

LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	2100F 1.8E	SPF
м - в	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EVOEDT				

PL	PLATES (table is in inches)									
JT	TYPE	PLATES	W	LEN	Υ	Х				
В	TMVW-p	MT20	4.0	12.0	1.00	5.50				
С	TTWW+m	MT20	6.0	6.0	2.25	2.75				
D	TMW+w	MT20	2.0	4.0						
E	TTWW+m	MT20	6.0	6.0	2,25	2,75				
F	TMVW-p	MT20	4.0	12.0	1.00	5.50				
Н	BMV1+p	MT20	3.0	4.0	2.25	1.50				
	BMWWV-t	MT20	6.0	6.0	3,00	1,50				
J	BS-t	MT20	4.0	8.0						
K	BMWWW-t	MT20	4.0	8.0	1.75	4.00				
L	BMWW-t	MT20	6.0	6.0	3.00	1.50				
M	BMV1+p	MT20	3.0	4.0	2,25	1.50				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

MAYIMM FACTORED NIGHT READD

	FACTOR	RED	MAXIMU	M FACTO	ORED	INPUT	REQRE
GROSS REACTION			GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	3039	0	3039	0	0	5-8	4 - 6
	3039	0	3039	0	0	5-8	4-6

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE V

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2123	1542 / 0	0/0	0/0	0/0	581 / 0	0/0
Н	2123	1542 / 0	0/0	0/0	0/0	581 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

Сн	CHORDS WEBS							
	MAX. FACTORED FACTO						MAX, FACTO	RED
MEMB.	FORCE	VERT, LOA	D LC1	MAX	MAX.	MEMB.		MAX
	(LBS)	(PLF	-) (CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM 1	го		LENGTH	FR-TO		
A-B	0 / 36	-119.4 -	119.4	0.11(1)	10.00	L-C	-330 / 106	0.09(1)
B-C	- 4244 / 0						0 / 1490	0.37(1)
C-D	-5035 / 0	225.2	225,2	0.68 (1)	3,09	K-D	-1538 / 0	0.40(1)
D-E	-5035 / 0						0 / 1490	0.37(1)
E-F	- 4244 / 0						-330 / 106	0.09(1)
F-G	0 / 36			0.11(1)			0 / 3844	0.95(1)
	- 2955 / 0					I- F	0 / 3844	0.95(1)
H-F	- 2955 / 0	0.0	0.0	0.21(1)	6.07			
M-L	0/0			0.30 (4)				
L-K	0 / 3813			0.83 (1)				
K-J	0 / 3813			0.83 (1)				
J-I	0 / 3813			0.83(1)				
I-H	0/0	-34.4	-34.4	0.30 (4)	10.00			
	EACTORED CONCENTRATED LOADS (LDS)							

FACTORED CONCENTRATED LOADS (LBS)

JT LOC. LC1 MAX- MAX+ FACE DIR. TYPE HEEL CONL.

C 6-0-0 - 537 - 537 — FRONT VERT TOTAL — C1

E 16-11-0 - 537 - 537 — FRONT VERT TOTAL — C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 6-0-0
END SETBACK = 6-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14 - TPIC 2014

> (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76") CALCULATED VERT. DEFL.(IL)= L/999 (0.17") ALLOWABLE DEFL.(TL)= L/360 (0.76") CALCULATED VERT. DEFL.(TL)= L/941 (0.29")

CSI: TC=0.84/1.00 (B-C:1) , BC=0.83/1.00 (I-K:1) , WB=0.95/1.00 (F-I:1) , SSI=0.49/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

PLATE PLACEMENT TOL. = 0.250 inches

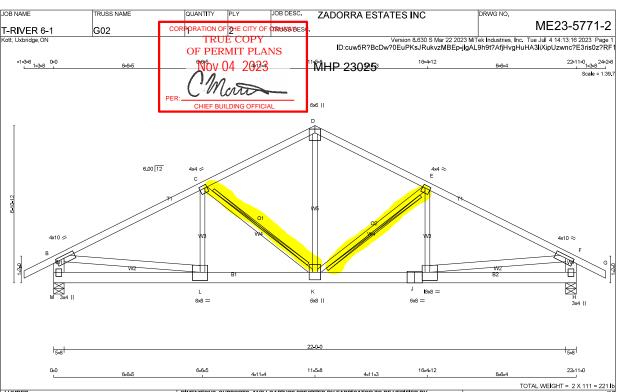
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90) JSI METAL= 0.87 (L) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not trust designer. Bracing shown is for lateral support of lateral support of actions only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer- nor consult.





LUMBER								
N. L. G. A. RULES								
CHORDS	SIZE		LUMBER	DESCR.				
A - D	2x4	DRY	2100F 1.8E	SPF				
D - G	2x4	DRY	2100F 1.8E	SPF				
M - B	2x6	DRY	No.2	SPF				
H - F	2x6	DRY	No.2	SPE				
M = .1	2x6	DRY	2100F 1.8E	SPF				
J - H	2x6	DRY	2100F 1.8E	SPF				
0 - 11	2.00	DIVI	21001 1.02	011				
ALL WEBS	2x3	DRY	No.2	SPE				
EXCEPT	2.45	DICI	140.2	511				
B - I	2x4	DRY	No.2	SPF				
i - F	2x4	DRY	No.2	SPE				
1	2X4	DIKT	140.2	SPF				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORE	S #ROWS		LOAD(PLF)
		SPACING (IN)	
TOP CH	IORDS: (0.1	22"X3") SPIRAL NAILS	
A-D	1	12	TOP
D-G	1	12	TOP
M-B	2	12	TOP
H-F	2	12	TOP
вотто	M CHORDS	: (0.122"X3") SPIRAL NA	JLS
M-J	2	12	SIDE(251.0)
J-H	2	12	SIDE(251.0)
WEBS:	(0,122"X3")	SPIRAL NAILS	
2x3	` 1	6	
201	4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING, REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES	(table	is in	inches)	
IT TVC) T	DΙ	ATEC	•

JT	TYPE	PLATES	W	LEN	Υ	Х	
В	TMVW-t	MT20	4.0	10.0	1.50	4.25	
С	TMWW-t	MT20	4.0	4.0	1.50	1.25	
D	TTW+p	MT20	6.0	6.0	2,00	3.00	
Е	TMWW-t	MT20	4.0	4.0	1.50	1,25	
F	TMVW-t	MT20	4.0	10.0	1.50	4.25	
Н	BMV1+p	MT20	3.0	4.0	2.25	1.50	
1	BMWW-t	MT20	8.0	8.0	4,25	4.00	
J	BS-t	MT20	6.0	8.0			
K	BMWWW+t	MT20	6.0	8.0			
L	BMWW-t	MT20	8.0	8.0	4.25	4.00	
M	BMV1+p	MT20	3.0	4.0	2.25	1.50	



NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARING.

FACTO		MAXIMU			INPUT	REQRE
GROSS RI	EACTION	GROSS	REACTIC	N	BRG	BRG
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
7491	0	7491	0	0	5-8	4-7
7491	0	7491	0	0	5-8	4-7

UNFACTORED REACT	HONS			
1ST LCASE	MAX./MIN	J. COMPONENT	I REACT	ONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	5233	3800 / 0	0/0	0/0	0/0	1432 / 0	0/0
Н	5233	3800 / 0	0/0	0/0	0/0	1432 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H BEARING SIZE FACTOR = 1.15 AT JNT(S) M, H (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 3,59 FT.

MAX, UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-K, C-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90%

OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

CHORDS				WEBS					
	MA.	X. FACTORED	FACTORED				MAX. FACTO	RED	
	MEMB.	FORCE	VERT, LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
		(LBS)	(PLF)	CSI (LC)	UNBRAC)	(LBS)	CSI (LC)	
	FR-TO		FROM TO		LENGTH	FR-TO			
	A-B	0 / 36	119.4 -119.4	0.06(1)	10,00	K-D	0 / 6142	0.76(1)	
	B-C	-10010 / 0	119.4 -119.4	0.58(1)	3.59	K-E	-3112 / 0	0.60(1)	
	C-D	- 7381 / 0	119.4 -119.4	0.31(1)	4.21	I-E	0 / 2348	0.29(1)	
	D-E	- 7381 / 0	119.4 -119.4	0.31(1)	4.21	C-K	-3112 / 0	0.60(1)	
	E-F	-10010 / 0	119.4 -119.4	0,58 (1)	3,59	L-C	0 / 2348	0.29(1)	
	F-G	0 / 36	-119.4 -119.4	0.06(1)	10.00	B-L	0 / 9041	0.80(1)	
	M-B	-6045 / 0	0.0 0.0	0.21(1)	6.01	I- F	0 / 9041	0.80(1)	
	H-F	-6045 / 0	0.0 0.0	0.21 (1)	6,01				
	M-L	0/0							
	L-K	0 / 8978	520.2 520.2						
		0 / 8978	520.2 520.2						
		0 / 8978	-520.2 -520.2						
	I-H	0/0	520.2 520.2	0.40 (1)	10.00				

DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	J IO	AD	=	48 1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 16-7-0
END DISTANCE = 22-11-0
END SPAN CARRIED = 16-7-0
END SWALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL)= L/360 (0.76")
CALCULATED VERT. DEFL (LL) = L/999 (0.17")
ALLOWABLE DEFL (TL)= L/360 (0.76")
CALCULATED VERT. DEFL (TL) = L/956 (0.29")

CSI: TC=0.58/1.00 (B-C:1) , BC=0.56/1.00 (K-L:1) , WB=0.80/1.00 (F-I:1) , SSI=0.65/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

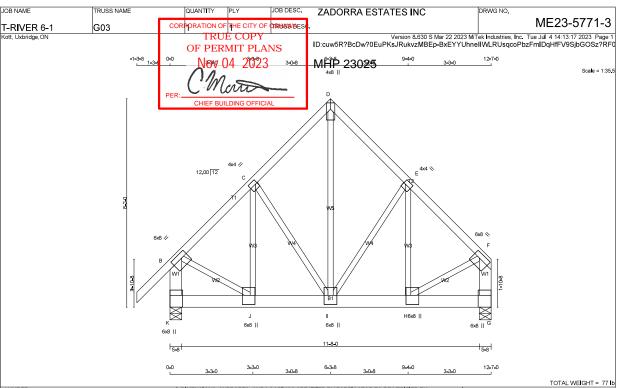
JSI GRIP= 0.90 (F) (INPUT = 0.90) JSI METAL= 0.96 (J) (INPUT = 1.00)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mites connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of the property parameters and proper
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com

KOTI



LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN	Y X
В	TMVW-t	MT20	6.0	8.0	2.25 3.00
С	TMWW-t	MT20	4.0	4.0	2.00 1.00
D	TTW+p	MT20	4.0	8.0	Edge
Е	TMWW-t	MT20	4.0	4.0	2.00 1.00
F	TMVW-t	MT20	6.0	8.0	2.25 3.00
G	BMV1+t	MT20	6.0	8.0	Edge 0,50
Н	BMVVVV+t	MT20	6.0	8.0	4.25 2.25
1	BMWWW+t	MT20	6.0	8.0	4.25 3.00
J	BMVVV+t	MT20	6.0	8.0	4.25 2.25
K	BMV1+t	MT20	6.0	8.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS MAXIMUM FACTORED NIGHT RECEP

	FACTOR	ED	MAXIMUN	/ FACTO	INPUT	REQRD	
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
IT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
(4191	0	4191	0	0	5-8	5-8
€	4024	0	4024	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	2926	2132 / 0	0/0	0/0	0/0	794 / 0	0/0
G	2812	2035 / 0	0/0	0/0	0/0	778 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G BEARING SIZE FACTOR = 1.15 AT JNT(S) K, G (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 3,70 FT.

MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS MAX. FACTO				WE	MAX. FACTO	DRED
	RCE VERT, LOAD LC				FORCE	MAX
(LB	S) (PLF)	CSI (LC)			(LBS)	CSI (LC)
FR-TO	FROM TO		LENGTH			
A-B 0/5					0 / 3065	0.76 (1)
B-C -3047 / 0					-898 / 0	0.48 (1)
C-D -2401/0					0 / 802	0.20(1)
D-E -2401/0				C-1	-897 / 0	0.48 (1)
E-F -3047/0					0 / 802	0.20(1)
K-B -3461/0		0.27(1)			0 / 2373	0.59(1)
G-F -3294 / 0	0.0 0.0	0.25 (1)	5,80	H-F	0 / 2373	0.59(1)
K-J 0/0						
J-I 0/2						
I-H 0/2						
H-G 0/0	-520.2 -520.2	0.38 (1)	10.00			

DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO.	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START DISTANCE = 00
START SPAN CARRIED = 16-7-0
END DISTANCE = 12-7-0
END SPAN CARRIED = 16-7-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CSI: TC=0.33/1.00 (E-F:1) , BC=0.56/1.00 (H-I:1) , WB=0.76/1.00 (D-I:1) , SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

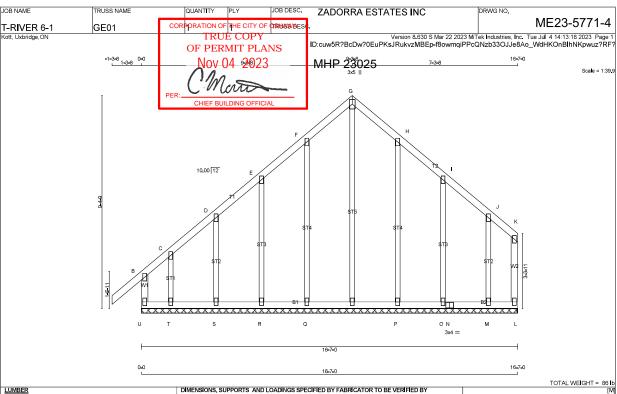
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90) JSI METAL= 0.65 (H) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding





LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCI
U - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
U - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE	WEBS			
	2x3	DRY	No.2	SPF
DRY: SEAS	ONED L	JMBER.		

GABLE STUDS SPACED AT 2-0-0 OC.

PLA	PLATES (table is in inches)										
JT	TYPE	PLATES	W	LEN Y	X						
	TMV+p		2.0	4.0							
C, I	D, E, F, H, I, J										
С	TMW+w	MT20	2.0	4.0							
	TTW+p	MT20	3.0	5.0							
	TMV+p	MT20	2.0	4.0							
L	BMV1+p	MT20	2,0	4.0							
	O, P, Q, R, S,										
М	BMW1+w	MT20	2.0	4.0							
Ν	BS-t	MT20	3.0	4.0							
U		MT20	2.0	4.0							
V	NP+w	MT20	2.0	4.0							

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
BEARINGS	

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6,25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHORDS WEBS								
MAX	. FACTORED	FACTO	RED				MAX, FACTO	RED
MEMB.	FORCE	VERT. LC	DAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(Pi	LF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
U-B	-388 / 0	0.0	0.0	0.09(1)	7,81	Q-F	-373 / 0	0.41(1)
A - B	0 / 53	-119.4	-119.4	0.16(1)	10.00	R-E	-180 / 0	0.10(1)
B-C	-144 / 0	-119.4	-119.4	0.18(1)	6.25	S-D	-266 / 0	0.08(1)
C- D	-63 / 0	119.4	-119.4	0.07(1)	6.25	T-C	-68 / 0	0.01(1)
D-E	-76 / 0	-119.4	-119.4	0.07(1)	6.25	P- H	- 369 / 0	0.41(1)
E-F	-36 / 0	-119.4	-119.4	0.15(1)	6.25	O-1	-201 / 0	0.12(1)
F-G	-114 / 0	-119.4	-119.4	0.15(1)	6.25	M-J	-129 / 0	0.04(1)
G-H	-114 / 0	119.4	-119.4	0.15(1)	6.25			
H-I	-38 / 0	-119.4	-119.4	0.15(1)	6.25			
I- J	-65 / 0	-119.4	-119.4	0.05(1)	6.25			
J-K	-107 / 0	119.4	-119.4	0.11(1)	6.25			
L-K	-171 / 0	0.0	0.0	0.11(1)	7.81			
U-T	0 / 61	-18.2	-18.2	0.10(1)				
T-S	0 / 58	-18.2		0.02(1)				
S-R	0 / 52	-18.2		0.02(1)				
R-Q	0 / 49	-18.2	-18.2	0.05 (4)	10.00			
Q-P	0 / 44	-18.2	-18.2	0.05 (4)	10.00			
P- 0	0 / 49	-18.2	-18.2	0.05(4)	10,00			
O- N	0 / 53	-18.2	-18.2	0.03(1)				
N- M	0 / 53	-18.2	-18.2	0.03(1)	10.00			
M-L	0 / 56	-18.2	-18.2	0.12(1)	10.00			

DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (B-C:1) , BC=0.12/1.00 (L-M:1) , WB=0.41/1.00 (F-Q:1) , SSI=0.13/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY) (PSI)
 SHEAR
 SECTION (PLI)

 MAX
 MIN
 MAX
 MIN
 MAX
 MIN

 MT20
 650
 371
 1747
 788
 1873

PLATE PLACEMENT TOL = 0,250 inches

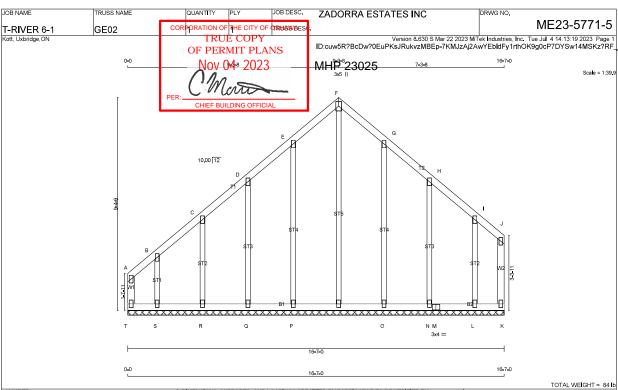
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (G) (INPUT = 0.90) JSI METAL= 0.25 (B) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of lateral suppor





LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR
T - A	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
T - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE	WEBS			
	2x3	DRY	No.2	SPF
DRY: SEAS	ONED LI	JMBER.		

GABLE STUDS SPACED AT 2-0-0 OC.

PLA	PLATES (table is in inches)											
JT	TYPE	PLATES	W	LEN Y	X							
Α	TMV+p	MT20	2.0	4.0								
В, 0	C, D, E, G, H,	l										
В	TMW+w	MT20	2.0	4.0								
	TTW+p	MT20	3.0	5.0								
J	TMV+p	MT20	2.0	4.0								
K	BMV1+p	MT20	2.0	4.0								
L, N	N, O, P, Q, R,											
L	BMW1+w	MT20	2.0	4.0								
М	BS-t	MT20	3.0	4.0								
Т	BMV1+p	MT20	2.0	4.0								
U	NP+w	MT20	2.0	4.0								

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY
BUILDING DESIGNER	
BEARINGS	

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT,
TWAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

CHC	ORDS					WE	BS	
MAX.	FACTORED	FACTO	RED				MAX, FACTO	RED
MEMB.	FORCE	VERT. LC	DAD LC1	MAX	MAX.	MEMB.		MAX
	(LBS)	(Pi	LF) (CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM			LENGTH			
T-A	-149 / 0			0.06(1)		P-E	-367 / 0	0.40(1)
A-B	-103 / 0			0.06(1)		Q-D	-184 / 0	0.11(1)
	- 75 / 0			0.06(1)		R-C	-256 / 0	0.07(1)
C-D	-81 / 0			0.06(1)		S-B	-155 / 0	0.03(1)
D-E	-44 / 0			0.15(1)		0- G	-363 / 0	0.40(1)
E-F	-118 / 0			0.15(1)		N-H	-205 / 0	0.12(1)
F-G	-118 / 0			0.15(1)		L-T	-121 / 0	0.03(1)
	- 46 / 0			0.15(1)				
H-I	-70 / 0			0.05 (1)				
I- J	-117 / 0			0.12(1)				
K-J	-182 / 0	0.0	0.0	0.12(1)	7,81			
T- S	0 / 69			0.07 (1)				
S-R	0 / 63			0.02(1)				
R-Q	0 / 57			0.02(1)				
Q-P	0 / 54	18.2		0.05 (4)				
P- 0	0 / 49	-18.2		0.05 (4)				
O- N	0 / 54	-18.2		0.05 (4)				
N- M	0 / 57			0.03 (1)				
M-L	0 / 57			0.03(1)				
L-K	0/60	-18.2	-18.2	0.13(1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:											
TOP	CH.	LL	=	34.8	PSF						
		DL	=	6.0	PSF						
BOT	CH.	LL	=	0.0	PSF						
		DL	=	7.3	PSF						
TOTA	L LO	AD	=	48.1	PSF						

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (E-F:1) , BC=0.13/1.00 (K-L:1) , WB=0.40/1.00 (E-P:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

PLATE PLACEMENT TOL. = 0.250 inches

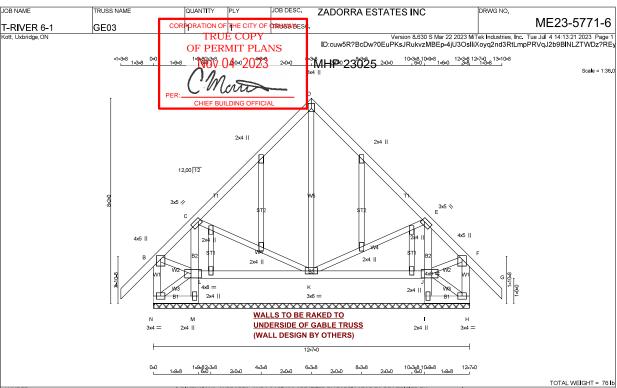
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (F) (INPUT = 0.90) JSI METAL= 0.20 (E) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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LUMBER N. L. G. A.	RULES			
CHORDS	SIZE		LUMBER	DESC
A - D	2x4	DRY	No.2	SP
D - G	2x4	DRY	No.2	SP
N - B	2x4	DRY	No.2	SP
H - F	2x4	DRY	No.2	SP
N - M	2x4	DRY	No.2	SP
M - C	2x4	DRY	No.2	SP
L - J	2x4	DRY	No.2	SP
1 - E	2x4	DRY	No.2	SP
I - H	2x4	DRY	No.2	SP
ALL WEB: EXCEPT	S 2x3	DRY	No.2	SP
ALL GABL	E WEBS	DRY	No.2	SP
DRY: SEA	SONED L			O.

GABLE STUDS SPACED AT 2-0-0 OC.

PL	ATES (table is	s in inches)				
JT	TYPE	PLATES	W	LEN	Υ	Х
В	TMVW+p	MT20	4.0	5.0	1.75	2.00
C	TMVW-t	MT20	3.0	5.0	1.50	1.25
D	TTW+p	MT20	3.0	4.0	Edge	
E	TMVW-t	MT20	3.0	5.0	1.50	1.25
F	TMVW+p	MT20	4.0	5.0	1.75	2.00
H	BMVW1-t	MT20	3.0	4.0		
1	BMV1+p	MT20	2.0	4.0		
J	BVMWW-I	MT20	4.0	8.0	2,00	5.00
K	BMWWW1-t	MT20	3.0	6.0		
L	BVMWW-I	MT20	4.0	8.0	2.00	5.00
M	BMV1+p	MT20	2.0	4.0		
N	BMVW1-t	MT20	3.0	4.0		
0,1	P, Q, R, S, T, I					
0	NP+w	MT20	2.0	4.0		
I						

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS	AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED B
BUILDING DESIGNER	
DEADINGS	

EA	KINGS						
	FACTO	RED	MAXIMUM FACTORED			INPUT	REQRD
	GROSS RI	EACTION	GROSS REACTION			BRG	BRG
Τ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
1	457	0	457	0	0	12-7-0 (1-9-8	1-8
	457	0	457	0	0	12-7-0 (1-9-8	1-8
	450	0	450	0	0	12-7-0 (1-9-8	1-8
	351	0	351	0	0	12-7-0 (1-9-8	1-8
	351	0	351	0	0	12-7-0 (1-9-8	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): $\mbox{\it K}$

UNF	JNFACTORED REACTIONS										
1ST LCASE MAX./MIN. COMPONENT REACTIONS											
JT	COMBINED	SNOW	LIVE	PERM,LIVE	WIND	DEAD	SOIL				
M	317	241 / 0	0/0	0/0	0/0	76 / 0	0/0				
1	317	241 / 0	0/0	0/0	0/0	76 / 0	0/0				
K	320	199 / 0	0/0	0/0	0/0	121 / 0	0/0				
N	242	195 / 0	0/0	0/0	0/0	47 / 0	0/0				
Н	242	195 / 0	0/0	0/0	0/0	47 / 0	0/0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I, K, N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CHUNG DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СН	ORDS					WE	BS	
MAX	. FACTORED	FACTO	RED				MAX, FACTO	RED
MEMB.	FORCE	VERT. LO	DAD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(Pi	LF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 59	119.4	-119.4	0.17(1)	10.00	K-D	-311 / 0	0.27(1)
B-C	-108 / 0	-119.4	-119.4	0.32(1)	6.25	K-E	-45 / 0	0.02(1)
C-D	-137 / 0	-119.4	-119.4	0.32(1)	6.25	C-K	-4 5 / 0	0.02(1)
D-E	-137 / 0	119.4	-119.4	0.32(1)	6,25	N-L	-1 / 0	0.00(1)
E-F	-108 / 0			0.32(1)		J- H	- 1/0	0.00(1)
F-G	0 / 59	-119.4		0.17(1)		B-L	0 / 116	0.03(1)
N-B	-335 / 0	0.0	0.0	0.04(1)	7,81	J-F	0 / 116	0.03(1)
H-F	-335 / 0	0.0	0.0	0.04(1)	7.81			
N-M	0/1			0.01 (4)				
M-L	-442 / 0			0.00(1)				
L-C	-4 38 / 0	0.0		0.00(1)				
L-K	0 / 112	-18.3		0.12 (4)				
K-J	0 / 112	-18.2		0.12 (4)				
I-J	-442 / 0	0.0		0.00(1)				
J-E		0.0		0.00(1)				
I-H	0/1	-18.2	-18.2	0.01(4)	10.00			

DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48,1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-D:1) , BC=0.12/1.00 (K-L:4) , WB=0.27/1.00 (D-K:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY) (PSI)
 SHEAR
 SECTION (PLI)

 MAX
 MIN
 MAX
 MIN
 MAX
 MIN

 MT20
 650
 371
 1747
 788
 1873

PLATE PLACEMENT TOL = 0,250 inches

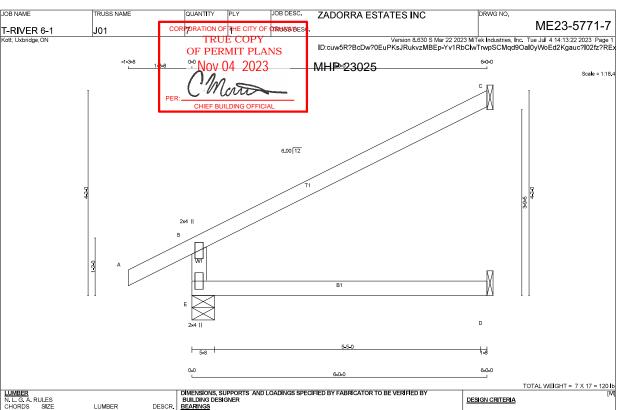
PLATE ROTATION TOL. = 5.0 Deg.



NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mites connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of the property parameters and proper
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches) W LEN Y X 2.0 4.0 2.0 4.0 B TMV+p E BMV1+p MT20 MT20

	41103						
	FACTORED		MAXIMU		INPUT	REQRD	
	GROSS R	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	674	0	674	0	0	5-8	1-8
С	269	0	269	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNF	ACTORED RE	ACTIONS				
	1ST LCASE	MAX./	MIN. COMPON	IENT REACTION	4S	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DE.
F	468	355 / 0	0/0	0/0	0/0	113 /
C	184	157 / 0	0/0	0/0	0/0	27 /
l n	26	0.10	0.40	0.70	0.10	26 /

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	ORDS			W	EBS	
MA>	(. FACTORED	FACTORED			MAX. FACTO	RED
MEMB.	FORCE	VERT, LOAD LC1	MAX	MAX. MEME	. FORCE	MAX
	(LBS)	(PLF) C	SI (LC)	UNBRAC	(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH FR-TO) ' '	
E-B	-610 / 0	0.0 0.0	0.13 (4)	7.81		
A-B	0 / 36	-119.4 -119.4	0.16(1)	10.00		
B-C	-40 / 0	119.4 -119.4	0.73(1)	6.25		
E-D	0/0	-18.2 -18.2	0.13(4)	10.00		

SPE	CIFIED	LOA	os:		
TOP	CH.	LL	=	34.8	PS
		DL	=	6.0	PS
BOT	CH.	LL	=	0.0	PS
		DL	=	7.3	PS
TOT	AL LO	AD	=	48,1	PS

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

SOIL 0/0 0/0 0/0

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

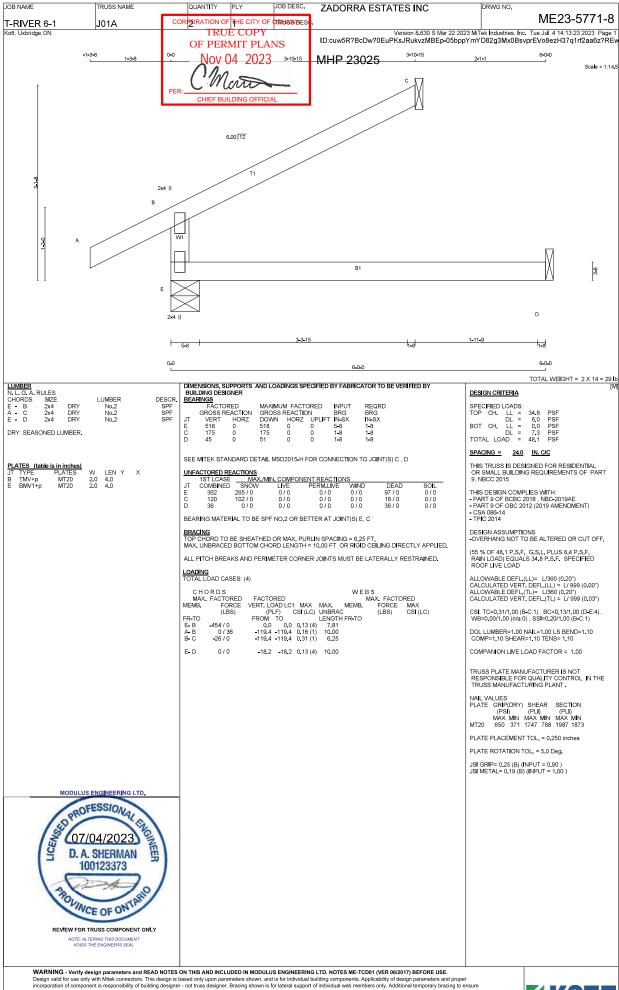
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90) JSI METAL= 0.25 (B) (INPUT = 1.00)



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incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding





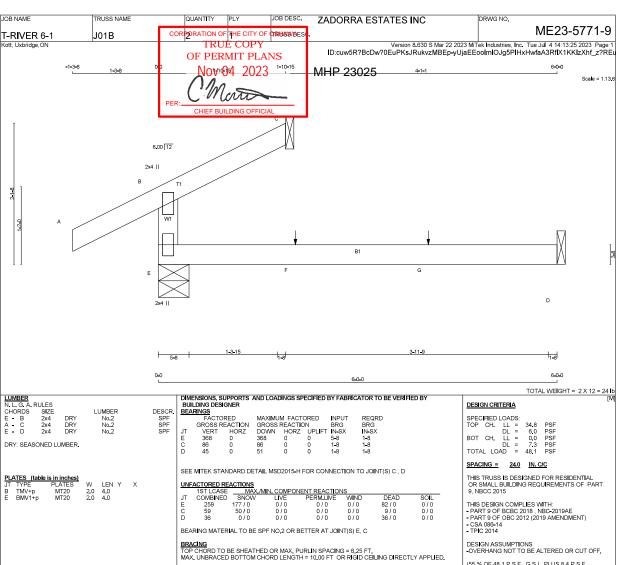
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not fuse designer. Brancing shown is for lateral support of individual web members only. Additional reportance to result in the properties of the control of the cont labrication, quality control, storage, delivery, erection and bracing, consult

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Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com





ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СНС	RDS				WE	BS		
MAX.	FACTORED	FACTORED				MAX. FACT	ORED	
MEMB.	FORCE	VERT. LOAD L	C1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAG		(LBS)	CSI (LC	2)
FR-TO		FROM TO		LENGTH	I FR-TO			
E-B	-304 / 0	0.0 0	0 0.13 (4)	7.81				
A– B	0 / 36	-119.4 -119	4 0.16 (1)	10.00				
B-C	- 12 / 0	-119.4 -119	4 0.07 (1	6.25				
E-F	0/0	-18.2 -18						
F-G	0/0	-18.2 -18						
G-D	0/0	-18.2 -18	2 0.13 (4)	10,00				
FACTOR	ED CONCENT	RATED LOADS						

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)

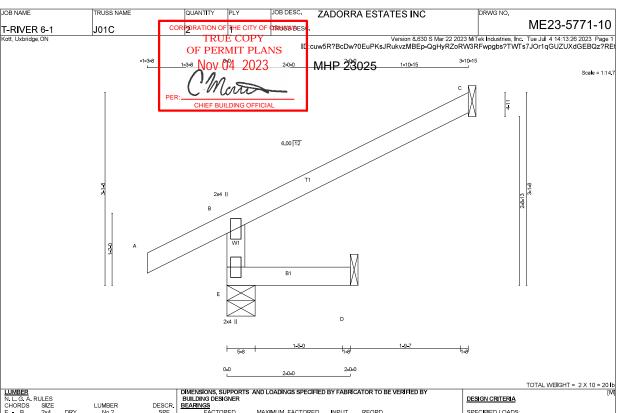


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not fuse designer. Brancing shown is for lateral support of individual web members only. Additional reportance to result in the properties of the control of the cont







LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches) W LEN Y X 2.0 4.0 2.0 4.0 B TMV+p E BMV1+p MT20 MT20

_	FACTO	FACTORED		M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	N-SX	IN-SX
E	474	0	474	0	0	5-8	1-8
С	175	0	175	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS										
	1ST LCASE	MAX./	MIN. COMPONE	ENT REACTION	IS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND					
E	326	265 / 0	0/0	0/0	0/0					
С	120	102 / 0	0/0	0/0	0/0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

СНО	ORDS					WE	BS		
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED	
MEMB.	FORCE	VERT. LO	DAD LC1	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(Pi	LF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
E-B	- 454 / 0	0.0	0.0	0.01(4)	7.81				
A-B	0 / 36	-119.4	-119.4	0.16(1)	10.00				
B-C	-26 / 0	-119.4	-119.4	0.31 (1)	6.25				
E-D	0/0	-18.2	-18.2	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA									
SPEC	IFIED	LOA	os:						
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA	L LO	AD	=	48.1	PSF				

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

SOIL 0/0 0/0 0/0

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90) JSI METAL= 0.19 (B) (INPUT = 1.00)

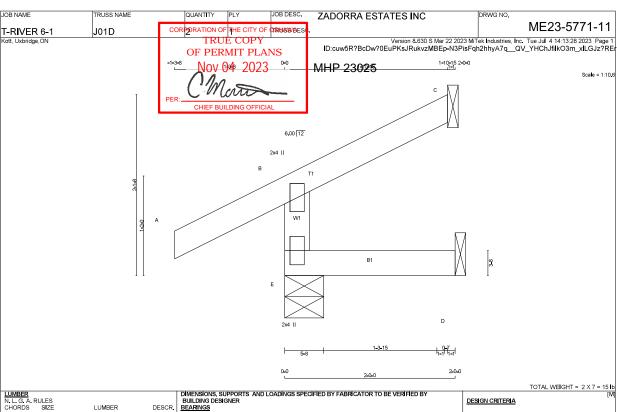


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

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Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com

KOT'



LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches) W LEN Y X 2.0 4.0 2.0 4.0 B TMV+p E BMV1+p

	41103						
	FACTORED		MAXIMU		INPUT	REQRD	
	GROSS RI	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	324	0	324	0	0	5-8	1-8
С	86	0	86	0	0	1-8	1-8
D	16	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNF	UNFACTORED REACTIONS											
	1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	Ī					
E	224	177 / 0	0/0	0/0	0/0	47 / 0						
С	59	50/0	0/0	0/0	0/0	9/0						
D	13	0/0	0/0	0/0	0/0	13 / 0						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

СН	ORDS				WE	BS	
MAX	. FACTORED	FACTORED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LOAD LO	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-304 / 0		0.01 (4)				
A-B	0 / 36	-119.4 -119.4					
B-C	- 12 / 0	-119.4 -119.4	1 0.07 (1)	6,25			
I F. D	0/0	-18.2 -18.3	0.02(4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	1 10	۸D	_	40.4	DOL

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

SOIL 0/0 0/0 0/0

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

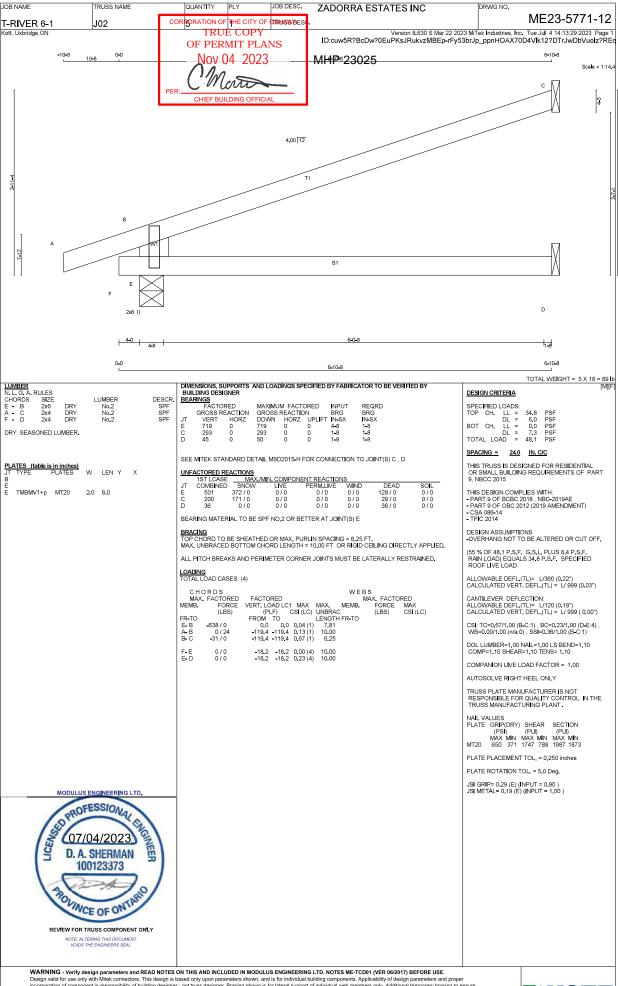
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90) JSI METAL= 0.13 (B) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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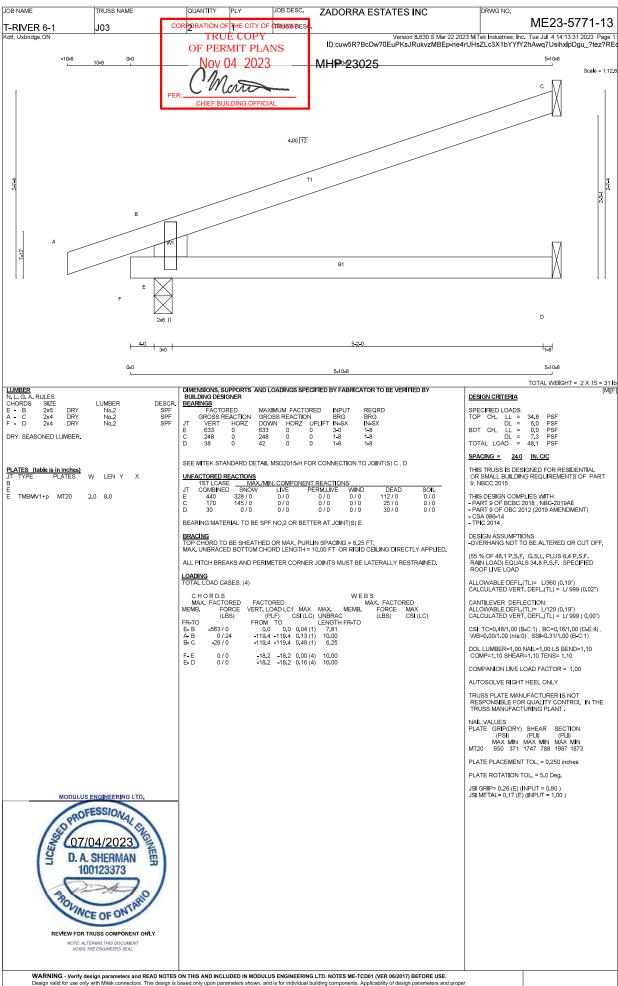
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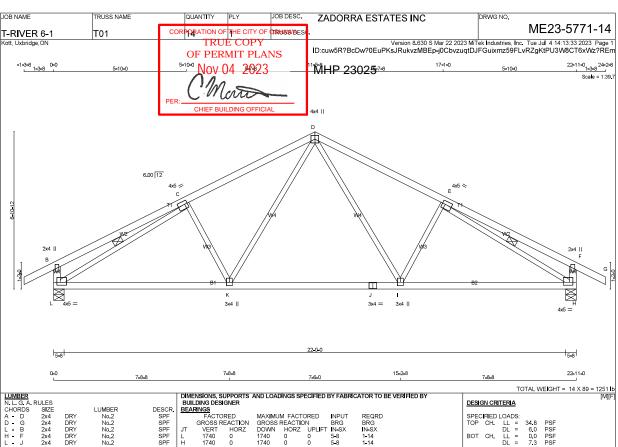
KOTI



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LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

PL	PLATES (table is in inches)								
JT	TYPE	PLATES	W	LEN	Υ	X			
В	TMV+p	MT20	2,0	4.0					
С	TMWW-t	MT20	4.0	5.0	1.75	1.50			
D	TTWW+p	MT20	4.0	4.0					
Е	TMWW-t	MT20	4.0	5.0	1,75	1.50			
F	TMV+p	MT20	2.0	4.0					
Н	BMVW1-t	MT20	4.0	5.0	1.50	1.75			
1	BMWW+t	MT20	3.0	4.0	1.75	1.50			
J	BS-t	MT20	3.0	4.0					
K	BMWW+t	MT20	3.0	4.0	1.75	1.50			
L	BMVW1-t	MT20	4.0	5.0	1.50	1.75			

	FACTOR	RED	MAXIMUM FACTORED I			INPUT	REQRD
	GROSS RE	ACTION	GROSS	REACTIO	N	BRG	BRG
Τ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	1740	0	1740	0	0	5-8	1-14
	1740	0	1740	0	0	5-8	1-14

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1213	892 / 0	0/0	0/0	0/0	321 / 0	0/0
Н	1213	892 / 0	0/0	0/0	0/0	321 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H,

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX, UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

Сн	ORDS				WE	BS	
		FACTORED				MAX. FACTO	DRED
MEMB.		VERT, LOAD L		MAX	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAG	3	(LBS)	CSI (LC)
FR-TO		FROM TO					
A-B	0 / 36	-119.4 -119	.4 0.16 (1)	10.00	D-I	0 / 679	0.15(1)
B-C	0 / 36	-119.4 -119					0.15(1)
	- 1939 / 0	-119.4 -119				0 / 679	0.15(1)
D-E	- 1939 / 0	-119.4 -119	.4 0.56 (1)	4.20	C-K	- 497 / 0	0.15(1)
E-F	0 / 36	-119.4 -119				-2308 / 0	0.72(1)
F-G	0 / 36	-119.4 -119			E- H	-2308 / 0	0.72(1)
	-424 / 0	0.0 0	.0 0.04 (1)	7.81			
H-F	-424 / 0	0.0 0	.0 0.04 (1)	7.81			
L-K	0 / 1936						
K-J	0 / 1367						
J-I	0 / 1367						
l I- H	0 / 1936	-18.2 -18	.2 0.44 (1)	10.00			

SPECIFIED LOADS:							
TOP	CH.	LL	=	34.8	PSF		
		DL	=	6.0	PSF		
BOT	CH.	LL	=	0.0	PSF		
		DL	=	7.3	PSF		
TOTAL LOAD = 48.1 PSF							

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.17")

CSI: TC=0.63/1.00 (B-C:1) , BC=0.44/1.00 (H-I:1) , WB=0.72/1.00 (C-L:1) , SSI=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90) JSI METAL= 0.62 (C) (INPUT = 1.00)



REVIEW FOR TRUSS COMPONENT ONLY

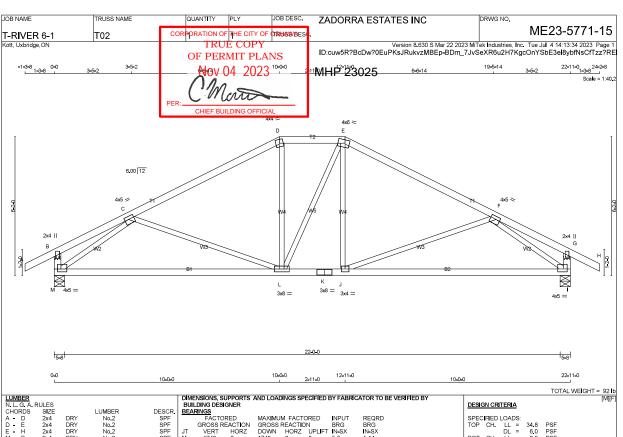
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

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KOTT



LUMBER				
N.L.G.A.R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DDV 0510	SMED III			

PL/	PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Υ	Х		
В	TMV+p	MT20	2.0	4.0				
C	TMWW-t	MT20	4.0	5.0	1.50	2.00		
D	TTW-m	MT20	4.0	4.0				
E	TTVVV-m	MT20	4.0	6.0		2,25		
F	TMWW-t	MT20	4.0	5.0	1.50	2.00		
G	TMV+p	MT20	2.0	4.0				
1	BMVW1-t	MT20	4.0	5.0	1,50	1,75		
J	BMWW-t	MT20	3.0	4.0				
K	BS-t	MT20	3.0	8.0				
L	BMWWW-t	MT20	3.0	8.0				
М	BMVW1-t	MT20	4.0	5.0	1,50	1.75		

7	NINGS						
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS I	REACTIO	N	BRG	BRG
Т	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Λ	1740	0	1740	0	0	5-8	1-14
	1740	0	1740	0	0	5-8	1-14

	1ST LCASE	MAX./I	MIN. COMPO	NENT REACTION	٧S
JΤ	COMBINED	SNOW	LIVE	PERM.LIVE	7

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1213	892 / 0	0/0	0/0	0/0	321 / 0	0/0
1	1213	892 / 0	0/0	0/0	0/0	321 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СН	ORDS					W E	BS	
MA>	. FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT, LC	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM			LENGTH			
A-B	0 / 36	-119.4	-119.4	0.16(1)	10.00	C-L	-390 / 0	0.42(1)
B-C	0 / 62	-119.4	-119.4	0.64(1)	10.00	L- D	0 / 269	0.06(1)
C-D	- 1752 / 0	119.4	-119.4	0,70(1)	4,10	L-E	0 / 1	0.00(1)
D-E	-1543 / 0	119.4	-119.4	0.16(1)	5.16	J-E	0 / 268	0.06(1)
E-F	-1752 / 0	119.4	-119.4	0.70(1)	4.10	J-F	-390 / 0	0.42(1)
F-G	0 / 62	-119.4	-119.4	0.64(1)	10.00	M-C	-2386 / 0	0.70(1)
G-H	0 / 36	119.4	-119.4	0.16(1)	10,00	F-I	-2386 / 0	0.70(1)
M-B	-223 / 0	0.0	0.0	0.02(1)	7,81			
I- G	-223 / 0	0.0	0.0	0.02(1)	7.81			
M-L	0 / 1903	-18.2	-18.2	0.52(1)	10,00			
L-K	0 / 1543	-18.2	-18.2	0.50(4)	10.00			
K-J	0 / 1543	-18.2	-18.2	0.50(4)	10.00			
J- I	0 / 1903	-18.2	-18.2	0.53(1)	10.00			

SPECIFIED LOADS:									
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA	L LO.	AD	=	48.1	PSF				

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/813 (0.34")

CSI: TC=0.70/1.00 (C-D:1) , BC=0.53/1.00 (I-J:1) , WB=0.70/1.00 (C-M:1) , SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

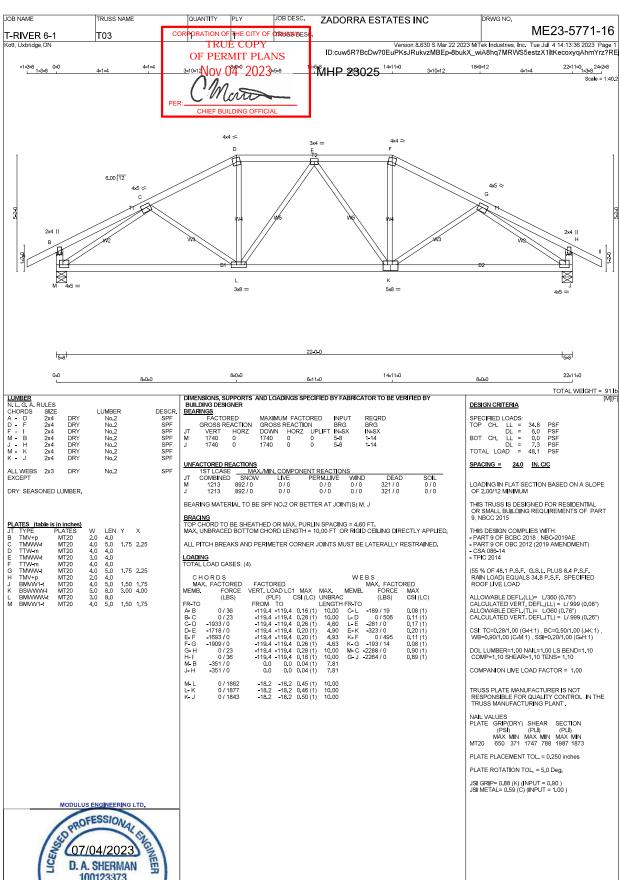
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90) JSI METAL= 0.89 (K) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mites connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of the property parameters and proper
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding







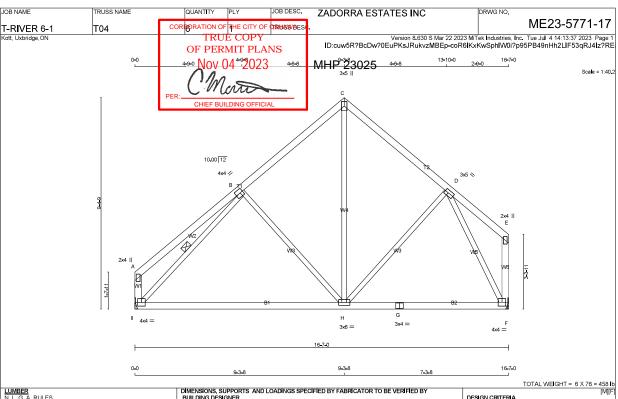
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not fuse designer. Brancing shown is for lateral support of individual web members only. Additional reportance to result in the properties of the control of the cont







LUMBER				
N. L. G. A. R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
1 - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

W LEN Y

DRY: SEASONED LUMBER.

PLATES (table is in inches)

Α	TMV+p	MT20	2.0	4.0		
В	TMWW-t	MT20	4.0	4.0	2.00	1,50
С	TTW+p	MT20	3.0	5.0		
D	TMWW-t	MT20	3.0	5.0	1,50	2,00
Е	TMV+p	MT20	2.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BS-t	MT20	3.0	4.0		
Н	BMWWW-t	MT20	3.0	6.0		
1	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEADINGS

DLA	BLANNOS									
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD			
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX			
1	1142	0	1142	0	0	MECHANIC	CAL			
F	1142	0	1142	0	0	MECHANIC	CAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, F. MINIMUM BEARING LENGTH AT JOINT I = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS 1ST I CASE MAX,/MIN, COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
1	798	577 / 0	0/0	0/0	0/0	221/0	0/0			
F	798	577 / 0	0/0	0/0	0/0	221/0	0/0			

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

р сно	DRDS					W E	BS	
MAX.	FACTORED	FACTOR	RED				MAX, FACTO	DRED
MEMB.	FORCE	VERT, LOA	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLI	F) (CSI (LC)	UNBRAC	:	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A-B	0 / 43	119.4	119.4	0.43(1)	10,00	B- H	-361 / 0	0.33(1)
B-C	-736 / 0	119.4	119.4	0.33(1)	6.25	H-C	0 / 447	0.10(1)
C-D	-725 / 0	-119.4	119.4	0.30(1)	6.25	H-D	0 / 49	0.02(4)
D-E	0 / 55	119.4	119.4	0.32(1)	10,00	I- B	-1161 / 0	0.37(1)
I- A	-209 / 0	0.0	0.0	0.02(1)	7.81	D-F	-1150 / 0	0.64(1)
F-E	- 72 / 0	0.0	0.0	0.01(1)	7.81			
I-H	0/775	-18.2	18.2	0.42(4)	10.00			
H-G	0 / 524	-18.2	-18.2	0.41(4)	10.00			
G-F	0 / 524	-18.2	-18.2	0.41(4)	10.00			

	,,,,,,,		-			
SPEC	IFIED	LOAI	os:			
TOP	CH.	LL	=	34.8	PSF	
		DL	=	6.0	PSF	
BOT	CH.	LL	=	0.0	PSF	
		DL	=	7.3	PSF	
TOTA	L LO	AD	=	48.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL)= L/999 (0.19")

CSI: TC=0.43/1.00 (A-B:1) , BC=0.42/1.00 (H-I:4) , WB=0.64/1.00 (D-F:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

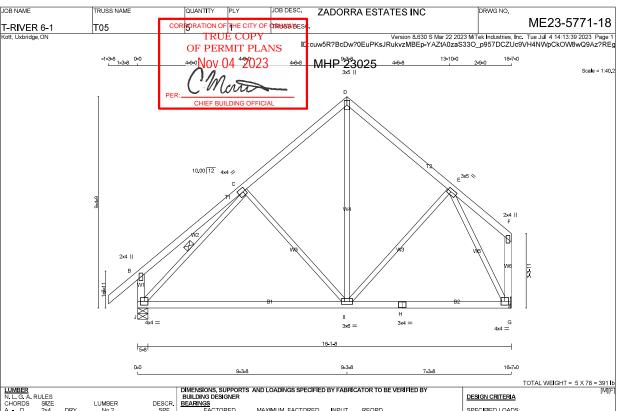
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90) JSI METAL= 0.39 (B) (INPUT = 1.00)



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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER				
N.L.G.A.R	ULES			
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

PL	ATES (table	is in inches)	
JT	TYPE	PLATES	W
В	TMV+p	MT20	2,0
C	TM/M/M/#	MT20	4.0

LEN Y X 4.0 4.0 2.00 1.50 5.0 1.50 2.00 4.0 4.0 4.0 6.0 4.0 TMWW-t TTW+p TMWW-t TMV+p BMVW1-t BS-t BMWWWV-t BMVW1-t 3.0 2.0 4.0 3.0 4.0

	VINOS						
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION GROSS REAC			REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1307	0	1307	0	0	5-8	1-8
G	1142	0	1142	0	0	MECHANIC	AL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

	151 LUASE	IVIAA./	MIN. COMPO	NEINT REACTION	VS CV		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	911	674 / 0	0/0	0/0	0/0	237 / 0	0/0
G	798	577 / 0	0/0	0/0	0/0	221 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

CHO	DRDS				WEBS					
MAX.	FACTORED	FACTOR	ED				MAX. FACTO	RED		
MEMB.	FORCE	VERT, LOA						MAX		
	(LBS)	(PLF	-) (UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM 1	го		LENGTH	I FR-TO				
A− B	0 / 53	-119.4 -					-361 / 0	0.33(1)		
B-C	0 / 43				10,00		0 / 447	0.10(1)		
C-D	-736 / 0	-119.4 -	119.4	0.33(1)	6.25	I-E	0 / 49	0.02(4)		
D-E	- 725 / 0	-119.4 -	119.4	0.30(1)	6.25	J-C	-1161 / 0	0.37(1)		
E-F	0 / 55	-119.4 -	119.4	0.32(1)	10.00	E-G	-1150 / 0	0.64(1)		
J-B	-374 / 0	0.0		0.04(1)						
G-F	- 72 / 0	0.0	0.0	0.01(1)	7.81					
J-I	0/775			0.42 (4)						
I- H	0 / 524			0.41 (4)						
H-G	0 / 524	-18.2	-18.2	0.41(4)	10.00					

DESIGN CRITERIA

enco	IFIED	1001	- -		
SPEC		LUMI	JO.		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL)= L/999 (0.19")

CSI: TC=0.43/1.00 (B-C:1) , BC=0.42/1.00 (I-J:4) , WB=0.64/1.00 (E-G:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

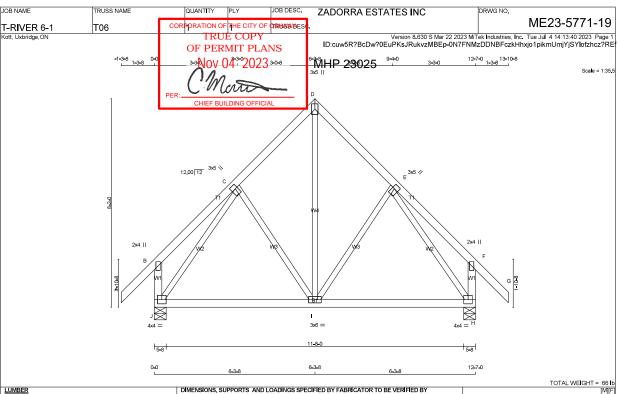
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90) JSI METAL= 0.39 (C) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of the property parameters and proper
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





N. L. G. A. RULES
CHORDS SIZE
A - D 2x4
D - G 2x4
H - F 2x4
J - H 2x4 LUMBER No.2 No.2 No.2 No.2 No.2 DESCR. SPF SPF SPF SPF SPF DRY DRY DRY DRY DRY DRY ALL WEBS 2x3 EXCEPT No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

Л	TYPE	PLATES	VV	LEN	Υ	Х	
3	TMV+p	MT20	2.0	4.0			
2	TMWW+t	MT20	3.0	5.0	2.00	1,00	
)	TTW+p	MT20	3.0	5.0			
	TMWW+t	MT20	3.0	5.0	2.00	1.00	
=	TMV+p	MT20	2.0	4.0			
+	BMVW1-t	MT20	4.0	4.0			
	BMWWW-t	MT20	3.0	6.0			
J	BMVW1-t	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARING.

DEA	RINGS						
	FACTOR			M FACTO	INPUT	REQRD	
	GROSS RE	EACTION	GROSS REACTION			BRG	BRG
T	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
1	1033	0	1033	0	0	5-8	1-8
H	1033	0	1033	0	0	5-8	1-8

UNFACTORED REACTIONS

	151 LUASE	IVIAA./I	MIN. COMPO				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	719	535 / 0	0/0	0/0	0/0	184 / 0	0/0
Н	719	535 / 0	0/0	0/0	0/0	184 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

СН	ORDS				WE	BS	
MAX	. FACTORED	FACTORED				MAX, FACTO	RED
MEMB.	FORCE	VERT, LOAD LO	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 59	-119.4 -119.4	0.17(1)	10.00	I- D	0 / 422	0.09(1)
B-C	0 / 31	-119.4 -119.4	0.19(1)	10.00	I-E	-171 / 0	0.09(1)
C-D	-538 / 0	119.4 -119.4	0,14(1)	6,25	C-1	-171 / 0	0.09(1)
D-E	-538 / 0	119.4 -119.4	0.14(1)	6,25	J- C	- 819 / 0	0.42(1)
E-F	0 / 31	119.4 -119.4	0.19(1)	10.00	E-H	-819 / 0	0.42(1)
F-G	0 / 59	-119.4 -119.4	0.17(1)	10.00			
J-B	-308 / 0	0.0 0.0	0.03(1)	7.81			
H-F	-308 / 0	0.0 0.0	0.03(1)	7,81			
J-1	0 / 456	18.2 18.2	0.23 (4)	10.00			
LH	0 / 456	_18.2 _18.2	0.23 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:										
TOP	CH.	LL	=	34.8	PSF					
		DL	=	6.0	PSF					
BOT	CH.	LL	=	0.0	PSF					
		DL	=	7.3	PSF					
TOTA	L LO	٩D	=	48.1	PSF					

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.23/1.00 (H-I:4) , WB=0.42/1.00 (C-J:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90) JSI METAL= 0.49 (C) (INPUT = 1.00)

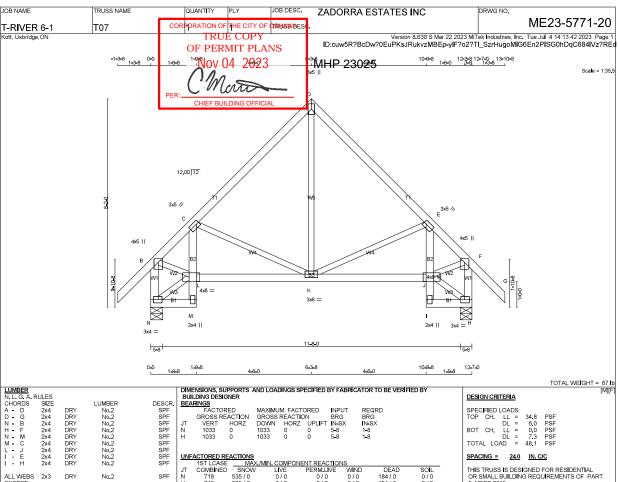


REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





| LUMBER | N. L. G. A. RULES | CHORDS | SIZE | CHORDS | C LUMBER No. 2 DRY DRY DRY DRY DRY DRY DRY DRY SPF ALL WEBS EXCEPT 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JΤ	TYPE	PLATES	W	LEN	Υ	Х	
В	TMVW+p	MT20	4.0	5.0	1,75	2,00	
С	TMVW-t	MT20	3.0	5.0	1,50	1,25	
D	TTW+p	MT20	3.0	5.0			
Е	TMVW-t	MT20	3.0	5.0	1.50	1.25	
F	TMVW+p	MT20	4.0	5.0	1.75	2,00	
Н	BMVW1-t	MT20	3.0	4.0	1.50	1.75	
1	BMV+p	MT20	2.0	4.0			
J	BVMWW-I	MT20	4.0	8.0	2.00	5.00	
K	BMWWW-t	MT20	3.0	6.0			
L	BVMWW-I	MT20	4.0	8.0	2.00	5.00	
M	BMV+p	MT20	2.0	4.0			
N	BMVW1-t	MT20	3.0	4.0	1,50	1,75	

	FACIO	KEU	IMAAINU	IVI FACT	JKED	INPUT	REURD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1033	0	1033	0	0	5-8	1-8
Н	1033	0	1033	0	0	5-8	1-8

184 / 0 184 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

10	IAL	LUAD	CASES.	(4)

	ORDS					WE		
MA>	. FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LC	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(Pl	.F) (CSI (LC)	UNBRAC)	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A-B	0 / 59	-119.4	-119.4	0.17(1)	10.00	K-D	0 / 319	0.07(1)
B-C	- 856 / 0	-119.4	-119.4	0.24(1)	6.25	K-E	-284 / 0	0.12(1)
C-D	-584 / 0	119.4	-119.4	0.31(1)	6,25	C-K	-284 / 0	0.12(1)
D-E	-584 / 0	119.4	-119.4	0.31(1)	6.25	N-L	-21/0	0.00(1)
E-F	-856 / 0	-119.4	-119.4	0.24(1)	6.25	J- H	-21/0	0.00(1)
F-G	0 / 59	-119.4	-119.4	0.17(1)	10.00	B-L	0 / 659	0.15(1)
N-B	-1 006 / 0	0.0	0.0	0,11(1)	7.80	J-F	0 / 659	0.15(1)
H-F	-1 006 / 0	0.0	0.0	0.11 (1)	7,80			
N-M	0 / 18	-18.2	-18.2	0.02(4)	10.00			
M-L	0 / 17	0.0	0.0	0.03(1)	10,00			
L-C	-119 / 10	0.0	0.0	0.02(1)	7.81			
L-K	0 / 647	-18.3	-18.3	0.16(1)	10.00			
K-J	0 / 647	-18.2	-18.2	0.16(1)	10.00			
I- J	0 / 17	0.0	0.0	0.03(1)	10,00			
J-E	- 119 / 10	0.0	0.0	0.02(1)	7.81			
I- H	0 / 18	-18.2	-18.2	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:					
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD			=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.31/1.00 (C-D:1) , BC=0.16/1.00 (J-K:1) , WB=0.15/1.00 (F-J:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (N) (INPUT = 0.90) JSI METAL= 0.35 (B) (INPUT = 1.00)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding

