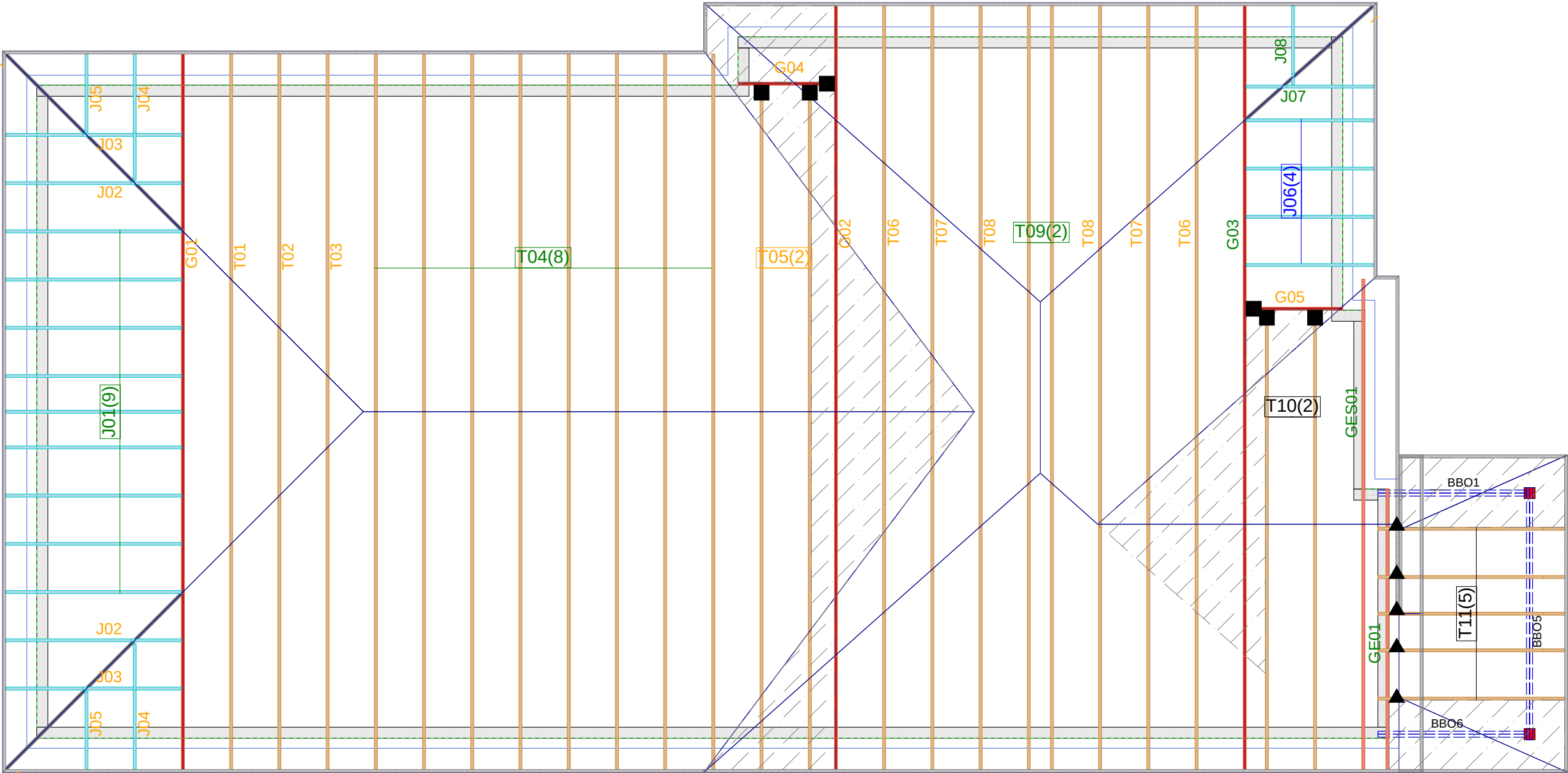




Hanger Name	Symbol	QTY
LUS24	▲	5
LJS26DS	■	6



CONVENTIONAL
FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE Laterally BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK HOMES
Job #	23-00111R0
Address	ZADORRA ETATES OSHAWA,ON
Model	VILLA 5-ELEV 2
Sales Rep	RALPH MIRIGELLO
Designer	RB
Date	6/06/23
Path	C:\MITEK\CA\JOBS\GREENPARK\ZADORRA ETATES\VILLA 5\VILLA 5-ELEV 2\

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft²
TC DL	6.0 lb/ft²
BC LL	0.0 lb/ft²
BC DL	7.3 lb/ft²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400



NE0723-111

GREENPARK - ZANORRA

ESTATES - VILLA 5-2

PER: *C. Morris*
CHIEF BUILDING OFFICIAL

PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILITIES

THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON CALCULATION PAGE. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. IT IS THE RESPONSIBILITY OF KOTT Inc. TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DESIGN INFORMATION

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN.

1. THE BUILDING USE AND OCCUPANCY TYPE IS AS INDICATED ON THE DRAWING.
2. GEOMETRY OF THE TRUSS AND DIMENSIONS INDICATED ON THE DRAWING ARE IDENTICAL TO THOSE OF THE INSTALLED TRUSS.
3. THE TRUSS LOADING INTENSITY AND DISTRIBUTION AS WELL AS LOAD TRANSFER MECHANISM IS THAT INDICATED ON THE DRAWING. NO BUILDINGS, TREES, PARAPETS OR OTHER PROJECTIONS HIGHER THAN THE ROOF FOR WHICH THE TRUSSES ARE USED ARE LOCATED WITHIN A DISTANCE LESS THAN TEN (10) TIMES THE DIFFERENCE IN HEIGHT, OR FIVE METERS (16 FT) WHICHEVER IS GREATER, UNLESS THE DRAWING INDICATES THAT THE SNOW DRIFTING HAS BEEN TAKEN INTO ACCOUNT.
4. THE TRUSSES ARE TO BE SUPPORTED AT THE BEARING POINTS INDICATED AND ANCHORED TO THE SUPPORTS WHERE CONSIDERED NECESSARY BY THE DESIGNER OF THE OVERALL STRUCTURE. BEARING SIZES SHOWN ARE THE MINIMUM REQUIRED TO PREVENT CRUSHING OF THE TRUSS MEMBERS AND DO NOT NECESSARILY TAKE INTO ACCOUNT STABILITY OF THE OVERALL BUILDING STRUCTURE. ELEVATION OF BEARINGS MUST BE CAREFULLY CHECKED AND SHIMMED TO ALIGNMENT FOR SOLID BEARINGS. ADEQUATE WOOD TRUSS BEARING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

CODE

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING, THE ONTARIO BUILDING CODE, TPIC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

HANDLING, INSTALLATION AND BRACING

1. THE TRUSSES MUST BE HANDLED AND INSTALLED BY A QUALIFIED PROFESSIONAL AS PER THE SUPPLIED DOCUMENT TITLED INFORMATION FOR TRUSS INSTALLERS AND THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS.
 2. THE COMPRESSION CHORDS ARE Laterally Braced by Continuous Rigid Diaphragm Sheathing or as Specified on the Drawing.
 3. TEMPORARY AND PERMANENT BRACING MUST BE INSTALLED AS INDICATED ON THE TRUSS DRAWING AND ACCORDING TO THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS. BRACING FOR THE LATERAL STABILITY OF THE TRUSS IS TO BE PROVIDED BY THE DESIGNER.
- IT IS RECOMMENDED THAT A PROFESSIONAL ENGINEER'S ADVICE BE OBTAINED FOR THE BRACING OF TRUSSES WITH A SPAN IN EXCESS OF 12.37M (40'-7").



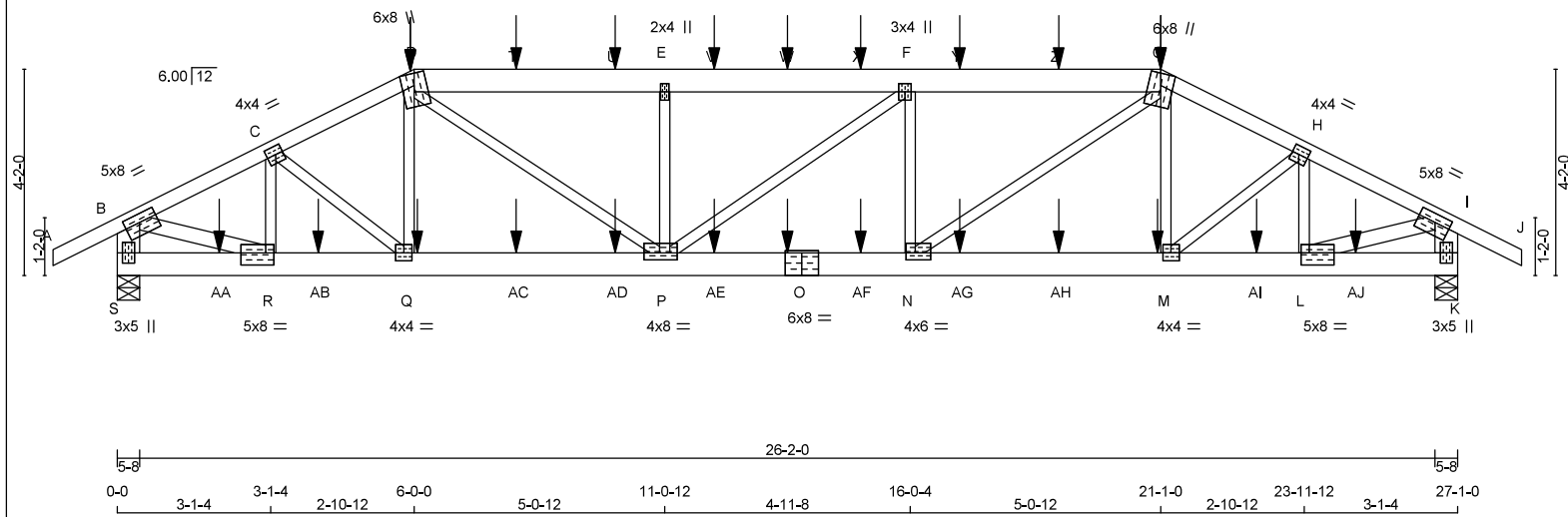
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME NE0723-111	TRUSS NAME G01	QUANTITY 1	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - VILLA 5-2	DRWG NO.
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CORPORATION OF THE CITY OF OSHAWA
TRUE COPY OF PERMIT PLANS
Nov 03 2023
CHIEF BUILDING OFFICIAL

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 18 14:35:41 2023 Page 1
ID: aX72BI6hiO7VoZWMeLJw?z93Pm-I2BDJTIfcRosEbMQjMTihauQv5BIuJ0WciC6Qlywpc0

Scale = 1:46.5



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x6	DRY	No.2
G - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - O	2x6	DRY	No.2
O - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
B - R	2x4	DRY	No.2
L - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	SIZE	W	LEN	Y X
B	TMWWH	MT20	5.0	8.0	2.00 3.75
C	TMWWH	MT20	4.0	4.0	2.00 1.75
D	TTWWH+m	MT20	6.0	8.0	Edge
E	TMWWH	MT20	2.0	4.0	
F	TMWWH+t	MT20	3.0	4.0	
G	TTWWH+m	MT20	6.0	8.0	Edge
H	TMWWH	MT20	4.0	4.0	2.00 1.75
I	TMWWH	MT20	5.0	8.0	2.00 3.75
K	BMV1+p	MT20	3.0	5.0	
L	BMWWH	MT20	5.0	8.0	3.00 2.00
M	BMWWH	MT20	4.0	4.0	
N	BMWWH	MT20	4.0	6.0	1.75 2.25
O	BS4	MT20	6.0	8.0	
P	BMWWH	MT20	4.0	8.0	1.75 3.75
Q	BMWWH	MT20	4.0	4.0	
R	BMWWH	MT20	5.0	8.0	3.00 2.00
S	BMV1+p	MT20	3.0	5.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQ'D	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	3335	0	3335	0	0	5-8	5-5
K	3335	0	3335	0	0	5-8	5-5

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	2329	1694 / 0	0 / 0	0 / 0	0 / 0	635 / 0	0 / 0
K	2329	1694 / 0	0 / 0	0 / 0	0 / 0	636 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 36	-119.4	-119.4 0.17 (1)	R-C	-1021 / 0	0.18 (1)	
B-C	-4353 / 0	-119.4	-119.4 0.43 (1)	C-Q	0 / 638	0.16 (1)	
C-D	-4895 / 0	-119.4	-119.4 0.48 (1)	Q-D	-171 / 75	0.04 (1)	
D-T	-6108 / 0	-119.4	-119.4 0.59 (1)	D-P	0 / 2123	0.53 (1)	
T-U	-6108 / 0	-119.4	-119.4 0.59 (1)	P-E	-1116 / 0	0.27 (1)	
U-E	-6108 / 0	-119.4	-119.4 0.59 (1)	E-F	-19 / 0	0.01 (1)	
E-V	-6107 / 0	-119.4	-119.4 0.58 (1)	N-F	-1118 / 0	0.27 (1)	
V-W	-6107 / 0	-119.4	-119.4 0.58 (1)	N-G	0 / 2143	0.53 (1)	
W-X	-6107 / 0	-119.4	-119.4 0.58 (1)	M-G	-179 / 74	0.05 (1)	
X-F	-6107 / 0	-119.4	-119.4 0.58 (1)	M-H	0 / 634	0.16 (1)	
F-Y	-6123 / 0	-119.4	-119.4 0.58 (1)	L-H	-1017 / 0	0.18 (1)	
Y-Z	-6123 / 0	-119.4	-119.4 0.58 (1)	B-R	0 / 4024	0.71 (1)	
Z-G	-6123 / 0	-119.4	-119.4 0.58 (1)	L-I	0 / 4026	0.71 (1)	
G-H	-4903 / 0	-119.4	-119.4 0.46 (1)				
H-I	-4355 / 0	-119.4	-119.4 0.43 (1)				
I-J	0 / 36	-119.4	-119.4 0.17 (1)				
S-B	-3245 / 0	0.0	0.0 0.23 (1)				
K-I	-3247 / 0	0.0	0.0 0.23 (1)				

S-AA	0 / 0	-18.2	-18.2 0.12 (1)	10.00			
AA-R	0 / 0	-18.2	-18.2 0.12 (1)	10.00			
R-AB	0 / 3905	-18.2	-18.2 0.63 (1)	10.00			
AB-Q	0 / 3905	-18.2	-18.2 0.63 (1)	10.00			
Q-AC	0 / 4400	-18.2	-18.2 0.65 (1)	10.00			
AC-AD	0 / 4400	-18.2	-18.2 0.65 (1)	10.00			
AD-P	0 / 4400	-18.2	-18.2 0.65 (1)	10.00			
P-AE	0 / 6122	-18.2	-18.2 0.91 (1)	10.00			
AE-O	0 / 6122	-18.2	-18.2 0.91 (1)	10.00			
O-AF	0 / 6122	-18.2	-18.2 0.91 (1)	10.00			
AF-N	0 / 6122	-18.2	-18.2 0.91 (1)	10.00			
N-AG	0 / 4399	-18.2	-18.2 0.63 (1)	10.00			
AG-AH	0 / 4399	-18.2	-18.2 0.63 (1)	10.00			
AH-M	0 / 4399	-18.2	-18.2 0.63 (1)	10.00			
M-AI	0 / 3906	-18.2	-18.2 0.62 (1)	10.00			
AI-L	0 / 3906	-18.2	-18.2 0.62 (1)	10.00			
L-AJ	0 / 0	-18.2	-18.2 0.12 (1)	10.00			
AJ-K	0 / 0	-18.2	-18.2 0.12 (1)	10.00			

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-0-0	-418	-418	—	FRONT	VERT	TOTAL	—	C1
G	21-1-0	-418	-418	—	FRONT	VERT	TOTAL	—	C1
M	21-0-0	-22	-22	—	FRONT	VERT	TOTAL	—	C1
O	13-6-8	-22	-22	—	FRONT	VERT	TOTAL	—	C1
Q	6-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
T	8-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	C1
U	10-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	C1
V	12-0-12	-102	-102	—	FRONT	VERT	TOTAL	—	C1
W	13-6-8	-102	-102	—	FRONT	VERT	TOTAL	—	C1
X	15-0-4	-102	-102	—	FRONT	VERT	TOTAL	—	C1
Y	17-0-4	-102	-102	—	FRONT	VERT	TOTAL	—	C1
Z	19-0-4	-102	-102	—	FRONT	VERT	TOTAL	—	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/842 (0.39")

CSI: TC=0.59/0.97 (D-E-1), BC=0.91/0.97 (N-P-1),
WB=0.71/0.97 (L-I-1), SSI=0.37/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL = 0.97 (O) (INPUT = 1.00)

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-111	G01	1	1	GREENPARK - ZADORRA ESTATES - VILLA 5-2	

TRUE COPY
OF PERMIT PLANS

Nov 03 2023

PER:

C. M. ...

CHIEF BUILDING OFFICIAL

ID: aX72BI6hiO7VoZWieLJw?z93Pm-mElbXpJHNlwjrtxdH3_xEnQbfVXXdmFqMyfzkywpc?

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	2-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AB	4-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AC	8-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AD	10-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AE	12-0-12	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AF	15-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AG	17-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AH	19-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AI	23-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1
AJ	25-0-4	-22	-22	—	FRONT	VERT	TOTAL	—	C1

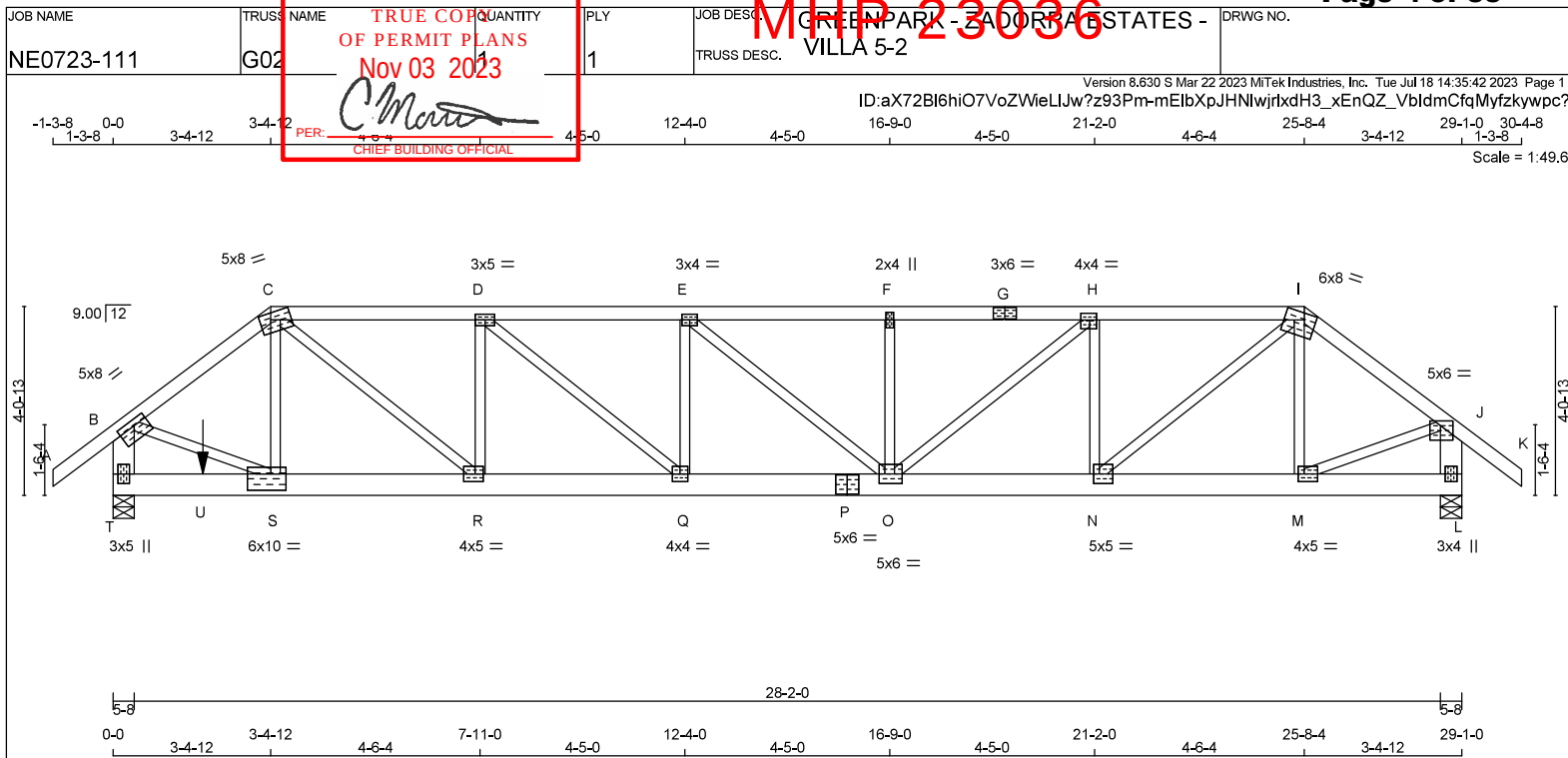
CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV4	MT20	5.0	8.0	1.50	3.50
C TTMWV-m	MT20	5.0	8.0	1.75	2.75
D TMWV4	MT20	3.0	5.0		
E TMWV4	MT20	3.0	4.0		
F TMWV4	MT20	2.0	4.0		
G TS4	MT20	3.0	6.0		
H TMWV4	MT20	4.0	4.0	1.75	1.75
I TTMWV-m	MT20	6.0	8.0	1.75	3.00
J TMWV-p	MT20	5.0	6.0	1.25	3.25
L BMV1-p	MT20	3.0	4.0		
M BMWV4	MT20	4.0	5.0	2.00	1.50
N BMWV4	MT20	5.0	5.0	2.50	1.50
O BMWVW4	MT20	5.0	6.0	2.50	1.50
P BS4	MT20	5.0	6.0		
Q BMWV4	MT20	4.0	4.0		
R BMWV4	MT20	4.0	5.0	2.00	2.00
S BMWV4	MT20	6.0	10.0	4.25	4.00
T BMV1-p	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG
JT VERT	4112	0	5-8
T	4112	0	5-8
L	2306	0	2-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	2868
T	2109 / 0
L	1608

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L
 BEARING SIZE FACTOR = 1.15 AT JNT(S) T (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.55 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 49	-119.4	-119.4	0.18 (1)	S-C	0 / 698	0.17 (1)	
B-C	-3448 / 0	-119.4	-119.4	0.47 (1)	C-R	0 / 1495	0.37 (1)	
C-D	-3931 / 0	-119.4	-119.4	0.87 (1)	R-D	-1073 / 0	0.27 (1)	
D-E	-4515 / 0	-119.4	-119.4	0.76 (1)	D-Q	0 / 762	0.19 (1)	
E-F	-4331 / 0	-119.4	-119.4	0.61 (1)	Q-E	-360 / 0	0.09 (1)	
F-G	-4331 / 0	-119.4	-119.4	0.73 (1)	E-O	-241 / 0	0.14 (1)	
G-H	-4331 / 0	-119.4	-119.4	0.73 (1)	O-F	-508 / 0	0.13 (1)	
H-I	-3475 / 0	-119.4	-119.4	0.61 (1)	O-H	0 / 1115	0.28 (1)	
I-J	-2242 / 0	-119.4	-119.4	0.35 (1)	H-N	-1304 / 0	0.33 (1)	
J-K	0 / 49	-119.4	-119.4	0.18 (1)	N-I	0 / 2191	0.54 (1)	
T-B	-3318 / 0	0.0	0.0	0.24 (1)	M-I	-467 / 0	0.12 (1)	
L-J	-2286 / 0	0.0	0.0	0.17 (1)	B-S	0 / 2886	0.71 (1)	
					M-J	0 / 1877	0.46 (1)	
T-U	0 / 0	-18.2	-18.2	0.60 (1)				
U-S	0 / 0	-18.2	-18.2	0.60 (1)				
S-R	0 / 2771	-18.2	-18.2	0.38 (1)				
R-Q	0 / 3931	-18.2	-18.2	0.28 (1)				
Q-P	0 / 4515	-18.2	-18.2	0.67 (1)				
P-O	0 / 4515	-18.2	-18.2	0.67 (1)				
O-N	0 / 3476	-18.2	-18.2	0.53 (1)				
N-M	0 / 1776	-18.2	-18.2	0.26 (1)				
M-L	0 / 0	-18.2	-18.2	0.04 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	1-114	-1453	-1453	—	FRONT	VERT	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOT CH. LL	=	34.8	PSF
DOL CH. LL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DOL CH. LL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/965 (0.36")

CSI: TC=0.78/0.97 (D-E-1), BC=0.67/0.97 (O-Q-1),
 WB=0.71/0.97 (B-S-1), SSI=0.90/1.00 (S-T-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

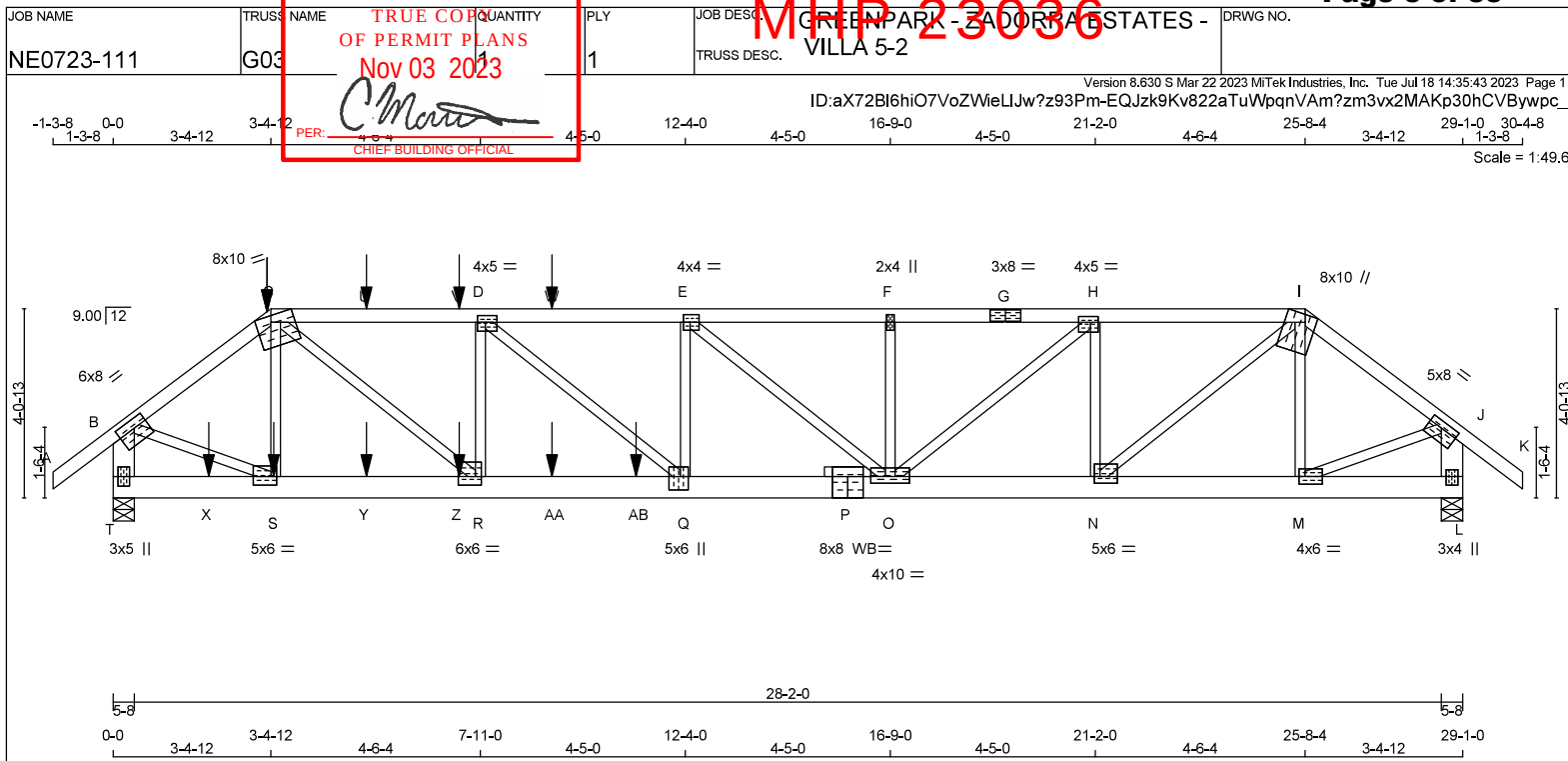
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
 JSI METAL = 0.86 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	2100F 1.8E	SPF
G - I	2x4	DRY	2100F 1.8E	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - L	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW4	MT20	6.0	8.0	2.25	3.50
C TTVW+m	MT20	8.0	10.0	Edge	4.00
D TMVW4	MT20	4.0	5.0	1.75	2.00
E TMVW4	MT20	4.0	4.0	2.00	1.75
F TMVW4	MT20	2.0	4.0		
G TS4	MT20	3.0	8.0		
H TMVW4	MT20	4.0	5.0	1.50	2.00
I TTVW+m	MT20	8.0	10.0	Edge	3.00
J TMVW4	MT20	5.0	8.0	1.75	3.50
L BMV1+p	MT20	3.0	4.0	2.25	1.50
M BMVW4	MT20	4.0	6.0	2.00	1.50
N BMVW4	MT20	5.0	6.0	1.75	1.50
O BMVW4	MT20	4.0	10.0	1.75	5.00
P BS4	MT20	8.0	8.0		
Q BMVW4	MT20	5.0	6.0	3.50	2.00
R BMVW4	MT20	6.0	6.0	2.25	1.50
S BMVW4	MT20	5.0	6.0	2.25	1.50
T BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
 EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM	FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
T	3554	0	3554	0	5-8
L	2860	0	2860	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS	
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND
T	2480	1820 / 0	0 / 0	0 / 0
L	1995	1465 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L
 BEARING SIZE FACTOR = 1.15 AT JNT(S) T, L (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.81 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	FORCE	FROM	TO	CSI (LC)	LENGTH	MEMB.	FORCE	MAX
FR-TO							FR-TO		
A-B	0 / 49	-119.4	-119.4	0.18 (1)	10.00	S-C	-876 / 0	0.22 (1)	
B-C	-3684 / 0	-119.4	-119.4	0.49 (1)	3.15	C-F	0 / 3695	0.91 (1)	
C-U	-5781 / 0	-119.4	-119.4	0.51 (1)	3.22	R-D	-1839 / 0	0.46 (1)	
L-U	-5781 / 0	-119.4	-119.4	0.51 (1)	3.22	D-Q	0 / 1748	0.43 (1)	
V-D	-5781 / 0	-119.4	-119.4	0.51 (1)	3.22	Q-E	0 / 298	0.07 (1)	
D-W	-7122 / 0	-119.4	-119.4	0.61 (1)	2.81	E-O	-1273 / 0	0.71 (1)	
W-E	-7122 / 0	-119.4	-119.4	0.61 (1)	2.81	O-F	-513 / 0	0.13 (1)	
E-F	-6145 / 0	-119.4	-119.4	0.39 (1)	3.27	O-H	0 / 1935	0.48 (1)	
F-G	-6145 / 0	-119.4	-119.4	0.47 (1)	3.17	H-I	-1822 / 0	0.46 (1)	
G-H	-6145 / 0	-119.4	-119.4	0.47 (1)	3.17	I-N	0 / 3055	0.76 (1)	
H-I	-4661 / 0	-119.4	-119.4	0.38 (1)	3.70	M-I	-626 / 0	0.16 (1)	
I-J	-2894 / 0	-119.4	-119.4	0.41 (1)	3.63	B-S	0 / 3083	0.76 (1)	
J-K	0 / 49	-119.4	-119.4	0.18 (1)	10.00	M-J	0 / 2422	0.60 (1)	
T-B	-3519 / 0	0.0	0.0	0.26 (1)	5.55				
L-J	-2843 / 0	0.0	0.0	0.21 (1)	6.09				

T-X	0 / 0	-18.2	-18.2	0.02 (4)	10.00
X-S	0 / 0	-18.2	-18.2	0.02 (4)	10.00
S-Y	0 / 2915	-18.2	-18.2	0.23 (1)	10.00
Y-Z	0 / 2915	-18.2	-18.2	0.23 (1)	10.00
Z-R	0 / 2915	-18.2	-18.2	0.23 (1)	10.00
R-AA	0 / 5781	-18.2	-18.2	0.70 (1)	10.00
AA-AB	0 / 5781	-18.2	-18.2	0.70 (1)	10.00
AB-Q	0 / 5781	-18.2	-18.2	0.70 (1)	10.00
Q-P	0 / 7122	-18.2	-18.2	0.51 (1)	10.00
P-O	0 / 7122	-18.2	-18.2	0.51 (1)	10.00
O-N	0 / 4661	-18.2	-18.2	0.33 (1)	10.00
N-M	0 / 2291	-18.2	-18.2	0.14 (1)	10.00
M-L	0 / 0	-18.2	-18.2	0.02 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-4-12	-198	-198		BACK	VERT	TOTAL		C1
S	3-5-9	-11	-11		BACK	VERT	TOTAL		C1
U	5-5-9	-41	-41		BACK	VERT	TOTAL		C1
V	7-5-9	-41	-41		BACK	VERT	TOTAL		C1
W	9-5-9	-41	-41		BACK	VERT	TOTAL		C1
X	2-0-12	-8	-8		BACK	VERT	TOTAL		C1
Y	5-5-9	-11	-11		BACK	VERT	TOTAL		C1
Z	7-5-9	-11	-11		BACK	VERT	TOTAL		C1
AA	9-5-9	-11	-11		BACK	VERT	TOTAL		C1
AB	11-3-4	-1079	-1079		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/729 (0.48")

CSI: TC=0.61/0.97 (D-E-1), BC=0.70/0.97 (Q-R-1),
 WB=0.91/0.97 (C-R-1), SSI=0.83/1.00 (Q-R-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
 JSI METAL= 0.99 (P) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.



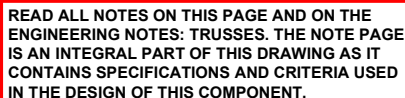
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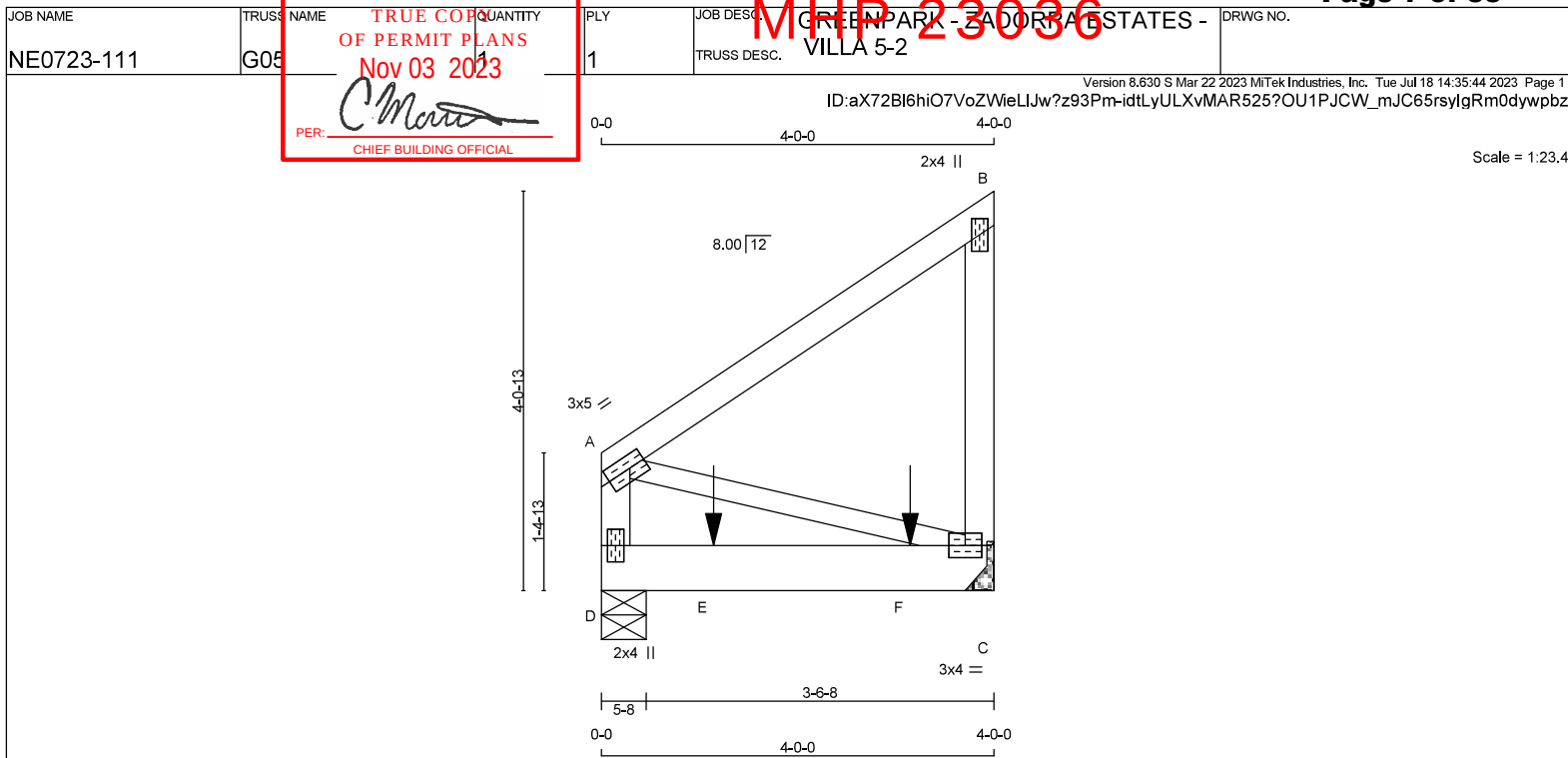
C. Morris

PER: _____

CHIEF BUILDING OFFICIAL

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.





TOTAL WEIGHT = 20 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	3.0	5.0	1.50	2.00
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-t	MT20	3.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	1568	0	1568	0	0	MECHANICAL	
D	1391	0	1391	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-11.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	1094	802 / 0	0 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
D	971	712 / 0	0 / 0	0 / 0	0 / 0	0 / 0	259 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
FR-TO				A-C	0 / 0	0.00 (1)
A-B	0 / 0	-119.4	-119.4 0.36 (1)	10.00		
C-B	-239 / 0	0.0	0.0 0.06 (1)	7.81		
D-A	-239 / 0	0.0	0.0 0.03 (1)	7.81		
D-E	0 / 0	-18.2	-18.2 0.96 (1)	10.00		
E-F	0 / 0	-18.2	-18.2 0.96 (1)	10.00		
F-C	0 / 0	-18.2	-18.2 0.96 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-1-12	-839	-839	—	BACK	VERT	TOTAL	—	C1
F	3-1-12	-840	-840	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 34.8	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.3	PSF
TOTAL LOAD	= 48.1	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/707 (0.07")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/407 (0.12")

CSI: TC=0.36/0.97 (A-B-1), BC=0.96/0.97 (C-D-1),
 WB=0.00/0.97 (A-C-1), SSI=0.88/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00
 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

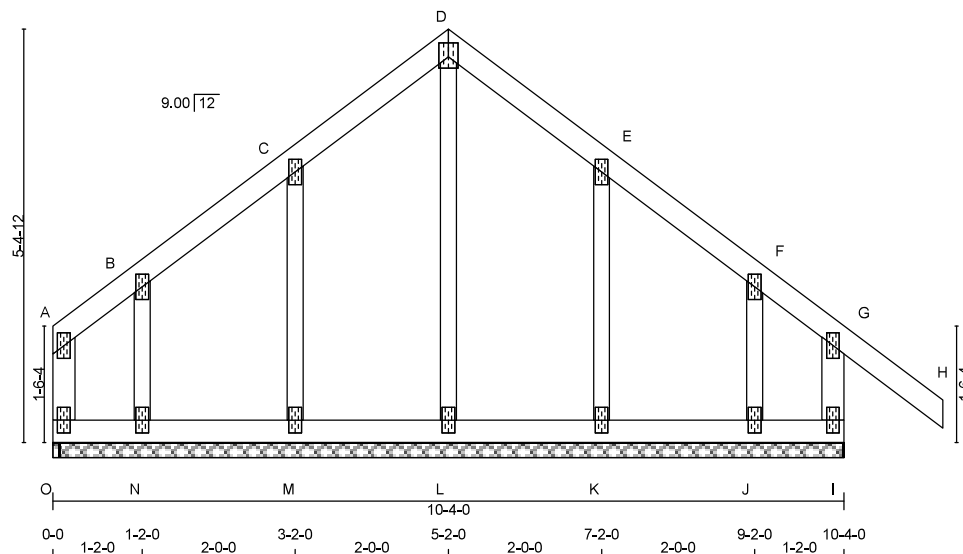
JSI GRIP= 0.23 (C) (INPUT = 0.90)
 JSI METAL= 0.12 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME NE0723-111	TRUSS NAME GE01	QUANTITY 1	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - VILLA 5-2	DRWG NO.
CORPORATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Monte</i> CHIEF BUILDING OFFICIAL Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 18 14:35:45 2023 Page 1 ID: aX72BI6hiO7VoZWleLIJw?z93Pm-ApRk9qL9gglliCgByCYerQ2CZjmzqHM6XKAJY3ywpby					
3-2-0 2-0-0 5-2-0 2-0-0 7-2-0 2-0-0 9-2-0 1-2-0 10-4-0 11-7-8 3x4 II Scale = 1:30.1					



TOTAL WEIGHT = 44 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
O - A	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - H	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, E, F						
B	TMV+w	MT20	2.0	4.0		
D	TMV+p	MT20	3.0	4.0	2.25	1.50
G	TMV+p	MT20	2.0	4.0		
I	BMV1+p	MT20	2.0	4.0		
J, K, L, M, N						
J	BMV1+w	MT20	2.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO			FR-TO		
O-A	-37 / 0	0.0 0.0 0.0 (1)	7.81	L-D	-267 / 0	0.11 (1)	
A-B	0 / 18	-119.4 -119.4 0.04 (1)	10.00	M-C	-250 / 0	0.06 (1)	
B-C	0 / 18	-119.4 -119.4 0.06 (1)	10.00	N-B	-196 / 0	0.03 (1)	
C-D	0 / 19	-119.4 -119.4 0.06 (1)	10.00	K-E	-259 / 0	0.06 (1)	
D-E	0 / 18	-119.4 -119.4 0.07 (1)	10.00	J-F	-100 / 0	0.02 (1)	
E-F	0 / 22	-119.4 -119.4 0.07 (1)	10.00				
F-G	-32 / 0	-119.4 -119.4 0.12 (1)	6.25				
G-H	0 / 49	-119.4 -119.4 0.16 (1)	10.00				
I-G	-290 / 0	0.0 0.0 0.04 (1)	7.81				
O-N	-2 / 0	-18.2 -18.2 0.01 (4)	10.00				
N-M	-10 / 0	-18.2 -18.2 0.02 (4)	6.25				
M-L	-17 / 0	-18.2 -18.2 0.02 (4)	6.25				
L-K	-17 / 0	-18.2 -18.2 0.02 (4)	6.25				
K-J	-10 / 0	-18.2 -18.2 0.02 (4)	6.25				
J-I	-6 / 0	-18.2 -18.2 0.03 (1)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 2.40 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/0.97 (G-H-1), BC=0.03/0.97 (I-J-1),
WB=0.11/0.97 (D-L-1), SSI=0.10/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

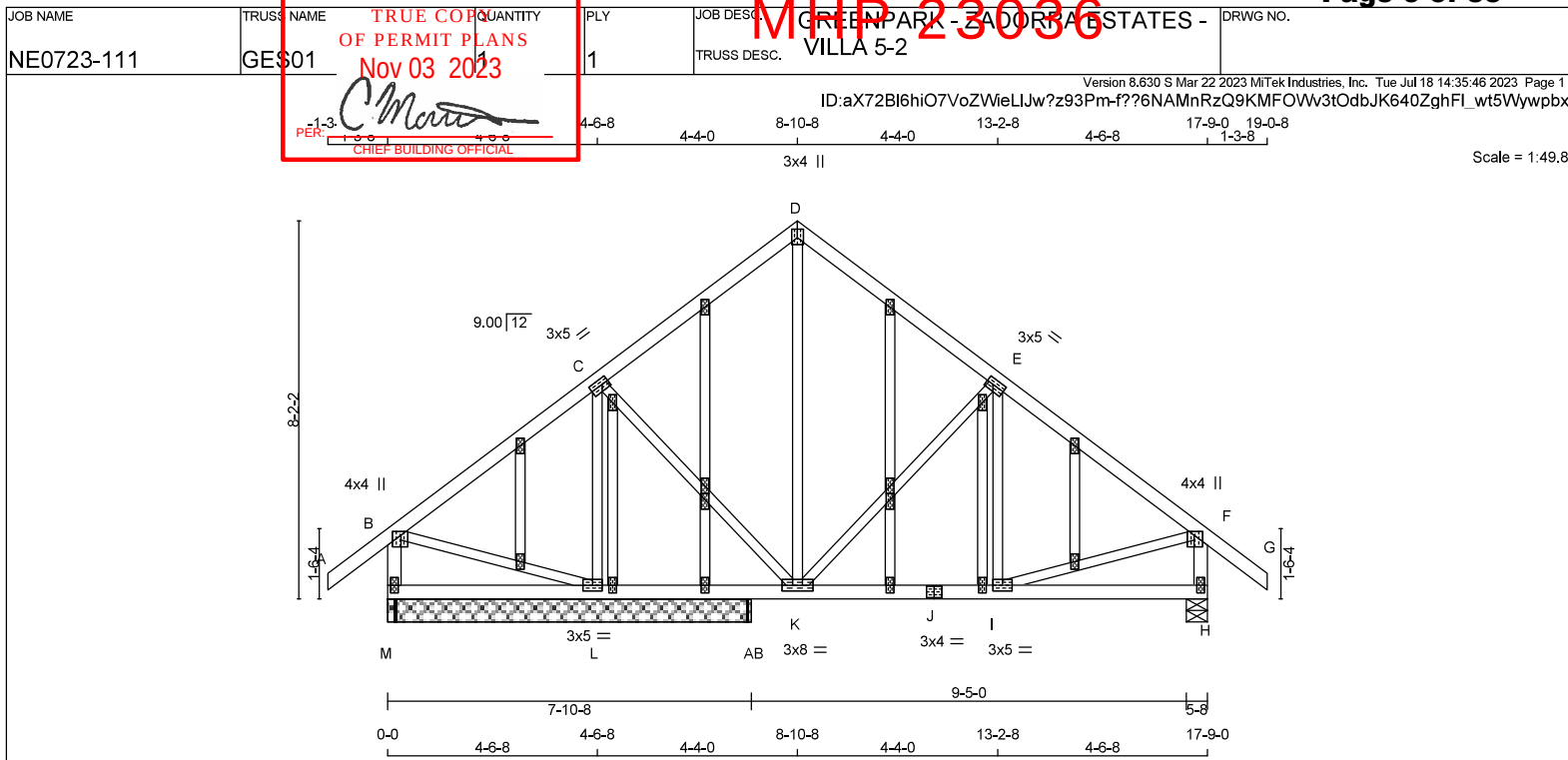
JSI GRIP= 0.18 (G) (INPUT = 0.90)

JSI METAL= 0.15 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TMVW-H	MT20	3.0	5.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-H	MT20	3.0	5.0	1.50	1.75
F	TMVW+p	MT20	4.0	4.0	1.00	2.25
H	BMV1+p	MT20	2.0	4.0		
I	BMVW-H	MT20	3.0	5.0		
J	BS-H	MT20	3.0	4.0		
K	BMVW-H	MT20	3.0	8.0		
L	BMVW-H	MT20	3.0	5.0		
M	BMV1+p	MT20	2.0	4.0		
N	NP+w	MT20	2.0	4.0	1.75	1.00
N, O, P, Q, R, S, T, U, V, W, X, Y, Z, AA						
N	NP+w	MT20	2.0	4.0		
W	NP+w	MT20	2.0	4.0	1.75	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
JT	512	0	512	0
M	1049	0	1049	0
L	1043	0	1043	0
H	169	0	169	0
AB				

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
M	353	281 / 0	0 / 0	72 / 0	0 / 0
L	735	522 / 0	0 / 0	214 / 0	0 / 0
H	725	545 / 0	0 / 0	180 / 0	0 / 0
AB	119	80 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, L, H, AB

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-B	0 / 49	-119.4	-119.4	0.16 (1)	10.00	K-D	0 / 63
B-C	-89 / 0	-119.4	-119.4	0.40 (1)	6.25	K-E	-485 / 0
C-D	-413 / 0	-119.4	-119.4	0.41 (1)	6.25	I-E	-57 / 67
D-E	-412 / 0	-119.4	-119.4	0.40 (1)	6.25	C-K	0 / 285
E-F	-750 / 0	-119.4	-119.4	0.42 (1)	6.22	L-C	-1016 / 0
F-G	0 / 49	-119.4	-119.4	0.16 (1)	10.00	B-L	0 / 106
M-B	-475 / 0	0.0	0.0	0.05 (1)	7.81	I-F	0 / 652
H-F	-1011 / 0	0.0	0.0	0.11 (1)	7.69		
M-L	0 / 0	-18.2	-18.2	0.08 (4)	10.00		
L-AB	0 / 102	-18.2	-18.2	0.12 (1)	10.00		
AB-K	0 / 102	-18.2	-18.2	0.12 (1)	10.00		
K-J	0 / 631	-18.2	-18.2	0.17 (1)	10.00		
J-I	0 / 631	-18.2	-18.2	0.17 (1)	10.00		
I-H	0 / 0	-18.2	-18.2	0.10 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 2.40 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.42/0.97 (E-F:1), BC=0.17/0.97 (I-K:1),
WB=0.36/0.97 (C-L:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

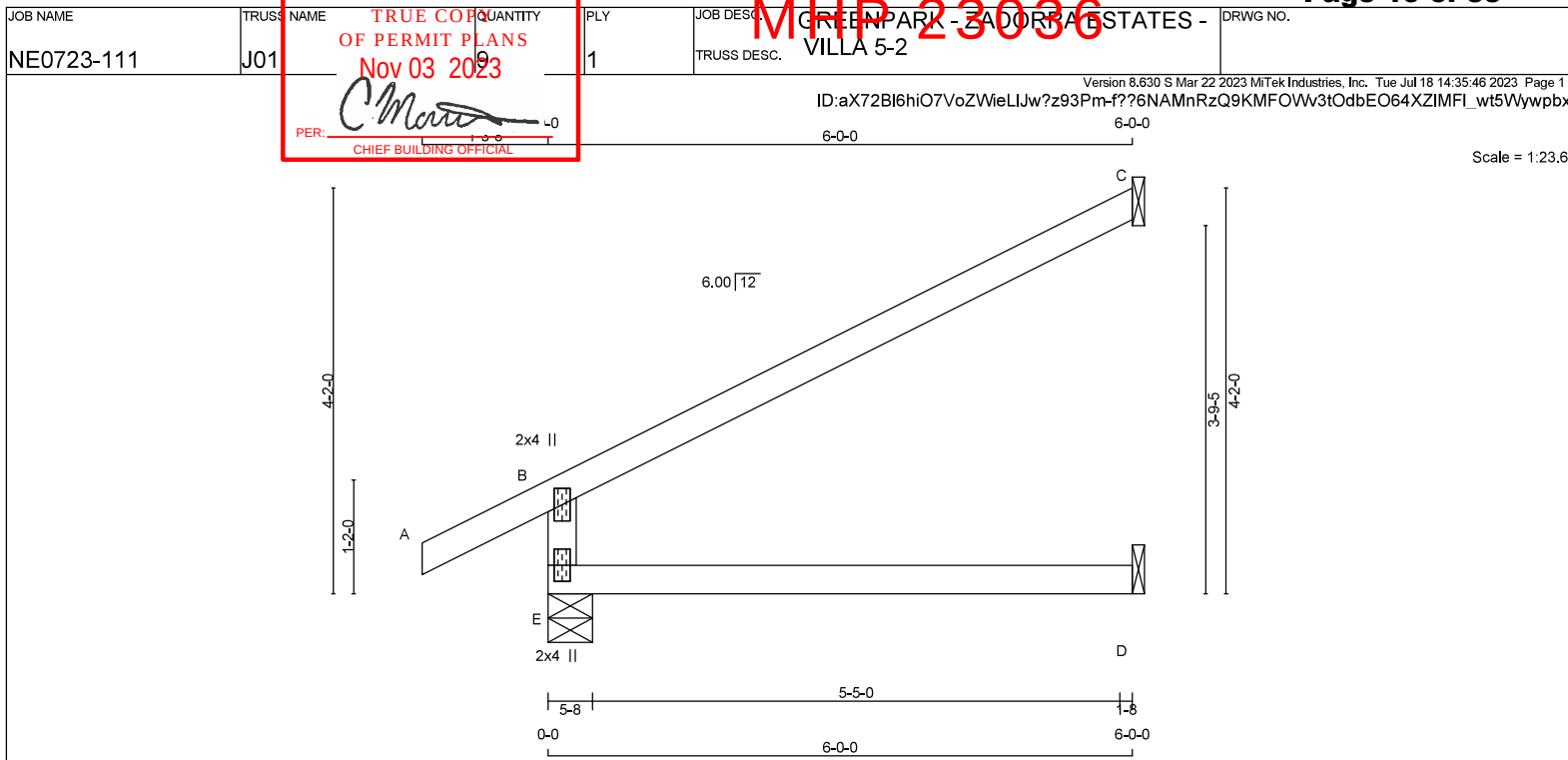
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	674	0	674	0	0	5-8	1-8
C	269	0	269	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	468	355 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0	0 / 0
C	184	157 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MEMB. LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO							
E-B	-610 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-40 / 0	-119.4	-119.4	0.73 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.73/0.97 (B-C-1), BC=0.13/0.97 (D-E-4),
 WB=0.00/0.97 (n/a-0), SSH=0.31/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

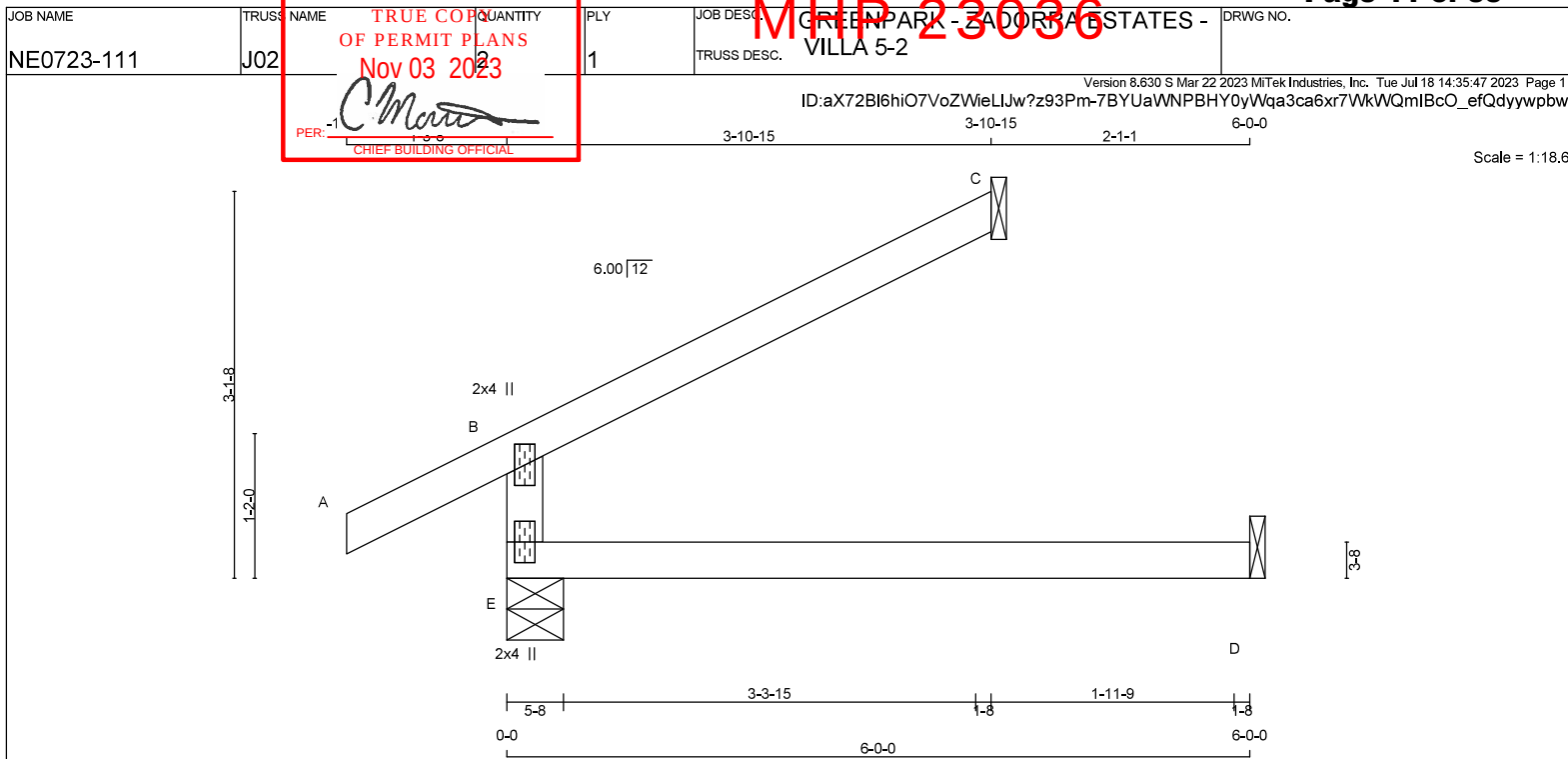
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
 JSI METAL= 0.25 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	518	0	518	0	0	5-8	1-8
C	175	0	175	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	362	265 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MEMB. MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LC2)
FR-TO		FROM	TO	LENGTH	FR-TO	
E-B	-454 / 0	0.0	0.0	0.13 (4)	7.81	
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00	
B-C	-26 / 0	-119.4	-119.4	0.31 (1)	6.25	
E-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00	

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.31/0.97 (B-C:1) , BC=0.13/0.97 (D-E:4) ,
 WB=0.00/0.97 (n/a:0) , SSH=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLD) (PSI)	SECTION (PLD) (PSI)
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

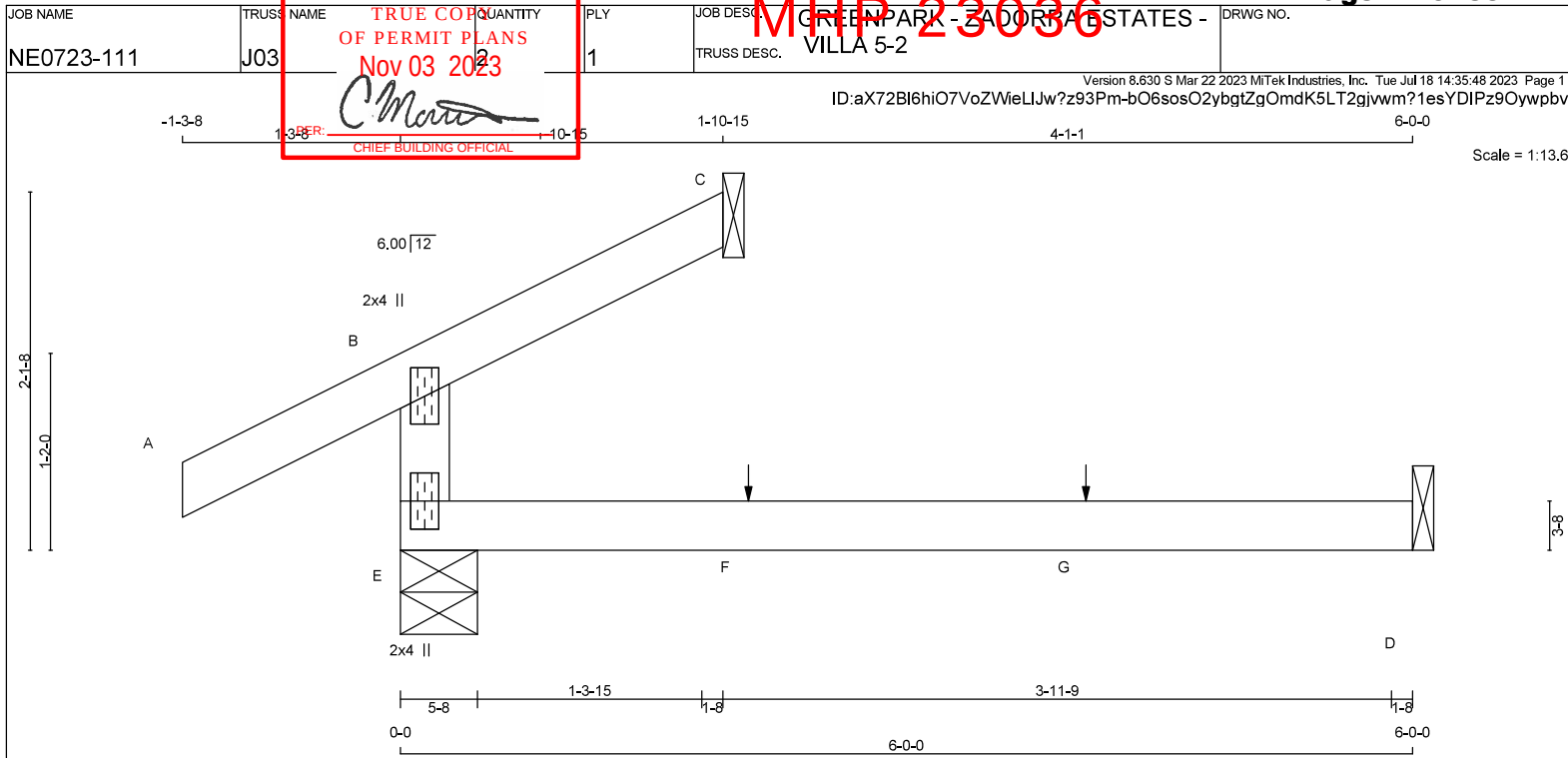
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
 JSI METAL= 0.19 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 12 = 24 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	368	0	368	0	0	5-8	1-8
C	86	0	86	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	259	177 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-304 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-12 / 0	-119.4	-119.4	0.07 (1)	6.25		
E-F	0 / 0	-18.2	-18.2	0.13 (4)	10.00		
F-G	0 / 0	-18.2	-18.2	0.13 (4)	10.00		
G-D	0 / 0	-18.2	-18.2	0.13 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1
G	4-0-12	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.16/0.97 (A-B:1), BC=0.13/0.97 (D-E:4),
 WB=0.00/0.97 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

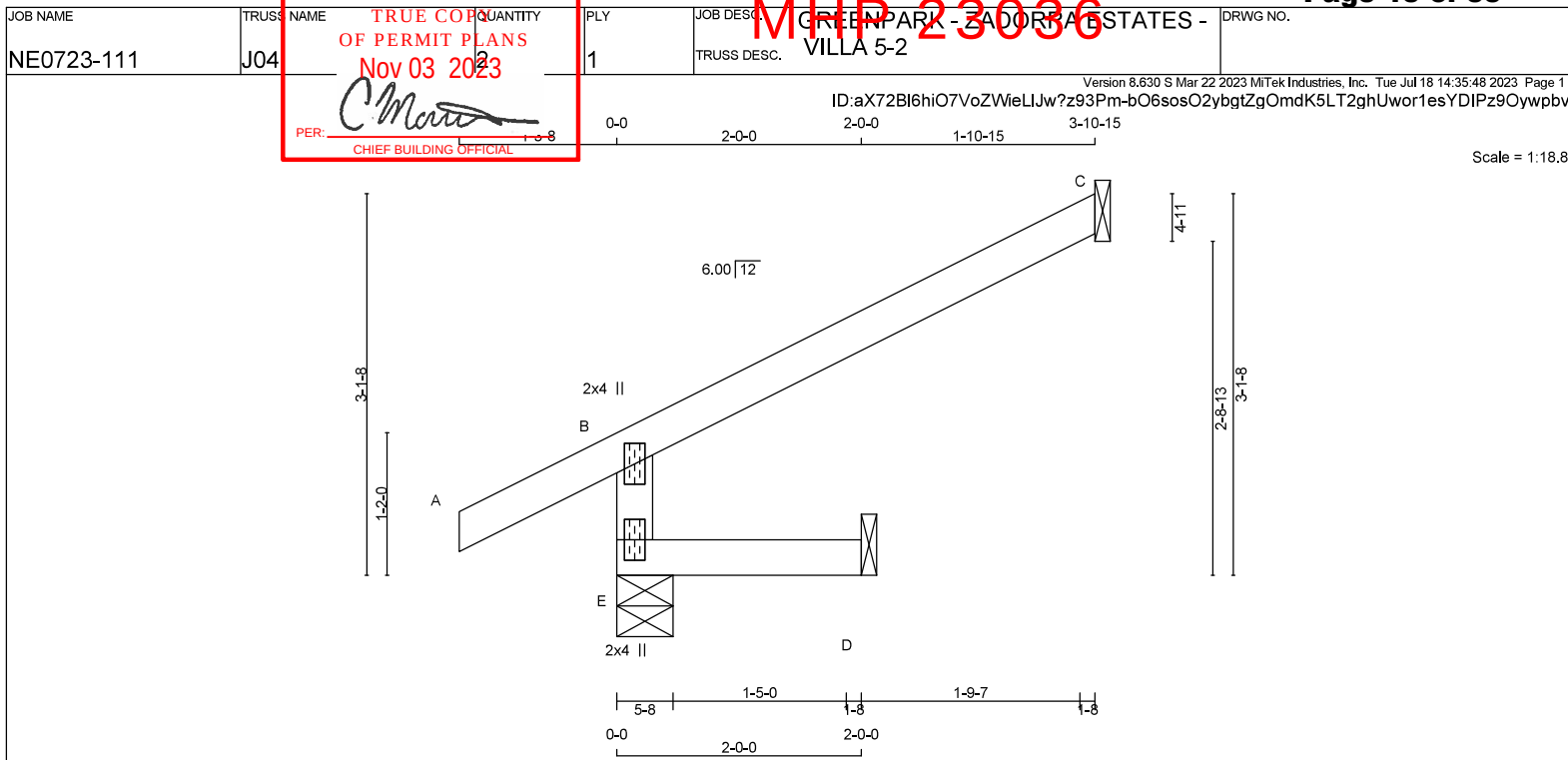
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
 JSI METAL = 0.13 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 10 = 20 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	474	0	474	0	5-8	1-8
C	175	0	175	0	1-8	1-8
D	16	0	16	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	326	265 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	120	102 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO						FR-TO		
E-B	-454 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00			
B-C	-26 / 0	-119.4	-119.4	0.31 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")CSI: TC=0.31/0.97 (B-C-1) , BC=0.02/0.97 (D-E-4) ,
WB=0.00/0.97 (n/a:0) , SSH=0.20/1.00 (B-C-1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PL)	(PSI)	(PL)
MT20	650	371	1747
		788	1987
			1873

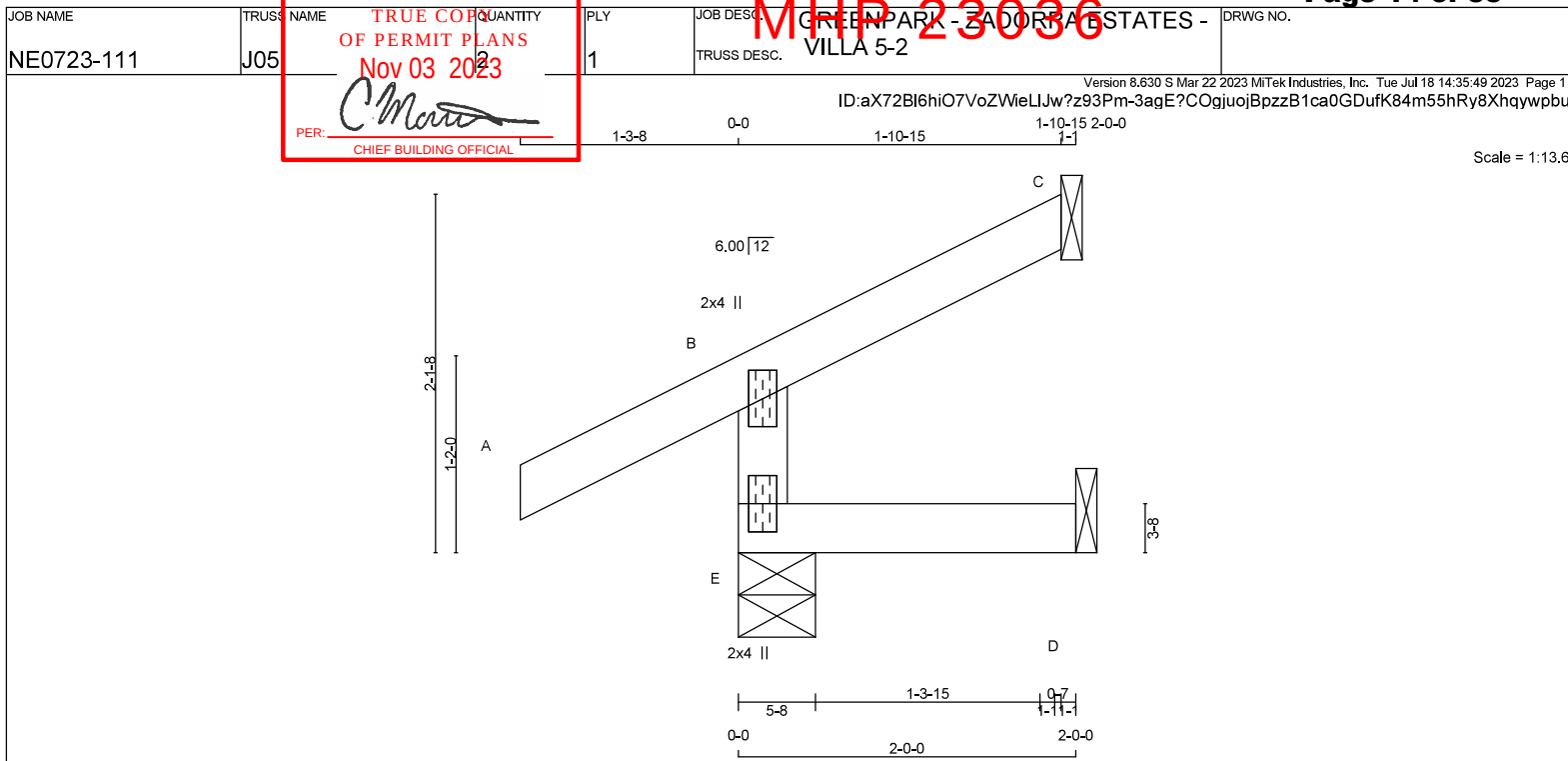
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
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TOTAL WEIGHT = 2 X 7 = 15 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	324	0	324	0	0	5-8	1-8
C	86	0	86	0	0	1-8	1-8
D	16	0	16	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	224	177 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MEMB. UNBRACED LENGTH FR-TO	MAX. FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO							
E-B	-304 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 36	-119.4	-119.4	0.16 (1)	10.00		
B-C	-12 / 0	-119.4	-119.4	0.07 (1)	6.25		
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")CSI: TC=0.16/0.97 (A-B:1) , BC=0.02/0.97 (D-E:4) ,
WB=0.00/0.97 (n/a:0) , SSI=0.11/1.00 (A-B:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PL)	(PSI)	(PL)	(PL)
MT20	650	371	1747
		788	1987
		1873	

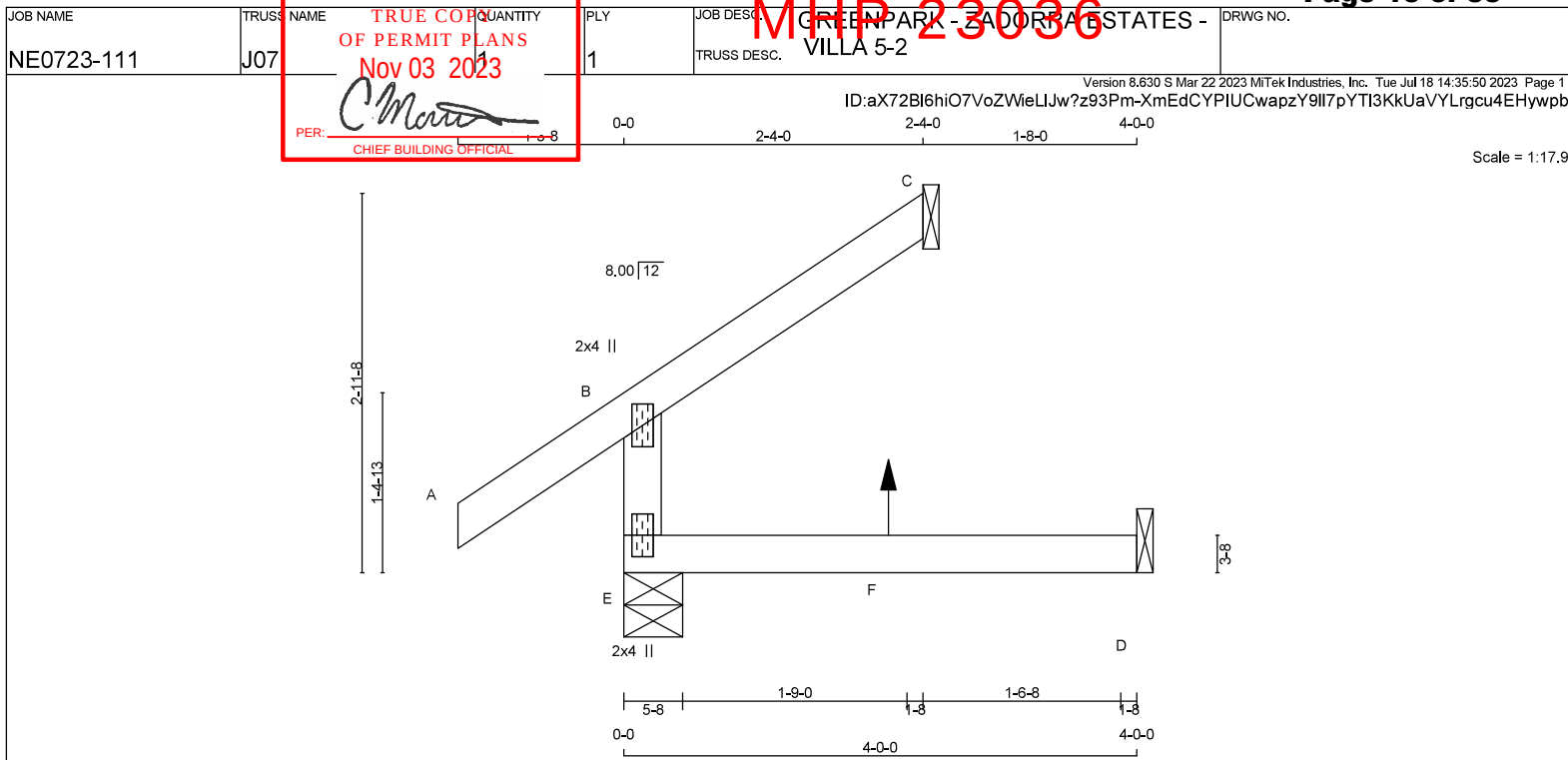
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 11 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	374	0	374	0	5-8	1-8
C	105	0	105	0	1-8	1-8
D	28	0	35	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	260	193 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0	0 / 0
C	72	61 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0	0 / 0
D	23	0 / -3	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
FR-TO								
A-B		-338 / 0	0.0	0.0	0.05 (4)		7.81	
A-B		0 / 45	-119.4	-119.4	0.16 (1)		10.00	
B-C		-19 / 0	-119.4	-119.4	0.11 (1)		6.25	
E-F		0 / 0	-18.2	-18.2	0.06 (4)		10.00	
F-D		0 / 0	-18.2	-18.2	0.06 (4)		10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	6	1	6	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN. C/C***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.16/0.97 (A-B:1) , BC=0.06/0.97 (D-E:4) ,
WB=0.00/0.97 (n/a:0) , SSI=0.11/1.00 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

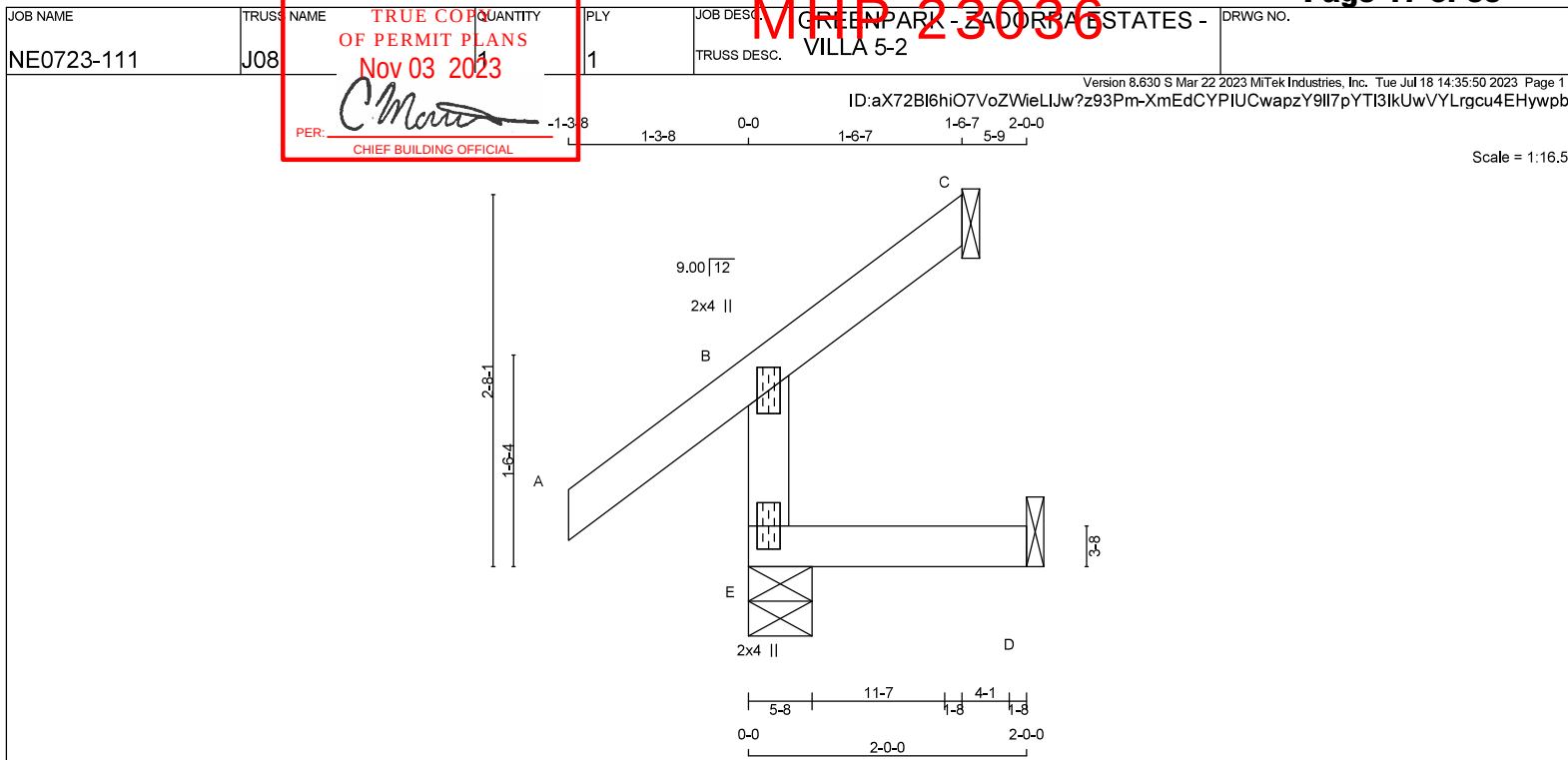
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

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TOTAL WEIGHT = 8 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	345	0	345	0	5-8	1-8
C	33	0	33	0	1-8	1-8
D	7	0	18	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT**UNFACTORED REACTIONS**

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	238	191 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	18 / -32	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	7	0 / -9	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB. UNBRAC LENGTH FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			
E-B	-315 / 0	0.0	0.0	0.04 (5)	7.81	
A-B	0 / 49	-119.4	-119.4	0.16 (1)	10.00	
B-C	-35 / 0	-119.4	-119.4	0.12 (1)	6.25	
E-D	0 / 0	-18.2	-18.2	0.04 (5)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")CSI: TC=0.16/0.97 (A-B:1) , BC=0.04/0.97 (D-E:5) ,
WB=0.00/0.97 (n/a:0) , SSH=0.10/1.00 (A-B:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
		1873	

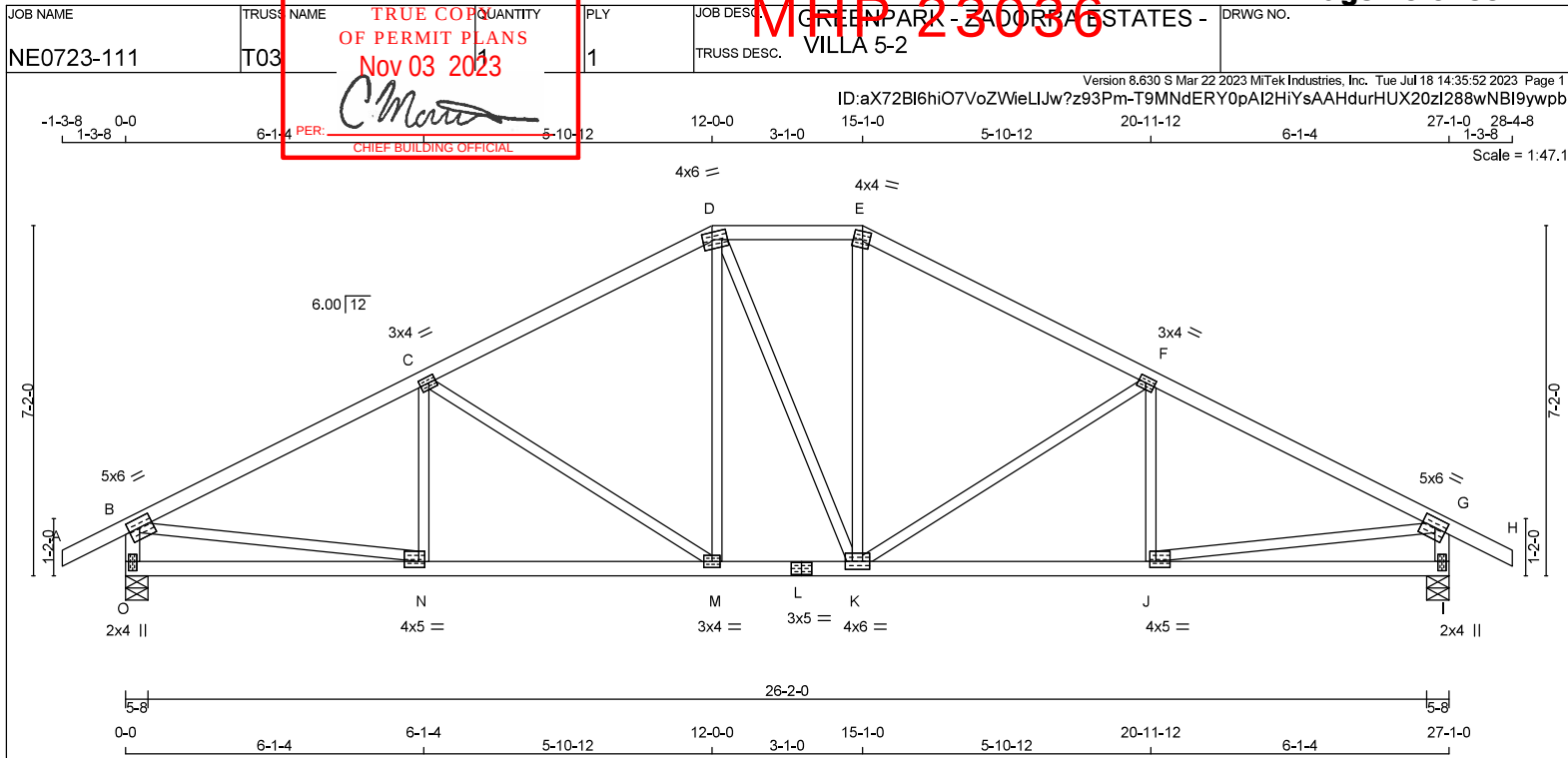
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
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IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	5.0	6.0	2.25	2.75
C	TMVW4	MT20	3.0	4.0	1.50	1.75
D	TTVW-m	MT20	4.0	6.0	1.75	2.25
E	TTVW-m	MT20	4.0	4.0	2.00	1.75
F	TMVW4	MT20	3.0	4.0	1.50	1.75
G	TMVW4	MT20	5.0	6.0	2.25	2.75
I	BMV1+p	MT20	2.0	4.0	2.25	1.00
J	BMVW4	MT20	4.0	5.0	1.50	1.50
K	BMVW4	MT20	4.0	6.0		
L	BS4	MT20	3.0	5.0		
M	BMVW4	MT20	3.0	4.0		
N	BMVW4	MT20	4.0	5.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	2026	0	2026	0
I	2026	0	2026	0

UNFACTORED REACTIONS**1ST CASE MAX./MIN. COMPONENT REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0
I	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 36	-119.4	-119.4 0.16 (1)	10.00	N-C	-216 / 53	0.06 (1)
B-C	-2643 / 0	-119.4	-119.4 0.69 (1)	3.43	C-M	-698 / 0	0.69 (1)
C-D	-2066 / 0	-119.4	-119.4 0.62 (1)	3.92	M-D	0 / 456	0.10 (1)
D-E	-1821 / 0	-119.4	-119.4 0.18 (1)	4.77	D-K	0 / 4	0.00 (1)
E-F	-2068 / 0	-119.4	-119.4 0.62 (1)	3.92	K-E	0 / 460	0.10 (1)
F-G	-2642 / 0	-119.4	-119.4 0.69 (1)	3.43	K-F	-696 / 0	0.69 (1)
G-H	0 / 36	-119.4	-119.4 0.16 (1)	10.00	J-F	-218 / 52	0.06 (1)
O-B	-1979 / 0	0.0	0.0 0.20 (1)	5.92	B-N	0 / 2421	0.54 (1)
I-G	-1978 / 0	0.0	0.0 0.20 (1)	5.92	J-G	0 / 2421	0.54 (1)
O-N	0 / 0	-18.2	-18.2 0.16 (4)	10.00			
N-M	0 / 2398	-18.2	-18.2 0.45 (1)	10.00			
M-L	0 / 1819	-18.2	-18.2 0.35 (1)	10.00			
L-K	0 / 1819	-18.2	-18.2 0.35 (1)	10.00			
K-J	0 / 2397	-18.2	-18.2 0.45 (1)	10.00			
J-I	0 / 0	-18.2	-18.2 0.16 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.69/0.97 (B-C:1) , BC=0.45/0.97 (M-N:1) ,
 WB=0.69/0.97 (C-M:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PL)
 (PSI) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

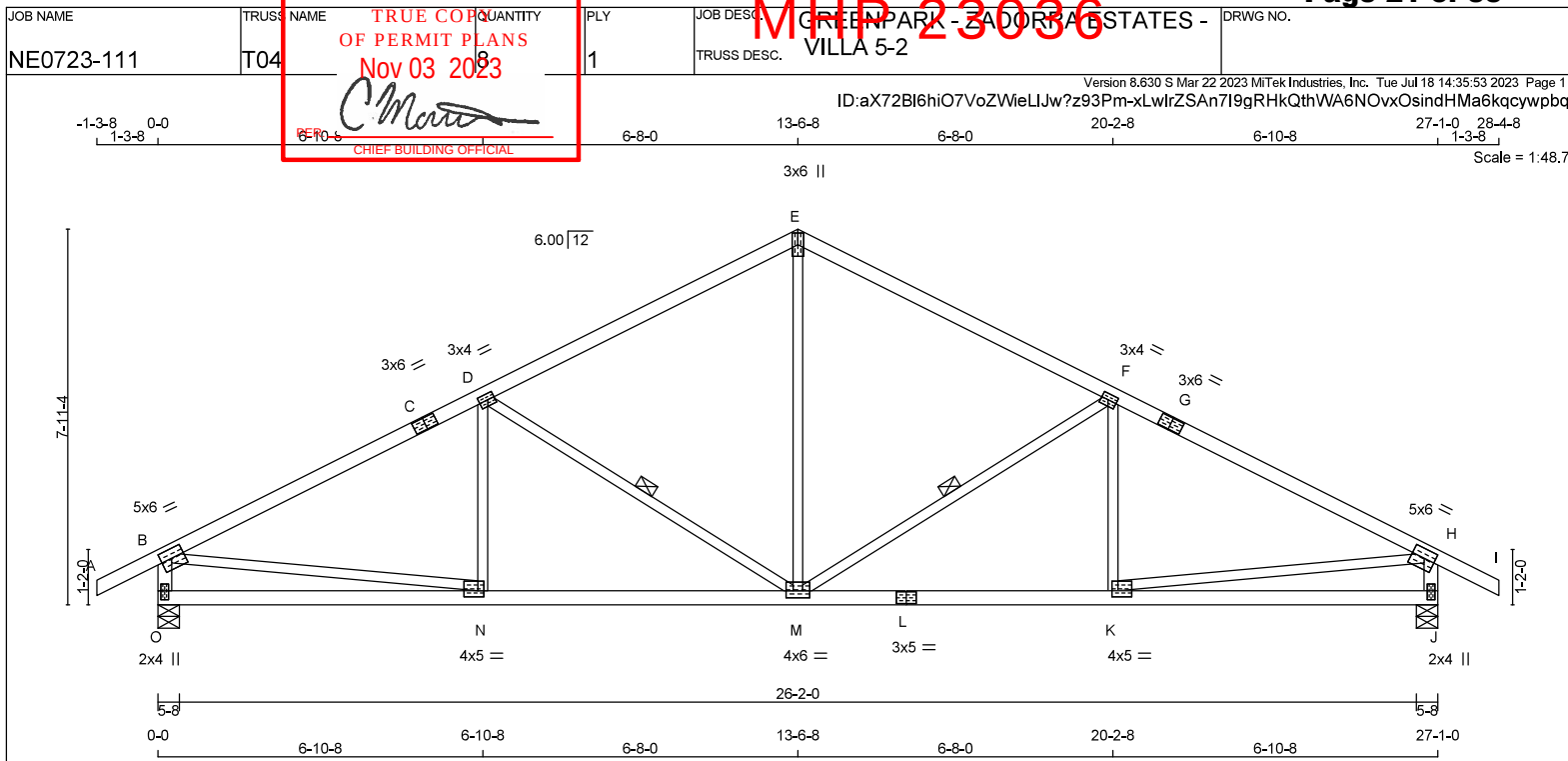
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
 JSI METAL = 0.68 (N) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	5.0	6.0	2.25	2.75
C	TS4	MT20	3.0	6.0		
D	TMVW4	MT20	3.0	4.0	1.50	1.75
E	TTW+p	MT20	3.0	6.0		
F	TMVW4	MT20	3.0	4.0	1.50	1.75
G	TS4	MT20	3.0	6.0		
H	TMVW4	MT20	5.0	6.0	2.25	2.75
J	BMV1+p	MT20	2.0	4.0	2.25	1.00
K	BMVW4	MT20	4.0	5.0	1.50	1.50
L	BS4	MT20	3.0	5.0		
M	BMVW4	MT20	4.0	6.0	1.75	3.00
N	BMVW4	MT20	4.0	5.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	GROSS REACTION	MAXIMUM FACTORED	GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
O	2026	0	2026	0	5-8	3-1
J	2026	0	2026	0	5-8	3-1

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0
J	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF F-M, D-M, DBS = 20'-0". CBF = 109 LBS.

DBS = DIAGONAL BRACE SPACING (MAX), CBF = CUMULATIVE BRACING FORCE (PER BRACE), FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 36	-119.4	-119.4 0.16 (1)	10.00	M-E	0 / 1065	0.24 (1)
B-C	-2634 / 0	-119.4	-119.4 0.91 (1)	2.97	M-F	-870 / 0	0.37 (1)
C-D	-2634 / 0	-119.4	-119.4 0.91 (1)	2.97	K-F	-167 / 76	0.05 (1)
D-E	-1904 / 0	-119.4	-119.4 0.79 (1)	3.65	D-M	-870 / 0	0.37 (1)
E-F	-1904 / 0	-119.4	-119.4 0.79 (1)	3.65	N-D	-167 / 76	0.05 (1)
F-G	-2634 / 0	-119.4	-119.4 0.91 (1)	2.97	B-N	0 / 2413	0.54 (1)
G-H	-2634 / 0	-119.4	-119.4 0.91 (1)	2.97	K-H	0 / 2413	0.54 (1)
H-I	0 / 36	-119.4	-119.4 0.16 (1)	10.00			
O-B	-1973 / 0	0.0	0.0 0.20 (1)	5.92			
J-H	-1973 / 0	0.0	0.0 0.20 (1)	5.92			
O-N	0 / 0	-18.2	-18.2 0.20 (4)	10.00			
N-M	0 / 2394	-18.2	-18.2 0.47 (1)	10.00			
M-L	0 / 2394	-18.2	-18.2 0.47 (1)	10.00			
L-K	0 / 2394	-18.2	-18.2 0.47 (1)	10.00			
K-J	0 / 0	-18.2	-18.2 0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
 ALLOWABLE DEFL.(TL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.91/0.97 (F-H:1), BC=0.47/0.97 (K-M:1), WB=0.54/0.97 (H-K:1), SSI=0.35/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

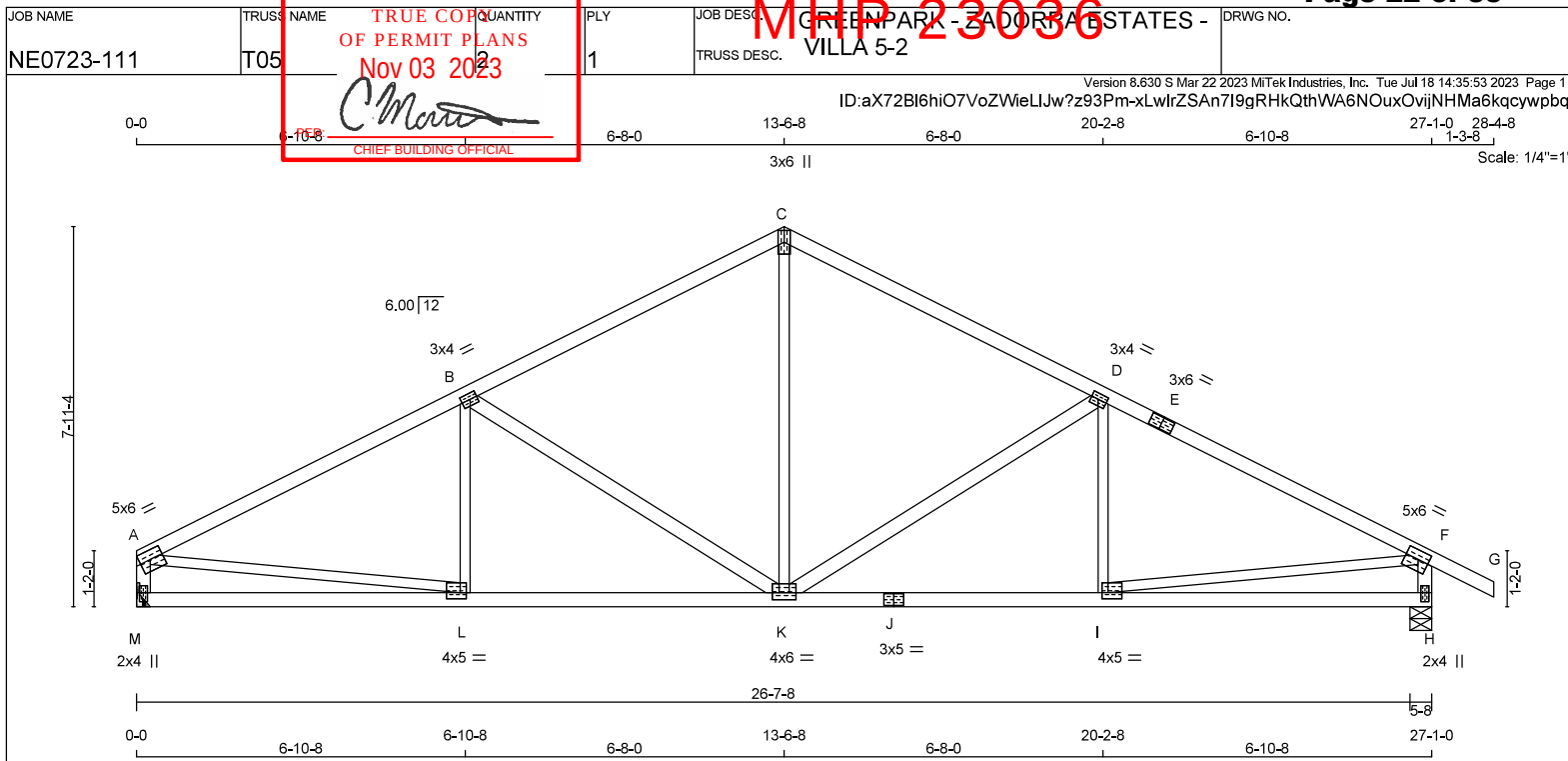
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
 JSI METAL = 0.74 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - A	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
K - D	2x4	DRY	No.2
B - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	5.0	6.0	2.25	Edge
B	TMVW4	MT20	3.0	4.0	1.50	1.75
C	TTW+p	MT20	3.0	6.0		
D	TMVW4	MT20	3.0	4.0	1.50	1.75
E	TS4	MT20	3.0	6.0		
F	TMVW4	MT20	5.0	6.0	2.25	2.75
H	BMV1+p	MT20	2.0	4.0	2.25	1.00
I	BMVW4	MT20	4.0	5.0	1.50	1.50
J	BS4	MT20	3.0	5.0		
K	BMVW4	MT20	4.0	6.0	1.75	3.00
L	BMVW4	MT20	4.0	5.0	1.50	1.50
M	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
 EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
M	1864	0	1864	0
H	2026	0	2026	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 2-9.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1303	943 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
H	1414	1037 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	-2635 / 0	-119.4 -119.4	0.91 (1)	2.95	K-C	0 / 1065	0.24 (1)
B-C	-1904 / 0	-119.4 -119.4	0.79 (1)	3.65	K-D	-871 / 0	0.81 (1)
C-D	-1904 / 0	-119.4 -119.4	0.79 (1)	3.65	I-D	-166 / 76	0.05 (1)
D-E	-2635 / 0	-119.4 -119.4	0.91 (1)	2.95	B-K	-871 / 0	0.81 (1)
E-F	-2635 / 0	-119.4 -119.4	0.91 (1)	2.95	L-B	-166 / 76	0.05 (1)
F-G	0 / 36	-119.4 -119.4	0.16 (1)	10.00	A-L	0 / 2414	0.54 (1)
M-A	-1811 / 0	0.0	0.0 0.18 (1)	6.13	I-F	0 / 2414	0.54 (1)
H-F	-1973 / 0	0.0	0.0 0.20 (1)	5.92			
M-L	0 / 0	-18.2	-18.2 0.20 (4)	10.00			
L-K	0 / 2395	-18.2	-18.2 0.47 (1)	10.00			
K-J	0 / 2395	-18.2	-18.2 0.47 (1)	10.00			
J-I	0 / 2395	-18.2	-18.2 0.47 (1)	10.00			
I-H	0 / 0	-18.2	-18.2 0.20 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 2.40 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
 ALLOWABLE DEFL.(TL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.91/0.97 (A-B:1) , BC=0.47/0.97 (K-L:1) ,
 WB=0.81/0.97 (B-K:1) , SSI=0.35/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

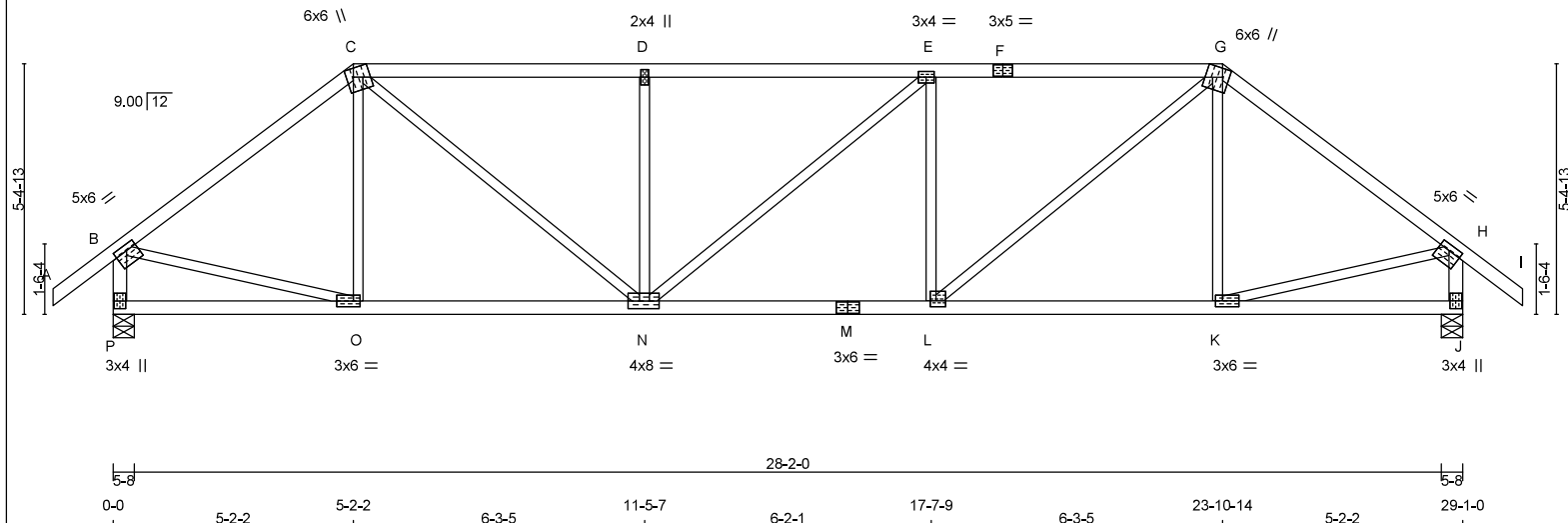
JSI GRIP= 0.89 (A) (INPUT = 0.90)
 JSI METAL= 0.74 (J) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.



JOB NAME NE0723-111	TRUSS NAME T06	QUANTITY 1	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - VILLA 5-2	DRWG NO.
Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Tue Jul 18 14:35:54 2023 Page 1 ID: aX72BI6hiO7VoZWMeLIJw?z93Pm-QYU72vSpYRR0Hbsw_bCijJwZQLJURGFQbEsIN2ywpbp					
Scale = 1:49.6					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW4	MT20	5.0	6.0	1.50 2.75
C	TTWW+m	MT20	6.0	6.0	Edge 4.25
D	TMW+w	MT20	2.0	4.0	
E	TMWW4	MT20	3.0	4.0	
F	TS4	MT20	3.0	5.0	
G	TTWW+m	MT20	6.0	6.0	Edge 4.25
H	TMWW4	MT20	5.0	6.0	1.50 2.75
J	BMV1+p	MT20	3.0	4.0	
K	BMWW4	MT20	3.0	6.0	1.50 1.75
L	BMWW4	MT20	4.0	4.0	1.50 1.50
M	BS4	MT20	3.0	6.0	
N	BMWW4	MT20	4.0	8.0	2.00 3.00
O	BMWW4	MT20	3.0	6.0	1.50 1.75
P	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
P	2167	0	2167	0
J	2167	0	2167	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1512	1108 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0
J	1512	1108 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 49	-119.4	-119.4 0.16 (1)	10.00	O-C	-263 / 25	0.11 (1)
B-C	-2117 / 0	-119.4	-119.4 0.74 (1)	3.74	C-N	0 / 1454	0.33 (1)
C-D	-2815 / 0	-119.4	-119.4 0.92 (1)	3.02	N-D	-807 / 0	0.35 (1)
D-E	-2815 / 0	-119.4	-119.4 0.91 (1)	3.02	N-E	0 / 0	0.00 (1)
E-F	-2816 / 0	-119.4	-119.4 0.92 (1)	3.03	L-E	-805 / 0	0.35 (1)
F-G	-2816 / 0	-119.4	-119.4 0.92 (1)	3.03	L-G	0 / 1455	0.33 (1)
G-H	-2117 / 0	-119.4	-119.4 0.74 (1)	3.74	K-G	-265 / 25	0.12 (1)
H-I	0 / 49	-119.4	-119.4 0.16 (1)	10.00	B-O	0 / 1733	0.39 (1)
P-B	-2130 / 0	0.0	0.0 0.22 (1)	5.74	K-H	0 / 1733	0.39 (1)
J-H	-2129 / 0	0.0	0.0 0.22 (1)	5.74			
P-O	0 / 0	-18.2	-18.2 0.14 (4)	10.00			
O-N	0 / 1685	-18.2	-18.2 0.35 (1)	10.00			
N-M	0 / 2815	-18.2	-18.2 0.51 (1)	10.00			
M-L	0 / 2815	-18.2	-18.2 0.51 (1)	10.00			
L-K	0 / 1685	-18.2	-18.2 0.35 (1)	10.00			
K-J	0 / 0	-18.2	-18.2 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.92/0.97 (C-D:1), BC=0.51/0.97 (L-N:1),
WB=0.39/0.97 (B-O:1), SS=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PL)
(PSI) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

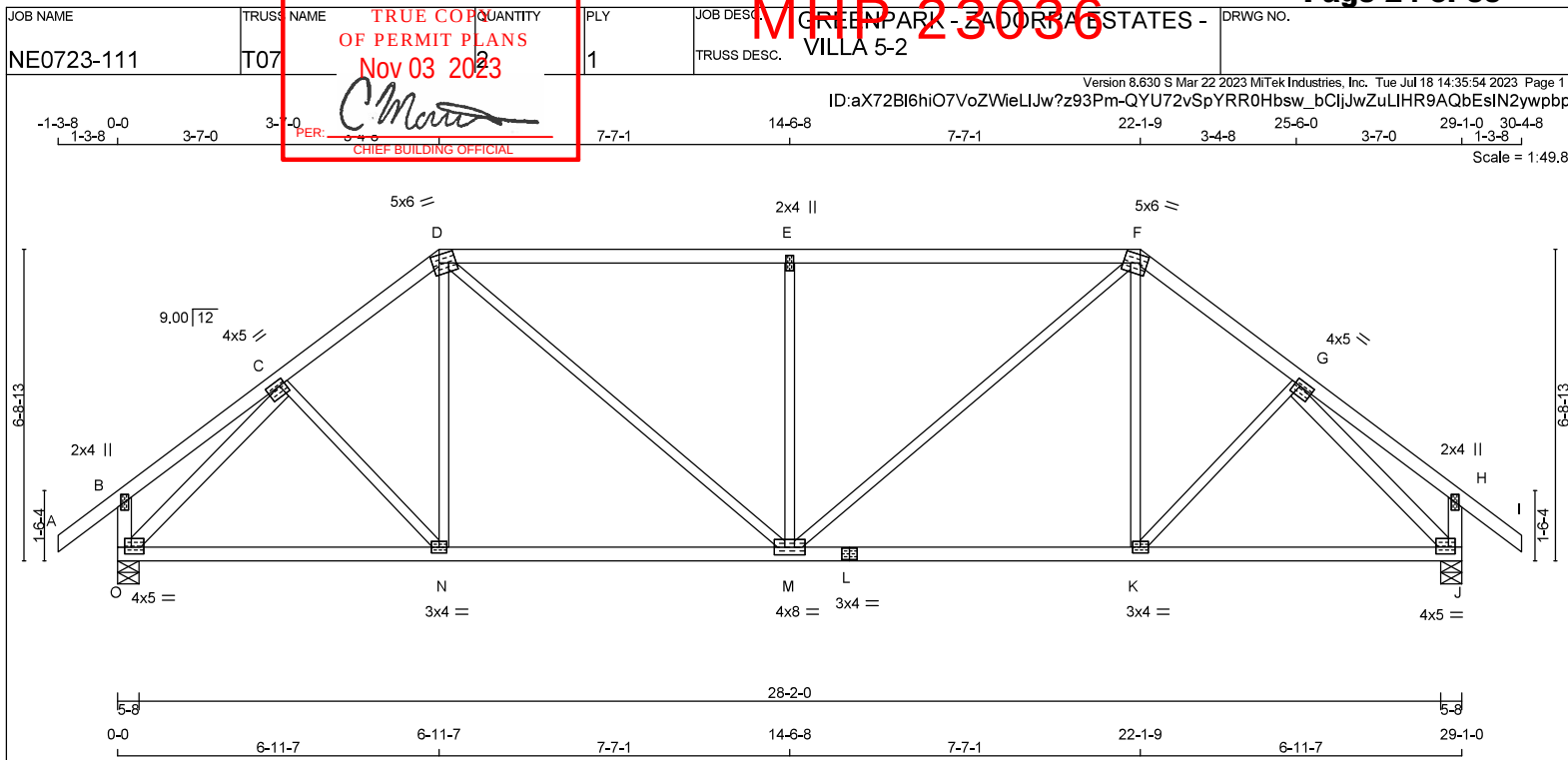
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.86 (M) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	2100F 1.8E
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
O - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - C	2x4	DRY	No.2
G - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWWH	MT20	4.0	5.0	1.50	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.75
G	TMWWH	MT20	4.0	5.0	1.50	1.50
H	TMV+p	MT20	2.0	4.0		
J	BMVW1-t	MT20	4.0	5.0	1.75	1.75
K	BMWW-4	MT20	3.0	4.0		
L	BS-t	MT20	3.0	4.0		
M	BMWWH	MT20	4.0	8.0		
N	BMWW-4	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	5.0	1.75	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	2167	0	2167	0
J	2167	0	2167	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	
O	1512	1108 / 0	0 / 0	0 / 0	403 / 0
J	1512	1108 / 0	0 / 0	0 / 0	403 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	VERT. LOAD	LC1	MAX	W E B S	MAX. FACTORED
		FORCE					FORCE
		(LBS)					(LBS)
FR-TO							
A-B	0 / 49	-119.4	-119.4	0.16 (1)	10.00	C-N	0 / 59
B-C	0 / 27	-119.4	-119.4	0.22 (1)	10.00	N-D	0 / 125
C-D	-2111 / 0	-119.4	-119.4	0.28 (1)	4.36	D-M	0 / 962
D-E	-2399 / 0	-119.4	-119.4	0.96 (1)	3.69	M-E	-1115 / 0
E-F	-2399 / 0	-119.4	-119.4	0.96 (1)	3.69	M-F	0 / 962
F-G	-2111 / 0	-119.4	-119.4	0.29 (1)	4.36	K-F	0 / 125
G-H	0 / 27	-119.4	-119.4	0.22 (1)	10.00	K-G	0 / 59
H-I	0 / 49	-119.4	-119.4	0.16 (1)	10.00	O-C	-2425 / 0
O-B	-325 / 0	0.0	0.0	0.03 (1)	7.81	G-J	-2425 / 0
J-H	-325 / 0	0.0	0.0	0.03 (1)	7.81		
O-N	0 / 1635	-18.2	-18.2	0.39 (1)	10.00		
N-M	0 / 1665	-18.2	-18.2	0.40 (1)	10.00		
M-L	0 / 1665	-18.2	-18.2	0.40 (1)	10.00		
L-K	0 / 1665	-18.2	-18.2	0.40 (1)	10.00		
K-J	0 / 1635	-18.2	-18.2	0.39 (1)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.96/0.97 (D-E:1), BC=0.40/0.97 (M-N:1),
 WB=0.84/0.97 (E-M:1), SSI=0.44/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSD)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

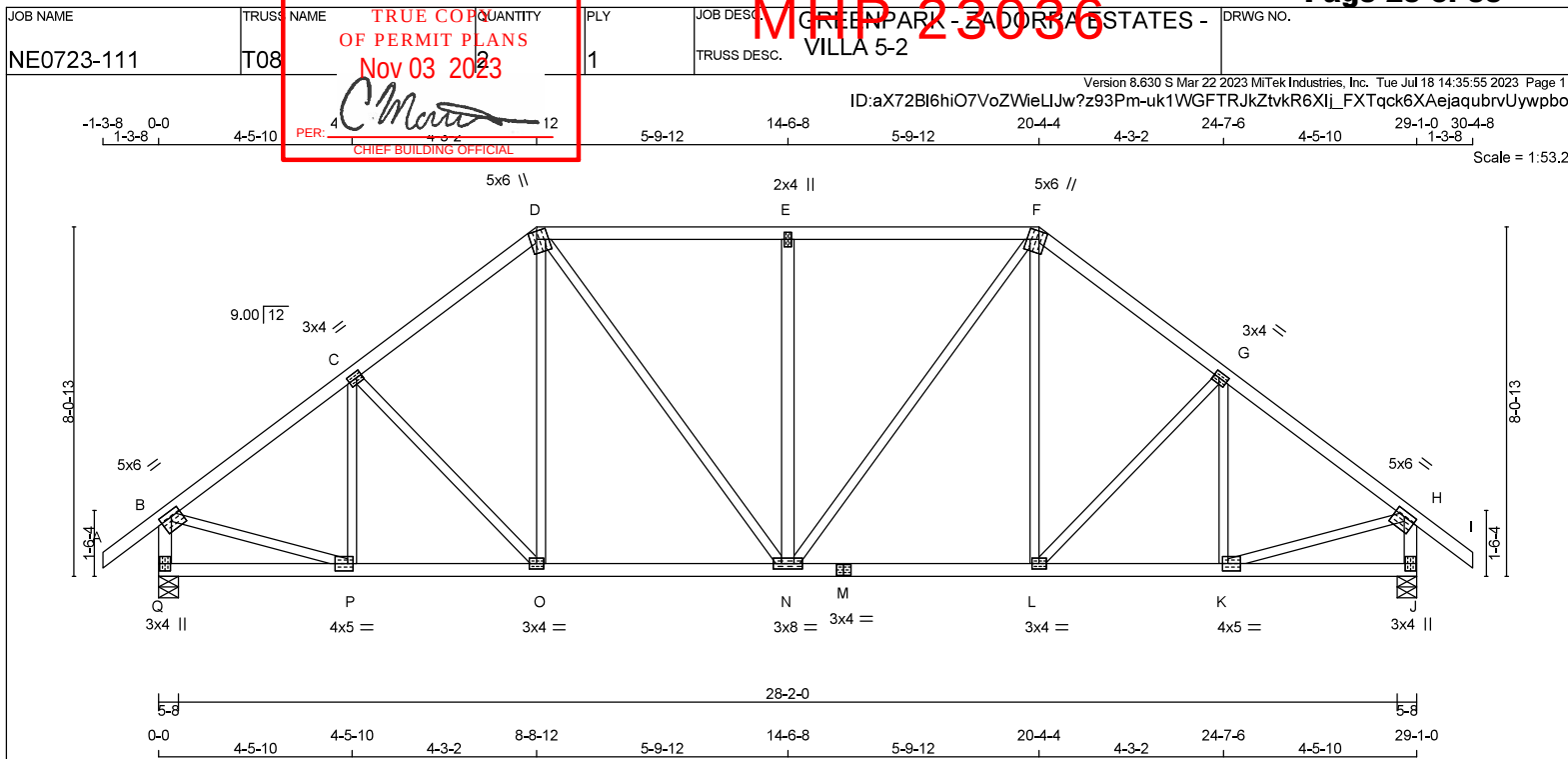
JSI GRIP= 0.88 (F) (INPUT = 0.90)

JSI METAL = 0.68 (G) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 137 = 275 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	5.0	6.0	1.50	2.75
C	TMVW4	MT20	3.0	4.0	1.50	1.50
D	TTVW+m	MT20	5.0	6.0	2.25	1.75
E	TMVW4	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.25	1.75
G	TMVW4	MT20	3.0	4.0	1.50	1.50
H	TMVW4	MT20	5.0	6.0	1.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMVW4	MT20	4.0	5.0	2.00	1.50
L	BMVW4	MT20	3.0	4.0		
M	BS4	MT20	3.0	4.0		
N	BMVW4	MT20	3.0	4.0		
O	BMVW4	MT20	3.0	4.0		
P	BMVW4	MT20	4.0	5.0	2.00	1.50
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
Q	2167	0	2167	0
J	2167	0	2167	0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1512	1108 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0
J	1512	1108 / 0	0 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX
FR-TO							FR-TO			
A-B	0 / 49	-119.4	-119.4	0.16 (1)	10.00	P-C	-383 / 0	0.13 (1)		
B-C	-2125 / 0	-119.4	-119.4	0.35 (1)	4.30	C-O	-216 / 0	0.15 (1)		
C-D	-2016 / 0	-119.4	-119.4	0.34 (1)	4.30	O-D	0 / 253	0.06 (1)		
D-E	-1940 / 0	-119.4	-119.4	0.57 (1)	4.09	D-N	0 / 597	0.13 (1)		
E-F	-1940 / 0	-119.4	-119.4	0.57 (1)	4.09	N-E	-850 / 0	0.76 (1)		
F-G	-2016 / 0	-119.4	-119.4	0.34 (1)	4.30	N-F	0 / 597	0.13 (1)		
G-H	-2125 / 0	-119.4	-119.4	0.35 (1)	4.30	L-F	0 / 253	0.06 (1)		
H-I	0 / 49	-119.4	-119.4	0.16 (1)	10.00	L-G	-216 / 0	0.15 (1)		
Q-B	-2130 / 0	0.0	0.0	0.22 (1)	5.74	K-G	-383 / 0	0.13 (1)		
J-H	-2130 / 0	0.0	0.0	0.22 (1)	5.74	B-P	0 / 1789	0.40 (1)		
Q-P	0 / 0	-18.2	-18.2	0.07 (4)	10.00	K-H	0 / 1789	0.40 (1)		
P-O	0 / 1729	-18.2	-18.2	0.33 (1)	10.00					
O-N	0 / 1582	-18.2	-18.2	0.31 (1)	10.00					
N-M	0 / 1582	-18.2	-18.2	0.31 (1)	10.00					
M-L	0 / 1582	-18.2	-18.2	0.31 (1)	10.00					
L-K	0 / 1729	-18.2	-18.2	0.33 (1)	10.00					
K-J	0 / 0	-18.2	-18.2	0.07 (4)	10.00					

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.57/0.97 (D-E:1), BC=0.33/0.97 (O-P:1),
 WB=0.76/0.97 (E-N:1), SS=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

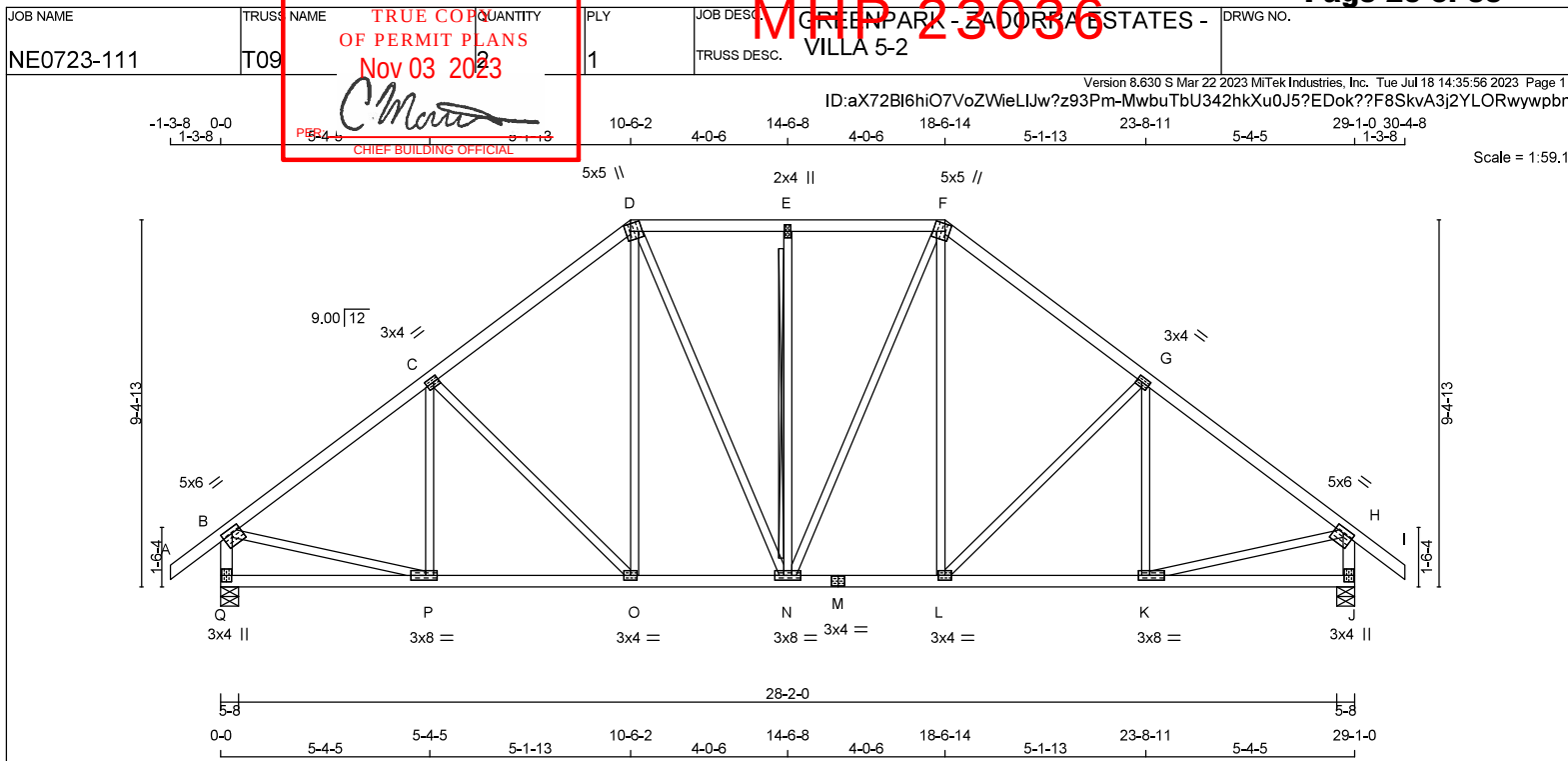
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL = 0.57 (H) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 144 = 288 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	5.0	6.0	1.50	2.75
C	TMVW4	MT20	3.0	4.0	1.50	1.50
D	TMVW+m	MT20	5.0	5.0	2.25	1.50
E	TMVW+m	MT20	2.0	4.0		
F	TMVW+m	MT20	5.0	5.0	2.25	1.50
G	TMVW4	MT20	3.0	4.0	1.50	1.50
H	TMVW4	MT20	5.0	6.0	1.50	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMVW4	MT20	3.0	8.0	1.50	3.50
L	BMVW4	MT20	3.0	4.0		
M	BS4	MT20	3.0	4.0		
N	BMVW4	MT20	3.0	8.0		
O	BMVW4	MT20	3.0	4.0		
P	BMVW4	MT20	3.0	8.0	1.50	3.50
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
Q	2167	0	2167	0
J	2167	0	2167	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
Q	COMBINED	SNOW
J	1512	1108 / 0
J	1512	1108 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO					FR-TO			
A-B	0 / 49	-119.4	-119.4	0.16 (1)	P-C	-287 / 18	0.13 (1)	
B-C	-2159 / 0	-119.4	-119.4	0.52 (1)	C-O	-402 / 0	0.43 (1)	
C-D	-1896 / 0	-119.4	-119.4	0.49 (1)	O-D	0 / 370	0.08 (1)	
D-E	-1623 / 0	-119.4	-119.4	0.26 (1)	D-N	0 / 350	0.08 (1)	
E-F	-1623 / 0	-119.4	-119.4	0.26 (1)	N-E	-577 / 0	0.38 (1)	
F-G	-1896 / 0	-119.4	-119.4	0.49 (1)	N-F	0 / 350	0.08 (1)	
G-H	-2159 / 0	-119.4	-119.4	0.52 (1)	L-F	0 / 370	0.08 (1)	
H-I	0 / 49	-119.4	-119.4	0.16 (1)	L-G	-402 / 0	0.43 (1)	
I-J	-2125 / 0	0.0	0.0	0.22 (1)	K-G	-287 / 18	0.13 (1)	
J-H	-2125 / 0	0.0	0.0	0.22 (1)	B-P	0 / 1806	0.41 (1)	
					K-H	0 / 1806	0.41 (1)	
Q-P	0 / 0	-18.2	-18.2	0.12 (4)				
P-O	0 / 1763	-18.2	-18.2	0.34 (1)				
O-N	0 / 1482	-18.2	-18.2	0.28 (1)				
N-M	0 / 1482	-18.2	-18.2	0.28 (1)				
M-L	0 / 1482	-18.2	-18.2	0.28 (1)				
L-K	0 / 1763	-18.2	-18.2	0.34 (1)				
K-J	0 / 0	-18.2	-18.2	0.12 (4)				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.52/0.97 (B-C:1), BC=0.34/0.97 (O-P:1), WB=0.43/0.97 (C-O:1), SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PSI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

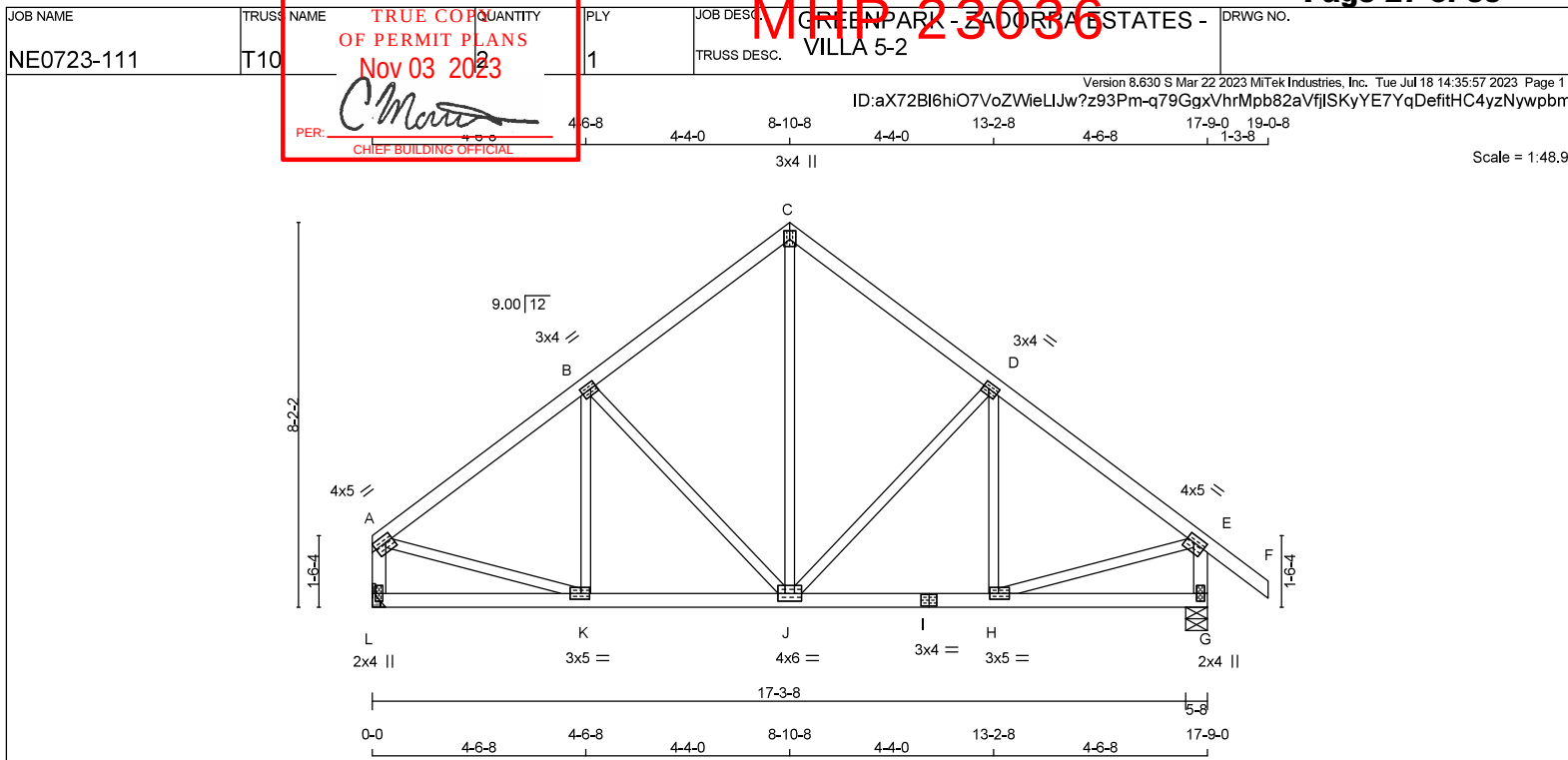
JSI GRIP= 0.89 (N) (INPUT = 0.90)

JSI METAL = 0.58 (H) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
L - A	2x4	DRY	No.2
G - E	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	4.0	5.0	1.75	Edge
B	TMVW4	MT20	3.0	4.0	1.50	1.50
C	TTV+p	MT20	3.0	4.0	2.25	1.50
D	TMVW4	MT20	3.0	4.0	1.50	1.50
E	TMVW4	MT20	4.0	5.0	1.75	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMVW4	MT20	3.0	5.0	1.50	2.25
I	BS-t	MT20	3.0	4.0		
J	BMVW4	MT20	4.0	6.0		
K	BMVW4	MT20	3.0	5.0	1.50	2.25
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
 EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	1222	0	1222	0
G	1387	0	1387	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	854	618 / 0	0 / 0	0 / 0	0 / 0	0 / 0	236 / 0	0 / 0
G	966	714 / 0	0 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH FR-TO
FR-TO	-1169 / 0	-119.4	-119.4	0.32 (1)	5.47	J-C	0 / 653
B-C	-906 / 0	-119.4	-119.4	0.31 (1)	6.04	J-D	-395 / 0
C-D	-906 / 0	-119.4	-119.4	0.31 (1)	6.04	H-D	-166 / 34
D-E	-1169 / 0	-119.4	-119.4	0.32 (1)	5.47	B-J	-395 / 0
E-F	0 / 49	-119.4	-119.4	0.16 (1)	10.00	K-B	-166 / 34
L-A	-1186 / 0	0.0	0.0	0.12 (1)	7.25	A-K	0 / 998
G-E	-1351 / 0	0.0	0.0	0.14 (1)	6.90	H-E	0 / 998
L-K	0 / 0	-18.2	-18.2	0.08 (4)	10.00		
K-J	0 / 965	-18.2	-18.2	0.19 (1)	10.00		
J-I	0 / 965	-18.2	-18.2	0.19 (1)	10.00		
I-H	0 / 965	-18.2	-18.2	0.19 (1)	10.00		
H-G	0 / 0	-18.2	-18.2	0.08 (4)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.59")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.59")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.32/0.97 (A-B:1), BC=0.19/0.97 (J-K:1),
 WB=0.28/0.97 (B-J:1), SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

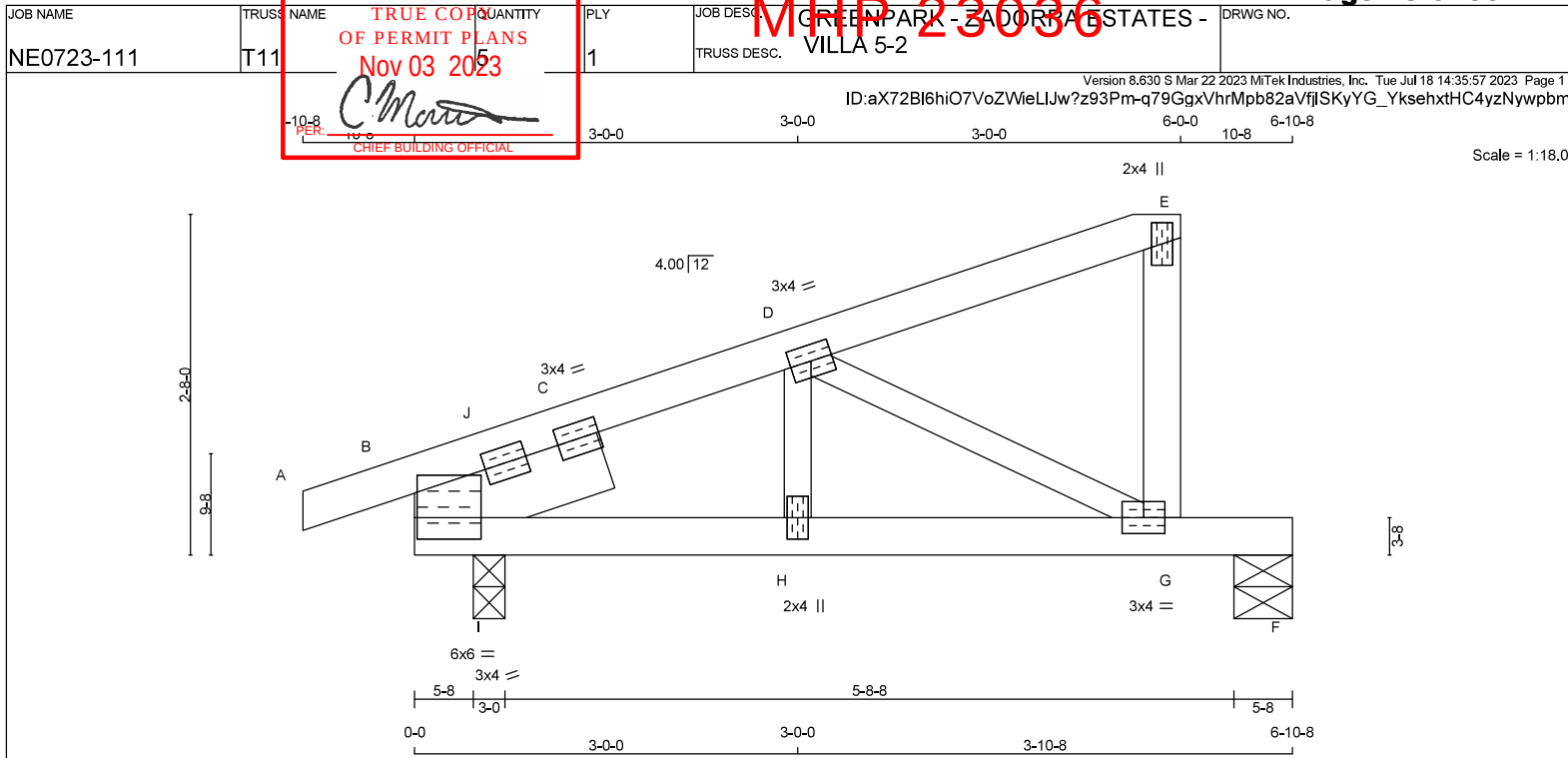
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
 JSI METAL= 0.41 (A) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
G - E	2x4	DRY	No.2
B - F	2x4	DRY	No.2

REINFORCING MEMBERS
 HW1 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	MT20	6.0	6.0	4.25	0.25
B	RT4	MT20	3.0	4.0		
B	RT4	MT20	3.0	4.0		
D	TMWW4	MT20	3.0	4.0		
E	TMV+p	MT20	2.0	4.0		
G	BMVW4	MT20	3.0	4.0		
H	BMV+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REGRD
B	577	0	577	0	0	3-0	1-8
F	375	0	375	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
B	401	300 / 0	0 / 0
F	264	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.		MAX. FACTORED	FACTORED			MEMB.		MAX. FACTORED			
		FORCE	VERT. LOAD	LC1	MAX			FORCE	MAX		
		(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)		
FR-TO			FROM	TO	LENGTH	FR-TO					
A-B	-11 / 0		-119.4	-119.4	0.05 (1)	6.25	H-D	0 / 210	0.05 (1)		
B-J	-575 / 0		-119.4	-119.4	0.02 (1)	6.25	D-G	-678 / 0	0.14 (1)		
J-C	-493 / 0		-119.4	-119.4	0.09 (1)	6.25	I-J	0 / 350	0.00 (1)		
C-D	-493 / 0		-119.4	-119.4	0.09 (1)	6.25	I-C	-411 / 0	0.03 (1)		
D-E	-5 / 0		-119.4	-119.4	0.13 (1)	10.00					
G-E	-162 / 0		0.0	0.0	0.02 (1)	7.81					
B-I	0 / 384		-18.2	-18.2	0.07 (1)	10.00					
I-H	0 / 607		-18.2	-18.2	0.20 (1)	10.00					
H-G	0 / 607		-18.2	-18.2	0.53 (1)	10.00					
G-F	0 / 0		-18.2	-18.2	0.44 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.23")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.23")
 CALCULATED VERT. DEFL.(TL) = L/886 (0.09")

CSI: TC=0.13/0.97 (D-E:1), BC=0.53/0.97 (G-H:1),
 WB=0.14/0.97 (D-G:1), SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
 JSI METAL = 0.21 (G) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



TRUE COPY
OF PERMIT PLANS
Nov 03 2023

MHP 23036

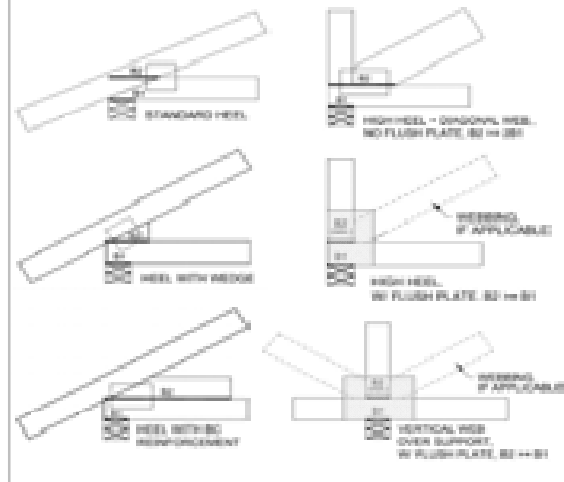
KOTT TRUSS BEARING CAPACITY TABLE

(INTERNAL USE ONLY)

TRUSS BEARING CAPACITIES (LBS.) PER INCH OF BEARING SURFACE (ED ON SPF #2 TOP PLATE) 7-Jun-21

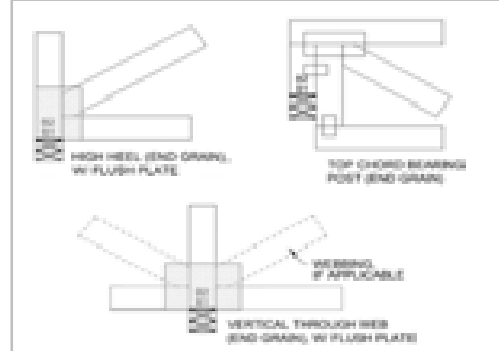
	BEARING PLATE (B1)	TYPE 1, NO FLUSH PLATE, B2=2B1							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
NO BEARING ENHANCER	1 1/2"	1383		2767		4151		5534	
	2x4	3712	3228	7425	6457	11138	9685	14851	12914
	2x6	5834	5073	11668	10146	17503	15220	23337	20293
	2x8	7690	6687	15381	13375	23072	20063	30763	26750
FLUSH PLATE	BEARING PLATE (B1)	TYPE 1, FLUSH PLATE							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
	1 1/2"	1383		2767		4151		5534	
	2x4	3712		7425		11138		14851	
BEARING ENHANCER	2x6	5834		11668		17503		23337	
	2x8	7690		15381		23072		30763	
CPn-4 (Simpson)	2x4	TYPE 1, FLUSH PLATE + BEARING ENHANCER							
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
CPn-6 (Simpson)	2x4	4515		9030		13545		18065	
	2x6	7095		14190		21285		28390	
CP4-9 (KOTT)	2x4	6007	4898	12014	9796	18021	14694	19801	19592
	2x6	8677	7075	17354	14150	26031	21225	31117	28300
SBP4 (MiTek)	2x4	7288		11001		14714		18427	
	2x6	11030		16865		22699		28534	
SBP6 (MiTek)	2x4	12886		20578		28269		35960	
	2x6								

TYPE 1 CONFIGURATIONS



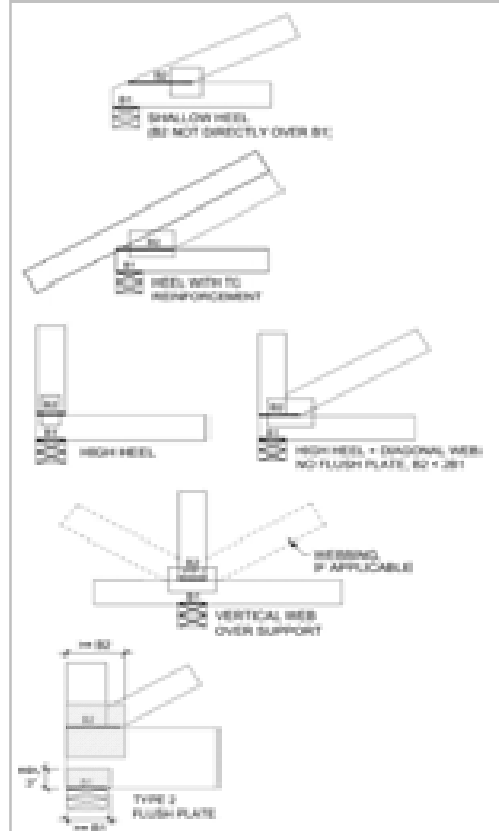
	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 1 (END GRAIN)							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)
END GRAIN	2x4	2x4	3712		7425		11138		14851	
	2x6	2x6	5834		11668		17503		23337	
			MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)
	2x8	2x8	7690		15381		23072		30763	
END GRAIN, BEARING ENHANCER	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 1 (END GRAIN), FLUSH PLATE + BEARING ENHANCER							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)
	CPn-4 (Simpson)	2x4	4515		9030		13545		18065	
	CPn-6 (Simpson)	2x6	7095		14190		21285		28390	
CP4-9 (KOTT)	2x4	2x6	15585	9006	19801	18013	19801		19801	
			MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)
CP6-9 (KOTT)	2x6	2x8	21834	13009	31117	26019	31117		31117	

TYPE 1 (END GRAIN) CONFIGURATIONS



	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, NO FLUSH PLATE, B2<2B1							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
NO BEARING ENHANCER	2x4	2x4	2639	2152	5279	4304	7919	6457	10588	8609
		2x6	3393	2767	6787	5534	10181	8302	13575	11069
	2x6	2x4	3393	2767	6787	5534	10181	8302	13575	11069
		2x6	4147	3382	8296	6764	12444	10146	16592	13529
		2x8	4808	3920	9616	7840	14424	11761	19232	15681
		2x10	5562	4535	11124	9070	16686	13606	22248	18141
	2x8	2x4	3959	3228	7919	6457	11878	9685	15838	12914
		2x6	4808	3920	9616	7840	14424	11761	19232	15681
		2x8	5467	4458	10935	8916	16403	13375	21871	17833
FLUSH PLATE	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, FLUSH PLATE							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
	2x4	2x4								
		2x6	TYPE 1 APPLIES							
	2x6	2x4	4672	3809	9344	7619	14016	11429	18689	15238
		2x6								
		2x8	TYPE 1 APPLIES							
		2x10								
	2x8	2x4	4672	3809	9344	7619	14016	11429	18689	15238
		2x6	7342	5986	14684	11973	22026	17959	29368	23946
		2x8	TYPE 1 APPLIES							

TYPE 2 CONFIGURATIONS



NOTES:

- Factored truss reaction shall not exceed bearing capacity corresponding to: configuration type, size of bearing surfaces, truss lumber, # of plies, and applicable enhancers.
- Values in table are in conformance with CSA O86-14 Cl. 6.5.7 and TPIC 2014-Update 2, and may be used for residential or commercial designs.
- Values in table are in conformance with MiTek Canada Detail B37821Q "SPF Bearing Capacities".
- Values in table are in conformance with Simpson Catalogue C-C-CAN2020.
- Conditions for use of table values include: standard duration (K=1), dry lumber (K_{dp}=1), untreated lumber (K_t=1), length of bearing factor not applied (K_b=1).
- Size factor (K_{sz}) applied to support material calculation when acceptable. Flush plate factor (K_p) applied to truss material calculation when acceptable (ie. excludes end grain).
- Flat roof factor (K_r) must applied for trusses making up a flat roof system; to do so, multiply bearing capacity values by 0.75 for this application.
- Bearing plate is to be specified by the project engineer; values in table assume a bearing material of SPF #2 (or better).
- When required, flush plate must not be located further than 1/4" away from bearing surface, and must cover the entire bearing plate length (B1).
- When required, bearing enhancer must be installed as per manufacturer's guidelines.
- Type 2 bearing configurations can be converted to use Type 1 table values as outlined in TPIC 2014-Update 1 Cl. 7.5.9.
- This table is not valid after April 30, 2022.

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	Length (in)	Diameter (in)	LATERAL Resistance per nail (Lbs.)		WITHDRAWAL Resistance per nail (Lbs.)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

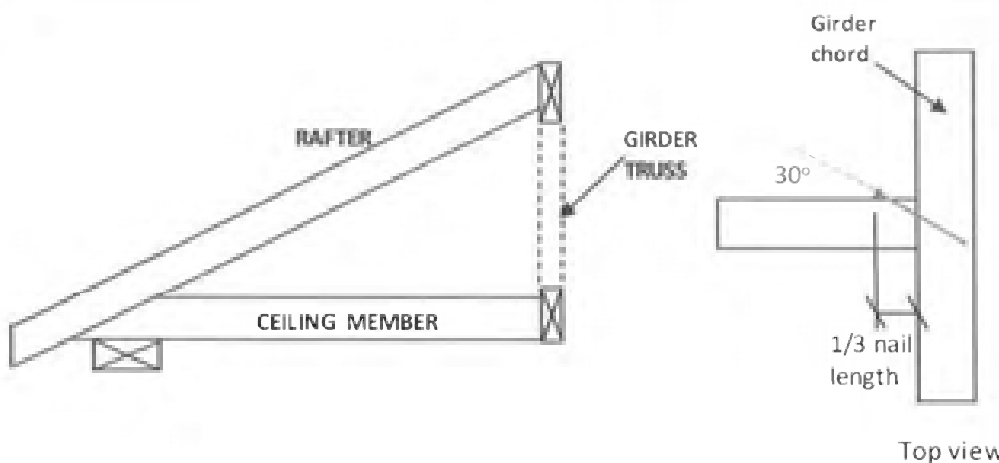
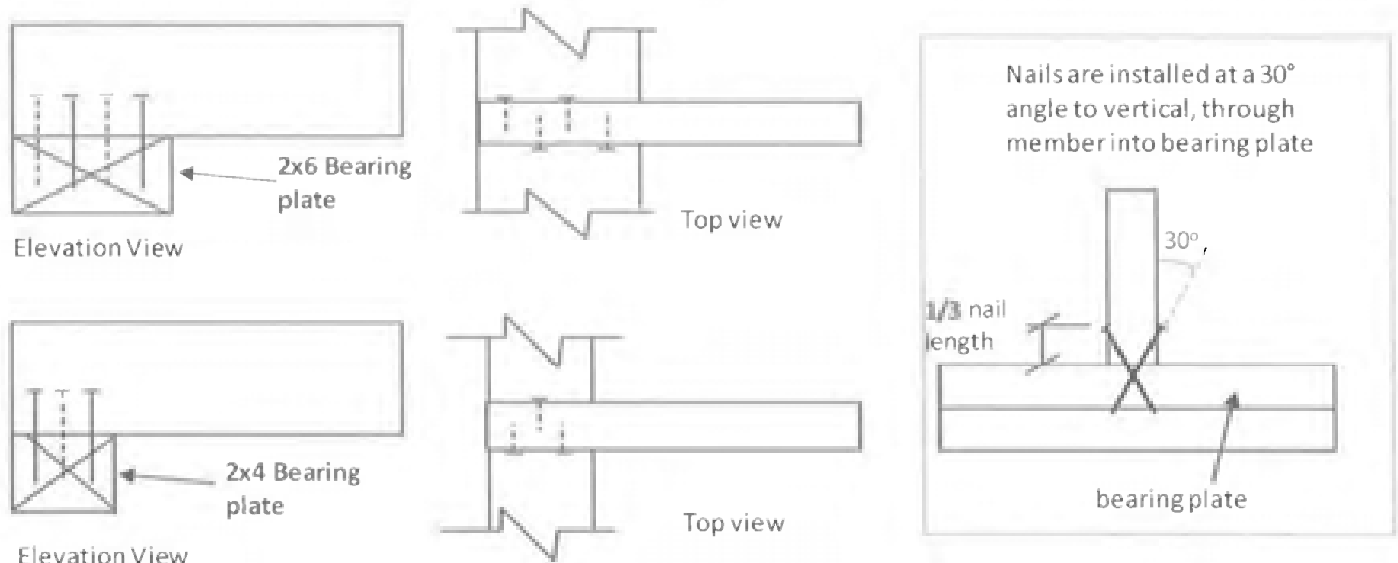


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

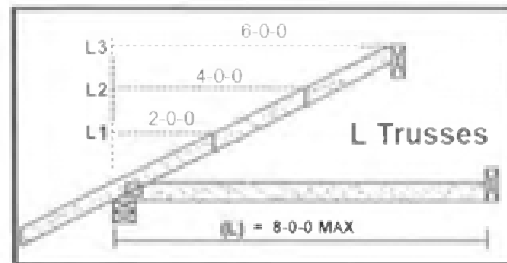
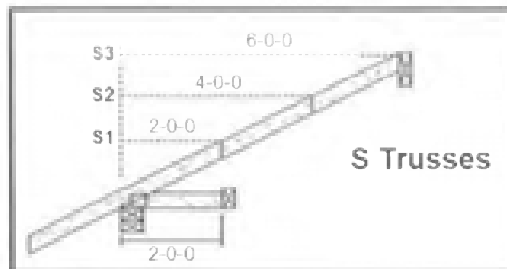
1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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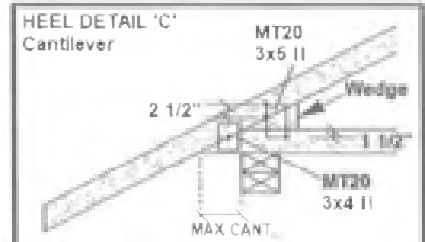
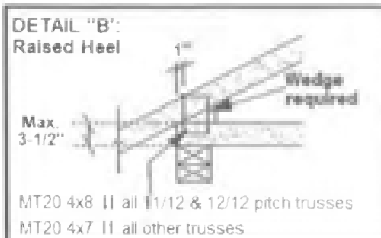
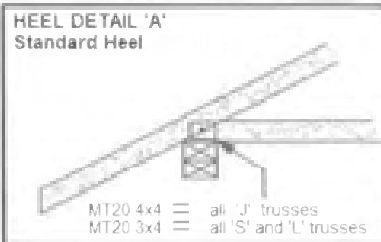
STANDARD HIP END FRAMING

PLAN VIEW



Specified Load Rating:

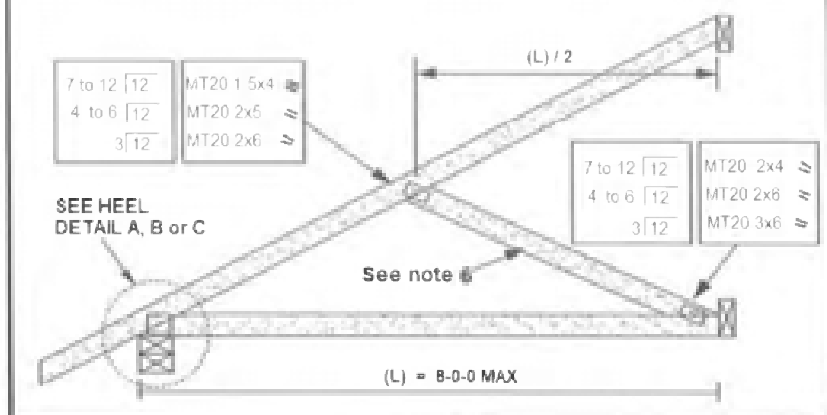
Top chord Live:	51.0 PSF or less
Top chord Dead:	6.0 PSF or less
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF or less



CANTILEVER DETAIL "C"

SLOPE	MAX CANT.	WEDGE PLATE	WEDGE SIZE
3/12	17"	3 x 5	2 x 3
4/12	14"	3 x 5	2 x 3
5/12	12"	3 x 5	2 x 4
6/12	10"	3 x 5	2 x 4
7/12	9"	3 x 5	2 x 5
8/12	8 1/2"	3 x 5	2 x 6
9/12	8"	3 x 5	2 x 6
10/12	7 1/2"	3 x 5	2 x 6

J Trusses



NOTES:

1. This detail is valid only for projects conforming to **PART 9 NBCC 2015** that do not require a wind analysis to be incorporated into the design of the trusses.
2. Overhang length shall not exceed 24 inches.
3. All lumber shall be 2x4 SPF (or D-Fir) DRY No. 2 grade or better.
4. All plates specified are MITEK MT20, pressed into both faces of each truss. Heel plates of all trusses shall conform to heel details 'A', 'B' or 'C'.
5. Diagonal hip rafter design shall conform to section 9.23.14.6 of NBCC 2015.
6. For 6.0 ft. or less span, diagonal web on truss 'J' is optional. Girder design must reflect choice of partial jack ('J' with diagonal web) or open jack ('J' without diagonal web)
7. All truss-to-rafter and truss-to-truss connections shall be specified as per MITEK standard detail 'MSD2015-H: Toe-Nail Capacity Details'

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STANDARD GABLE END DETAIL

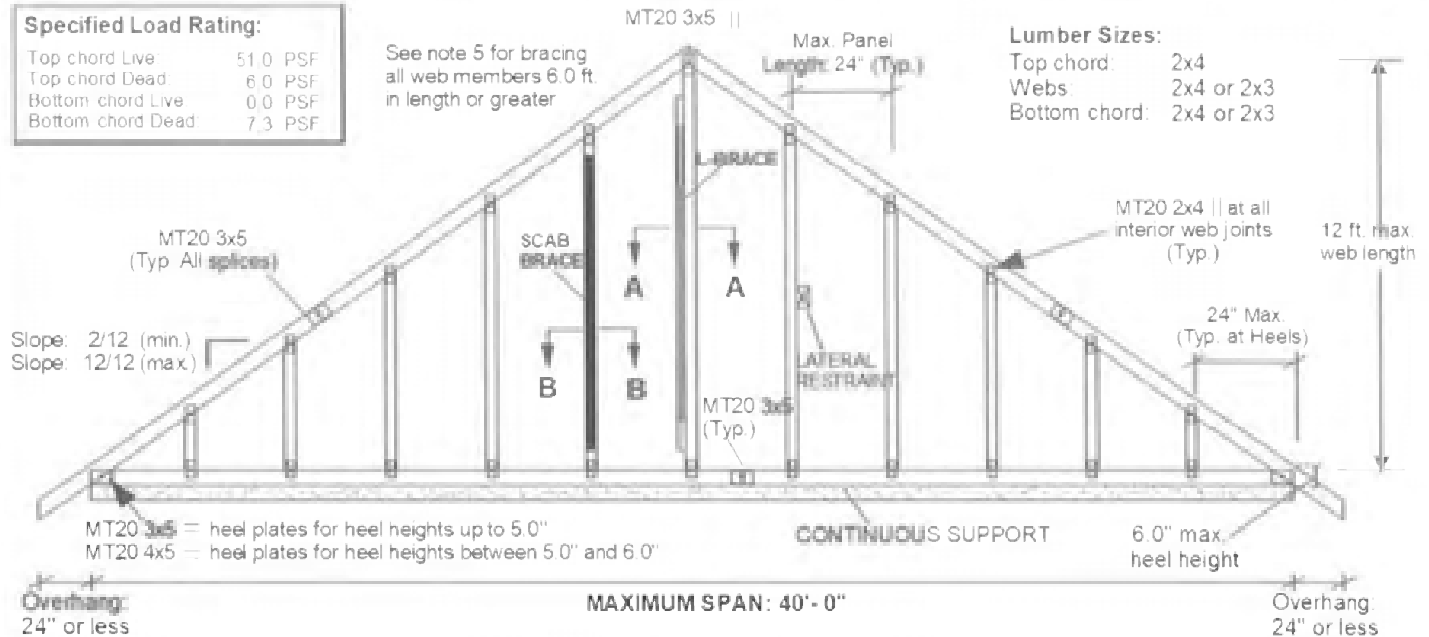
Specified Load Rating:

Top chord Live	51.0 PSF
Top chord Dead	6.0 PSF
Bottom chord Live	0.0 PSF
Bottom chord Dead	7.3 PSF

See note 5 for bracing
all web members 6.0 ft.
in length or greater

Lumber Sizes:

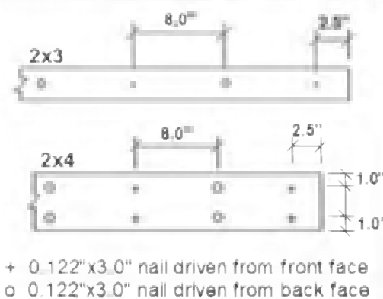
Top chord:	2x4
Webs:	2x4 or 2x3
Bottom chord:	2x4 or 2x3



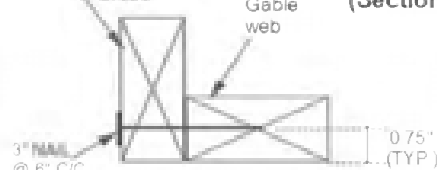
SCAB BRACE DETAIL (Section B-B)

Gable Web

SPF No. 2 DRY Scab, same size as web. Scab brace must cover 90% of web length



L BRACE DETAIL (Section A-A)

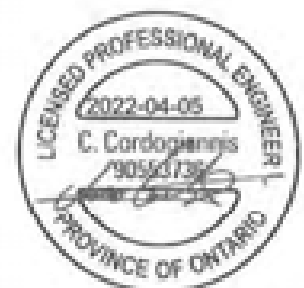


Fasten L-Brace to narrow edge of web with one row of 0.122" x 3.0" nails spaced at 6.0" c/c along entire length of web. Brace must cover 90% of the web length. Respect a 2.5" minimum end distance.

Notes:

1. This detail is only valid for projects conforming to **Part 9, NBCC 2015** that do not require a wind analysis to be incorporated into the design of the truss.
2. This detail is for vertical (gravity) load rating of the truss only. Truss must be continuously supported over the entire length of bottom chord.
3. Maximum web length not to exceed 12.0 ft. Spacing of gable stud webs in the truss not to exceed 24 inches cc.
4. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint.
5. Lateral restraint required at half-length of all webs over 6.0 ft. long. Alternatively install an L-Brace or scab brace as shown above. Scab braces shall be limited to 10 ft. long webs or less.
6. All plates are MITek MT20 pressed into both faces of truss.
7. All lumber to be SPF (or D-Fir) DRY and of No.2 grade or better.
8. Additional building bracing is typically installed to brace the face of the end wall assembly. See BCSI Canada 'Building Designer Responsibilities for Gable End Frame Bracing' for additional information on building bracing for gable-end assemblies.

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Groups to See *Young Women*
Groups pour *jeunes femmes*

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with groups of four (russes and all russes are set)

quatre fermes jusqu'à ce que toutes les fermes soient installées.

Reste in BCS1-B2C* for more information

Rejoignez-vous au BCS1-B2C* pour obtenir plus de renseignements

Restraint/Bracing for All Planes of Tension
Renfou et contreventement pour tous les plans de tension

Minimum modulus (used for lateral restraint and diagonal bracing) is 245 in⁴ (32.1 dm⁴) for 30' (9.14 m) and 300 in⁴ (39.2 dm⁴) for 36' (10.97 m).

Minimum modulus (utilisé pour le contreventement latéral et le contreventement diagonal) est de 245 in⁴ (32.1 dm⁴) pour 30' (9.14 m) et de 300 in⁴ (39.2 dm⁴) pour 36' (10.97 m).

This restraint and bracing method is for all business groups 302 and 402 next column for temporary restraint and bracing of PCFA.

Cette méthode de renfou et de contreventement correspond à toutes les catégories 302 et 402 (NFP). Reportez-vous à la partie supérieure de la colonne temporaire des Renfous.

— MEMBRE SUPERIEURE

Up to 30' (9.14 m) 10' (3.05 m)

30° to 45° (9.3m to 13.7m)	8 00 m (2.4m) c/c
45° to 60° (13.7m to 18.3m)	8 00 m (1.8m) c/c
60° to 90° (18.3m to 24.4m)	4 00 m (1.2m) c/c

[illegible]

Figure 1 is a schematic diagram of a human eye. At the top, a yellow circle represents the sun. A line representing a light ray from the sun passes through a lens and focuses on the retina. Labels include 'Sun', 'Lens', 'Retina', and 'Image of Sun'.

3) BOTTOM CORD - MEMBRANE

OM, LA RETENUE ET LE
e confortablement penché

Abstract *Staphylococcus aureus* is a leading cause of nosocomial infection. The purpose of this study was to determine the prevalence of *S. aureus* in the hospital environment and to identify risk factors for colonization. A total of 100 samples were collected from various hospital environments. The results showed that 60% of the samples were positive for *S. aureus*. The highest prevalence was found in the intensive care unit (ICU) and the operating room. Risk factors for colonization included contact with the ICU, contact with the operating room, and contact with medical equipment.

Les arbres doivent être taillés au plus tard le 15 mai.

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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Q (lb) (lb/ton)	Max. Bow (lb/ton)	Truss Load (lb/ton)
1 (1.000)	31.8 (7.16)	19.7 (4.43)
2 (2.000)	27.6 (6.18)	18.5 (4.15)
3 (3.000)	17.2 (3.87)	16.7 (3.73)
4 (4.000)	16.75 (3.75)	15.6 (3.49)
5 (5.000)	16.0 (3.60)	13.8 (3.09)
6 (6.000)	1.17 (0.26)	12.4 (2.79)
7 (7.000)	3.0 (0.68)	11.1 (2.49)
8 (8.000)	7.0 (1.56)	9.7 (2.19)
9 (9.000)	1.0 (0.22)	8.3 (1.86)
10 (10.000)	1.0 (0.22)	6.3 (1.41)

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[illegible]

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