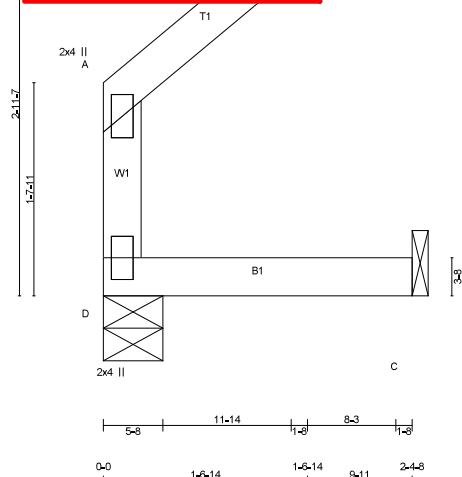




MHP 23026

Scale = 1:13.9



LUMBER				DESCR.	SPF
N, L, G, A, RULES	CHORDS	SIZE	LUMBER		
D - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	2.0	4.0	
D	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
D	117	0	117	0	0	5-8	1-8	1-8	1-8
B	91	0	91	0	0	1-8	1-8	1-8	1-8
C	24	0	24	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	82	58/0	0/0	0/0	0/0	25/0	0/0
B	62	51/0	0/0	0/0	0/0	11/0	0/0
C	19	2/0	0/0	0/0	0/0	16/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MEMB. FORCE (LBS)	MAX. FACTORED (PLF)
FR-TO		FROM	TO	UNBRAC LENGTH	FR-TO
D-A	-97/0	0.0	0.0	0.01 (1)	7.81
A-B	-2/1	-119.4	-119.4	0.04 (1)	10.00
D-C	0/0	-18.2	-18.2	0.03 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.03/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SS=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

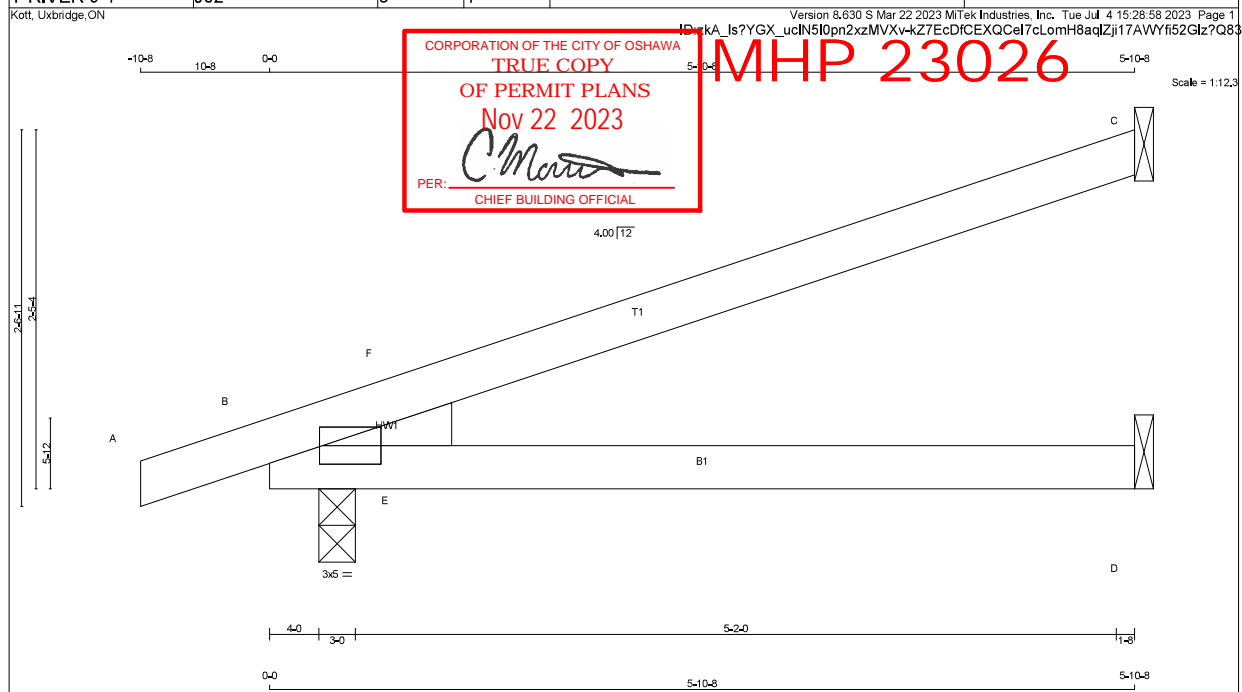


REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





LUMBER		N. L. G. A. RULES		CHORDS		SIZE		LUMBER		DESCR.	
A - C		2x4		DRY		No.2		SPF		SPF	
B - D		2x4		DRY		No.2		SPF		SPF	
DRY: SEASONED LUMBER.											
PLATES (table is in inches)											
JT		TYPE		PLATES		W		LEN		Y X	
B		TMBH14		MT20		3.0		5.0		1.50 0.25	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER											
BEARINGS											
JT		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG		HEEL WEDGE	
C		299 0		299 0		0 0		1-8 1-8		2x4 L	
B		514 0		514 0		0 0		3-0 1-8			
D		105 0		105 0		0 0		1-8 1-8			
SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D											
UNFACTORED REACTIONS											
JT		COMBINED		SNOW		LIVE		PERM.LIVE		WIND DEAD SOIL	
C		205		169 / 0		0 / 0		0 / 0		36 / 0 0 / 0	
B		358		269 / 0		0 / 0		0 / 0		89 / 0 0 / 0	
D		77		35 / 0		0 / 0		0 / 0		42 / 0 0 / 0	
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B											
BRACING											
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.											
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.											
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.											
LOADING											
TOTAL LOAD CASES: (4)											
FR-TO		CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.		FORCE (LBS)		VERT. LOAD (PLF)		LC1 MAX. FACTORED (LC)		MEMB.		FORCE (LBS)	
A-B		0 / 8		-119.4		-119.4 0.07 (1)		10.00		E-F -328 / 8 0.00 (1)	
B-F		-23 / 0		-119.4		-119.4 0.07 (4)		6.25			
F-C		0 / 3		-119.4		-119.4 0.51 (1)		10.00			
B-E		0 / 0		-18.2		-18.2 0.41 (1)		10.00			
E-D		0 / 0		-18.2		-18.2 0.41 (1)		10.00			

DESIGN CRITERIA											
SPECIFIED LOADS:											
TOP CH.		LL		=		34.8		PSF			
TOP CH.		DL		=		8.0		PSF			
BOT CH.		LL		=		0.0		PSF			
BOT CH.		DL		=		7.3		PSF			
TOTAL LOAD		=		48.1		PSF					
SPACING = 24.0 IN. C/C											
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015											
THIS DESIGN COMPLIES WITH:											
- PART 9 OF CBC 2018, NBC-2019AE											
- PART 9 OF OBC 2012 (2019 AMENDMENT)											
- CSA 086-14											
- TPIC 2014											
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD											
ALLOWABLE DEFL.(LL)= L/360 (0.20")											
CALCULATED VERT. DEFL.(LL) = L/683 (0.10")											
ALLOWABLE DEFL.(TL)= L/360 (0.20")											
CALCULATED VERT. DEFL.(TL) = L/377 (0.19")											
CSI TC=0.51/1.00 (C-F:1), BC=0.41/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)											
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10											
COMPANION LIVE LOAD FACTOR = 1.00											
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.											
NAIL VALUES											
PLATE GRIP(DRY)		SHEAR		SECTION		(PSI)		(PL)		(PL)	
MAX		MIN		MAX		MIN		MAX		MIN	
MT20		650 371		1747 788		1987 1873					
PLATE PLACEMENT TOL. = 0.250 inches											
PLATE ROTATION TOL. = 5.0 Deg.											
JSI GRIP= 0.73 (B) (INPUT = 0.90)											
JSI METAL= 0.07 (B) (INPUT = 1.00)											

MODULUS ENGINEERING LTD.

07/04/2023

LICENSED PROFESSIONAL ENGINEER

P. R. HEALY

PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL



DRY, SEASONED LUMBER.

PLATES (table is in inches) 1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

D	TS-t	MT20	3.0	6.0	<u>LOADING</u>	ALLOWABLE DEFL.(TL)= L/360 (0.76")
E	TTWW+p	MT20	3.0	5.0		CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

J	BMVW1+	MT20	4.0	5.0	1.75	2.25	MAX. FACTORED	FACTORED	MAX. FACTORED	
K	BMVW1+	MT20	3.0	4.0			MEMB. FORCE	VERT. LOAD LC1 MAX	MAX	DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

N	DMVVV14	MT20	4.0	5.0	1.75	2.25	A-D	0/49	-119.4	-119.4	0.10(1)	10.00	E-R	0/045	0.10(1)	COMPARISON LIVE LOAD FACTOR = 1.00
							B-C	0/50	-119.4	-119.4	0.65(1)	10.00	K-G	-479/0	0.29(1)	
							C-D	-1487/0	-119.4	-119.4	0.55(1)	4.68	M-E	0/643	0.10(1)	

F-G	-148/70	-119.4	-119.4	0.55 (1)	4.68	G-J	-1824/70	0.76 (1)	TRUSS MANUFACTURING PLANT.
G-H	0/50	-119.4	-119.4	0.65 (1)	10.00				
H-I	0/10	-119.4	-119.4	0.16 (1)	10.00				MAN. VALUES

[illegible]

		PLATE ROTATION TOL. = 5.0 Deg.
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[illegible]

MODULUS ENGINEERING LTD		
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07/01/0000




VOIDS THE ENGINEERS SEAL

incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding

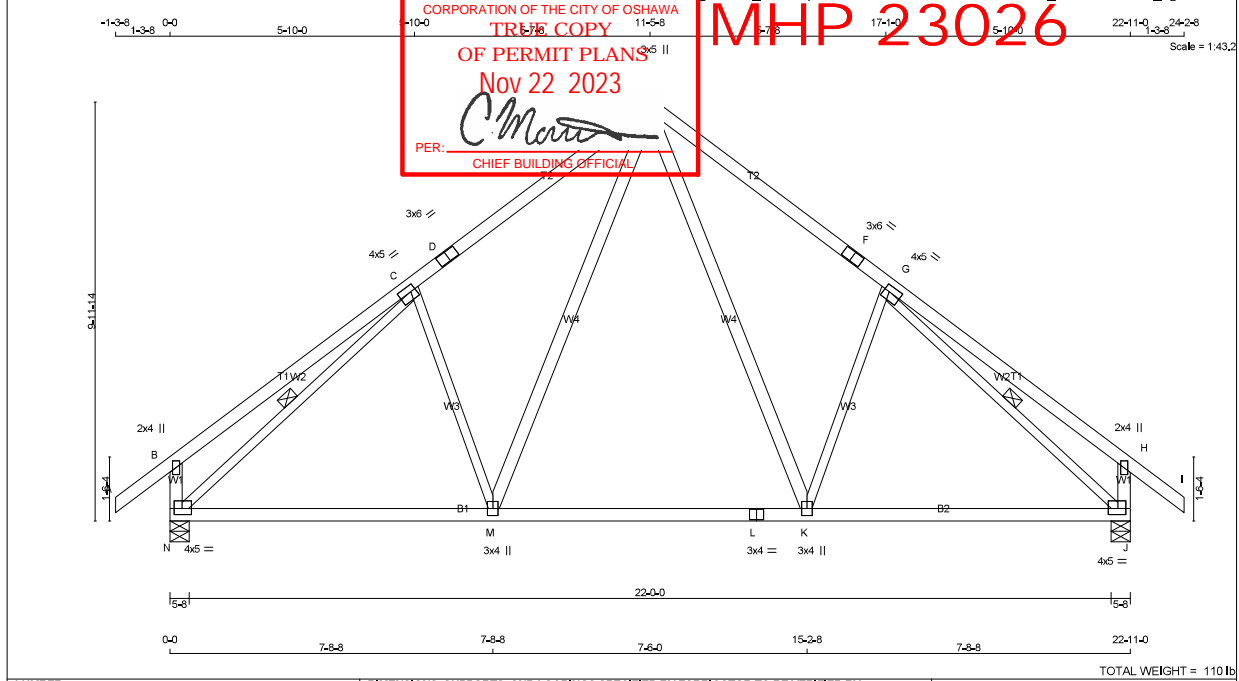
Street, Suite 512, Alexandria, VA 22314 or www.secdatabase.com



incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding

Street, Suite 512, Alexandria, VA 22314 or www.secdatabase.com





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
CHORDS	SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:			
A - D	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD	TOP CH.	LL	= 34.8 PSF
D - E	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	DL	= 6.0 PSF	
E - F	2x4	DRY	No.2	SPF	DOWN	DOWN	UPLIFT	IN-SX	BOT CH.	LL	= 0.0 PSF
F - I	2x4	DRY	No.2	SPF	N	1742	0	1742	0	DL	= 7.3 PSF
N - B	2x4	DRY	No.2	SPF	J	1742	0	1742	0	TOTAL LOAD	= 48.1 PSF
J - H	2x4	DRY	No.2	SPF							
N - L	2x4	DRY	No.2	SPF							
L - J	2x4	DRY	No.2	SPF							

ALL WEBS EXCEPT				UNFACTORED REACTIONS				SPACING = 24.0 IN. GIG				
K - G	2x3	DRY	No.2	SPF	1ST CASE	MAX./MIN.	COMPONENT REACTIONS		THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
C - M	2x3	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N - C	2x3	DRY	No.2	SPF	N	1215	894 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0
G - J	2x3	DRY	No.2	SPF	J	1215	894 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0
				SPF	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J							
				SPF	THIS DESIGN COMPLIES WITH:							
					- PART 9 OF BCBC 2018, NBC-2019AE							
					- PART 9 OF OBC 2012 (2019 AMENDMENT)							
					- CSA 086-14							
					- TPIC 2014							
DRY, SEASONED LUMBER.												

DRY: SEASONED LUMBER.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH
A-B	0 / 49	-119.4	-119.4	0.16 (1)	E-K	0 / 643	0.10 (1)
B-C	0 / 50	-119.4	-119.4	0.65 (1)	K-G	-479 / 0	0.29 (1)
C-D	-1487 / 0	-119.4	-119.4	0.55 (1)	M-E	0 / 643	0.10 (1)
D-E	-1487 / 0	-119.4	-119.4	0.55 (1)	C-M	-479 / 0	0.29 (1)
E-F	-1487 / 0	-119.4	-119.4	0.55 (1)	N-C	-1824 / 0	0.76 (1)
F-G	-1487 / 0	-119.4	-119.4	0.55 (1)	G-J	-1824 / 0	0.76 (1)
G-H	0 / 50	-119.4	-119.4	0.65 (1)			
H-I	0 / 49	-119.4	-119.4	0.16 (1)			
N-B	-422 / 0	0.0	0.0	0.04 (1)			
J-H	-422 / 0	0.0	0.0	0.04 (1)			
N-M	0 / 1316	-18.2	-18.2	0.34 (1)			
M-L	0 / 920	-18.2	-18.2	0.30 (4)			
L-K	0 / 920	-18.2	-18.2	0.30 (4)			
K-J	0 / 1316	-18.2	-18.2	0.34 (1)			

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PL) (PL)

MAX MIN MAX MIN MAX MIN

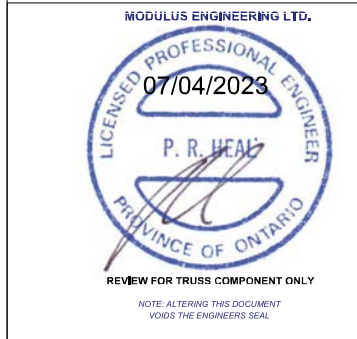
MT20 650 371 1747 788 1987 1873

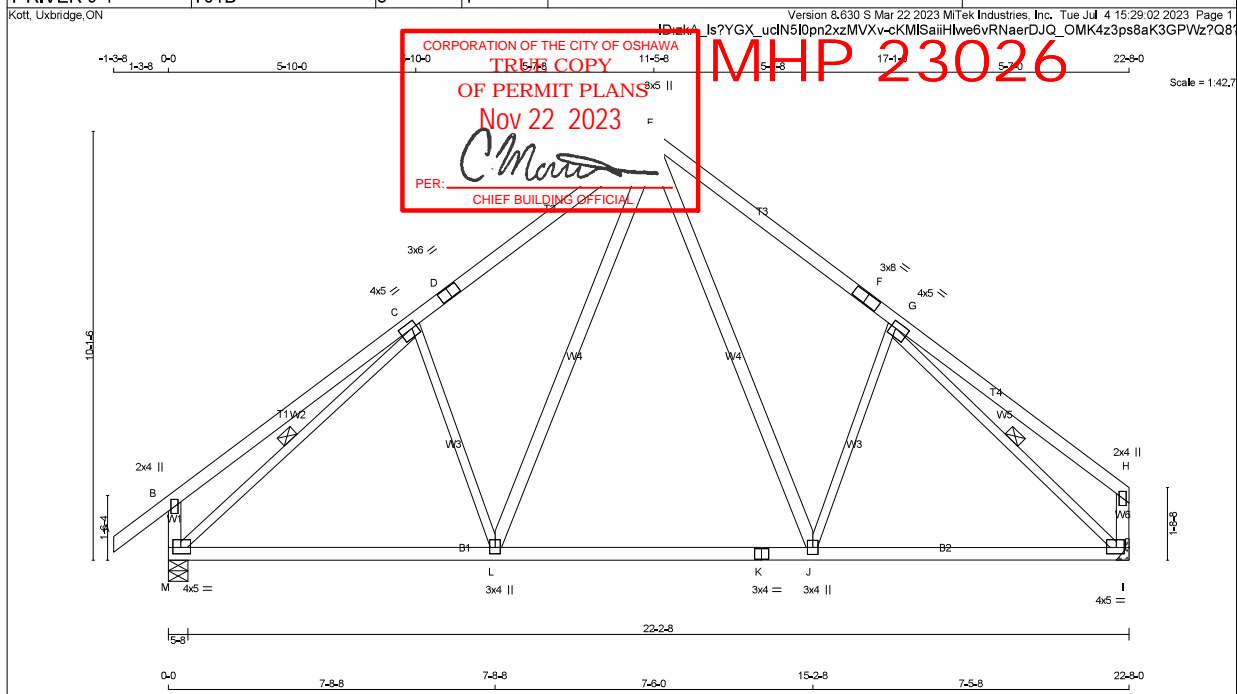
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

JSI METAL= 0.53 (G) (INPUT = 1.00)





<div><div><div>LUMBER</div><div>N. L. G. A. RULES</div><div>CHORDS SIZE</div><div>LUMBER</div><div>DESCR.</div></div><div><div>A - D</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>D - E</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>E - F</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>F - H</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>M - B</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>I - H</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>M - K</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>K - I</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>ALL WEBS</div><div>2x4</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>EXCEPT</div><div></div><div></div><div></div><div></div></div><div><div>J - G</div><div>2x3</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>C - L</div><div>2x3</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>M - C</div><div>2x3</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>G - I</div><div>2x3</div><div>DRY</div><div>No.2</div><div>SPF</div></div><div><div>DRY: SEASONED LUMBER.</div></div></div>										<div><div><div>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</div><div>BEARINGS</div><div>FACTORED GROSS REACTION</div><div>MAXIMUM FACTORED GROSS REACTION</div><div>INPUT BRG</div><div>REQD BRG</div></div><div><div>JT VERT HORZ</div><div>DOWN HORZ UPLIFT</div><div>IN-SX IN-SX</div></div><div><div>M 1725 0</div><div>1725 0</div><div>0 5-8</div><div>1-14</div></div><div><div>I 1560 0</div><div>1560 0</div><div>0 0</div><div>MECHANICAL</div></div><div><div>A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-11.</div></div><div><div>UNFACTORED REACTIONS</div><div>1ST LCASE</div><div>MAX./MIN. COMPONENT REACTIONS</div></div><div><div>JT COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL</div><div>M 1203 885 / 0</div><div>0 / 0</div><div>0 / 0</div><div>318 / 0</div><div>0 / 0</div></div><div><div>I 1090 789 / 0</div><div>0 / 0</div><div>0 / 0</div><div>0 / 0</div><div>301 / 0</div><div>0 / 0</div></div><div><div>BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M</div></div><div><div>BRACING</div><div>TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4,71 FT.</div><div>MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED.</div><div>ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.</div><div>1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, G-I.</div><div>END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW</div><div>LOADING</div><div>TOTAL LOAD CASES: (4)</div></div><div><div>CHORDS</div><div>WEBS</div><div>MAX. FACTORED</div><div>MAX. FACTORED</div><div>MEMB. FORCE (LBS)</div><div>VERT. LOAD (PLF)</div><div>MAX. FACTORED VERT. LOAD (LC1)</div><div>MAX. FACTORED VERT. LOAD (LC)</div><div>MEMB. FORCE (LBS)</div><div>MAX. FACTORED VERT. LOAD (LC1)</div><div>MAX. FACTORED VERT. LOAD (LC)</div></div><div><div>FR-TO</div><div>A-B 0 / 49</div><div>-119,4 -119,4</div><div>0,16 (1)</div><div>10,00</div><div>FR-TO</div><div>E-J 0 / 588</div><div>0,09 (1)</div></div><div><div>B-C 0 / 50</div><div>-119,4 -119,4</div><div>0,65 (1)</div><div>10,00</div><div>J-G -427 / 0</div><div>0,26 (1)</div></div><div><div>C-D -1464 / 0</div><div>-119,4 -119,4</div><div>0,55 (1)</div><div>4,71</div><div>L-E 0 / 647</div><div>0,10 (1)</div></div><div><div>D-E -1464 / 0</div><div>-119,4 -119,4</div><div>0,55 (1)</div><div>4,71</div><div>C-L -482 / 0</div><div>0,29 (1)</div></div><div><div>E-F -1435 / 0</div><div>-119,4 -119,4</div><div>0,54 (1)</div><div>4,78</div><div>M-C -1799 / 0</div><div>0,75 (1)</div></div><div><div>F-G -1435 / 0</div><div>-119,4 -119,4</div><div>0,54 (1)</div><div>4,78</div><div>G-I -1783 / 0</div><div>0,70 (1)</div></div><div><div>G-H 0 / 50</div><div>-119,4 -119,4</div><div>0,62 (1)</div><div>10,00</div><div></div><div></div></div><div><div>M-B -422 / 0</div><div>0,0 0,0</div><div>0,04 (1)</div><div>7,81</div><div></div><div></div></div><div><div>I-H -242 / 0</div><div>0,0 0,0</div><div>0,03 (1)</div><div>7,81</div><div></div><div></div></div><div><div>M-L 0 / 1298</div><div>-18,2 -18,2</div><div>0,34 (1)</div><div>10,00</div><div></div><div></div></div><div><div>L-K 0 / 900</div><div>-18,2 -18,2</div><div>0,31 (4)</div><div>10,00</div><div></div><div></div></div><div><div>K-J 0 / 900</div><div>-18,2 -18,2</div><div>0,31 (4)</div><div>10,00</div><div></div><div></div></div><div><div>J-I 0 / 1258</div><div>-18,2 -18,2</div><div>0,33 (1)</div><div>10,00</div><div></div><div></div></div></div>										<div><div><div>DESIGN CRITERIA</div><div>SPECIFIED LOADS:</div><div>TOP CH. LL = 34,8 PSF</div><div>DL = 6,0 PSF</div><div>BOT CH. LL = 0,0 PSF</div><div>DL = 7,3 PSF</div><div>TOTAL LOAD = 48,1 PSF</div></div><div><div>SPACING = 24,0 IN. GIG</div><div>THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015</div><div>THIS DESIGN COMPLIES WITH:</div><div>- PART 9 OF BCBC 2018, NBC-2019AE</div><div>- PART 9 OF OBC 2012 (2019 AMENDMENT)</div><div>- CSA 086-14</div><div>- TPIC 2014</div><div>(55 % OF 48,1 P.S.F. G.S.L. PLUS 8,4 P.S.F. RAIN LOAD) EQUALS 34,8 P.S.F. SPECIFIED ROOF LIVE LOAD</div><div>ALLOWABLE DEFL.(LL)= L/360 (0,76")</div><div>CALCULATED VERT. DEFL.(LL) = L/999 (0,06")</div><div>ALLOWABLE DEFL.(TL)= L/360 (0,76")</div><div>CALCULATED VERT. DEFL.(TL) = L/999 (0,13")</div><div>CSI TC=0,65/1,00 (B-C:1) , BC=0,34/1,00 (L-M:1) , WB=0,75/1,00 (C-M:1) , SSI=0,27/1,00 (C-E:1)</div><div>DOL LUMBER=1,00 NAIL=1,00 LS BEND=1,10</div><div>COMP=1,10 SHEAR=1,10 TENS= 1,10</div><div>COMPANION LIVE LOAD FACTOR = 1,00</div><div>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .</div><div>NAIL VALUES</div><div>PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)</div><div>MAX MIN MAX MIN MAX MIN</div><div>MT20 650 371 1747 788 1987 1873</div><div>PLATE PLACEMENT TOL. = 0,250 inches</div><div>PLATE ROTATION TOL. = 5,0 Deg.</div><div>JSI GRIP= 0,86 (G) (INPUT = 0,90)</div><div>JSI METAL= 0,52 (C) (INPUT = 1,00)</div></div></div>									
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MODULUS ENGINEERING LTD.

07/04/2023

LICENSED PROFESSIONAL ENGINEER

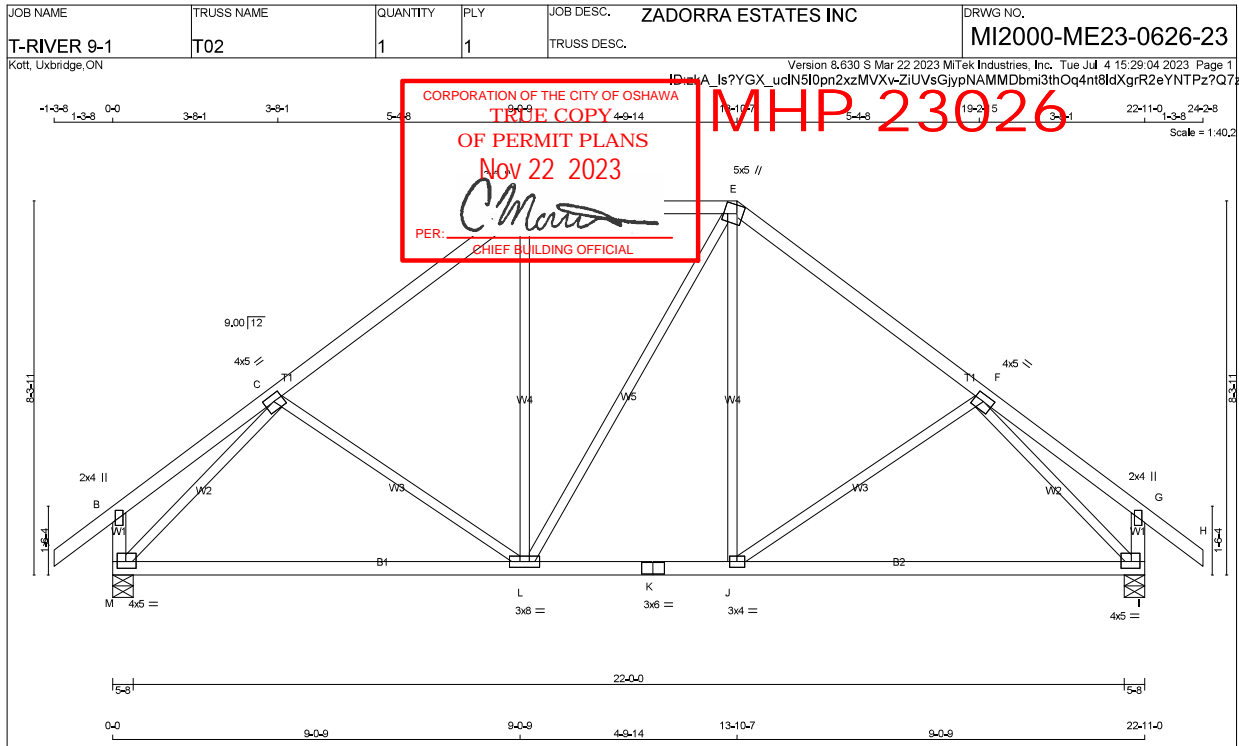
P. R. HEAL

PROVINCE OF ONTARIO

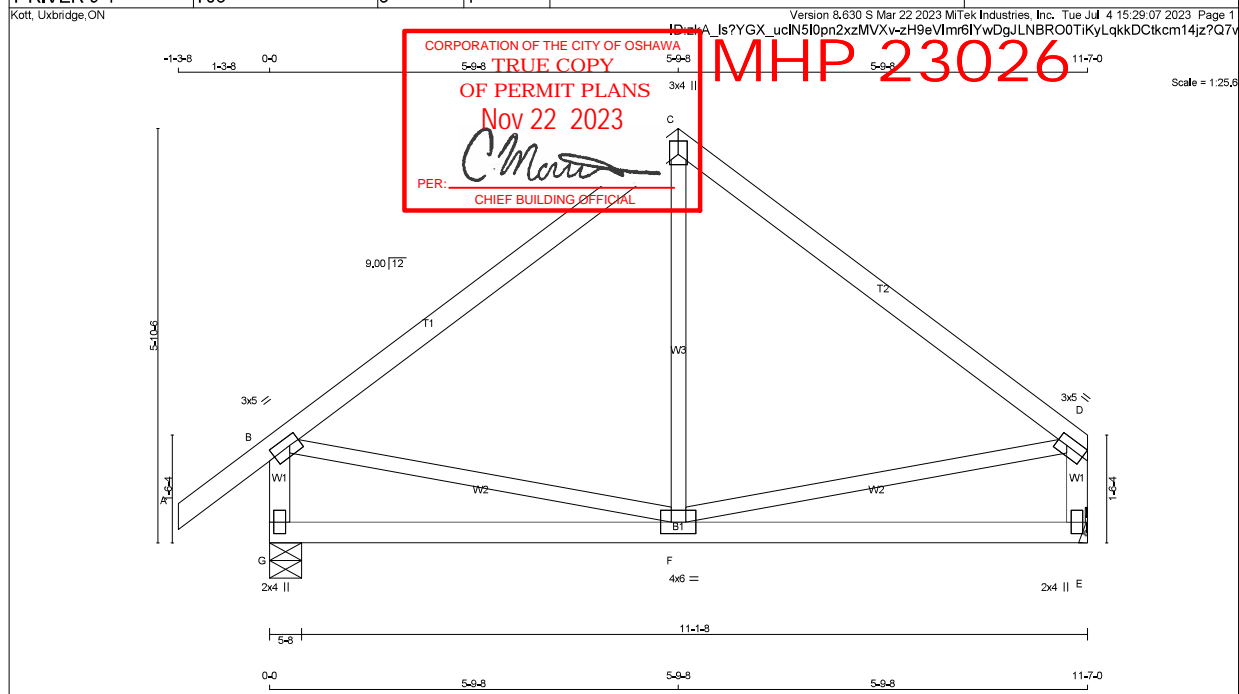
REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT

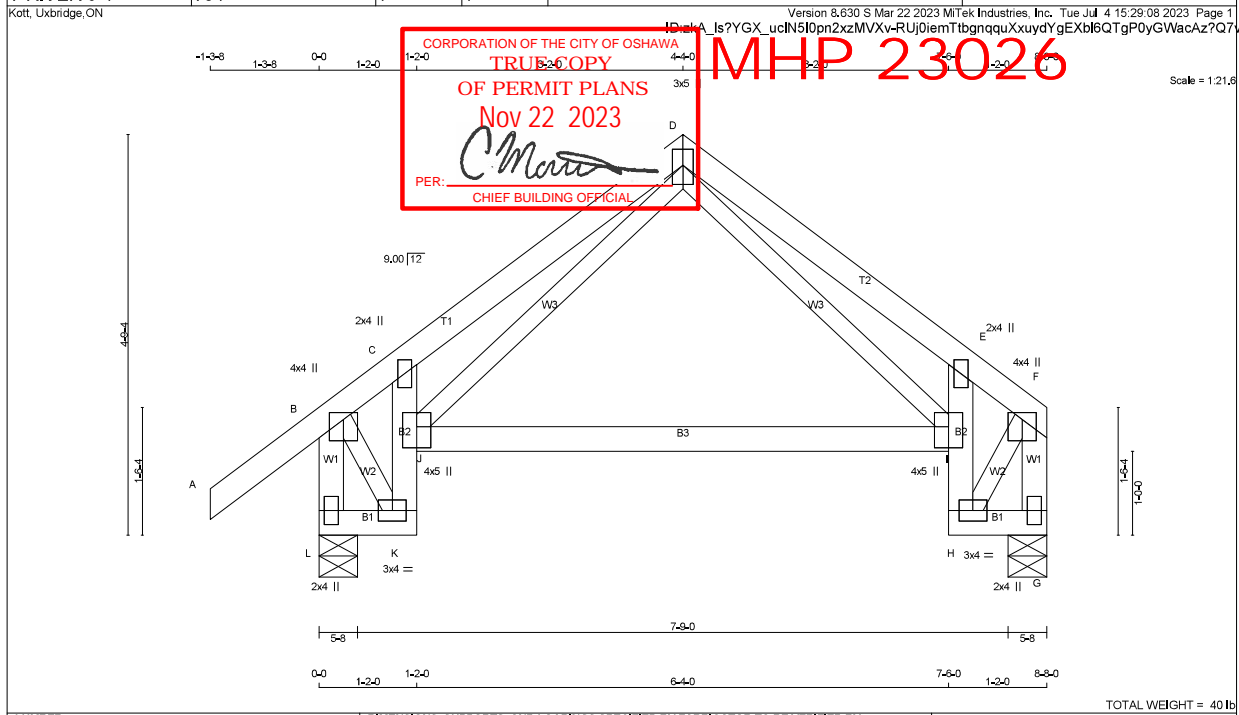
VOIDS THE ENGINEERS SEAL



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - E 2x4 DRY No.2 SPF E - H 2x4 DRY No.2 SPF M - B 2x4 DRY No.2 SPF I - G 2x4 DRY No.2 SPF M - K 2x4 DRY No.2 SPF K - I 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY, SEASONED LUMBER.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION DOWN VERT M 1742 0 I 1742 0 MAXIMUM FACTORED GROSS REACTION UP LIFT J 5-8 K 5-8 L 5-8 INPUT BRG IN-SX 5-8 1-14 5-8 1-14 REQRD BRG IN-SX 1-14 1-14 UNFACTORED REACTIONS 1ST CASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL M 1215 894 / 0 0 / 0 0 / 0 321 / 0 0 / 0 I 1215 894 / 0 0 / 0 0 / 0 321 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX. FACTORED (PLF) CSI (LC) UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) CSI (LC) WEBS MEMB. MAX. FACTORED FORCE (LBS) CSI (LC) FR-TO A-B 0 / 49 -119.4 -119.4 0.16 (1) 10.00 C-L -225 / 4 0.20 (1) B-C 0 / 55 -119.4 -119.4 0.45 (1) 10.00 L-D 0 / 266 0.06 (1) C-D -1403 / 0 -119.4 -119.4 0.46 (1) 4.96 L-E 0 / 0 0.00 (1) D-E -1095 / 0 -119.4 -119.4 0.37 (1) 5.57 J-F 0 / 266 0.06 (1) E-F -1403 / 0 -119.4 -119.4 0.46 (1) 4.96 J-F -225 / 4 0.20 (1) F-G 0 / 55 -119.4 -119.4 0.45 (1) 10.00 M-C -1885 / 0 0.84 (1) G-H 0 / 49 -119.4 -119.4 0.16 (1) 10.00 F-I -1885 / 0 0.84 (1) M-B -285 / 0 0.0 0.0 0.03 (1) 7.81 I-G -285 / 0 0.0 0.0 0.03 (1) 7.81 M-L 0 / 1277 -18.2 -18.2 0.39 (4) 10.00 L-K 0 / 1095 -18.2 -18.2 0.36 (4) 10.00 K-J 0 / 1095 -18.2 -18.2 0.36 (4) 10.00 J-I 0 / 1277 -18.2 -18.2 0.39 (1) 10.00	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIG LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018 - NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F., SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.76") CALCULATED VERT. DEFL.(LL) = L/999 (0.05") ALLOWABLE DEFL.(TL) = L/360 (0.76") CALCULATED VERT. DEFL.(TL) = L/999 (0.24") CSI TC=0.46/1.00 (C-D-1), BC=0.39/1.00 (I-J-1), WB=0.84/1.00 (C-M-1), SS=0.24/1.00 (E-F-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PU) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (D) (INPUT = 0.90) JSI METAL= 0.49 (K) (INPUT = 1.00)
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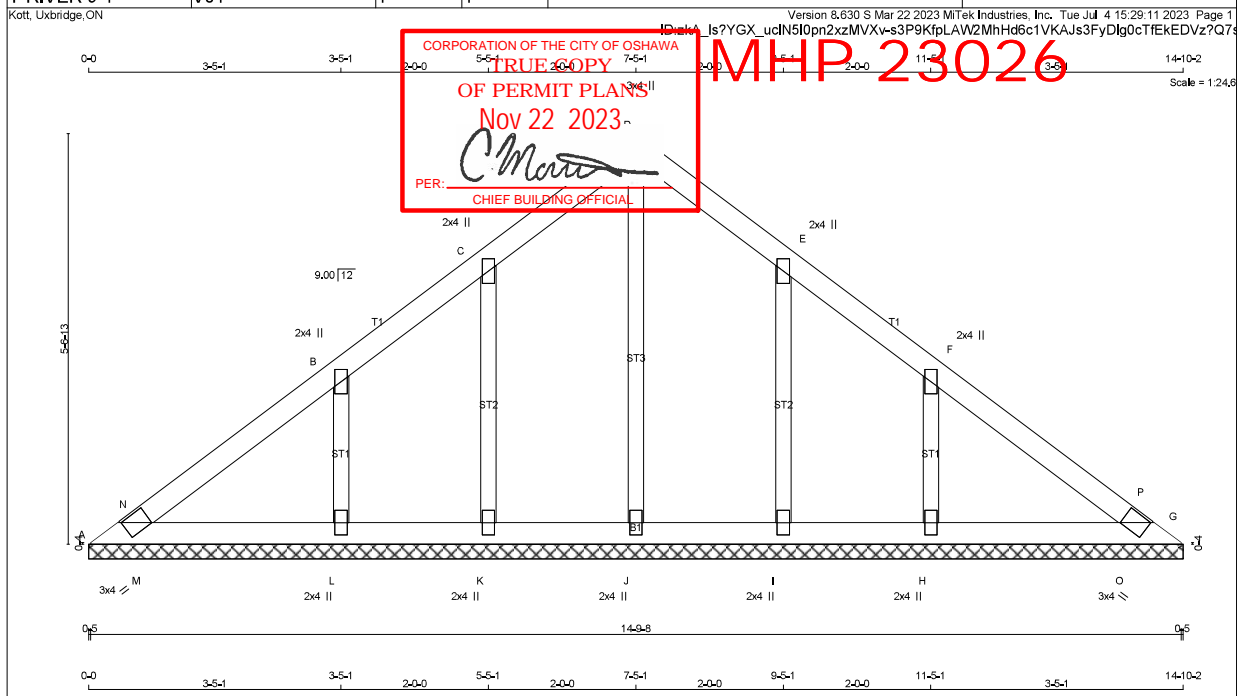
<div>LUMBER</div> <div>N. L. G. A. RULES</div> <div>CHORDS SIZE LUMBER DESCR.</div> <div>A - C 2x4 DRY 2100F 1.8E SPF</div> <div>C - D 2x4 DRY 2100F 1.8E SPF</div> <div>G - B 2x4 DRY No.2 SPF</div> <div>E - D 2x4 DRY No.2 SPF</div> <div>G - E 2x4 DRY No.2 SPF</div> <div>ALL WEBS 2x3 DRY No.2 SPF</div> <div>EXCEPT</div> <div>DRY: SEASONED LUMBER.</div>		<div>DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER</div> <div>BEARINGS</div> <table><thead><tr><th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQRD BRG</th></tr><tr><th>JT</th><th>VERT</th><th>DOWN</th><th>HORZ</th><th>UPLIFT</th></tr></thead><tbody><tr><td>G</td><td>962</td><td>0</td><td>962</td><td>0</td></tr><tr><td>E</td><td>797</td><td>0</td><td>797</td><td>0</td></tr></tbody></table> <div>A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, MINIMUM BEARING LENGTH AT JOINT E = 1'-8".</div>			FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT	VERT	DOWN	HORZ	UPLIFT	G	962	0	962	0	E	797	0	797	0	<div>DESIGN CRITERIA</div> <div>SPECIFIED LOADS:</div> <div>TOP CH. LL = 34.8 PSF</div> <div>DL = 6.0 PSF</div> <div>BOT CH. LL = 0.0 PSF</div> <div>DL = 7.3 PSF</div> <div>TOTAL LOAD = 48.1 PSF</div> <div>SPACING = 24.0 IN. G/C</div> <div>THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015</div> <div>THIS DESIGN COMPLIES WITH:</div> <div>- PART 9 OF BCBC 2018, NBC-2019AE</div> <div>- PART 9 OF OBC 2012 (2019 AMENDMENT)</div> <div>- CSA 086-14</div> <div>- TPIC 2014</div> <div>(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD</div> <div>ALLOWABLE DEFL.(LL)= L/360 (0.39")</div> <div>CALCULATED VERT. DEFL.(LL) = L/999 (0.01")</div> <div>ALLOWABLE DEFL.(TL)= L/360 (0.39")</div> <div>CALCULATED VERT. DEFL.(TL) = L/999 (0.03")</div> <div>CSI TC=0.34/1.00 (C-D:1) , BC=0.18/1.00 (F-G:4) , WB=0.10/1.00 (B-F:1) , SSI=0.22/1.00 (C-D:1)</div> <div>DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10</div> <div>COMPANION LIVE LOAD FACTOR = 1.00</div> <div>TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .</div> <div>NAIL VALUES</div> <div>PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL) (PL)</div> <div>MAX MIN MAX MIN MAX MIN</div> <div>MT20 650 371 1747 788 1987 1873</div> <div>PLATE PLACEMENT TOL. = 0.250 inches</div> <div>PLATE ROTATION TOL. = 5.0 Deg.</div> <div>JSI GRIP= 0.74 (D) (INPUT = 0.90)</div> <div>JSI METAL= 0.24 (B) (INPUT = 1.00)</div>																																																																																																																												
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<div>PLATES (table is in inches)</div> <table><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>B</td><td>TMVW4</td><td>MT20</td><td>3.0</td><td>5.0</td><td>1.50</td><td>1.75</td></tr><tr><td>C</td><td>TTW+p</td><td>MT20</td><td>3.0</td><td>4.0</td><td>2.25</td><td>1.50</td></tr><tr><td>D</td><td>TMVW4</td><td>MT20</td><td>3.0</td><td>5.0</td><td>1.50</td><td>Edge</td></tr><tr><td>E</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>F</td><td>BMVW4</td><td>MT20</td><td>4.0</td><td>6.0</td><td></td><td></td></tr><tr><td>G</td><td>BMV1+p</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr></tbody></table> <div>Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.</div>		JT	TYPE	PLATES	W	LEN	Y	X	B	TMVW4	MT20	3.0	5.0	1.50	1.75	C	TTW+p	MT20	3.0	4.0	2.25	1.50	D	TMVW4	MT20	3.0	5.0	1.50	Edge	E	BMV1+p	MT20	2.0	4.0			F	BMVW4	MT20	4.0	6.0			G	BMV1+p	MT20	2.0	4.0			<div>UNFACTORED REACTIONS</div> <table><thead><tr><th>JT</th><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM.LIVE</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr></thead><tbody><tr><td>G</td><td>670</td><td>498 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>171 / 0</td><td>0 / 0</td></tr><tr><td>E</td><td>557</td><td>403 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>154 / 0</td><td>0 / 0</td></tr></tbody></table> <div>BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G</div> <div>BRACING</div> <div>TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.</div> <div>MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.</div> <div>ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.</div> <div>LOADING</div> <div>TOTAL LOAD CASES: (4)</div> <table><thead><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PLF)</th><th>MAX. LOAD LC1</th><th>MEMB.</th><th>MAX. FACTORED FORCE (LBS)</th><th>MAX. LOAD LC1</th><th>MAX. UNBRACED LENGTH FR-TO</th></tr></thead><tbody><tr><td>A-B</td><td>0 / 49</td><td>-119.4</td><td>-119.4 0.11 (1)</td><td>10.00</td><td>F-C</td><td>-47 / 95</td><td>0.03 (4)</td></tr><tr><td>B-C</td><td>-537 / 0</td><td>-119.4</td><td>-119.4 0.34 (1)</td><td>6.25</td><td>B-F</td><td>0 / 439</td><td>0.10 (1)</td></tr><tr><td>C-D</td><td>-537 / 0</td><td>-119.4</td><td>-119.4 0.34 (1)</td><td>6.25</td><td>F-D</td><td>0 / 439</td><td>0.10 (1)</td></tr><tr><td>G-B</td><td>-922 / 0</td><td>0.0</td><td>0.0 0.10 (1)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>E-D</td><td>-757 / 0</td><td>0.0</td><td>0.0 0.08 (1)</td><td>7.81</td><td></td><td></td><td></td></tr><tr><td>G-F</td><td>0 / 0</td><td>-18.2</td><td>-18.2 0.18 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>F-E</td><td>0 / 0</td><td>-18.2</td><td>-18.2 0.18 (4)</td><td>10.00</td><td></td><td></td><td></td></tr></tbody></table>		JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	G	670	498 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0	E	557	403 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LOAD LC1	MAX. UNBRACED LENGTH FR-TO	A-B	0 / 49	-119.4	-119.4 0.11 (1)	10.00	F-C	-47 / 95	0.03 (4)	B-C	-537 / 0	-119.4	-119.4 0.34 (1)	6.25	B-F	0 / 439	0.10 (1)	C-D	-537 / 0	-119.4	-119.4 0.34 (1)	6.25	F-D	0 / 439	0.10 (1)	G-B	-922 / 0	0.0	0.0 0.10 (1)	7.81				E-D	-757 / 0	0.0	0.0 0.08 (1)	7.81				G-F	0 / 0	-18.2	-18.2 0.18 (4)	10.00				F-E	0 / 0	-18.2	-18.2 0.18 (4)	10.00			
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B-C	-537 / 0	-119.4	-119.4 0.34 (1)	6.25	B-F	0 / 439	0.10 (1)																																																																																																																																													
C-D	-537 / 0	-119.4	-119.4 0.34 (1)	6.25	F-D	0 / 439	0.10 (1)																																																																																																																																													
G-B	-922 / 0	0.0	0.0 0.10 (1)	7.81																																																																																																																																																
E-D	-757 / 0	0.0	0.0 0.08 (1)	7.81																																																																																																																																																
G-F	0 / 0	-18.2	-18.2 0.18 (4)	10.00																																																																																																																																																
F-E	0 / 0	-18.2	-18.2 0.18 (4)	10.00																																																																																																																																																
<div>MODULUS ENGINEERING LTD.</div> <div>07/04/2023</div> <div>P. R. HEAL</div> <div>PROVINCE OF ONTARIO</div> <div>REVIEW FOR TRUSS COMPONENT ONLY</div> <div>NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL</div>																																																																																																																																																				



N, L, G, A, RULES										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
CHORDS SIZE LUMBER DESCR.										BEARINGS										SPECIFIED LOADS:									
A - D 2x4 DRY No.2 SPF										FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD										TOP CH. LL = 34.8 PSF									
D - F 2x4 DRY No.2 SPF										JT GROSS REACTION DOWN BRG BRG										DL = 6.0 PSF									
L - B 2x4 DRY No.2 SPF										L VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX										BOT CH. LL = 0.0 PSF									
G - F 2x4 DRY No.2 SPF										L 774 0 774 0 0 5-8 1-8										DL = 7.3 PSF									
L - K 2x4 DRY No.2 SPF										G 583 0 583 0 0 5-8 1-8										TOTAL LOAD = 48.1 PSF									
K - C 2x4 DRY No.2 SPF																													
J - I 2x4 DRY No.2 SPF																													
H - E 2x4 DRY No.2 SPF																													
H - G 2x4 DRY No.2 SPF																													
ALL WEBS 2x3 DRY No.2 SPF										UNFACTORED REACTIONS										SPACING = 24.0 IN. G.C									
EXCEPT										1ST LCASE MAX./MIN. COMPONENT REACTIONS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015									
										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL																			
										L 538 405 / 0 0 / 0 0 / 0 133 / 0 0 / 0																			
										G 408 294 / 0 0 / 0 0 / 0 114 / 0 0 / 0																			
DRY: SEASONED LUMBER.										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G										THIS DESIGN COMPLIES WITH:									
																				- PART 9 OF BCBC 2018, NBC-2019AE									
																				- PART 9 OF OBC 2012 (2019 AMENDMENT)									
																				- CSA 086-14									
																				- TPIC 2014									
																				DESIGN ASSUMPTIONS									
																				-OVERHANG NOT TO BE ALTERED OR CUT OFF.									
																				(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD									
																				ALLOWABLE DEFL.(LL)= L/360 (0.29")									
																				CALCULATED VERT. DEFL.(LL) = L/999 (0.02")									
																				ALLOWABLE DEFL.(TL)= L/360 (0.29")									
																				CALCULATED VERT. DEFL.(TL) = L/897 (0.12")									
																				CSI TC=0.22/1.00 (D-E), BC=0.40/1.00 (E-I), WB=0.10/1.00 (F-H), SSI=0.26/1.00 (E-I),									
																				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10									
																				COMPANION LIVE LOAD FACTOR = 1.00									
																				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
																				NAIL VALUES									
																				PLATE GRIP(DRY) SHEAR SECTION									
																				(PSI) (PLI) (PLI)									
																				MAX MIN MAX MIN MAX MIN									
																				MT20 650 371 1747 788 1987 1873									
																				PLATE PLACEMENT TOL. = 0.250 inches									
																				PLATE ROTATION TOL. = 5.0 Deg.									
																				JSI GRIP= 0.83 (K) (INPUT = 0.90)									
																				JSI METAL= 0.49 (E) (INPUT = 1.00)									

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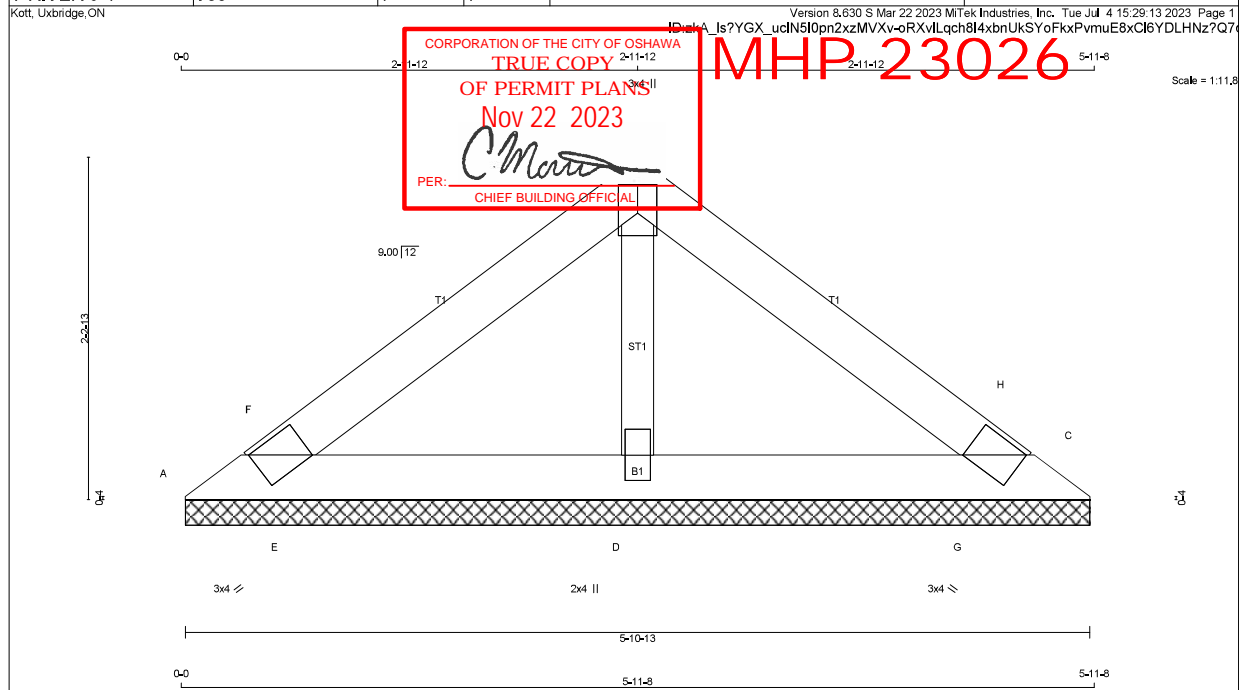




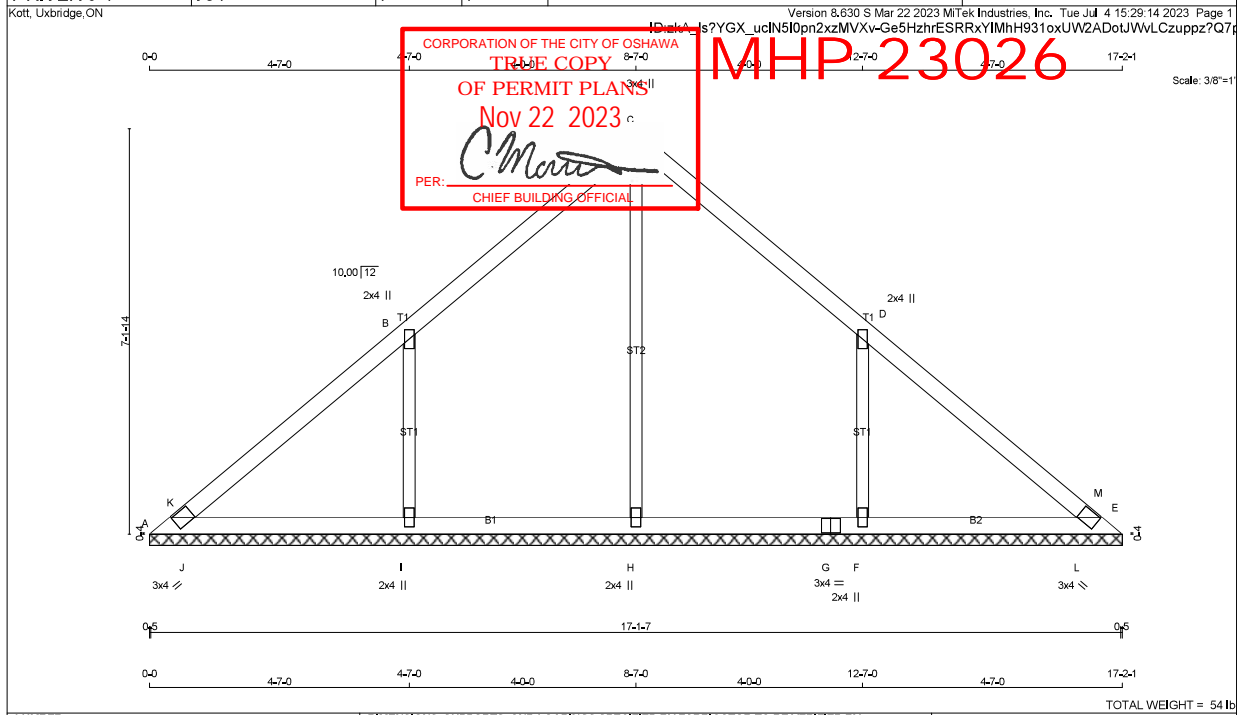
LUMBER N, L, G, A, RULES CHORDS SIZE LUMBER DESCR. SPF A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF A - G 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER.		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIG THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.13/1.00 (B-N.1), BC=0.11/1.00 (L-M.1), WB=0.14/1.00 (D-J.1), SS=0.12/1.00 (B-N.1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.22 (F) (INPUT = 0.90) JSI METAL= 0.17 (F) (INPUT = 1.00)	
PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM1+h MT20 3.0 4.0 B, C, E, F B TWM+w MT20 2.0 4.0 D TTV+w MT20 3.0 4.0 2.25 1.50 G TBM1+h MT20 3.0 4.0 H, I, J, K, L H BMV1+w MT20 2.0 4.0		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQRD VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX JT A 161 0 161 0 0 14-10-2 (14-948) G 161 0 161 0 0 14-10-2 (14-948) J 356 0 356 0 0 14-10-2 (14-948) K 240 0 240 0 0 14-10-2 (14-948) L 439 0 439 0 0 14-10-2 (14-948) I 240 0 240 0 0 14-10-2 (14-948) H 439 0 439 0 0 14-10-2 (14-948) VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL A 112 83 / 0 0 / 0 0 / 0 29 / 0 0 / 0 G 112 83 / 0 0 / 0 0 / 0 29 / 0 0 / 0 J 250 174 / 0 0 / 0 0 / 0 77 / 0 0 / 0 K 167 124 / 0 0 / 0 0 / 0 43 / 0 0 / 0 L 307 221 / 0 0 / 0 0 / 0 87 / 0 0 / 0 I 167 124 / 0 0 / 0 0 / 0 43 / 0 0 / 0 H 307 221 / 0 0 / 0 0 / 0 87 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, K, L, I, H BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MEMB. MAX. FACTORED FORCE VERT. LOAD LC1 MAX. MAX. MEMB. MAX. FACTORED FORCE MAX. FACTORED (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO FROM TO A-N 0 / 42 -119.4 -119.4 0.03 (1) 10.00 J-D -302 / 0 0.14 (1) N-B 0 / 78 -119.4 -119.4 0.13 (1) 10.00 K-C -240 / 0 0.06 (1) B-C 0 / 59 -119.4 -119.4 0.12 (1) 10.00 L-B -322 / 0 0.05 (1) C-D 0 / 66 -119.4 -119.4 0.07 (1) 10.00 I-E -240 / 0 0.06 (1) D-E 0 / 66 -119.4 -119.4 0.07 (1) 10.00 H-F -322 / 0 0.05 (1) E-F 0 / 59 -119.4 -119.4 0.12 (1) 10.00 M-N -133 / 0 0.00 (1) F-P 0 / 78 -119.4 -119.4 0.13 (1) 10.00 O-P -133 / 0 0.00 (1) P-G 0 / 42 -119.4 -119.4 0.03 (1) 10.00 A-M -66 / 0 -18.2 -18.2 0.11 (1) 6.25 M-L -44 / 0 -18.2 -18.2 0.11 (1) 6.25 L-K -57 / 0 -18.2 -18.2 0.07 (1) 6.25 K-J -63 / 0 -18.2 -18.2 0.02 (4) 6.25 J-I -53 / 0 -18.2 -18.2 0.02 (4) 6.25 I-H -57 / 0 -18.2 -18.2 0.07 (1) 6.25 H-O -44 / 0 -18.2 -18.2 0.11 (1) 6.25 O-G -66 / 0 -18.2 -18.2 0.11 (1) 6.25	



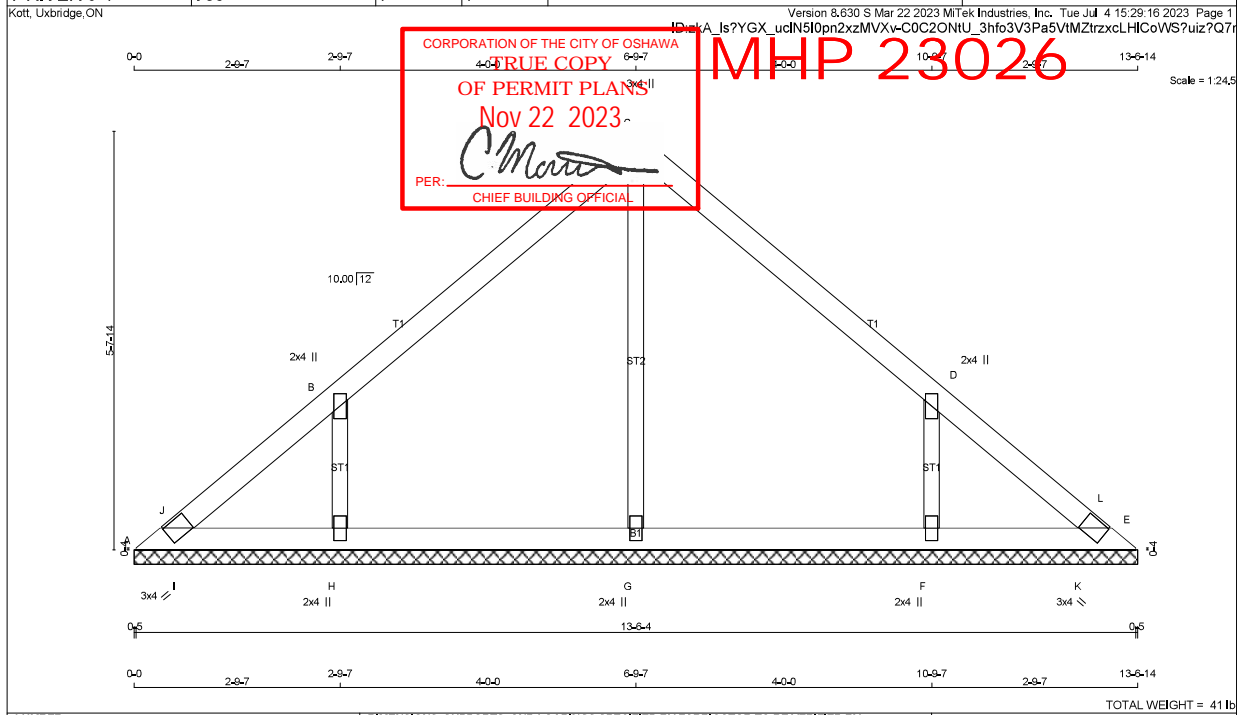
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
 Design valid for use only with Mitak connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com



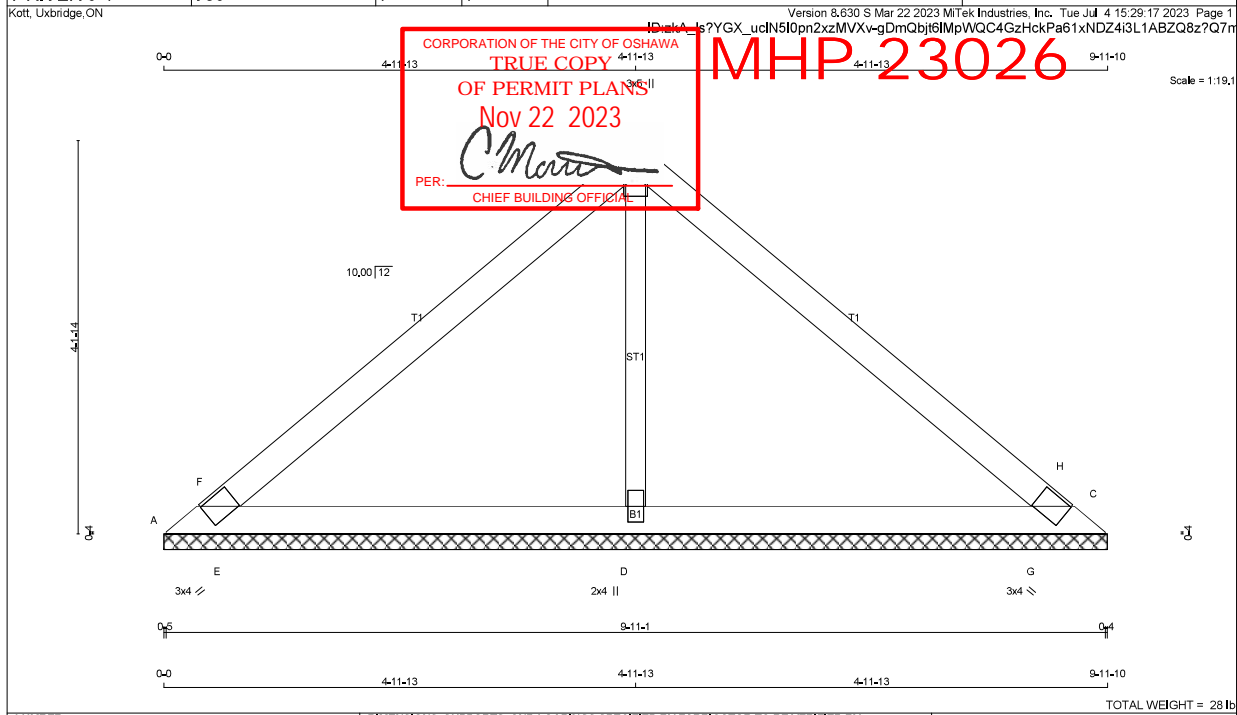
TOTAL WEIGHT = 15 (M)																																																																																									
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER A - B 2x4 DRY No.2 B - C 2x4 DRY No.2 A - C 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER.																																																																																									
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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><td></td><td>FACTORED GROSS REACTION</td><td>MAXIMUM FACTORED GROSS REACTION</td><td>INPUT BRG</td><td>REQRD BRG</td></tr><tr><td>JT</td><td>VERT</td><td>HORZ</td><td>DOWN</td><td>HORZ</td></tr><tr><td>A</td><td>98</td><td>0</td><td>98</td><td>0</td></tr><tr><td>C</td><td>98</td><td>0</td><td>98</td><td>0</td></tr><tr><td>D</td><td>616</td><td>0</td><td>616</td><td>0</td></tr></table>			FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	JT	VERT	HORZ	DOWN	HORZ	A	98	0	98	0	C	98	0	98	0	D	616	0	616	0																																																															
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UNFACTORED REACTIONS <table><tr><td></td><td>1ST LCASE</td><td colspan="5">MAX./MIN. COMPONENT REACTIONS</td></tr><tr><td>JT</td><td>COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM. LIVE</td><td>WIND</td><td>DEAD</td><td>SOIL</td></tr><tr><td>A</td><td>69</td><td>51 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>18 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>69</td><td>51 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>18 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>431</td><td>310 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td><td>121 / 0</td><td>0 / 0</td></tr></table>			1ST LCASE	MAX./MIN. COMPONENT REACTIONS					JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	A	69	51 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	C	69	51 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	D	431	310 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0																																																	
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<table><tr><td colspan="4">CHORDS</td><td colspan="4">WEBS</td></tr><tr><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>FACTORED VERT. LOAD LC1 (PLF)</td><td>MAX. CSI (LC)</td><td>MEMB.</td><td>MAX. FACTORED FORCE (LBS)</td><td>MAX. CSI (LC)</td><td></td></tr><tr><td>FR-TO</td><td></td><td>FROM</td><td>TO</td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>A-F</td><td>0 / 150</td><td>-119.4</td><td>-119.4, 0.04 (1)</td><td>10.00</td><td>D-B</td><td>-443 / 0</td><td>0.07 (1)</td></tr><tr><td>F-B</td><td>0 / 154</td><td>-119.4</td><td>-119.4, 0.12 (1)</td><td>10.00</td><td>E-F</td><td>-171 / 0</td><td>0.00 (1)</td></tr><tr><td>B-H</td><td>0 / 154</td><td>-119.4</td><td>-119.4, 0.12 (1)</td><td>10.00</td><td>G-H</td><td>-171 / 0</td><td>0.00 (1)</td></tr><tr><td>H-C</td><td>0 / 150</td><td>-119.4</td><td>-119.4, 0.04 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>A-E</td><td>-149 / 0</td><td>-18.2</td><td>-18.2, 0.11 (1)</td><td>6.25</td><td></td><td></td><td></td></tr><tr><td>E-D</td><td>-126 / 0</td><td>-18.2</td><td>-18.2, 0.11 (1)</td><td>6.25</td><td></td><td></td><td></td></tr><tr><td>D-G</td><td>-126 / 0</td><td>-18.2</td><td>-18.2, 0.11 (1)</td><td>6.25</td><td></td><td></td><td></td></tr><tr><td>G-C</td><td>-149 / 0</td><td>-18.2</td><td>-18.2, 0.11 (1)</td><td>6.25</td><td></td><td></td><td></td></tr></table>		CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		FR-TO		FROM	TO	FR-TO				A-F	0 / 150	-119.4	-119.4, 0.04 (1)	10.00	D-B	-443 / 0	0.07 (1)	F-B	0 / 154	-119.4	-119.4, 0.12 (1)	10.00	E-F	-171 / 0	0.00 (1)	B-H	0 / 154	-119.4	-119.4, 0.12 (1)	10.00	G-H	-171 / 0	0.00 (1)	H-C	0 / 150	-119.4	-119.4, 0.04 (1)	10.00				A-E	-149 / 0	-18.2	-18.2, 0.11 (1)	6.25				E-D	-126 / 0	-18.2	-18.2, 0.11 (1)	6.25				D-G	-126 / 0	-18.2	-18.2, 0.11 (1)	6.25				G-C	-149 / 0	-18.2	-18.2, 0.11 (1)	6.25			
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DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 240 IN. CIC THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.12/1.00 (B-H:1), BC=0.11/1.00 (D-G:1), WB=0.07/1.00 (B-D:1), SSI=0.09/1.00 (C-G:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.30 (B) (INPUT = 0.90) JSI METAL= 0.10 (B) (INPUT = 1.00)																																																																																									
MODULUS ENGINEERING LTD. 07/04/2023 P. R. HEAL PROVINCE OF ONTARIO REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL																																																																																									



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - C 2x4 DRY No.2 C - E 2x4 DRY No.2 A - G 2x4 DRY No.2 G - E 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX A 185 0 185 0 0 17'-2-1 (5'-1-7 1-8 E 185 0 185 0 0 17'-2-1 (5'-1-7 1-8 H 555 0 555 0 0 17'-2-1 (5'-1-7 1-8 I 716 0 716 0 0 17'-2-1 (5'-1-7 1-8 F 716 0 716 0 0 17'-2-1 (5'-1-7 1-8 VALUE IN PARENTHESES INDICATES EFFECTIVE BEARING LENGTH UNFACTORED REACTIONS JT COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL A 128 98 / 0 0 / 0 0 / 0 31 / 0 0 / 0 E 128 98 / 0 0 / 0 0 / 0 31 / 0 0 / 0 H 391 264 / 0 0 / 0 0 / 0 127 / 0 0 / 0 I 500 366 / 0 0 / 0 0 / 0 134 / 0 0 / 0 F 500 366 / 0 0 / 0 0 / 0 134 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 MAX. MAX. WEBS MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) MEMB. FR-TO FROM TO LENGTH FR-TO A-K 0 / 82 -119.4 -119.4 0.04 (4) 10.00 H-C -517 / 0 0.45 (1) K-B 0 / 134 -119.4 -119.4 0.35 (1) 10.00 I-B -581 / 0 0.13 (1) B-C 0 / 76 -119.4 -119.4 0.34 (1) 10.00 F-D -581 / 0 0.13 (1) C-D 0 / 76 -119.4 -119.4 0.34 (1) 10.00 J-K -184 / 5 0.00 (1) D-M 0 / 134 -119.4 -119.4 0.35 (1) 10.00 L-M -184 / 5 0.00 (1) M-E 0 / 82 -119.4 -119.4 0.04 (4) 10.00 A-J -108 / 0 -18.2 -18.2 0.15 (1) 6.25 J-I -71 / 0 -18.2 -18.2 0.15 (1) 6.25 I-H -87 / 0 -18.2 -18.2 0.10 (1) 6.25 H-G -87 / 0 -18.2 -18.2 0.10 (1) 6.25 G-F -87 / 0 -18.2 -18.2 0.10 (1) 6.25 F-L -71 / 0 -18.2 -18.2 0.15 (1) 6.25 L-E -108 / 0 -18.2 -18.2 0.15 (1) 6.25	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIG THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN/LD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.35/1.00 (B-K 1), BC=0.15/1.00 (I-J 1), WB=0.45/1.00 (C-H 1), SSI=0.18/1.00 (B-C 1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.65 (C) (INPUT = 0.90) JSI METAL= 0.31 (D) (INPUT = 1.00)
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - C 2x4 DRY No.2 C - E 2x4 DRY No.2 A - E 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER.		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. GIG THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.27/1.00 (B-C:1), BC=0.06/1.00 (G-H:4), WB=0.14/1.00 (C-G:1), SSI=0.17/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.38 (C) (INPUT = 0.90) JSI METAL= 0.27 (D) (INPUT = 1.00)	
PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM1-h MT20 3.0 4.0 B TMW1+w MT20 2.0 4.0 C TTW1+p MT20 3.0 4.0 2.50 1.50 D TMW1+w MT20 2.0 4.0 E TBM1-h MT20 3.0 4.0 F, G, H F BMW1+w MT20 2.0 4.0		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX JT 159 0 159 0 0 13'-6" (13'-6") A 159 0 159 0 0 13'-6" (13'-6") E 377 0 377 0 0 13'-6" (13'-6") G 583 0 583 0 0 13'-6" (13'-6") H 583 0 583 0 0 13'-6" (13'-6") F 583 0 583 0 0 13'-6" (13'-6") VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH UNFACTORED REACTIONS JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL A 111 84/0 0/0 0/0 0/0 27/0 0/0 E 111 84/0 0/0 0/0 0/0 27/0 0/0 G 267 172/0 0/0 0/0 0/0 95/0 0/0 H 406 301/0 0/0 0/0 0/0 106/0 0/0 F 406 301/0 0/0 0/0 0/0 106/0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. MAX. MEMB. FORCE (LBS) MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) FR-TO FROM TO LENGTH FR-TO A-J -119/0 -119.4 -119.4 0.05 (1) 6.25 G-C -304/0 0.14 (1) J-B -29/2 -119.4 -119.4 0.26 (1) 6.25 H-B -507/0 0.08 (1) B-C -101/0 -119.4 -119.4 0.27 (1) 6.25 F-D -507/0 0.08 (1) C-D -101/0 -119.4 -119.4 0.27 (1) 6.25 I-J -8/7 0.00 (1) D-L -29/2 -119.4 -119.4 0.26 (1) 6.25 K-L -8/7 0.00 (1) L-E -119/0 -119.4 -119.4 0.05 (1) 6.25 A-I 0/75 -18.2 -18.2 0.04 (1) 10.00 I-H 0/79 -18.2 -18.2 0.05 (4) 10.00 H-G 0/54 -18.2 -18.2 0.06 (4) 10.00 G-F 0/54 -18.2 -18.2 0.06 (4) 10.00 F-K 0/79 -18.2 -18.2 0.05 (4) 10.00 K-E 0/75 -18.3 -18.3 0.04 (1) 10.00	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - B 2x4 DRY No.2 B - C 2x4 DRY No.2 A - C 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 DRY, SEASONED LUMBER, SPF		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION JT VERT HORZ A 29 0 C 29 0 D 1308 0 MAXIMUM FACTORED GROSS REACTION DOWN HORZ UPLIFT 29 0 0 29 0 0 1308 0 0 INPUT BRG IN-SX 9-11-10 (9-11+12 9-11-10 (9-11+12 9-11-10 (9-11+12 REQD BRG IN-SX 9-11-10 (9-11+12 9-11-10 (9-11+12 9-11-10 (9-11+12		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD CSI TC=0.37/1.00 (B-F:1) , BC=0.27/1.00 (D-E:1) , WB=0.27/1.00 (B-D:1) , SSI=0.23/1.00 (A-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT . NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.53 (D) (INPUT = 0.90) JSI METAL= 0.22 (D) (INPUT = 1.00)
PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TBM1+h MT20 3.0 4.0 B TTV+h MT20 3.0 5.0 C TBM1+h MT20 3.0 4.0 D BMW1+w MT20 2.0 4.0		UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL A 20 14/0 0/0 0/0 0/0 6/0 0/0 C 20 14/0 0/0 0/0 0/0 6/0 0/0 D 914 662/0 0/0 0/0 0/0 251/0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS W E B S MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX (PLF) MAX. UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. UNBRAC LENGTH FR-TO A-F 0/509 -119.4 -119.4 0.25 (1) 10.00 D-B -1061/0 0.27 (1) F-B 0/454 -119.4 -119.4 0.37 (1) 10.00 E-F -413/0 0.00 (1) B-H 0/454 -119.4 -119.4 0.37 (1) 10.00 G-H -413/0 0.00 (1) H-C 0/509 -119.4 -119.4 0.25 (1) 10.00 A-E -434/0 -18.2 -18.2 0.27 (1) 6.25 E-D -362/0 -18.2 -18.2 0.27 (1) 6.25 D-G -362/0 -18.2 -18.2 0.27 (1) 6.25 G-C -434/0 -18.3 -18.3 0.27 (1) 6.25		MODULUS ENGINEERING LTD. 07/04/2023 P. R. HEAL PROVINCE OF ONTARIO REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.
Design valid for use only with Miltek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult
TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.bscindustry.com