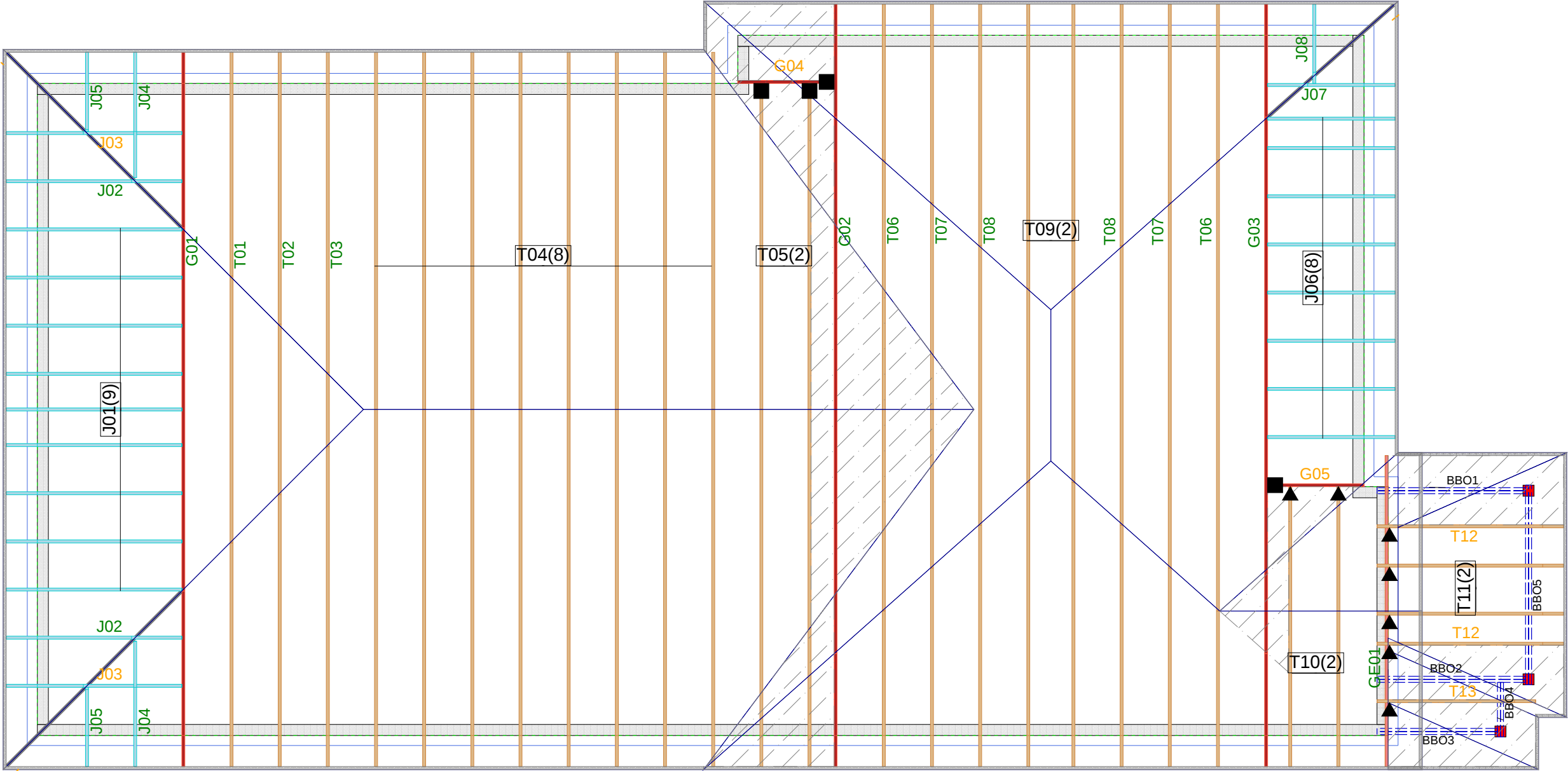




Hanger Name	Symbol	QTY
LUS24	▲	7
LJS26DS	■	4



CONVENTIONAL
FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE Laterally BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK HOMES
Job #	23-00110R0
Address	ZADORRA ESTATES OSHAWA,ON
Model	VILLA 5-ELEV-1
Sales Rep	RALPH MIRIGELLO
Designer	RB
Date	6/06/23
Path	C:\MITEK\CA\JOBS\GREENPARK\ZADORRA ESTATES\VILLA 5\VILLA 5-ELEV 1\

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft²
TC DL	6.0 lb/ft²
BC LL	0.0 lb/ft²
BC DL	7.3 lb/ft²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400





NE0723-110

GREENPARK - ZADORRA

ESTATES - VILLA 5-1

PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILITIES

THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON CALCULATION PAGE. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. IT IS THE RESPONSIBILITY OF KOTT Inc. TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DESIGN INFORMATION

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN.

1. THE BUILDING USE AND OCCUPANCY TYPE IS AS INDICATED ON THE DRAWING.
2. GEOMETRY OF THE TRUSS AND DIMENSIONS INDICATED ON THE DRAWING ARE IDENTICAL TO THOSE OF THE INSTALLED TRUSS.
3. THE TRUSS LOADING INTENSITY AND DISTRIBUTION AS WELL AS LOAD TRANSFER MECHANISM IS THAT INDICATED ON THE DRAWING. NO BUILDINGS, TREES, PARAPETS OR OTHER PROJECTIONS HIGHER THAN THE ROOF FOR WHICH THE TRUSSES ARE USED ARE LOCATED WITHIN A DISTANCE LESS THAN TEN (10) TIMES THE DIFFERENCE IN HEIGHT, OR FIVE METERS (16 FT) WHICHEVER IS GREATER, UNLESS THE DRAWING INDICATES THAT THE SNOW DRIFTING HAS BEEN TAKEN INTO ACCOUNT.
4. THE TRUSSES ARE TO BE SUPPORTED AT THE BEARING POINTS INDICATED AND ANCHORED TO THE SUPPORTS WHERE CONSIDERED NECESSARY BY THE DESIGNER OF THE OVERALL STRUCTURE. BEARING SIZES SHOWN ARE THE MINIMUM REQUIRED TO PREVENT CRUSHING OF THE TRUSS MEMBERS AND DO NOT NECESSARILY TAKE INTO ACCOUNT STABILITY OF THE OVERALL BUILDING STRUCTURE. ELEVATION OF BEARINGS MUST BE CAREFULLY CHECKED AND SHIMMED TO ALIGNMENT FOR SOLID BEARINGS. ADEQUATE WOOD TRUSS BEARING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

CODE

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING, THE ONTARIO BUILDING CODE, TPIC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

HANDLING, INSTALLATION AND BRACING

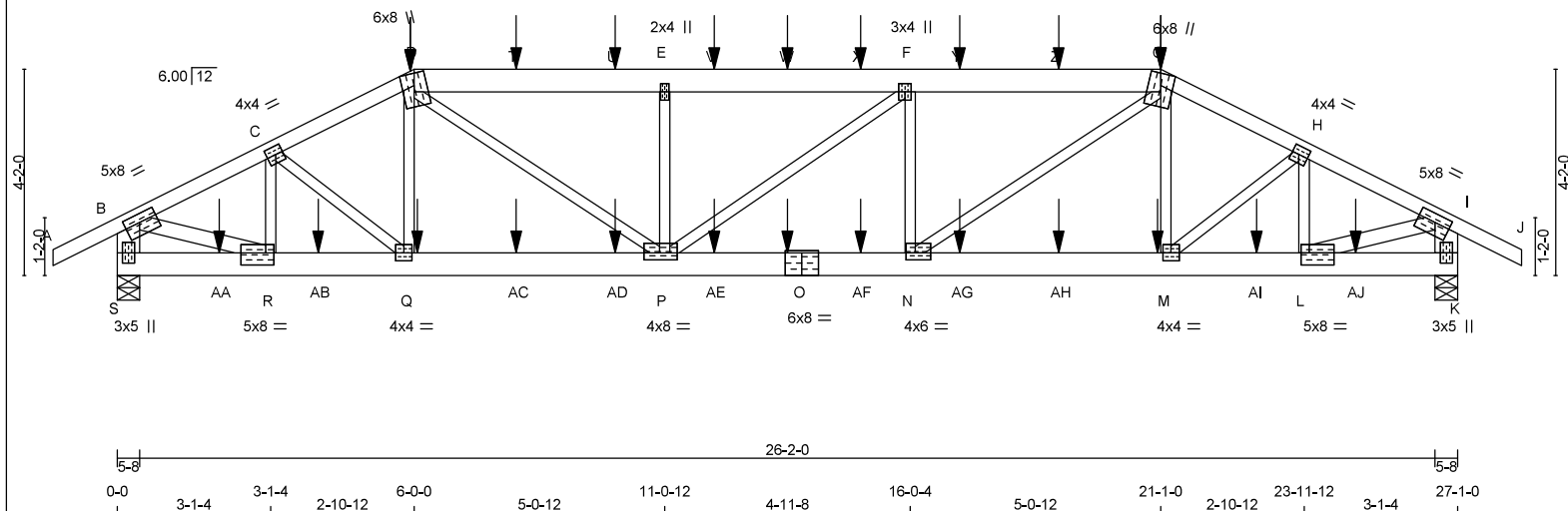
1. THE TRUSSES MUST BE HANDLED AND INSTALLED BY A QUALIFIED PROFESSIONAL AS PER THE SUPPLIED DOCUMENT TITLED INFORMATION FOR TRUSS INSTALLERS AND THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS.
2. THE COMPRESSION CHORDS ARE Laterally Braced by Continuous Rigid Diaphragm Sheathing or as Specified on the Drawing.
3. TEMPORARY AND PERMANENT BRACING MUST BE INSTALLED AS INDICATED ON THE TRUSS DRAWING AND ACCORDING TO THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS. BRACING FOR THE LATERAL STABILITY OF THE TRUSS IS TO BE PROVIDED BY THE BUILDING DESIGNER.
4. IT IS RECOMMENDED THAT A PROFESSIONAL ENGINEER'S ADVICE BE OBTAINED FOR THE BRACING OF TRUSSES SPANNING MORE THAN 12.37M (40'-7").

JOB NAME NE0723-110	TRUSS NAME G01	QUANTITY 1	PLY 1	JOB DESC. CHIEF PARK - 2ND FLOOR STATES - VILLA 5-1	DRWG NO.
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TRUE COPY OF PERMIT PLANS
Nov 03 2023
C. Monte
CHIEF BUILDING OFFICIAL

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 18 14:02:23 2023 Page 1
ID: aX72BI6hiO7VoZWleLIJw?z93Pm-ofTu_H7N8Y3MXXdQgkdAIE7z8hGslgQQAsDNI6yww5E

Scale = 1:46.5



LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES										DESIGNER										*** SPECIAL LOADS ANALYSIS ***									
CHORDS SIZE LUMBER DESCR. SPF										BEARINGS										GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.									
A - D 2x4 DRY No.2										FACTORED MAXIMUM FACTORED INPUT REQD										LOADS WERE DERIVED FROM USER INPUT									
D - G 2x6 DRY No.2										GROSS REACTION GROSS REACTION BRG BRG										NO FURTHER MODIFICATIONS WERE MADE									
G - J 2x4 DRY No.2										JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX																			
S - B 2x6 DRY No.2										S 3335 0 3335 0 0 5-8 5-5																			
K - I 2x6 DRY No.2										K 3335 0 3335 0 0 5-8 5-5																			
S - O 2x6 DRY No.2																													
O - K 2x6 DRY No.2																													
										UNFACTORED REACTIONS																			
ALL WEBS 2x3 DRY No.2										1ST LCASE MAX./MIN. COMPONENT REACTIONS																			
EXCEPT										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL																			
B - R 2x4 DRY No.2										S 2329 1694 / 0 0 / 0 0 / 0 0 / 0 635 / 0 0 / 0																			
L - I 2x4 DRY No.2										K 2329 1694 / 0 0 / 0 0 / 0 0 / 0 636 / 0 0 / 0																			
DRY: SEASONED LUMBER.										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K																			
										BRACING																			
										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.73 FT.																			
										MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.																			
										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.																			
PLATES (table is in inches)										LOADING																			
JT TYPE PLATES W LEN Y X										TOTAL LOAD CASES: (4)																			
B TMVW+ MT20 5.0 8.0 2.00 3.75										CHORDS																			
C TMVW+ MT20 4.0 4.0 2.00 1.75										MAX. FACTORED																			
D TTVW+m MT20 6.0 8.0 Edge										MEMB. FORCE (LBS)																			
E TMV+w MT20 2.0 4.0										VERT. LOAD LC1																			
F TMVW+t MT20 3.0 4.0										(PLF)																			
G TTVW+m MT20 6.0 8.0 Edge										MAX. CSI (LC)																			
H TMVW+ MT20 4.0 4.0 2.00 1.75										MAX. UNBRAC																			
I TMVW+ MT20 5.0 8.0 2.00 3.75										MEMB. FORCE (LBS)																			
K BMV1+p MT20 3.0 5.0										FR-TO																			
L BMVW+ MT20 5.0 8.0 3.00 2.00										FROM TO																			
M BMVW+ MT20 4.0 4.0										A-B 0 / 36																			
N BMVW+ MT20 4.0 6.0 1.75 2.25										B-C -4353 / 0																			
O BS+ MT20 6.0 8.0										C-D -4905 / 0																			
P BMVW+ MT20 4.0 8.0 1.75 3.75										D-T -6108 / 0																			
Q BMVW+ MT20 4.0 4.0										T-U -6108 / 0																			
R BMVW+ MT20 5.0 8.0 3.00 2.00										U-E -6108 / 0																			
S BMV1+p MT20 3.0 5.0										E-V -6107 / 0																			
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES										V-W -6107 / 0																			
EDGE OF CHORD.										W-X -6107 / 0																			
										X-F -6107 / 0																			
										F-Y -6123 / 0																			
										Y-Z -6123 / 0																			
										Z-G -6123 / 0																			
										G-H -4903 / 0																			
										H-I -4355 / 0																			
										I-J 0 / 36																			
										J-S -3245 / 0																			
										K-I -3247 / 0																			

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-110	G01	1	1	GRIFFIN PARK - 240 DORMER STATES - VILLA 5-1	

Nov 03 2023

PER:



CHIEF BUILDING OFFICIAL

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 18 14:02:23 2023 Page 2

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SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	2-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AB	4-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AC	8-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AD	10-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AE	12-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AF	15-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AG	17-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AH	19-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AI	23-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AJ	25-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1

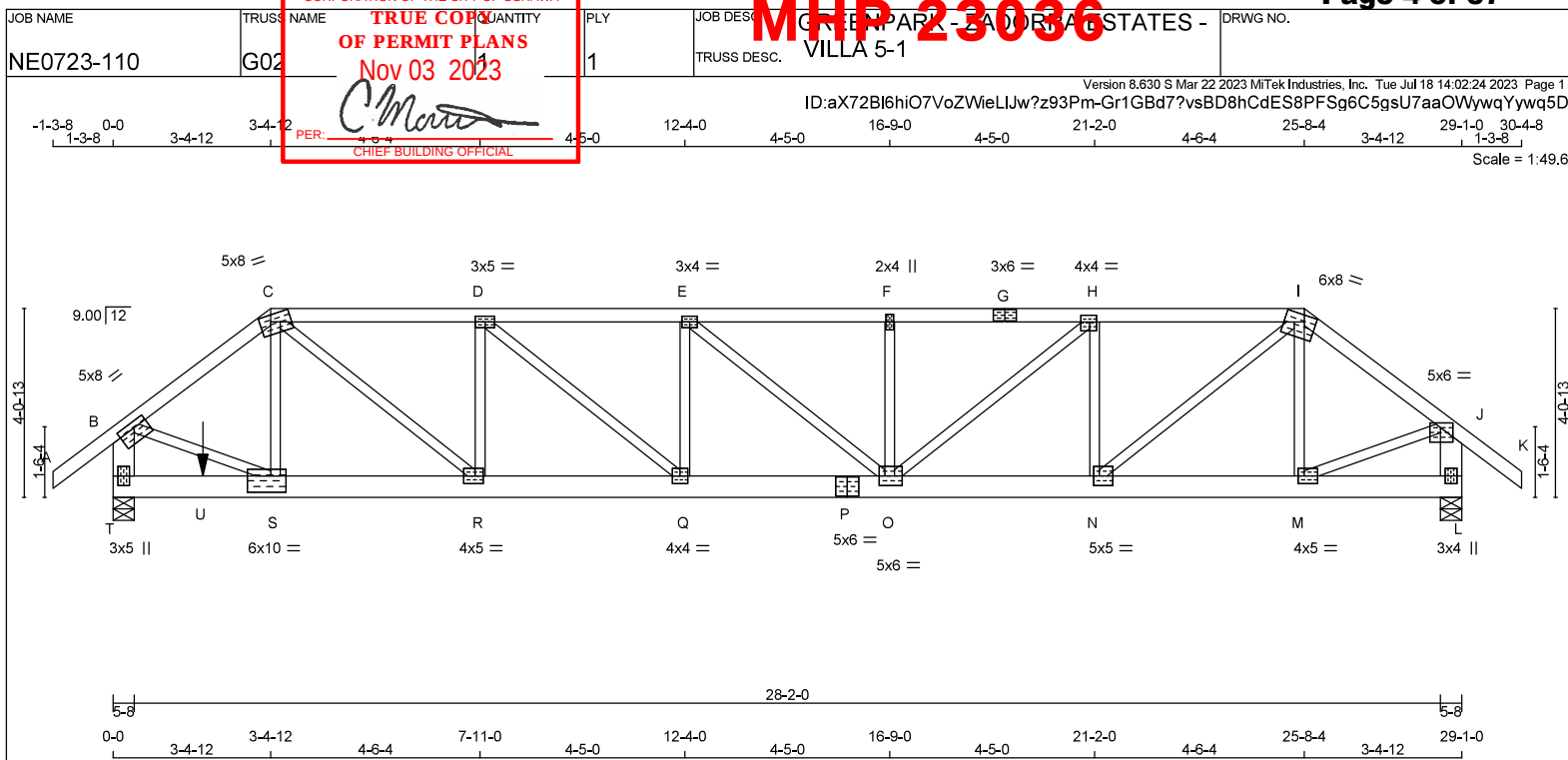
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW4	MT20	5.0	8.0	1.50	3.50
C TTWW-m	MT20	5.0	8.0	1.75	2.75
D TMWW4	MT20	3.0	5.0		
E TMWW4	MT20	3.0	4.0		
F TMWW4	MT20	2.0	4.0		
G TS4	MT20	3.0	6.0		
H TMWW4	MT20	4.0	4.0	1.75	1.75
I TTWW-m	MT20	6.0	8.0	1.75	3.00
J TMWW-p	MT20	5.0	6.0	1.25	3.25
L BMV1-p	MT20	3.0	4.0		
M BMWW4	MT20	4.0	5.0	2.00	1.50
N BMWW4	MT20	5.0	5.0	2.50	1.50
O BMWWWW4	MT20	5.0	6.0	2.50	1.50
P BS4	MT20	5.0	6.0		
Q BMWW4	MT20	4.0	4.0		
R BMWW4	MT20	4.0	5.0	2.00	2.00
S BMWW4	MT20	6.0	10.0	4.25	4.00
T BMV1-p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	4112	0	5-8
T 4112	0	0	5-8
L 2306	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	2868
T 2868	2109 / 0
L 1608	1180 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L
 BEARING SIZE FACTOR = 1.15 AT JNT(S) T (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.55 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO	A-B	0 / 49	-119.4	-119.4	0.18 (1)	10.00	S-C	0 / 698
	B-C	-3448 / 0	-119.4	-119.4	0.47 (1)	3.28	C-R	0 / 1495
	C-D	-3931 / 0	-119.4	-119.4	0.87 (1)	2.86	R-D	-1073 / 0
	D-E	-4515 / 0	-119.4	-119.4	0.76 (1)	2.55	D-Q	0 / 762
	E-F	-4331 / 0	-119.4	-119.4	0.61 (1)	2.83	Q-E	-360 / 0
	F-G	-4331 / 0	-119.4	-119.4	0.73 (1)	2.64	E-O	-241 / 0
	G-H	-4331 / 0	-119.4	-119.4	0.73 (1)	2.64	O-F	-508 / 0
	H-I	-3475 / 0	-119.4	-119.4	0.61 (1)	3.11	O-H	0 / 1115
	I-J	-2242 / 0	-119.4	-119.4	0.35 (1)	4.13	H-N	-1304 / 0
	J-K	0 / 49	-119.4	-119.4	0.18 (1)	10.00	N-I	0 / 2191
	T-B	-3318 / 0	0.0	0.0	0.24 (1)	5.71	M-I	-467 / 0
	L-J	-2286 / 0	0.0	0.0	0.17 (1)	6.66	B-S	0 / 2886
							M-J	0 / 1877
	T-U	0 / 0	-18.2	-18.2	0.60 (1)	10.00		
	U-S	0 / 0	-18.2	-18.2	0.60 (1)	10.00		
	S-R	0 / 2771	-18.2	-18.2	0.38 (1)	10.00		
	R-Q	0 / 3931	-18.2	-18.2	0.28 (1)	10.00		
	Q-P	0 / 4515	-18.2	-18.2	0.67 (1)	10.00		
	P-O	0 / 4515	-18.2	-18.2	0.67 (1)	10.00		
	O-N	0 / 3476	-18.2	-18.2	0.53 (1)	10.00		
	N-M	0 / 1776	-18.2	-18.2	0.26 (1)	10.00		
	M-L	0 / 0	-18.2	-18.2	0.04 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U 1-11-4	-1453	-1453						C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOT CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/965 (0.36")

CSI: TC=0.78/0.97 (D-E-1), BC=0.67/0.97 (O-Q-1),
 WB=0.71/0.97 (B-S-1), SSI=0.90/1.00 (S-T-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

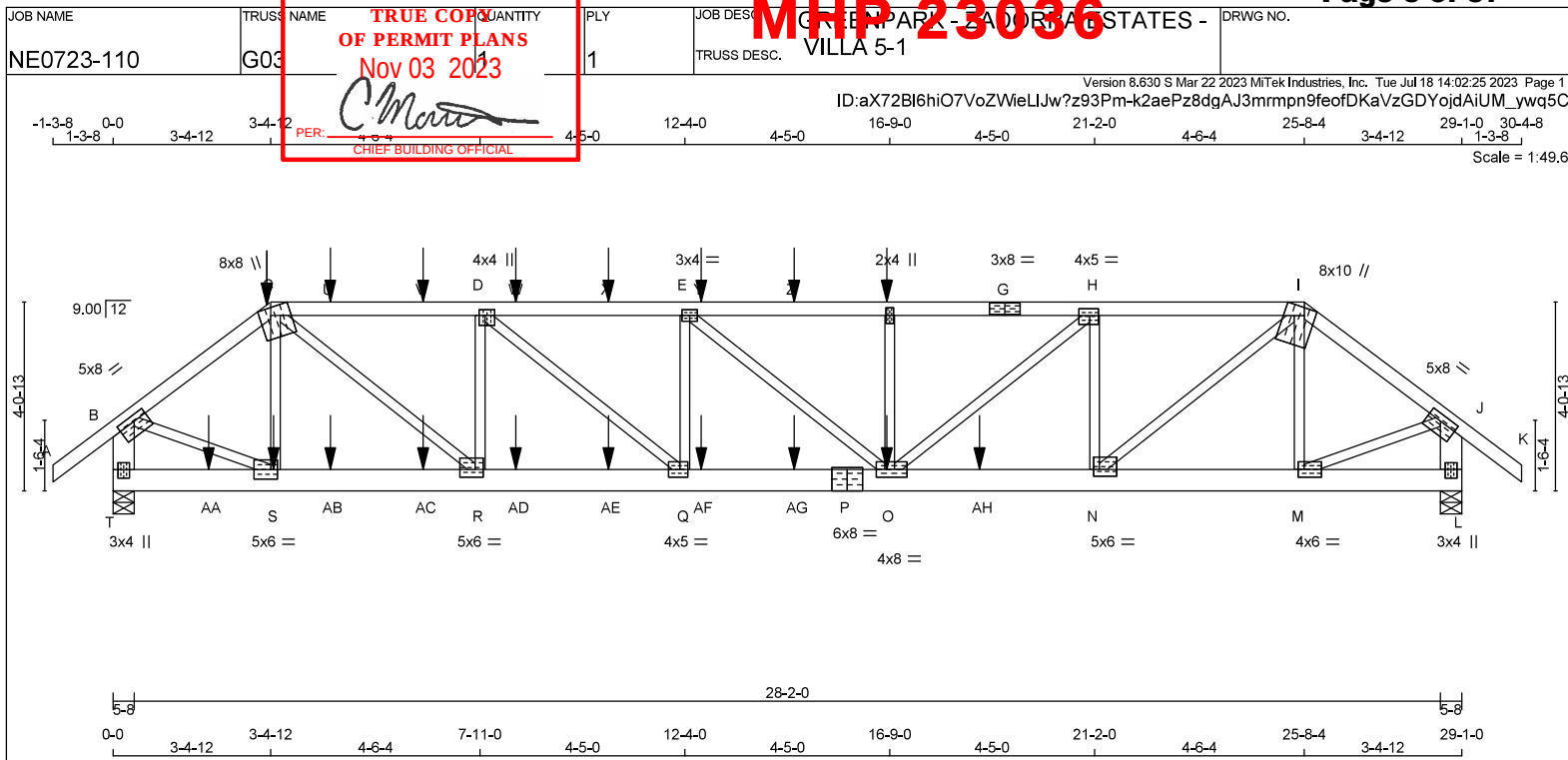
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
 JSI METAL = 0.86 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	2100F 1.8E	SPF
G - I	2x4	DRY	2100F 1.8E	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	5.0	8.0	1.50 3.50
C	TTVW+m	MT20	8.0	8.0	Edge 3.00
D	TMVW+1	MT20	4.0	4.0	1.50 1.50
E	TMVW4	MT20	3.0	4.0	
F	TMVW+1	MT20	2.0	4.0	
G	TS4	MT20	3.0	8.0	
H	TMVW4	MT20	4.0	5.0	1.75 2.25
I	TTVW+m	MT20	8.0	10.0	Edge 3.00
J	TMVW4	MT20	5.0	8.0	1.75 3.50
L	BMV1+p	MT20	3.0	4.0	2.25 1.50
M	BMVW4	MT20	4.0	6.0	2.00 1.50
N	BMVW4	MT20	5.0	6.0	1.75 1.50
O	BMVW4	MT20	4.0	8.0	2.00 2.25
P	BS4	MT20	6.0	8.0	
Q	BMVW4	MT20	4.0	5.0	2.00 2.00
R	BMVW4	MT20	5.0	6.0	2.00 2.00
S	BMVW4	MT20	5.0	6.0	2.50 1.75
T	BMV1+p	MT20	3.0	4.0	2.25 1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
 EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
T	3113	0	3113	0
L	3016	0	3016	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
T	2173	1588 / 0	0 / 0
L	2104	1545 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L
 BEARING SIZE FACTOR = 1.15 AT JNT(S) L (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO		
A-B	0 / 49	-119.4	-119.4 0.18 (1)	S-C	-672 / 0	0.17 (1)
B-C	-3183 / 0	-119.4	-119.4 0.44 (1)	C-R	0 / 2873	0.71 (1)
C-U	-4750 / 0	-119.4	-119.4 0.45 (1)	P-D	-1701 / 0	0.43 (1)
L-V	-4750 / 0	-119.4	-119.4 0.45 (1)	D-Q	0 / 1516	0.38 (1)
V-D	-4750 / 0	-119.4	-119.4 0.45 (1)	Q-E	-948 / 0	0.24 (1)
D-W	-5913 / 0	-119.4	-119.4 0.53 (1)	E-O	0 / 461	0.11 (1)
W-X	-5913 / 0	-119.4	-119.4 0.53 (1)	O-F	-599 / 0	0.15 (1)
X-E	-5913 / 0	-119.4	-119.4 0.53 (1)	O-H	0 / 1569	0.39 (1)
E-Y	-6266 / 0	-119.4	-119.4 0.47 (1)	H-I	-1581 / 0	0.40 (1)
Y-Z	-6266 / 0	-119.4	-119.4 0.47 (1)	I-J	0 / 3411	0.84 (1)
Z-F	-6266 / 0	-119.4	-119.4 0.47 (1)	F-G	-756 / 0	0.19 (1)
F-G	-6266 / 0	-119.4	-119.4 0.47 (1)	G-H	0 / 2664	0.66 (1)
G-H	-6266 / 0	-119.4	-119.4 0.47 (1)	H-I	0 / 2558	0.63 (1)
I-J	-5063 / 0	-119.4	-119.4 0.39 (1)			
J-K	-3056 / 0	-119.4	-119.4 0.42 (1)			
K-L	0 / 49	-119.4	-119.4 0.18 (1)			
T-B	-3091 / 0	0.0	0.0 0.22 (1)			
L-J	-2982 / 0	0.0	0.0 0.22 (1)			

T-AA	0 / 0	-18.2	-18.2 0.05 (4)	10.00
AA-S	0 / 0	-18.2	-18.2 0.05 (4)	10.00
S-AB	0 / 2521	-18.2	-18.2 0.37 (1)	10.00
AB-AC	0 / 2521	-18.2	-18.2 0.37 (1)	10.00
AC-R	0 / 2521	-18.2	-18.2 0.37 (1)	10.00
R-AD	0 / 4750	-18.2	-18.2 0.72 (1)	10.00
AD-AE	0 / 4750	-18.2	-18.2 0.72 (1)	10.00
AE-Q	0 / 4750	-18.2	-18.2 0.72 (1)	10.00
Q-AF	0 / 5913	-18.2	-18.2 0.85 (1)	10.00
AF-AG	0 / 5913	-18.2	-18.2 0.85 (1)	10.00
AG-P	0 / 5913	-18.2	-18.2 0.85 (1)	10.00
P-O	0 / 5913	-18.2	-18.2 0.85 (1)	10.00
O-AH	0 / 5063	-18.2	-18.2 0.62 (1)	10.00
AH-N	0 / 5063	-18.2	-18.2 0.62 (1)	10.00
N-M	0 / 2417	-18.2	-18.2 0.22 (1)	10.00
M-L	0 / 0	-18.2	-18.2 0.01 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-4-12	-198	-198		BACK	VERT	TOTAL		C1
F	16-8-4	-41	-41		BACK	VERT	TOTAL		C1
O	16-8-4	-11	-11		BACK	VERT	TOTAL		C1
S	3-5-0	-11	-11		BACK	VERT	TOTAL		C1
U	4-8-4	-41	-41		BACK	VERT	TOTAL		C1
V	6-8-4	-41	-41		BACK	VERT	TOTAL		C1
W	8-8-4	-41	-41		BACK	VERT	TOTAL		C1
X	10-8-4	-41	-41		BACK	VERT	TOTAL		C1
Y	12-8-4	-41	-41		BACK	VERT	TOTAL		C1
Z	14-8-4	-41	-41		BACK	VERT	TOTAL		C1
AA	2-0-12	-8	-8		BACK	VERT	TOTAL		C1
AB	4-8-4	-11	-11		BACK	VERT	TOTAL		C1
AC	6-8-4	-11	-11		BACK	VERT	TOTAL		C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL	LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/779 (0.45")

CSI: TC=0.53/0.97 (D-E-1), BC=0.85/0.97 (O-Q-1),
 WB=0.84/0.97 (H-N-1), SSI=0.38/1.00 (N-O-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
 JSI METAL = 0.89 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-110	G03	1	1	GRIFFIN PARK - 2400 RIVA STATES - VILLA 5-1	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 18 14:02:25 2023 Page 2

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PER:

CORPORATION OF THE CITY OF OSHAWA
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 OF PERMIT PLANS
 Nov 03 2023


CHIEF BUILDING OFFICIAL

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AD	8-8-4	-11	-11	—	BACK	VERT	TOTAL	—	C1
AE	10-8-4	-11	-11	—	BACK	VERT	TOTAL	—	C1
AF	12-8-4	-11	-11	—	BACK	VERT	TOTAL	—	C1
AG	14-8-4	-11	-11	—	BACK	VERT	TOTAL	—	C1
AH	18-8-4	-676	-676	—	BACK	VERT	TOTAL	—	C1

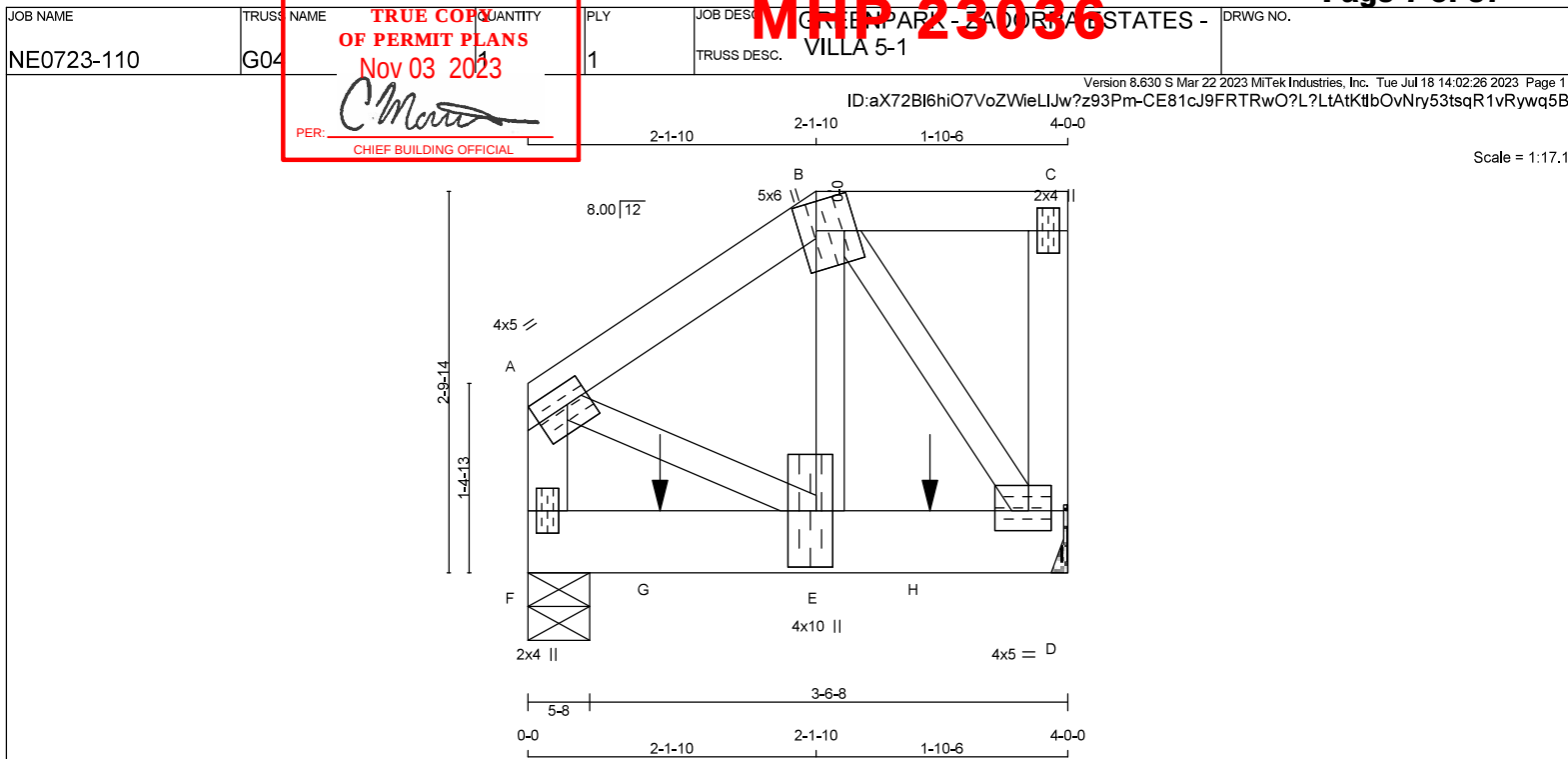
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 21 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW4	MT20	4.0	5.0	1.75	Edge
B	TTWW+m	MT20	5.0	6.0	2.50	1.50
C	TMV+p	MT20	2.0	4.0		
D	BMWW14	MT20	4.0	5.0	1.75	2.00
E	BMWW+t	MT20	4.0	10.0	5.00	1.50
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
 EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	2102	0	2102	0	0	MECHANICAL	
F	2141	0	2141	0	0	5-8	3-7

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-4.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1467	1072 / 0	0 / 0	0 / 0	0 / 0	395 / 0	0 / 0
F	1494	1092 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1214 / 0	-119.4 -119.4	0.12 (1)	5.62	E-B	0 / 1857	0.46 (1)
B-C	0 / 0	-119.4 -119.4	0.08 (1)	10.00	B-D	-1785 / 0	0.35 (1)
D-C	-111 / 0	0.0 0.0	0.02 (1)	7.81	A-E	0 / 1102	0.27 (1)
F-A	-1253 / 0	0.0 0.0	0.14 (1)	7.06			
F-G	0 / 0	-18.2 -18.2	0.63 (1)	10.00			
G-E	0 / 0	-18.2 -18.2	0.63 (1)	10.00			
E-H	0 / 1086	-18.2 -18.2	0.57 (1)	10.00			
H-D	0 / 1086	-18.2 -18.2	0.57 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	11-12	-1289	-1289		FRONT	VERT	TOTAL		C1
H	2-11-12	-1288	-1288		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.14/0.97 (A-F:1), BC=0.63/0.97 (E-F:1), WB=0.46/0.97 (B-E:1), SSI=0.86/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

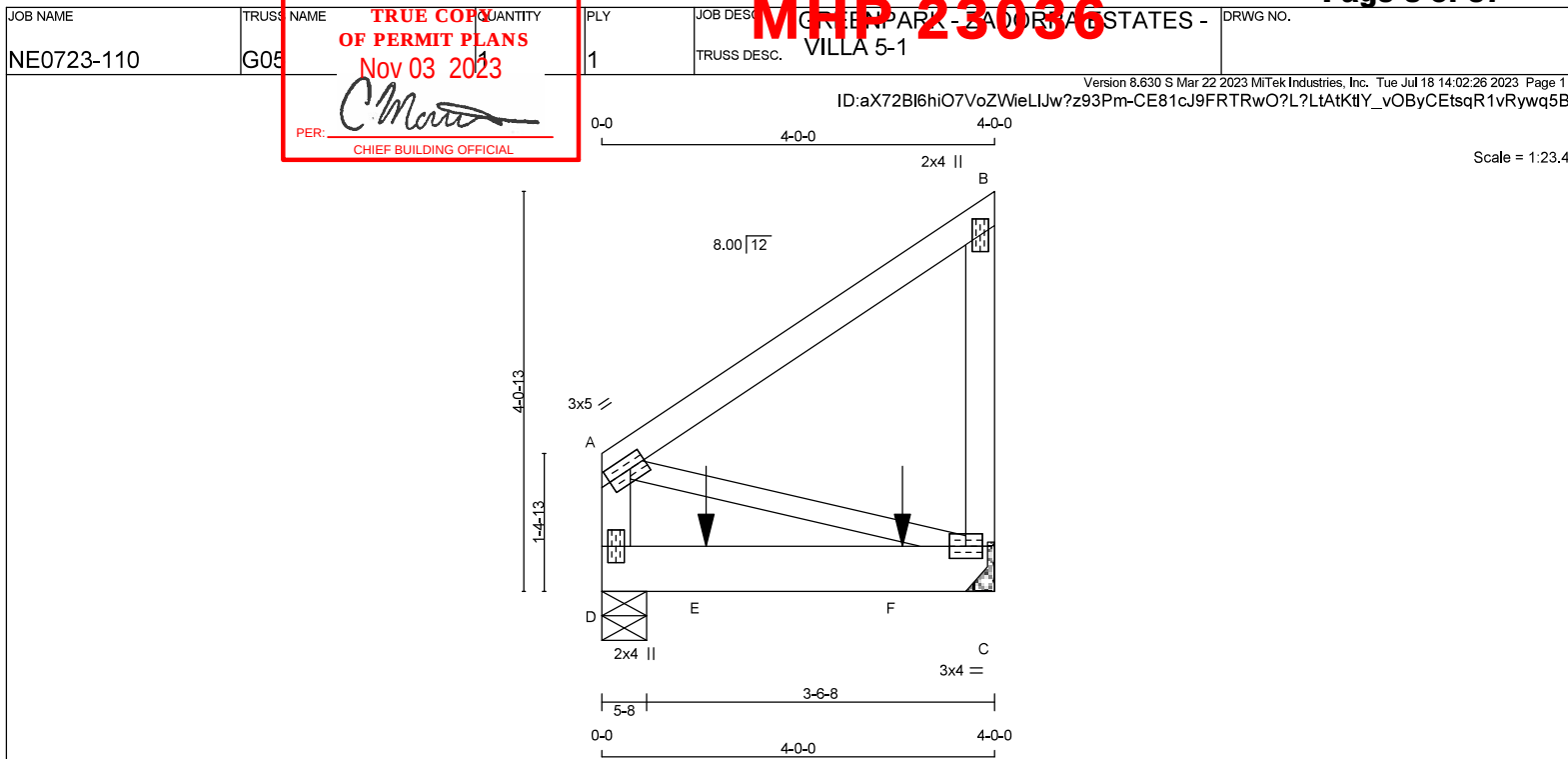
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
 JSI METAL = 0.49 (E) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 20 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - B	2x4	DRY No.2
C - B	2x4	DRY No.2
D - A	2x4	DRY No.2
D - C	2x6	DRY No.2

ALL WEBS 2x3 DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	3.0	5.0	1.50	2.00
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	991	0	991	0	0	MECHANICAL	
D	947	0	947	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS
C	691	510 / 0
D	660	488 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO				FR-TO		
A-B	0 / 0	-119.4	0.36 (1)	A-C	0 / 0	0.00 (1)
C-B	-239 / 0	0.0	0.06 (1)			
D-A	-239 / 0	0.0	0.03 (1)			
D-E	0 / 0	-18.2	0.55 (1)			
E-F	0 / 0	-18.2	0.55 (1)			
F-C	0 / 0	-18.2	0.55 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-0-12	-483	-483		BACK	VERT	TOTAL		C1
F	3-0-12	-483	-483		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/691 (0.07")

CSI: TC=0.36/0.97 (A-B-1), BC=0.55/0.97 (C-D-1), WB=0.00/0.97 (A-C-1), SSI=0.50/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

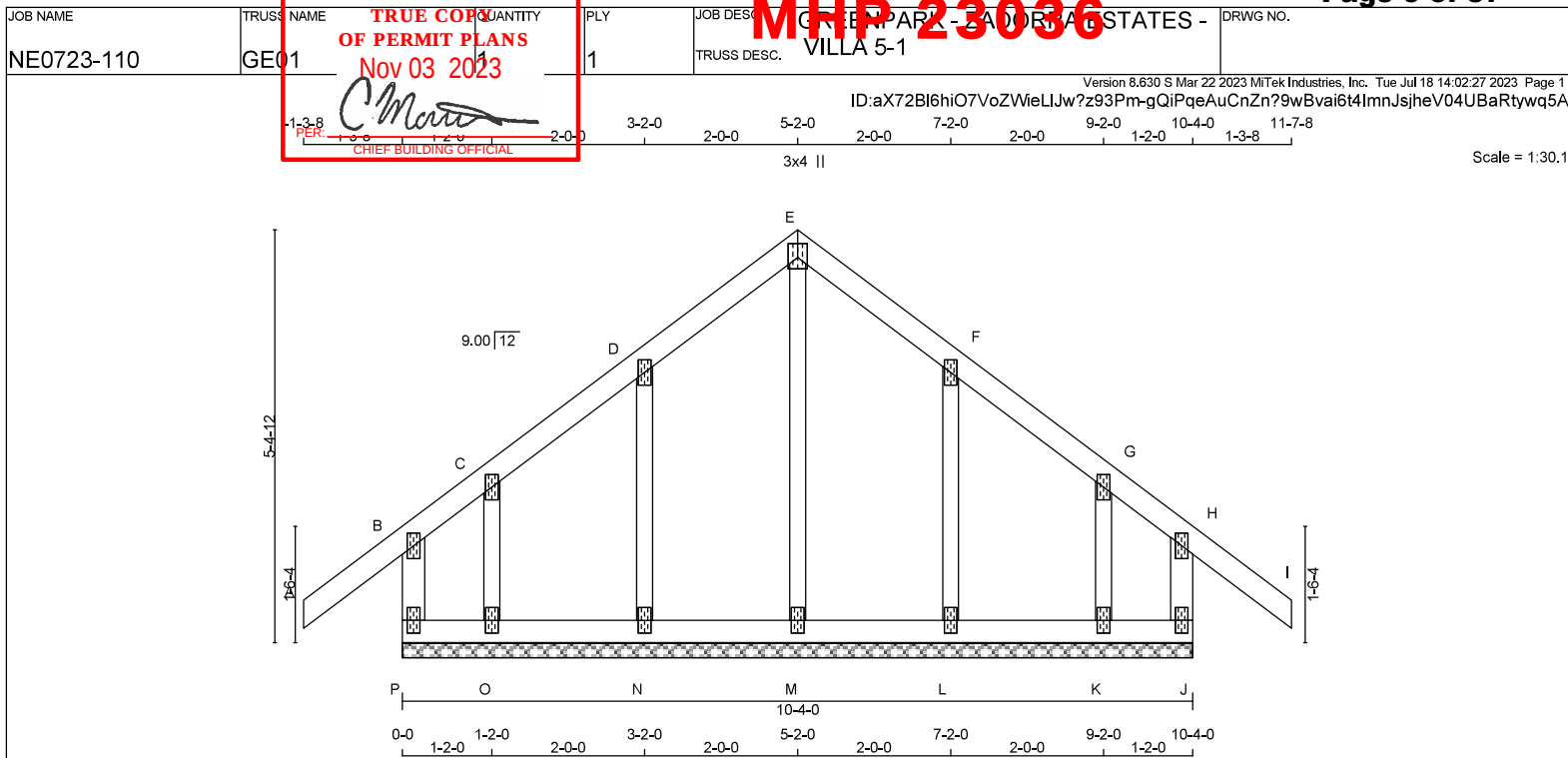
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 ALL GABLE WEBS 2x3 DRY No.2
 DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C, D, F, G						
C	TMV+w	MT20	2.0	4.0		
E	TTW+p	MT20	3.0	4.0	2.25	1.50
H	TMV+p	MT20	2.0	4.0		
J	BMV1+p	MT20	2.0	4.0		
K, L, M, N, O						
K	BMV1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-257 / 0	0.0 0.0 0.06 (1)	7.81	M-E	-302 / 0	0.13 (1)	
A-B	0 / 49	-119.4 -119.4 0.16 (1)	10.00	N-D	-259 / 0	0.06 (1)	
B-C	0 / 8	-119.4 -119.4 0.10 (1)	10.00	O-C	-115 / 0	0.02 (1)	
C-D	0 / 53	-119.4 -119.4 0.07 (1)	10.00	L-F	-259 / 0	0.06 (1)	
D-E	0 / 49	-119.4 -119.4 0.07 (1)	10.00	K-G	-115 / 0	0.02 (1)	
E-F	0 / 49	-119.4 -119.4 0.07 (1)	10.00				
F-G	0 / 53	-119.4 -119.4 0.07 (1)	10.00				
G-H	0 / 8	-119.4 -119.4 0.10 (1)	10.00				
H-I	0 / 49	-119.4 -119.4 0.16 (1)	10.00				
J-H	-257 / 0	0.0 0.0 0.06 (1)	7.81				
P-O	-30 / 0	-18.2 -18.2 0.01 (4)	6.25				
O-N	-35 / 0	-18.2 -18.2 0.02 (4)	6.25				
N-M	-42 / 0	-18.2 -18.2 0.02 (4)	6.25				
M-L	-42 / 0	-18.2 -18.2 0.02 (4)	6.25				
L-K	-35 / 0	-18.2 -18.2 0.02 (4)	6.25				
K-J	-30 / 0	-18.2 -18.2 0.01 (4)	6.25				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 2.40 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/0.97 (A-B:1), BC=0.02/0.97 (L-M:4),
 WB=0.13/0.97 (E-M:1), SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

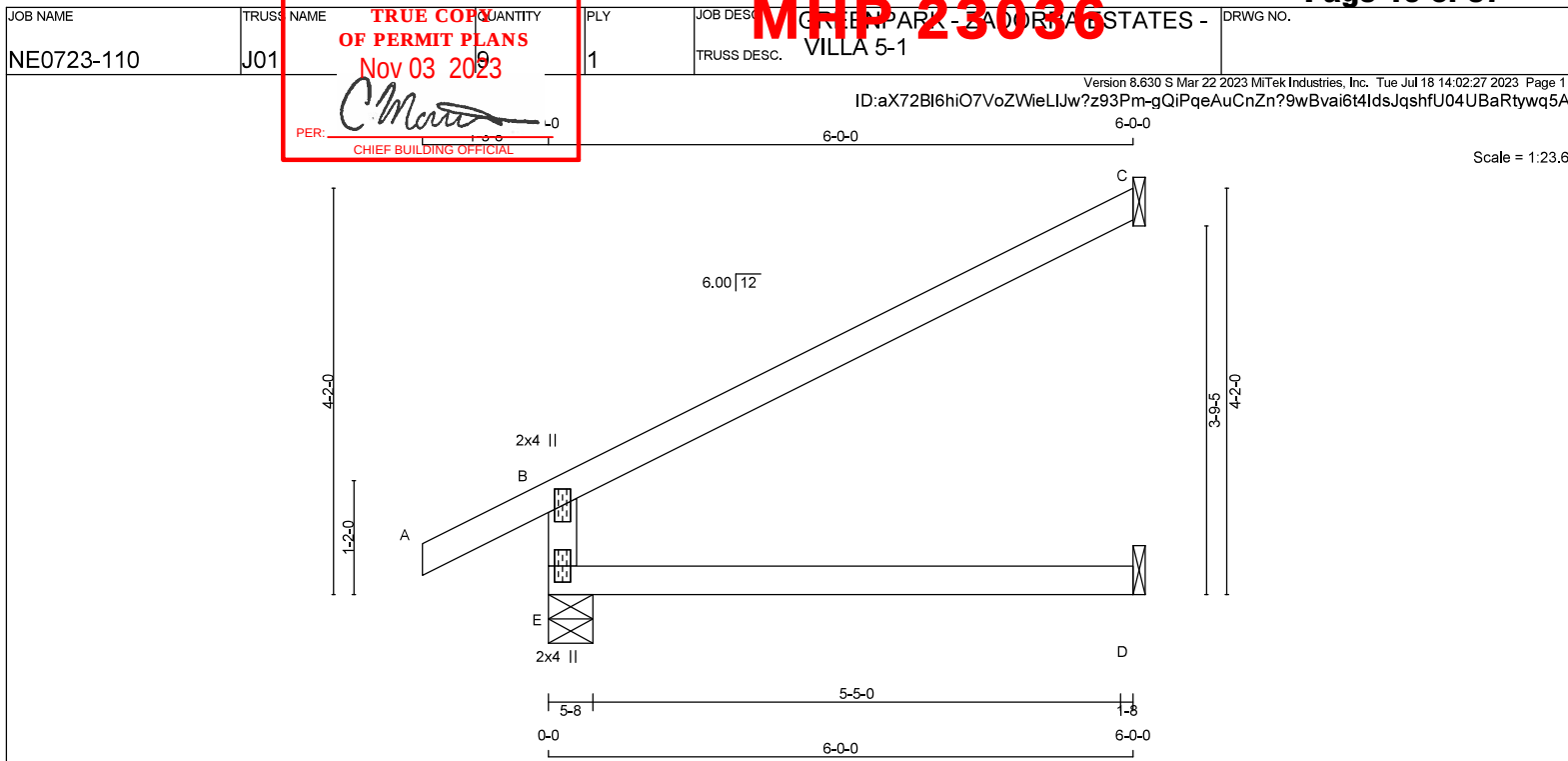
JSI GRIP= 0.21 (E) (INPUT = 0.90)

JSI METAL= 0.14 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 9 X 17 = 154 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	674	0	674	0	0	5-8	1-8
C	269	0	269	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
E	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
E	468	355 / 0 0 / 0 0 / 0 113 / 0 0 / 0
C	184	157 / 0 0 / 0 0 / 0 27 / 0 0 / 0
D	36	0 / 0 0 / 0 0 / 0 36 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MEMB. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO							
E-B	-610 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-40 / 0	-119.4	-119.4	0.73 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")CSI: TC=0.73/0.97 (B-C:1) , BC=0.13/0.97 (D-E:4) ,
WB=0.00/0.97 (n/a:0) , SSH=0.31/1.00 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PL)	(PL)	(PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

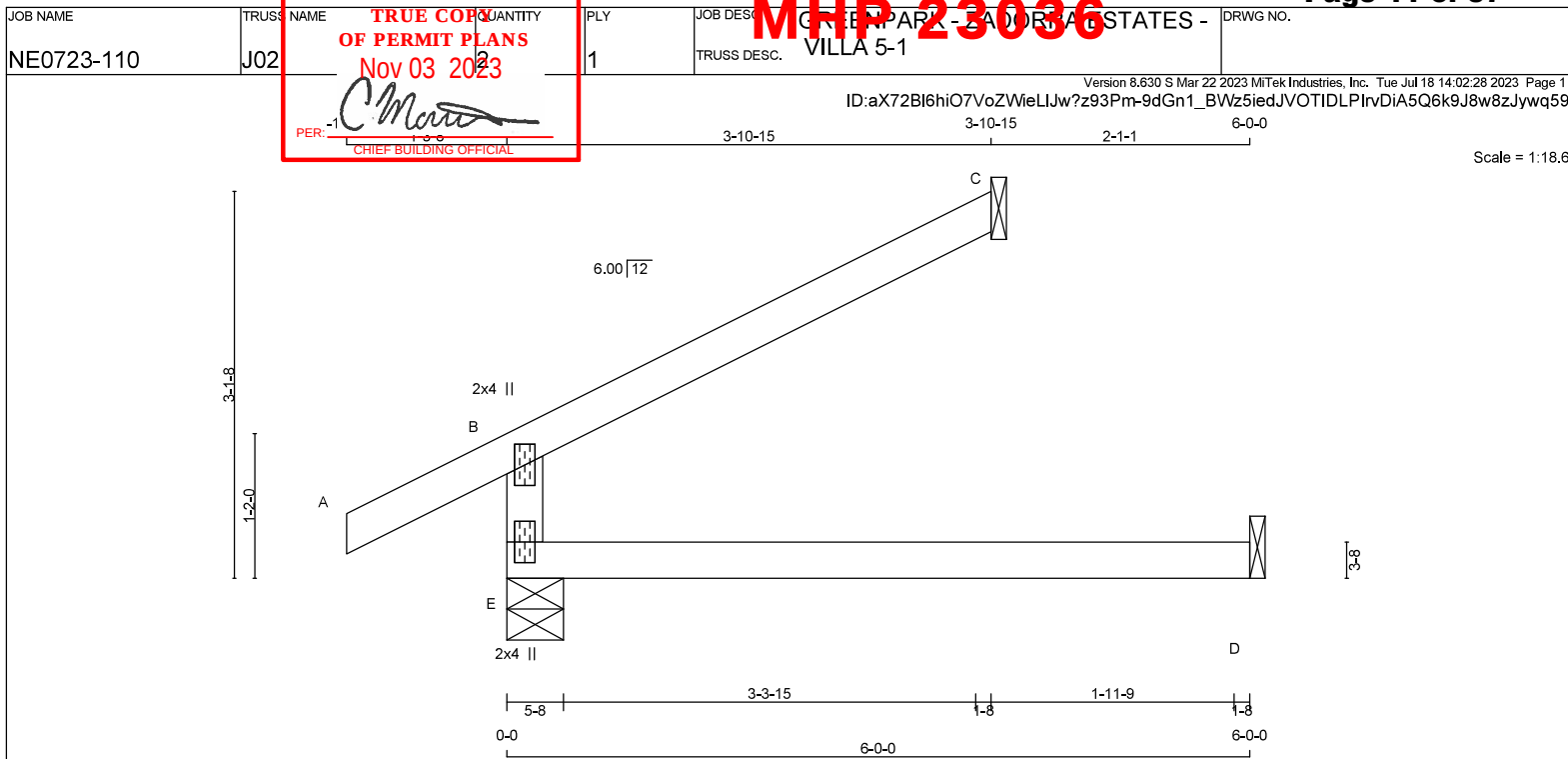
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	518	0	518	0	0	5-8	1-8
C	175	0	175	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	362	265 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0	0 / 0
C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MEMB. LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO										
E-B		-454 / 0	0.0	0.0	0.13 (4)	7.81				
A-B		0 / 36	-119.4	-119.4	0.16 (1)	10.00				
B-C		-26 / 0	-119.4	-119.4	0.31 (1)	6.25				
E-D		0 / 0	-18.2	-18.2	0.13 (4)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.31/0.97 (B-C:1) , BC=0.13/0.97 (D-E:4) ,
 WB=0.00/0.97 (n/a:0) , SSH=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PL)	(PL)	(PL)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

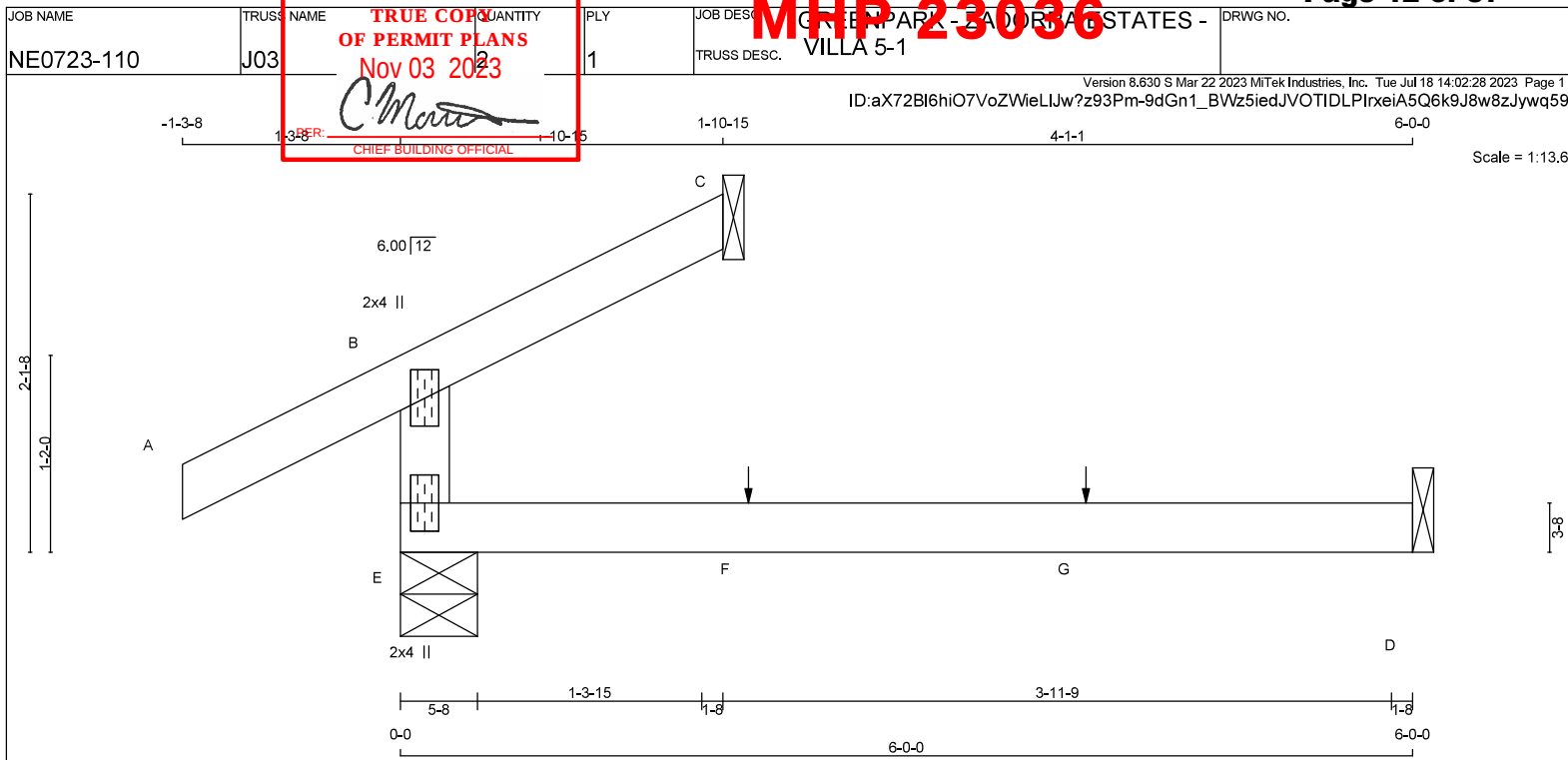
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
 JSI METAL= 0.19 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 12 = 24 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	REQD	BRG
E	368	0	368	0	0	5-8	1-8		
C	86	0	86	0	0	1-8	1-8		
D	45	0	51	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	259	177 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
E-B	-304 / 0	0.0	0.0 0.13 (4)	7.81			
A-B	0 / 36	-119.4	-119.4 0.16 (1)	10.00			
B-C	-12 / 0	-119.4	-119.4 0.07 (1)	6.25			
E-F	0 / 0	-18.2	-18.2 0.13 (4)	10.00			
F-G	0 / 0	-18.2	-18.2 0.13 (4)	10.00			
G-D	0 / 0	-18.2	-18.2 0.13 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1
G	4-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.16/0.97 (A-B:1) , BC=0.13/0.97 (D-E:4) ,
WB=0.00/0.97 (n/a:0) , SSI=0.11/1.00 (A-B:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

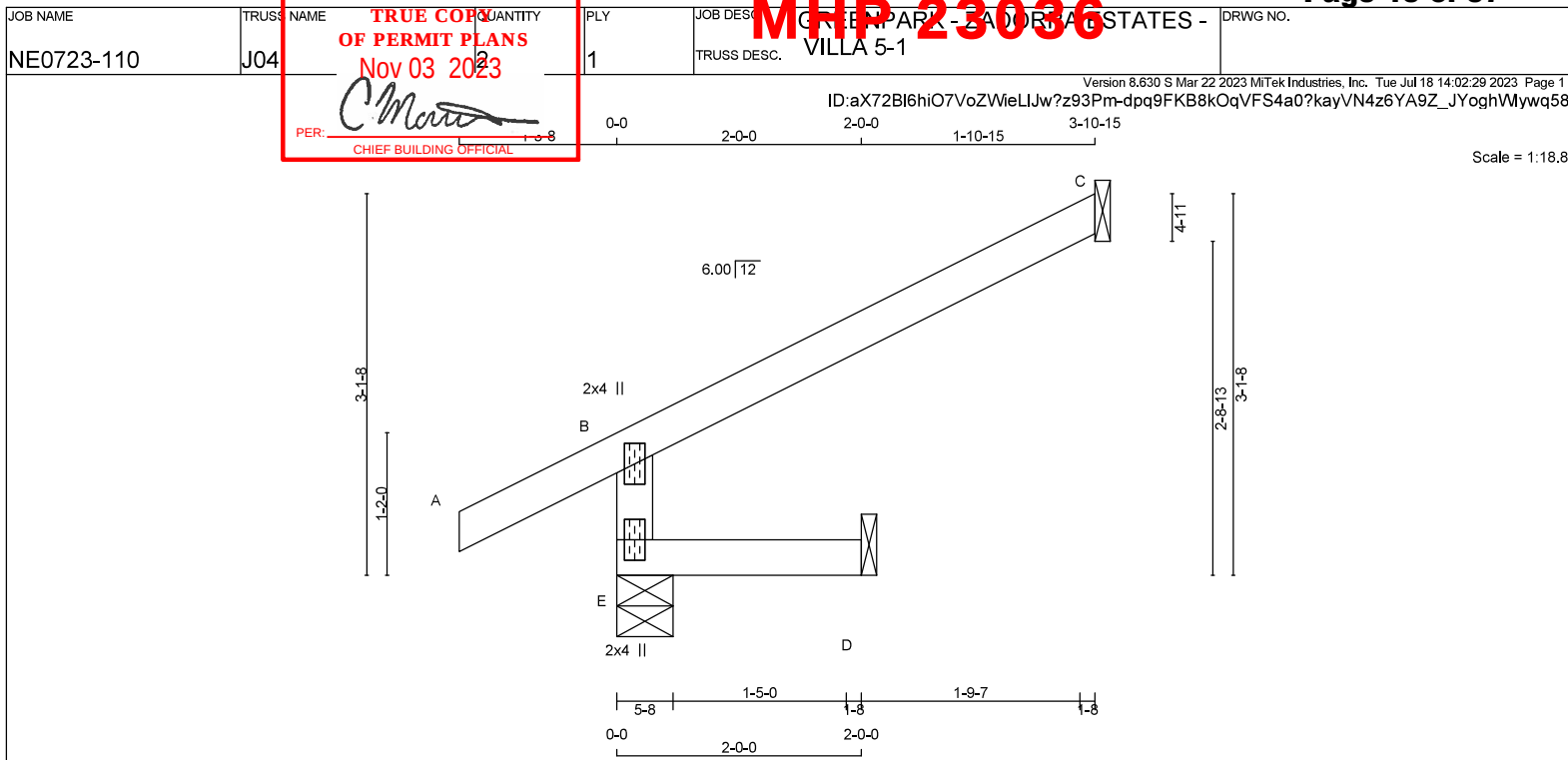
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 10 = 20 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	474	0	474	0	5-8	1-8
C	175	0	175	0	1-8	1-8
D	16	0	16	0	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	326	265 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORZ. LOAD (LC2)	MEMB. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)	MAX. HORZ. LOAD (LC2)
FR-TO							
E-B	-454 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-26 / 0	-119.4	-119.4	0.31 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")CSI: TC=0.31/0.97 (B-C-1) , BC=0.02/0.97 (D-E-4) ,
WB=0.00/0.97 (n/a-0) , SSH=0.20/1.00 (B-C-1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PL)	(PL)	(PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

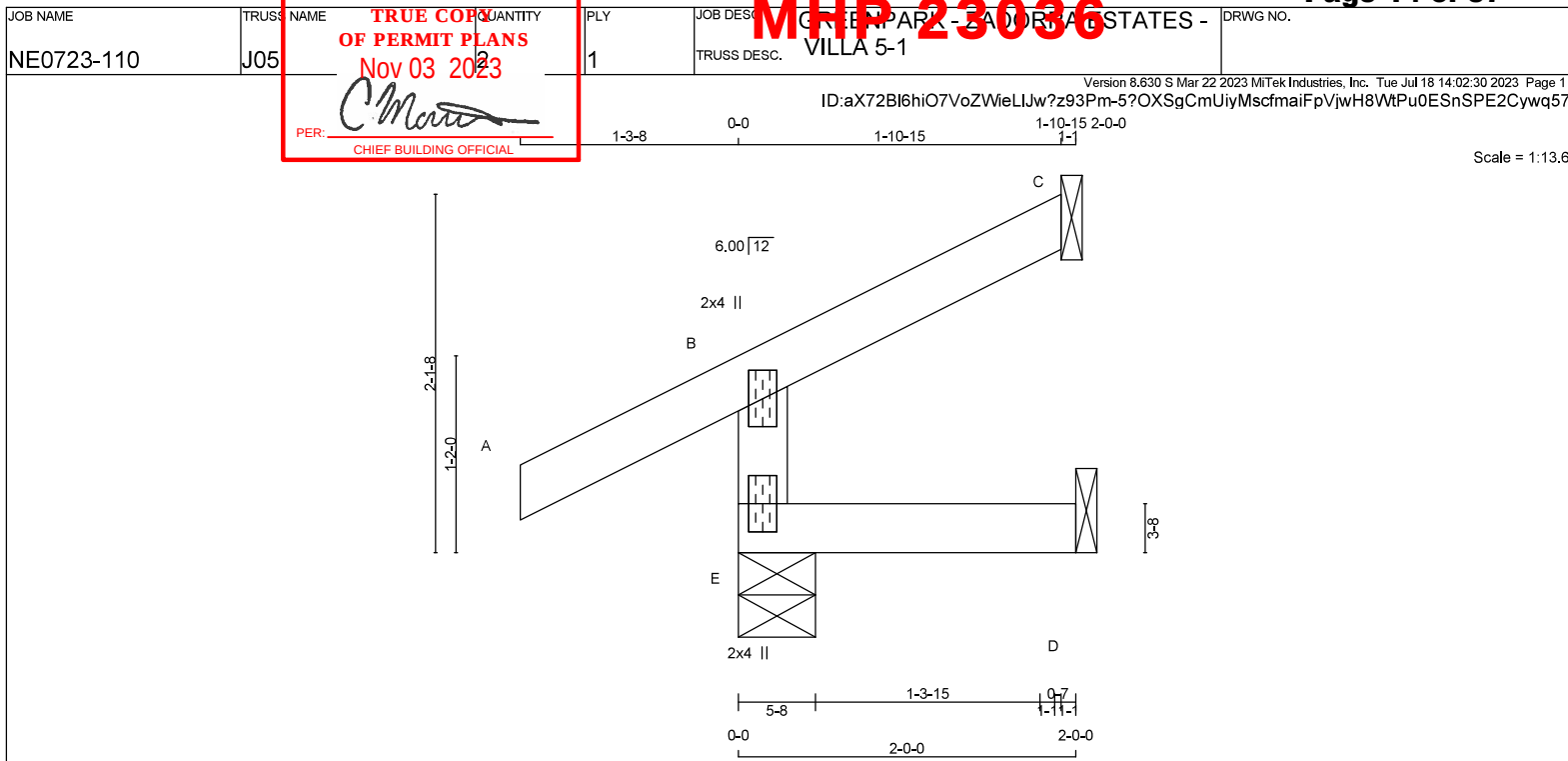
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL = 0.19 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	324	0	324	0	0	5-8	1-8
C	86	0	86	0	0	1-8	1-8
D	16	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	224	177 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO			
E-B	-304 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-12 / 0	-119.4	-119.4	0.07 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.16/0.97 (A-B:1) , BC=0.02/0.97 (D-E:4) ,
 WB=0.00/0.97 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PL)	(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

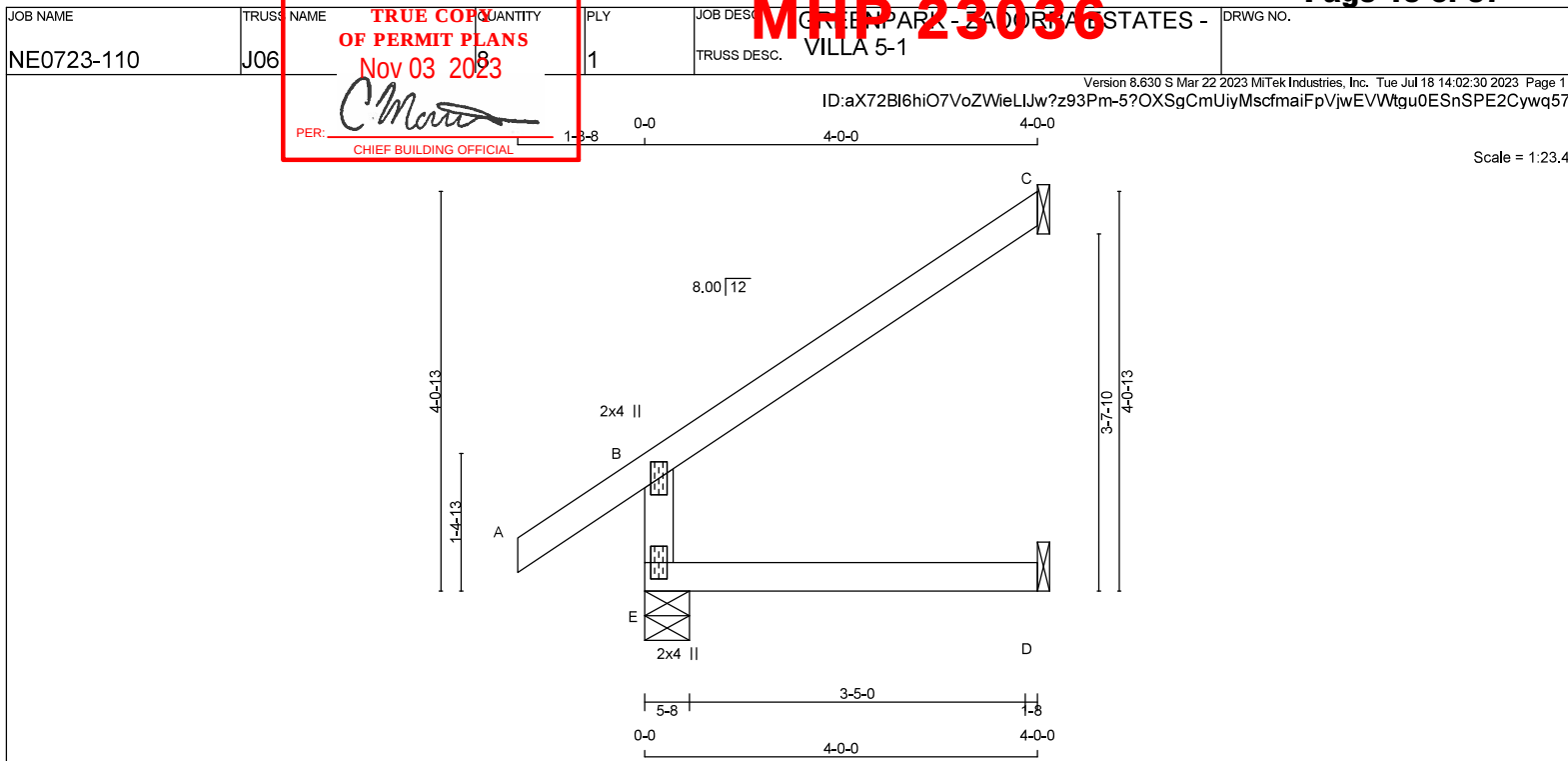
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
 JSI METAL= 0.13 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 8 X 13 = 105 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	504	0	504	0	5-8	1-8
C	179	0	179	0	1-8	1-8
D	31	0	35	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	349	270 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0	0 / 0
C	123	105 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	0 / 0
D	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO						FR-TO			
E-B	-462 / 0	0.0	0.0	0.05 (4)	7.81				
A-B	0 / 45	-119.4	-119.4	0.16 (1)	10.00				
B-C	-33 / 0	-119.4	-119.4	0.32 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.06 (4)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.32/0.97 (B-C:1) , BC=0.06/0.97 (D-E:4) ,
 WB=0.00/0.97 (n/a:0) , SSH=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
 FOR QUALITY CONTROL IN THE TRUSS
 MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PL)	(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

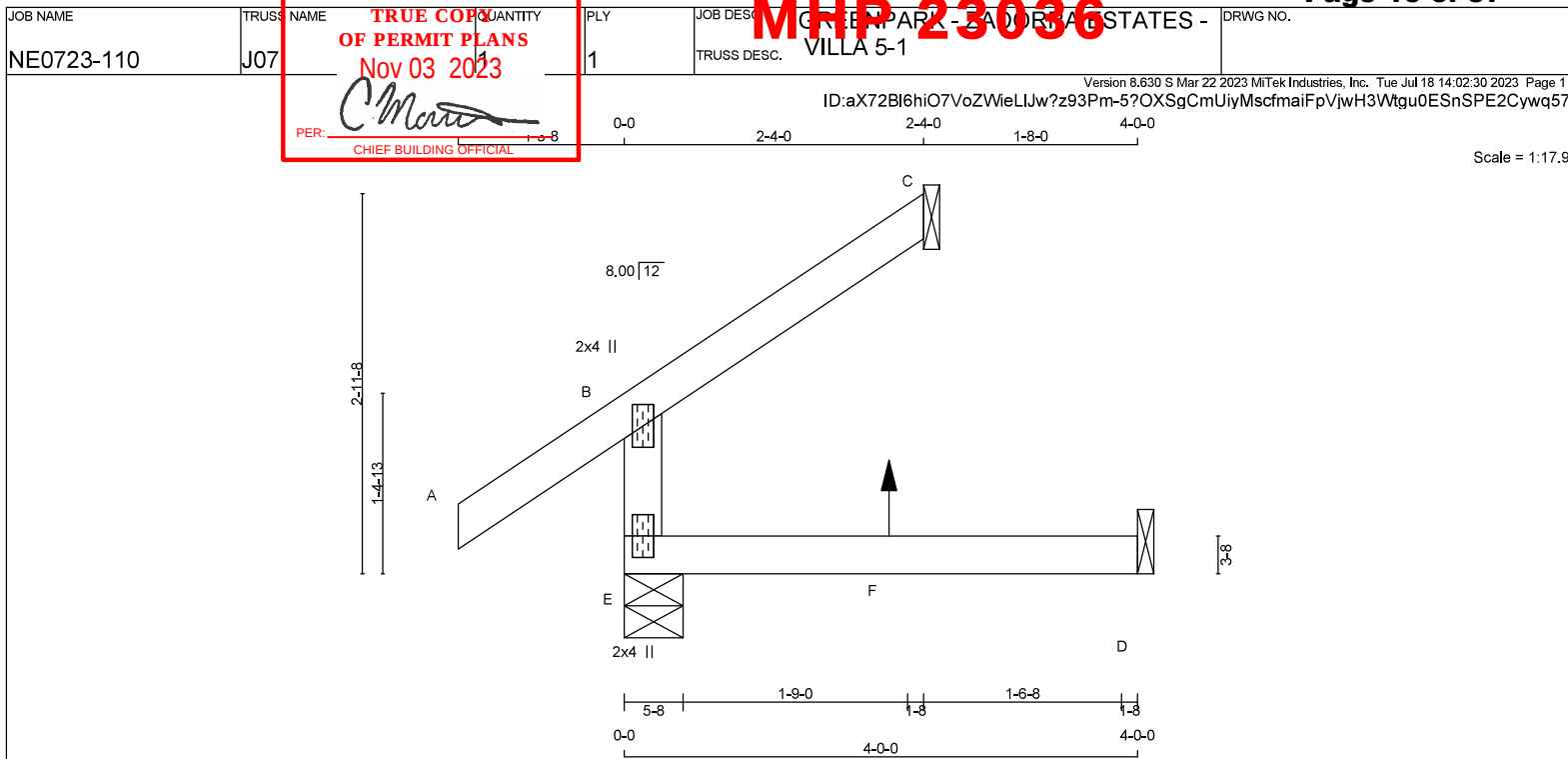
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
 JSI METAL = 0.24 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 11 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	374	0	374	0	5-8	1-8
C	105	0	105	0	1-8	1-8
D	28	0	35	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	260	193 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0	0 / 0
C	72	61 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0	0 / 0
D	23	0 / -3	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO								
E-B	-338 / 0		0.0	0.0	0.05 (4)		7.81	
A-B	0 / 45		-119.4	-119.4	0.16 (1)		10.00	
B-C	-19 / 0		-119.4	-119.4	0.11 (1)		6.25	
E-F	0 / 0		-18.2	-18.2	0.06 (4)		10.00	
F-D	0 / 0		-18.2	-18.2	0.06 (4)		10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	6	1	6	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.16/0.97 (A-B:1) , BC=0.06/0.97 (D-E:4) ,
 WB=0.00/0.97 (n/a:0) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

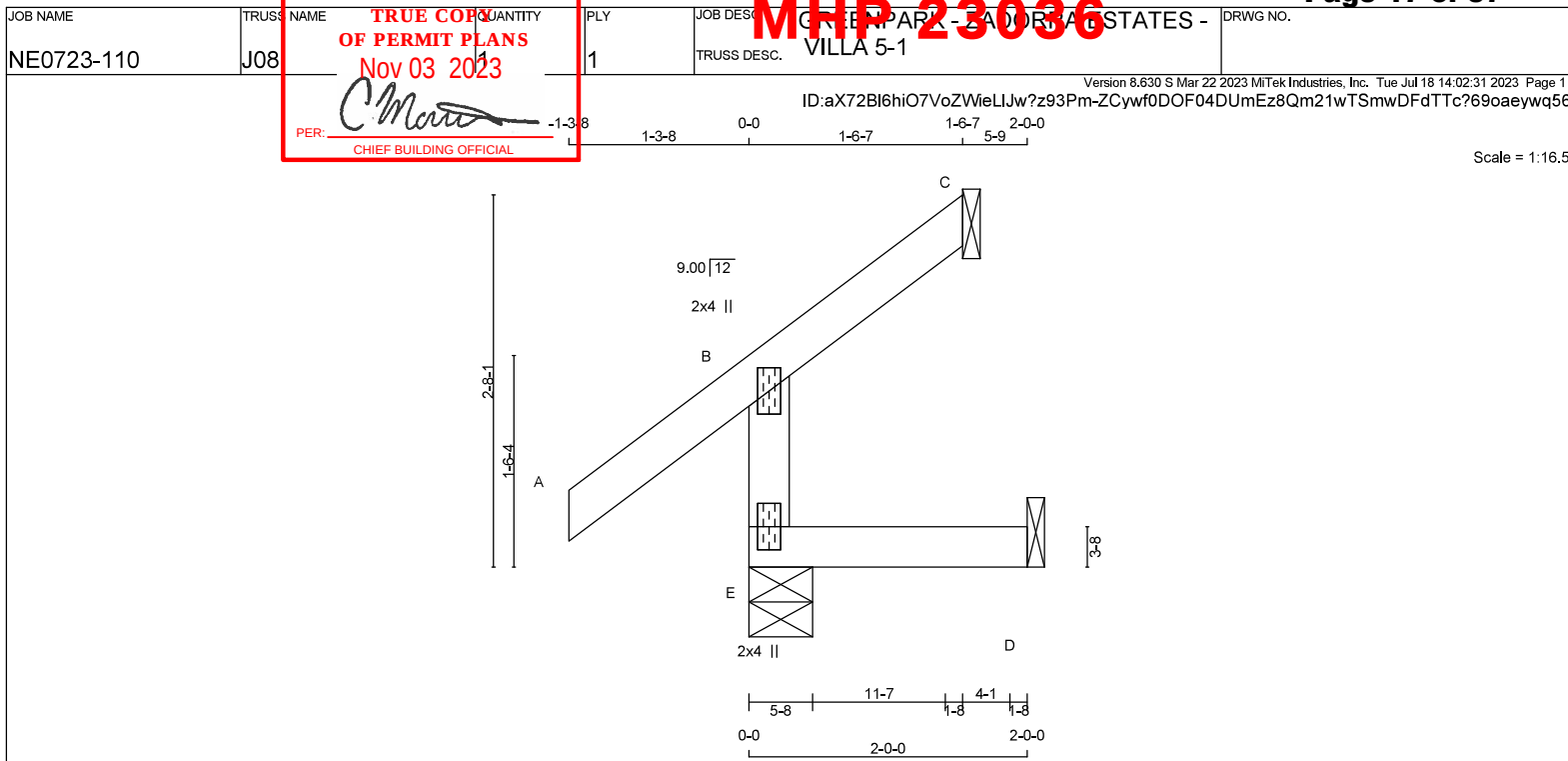
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
 JSI METAL= 0.17 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 8 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	345	0	345	0	0	5-8	1-8
C	33	0	33	0	-46	1-8	1-8
D	7	0	18	0	-2	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	238	191 / 0	0 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	18 / -32	0 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	7	0 / -9	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD LC1 (PLF)
E-B	-315 / 0	0.0	0.0	0.04 (5)	7.81				
A-B	0 / 49	-119.4	-119.4	0.16 (1)	10.00				
B-C	-35 / 0	-119.4	-119.4	0.12 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.04 (5)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.16/0.97 (A-B:1) , BC=0.04/0.97 (D-E:5) ,
 WB=0.00/0.97 (n/a:0) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
 FOR QUALITY CONTROL IN THE TRUSS
 MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

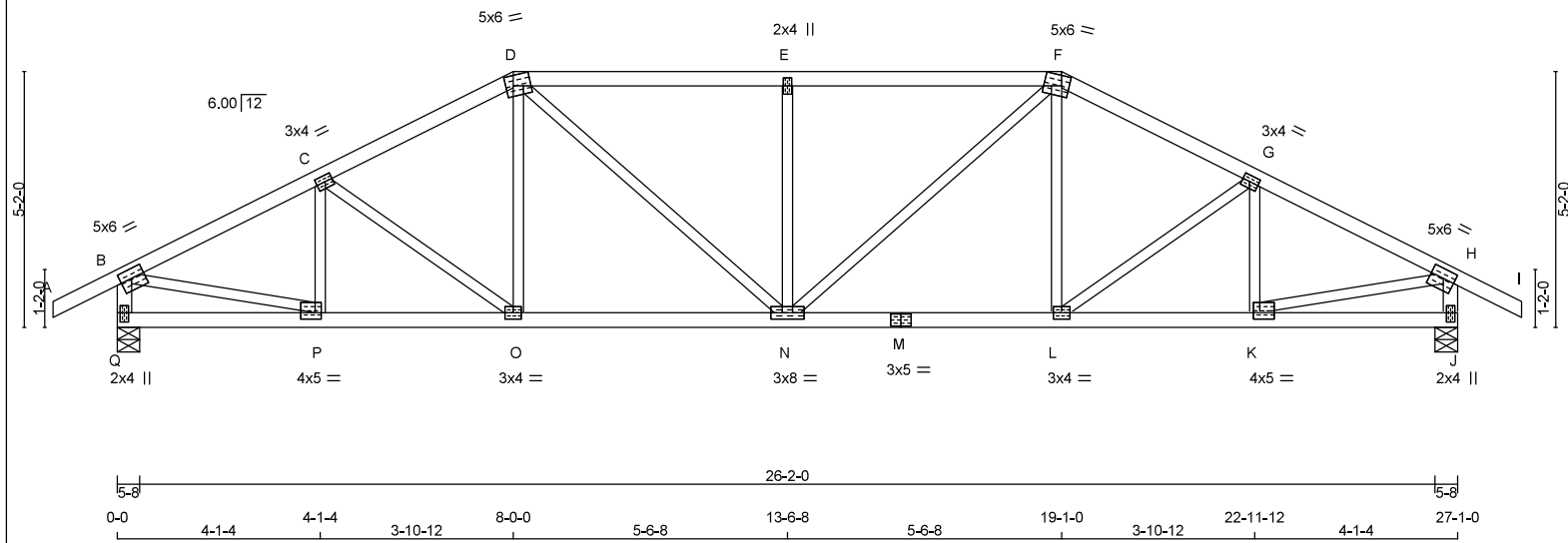
JSI GRIP= 0.21 (B) (INPUT = 0.90)
 JSI METAL= 0.17 (B) (INPUT = 1.00)

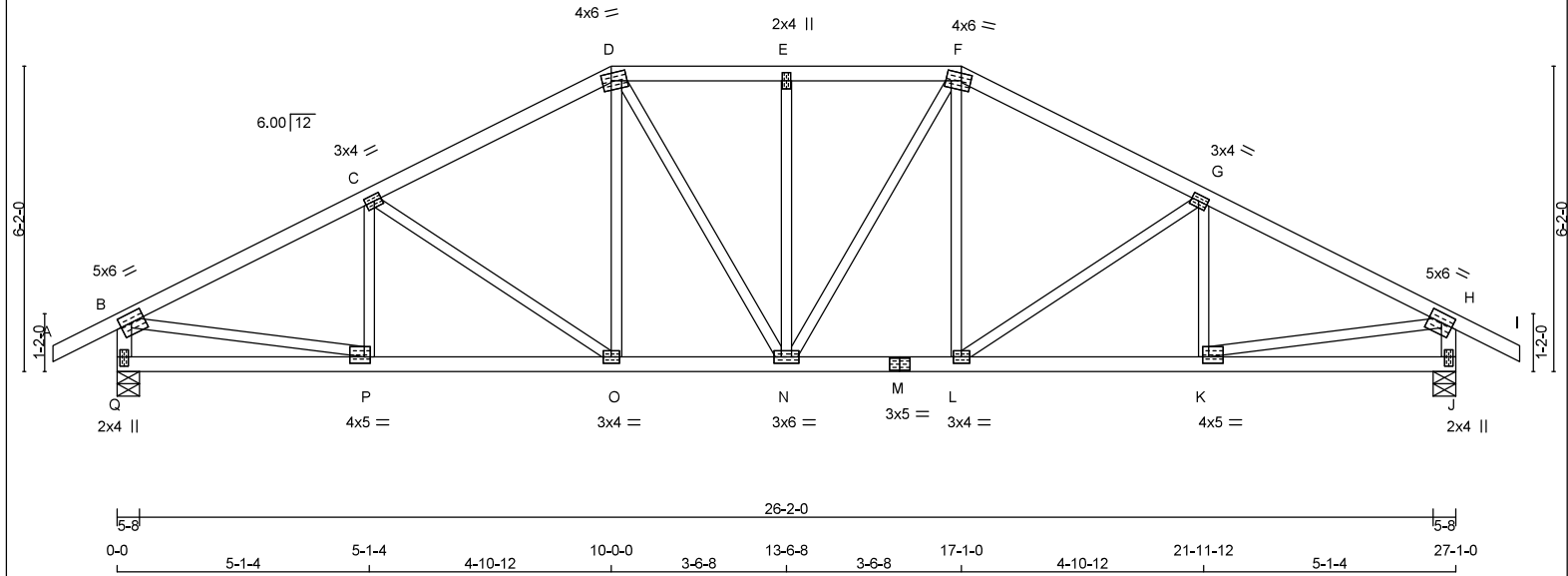


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 IN THE DESIGN OF THIS COMPONENT.



JOB NAME NE0723-110	TRUSS NAME T01	QUANTITY 1	PLY 1	JOB DESCRIPTION CHEMPARK - 240' R/W STATES - VILLA 5-1	DRWG NO.
Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 18 14:02:31 2023 Page 1 ID: aX72B16hiO7VoZWleLIJw?z93Pm-ZCywf0DOF04DUmEz8Qm21wTMQw7HdLdc?69oaeywq56 Scale = 1:46.5					





TOTAL WEIGHT = 116 lb

LUMBER

N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
Q - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
Q - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

J	T	PLATES	W	L	Y	X
B	TMWV+	MT20	5.0	6.0	2.25	2.75
C	TMWV+	MT20	3.0	4.0	1.50	1.75
D	TMWV-m	MT20	4.0	6.0	1.75	2.25
E	TMWV+	MT20	2.0	3.0	1.00	1.50
F	TTWV-m	MT20	4.0	6.0	1.75	2.25
G	TMWV+	MT20	3.0	4.0	1.50	1.75
H	TMWV+	MT20	5.0	6.0	2.25	2.75
I	BMV+1p	MT20	2.0	4.0	2.25	1.00
K	BMWV+	MT20	4.0	5.0	1.50	1.50
L	BMWV+	MT20	3.0	4.0		
M	BS+	MT20	3.0	5.0		
N	BMWV-m	MT20	3.0	4.0	3.0	6.0
O	BMWV+	MT20	3.0	4.0		
P	BMWV+	MT20	4.0	5.0	1.50	1.50
Q	BMV+1p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2026	0	2026	0	0	5-8	3-1
J	2026	0	2026	0	0	5-8	3-1

UNFACTORED REACTIONS

JT	1ST LCASE	MAX, MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0
J	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 36	-119.4	1.0	1.0	P-C	-305 / 9	0.07 (1)
B-C	-2621 / 0	-119.4	1.0	1.0	P-C	-444 / 0	0.28 (1)
C-D	-2274 / 0	-119.4	1.0	1.0	O-D	0 / 338	0.08 (1)
D-E	-2177 / 0	-119.4	1.0	1.0	D-N	0 / 323	0.07 (1)
E-F	-2177 / 0	-119.4	1.0	1.0	N-E	-506 / 0	0.30 (1)
F-G	-2274 / 0	-119.4	1.0	1.0	N-E	0 / 323	0.07 (1)
G-H	-2621 / 0	-119.4	1.0	1.0	L-F	0 / 338	0.08 (1)
H-I	0 / 36	-119.4	1.0	1.0	K-G	-444 / 0	0.28 (1)
Q-B	-1884 / 0	0.0	0.0	0.0	K-H	-305 / 9	0.07 (1)
J-H	-1884 / 0	0.0	0.0	0.0	B-P	0 / 2405	0.54 (1)
					K-H	0 / 2405	0.54 (1)
Q-P	0 / 0	-18.2	0.1	10.0			
P-O	0 / 2372	-18.2	0.2	10.0			
Q-N	0 / 2011	-18.2	0.3	10.0			
N-M	0 / 2011	-18.2	0.3	10.0			
M-L	0 / 2011	-18.2	0.3	10.0			
L-K	0 / 2372	-18.2	0.2	10.0			
K-J	0 / 0	-18.2	0.1	10.0			

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD =		48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.47/0.97 (G-H:1) , BC=0.42/0.97 (K-L:1) ,
WB=0.54/0.97 (H-K:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION | ME | LOAD | F

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

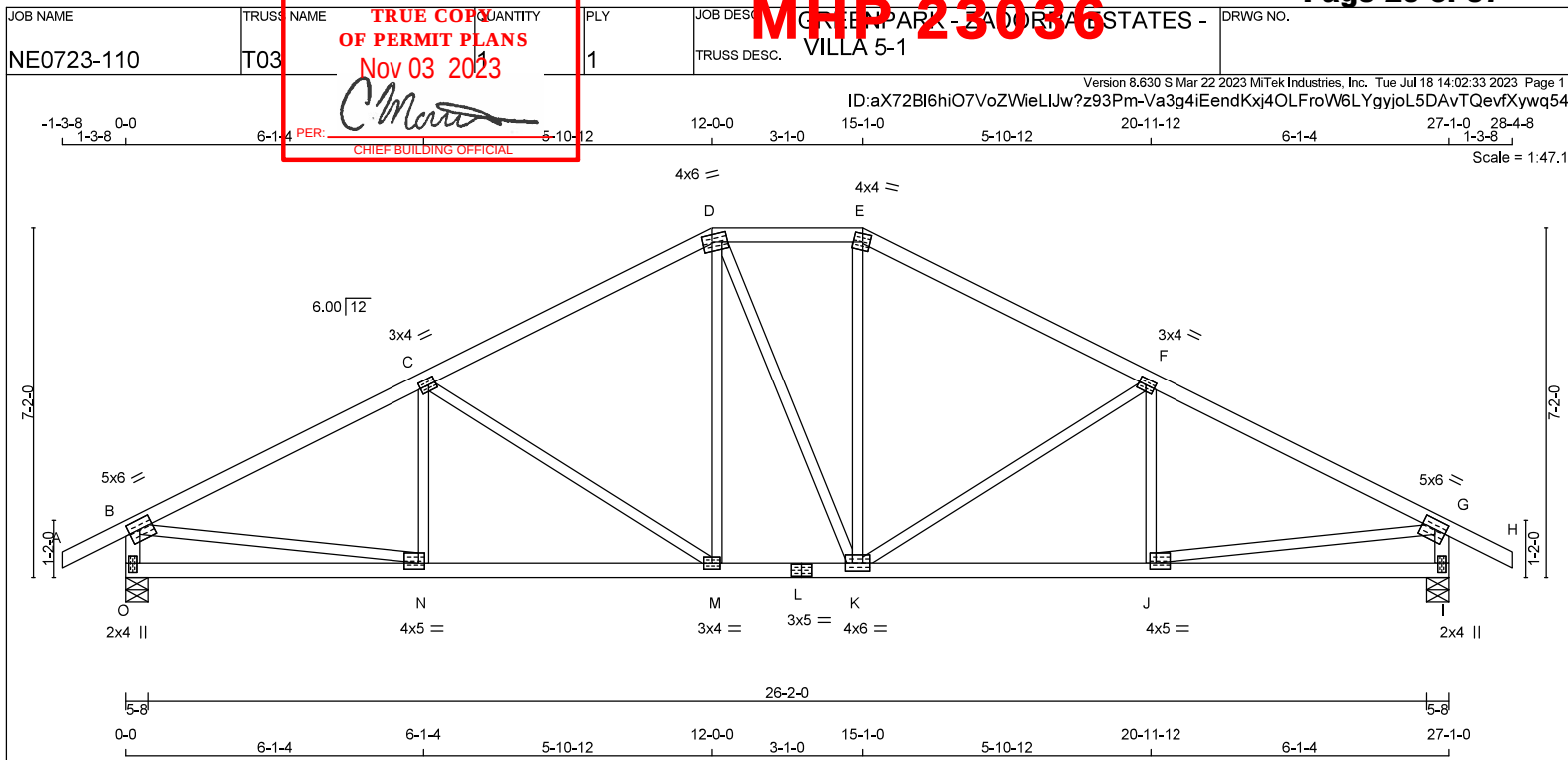
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.68 (K) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	5.0	6.0	2.25	2.75
C	TMVW4	MT20	3.0	4.0	1.50	1.75
D	TTVW-m	MT20	4.0	6.0	1.75	2.25
E	TTVW-m	MT20	4.0	4.0	2.00	1.75
F	TMVW4	MT20	3.0	4.0	1.50	1.75
G	TMVW4	MT20	5.0	6.0	2.25	2.75
I	BMV1+p	MT20	2.0	4.0	2.25	1.00
J	BMVW4	MT20	4.0	5.0	1.50	1.50
K	BMVW4	MT20	4.0	6.0		
L	BS4	MT20	3.0	5.0		
M	BMVW4	MT20	3.0	4.0		
N	BMVW4	MT20	4.0	5.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
O	2026	0	2026	0
I	2026	0	2026	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0
I	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	N-C	-216 / 53	0.06 (1)	
B-C	-2643 / 0	-119.4	-119.4	0.69 (1)	3.43	C-M	-698 / 0	0.69 (1)	
C-D	-2066 / 0	-119.4	-119.4	0.62 (1)	3.92	M-D	0 / 456	0.10 (1)	
D-E	-1821 / 0	-119.4	-119.4	0.18 (1)	4.77	D-K	0 / 4	0.00 (1)	
E-F	-2068 / 0	-119.4	-119.4	0.62 (1)	3.92	K-E	0 / 460	0.10 (1)	
F-G	-2642 / 0	-119.4	-119.4	0.69 (1)	3.43	K-F	-696 / 0	0.69 (1)	
G-H	0 / 36	-119.4	-119.4	0.16 (1)	10.00	J-F	-218 / 52	0.06 (1)	
O-B	-1979 / 0	0.0	0.0	0.20 (1)	5.92	B-N	0 / 2421	0.54 (1)	
I-G	-1978 / 0	0.0	0.0	0.20 (1)	5.92	J-G	0 / 2421	0.54 (1)	
O-N	0 / 0	-18.2	-18.2	0.16 (4)	10.00				
N-M	0 / 2398	-18.2	-18.2	0.45 (1)	10.00				
M-L	0 / 1819	-18.2	-18.2	0.35 (1)	10.00				
L-K	0 / 1819	-18.2	-18.2	0.35 (1)	10.00				
K-J	0 / 2397	-18.2	-18.2	0.45 (1)	10.00				
J-I	0 / 0	-18.2	-18.2	0.16 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL)= L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.69/0.97 (B-C:1) , BC=0.45/0.97 (M-N:1) ,
 WB=0.69/0.97 (C-M:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PL)
 (PSI) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

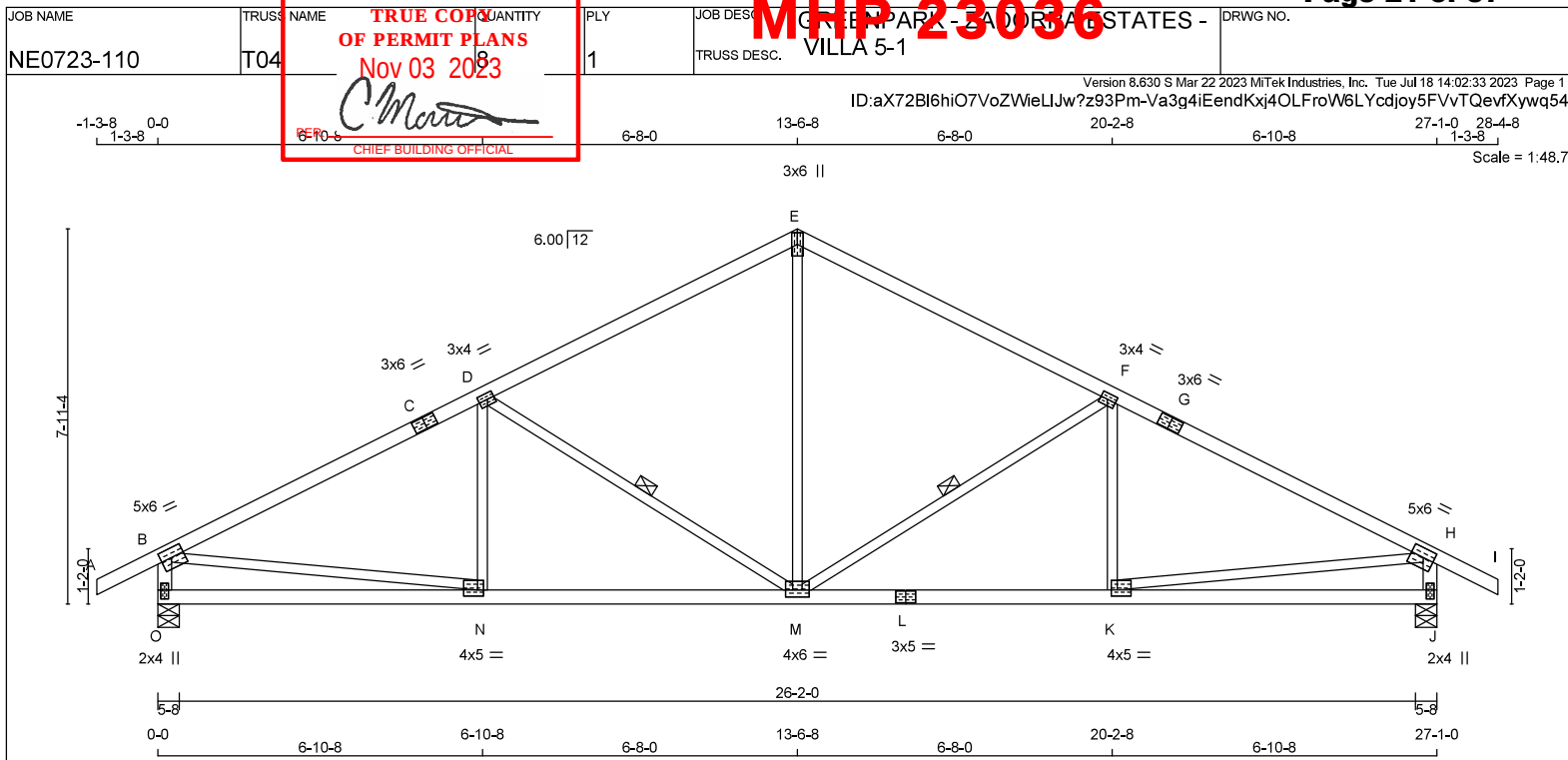
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
 JSI METAL = 0.68 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 8 X 106 = 849 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW4	MT20	5.0	6.0	2.25	2.75
C TS4	MT20	3.0	6.0		
D TMWW4	MT20	3.0	4.0	1.50	1.75
E TTW+p	MT20	3.0	6.0		
F TMWW4	MT20	3.0	4.0	1.50	1.75
G TS4	MT20	3.0	6.0		
H TMWW4	MT20	5.0	6.0	2.25	2.75
J BMV1+p	MT20	2.0	4.0	2.25	1.00
K BMWW4	MT20	4.0	5.0	1.50	1.50
L BS4	MT20	3.0	5.0		
M BMWWWW4	MT20	4.0	6.0	1.75	3.00
N BMWW4	MT20	4.0	5.0	1.50	1.50
O BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	0	2026	0	5-8
O	2026	0	5-8	3-1
J	2026	0	5-8	3-1

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1414	1037 / 0	0 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0
J	1414	1037 / 0	0 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M, DBS = 20'-0". CBF = 109 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE (PER BRACE), FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LBS)	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 36	-119.4	-119.4 0.16 (1)	10.00	M-E	0 / 1065	0.24 (1)
B-C	-2634 / 0	-119.4	-119.4 0.91 (1)	2.97	M-F	-870 / 0	0.37 (1)
C-D	-2634 / 0	-119.4	-119.4 0.91 (1)	2.97	K-F	-167 / 76	0.05 (1)
D-E	-1904 / 0	-119.4	-119.4 0.79 (1)	3.65	D-M	-870 / 0	0.37 (1)
E-F	-1904 / 0	-119.4	-119.4 0.79 (1)	3.65	N-D	-167 / 76	0.05 (1)
F-G	-2634 / 0	-119.4	-119.4 0.91 (1)	2.97	B-N	0 / 2413	0.54 (1)
G-H	-2634 / 0	-119.4	-119.4 0.91 (1)	2.97	K-H	0 / 2413	0.54 (1)
H-I	0 / 36	-119.4	-119.4 0.16 (1)	10.00			
O-B	-1973 / 0	0.0	0.0 0.20 (1)	5.92			
J-H	-1973 / 0	0.0	0.0 0.20 (1)	5.92			
O-N	0 / 0	-18.2	-18.2 0.20 (4)	10.00			
N-M	0 / 2394	-18.2	-18.2 0.47 (1)	10.00			
M-L	0 / 2394	-18.2	-18.2 0.47 (1)	10.00			
L-K	0 / 2394	-18.2	-18.2 0.47 (1)	10.00			
K-J	0 / 0	-18.2	-18.2 0.20 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.90")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.90")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.21")

CSI: TC=0.91/0.97 (F-H:1), BC=0.47/0.97 (K-M:1),
 WB=0.54/0.97 (H-K:1), SSI=0.35/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

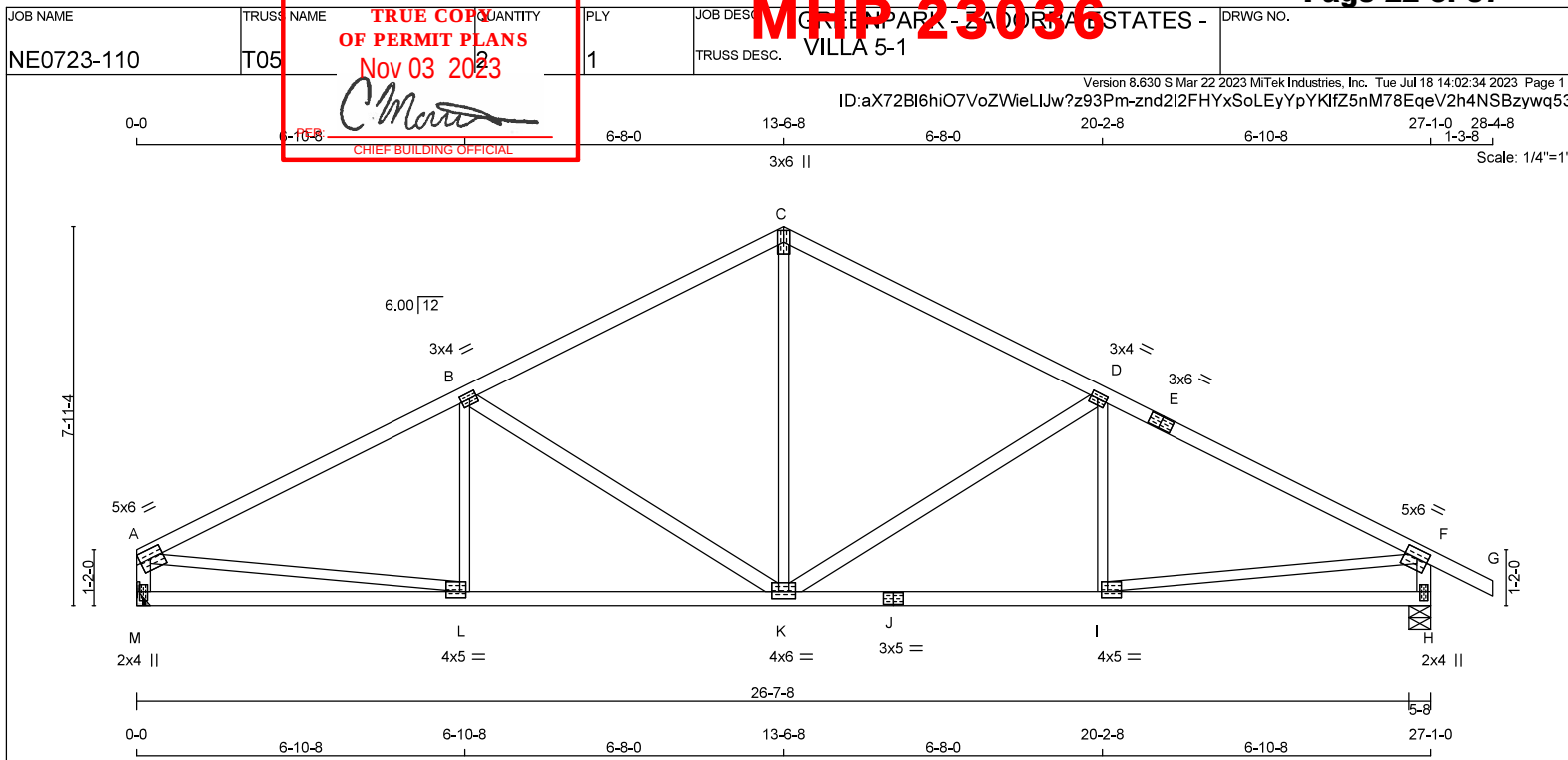
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
 JSI METAL = 0.74 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 110 = 219 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - A	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - D	2x4	DRY	No.2
B - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	5.0	6.0	2.25	Edge
B	TMVW4	MT20	3.0	4.0	1.50	1.75
C	TTW+p	MT20	3.0	6.0		
D	TMVW4	MT20	3.0	4.0	1.50	1.75
E	TS4	MT20	3.0	6.0		
F	TMVW4	MT20	5.0	6.0	2.25	2.75
H	BMV1+p	MT20	2.0	4.0	2.25	1.00
I	BMVW4	MT20	4.0	5.0	1.50	1.50
J	BS4	MT20	3.0	5.0		
K	BMVW4	MT20	4.0	6.0	1.75	3.00
L	BMVW4	MT20	4.0	5.0	1.50	1.50
M	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
M	1864	0	1864	0
H	2026	0	2026	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 2-9.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1303	943 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
H	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO (LC)	
FR-TO				FR-TO			
A-B	-2635 / 0	-119.4	-119.4 0.91 (1)	2.95	K-C	0 / 1065	0.24 (1)
B-C	-1904 / 0	-119.4	-119.4 0.79 (1)	3.65	K-D	-871 / 0	0.81 (1)
C-D	-1904 / 0	-119.4	-119.4 0.79 (1)	3.65	I-D	-166 / 76	0.05 (1)
D-E	-2635 / 0	-119.4	-119.4 0.91 (1)	2.95	B-K	-871 / 0	0.81 (1)
E-F	-2635 / 0	-119.4	-119.4 0.91 (1)	2.95	L-B	-166 / 76	0.05 (1)
F-G	0 / 36	-119.4	-119.4 0.16 (1)	10.00	A-L	0 / 2414	0.54 (1)
M-A	-1811 / 0	0.0	0.0 0.18 (1)	6.13	I-F	0 / 2414	0.54 (1)
H-F	-1973 / 0	0.0	0.0 0.20 (1)	5.92			
M-L	0 / 0	-18.2	-18.2 0.20 (4)	10.00			
L-K	0 / 2395	-18.2	-18.2 0.47 (1)	10.00			
K-J	0 / 2395	-18.2	-18.2 0.47 (1)	10.00			
J-I	0 / 2395	-18.2	-18.2 0.47 (1)	10.00			
I-H	0 / 0	-18.2	-18.2 0.20 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")CSI: TC=0.91/0.97 (A-B:1) , BC=0.47/0.97 (K-L:1) ,
WB=0.81/0.97 (B-K:1) , SSI=0.35/1.00 (A-B:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL = 0.74 (J) (INPUT = 1.00)

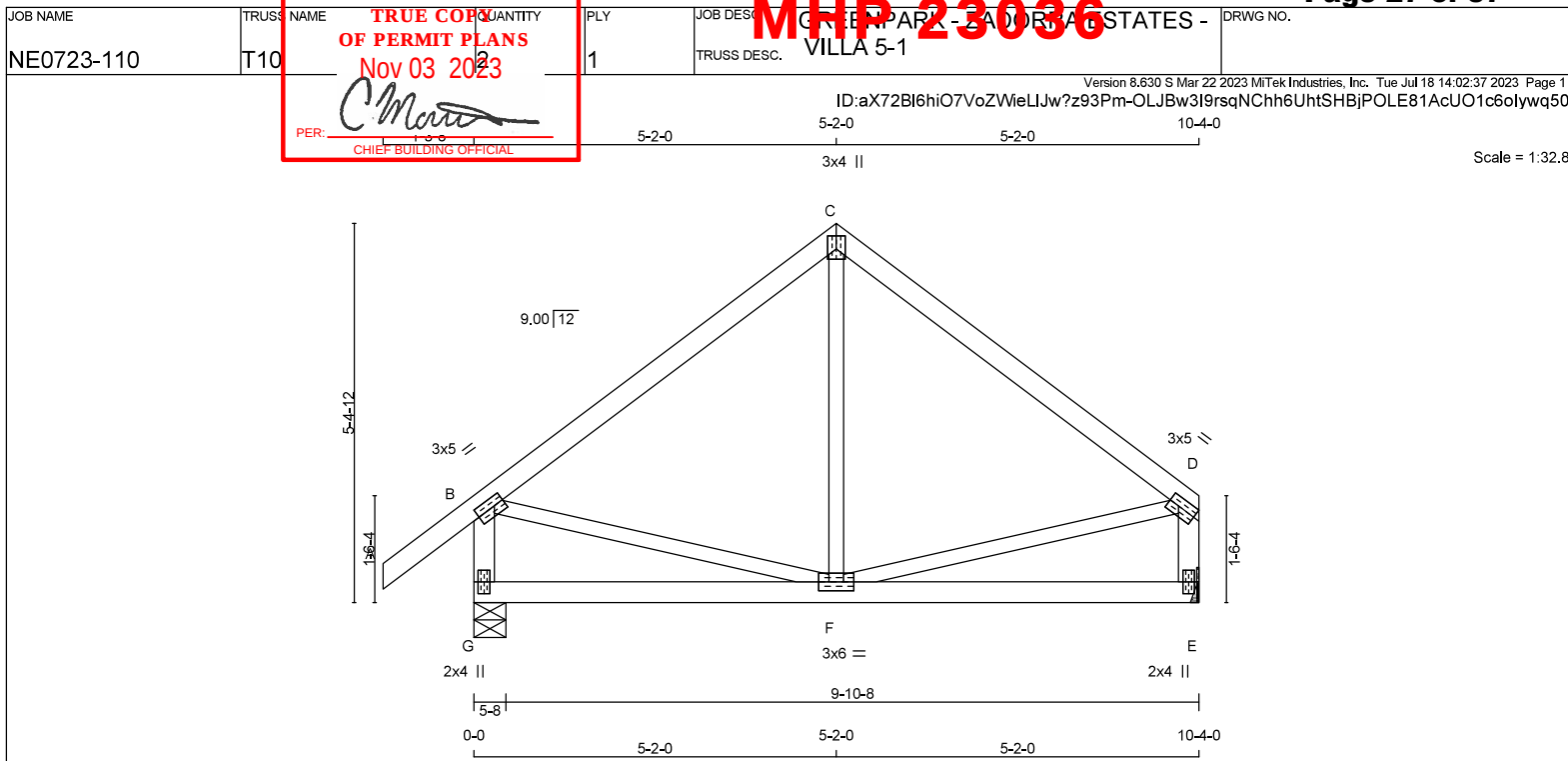
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TOTAL WEIGHT = 2 X 43 = 86 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWH	MT20	3.0	5.0	1.50	1.75
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVWH	MT20	3.0	5.0	1.50	Edge
E	BMV1+p	MT20	2.0	4.0		
F	BMVWH	MT20	3.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	876	0	876	0	0	5-8	1-8
E	711	0	711	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	610	456 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0
E	497	360 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED HORIZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 49	-119.4	-119.4	0.16 (1)	10.00	F-C	-57 / 80	
B-C	-467 / 0	-119.4	-119.4	0.41 (1)	6.25	B-F	0 / 383	
C-D	-467 / 0	-119.4	-119.4	0.41 (1)	6.25	F-D	0 / 383	
G-B	-840 / 0	0.0	0.0	0.09 (1)	7.81			
E-D	-675 / 0	0.0	0.0	0.07 (1)	7.81			
G-F	0 / 0	-18.2	-18.2	0.14 (4)	10.00			
F-E	0 / 0	-18.2	-18.2	0.14 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")CSI: TC=0.41/0.97 (B-C:1), BC=0.14/0.97 (F-G:4),
WB=0.09/0.97 (B-F:1), SSI=0.19/1.00 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
		788	1987
		1873	

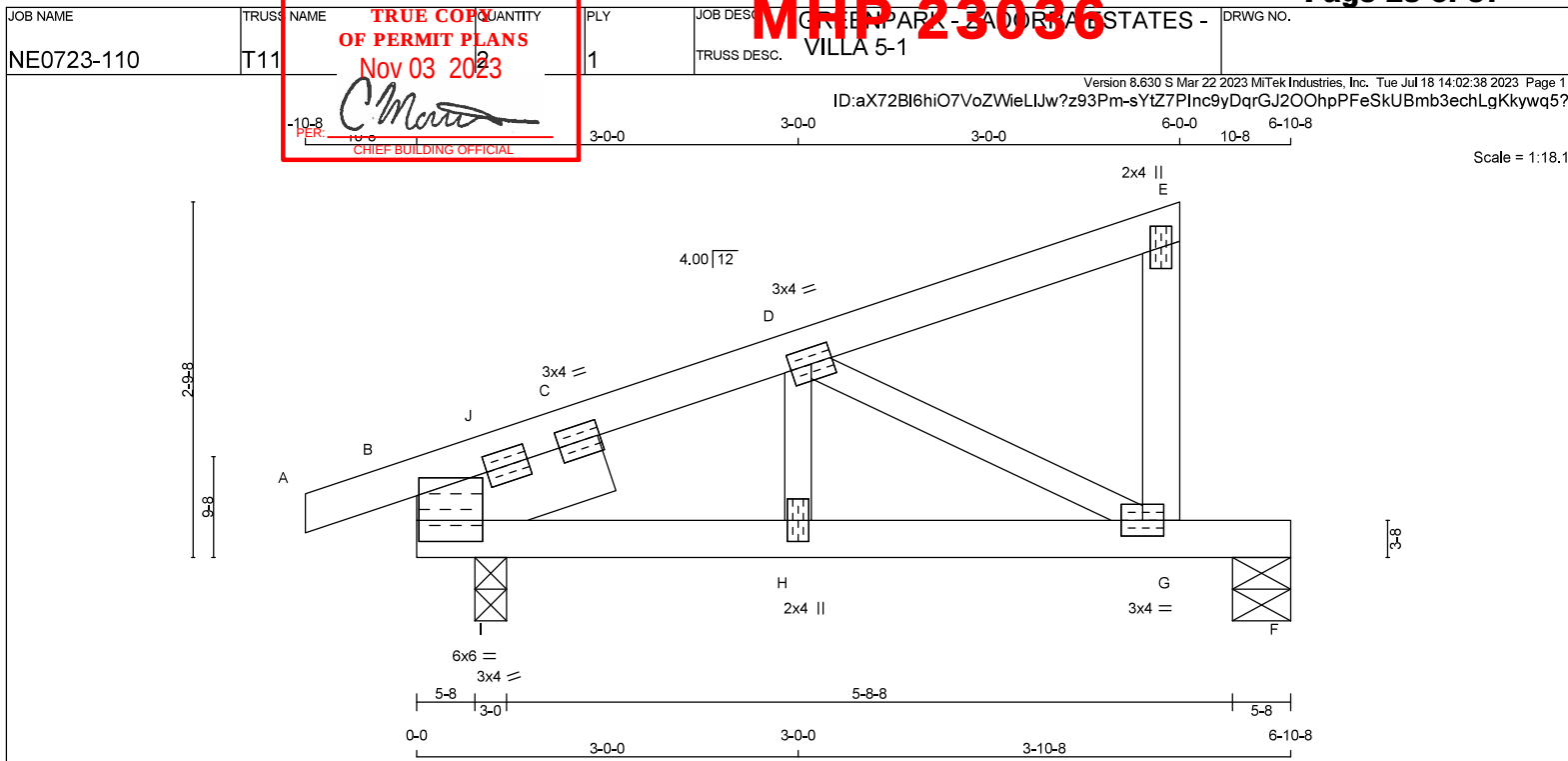
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL = 0.21 (B) (INPUT = 1.00)

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TOTAL WEIGHT = 2 X 25 = 50 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
G - E	2x4	DRY	No.2
B - F	2x4	DRY	No.2

REINFORCING MEMBERS
HW1 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	6.0	6.0	4.25	0.25
B	RT-I	MT20	3.0	4.0		
B	RT-I	MT20	3.0	4.0		
D	TMVW-H	MT20	3.0	4.0		
E	TMV+p	MT20	2.0	4.0		
G	BMVW-H	MT20	3.0	4.0		
H	BMV+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	REQD
B	577	0	577	0	0	3-0	1-8	
F	375	0	375	0	0	5-8	1-8	

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS
B	401	300 / 0	0 / 0
F	264	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX	MAX
A-B	-11 / 0	-119.4	-119.4	0.05 (1)	6.25	H-D	0 / 210	0.05 (1)	
B-J	-575 / 0	-119.4	-119.4	0.02 (1)	6.25	D-G	-678 / 0	0.14 (1)	
J-C	-493 / 0	-119.4	-119.4	0.09 (1)	6.25	I-J	0 / 350	0.00 (1)	
C-D	-493 / 0	-119.4	-119.4	0.09 (1)	6.25	I-C	-411 / 0	0.03 (1)	
D-E	-5 / 0	-119.4	-119.4	0.13 (1)	10.00				
G-E	-162 / 0	0.0	0.0	0.02 (1)	7.81				
B-I	0 / 384	-18.2	-18.2	0.07 (1)	10.00				
I-H	0 / 607	-18.2	-18.2	0.20 (1)	10.00				
H-G	0 / 607	-18.2	-18.2	0.53 (1)	10.00				
G-F	0 / 0	-18.2	-18.2	0.44 (1)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

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- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/886 (0.09")

CSI: TC=0.13/0.97 (D-E:1), BC=0.53/0.97 (G-H:1),
WB=0.14/0.97 (D-G:1), SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

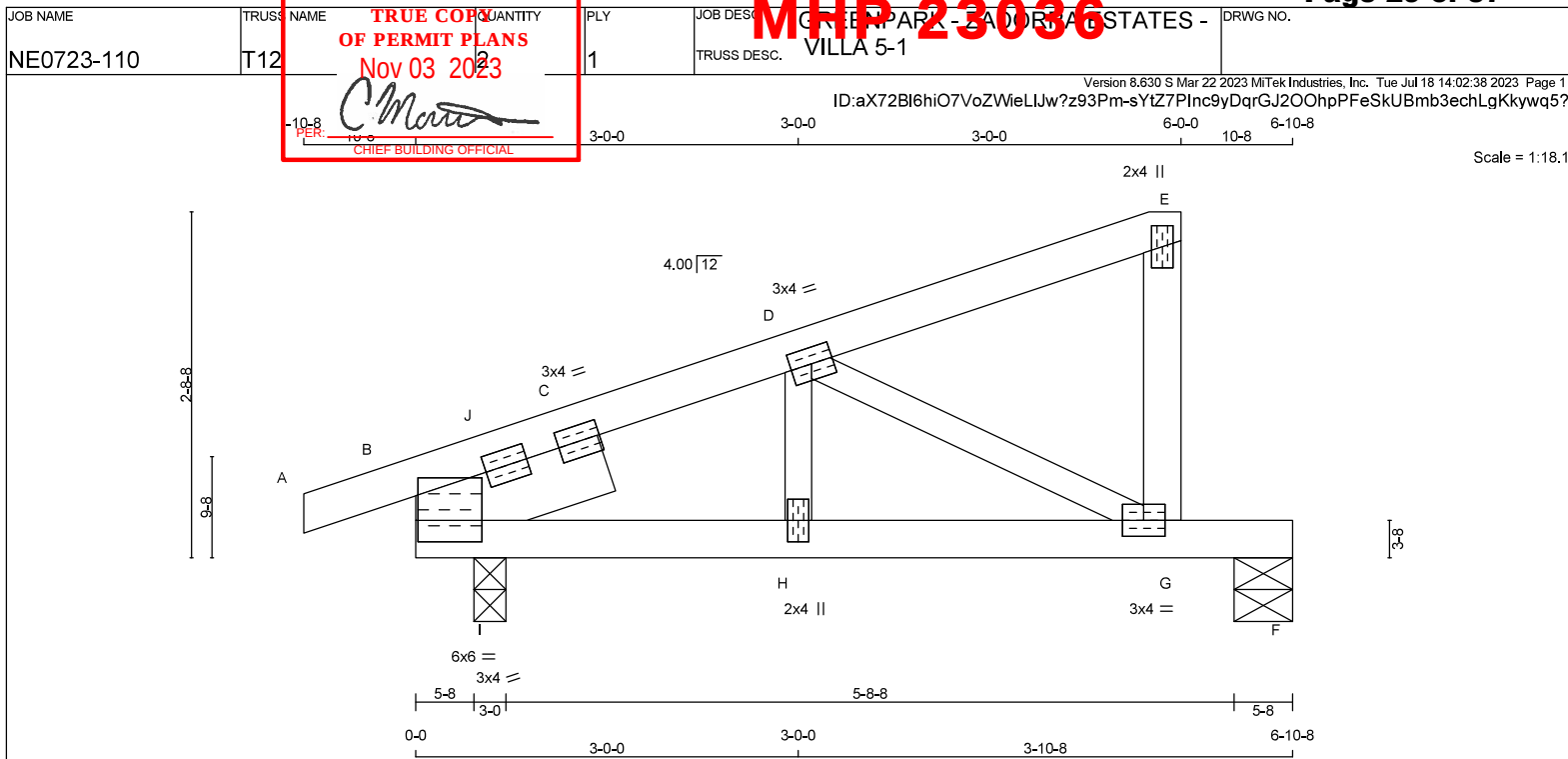
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL = 0.21 (G) (INPUT = 1.00)



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TOTAL WEIGHT = 2 X 25 = 50 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
G - E	2x4	DRY	No.2
B - F	2x4	DRY	No.2

REINFORCING MEMBERS
 HW1 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	6.0	6.0	4.25	0.25
B	RT-I	MT20	3.0	4.0		
B	RT-I	MT20	3.0	4.0		
D	TMVW-H	MT20	3.0	4.0		
E	TMV+p	MT20	2.0	4.0		
G	BMVW-H	MT20	3.0	4.0		
H	BMV+W	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	REQD
B	577	0	577	0	0	3-0	1-8	
F	375	0	375	0	0	5-8	1-8	

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS
B	401	300 / 0	0 / 0
F	264	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED FORCE (LBS)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						FR-TO			
A-B	-11 / 0	-119.4	-119.4	0.05 (1)	6.25	H-D	0 / 210	0.05 (1)	
B-J	-575 / 0	-119.4	-119.4	0.02 (1)	6.25	D-G	-678 / 0	0.14 (1)	
J-C	-493 / 0	-119.4	-119.4	0.09 (1)	6.25	I-J	0 / 350	0.00 (1)	
C-D	-493 / 0	-119.4	-119.4	0.09 (1)	6.25	I-C	-411 / 0	0.03 (1)	
D-E	-5 / 0	-119.4	-119.4	0.13 (1)	10.00				
G-E	-162 / 0	0.0	0.0	0.02 (1)	7.81				
B-I	0 / 384	-18.2	-18.2	0.07 (1)	10.00				
I-H	0 / 607	-18.2	-18.2	0.20 (1)	10.00				
H-G	0 / 607	-18.2	-18.2	0.53 (1)	10.00				
G-F	0 / 0	-18.2	-18.2	0.44 (1)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.23")
 CALCULATED VERT. DEFL.(TL) = L/886 (0.09")

CSI: TC=0.13/0.97 (D-E:1), BC=0.53/0.97 (G-H:1),
 WB=0.14/0.97 (D-G:1), SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

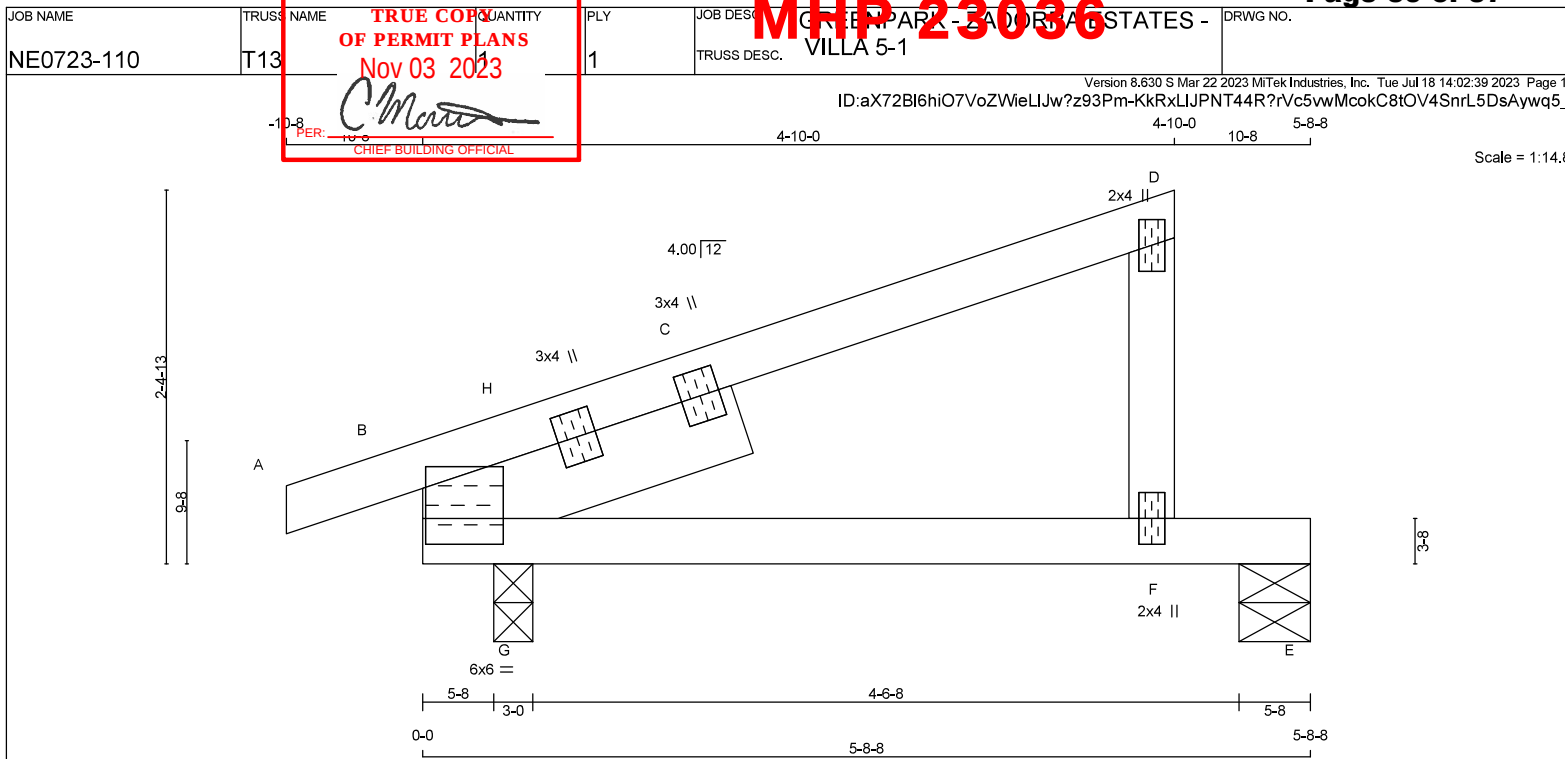
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
 JSI METAL = 0.21 (G) (INPUT = 1.00)



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TOTAL WEIGHT = 19 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
F - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2

REINFORCING MEMBERS
HW1 2x6 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	6.0	6.0	4.25	0.25
B	RT+H	MT20	3.0	4.0		
B	RT+H	MT20	3.0	4.0		
D	TMV+p	MT20	2.0	4.0		
F	BMV+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	495	0	495	0	0	3-0	1-8
E	296	0	296	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS
B	344	258 / 0	0 / 0
E	209	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						FR-TO			
A-B	-11 / 0	-119.4	-119.4	0.05 (1)	6.25	G-H	-339 / 0	0.00 (1)	
B-H	-286 / 0	-119.4	-119.4	0.07 (1)	6.25	G-C	0 / 342	0.04 (1)	
H-C	-147 / 0	-119.4	-119.4	0.45 (1)	6.25				
C-D	-147 / 0	-119.4	-119.4	0.45 (1)	6.25				
F-D	-275 / 0	0.0	0.0	0.03 (1)	7.81				
B-G	0 / 247	-18.2	-18.2	0.20 (1)	10.00				
G-F	0 / 0	-18.2	-18.2	0.34 (1)	10.00				
F-E	0 / 0	-18.2	-18.2	0.34 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/706$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/392$ (0.17")

CSI: TC=0.45/0.97 (D+H:1), BC=0.34/0.97 (F-G:1),
WB=0.04/0.97 (C-G:1), SSI=0.28/1.00 (D+H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL = 0.08 (D) (INPUT = 1.00)



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OF PERMIT PLANS
Nov 03 2023

MHP 23036

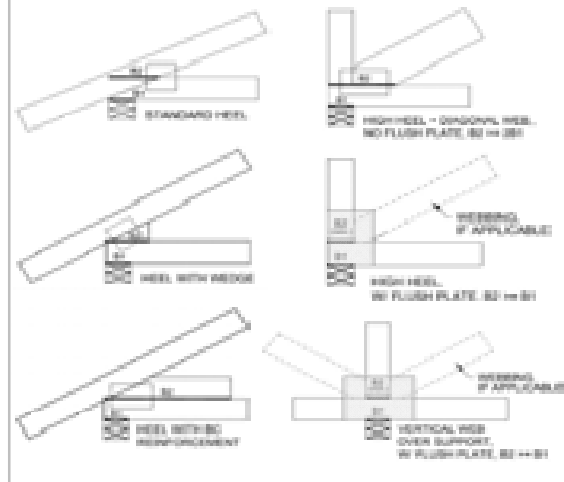
KOTT TRUSS BEARING CAPACITY TABLE

(INTERNAL USE ONLY)

TRUSS BEARING CAPACITIES (LBS.) PER INCH OF BEARING SURFACE (ED ON SPF #2 TOP PLATE) 7-Jun-21

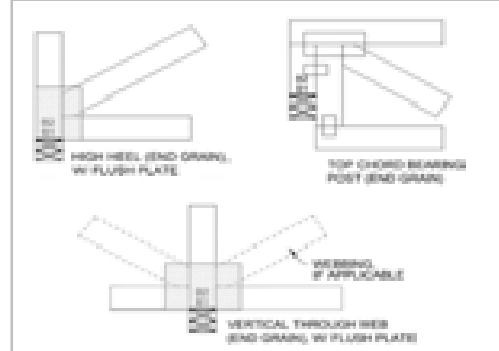
	BEARING PLATE (B1)	TYPE 1, NO FLUSH PLATE, B2=2B1							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
NO BEARING ENHANCER	1 1/2"	1383		2767		4151		5534	
	2x4	3712	3228	7425	6457	11138	9685	14851	12914
	2x6	5834	5073	11668	10146	17503	15220	23337	20293
	2x8	7690	6687	15381	13375	23072	20063	30763	26750
FLUSH PLATE	BEARING PLATE (B1)	TYPE 1, FLUSH PLATE							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
	1 1/2"	1383		2767		4151		5534	
	2x4	3712		7425		11138		14851	
BEARING ENHANCER	2x6	5834		11668		17503		23337	
	2x8	7690		15381		23072		30763	
	BEARING PLATE (B1)	TYPE 1, FLUSH PLATE + BEARING ENHANCER							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
CPn-4 (Simpson)	2x4		4515		9030		13545		18065
CPn-6 (Simpson)	2x6		7095		14190		21285		28390
CP4-9 (KOTT)	2x4		6007		4898		12014		9796
CP6-9 (KOTT)	2x6		8677		7075		17354		14150
SBP4 (MiTek)	2x4		7288		11001		14714		18427
SBP6 (MiTek)	2x6		11030		16865		22699		28534
SBP6 (MiTek)	2x8		12886		20578		28269		35960

TYPE 1 CONFIGURATIONS



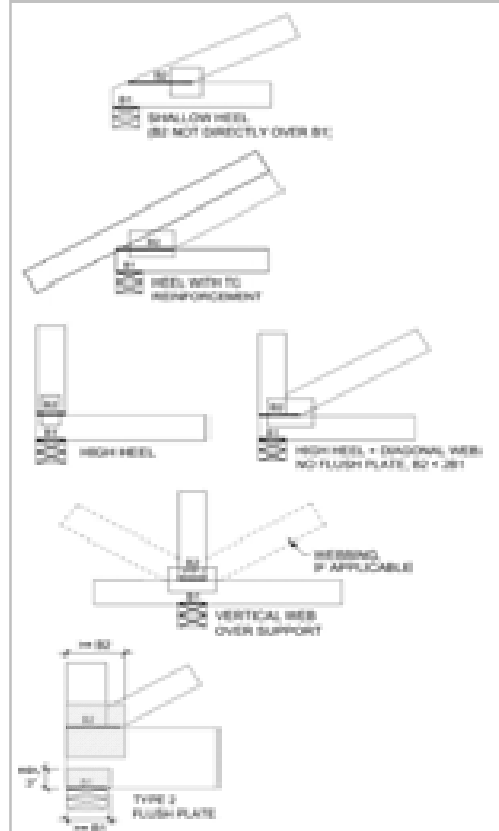
	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 1 (END GRAIN)							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)
END GRAIN	2x4	2x4		3712		7425		11138		14851
	2x6	2x6		5834		11668		17503		23337
			MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)
	2x8	2x8		7690		15381		23072		30763
END GRAIN, BEARING ENHANCER	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 1 (END GRAIN), FLUSH PLATE + BEARING ENHANCER							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)
	CPn-4 (Simpson)	2x4		4515		9030		13545		18065
	CPn-6 (Simpson)	2x6		7095		14190		21285		28390
CP4-9 (KOTT)	2x4	2x6		15585		9006		18013		19801
			MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)
CP6-9 (KOTT)	2x6	2x8		21834		13009		31117		31117

TYPE 1 (END GRAIN) CONFIGURATIONS



	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, NO FLUSH PLATE, B2<2B1							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
NO BEARING ENHANCER	2x4	2x4	2639	2152	5279	4304	7919	6457	10588	8609
		2x6	3393	2767	6787	5534	10181	8302	13575	11069
	2x6	2x4	3393	2767	6787	5534	10181	8302	13575	11069
		2x6	4147	3382	8296	6764	12444	10146	16592	13529
		2x8	4808	3920	9616	7840	14424	11761	19232	15681
		2x10	5562	4535	11124	9070	16686	13606	22248	18141
	2x8	2x4	3959	3228	7919	6457	11878	9685	15838	12914
		2x6	4808	3920	9616	7840	14424	11761	19232	15681
		2x8	5467	4458	10935	8916	16403	13375	21871	17833
FLUSH PLATE	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, FLUSH PLATE							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
	2x4	2x4								
		2x6	TYPE 1 APPLIES							
	2x6	2x4	4672	3809	9344	7619	14016	11429	18689	15238
		2x6								
		2x8	TYPE 1 APPLIES							
		2x10								
	2x8	2x4	4672	3809	9344	7619	14016	11429	18689	15238
		2x6	7342	5986	14684	11973	22026	17959	29368	23946
		2x8	TYPE 1 APPLIES							

TYPE 2 CONFIGURATIONS



NOTES:

- Factored truss reaction shall not exceed bearing capacity corresponding to: configuration type, size of bearing surfaces, truss lumber, # of plies, and applicable enhancers.
- Values in table are in conformance with CSA O86-14 Cl. 6.5.7 and TPIC 2014-Update 2, and may be used for residential or commercial designs.
- Values in table are in conformance with MiTek Canada Detail B37821Q "SPF Bearing Capacities".
- Values in table are in conformance with Simpson Catalogue C-C-CAN2020.
- Conditions for use of table values include: standard duration (K_d=1), dry lumber (K_{sp}=1), untreated lumber (K_t=1), length of bearing factor not applied (K_b=1).
- Size factor (K_{sz}) applied to support material calculation when acceptable. Flush plate factor (K_p) applied to truss material calculation when acceptable (ie. excludes end grain).
- Flat roof factor (K_r) must applied for trusses making up a flat roof system; to do so, multiply bearing capacity values by 0.75 for this application.
- Bearing plate is to be specified by the project engineer; values in table assume a bearing material of SPF #2 (or better).
- When required, flush plate must not be located further than 1/4" away from bearing surface, and must cover the entire bearing plate length (B1).
- When required, bearing enhancer must be installed as per manufacturer's guidelines.
- Type 2 bearing configurations can be converted to use Type 1 table values as outlined in TPIC 2014-Update 1 Cl. 7.5.9.
- This table is not valid after April 30, 2022.

TOE-NAIL CAPACITY DETAILS**LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS**

NAIL TYPE	Length (in)	Diameter (in)	LATERAL Resistance per nail (Lbs.)		WITHDRAWAL Resistance per nail (Lbs.)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

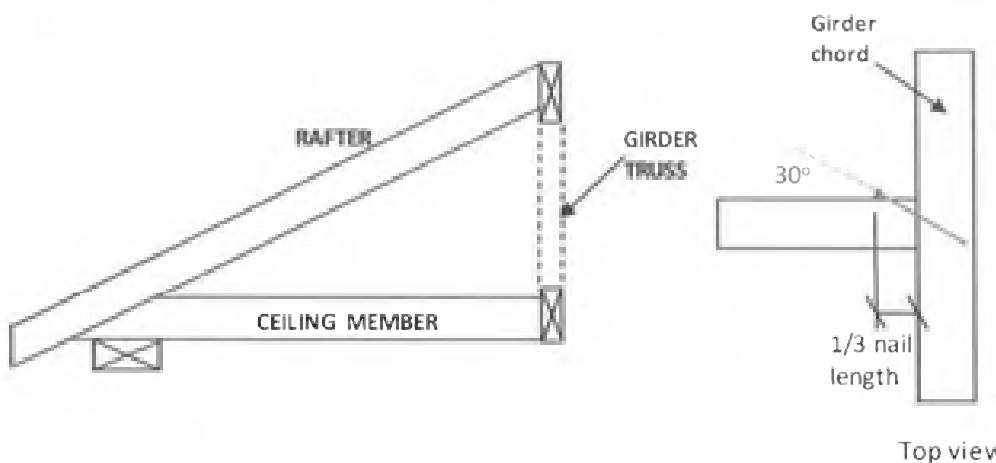
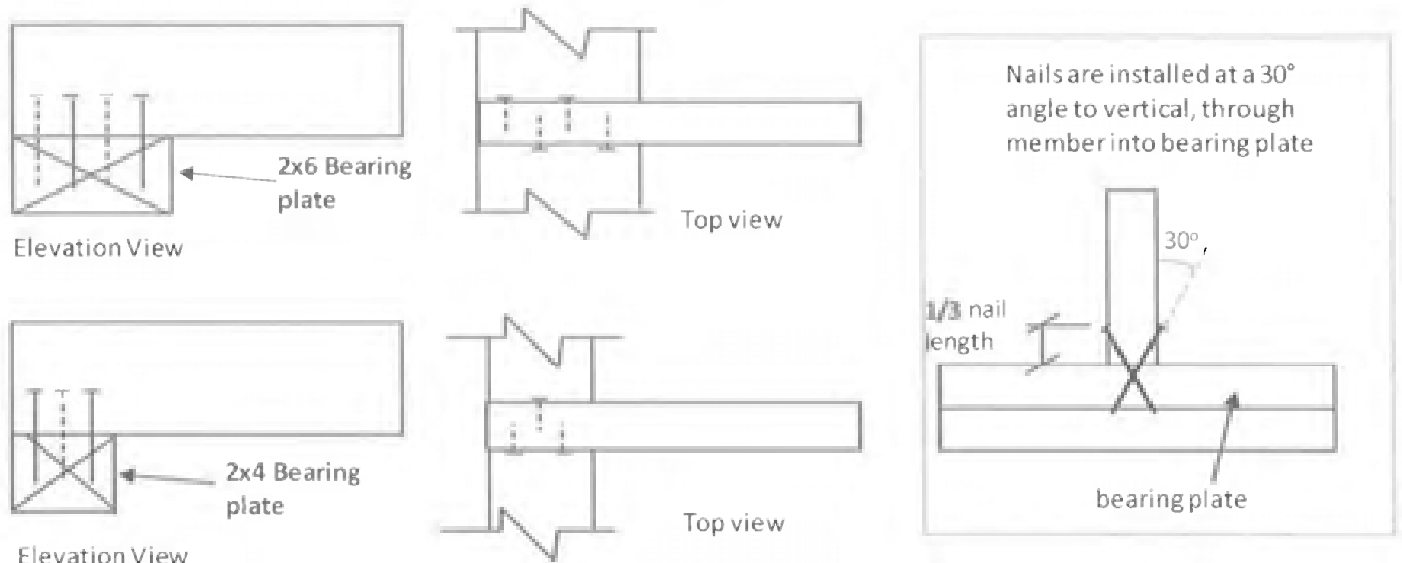


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

TOE-NAIL CAPACITY DETAILS**Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift****NOTES:**

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

Nov 03 2023

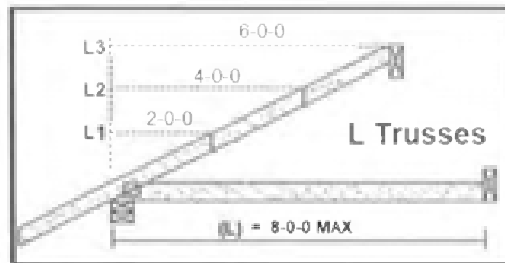
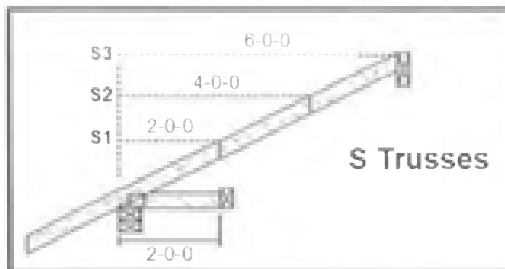
PER: *C. Cordogianis*
CHIEF BUILDING OFFICIAL**MHP 23036**STANDARD DETAIL **MSD2015-J**

Issued: MARCH 17, 2021

Expiry: APRIL 30, 2023

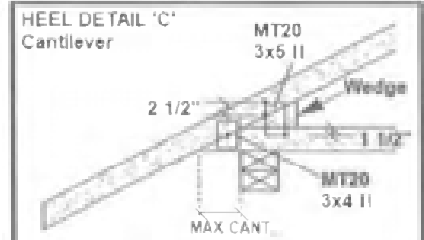
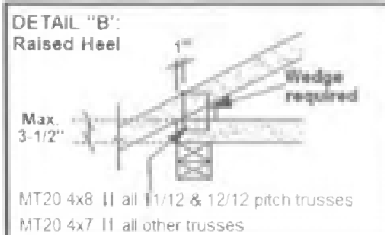
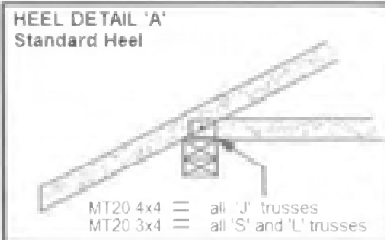
STANDARD HIP END FRAMING

PLAN VIEW



Specified Load Rating:

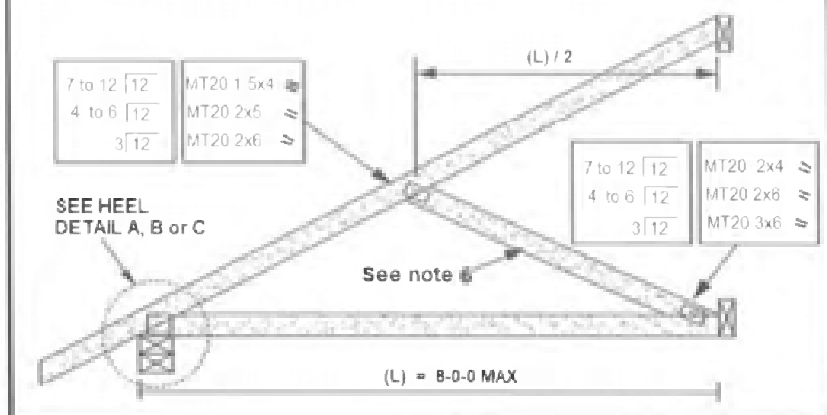
Top chord Live:	51.0 PSF or less
Top chord Dead:	6.0 PSF or less
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF or less



CANTILEVER DETAIL 'C'

SLOPE	MAX CANT.	WEDGE PLATE	WEDGE SIZE
3/12	17"	3 x 5	2 x 3
4/12	14"	3 x 5	2 x 3
5/12	12"	3 x 5	2 x 4
6/12	10"	3 x 5	2 x 4
7/12	9"	3 x 5	2 x 5
8/12	8 1/2"	3 x 5	2 x 6
9/12	8"	3 x 5	2 x 6
10/12	7 1/2"	3 x 5	2 x 6

J Trusses



PEO

Certificate No. 10889485

NOTES:

1. This detail is valid only for projects conforming to **PART 9 NBCC 2015** that do not require a wind analysis to be incorporated into the design of the trusses.
2. Overhang length shall not exceed 24 inches.
3. All lumber shall be 2x4 SPF (or D-Fir) DRY No. 2 grade or better.
4. All plates specified are MITEK MT20, pressed into both faces of each truss. Heel plates of all trusses shall conform to heel details 'A', 'B' or 'C'.
5. Diagonal hip rafter design shall conform to section 9.23.14.6 of NBCC 2015.
6. For 6.0 ft. or less span, diagonal web on truss 'J' is optional. Girder design must reflect choice of partial jack ('J' with diagonal web) or open jack ('J' without diagonal web)
7. All truss-to-rafter and truss-to-truss connections shall be specified as per MITEK standard detail 'MSD2015-H: Toe-Nail Capacity Details'



STANDARD DETAIL MSD2015-K

Issued: MARCH 1, 2022

Expiry: APRIL 30, 2024

STANDARD GABLE END DETAIL

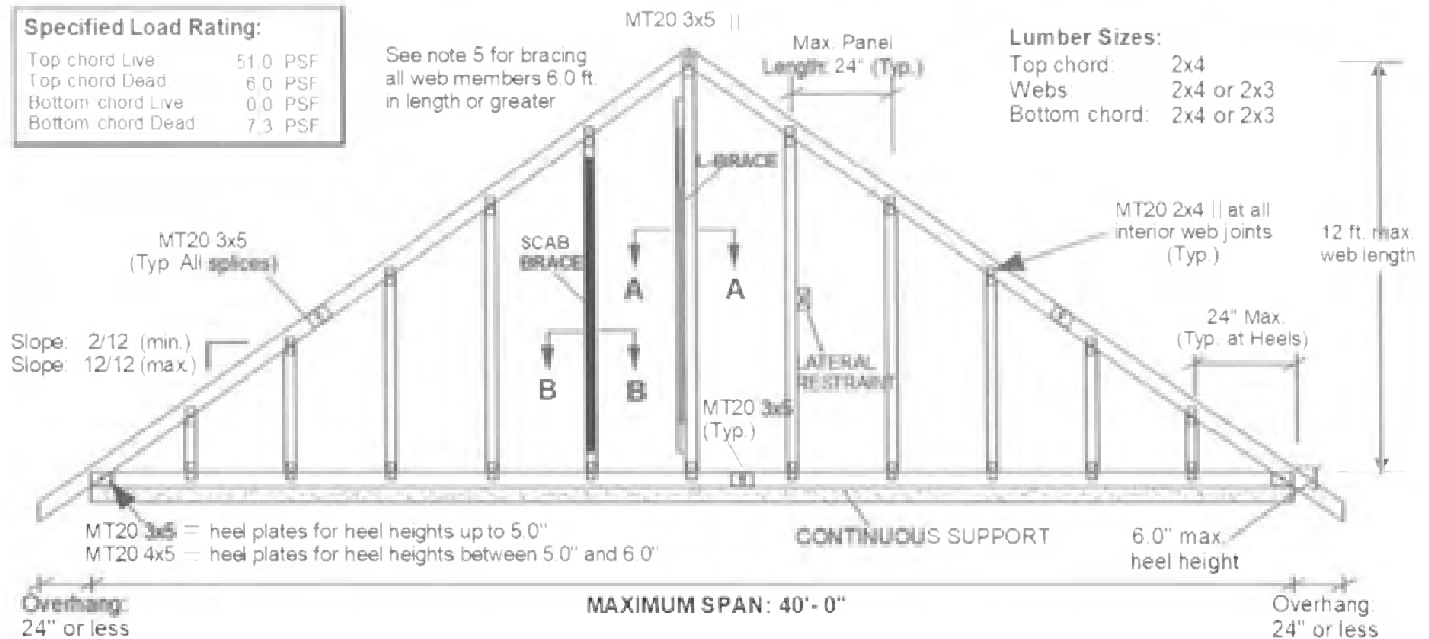
Specified Load Rating:

Top chord Live	51.0 PSF
Top chord Dead	6.0 PSF
Bottom chord Live	0.0 PSF
Bottom chord Dead	7.3 PSF

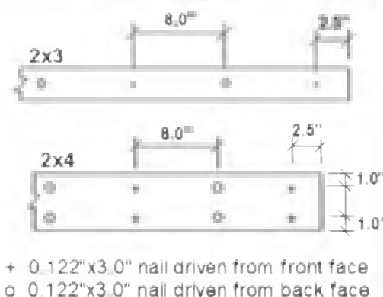
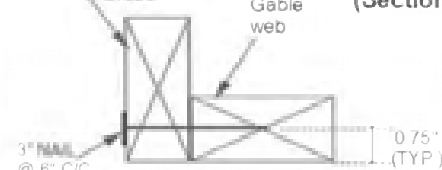
See note 5 for bracing
all web members 6.0 ft.
in length or greater

Lumber Sizes:

Top chord:	2x4
Webs:	2x4 or 2x3
Bottom chord:	2x4 or 2x3

SCAB BRACE DETAIL
(Section B-B)

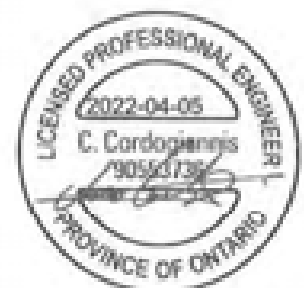

SPF No. 2 DRY Scab, same size as web. Scab brace must cover 90% of web length

2x4 SPF No. 2
DRY L-BraceL BRACE DETAIL
(Section A-A)

Fasten L-Brace to narrow edge of web with one row of 0.122" x 3.0" nails spaced at 6.0" c/c along entire length of web. Brace must cover 90% of the web length. Respect a 2.5" minimum end distance.

Notes:

1. This detail is only valid for projects conforming to **Part 9, NBCC 2015** that do not require a wind analysis to be incorporated into the design of the truss.
2. This detail is for vertical (gravity) load rating of the truss only. Truss must be continuously supported over the entire length of bottom chord.
3. Maximum web length not to exceed 12.0 ft. Spacing of gable stud webs in the truss not to exceed 24 inches cc.
4. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint.
5. Lateral restraint required at half-length of all webs over 6.0 ft. long. Alternatively install an L-Brace or scab brace as shown above. Scab braces shall be limited to 10 ft. long webs or less.
6. All plates are MITEK MT20 pressed into both faces of truss.
7. All lumber to be SPF (or D-Fir) DRY and of No.2 grade or better.
8. Additional building bracing is typically installed to brace the face of the end wall assembly. See BCSI Canada 'Building Designer Responsibilities for Gable End Frame Bracing' for additional information on building bracing for gable-end assemblies.

PEO
Certificate No. 10869485

November 2, Monday: Ten full and half-filled closed brown glass bottles of water placed on the ground in front of the house.

Installing — Installation

Two shims for out-of-plumb

Pump line through exterior wall

1/2" (12.7mm)	3/4" (19.0mm)	Max. Size	Trace Length
1/2"	3/4"	1/2"	15.0'
3/4"	1"	3/4"	17.5'
1"	1 1/4"	1"	20.0'
1 1/4"	1 3/4"	1 1/4"	22.5'
1 3/4"	2"	1 3/4"	25.0'
2"	2 1/4"	2"	27.5'
2 1/4"	2 3/4"	2 1/4"	30.0'
2 3/4"	3"	2 3/4"	32.5'
3"	3 1/4"	3"	35.0'
3 1/4"	3 3/4"	3 1/4"	37.5'
3 3/4"	4"	3 3/4"	40.0'
4"	4 1/4"	4"	42.5'
4 1/4"	4 3/4"	4 1/4"	45.0'
4 3/4"	5"	4 3/4"	47.5'
5"	5 1/4"	5"	50.0'
5 1/4"	5 3/4"	5 1/4"	52.5'
5 3/4"	6"	5 3/4"	55.0'
6"	6 1/4"	6"	57.5'
6 1/4"	6 3/4"	6 1/4"	60.0'
6 3/4"	7"	6 3/4"	62.5'
7"	7 1/4"	7"	65.0'
7 1/4"	7 3/4"	7 1/4"	67.5'
7 3/4"	8"	7 3/4"	70.0'
8"	8 1/4"	8"	72.5'
8 1/4"	8 3/4"	8 1/4"	75.0'
8 3/4"	9"	8 3/4"	77.5'
9"	9 1/4"	9"	80.0'
9 1/4"	9 3/4"	9 1/4"	82.5'
9 3/4"	10"	9 3/4"	85.0'
10"	10 1/4"	10"	87.5'
10 1/4"	10 3/4"	10 1/4"	90.0'
10 3/4"	11"	10 3/4"	92.5'
11"	11 1/4"	11"	95.0'
11 1/4"	11 3/4"	11 1/4"	97.5'
11 3/4"	12"	11 3/4"	100.0'
12"	12 1/4"	12"	102.5'
12 1/4"	12 3/4"	12 1/4"	105.0'
12 3/4"	13"	12 3/4"	107.5'
13"	13 1/4"	13"	110.0'
13 1/4"	13 3/4"	13 1/4"	112.5'
13 3/4"	14"	13 3/4"	115.0'
14"	14 1/4"	14"	117.5'
14 1/4"	14 3/4"	14 1/4"	120.0'
14 3/4"	15"	14 3/4"	122.5'
15"	15 1/4"	15"	125.0'
15 1/4"	15 3/4"	15 1/4"	127.5'
15 3/4"	16"	15 3/4"	130.0'
16"	16 1/4"	16"	132.5'
16 1/4"	16 3/4"	16 1/4"	135.0'
16 3/4"	17"	16 3/4"	137.5'
17"	17 1/4"	17"	140.0'
17 1/4"	17 3/4"	17 1/4"	142.5'
17 3/4"	18"	17 3/4"	145.0'
18"	18 1/4"	18"	147.5'
18 1/4"	18 3/4"	18 1/4"	150.0'
18 3/4"	19"	18 3/4"	152.5'
19"	19 1/4"	19"	155.0'
19 1/4"	19 3/4"	19 1/4"	157.5'
19 3/4"	20"	19 3/4"	160.0'
20"	20 1/4"	20"	162.5'
20 1/4"	20 3/4"	20 1/4"	165.0'
20 3/4"	21"	20 3/4"	167.5'
21"	21 1/4"	21"	170.0'
21 1/4"	21 3/4"	21 1/4"	172.5'
21 3/4"	22"	21 3/4"	175.0'
22"	22 1/4"	22"	177.5'
22 1/4"	22 3/4"	22 1/4"	180.0'
22 3/4"	23"	22 3/4"	182.5'
23"	23 1/4"	23"	185.0'
23 1/4"	23 3/4"	23 1/4"	187.5'
23 3/4"	24"	23 3/4"	190.0'
24"	24 1/4"	24"	192.5'
24 1/4"	24 3/4"	24 1/4"	195.0'
24 3/4"	25"	24 3/4"	197.5'
25"	25 1/4"	25"	200.0'
25 1/4"	25 3/4"	25 1/4"	202.5'
25 3/4"	26"	25 3/4"	205.0'
26"	26 1/4"	26"	207.5'
26 1/4"	26 3/4"	26 1/4"	210.0'
26 3/4"	27"	26 3/4"	212.5'
27"	27 1/		

[illegible][illegible][illegible]

AVERTISSEMENT! GUIDE POUR LA MANIPULATION, L'INSTALLATION, LA RETENUE ET LE CONTRÔLEMENT DES FERMES Les parties supérieures à 10 ou 100 psi peuvent causer un contreventement permanent complexe. Veuillez lire le manuel professionnel enregistré.

1. Permanent Restraint/Bracing
Permanent restraint/bracing is a major cause of truss roof performance problems and has been known to fail on roof or floor system collapse.
The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.
The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.

Minimum Truss Size	Minimum Truss Size	Minimum Truss Size
1. Truss span greater than 60' (18.3m)	2. Truss span greater than 60' (18.3m)	3. Truss span greater than 60' (18.3m)
4. Truss span greater than 60' (18.3m)	5. Truss span greater than 60' (18.3m)	6. Truss span greater than 60' (18.3m)

2. Permanent Bracing for the Top Chord Plane
Permanent bracing is important because it prevents out-of-plane buckling of truss members.
It helps maintain proper truss spacing and resists and transfers lateral loads from wind and seismic forces.
The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.

3. Permanent Bracing for the Web Member Plane
Permanent bracing is important because it prevents out-of-plane buckling of truss members.
It helps maintain proper truss spacing and resists and transfers lateral loads from wind and seismic forces.
The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.

4. Continuous Lateral Restraint (CLR) & Diagonal Bracing
Continuous lateral restraint (CLR) is a method of providing lateral support to the truss system.
Diagonal bracing is a method of providing lateral support to the truss system.

5. Permanent Bracing for the Web Member Plane
Permanent bracing is important because it prevents out-of-plane buckling of truss members.
It helps maintain proper truss spacing and resists and transfers lateral loads from wind and seismic forces.
The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.

6. Continuous Lateral Restraint (CLR) & Diagonal Bracing
Continuous lateral restraint (CLR) is a method of providing lateral support to the truss system.
Diagonal bracing is a method of providing lateral support to the truss system.

7. Permanent Bracing for the Web Member Plane
Permanent bracing is important because it prevents out-of-plane buckling of truss members.
It helps maintain proper truss spacing and resists and transfers lateral loads from wind and seismic forces.
The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.

8. Continuous Lateral Restraint (CLR) & Diagonal Bracing
Continuous lateral restraint (CLR) is a method of providing lateral support to the truss system.
Diagonal bracing is a method of providing lateral support to the truss system.

9. Permanent Bracing for the Web Member Plane
Permanent bracing is important because it prevents out-of-plane buckling of truss members.
It helps maintain proper truss spacing and resists and transfers lateral loads from wind and seismic forces.
The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.

10. Continuous Lateral Restraint (CLR) & Diagonal Bracing
Continuous lateral restraint (CLR) is a method of providing lateral support to the truss system.
Diagonal bracing is a method of providing lateral support to the truss system.

11. Permanent Bracing for the Web Member Plane
Permanent bracing is important because it prevents out-of-plane buckling of truss members.
It helps maintain proper truss spacing and resists and transfers lateral loads from wind and seismic forces.
The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.

12. Continuous Lateral Restraint (CLR) & Diagonal Bracing
Continuous lateral restraint (CLR) is a method of providing lateral support to the truss system.
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13. Permanent Bracing for the Web Member Plane
Permanent bracing is important because it prevents out-of-plane buckling of truss members.
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The building code and the building designer are responsible for ensuring that the truss system is designed to resist the full range of loads and moments that may be applied to the truss system.

14. Continuous Lateral Restraint (CLR) & Diagonal Bracing
Continuous lateral restraint (CLR) is a method of providing lateral support to the truss system.
Diagonal bracing is a method of providing lateral support to the truss system.

15. Permanent Bracing for the Web Member Plane
Permanent bracing is important because it prevents out-of-plane buckling of truss members.
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