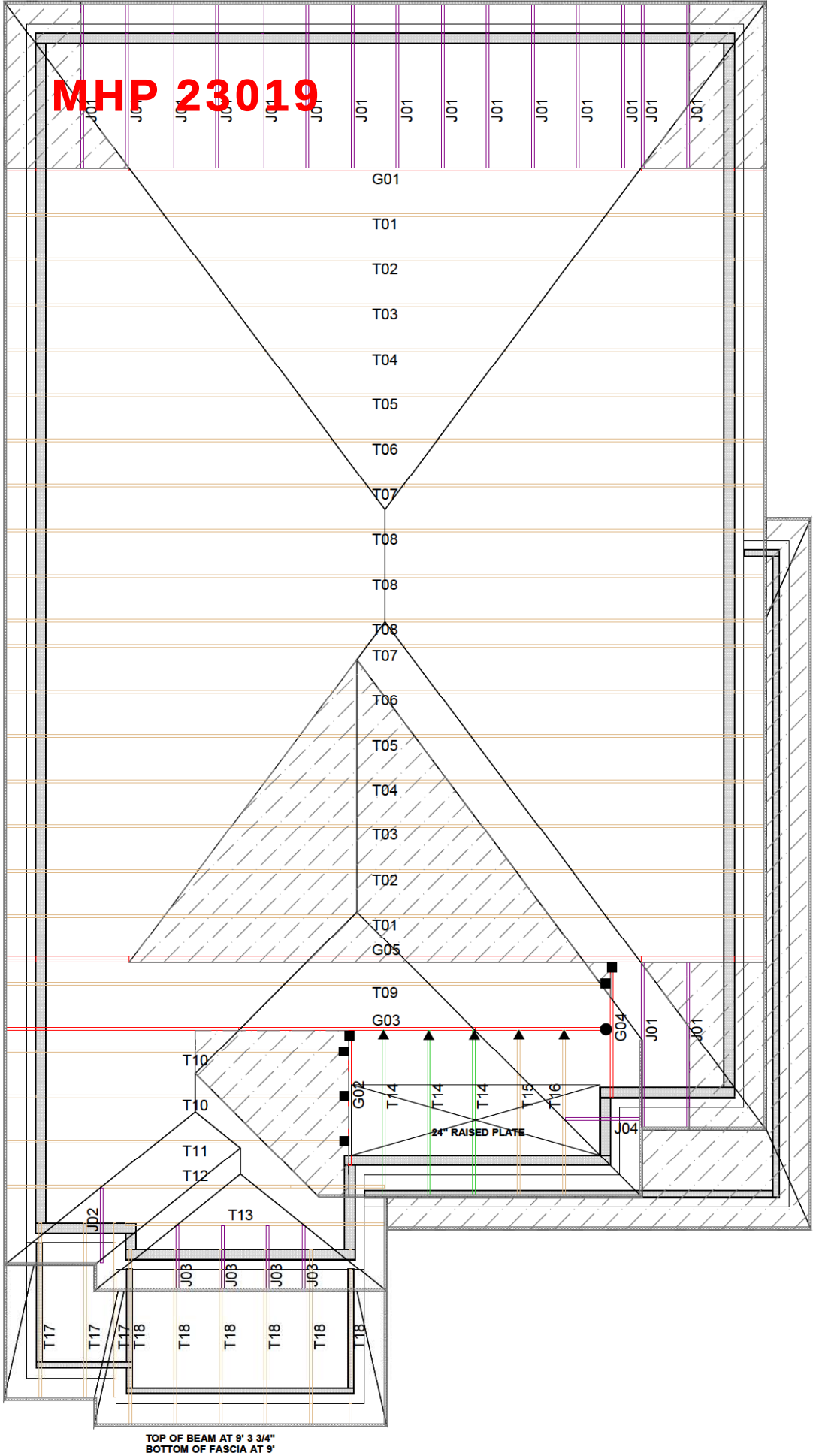




Hanger Name	Symbol	QTY
LUS24	▲	5
LJS26DS	■	6
HGUS26	●	1
	◆	0
	△	0
	□	0
	◡	0
	○	0



**CONVENTIONAL
FRAMING BY OTHERS**

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT CROSS OVER TRUSSES TO BE MIN. 2x4 SPF @ 24" C/C WITH A 2x4 VERTICAL POST TO THE TRUSS BELOW. VERTICAL POSTS TO BE LATERALLY BRACED SO THAT UNBRACED LENGTH DOES NOT EXCEED 6'. DESIGN OF CONVENTIONAL FRAMING IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

JOB INFORMATION

Customer	GREENPARK HOMES
Job #	22-00198R0
Address	ZADORRA PENROSE 2-1 OSHAWA, ON
Model	PENROSE 2-1
Sales Rep	RALPH MIRIGELLO
Designer	BB
Date	6/1/2022
Path	S:\DESIGN\KLU\CUSTOMERS\GREENPARK\ZADORRA ESTATES\MODELS\PENROSE 2\PENROSE 2-1\T

DESIGN INFORMATION

Code	NBCC 2015
Bldg	Residential - HSB (NBCC Part 9)
TC LL	34.8 lb/ft ²
TC DL	3.0 lb/ft ²
BC LL	0.0 lb/ft ²
BC DL	7.3 lb/ft ²
Deflection	LL=L/360 TL=L/360
Spacing	24" O/C unless otherwise noted
Complies With	OBC 2012 (2019 Amendment) CSA O86-14 and TPIC 2014

IMPORTANT INFORMATION

Hangers and Fasteners to be installed as per manufacturer

Refer to truss drawings in the Truss Engineering Package for ply-to-ply attachment notes

For site-framed valleys: top chords of all roof trusses must be laterally supported using 2x4 continuous bracing @24 O/C - all bracing must be anchored at ends as per TPIC Installation Guidelines

Read all notes on this page in addition to those shown on the KOTT Truss Engineering package

Field erection, handling and bracing are not the responsibility of KOTT, or KOTT Engineering

Unless noted otherwise, hurricane ties are to be installed at the bearings of all trusses > 40 ft clear span, and any girder or beam supporting trusses with a clear span >40 ft. See hanger legend for type.

Unless noted otherwise, for Part 9 bldgs, all trusses are to be anchored to the top of supporting walls as follows: trusses with a clear span <40 ft use 3-1/4" nails @ each bearing; trusses with a clear span >40 ft use 3-1/4" nails @ each bearing in addition to the appropriate hurricane tie.

KOTT Inc.
14 Anderson Blvd.
Uxbridge, ON
905.642.4400





ENGINEERING NOTE PAGE (ENP-1) PLEASE READ PRIOR TO INSTALLATION

MHP 23019

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY KOTT DESIGN. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

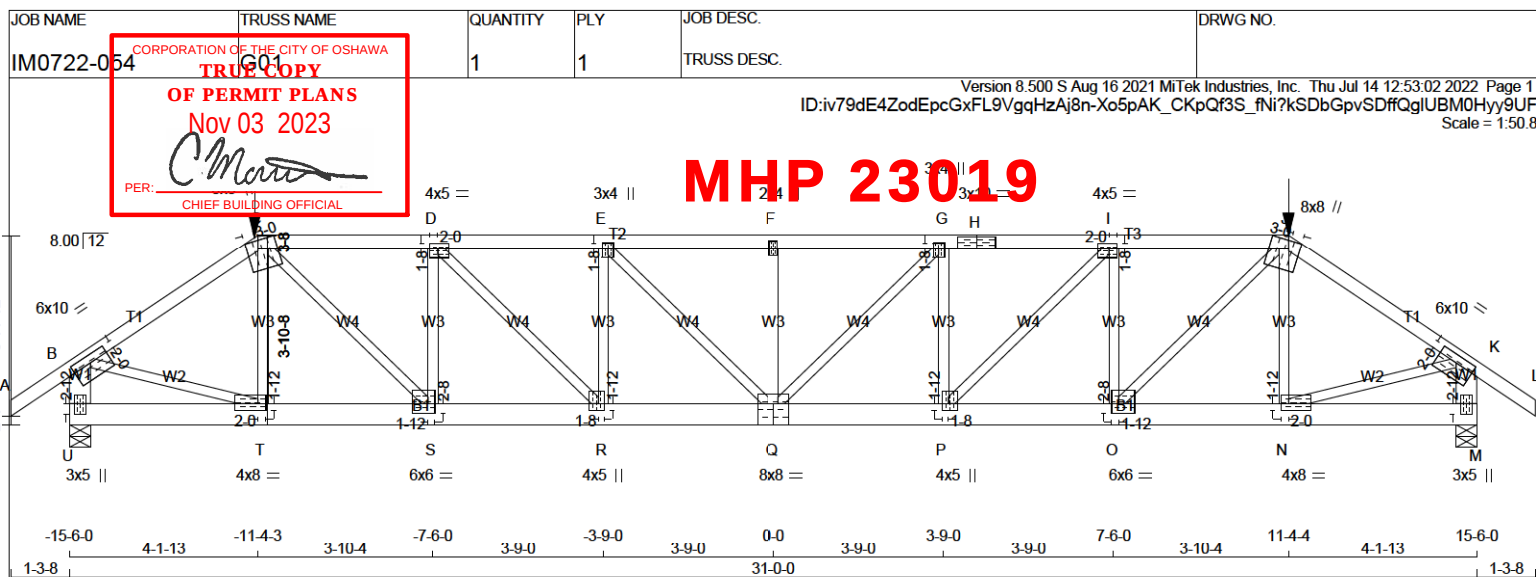
SUPPORTS

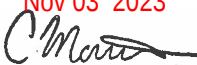
- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

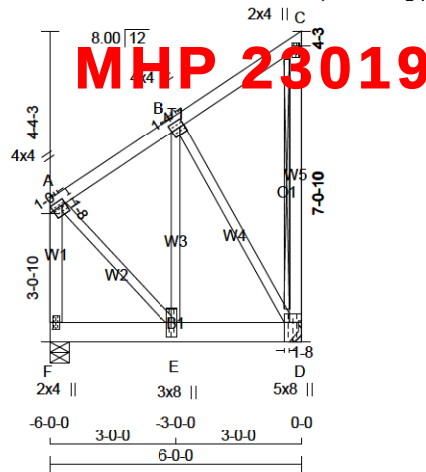
- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA G02 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER:  CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 12:53:03 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VggHzAj8n-?_fCOg?q57YWhcZsxPWz?Q7Z2JmOCfpz8xwYjy9UE
Scale = 1:54.9



TOTAL WEIGHT = 41 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS
--------	--------	------	--------	--------	----------

N. L. G. A. RULES					
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-t	MT20	4.0	4.0	1.50	1.00
B	TMW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	2.0	4.0		
D	BMVW-t	MT20	5.0	8.0	Edge	1.50
E	BMW-t	MT20	3.0	8.0		
F	BMV-t	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	JT VERT HORZ	IN-SX	IN-SX
F 1505 0	1505 0	5-8	1-10
D 1505 0	1505 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	1043	805 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0
D	1043	805 / 0	0 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX
FR-TO		FORCE (LBS)	FROM TO	(PLF)	CSI (LC)	UNBRACED LENGTH	FR-TO		FORCE (LBS)	MAX	CSI (LC)			
F-A		-1024 / 0	0.0	0.0	0.19 (1)	7.72	A-E		0 / 760	0.19 (1)				
A-B		-625 / 0	-111.9	-111.9	0.18 (1)	6.25	E-B		0 / 841	0.21 (1)				
B-C		-21 / 0	-111.9	-111.9	0.17 (1)	6.25	B-D		-1044 / 0	0.58 (1)				
D-C		-129 / 0	0.0	0.0	0.04 (1)	7.81								
F-E		0 / 0	-389.7	-389.7	0.23 (1)	10.00								
E-D		0 / 538	-389.7	-389.7	0.30 (1)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 13-10-8
END DISTANCE = 6-0-0
END SPAN CARRIED = 13-10-8
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.19/1.00 (A-F:1), BC=0.30/1.00 (D-E:1), WB=0.58/1.00 (B-D:1), SSI=0.46/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

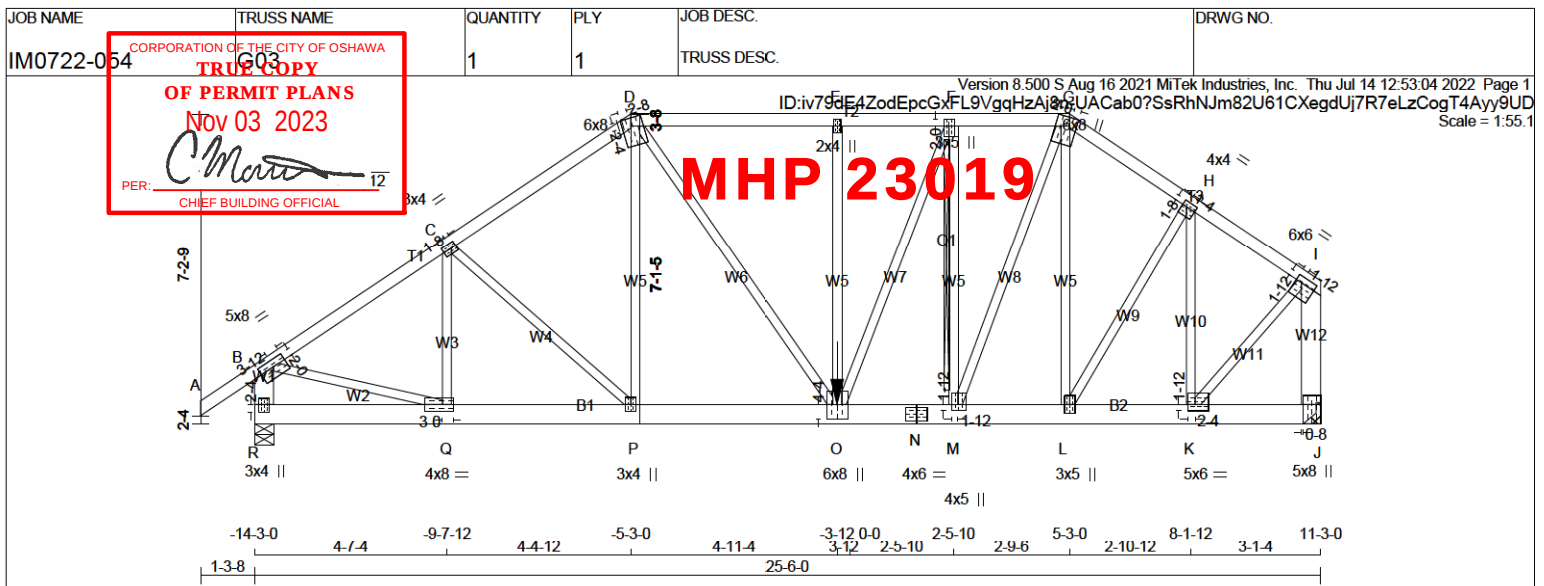
JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
R - B	2x6	DRY	No.2
J - I	2x6	DRY	No.2
R - N	2x6	DRY	No.2
N - J	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-t	MT20	5.0	8.0	2.00	3.75
C	TMW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW+m	MT20	6.0	8.0	Edge	2.50
E	TMW-w	MT20	2.0	4.0		
F	TMW-t	MT20	3.0	5.0	2.00	1.50
G	TTWW+m	MT20	6.0	8.0	Edge	2.00
H	TMW-t	MT20	4.0	4.0	1.50	1.25
I	TMW-t	MT20	6.0	6.0	1.75	1.75
J	BMV1+t	MT20	5.0	8.0	Edge	0.50
K	BMW-t	MT20	5.0	6.0	1.75	2.25
L	BMW-t	MT20	3.0	5.0		
M	BMW-t	MT20	4.0	5.0	1.75	1.75
N	BS-t	MT20	4.0	6.0		
O	BMW-t	MT20	6.0	8.0	4.25	3.00
P	BMW-t	MT20	3.0	4.0		
Q	BMW-t	MT20	4.0	8.0	2.00	3.00
R	BMV1+p	MT20	3.0	4.0	2.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
R	2797	0	2797	0
J	3512	0	3512	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 5-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
R	1936	1509 / 0	0 / 0
J	2434	1878 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 43	-111.9 -111.9	0.17 (1)	Q-C	-556 / 0	0.16 (1)	
B-C	-3213 / 0	-111.9 -111.9	0.66 (1)	C-P	-61 / 0	0.04 (1)	
C-D	-3227 / 0	-111.9 -111.9	0.65 (1)	P-D	0 / 214	0.05 (1)	
D-E	-3454 / 0	-111.9 -111.9	0.63 (1)	D-O	0 / 1386	0.34 (1)	
E-F	-3455 / 0	-111.9 -111.9	0.48 (1)	O-E	-530 / 0	0.49 (1)	
F-G	-3061 / 0	-111.9 -111.9	0.27 (1)	O-F	0 / 1069	0.26 (1)	
G-H	-2911 / 0	-111.9 -111.9	0.29 (1)	M-F	-1233 / 0	0.46 (1)	
H-I	-2337 / 0	-111.9 -111.9	0.25 (1)	M-G	0 / 1811	0.45 (1)	
R-B	-2738 / 0	0.0 0.0	0.20 (1)	L-G	-358 / 0	0.33 (1)	
J-I	-3338 / 0	0.0 0.0	0.42 (1)	L-H	0 / 860	0.21 (1)	
				K-H	-1453 / 0	0.63 (1)	
R-Q	0 / 0	-18.2 -18.2	0.07 (1)	B-Q	0 / 2762	0.68 (1)	
Q-P	0 / 2699	-18.2 -18.2	0.42 (1)	K-I	0 / 2725	0.67 (1)	
P-O	0 / 2657	-18.2 -18.2	0.45 (1)				
O-N	0 / 3061	-133.5 -133.5	0.53 (1)				
N-M	0 / 3061	-133.5 -133.5	0.53 (1)				
M-L	0 / 2394	-133.5 -133.5	0.39 (1)				
L-K	0 / 1961	-133.5 -133.5	0.33 (1)				
K-J	0 / 0	-133.5 -133.5	0.08 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	CONN.
O	13-11.4	-1505	-1505	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 13-11.4
START SPAN CARRIED = 6-0-0
END DISTANCE = 25-6-0
END SPAN CARRIED = 6-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.85")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (0.85")
CALCULATED VERT. DEFL. (TL) = L/999 (0.20")

CSI: TC=0.66/1.00 (B-C-1), BC=0.53/1.00 (M-O-1), WB=0.68/1.00 (B-Q-1), SSI=0.27/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA G03 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER:  CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 12:53:04 2022 Page 2
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-UACab0?SsRhNjM82U61CXegdUj7R7eLzCogT4Ayy9UD

MHP 23019

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
 JSI METAL= 0.78 (N) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA G04 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: [Signature] CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 12:53:04 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-UA Cab0?SsRhNjm82U61CXegiej1Z7 aizCogT4 Ayy9UD Scale = 1:34.4

MHP 23019

LUMBER
N L G A RULES
CHORDS SIZE LUMBER DESCR.
F - A 2x4 DRY No.2 SPF
A - C 2x4 DRY No.2 SPF
D - C 2x4 DRY No.2 SPF
F - D 2x8 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A B TMWW-t MT20 6.0 8.0 1.75 3.50
C TMV+p MT20 2.0 4.0
D BMVV+1+t MT20 8.0 12.0 7.25 Edge
E BMVVW+t MT20 8.0 10.0 5.50 3.50
F TMBMVW1*t+mt20 8.0 10.0 3.00 Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS
FACTORED MAXIMUM FACTORED INPUT REQ'D
GROSS REACTION GROSS REACTION BRG BRG
JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX
F 2649 0 2649 0 0 5-8 5-2
D 3846 0 3846 0 0 MECHANICAL

A SUITABLE HANGER/Mechanical CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3-8.

UNFACTORED REACTIONS
1ST LCASE MAX./MIN. COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
F 1836 1416 / 0 0 / 0 0 / 0 419 / 0 0 / 0
D 2665 2057 / 0 0 / 0 0 / 0 609 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS WEBS
MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX LC1 MAX CSI (LC) MAX UNBRAC LENGTH FR-TO MEMB. MAX. FACTORED FORCE (LBS) MAX CSI (LC)
FR-TO FROM TO
F-A -2422 / 0 0.0 0.0 0.27 (1) 5.44 A-E 0 / 3192 0.79 (1)
A-B -3471 / 0 -111.9 -111.9 0.33 (1) 3.45 E-B 0 / 3549 0.88 (1)
B-C -8 / 0 -111.9 -111.9 0.14 (1) 10.00 B-D -3908 / 0 0.92 (1)
D-C -146 / 0 0.0 0.0 0.03 (1) 7.81

F-E 0 / 0 -18.2 -18.2 0.30 (1) 10.00
E-D 0 / 3111 -767.8 -767.8 0.91 (1) 10.00

FACTORED CONCENTRATED LOADS (LBS)
JT LOC. LC1 MAX MAX+ FACE DIR TYPE HEEL CONN.
E 3-0-12 -3512 -3512 — FRONT VERT TOTAL — C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/Mechanical CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CSStd Girder
START DISTANCE = 3-0-12
START SPAN CARRIED = 25-6-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 25-6-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/993 (0.07")

CSI: TC=0.33/1.00 (A-B:1), BC=0.91/1.00 (D-E:1), WB=0.92/1.00 (B-D:1), SSI=0.78/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	G04	1	1	TRUSS DESC.	

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 03 2023

 PER: _____
 CHIEF BUILDING OFFICIAL

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 12:53:04 2022 Page 2
 ID:iv79dE4ZodEpcGxFL9VggHzAj8n-UACab0?SsRhNJm82U61CXegiej1Z7aizCogT4Ayy9UD

MHP 23019

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)

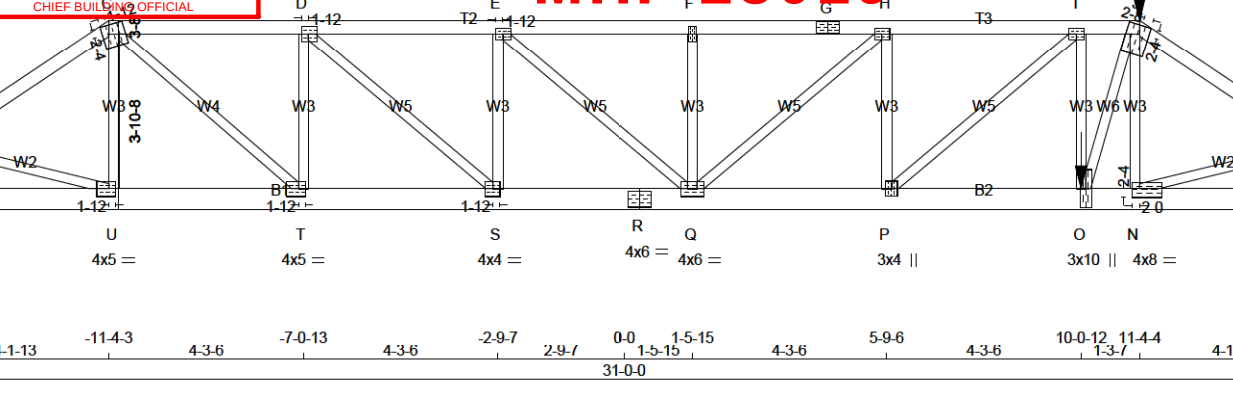
JSI METAL= 0.70 (E) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	TRUSS DESC.	DRWG NO.																																																																																																																																																																																																																								
IM0722-054	CORPORATION OF THE CITY OF OSHAWA G05 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: [Signature] CHIEF BUILDING OFFICIAL	1	2																																																																																																																																																																																																																										
Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 12:53:05 2022 Page 1 ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-yNmyPM04dkpEwwjE2qYR4rDsu7S1s4P6RSQ0ccy9UO Scale = 1:50.8																																																																																																																																																																																																																													
																																																																																																																																																																																																																													
TOTAL WEIGHT = 2 X 150 = 300 lb																																																																																																																																																																																																																													
LUMBER N.L.G.A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - G 2x4 DRY No.2 SPF G - J 2x4 DRY No.2 SPF J - L 2x4 DRY No.2 SPF V - B 2x6 DRY No.2 SPF M - K 2x6 DRY No.2 SPF V - R 2x6 DRY No.2 SPF R - M 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 TOP C-G 1 12 TOP G-J 1 12 SIDE(49.5) J-L 1 12 SIDE(0.0) V-B 2 12 TOP M-K 2 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS V-R 2 12 TOP R-M 2 12 SIDE(9.0) WEBS : (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQ'D GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX V 2919 0 2919 0 0 5-8 1-9 M 5831 0 5831 0 0 5-8 4-0 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL V 2020 1576 / 0 0 / 0 0 / 0 444 / 0 0 / 0 M 4040 3124 / 0 0 / 0 0 / 0 916 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table border="1"><thead><tr><th colspan="4">CHORDS</th><th colspan="4">WEBS</th></tr><tr><th>MEMB.</th><th>FORCE (LBS)</th><th>VERT. LOAD (PLF)</th><th>MAX. CSI (LC)</th><th>MEMB.</th><th>FORCE (LBS)</th><th>MAX. CSI (LC)</th><th></th></tr></thead><tbody><tr><td>FR-TO</td><td></td><td>FROM TO</td><td></td><td>FR-TO</td><td></td><td></td><td></td></tr><tr><td>A-B</td><td>0 / 43</td><td>-111.9 -111.9</td><td>0.08 (1)</td><td>10.00</td><td>U-C</td><td>-546 / 0</td><td>0.07 (1)</td></tr><tr><td>B-C</td><td>-3329 / 0</td><td>-111.9 -111.9</td><td>0.24 (1)</td><td>4.88</td><td>C-T</td><td>0 / 2955</td><td>0.37 (1)</td></tr><tr><td>C-D</td><td>-4967 / 0</td><td>-111.9 -111.9</td><td>0.26 (1)</td><td>4.13</td><td>T-D</td><td>-1895 / 0</td><td>0.25 (1)</td></tr><tr><td>D-E</td><td>-6521 / 0</td><td>-111.9 -111.9</td><td>0.34 (1)</td><td>3.59</td><td>D-S</td><td>0 / 2076</td><td>0.26 (1)</td></tr><tr><td>E-F</td><td>-7435 / 0</td><td>-111.9 -111.9</td><td>0.35 (1)</td><td>3.39</td><td>S-E</td><td>-1278 / 0</td><td>0.17 (1)</td></tr><tr><td>F-G</td><td>-7435 / 0</td><td>-111.9 -111.9</td><td>0.38 (1)</td><td>3.36</td><td>E-Q</td><td>0 / 1221</td><td>0.15 (1)</td></tr><tr><td>G-H</td><td>-7435 / 0</td><td>-111.9 -111.9</td><td>0.38 (1)</td><td>3.36</td><td>Q-F</td><td>-467 / 0</td><td>0.06 (1)</td></tr><tr><td>H-I</td><td>-7799 / 0</td><td>-111.9 -111.9</td><td>0.40 (1)</td><td>3.26</td><td>P-H</td><td>-486 / 0</td><td>0.13 (1)</td></tr><tr><td>I-J</td><td>-7327 / 0</td><td>-210.9 -210.9</td><td>0.21 (1)</td><td>3.51</td><td>P-H</td><td>-200 / 25</td><td>0.03 (1)</td></tr><tr><td>J-K</td><td>-7194 / 0</td><td>-111.9 -111.9</td><td>0.43 (1)</td><td>3.35</td><td>P-I</td><td>0 / 631</td><td>0.08 (1)</td></tr><tr><td>K-L</td><td>0 / 43</td><td>-111.9 -111.9</td><td>0.08 (1)</td><td>10.00</td><td>O-I</td><td>-704 / 0</td><td>0.09 (1)</td></tr><tr><td>V-B</td><td>-2887 / 0</td><td>0.0 0.0</td><td>0.10 (1)</td><td>7.81</td><td>O-J</td><td>0 / 4250</td><td>0.53 (1)</td></tr><tr><td>M-K</td><td>-5791 / 0</td><td>0.0 0.0</td><td>0.21 (1)</td><td>6.13</td><td>N-J</td><td>-791 / 0</td><td>0.10 (1)</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td>B-U</td><td>0 / 2840</td><td>0.35 (1)</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td>N-K</td><td>0 / 6137</td><td>0.76 (1)</td></tr><tr><td>V-U</td><td>0 / 0</td><td>-18.2 -18.2</td><td>0.02 (4)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>U-T</td><td>0 / 2750</td><td>-18.2 -18.2</td><td>0.20 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>T-S</td><td>0 / 4967</td><td>-18.2 -18.2</td><td>0.36 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>S-R</td><td>0 / 6521</td><td>-18.2 -18.2</td><td>0.48 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>R-Q</td><td>0 / 6521</td><td>-18.2 -18.2</td><td>0.48 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>Q-P</td><td>0 / 7799</td><td>-18.2 -18.2</td><td>0.56 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>P-O</td><td>0 / 7327</td><td>-18.2 -18.2</td><td>0.63 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>O-N</td><td>0 / 5954</td><td>-34.4 -34.4</td><td>0.55 (1)</td><td>10.00</td><td></td><td></td><td></td></tr><tr><td>N-M</td><td>0 / 0</td><td>-34.4 -34.4</td><td>0.05 (1)</td><td>10.00</td><td></td><td></td><td></td></tr></tbody></table> FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX+ FACE DIR TYPE HEEL CONN. J 26-10-3 -348 -348 — FRONT VERT TOTAL — C1 O 25-6-12 -3845 -3845 — FRONT VERT TOTAL — C1 CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED. DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 3.0 PSF BOT CH. LL = 0.0 PSF DL = 7.3 PSF TOTAL LOAD = 45.1 PSF SPACING = 24.0 IN./C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM GIRDER TYPE: CPrimeHip SIDE SETBACK = 4-1-13 END SETBACK = 6-0-0 END WALL WIDTH = 5-8 CORNER FRAMING TYPE: CONVENTIONAL END JACK TYPE: CONVENTIONAL APPLIED TO FRONT SIDE - ADDTL LOADS BASED ON .55 % OF GSL. LOADS APPLIED TO FIRST 5-5.4 OF SPAN MEASURED FROM THE RIGHT. *** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL(LL)= L/360 (1.03") CALCULATED VERT. DEFL(LL)= L/999 (0.21") ALLOWABLE DEFL(TL)= L/360 (1.03") CALCULATED VERT. DEFL(TL)= L/999 (0.34") CSI: TC=0.43/1.00 (J-K-1), BC=0.63/1.00 (O-P-1), WB=0.76/1.				CHORDS				WEBS				MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)		FR-TO		FROM TO		FR-TO				A-B	0 / 43	-111.9 -111.9	0.08 (1)	10.00	U-C	-546 / 0	0.07 (1)	B-C	-3329 / 0	-111.9 -111.9	0.24 (1)	4.88	C-T	0 / 2955	0.37 (1)	C-D	-4967 / 0	-111.9 -111.9	0.26 (1)	4.13	T-D	-1895 / 0	0.25 (1)	D-E	-6521 / 0	-111.9 -111.9	0.34 (1)	3.59	D-S	0 / 2076	0.26 (1)	E-F	-7435 / 0	-111.9 -111.9	0.35 (1)	3.39	S-E	-1278 / 0	0.17 (1)	F-G	-7435 / 0	-111.9 -111.9	0.38 (1)	3.36	E-Q	0 / 1221	0.15 (1)	G-H	-7435 / 0	-111.9 -111.9	0.38 (1)	3.36	Q-F	-467 / 0	0.06 (1)	H-I	-7799 / 0	-111.9 -111.9	0.40 (1)	3.26	P-H	-486 / 0	0.13 (1)	I-J	-7327 / 0	-210.9 -210.9	0.21 (1)	3.51	P-H	-200 / 25	0.03 (1)	J-K	-7194 / 0	-111.9 -111.9	0.43 (1)	3.35	P-I	0 / 631	0.08 (1)	K-L	0 / 43	-111.9 -111.9	0.08 (1)	10.00	O-I	-704 / 0	0.09 (1)	V-B	-2887 / 0	0.0 0.0	0.10 (1)	7.81	O-J	0 / 4250	0.53 (1)	M-K	-5791 / 0	0.0 0.0	0.21 (1)	6.13	N-J	-791 / 0	0.10 (1)						B-U	0 / 2840	0.35 (1)						N-K	0 / 6137	0.76 (1)	V-U	0 / 0	-18.2 -18.2	0.02 (4)	10.00				U-T	0 / 2750	-18.2 -18.2	0.20 (1)	10.00				T-S	0 / 4967	-18.2 -18.2	0.36 (1)	10.00				S-R	0 / 6521	-18.2 -18.2	0.48 (1)	10.00				R-Q	0 / 6521	-18.2 -18.2	0.48 (1)	10.00				Q-P	0 / 7799	-18.2 -18.2	0.56 (1)	10.00				P-O	0 / 7327	-18.2 -18.2	0.63 (1)	10.00				O-N	0 / 5954	-34.4 -34.4	0.55 (1)	10.00				N-M	0 / 0	-34.4 -34.4	0.05 (1)	10.00			
CHORDS				WEBS																																																																																																																																																																																																																									
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)																																																																																																																																																																																																																							
FR-TO		FROM TO		FR-TO																																																																																																																																																																																																																									
A-B	0 / 43	-111.9 -111.9	0.08 (1)	10.00	U-C	-546 / 0	0.07 (1)																																																																																																																																																																																																																						
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C-D	-4967 / 0	-111.9 -111.9	0.26 (1)	4.13	T-D	-1895 / 0	0.25 (1)																																																																																																																																																																																																																						
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E-F	-7435 / 0	-111.9 -111.9	0.35 (1)	3.39	S-E	-1278 / 0	0.17 (1)																																																																																																																																																																																																																						
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G-H	-7435 / 0	-111.9 -111.9	0.38 (1)	3.36	Q-F	-467 / 0	0.06 (1)																																																																																																																																																																																																																						
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I-J	-7327 / 0	-210.9 -210.9	0.21 (1)	3.51	P-H	-200 / 25	0.03 (1)																																																																																																																																																																																																																						
J-K	-7194 / 0	-111.9 -111.9	0.43 (1)	3.35	P-I	0 / 631	0.08 (1)																																																																																																																																																																																																																						
K-L	0 / 43	-111.9 -111.9	0.08 (1)	10.00	O-I	-704 / 0	0.09 (1)																																																																																																																																																																																																																						
V-B	-2887 / 0	0.0 0.0	0.10 (1)	7.81	O-J	0 / 4250	0.53 (1)																																																																																																																																																																																																																						
M-K	-5791 / 0	0.0 0.0	0.21 (1)	6.13	N-J	-791 / 0	0.10 (1)																																																																																																																																																																																																																						
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V-U	0 / 0	-18.2 -18.2	0.02 (4)	10.00																																																																																																																																																																																																																									
U-T	0 / 2750	-18.2 -18.2	0.20 (1)	10.00																																																																																																																																																																																																																									
T-S	0 / 4967	-18.2 -18.2	0.36 (1)	10.00																																																																																																																																																																																																																									
S-R	0 / 6521	-18.2 -18.2	0.48 (1)	10.00																																																																																																																																																																																																																									
R-Q	0 / 6521	-18.2 -18.2	0.48 (1)	10.00																																																																																																																																																																																																																									
Q-P	0 / 7799	-18.2 -18.2	0.56 (1)	10.00																																																																																																																																																																																																																									
P-O	0 / 7327	-18.2 -18.2	0.63 (1)	10.00																																																																																																																																																																																																																									
O-N	0 / 5954	-34.4 -34.4	0.55 (1)	10.00																																																																																																																																																																																																																									
N-M	0 / 0	-34.4 -34.4	0.05 (1)	10.00																																																																																																																																																																																																																									

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA G05 TRUE COPY OF PERMIT PLANS Nov 03 2023	1	2	TRUSS DESC.	

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PLATES (table is in it)

JT	TYPE	PLA	PERM	CHIEF BLDG OFF	PLI	PSI	PLI	PSI
B	TMVW-p	MT20	4.0	4.0	2.00	1.75		
C	TTWW-t	MT20	3.0	4.0	1.50	1.75		
D	TMVW-t	MT20	4.0	4.0	2.00	1.75		
E	TMVW-t	MT20	3.0	4.0	1.50	1.75		
F	TMVW-w	MT20	2.0	4.0				
G	TS-t	MT20	3.0	6.0				
H	TMVW-t	MT20	3.0	4.0				
I	TMVW-t	MT20	3.0	4.0				
J	TTWW+m	MT20	6.0	8.0	Edge 2.50			
K	TMVW-t	MT20	5.0	8.0	1.50	3.75		
M	BMV1+p	MT20	3.0	4.0	2.25	1.50		
N	BMVW-t	MT20	4.0	8.0	2.25	2.00		
O	BMVW-t	MT20	3.0	10.0				
P	BMVW-t	MT20	3.0	4.0				
Q	BMVWW-t	MT20	4.0	6.0				
R	BS-t	MT20	4.0	6.0				
S	BMVW-t	MT20	4.0	4.0	2.00	1.75		
T	BMVW-t	MT20	4.0	5.0	2.00	1.75		
U	BMVW-t	MT20	4.0	5.0	2.00	1.75		
V	BMV1+p	MT20	2.0	4.0				

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

MHP 23019

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)

JSI METAL= 0.80 (R) (INPUT = 1.00)



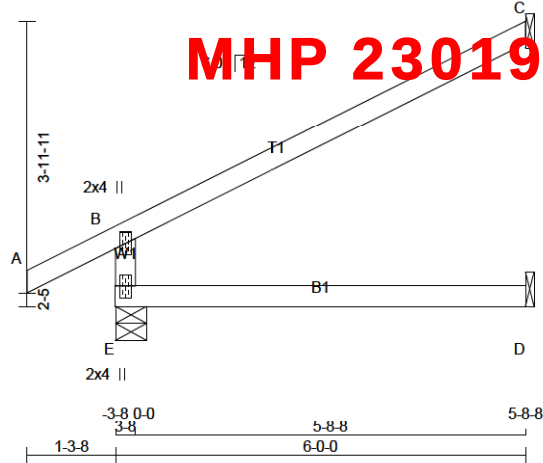
July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	17	1	TRUSS DESC.	

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ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-yNmpM04dkpEwwjE2qYR4rDos7ZrsFG6RSQ0ccy9UC
Scale = 1:33.7



TOTAL WEIGHT = 17 X 17 = 291 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS						
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX IN-SX
E	636	0	636	0	0	5-8 1-8
C	252	0	252	0	0	1-8 1-8
D	45	0	51	0	0	1-8 1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	437	355 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	170	157 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. CS1 (LC)	UNBRACED LENGTH	FR-TO
E-B		-571 / 0	0.0	0.0	0.13 (4)	7.81	
A-B		0 / 34	-111.9	-111.9	0.15 (1)	10.00	
B-C		-38 / 0	-111.9	-111.9	0.69 (1)	6.25	
E-D		0 / 0	-18.2	-18.2	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.69/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)



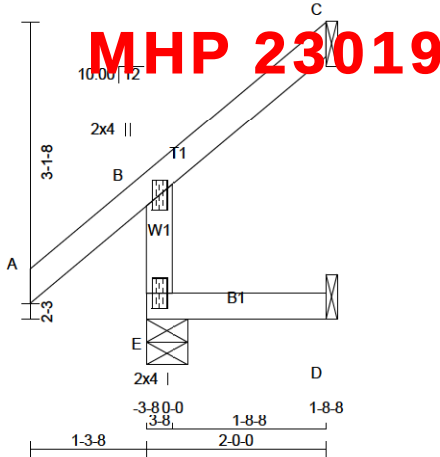
July 14, 2022

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA 102 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

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Scale = 1:25.6



TOTAL WEIGHT = 9 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	314	0	314	0	5-8	1-8
C	85	0	85	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
E	214	183 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0	
C	57	53 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0	
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED CSI (LC)
FR-TO		FROM	TO			FR-TO		
E-B	-294 / 0	0.0	0.0	0.01 (4)	7.81			
A-B	0 / 50	-111.9	-111.9	0.15 (1)	10.00			
B-C	-17 / 0	-111.9	-111.9	0.07 (1)	6.25			
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA 103 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	4	1	TRUSS DESC.	

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Scale = 1:22.9

MHP 23019

TOTAL WEIGHT = 4 X 8 = 30 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF	SPF	SPECIFIED LOADS:	TOP CH.	LL	
E - B	2x4	DRY	No.2		SPF			TOP CH. LL = 34.8 PSF			
A - C	2x4	DRY	No.2		SPF			DL = 3.0 PSF			
E - D	2x4	DRY	No.2		SPF			BOT CH. LL = 0.0 PSF			
								DL = 7.3 PSF			
								TOTAL LOAD = 45.1 PSF			
DRY: SEASONED LUMBER.				SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D				SPACING = 24.0 IN. C/C			
PLATES (table is in inches)				PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015			
JT	TYPE	PLATES	W	LEN	Y	X		THIS DESIGN COMPLIES WITH:			
B	TMV+p	MT20	2.0	4.0				- PART 9 OF CBC 2018, ABC 2019			
E	BMV1+p	MT20	2.0	4.0				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
								- CSA 086-14			
								- TPIC 2014			
				UNFACTORED REACTIONS				DESIGN ASSUMPTIONS			
JT	1ST CASE	MAX. MIN.	COMPONENT REACTIONS					- OVERHANG NOT TO BE ALTERED OR CUT OFF.			
E	221	193 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0	(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD			
C	18	16 / -32	0 / 0	0 / 0	0 / 0	2 / 0	0 / 0	ALLOWABLE DEFL. (LL) = L/360 (0.19")			
D	0	0 / -12	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0	CALCULATED VERT. DEFL. (LL) = L/999 (0.00")			
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E				BRACING				ALLOWABLE DEFL. (TL) = L/360 (0.19")			
				TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				CALCULATED VERT. DEFL. (TL) = L/999 (0.00")			
				MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.				CSI: TC=0.15/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),			
				LOADING				WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)			
				TOTAL LOAD CASES: (5)				DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
				CHORDS				COMP=1.10 SHEAR=1.10 TENS= 1.10			
				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	COMPANION LIVE LOAD FACTOR = 1.00			
				FR-TO		FROM TO	LENGTH FR-TO	TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
				E-B	-296 / 0	0.0	0.0 (5)	NAIL VALUES			
				A-B	0 / 50	-111.9	-111.9 (1)	PLATE GRIP(DRY) SHEAR SECTION			
				B-C	-36 / 0	-111.9	-111.9 (1)	(PSI) (PLI) (PLI)			
				E-D	0 / 0	-18.2	-18.2 (5)	MAX MIN MAX MIN MAX MIN			
								MT20 650 371 1747 788 1987 1873			
				CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN				PLATE PLACEMENT TOL. = 0.250 inches			
				PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.				PLATE ROTATION TOL. = 5.0 Deg.			



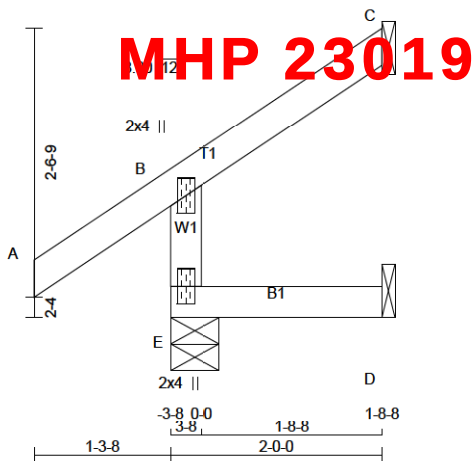
July 14, 2022

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 12:53:06 2022 Page 1
ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-QZKK0h1jO2x5Y4lQcX3gd3l5_WwvbiWFg69a92yy9UB
Scale = 1:21.8



TOTAL WEIGHT = 8 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	313	0	313	0	0	5-8	1-8	
C	85	0	85	0	0	1-8	1-8	
D	17	0	18	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	214	182 / 0	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
C	57	53 / 0	0 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO									
E-B	-293 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 43	-111.9	-111.9	0.15 (1)	10.00				
B-C	-15 / 0	-111.9	-111.9	0.07 (1)	6.25				
E-D	0 / 0	-18.2	-18.2	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

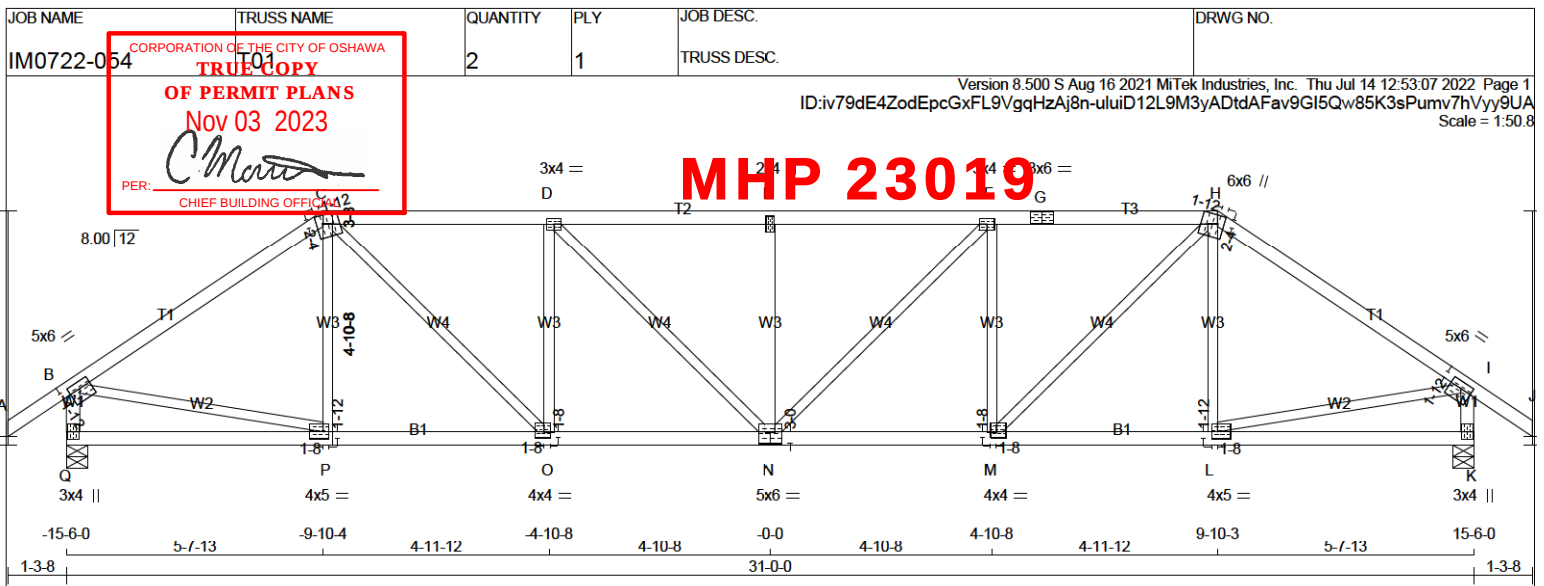
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JSI METAL= 0.15 (B) (INPUT = 1.00)

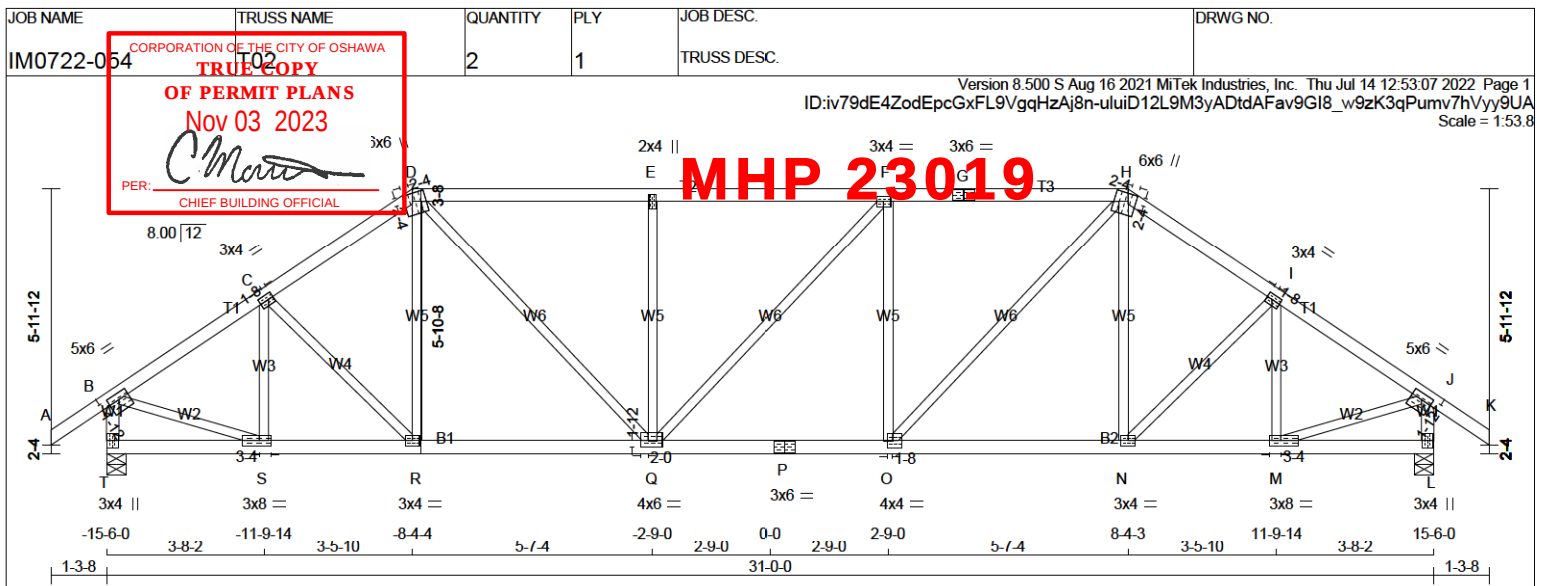


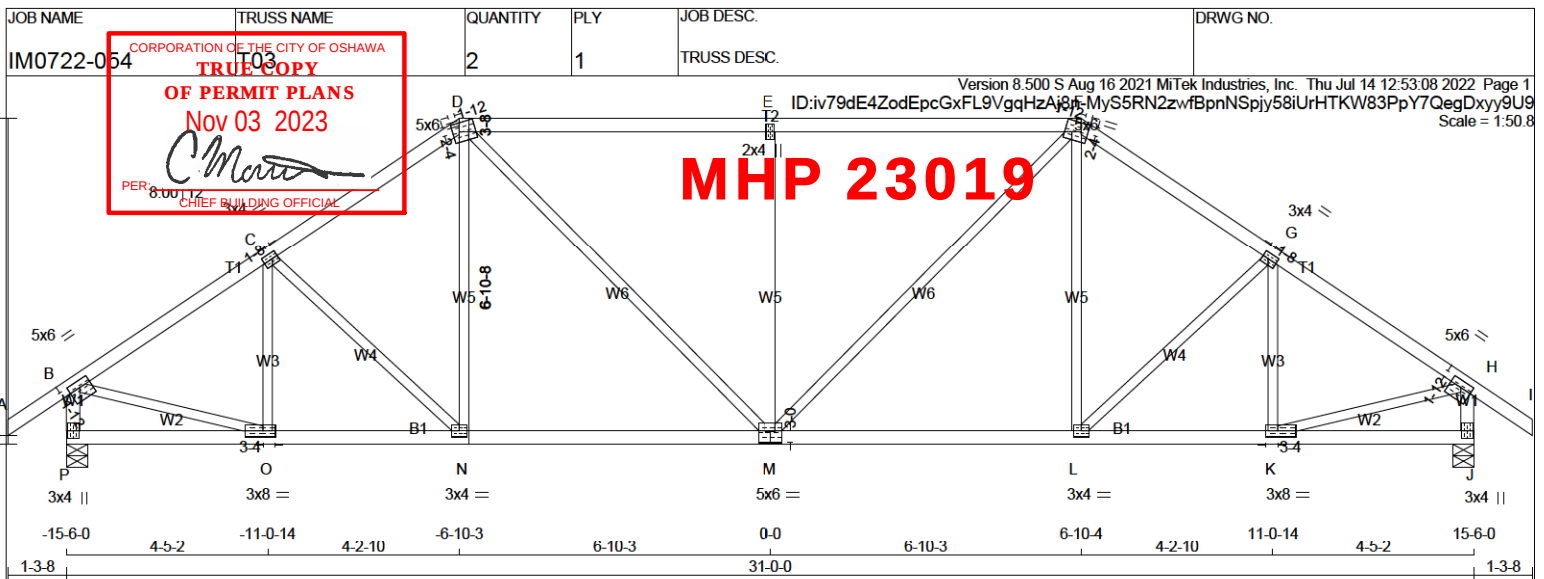
July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.









TOTAL WEIGHT = 2 X 134 = 268 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - I	2x4	DRY	No.2 SPF
P - B	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
P - M	2x4	DRY	No.2 SPF
M - J	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	1.75	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	TMVW-t	MT20	3.0	4.0	1.50	1.50
H	TMVW-t	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	3.0	8.0	1.50	3.25
L	BMVW-t	MT20	3.0	4.0		
M	BSWWW-I	MT20	5.0	6.0	3.00	3.00
N	BMVW-t	MT20	3.0	4.0		
O	BMVW-t	MT20	3.0	8.0	1.50	3.25
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY**BUILDING DESIGNER****BEARINGS**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
P	2171	0	2171	0	5-8	3-9
J	2171	0	2171	0	5-8	3-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. SNOW		LIVE		PERM. LIVE		WIND	DEAD	SOIL
	VERT	HORZ	MAX	MIN	MAX	MIN	MAX	MIN			
P	1502	1174 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0	0 / 0
J	1502	1174 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	UNBRACED LENGTH (FR-TO)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 43	-111.9	-111.9	0.15 (1)	10.00	O-C	-414 / 0
B-C	-2325 / 0	-111.9	-111.9	0.33 (1)	4.20	C-N	-161 / 0
C-D	-2249 / 0	-111.9	-111.9	0.32 (1)	4.27	N-D	0 / 238
D-E	-2366 / 0	-111.9	-111.9	0.79 (1)	3.41	D-M	0 / 738
E-F	-2366 / 0	-111.9	-111.9	0.79 (1)	3.41	M-E	-943 / 0
F-G	-2249 / 0	-111.9	-111.9	0.32 (1)	4.27	M-F	0 / 738
G-H	-2325 / 0	-111.9	-111.9	0.33 (1)	4.20	L-F	0 / 238
H-I	0 / 43	-111.9	-111.9	0.15 (1)	10.00	L-G	-161 / 0
P-B	-2131 / 0	0.0	0.0	0.22 (1)	5.81	K-G	-414 / 0
J-H	-2131 / 0	0.0	0.0	0.22 (1)	5.81	B-O	0 / 2017
P-O	0 / 0	-18.2	-18.2	0.08 (4)	10.00	K-H	0 / 2017
O-N	0 / 1959	-18.2	-18.2	0.42 (1)	10.00		
N-M	0 / 1845	-18.2	-18.2	0.40 (1)	10.00		
M-L	0 / 1845	-18.2	-18.2	0.40 (1)	10.00		
L-K	0 / 1959	-18.2	-18.2	0.42 (1)	10.00		
K-J	0 / 0	-18.2	-18.2	0.08 (4)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	45.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.11")
ALLOWABLE DEFL. (TL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.22")

CSI: TC=0.79/1.00 (D-E-1), BC=0.42/1.00 (K-L-1),
WB=0.84/1.00 (E-M-1), SSI=0.37/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

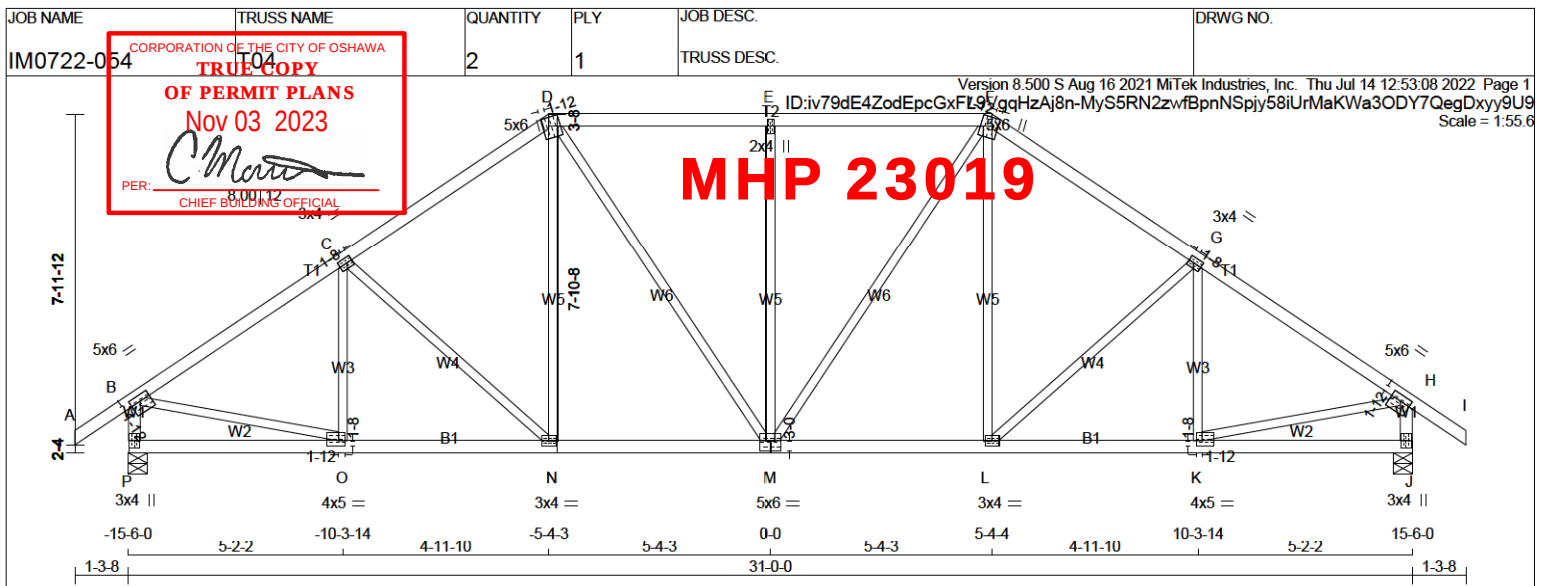
JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.61 (H) (INPUT = 1.00)



July 14, 2022

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IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.





LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N - L, G, A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMWV-t	MT20	5.0	6.0	1.75	3.00	
C	TMWV-t	MT20	3.0	4.0	1.50	1.50	
D	TTWV+m	MT20	5.0	6.0	Edge	1.75	
E	TMWV-w	MT20	2.0	4.0			
F	TTWV+m	MT20	5.0	6.0	Edge	1.75	
G	TMWV-t	MT20	3.0	4.0	1.50	1.50	
H	TMWV-t	MT20	5.0	6.0	1.75	3.00	
J	BMV1+p	MT20	3.0	4.0			
K	BMWV-t	MT20	4.0	5.0	1.50	1.75	
L	BMWV-t	MT20	3.0	4.0			
M	BSWWW-I	MT20	5.0	6.0	3.00	3.00	
N	BMWV-t	MT20	3.0	4.0			
O	BMWV-t	MT20	4.0	5.0	1.50	1.75	
P	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
P	2171	0	2171	0	5-8	3-9
J	2171	0	2171	0	5-8	3-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	UNBRAC			
P	1502	1174 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0
J	1502	1174 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOA. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 43	-111.9	-111.9	0.15 (1)	10.00	O-C	-323 / 21 0.11 (1)	
B-C	-2368 / 0	-111.9	-111.9	0.46 (1)	4.02	C-N	-330 / 0 0.28 (1)	
C-D	-2149 / 0	-111.9	-111.9	0.44 (1)	4.21	N-D	0 / 328 0.07 (1)	
D-E	-2033 / 0	-111.9	-111.9	0.46 (1)	4.25	D-M	0 / 489 0.11 (1)	
E-F	-2033 / 0	-111.9	-111.9	0.46 (1)	4.25	M-E	-731 / 0 0.95 (1)	
F-G	-2149 / 0	-111.9	-111.9	0.44 (1)	4.21	M-F	0 / 489 0.11 (1)	
G-H	-2368 / 0	-111.9	-111.9	0.46 (1)	4.02	L-F	0 / 328 0.07 (1)	
H-I	0 / 43	-111.9	-111.9	0.15 (1)	10.00	L-G	-330 / 0 0.28 (1)	
P-B	-2128 / 0	0.0	0.0	0.22 (1)	5.81	K-G	-323 / 21 0.11 (1)	
J-H	-2128 / 0	0.0	0.0	0.22 (1)	5.81	B-O	0 / 2044 0.46 (1)	
						K-H	0 / 2044 0.46 (1)	
P-O	0 / 0	-18.2	-18.2	0.10 (4)	10.00			
O-N	0 / 2001	-18.2	-18.2	0.39 (1)	10.00			
N-M	0 / 1759	-18.2	-18.2	0.35 (1)	10.00			
M-L	0 / 1759	-18.2	-18.2	0.35 (1)	10.00			
L-K	0 / 2001	-18.2	-18.2	0.39 (1)	10.00			
K-J	0 / 0	-18.2	-18.2	0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.10")
ALLOWABLE DEFL. (TL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.17")

CSI: TC=0.46/1.00 (D-E:1), BC=0.39/1.00 (K-L:1), WB=0.95/1.00 (E-M:1), SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.62 (H) (INPUT = 1.00)

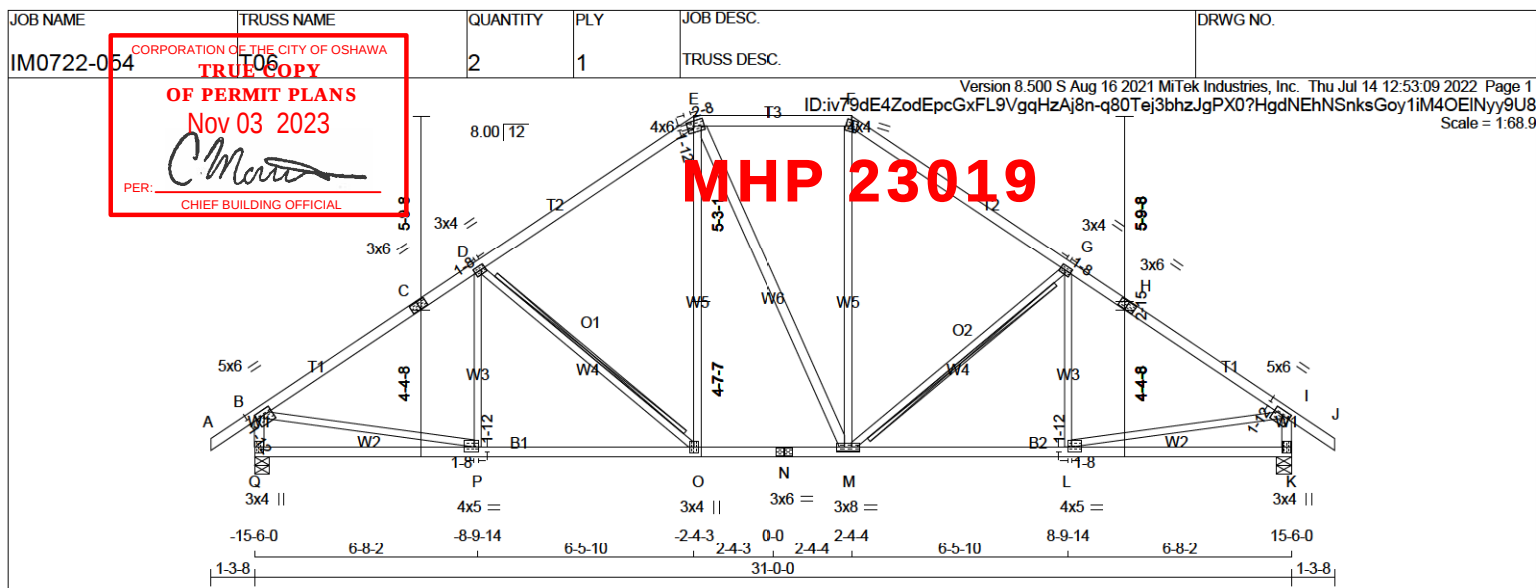


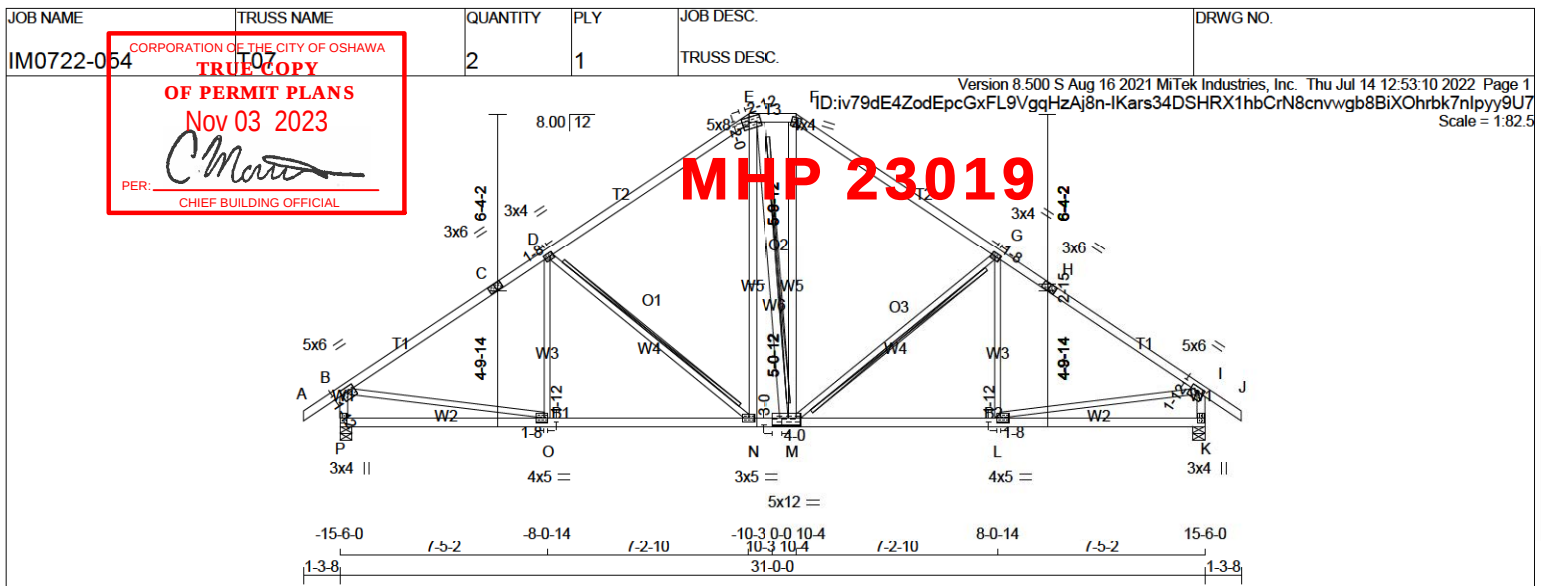
July 14, 2022

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[illegible]



**LUMBER**

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

C - E 2x4 DRY

E - F 2x4 DRY

F - H 2x4 DRY

H - J 2x4 DRY

P - B 2x4 DRY

K - I 2x4 DRY

P - M 2x4 DRY

M - K 2x4 DRY

ALL WEBS 2x3 DRY

EXCEPT

N - E 2x4 DRY

E - M 2x4 DRY

M - F 2x4 DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES

B TMW-t MT20

C TS-t MT20

D TMW-t MT20

E TTW-m MT20

F TTW-m MT20

G TMW-t MT20

H TS-t MT20

I TMW-t MT20

K BMV1-p MT20

L BMW-t MT20

M BSWWW-I MT20

N BMW-t MT20

O BMW-t MT20

P BMV1-p MT20

LUMBER

2100F 1.8E

2100F 1.8E

No.2

2100F 1.8E

2100F 1.8E

No.2

No.2

No.2

No.2

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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

DOWN

UP

BRG

IN-SX

BRG

IN-SX

BRG

IN-SX

BRG

IN-SX

BRG

IN-SX

BRG

IN-SX

BRG

IN-SX

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UNFACTORED REACTIONS

1ST LCASE

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

0/0

0/0

0/0

0/0

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FACTORED

MAXIMUM FACTORED

INPUT

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BRG

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY

APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-N, E-M, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3"

COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER

90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN

THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FACTORED

VERT. LOAD LC1

MAX

UNBRAC

LENGTH

FR-TO

A-B

B-C

C-D

D-E

E-F

F-G

G-H

H-I

I-J

J-K

K-L

L-M

M-N

N-O

O-P

P-Q

Q-R

R-S

S-T

T-U

U-V

V-W

W-X

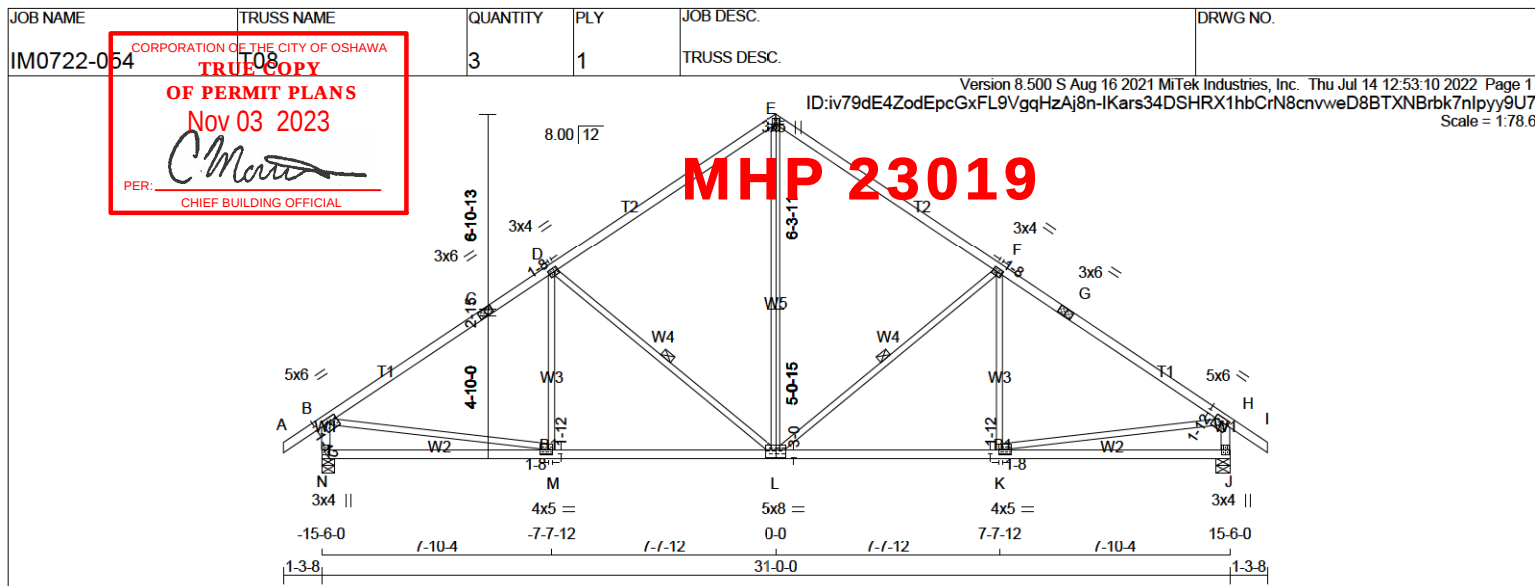
X-Y

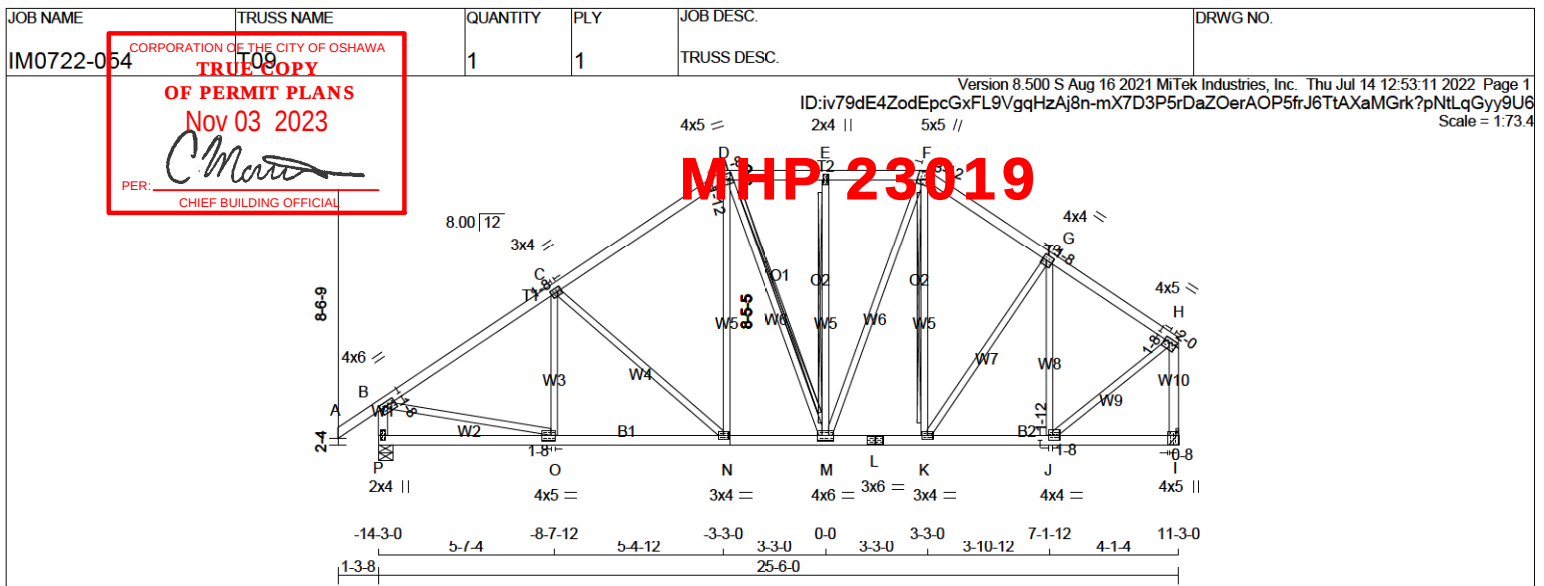
Y-Z

Z-AA

AA-BB

BB-CC





TOTAL WEIGHT = 131 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	3.00
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	4.0	5.0	1.75	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	5.0	Edge	3.75
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-t	MT20	4.0	5.0	1.50	2.00
I	BMV1+t	MT20	4.0	5.0	Edge	0.50
J	BMVW-t	MT20	4.0	4.0	1.75	1.50
K	BMVW-t	MT20	3.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVWW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	3.0	4.0		
O	BMVW-t	MT20	4.0	5.0	2.00	1.50
P	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
P	1813	0	1813	0
I	1659	0	1659	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1254	983 / 0	0 / 0	0 / 0	0 / 0	271 / 0	0 / 0
I	1150	887 / 0	0 / 0	0 / 0	0 / 0	263 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-M, E-M, F-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 43	-111.9 -111.9	0.15 (1)	10.00	O-C	-196 / 63	0.08 (1)
B-C	-1883 / 0	-111.9 -111.9	0.51 (1)	4.35	C-N	-492 / 0	0.51 (1)
C-D	-1522 / 0	-111.9 -111.9	0.47 (1)	4.77	N-D	0 / 408	0.09 (1)
D-E	-1251 / 0	-111.9 -111.9	0.15 (1)	5.60	D-M	-3 / 44	0.01 (1)
E-F	-1251 / 0	-111.9 -111.9	0.15 (1)	5.60	M-E	-431 / 0	0.24 (1)
F-G	-1336 / 0	-111.9 -111.9	0.25 (1)	5.34	M-F	0 / 466	0.10 (1)
G-H	-1201 / 0	-111.9 -111.9	0.24 (1)	5.57	K-F	-19 / 49	0.02 (4)
P-B	-1770 / 0	0.0	0.0	18 (1)	K-G	0 / 106	0.02 (1)
I-H	-1628 / 0	0.0	0.0	32 (1)	J-G	-687 / 0	0.41 (1)
				6.48	B-O	0 / 1630	0.37 (1)
P-O	0 / 0	-18.2 -18.2	0.14 (4)	10.00	J-H	0 / 1280	0.29 (1)
O-N	0 / 1600	-18.2 -18.2	0.32 (1)	10.00			
N-M	0 / 1235	-18.2 -18.2	0.24 (1)	10.00			
M-L	0 / 1084	-18.2 -18.2	0.20 (1)	10.00			
L-K	0 / 1084	-18.2 -18.2	0.20 (1)	10.00			
K-J	0 / 1024	-18.2 -18.2	0.20 (1)	10.00			
J-I	0 / 0	-18.2 -18.2	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 3.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.3 PSF

TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.85")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.85")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.10")

CSI: TC=0.51/1.00 (B-C:1), BC=0.32/1.00 (N-O:1), WB=0.51/1.00 (C-N:1), SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 1.00)

July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

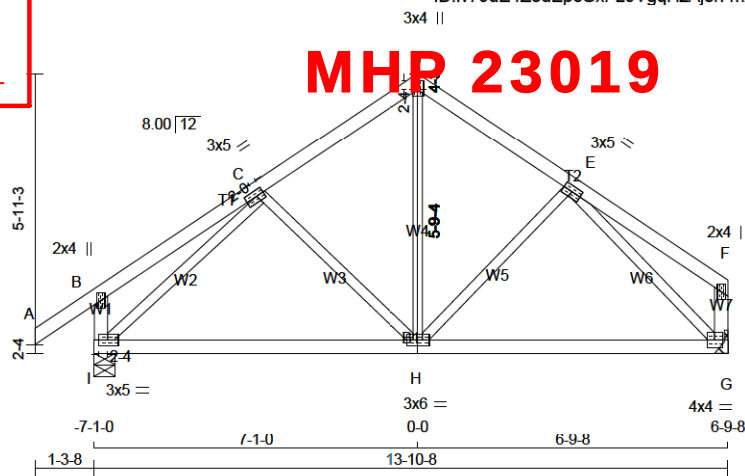


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA T-10 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	2	1	TRUSS DESC.	

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 12:53:11 2022 Page 1

ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-mX7D3P5rDaZoerAOP5frJ6TxcXatGth?pnLqGyy9U6

Scale = 1:50.5



TOTAL WEIGHT = 2 X 59 = 118 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW-t	MT20	3.0	5.0		
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVWW-t	MT20	3.0	6.0		
I	BMVW1-t	MT20	3.0	5.0	1.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
I	1056	0	1056	0
G	903	0	903	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
I	729	578 / 0	0 / 0
G	626	483 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 43	-111.9	-111.9	0.15 (1)	10.00	C-H	-227 / 5 0.09 (1)
B-C	0 / 25	-111.9	-111.9	0.22 (1)	10.00	H-D	0 / 442 0.10 (1)
C-D	-694 / 0	-111.9	-111.9	0.17 (1)	6.25	H-E	-178 / 13 0.07 (1)
D-E	-693 / 0	-111.9	-111.9	0.16 (1)	6.25	I-C	-1004 / 0 0.39 (1)
E-F	0 / 24	-111.9	-111.9	0.20 (1)	10.00	E-G	-982 / 0 0.37 (1)
I-B	-306 / 0	0.0	0.0	0.03 (1)	7.81		
G-F	-147 / 0	0.0	0.0	0.02 (1)	7.81		
I-H	0 / 720	-18.2	-18.2	0.29 (4)	10.00		
H-G	0 / 680	-18.2	-18.2	0.29 (4)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD		=	45.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.46")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.46")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.07")

CSI: TC=0.22/1.00 (B-C-1), BC=0.29/1.00 (H-I-4), WB=0.39/1.00 (C-I-1), SSI=0.16/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

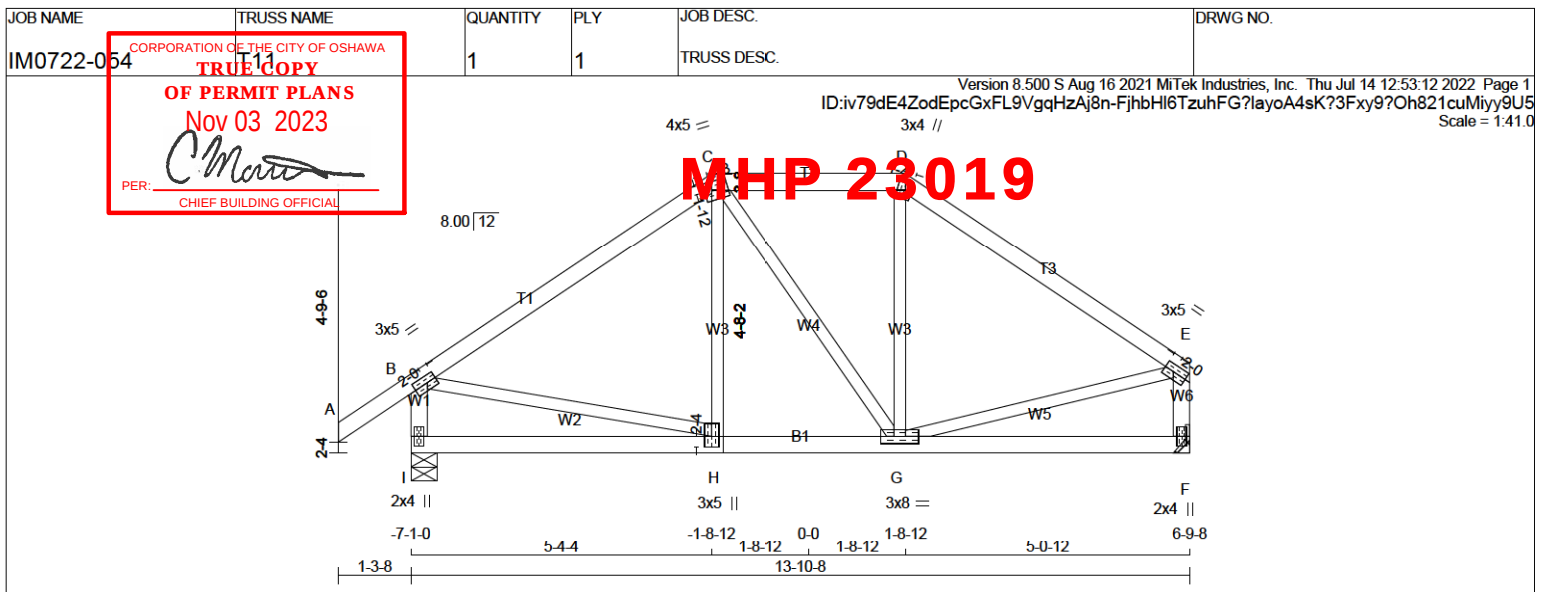
PLATE ROTATION TOL. = 5.0 Deg.

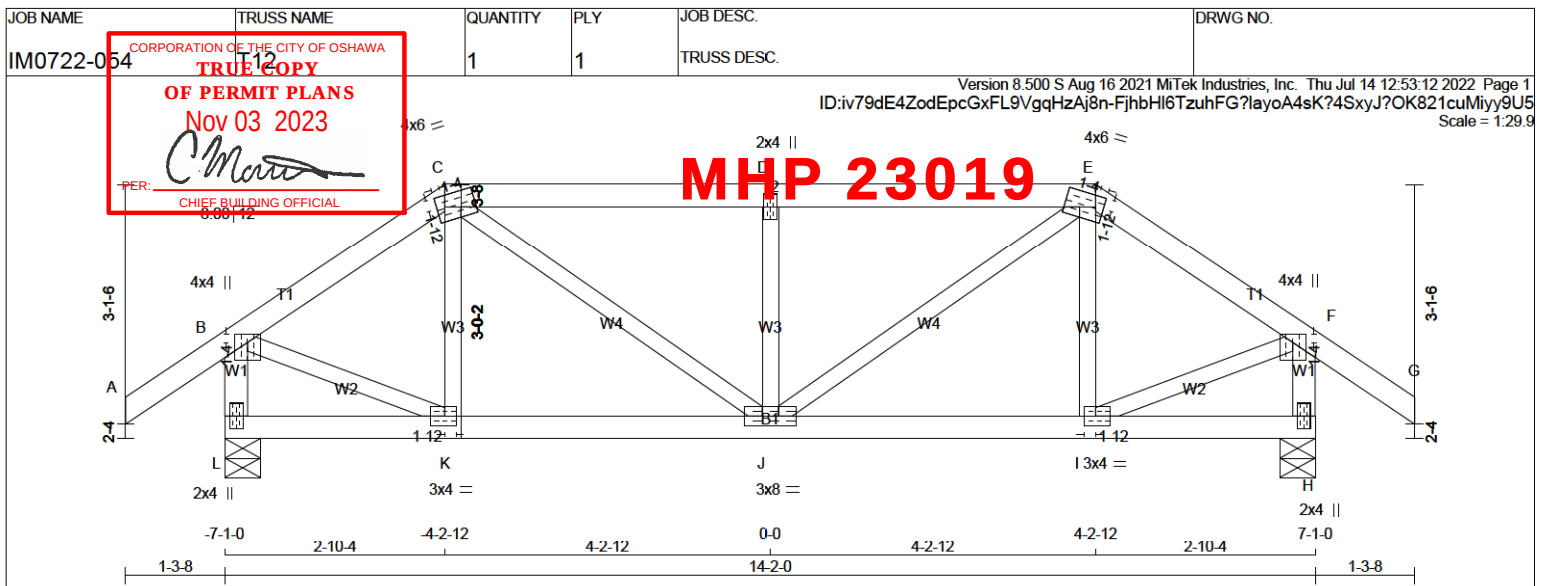
JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

July 14, 2022

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	1.25
D	TMVW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	1.25
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMVW-t	MT20	3.0	4.0	1.50	1.75
J	BMVWW-t	MT20	3.0	8.0		
K	BMVW-t	MT20	3.0	4.0	1.50	1.75
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1075	0	1075	0
H	1075	0	1075	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	743	588 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0
H	743	588 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		FROM	TO	LENGTH
A-B	0 / 43	-111.9	-111.9	0.15 (1)	10.00	K-C	-186 / 20		0.04 (1)
B-C	-863 / 0	-111.9	-111.9	0.17 (1)	6.25	C-J	0 / 556		0.13 (1)
C-D	-1163 / 0	-111.9	-111.9	0.34 (1)	5.45	J-D	-574 / 0		0.11 (1)
D-E	-1163 / 0	-111.9	-111.9	0.34 (1)	5.45	J-E	0 / 556		0.13 (1)
E-F	-863 / 0	-111.9	-111.9	0.17 (1)	6.25	I-E	-186 / 20		0.04 (1)
F-G	0 / 43	-111.9	-111.9	0.15 (1)	10.00	B-K	0 / 763		0.17 (1)
L-B	-1056 / 0	0.0	0.0	0.11 (1)	7.66	I-F	0 / 763		0.17 (1)
H-F	-1056 / 0	0.0	0.0	0.11 (1)	7.66				
L-K	0 / 0	-18.2	-18.2	0.05 (4)	10.00				
K-J	0 / 710	-18.2	-18.2	0.15 (1)	10.00				
J-I	0 / 710	-18.2	-18.2	0.15 (1)	10.00				
I-H	0 / 0	-18.2	-18.2	0.05 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.47")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.47")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.34/1.00 (C-D-1), BC=0.15/1.00 (J-K-1), WB=0.17/1.00 (B-K-1), SSI=0.23/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

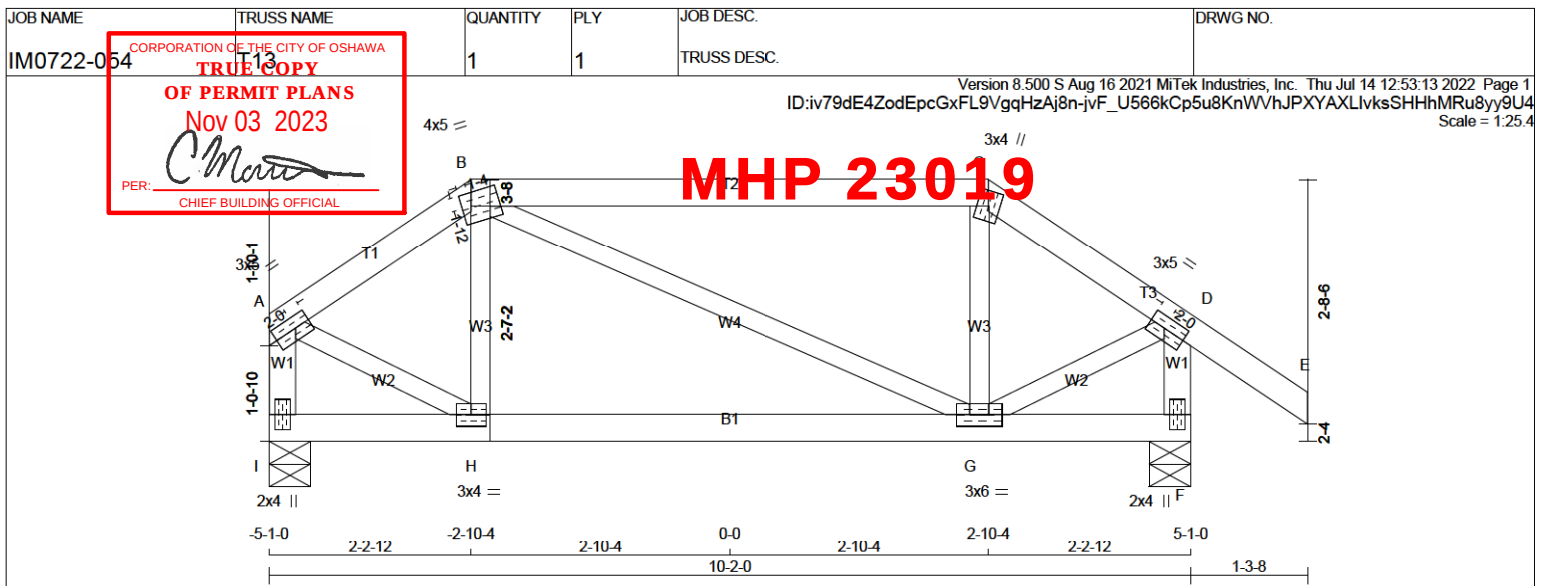
JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.24 (K) (INPUT = 1.00)



July 14, 2022

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
I - A	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	2.00
B	TTWW-m	MT20	4.0	5.0	1.75	1.25
C	TTW+m	MT20	3.0	4.0		
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	3.0	6.0		
H	BMVW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	DOWN	IN-SX	IN-SX
I	661	0	5-8	1-8
F	815	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	458	354 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
F	562	449 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-562 / 0	-111.9 -111.9	0.10 (1)	6.25	H-B	-130 / 44	0.02 (1)
B-C	-461 / 0	-111.9 -111.9	0.64 (1)	6.25	B-G	0 / 0	0.00 (1)
C-D	-562 / 0	-111.9 -111.9	0.10 (1)	6.25	G-C	-130 / 45	0.02 (1)
D-E	0 / 43	-111.9 -111.9	0.15 (1)	10.00	A-H	0 / 514	0.12 (1)
I-A	-655 / 0	0.0 0.0	0.07 (1)	7.81	G-D	0 / 514	0.12 (1)
F-D	-808 / 0	0.0 0.0	0.08 (1)	7.81			
I-H	0 / 0	-18.2 -18.2	0.09 (4)	10.00			
H-G	0 / 461	-18.2 -18.2	0.12 (1)	10.00			
G-F	0 / 0	-18.2 -18.2	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.34")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
 ALLOWABLE DEFL. (TL) = L/360 (0.34")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.64/1.00 (B-C:1), BC=0.12/1.00 (G-H:1), WB=0.12/1.00 (A-H:1), SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
 JSI METAL= 0.20 (D) (INPUT = 1.00)



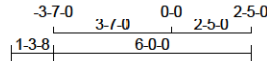
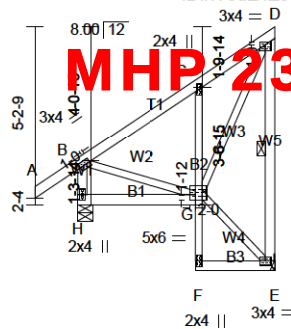
July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	T-14 CORPORATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	3	1	TRUSS DESC.	

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ID:iv79dE4ZodEpcGxFL9VggHzAj8n-jvF_U566kCp5u8KnWVhJPXYlpLlhksgHHhMRu8yy9U4
Scale = 1:70.0

TOTAL WEIGHT = 3 X 39 = 118 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x3	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TMV+p	MT20	2.0	4.0		
D	TMVW+p	MT20	3.0	4.0	1.00	2.00
E	BMVW1-t	MT20	3.0	4.0		
F	BMV+p	MT20	2.0	4.0		
G	BMVWWW-l	MT20	5.0	6.0	1.75	2.00
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	544	0	544	0
E	390	0	390	0
			MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW
H	374	304 / 0
E	271	209 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E. DBS = 20-0-0. CBF = 46 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	MAX
FR-TO		FROM	TO	CS1 (LC)	UNBRAC	FR-TO			
H-B	-510 / 0	0.0	0.0	0.05 (1)	7.81	B-G	0 / 207	0.05 (1)	
A-B	0 / 43	-111.9	-111.9	0.15 (1)	10.00	G-E	-4 / 0	0.00 (1)	
B-C	-219 / 0	-111.9	-111.9	0.18 (1)	6.25	G-D	0 / 451	0.10 (1)	
C-D	-254 / 0	-111.9	-111.9	0.17 (1)	6.25				
E-D	-367 / 0	0.0	0.0	0.09 (1)	6.25				
H-G	0 / 0	-18.2	-18.2	0.07 (4)	10.00				
F-G	0 / 24	0.0	0.0	0.02 (1)	10.00				
G-C	-417 / 0	0.0	0.0	0.04 (1)	7.81				
F-E	0 / 3	-18.2	-18.2	0.03 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.18/1.00 (B-C-1), BC=0.07/1.00 (G-H-4), WB=0.10/1.00 (D-G-1), SSI=0.15/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)



July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

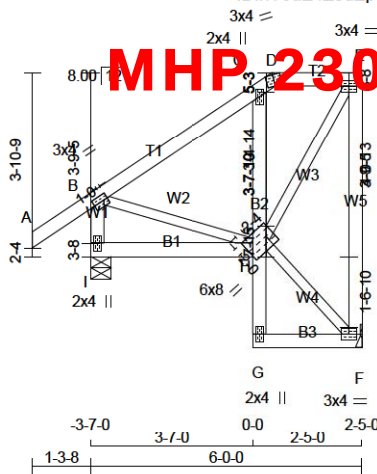


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morin</i> CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

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ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-B5pMiR7kVvVyVlvz4DCYx15SCfETJVRVL5?Rbyy9U3

Scale = 1:51.0



TOTAL WEIGHT = 38 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
G - C	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TMV+p	MT20	2.0	4.0		
D	TT-m	MT20	3.0	4.0	Edge	2.00
E	TMVW-t	MT20	3.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMV+p	MT20	2.0	4.0		
H	BVMWW-w	MT20	6.0	8.0	2.00	2.25
I	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	368	0	0	0
I	566	0	5.8	1.8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1.8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	255	194 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
I	389	319 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 43	-111.9	-111.9 0.15 (1)	10.00	H-F	-8 / 3	0.00 (1)
B-C	-216 / 0	-111.9	-111.9 0.20 (1)	6.25	B-H	0 / 147	0.03 (1)
C-D	-206 / 0	-111.9	-111.9 0.08 (1)	6.25	H-E	0 / 285	0.06 (1)
D-E	-151 / 0	-111.9	-111.9 0.05 (1)	6.25			
F-E	-340 / 0	0.0	0.0 0.08 (1)	7.81			
I-B	-528 / 0	0.0	0.0 0.05 (4)	7.81			
I-H	0 / 32	-18.2	-18.2 0.05 (4)	10.00			
G-H	0 / 23	0.0	0.0 0.02 (1)	10.00			
H-C	-242 / 1	0.0	0.0 0.05 (1)	7.81			
G-F	0 / 10	-18.2	-18.2 0.02 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
DL	=	3.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.3	PSF	
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (0.20")
CALCULATED VERT. DEFL (LL) = L/999 (0.00")
ALLOWABLE DEFL (TL) = L/360 (0.20")
CALCULATED VERT. DEFL (TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (B-C-1), BC=0.05/1.00 (C-H-1), WB=0.06/1.00 (E-H-1), SSI=0.14/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

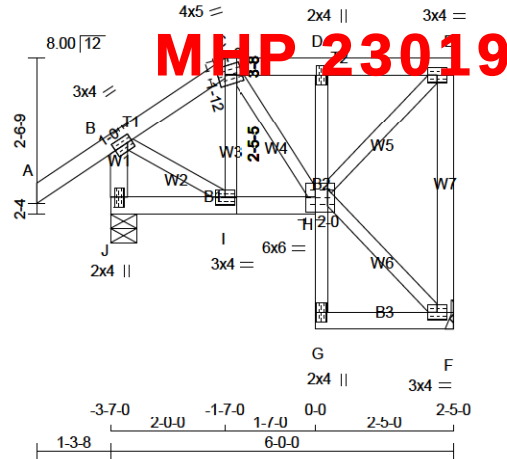
July 14, 2022

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
IM0722-054	CORPORATION OF THE CITY OF OSHAWA T16 TRUE COPY OF PERMIT PLANS Nov 03 2023 PER: <i>C. Morris</i> CHIEF BUILDING OFFICIAL	1	1	TRUSS DESC.	

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ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-B5pMiR7kVvxyVlvz4DCYxl5TyfQTJFRVL5?Rbyy9U3
Scale = 1:40.3



TOTAL WEIGHT = 35 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
G - D	2x3	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TTWW-m	MT20	4.0	5.0	1.75	1.50
D	TMV+p	MT20	2.0	4.0		
E	TMVW-t	MT20	3.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		
G	BMV+p	MT20	2.0	4.0		
H	BMVWWW-I	MT20	6.0	6.0	3.00	2.00
I	BMVW-t	MT20	3.0	4.0		
J	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	390	0	390	0
J	544	0	544	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS				
F	271	209 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
J	374	304 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 43	-111.9	-111.9	0.15 (1)	10.00	I-C	-75 / 20
B-C	-265 / 0	-111.9	-111.9	0.08 (1)	6.25	C-H	0 / 52
C-D	-246 / 0	-111.9	-111.9	0.06 (1)	6.25	H-F	-2 / 0
D-E	-245 / 0	-111.9	-111.9	0.07 (1)	6.25	H-E	0 / 356
F-E	-368 / 0	0.0	0.0	0.13 (1)	7.81	B-I	0 / 247
J-B	-527 / 0	0.0	0.0	0.05 (1)	7.81		
J-I	0 / 0	-18.2	-18.2	0.02 (4)	10.00		
I-H	0 / 217	-18.2	-18.2	0.04 (1)	10.00		
G-H	0 / 24	0.0	0.0	0.01 (1)	10.00		
H-D	-270 / 0	0.0	0.0	0.01 (1)	7.81		
G-F	0 / 2	-18.2	-18.2	0.03 (4)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	45.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.15/1.00 (A-B:1), BC=0.04/1.00 (H-I:1), WB=0.08/1.00 (E-H:1), SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME

TRUSS NAME

JOB DESC.

DRWG NO.

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 03 2023

CHIEF BUILDING OFFICIAL

IM0722-054

QUANTITY

PLY

TRUSS DESC.

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ID:iv79dE4ZodEpcGxFL9VgqHzAj8n-B5pMIR7kVVxyIvz4DCYxl5UZIxmTIARVL5?Rbyy9U3

Scale = 1:20.6

MHP 23019

TOTAL WEIGHT = 3 X 22 = 67 lb [M][F]

LUMBER

N.L.G.A. RULES

CHORDS SIZE LUMBER DESCR.

A - D 2x4 DRY No.2 SPF

F - D 2x4 DRY No.2 SPF

B - E 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

GROSS REACTION FACTORED MAXIMUM FACTORED INPUT REQD HEEL

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX WEDGE

B 544 0 544 0 0 3-0 1-8 2x4 L

E 356 0 356 0 0 5-8 1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL

B 375 299 / 0 0 / 0 0 / 0 0 / 0 76 / 0 0 / 0

E 248 182 / 0 0 / 0 0 / 0 0 / 0 66 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS WEBS

MEMB. FORCE FACTORED VERT. LOAD LC1 MAX. MAX. MEMB. FORCE MAX.

(LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)

FR-TO FROM TO

A-B 0 / 7 -111.9 -111.9 0.07 (1) 10.00 G-C 0 / 218 0.05 (1)

B-I -806 / 0 -111.9 -111.9 0.03 (1) 6.25 C-F -782 / 0 0.15 (1)

I-C -756 / 0 -111.9 -111.9 0.07 (1) 6.25 H-I -39 / 14 0.00 (1)

C-D -6 / 0 -111.9 -111.9 0.11 (1) 10.00

F-D -143 / 0 0.0 0.02 (1) 7.81

B-H 0 / 723 -18.2 -18.2 0.15 (1) 10.00

H-G 0 / 723 -18.2 -18.2 0.21 (1) 10.00

G-F 0 / 723 -18.2 -18.2 0.53 (1) 10.00

F-E 0 / 0 -18.2 -18.2 0.41 (1) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF DL = 3.0 PSF

BOT CH. LL = 0.0 PSF DL = 7.3 PSF

TOTAL LOAD = 45.1 PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05") ALLOWABLE DEFL(TL)= L/360 (0.23") CALCULATED VERT. DEFL.(TL) = L/ 930 (0.09")

CSI: TC=0.11/1.00 (C-D:1), BC=0.53/1.00 (F-G:1), WB=0.15/1.00 (C-F:1), SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90) JSI METAL= 0.25 (F) (INPUT = 1.00)

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMBH1-m MT20 3.0 5.0 1.50 0.25

C TMWW-t MT20 3.0 4.0

D TMW+p MT20 2.0 4.0

F BMVW-t MT20 3.0 4.0 1.50 1.75

G BMW+w MT20 2.0 4.0

LICENSED PROFESSIONAL ENGINEER

I.MATUEVIC

100528832

PROVINCE OF ONTARIO

July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

KOTT

JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

DRWG NO.

Version 8.500 S Aug 16 2021 MiTek Industries, Inc. Thu Jul 14 12:53:15 2022 Page 1

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CORPORATION OF THE CITY OF OSHAWA
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OF PERMIT PLANS
Nov 03 2023

CHIEF BUILDING OFFICIAL

MHP 23019

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY				DESIGN CRITERIA			
N.L.G.A. RULES				BUILDING DESIGNER							
CHORDS SIZE LUMBER DESCR.				BEARINGS				SPECIFIED LOADS:			
A - D	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD	TOP CH. LL = 34.8 PSF		
F - D	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	DL = 3.0 PSF		
B - E	2x4	DRY	No.2	SPF	VERT HORZ	DOWN HORZ UPLIFT	IN-SX	HEEL	BOT CH. LL = 0.0 PSF		
									DL = 7.3 PSF		
									TOTAL LOAD = 45.1 PSF		
ALL WEBS 2x3 DRY No.2 SPF											
DRY: SEASONED LUMBER.											
				UNFACTORED REACTIONS				SPACING = 24.0 IN./C/C			
				1ST LCASE MAX./MIN. COMPONENT REACTIONS				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015			
				JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL				THIS DESIGN COMPLIES WITH:			
				B 375 299 / 0 0 / 0 0 / 0 0 / 0 76 / 0 0 / 0				- PART 9 OF BCBC 2018 , ABC 2019			
				E 248 182 / 0 0 / 0 0 / 0 0 / 0 66 / 0 0 / 0				- PART 9 OF OBC 2012 (2019 AMENDMENT)			
								- CSA 086-14			
								- TPIC 2014			
								(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD			
								ALLOWABLE DEFL.(LL)= L/360 (0.23")			
								CALCULATED VERT. DEFL.(LL)= L/ 999 (0.04")			
								ALLOWABLE DEFL.(TL)= L/360 (0.23")			
								CALCULATED VERT. DEFL.(TL)= L/ 999 (0.07")			
								CSI: TC=0.07/1.00 (C-I:1) , BC=0.52/1.00 (F-G:1) , WB=0.13/1.00 (C-F:1) , SSI=0.28/1.00 (E-F:1)			
								DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
								COMPANION LIVE LOAD FACTOR = 1.00			
								TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
								NAIL VALUES			
								PLATE GRIP(DRY) SHEAR SECTION			
								(PSI) (PLI) (PLI)			
								MAX MIN MAX MIN MAX MIN			
								MT20 650 371 1747 788 1987 1873			
								PLATE PLACEMENT TOL. = 0.250 inches			
								PLATE ROTATION TOL. = 5.0 Deg.			
								JSI GRIP= 0.81 (F) (INPUT = 0.90)			
								JSI METAL= 0.23 (F) (INPUT = 1.00)			

LICENSED PROFESSIONAL ENGINEER
I.MATJUEVIC
100528832
PROVINCE OF ONTARIO

July 14, 2022

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



STANDARD DETAIL MSD2015-H

Issued: MARCH 1, 2022

Expiry: APRIL 30, 2024

MHP 23019**TOE-NAIL CAPACITY DETAILS****LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS**

NAIL TYPE	Length (in)	Diameter (in)	LATERAL Resistance per nail (Lbs.)		WITHDRAWAL Resistance per nail (Lbs.)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

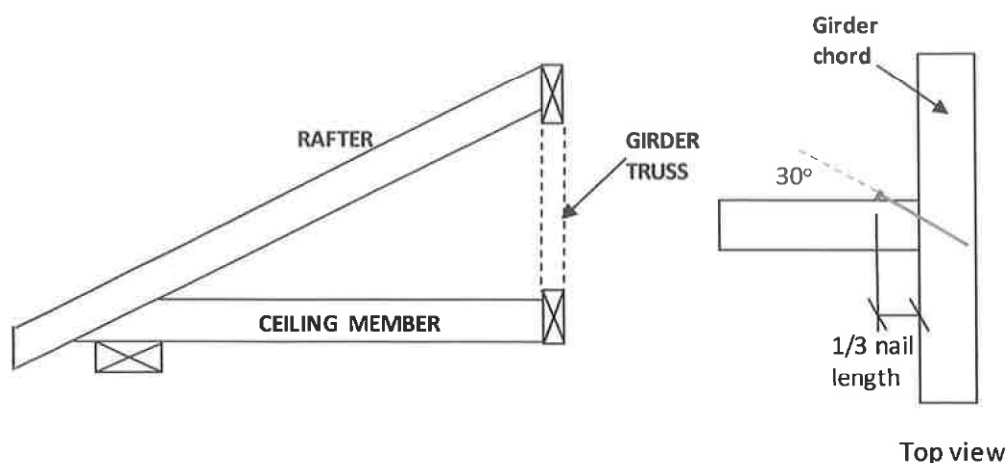


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

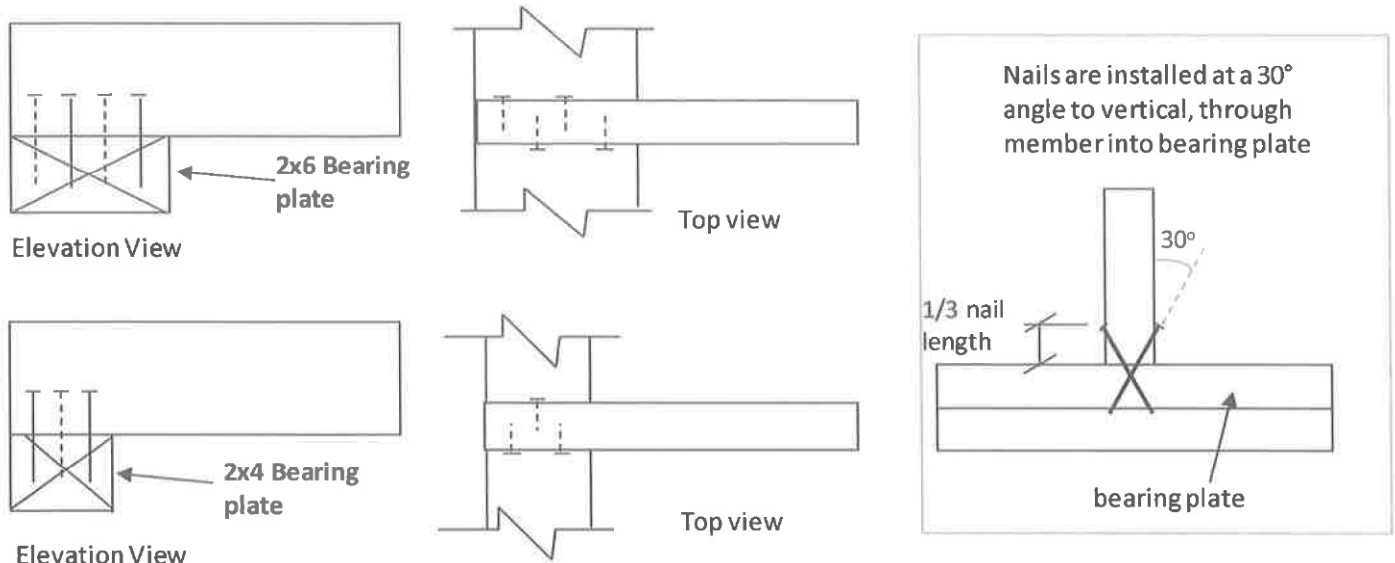




STANDARD DETAIL MSD2015-H

Issued: MARCH 1, 2022

Expiry: APRIL 30, 2024

MHP 23019**TOE-NAIL CAPACITY DETAILS****Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift****NOTES:**

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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STANDARD DETAIL MSD2015-J

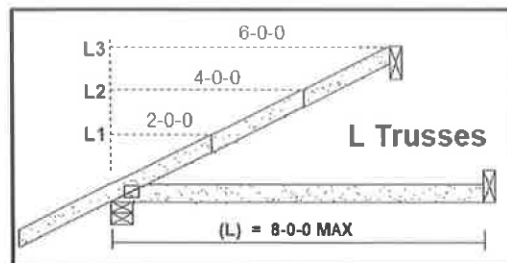
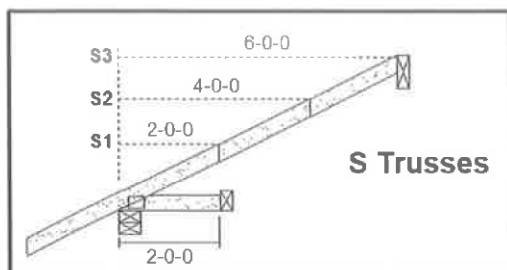
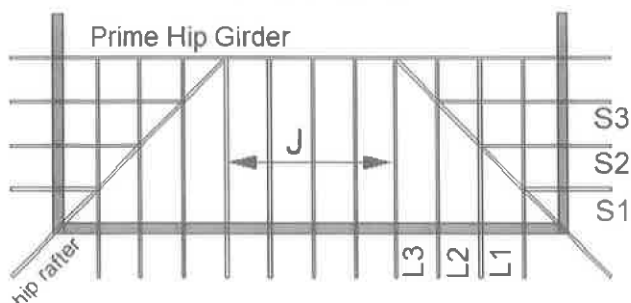
Issued: MARCH 17, 2021

Expiry: APRIL 30, 2023

MHP 23019

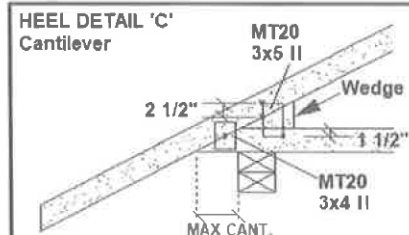
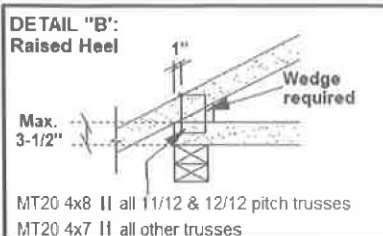
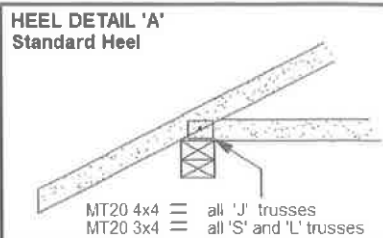
STANDARD HIP END FRAMING

PLAN VIEW



Specified Load Rating:

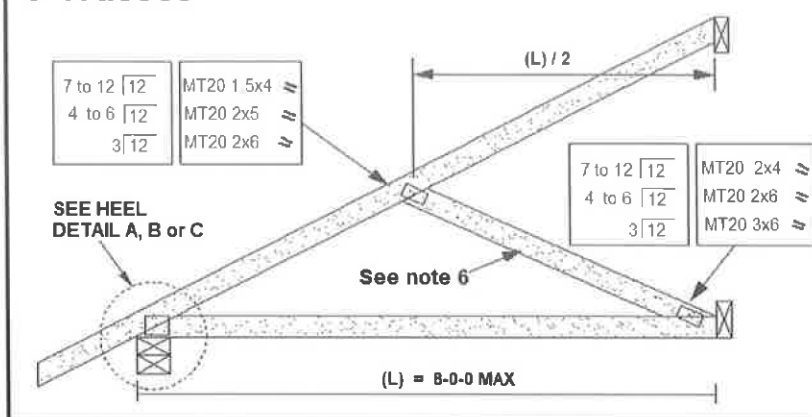
Top chord Live:	51.0 PSF or less
Top chord Dead:	6.0 PSF or less
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF or less



CANTILEVER DETAIL "C"

SLOPE	MAX CANT.	WEDGE PLATE	WEDGE SIZE
3/12	17"	3 X 5	2 X 3
4/12	14"	3 X 5	2 X 3
5/12	12"	3 X 5	2 X 4
6/12	10"	3 X 5	2 X 4
7/12	9"	3 X 5	2 X 6
8/12	8 5/8"	3 X 5	2 X 6
9/12	8"	3 X 5	2 X 6
10/12	7 5/8"	3 X 5	2 X 6

J Trusses



PEO

Certificate No. 10889485

NOTES:

1. This detail is valid only for projects conforming to **PART 9 NBCC 2015** that do not require a wind analysis to be incorporated into the design of the trusses.
2. Overhang length shall not exceed 24 inches.
3. All lumber shall be 2x4 SPF (or D-Fir) DRY No. 2 grade or better.
4. All plates specified are MITEK MT20, pressed into both faces of each truss. Heel plates of all trusses shall conform to heel details 'A', 'B' or 'C'.
5. Diagonal hip rafter design shall conform to section 9.23.14.6 of NBCC 2015.
6. For 6.0 ft. or less span, diagonal web on truss 'J' is optional. Girder design must reflect choice of partial jack ('J' with diagonal web) or open jack ('J' without diagonal web)
7. All truss-to-rafter and truss-to-truss connections shall be specified as per MITEK standard detail 'MSD2015-H: Toe-Nail Capacity Details'

