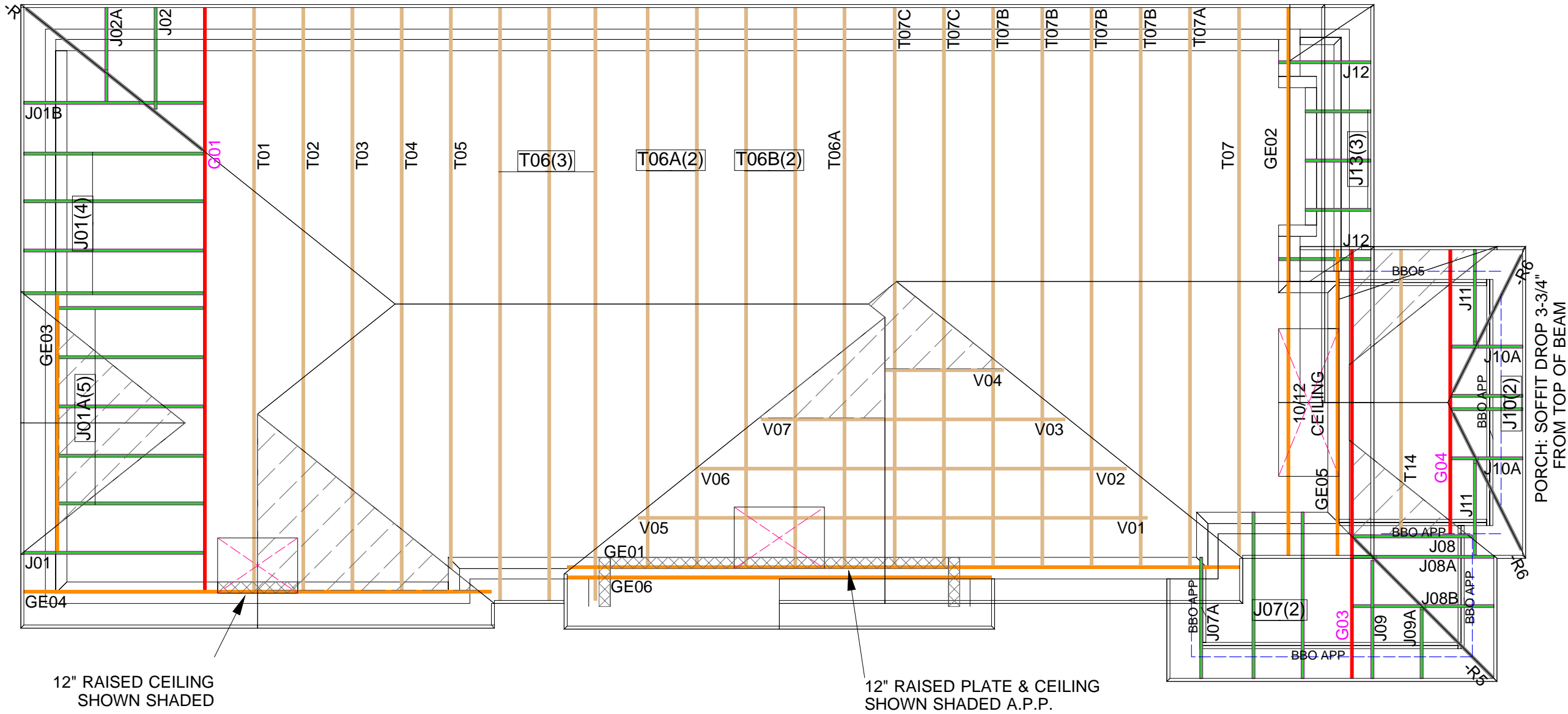


CORPORATION OF THE CITY OF OSHAWA  
 TRUE COPY  
 OF PERMIT PLANS  
 Nov 16 2023  
 PER: *C. Morin*  
 CHIEF BUILDING OFFICIAL

MHP 23027

UPPER LEVEL: TOP OF PLATE FLUSH WITH U/S SOFFIT



JOB INFORMATION	
Customer	GREENPARK GROUP
Job #	23-00091R0
Address	ZADORRA ESTATES ZADORRA ESTATES INC OSHAWA, ON
Model	RIVER 11-2
Sales Rep	RALPH MIRIGELLO
Designer	LI
Date	2023-07-05
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 11\T-RIVER 11-2\

DESIGN INFORMATION	
Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft <sup>2</sup>
TC DL	6.0 lb/ft <sup>2</sup>
BC LL	0.0 lb/ft <sup>2</sup>
BC DL	7.3 lb/ft <sup>2</sup>
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

**IMPORTANT INFORMATION**

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes


For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

 **CONVENTIONAL FRAMING BY OTHERS**

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

**KOTT Inc.**  
 14 Anderson Blvd.  
 Uxbridge, ON  
 905.642.4400





# General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



Read Carefully Prior to Manufacture and Installation

**Note:** It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

### Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

### Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

### Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

### Lumber size and Grade:

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

### Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

### Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

### Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

### Ply to ply connection:

Where the truss is designed for 2 or more plies, the individual truss plies must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

### Building Code:

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-triangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

### Chord Bracing:

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

### Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

### Design Results:

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

### Manufacturing tolerances:

Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

### Failure to follow these guidelines could cause property damage and personal injury

1. Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult **BCSI-CANADA** for installation requirements (copies available from your truss supplier or from [www.sbcindustry.com](http://www.sbcindustry.com))

2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.

3. Never exceed the design loading shown and never stack building materials on inadequately braced trusses

4. Provide copies of truss component drawings to the building department, erection supervisor, property owner and all other interested parties (e.g. Building designer where required)

5. Cut members to bear tightly against one another

6. Place plates on each face of truss at each joint and embed fully, using proper roller or hydraulic press. Knots and wane at joint locations are regulated by TPIC Appendix G

7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC

8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture

9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use in a corrosive environment

10. Connections not shown are the responsibility of others

11. Do not cut or alter truss members or plates without prior approval of an engineer

12. Install and load vertically unless otherwise noted

13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient

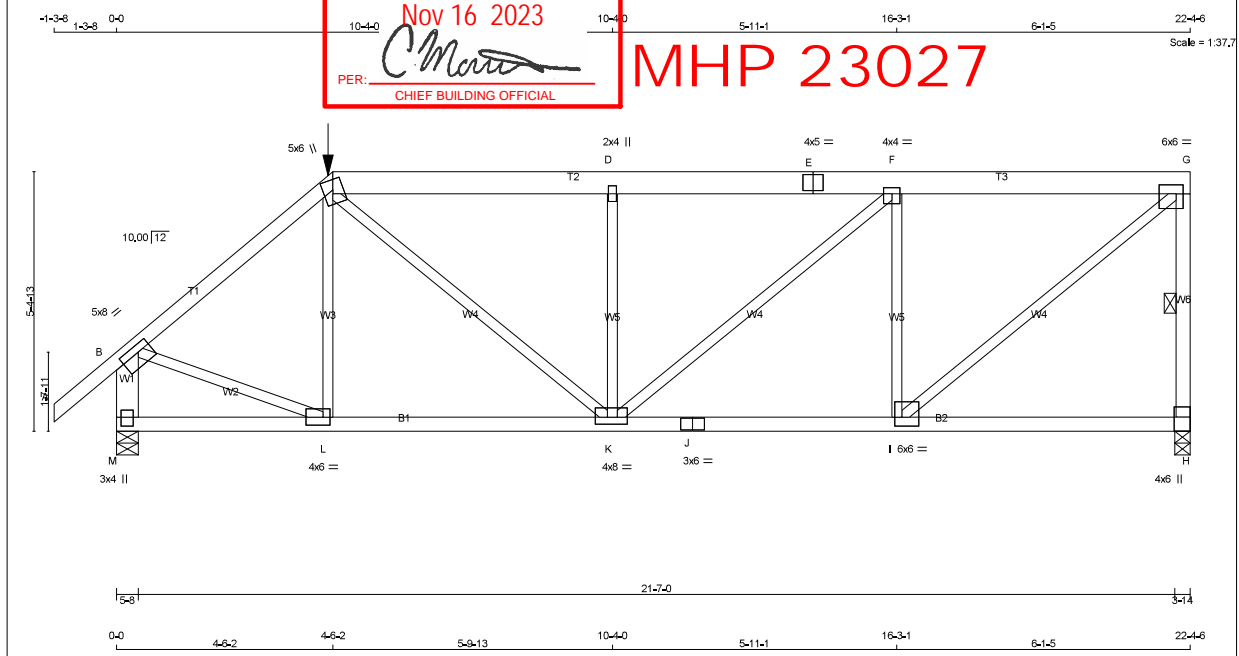
14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G

16. Building designer must review individual component drawings to ensure they are suitable for the structure

15. Not designed for solar panels unless specifically noted

PROVINCIAL ENGINEER  
CHIEF BUILDING OFFICIAL  
M. H. P.  
Nov 16 2023  
TRUE COPY  
OF PERMIT PLANS

MHP 23027



**LUMBER**  
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - G	2x8	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - B	2x6	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X	
B	TMWw4	MT20	5.0	8.0	1.75	3.25
C	TMWw+m	MT20	5.0	6.0	2.50	2.50
D	TMWw	MT20	2.0	4.0		
E	TS-t	MT20	4.0	5.0		
F	TMWw4	MT20	4.0	4.0	1.50	2.00
G	TMWw4	MT20	6.0	6.0	2.25	1.75
H	BMV1+1	MT20	4.0	6.0	Edge	0.50
I	BMWw4	MT20	6.0	6.0	2.25	1.75
J	BS-t	MT20	3.0	6.0		
K	BMWw4	MT20	4.0	8.0	1.75	3.00
L	BMWw4	MT20	4.0	6.0	2.00	1.75
M	BMV1+p	MT20	3.0	4.0	2.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	2986	0	3-14	3-14
H	2986	0	3-14	3-14
M	2997	0	5-8	4-4

**UNFACTORED REACTIONS**

JT	1ST CASE COMBINED	MAX./MIN. SNOW	MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
H	2087	1513 / 0	0 / 0	0 / 0	574 / 0	0 / 0
M	2094	1519 / 0	0 / 0	0 / 0	575 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M  
BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.  
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G4.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC1)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO		
A-B	0 / 53	-119.4 -119.4	0.18 (1)	L-C	-454 / 48	0.20 (1)
B-C	-2894 / 0	-119.4 -119.4	0.72 (1)	B-L	0 / 2323	0.58 (1)
C-D	-3494 / 0	-229.2 -229.2	0.61 (1)	I-G	0 / 3660	0.91 (1)
D-E	-3494 / 0	-229.2 -229.2	0.69 (1)	C-K	0 / 1663	0.41 (1)
E-F	-3494 / 0	-229.2 -229.2	0.69 (1)	I-F	-2088 / 0	0.88 (1)
F-G	-2826 / 0	-229.2 -229.2	0.65 (1)	K-D	-1459 / 0	0.81 (1)
H-G	-2898 / 0	0.0 0.0	0.45 (1)	K-F	0 / 875	0.22 (1)
M-B	-2941 / 0	0.0 0.0	0.22 (1)			
M-L	0 / 0	-35.0 -35.0	0.24 (4)			10.00
L-K	0 / 2237	-35.0 -35.0	0.53 (1)			10.00
K-J	0 / 2826	-35.0 -35.0	0.67 (1)			10.00
J-I	0 / 2826	-35.0 -35.0	0.67 (1)			10.00
I-H	0 / 0	-35.0 -35.0	0.34 (4)			10.00

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-6-2	-404	-404						C1

**CONNECTION REQUIREMENTS**  
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. GIC**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: C-Profile  
LEFT SETBACK = 4-8-2  
RIGHT SETBACK = 0-0  
END SETBACK = 6-0-0  
END WALL WIDTH = 3-14  
CORNER FRAMING TYPE: CONVENTIONAL  
END JACK TYPE: CONVENTIONAL  
APPLIED TO FRONT SIDE  
- ADDTL LOADS BASED ON 55% OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.75")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")  
ALLOWABLE DEFL.(TL) = L/360 (0.75")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI. TC=0.72/1.00 (B-C-1), BC=0.67/1.00 (I-K-1), WB=0.91/1.00 (G-I-1), SSF=0.55/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00  
AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)  
JSI METAL= 0.89 (J) (INPUT = 1.00)

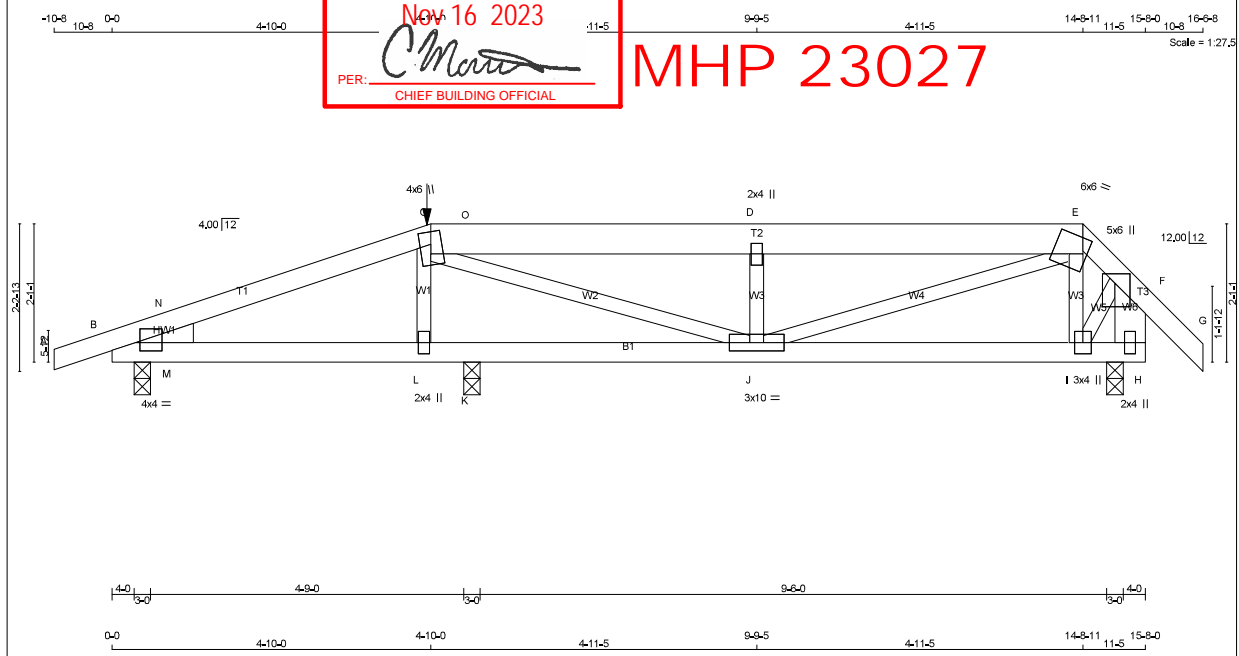
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY  
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.  
Design valid for use only with Mitek connectors. This design is based only on parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com



NOV 16 2023  
 PER: *C. Moran*  
 CHIEF BUILDING OFFICIAL



**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH-I	MT20	4.0	4.0	1.50 0.75
C	TTWW+m	MT20	4.0	6.0	2.25 2.00
D	TMW+w	MT20	2.0	4.0	
E	TTWW-m	MT20	6.0	6.0	3.75 3.00
F	TMW+p	MT20	5.0	6.0	1.75 2.75
H	BMV1+p	MT20	2.0	4.0	
I	BMVW+t	MT20	3.0	4.0	
J	BMVWW-t	MT20	3.0	10.0	
L	BMV+w	MT20	2.0	4.0	

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	GROSS REACTION VERT	HORZ	MAXIMUM FACTORED DOWN	GROSS REACTION UP	INPUT BRG	REQRD BRG	HEEL WEDGE
B	901	0	901	0	3-0	1-8	2x4 L
H	1011	0	1011	0	3-0	1-8	
K	848	0	848	0	3-0	1-8	

**UNFACTORED REACTIONS**

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	630	456 / 0	0 / 0	0 / 0	0 / 0	174 / 0	0 / 0
H	704	523 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
K	592	431 / 0	0 / 0	0 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, K

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (6)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	UNBRAC MAX. CSI (LC)	MEMB. MAX. FORCE (LBS)	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 8	-119.4	-119.4	0.08 (1)	10.00	L-C	-638 / 0 0.10 (1)
B-N	-1255 / 0	-119.4	-119.4	0.08 (4)	5.68	C-J	-240 / 856 0.21 (6)
N-C	-1160 / 0	-119.4	-119.4	0.38 (1)	5.36	J-D	-702 / 0 0.11 (1)
C-O	-1711 / 0	-119.4	-119.4	0.21 (1)	5.79	J-E	0 / 1298 0.32 (1)
O-D	-1711 / 0	-119.4	-119.4	0.21 (1)	5.79	I-E	-177 / 7 0.03 (1)
D-E	-1711 / 0	-119.4	-119.4	0.21 (1)	5.79	I-F	0 / 623 0.15 (1)
E-F	-740 / 0	-119.4	-119.4	0.09 (1)	6.25	M-N	-105 / 158 0.00 (1)
F-G	0 / 41	-119.4	-119.4	0.09 (1)	10.00		
H-F	-1100 / 0	0.0	0.0	0.08 (1)	7.81		
B-M	0 / 1104	-30.0	-30.0	0.37 (5)	10.00		
M-L	0 / 1104	-30.0	-30.0	0.48 (1)	10.00		
L-K	0 / 1142	-18.2	-18.2	0.74 (1)	10.00		
K-J	0 / 1142	-18.2	-18.2	0.74 (1)	10.00		
J-I	0 / 486	-18.2	-18.2	0.26 (1)	10.00		
I-H	0 / 0	-18.2	-18.2	0.16 (1)	10.00		

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-10-0	-321	-321	--	FRONT	VERT	TOTAL	--	C1

**CONNECTION REQUIREMENTS**  
 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 34.8 PSF  
 DL = 8.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. GIC**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip  
 LEFT SETBACK = 4-10-0  
 RIGHT SETBACK = 11-5  
 END SETBACK = 4-10-0  
 END WALL WIDTH = 3-0  
 CORNER FRAMING TYPE: CONVENTIONAL  
 END JACK TYPE: CONVENTIONAL  
 APPLIED TO FRONT SIDE  
 - ADDTL LOADS BASED ON 5% OF G.S.L.  
 LOADS APPLIED TO FIRST 5'-0" OF SPAN  
 MEASURED FROM THE LEFT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF CBC2018 - NBC-2019AE  
 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

**DESIGN ASSUMPTIONS**  
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(LL) = L/970 (0.07")  
 ALLOWABLE DEFL.(TL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(TL) = L/543 (0.12")

CSI: TC=0.38/1.00 (C-N-1), BC=0.74/1.00 (K-L-1), WB=0.32/1.00 (E-J-1), SSI=0.60/1.00 (K-L-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00  
 AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PLI)
MT20	650	371	1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches  
 PLATE ROTATION TOL. = 5.0 Deg.

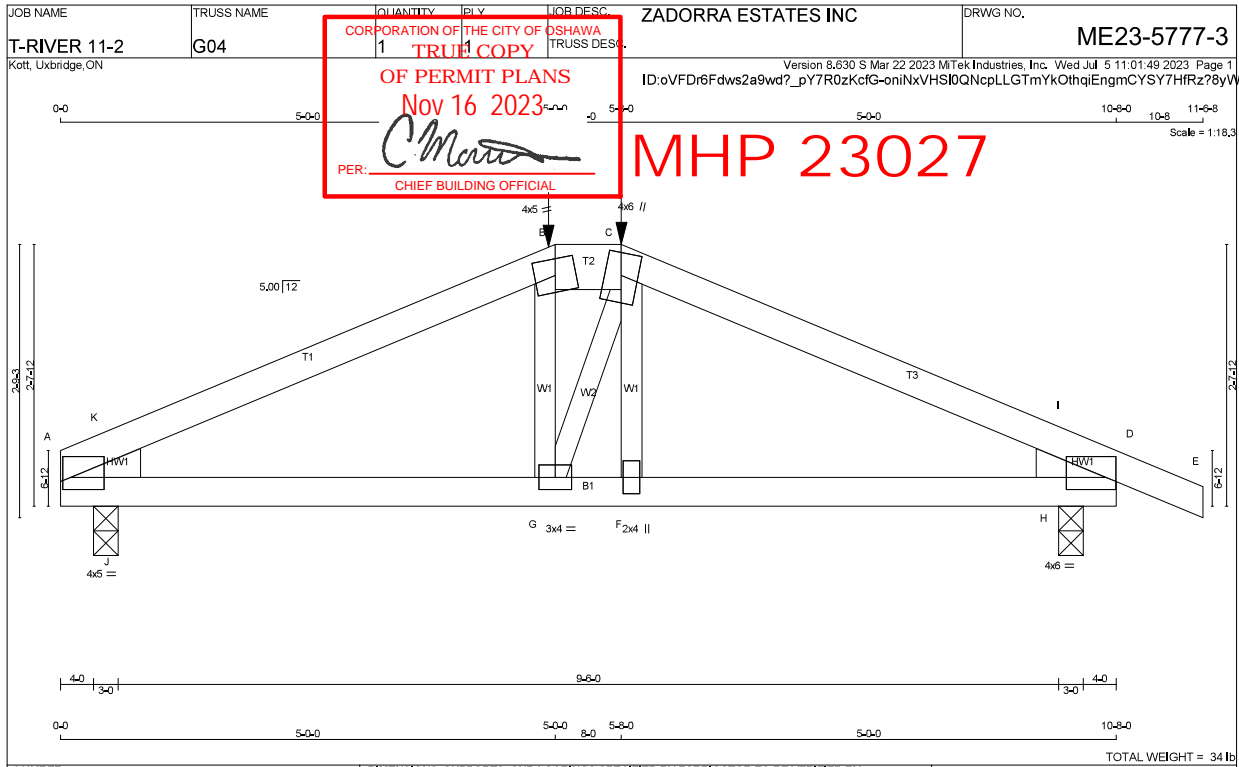
JSI GRIP= 0.90 (E) (INPUT = 0.90)  
 JSI METAL= 0.45 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY  
 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only on parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Overall permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult  
 TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





**LUMBER**  
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x6	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF  
DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH-I	MT20	4.0	5.0	1.50	1.00
B	TTW-m	MT20	4.0	5.0		
C	TTWW+m	MT20	4.0	6.0	2.75	2.00
D	TMBH-I	MT20	4.0	6.0	1.50	1.25
F	BMW+H	MT20	2.0	4.0		
G	BMW+H	MT20	3.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	HEEL	BRG	BRG	WEDGE
D	1004	0	1004	0	0	3-0	1-8	2x4 R			
A	893	0	893	0	0	3-0	1-8	2x4 L			

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	699	520 / 0	0 / 0	0 / 0	0 / 0	180 / 0	0 / 0
A	623	455 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, A

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	C H O R D S				W E B S			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. (LC)	
A-K	-1391 / 0	-119.4	-119.4	0.28 (1)	5.13	G-B	0 / 85	0.03 (4)
K-B	-1314 / 0	-119.4	-119.4	0.36 (1)	5.16	G-C	0 / 4	0.00 (4)
B-C	-1198 / 0	-138.1	-138.1	0.01 (1)	6.25	F-C	0 / 81	0.02 (4)
C-I	-1312 / 0	-119.4	-119.4	0.36 (1)	5.16	H-I	-96 / 65	0.00 (1)
I-D	-1390 / 0	-119.4	-119.4	0.28 (1)	5.14	J-K	-97 / 65	0.00 (1)
D-E	0 / 8	-119.4	-119.4	0.08 (1)	10.00			
A-J	0 / 1202	-21.1	-21.1	0.32 (1)	10.00			
J-G	0 / 1202	-21.1	-21.1	0.33 (1)	10.00			
G-F	0 / 1197	-21.1	-21.1	0.30 (1)	10.00			
F-H	0 / 1201	-21.1	-21.1	0.33 (1)	10.00			
H-D	0 / 1201	-21.1	-21.1	0.32 (1)	10.00			

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-0-0	-137	-137	--	FRONT	VERT	TOTAL	--	C1
C	5-8-0	-137	-137	--	FRONT	VERT	TOTAL	--	C1

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip  
SIDE SETBACK = 5'-0"  
END SETBACK = 2'-10"  
END WALL WIDTH = 3'-0"  
CORNER FRAMING TYPE: CONVENTIONAL  
END JACK TYPE: CONVENTIONAL  
APPLIED TO FRONT SIDE  
-ADDTL LOADS BASED ON 55% OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 48.1 P.S.F., G.S.L. PLUS 6.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F., SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF., (LL) = L/360 (0.36")  
CALCULATED VERT. DEF., (LL) = L/999 (0.03")  
ALLOWABLE DEF., (TL) = L/360 (0.36")  
CALCULATED VERT. DEF., (TL) = L/999 (0.05")

CSI: TC=0.36/1.00 (C-I-1), BC=0.33/1.00 (F-H-1),  
WB=0.03/1.00 (B-G-4), SSI=0.24/1.00 (B-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (A) (INPUT = 0.90)  
JSI METAL = 0.33 (A) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

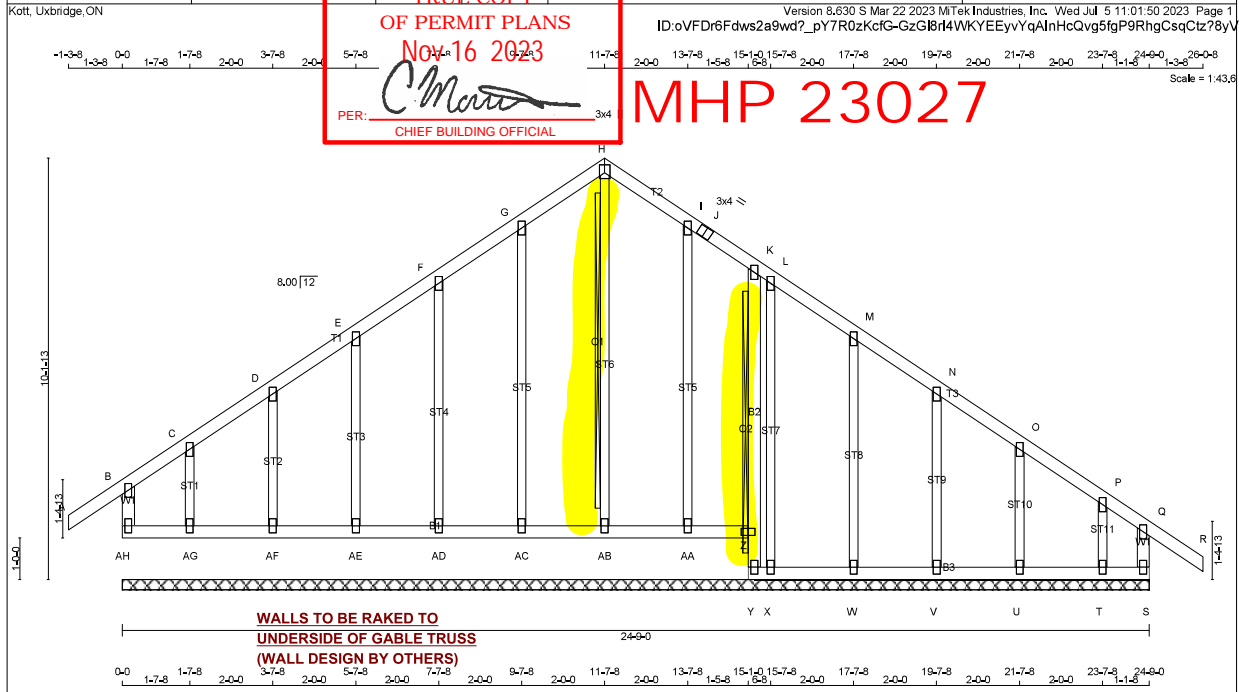
REVIEW FOR TRUSS COMPONENT ONLY  
NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.  
Design valid for use only with Mitek connectors. This design is based on input parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Overall permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult  
TPIC Appendix G - Minimum quality Manufacturing Criteria available from [www.tpica.ca](http://www.tpica.ca) and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or [www.sbciindustry.com](http://www.sbciindustry.com)



CORPORATION OF THE CITY OF OSHAWA  
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**OF PERMIT PLANS**  
**Nov 16 2023**  
 PER: *[Signature]*  
**CHIEF BUILDING OFFICIAL**

**MHP 23027**



**WALLS TO BE RAKED TO UNDERSIDE OF GABLE TRUSS (WALL DESIGN BY OTHERS)**

**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR. SPF

AH	B	2x4	DRY	No.2	SPF
A	H	2x4	DRY	No.2	SPF
H	J	2x4	DRY	No.2	SPF
J	R	2x4	DRY	No.2	SPF
S	Q	2x4	DRY	No.2	SPF
AH	Z	2x4	DRY	No.2	SPF
Y	K	2x4	DRY	No.2	SPF
Y	S	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF  
 ALL GABLE WEBS 2x3 DRY No.2 SPF  
 DRYS: SEASONED LUMBER.  
 GABLE STUDS SPACED AT 2-0-0 OC.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**  
 THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.  
 THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JTS: AH, AB, AC, AD, AE, AF, AG, AA  
 BEARING MATERIAL TO BE SPF No.2 OR BETTER AT JOINT(S)

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.  
 2x4 DRY SPF No.2 T-BRACE AT K-Z, H-AB

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 34.8 PSF  
 DL = 6.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF CBC 2018, NBC-2019AE  
 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

**DESIGN ASSUMPTIONS**  
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.  
 (55% OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI TC=0.16/1.00 (Q-R-1), BC=0.03/1.00 (S-T-1),  
 WB=0.28/1.00 (G-AC-1), SSI=0.11/1.00 (Q-R-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
 MAX MIN MAX MIN MAX MIN  
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches  
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (H) (INPUT = 0.90)  
 JSI METAL= 0.14 (Q) (INPUT = 1.00)

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B, K, Q					
C	TMV+p	MT20	2.0	4.0	
C, D, E, F, G, I, L, M, N, O, P					
C	TMV+w	MT20	2.0	4.0	
H	TMV+p	MT20	3.0	4.0	2.25 1.50
J	TS+	MT20	3.0	4.0	
S, Y, AH					
S	BMV1+p	MT20	2.0	4.0	
T, U, V, W, X, AA, AB, AC, AD, AE, AF, AG					
T	BMV1+w	MT20	2.0	4.0	
Z	BVM+	MT20	2.0	4.0	

**FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.**

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**  
 TOTAL LOAD CASES: (4)

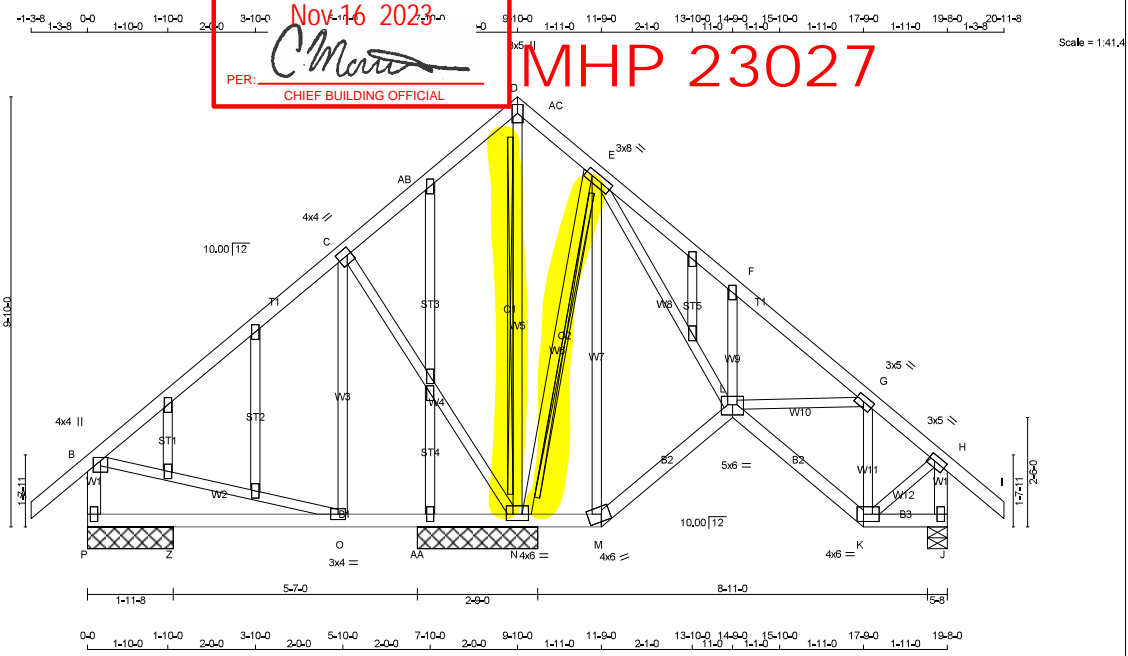
MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED (PLF) VERT. LOAD	MAX. UNBRAC. (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
AH-B	-294 / 0	0.0 0.0 0.04 (1)	7.81	AB-H -288 / 0 0.17 (1)
A-B	0 / 45	-119.4 -119.4 0.16 (1)	10.00	AC-G -245 / 0 0.28 (1)
B-C	-15 / 0	-119.4 -119.4 0.11 (1)	6.25	AD-F -235 / 0 0.16 (1)
C-D	0 / 22	-119.4 -119.4 0.06 (1)	10.00	AE-E -233 / 0 0.09 (1)
D-E	0 / 21	-119.4 -119.4 0.06 (1)	10.00	AF-D -250 / 0 0.06 (1)
E-F	0 / 27	-119.4 -119.4 0.06 (1)	10.00	AG-C -153 / 0 0.02 (1)
F-G	0 / 32	-119.4 -119.4 0.06 (1)	10.00	AA-I -218 / 0 0.25 (1)
G-H	0 / 33	-119.4 -119.4 0.06 (1)	10.00	X-L -128 / 0 0.13 (1)
H-I	0 / 35	-119.4 -119.4 0.06 (1)	10.00	W-M -244 / 0 0.15 (1)
I-J	0 / 32	-119.4 -119.4 0.05 (1)	10.00	V-N -232 / 0 0.08 (1)
J-K	0 / 32	-119.4 -119.4 0.05 (1)	10.00	U-O -251 / 0 0.05 (1)
K-L	0 / 33	-119.4 -119.4 0.05 (1)	10.00	T-P -99 / 0 0.01 (1)
L-M	0 / 24	-119.4 -119.4 0.06 (1)	10.00	
M-N	0 / 24	-119.4 -119.4 0.06 (1)	10.00	
N-O	0 / 17	-119.4 -119.4 0.06 (1)	10.00	
O-P	0 / 18	-119.4 -119.4 0.06 (1)	10.00	
P-Q	-32 / 0	-119.4 -119.4 0.12 (1)	6.25	
Q-R	0 / 45	-119.4 -119.4 0.16 (1)	10.00	
S-Q	-290 / 0	0.0 0.0 0.04 (1)	7.81	



**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only on parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Overall permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult  
 TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com



CORPORATION OF THE CITY OF OSHAWA  
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**OF PERMIT PLANS**  
**Nov 16 2023**  
 PER: *C. Moran*  
**CHIEF BUILDING OFFICIAL**



**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR. SPF

A - D	2x4	DRY	2100F 1.8E	SPF
D - I	2x4	DRY	2100F 1.8E	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT	2'-0" OC.			

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+4	MT20	4.0	4.0	2.00	1.25
D	TMV+p	MT20	3.0	5.0		
E	TMVW+4	MT20	3.0	8.0	1.50	2.75
F	TMV+w	MT20	2.0	4.0		
G	TMVW+4	MT20	3.0	5.0	1.50	2.25
H	TMVW+4	MT20	3.0	5.0	1.50	1.75
J	BMV1+p	MT20	2.0	4.0		
K	BBW+p	MT20	4.0	6.0	2.00	4.25
L	BBW+p	MT20	5.0	6.0	2.75	3.00
M	BBW+m	MT20	4.0	6.0	2.00	2.25
N	BBW+1+4	MT20	4.0	6.0	1.75	3.00
O	BMVW+4	MT20	3.0	4.0		
P	BMV1+p	MT20	2.0	4.0		
Q	R, S, T, U, W, X, Y					
R	NP+w	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	DOWN	IN	IN
P	928	0	1-11-8
J	676	0	5-8
N	2519	0	2-9-0
Z	119	0	1-11-8
AA	93	0	2-9-0

**UNFACTORED REACTIONS**

1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
P	641	603 / 0	0 / 0	0 / 0	137 / 0	0 / 0
J	471	375 / -8	0 / 0	0 / 0	119 / 0	0 / 0
N	1768	1239 / 0	0 / 0	0 / 0	529 / 0	0 / 0
Z	93	15 / 0	0 / 0	0 / 0	80 / 0	0 / 0
AA	70	33 / -8	0 / 0	0 / 0	51 / 0	0 / 0

**DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*  
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.  
 LOADS WERE DERIVED FROM USER INPUT  
 NO FURTHER MODIFICATIONS WERE MADE

**SPECIFIED LOADS:**  
 TOP CH. LL = 34.8 PSF  
 DL = 6.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. C/C**

\*\*\* NON STANDARD GIRDER \*\*\*  
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J, N, Z, AA

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.  
 2x4 DRY SPF No.2 T-BRACE AT D-N, E-N

**FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.**

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**  
 TOTAL LOAD CASES: (10)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD			WEBS MAX. FACTORED FORCE			
		LC1	MAX	MAX.	MEMB.	FORCE	MAX	
FR-TO		FROM	TO	UNBRAC	FR-TO			
A-B	0 / 83	-188.3	-188.3	0.19 (1)	10.00	O-C	-46 / 82	0.03 (7)
B-C	-396 / 47	-188.3	-188.3	0.63 (7)	6.25	C-N	-917 / 0	0.99 (1)
C-AB	0 / 306	-188.3	-188.3	0.64 (1)	6.25	N-D	-775 / 0	0.57 (1)
AB-D	0 / 306	-188.3	-188.3	0.64 (1)	6.25	N-E	-890 / 0	0.49 (1)
D-AC	0 / 337	-188.3	-188.3	0.18 (1)	10.00	M-E	0 / 144	0.04 (9)
AC-E	0 / 337	-188.3	-188.3	0.18 (1)	10.00	E-L	0 / 801	0.20 (10)
E-F	-446 / 0	-188.3	-188.3	0.16 (10)	6.25	L-F	-485 / 0	0.10 (9)
F-G	-459 / 0	-119.4	-119.4	0.12 (9)	6.25	L-G	0 / 125	0.03 (10)
G-H	-355 / 0	-119.4	-119.4	0.11 (10)	6.25	K-G	-327 / 0	0.06 (10)
H-I	0 / 53	-119.4	-119.4	0.12 (1)	10.00	B-O	-13 / 365	0.09 (7)
P-B	-1080 / 0	0.0	0.0	0.13 (7)	7.57	K-H	0 / 304	0.08 (10)
J-H	-895 / 0	0.0	0.0	0.08 (10)	7.81			
P-Z	0 / 0	-22.0	-25.4	0.09 (4)	10.00			
Z-O	0 / 0	-42.0	-32.0	0.09 (4)	10.00			
O-AA	-12 / 356	-32.0	-34.9	0.13 (7)	6.25			
AA-N	-12 / 356	-34.9	-38.9	0.13 (7)	6.25			
N-M	-113 / 5	-38.9	-32.3	0.08 (8)	6.25			
M-L	-146 / 6	-32.3	-22.0	0.08 (4)	6.25			
L-K	0 / 314	-18.3	-18.3	0.09 (10)	10.00			
K-J	0 / 0	-18.3	-18.3	0.02 (4)	10.00			

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF CBC 2018, NBC-2019AE  
 - PART 9 OF CBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

**DESIGN ASSUMPTIONS**  
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.  
 (55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")  
 ALLOWABLE DEFL.(TL) = L/360 (0.33")  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.64/1.00 (C-D-1), BC=0.13/1.00 (C-AA-7), WB=0.99/1.00 (C-N-1), SSI=0.43/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (FLU) (PLU)  
 MAX MIN MAX MIN MAX MIN  
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches  
 PLATE ROTATION TOL. = 5.0 Deg.

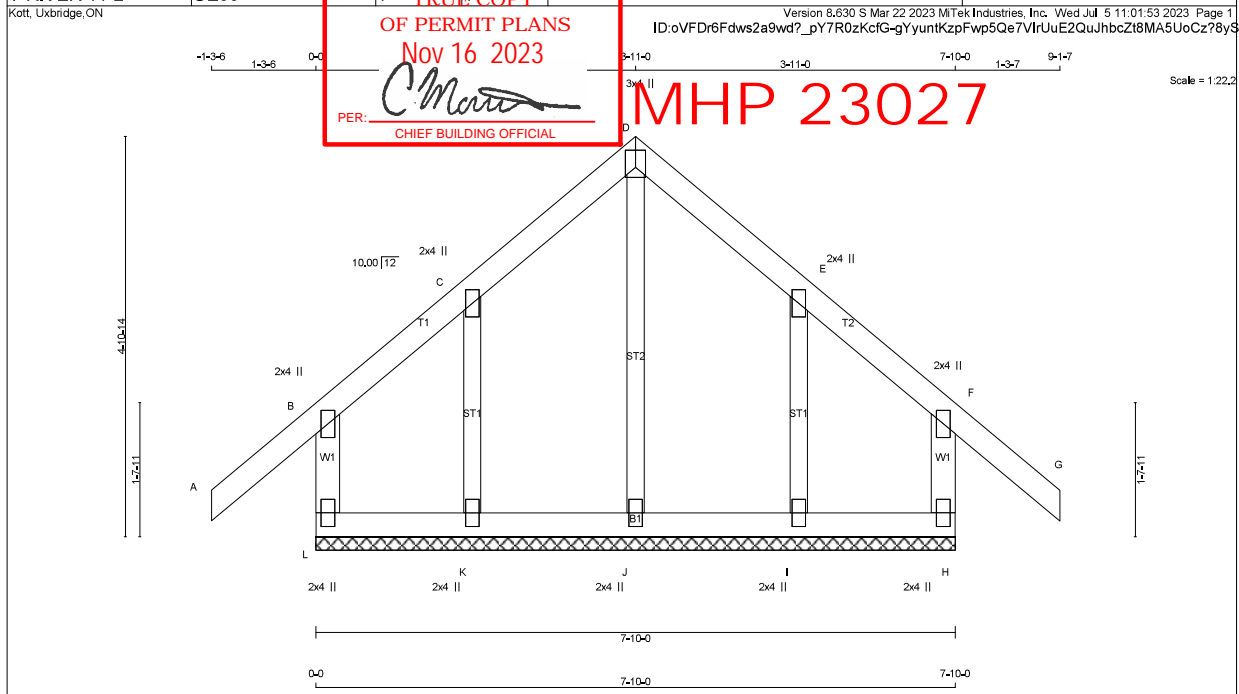
JSI GRIP= 0.89 (H) (INPUT = 0.90)  
 JSI METAL= 0.29 (L) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

CORPORATION OF THE CITY OF OSHAWA  
**TRUSS DES.**  
**OF PERMIT PLANS**  
**Nov 16 2023**  
 PER: *C. Moran*  
**CHIEF BUILDING OFFICIAL**



TOTAL WEIGHT = 35 lb

**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR.

L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
GABLE STUDS SPACED AT 2'-0" OC.				

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**  
 THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.  
 THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.  
 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 34.8 PSF  
 DL = 6.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

**SPACING = 240 IN. GIC**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	4.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	2.0	4.0		
H	BMV1+p	MT20	2.0	4.0		
I, J, K						
I	BMV1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

**LOADING**  
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED	FACTORED	MAX. FACTORED	FACTORED
	MEMB. FORCE (LBS)	VERT. LOAD (PLF)	MEMB. FORCE (LBS)	MEMB. FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO
L-B	-272 / 0	0.0 0.0 0.06 (1)	7.81	J-D -322 / 0 0.11 (1)
A-B	0 / 53	-119.4 -119.4 0.16 (1)	10.00	K-C -199 / 0 0.04 (1)
B-C	0 / 22	-119.4 -119.4 0.10 (1)	10.00	I-E -198 / 0 0.04 (1)
C-D	0 / 50	-119.4 -119.4 0.07 (1)	10.00	
D-E	0 / 50	-119.4 -119.4 0.07 (1)	10.00	
E-F	0 / 22	-119.4 -119.4 0.10 (1)	10.00	
F-G	0 / 53	-119.4 -119.4 0.16 (1)	10.00	
H-F	-273 / 0	0.0 0.0 0.06 (1)	7.81	
L-K	-28 / 0	-18.2 -18.2 0.02 (4)	6.25	
K-J	-35 / 0	-18.2 -18.2 0.02 (4)	6.25	
J-I	-35 / 0	-18.2 -18.2 0.02 (4)	6.25	
I-H	-28 / 0	-18.2 -18.2 0.02 (4)	6.25	

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF BCBC 2018, NBC-2019AE  
 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

**DESIGN ASSUMPTIONS**  
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.  
 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD), EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (F-G-1), BC=0.02/1.00 (L-K-4),  
 WB=0.11/1.00 (D-J-1), SSI=0.10/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

MODULUS ENGINEERING LTD.



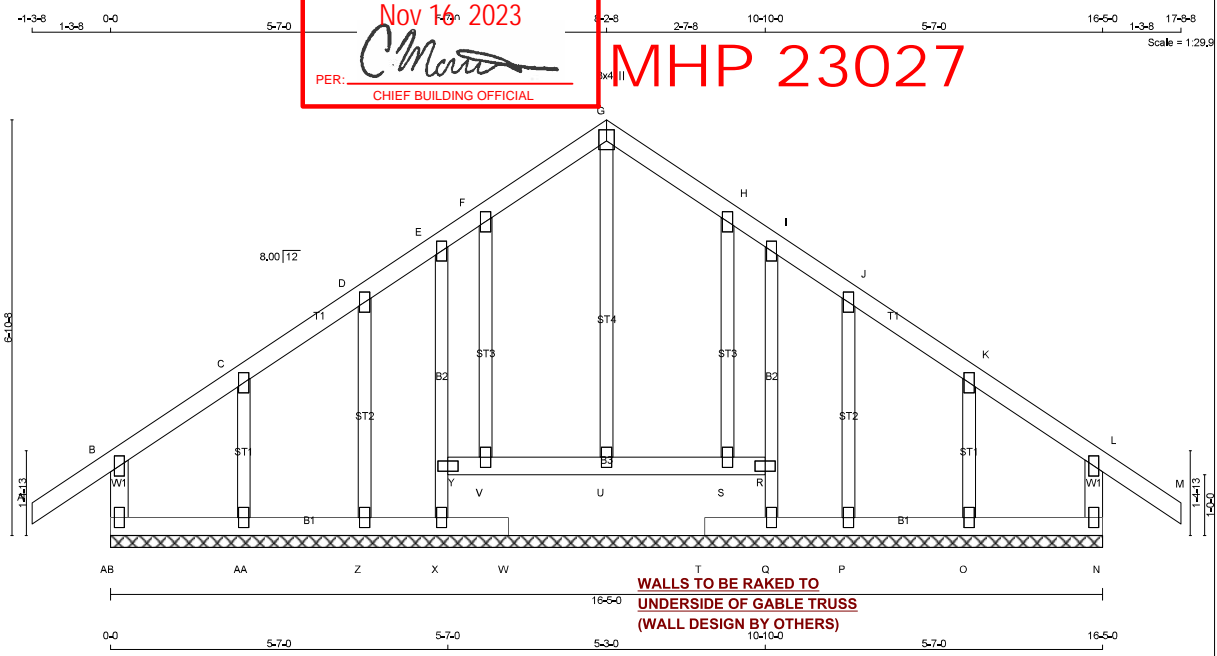
REVIEW FOR TRUSS COMPONENT ONLY  
 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult  
 TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com





CORPORATION OF THE CITY OF OSHAWA  
 TRUSS DESG.  
**TRUSS COPY**  
**OF PERMIT PLANS**  
**Nov 16 2023**  
 PER: *C. Moran*  
**CHIEF BUILDING OFFICIAL**



**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR. SPF

AB - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
AB - W	2x4	DRY	No.2	SPF
X - E	2x3	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
Q - I	2x3	DRY	No.2	SPF
T - N	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF  
 ALL GABLE WEBS 2x3 DRY No.2 SPF  
 DRY: SEASONED LUMBER.  
 GABLE STUDS SPACED AT 2'-0" OC.

**PLATES (table is in inches)**

JF TYPE	PLATES	W	LEN	Y	X
B, E, I, L					
B	TMV+p	MT20	2.0	4.0	
C, D, F, H, J, K					
C	TMV+w	MT20	2.0	4.0	
G	TTW+p	MT20	3.0	4.0	2.25 1.50
N, Q, X, AB					
N	BMV1+p	MT20	2.0	4.0	
O, P, U, Z, AA					
O	BMV1+w	MT20	2.0	4.0	
R	BVMH	MT20	2.0	4.0	
S	BMV+w	MT20	2.0	4.0	
V	BMV+w	MT20	2.0	4.0	
Y	BVMH	MT20	2.0	4.0	

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**  
**BEARINGS**  
 THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.  
 THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.  
 BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JTS: U  
 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS					
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)				
FR-TO		FROM TO	LENGTH	FR-TO				
AB-B	-298 / 0	0,0	0,0	0,01 (1)	7,81	U-G	-310 / 0	0,15 (1)
A-B	0 / 45	-119,4	-119,4	0,16 (1)	10,00	V-F	-100 / 0	0,03 (1)
B-C	-11 / 0	-119,4	-119,4	0,06 (1)	6,25	Z-D	-187 / 0	0,05 (1)
C-D	-5 / 0	-119,4	-119,4	0,06 (1)	10,00	A-C	-248 / 0	0,04 (1)
D-E	0 / 3	-119,4	-119,4	0,04 (1)	10,00	S-H	-100 / 0	0,03 (1)
E-F	-19 / 0	-119,4	-119,4	0,05 (1)	6,25	P-J	-187 / 0	0,05 (1)
F-G	0 / 20	-119,4	-119,4	0,09 (1)	10,00	O-K	-248 / 0	0,04 (1)
G-H	0 / 20	-119,4	-119,4	0,09 (1)	10,00			
H-I	-19 / 0	-119,4	-119,4	0,05 (1)	6,25			
I-J	0 / 3	-119,4	-119,4	0,04 (1)	10,00			
J-K	-5 / 0	-119,4	-119,4	0,06 (1)	10,00			
K-L	-11 / 0	-119,4	-119,4	0,06 (1)	6,25			
L-M	0 / 45	-119,4	-119,4	0,16 (1)	10,00			
N-L	-298 / 0	0,0	0,0	0,01 (1)	7,81			
AB-AA	0 / 8	-18,2	-18,2	0,02 (4)	10,00			
AA-Z	0 / 0	-18,2	-18,2	0,02 (4)	10,00			
Z-X	-4 / 0	-18,2	-18,2	0,01 (4)	10,00			
X-W	0 / 0	-18,2	-18,2	0,00 (4)	10,00			
X-Y	-247 / 0	0,0	0,0	0,02 (1)	7,81			
Y-E	-156 / 0	0,0	0,0	0,03 (1)	7,81			
Y-V	-2 / 0	-18,2	-18,2	0,06 (1)	10,00			
V-U	-4 / 0	-18,2	-18,2	0,06 (1)	10,00			
U-S	-4 / 0	-18,2	-18,2	0,06 (1)	10,00			
S-R	-2 / 0	-18,2	-18,2	0,06 (1)	10,00			
Q-R	-247 / 0	0,0	0,0	0,02 (1)	7,81			
R-I	-156 / 0	0,0	0,0	0,03 (1)	7,81			
T-Q	0 / 0	-18,2	-18,2	0,00 (4)	10,00			
Q-P	-4 / 0	-18,2	-18,2	0,01 (4)	10,00			
P-O	0 / 0	-18,2	-18,2	0,02 (4)	10,00			
O-N	0 / 8	-18,2	-18,2	0,02 (4)	10,00			

**DESIGN CRITERIA**  
 SPECIFIED LOADS:  
 TOP CH. LL = 34,8 PSF  
 DL = 6,0 PSF  
 BOT CH. LL = 0,0 PSF  
 DL = 7,3 PSF  
 TOTAL LOAD = 48,1 PSF

**SPACING = 24.0 IN. GIC**  
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF CBC 2018, NBC-2019AE  
 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

**DESIGN ASSUMPTIONS**  
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.  
 (55% OF 48,1 P.S.F., G.S.L., PLUS 8,4 P.S.F. RAIN LOAD) EQUALS 34,8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI TC=0,16/1,00 (A-B-1), BC=0,08/1,00 (L-V-1),  
 WB=0,16/1,00 (G-U-1), SSI=0,11/1,00 (A-B-1)

DOL LUMBER=1,00 NAIL=1,00 LS BEND=1,10  
 COMP=1,10 SHEAR=1,10 TENS=1,10  
 COMPANION LIVE LOAD FACTOR = 1,00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
 MAX MIN MAX MIN MAX MIN  
 MT20 650 371 1747 788 1987 1873  
 PLATE PLACEMENT TOL. = 0,250 inches  
 PLATE ROTATION TOL. = 5,0 Deg.  
 JSI GRIP= 0,52 (G) (INPUT = 0,90)  
 JSI METAL= 0,16 (L) (INPUT = 1,00)

MODULUS ENGINEERING LTD.

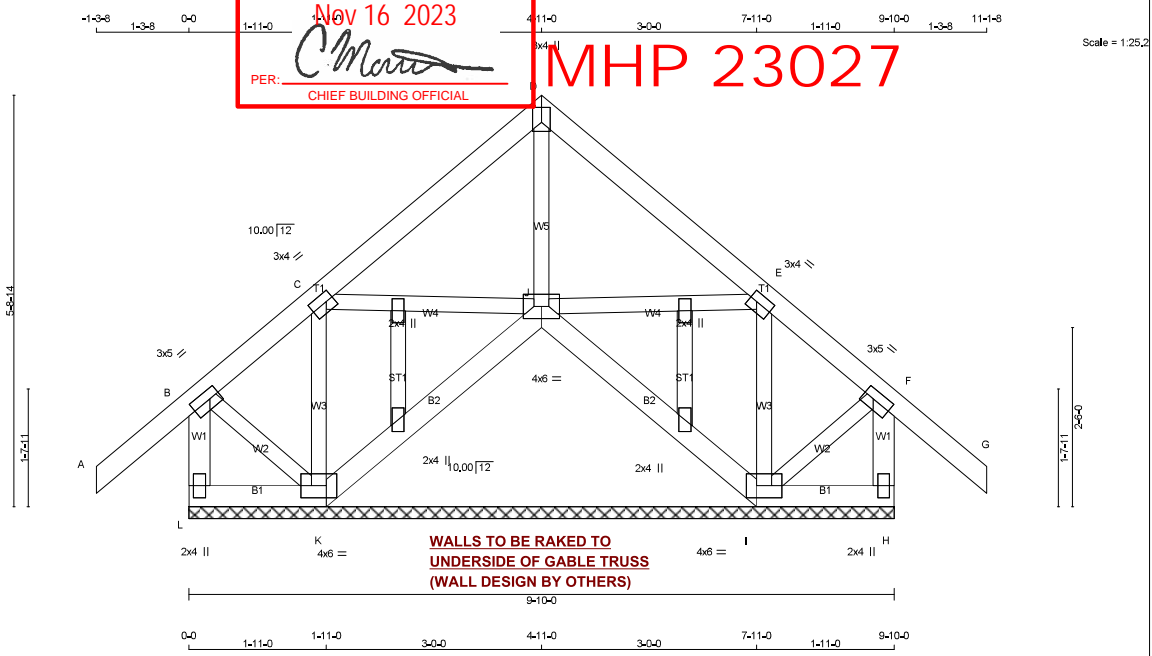
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TRUSS COPY  
 OF PERMIT PLANS  
 Nov 16 2023  
 PER: *C. Moran*  
 CHIEF BUILDING OFFICIAL

MHP 23027



**LUMBER**  
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
L - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - K	2x4	DRY No.2	SPF
K - J	2x4	DRY No.2	SPF
J - I	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2'-0" OC.		

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	3.0	5.0	1.50	1.75
C	TMVW4	MT20	3.0	4.0	1.50	1.25
D	TMV+p	MT20	3.0	4.0	2.50	1.50
E	TMVW4	MT20	3.0	4.0	1.50	1.25
F	TMVW4	MT20	3.0	5.0	1.50	1.75
H	BMV1+p	MT20	2.0	4.0		
I	BBVW1-p	MT20	4.0	6.0	2.00	4.25
J	BBVW1-p	MT20	4.0	6.0		
K	BBVW1-p	MT20	4.0	6.0	2.00	4.25
L	BMV1+p	MT20	2.0	4.0		
M	N, O, P					
M	NP+w	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX
JT				
L	342	0	342	0
K	302	0	302	0
J	395	0	395	0
I	302	0	302	0
H	342	0	342	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): J

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	236	189/0	0/0	0/0	0/0	47/0	0/0
K	212	150/0	0/0	0/0	0/0	62/0	0/0
J	277	199/0	0/0	0/0	0/0	78/0	0/0
I	212	150/0	0/0	0/0	0/0	62/0	0/0
H	236	189/0	0/0	0/0	0/0	47/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, K, J, I, H

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		VERT. LOAD (PLF)	CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO					
A-B	0/53	-119.4	-119.4	0.16 (1)	10.00
B-C	-41/0	-119.4	-119.4	0.16 (1)	6.25
C-D	-17/0	-119.4	-119.4	0.14 (1)	6.25
D-E	-17/0	-119.4	-119.4	0.14 (1)	6.25
E-F	-41/0	-119.4	-119.4	0.16 (1)	6.25
F-G	0/53	-119.4	-119.4	0.16 (1)	10.00
L-B	-325/0	0.0	0.0	0.03 (1)	7.81
H-F	-325/0	0.0	0.0	0.03 (1)	7.81
L-K	0/0	-18.3	-18.3	0.02 (4)	10.00
K-J	0/17	-18.2	-18.2	0.05 (4)	10.00
J-I	0/17	-18.3	-18.3	0.05 (4)	10.00
I-H	0/0	-18.3	-18.3	0.02 (4)	10.00

**DESIGN CRITERIA**

SPECIFIED LOADS:  
TOP CH. LL = 34.8 PSF  
DL = 6.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G.C.**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

DESIGN ASSUMPTIONS  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSJ TC=0.16/1.00 (F-G.1), BC=0.05/1.00 (I-J.4), WB=0.06/1.00 (D-J.1), SSI=0.13/1.00 (C-D.1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (D) (INPUT = 0.90)  
JSI METAL= 0.13 (K) (INPUT = 1.00)

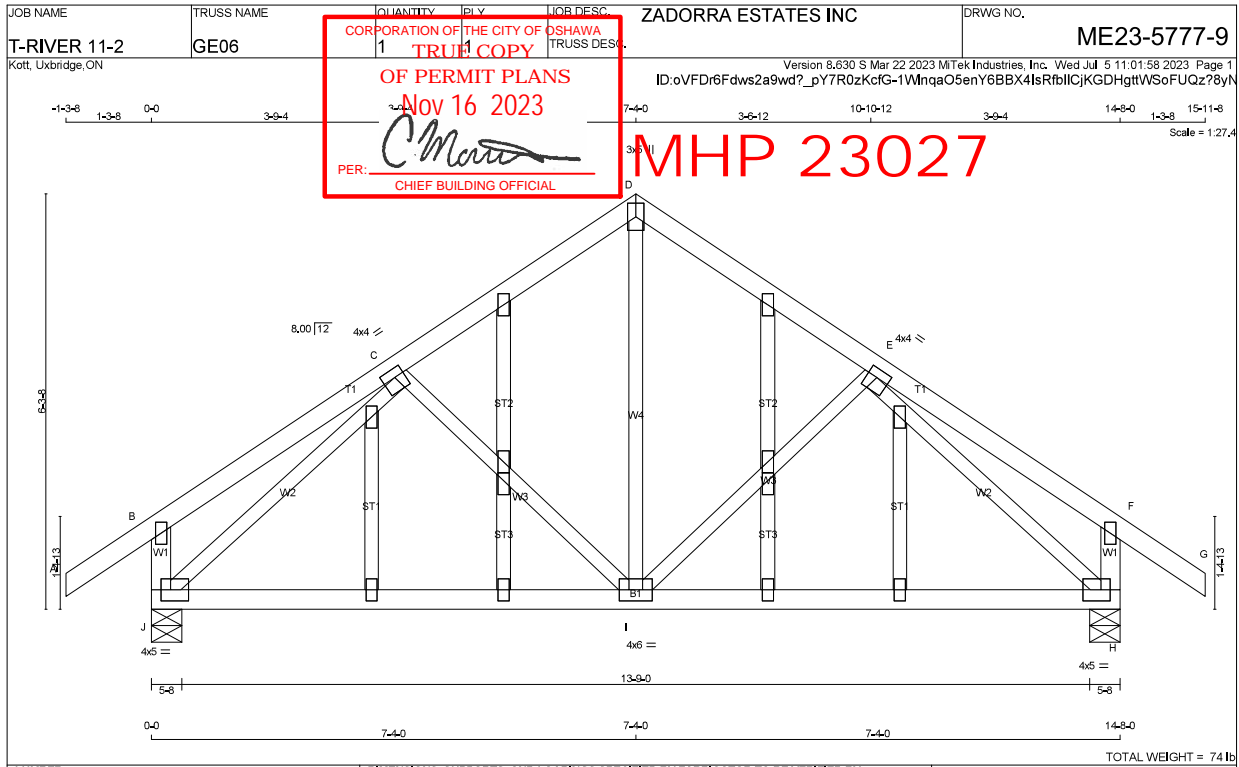
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

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**LUMBER**  
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
J - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0	
C	TMWW+4	MT20	4.0	4.0	1.50 1.75
D	TTW+p	MT20	3.0	5.0	
E	TMWW+4	MT20	4.0	4.0	1.50 1.75
F	TMV+p	MT20	2.0	4.0	
H	BMWW+4	MT20	4.0	5.0	2.00 1.75
I	BMWW+4	MT20	4.0	6.0	
J	BMWW+4	MT20	4.0	5.0	2.00 1.75
K	NP+w	MT20	2.0	4.0	1.50 1.00
K, L, M, N, O, P, Q, R, S, T					
K	NP+w	MT20	2.0	4.0	
R	NP+w	MT20	2.0	4.0	1.50 1.00

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG UPLIFT	REQRD BRG IN-SX
JT	1837	0	0	5-8
J	1837	0	0	5-8
H	1837	0	0	5-8

**UNFACTORED REACTIONS**

	1ST CASE	SNOW	MAX./MIN. LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1288	909 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
H	1288	909 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	C H O R D S			W E B S			
	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED UNBRAC. LENGTH FR-TO	MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO				
A-B	0 / 72	-188.3	-188.3	0.28 (1)	10.00	I-D	0 / 805
B-C	0 / 44	-188.3	-188.3	0.44 (1)	10.00	I-E	-387 / 0
C-D	-1252 / 0	-188.3	-188.3	0.48 (1)	5.04	C-I	-387 / 0
D-E	-1252 / 0	-188.3	-188.3	0.48 (1)	5.04	J-C	-1793 / 0
E-F	0 / 44	-188.3	-188.3	0.44 (1)	10.00	E-H	-1793 / 0
F-G	0 / 72	-188.3	-188.3	0.28 (1)	10.00		
J-B	-524 / 0	0.0	0.0	0.06 (1)	7.81		
H-F	-524 / 0	0.0	0.0	0.06 (1)	7.81		
J-I	0 / 1292	-22.0	-32.0	0.56 (4)	10.00		
I-H	0 / 1292	-32.0	-22.0	0.56 (4)	10.00		

**DESIGN CRITERIA**

\*\*\* SPECIAL LOADS ANALYSIS \*\*\*  
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.  
LOADS WERE DERIVED FROM USER INPUT  
NO FURTHER MODIFICATIONS WERE MADE

**SPECIFIED LOADS:**

TOP CH. LL	= 34.8 PSF
BY USER	
BOT CH. LL	= 0.0 PSF
DL	= 7.3 PSF
TOTAL LOAD	= 48.1 PSF

**SPACING = 240 IN. G/C**

\*\*\* NON STANDARD GIRDER \*\*\*  
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, NBC-2019AE  
- PART 9 OF CBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")  
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")  
ALLOWABLE DEFL.(TL)= L/360 (0.49")  
CALCULATED VERT. DEFL.(TL)= L/999 (0.12")

CSI: TC=0.48/1.00 (D-E-1), BC=0.56/1.00 (H-I-4), WB=0.76/1.00 (E-H-1), SSI=0.30/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00  
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)  
JSI METAL= 0.57 (C) (INPUT = 1.00)

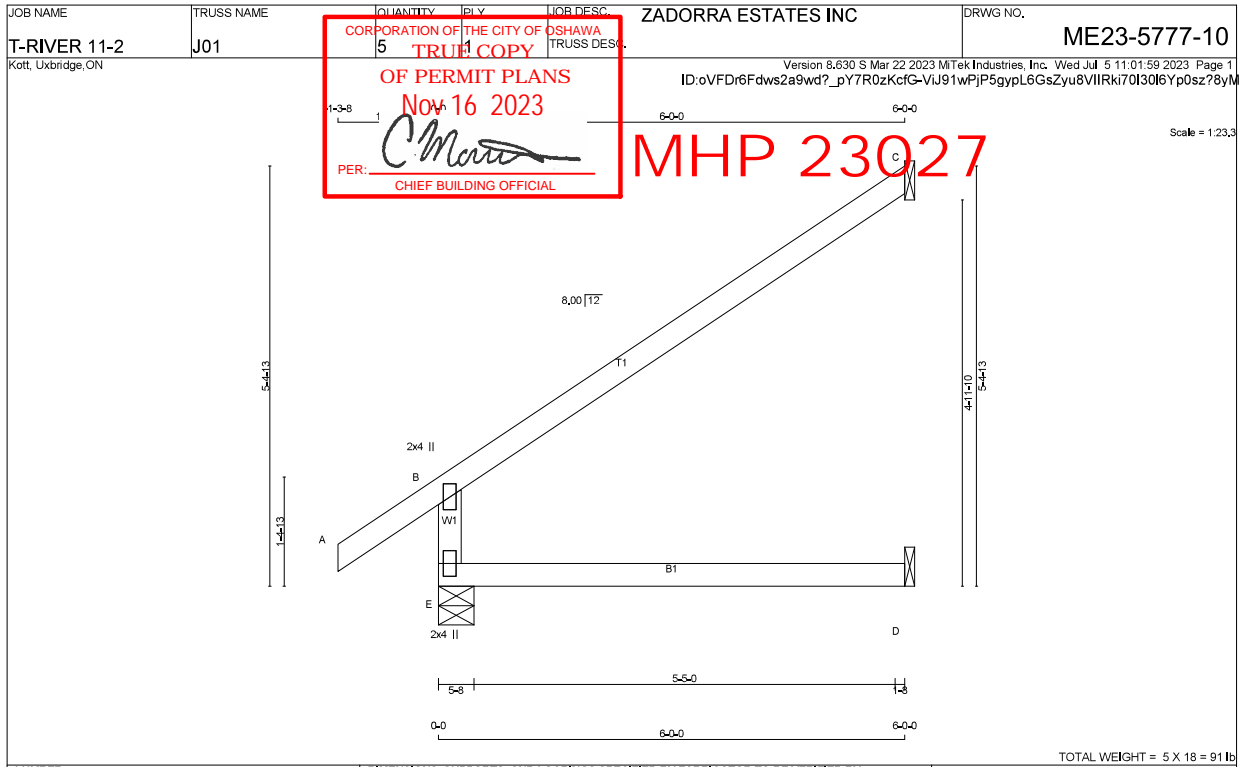
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

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**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE LUMBER DESCR. SPF

A - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
E	675	0	675	0
C	269	0	269	0
D	46	0	51	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
E	469	357 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
C	184	157 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO
E-B	-612 / 0	0.0 0.0	0.13 (4)	7.81
A-B	0 / 45	-119.4 -119.4	0.16 (1)	10.00
B-C	-50 / 0	-119.4 -119.4	0.73 (1)	6.25
E-D	0 / 0	-18.2 -18.2	0.14 (4)	10.00

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 240 IN. GIG**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018 - NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

**DESIGN ASSUMPTIONS**  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.  
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL) = L/360 (0.20")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI TC=0.73/1.00 (B-C-1) BC=0.14/1.00 (D-E-4) ,  
WB=0.00/1.00 (n/a.0) , SSI=0.29/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.90 )  
JSI METAL= 0.31 (B) (INPUT = 1.00 )

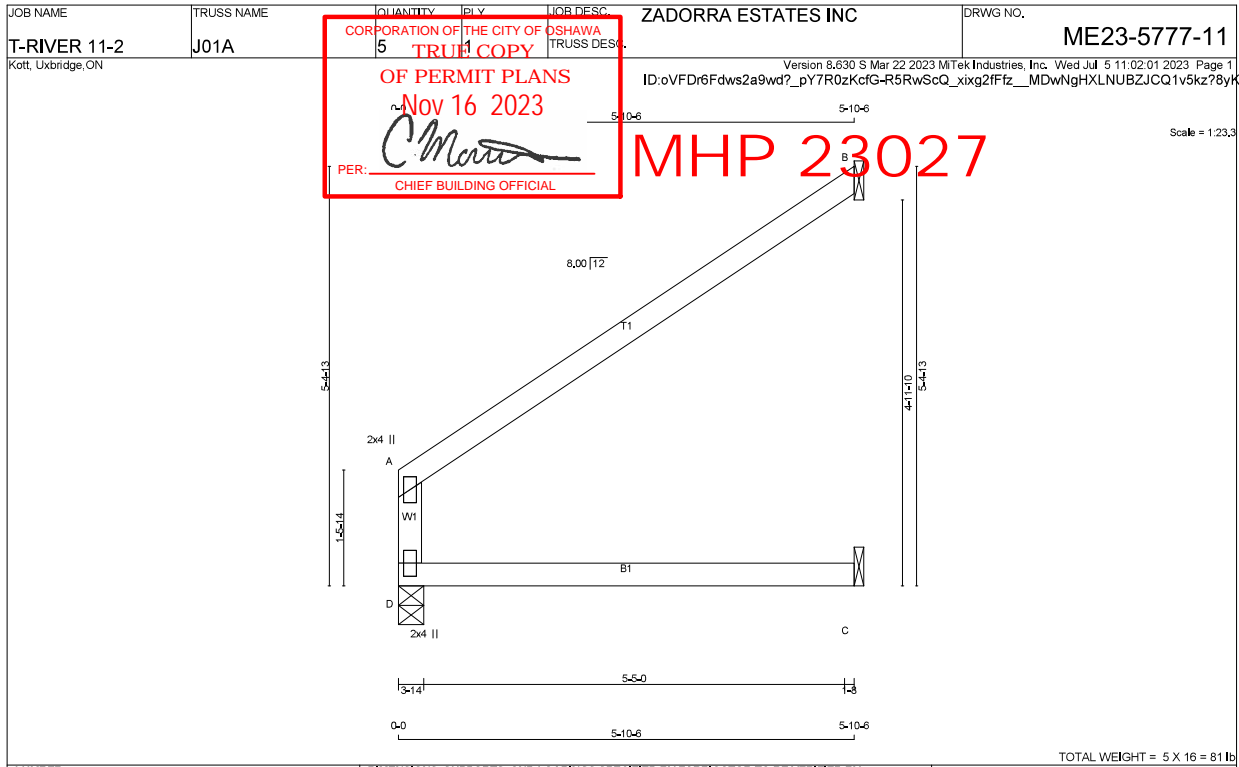
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

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**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE LUMBER DESCR. SPF

D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG UPLIFT	REQRD BRG IN-SX
D	404	0	0	3-14
B	317	0	0	1-8
C	86	0	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
D	282	234/0	0/0	0/0	0/0	78/0	0/0
B	217	182/0	0/0	0/0	0/0	35/0	0/0
C	65	22/0	0/0	0/0	0/0	43/0	0/0

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH (LC)
FR-TO				
D-A	-383/0	0.0	0.0	0.23 (1)
A-B	-18/0	-119.4	-119.4	0.58 (1)
D-C	0/0	-18.2	-18.2	0.28 (1)

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 240 IN. G/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")  
ALLOWABLE DEFL.(TL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(TL) = L/503 (0.14")

CSI TC=0.58/1.00 (A-B:1), BC=0.28/1.00 (C-D:1), WB=0.00/1.00 (n/a:0), SSI=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP (DRY) (PSI)	SHEAR (FL) (PSI)	SECTION (FL) (PSI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (A) (INPUT = 0.90 )  
JSI METAL= 0.20 (A) (INPUT = 1.00 )

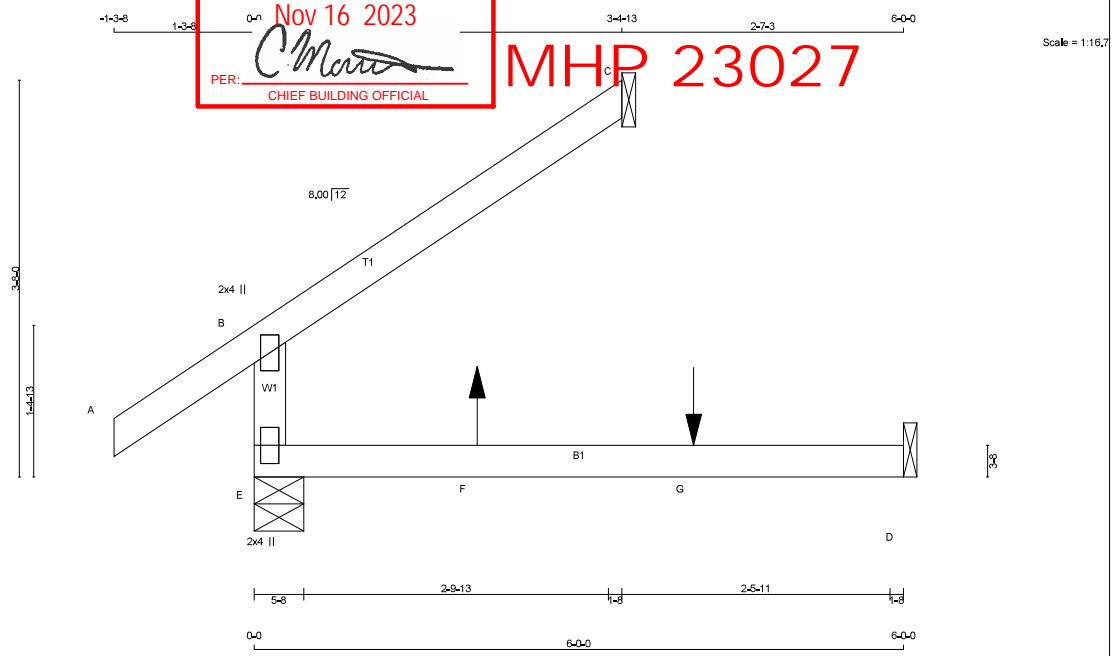
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

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CORPORATION OF THE CITY OF OSHAWA  
**TRUSS COPY**  
**OF PERMIT PLANS**  
**Nov 16 2023**  
 PER: *C. Moran*  
**CHIEF BUILDING OFFICIAL**

MHP 23027

**LUMBER**  
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	478	478	5-8	1-8
C	152	152	0	1-8
D	46	53	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
E	335	240 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	104	89 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	37	0 / -1	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO
E-B	-417 / 0	0.0 0.0	0.13 (4)	7.81
A-B	0 / 45	-119.4 -119.4	0.16 (1)	10.00
B-C	-28 / 0	-119.4 -119.4	0.23 (1)	6.25
E-F	0 / 0	-18.2 -18.2	0.14 (4)	10.00
F-G	0 / 0	-18.2 -18.2	0.14 (4)	10.00
G-D	0 / 0	-18.2 -18.2	0.14 (4)	10.00

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	-5	1	5	BACK	VERT	TOTAL	-	C1
G	4-0-12	-2	-2	-	BACK	VERT	TOTAL	-	C1

**CONNECTION REQUIREMENTS**  
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 240 IN. GIG**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

**DESIGN ASSUMPTIONS**  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.  
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL) = L/360 (0.20")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.23/1.00 (B-C-1) BC=0.14/1.00 (D-E-4) WB=0.00/1.00 (n/a.0) SSI=0.16/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
MAX MIN MAX MIN MAX MIN  
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.  
JSI GRIP= 0.26 (B) (INPUT = 0.90 )  
JSI METAL= 0.21 (B) (INPUT = 1.00 )

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

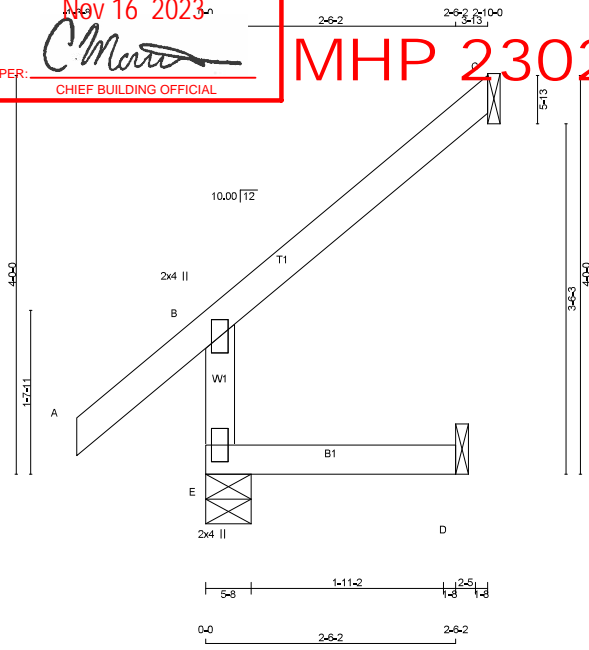
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CORPORATION OF THE CITY OF OSHAWA  
 TRUSS DESIG.  
**TRUSS COPY**  
**OF PERMIT PLANS**  
**Nov 16 2023**  
  
 PER: \_\_\_\_\_  
 CHIEF BUILDING OFFICIAL

Version 8.630 S Mar 22 2023 Mitek Industries, Inc. Wed Jul 5 11:02:03 2023 Page 1  
 ID: oVFDr6FdwS2a9wd?\_pY7R0zKcIG-OTZgttISESJBOLYp25P1qLLS7KL5y52cgjW09dz?8y



TOTAL WEIGHT = 11 lb

**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR. SPF

E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
E	402	402	0	5-8
C	127	127	0	1-8
D	21	23	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
E	277	219/0	0/0	0/0	0/0	58/0	0/0
C	87	74/0	0/0	0/0	0/0	13/0	0/0
D	17	0/0	0/0	0/0	0/0	17/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (7)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH FR-TO
FR-TO				
E-B	-376/0	0.0	0.0	0.01 (4)
A-B	0/53	-119.4	-119.4	0.16 (1)
B-C	-27/0	-119.4	-119.4	0.16 (6)
E-D	0/0	-18.2	-18.2	0.03 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 34.8 PSF  
 DL = 8.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

**SPACING = 240 IN. GIG**  
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF BCBC 2018, NBC-2019AE  
 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

**DESIGN ASSUMPTIONS**  
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.  
 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
 ALLOWABLE DEFL.(TL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI TC=0.16/1.00 (A-B-1) BC=0.03/1.00 (D-E-4) WB=0.00/1.00 (n/a.0) SSI=0.13/1.00 (B-C-6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**


PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)  
 JSI METAL= 0.20 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.



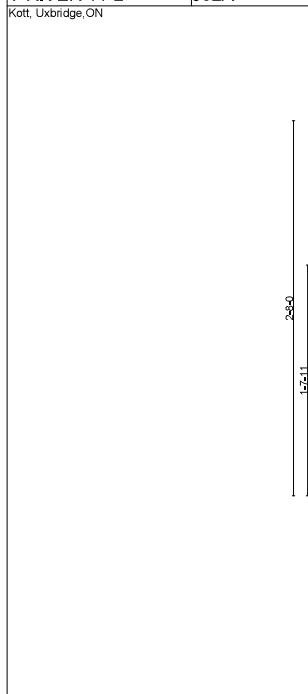
REVIEW FOR TRUSS COMPONENT ONLY  
 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.  
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CORPORATION OF THE CITY OF OSHAWA  
**TRUSS COPY**  
**OF PERMIT PLANS**  
**Nov 16 2023**  
 PER: *C. Moore*  
 CHIEF BUILDING OFFICIAL

Version 8.630 S Mar 22 2023 Mitek Industries, Inc. Wed Jul 5 11:02:04 2023 Page 1  
 ID:eVFDr6FdwS2a9wd7\_pY7R0zKcFG-sg624eTsDdJFv6\_EfBY3rZ?I4IRhYllUuNFah3z78yH



**MHP 23027**

Scale = 1/12,8

**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR.  
 A - B 2x4 DRY No.2 SPF  
 A - C 2x4 DRY No.2 SPF  
 E - D 2x4 DRY No.2 SPF  
 DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION VERT	MAXIMUM GROSS REACTION DOWN	FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	345	0	0	5-8	1-8
C	0	4	0	6-5	1-8
D	13	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C, D  
 PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

**UNFACTORED REACTIONS**

JT	1ST CASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	238	189/0	0/0	0/0	0/0	49/0	0/0
C	0	0/-43	0/0	0/0	0/0	3/0	0/0
D	12	0/-5	0/0	0/0	0/0	16/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C  
**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (5)

MEMB.	CHORDS		W E B S	
	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO			FROM	TO
E-B	-313/0	0.0	0.0	0.04 (5)
A-B	0/53	-119.4	-119.4	0.16 (1)
B-C	-47/0	-119.4	-119.4	0.12 (1)
E-D	0/0	-18.2	-18.2	0.04 (5)

**CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN**  
 PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 34.8 PSF  
 DL = 8.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. C/C**  
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015  
 THIS DESIGN COMPLIES WITH:  
 - PART 9 OF BCBC 2018, NBC-2019AE  
 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

**DESIGN ASSUMPTIONS**  
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.  
 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
 ALLOWABLE DEFL.(TL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")  
 CSI TC=0.16/1.00 (A-B 1) BC=0.04/1.00 (D-E 5)  
 WB=0.00/1.00 (n/a 0) SSI=0.10/1.00 (A-B 1)  
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
 COMP=1.10 SHEAR=1.10 TENS=1.10  
 COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches  
 PLATE ROTATION TOL. = 5.0 Deg.  
 JSI GRIP = 0.21 (B) (INPUT = 0.90)  
 JSI METAL = 0.17 (B) (INPUT = 1.00)

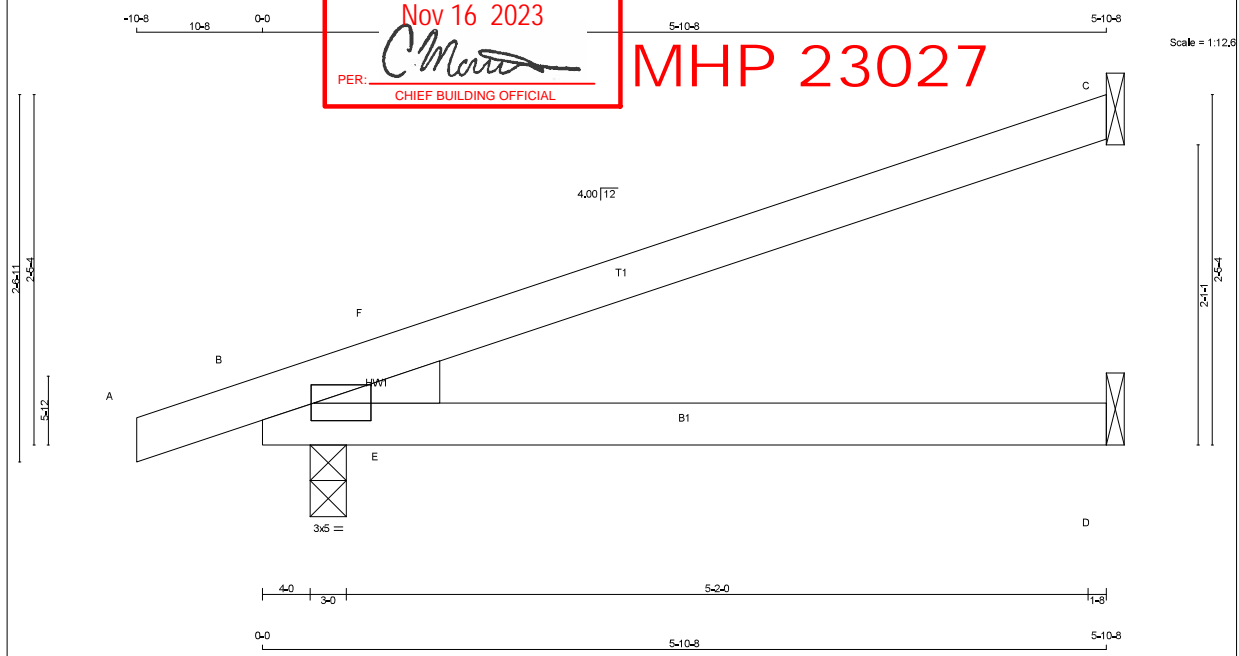
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY  
 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.  
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 TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbciindustry.com







**LUMBER**  
N.L.G.A. RULES  
CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X	
B	TMBH14	MT20	3.0	5.0	1.50	0.25

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM GROSS REACTION	INPUT BRG	REQRD BRG	HEEL WEDGE
	VERT	DOWN	IN-SX	IN-SX	
C	299	0	299	0	1-8
B	514	0	514	0	3-0
D	105	0	105	0	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
C	205	169 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
B	358	289 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	77	35 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-B	0 / 8	-119.4	-119.4	0.07 (1)	10.00	E-F	-328 / 8
B-F	-23 / 0	-119.4	-119.4	0.07 (4)	6.25		0.00 (1)
F-C	0 / 3	-119.4	-119.4	0.51 (1)	10.00		
B-E	0 / 0	-18.2	-18.2	0.41 (1)	10.00		
E-D	0 / 0	-18.2	-18.2	0.41 (1)	10.00		

**DESIGN CRITERIA**

SPECIFIED LOADS:  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. GIG**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(LL) = L/683 (0.10")  
ALLOWABLE DEFL.(TL)= L/360 (0.20")  
CALCULATED VERT. DEFL.(TL) = L/377 (0.19")

CSI TC=0.51/1.00 (C-F:1), BC=0.41/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE (PSI)	GRIP (FL)	(DRY)	SHEAR (FL)	SECTION (FL)
MT20	650	371	1747	788
			1987	1873

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.  
JSI GRIP= 0.73 (B) (INPUT = 0.90)  
JSI METAL= 0.07 (B) (INPUT = 1.00)

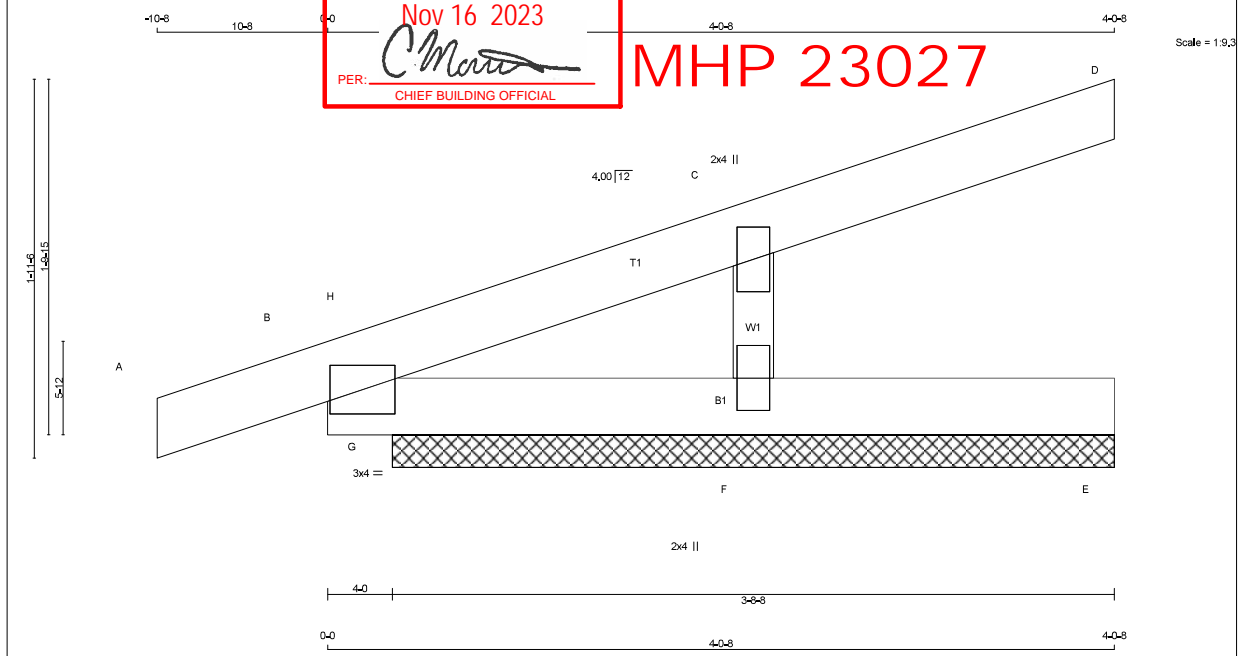
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

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**LUMBER**  
N.L.G.A. RULES  
CHORDS SIZE LUMBER DESCR. SPF  
A - D 2x4 DRY No.2  
B - E 2x4 DRY No.2  
ALL WEBS 2x3 DRY No.2  
DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TM31-J	MT20	3.0	4.0		
C	TMW+ww	MT20	2.0	4.0		
F	BMV1+ww	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX
B	162	0	3-8-8	1-8
E	17	0	3-8-8	1-8
F	493	0	3-8-8	1-8

**UNFACTORED REACTIONS**

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	113	85 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
E	13	3 / -0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
F	342	281 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, F

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (7)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD			MEMB. FORCE (LBS)	WEBS MAX. FACTORED FORCE (LBS)
		LC1	MAX	MIN		
FR-TO		FROM	TO	LENGTH	FR-TO	
A-B	0 / 8	-119.4	-119.4	0.08 (7)	10.00	F-C -473 / 0
B-H	-108 / 0	-119.4	-119.4	0.04 (1)	6.25	G-H 0 / 137
H-C	0 / 17	-119.4	-119.4	0.31 (1)	10.00	
C-D	-37 / 0	-119.4	-119.4	0.31 (1)	6.25	
B-G	0 / 24	-18.2	-18.2	0.04 (6)	10.00	
G-F	0 / 24	-18.2	-18.2	0.04 (6)	10.00	
F-E	0 / 0	-18.2	-18.2	0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

**DESIGN ASSUMPTIONS**  
- OVERHANG NOT TO BE ALTERED OR CUT OFF.  
(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI TC=0.31/1.00 (C-H-1), BC=0.04/1.00 (B-G-6),  
WB=0.07/1.00 (C-F-1), SSI=0.18/1.00 (C-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP (DRY) (PSI)	SHEAR (FL)	SECTION (FL)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.  
JSI GRIP= 0.25 (C) (INPUT = 0.90)  
JSI METAL= 0.14 (C) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

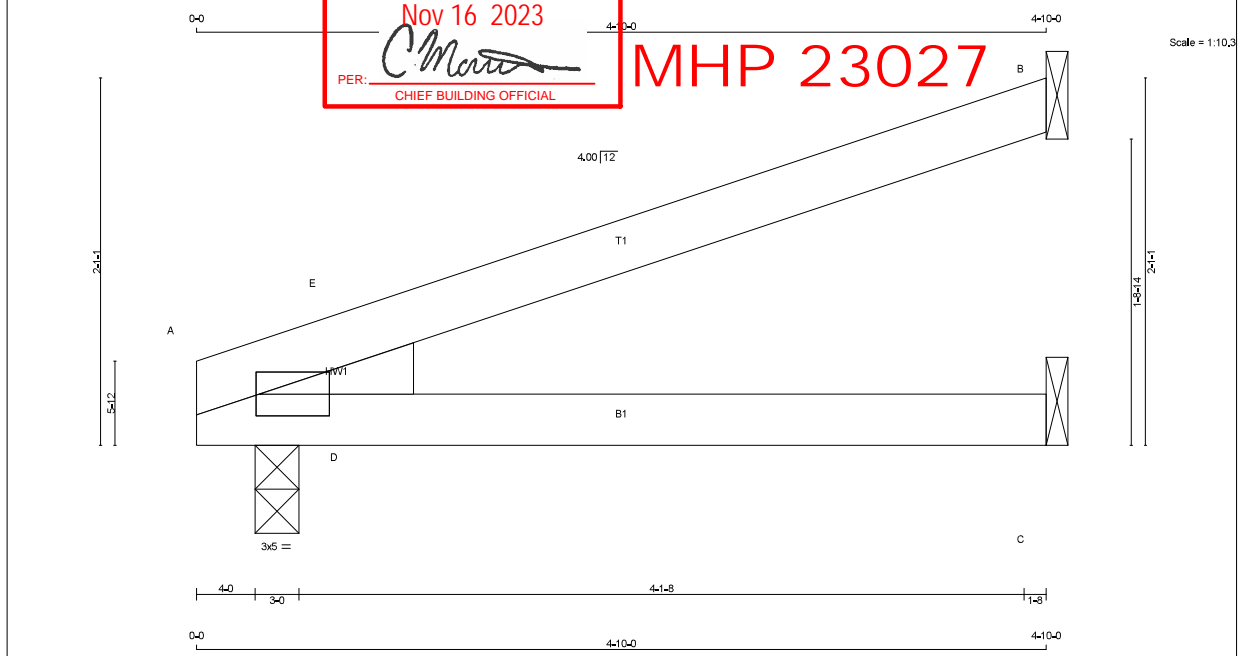
REVIEW FOR TRUSS COMPONENT ONLY

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CORPORATION OF THE CITY OF OSHAWA  
**TRUSS COPY**  
**OF PERMIT PLANS**  
**Nov 16 2023**  
 PER: *C. Moore*  
**CHIEF BUILDING OFFICIAL**



**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR. SPF  
 A - B 2x4 DRY No.2  
 A - C 2x4 DRY No.2

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
A	TMBH14 MT20	3.0	5.0	1.50	0.25

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION VERT	MAXIMUM GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	243	0	243	0	1-8
C	89	0	89	0	1-8
A	333	0	333	0	3-0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B, C

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
B	167	137 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
C	65	31 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
A	233	168 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	MEMB. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)	CS1 (LC)	MAX.
A-E	-20 / 0	-119.4	-119.4	0.06 (4)	6.25	D-E	-236 / 6	0.00 (1)
E-B	0 / 3	-119.4	-119.4	0.34 (1)	10.00			
A-D	0 / 0	-18.2	-18.2	0.29 (1)	10.00			
D-C	0 / 0	-18.2	-18.2	0.29 (1)	10.00			

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 34.8 PSF  
 DL = 6.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**  
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF BCBC 2018, NBC-2019AE  
 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05")  
 ALLOWABLE DEFL.(TL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(TL) = L/656 (0.09")

CSI TC=0.34/1.00 (B-E:1), BC=0.29/1.00 (A-D:1), WB=0.00/1.00 (D-E:1), SSI=0.18/1.00 (A-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10  
 COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE (PSI)	GRIP (FL)	DRY (FL)	SHEAR (FL)	SECTION (FL)
MT20	650	371	1747	788
			1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (A) (INPUT = 0.90 )  
 JSI METAL= 0.04 (A) (INPUT = 1.00 )

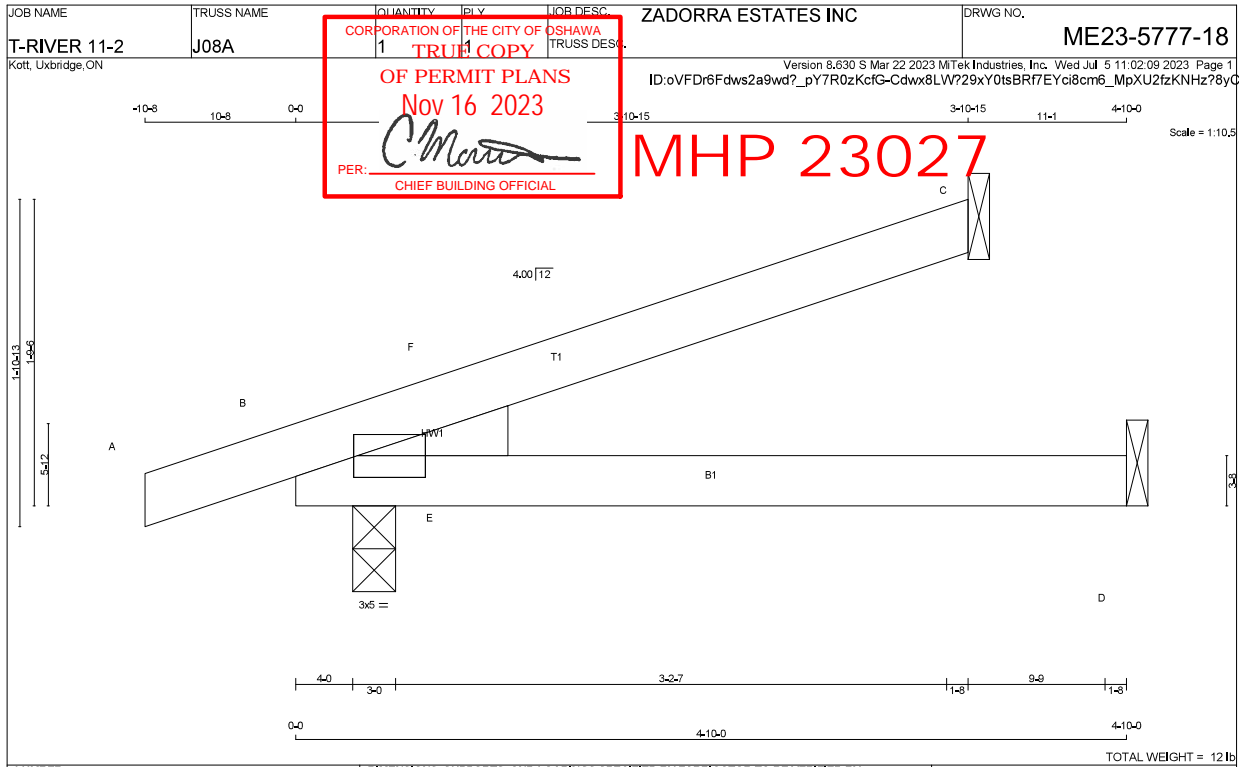
MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

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**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE LUMBER DESCR. SPF  
A - C 2x4 DRY No.2  
B - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH14	MT20	3.0	5.0	1.50	0.25

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

JT	FACTORED GROSS REACTION		MAXIMUM GROSS REACTION		INPUT BRG	REQRD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
C	205	0	205	0	1-8	1-8	
B	393	0	393	0	3-0	1-8	2x4 L
D	67	0	67	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	1ST LCASE MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
C	142	113/0	0/0	0/0	0/0	28/0	0/0
B	274	205/0	0/0	0/0	0/0	69/0	0/0
D	50	18/0	0/0	0/0	0/0	31/0	0/0

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
A-B	0/8	-119.4	10.00	-119/29
B-F	-41/0	-119.4	6.25	
F-C	0/9	-119.4	10.00	
B-E	0/0	-18.2	10.00	
E-D	0/0	-18.2	10.00	

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, NBC-2019AE  
- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")  
ALLOWABLE DEFL.(TL) = L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.24/1.00 (C-F:1), BC=0.17/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.15/1.00 (C-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP (DRY) (PSI)	SHEAR (FL) (PSI)	SECTION (FL) (PSI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.  
JSI GRIP= 0.49 (B) (INPUT = 0.90 )  
JSI METAL= 0.05 (B) (INPUT = 1.00 )

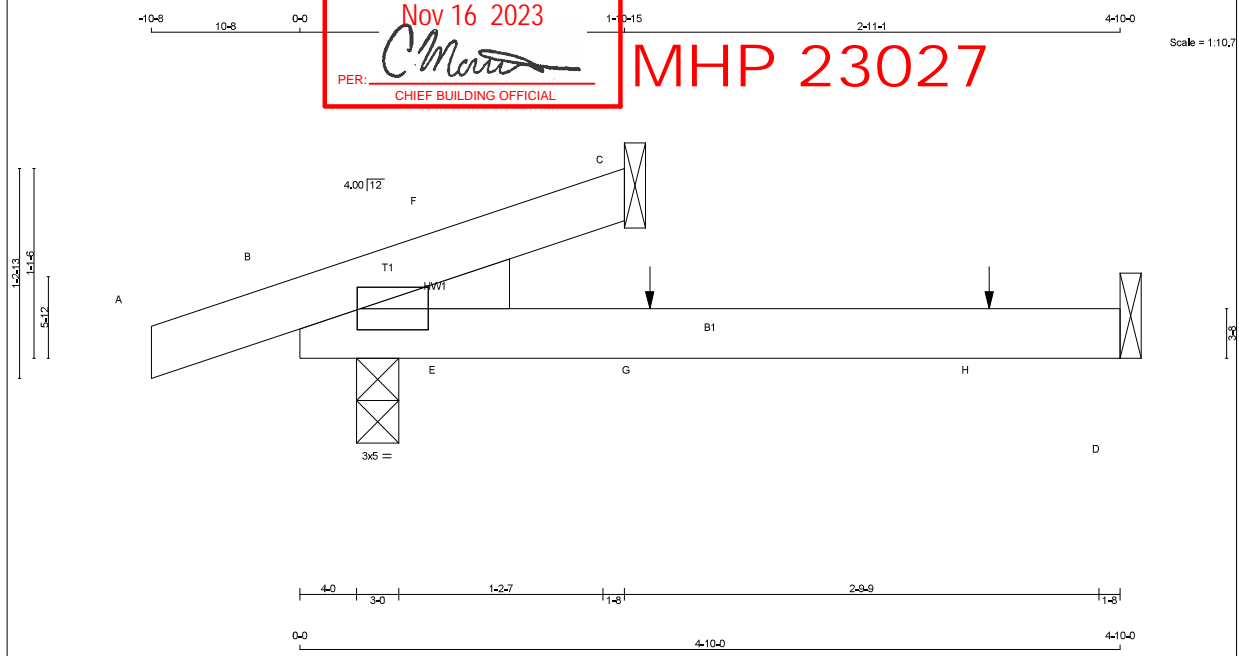
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**LUMBER**  
N.L.G.A. RULES  
CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	
B - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH14	MT20	3.0	5.0	1.50 0.25

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL WEDGE
C	189 0	189 0	0 0	1-8	1-8
B	262 0	262 0	0 0	3-0	1-8 2x4 L
D	145 0	145 0	0 0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
C	133	92/0	0/0	0/0	0/0	41/0	0/0
B	181	139/0	0/0	0/0	0/0	42/0	0/0
D	103	67/0	0/0	0/0	0/0	36/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
A-B	0/8	-119.4 -119.4	0.07 (1)	10.00
B-F	-65/0	-119.4 -119.4	0.13 (1)	6.25
F-C	0/41	-119.4 -119.4	0.18 (1)	10.00
B-E	0/0	-18.2	-18.2	0.06 (1) 10.00
E-G	0/0	-18.2	-18.2	0.15 (1) 10.00
G-H	0/0	-18.2	-18.2	0.15 (1) 10.00
H-D	0/0	-18.2	-18.2	0.15 (1) 10.00

**FACTORED CONCENTRATED LOADS (LBS)**

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-28	-28	--	BACK	VERT	TOTAL	--	C1
H	4-0-12	-142	-142	--	BACK	VERT	TOTAL	--	C1

**CONNECTION REQUIREMENTS**

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:  
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- PART 9 OF OBC 2012 (2019 AMENDMENT)  
- CSA 086-14  
- TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")  
ALLOWABLE DEFL.(TL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI TC=0.18/1.00 (C-F:1), BC=0.15/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SH=0.14/1.00 (C-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.  
JSI GRIP= 0.08 (B) (INPUT = 0.90)  
JSI METAL= 0.02 (B) (INPUT = 1.00)

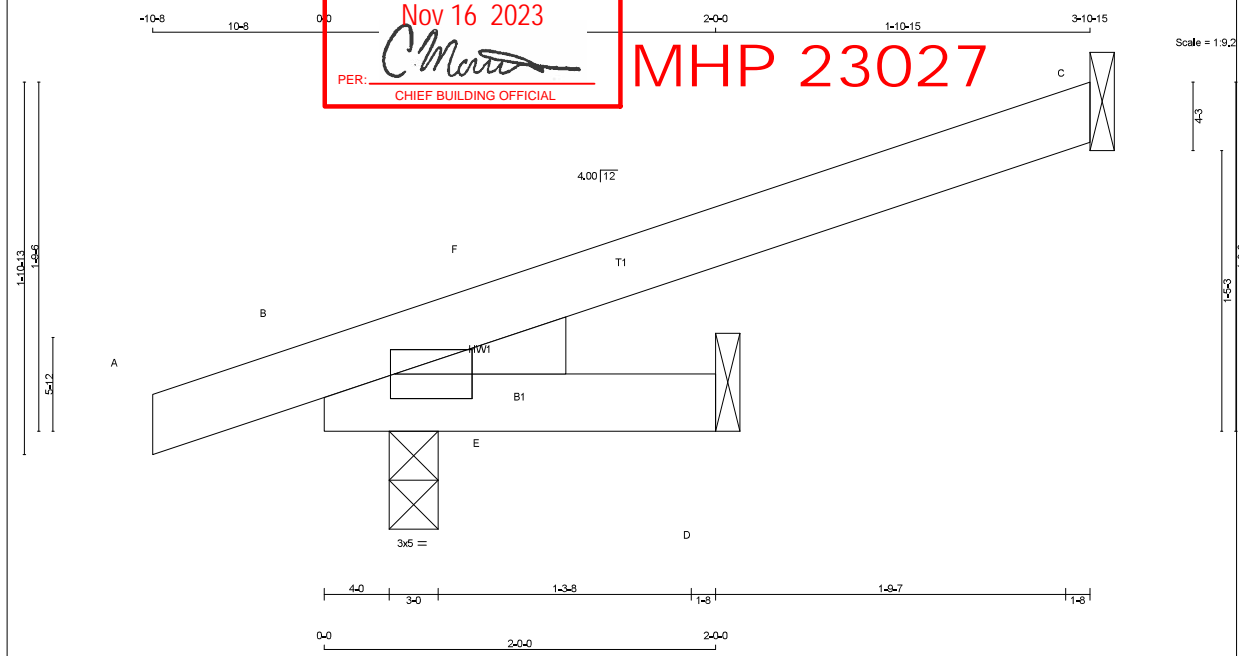
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**LUMBER**  
 N.L.G.A. RULES  
 CHORDS SIZE LUMBER DESCR. SPF  
 A - C 2x4 DRY No.2  
 B - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH14	MT20	3.0	5.0	1.50	0.25

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	162	0	1-8	1-8	
B	294	0	3-0	1-8	2x4 L
D	158	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
C	111	94/0	0/0	0/0	0/0	17/0	0/0
B	203	160/0	0/0	0/0	0/0	43/0	0/0
D	110	82/0	0/0	0/0	0/0	27/0	0/0

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC LENGTH	MAX. FACTORED FORCE (LBS)
A-B	0/8	-119.4	0.08 (5)	10.00
B-F	0/24	-119.4	0.07 (1)	10.00
F-C	-5/0	-119.4	0.15 (1)	10.00
B-E	0/0	-18.2	0.22 (1)	10.00
E-D	0/0	-18.2	0.22 (1)	10.00

**CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN**

**PATTERN LOADING CHECK APPLIED TO THIS TRUSS.**

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 34.8 PSF  
 DL = 8.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**

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 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")  
 ALLOWABLE DEFL.(TL)= L/360 (0.19")  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.15/1.00 (C-F:1), BC=0.22/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SSI=0.14/1.00 (C-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP (DRY) (PSI)	SHEAR (FL) (PSI)	SECTION (FL) (PSI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches  
 PLATE ROTATION TOL. = 5.0 Deg.  
 JSI GRIP= 0.33 (B) (INPUT = 0.90 )  
 JSI METAL= 0.03 (B) (INPUT = 1.00 )

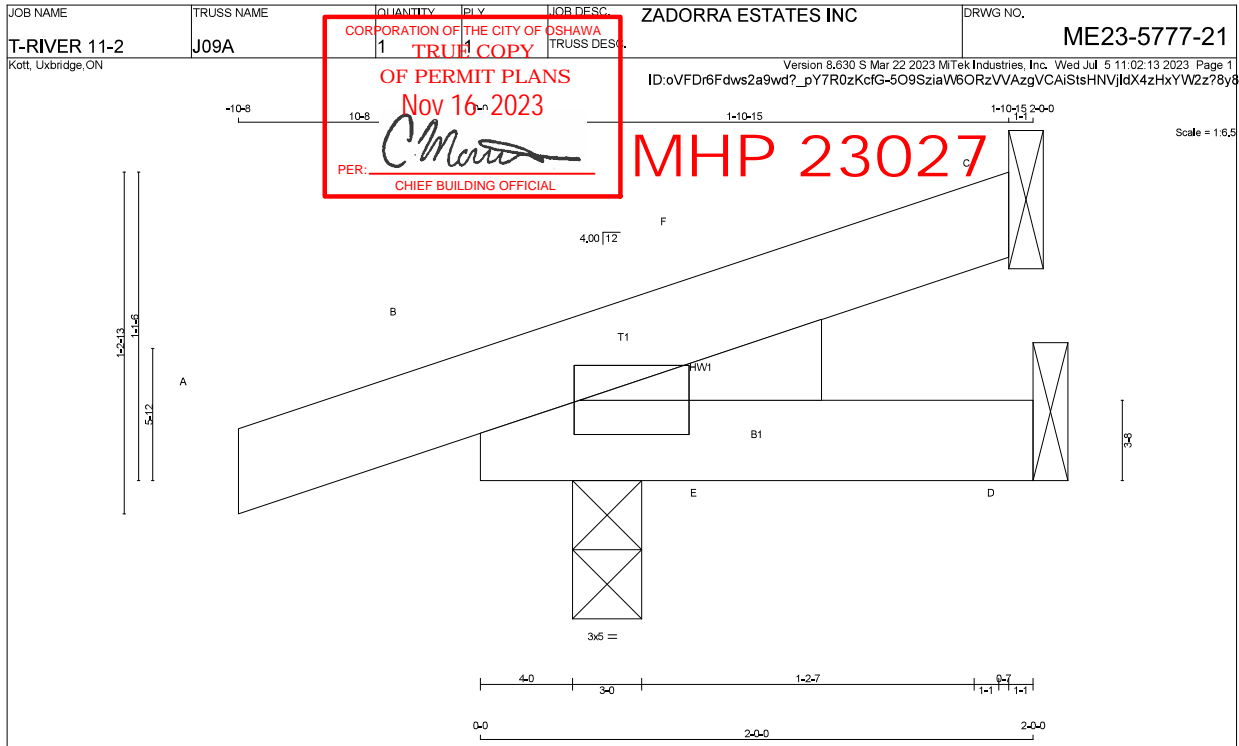
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**LUMBER**  
N.L.G.A. RULES  
CHORDS SIZE LUMBER DESCR.  
A - C 2x4 DRY No.2 SPF  
B - D 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH14	MT20	3.0	5.0	1.50 0.25

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION VERT	MAXIMUM GROSS REACTION DOWN	FACTORED GROSS REACTION UP	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL BRG WEDGE
C	86	0	86	0	1-8	1-8
B	244	0	244	0	3-0	1-8
D	45	0	45	0	1-8	1-8

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 34.8 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.3 PSF  
TOTAL LOAD = 48.1 PSF

**SPACING = 24.0 IN. G/C**

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(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
ALLOWABLE DEFL.(TL)= L/360 (0.19")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI TC=0.07/1.00 (A-B:1), BC=0.05/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SH=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

**UNFACTORED REACTIONS**

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	60	47/0	0/0	0/0	0/0	13/0	0/0
B	159	132/0	0/0	0/0	0/0	37/0	0/0
D	32	19/0	0/0	0/0	0/0	14/0	0/0

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (5)

MEMB.	CHORDS			WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
A-B	0/8	-119.4	-119.4	0.07 (1)	10.00	E-F	-58/3
B-F	-14/0	-119.4	-119.4	0.04 (1)	6.25		0.00 (1)
F-C	0/9	-119.4	-119.4	0.04 (1)	10.00		
B-E	0/0	-18.2	-18.2	0.05 (1)	10.00		
E-D	0/0	-18.2	-18.2	0.05 (1)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

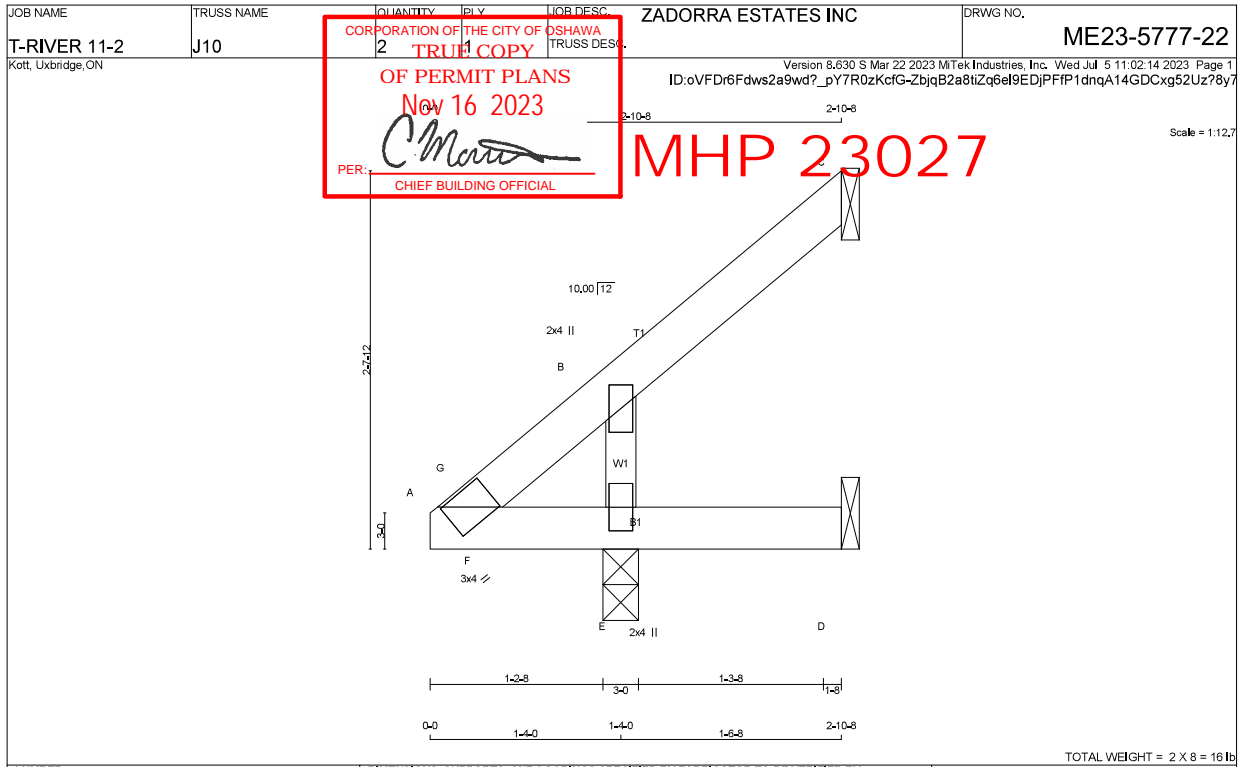
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**LUMBER**  
N. L. G. A. RULES  
CHORDS SIZE LUMBER DESCR. SPF  
A - C 2x4 DRY No.2  
A - D 2x4 DRY No.2  
ALL WEBS 2x3 DRY No.2  
SPF  
DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
E	BWV1+w	MT20	2.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQRD BRG IN-SX
JT	55	0	-23	1-8
C	-29	0	-37	1-8
D	369	0	0	3-0

SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) C, D  
 PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT  
 PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

**UNFACTORED REACTIONS**

JT	1ST CASE	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
C	37	34 / -16	0 / 0	0 / 0	0 / 0	3 / 0	0 / 0	
D	-19	0 / -24	0 / 0	0 / 0	0 / 0	2 / 0	0 / 0	
E	258	187 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E  
**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
TOTAL LOAD CASES: (5)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED (LC)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED (LC)
FR-TO		FROM TO	LENGTH	FR-TO		
A-G	-12 / 5	-119.4	-119.4 0.02 (5)	6.25	E-B	-240 / 0 0.03 (1)
G-B	0 / 13	-119.4	-119.4 0.10 (1)	10.00	F-G	-81 / 0 0.00 (1)
B-C	-27 / 0	-119.4	-119.4 0.09 (1)	6.25		
A-F	0 / 27	-18.3	-18.3 0.01 (1)	10.00		
F-E	0 / 24	-18.2	-18.2 0.10 (5)	10.00		
E-D	0 / 0	-18.2	-18.2 0.10 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN  
 PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

**DESIGN CRITERIA**  
 SPECIFIED LOADS:  
 TOP CH. LL = 34.8 PSF  
 DL = 8.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.3 PSF  
 TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C  
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015  
 THIS DESIGN COMPLIES WITH:  
 - PART 9 OF BCBC 2018, NBC-2019AE  
 - PART 9 OF OBC 2012 (2019 AMENDMENT)  
 - CSA 086-14  
 - TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD  
 ALLOWABLE DEFL.(LL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")  
 ALLOWABLE DEFL.(TL) = L/360 (0.19")  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CANTILEVER DEFLECTION:  
 ALLOWABLE DEFL.(LL) = L/120 (0.19")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")  
 ALLOWABLE DEFL.(TL) = L/120 (0.19")  
 CALCULATED VERT. DEFL.(TL) = L/971 (0.02")  
 CSI TC=0.10/1.00 (B-G:1), BC=0.10/1.00 (E-F:5), WB=0.03/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)  
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10  
 COMPANION LIVE LOAD FACTOR = 1.00  
 AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.  
 NAIL VALUES  
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
 MAX MIN MAX MIN MAX MIN  
 MT20 650 371 1747 788 1987 1873  
 PLATE PLACEMENT TOL. = 0.250 inches  
 PLATE ROTATION TOL. = 5.0 Deg.  
 JSI GRIP= 0.17 (B) (INPUT = 0.90)  
 JSI METAL= 0.13 (B) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY  
 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEER'S SEAL

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult  
 TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com

