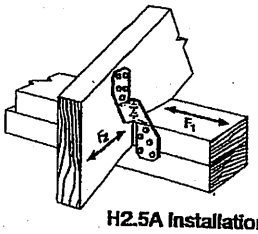
H2A



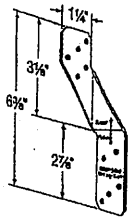
H2A Installation



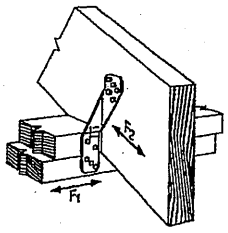
H2.5A



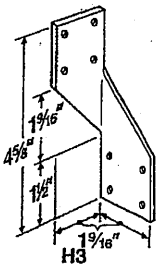
H2.5A Installation



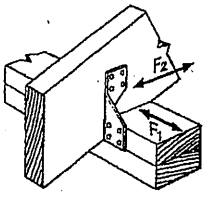
H2.5T



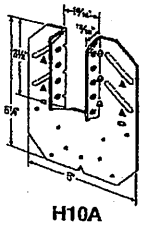
H2.5T Installation
(Nails into both top plates)



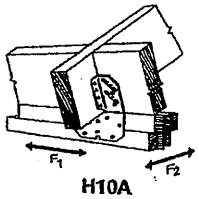
H3



H3 Installation



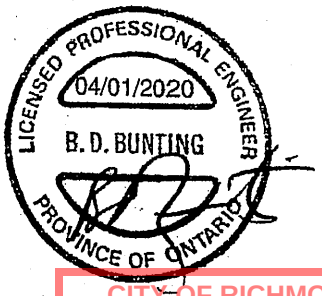
H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
					Uplift	Normal		Uplift	Normal	
		To Rafter	To Plates	To Studs		F_1	F_2		F_1	F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECH20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON Strong-Tie

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

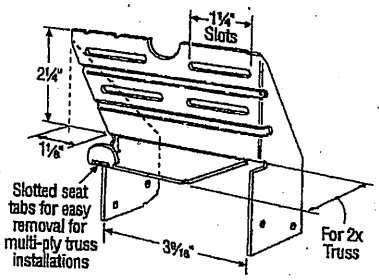
Design: Factored resistances are in accordance with CSA 086-14

Installation:

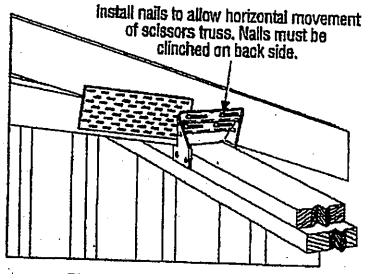
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

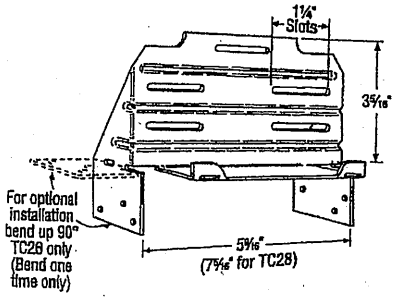
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



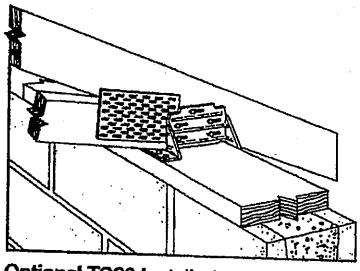
TC24
U.S. Patent 4,932,173



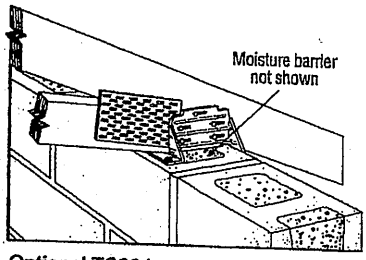
Typical TC24 Installation



TC26
(TC28 Similiar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECTC20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HTU

SIMPSON Strong-Tie®

Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners		Factored Resistance			
					D.Fir-L		S-P-F	
			Header	Joist	Uplift (K_D = 1.15)	Normal (K_D = 1.00)	Uplift (K_D = 1.15)	Normal (K_D = 1.00)
					lb.	lb.	lb.	lb.
					kN	kN	kN	kN
HTU26 (Min.)	3⅞	(2) 2x4	(10) 16d	(14) 10d x 1½"	1740	3340	1235	2370
					7.74	14.86	5.49	10.54
HTU26 (Max.)	5½	(2) 2x4	(10) 16d	(20) 10d x 1½"	2470	4015	1755	2850
					10.99	17.86	7.81	12.68
HTU28 (Max.)	3⅞	(2) 2x6	(20) 16d	(26) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19
HTU210 (Max.)	7¼	(2) 2x6	(20) 16d	(32) 10d x 1½"	4150	6395	2945	4540
					18.46	28.45	13.10	20.19

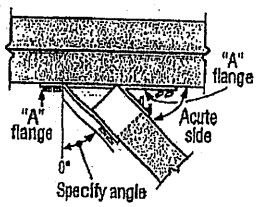
See table footnotes on p. 260.

Hanger Options

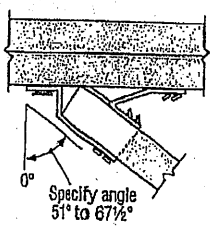
See Hanger Options information on pp. 125–127.

Skewed Seat

- Skewable up to 67½°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners		Factored Resistance			
				D.Fir-L		S-P-F	
		Header	Joist	Uplift (KD=1.15)	Normal (KD=1.00)	Uplift (KD=1.15)	Normal (KD=1.00)
				lbs	lbs	lbs	lbs
				kN	kN	kN	kN
HTU26	< 51	(20) 16d	(14) 10d x 1½"	1835	4110	1300	2905
				8.16	18.28	5.78	12.92
	51–67½	(20) 16d	(12) 10d x 1½"	1350	3820	955	2560
				6.01	16.10	4.25	11.39
HTU28	< 51	(26) 16d	(20) 10d x 1½"	2810	4270	1985	3030
				12.50	18.99	8.83	13.48
	51–67½	(26) 16d	(17) 10d x 1½"	2075	3930	1465	2780
				9.23	17.48	6.52	12.37
HTU210	< 51	(32) 16d	(26) 10d x 1½"	3785	4430	2675	3135
				16.84	19.71	11.90	13.95
	51–67½	(32) 16d	(22) 10d x 1½"	2795	4240	1980	3000
				12.43	18.86	8.81	13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140	3715	1515	2625
				9.52	16.53	6.74	11.68
	51–67½	(20) 16d	(12) 10d	1610	3920	1140	2785
				7.16	17.44	5.07	12.39
HTU28-2	< 51	(26) 16d	(20) 10d	3960	5425	2815	3855
				17.62	24.13	12.52	17.15
	51–67½	(26) 16d	(17) 10d	2385	5425	1695	3855
				10.61	24.13	7.54	17.15
HTU210-2	< 51	(32) 16d	(26) 10d	5025	6890	3570	4890
				22.35	30.65	15.88	21.75
	51–67½	(36) 16d	(22) 10d	3145	6680	2225	4745
				13.99	29.72	9.90	21.10

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3½" long, 10d x 1½" = 0.148" dia. x 1½" long, 10d = 0.148" dia. x 3" long. See pp. 27–28 for other nail sizes and information.

ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

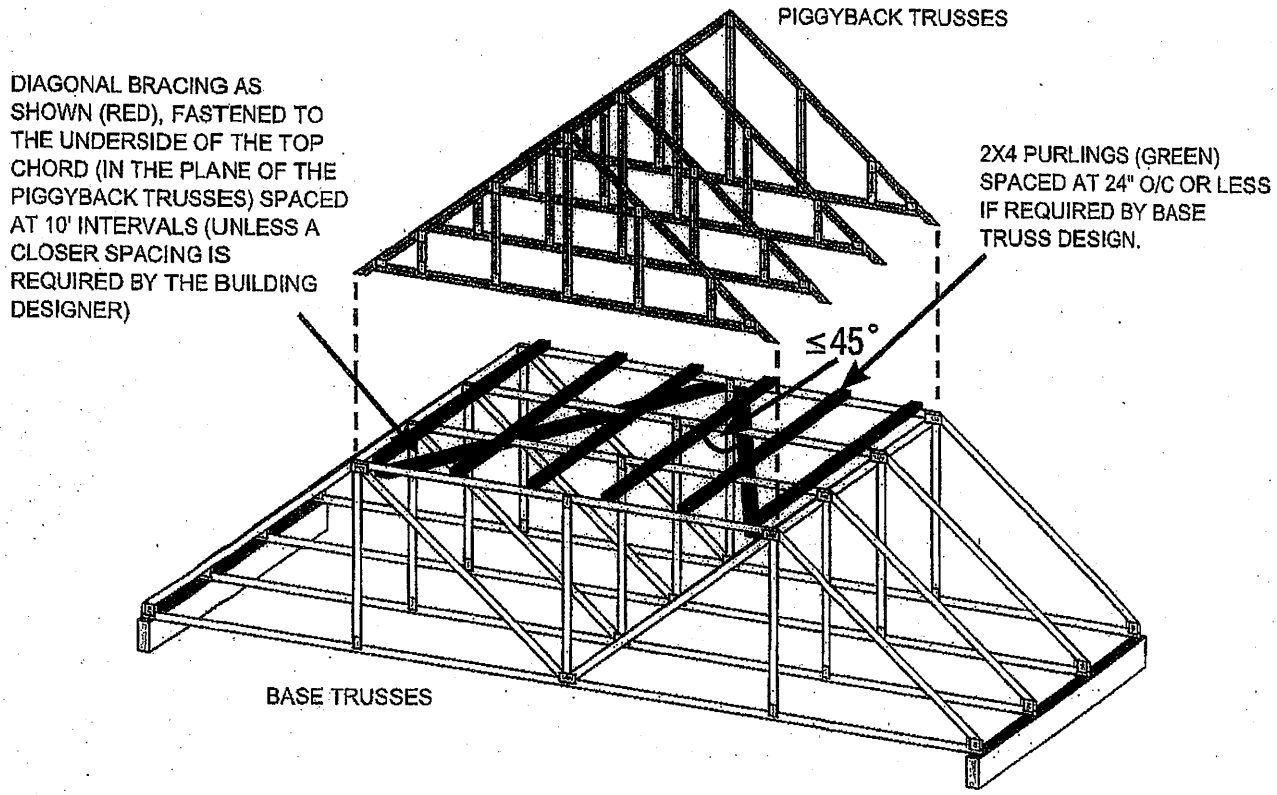
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

SIMPSON Strong-Tie®

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

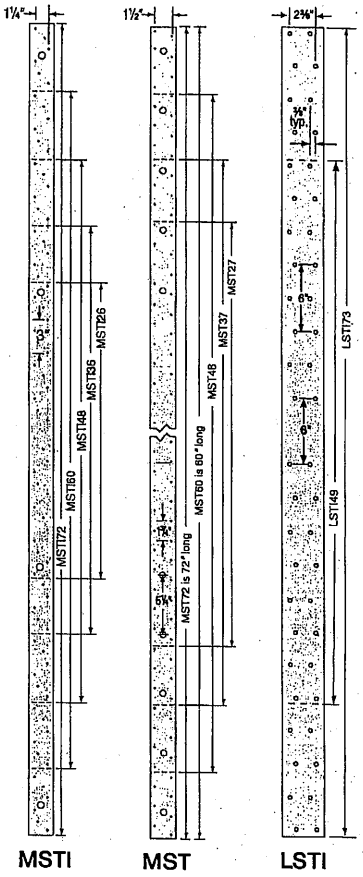
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

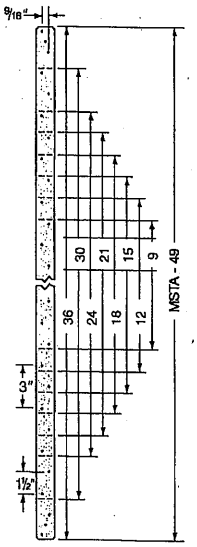
MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

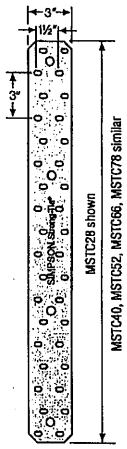
Installation: Use all specified fasteners; see General Notes

Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

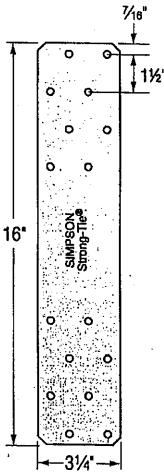




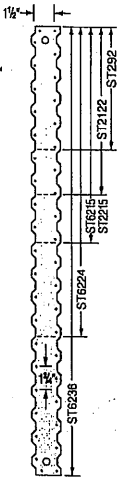
LSTA and MSTA
(Pilot holes not shown)



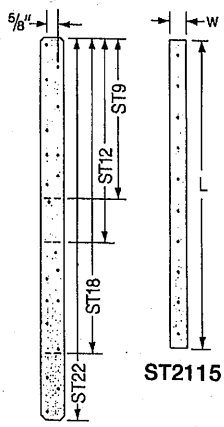
MSTC



HRS416Z

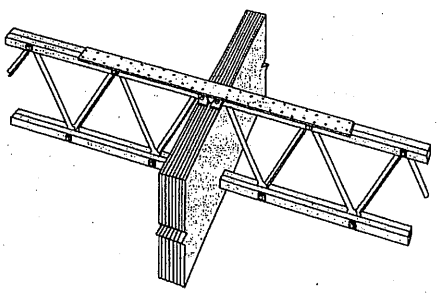


ST

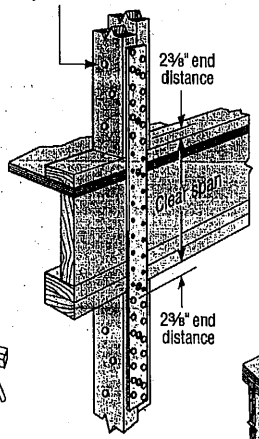


ST9, ST12, ST18, ST22

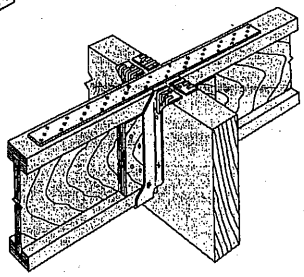
ST2115



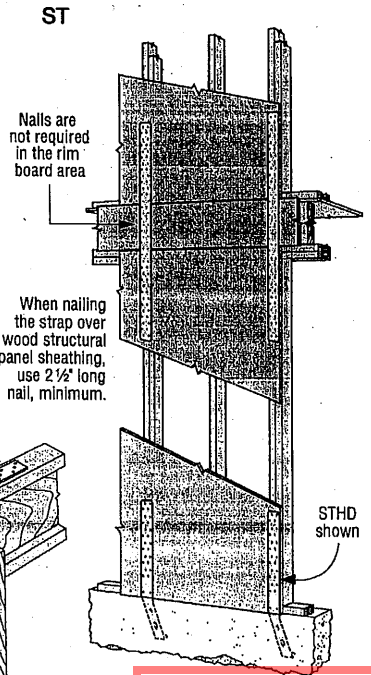
Typical LSTI Installation



Floor-to-Floor Tie Installation Showing a Clear Span



Typical MSTI Installation
(MIT hanger shown)
LSTI similar



Typical Detail with Strap Installed over Wood Structural Panel Sheathing