

CONVENTIONAL
FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK GROUP
Job #	23-00093R0
Address	ZADORRA ESTATES ZADORRA ESTATES INC OSHAWA, ON
Model	RIVER 11-3
Sales Rep	RALPH MIRIGELLO
Designer	LI
Date	2023-05-02
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 11\T-RIVER 11-3\

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft ²
TC DL	6.0 lb/ft ²
BC LL	0.0 lb/ft ²
BC DL	7.3 lb/ft ²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

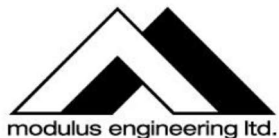
Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400





General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



**Read Carefully Prior to
Manufacture and Installation**

Note: It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-triangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

Failure to follow these guidelines could cause property damage and personal injury

1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult **BCSI-CANADA** for installation requirements (copies available from your truss supplier or from www.sbcindustry.com)

2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.

3. Never exceed the design loading shown and never stack building materials on inadequately braced trusses

4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all other interested parties (e.g. Building designer where required)

5. Cut members to bear tightly against one another

6. Place plates on each face of truss at each joint and embed fully, using proper roller or hydraulic press. Knots and wane at joint locations are regulated by TPIC Appendix G

7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC

8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture

9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in a corrosive environment

10. Connections not shown are the responsibility of others

11. Do not cut or alter truss members or plates without prior approval of an engineer

12. Install and load vertically unless otherwise noted

13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient

14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G

16. Building designer must review individual component drawings to ensure they are suitable for the structure

15. Not designed for solar panels unless specifically noted

PROVIDED TO THE CITY OF CANADA
TRUE COPY
OF PERMIT PLANS
Nov 16 2023
CHIEF BUILDING OFFICIAL

MHP 23027

JOB NAME T-RIVER 11-3 Kott, Uxbridge, ON	TRUSS NAME G03	QUANTITY 1	BY TRUE COPY	JOB DESC. CORPORATION OF THE CITY OF OSHAWA TRUSS DESIG.	ZADORRA ESTATES INC	DRWG NO. ME23-5781-3
				Version 8.630 S Mar 22 2023 Mitek Industries, Inc. Wed Jul 5 11:52:10 2023 Page 1 ID: QRDxuUhhXJBjX8m6PaVhmZzKxFR-rzIR_UrQJD1Ky3nzQT_gSV00UNI749C73QdCqz78D.		
				Scale = 1:10.2		
				TOTAL WEIGHT = 12 lb		
				[M]		
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. E - A 2x4 DRY No.2 SPF A - B 2x4 DRY No.2 SPF B - C 2x6 DRY No.2 SPF D - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY, SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX E 215 0 215 0 0 5-8 1-8 D 212 0 212 0 0 MECHANICAL A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D, MINIMUM BEARING LENGTH AT JOINT D = 1-8. UNFACTORED REACTIONS 1ST LOASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL E 150 110/0 0/0 0/0 0/0 40/0 0/0 D 148 108/0 0/0 0/0 0/0 39/0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10,00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) C H O R D S W E B S MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD (PLF) MAX. (LC) UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX. (LC) FR-TO FROM TO E-A -102/0 0,0 0,0 0,01 (1) 7,81 E-B -87/0 0,02 (1) A-B 0/0 -119,4 -119,4 0,07 (1) 10,00 B-D -125/0 0,02 (1) B-C 0/0 -105,7 -105,7 0,01 (1) 10,00 D-C -51/0 0,0 0,0 0,01 (1) 7,81 E-F 0/62 -39,1 -39,1 0,06 (1) 10,00 F-D 0/62 -39,1 -39,1 0,06 (1) 10,00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX+ MAX- FACE DIR. TYPE HEEL CONN. B 1-8-8 -17 -17 - FRONT VERT TOTAL - C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.		
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34,8 PSF DL = 6,0 PSF BOT CH. LL = 0,0 PSF DL = 7,3 PSF TOTAL LOAD = 48,1 PSF SPACING = 24.0 IN. G/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2,00/12 MINIMUM GIRDER TYPE: CPrimeHip LEFT SETBACK = 1-8-8 RIGHT SETBACK = 0-0 END SETBACK = 2-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55 % OF GSL. GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 2-8-0 END DISTANCE = 2-8-0 END SPAN CARRIED = 2-8-0 END WALL WIDTH = 0-0 APPLIED TO BACK SIDE OF BOTTOM CHORD. - ADDTL LOADS BASED ON 55 % OF GSL. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2015 (NBC-2019AE) - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48,1 P.S.F. G.S.L. PLUS 8,4 P.S.F. RAIN LOAD) EQUALS 34,8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0,19") CALCULATED VERT. DEFL.(LL) = L/999 (0,00") ALLOWABLE DEFL.(TL)= L/360 (0,19") CALCULATED VERT. DEFL.(TL) = L/999 (0,01") CSI TC=0,07/1,00 (A-B:1) , BC=0,06/1,00 (D-E:1) , WB=0,02/1,00 (B-D:1) , SSI=0,08/1,00 (A-B:1) DOL LUMBER=1,00 NAIL=1,00 LS BEND=1,00 COMP=1,00 SHEAR=1,00 TENS= 1,00 COMPANION LIVE LOAD FACTOR = 1,00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0,250 inches PLATE ROTATION TOL. = 5,0 Deg. JSI GRIP= 0,16 (D) (INPUT = 0,90) JSI METAL= 0,04 (B) (INPUT = 1,00)						
MODULUS ENGINEERING LTD. 07/05/2023 D. A. SHERMAN 100123373 PROVINCE OF ONTARIO REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL						
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com				KOTT		



TOTAL WEIGHT = 5 X 17 = 85 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.73/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

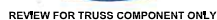
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of design is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-7C001 (VER 06/2017) BEFORE USE.
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IVIMP 23027



[M

DESCR
SPF
SPF
SPF

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	368	0	368	0	0	5-8	1-8
C	86	0	86	0	0	1-8	1-8
D	45	0	51	0	0	1-8	1-8

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	259	177 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	59	50 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD L1 (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
E-B	-304/0	0,0	0,0	13,4		7,81	
A-B	0/36	-119,4	-119,4	0,16	(1)	10,00	
B-C	-12/0	-119,4	-119,4	0,07	(1)	6,25	
E-F	0/0	-18,2	-18,2	0,13	(4)	10,00	
F-G	0/0	-18,2	-18,2	0,13	(4)	10,00	
G-D	0/0	-18,2	-18,2	0,13	(4)	10,00	

4) Q4: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA				
SPECIFIED LOADS:				
TOP CH.	LL =	34.8	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.3	PSF	
TOTAL LOAD	=	48.1	PSF	

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.16/1.00 (A-B:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

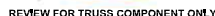
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 187

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



NOTE: ALTERING THIS DOCUMENT
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.com and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindstry.com





KOTT

JOB NAME

T-RIVER 11-3

TRUSS NAME

J02

QUANTITY

7

PLY

1

JOB DESC.

ZADORRA ESTATES INC

DRWG NO.

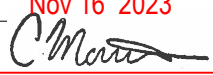
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CORPORATION OF THE CITY OF OSHAWA

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OF PERMIT PLANS

Nov 16 2023

PER: 

CHIEF BUILDING OFFICIAL

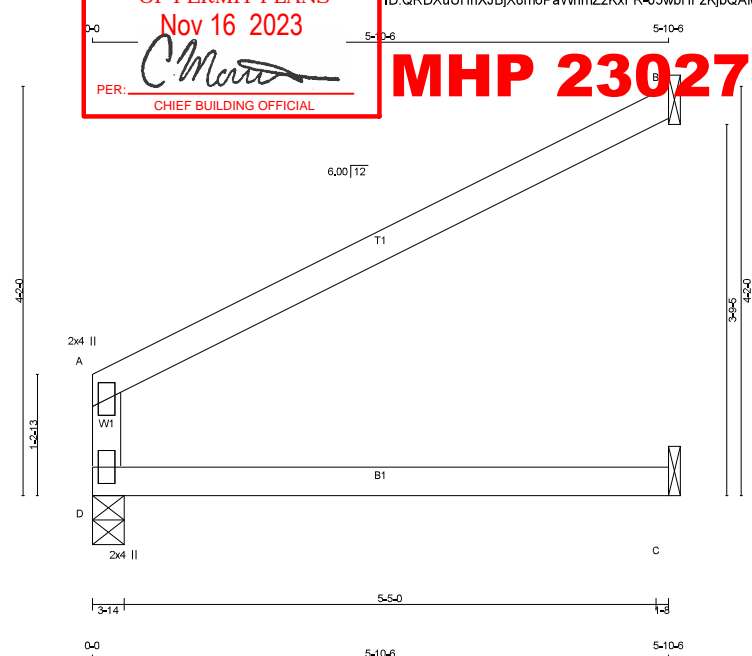
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Version 8.630 S Mar 22 2023 Mitek Industries, Inc. Wed Jul 5 11:52:21 2023 Page 1

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Kott, Uxbridge, ON

Scale = 1:18,4



LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER

D - A 2x4 DRY No.2

A - B 2x4 DRY No.2

D - C 2x4 DRY No.2

DRY: SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
D	404 0	404 0	3-14	1-8
B	318 0	318 0	1-8	1-8
C	86 0	86 0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	282	204 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
B	218	182 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0
C	64	22 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6,25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
D-A	-383 / 0	0,0	0,0 0,22 (1)	D-A	-383 / 0	0,0	0,22 (1)
A-B	-15 / 0	-119,4	-119,4 0,58 (1)	A-B	-15 / 0	-119,4	0,58 (1)
D-C	0 / 0	-18,2	-18,2 0,28 (1)	D-C	0 / 0	-18,2	0,28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34,8 PSF

DL = 6,0 PSF

BOT CH. LL = 0,0 PSF

DL = 7,3 PSF

TOTAL LOAD = 48,1 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 48,1 P.S.F. G.S.L. PLUS 8,4 P.S.F. RAIN LOAD) EQUALS 34,8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0,20")

CALCULATED VERT. DEFL.(LL) = L/999 (0,07")

ALLOWABLE DEFL.(TL) = L/360 (0,20")

CALCULATED VERT. DEFL.(TL) = L/504 (0,14")

CS1 TC=0,58/1,00 (A-B:1), BC=0,28/1,00 (C-D:1), WB=0,00/1,00 (n/a:0), SS1=0,27/1,00 (A-B:1)

DOL LUMBER=1,00 NAIL=1,00 LS BEND=1,10 COMP=1,10 SHEAR=1,10 TENS=1,10

COMPANION LIVE LOAD FACTOR = 1,00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
	(PSI)	(PL)	(PL)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0,250 inches

PLATE ROTATION TOL. = 5,0 Deg.

JSI GRIP= 0,21 (A) (INPUT = 0,90)

JSI METAL= 0,16 (A) (INPUT = 1,00)

TOTAL WEIGHT = 7 X 15 = 106 lb

[M]F

MODULUS ENGINEERING LTD.

07/05/2023

D. A. SHERMAN

100123373

PROVINCE OF ONTARIO


REVIEW FOR TRUSS COMPONENT ONLY

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com



JOB NAME

T-RIVER 11-3

TRUSS NAME

J02A

QUANTITY

2

BY

1

JOB DESC.

ZADORRA ESTATES INC

DRWG NO.

ME23-5781-13

CORPORATION OF THE CITY OF OSHAWA

TRUE COPY

OF PERMIT PLANS

Nov 16 2023

PER:

CHIEF BUILDING OFFICIAL

MHP 23027

Version 8.630 S Mar 22 2023 Mitek Industries, Inc. Wed Jul 5 11:52:22 2023 Page 1

D:QRDXuUHhXJBjX8m6PaWhmZzKxFR-UHT_Vb_yUvY1_vtGSyho9_?17JR5pVRzuwKGd8z78D7

0-0

3-4-5

2-1-1

5-10-6

6.00 [12]

T1

2x4 II

A

WI

B1

D

2x4 II

C

3-1-4

3-3-15

1-8

1-11-9

1-8

0-0

3-8-9

3-8-9 3-4-5 0-1-2

2-1-1

5-10-6

Scale = 1:14.3

TOTAL WEIGHT = 2 X 13 = 25 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

D - A

2x4

DRY

No.2

A - B

2x4

DRY

No.2

D - C

2x4

DRY

No.2

DESCR.

SPF

SPF

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

A

TMV+p

MT20

2.0

4.0

D

BMV1+p

MT20

2.0

4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

INPUT

REQRD

GROSS REACTION

GROSS REACTION

BRG

BRG

JT

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

D

282

0

282

0

0

3-14

1-8

B

216

0

216

0

0

1-8

1-8

C

59

0

59

0

0

1-8

1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

D

199

135 / 0

0 / 0

0 / 0

0 / 0

63 / 0

0 / 0

B

149

121 / 0

0 / 0

0 / 0

0 / 0

28 / 0

0 / 0

C

46

7 / 0

0 / 0

0 / 0

0 / 0

39 / 0

0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

W E B S

MEMB.

MAX. FACTORED FORCE (LBS)

FACTORED VERT. LOAD (PLF)

MAX. UNBRACED LENGTH (LC)

MEMB.

MAX. FACTORED FORCE (LBS)

MAX. UNBRACED LENGTH (LC)

FR-TO

FR-TO

D-A

-234 / 0

0.0

0.0

0.05 (4)

7.81

A-B

-4 / 4

-119.4

-119.4

0.25 (1)

10.00

D-C

0 / 0

-18.2

-18.2

0.15 (4)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:

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BOT CH. LL = 0.0 PSF

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(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.02")

ALLOWABLE DEFL.(TL) = L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI TC=0.25/1.00 (A-B:1), BC=0.15/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (A) (INPUT = 0.90)

JSI METAL= 0.10 (A) (INPUT = 1.00)

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KOTT