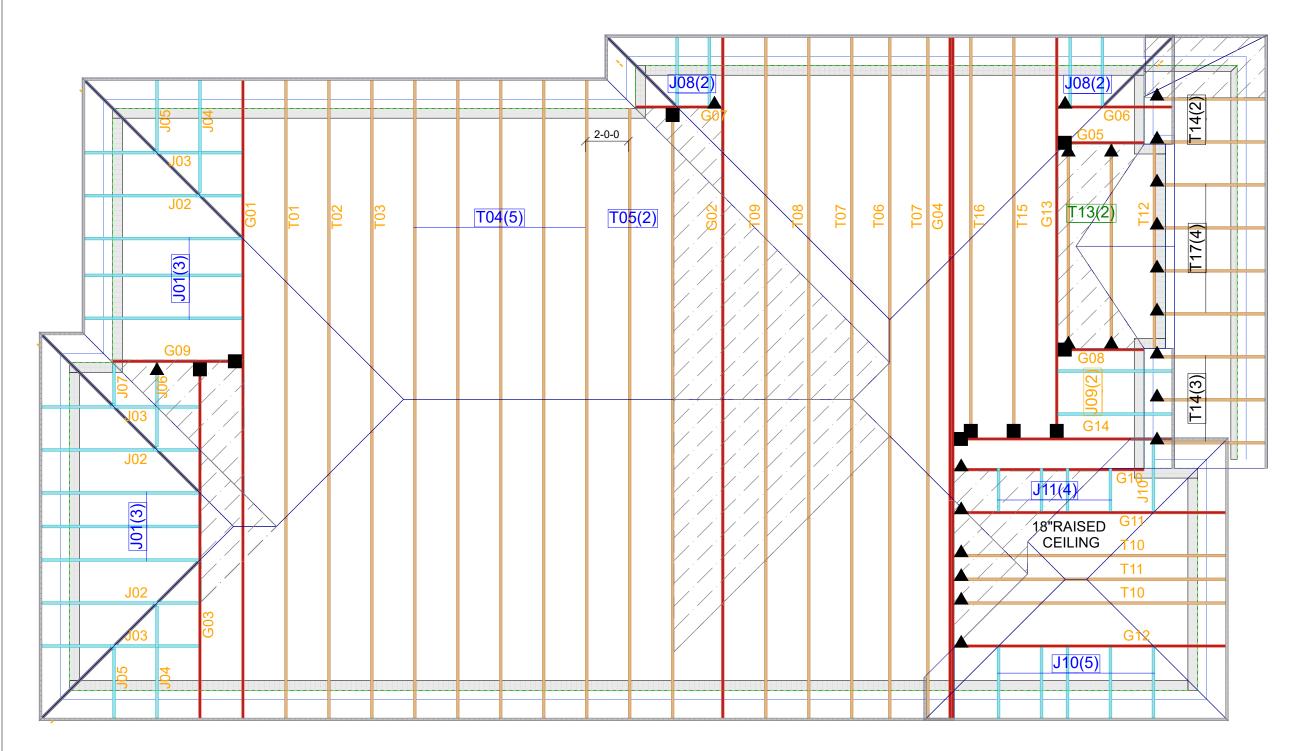


MHP 23035



Hanger Name	Symbol	QTY
LUS24		22
LJS26DS		9



ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9
OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES
TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST
TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY
BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED
6'. DESIGN OF CONVENTIONAL FRAMING IS THE
RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB IN	FORMATION
Customer	GREENPARK HOMES
Job #	23-00109R0
Address	ZADORRA ESTATES OSHAWA,ON
Model	VILLA 3-ELEV-3
Sales Rep	RALPH MIRIGELLO
Designer	RB
Date	6/05/23
Path	C:\MITEK\CA\JOBS\GREENPARK\ZADORRA ESTATES\VILLA 3-ELEV 3\VILLA 3-ELEV 3\

DESIGN INFORMATION						
Code	NBCC 2015					
Bldg	Residential - HSB (NBCC Part 9)					
TC LL	38.4 lb/ft²					
TC DL	6.0 lb/ft ²					
BC LL	0.0 lb/ft²					
BC DL	7.3 lb/ft²					
Deflection	LL=L/360 TL=L/360					
Spacing	24" O/C unless otherwise					
Spacing	noted					
Complies With	OBC 2012 (2019 Amendment) CSA 086-14 and TPIC 2014					

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.

14 Anderson Blvd. Uxbridge, ON 905.642.4400





NE0723-109 GREENPARK - ZADORRA ESTATES - VILLA 3-3



PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILITIES

THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON CALCULATION PAGE. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER.

IT IS THE RESPONSIBILITY OF KOTT Inc. TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DESIGN INFORMATION

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN.

- 1. THE BUILDING USE AND OCCUPANCY TYPE IS AS INDICATED ON THE DRAWING.
- 2. GEOMETRY OF THE TRUSS AND DIMENSIONS INDICATED ON THE DRAWING ARE IDENTICAL TO THOSE OF THE INSTALLED TRUSS.
- 3. THE TRUSS LOADING INTENSITY AND DISTRIBUTION AS WELL AS LOAD TRANSFER MECHANISM IS THAT INDICATED ON THE DRAWING. NO BUILDINGS, TREES, PARAPETS OR OTHER PROJECTIONS HIGHER THAN THE ROOF FOR WHICH THE TRUSSES ARE USED ARE LOCATED WITHIN A DISTANCE LESS THAN TEN (10) TIMES THE DIFFERENCE IN HEIGHT, OR FIVE METERS (16 FT) WHICHEVER IS GREATER, UNLESS THE DRAWING INDICATES THAT THE SNOW DRIFTING HAS BEEN TAKEN INTO ACCOUNT.
- 4. THE TRUSSES ARE TO BE SUPPORTED AT THE BEARING POINTS INDICATED AND ANCHORED TO THE SUPPORTS WHERE CONSIDERED NECESSARY BY THE DESIGNER OF THE OVERALL STRUCTURE. BEARING SIZES SHOWN ARE THE MINIMUM REQUIRED TO PREVENT CRUSHING OF THE TRUSS MEMBERS AND DO NOT NECESSARILY TAKE INTO ACCOUNT STABILITY OF THE OVERALL BUILDING STRUCTURE. ELEVATION OF BEARINGS MUST BE CAREFULLY CHECKED AND SHIMMED TO ALIGNMENT FOR SOLID BEARINGS. ADEQUATE WOOD TRUSS BEARING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

CODE

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING, THE ONTARIO BUILDING CODE, TPIC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

HANDLING, INSTALLATION AND BRACING

- 1. THE TRUSSES MUST BE HANDLED AND INSTALLED BY A QUALIFIED PROFESSIONAL AS PER THE SUPPLIED DOCUMENT TITLED INFORMATION FOR TRUSS INSTALLERS AND THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS.
- 2. THE COMPRESSION CHORDS ARE LATERALLY BRACED BY CONTINUOUS RIGID DIAPHRAGM SHEATHING OR AS SPECIFIED ON THE DRAWING.
- 3. TEMPORARY AND PERMANENT BRACING MUST BE INSTALLED AS INDICATED ON THE TRUSS DRAWING AND ACCORDING TO THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS. BRACING FOR THE LATERAL STABILITY OF THE TRUSS IS TO BE PROVIDED BY THE BUILDING DESIGNER.
- 4. IT IS RECOMMENDED THAT A PROFESSIONAL ENGINEER'S ADVICE BE OBTAINED FOR THE BRACING OF TRUSSES SPANNING MORE THAN 12.37M (40'-7").

(INTERNALIDATION TO THE CONTROL OF T MM

TRUSS BEARING CA	APACITIES [L	.BS.], BY TRU	SS LUMB (111	coto	,	PLATE)		7-Jun-21
			PER:	TITE 1, NO	LUSHILAI		1		
	BEARING	1-	IV	CHIEF BUI	DING OFF	CIAL	DI V	4-	PLY
NO BEARING	PLATE (B1)	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
ENHANCER	1 1/2"	13	83	27	67	41	51	5!	534
	2x4	3712	3228	7425	6457	11138	9685	14851	12914
	2x6	5834	5073	11668	10146	17503	15220	23337	20293
	2x8	7690	6687	15381	13375	23072	20063	30763	26750
	TYPE 1, FLUSH PLATE								
	BEARING	1-P	LY	2-1	PLY	3-1	PLY	4-	PLY
	PLATE (B1)	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
FLUSH PLATE	1 1/2"	13		2767		4151		5534	
	2x4		3712		7425		138		851
	2x6		834 116			17503		23337	
	2x8	76	90	15	15381 2307		072	30	763
			TYPE	1, FLUSH P	LATE + BEA	RING ENHA	NCER		
BEARING	BEARING	1-P	LY	2-1	PLY	3-1	PLY	4-PLY	
ENHANCER	PLATE (B1)	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
CPn-4 (Simpson)	2x4	4515		9030		13545		18065	
CPn-6 (Simpson)	2x6	7095		14	190	21285		28	390
CP4-9 (KOTT)	2x4	6007	4898	12014	9796	18021	14694	19801	19592
CP6-9 (KOTT)	2x6	8677	7075	17354	14150	26031	21225	31117	28300
SBP4 (MiTek)	2x4	72	88	11	001	14	714	18	427
SBP6 (MiTek)	2x6	110	30	16	865	220	599	28	534
SBP6 (MiTek)	2x8	128	886	20	578	28	269	35960	

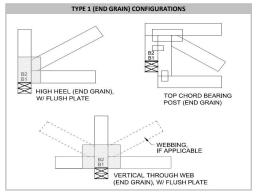
TYPE 1 CONFIGUR	ATIONS
82 81 STANDARD HEEL	BI HIGH HEEL + DIACONAL WEB, NO FLUSH PLATE, B2 >= 2B1
HEEL WITH WEDGE	WEBBING, IF APPLICABLE B1 HIGH HEEL, W/FLUSH PLATE, B2 >= B1
B2 HEEL WITH BC REINFORCEMENT	WEBBING, IF APPLICABLE WERTICAL WEB OVER SUPPORT, W/FLUSH PLATE, B2 >= B1

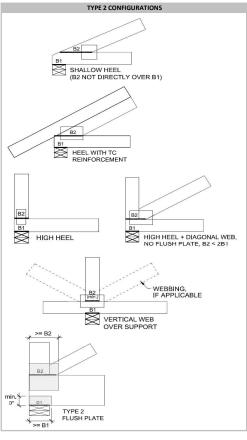
	TYPE 1 (END GRA						· ,					
	BEARING POST (B2)		1-1	PLY	2-PLY		3-PLY		4-PLY			
	PLATE	ABOVE	MSR2100	SPF No.2	MSR2100	SPF No.2	MSR2100	SPF No.2	MSR2100	SPF No.2		
	(B1)	BEARING	(EG)	(EG)	(EG)	(EG)	(EG)	(EG)	(EG)	(EG)		
END GRAIN	2x4	2x4	37	12	74	25	11:	138	148	51		
	2x6	2x6	58	34	110	568	175	503	233	37		
			MSR1950		MSR1950		MSR1950		MSR1950			
			(EG)	SPF No.2	(EG)	SPF No.2	(EG)	SPF No.2	(EG)	SPF No.2		
	2x8	2x8 2x8 7690		90	15381 23072			30763				
END GRAIN.			TYPE :	1 (END GRA	IN), FLUSH I	PLATE + BEA	RING ENHA	NCER				
BFARING	BEARING	POST (B2)	1-1	PLY	2-1	PLY	3-F	PLY	4-P	LY		
	PLATE	ABOVE	MSR2100	SPF No.2	MSR2100	SPF No.2	MSR2100	SPF No.2	MSR2100	SPF No.2		
ENHANCER	(B1)	BEARING	(EG)	(EG)	(EG)	(EG)	(EG)	(EG)	(EG)	(EG)		
CPn-4 (Simpson)	2x4	2x4	45	15	90	30	13!	545	180	65		
CPn-6 (Simpson)	2x6	2x6	70	195	14:	190	212	285	283	90		
CP4-9 (KOTT)	2x4	2x6	15585	15585 9006		18013	19801		198	01		
			MSR1950	SPF No.2	MSR1950	SPF No.2	MSR1950	SPF No.2	MSR1950	SPF No.2		
			(EG)	(EG)	(EG)	(EG)	(EG)	(EG)	(EG)	(EG)		
CP6-9 (KOTT)	2x6	2x8	21834	13009	31117	26019	31:	117	311	17		

				TVDE	2 NO FILIS	H PLATE, B2	-2D4					
	BEARING	POST (B2)	1-	PLY	2-PLY		3-PLY		4-P	LY		
	PLATE	ABOVE	MSR2100	SPF No.2	MSR2100	SPF No.2	MSR2100	SPF No.2	MSR2100	SPF No.2		
	(B1)	BEARING	IVISKZIUU	MSR1950	IVISKZ100	MSR1950	IVISKZIUU	MSR1950	IVISKZ1UU	MSR1950		
	2x4	2x4	2639	2152	5279	4304	7919	6457	10588	8609		
NO BEARING	2X4	2x6	3393	2767	6787	5534	10181	8302	13575	11069		
FNHANCER		2x4	3393	2767	6787	5534	10181	8302	13575	11069		
ENHANCER	2x6	2x6	4147	3382	8296	6764	12444	10146	16592	13529		
	ZXb	2x8	4808	3920	9616	7840	14424	11761	19232	15681		
		2x10	5562	4535	11124	9070	16686	13606	22248	18141		
	2x8	2x4	3959	3228	7919	6457	11878	9685	15838	12914		
		2x6	4808	3920	9616	7840	14424	11761	19232	15681		
		2x8	5467	4458	10935	8916	16403	13375	21871	17833		
					TYPE 2, FLI	JSH PLATE	(TE					
	BEARING	POST (B2)	1-PLY		2-PLY		3-1	PLY	4-P	LY		
	PLATE	ABOVE	MSR2100	SPF No.2	MSR2100	SPF No.2	MSR2100	SPF No.2	MSR2100	SPF No.2		
	(B1)	BEARING	IVI5R2100	MSR1950	IVISK2100	MSR1950	IVISK2100	MSR1950	IVISKZ100	MSR1950		
	2x4	2x4	TYPE 1 APPLIES									
	2.44	2x6	TIFE I APPLIES									
FLUSH PLATE		2x4	4672	3809	9344	7619	14016	11429	18689	15238		
	2x6	2x6										
	2.00	2x8	l			TYPE 1	APPLIES					
		2x10										
		2x4	4672	3809	9344	7619	14016	11429	18689	15238		
	2x8	2x6	7342	5986	14684	11973	22026	17959	29368	23946		
		2x8				TYPE 1	APPLIES					
•												

NOTES:

- $\textbf{1.} \ \mathsf{Factored} \ \mathsf{truss} \ \mathsf{reaction} \ \mathsf{shall} \ \mathsf{not} \ \mathsf{exceed} \ \mathsf{bearing} \ \mathsf{capacity} \ \mathsf{corresponding} \ \mathsf{to} : \mathsf{configuration} \ \mathsf{type}, \ \mathsf{size} \ \mathsf{of} \ \mathsf{bearing} \ \mathsf{surfaces}, \ \mathsf{truss}$ lumber, # of plies, and applicable enhancers.
- 2a. Values in table are in conformance with CSA 086-14 Cl. 6.5.7 and TPIC 2014-Update 2, and may be used for residential or commercial designs.
- 2b. Values in table are in conformance with MiTek Canada Detail B37821Q "SPF Bearing Capacities".
- 2c. Values in table are in conformance with Simpson Catalogue C-C-CAN2020.
- 3a. Conditions for use of table values include: standard duration (Ko=1), dry lumber (Kscp=1), untreated lumber (KT=1), length of bearing factor not applied (KB=1).
- 3b. Size factor (Kzcp) applied to support material calculation when acceptable. Flush plate factor (KP) applied to truss material calculation when acceptable (ie. excludes end grain).
- 3c. Flat roof factor (Kr) must applied for trusses making up a flat roof system; to do so, multiply bearing capacity values by 0.75 for this application.
- 4. Bearing plate is to be specified by the project engineer; values in table assume a bearing material of SPF #2 (or better).
- 5. When required, flush plate must not be located further than 1/4" away from bearing surface, and must cover the entire bearing plate length (B1).
- 6. When required, bearing enhancer must be installed as per manufacturer's guidelines.
- 7. Type 2 bearing configurations can be converted to use Type 1 table values as outlined in TPIC 2014-Update 1 Cl. 7.5.9.
- 8. This table is not valid after April 30, 2022.







MHPa23035 AIL MSD2015-H

Issued:

MARCH 1, 2022 APRIL 30, 2024

Expiry:

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	Length	Diameter		istance per nail .bs.)	WITHDRAWAL Resistance per nail (Lbs.)		
	(in)	(in)	SPF	D. FIR	SPF	D. FIR	
CONANAON	3.00	0.144	122	139	30	42	
COMMON 3.25	3.25	0.144	127	144	32	45	
WIKE	3.50	0.160	152	173	38	52	
COMMON	3.00	0.122	96	108	26	36	
SPIRAL	3.25	0.122	97	108	28	40	
SFIRAL	3.50	0.152	142	161	36	50	
3.25" Gun nail	3.25	0.120	94	105	28	39	

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:		Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter	(in.)	0.160	0.152	0.144	0.122	0.120
Length	(in.)	3.50	3.50	3.00	3.00	3.25
LUMBER			MAXIMU	M NUMBER OF TO	DE-NAILS	
2x4 SPF		2	2	3	3	3
2x6 SPF		4	4	4	5	5
2x4 D. FI	R	2	2	2	2	2
2x6 D. FI	R	3	3	3	4	4

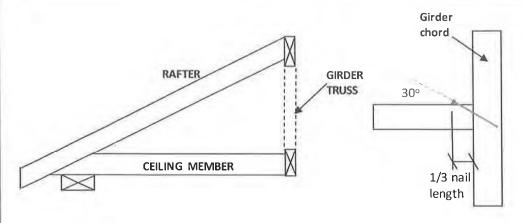


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

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Page 1 of 2

PEO Certificate No 10889485

Top view





MHPa23035 TAIL MSD2015-H

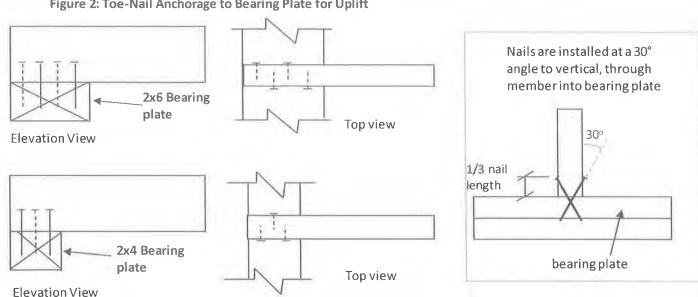
Issued:

MARCH 1, 2022

Expirv: **APRIL 30, 2024**

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

- 1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
- 2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to wind or earthquake loads do not exceed the withdrawal resistance of the toe-nails. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is NOT permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
- 3. Tabulated toe-nail resistances on page 1 are for one toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
- Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).
- 5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
- 6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
- 7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
- 8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PROFESSIONAL CLA **PEQ** Certificate No 10889485 POVINCE OF ONTAR

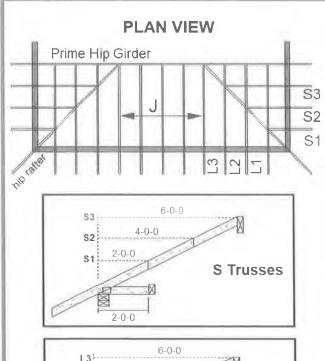
Page 2 of 2

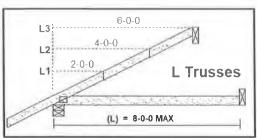


MHPs 23025 ETAIL MSD2015-J

Issued: MARCH 17, 2021 Expiry: APRIL 30, 2023

STANDARD HIP END FRAMING

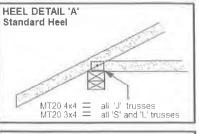


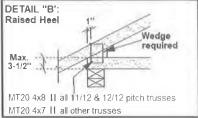


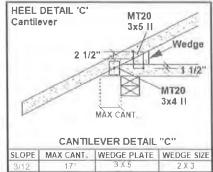
Specified Load Rating:

Top chord Live:
Top chord Dead:
Bottom chord Live:
Bottom chord Dead:

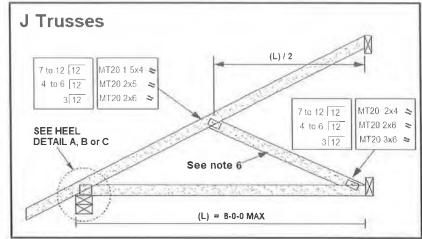
51.0 PSF or less
6.0 PSF or less
0.0 PSF
7.3 PSF or less







SLOPE	MAX CANT.	WEDGE PLATE	WEDGE SIZE
3/12	17"	3 X 5	2 X 3
4/12	14"	3 X 5	2 X 3
5/12	12"	3 X 5	2 X 4
6/12	10"	3 X 5	2 X 4
7/12	9=	3 X 5	2 X 6
8/12	8.5"	3 X 5	2 X 6
9/12	8"	3 X 5	2 X 6
10/12	7.5"	3 X 5	2 X 6



NOTES:

- 1. This detail is valid only for projects conforming to **PART 9 NBCC 2015** that do not require a wind analysis to be incorporated into the design of the trusses.
- 2. Overhang length shall not exceed 24 inches.
- 3. All lumber shall be 2x4 SPF (or D-Fir) DRY No. 2 grade or better.
- 4. All plates specified are MITEK MT20, pressed into both faces of each truss. Heel plates of all trusses shall conform to heel details 'A', 'B' or 'C'.
- 5. Diagonal hip rafter design shall conform to section 9.23.14.6 of NBCC 2015.
- **6.** For 6.0 ft. or less span, diagonal web on truss 'J" is optional. Girder design must reflect choice of partial jack ('J' with diagonal web) or open jack ('J' without diagonal web)
- 7. All truss-to-rafter and truss-to-truss connections shall be specified as per MITEK standard detail 'MSD2015-H: Toe-Nail Capacity Details'



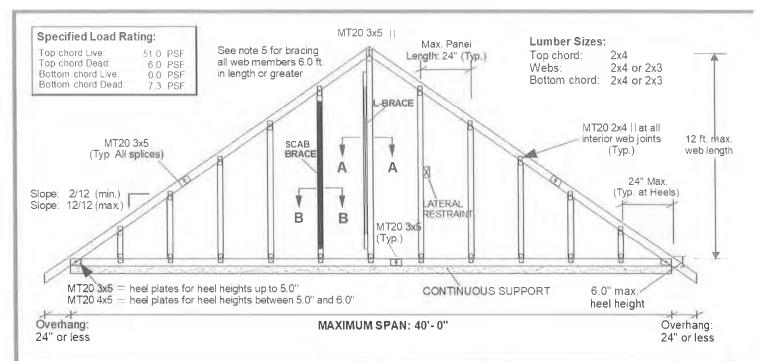


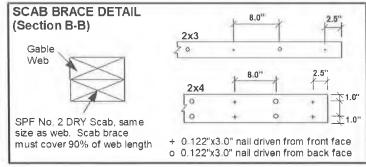


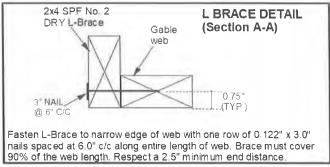
MHPst23035etail MSD2015-K

Issued: MARCH 1, 2022 Expiry: APRIL 30, 2024

STANDARD GABLE END DETAIL







Notes:

- 1. This detail is only valid for projects conforming to Part 9, NBCC 2015 that do not require a wind analysis to be incorporated into the design of the truss.
- 2. This detail is for vertical (gravity) load rating of the truss only. Truss must be continuously supported over the entire length of bottom chord.
- 3. Maximum web length not to exceed 12.0 ft. Spacing of gable stud webs in the truss not to exceed 24 inches cc.
- 4. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint.
- **5.** Lateral restraint required at half-length of all webs over 6.0 ft. long. Alternatively install an L-Brace or scab brace as shown above. Scab braces shall be limited to 10 ft. long webs or less.
- 6. All plates are MITEK MT20 pressed into both faces of truss.
- 7. All lumber to be SPF (or D-Fir) DRY and of No.2 grade or better.
- **8.** Additional building bracing is typically installed to brace the face of the end wall assembly. See BCSI Canada 'Building Designer Responsibilities for Gable End Frame Bracing' for additional information on building bracing for gable-end assemblies.

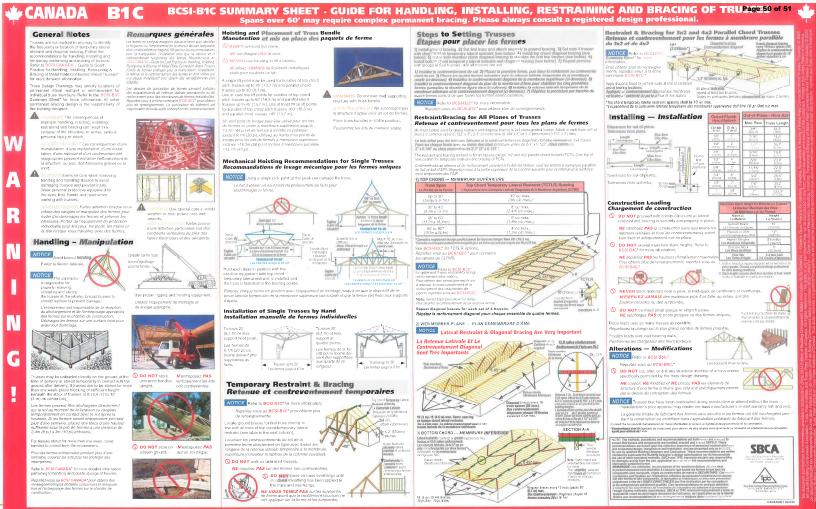




★ CANADA

B1C

MHP 23035



GUIDE POUR LA MANIPULATION, L'INSTALLATION, LA RETENUE ET LE CONTREVENTEMENT DES FERMES Les portées supérieures à 18,3m (60 pi) peuvent exiger un contreventement permanent complexe. Veuillez toujours concepteur professionnel enregi AVERTISSEMENT!



A, continuous lateral restraint & diagonal bracing, B, individual member veb reinforcement, Relenue et controvenitement a l'inde : A, d'une retonus latérale continuo et d'un contravenitement a l'inde : A, d'une retonus latérale continuo et d'un contravenitement diagonal. B, d'un reinforcement des membrures d'âme individualles,

A. Continuous Lateral Restraint (CLR) & Diagonal Bracing Retenue latérale continue (RLC) et contreventement diagonal

(i.e., fail) Without permanent bracing the truss, or a partian of its members, will buckle (i.e., fail) at loads far less than design,

Permanent Bracing for the Top Chord Plane
 Contreventement permanent pour le plan de membrure supérieure

plywood, oriented strand board (OSB), or wood notal structural purlins that are properly braced, ich to each truss,

The truss design drawing (TDD) provides informs on the assumed support for the top chord,

MHP 23035



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Some trans disciple of provide general trades design beliefs and obtails to assist the subtray designer in determination like to-come required set transfer latural loads, due to wind and/or selection forces from the gabo
and frame in the transfer and reading college displants and reading college displants of an extra transfer and the college of the composition global reading and displants of the selection of the composition global reading and displants of the college of the colleg RETENUE ET CONTREVENTEMENT PERMANENTS DES MEMBRURES ET DES MEMBRURES D'ÂME Les portées de 18,3m (60 pi) et plus peuvent exiger un contreventement permanent complexe. Veuillez toujours consulter un ingénieur profe

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