

JOB NAME

T-RIVER 3-1

TRUSS NAME

J04

CORPORATION OF ONTARIO
TRUE COPY
OF PERMIT PLANS
Dec 04 2023

PER: CHIEF BUILDING OFFICIAL

JOB DESC.

ZADORRA ESTATES INC

DRWG NO.

ME23-5764-12

ID:NcclGzmhd_IdQzH3VGMNIRZos0a-S4xt6OV6wXeemsXuMmUp0Tj6ErK9UBQyptlyz?VIK

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Kott, Uxbridge, ON

Scale = 1:16,2

TOTAL WEIGHT = 6 X 14 = 81 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER DESCR.

G - B 2x4 DRY No.2 SPF

A - D 2x4 DRY No.2 SPF

G - E 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER,

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMWVW MT20 3.0 4.0 1.50 1.00

C TMW+w MT20 2.0 4.0

F BMWW1-t MT20 3.0 4.0

G BMV+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD

JT VERT HORZ DOWN HORZ UPLIFT IN-SX BRG

D 24 0 24 0 -77 1-8 1-8

E -100 0 0 0 -111 1-8 1-8

F 683 0 683 0 0 3-8 1-8

SEE MITEK STANDARD DETAIL, MSD2015-H FOR CONNECTION TO JOINT(S) D, E

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

D 16 15/-50 0/0 0/0 0/0 1/0 0/0

E -67 0/-69 0/0 0/0 0/0 0/-2 0/0

F 473 369/0 0/0 0/0 0/0 104/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. MAX. CSI (LC) UNBRAC FR-TO

G-B -175 / 0 0.0 0.0 0.02 (5) 7.81 F-C -371 / 0 0.06 (1)

A-B 0 / 43 -119.4 -119.4 0.14 (1) 10.00 B-F 0 / 21 0.00 (1)

B-C 0 / 28 -119.4 -119.4 0.28 (1) 10.00

C-D -55 / 0 -119.4 -119.4 0.28 (1) 6.25

G-F 0 / 0 -18.2 -18.2 0.33 (5) 10.00

F-E 0 / 0 -18.2 -18.2 0.33 (5) 10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBCBC 2018 , NBC-2019AE

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F., G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")

CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")

ALLOWABLE DEFL.(TL)= L/360 (0.19")

CALCULATED VERT. DEFL.(TL)= L/ 999 (0.01")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL)= L/120 (0.19")

CALCULATED VERT. DEFL.(LL)= L/ 340 (0.05")

ALLOWABLE DEFL.(TL)= L/120 (0.19")

CALCULATED VERT. DEFL.(TL)= L/ 223 (0.07")

CSI TC=0.28/1.00 (B-C), BC=0.33/1.00 (F-G), WB=0.06/1.00 (C-F). SSI=0.16/1.00 (F-G-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (F) (INPUT = 0.90)

JSI METAL= 0.19 (C) (INPUT = 1.00)

MODULUS ENGINEERING LTD.

REVIEW FOR TRUSS COMPONENT ONLY

NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-TCDD1 (VER 06/2017) BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building components. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult

TPIC Appendix G - Minimum quality Manufacturing Criteria available from www.tpica.ca and BCSI-CANADA (Building Component Safety Information) available from TPI, 781 N. Lee Street, Suite 312, Alexandria, VA 22314 or www.sbindustry.com

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED IN MODULUS ENGINEERING LTD. NOTES ME-7C001 (VER 06/2017) BEFORE USE.
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