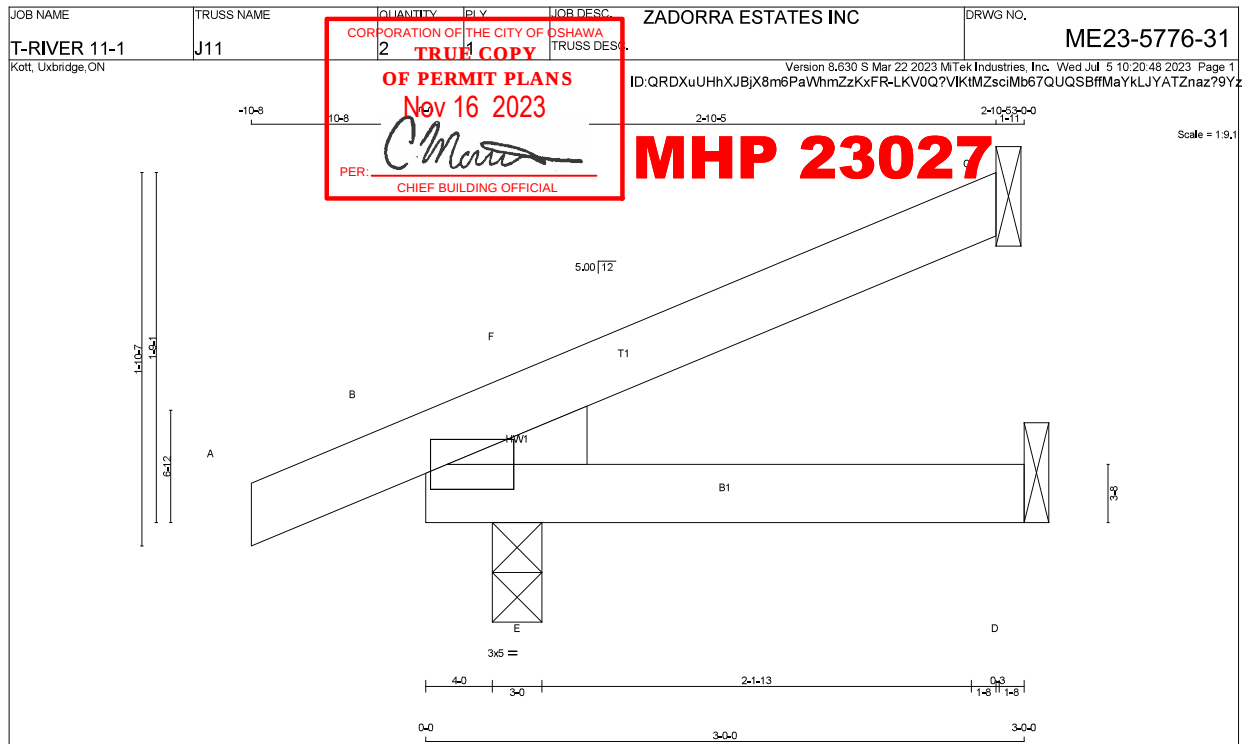




LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C2x4

DRY

No.2

LUMBER

DESCR.

SPF

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

B TMBH14

MT20

3.0

5.0

1.50

1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

HEEL

WEDGE

JT

C

B

D

144

0

144

0

0

1-8

1-8

2x4 L

311

0

311

0

0

3-0

1-8

53

0

53

0

0

1-8

1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

JT

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

C

B

D

99

215

39

81/0

165/0

18/0

0/0

0/0

0/0

0/0

0/0

0/0

18/0

50/0

21/0

0/0

0/0

0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MAX. FACTORED

MEMB.

FORCE (LBS)

FACTORED

VERT. LOAD (PLF)

LC1

MAX. UNBRAC

FORCE (LBS)

MEMB.

FORCE (LBS)

WEBS

MAX. FACTORED

MAX. CSI (LC)

FR-TO

A-B

B-F

F-C

B-E

E-D

0/8

-23/0

0/4

0/0

0/0

FROM TO

-119.4 -119.4

-119.4 -119.4

-119.4 -119.4

-18.2 -18.2

-18.2 -18.2

0.08 (7)

0.03 (4)

0.12 (1)

0.10 (1)

0.10 (1)

10.00

6.25

10.00

FR-TO

E-F

-126/6

0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.

LL

=

34.8

PSF

DL

=

6.0

PSF

BOT CH.

LL

=

0.0

PSF

DL

=

7.3

PSF

TOTAL LOAD

=

48.1

PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/999 (0.01")

ALLOWABLE DEFL.(TL)= L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI TC=0.12/1.00 (C-F:1), BC=0.10/1.00 (B-E:1), WB=0.00/1.00 (E-F:1), SS=0.10/1.00 (C-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)

MAX

MIN

MAX

MIN

MAX

MIN

MT20

650

371

1747

788

1987

1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)

JSI METAL= 0.04 (B) (INPUT = 1.00)

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07/05/2023

D. A. SHERMAN

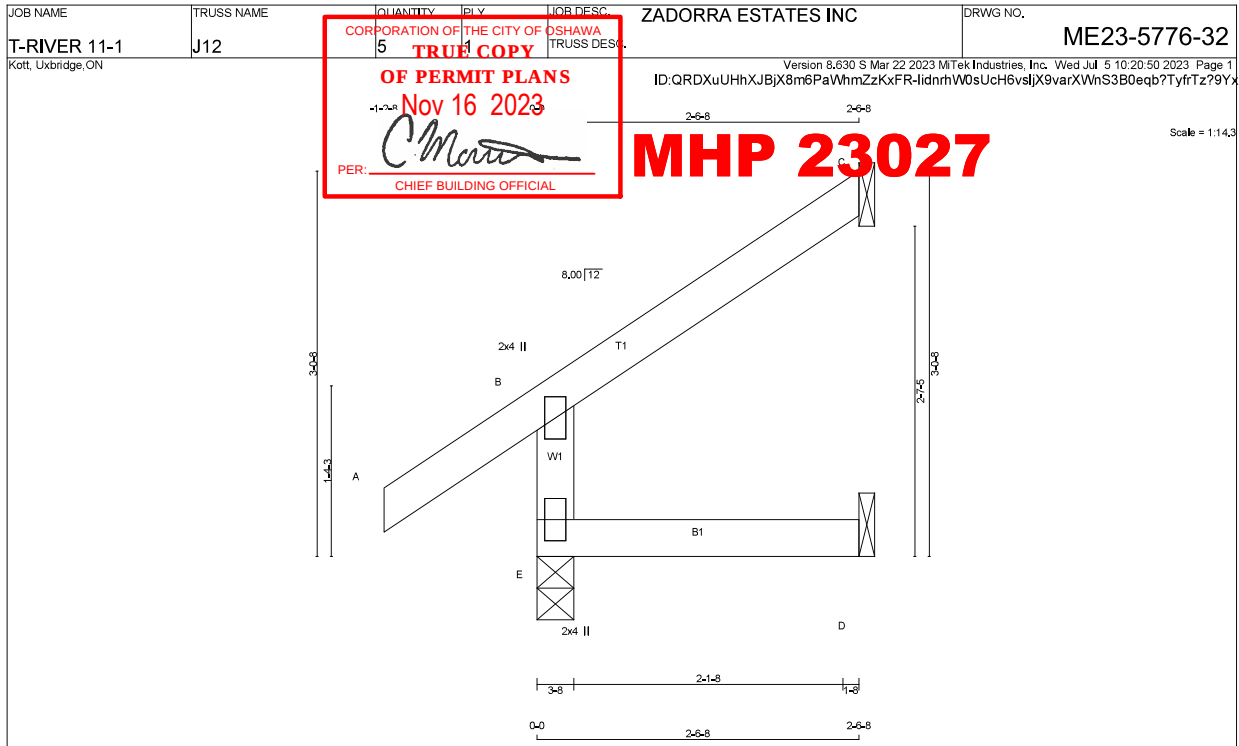
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PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT

VOIDS THE ENGINEERS SEAL



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER E - B 2x4 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY: SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table><tr><th>JT</th><th>VERT</th><th>HORZ</th><th>DOWN</th><th>HORZ</th><th>UPLIFT</th><th>IN-SX</th><th>REQRD</th></tr><tr><td>E</td><td>369</td><td>0</td><td>369</td><td>0</td><td>0</td><td>3-8</td><td>1-8</td></tr><tr><td>C</td><td>114</td><td>0</td><td>114</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td></tr><tr><td>D</td><td>21</td><td>0</td><td>23</td><td>0</td><td>0</td><td>1-8</td><td>1-8</td></tr></table> SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D UNFACTORED REACTIONS <table><tr><th>JT</th><th>1ST LCASE</th><th>FACTORED</th><th>MAX./MIN.</th><th>COMPONENT REACTIONS</th></tr><tr><td>JT</td><td>COMBINED</td><td>SNOW</td><td>LIVE</td><td>PERM.LIVE</td></tr><tr><td>E</td><td>255</td><td>200 / 0</td><td>0 / 0</td><td>0 / 0</td></tr><tr><td>C</td><td>78</td><td>67 / 0</td><td>0 / 0</td><td>0 / 0</td></tr><tr><td>D</td><td>16</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td></tr></table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (7) <table><tr><th colspan="2">CHORDS</th><th colspan="2">WEBS</th></tr><tr><th>MEMB.</th><th>FACTORED</th><th>FACTORED</th><th>MEMB.</th></tr><tr><td></td><td>FORCE</td><td>VERT. LOAD</td><td>FORCE</td></tr><tr><td></td><td>(LBS)</td><td>(PLF)</td><td>(LBS)</td></tr><tr><td>FR-TO</td><td></td><td>FROM TO</td><td>UNBRACED LENGTH FR-TO</td></tr><tr><td>E-B</td><td>-343 / 0</td><td>0.0 0.0 0.02 (4)</td><td>7.81</td></tr><tr><td>A-B</td><td>0 / 43</td><td>-119.4 -119.4 0.14 (1)</td><td>10.00</td></tr><tr><td>B-C</td><td>-21 / 0</td><td>-119.4 -119.4 0.13 (6)</td><td>6.25</td></tr><tr><td>E-D</td><td>0 / 0</td><td>-18.2 -18.2 0.03 (4)</td><td>10.00</td></tr></table> CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.		JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REQRD	E	369	0	369	0	0	3-8	1-8	C	114	0	114	0	0	1-8	1-8	D	21	0	23	0	0	1-8	1-8	JT	1ST LCASE	FACTORED	MAX./MIN.	COMPONENT REACTIONS	JT	COMBINED	SNOW	LIVE	PERM.LIVE	E	255	200 / 0	0 / 0	0 / 0	C	78	67 / 0	0 / 0	0 / 0	D	16	0 / 0	0 / 0	0 / 0	CHORDS		WEBS		MEMB.	FACTORED	FACTORED	MEMB.		FORCE	VERT. LOAD	FORCE		(LBS)	(PLF)	(LBS)	FR-TO		FROM TO	UNBRACED LENGTH FR-TO	E-B	-343 / 0	0.0 0.0 0.02 (4)	7.81	A-B	0 / 43	-119.4 -119.4 0.14 (1)	10.00	B-C	-21 / 0	-119.4 -119.4 0.13 (6)	6.25	E-D	0 / 0	-18.2 -18.2 0.03 (4)	10.00	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. G/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/999 (0.00") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/999 (0.00") CSI TC=0.14/1.00 (A-B 1), BC=0.03/1.00 (D-E 4), WB=0.00/1.00 (n/a 0), SSI=0.12/1.00 (B-C 6) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.21 (B) (INPUT = 0.90) JSI METAL= 0.18 (B) (INPUT = 1.00)
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REQRD																																																																																										
E	369	0	369	0	0	3-8	1-8																																																																																										
C	114	0	114	0	0	1-8	1-8																																																																																										
D	21	0	23	0	0	1-8	1-8																																																																																										
JT	1ST LCASE	FACTORED	MAX./MIN.	COMPONENT REACTIONS																																																																																													
JT	COMBINED	SNOW	LIVE	PERM.LIVE																																																																																													
E	255	200 / 0	0 / 0	0 / 0																																																																																													
C	78	67 / 0	0 / 0	0 / 0																																																																																													
D	16	0 / 0	0 / 0	0 / 0																																																																																													
CHORDS		WEBS																																																																																															
MEMB.	FACTORED	FACTORED	MEMB.																																																																																														
	FORCE	VERT. LOAD	FORCE																																																																																														
	(LBS)	(PLF)	(LBS)																																																																																														
FR-TO		FROM TO	UNBRACED LENGTH FR-TO																																																																																														
E-B	-343 / 0	0.0 0.0 0.02 (4)	7.81																																																																																														
A-B	0 / 43	-119.4 -119.4 0.14 (1)	10.00																																																																																														
B-C	-21 / 0	-119.4 -119.4 0.13 (6)	6.25																																																																																														
E-D	0 / 0	-18.2 -18.2 0.03 (4)	10.00																																																																																														

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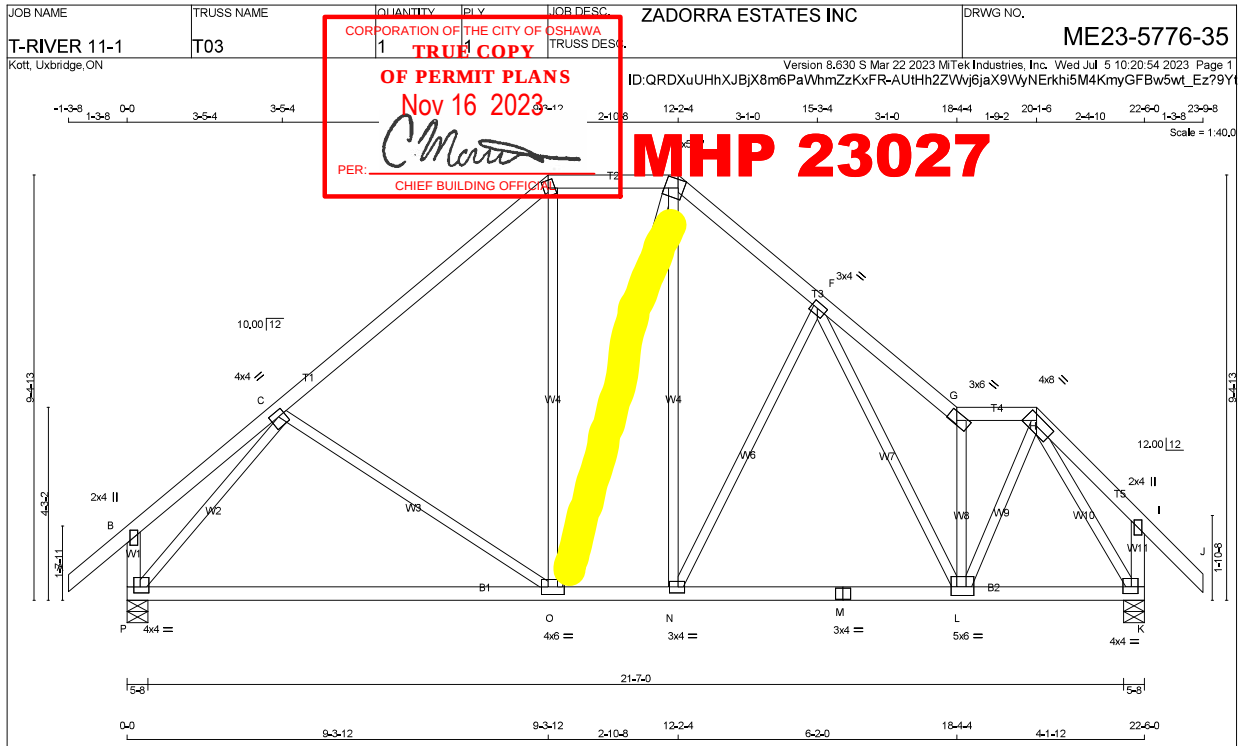
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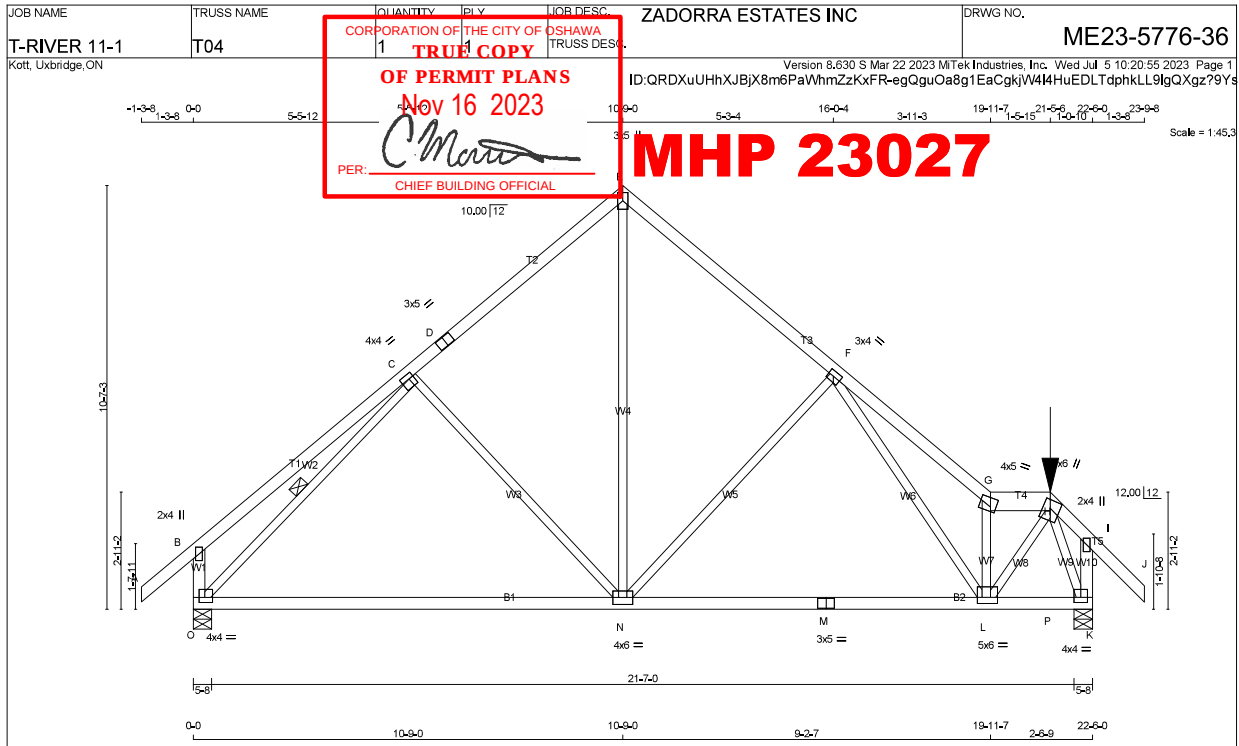
LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - D 2x4 DRY No.2 D - E 2x4 DRY No.2 E - G 2x4 DRY No.2 G - H 2x6 DRY No.2 H - J 2x4 DRY No.2 O - B 2x4 DRY No.2 O - I 2x4 DRY No.2 O - M 2x4 DRY No.2 M - K 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 EXCEPT DRY: SEASONED LUMBER.										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS JT GROSS REACTION VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX K O 1709 0 1709 0 0 5-8 1-14 1731 0 1731 0 0 5-8 1-14 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL O K 1192 877 / 0 0 / 0 0 / 0 0 / 0 315 / 0 0 / 0 1207 890 / 0 0 / 0 0 / 0 0 / 0 317 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.30 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 (PLF) MAX. CSI (LC) MAX. UNBRAC LENGTH FR-TO MEMB. FORCE (LBS) MAX. FACTORED FORCE MAX. CSI (LC) FR-TO FROM TO A-B 0 / 53 -119.4 -119.4 0.18 (1) 10.00 C-N -399 / 0 0.52 (1) B-C 0 / 50 -119.4 -119.4 0.63 (1) 10.00 N-E 0 / 940 0.23 (1) C-D -1172 / 0 -119.4 -119.4 0.73 (1) 4.71 N-F -577 / 0 0.75 (1) D-E -1172 / 0 -119.4 -119.4 0.73 (1) 4.71 F-L 0 / 160 0.04 (4) E-F -1163 / 0 -119.4 -119.4 0.58 (1) 5.06 L-G -1351 / 0 0.26 (1) F-G -1689 / 0 -119.4 -119.4 0.58 (1) 4.30 L-H 0 / 1554 0.38 (1) G-H -1390 / 0 -115.7 -115.7 0.03 (1) 6.25 O-C -1669 / 0 0.71 (1) H-I -78 / 0 -119.4 -119.4 0.17 (1) 6.25 H-K -1495 / 0 0.28 (1) I-J 0 / 59 -119.4 -119.4 0.18 (1) 10.00 O-B -407 / 0 0.0 0.0 0.05 (1) 7.81 K-I -340 / 0 0.0 0.0 0.04 (1) 7.81 O-N 0 / 1136 -18.2 -18.2 0.63 (4) 10.00 N-M 0 / 1248 -18.2 -18.2 0.64 (4) 10.00 M-L 0 / 1248 -18.2 -18.2 0.64 (4) 10.00 L-P 0 / 568 -17.7 -17.7 0.20 (4) 10.00 P-K 0 / 568 -17.7 -17.7 0.20 (4) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC LC1 MAX+ MAX+ FACE DIR. TYPE HEEL CONN. H 21-56 -17 -17 - FRONT VERT TOTAL - C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.										DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 48.1 PSF SPACING = 24.0 IN. C/C LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM GIRDER TYPE: CPrimeHp LEFT SETBACK = 10'-0" RIGHT SETBACK = 1'-0" END SETBACK = 2'-0" END WALL WIDTH = 5'-8" CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON 55 % OF GSL. LOADS APPLIED TO FIRST 2'-6" OF SPAN MEASURED FROM THE RIGHT. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC2018 - NBC-2019AE - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.75") CALCULATED VERT. DEFL.(LL) = L / 999 (0.06") ALLOWABLE DEFL.(TL)= L/360 (0.75") CALCULATED VERT. DEFL.(TL) = L / 760 (0.36") CSI TC=0.73/1.00 (C-E 1), BC=0.64/1.00 (L-N 4), WB=0.75/1.00 (F-N 1), SS=0.26/1.00 (C-E 1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE LEFT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (G) (INPUT = 0.90) JSI METAL= 0.56 (C) (INPUT = 1.00)									
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07/05/2023

D. A. SHERMAN

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PROVINCE OF ONTARIO

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
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Kott Uxbridge ON



TOTAL WEIGHT = 94.11

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC2018 NBC-2019/AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

((5% OF 48.1 P.S.F., G.S.L PLUS 8.4 P.S.F., RAIN LOAD) EQUALS 34.8 P.S.F., SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= U/360 (0.72")
CALCULATED VERT. DEFL.(LL) = U/999 (0.055")
ALLOWABLE DEFL.(TL)= U/360 (0.72")
CALCULATED VERT. DEFL.(TL) = U/873 (0.30")

CSL TC=0.57/1.00 (F-G-1), BC=0.68/1.00 (H-J-4)
WB=0.65/1.00 (F+H-I), SC=0.24/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI MEAL= 0.53 (C) (INPUT = 1.00)

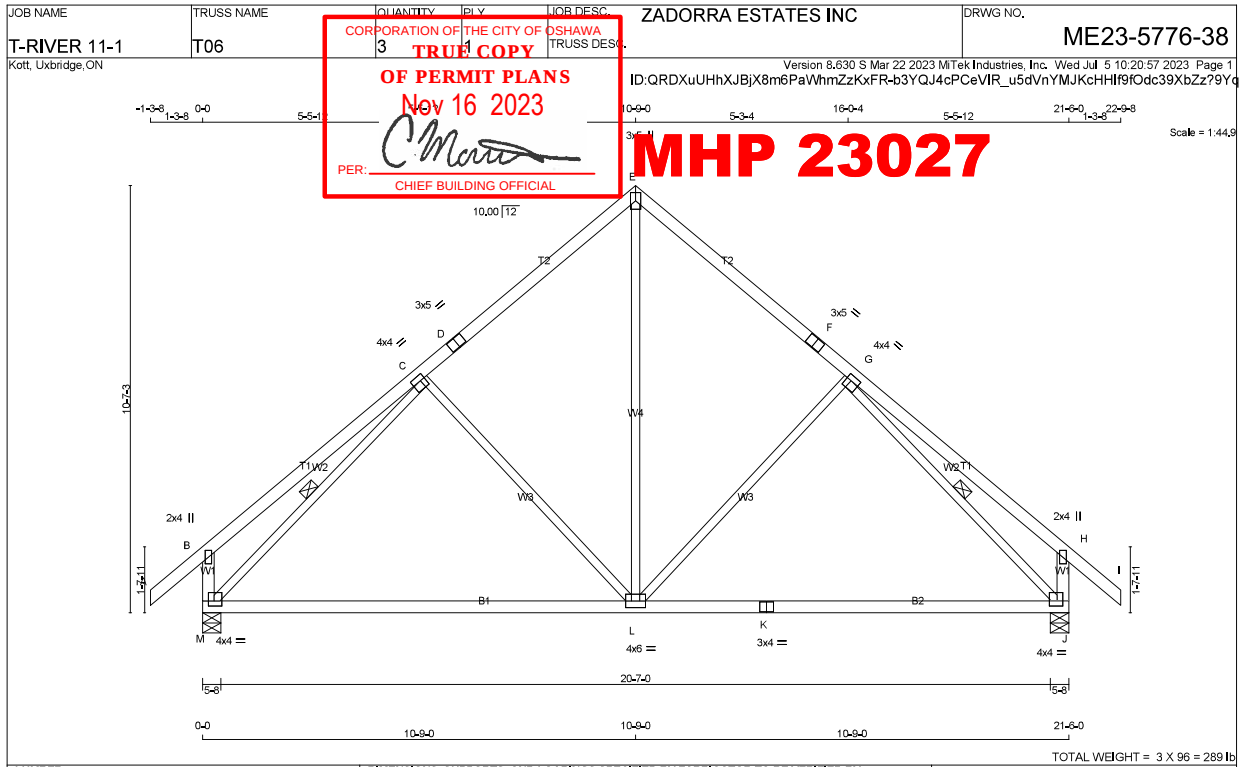
REVIEW FOR TRUSS COMPONENT ONLY

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TC001 (VER 06/2017) BEFORE USE.

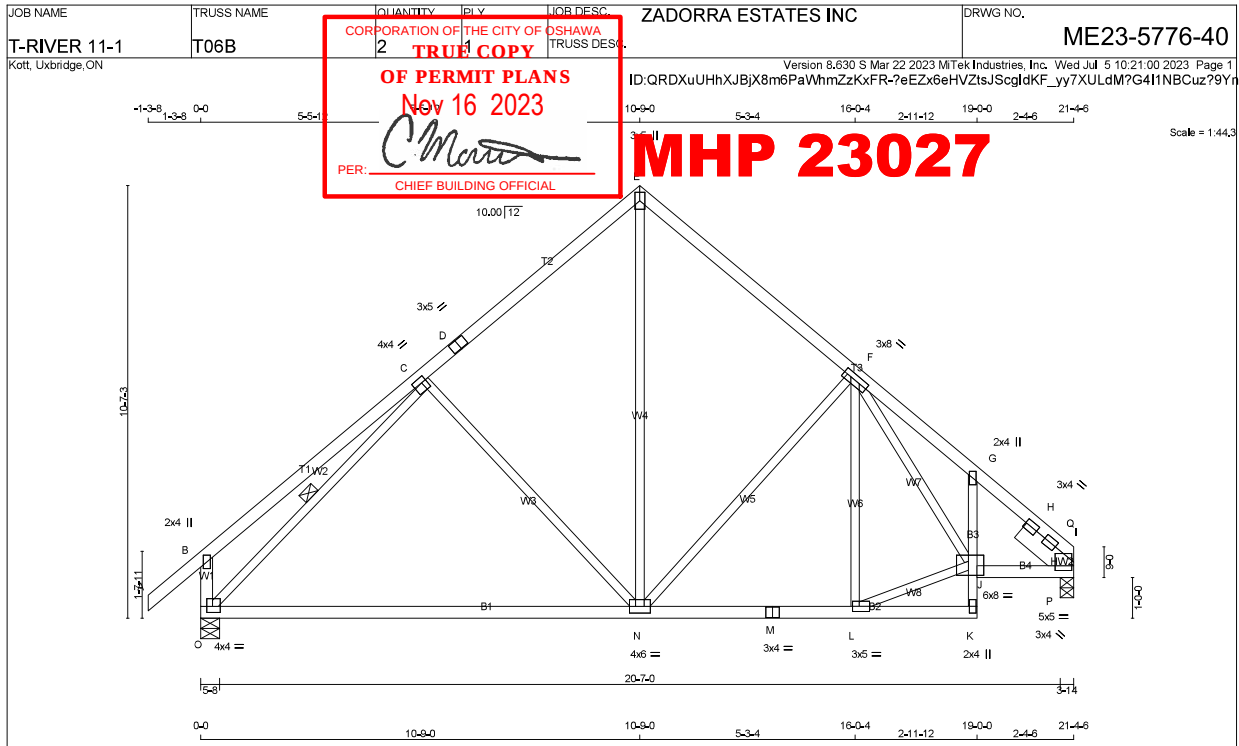
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.com and **BCSI-CANADA (Building Component Safety Information)** available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindstry.com





LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
A - D 2x4 DRY No.2 SPF				BEARINGS				SPECIFIED LOADS:			
D - E 2x4 DRY No.2 SPF				FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD				TOP CH. LL = 34.8 PSF			
E - F 2x4 DRY No.2 SPF				JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX				DL = 6.0 PSF			
F - I 2x4 DRY No.2 SPF				M 1645 0 1645 0 0 5-8 1-13				BOT CH. LL = 0.0 PSF			
M - B 2x4 DRY No.2 SPF				J 1645 0 1645 0 0 5-8 1-13				DL = 7.3 PSF			
J - H 2x4 DRY No.2 SPF				UNFACTORED REACTIONS				TOTAL LOAD = 48.1 PSF			
M - K 2x4 DRY No.2 SPF				1ST CASE MAX./MIN. COMPONENT REACTIONS				SPACING = 24.0 IN. G/G			
K - J 2x4 DRY No.2 SPF				JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
ALL WEBS EXCEPT 2x3 DRY No.2 SPF				M 1147 845 / 0 0 / 0 0 / 0 303 / 0 0 / 0				THIS DESIGN COMPLIES WITH:			
DRY, SEASONED LUMBER.				J 1147 845 / 0 0 / 0 0 / 0 303 / 0 0 / 0				- PART 9 OF CBC 2018, NBC-2019AE			
				BRACING				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.				- CSA 086-14			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.				- TPIC 2014			
				ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD			
				1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, G-J.				ALLOWABLE DEFL.(LL) = L/360 (0.72")			
				END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				CALCULATED VERT. DEFL.(LL) = L/999 (0.05")			
				LOADING				ALLOWABLE DEFL.(TL) = L/360 (0.72")			
				TOTAL LOAD CASES: (4)				CALCULATED VERT. DEFL.(TL) = L/673 (0.30")			
								CSI: TC=0.57/1.00 (G-H 1), BC=0.68/1.00 (J-L 4), WB=0.65/1.00 (G-J 1), SS=0.24/1.00 (E-G 1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
								COMPANION LIVE LOAD FACTOR = 1.00			
								TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 650 371 1747 788 1987 1873			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.81 (G) (INPUT = 0.90)			
								JSI METAL= 0.53 (C) (INPUT = 1.00)			



LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					DESIGN CRITERIA				
N. L. G. A. RULES					BEARINGS					SPECIFIED LOADS:				
CHORDS					FACTORED GROSS REACTION					TOP CH. LL = 34.8 PSF				
A - D 2x4 DRY No.2 SPF					JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX					DL = 6.0 PSF				
D - E 2x4 DRY No.2 SPF					I 1471 0 1471 0 0 3-14 1-9					BOT CH. LL = 0.0 PSF				
E - I 2x4 DRY No.2 SPF					O 1636 0 1636 0 0 5-8 1-12					DL = 7.3 PSF				
O - B 2x4 DRY No.2 SPF										TOTAL LOAD = 48.1 PSF				
O - M 2x4 DRY No.2 SPF														
M - K 2x4 DRY No.2 SPF														
K - G 2x3 DRY No.2 SPF														
J - I 2x4 DRY No.2 SPF														
REINFORCING MEMBERS					UNFACTORED REACTIONS					SPACING = 24.0 IN. C/C				
HW2 2x6 DRY No.2 SPF					1ST CASE MAX./MIN. COMPONENT REACTIONS					THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015				
ALL WEBS 2x3 DRY No.2 SPF					JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL					THIS DESIGN COMPLIES WITH:				
					I 1028 744 / 0 0 / 0 0 / 0 0 / 0 284 / 0 0 / 0					- PART 9 OF BCBC 2018, NBC-2019AE				
					O 1141 840 / 0 0 / 0 0 / 0 0 / 0 301 / 0 0 / 0					- PART 9 OF OBC 2012 (2019 AMENDMENT)				
DRY: SEASONED LUMBER.					BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O					- CSA 086-14				
					BRACING					- TPC 2014				
					TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.					(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD				
					MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.					ALLOWABLE DEFL.(LL) = L/360 (0.71")				
					ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.					CALCULATED VERT. DEFL.(LL) = L/999 (0.05")				
					1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O.					ALLOWABLE DEFL.(TL) = L/360 (0.71")				
					END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW					CALCULATED VERT. DEFL.(TL) = L/679 (0.38")				
					LOADING					CSI: TC=0.57/1.00 (B-C:1), BC=0.53/1.00 (L-N:4), WB=0.64/1.00 (C-O:1), SSI=0.24/1.00 (G-Q:1)				
					TOTAL LOAD CASES: (4)					DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10				
					CHORDS					COMPANION LIVE LOAD FACTOR = 1.00				
					MEMB. MAX. FACTORED FORCE (LBS)					TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.				
					FR-TO					NAIL VALUES				
					A-B 0 / 53					PLATE GRIP(DRY) SHEAR SECTION				
					B-C 0 / 50					(PSI) (PLI) (PLI)				
					C-D -1070 / 0					MAX MIN MAX MIN MAX MIN				
					D-E -1070 / 0					MT20 650 371 1747 788 1987 1873				
					E-F -1060 / 0					PLATE PLACEMENT TOL. = 0.250 inches				
					F-G -1651 / 0					PLATE ROTATION TOL. = 5.0 Deg.				
					G-H -1413 / 0					JSI GRIP= 0.89 (O) (INPUT = 0.90)				
					H-Q -1413 / 0					JSI METAL= 0.52 (C) (INPUT = 1.00)				
					Q-I -1859 / 0									
					O-B -407 / 0									
					O-N 0 / 1066									
					N-M 0 / 1057									
					M-L 0 / 1057									
					L-K 0 / 24									
					K-J 0 / 36									
					J-G 0 / 98									
					J-P 0 / 1351									
					P-I 0 / 932									

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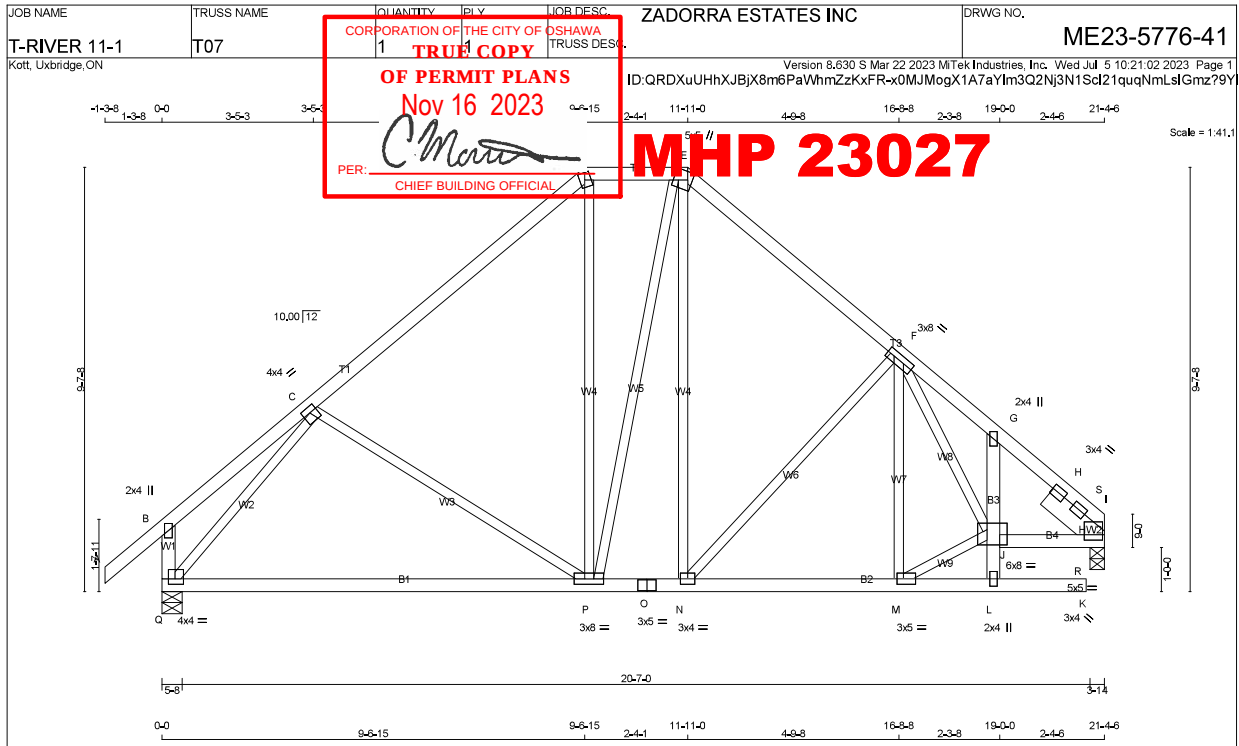
D. A. SHERMAN

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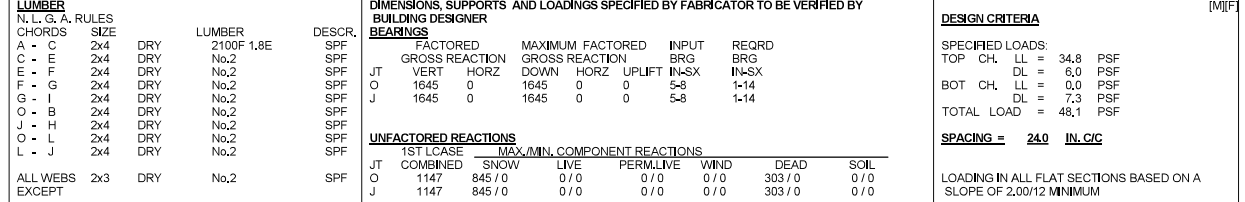
REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT
VOIDS THE ENGINEERS SEAL

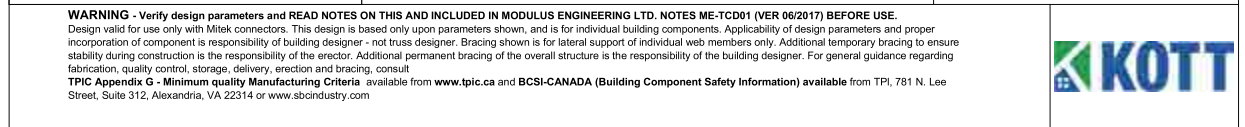


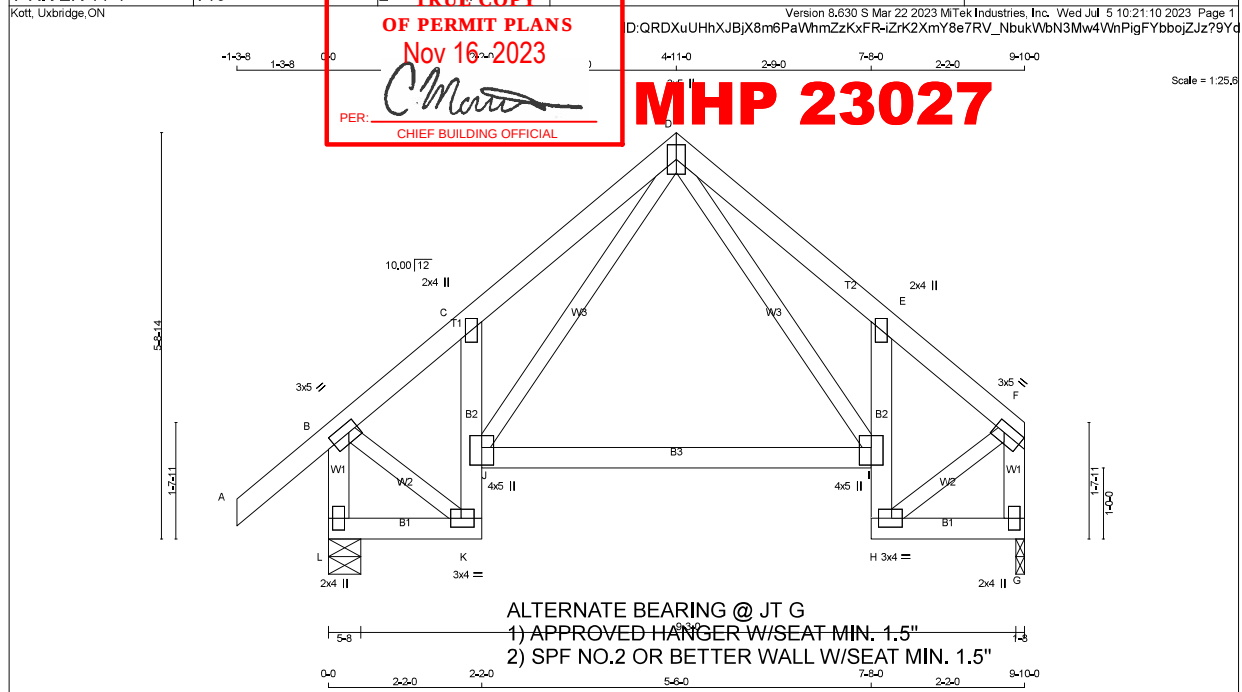
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA				
A - D	2x4	DRY	No.2	SPF	BEARINGS				SPECIFIED LOADS:			
D - E	2x4	DRY	No.2	SPF	FACTORED				TOP CH. LL = 34.8 PSF			
E - I	2x4	DRY	No.2	SPF	GROSS REACTION				DL = 6.0 PSF			
Q - B	2x4	DRY	No.2	SPF	DOWN				BOT CH. LL = 0.0 PSF			
Q - O	2x4	DRY	No.2	SPF	UP				DL = 7.3 PSF			
O - K	2x4	DRY	No.2	SPF	BRG				TOTAL LOAD = 48.1 PSF			
L - G	2x4	DRY	No.2	SPF	IN-SX				SPACING = 24.0 IN. CG			
J - I	2x4	DRY	No.2	SPF	WIND				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
REINFORCING MEMBERS				UNFACTORED REACTIONS				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015				
HW2	2x6	DRY	No.2	SPF	1ST CASE				THIS DESIGN COMPLIES WITH:			
ALL WEBS EXCEPT				2ND CASE				- PART 9 OF CBC 2018 - NBC-2019AE				
DRY: SEASONED LUMBER.				3RD CASE				- PART 9 OF OBC 2012 (2019 AMENDMENT)				
				4TH CASE				- CSA 086-14				
				5TH CASE				DESIGN ASSUMPTIONS				
				6TH CASE				- OVERHANG NOT TO BE ALTERED OR CUT OFF.				
				7TH CASE				(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD				
				8TH CASE				ALLOWABLE DEFL.(LL)= L/360 (0.71")				
				9TH CASE				CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")				
				10TH CASE				ALLOWABLE DEFL.(TL)= L/360 (0.71")				
				11TH CASE				CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")				
				12TH CASE				CSI TC=0.60/1.00 (C-D-1), BC=0.47/1.00 (P-Q-4), WB=0.77/1.00 (C-Q-1), SSI=0.26/1.00 (C-D-1)				
				13TH CASE				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10				
				14TH CASE				COMPANION LIVE LOAD FACTOR = 1.00				
				15TH CASE				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.				
				16TH CASE				NAIL VALUES				
				17TH CASE				PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)				
				18TH CASE				MAX MIN MAX MIN MAX MIN				
				19TH CASE				MT20 650 371 1747 788 1987 1873				
				20TH CASE				PLATE PLACEMENT TOL. = 0.250 inches				
				21TH CASE				PLATE ROTATION TOL. = 5.0 Deg.				
				22TH CASE				JSI GRIP= 0.90 (D) (INPUT = 0.90)				
				23TH CASE				JSI METAL= 0.55 (C) (INPUT = 1.00)				

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NAIL VALUES	
PLATE	GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
	MAX MIN MAX MIN MAX MIN
MT20	650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0,250 inches	
PLATE ROTATION TOL. = 5,0 Deg.	
JSI GRIP= 0,89 (C) (INPUT = 0,90)	
JSI METAL= 0,48 (R) (INPUT = 1,00)	





ALTERNATE BEARING @ JT G
1) APPROVED HANGER W/SEAT MIN. 1.5"
2) SPF NO.2 OR BETTER WALL W/SEAT MIN. 1.5"

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N, L, G, A, RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	JT	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL = 34.8 PSF	
D - F	2x4	DRY	No.2	SPF	L	VERT	DOWN	UP	IN-SX	DL = 6.0 PSF	
L - B	2x4	DRY	No.2	SPF	G	VERT	DOWN	UP	IN-SX	BOT CH. LL = 0.0 PSF	
G - F	2x4	DRY	No.2	SPF	L	854	0	854	0	DL = 7.3 PSF	
L - K	2x4	DRY	No.2	SPF	G	665	0	665	0	TOTAL LOAD = 48.1 PSF	
K - C	2x4	DRY	No.2	SPF							
J - I	2x4	DRY	No.2	SPF							
H - E	2x4	DRY	No.2	SPF							
H - G	2x4	DRY	No.2	SPF							
ALL WEBS	2x3	DRY	No.2	SPF							
EXCEPT											

DRY: SEASONED LUMBER.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE	VERT. LOAD	MAX	MEMB.	FORCE	MAX	
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH
A-B	0 / 53	-119.4	-119.4	0.16 (1)	10.00	D-I	0 / 346
B-C	-478 / 0	-119.4	-119.4	0.16 (1)	6.25	J-D	0 / 271
C-D	-548 / 0	-119.4	-119.4	0.18 (1)	6.25	B-K	0 / 402
D-E	-705 / 0	-119.4	-119.4	0.18 (1)	6.25	H-F	0 / 438
E-F	-471 / 0	-119.4	-119.4	0.06 (1)	6.25		
L-B	-836 / 0	0.0	0.0	0.09 (1)	7.81		
G-F	-647 / 0	0.0	0.0	0.07 (1)	7.81		
L-K	0 / 0	-18.2	-18.2	0.02 (4)	10.00		
K-J	-200 / 0	0.0	0.0	0.46 (1)	7.81		
J-C	-366 / 0	0.0	0.0	0.46 (1)	7.81		
J-I	0 / 348	-18.2	-18.2	0.20 (4)	10.00		
H-I	-219 / 0	0.0	0.0	0.50 (1)	7.81		
I-E	-447 / 0	0.0	0.0	0.50 (1)	7.81		
H-G	0 / 0	-18.2	-18.2	0.02 (4)	10.00		

ALLOWABLE DEFLECTIONS:
CALCULATED VERT. DEFLECTION (LL) = 1/999 (0.03")
ALLOWABLE DEFLECTION (LL) = 1/999 (0.04")
CALCULATED VERT. DEFLECTION (TL) = 1/960 (0.33")
ALLOWABLE DEFLECTION (TL) = 1/999 (0.11")

CSL TC=0.18/1.00 (C-D-1), BC=0.50/1.00 (E-I-1), WB=0.10/1.00 (F-H-1), SSI=0.29/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

07/05/2023

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REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

JOB NAME

TRUSS NAME

QUANTITY

P/LY

JOB DESC.

ZADORRA ESTATES INC

DRWG NO.

T-RIVER 11-1

T14

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 16 2023
PER: [Signature]
CHIEF BUILDING OFFICIAL

ME23-5776-48

Kott, Uxbridge, ON

ID.QRDXuUHhXJBjX8m6PaWhmZzKxFR-AIOJfTnBvx7F17yNsr1qwGv2Fw9R8lHqFYH5lz79Yc

Version 8.630 S Mar 22 2023 Mitek Industries, Inc. Wed Jul 5 10:21:11 2023 Page 1

0'-0"

5'-0"

5'-4"

5'-4"

5'-4"

5'-4"

10'-8"

10'-8"

11'-6"

Scale = 1:18.0

5.00 T₂

T₁

B

I

A

H

4x4

E_{2x4 II}

G

C

D

F

4x4

0'-0"

4'-0"

3'-0"

5'-4"

5'-4"

5'-4"

10'-8"

TOTAL WEIGHT = 30 Lbs

LUMBER

N.L.G.A. RULES

CHORDS SIZE

LUMBER

DESCR.

SPF

SPF

ALL WEBS 2x3 DRY SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

A TMBH1-m MT20 4.0 4.0 1.75

B TTW+p MT20 3.0 4.0

C TMBH1-m MT20 4.0 4.0 1.75

D BMW+w MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION

MAXIMUM FACTORED GROSS REACTION

INPUT BRG

REQRD BRG

HEEL WEDGE

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

C 845 0 845 0 0 3-0 1-8 2x4 R

A 734 0 734 0 0 3-0 1-8 2x4 L

UNFACTORED REACTIONS

1ST LCASE

JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL

C 589 436 / 0 0 / 0 0 / 0 153 / 0 0 / 0

A 513 371 / 0 0 / 0 0 / 0 142 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

FORCE (LBS)

VERT. LOAD

LC1

MAX. UNBRACED

FROM TO

LENGTH

FR-TO

A-I -930 / 0 -119.4 -119.4 0.18 (1) 6.20 E-B 0 / 210 0.05 (1)

I-B -930 / 0 -119.4 -119.4 0.35 (1) 5.96 F-G -267 / 30 0.00 (1)

B-G -930 / 0 -119.4 -119.4 0.35 (1) 5.96 H-I -267 / 30 0.00 (1)

G-C -930 / 0 -119.4 -119.4 0.18 (1) 6.20

C-D 0 / 8 -119.4 -119.4 0.07 (1) 10.00

A-H 0 / 846 -18.2 -18.2 0.34 (1) 10.00

H-E 0 / 846 -18.2 -18.2 0.34 (1) 10.00

E-F 0 / 846 -18.2 -18.2 0.34 (1) 10.00

F-C 0 / 846 -18.2 -18.2 0.34 (1) 10.00

WEBS

MEMB.

FORCE (LBS)

MAX. FACTORED

CS1 (LC)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018 - NBC-2019AE

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")

ALLOWABLE DEFL.(TL)= L/360 (0.36")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI TC=0.35/1.00 (B-I), BC=0.34/1.00 (A-H 1), WB=0.05/1.00 (B-E 1), SS=0.23/1.00 (B-G 1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)

JSI METAL= 0.38 (A) (INPUT = 1.00)

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07/05/2023

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCD01 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbcindustry.com

KOTT