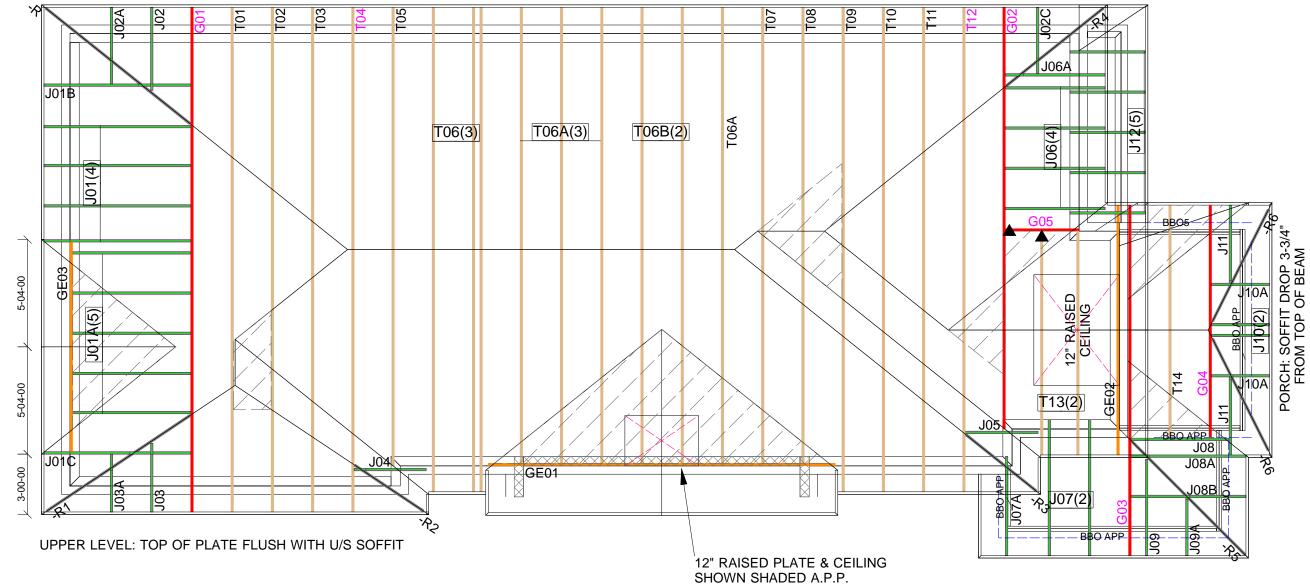


Please Note: Truss layout revised from plans to reduce girder point loads.



Hanger Name	Symbol	QTY
LUS24	<b>A</b>	2
LJS26DS		0



# CONVENTIONAL FRAMING BY OTHERS

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB IN	FORMATION
Customer	GREENPARK GROUP
Job #	23-00090R0
Address	ZADORRA ESTATES ZADORRA ESTATES INC.
Address	OSHAWA,ON
Model	RIVER 11-1
Sales Rep	RALPH MIRIGELLO
Designer	LI
Date	2023-07-05
Path	C:\MITEK\CA\JOBS\GREENPARK GROUP\ZADORRA ESTATES\MODELS\RIVER 11\T-RIVER 11-1\

# DESIGN INFORMATION Code NBCC 2015 Bldg Residential - HSB (NBCC Part 9) TC LL 34.8 lb/ft² TC DL 6.0 lb/ft² BC LL 0.0 lb/ft² BC DL 7.3 lb/ft²

DeflectionLL=L/360TL=L/360Spacing24" O/C unless otherwise noted

Complies OBC 2012 (2019 Amendment)
With CSA O86-14 and TPIC 2014

# IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

# KOTT Inc.

14 Anderson Blvd. Uxbridge, ON 905.642.4400





# General Guidelines for Truss Manufacturer and Installer on Reading Truss Component Drawings



# Read Carefully Prior to Manufacture and Installation

Note: It is important that all information on the truss component drawing is understood by all interested parties. If clarification is required, please contact your truss supplier prior to installation of the trusses

#### Standard Design Loading:

Standard loading is indicated on the drawing legend for the top and bottom chords, for snow, live and dead loads where indicated. Actual panel UDL is further indicated for individual panels in the body of the truss drawing.

#### Non-Standard Loading:

Additional uniform loading is included in individual panel loading. Concentrated loads are noted in a separate table in the body of the drawing.

#### Reactions:

Factored gross reactions are indicated as Maximum Factored Reactions, not necessarily for the load case outlined on the drawing. Includes vertical, horizontal and uplift.

#### **Lumber size and Grade:**

The member size and grade is indicated in the lumber table. The truss must be manufactured with the same size and species noted but may be an equal or better grade than indicated.

#### Plates sizes:

Plate sizes are noted as Width x Length, where the plate slot direction is parallel to the plate length. Plate sizes indicated are the minimum required and may be increased.

#### Plate location:

Plates are centred on the joint unless an x-y offset is indicated. If clarification of placement is required prior to manufacture or during inspection, additional detail on plate placement is available from the truss manufacturer.

#### Bearing:

In most cases, input bearing size (input by designer) and minimum required bearing are indicated on the drawing. In cases where the bearing capacity has been enhanced by using a bearing block, bearing enhancer or flush plate, the bearing required will match the input bearing even where the required bearing might be less than what is indicated

#### Ply to ply connection:

Where the truss is designed for 2 or more plys, the individual truss plys must be fastened together. A nailing chart will be included which includes nails size, type, spacing and rows for each member. For 4 ply trusses, bolts or structural screws may also be noted

#### **Building Code:**

The truss will be designed as Part 9, Part 4 or Farm and will be noted in the legend. In certain cases, wind loading will also be required and will be outlined on the drawing, including information pertaining to location, building height, exposure class and opening size. TPIC requires that some non-trangulated frames such as attic trusses and gambrel arches be designed Part 4 even though the building itself might meet the requirements of Part 9.

#### **Chord Bracing:**

Minimum spacing for bracing for the top and bottom chord is clearly indicated. This can also be achieved when suitable sheathing is directly connected to the top chord and when a suitable ceiling is directly connected to the bottom chord. For large cantilevers where there is typically not a directly connected ceiling, care should be taken to meet the bracing criteria noted. The base truss for piggyback situations must have 2x4 purlins (max truss spacing 24" o/c) connected at a maximum of 24" o/c along the flat top chord section. Additional x-bracing may be required in the plane of the purlins.

#### Web Bracing:

Requirements for individual web bracing will be indicated on the drawing. This will either be a lateral brace or T-brace. Where a T-brace is specified, size, grade and nailing requirement will be noted. For a lateral brace, a 1x4 minimum is required. Note: The building designer is responsible for ensuring adequate load transfer from the individual lateral braces into the overall structure.

#### **Design Results:**

Axial forces for load case 1 are indicated on the drawing. Other load case results can be supplied upon request. Maximum stress indices are also indicated for both the lumber and plates. Maximum deflection is indicated, both allowable and calculated.

## Manufacturing tolerances:

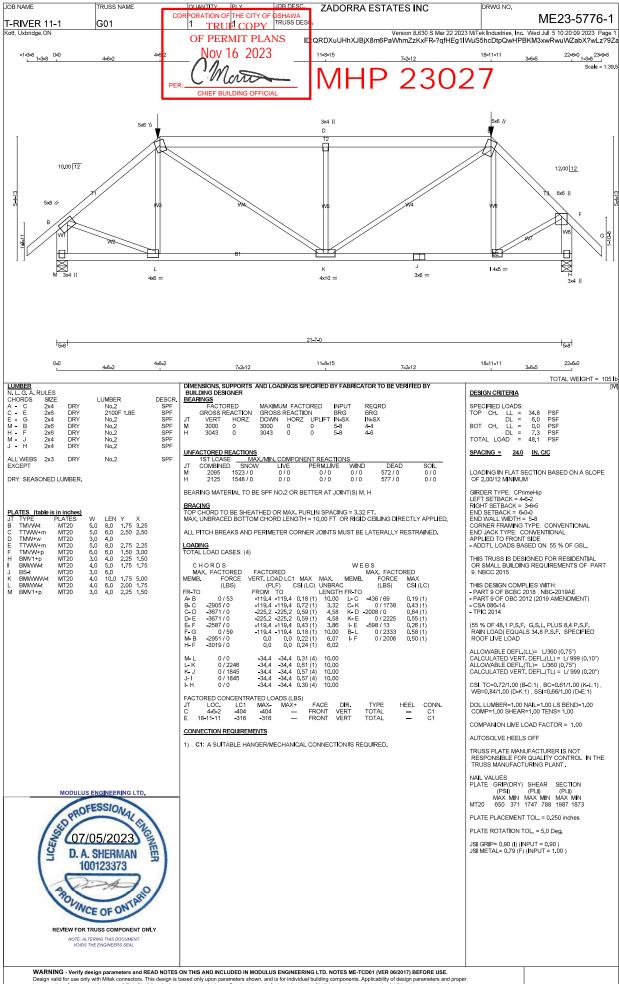
Tolerances for plate placement as outlined in TPIC Appendix G are noted on each truss component drawing.

# Failure to follow these guidelines could cause property damage and personal injury

- Additional stability bracing for truss system, e.g. diagonal or xbracing is always required. Consult <u>BCSI-CANADA</u> for installation requirements (copies available from your truss supplier or from www.sbcindustry.com)
- 2. Truss bracing must be designed by an engineer. Individual lateral braces shown in truss drawings must be incorporated into overall structure through connection to diaphragm or other means.
- 3. Never exceed the design loading shown and never stack building materials on inadequately braced trusses
- Provide copies of truss component drawings to the building department, erection supervisor, property owner and all othe interested parties (e.g. Building designer where required)
- 5. Cut members to bear tightly against one another
- 6. Place plates on each face of truss at each joint and embed fulusing proper roller or hydraulic press. Knots and wane at joir t locations are regulated by TPIC Appendix G
- 7. Design assumes trusses will be suitably protected from the environment in accordance with TPIC
- 8. Unless otherwise noted, MC of lumber shall not exceed 19% at time of manufacture
- 9. Unless expressly noted, this design is not applicable for fire retardant, preservative treatment or green lumber nor for use corrosive environment
- 10. Connections not shown are the responsibility of others
- 11. Do not cut or alter truss members or plates without prior appreal a of an engineer
- 12. Install and load vertically unless otherwise noted
- 13. Review all portions of this design including all notes. Reviewing pictures alone is not sufficient
- 14. Design assumes manufactured in accordance with TPIC Quality criteria as outlined in Appendix G
- 16. Building designer must review individual component drawings to ensure they are suitable for the structure
- 15. Not designed for solar panels unless specifically noted

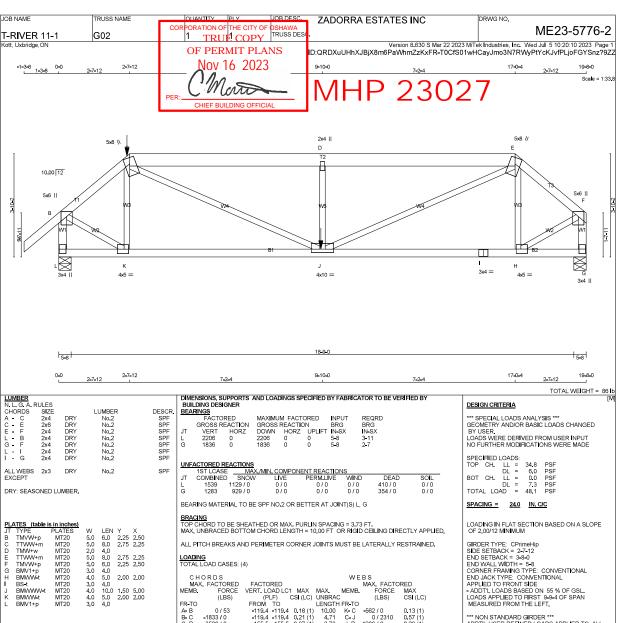
ME-TCD01 (VER. 06/2017)





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Milet connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is representability of two littures designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult





		ORDS				WE			
- 1	MAX		FACTORED				MAX. FACTO	DRED	
- 1	MEMB.	FORCE	VERT. LOAD L	C1 MAX	MAX.	MEMB.	FORCE	MAX	
1		(LBS)	(PLF)	CSI (LC)	UNBRAG	С	(LBS)	CSI (LC)	
1	FR-TO		FROM TO		LENGTH	I FR-TO			
1	A− B	0 / 53	119.4 -119	4 0.18 (1	10.00	K-C	<b>-</b> 562 / 0	0.13(1)	
1	B-C	-1833 / 0	119.4 -119	4 0.21 (1	4.71	C-J	0 / 2310	0.57(1)	
1	C- D	-3509 / 0	155.5 -155	5 0.67 (1)	3,73	J-D -	<b>-</b> 1209 / 0	0.28 (1)	
1	D-E	-3509 / 0	119.4 -119	4 0.67 (1)	3,73	J <del>-</del> E	0 / 2484	0.61(1)	
1	E-F	-1632 / 0	119.4 119	4 0.20 (1	4.95	H-E	-523 / 0	0.12(1)	
1	L-B	-2217 / 0	0.0 0	.0 0.26 (1	5.66	B-K	0 / 1589	0.39(1)	
	G-F	-1843 / 0	0.0	.0 0.21 (1	6,12	H-F	0 / 1414	0.35 (1)	
	L- K	0/0	-23.8 -23	8 0 22 (4)	10.00				
1	K-J	0 / 1430	23.8 23						
1	J-1	0 / 1274		2 0.34 (1					
1	I- H	0 / 1274	18.2 18						
1	H-G	0/0		2 0.15 (4					
1	11-0	0,0	-10.2 -10	.2 0.10 (4	10.00				
	FACTO	RED CONCENT	RATED LOADS	(LBS)					

MAX--114 -741 HEEL CONN. -- C1 -- C1

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

\*\*\* NON STANDARD GIRDER \*\*\* ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF BCBC 2018, NBC-2019AE
-PART 9 OF DBC 2012 (2019 AMENDMENT)
-CSA 086-14
-TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (0.66")
CALCULATED VERT. DEFL (LL) = L/999 (0.11")
ALLOWABLE DEFL (TL) = L/360 (0.66")
CALCULATED VERT. DEFL (TL) = L/999 (0.20")

CSI: TC=0.67/1.00 (C-D:1) , BC=0.41/1.00 (J-K:1) , WB=0.61/1.00 (E-J:1) , SSI=0.44/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

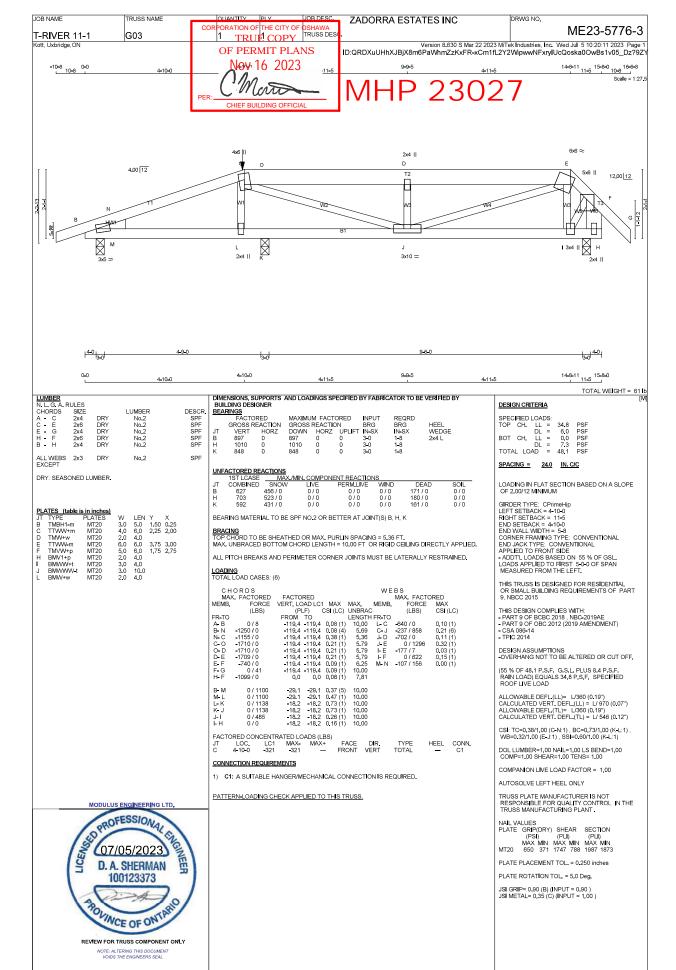
JSI GRIP= 0.90 (C) (INPUT = 0.90 ) JSI METAL= 0.41 (I) (INPUT = 1.00 )



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

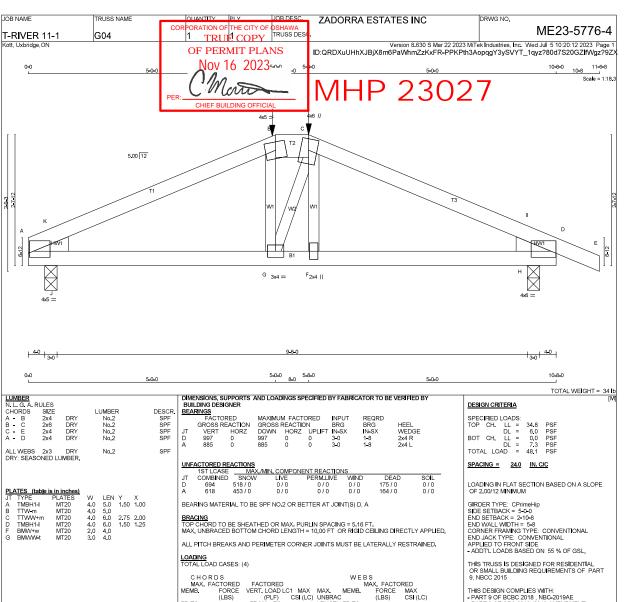
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not fuse designer. Brancing shown is for lateral support of individual web members only. Additional reportance to result in the properties of the control of the cont





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust sedesigner. Breating shown is for lateral support of individual web members only. Additional reportance to result of the control of the contr





FACTORED
VERT.LOAD LC1
MAX
MAX. MEMB.
(PLF) CSI (LC) UNISRAC
FROM TO LENGTH FR-TO
-119.4 -119.4 0.28 (1) 5.16 G-B
-119.4 -119.4 0.28 (1) 5.16 G-C
-131.9 -131.9 0.01 (1) 6.25 F-C
-119.4 -119.4 0.28 (1) 5.16 H-I
-119.4 -119.4 0.28 (1) 5.16 J-K
-119.4 -119.4 0.28 (1) 5.16 J-K FR-TO -1377 / 0 -1302 / 0 -1188 / 0 -1300 / 0 -1376 / 0 0 / 8 0 / 83 0 / 3 0 / 79 -101 / 61 -102 / 60 0.02 (4) 0.00 (4) 0.02 (4) 0.00 (1) 0.00 (1) 0 / 1191 0 / 1191 0 / 1187 0 / 1190 0 / 1190 -20.2 -20.2 0.32 (1) -20.2 -20.2 0.33 (1) -20.2 -20.2 0.30 (1) -20.2 -20.2 0.33 (1) -20.2 -20.2 0.32 (1) A- J J- G G- F F- H H- D 10.00 10.00 10.00 10.00 10.00 FACTORED CONCENTRATED LOADS (LBS)

JT LOC. LC1 MAX- MAX+ FACE DIR.

5-0-0 -137 -137 --- FRONT VERT

C 5-8-0 -137 -137 --- FRONT VERT HEEL

CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED. THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL)= L/360 (0.36")
CALCULATED VERT. DEFL (LL)= L/999 (0.03")
ALLOWABLE DEFL (TL)= L/360 (0.36")
CALCULATED VERT. DEFL (TL)= L/999 (0.05")

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

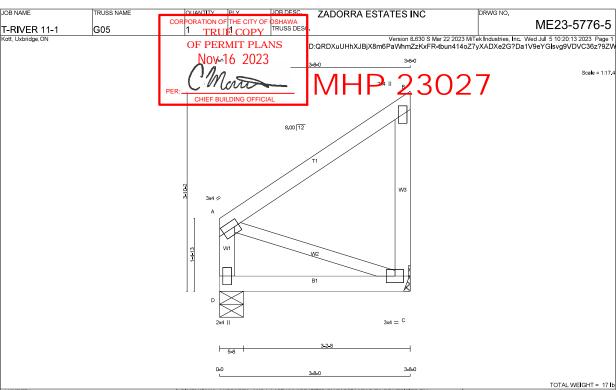
JSI GRIP= 0.86 (A) (INPUT = 0.90 ) JSI METAL= 0.33 (A) (INPUT = 1.00 )



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not fuse designer. Brancing shown is for lateral support of individual web members only. Additional reportance to result in the properties of the control of the cont





LUMBER				
N. L. G. A. F	ULES			
CHORDS	SIZE		LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
С- В	2x4	DRY	No.2	SPF
D-C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRV: SEAS	ONEDI	IMBED		

# PLATES (table is in inches)

JI	IIIFE	FLAIES	٧v	LEIN	1	^	
Α	TMVW-t	MT20	3.0	4.0	1.50	1.00	
В	TMV+p	MT20	2.0	4.0			
С	BMVW1-t	MT20	3.0	4.0			
D	RMV/1+n	MT20	2.0	4.0			

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEA	BEARINGS										
	FACTO	MAXIMUM FACTORED			INPUT	REQRD					
GROSS REACTION			GROSS REACTION			BRG	BRG				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
D	742	0	742	0	0	5-8	1-8				
С	742	0	742	0	0	MECHANIC	CAL				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

# UNFACTORED REACTIONS

	1ST LCASE	MAX./	MIN. COMPON	JENT REACTION	VS.		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	518	379 / 0	0/0	0/0	0/0	139 / 0	0/0
C	518	379 / 0	0/0	0/0	0/0	139 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 10,00 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (4)

СНО	ORDS					WE	BS		
MAX.	FACTORED	FACTORED					MAX. FACTOR	RED	
MEMB.	FORCE	VERT. LOAD	LC1 M	AX N	ΛAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CS	(LC) L	JNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		L	ENGTH	FR-TO			
D- A	-219 / 0		0.0		7.81	A-C	0/0	0.00(1)	
A− B	0/0	-119.4 -11	9.4 0.3	0 (1)	10.00				
C-B	-219 / 0	0.0	0.0	5 (1)	7.81				
D- C	0/0	285.4 -28	5.4 0.7	2(1)	10.00				

## DESIGN CRITERIA

SPECIFIED LOADS:									
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA	L LO	AD	=	48 1	PSF				

### SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 9-10-0
END DISTANCE = 3-8-0
END SPAN CARRIED = 9-10-0
END SPAN CARRIED = 9-10-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
-ADDTL LOADS BASED ON 60 % OF GSL.
(DEFINED BY USER)

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL)= L/360 (0.19")
CALCULATED VERT. DEFL (LL) = L/582 (0.08")
ALLOWABLE DEFL (TL)= L/360 (0.19")
CALCULATED VERT. DEFL (TL) = L/360 (0.14")

CSI: TC=0.30/1.00 (A-B:1), BC=0.72/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SSI=0.45/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

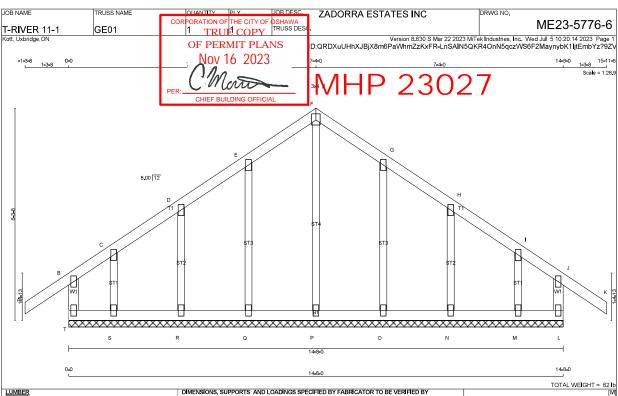
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (C) (INPUT = 0.90 ) JSI METAL= 0.11 (B) (INPUT = 1.00 )



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of lateral suppor





LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3 WEBS	DRY	No.2	SPF
/ LL O/ LDLL	2x3	DRY	No.2	SPF
DRY: SEAS	ONED L	UMBER.		

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table	is in inches)				
JT TYPE	PLATES	W	LEN	Υ	Х
B TMV+p	MT20	2.0	4.0		
C, D, E, G, H, I					
C TMW+w	MT20	2.0	4.0		
F TTW+p	MT20	3.0	4.0	2,25	1,50
J TMV+p	MT20	2.0	4.0		
L BMV1+p	MT20	2.0	4.0		
M, N, O, P, Q, R	, S				
M BMW1+w	MT20	2.0	4.0		
T BMV1+p	MT20	2.0	4.0		

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S)

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10,00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (4)

CHC	RDS					WE	BS	
MAX.	FACTORED	FACTO	RED				MAX, FACTO	RED
MEMB.	FORCE	VERT. LC	DAD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(Pi	LF) (	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM			LENGTH	FR-TO		
T-B	-266 / 0	0.0	0.0	0.05(1)	7,81	P-F	-300 / 0	0.19(1)
A− B	0 / 45			0.16(1)		Q-E	-242 / 0	0.09(1)
B-C	0/6			0.10(1)		R-D	-247 / 0	0.05(1)
C-D	0 / 44	-119.4	-119.4	0.07(1)	10,00	S-C	-134 / 0	0.02(1)
D-E	0 / 44			0.07(1)		0- G	-242 / 0	0.09(1)
E-F	0 / 49	-119.4	-119.4	0.07(1)	10.00	N-H	<b>-</b> 247 / 0	0.05(1)
F-G	0 / 49			0.07 (1)		M- I	-134 / 0	0.02(1)
G-H	0 / 44			0.07(1)				
H-I	0 / 44			0.07(1)				
I- J	0/6			0.10(1)				
J-K	0 / 45			0.16(1)				
L- J	-266 / 0	0.0	0.0	0.05 (1)	7.81			
T-S	-25 / 0	-18.2	-18.2	0.01 (1)	6.25			
S-R	31/0			0.02(4)				
R-Q	-37 / 0			0.02(4)				
Q-P	-41/0			0.01(4)				
P- 0	-41/0	18.2		0.01(4)				
O- N	-37 / 0	-18.2		0.02(4)				
N- M	-31 / 0			0.02(4)				
M- L	-25 / 0	-18.2		0.01 (1)				

## DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48 1	PSF

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (J-K:1) , BC=0.02/1.00 (N-O:4) , WB=0.19/1.00 (F-P:1) , SSI=0.11/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
(PX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0,250 inches

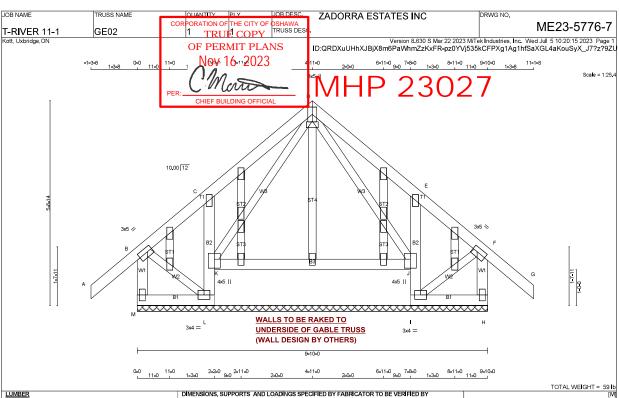
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (F) (INPUT = 0.90 ) JSI METAL= 0.13 (D) (INPUT = 1.00 )



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing shown is for lateral support of lateral suppor





LUMBER N. L. G. A. R CHORDS A - D D - G M - B H - F M - L L - C	SIZE 2x4 2x4 2x4 2x4 2x4 2x4 2x4	DRY DRY DRY DRY DRY DRY DRY	LUMBER No.2 No.2 No.2 No.2 No.2 No.2	DESCR. SPF SPF SPF SPF SPF SPF
Î - E I - H	2x4 2x4 2x4	DRY DRY	No.2 No.2 No.2	SPF SPF
ALL WEBS EXCEPT ALL GABLE	2x3 WEBS	DRY	No.2	SPF
DRY: SEASO	2x3 ONED LU	DRY JMBER.	No.2	SPF

PLATES (table is in inches)	
JT TYPE PLATES W LEN Y X	
B TMVW-t MT20 3.0 5.0 1.50 1	.75
C TMV+p MT20 2.0 4.0	
D TTWW+p MT20 3.0 5.0	
D NP-p MT20 2.0 4.0 0.25 2	.00
E TMV+p MT20 2.0 4.0	
F TMVW+ MT20 3.0 5.0 1.50 1	.75
H BMV1+p MT20 2.0 4.0	
I BMVW1-t MT20 3.0 4.0	
J BVMW+p MT20 4.0 5.0 3.00 2	.00
K BVMW+p MT20 4.0 5.0 3.00 2	.00
L BMVVV1-t MT20 3.0 4.0	
M BMV1+p MT20 2.0 4.0	
N, O, O, P, Q, R, S, T, U, V, V, W, X	
N NP+w MT20 2.0 4.0	

GABLE STUDS SPACED AT 2-0-0 OC.

# DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS MANNING FACTORED MANNING FACTORED NIGHT BEOODS

H	KINGS						
	FACTO	RED	MAXIMU	M FACT	ORED	INPUT	REQRD
	GROSS RI	EACTION	GROSS	REACTIC	N	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	329	0	329	0	0	9-10-0 (2-2-0	1-8
	513	0	513	0	0	9-10-0 (2-2-0	1-8
	329	0	329	0	0	9-10-0 (2-2-0	1-8
	513	0	513	0	0	9-10-0 (2-2-0	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

<u>UNF</u>	ACTORED RE	ACTIONS					
	1ST LCASE	MAX./	MIN. COMPON	VENT REACTION	4S		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	227	181 / 0	0/0	0/0	0/0	46 / 0	0/0
1	359	258 / 0	0/0	0/0	0/0	101 / 0	0/0
Н	227	181 / 0	0/0	0/0	0/0	46 / 0	0/0
L	359	258 / 0	0/0	0/0	0/0	101 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I, H, L

BRACINS
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6,25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CHUNG DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

	DRDS FACTORED	FACTOR	RED.			WE	B S MAX. FACTO	RED
MEMB.		VERT, LO			MAX.	MEMB.		MAX
	(LBS)	(PLI	F) (	CSI (LC)	UNBRAC	:	(LBS)	CSI (LC)
FR-TO		FROM :						
A-B	0 / 53				10.00		<b>-</b> 183 / 0	0.08(1)
B-C					6,25			0.08(1)
C-D	-16 / 0							0.00(1)
D-E	-16 / 0					I- F	0 / 10	0.00(1)
E-F	-23 / 0			0.16 (1)				
	0 / 53			0.16 (1)				
	-311 / 0							
H-F	-311 / 0	0.0	0.0	0.03 (1)	7.81			
M- L	0/0	10.0	10.7	0.02 (4)	10.00			
L-K	-500 / 0	0.0						
K C	-295 / 0			0.01 (1)				
K-J	0/99	18.2		0.18 (4)				
I- J	-500 / 0			0.18 (4)				
J-E	-295 / 0			0.01 (1)				
1- H	0/0	18.2		0.02 (4)				
15.11	070	-10.2	-10.2	0.02 (4)	10.00			

# DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48 1	PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (A-B:1) , BC=0.18/1.00 (J-K:4) , WB=0.08/1.00 (D-J:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY) (PSI)
 SHEAR
 SECTION (PLI)

 MAX
 MIN
 MAX
 MIN
 MAX
 MIN

 MT20
 650
 371
 1747
 788
 1873

PLATE PLACEMENT TOL = 0,250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (L) (INPUT = 0.90 ) JSI METAL= 0.16 (C) (INPUT = 1.00 )



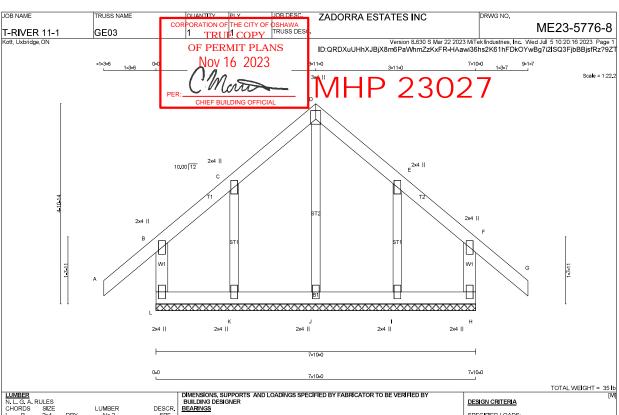
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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com





N. L. G. A. RULES CHORDS SIZE L - B 2x4 A - D 2x4 D - G 2x4 H - F 2x4 L - H 2x4 LUMBER No.2 No.2 No.2 No.2 No.2 DESCR. SPF SPF SPF SPF SPF ALL WEBS 2x3 ALL GABLE WEBS No.2 SPF 2x3 DRY DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PL	PLATES (table is in inches)								
JΤ	TYPE	PLATES	W	LEN	Υ	Х			
В	TMV+p	MT20	2.0	4.0					
С	TMW+w	MT20	2.0	4.0					
D	TTW+p	MT20	3.0	4.0	2,50	1.50			
Е	TMW+w	MT20	2.0	4.0					
F	TMV+p	MT20	2.0	4.0					
Н	BMV1+p	MT20	2.0	4.0					
I, J,	K								
1	BMW1+w	MT20	2.0	4.0					
L	BMV1+p	MT20	2.0	4.0					

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TO FLORD TO BE SHEATHED OR MAX, PURLIN SPACING = 10,00 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 6,25 FT OR RIGID CHUING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

Сно	ORDS					WE	BS	
MAX,	FACTORED	FACTORE	D				MAX, FACTO	RED
MEMB.	FORCE	VERT, LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)		CSI (LC)	<b>UNBRAG</b>	3	(LBS)	CSI (LC)
FR-TO		FROM TO			LENGTH			
L-B	-272 / 0	0.0	0,0	0.06(1)	7,81	J- D	-322 / 0	0.11(1)
A-B	0 / 53	119.4 -1	19.4	0.16(1)	10.00	K-C	-199 / 0	0.04(1)
B-C	0 / 22	119.4 -1	19.4	0.10(1)	10.00	I-E	-198 / 0	0.04(1)
C-D	0 / 50	119.4 -1	19,4	0.07(1)	10,00			
D-E	0 / 50	119.4 -1	19.4	0.07(1)	10.00			
E-F	0 / 22	-119.4 -1						
F-G	0 / 53	-119.4 -1	19.4	0.16(1)	10.00			
H-F	-273 / 0	0.0	0.0	0.06 (1)	7.81			
L-K	-28 / 0	-18.2	18.2	0.02(4)	6.25			
K-J	-35 / 0			0.02 (4)				
J-I	-35 / 0	-18.2						
I- H	-28 / 0	-18.2	18.2	0.02 (4)	6.25			

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (F-G:1) , BC=0.02/1.00 (J-K:4) , WB=0.11/1.00 (D-J:1) , SSI=0.10/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

 NAIL VALUES

 PLATE
 GRIP(DRY) (PSI)
 SHEAR
 SECTION (PLI)

 MAX
 MIN
 MAX
 MIN
 MAX
 MIN

 MT20
 650
 371
 1747
 788
 1873

PLATE PLACEMENT TOL = 0,250 inches

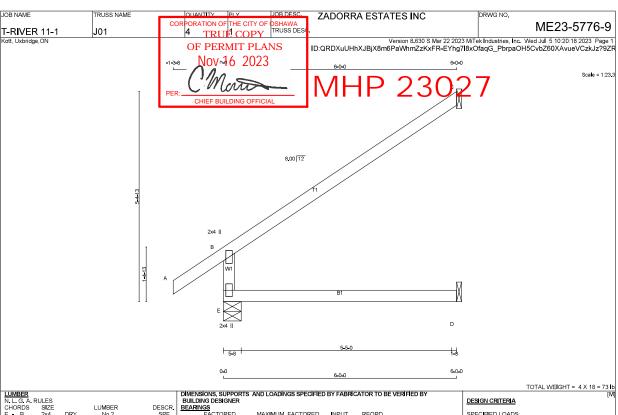
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.90 ) JSI METAL= 0.13 (B) (INPUT = 1.00 )



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY

DRY: SEASONED LUMBER.

#### PLATES (table is in inches) W LEN Y X 2.0 4.0 2.0 4.0 B TMV+p E BMV1+p

BEA	RINGS						
	FACTO	RED	MAXIMU	M FACTO	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	675	0	675	0	0	5-8	1-8
С	269	0	269	0	0	1-8	1-8
ח	46	0	51	0	Λ	1_8	1_8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

45T 10ASE MAX /MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD :	SOIL
E 469 357/0 0/0 0/0 0/0 112/0	0/0
C 184 157/0 0/0 0/0 0/0 27/0	0/0
D 37 0/0 0/0 0/0 0/0 37/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (4)

CHORDS			WEBS			
MAX.	FACTORED	FACTORED	MA	X. FACTORED		
MEMB.	FORCE	VERT, LOAD LC1 MAX	MAX. MEMB.	FORCE MAX		
	(LBS)	(PLF) CSI (LC	UNBRAC	(LBS) CSI (LC)		
FR-TO		FROM TO	LENGTH FR-TO			
E-B	-612 / 0	0.0 0.0 0.13 (4	) 7.81			
E-B A-B	0 / 45	-119.4 -119.4 0.16 (1	) 10.00			
B-C	-50 / 0	-119.4 -119.4 0.73 (1	) 6.25			
E-D	0/0	-18.2 -18.2 0.14 (4	10.00			

DESIGN CRITERIA 

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.90 ) JSI METAL= 0.31 (B) (INPUT = 1.00 )

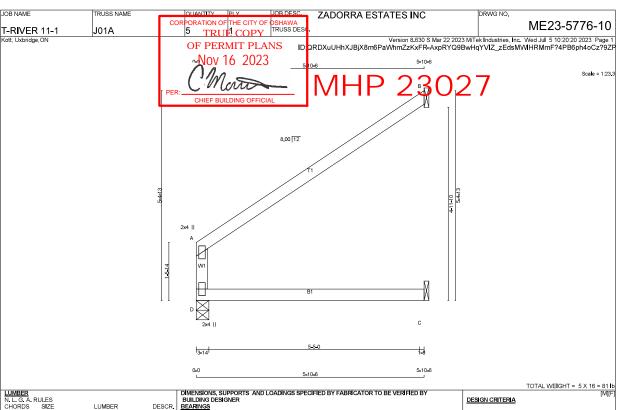


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incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of the story. Additional temporary bracing to ensure
stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com





LUMBER
N. L. G. A. RULES
CHORDS SIZE
D - A 2x4
A - B 2x4
D - C 2x4 LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY

DRY: SEASONED LUMBER.

#### PLATES (table is in inches) JT TYPE PLATES W LEN Y X 2.0 4.0 2.0 4.0 A TMV+p D BMV1+p

	41103						
	FACTORED		MAXIMU		INPUT	REQRD	
	GROSS RE	EACTION	GROSS	REACTIC	BRG	BRG	
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	404	0	404	0	0	3-14	1-8
В	317	0	317	0	0	1-8	1-8
С	86	0	86	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

	1ST LCASE	MAX_/N	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
D	282	204 / 0	0/0	0/0	0/0	78 / 0	0/0				
В	217	182 / 0	0/0	0/0	0/0	35 / 0	0/0				
С	65	22/0	0/0	0/0	0/0	43 / 0	0/0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (4)

сно	RDS					WE	BS	
MAX.	FACTORED	FACTO	RED				MAX. FACTO	RED
MEMB.	FORCE	VERT. LC	AD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)			CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
D-A A-B	-383 / 0	0.0	0.0	0.23 (1)	7.81			
A-B	-18 / 0	-119.4	-119.4	0.58 (1)	6.25			
D C	0.10	40.0		0.00 (4)				

DESIGN CRITERIA 

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

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- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/503 (0.14")

CSI: TC=0.58/1.00 (A-B:1) , BC=0.28/1.00 (C-D:1) , WB=0.00/1.00 (n/a:0) , SSI=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (A) (INPUT = 0.90 ) JSI METAL= 0.20 (A) (INPUT = 1.00 )



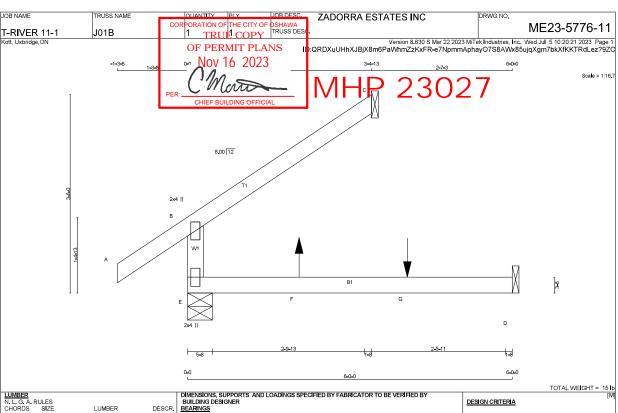
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Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com

KOTI



LUMBER				
N. L. G. A. F	RULES			
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

PL	ATES (table	is in inches)			
JT	TYPE	PLATES	W	LEN Y	Х
В	TMV+p	MT20	2.0	4.0	
Е	BMV1+p	MT20	2.0	4.0	

BEAL	NGS						
	FACTO	RED	MAXIMU	M FACTO	INPUT	REQRD	
	GROSS RI	EACTION	GROSS	REACTIC	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	478	0	478	0	0	5-8	1-8
С	152	0	152	0	0	1-8	1-8
D	46	0	53	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNF	UNFACTORED REACTIONS											
	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
E	335	240 / 0	0/0	0/0	0/0	95 / 0	0/0					
C	104	89 / 0	0/0	0/0	0/0	15 / 0	0/0					
D	37	0 / -1	0/0	0/0	0/0	38 / 0	0/0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (4)

CHO	RUS				VV E	BS		
MAX.	FACTORED	FACTORED				MAX. FACT	ORED	
мемв.	FORCE	VERT, LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (L	_C)
FR-TO		FROM TO		LENGTH				
E-B	-417 / 0	0.0 0.0	0.13(4)	7.81				
Ā-B	0 / 45							
B-C	-28 / 0	-119.4 -119.4	0.23(1)	6.25				
E-F	0/0	-18.2 -18.2	0.14(4)	10.00				
F-G	0/0	-18.2 -18.2	0.14(4)	10.00				
G-D	0/0	18.2 18.2	0,14 (4)	10,00				
FACTOR	ED CONCENT	RATED LOADS (L	BS)					
JT	LOC. LC1	MAX- MAX	+ F	ACE D	IR.	TYPE	HEEL	CONN
	-0-12 5	5 1	5 BA	CK VE	RT	TOTAL		C1
	0-12 -2		- BA	CK VE	RT	TOTAL		C1
		_						

## CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48 1	PSF

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

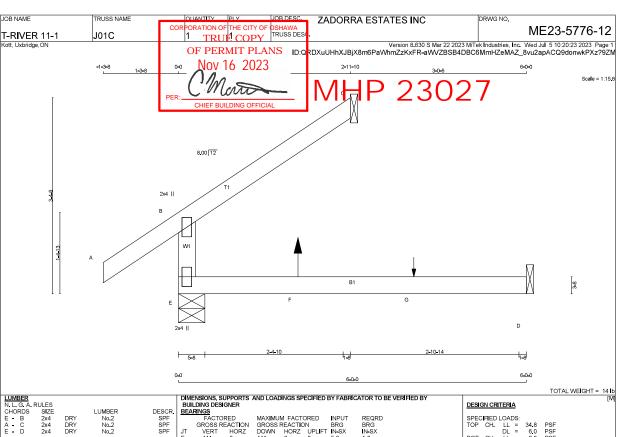
JSI GRIP= 0.26 (B) (INPUT = 0.90 ) JSI METAL= 0.21 (B) (INPUT = 1.00 )



NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

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incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of its early applicable sonly. Additional temporary bracing to ensure
stability during construction is the responsibility of the exector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY

DRY: SEASONED LUMBER.

#### PLATES (table is in inches) W LEN Y X 2.0 4.0 2.0 4.0 B TMV+p E BMV1+p

BEAR	<u>divos</u>						
	FACTOR	ED	MAXIMUN	/ FACTO	INPUT	REQRD	
	<b>GROSS RE</b>	ACTION	GROSS F	REACTIO	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	441	0	441	0	0	5-8	1-8
С	133	0	133	0	0	1-8	1-8
D	44	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNF	UNFACTORED REACTIONS											
	1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL					
E	309	219 / 0	0/0	0/0	0/0	90 / 0	0/0					
С	91	78/0	0/0	0/0	0/0	13 / 0	0/0					
D	35	0/-2	0/0	0/0	0/0	37 / 0	0/0					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (7)

CHC	RDS			WEBS						
MAX.	FACTORE	D FACTO	RED				MAX. FACT	ORED		
MEMB.	FORCE	VERT. LC	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(Pi	_F) (	CSI (LC)	UNBRAG		(LBS)	CSI (	LC)	
FR-TO		FROM	TO		LENGTH	FR-TO				
E-B	-385 / 0	0.0	0.0	0.13(4)	7.81					
A <del>-</del> B	0 / 45	-119.4	-119.4	0.16(1)	10.00					
B-C	-24 / 0	-119.4	-119.4	0.18(1)	6.25					
E-F	0/0	-18.2	-18.2	0.14(4)	10.00					
F-G	0/0	-18.2		0.14(4)						
G-D	0/0	-18.2	-18.2	0.14(4)	10.00					
		NTRATED LO								
		C1 MAX-	MAX-			DIR.	TYPE	HEEL	CONN	
		10 1	11			ERT	TOTAL		C1	
G 4	-0-12	1 1	_	<ul> <li>FR</li> </ul>	IV TNC	ERT	TOTAL		C1	

## CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

SPECIFIED LOADS:									
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA	L LO	AD	=	48.1	PSF				

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.14/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90 ) JSI METAL= 0.20 (B) (INPUT = 1.00 )

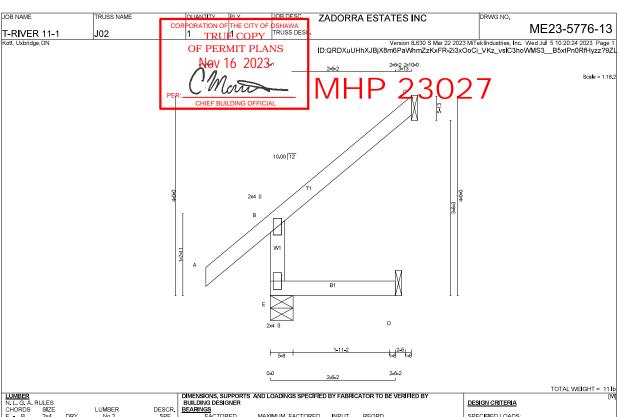


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not fuse designer. Brancing shown is for lateral support of individual web members only. Additional reportance to result in the properties of the control of the cont







LUMBER				
N. L. G. A.	RULES			
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

# 

	FACTORED			M FACT	INPUT	REQRD	
	GROSS R	EACTION	GROSS	REACTIO	BRG	BRG	
Γ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	402	0	402	0	0	5-8	1-8
	127	0	127	0	0	1-8	1-8
	21	0	23	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS							
	1ST LCASE	MAX.	/MIN. COMPO	NENT REA			
JT	COMBINED	SNOW	LIVE	PERM.			
F	277	219 / 0	0/0	0.7			

	IOI LUMOE	IVIAA./II	ALIA, COMICO	NENT REACTION	VO		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	277	219/0	0/0	0/0	0/0	58 / 0	0/0
С	87	74/0	0/0	0/0	0/0	13/0	0/0
D	17	0/0	0/0	0/0	0/0	17 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (7)

CHC	RDS	WEBS						
MAX.	FACTORED	FACTORED			- 1	MAX. FACTO	RED	
MEMB.	FORCE	VERT, LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
E-B	-376 / 0	0.0 0.0	0.01 (4)	7.81				
A-B	0 / 53	-119.4 -119.4						
B-C	<del>-</del> 27 / 0	-119.4 -119.4	0.16 (6)	6.25				
E-D	0/0	-18.2 -18.2	0.03 (4)	10.00				
E-D	0/0	-18.2 -18.2	0.03 (4)	10.00				

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

## DESIGN CRITERIA

SPECIFIED LOADS:									
OP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
OTA	L LO	AD	=	48 1	PSF				

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.03/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.13/1.00 (B-C:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90 ) JSI METAL= 0.20 (B) (INPUT = 1.00 )

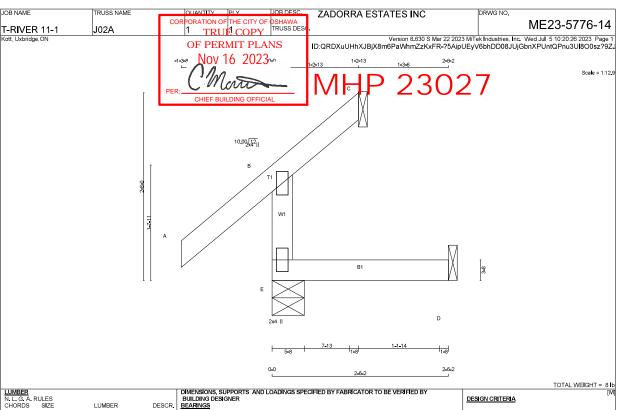


REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of its early applicable sonly. Additional temporary bracing to ensure
stability during construction is the responsibility of the exector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DRY DRY DRY DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X 2.0 4.0 2.0 4.0

BEAL	NINGS						
	FACTORED		MAXIMU	M FACTO	INPUT	REQRD	
	GROSS R	EACTION	GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	N-SX	IN-SX
E	345	0	345	0	0	5-8	1-8
С	0	0	4	0	-65	1-8	1-8
D	13	0	23	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

<u>UNF</u>	UNFACTORED REACTIONS								
	1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
E	238	189 / 0	0/0	0/0	0/0	49 / 0	0/0		
С	0	0 / -43	0/0	0/0	0/0	3/0	0/0		
D	12	0/-5	0/0	0/0	0/0	16 / 0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT, MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

	ORDS FACTORED	FACTORED			WE	BS MAX. FACTO	RED
мемв.	FORCE	VERT, LOAD LC	1 MAX	MAX. N	VEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH F	R-TO		
E-B	-313 / 0	0.0 0.0	0.04 (5)	7.81			
A-B	0 / 53	119.4 119.4	0.16(1)	10.00			
B-C	<del>-4</del> 7 / 0	-119.4 -119.4	0.12(1)	6.25			
E- D	0/0	-18.2 -18.2	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90 ) JSI METAL= 0.17 (B) (INPUT = 1.00 )



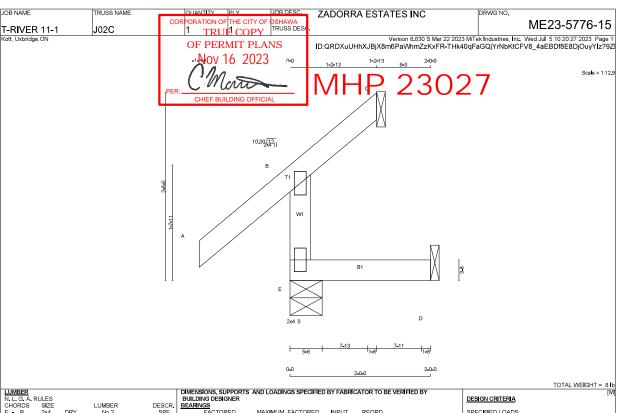
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TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com

KOT'



LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DRY DRY DRY

DRY: SEASONED LUMBER.

PL	ATES (table	e is in inches)				
JΤ	TYPE	PLATES	W	LEN	Υ	X
В	TMV+p	MT20	2.0	4.0		
	DMN/1+n	MTOD	2.0	4.0		

2	NINGS						
	FACTORED GROSS REACTION			M FACTO	INPUT BRG	REQRD BRG	
Γ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	343	0	343	0	0	5-8	1-8
	0	0	2	0	-65	1-8	1-8
	7	0	18	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNF	UNFACTORED REACTIONS										
	1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL				
E	236	190 / 0	0/0	0/0	0/0	47 / 0	0/0				
C	0	0 / -42	0/0	0/0	0/0	1/0	0/0				
D	7	0/-7	0/0	0/0	0/0	13/0	0/0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT, MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

	CHORDS MAX. FACTORED FACTORED				WEBS			
IVIAA.		FACTOR	KEU				MAX. FACTO	KEU
MEMB.	FORCE	VERT, LO.	AD LC1	MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PL	.F) (	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
E-B	-313 / 0	0.0	0.0	0.04 (5)	7.81			
A-B	0 / 53	119.4	-119.4	0.16(1)	10.00			
B-C	-48/0	119.4	-119.4	0.12(1)	6.25			
E- D	0/0	-18.2	-18.2	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:									
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA	L LO	AD	=	48 1	PSF				

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90 ) JSI METAL= 0.17 (B) (INPUT = 1.00 )



REVIEW FOR TRUSS COMPONENT ONLY

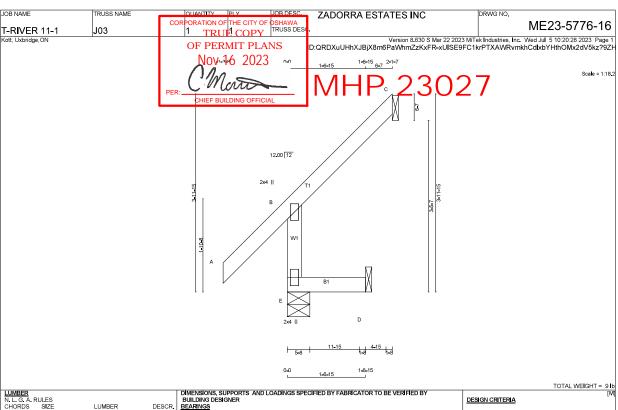
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Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com

KOT'



LUMBER
N. L. G. A. RULES
CHORDS SIZE
E - B 2x4
A - C 2x4
E - D 2x4 LUMBER No.2 No.2 No.2 DESCR. SPF SPF SPF DRY DRY DRY DRY: SEASONED LUMBER.

#### PLATES (table is in inches) W LEN Y X 2.0 4.0 2.0 4.0 B TMV+p E BMV1+p

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIO	N	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Ε	339	0	339	0	0	5-8	1-8
С	96	0	96	0	0	1-8	1-8
D	13	0	15	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	151 LUASE	IVIAA./I	VIIN. COMPO	NEINT REACTION	VS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	234	189 / 0	0/0	0/0	0/0	45 / 0	0/0
C	65	56 / 0	0/0	0/0	0/0	10 / 0	0/0
D	11	0/0	0/0	0/0	0/0	11/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (5)

CH	ORDS				WE	BS	
MAX	. FACTORED	FACTORED				MAX. FACTO	RED
MEMB.	FORCE	VERT, LOAD LC	1 MAX	MAX. N	ΛΕΜΒ.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH F	R-TO		
E-B	-324 / 0	0.0 0.0	0.00(4)	7.81			
A-B	0 / 59	-119.4 -119.4	0.17(1)	10.00			
B-C	-22 / 0	119.4 -119.4	0.09(1)	6.25			
E-D	0.40	40.0 40.0	0.04 (4)	40.00			
E-D	0/0	-18.2 -18.2	0.01 (4)	10.00			

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

## DESIGN CRITERIA

PEC	IFIED	LOAI	os:		
OP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
OT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
OTA	L LO	AD	=	48 1	PSF

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.01/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90 ) JSI METAL= 0.17 (B) (INPUT = 1.00 )



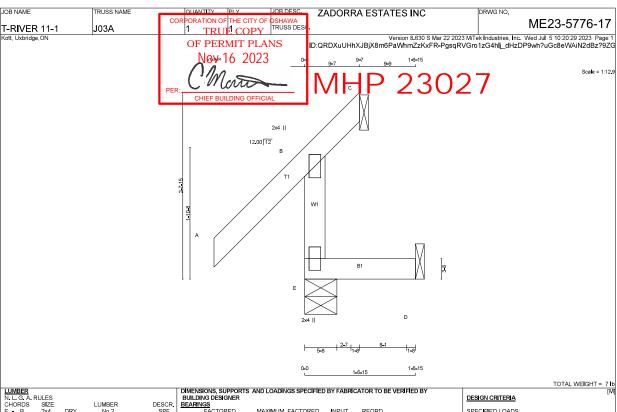
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Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com





LUMBER				
N. L. G. A. I	RULES			
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

# 

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RI	EACTION	GROSS	REACTIC	N	BRG	BRG
JΤ	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	363	0	363	0	0	5-8	1-8
2	<b>-</b> 76	0	0	0	-118	1-8	1-8
)	2	0	14	0	-3	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

# PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./I	MIN. COMPOR	VENT REACTION	VS.		
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	250	204 / 0	0/0	0/0	0/0	46 / 0	0/0
l c	<b>-</b> 52	0/-72	0/0	0/0	0/0	0 / -7	0/0
D	3	0/-7	0/0	0/0	0/0	10 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACINS
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

	R D S FACTORED	FACTORED			WE	BS MAX. FACTO	RED
MEMB.	FORCE	VERT, LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-336 / 0	0.0 0.0	0.03 (5)	7.81			
A− B	0 / 59	119.4 -119.4	0.17(1)	10.00			
B-C	-87 / 0	119.4 -119.4	0.13(1)	6.25			
E- D	0/0	-18.2 -18.2	0.03 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

## DESIGN CRITERIA

SPECIFIED LOADS:									
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
зот	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA	L LO	AD	=	48 1	PSF				

#### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

# DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.03/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

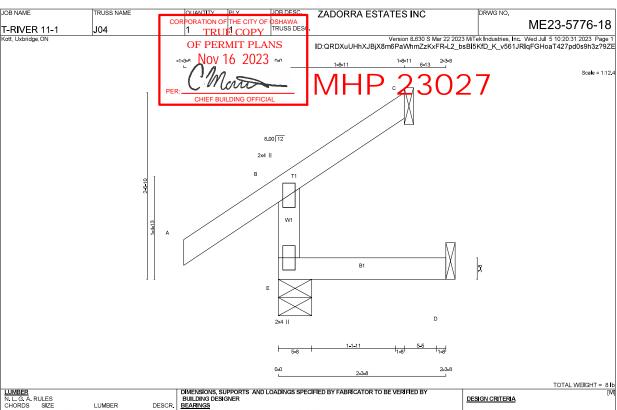
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90 ) JSI METAL= 0.18 (B) (INPUT = 1.00 )



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stability during construction is the responsibility of the exector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER				
N. L. G. A.	RULES			
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
Ā - Č	2x4	DRY	No.2	SPF
E D	2x4	DRY	No.2	SPF

PL	ATES (table	is in inches)			
JT	TYPE	PLATES	W	LEN Y	×
В	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

BEAR	divido						
	FACTO	RED	MAXIMU	M FACTO	ORED	INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIC	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	350	0	350	0	0	5-8	1-8
С	51	0	51	0	-37	1-8	1-8
D	11	0	20	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

<u>UNF</u>	UNFACTORED REACTIONS									
	1ST LCASE	MAX./I	MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL			
E	241	192 / 0	0/0	0/0	0/0	49 / 0	0/0			
С	35	28 / <del>-</del> 28	0/0	0/0	0/0	7/0	0/0			
D	10	0/-8	0/0	0/0	0/0	15 / 0	0/0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING TOTAL LOAD CASES: (5)

	ORDS		WEBS				
MAX	. FACTORED	FACTORED	)			MAX. FACTO	RED
MEMB.	FORCE	VERT, LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-319 / 0	0.0	0.0 0.04 (5)	7.81			
A-B	0 / 45	119.4 -119	9.4 0.16(1)	10,00			
B-C	<del>-</del> 29 / 0	-119.4 -11	9.4 0.12 (1)	6.25			
E-D	0/0	-18.2 -1	8.2 0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

## DESIGN CRITERIA

SPEC	IFIED	LOAI	DS:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	L LO	AD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90 ) JSI METAL= 0.16 (B) (INPUT = 1.00 )

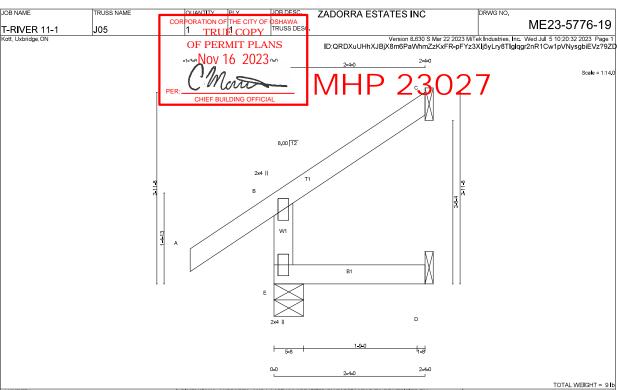


REVIEW FOR TRUSS COMPONENT ONLY

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stability during construction is the responsibility of the exector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding





LUMBER				
N. L. G. A. I	RULES			
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

# 

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS R	EACTION	GROSS	REACTIC	N	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	361	0	361	0	0	5-8	1-8
С	105	0	105	0	0	1-8	1-8
D	19	0	21	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNF	UNFACTORED REACTIONS									
	1ST LCASEMAX./MIN. COMPONENT REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
E	249	197 / 0	0/0	0/0	0/0	53 / 0	0/0			
C	72	61/0	0/0	0/0	0/0	11/0	0/0			
D	15	0/0	0/0	0/0	0/0	15/0	0/0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (7)

СН	ORDS		WEBS				
MAX	. FACTORED	FACTORED				MAX. FACTO	DRED
MEMB.	FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC	;	(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
E-B	-338 / 0	0.0 0.0	0.01(4)	7.81			
A-B	0 / 45	-119.4 -119.4	0.16(1)	10.00			
B-C	-19 / 0	119.4 -119.4	0.11 (6)	6.25			
E-D	0/0	-18.2 -18.2	0.02(4)	10.00			

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

#### DESIGN CRITERIA

SPEC	IFIED	LOAI	os:		
TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTA	J IO	AD	=	48 1	PSF

### SPACING = 24.0 IN. C/C

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(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.11/1.00 (B-C:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

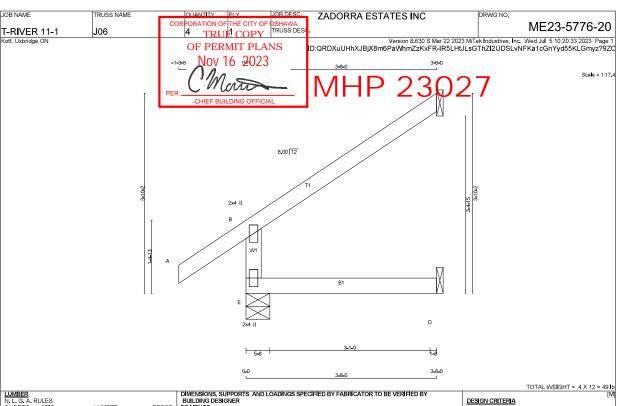
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90 ) JSI METAL= 0.17 (B) (INPUT = 1.00 )



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LUMBER				
N. L. G. A. I	RULES			
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
Ā - Č	2x4	DRY	No.2	SPF
F D	2x4	DRY	No.2	SPF
1				0

# 

RINGS						
FACTORED		MAXIMUM FACTORED			INPUT	REQRD
GROSS R	GROSS REACTION			BRG	BRG	
VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
475	0	475	0	0	5-8	1-8
164	0	164	0	0	1-8	1-8
29	0	32	0	0	1-8	1-8
	FACTO GROSS R VERT 475 164	FACTORED GROSS REACTION VERT HORZ 475 0 164 0	FACTORED MAXIMU GROSS REACTION GROSS VERT HORZ DOWN 475 0 475 164 0 164	FACTORED MAXIMUM FACTOR GROSS REACTION GROSS REACTION VERT HORZ DOWN HORZ 475 0 164 0 164 0	FACTORED	FACTORED   MAXIMUM FACTORED   INPUT GROSS REACTION   GROSS REACTION   BRG VERT   HORZ   DOWN   HORZ   UPLIFT IN-SX 475   0   475   0   5-8   164   0   164   0   0   1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UN	UNFACTORED REACTIONS								
1	1ST LCASE	MAX./	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
E	329	255 / 0	0/0	0/0	0/0	74 / 0	0/0		
l c	112	96 / 0	0/0	0/0	0/0	17 / 0	0/0		
D	23	0/0	0/0	0/0	0/0	23 / 0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (7)

	RDS				WE			
MAX.	FACTORED	FACTORED				MAX. FACTO	RED	
MEMB.	FORCE	VERT, LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
E-B	-437 / 0		0.04(4)					
A-B	0 / 45	-119.4 -119.4	0.17 (5)	10.00				
B-C	-30 / 0	-119.4 -119.4	0.27 (6)	6.25				
E-D	0/0	-18.2 -18.2	0.05 (4)	10.00				

# CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA							
SPEC	IFIED	LOA	os:				
TOP	CH.	LL	=	34.8	PSF		
		DL	=	6.0	PSF		
BOT	CH.	LL	=	0.0	PSF		
		DL	=	7.3	PSF		
TOTA	1.10	AD	=	48 1	PSF		

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.27/1.00 (B-C:6) , BC=0.05/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.18/1.00 (B-C:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90 ) JSI METAL= 0.22 (B) (INPUT = 1.00 )

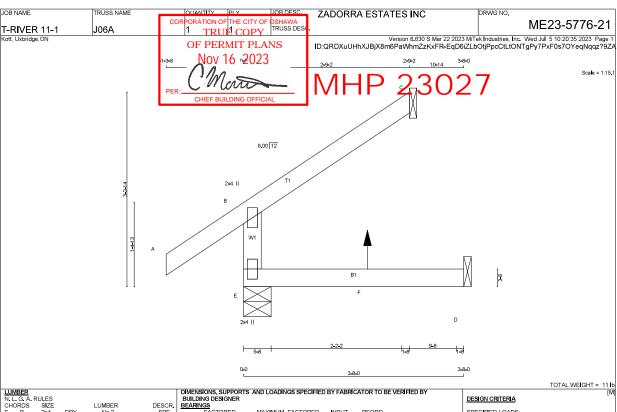


REVIEW FOR TRUSS COMPONENT ONLY NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of lateral support of its early applicable sonly. Additional temporary bracing to ensure
stability during construction is the responsibility of the exector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

Street, Suite 312, Alexandric, V. 92-2314 or www.sbcindustry.com



LUMBER				
N. L. G. A. I				
CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

# 

3EA	RINGS						
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS RE	ACTION	GROSS REACTION			BRG	BRG
ΙT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Ε	403	0	403	0	0	5-8	1-8
2	124	0	124	0	0	1-8	1-8
)	25	0	32	0	n	1_8	1_8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS							
	1ST LCASE	MAX./	MIN. COMPON	IENT REACTION	NS.		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DI	
E	280	212 / 0	0/0	0/0	0/0	67	
c	85	72/0	0/0	0/0	0/0	12	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX, PURLIN SPACING = 6,25 FT.
MAX, UNBRACED BOTTOM CHORD LENGTH = 10,00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

# LOADING TOTAL LOAD CASES: (13)

CHORDS				WEBS			
l N	MAX. FACTORED	FACTORED				MAX. FACTO	RED
MEM	B. FORCE	VERT. LOAD LC	1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)		CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-T		FROM TO		LENGTH	FR-TO		
E-B		0.0 0.0	0.04(4)	7.81			
A-B	0 / 45	119.4 -119.4	0.17 (7)	10.00			
B-C	-23 / 0	119.4 -119.4	0.15 (6)	6.25			
E-F		-18.2 -18.2					
F-D	0/0	-18.2 -18.2	0.05 (4)	10.00			
1							

FACTORED CONCENTRATED LOADS (LBS)
JT LOC. LC1 MAX- MAX+ FACE DIR.
F 2-0-12 9 1 11 FRONT VERT HEEL CONN. -- C1

## CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

# DESIGN CRITERIA

SPECIFIED LOADS:									
TOP	CH.	LL	=	34.8	PSF				
		DL	=	6.0	PSF				
BOT	CH.	LL	=	0.0	PSF				
		DL	=	7.3	PSF				
TOTA		۸D	_	10 1	DOE				

### SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-114
- TPIC 2014

SOIL 0/0 0/0 0/0

DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

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CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.17/1.00 (A-B:7) , BC=0.05/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.13/1.00 (B-C:6)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

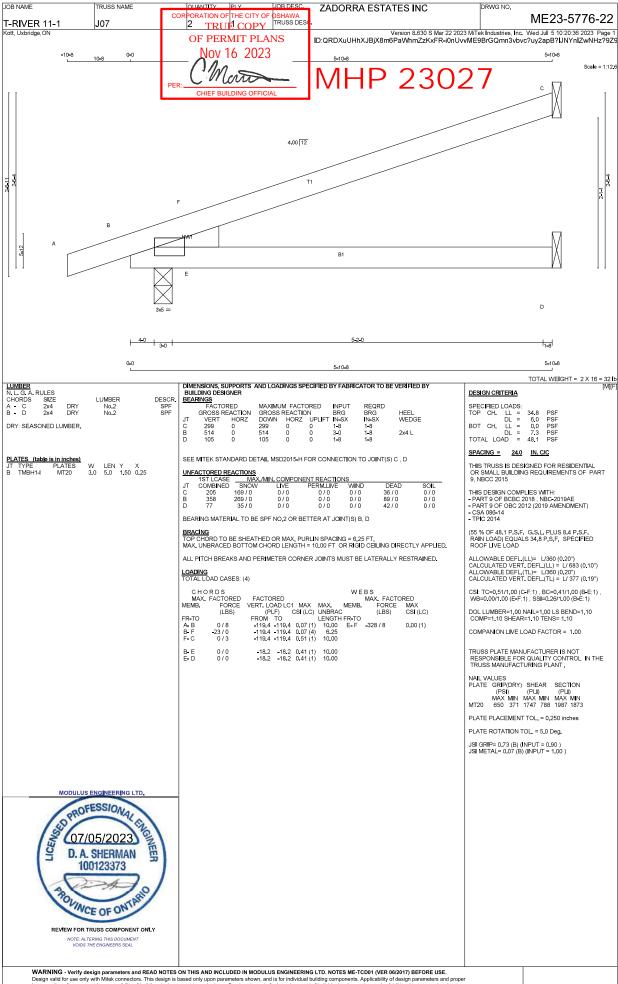
JSI GRIP= 0.23 (B) (INPUT = 0.90 ) JSI METAL= 0.19 (B) (INPUT = 1.00 )



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.
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stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer - for general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper
incorporation of component is responsibility of building designer - not fuse designer. Brancing shown is for lateral support of individual web members only. Additional reportance to result in the properties of the control of the cont labrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpic.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee

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