



ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/W WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6". DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB IN	FORMATION
Customer	GREENPARK HOMES
Job #	23-00113R0
Address	ZADORRA ESTATES ROSE 3 EL 2 OSHAWA,ON
Model	ROSE 3 EL 2
Sales Rep	RALPH MIRIGELLO
Designer	BB
Date	5/31/2023
Path	S:\DESIGN\KLU\CUSTOMERS\GREENPARK\ZADORRA ESTATES\MODELS\ROSE 3\ROSE 3-2\T-ROSE

DESIGN	DESIGN INFORMATION						
Code	NBCC 2015						
Bldg	Residential - HSB (NBCC Part 9)						
TC LL	23.3 lb/ft²						
TC DL	6.0 lb/ft²						
BC LL	0.0 lb/ft ²						
BC DL	7.3 lb/ft²						
Deflection	LL=L/360 TL=L/360						
Spacing	24" O/C unless otherwise						
Spacing	noted						
Complies With	OBC 2012 (2019 Amendment) CSA 086-14 and TPIC 2014						

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.

14 Anderson Blvd. Uxbridge, ON 905.642.4400







PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

RESPONSIBILITIES

THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON CALCULATION PAGE. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER.

IT IS THE RESPONSIBILITY OF KOTT Inc. TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

DESIGN INFORMATION

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN.

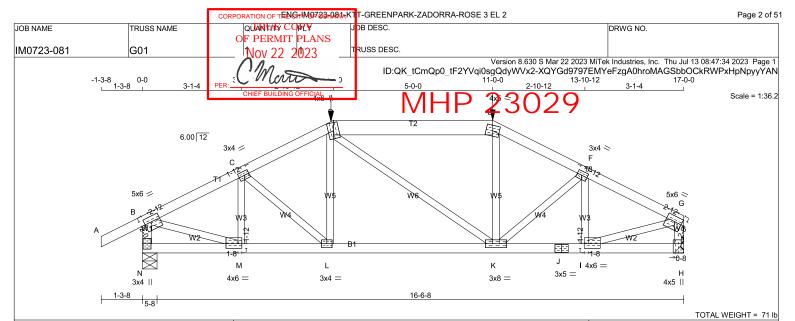
- 1. THE BUILDING USE AND OCCUPANCY TYPE IS AS INDICATED ON THE DRAWING.
- 2. GEOMETRY OF THE TRUSS AND DIMENSIONS INDICATED ON THE DRAWING ARE IDENTICAL TO THOSE OF THE INSTALLED TRUSS.
- 3. THE TRUSS LOADING INTENSITY AND DISTRIBUTION AS WELL AS LOAD TRANSFER MECHANISM IS THAT INDICATED ON THE DRAWING. NO BUILDINGS, TREES, PARAPETS OR OTHER PROJECTIONS HIGHER THAN THE ROOF FOR WHICH THE TRUSSES ARE USED ARE LOCATED WITHIN A DISTANCE LESS THAN TEN (10) TIMES THE DIFFERENCE IN HEIGHT, OR FIVE METERS (16 FT) WHICHEVER IS GREATER, UNLESS THE DRAWING INDICATES THAT THE SNOW DRIFTING HAS BEEN TAKEN INTO ACCOUNT.
- 4. THE TRUSSES ARE TO BE SUPPORTED AT THE BEARING POINTS INDICATED AND ANCHORED TO THE SUPPORTS WHERE CONSIDERED NECESSARY BY THE DESIGNER OF THE OVERALL STRUCTURE. BEARING SIZES SHOWN ARE THE MINIMUM REQUIRED TO PREVENT CRUSHING OF THE TRUSS MEMBERS AND DO NOT NECESSARILY TAKE INTO ACCOUNT STABILITY OF THE OVERALL BUILDING STRUCTURE. ELEVATION OF BEARINGS MUST BE CAREFULLY CHECKED AND SHIMMED TO ALIGNMENT FOR SOLID BEARINGS. ADEQUATE WOOD TRUSS BEARING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER.

CODE

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING, THE ONTARIO BUILDING CODE, TPIC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

HANDLING, INSTALLATION AND BRACING

- 1. THE TRUSSES MUST BE HANDLED AND INSTALLED BY A QUALIFIED PROFESSIONAL AS PER THE SUPPLIED DOCUMENT TITLED INFORMATION FOR TRUSS INSTALLERS AND THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS.
- 2. THE COMPRESSION CHORDS ARE LATERALLY BRACED BY CONTINUOUS RIGID DIAPHRAGM SHEATHING OR AS SPECIFIED ON THE DRAWING.
- 3. TEMPORARY AND PERMANENT BRACING MUST BE INSTALLED AS INDICATED ON THE TRUSS DRAWING AND ACCORDING TO THE BCSI-B1 AND BCSI-B3 SUMMARY SHEETS. BRACING FOR THE LATERAL STABILITY OF THE TRUSS IS TO BE PROVIDED BY THE BUILDING DESIGNER.
- 4. IT IS RECOMMENDED THAT A PROFESSIONAL ENGINEER'S ADVICE BE OBTAINED FOR THE BRACING OF TRUSSES SPANNING MORE THAN 12.37M (40'-7").



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EVOEDT				I

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JI	TYPE	PLATES	VV	LEN	Y X	
В	TMVW-t	MT20	5.0	6.0	2.00 2.75	
С	TMWW-t	MT20	3.0	4.0	1.50 1.75	
D	TTWW+m	MT20	4.0	6.0		
Ε	TTW-m	MT20	4.0	5.0		
F	TMWW-t	MT20	3.0	4.0	1.50 1.75	
G	TMVW-t	MT20	5.0	6.0	2.00 2.75	
Н	BMV1+t	MT20	4.0	5.0	Edge 0.50	
	DAMA/A/ 4	MTOO	4.0	0.0	4 7E 4 EO	

6.0 5.0 8.0 1.75 1.50 BS-t MT20 MT20 3.0 BMWWW-t BMWW-t MT20 3.0 4.0 BMWW-t 6.0 BMV1+p MT20 3.0 4.0

INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTOR	ED	MAXIMUN	/ FACTO	RED	INPUT	REQRD
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	2271	0	2271	0	0	5-8	3-14
+	2109	0	2109	0	0	MECHANIC	AL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

	151 LUASE		VIIN. COMPO	NEINT REACTION	<i>N</i> O		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Ν	1586	1154 / 0	0/0	0/0	0/0	433 / 0	0/0
Н	1476	1059 / 0	0/0	0/0	0/0	417 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

WERS

LOADING TOTAL LOAD CASES: (4)

CHORDS

ı	СП	UKUS					VV ⊏	00		
ı	MAX	. FACTORED	FACTO	RED				MAX. FACTO	RED	
ı	MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
ı		(LBS)	(Pl	_F)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
ı	FR-TO		FROM	TO		LENGTH	FR-TO			
ı	A-B	0 / 36	-119.4	-119.4	0.17(1)	10.00	M- C	-602 / 0	0.11 (1)	
ı	B- C	-2718 / 0	-119.4	-119.4	0.26(1)	3.91	C-L	0 / 203	0.05(1)	
ı	C- D	-2905 / 0	-119.4	-119.4	0.26(1)	3.78	L- D	0 / 152	0.06 (4)	
ı	D-E	-2598 / 0	-225.2	-225.2	0.57 (1)	4.28	D-K	0/2	0.00(1)	
ı	E-F	-2907 / 0	-119.4	-119.4	0.26(1)	3.78	K-E	0 / 153	0.06(4)	
ı	F- G	-2718 / 0	-119.4	-119.4	0.26(1)	3.91	K-F	0 / 205	0.05(1)	
ı	N- B	-2213 / 0	0.0	0.0	0.25(1)	5.58	I- F	-605 / 0	0.11(1)	
ı	H- G	-2051 / 0	0.0	0.0	0.23 (1)	5.78		0 / 2535	0.63 (1)	
ı							I- G	0 / 2535	0.63 (1)	
ı	N- M	0/0	-34.4	-34.4	0.09 (4)	10.00				
ı	M- L	0 / 2444	-34.4	-34.4	0.51(1)	10.00				
ı	L- K	0 / 2596	-34.4	-34.4	0.55 (1)	10.00				
ı	K- J	0 / 2443	-34.4	-34.4	0.52(1)	10.00				
ı	J- I	0 / 2443	-34.4	-34.4	0.52 (1)	10.00				
ı	I- H	0/0	-34.4	-34.4	0.09 (4)	10.00				
ı										
ı	SPECIF	TED CONCENTE	RATED LO	ADS (LI	BS)					

JT. LOC. LC1 MAX-MÀX+ FACE DIR TYPF HEEL -367 -367 11-0-0 -367 -367 FRONT VFRT TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS: 34.8 6.0 TOP CH. LL = 0.0 7.3 PSF TOTAL LOAD 48.1

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.57") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.07") ALLOWABLE DEFL.(TL)= L/360 (0.57") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.13")

CSI: TC=0.57/0.97 (D-E:1) , BC=0.55/0.97 (K-L:1) , WB=0.63/0.97 (B-M:1) , SSI=0.37/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

CONN.

C1 C1

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90) JSI METAL= 0.74 (J) (INPUT = 1.00)



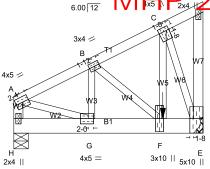


Nov 22 2023

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:47:35 2023 Page 1 ID:QK_tCmQp0_tF2YVqi0sgQdyWVx2-?d5erV8ntYUPGPYskjC4LajWk?zOxCefda1MvGyyYAM 4-8-12 1-3-4 6-0-0

2-5-10 2-3-2 0

Scale = 1:36.5



TOTAL WEIGHT = 33 lb

PSF

LUMBER				
N. L. G. A. R	RULES			
CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASO	ONED LI	JMBER.		

PL/	ATES (table	is in inches)					
JT	TYPE	PLATES	W	LEN	Υ	X	
Α	TMVW-t	MT20	4.0	5.0	2.00	2.25	
В	TMWW-t	MT20	3.0	4.0	1.50	1.75	
С	TMWW+t	MT20	4.0	5.0	1.50	1.00	
D	TMV+p	MT20	2.0	4.0			
Ε	BMVW1+t	MT20	5.0	10.0	Edge	1.50	
F	BMWW+t	MT20	3.0	10.0			
G	BMWW-t	MT20	4.0	5.0	2.00	2.00	
н	BMV1+p	MT20	2.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY	
BUILDING DESIGNER	
BEARINGS	

	FACTOR	(ED	MAXIMUN	/ FACTO	RED	INPUT	REQRD
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
T	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1255	0	1255	0	0	5-8	1-8
Ξ	2334	0	2334	0	0	MECHANIC	AL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	MIN. COMPO	NENT REACTION	NS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Н	877	635 / 0	0/0	0/0	0/0	242 / 0	0/0
E	1631	1180 / 0	0/0	0/0	0/0	451 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СН	ORDS		WEBS						
MAX	K. FACTORED	FACTORE	D				MAX. FACT	FORED	
MEMB.	FORCE	VERT. LOAD	LC1 M	AX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI	(LC)	UNBRAC	2	(LBS)	CSI ((LC)
FR-TO		FROM TO)		LENGTH	I FR-TO			
	-1210 / 0								(1)
	-1301 / 0								(4)
	-908 / 0								(1)
	-14 / 0								
E- D	-45 / 0	0.0	0.0 0.0	1 (1)	7.81	C-E	-2129 / 0	0.47	(1)
	0/0								
	0 / 1182								
F-E	0 / 799	-18.2 -	18.2 0.3	9 (1)	10.00				
SPECIF	FIED CONCENT	RATED LOAD	S (LBS)						
	LOC. LC			F/	CF [DIR	TYPF	HEEL	CONN
	4-8-12 -147						TOTAL		C1
CONNE	CTION PEOLIIR	EMENTS							

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: DI =

6.0 0.0 7.3 LL = DL = BOT CH. = 48.1 TOTAL LOAD

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip SIDE SETBACK = 0-0 END SETBACK = 6-0-0 END WALL WIDTH = 0-0 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 4-8-12 OF SPAN

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.04")

CSI: TC=0.22/0.97 (A-B:1) , BC=0.44/0.97 (F-G:1) , WB=0.50/0.97 (C-F:1) , SSI=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873



		CORPORATION OF TENGTIM0723108114K	KTT-GREENPARK-ZADORRA-ROSE 3 EL 2 Page 4 of
JOB NAME	TRUSS NAME	QUARTITY COPLY	JDB DESC. DRWG NO.
		OF PERMIT PLANS	
IM0723-081	G02	Nov 22 2023	TRUSS DESC.
	<u>.</u>		Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:47:35 2023 Page 3
			ID:QK_tCmQp0_tF2YVqi0sgQdyWVx2-?d5erV8ntYUPGPYskjC4LajWk?zOxCefda1MvGyyYAl
		PER: C WWW	
		CHIEF BUILDING OFFICIAL	
			PLATE PLACEMENT TOL. = 0.250 inches



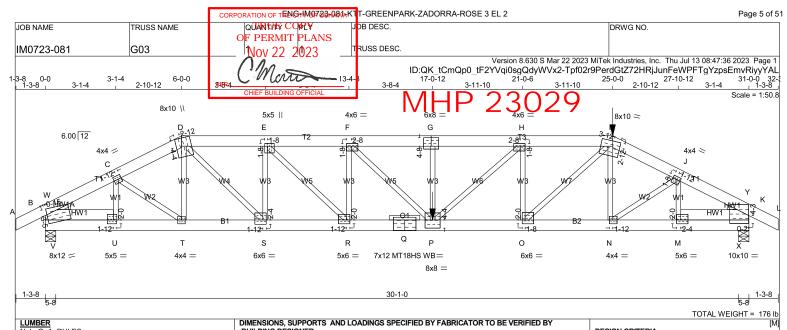
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PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)

JSI METAL= 0.47 (F) (INPUT = 1.00)



LUMBER N. L. G. A. R CHORDS A - D D - G G - I I - L B - Q	SIZE 2x6 2x6 2x6 2x6 2x6 2x6	DRY DRY DRY DRY DRY	LUMBER 2100F 1.8E 2100F 1.8E 2100F 1.8E 2100F 1.8E 2100F 1.8E	DESCR. SPF SPF SPF SPF SPF				
Q - K REINFORCIN HW1 HW2	2x6 NG MEMI 2x6 2x6	DRY BERS DRY DRY	2100F 1.8E No.2 No.2	SPF SPF SPF				
ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.								

W

LEN Y

PLATES (table is in inches) PLATES

٠.	—					
В	TMBMW1-m	MT20	8.0	12.0	3.25	1.00
С	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW+m	MT20	8.0	10.0	4.25	2.75
Е	TMWW+t	MT20	5.0	5.0	1.50	1.50
F	TMWW-t	MT20	4.0	6.0	1.50	2.50
G	TSW-I	MT20	6.0	8.0	4.50	4.00
Н	TMWW-t	MT20	4.0	6.0	1.50	2.50
1	TTWW-m	MT20	8.0	10.0	2.75	3.75
J	TMWW-t	MT20	4.0	4.0	1.50	1.75
K	TMBMW1-I	MT20	10.0	10.0	4.25	0.25
M	BMWW-t	MT20	5.0	6.0	2.00	2.25
Ν	BMWW-t	MT20	4.0	4.0	2.00	1.75
0	BMWW-t	MT20	6.0	6.0	2.00	1.50
Ρ	BMWWW-t	MT20	8.0	8.0	4.25	4.00
Q	BS-t	MT18HS	7.0	12.0		
R	BMWW-t	MT20	5.0	6.0	2.00	1.75
S	BMWW-t	MT20	6.0	6.0	2.25	1.75
Т	BMWW-t	MT20	4.0	4.0		
U	BMWW-t	MT20	5.0	5.0	2.00	1.75

WB - INDICATES BLOCKING REQUIRED



READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: TRUSSES. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

BUILDING DESIGNER

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX WED B 3777 0 3777 0 0 5-8 3-9 2x41										
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX WED B 3777 0 3777 0 0 5-8 3-9 2x41		FACTOR	ED	MAXIMUN	/ FACTO	RED	INPUT	REQRD		
B 3777 0 3777 0 0 5-8 3-9 2x41		GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG	HEEL	
D 0111 0 0 00 00 EATT	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE	
K 4766 0 4766 0 0 5-8 4-8 2x3 I	В	3777	0	3777	0	0	5-8	3-9	2x4 L	
	K	4766	0	4766	0	0	5-8	4-8	2x3 R	

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	11N. COMPO	NENT REACTION	NS .		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
В	2637	1926 / 0	0/0	0/0	0/0	710 / 0	0/0
K	3330	2412 / 0	0/0	0/0	0/0	918 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B. K BEARING SIZE FACTOR = 1.15 AT JNT(S) B, K (BASED ON SUPPORT DEPTH = 1-8)

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	HORDS				W E	BS	
MA	X. FACTORED	FACTORED				MAX. FACTO	DRED
MEMB	. FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)
FR-TO)	FROM TO		LENGTH	FR-TO		
A-B	0/0	-119.4 -119.4	0.05(1)	10.00		-994 / 0	0.17 (1)
B- W	-3999 / 0	-119.4 -119.4	0.09(1)	5.08	C-T	0 / 733	0.18 (1)
W-C	-5231 / 0	-119.4 -119.4	0.11(1)	4.55	T- D	-242 / 0	0.06(1)
C- D	-5856 / 0	-119.4 -119.4	0.12(1)	4.34	D-S	0 / 3477	0.86 (1)
D- E	-7685 / 0	-119.4 -119.4	0.19(1)	3.81	S-E	-2460 / 0	0.60(1)
E-F	-9687 / 0	-119.4 -119.4	0.27(1)	3.37	E-R	0 / 2839	0.70(1)
F- G	-11124 / 0	-119.4 -119.4	0.34(1)	3.09	R-F	-1861 / 0	0.46 (1)
G- H	-11114 / 0	-225.2 -225.2	0.41(1)	3.02	F-P	0 / 2038	0.50(1)
H- I	-9550 / 0	-225.2 -225.2	0.33(1)	3.33	P- G	-619 / 0	0.15(1)
I- J	-7564 / 0	-119.4 -119.4	0.17(1)	3.84	P- H	0 / 2140	0.53(1)
J-Y	-6715 / 0	-119.4 -119.4	0.16(1)	4.05	O- H	-2439 / 0	0.60(1)
Y-K	-5101 / 0	-119.4 -119.4	0.12(1)	4.58	O- I	0 / 3819	0.95 (1)
K-L	0/0	-119.4 -119.4	0.05(1)	10.00	N- I	-353 / 25	0.09(1)
						0 / 1000	0.25 (1)
B- V	0 / 1795	-18.2 -18.2				-1226 / 0	0.21 (1)
V- U	0 / 1795		0.14 (1)			0 / 65	0.00(1)
U- T	0 / 4647		0.29(1)			0 / 2998	0.39 (1)
T-S	0 / 5228		0.32 (1)			0 / 3873	0.50 (1)
S-R	0 / 7685		0.44 (1)		X-Y	0 / 100	0.00 (1)
R-Q	0 / 9687		0.64 (1)				
Q-P	0 / 9687		0.64 (1)				
_ P- O	0 / 9550		0.63 (1)				
O- N	0 / 6755		0.40 (1)				
N- M	0 / 5963		0.36 (1)				
M- X	0 / 2279		0.17 (1)				
X-K	0 / 2279	-34.4 -34.4	0.12 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
l	25-0-0	-367	-367		FRONT	VERT	TOTAL		C1
Ρ	17-0-12	-1631	-1631		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

34.8

SPECIFIED LOADS: TOP CH. LL =

		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE ADDT'L LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 13-11-4 OF SPAN MEASURED FROM THE RIGHT.

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT)

CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.03") CALCULATED VERT. DEFL.(TL) = L/651 (0.57")

CSI: TC=0.41/0.97 (G-H:1) , BC=0.64/0.97 (P-R:1) , WB=0.95/0.97 (I-O:1) , SSI=0.34/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



		CORPORATION OF TELNG+1M0723+084A	T-GREENPARK-ZADORRA-ROSE 3 EL 2	Page 6 of
JOB NAME	TRUSS NAME	QUANTITY COPLY OF PERMIT PLANS	JDB DESC.	DRWG NO.
IM0723-081	G03	Nov 22 2023	T <mark>R</mark> USS DESC.	
				2 2023 MiTek Industries, Inc. Thu Jul 13 08:47:36 2023 Page Fpf02r9PerdGtZ72HRjJunFeWPFTgYzpsEmvRiyyYA
		PER: CHIEF BUILDING OFFICIAL		NAII VALLIES

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
MT18HS 586 403 2455 1382 3163 3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90) JSI METAL= 0.99 (Q) (INPUT = 1.00)





RUSS DESC. Nov 22 2023 low 2-2-8

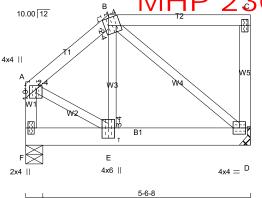
CHIEF BUILDING OFFICE

Version 8,630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:47:36 2023 Page 1 ID:QK_tCmQp0_tF2YVqi0sgQdyWVx2-Tpf02r9PerdGtZ72HRjJunFfsPJmgh1psEmvRiyyYAL

2x4)

Scale = 1:30.8

TOTAL WEIGHT = 30 lb



LUMBER N. L. G. A. RULES DESCR. SPF CHORDS LUMBER A - B B - C D - C F - A F - D DRY 2x4 No.2 2x4 2x4 DRY DRY No.2 No.2 SPF 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 SPF EXCEPT

G04

DRY: SEASONED LUMBER

IM0723-081

PL/	ATES (table	is in inches)			
JT	TYPE	PLATES	W	LEN	Υ
Α	TMVW+p	MT20	4.0	4.0	1.

TMVW+p	MT20	4.0	4.0	1.00 2.25
TTWW+m	MT20	5.0	5.0	2.25 1.25
TMV+p	MT20	2.0	4.0	
BMVW1-t	MT20	4.0	4.0	
BMWW+t	MT20	4.0	6.0	3.25 2.00
BMV1+p	MT20	2.0	4.0	
	TTWW+m TMV+p BMVW1-t BMWW+t	TTWW+m MT20 TMV+p MT20 BMVW1-t MT20 BMWW+t MT20	TTWW+m MT20 5.0 TMV+p MT20 2.0 BMVW1-t MT20 4.0 BMWW+t MT20 4.0	TTWW+m MT20 5.0 5.0 TMV+p MT20 2.0 4.0 BMVW1-t MT20 4.0 4.0 BMWW+t MT20 4.0 6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

<u> </u>	VIII VOO						
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	GROSS REACTION			BRG	BRG	
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
)	1428	0	1428	0	0	MECHANIC	CAL
=	1428	0	1428	0	0	5-8	1-9

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-9.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	998	722 / 0	0/0	0/0	0/0	276 / 0	0/0
F	998	722 / 0	0/0	0/0	0/0	276 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СН	ORDS			WEBS				
MA)	K. FACTORED	FACTORED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LOAD LO	1 MAX	MAX.	MEMB	. FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	-986 / 0	-119.4 -119.4	0.12 (1)	6.07	E-B	0 / 916	0.23(1)	
B- C	0/0	-119.4 -119.4	0.32 (1)	10.00	B- D	-1011 / 0	0.37(1)	
D- C	-226 / 0	0.0 0.0	0.04 (1)	7.81	A-E	0 / 852	0.21(1)	
F- A	-1166 / 0	0.0 0.0	0.13 (1)	7.26				
F-E	0/0	-356.7 -356.7	0.25 (1)	10.00				
E- D	0 / 785	-356 7 -356 7	0.43 (1)	10.00				



SPECIFIED LOADS:								
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSF			
TOTA	L LO	AD	=	48.1	PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 11-10-0 END DISTANCE = 6-0-0 END SPAN CARRIED = 11-10-0 END WALL WIDTH = 0-0 APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.32/0.97 (B-C:1) , BC=0.43/0.97 (D-E:1) , WB=0.37/0.97 (B-D:1) , SSI=0.50/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90) JSI METAL= 0.35 (E) (INPUT = 1.00)





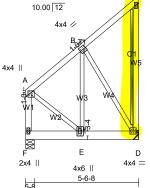
2x4

Nov 22 2023 CHIEF BUILDING OFFICIAL

Version 8,630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:47:37 2023 Page 1 ID:QK_tCmQp0_tF2YVqi0sgQdyWVx2-x?DOFBA2P9I7VjiFr8FYQ?osXphMP5yy5uWT_8yyYAK 6-0-0 3-0-0

23029

Scale = 1:61.2



TOTAL WEIGHT = 40 lb

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASO	NED L	JMBER.		

G05

PLATES (table is in inches)

IM0723-081

JΤ	TYPE	PLATES	W	LEN	Υ	Χ
Α	TMVW+p	MT20	4.0	4.0	1.00	2.00
В	TMWW-t	MT20	4.0	4.0	2.00	1.00
С	TMV+p	MT20	2.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
Е	BMWW+t	MT20	4.0	6.0	3.25	2.00
F	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

ב	ARINOS										
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD				
	GROSS RE	GROSS REACTION			BRG	BRG					
Т	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
	1428	0	1428	0	0	5-8	1-9				
)	1428	0	1428	0	0	MECHANIC	AL				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-9.

UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
F	998	722 / 0	0/0	0/0	0/0	276 / 0	0/0		
D	998	722 / 0	0/0	0/0	0/0	276 / 0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3' COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

СНС	RDS		WEBS						
MAX.	FACTORED	FACTORED				MAX. FACTO	RED		
MEMB.	FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB	. FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
F- A	-988 / 0	0.0 0.0	0.14(1)	7.74	A-E	0 / 672	0.17(1)		
A-B	-676 / 0	-119.4 -119.4	0.20(1)	6.25	E-B	0 / 859	0.21(1)		
B- C	-27 / 0	-119.4 -119.4	0.19(1)	6.25	B- D	-1007 / 0	0.51 (1)		
D- C	-137 / 0	0.0 0.0	0.04 (1)	7.81					
F-E	0/0	-356.7 -356.7	0.21 (1)	10.00					
E- D	0 / 540	-356.7 -356.7	0.28 (1)	10.00					



SPECIFIED LOADS:								
TOP	CH.	LL	=	34.8	PSF			
		DL	=	6.0	PSF			
BOT	CH.	LL	=	0.0	PSF			
		DL	=	7.3	PSF			
TOTAL LOAD = 48.1 PSF								

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 11-10-0 END DISTANCE = 6-0-0 END SPAN CARRIED = 11-10-0

END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT)

CSA 086-14 - TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")

CSI: TC=0.20/0.97 (A-B:1) , BC=0.28/0.97 (D-E:1) , WB=0.51/0.97 (B-D:1) , SSI=0.42/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90) JSI METAL= 0.26 (E) (INPUT = 1.00)





LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	2100F 1.8E	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN	Υ .	X
Α	TMVW-p	MT20	6.0	6.0	1.50	3.00
В	TMWW-t	MT20	4.0	4.0	1.50	1.00
С	TTWW-m	MT20	6.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
Е	TTWW-m	MT20	6.0	8.0	Edge	3.00
F	TTWW+m	MT20	6.0	6.0	3.00	2.25
G	TTWW+m	MT20	8.0	8.0	Edge	2.75
Н	TMWW-t	MT20	3.0	4.0	1.50	1.25
1	TMVW-t	MT20	6.0	8.0	1.50	3.25
K	BMV1+t	MT20	6.0	8.0	Edge	0.50
L	BMWW+t	MT20	6.0	8.0	4.25	2.00
M	BMWW+t	MT20	3.0	4.0		
N	BMWW+t	MT20	3.0	6.0	2.00	1.50
0	BMWW+t	MT20	4.0	4.0	1.75	1.75
Ρ	BS-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	8.0	8.0	4.25	4.00
R	BMWW+t	MT20	3.0	5.0		
S	BMWW-t	MT20	5.0	5.0	1.75	1.50
Т	BMV1+t	MT20	4.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTOR		MAXIMUN		INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Т	3601	0	3601	0	0	MECHANIC	AL
K	4208	0	4208	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT T. MINIMUM BEARING LENGTH AT JOINT T = 3-8.

UNFACTORED REACTIONS

	151 LUASE	IVIAA./	<u>IVIIN. COMPOI</u>	NEINT REACTION	VO		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2519	1809 / 0	0/0	0/0	0/0	710 / 0	0/0
K	2938	2145 / 0	0/0	0/0	0/0	793 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K BEARING SIZE FACTOR = 1.15 AT JNT(S) K (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-Q, F-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"
COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

	HORDS	FACTO	DED			W E	EBS MAX. FACT	ODED			
MEMB		VERT. LC		MAX	MAX.	MEMB					
	(LBS)			CSI (LC)			(LBS)				
FR-TO		FROM		(,		H FR-TC			()		
A-B	-2832 / 0	-119.4	-119.4	0.31(1)	3.78	8 S-B	-1495 / 0	0.58	(1)		
B- C	-3450 / 0			0.36 (1)) B-R	0 / 804	0.20			
C- D	-3717 / 0			0.69 (1)		R-C	-492 / 12	0.50			
D- E	-3716 / 0	-119.4	-119.4	0.64 (1)	3.59	0 - Q	0 / 1876	0.46	(1)		
E-F	-3945 / 0	-119.4	-119.4	0.22 (1)	3.33	3 Q-D	-1050 / 0	0.42	(1)		
F- G	-3198 / 0	-119.4	-119.4	0.25 (1)	3.63	3 Q-E	0 / 1282	0.32	(1)		
G- H	-3482 / 0			0.46(1)	3.31		0 / 1192	0.29	(1)		
H- I	-3630 / 0			0.37(1)		0- F	-1094 / 0	0.81	(1)		
l- J	0 / 53	-119.4	-119.4	0.18(1)	10.00) N- F	-1661 / 0	0.51	(1)		
T- A	-3547 / 0	0.0	0.0	0.33(1)	5.54	N- G	0 / 1609	0.40	(1)		
K-I	-4107 / 0	0.0	0.0	0.30(1)	5.17			0.09			
						M- H		0.09			
T-S	0/0	-34.4		0.04 (4)				0.04			
S-R	0 / 2195	-34.4		0.35 (1)			0 / 2699				
R-Q	0 / 2613	-34.4		0.41 (1)			0 / 3197	0.79	(1)		
Q-P	0 / 3020	-18.2		0.45 (1)							
P-O	0 / 3020	-18.2		0.45 (1)							
O- N	0 / 3226	-18.2		0.50(1)							
N- M	0 / 2657	-18.2		0.44 (1)							
M- L	0 / 2816	-18.2		0.50(1)							
L- K	0/0	-18.2	-18.2	0.13 (1)	10.00)					
SDEC	IFIED CONCENT	DATEDIO	ADS /I E	967							
JT	LOC. LC1		MAX.		ACE	DIR.	TYPE	HEEL	CONN		
C	6-0-0 -367		IVIAA			/ERT	TOTAL		CONIN		
	23-2-12 -998		-			/ERT	TOTAL		C1		
Q	11-3-4 -998		-			/ERT	TOTAL		C1		
Q	11-5-4 -330	-550	-	- 110	OINI '	V L I X I	TOTAL		O I		
CONN	CONNECTION REQUIREMENTS										

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIAL LOADS ANALYSIS ** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

TOTAL WEIGHT = 156 lb

SPECIFIED LOADS:

TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip SIDE SETBACK = 6-0-0 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE ADDT'L LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 11-3-4 OF SPAN MEASURED FROM THE LEFT.

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14 - TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.85") CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.69/0.97 (C-D:1) , BC=0.50/0.97 (N-O:1) , WB=0.81/0.97 (F-O:1) , SSI=0.59/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



		CORPORATION OF TELNG+1M0723+08/14	(T-GREENPARK-ZADORRA-ROS	E 3 EL 2	Page 10 of 51
JOB NAME	TRUSS NAME	QUANTITY COPLY OF PERMIT PLANS	JDB DESC.		DRWG NO.
IM0723-081	G06	Nov 22 2023	TRUSS DESC.		
		PER: CMorto	ID:QK_tCmQp0_tF2Y		MiTek Industries, Inc. Thu Jul 13 08:47:38 2023 Page 2 ATt 7sHRPrmnzCLwWC 48Uc5JYF0WayyYAJ
		CHIEF BUILDING OFFICIAL	¹ MHP	23029	NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (A) (INPUT = 0.90) JSI METAL= 0.85 (L) (INPUT = 1.00)



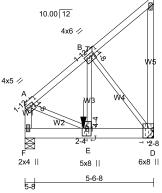


Nov 22 2023 RUSS DESC CHIEF BUILDING OFFICIAL

Version 8,630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:47:39 2023 Page 1 ID:QK_tCmQp0_tF2YVqi0sgQdyWVx2-uOL9gsBlxm?rk0sdzZH0VQtDJcJotzPFYC?Z21yyYAI 3-0-0 6-0-0 3-0-0

10.00 12 4x6 // Scale = 1:53.7

Page 11 of 51



TOTAL WEIGHT = 2 X 36 = 73 lb

JLES			
SIZE		LUMBER	DESCR.
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x6	DRY	No.2	SPF
	551		
		No.2	SPF
INED L	UIVIBER.		
	2x4 2x4 2x4 2x6 2x3	SIZE 2x4 DRY 2x4 DRY 2x4 DRY 2x4 DRY 2x6 DRY	SIZE LUMBER 2x4 DRY No.2 2x4 DRY No.2 2x4 DRY No.2 2x6 DRY No.2 2x3 DRY No.2

G07

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

IM0723-081

CHORL	S #ROWS	SURFACE	LOAD(PLF)						
		SPACING (IN)							
TOP CH	TOP CHORDS: (0.122"X3") SPIRAL NAILS								
F- A	1	12	TOP						
A- C	1	12	SIDE(45.9)						
C- D	1	12	TOP						
вотто	M CHORDS	: (0.122"X3") SPIRAL NAILS							
F- D	2	6	SIDE(396.5)						
WEBS:	WEBS: (0.122"X3") SPIRAL NAILS								
2x3	1	6							

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING. PATTERN SHALL BE CAPABLE OF TRANSFERING.
REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN	Y X	
Α	TMVW-t	MT20	4.0	5.0	1.50 1.75	
В	TMWW-t	MT20	4.0	6.0	1.50 1.75	
С	TMV+p	MT20	2.0	4.0		
D	BMVW1+t	MT20	6.0	8.0	Edge 2.50	
Ε	BMWW+t	MT20	5.0	8.0	4.25 2.25	
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	KINGS						
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
=	3120	0	3120	0	0	5-8	1-11
)	4103	0	4103	0	0	MECHANIC	:AI

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	151 LCASE	IVIAX./N	<u>/IIN. COMPO</u>	NENT REACTION	15		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2180	1578 / 0	0/0	0/0	0/0	603 / 0	0/0
D	2867	2075 / 0	0/0	0/0	0/0	793 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СН	ORDS		WEBS						
MAX	. FACTORED	FACTORED	FACTORED			MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB	. FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
F- A	-3080 / 0		0.18 (1)		A- E	0 / 2417	0.30 (1)		
A- B	-2879 / 0	-238.9 -238.9				0 / 3995	0.49 (1)		
	-35 / 0	-119.4 -119.4			B- D	-3567 / 0	0.64 (1)		
D- C	-124 / 0	0.0 0.0	0.05 (1)	7.81					
F-E	0/0	-36.5 -36.5							
E- D	0 / 2238	-811.3 -811.3	0.47 (1)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-8-12	-2519	-2519		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 2-8-12 START SPAN CARRIED = 25-6-0 END DISTANCE = 6-0-0 END SPAN CARRIED = 25-6-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 6-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 2-8-12 OF SPAN MEASURED FROM THE LEFT.

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 , NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.18/0.97 (A-B:1) , BC=0.47/0.97 (D-E:1) , WB=0.64/0.97 (B-D:1) , SSI=0.41/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



		CORPORATION OF TENGTIM0723-081AK	T-GREENPAR	RK-ZADORRA-ROS	E 3 EL 2	Page 12 o
JOB NAME	TRUSS NAME	QUANTITY COPLY OF PERMIT PLANS	JDB DESC.			DRWG NO.
IM0723-081	G07	Nov 22 2 0 23	TRUSS DESC.			
		7. A == ====			Version 8.630 S Mar 22 202	3 MiTek Industries, Inc. Thu Jul 13 08:47:39 2023 Page
				ID:QK_tCmQp0_		9gsBlxm?rk0sdzZH0VQtDJcJotzPFYC?Z21yyY
		PER:				
		CHIEF BUILDING OFFICIAL			22020	NAIL VALUES
			_ [MHP	23029	PLATE GRIP(DRY) SHEAR SECTION
			•	V	2002,	(PSI) (PLI) (PLI)
						MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.86 (B) (INPUT = 0.90) JSI METAL= 0.44 (A) (INPUT = 1.00)

I.MATIJEVIC 100528832

NCE OF ON



RULES			
SIZE		LUMBER	DESCR.
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x6	DRY	No.2	SPF
2x4	DRY	No.2	SPF
2x8	DRY	No.2	SPF
2x6	DRY	No.2	SPF
2x3	DRY	No.2	SPF
	2x4 2x4 2x4 2x4 2x4 2x6 2x4 2x8 2x6	SIZE 2x4 DRY 2x6 DRY 2x6 DRY 2x8 DRY 2x8 DRY	SIZE LUMBER 2x4 DRY No.2 2x6 DRY No.2 2x6 DRY No.2 2x8 DRY No.2 2x8 DRY No.2 2x8 DRY No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORD	S #ROWS	SURFACE	LOAD(PLF)
		SPACING (IN)	
TOP CH	ORDS : (0.1	122"X3") SPIRAL	NAILS
A- B	1	12	TOP
B- D	1	12	TOP
D- E	1	12	TOP
E- G	1	12	TOP
G- J	1	12	TOP
K-I	1	12	TOP
T- A	2	12	TOP
BOTTOM	M CHORDS	: (0.122"X3") SP	IRAL NAILS
T-O	2	12	SIDE(61.0)
O-K	2	12	TOP
WEBS:	(0.122"X3")	SPIRAL NAILS	
242	· 4	e	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

30-6-8

	FACTOR	ED	MAXIMUN	/ FACTO	RED	INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Т	6124	0	6124	0	0	MECHANIC	AL
K	3067	0	3067	0	0	5-8	1-11

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT T. MINIMUM BEARING LENGTH AT JOINT T = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./N	IIN. COMPO	NENT REACTION	NS .		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Т	4280	3096 / 0	0/0	0/0	0/0	1183 / 0	0/0
K	2141	1563 / 0	0/0	0/0	0/0	577 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

6x8 ||

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-P. E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3' COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

	ORDS					W E	BS	
MAX	(. FACTORED	FACTO	RED				MAX. FACTO	DRED
MEMB.	FORCE	VERT. LC	AD LC1	1 MAX				MAX
	(LBS)	(PL	_F) (CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A- B	-4976 / 0	-119.4	-119.4	0.31(1)	4.04	S-B	-551 / 0	0.18 (1)
B- C	-4576 / 0	-119.4	-119.4	0.21(1)	4.26	B-R	0 / 3577	0.44 (1)
C- D	-4578 / 0	-119.4	-119.4	0.31(1)	4.17	R- C	-458 / 0	0.15 (1)
D- E	-4804 / 0	-119.4	-119.4	0.13(1)	4.27	R- D	-571 / 0	0.35(1)
E-F	-3308 / 0	-119.4	-119.4	0.35(1)	4.71	Q- D	0 / 226	0.03 (1)
F- G	-3308 / 0	-119.4	-119.4	0.35(1)	4.71	D-P	-3494 / 0	0.70(1)
G- H	-3139 / 0	-119.4	-119.4	0.23(1)	4.97	P-E	0 / 3369	0.42(1)
H- I	-3041 / 0	-119.4	-119.4	0.23(1)	5.03	E- N	-798 / 0	0.39(1)
I- J	0 / 53	-119.4	-119.4	0.09(1)	10.00	N- F	-726 / 0	0.50(1)
T- A	-6052 / 0	0.0	0.0	0.35(1)	5.94	N- G	0 / 1746	0.22(1)
K- I	-3016 / 0	0.0	0.0	0.17(1)	6.56	M- G	0 / 87	0.02(4)
						M- H	0 / 19	0.00(1)
T-S	0/0	-140.2	-140.2	0.22(1)	10.00	L- H	-644 / 0	0.12(1)
S-R	0 / 3803	-140.2	-140.2	0.44(1)	10.00	A-S	0 / 4623	0.57 (1)
R-Q	0 / 4934	-18.2	-18.2	0.50(1)	10.00	L- I	0 / 2461	0.30(1)
Q-P	0 / 4931	-18.2	-18.2	0.30(1)	10.00			
P-O	0 / 3734	-18.3	-18.3	0.27 (1)	10.00			
O- N	0 / 3734	-18.2	-18.2	0.27(1)	10.00			
N- M	0 / 2378	-18.2	-18.2	0.18 (1)	10.00			
M- L	0 / 2364	-18.2	-18.2	0.17(1)	10.00			
L- K	0/0	-18.2	-18.2	0.03 (1)	10.00			
SDECIE	IED CONCENT	DATEDIO	VD6 (11	86/				
OF ECIF	SPECIFIED CONCENTRATED LOADS (LBS)							

FACE

DIR

TYPE

HEEL

CONN

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

MAX+

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

TOTAL WEIGHT = 2 X 192 = 385 II

SPECIFIED LOADS:

34.8 DI = 6.0 PSF LL = DL = 0.0 7.3 BOT CH. PSF TOTAL LOAD = 48.1

1-3-8 5-8

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 6-0-0 END DISTANCE = 5-4-7 END SPAN CARRIED = 6-0-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD.

- ADDT'L LOADS BASED ON 55 % OF GSL.

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT) CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (1.03") CALCULATED VERT. DEFL.(LL)= L/999 (0.11") ALLOWABLE DEFL.(TL)= L/360 (1.03") CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.35/0.97 (A-T:1) , BC=0.50/0.97 (Q-R:1) , WB=0.70/0.97 (D-P:1) , SSI=0.46/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
650 371 1747 788 1987 1873



CORPORATION OF TENGTIMO7031-0814K1 T-GREENPARK-ZADORRA-ROSE 3 EL 2 Page 14 of 51 QUANTITY COPY OF PERMIT PLANS JOB NAME TRUSS NAME DB DESC. DRWG NO. IM0723-081 G08 Nov 22 2023 RUSS DESC. Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 13:31:15 2023 Page 2 ID:QK_tCmQp0_tF2YVqi0sgQdyWVx2-f_WvzD7qexqTA0hznDZK9le4OssFlnzsNZv_HlyyU0Q

CHIEF BUILDING OFFICIAL

PLATES (table is in inches)
JT TYPE PLATES
A TMVW+p MT20 LEN Y
6.0 Edg
8.0 2.00
4.0
8.0 2.50
6.0 2.00
4.0
6.0 2.25
5.0 1.50
5.0 1.50
4.0
6.0 2.00
4.0
6.0 1.75 Y X Edge 2.00 2.25 6.0 6.0 2.0 6.0 5.0 2.0 TTWW+m TMW+w TTWWW-m MT20 MT20 MT20 2.50 4.00 2.00 2.25 TTWW-m TMW+w TTWW+m TMWW-t TMVW-t MT20 MT20 2.25 1.50 1.50 2.25 1.50 1.75 MT20 MT20 MT20 MT20 5.0 3.0 4.0 BMV1+p BMWW-t MT20 MT20 2.00 2.00 BMWW+t BMWWW-t MT20 MT20 1.75 3.00 8.0 5.0 2.25 4.0 2.25 8.0 4.00 6.0 2.50 10.0 7.25 6.0 4.0 2.0 6.0 5.0

2.25 1.75 2.25 1.00 4.00 2.00 2.50 2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

4.0

MT20

MT20 MT20 MT20 MT20

MT20

BS-t

BMWW+t BMW+w BMWWW+t BMWW-t

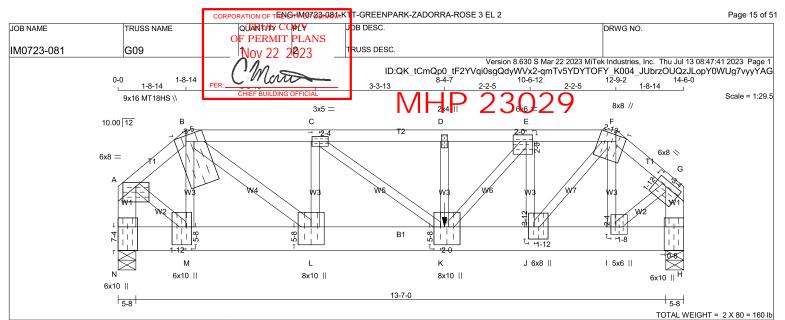
BMV1+t

MHP 23029

PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (H) (INPUT = 0.90) JSI METAL= 0.47 (I) (INPUT = 1.00)







LUMBER	LUMBER									
N. L. G. A. R	ULES									
CHORDS	SIZE		LUMBER	DESCR.						
A - B	2x4	DRY	No.2	SPF						
B - F	2x4	DRY	No.2	SPF						
F - G	2x4	DRY	No.2	SPF						
N - A	2x6	DRY	No.2	SPF						
H - G	2x6	DRY	No.2	SPF						
N - H	2x8	DRY	1950F 1.7E	SPF						
ALL WEBS	2x3	DRY	No.2	SPF						
B - L	2x4	DRY	No.2	SPF						
DRY: SEASO	ONED LU	JMBER.								

DESIGN CONSISTS OF <u>2</u> TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

011000	0 11001110	011054.05	LOAD(DLE)
CHORD	S #ROWS	SURFACE	LOAD(PLF)
		SPACING (IN)	
TOP CH	IORDS: (0.	122"X3") SPIRAL NAIL	S
A-B	1 `	12	TOP
B- F	1	12	TOP
F- G	1	12	TOP
N- A	2	12	TOP
H- G	2	12	TOP
BOTTO	M CHORDS	: (0.122"X3") SPIRAL	NAILS
N- H	2	5	SIDE(491.2)
WEBS:	(0.122"X3")	SPIRAL NAILS	
2x3	· 1	6	
2×4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN	Y X
Α	TMVW-p	MT20	6.0	8.0	Edge
В	TTWW+m	MT18HS	9.0	16.0	Edge 3.25
С	TMWW-t	MT20	3.0	5.0	1.50 2.25



READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: TRUSSES. THE NOTE PAGE** IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

טבתו	tii too						
	FACTOR	ED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	9437	0	9437	0	0	5-8	5-8
Н	6906	0	6906	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX.	MIN. COMPON	NENT REACTION	NS .		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	6595	4772 / 0	0/0	0/0	0/0	1823 / 0	0/0
H	4827	3492 / 0	0/0	0/0	0/0	1334 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N. H BEARING SIZE FACTOR = 1.15 AT JNT(S) N, H (BASED ON SUPPORT DEPTH = 1-8)

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 1.68 FT MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C+	HORDS				W E	BS	
MA	X. FACTORED	FACTORED				MAX. FACTO	RED
MEMB.	. FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB	. FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	-7814 / 0	-119.4 -119.4	0.23(1)	3.34	M-B	-771 / 0	0.07(1)
B- C	-12833 / 0	-119.4 -119.4	0.66(1)	2.13	B- L	0 / 8793	0.78 (1)
C-D	-14232 / 0	-119.4 -119.4	0.84 (1)	1.68	L- C	-1502 / 0	0.14(1)
D-E	-14232 / 0	-119.4 -119.4	0.74(1)	1.78	C-K	0 / 1789	0.22(1)
E-F	-9822 / 0	-119.4 -119.4	0.36(1)	2.84	K- D	-232 / 0	0.02(1)
F- G	-6383 / 0	-119.4 -119.4	0.16(1)	3.74	K-E	0 / 6902	0.85 (1)
N- A	-8867 / 0	0.0 0.0	0.32(1)	4.99	J- E	-5511 / 0	0.50(1)
H- G	-7262 / 0	0.0 0.0	0.26(1)	5.48		0 / 7843	0.97 (1)
					I- F	-2230 / 0	0.20(1)
N- M	0/0	-1000.6-1000.6	0.08 (1)	10.00	A- M	0 / 7029	0.87(1)
M-L	0 / 5935	-1000.6-1000.6	0.23(1)	10.00	I- G	0 / 5742	0.71(1)
L-K	0 / 12833	-1000.6-1000.6	0.60(1)	10.00			
K-J	0 / 9822	-18.2 -18.2	0.48 (1)	10.00			
J- I		-18.2 -18.2					
I- H	0/0	-18.2 -18.2	0.09(1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

				MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	8-4-7	-4280	-4280		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

34.8

SPECIFIED LOADS: TOP CH. LL =

		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 31-0-0 END DISTANCE = 8-4-7 END SPAN CARRIED = 31-0-0 END WALL WIDTH = 5-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD. - ADDT'L LOADS BASED ON 55 % OF GSL.

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT) CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED

ALLOWABLE DEFL.(LL)= L/360 (0.48") CALCULATED VERT. DEFL.(LL)= L/999 (0.14") ALLOWABLE DEFL.(TL)= L/360 (0.48") CALCULATED VERT. DEFL.(TL)= L/725 (0.24")

CSI: TC=0.84/0.97 (C-D:1) , BC=0.60/0.97 (K-L:1) , WB=0.97/0.97 (F-J:1) , SSI=0.71/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

MT18HS 586 403 2455 1382 3163 3004



CORPORATION OF TENGTIMOT 23-081/KTT-GREENPARK-ZADORRA-ROSE 3 EL 2

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JOB NAME

TRUSS NAME

OUANTITY CORY
OF PERMIT PLANS

IM0723-081

G09

TRUSS DESC.

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ID:QK tCmQp0 tF2YVqi0sgQdyWVx2-qmTv5YDYTOFY K004 JUbrzOUQzJLopY0WUg7vyyYAG

CHIEF BUILDING OFFICIAL

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

MHP 23029

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90) JSI METAL= 0.89 (M) (INPUT = 1.00)

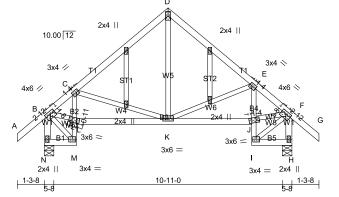




CORPORATION OF TENGTIMO7031-0814KTT-GREENPARK-ZADORRA-ROSE 3 EL 2 Page 17 of 51 TRUSS NAME QUANTITY COPLY DB DESC JOB NAME DRWG NO.

OF PERMIT PLANS Nov 22 2023

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Thu Jul 13 08:47:42 2023 Page 1 version 8.030 S Mar 22 2023 MI ek Industries, Inc. Thu Jul 13 08:47:42 2023 Page 1 ID:QK_tCmQp0_tF2YVqi0SqQdyWXx2-1z1HJuDAEhNPbUaCehqj72VhuqPl4SQhEADEfMyyYAF 5-11-0 2-0-0 -11-10 1-11-10 13-18 3 Scale = 1:55.1



TOTAL WEIGHT = 62 lb

LUMBER										
N. L. G. A. R	ULES									
CHORDS	SIZE		LUMBER	DESCR.						
A - D	2x4	DRY	No.2	SPF						
D - G	2x4	DRY	No.2	SPF						
N - B	2x4	DRY	No.2	SPF						
H - F	2x4	DRY	No.2	SPF						
N - M	2x4	DRY	No.2	SPF						
M - C	2x3	DRY	No.2	SPF						
L - J	2x4	DRY	No.2	SPF						
1 - E	2x3	DRY	No.2	SPF						
I - H	2x4	DRY	No.2	SPF						
ALL WEBS	2x3	DRY	No.2	SPF						
EXCEPT										
ALL GABLE	ALL GABLE WEBS									
	2x3	DRY	No.2	SPF						
DRY: SEASO	ONED LU	JMBER.								

GE01

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

IM0723-081

JI	TYPE	PLATES	VV	LEN	Y	Х
В	TMVWW-t	MT20	4.0	6.0	1.50	2.75
С	TMVW-t	MT20	3.0	4.0	1.50	1.25
D	TTW+p	MT20	3.0	5.0		
Е	TMVW-t	MT20	3.0	4.0	1.50	1.25
F	TMVWW-t	MT20	4.0	6.0	1.50	2.75
Н	BMV1+p	MT20	2.0	4.0		
1	BMVW-t	MT20	3.0	4.0		
J	BVMW+w	MT20	3.0	6.0	1.25	Edge
K	RMM/////////	MT20	3.0	6.0		

MT20 MT20 3.0 6.0 3.0 6.0 1.25 0.75 BVMW+w BMVW-t BMV1+p O, P, Q, R O NP+w NP+w 2.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTOR	ED	MAXIMUN	/ FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1047	0	1047	0	0	5-8	1-8
Н	1047	0	1047	0	0	5-8	1-8

UNFACTORED REACTIONS											
	1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
N	736	508 / 0	0/0	0/0	0/0	227 / 0	0/0				
Н	736	508 / 0	0/0	0/0	0/0	227 / 0	0/0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СН	ORDS					W E	BS	
MAX	(. FACTORED	FACTO	RED				MAX. FACTO	ORED
MEMB.	FORCE	VERT. LC	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PI	_F) (CSI (LC)	UNBRAC)	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A-B	0 / 53	-119.4	-119.4	0.16(1)	10.00	C-K	-322 / 0	0.12(1)
B- C	-994 / 0	-119.4	-119.4	0.22 (1)	5.96	K- D	0 / 397	0.09 (1)
C- D	-674 / 0	-119.4	-119.4	0.29 (1)	6.25	K-E	-302 / 0	0.10 (1)
D-E	-670 / 0	-119.4	-119.4	0.24(1)	6.25	B- L	0 / 797	0.18 (1)
E-F	-981 / 0	-119.4	-119.4	0.18(1)	6.05	J- F	0 / 763	0.17 (1)
F- G	0 / 53	-119.4	-119.4	0.16(1)	10.00	B- M	0 / 18	0.00(1)
N-B	-1029 / 0	0.0	0.0	0.11(1)	7.64	I- F	0 / 16	0.00(1)
H-F	-1024 / 0	0.0	0.0	0.11(1)	7.66			
N- M	0/0	-23.2	-26.3	0.02(4)	10.00			
M-L	0 / 16	0.0	0.0	0.04(1)	10.00			
L-C	-108 / 19	0.0	0.0	0.03(1)	7.81			
L-K	0 / 802	-26.3	-35.7	0.23(4)	10.00			
K-J	0 / 772	-35.8	-27.1	0.23(4)	10.00			
I- J	0 / 23	0.0	0.0	0.04(1)	10.00			
J- E	-67 / 31	0.0	0.0	0.03(4)	7.81			
I- H	0/0	-27.1	-23.2	0.02(4)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS: TOP CH. LL = 34.8

		DL	=	6.0	PS
BOT	CH.	LL	=	0.0	PS
			=	7.3	PS
TOTA	L LO	AD	=	48.1	PS

SPACING = <u>24.0</u> IN. C/C

NON STANDARD GIRDER *** ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39") CALCULATED VERT. DEFL.(LL)= L/999 (0.01") ALLOWABLE DEFL.(TL)= L/360 (0.39") CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.29/0.97 (C-D:1) , BC=0.23/0.97 (K-L:4) , WB=0.18/0.97 (B-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90) JSI METAL= 0.28 (F) (INPUT = 1.00)



