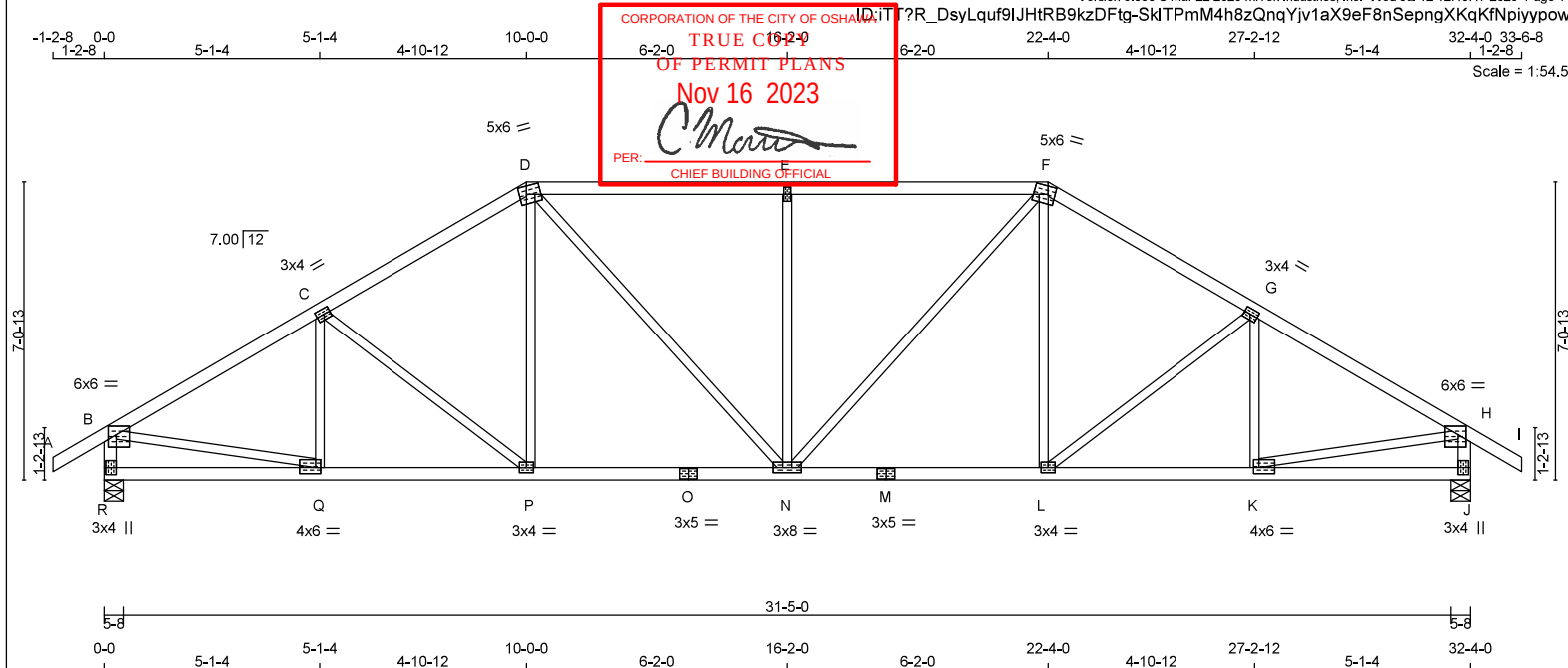


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-061	T02	1	1	TRUSS DESC.	CAROL 12-1	

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TOTAL WEIGHT = 136 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	6.0	2.50	2.25
C	TMVW-H	MT20	3.0	4.0	1.50	1.75
D	TTVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-H	MT20	3.0	4.0	1.50	1.75
H	TMVW-p	MT20	6.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0	2.00	
K	BMVW-4	MT20	4.0	6.0	1.75	1.50
L	BMVW-4	MT20	3.0	4.0		
M	BS-4	MT20	3.0	5.0		
N	BMVW-WH	MT20	3.0	8.0		
O	BS-4	MT20	3.0	5.0		
P	BMVW-4	MT20	3.0	4.0		
Q	BMVW-4	MT20	4.0	6.0	1.75	1.50
R	BMV1+p	MT20	3.0	4.0	2.00	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	2379	0	2379	0
J	2379	0	2379	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
R	1660	1215 / 0	0 / 0
J	1660	1215 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 39	-119.4	-119.4 0.14 (1)	Q-C	-370 / 0
B-C	-2916 / 0	-119.4	-119.4 0.51 (1)	C-P	-345 / 0
C-D	-2673 / 0	-119.4	-119.4 0.48 (1)	P-D	0 / 320
D-E	-2743 / 0	-119.4	-119.4 0.71 (1)	D-N	0 / 686
E-F	-2743 / 0	-119.4	-119.4 0.71 (1)	N-E	-904 / 0
F-G	-2673 / 0	-119.4	-119.4 0.48 (1)	N-F	0 / 686
G-H	-2916 / 0	-119.4	-119.4 0.51 (1)	L-F	0 / 320
H-I	0 / 39	-119.4	-119.4 0.14 (1)	L-G	-345 / 0
R-B	-2336 / 0	0.0	0.0 0.24 (1)	K-G	-370 / 0
J-H	-2336 / 0	0.0	0.0 0.24 (1)	B-G	0 / 2589
R-Q	0 / 0	-18.2	-18.2 0.10 (4)	K-H	0 / 2589
Q-P	0 / 2548	-18.2	-18.2 0.48 (1)		
P-O	0 / 2281	-18.2	-18.2 0.43 (1)		
O-N	0 / 2281	-18.2	-18.2 0.43 (1)		
N-M	0 / 2281	-18.2	-18.2 0.43 (1)		
M-L	0 / 2281	-18.2	-18.2 0.43 (1)		
L-K	0 / 2548	-18.2	-18.2 0.48 (1)		
K-J	0 / 0	-18.2	-18.2 0.10 (4)		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")CSI: TC=0.71/1.00 (E-F:1), BC=0.48/1.00 (K-L:1),
WB=0.78/1.00 (E-N:1), SS=0.36/1.00 (E-F:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

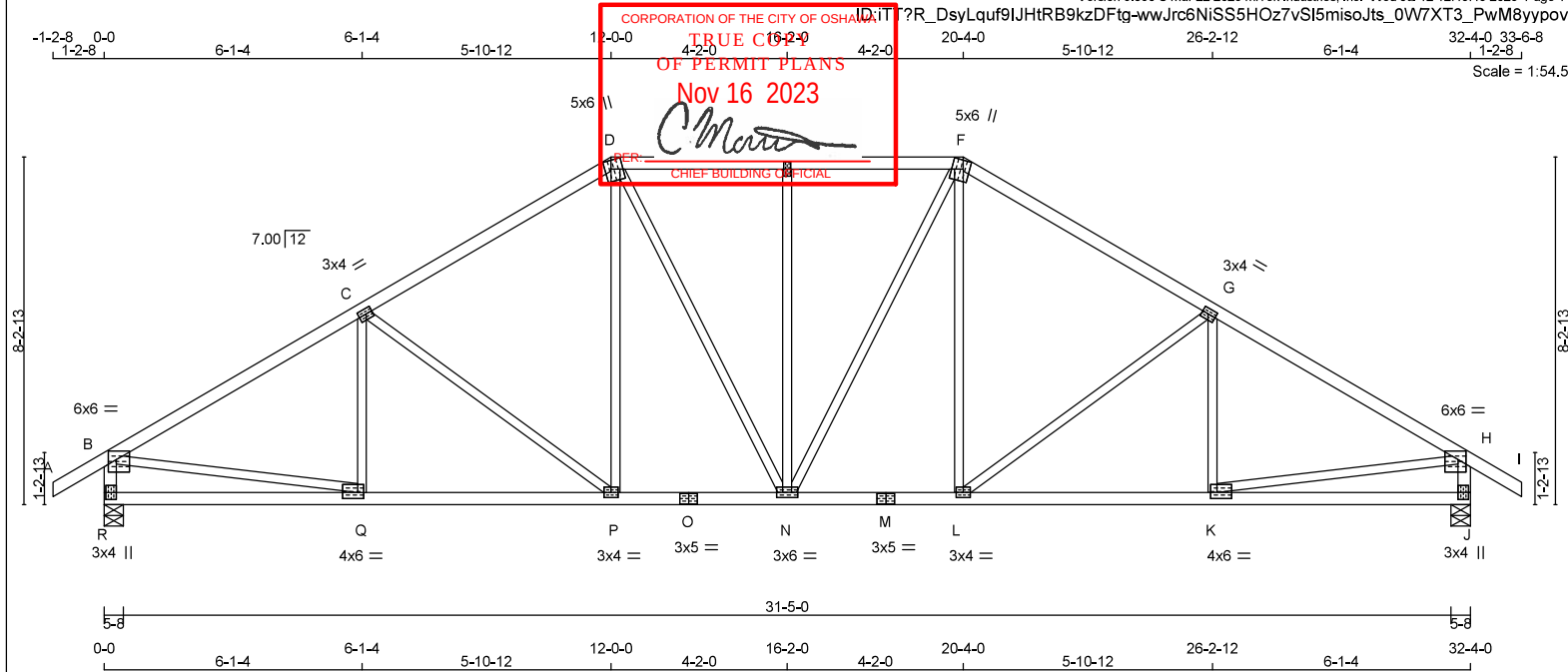
JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL = 0.72 (O) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-061	T03	1	1	TRUSS DESC.	CAROL 12-1	

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TOTAL WEIGHT = 144 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	6.0	2.50	2.25
C	TMVW-H	MT20	3.0	4.0	1.50	1.75
D	TTVW+m	MT20	5.0	6.0	2.50	1.75
E	TMVW+H	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.50	1.75
G	TMVW-H	MT20	3.0	4.0	1.50	1.75
H	TMVW-p	MT20	6.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0	2.00	
K	BMVW-4	MT20	4.0	6.0	1.75	1.75
L	BMVW-4	MT20	3.0	4.0		
M	BS-4	MT20	3.0	5.0		
N	BMVW-H	MT20	3.0	6.0		
O	BS-4	MT20	3.0	5.0		
P	BMVW-4	MT20	3.0	4.0		
Q	BMVW-4	MT20	4.0	6.0	1.75	1.75
R	BMV1+p	MT20	3.0	4.0	2.00	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2379	0	2379	0	0	5-8	4-4
J	2379	0	2379	0	0	5-8	4-4

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1660	1215 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0
J	1660	1215 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO					FR-TO		
A-B	0 / 39	-119.4	-119.4 0.14 (1)	10.00	Q-C	-271 / 36	0.09 (1)
B-C	-2950 / 0	-119.4	-119.4 0.75 (1)	3.28	C-P	-583 / 0	0.65 (1)
C-D	-2498 / 0	-119.4	-119.4 0.67 (1)	3.61	P-D	0 / 447	0.10 (1)
D-E	-2296 / 0	-119.4	-119.4 0.31 (1)	4.23	D-N	0 / 372	0.08 (1)
E-F	-2296 / 0	-119.4	-119.4 0.31 (1)	4.23	N-E	-599 / 0	0.79 (1)
F-G	-2498 / 0	-119.4	-119.4 0.67 (1)	3.61	N-F	0 / 372	0.08 (1)
G-H	-2950 / 0	-119.4	-119.4 0.75 (1)	3.28	L-F	0 / 447	0.10 (1)
H-I	0 / 39	-119.4	-119.4 0.14 (1)	10.00	L-G	-583 / 0	0.65 (1)
R-B	-2331 / 0	0.0	0.0 0.24 (1)	5.59	K-G	-271 / 36	0.09 (1)
J-H	-2331 / 0	0.0	0.0 0.24 (1)	5.59	B-Q	0 / 2614	0.59 (1)
					K-H	0 / 2614	0.59 (1)
R-Q	0 / 0	-18.2	-18.2 0.16 (4)	10.00			
Q-P	0 / 2585	-18.2	-18.2 0.48 (1)	10.00			
P-O	0 / 2124	-18.2	-18.2 0.40 (1)	10.00			
O-N	0 / 2124	-18.2	-18.2 0.40 (1)	10.00			
N-M	0 / 2124	-18.2	-18.2 0.40 (1)	10.00			
M-L	0 / 2124	-18.2	-18.2 0.40 (1)	10.00			
L-K	0 / 2585	-18.2	-18.2 0.48 (1)	10.00			
K-J	0 / 0	-18.2	-18.2 0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.75/1.00 (G-H:1) , BC=0.48/1.00 (P-Q:1) ,
WB=0.79/1.00 (E-N:1) , SS=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PL)
(PSI) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

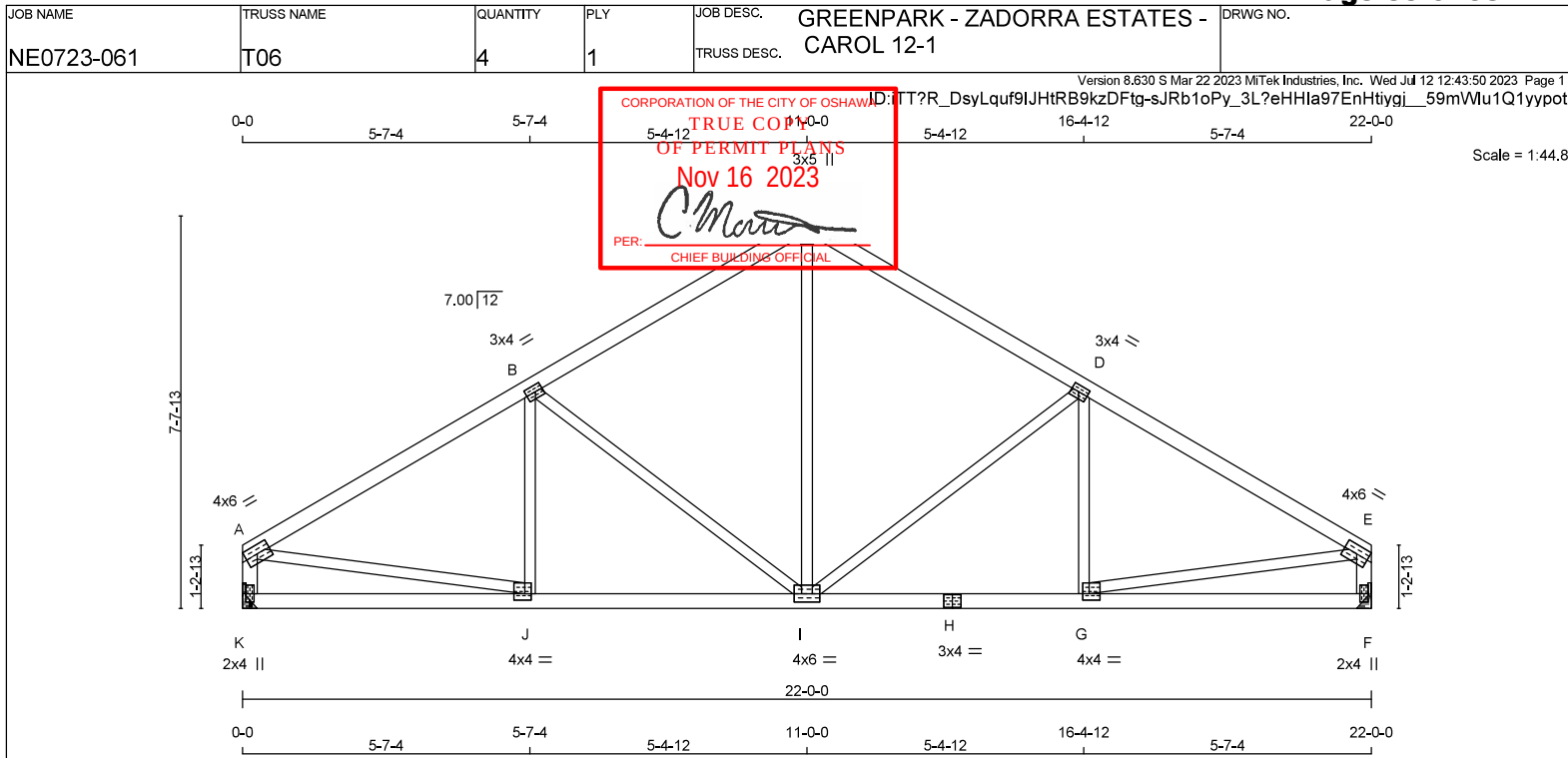
JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.62 (O) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-H	MT20	4.0	6.0		Edge
B	TMVW-H	MT20	3.0	4.0	1.50	1.75
C	TMV+p	MT20	3.0	5.0		
D	TMVW-H	MT20	3.0	4.0	1.50	1.75
E	TMVW-H	MT20	4.0	6.0		Edge
F	BMV1+p	MT20	2.0	4.0		
G	BMVW-H	MT20	4.0	4.0	1.50	1.50
H	BS-t	MT20	3.0	4.0		
I	BMVW-H	MT20	4.0	6.0		
J	BMVW-H	MT20	4.0	4.0	1.50	1.50
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1514	0	1514	0	0	MECHANICAL	MECHANICAL
F	1514	0	1514	0	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, F. MINIMUM BEARING LENGTH
AT JOINT K = 1-10, JOINT F = 1-10.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1058	766 / 0	0 / 0	0 / 0	0 / 0	293 / 0	0 / 0
F	1058	766 / 0	0 / 0	0 / 0	0 / 0	293 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	-1840 / 0	-119.4	-119.4 0.52 (1)	4.35	I-C	0 / 847	0.19 (1)
B-C	-1381 / 0	-119.4	-119.4 0.49 (1)	4.94	I-D	-610 / 0	0.53 (1)
C-D	-1381 / 0	-119.4	-119.4 0.49 (1)	4.94	G-D	-160 / 55	0.05 (1)
D-E	-1840 / 0	-119.4	-119.4 0.52 (1)	4.35	B-I	-610 / 0	0.53 (1)
K-A	-1470 / 0	0.0	0.0 0.15 (1)	6.75	J-B	-160 / 55	0.05 (1)
F-E	-1470 / 0	0.0	0.0 0.15 (1)	6.75	A-J	0 / 1644	0.37 (1)
					G-E	0 / 1644	0.37 (1)
K-J	0 / 0	-18.2	-18.2 0.13 (4)	10.00			
J-I	0 / 1623	-18.2	-18.2 0.32 (1)	10.00			
I-H	0 / 1623	-18.2	-18.2 0.32 (1)	10.00			
H-G	0 / 1623	-18.2	-18.2 0.32 (1)	10.00			
G-F	0 / 0	-18.2	-18.2 0.13 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL
BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.73")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.73")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.11")

CSI: TC=0.52/1.00 (D-E:1), BC=0.32/1.00 (G-I:1),
WB=0.53/1.00 (D-I:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.55 (J) (INPUT = 1.00)

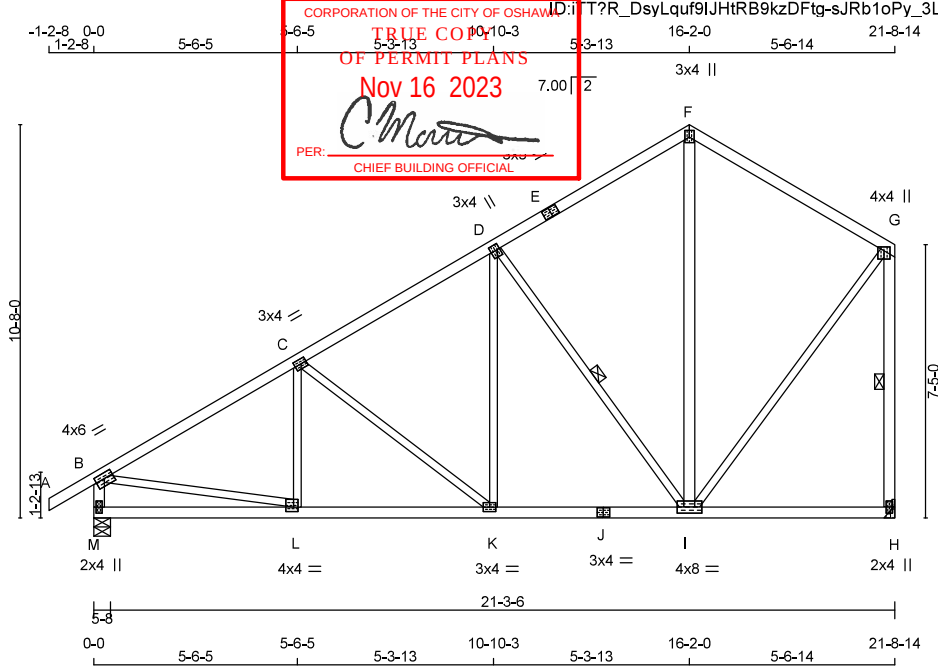


READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-061	T07	3	1	TRUSS DESC.	CAROL 12-1	

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Scale = 1:62.5

TOTAL WEIGHT = 3 X 108 = 324 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
I - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	6.0	1.75	3.00
C	TMVW4	MT20	3.0	4.0	1.50	1.75
D	TMVW4	MT20	3.0	4.0	1.75	0.75
E	TS4	MT20	3.0	5.0		
F	TTW+p	MT20	3.0	4.0	2.25	1.50
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMVW4	MT20	4.0	8.0		
J	BS4	MT20	3.0	4.0		
K	BMVW4	MT20	3.0	4.0		
L	BMVW4	MT20	4.0	4.0	1.50	1.50
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
M	1650	0	1650	0
H	1497	0	1497	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
M	1150	846 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1046	757 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D4, G-H. DBS = 20-0-0. CBF = 182 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 39	-119.4	-119.4 0.14 (1)	10.00	L-C	-154 / 55	0.05 (1)
B-C	-1799 / 0	-119.4	-119.4 0.48 (1)	4.47	C-K	-518 / 0	0.44 (1)
C-D	-1358 / 0	-119.4	-119.4 0.39 (1)	5.11	K-D	0 / 408	0.09 (1)
D-E	-731 / 0	-119.4	-119.4 0.39 (1)	6.25	D-I	-959 / 0	0.53 (1)
E-F	-731 / 0	-119.4	-119.4 0.39 (1)	6.25	I-F	0 / 117	0.03 (4)
F-G	-700 / 0	-119.4	-119.4 0.48 (1)	6.25	B-L	0 / 1602	0.36 (1)
M-B	-1607 / 0	0.0	0.0 0.16 (1)	6.52	I-G	0 / 979	0.22 (1)
H-G	-1456 / 0	0.0	0.0 0.37 (1)	5.42			
M-L	0 / 0	-18.2	-18.2 0.13 (4)	10.00			
L-K	0 / 1581	-18.2	-18.2 0.31 (1)	10.00			
K-J	0 / 1172	-18.2	-18.2 0.27 (1)	10.00			
J-I	0 / 1172	-18.2	-18.2 0.27 (1)	10.00			
I-H	0 / 0	-18.2	-18.2 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	8.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.48/1.00 (F-G:1), BC=0.31/1.00 (K-L:1), WB=0.53/1.00 (D-I:1), SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

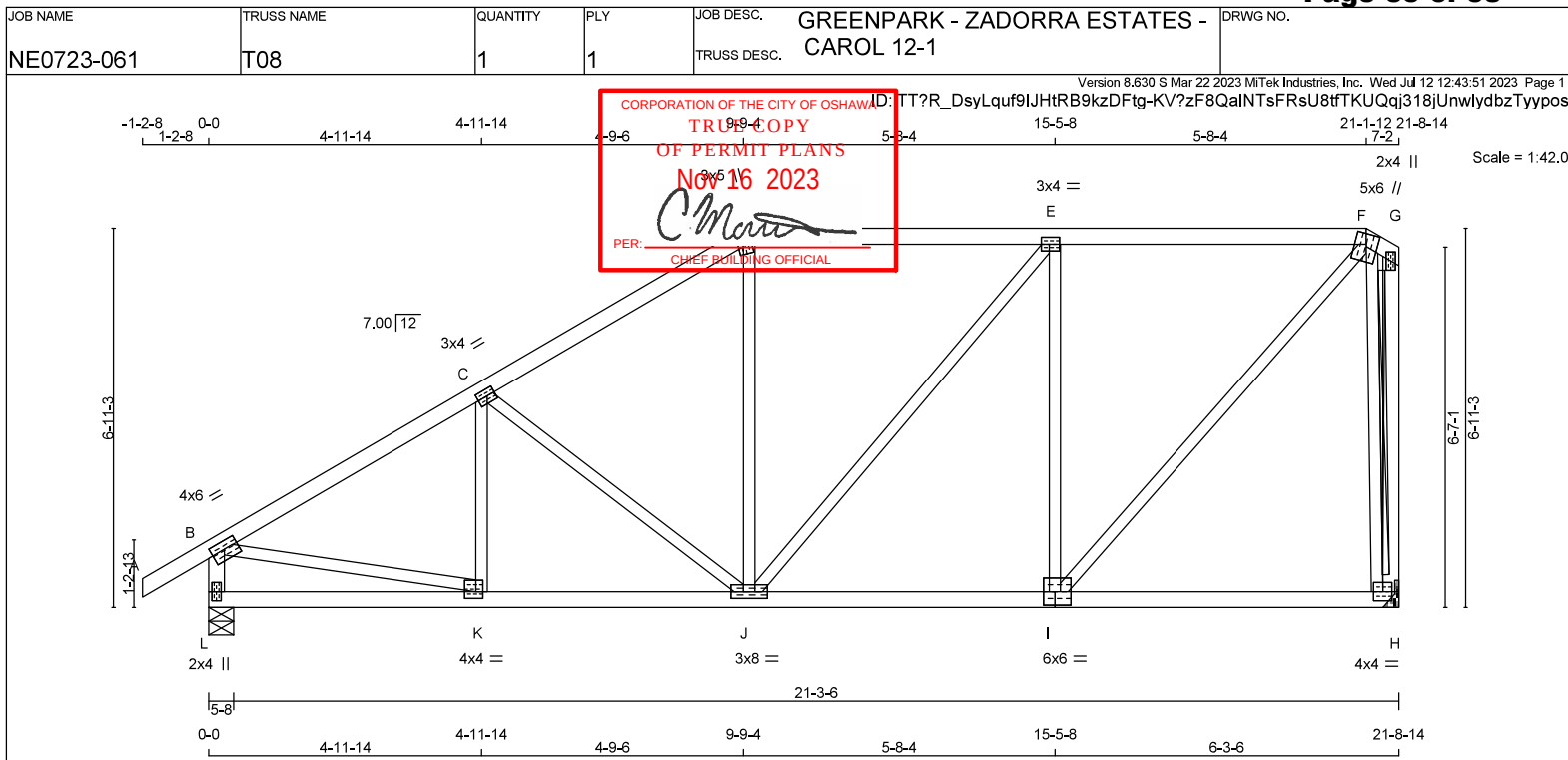
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL = 0.54 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	6.0	1.75	3.00
C	TMVW4	MT20	3.0	4.0	1.50	1.75
D	TTW+m	MT20	3.0	5.0	2.75	1.25
E	TMVW4	MT20	3.0	4.0	1.50	1.75
F	TTWV+m	MT20	5.0	6.0	2.25	2.50
G	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BSVW4	MT20	6.0	6.0	3.00	2.50
J	BMVWV4	MT20	3.0	8.0		
K	BMVW4	MT20	4.0	4.0	1.50	1.50
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	1650	0	1650	0
H	1497	0	1497	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	1150	846 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1046	757 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	(PLF)	(LBS)	(LBS)	(LBS)	UNBRAC	FORCE	FORCE	CSI (LC)
FR-TO	FROM	TO	FROM	TO	LENGTH	FR-TO	FROM	TO	CSI (LC)
A-B	0 / 39	-119.4	-119.4	0.14 (1)	10.00	K-C	-205 / 27	0.05 (1)	
B-C	-1814 / 0	-119.4	-119.4	0.57 (1)	4.28	C-J	-483 / 0	0.33 (1)	
C-D	-1445 / 0	-119.4	-119.4	0.53 (1)	4.72	J-D	0 / 238	0.05 (1)	
D-E	-1220 / 0	-119.4	-119.4	0.71 (1)	4.81	J-E	0 / 222	0.05 (1)	
E-F	-1077 / 0	-119.4	-119.4	0.70 (1)	4.86	I-E	-1010 / 0	0.83 (1)	
F-G	0 / 0	-119.4	-119.4	0.01 (1)	10.00	I-F	0 / 1461	0.33 (1)	
L-B	-1609 / 0	0.0	0.0	0.16 (1)	6.52	B-K	0 / 1623	0.37 (1)	
H-G	-35 / 0	0.0	0.0	0.03 (1)	7.81	F-H	-1409 / 0	0.47 (1)	
L-K	0 / 0	-18.2	-18.2	0.09 (4)	10.00				
K-J	0 / 1596	-18.2	-18.2	0.32 (1)	10.00				
J-I	0 / 1077	-18.2	-18.2	0.24 (1)	10.00				
I-H	0 / 126	-18.2	-18.2	0.22 (4)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.71/1.00 (D-E:1), BC=0.32/1.00 (J-K:1),
WB=0.83/1.00 (E-I:1), SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(FSD)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

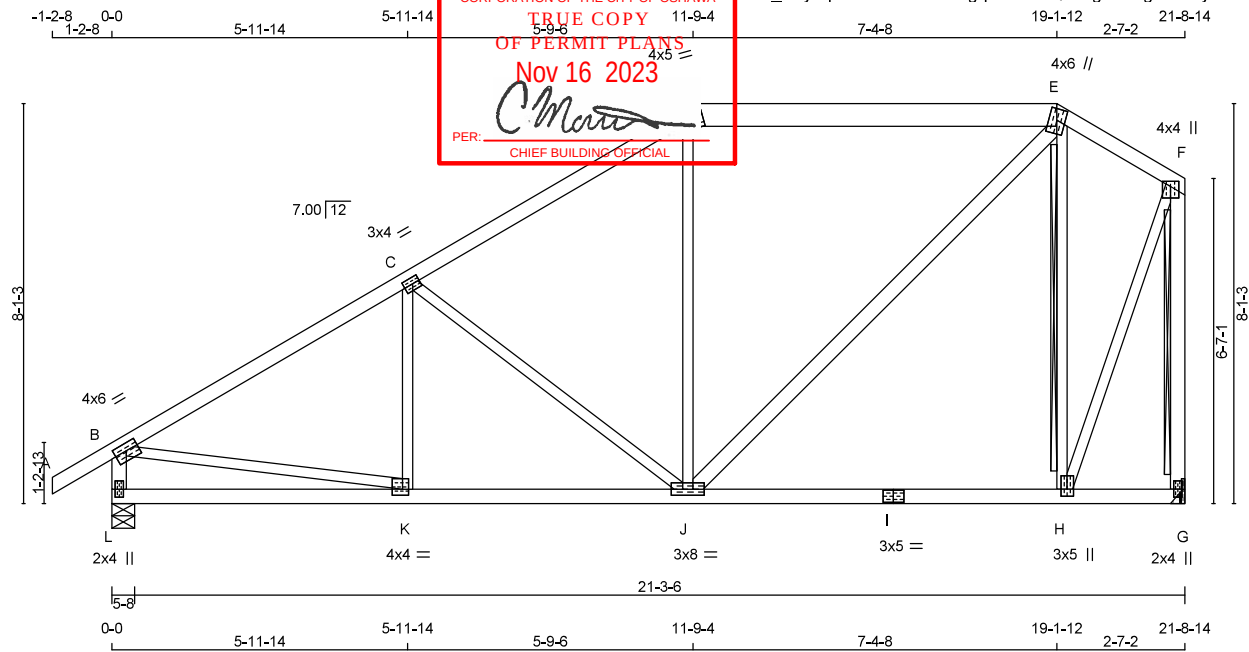
JSI GRIP= 0.90 (D) (INPUT = 0.90)

JSI METAL= 0.54 (K) (INPUT = 1.00)



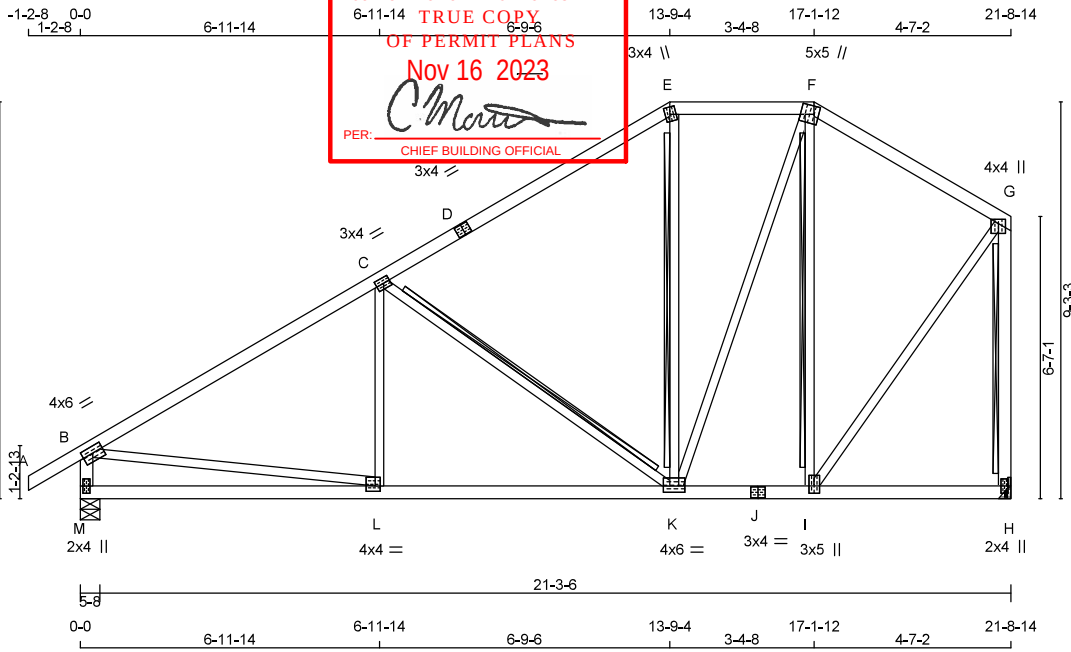
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-061	T10	1	1	TRUSS DESC.	CAROL 12-1	

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Scale = 1:53.8

TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - E	2x4	DRY No.2
E - F	2x4	DRY No.2
F - G	2x4	DRY No.2
M - B	2x4	DRY No.2
H - G	2x4	DRY No.2
M - J	2x4	DRY No.2
J - H	2x4	DRY No.2

ALL WEBS
EXCEPT

DRY: SEASONED LUMBER.

DESCR.

SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING****DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
M	1650	0	1650	0
H	1497	0	1497	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1150	846 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1046	757 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-K, E-K, F-I, G-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 39	-119.4	-119.4 0.14 (1)	10.00	L-C	-62 / 110	0.04 (4)
B-C	-1775 / 0	-119.4	-119.4 0.85 (1)	3.89	C-K	-910 / 0	0.51 (1)
C-D	-1024 / 0	-119.4	-119.4 0.77 (1)	4.81	K-E	-13 / 64	0.02 (4)
D-E	-1024 / 0	-119.4	-119.4 0.77 (1)	4.81	K-F	0 / 671	0.15 (1)
E-F	-840 / 0	-119.4	-119.4 0.18 (1)	6.25	I-F	-745 / 0	0.47 (1)
F-G	-718 / 0	-119.4	-119.4 0.33 (1)	6.25	B-L	0 / 1590	0.36 (1)
M-B	-1598 / 0	0.0	0.0 0.16 (1)	6.53	I-G	0 / 1031	0.23 (1)
H-G	-1462 / 0	0.0	0.0 0.36 (1)	7.81			
M-L	0 / 0	-18.2	-18.2 0.22 (4)	10.00			
L-K	0 / 1576	-18.2	-18.2 0.37 (1)	10.00			
K-J	0 / 609	-18.2	-18.2 0.15 (1)	10.00			
J-I	0 / 609	-18.2	-18.2 0.15 (1)	10.00			
I-H	0 / 0	-18.2	-18.2 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL)= L/999 (0.12")

CSI: TC=0.85/1.00 (B-C:1) , BC=0.37/1.00 (K-L:1) , WB=0.51/1.00 (C-K:1) , SS=0.35/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PSI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)

JSI METAL= 0.54 (L) (INPUT = 1.00)

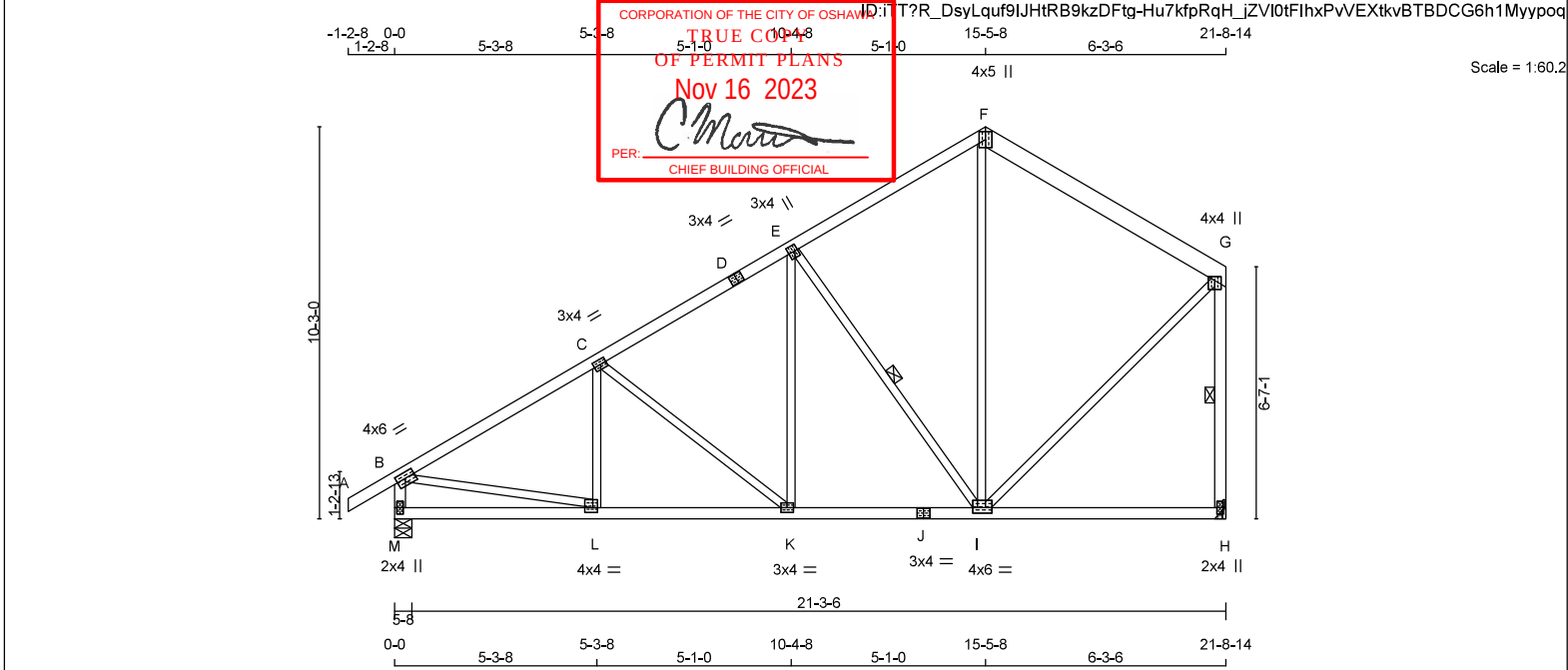


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-061	T11	5	1	TRUSS DESC.	CAROL 12-1	

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TOTAL WEIGHT = 5 X 107 = 536 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x6	DRY	No.2
M - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW4	MT20	4.0	6.0	1.75 3.00
C	TMVW4	MT20	3.0	4.0	1.50 1.75
D	TS4	MT20	3.0	4.0	
E	TMVW+1	MT20	3.0	4.0	2.00 0.75
F	TTW+p	MT20	4.0	5.0	
G	TMVW+p	MT20	4.0	4.0	1.25 2.00
H	BMV1+p	MT20	2.0	4.0	
I	BMVWVW4	MT20	4.0	6.0	1.75 1.50
J	BS4	MT20	3.0	4.0	
K	BMVW4	MT20	3.0	4.0	
L	BMVW4	MT20	4.0	4.0	1.50 1.50
M	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1650	0	1650	0	0	5-8	1-14
H	1497	0	1497	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1150	846 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1046	757 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF E-I, G-H, DBS = 20-0-0. CBF = 181 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 39	-119.4	-119.4 0.14 (1)	10.00	L-C	-170 / 48	0.05 (1)
B-C	-1802 / 0	-119.4	-119.4 0.44 (1)	4.52	C-K	-474 / 0	0.36 (1)
C-D	-1402 / 0	-119.4	-119.4 0.38 (1)	5.10	K-E	0 / 372	0.08 (1)
D-E	-1402 / 0	-119.4	-119.4 0.38 (1)	5.10	E-I	-893 / 0	0.45 (1)
E-F	-826 / 0	-119.4	-119.4 0.38 (1)	6.21	I-F	0 / 185	0.05 (4)
F-G	-801 / 0	-119.4	-119.4 0.30 (1)	6.25	B-L	0 / 1606	0.36 (1)
M-B	-1608 / 0	0.0	0.0 0.16 (1)	6.52	I-G	0 / 959	0.22 (1)
H-G	-1449 / 0	0.0	0.0 0.28 (1)	5.43			
M-L	0 / 0	-18.2	-18.2 0.12 (4)	10.00			
L-K	0 / 1582	-18.2	-18.2 0.30 (1)	10.00			
K-J	0 / 1210	-18.2	-18.2 0.29 (1)	10.00			
J-I	0 / 1210	-18.2	-18.2 0.29 (1)	10.00			
I-H	0 / 0	-18.2	-18.2 0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.44/1.00 (B-C:1), BC=0.30/1.00 (K-L:1), WB=0.45/1.00 (E-I:1), SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

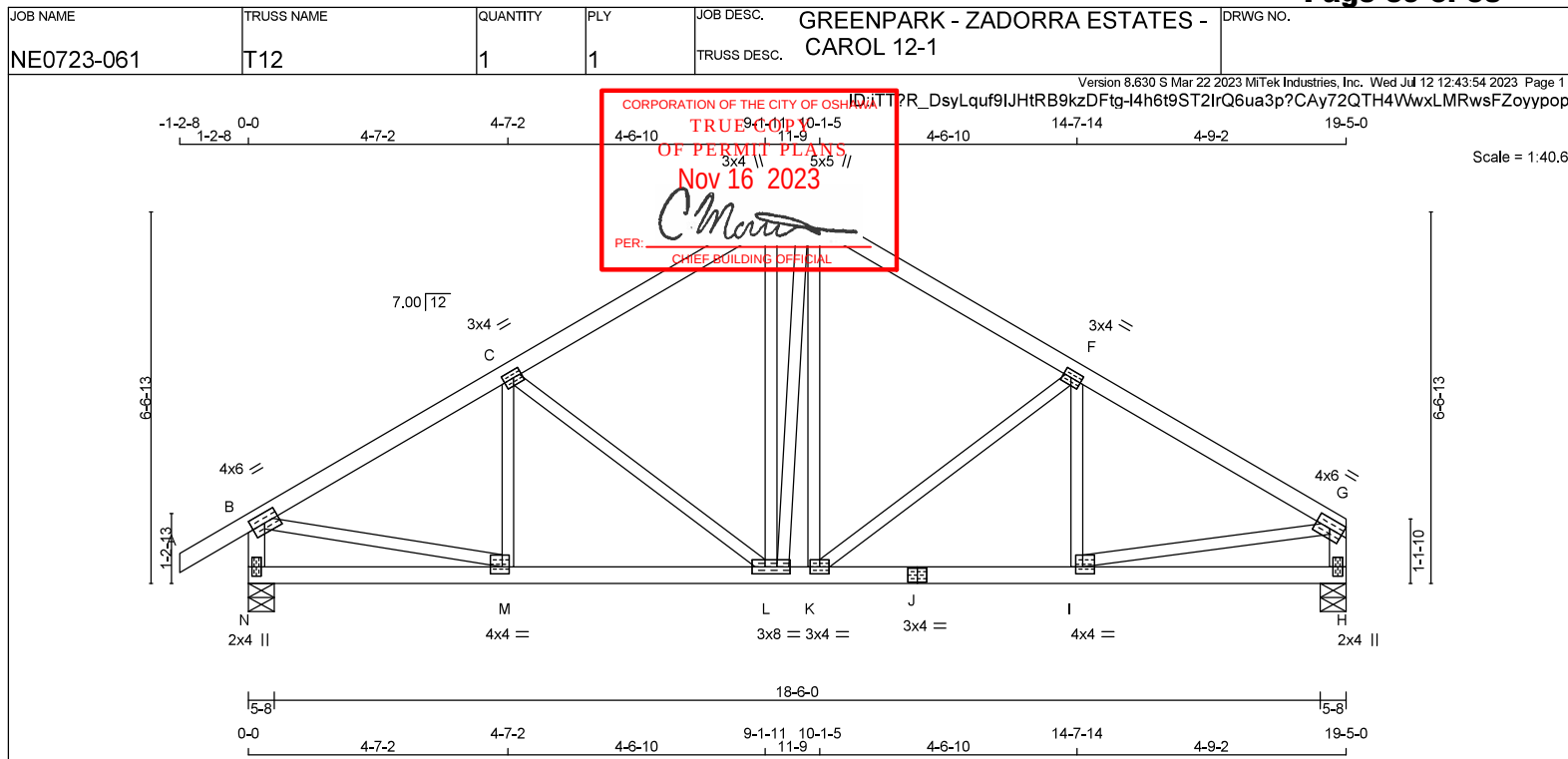
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.54 (L) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 87 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	6.0	2.00	3.00
C	TMVW4	MT20	3.0	4.0	1.50	1.75
D	TTW+m	MT20	3.0	4.0	2.00	1.25
E	TTW+m	MT20	5.0	5.0	2.50	1.25
F	TMVW4	MT20	3.0	4.0	1.50	1.75
G	TMVW4	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	2.0	4.0		
I	BMVW4	MT20	4.0	4.0	1.50	1.50
J	BS4	MT20	3.0	4.0		
K	BMVW4	MT20	3.0	4.0		
L	BMVW4	MT20	3.0	8.0		
M	BMVW4	MT20	4.0	4.0	1.50	1.50
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
N	1490	0	1490	0
H	1337	0	1337	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1039	765 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
H	934	676 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 39	-119.4	-119.4 0.14 (1)	10.00	M-C	-195 / 30	0.05 (1)
B-C	-1576 / 0	-119.4	-119.4 0.34 (1)	4.88	C-L	-450 / 0	0.27 (1)
C-D	-1231 / 0	-119.4	-119.4 0.33 (1)	5.40	L-D	0 / 341	0.08 (1)
D-E	-1039 / 0	-119.4	-119.4 0.02 (1)	6.15	L-E	0 / 30	0.01 (1)
E-F	-1228 / 0	-119.4	-119.4 0.34 (1)	5.38	K-E	0 / 312	0.07 (1)
F-G	-1620 / 0	-119.4	-119.4 0.36 (1)	4.82	K-F	-502 / 0	0.30 (1)
N-B	-1452 / 0	0.0	0.0 0.15 (1)	6.79	I-F	-151 / 45	0.04 (1)
H-G	-1299 / 0	0.0	0.0 0.13 (1)	7.08	B-M	0 / 1417	0.32 (1)
					I-G	0 / 1448	0.33 (1)
N-M	0 / 0	-18.2	-18.2 0.09 (4)	10.00			
M-L	0 / 1389	-18.2	-18.2 0.28 (1)	10.00			
L-K	0 / 1035	-18.2	-18.2 0.22 (1)	10.00			
K-J	0 / 1427	-18.2	-18.2 0.27 (1)	10.00			
J-I	0 / 1427	-18.2	-18.2 0.27 (1)	10.00			
I-H	0 / 0	-18.2	-18.2 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.3 PSF

TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.65")

CALCULATED VERT. DEFL.(LL) = L/999 (0.04")

ALLOWABLE DEFL.(TL) = L/360 (0.65")

CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.36/1.00 (F-G-1), BC=0.28/1.00 (L-M-1), WB=0.33/1.00 (G-I-1), SSI=0.23/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PL)

MT20 650 371 1747 788 1987 1873

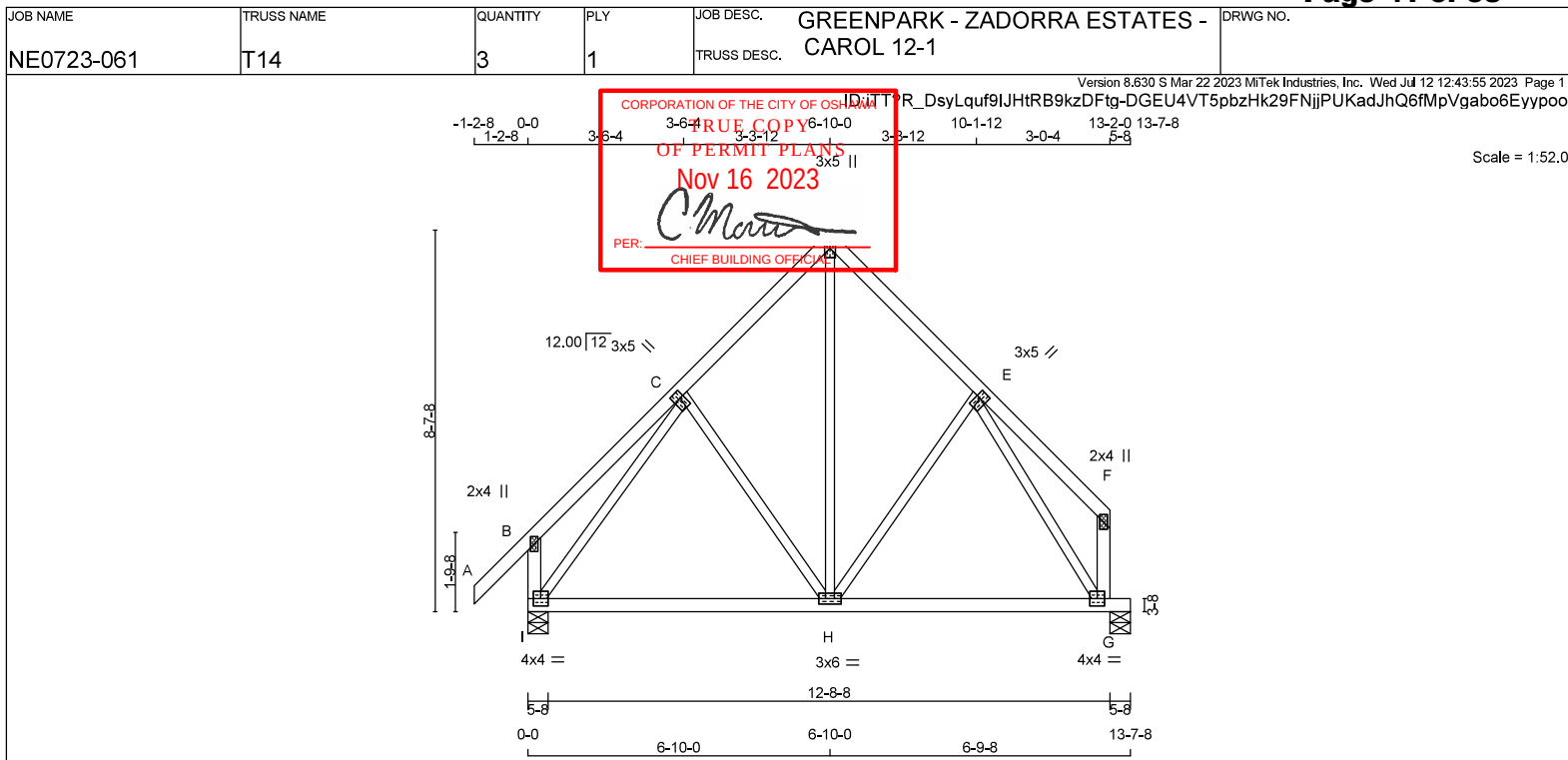
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (B) (INPUT = 0.90)
JSI METAL = 0.49 (I) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 68 = 203 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW+H	MT20	3.0	5.0	2.00	1.00
D	TMV+p	MT20	3.0	5.0		
E	TMVW+H	MT20	3.0	5.0	2.00	1.00
F	TMV+p	MT20	2.0	4.0		
G	BMVW1+H	MT20	4.0	4.0		
H	BMVW+H	MT20	3.0	6.0		
I	BMVW1+H	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
I	1063	1063	5-8	1-8
G	906	906	0	5-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	740	550 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0
G	633	458 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 55	-119.4	-119.4 0.15 (1)	10.00	C-H	-209 / 0	0.13 (1)
B-C	0 / 34	-119.4	-119.4 0.23 (1)	10.00	H-D	0 / 416	0.09 (1)
C-D	-558 / 0	-119.4	-119.4 0.17 (1)	6.25	H-E	-121 / 6	0.07 (1)
D-E	-554 / 0	-119.4	-119.4 0.18 (1)	6.25	I-C	-860 / 0	0.50 (1)
E-F	0 / 35	-119.4	-119.4 0.20 (1)	10.00	E-G	-862 / 0	0.45 (1)
I-B	-310 / 0	0.0	0.0 0.03 (1)	7.81			
G-F	-123 / 0	0.0	0.0 0.02 (1)	7.81			
I-H	0 / 493	-18.2	-18.2 0.26 (4)	10.00			
H-G	0 / 442	-18.2	-18.2 0.25 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")CSI: TC=0.23/1.00 (B-C-1), BC=0.26/1.00 (H-I-4),
WB=0.50/1.00 (C-I-1), SSI=0.14/1.00 (C-D-1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	371	788	1987

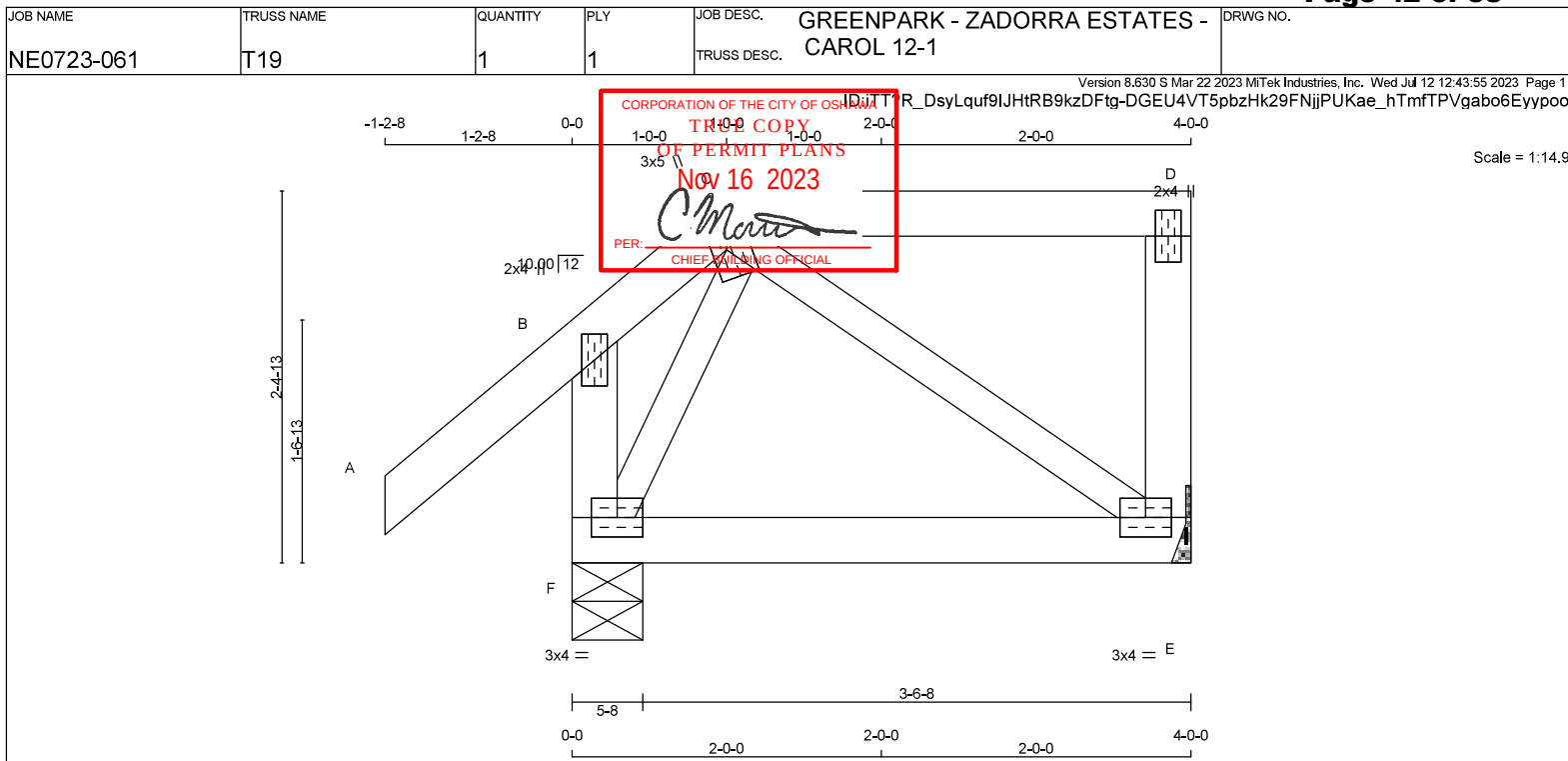
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL = 0.52 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
F - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TTWW+m	MT20	3.0	5.0	1.75	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW14	MT20	3.0	4.0		
F	BMVW14	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	250	0	250	0	0	MECHANICAL	
F	456	0	456	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	175	124 / 0	0 / 0	0 / 0	0 / 0	51 / 0	0 / 0
F	316	245 / 0	0 / 0	0 / 0	0 / 0	71 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MAX. UNBRAC LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)		
FR-TO		FR-TO		LENGTH	FR-TO				
A-B	0 / 50	-119.4	-119.4	0.15 (1)	10.00	C-E	-60 / 0	0.01 (1)	
B-C	-66 / 0	-119.4	-119.4	0.14 (1)	6.25	F-C	-114 / 0	0.02 (1)	
C-D	0 / 0	-119.4	-119.4	0.18 (1)	10.00				
E-D	-179 / 0	0.0	0.0	0.02 (1)	7.81				
F-B	-316 / 0	0.0	0.0	0.03 (1)	7.81				
F-E	0 / 49	-18.2	-18.2	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.18/1.00 (C-D:1), BC=0.09/1.00 (E-F:4),
WB=0.02/1.00 (C-F:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (C) (INPUT = 0.90)
JSI METAL = 0.17 (B) (INPUT = 1.00)

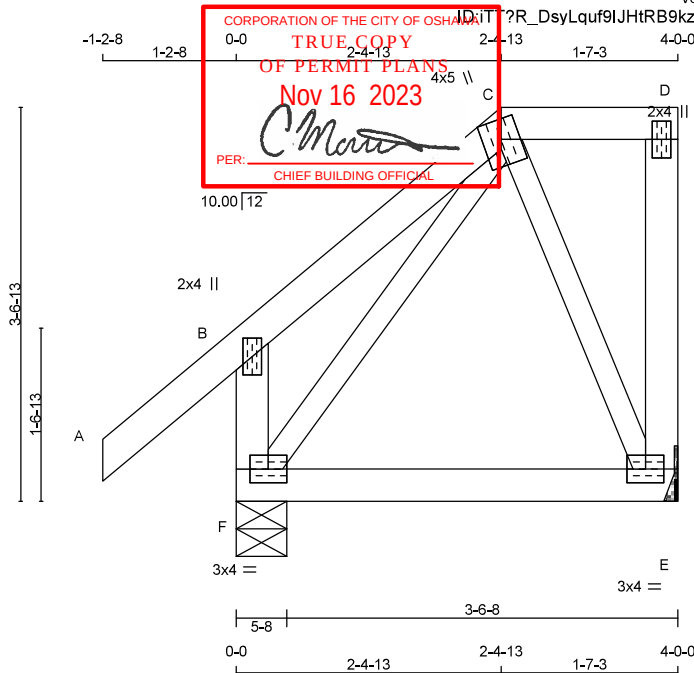


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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-061	T20	1	1	TRUSS DESC.	CAROL 12-1	

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Scale = 1:20.9

TOTAL WEIGHT = 22 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
F - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TTWW+m	MT20	4.0	5.0	Edge	
D	TMV+p	MT20	2.0	4.0		
E	BMVW14	MT20	3.0	4.0		
F	BMVW14	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	275	0	275	0	0	MECHANICAL	
F	431	0	431	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	192	139 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	299	230 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 50	-119.4	-119.4	0.15 (1)	10.00	C-E -160 / 0	0.04 (1)
B-C	0 / 0	-119.4	-119.4	0.12 (1)	10.00	F-C -119 / 0	0.03 (1)
C-D	0 / 0	-119.4	-119.4	0.05 (1)	10.00		
E-D	-96 / 0	0.0	0.0	0.02 (1)	7.81		
F-B	-299 / 0	0.0	0.0	0.03 (1)	7.81		
F-E	0 / 70	-18.2	-18.2	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.3 PSF
 TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")CSI: TC=0.15/1.00 (A-B:1) ; BC=0.09/1.00 (E-F:4) ,
WB=0.04/1.00 (C-E:1) , SSI=0.09/1.00 (A-B:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLD)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

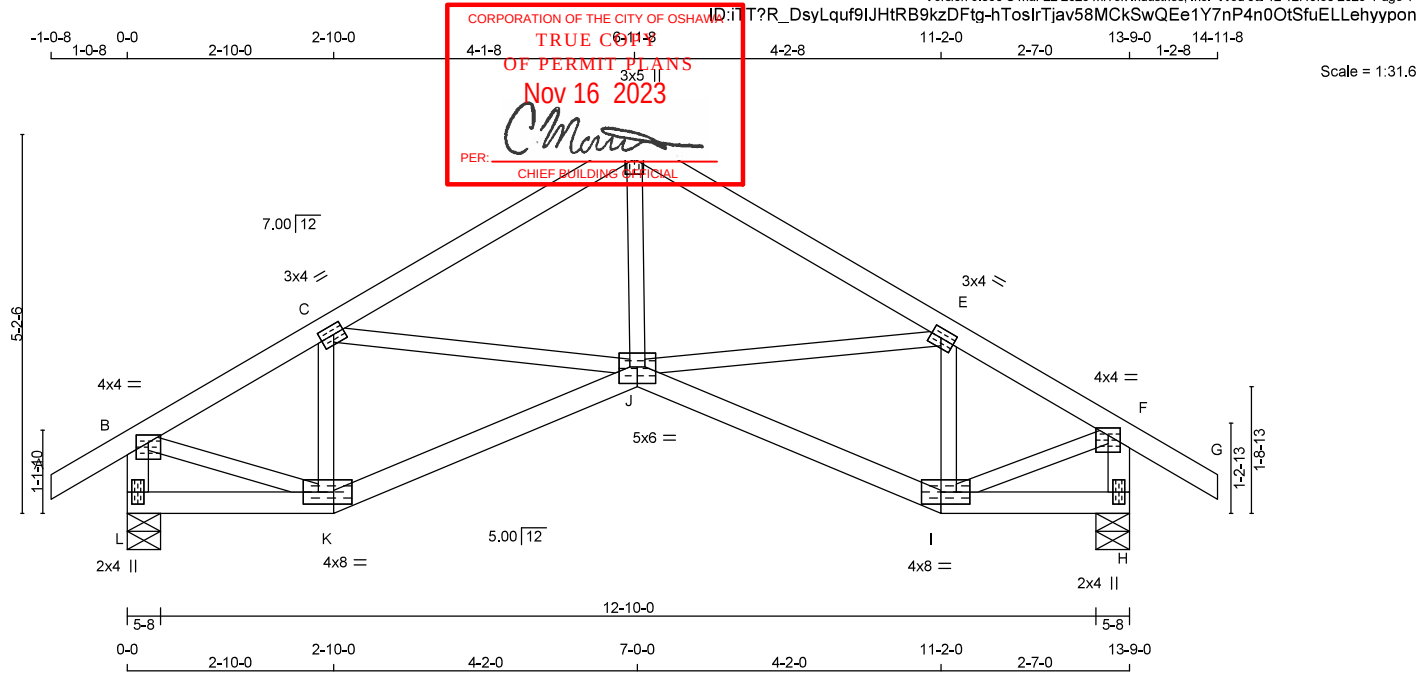
JSI GRIP= 0.28 (F) (INPUT = 0.90)
JSI METAL = 0.16 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
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 IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-061	T21	2	1	TRUSS DESC.	CAROL 12-1	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 12:43:56 2023 Page 1

**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.25	2.00
C	TMVW-H	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	5.0		
E	TMVW-H	MT20	3.0	4.0	1.50	1.75
F	TMVW-p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BBVW-p	MT20	4.0	8.0	2.00	4.75
J	BBVW-p	MT20	5.0	6.0	2.75	3.00
K	BBVW-p	MT20	4.0	8.0	2.00	5.00
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1080	0	1080	0
H	1100	0	1100	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
L	753	556 / 0
H	766	568 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0 / 34	-119.4	-119.4	0.11 (1)	10.00	K-C	-596 / 0	0.10 (1)	
B-C	-1064 / 0	-119.4	-119.4	0.21 (1)	5.87	C-J	0 / 111	0.03 (1)	
C-D	-1218 / 0	-119.4	-119.4	0.25 (1)	5.52	D-J	0 / 790	0.18 (1)	
D-E	-1230 / 0	-119.4	-119.4	0.27 (1)	5.48	J-E	0 / 163	0.04 (1)	
E-F	-1014 / 0	-119.4	-119.4	0.21 (1)	5.97	I-E	-630 / 0	0.10 (1)	
F-G	0 / 39	-119.4	-119.4	0.14 (1)	10.00	B-K	0 / 987	0.22 (1)	
L-B	-1054 / 0	0.0	0.0	0.11 (1)	7.66	I-F	0 / 965	0.22 (1)	
H-F	-1076 / 0	0.0	0.0	0.11 (1)	7.61				
L-K	0 / 0	-18.2	-18.2	0.04 (4)	10.00				
K-J	0 / 1000	-18.2	-18.2	0.21 (1)	10.00				
J-I	0 / 956	-18.2	-18.2	0.21 (1)	10.00				
I-H	0 / 0	-18.3	-18.3	0.04 (4)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.46")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.46")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.27/1.00 (D-E:1) , BC=0.21/1.00 (J-K:1) ,
 WB=0.22/1.00 (B-K:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

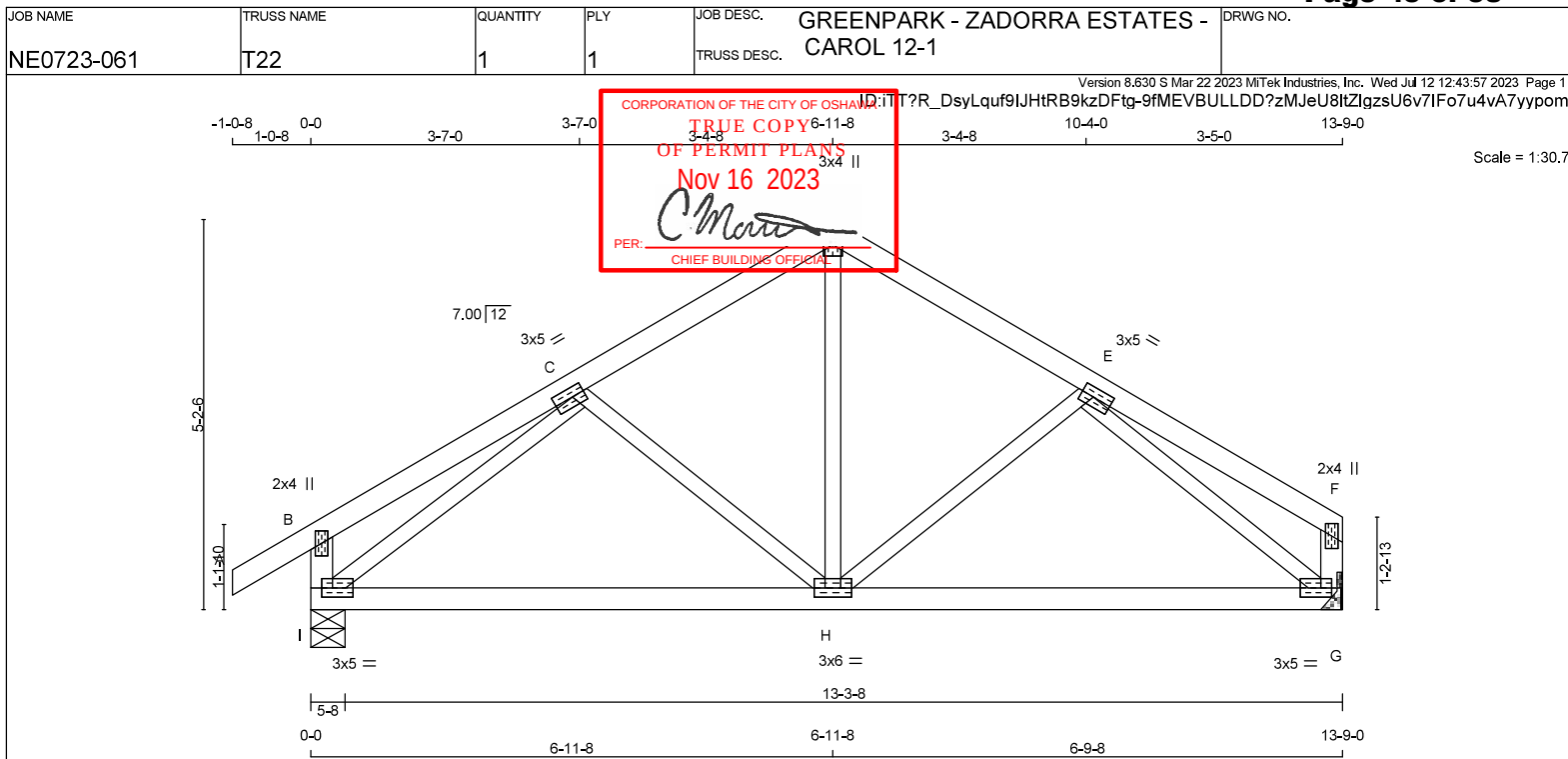
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
 JSI METAL = 0.32 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
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 IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 54 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-H	MT20	3.0	5.0	1.50	2.00
D	TTVW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-H	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-H	MT20	3.0	5.0	1.50	1.75
H	BMVW1-H	MT20	3.0	6.0		
I	BMVW1-H	MT20	3.0	5.0	1.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
I	1080	0	1080	0
G	947	0	947	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
I	753	556 / 0 0 / 0 0 / 0 0 / 0 196 / 0 0 / 0
G	661	479 / 0 0 / 0 0 / 0 0 / 0 183 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	
FR-TO							
A-B	0 / 34	-119.4	-119.4 0.11 (1)	10.00	C-H	-273 / 0	0.09 (1)
B-C	0 / 23	-119.4	-119.4 0.22 (1)	10.00	H-D	0 / 475	0.11 (1)
C-D	-818 / 0	-119.4	-119.4 0.18 (1)	6.25	H-E	-235 / 0	0.08 (1)
D-E	-816 / 0	-119.4	-119.4 0.17 (1)	6.25	I-C	-1160 / 0	0.38 (1)
E-F	0 / 23	-119.4	-119.4 0.21 (1)	10.00	E-G	-1145 / 0	0.35 (1)
I-B	-295 / 0	0.0	0.0 0.03 (1)	7.81			
G-F	-152 / 0	0.0	0.0 0.02 (1)	7.81			
I-H	0 / 897	-18.2	-18.2 0.30 (4)	10.00			
H-G	0 / 868	-18.2	-18.2 0.30 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = $L/360$ (0.46")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.46")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")CSI: TC=0.22/1.00 (B-C:1) , BC=0.30/1.00 (H-I:4) ,
WB=0.38/1.00 (C-I:1) , SSI=0.17/1.00 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

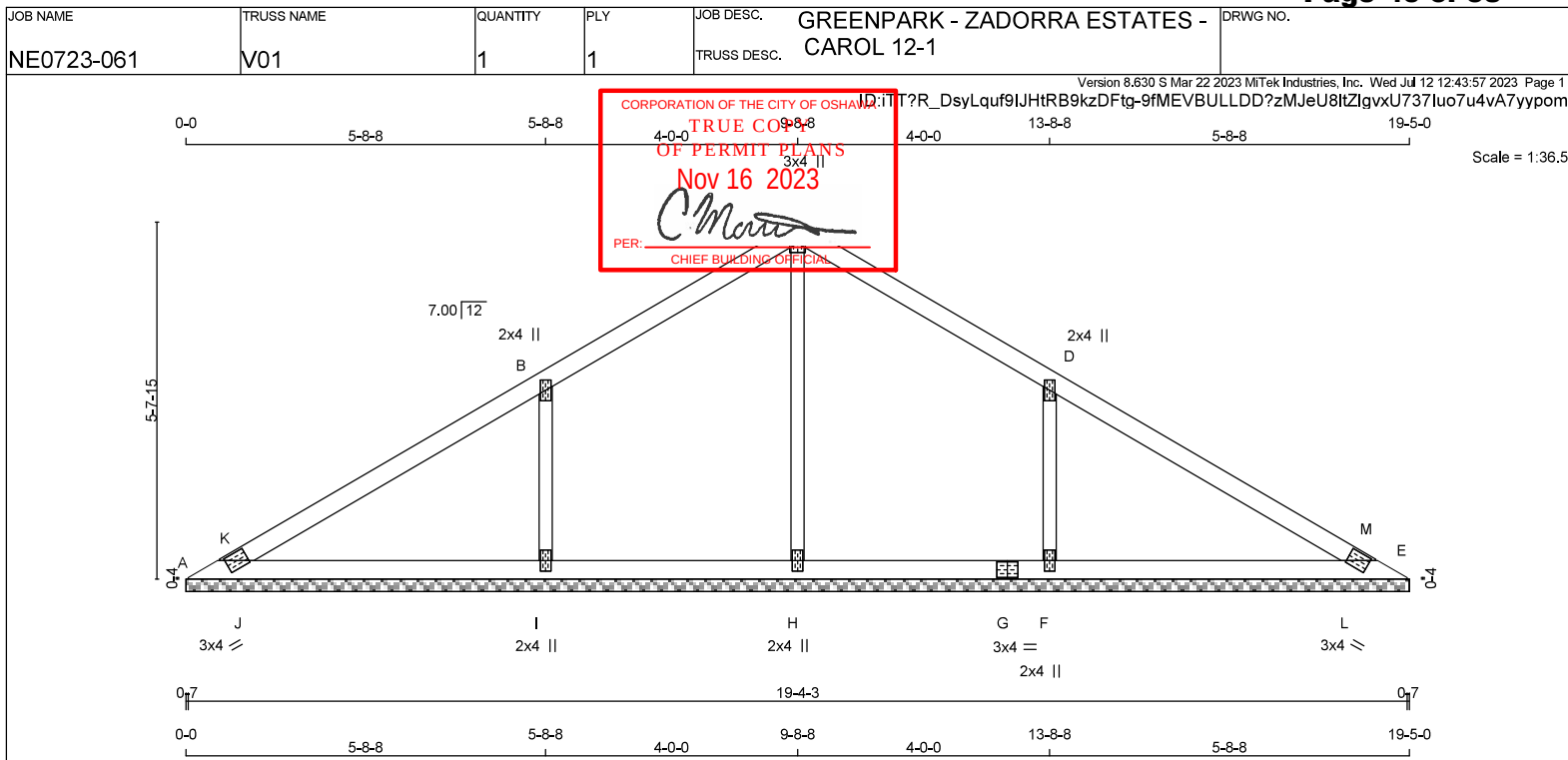
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	3.0	4.0		
B TMW+w	MT20	2.0	4.0		
C TTW+p	MT20	3.0	4.0	2.25	1.50
D TMW+w	MT20	2.0	4.0		
E TBM1-h	MT20	3.0	4.0		
F H-I	MT20	2.0	4.0		
F BMW1+w	MT20	2.0	4.0		
G BS4	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
A	153	0	153	0	19-5-0 (6-4-3) JI-8	
E	153	0	153	0	19-5-0 (6-4-3) JI-8	
H	717	0	717	0	19-5-0 (6-4-3) JI-8	
I	820	0	820	0	19-5-0 (6-4-3) JI-8	
F	820	0	820	0	19-5-0 (6-4-3) JI-8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	106	80 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
E	106	80 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
H	503	352 / 0	0 / 0	0 / 0	0 / 0	151 / 0	0 / 0
I	573	418 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0
F	573	418 / 0	0 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
A-K	0 / 332	-119.4	-119.4	0.07 (1)	10.00	H-C	-708 / 0	0.34 (1)	
K-B	0 / 380	-119.4	-119.4	0.47 (1)	10.00	I-B	-651 / 0	0.13 (1)	
B-C	0 / 330	-119.4	-119.4	0.47 (1)	10.00	F-D	-651 / 0	0.13 (1)	
C-D	0 / 330	-119.4	-119.4	0.47 (1)	10.00	J-K	-236 / 6	0.00 (1)	
D-M	0 / 380	-119.4	-119.4	0.47 (1)	10.00	L-M	-236 / 6	0.00 (1)	
M-E	0 / 332	-119.4	-119.4	0.07 (1)	10.00				
A-J	-335 / 0	-18.2	-18.2	0.22 (1)	6.25				
J-I	-302 / 0	-18.2	-18.2	0.23 (1)	6.25				
I-H	-318 / 0	-18.2	-18.2	0.15 (1)	6.25				
H-G	-318 / 0	-18.2	-18.2	0.15 (1)	6.25				
G-F	-318 / 0	-18.2	-18.2	0.15 (1)	6.25				
F-L	-302 / 0	-18.2	-18.2	0.23 (1)	6.25				
L-E	-335 / 0	-18.3	-18.3	0.22 (1)	6.25				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47/1.00 (B-K:1), BC=0.23/1.00 (F-L:1), WB=0.34/1.00 (C-H:1), SS=0.23/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

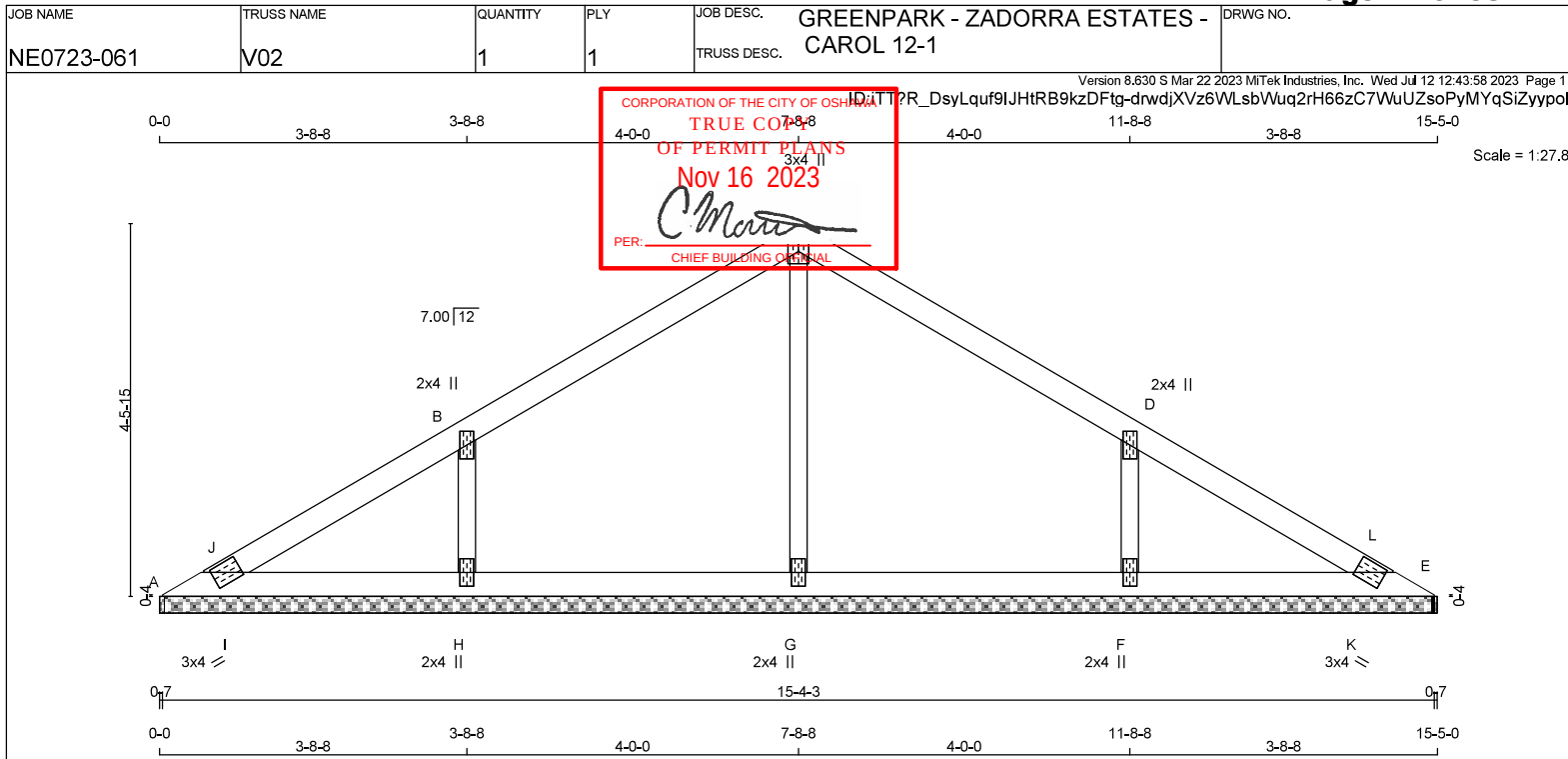
JSI GRIP= 0.65 (G) (INPUT = 0.90)

JSI METAL= 0.30 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	168	0	168	0	0	15'-5" (15'-4"-3)"	8
E	168	0	168	0	0	15'-5" (15'-4"-3)"	8
G	495	0	495	0	0	15'-5" (15'-4"-3)"	8
H	641	0	641	0	0	15'-5" (15'-4"-3)"	8
F	641	0	641	0	0	15'-5" (15'-4"-3)"	8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	117	87 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	117	87 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
G	349	236 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
H	447	329 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
F	447	329 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	0 / 13	-119.4 -119.4	0.06 (1)	G-C	-437 / 0	0.13 (1)	
J-B	0 / 73	-119.4 -119.4	0.29 (1)	H-B	-535 / 0	0.08 (1)	
B-C	0 / 26	-119.4 -119.4	0.29 (1)	F-D	-535 / 0	0.08 (1)	
C-D	0 / 26	-119.4 -119.4	0.29 (1)	I-J	-74 / 6	0.00 (1)	
D-L	0 / 73	-119.4 -119.4	0.29 (1)	K-L	-74 / 6	0.00 (1)	
L-E	0 / 13	-119.4 -119.4	0.06 (1)				
A-I	-34 / 0	-18.2 -18.2	0.08 (1)				
I-H	-23 / 0	-18.2 -18.2	0.08 (1)				
H-G	-44 / 0	-18.2 -18.2	0.08 (4)				
G-F	-44 / 0	-18.2 -18.2	0.08 (4)				
F-K	-23 / 0	-18.2 -18.2	0.08 (1)				
K-E	-34 / 0	-18.3 -18.3	0.08 (1)				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADCSI: TC=0.29/1.00 (B-J-1), BC=0.08/1.00 (H-I-1),
 WB=0.13/1.00 (C-G-1), SS=0.20/1.00 (C-D-1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987

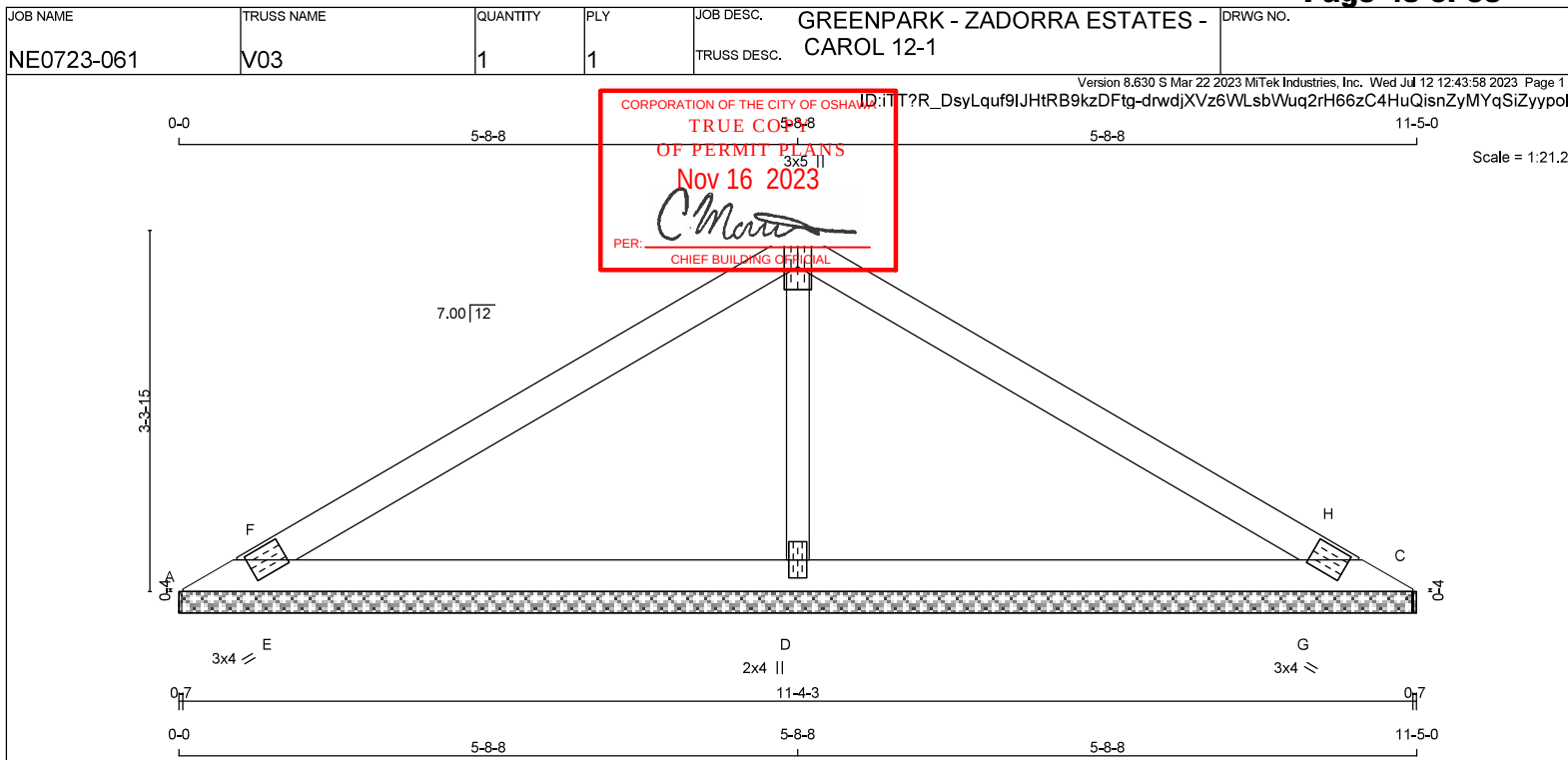
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
 JSI METAL= 0.25 (D) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
A - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESCR.
SPF
SPF
SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	3.0	5.0		
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	6	0	6	0	0	11-5-0 (11-4-32)	
C	6	0	6	0	0	11-5-0 (11-4-32)	
D	1551	0	1551	0	0	11-5-0 (11-4-32)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	4	1/0	0/0	0/0	0/0	0/0	4/0	0/0
C	4	1/0	0/0	0/0	0/0	0/0	4/0	0/0
D	1083	789/0	0/0	0/0	0/0	0/0	294/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-F	0/767	-119.4 -119.4	0.30 (1)	10.00	D-B	-1282/0	0.25 (1)
F-B	0/747	-119.4 -119.4	0.50 (1)	10.00	E-F	-402/0	0.00 (1)
B-H	0/747	-119.4 -119.4	0.50 (1)	10.00	G-H	-402/0	0.00 (1)
H-C	0/767	-119.4 -119.4	0.30 (1)	10.00			
A-E	-706/0	-18.2 -18.2	0.32 (1)	6.25			
E-D	-657/0	-18.2 -18.2	0.33 (1)	6.25			
D-G	-657/0	-18.2 -18.2	0.33 (1)	6.25			
G-C	-706/0	-18.2 -18.2	0.32 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.50/1.00 (B-F:1), BC=0.33/1.00 (D-E:1),
WB=0.25/1.00 (B-D:1), SSI=0.21/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

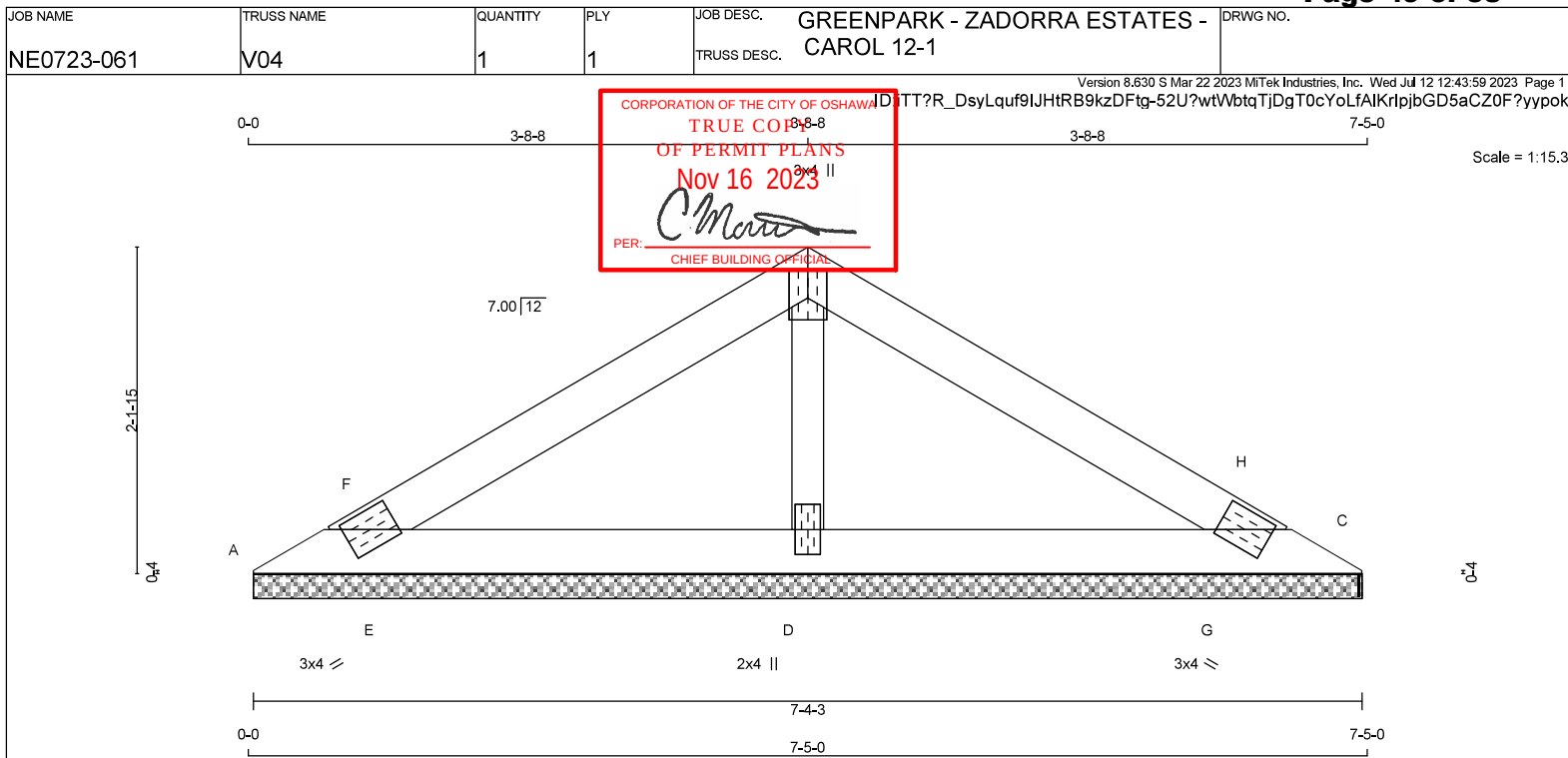
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.27 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 18 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
A - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESCR.
SPF
SPF
SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT UPLIFT	REQD BRG IN-SX	REQD BRG IN-SX
A	97	0	97	0	0	7-4-3	1-8
C	97	0	97	0	0	7-4-3	1-8
D	818	0	818	0	0	7-4-3	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM. LIVE	MAX./MIN. WIND	MAX./MIN. DEAD	MAX./MIN. SOIL
A	68	49 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
C	68	49 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	572	413 / 0	0 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-F	0 / 286	-119.4 -119.4	0.07 (1)	D-B	-817 / 0	0.09 (1)	
F-B	0 / 293	-119.4 -119.4	0.19 (1)	E-F	-205 / 0	0.00 (1)	
B-H	0 / 293	-119.4 -119.4	0.19 (1)	G-H	-205 / 0	0.00 (1)	
H-C	0 / 286	-119.4 -119.4	0.07 (1)				
A-E	-279 / 0	-18.2 -18.2	0.15 (1)				6.25
E-D	-256 / 0	-18.2 -18.2	0.15 (1)				6.25
D-G	-256 / 0	-18.2 -18.2	0.15 (1)				6.25
G-C	-279 / 0	-18.2 -18.2	0.15 (1)				6.25

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (B-F:1), BC=0.15/1.00 (D-E:1), WB=0.09/1.00 (B-D:1), SSI=0.12/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



KOTT TRUSS BEARING CAPACITY TABLE

(INTERNAL USE ONLY)

TRUSS BEARING CAPACITIES (LBS.), BY TRUSS LUMBER TYPE (SUPPORTED ON SP #2 TOP PLATE)

CORPORATION OF THE CITY OF OSHAWA

TRUE COPY

OF PERMIT PLANS

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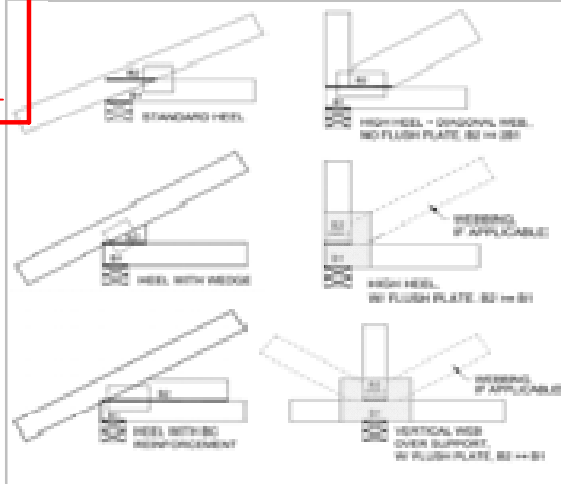
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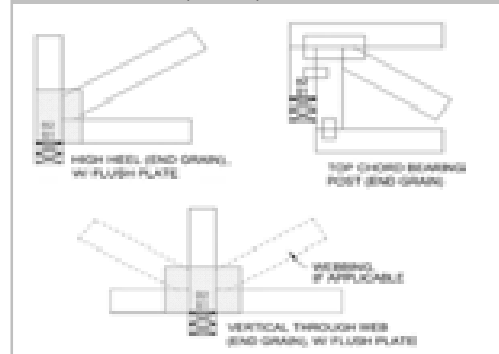
	BEARING PLATE (B1)	TYPE 1, NO FLUSH PLATE, B2=2B1							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
NO BEARING ENHANCER	1 1/2"	1383		2767		4151		5534	
	2x4	3712	3228	7425	6457	11138		14851	
	2x6	5834	5073	11668	10146	17503		23337	
	2x8	7690	6687	15381	13375	23072		30763	
FLUSH PLATE	BEARING PLATE (B1)	TYPE 1, FLUSH PLATE							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
		1 1/2"	1383	2767	4151	5534			
		2x4	3712	7425	11138	14851			
BEARING ENHANCER	2x6	5834	5073	11668	10146	17503		23337	
	2x8	7690	6687	15381	13375	23072		30763	
CPn-4 (Simpson)	2x4	TYPE 1, FLUSH PLATE + BEARING ENHANCER							
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
CPn-6 (Simpson)	2x4	4515		9030		13545		18065	
	2x6	7095		14190		21285		28390	
CP4-9 (KOTT)	2x4	6007	4898	12014	9796	18021	14694	19801	19592
	2x6	8677	7075	17354	14150	26031	21225	31117	28300
SBP4 (MiTek)	2x4	7288		11001		14714		18427	
	2x6	11030		16865		22699		28534	
SBP6 (MiTek)	2x4	12886		20578		28269		35960	
	2x6								

TYPE 1 CONFIGURATIONS



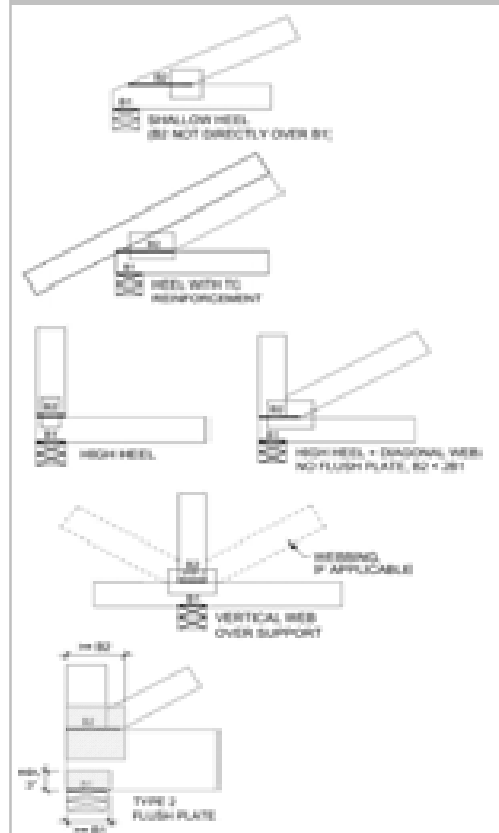
	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 1 (END GRAIN)							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)
END GRAIN	2x4	2x4	3712		7425		11138		14851	
	2x6	2x6	5834		11668		17503		23337	
			MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)
	2x8	2x8	7690		15381		23072		30763	
END GRAIN, BEARING ENHANCER	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 1 (END GRAIN), FLUSH PLATE + BEARING ENHANCER							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)	MSR2100 (EG)	SPF No.2 (EG)
			2x4	2x4	4515		9030		13545	
			2x6	2x6	7095		14190		21285	
CPn-4 (Simpson)	2x4	2x4	4515		9030		13545		18065	
	2x6	2x6	7095		14190		21285		28390	
CPn-6 (Simpson)	2x4	2x6	15585	9006	19801	18013	19801		19801	
			MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)	MSR1950 (EG)	SPF No.2 (EG)
CP4-9 (KOTT)	2x4	2x8	21834	13009	31117	26019	31117		31117	
	2x6	2x8								

TYPE 1 (END GRAIN) CONFIGURATIONS



	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, NO FLUSH PLATE, B2=2B1							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
NO BEARING ENHANCER	2x4	2x4	2639	2152	5279	4304	7919	6457	10588	8609
		2x6	3393	2767	6787	5534	10181	8302	13575	11069
		2x8	3393	2767	6787	5534	10181	8302	13575	11069
		2x10	4147	3382	8296	6764	12444	10146	16592	13529
	2x6	2x6	4808	3920	9616	7840	14424	11761	19232	15681
		2x8	5562	4535	11124	9070	16686	13606	22248	18141
		2x10	3959	3228	7919	6457	11878	9685	15838	12914
		2x12	4808	3920	9616	7840	14424	11761	19232	15681
	2x8	2x8	5467	4458	10935	8916	16403	13375	21871	17833
		2x10								
		2x12								
		2x14								
FLUSH PLATE	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, FLUSH PLATE							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
			2x4							
			2x6							
			2x8							
			2x10							
			2x12							
			2x14							
			2x16							
			2x18							
			2x20							

TYPE 2 CONFIGURATIONS



NOTES:

- Factored truss reaction shall not exceed bearing capacity corresponding to: configuration type, size of bearing surfaces, truss lumber, # of plies, and applicable enhancers.
- Values in table are in conformance with CSA O86-14 Cl. 6.5.7 and TPIC 2014-Update 2, and may be used for residential or commercial designs.
- Values in table are in conformance with MiTek Canada Detail B37821Q "SPF Bearing Capacities".
- Values in table are in conformance with Simpson Catalogue C-C-CAN2020.
- Conditions for use of table values include: standard duration (K=1), dry lumber (K_{dp}=1), untreated lumber (K=1), length of bearing factor not applied (K_b=1).
- Size factor (K_{zp}) applied to support material calculation when acceptable. Flush plate factor (K_p) applied to truss material calculation when acceptable (ie. excludes end grain).
- Flat roof factor (K_r) must applied for trusses making up a flat roof system; to do so, multiply bearing capacity values by 0.75 for this application.
- Bearing plate is to be specified by the project engineer; values in table assume a bearing material of SPF #2 (or better).
- When required, flush plate must not be located further than 1/4" away from bearing surface, and must cover the entire bearing plate length (B1).
- When required, bearing enhancer must be installed as per manufacturer's guidelines.
- Type 2 bearing configurations can be converted to use Type 1 table values as outlined in TPIC 2014-Update 1 Cl. 7.5.9.
- This table is not valid after April 30, 2022.

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	Length (in)	Diameter (in)	LATERAL Resistance per nail (Lbs.)		WITHDRAWAL Resistance per nail (Lbs.)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

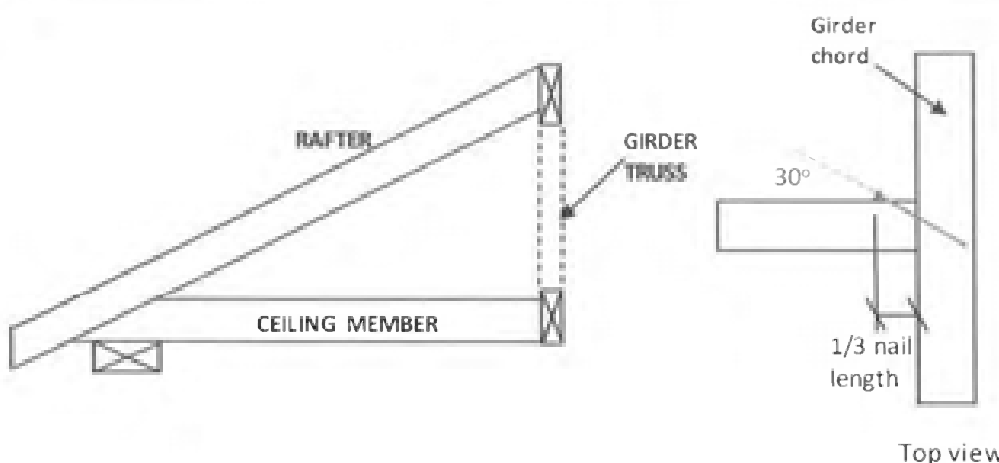
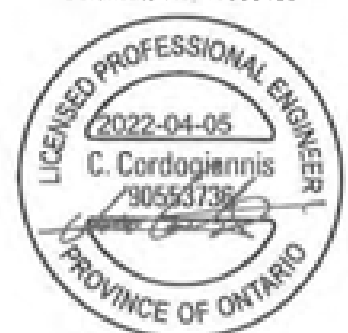
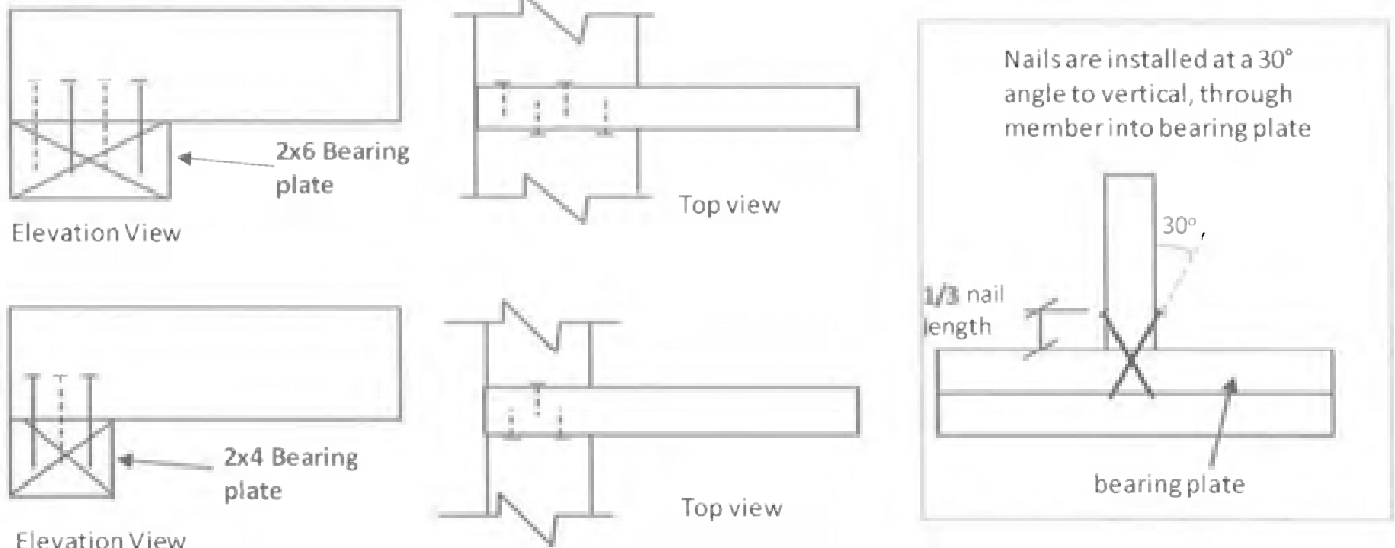


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

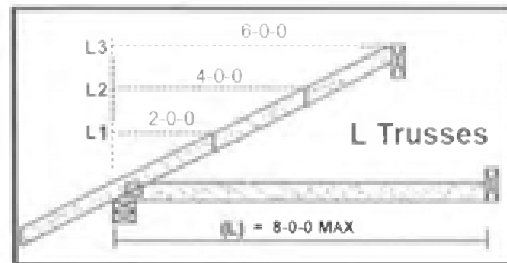
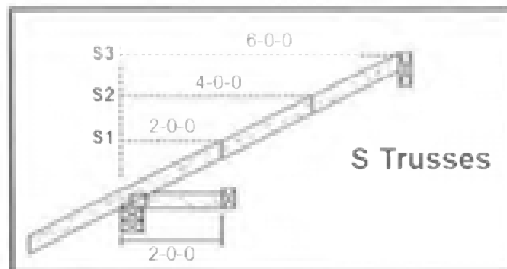
1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

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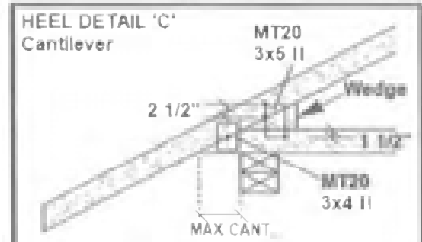
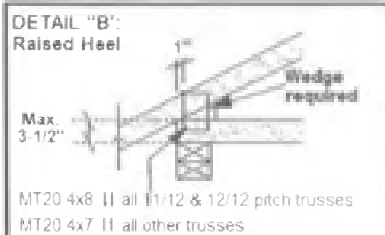
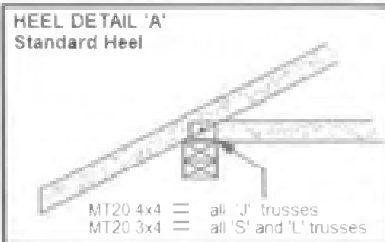
STANDARD HIP AND FRAMING

PLAN VIEW



Specified Load Rating:

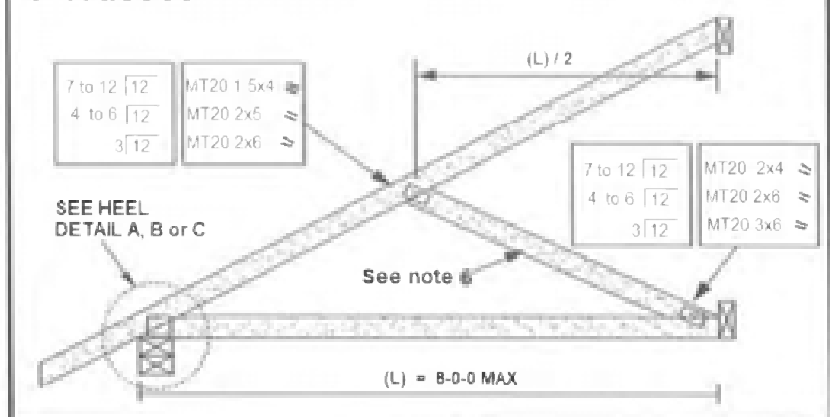
Top chord Live:	51.0 PSF or less
Top chord Dead:	6.0 PSF or less
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF or less



CANTILEVER DETAIL "C"

SLOPE	MAX CANT.	WEDGE PLATE	WEDGE SIZE
3/12	17"	3 x 5	2 x 3
4/12	14"	3 x 5	2 x 3
5/12	12"	3 x 5	2 x 4
6/12	10"	3 x 5	2 x 4
7/12	9"	3 x 5	2 x 5
8/12	8 1/2"	3 x 5	2 x 5
9/12	8"	3 x 5	2 x 6
10/12	7 1/2"	3 x 5	2 x 6

J Trusses



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NOTES:

1. This detail is valid only for projects conforming to **PART 9 NBCC 2015** that do not require a wind analysis to be incorporated into the design of the trusses.
2. Overhang length shall not exceed 24 inches.
3. All lumber shall be 2x4 SPF (or D-Fir) DRY No. 2 grade or better.
4. All plates specified are MITEK MT20, pressed into both faces of each truss. Heel plates of all trusses shall conform to heel details 'A', 'B' or 'C'.
5. Diagonal hip rafter design shall conform to section 9.23.14.6 of NBCC 2015.
6. For 6.0 ft. or less span, diagonal web on truss 'J' is optional. Girder design must reflect choice of partial jack ('J' with diagonal web) or open jack ('J' without diagonal web)
7. All truss-to-rafter and truss-to-truss connections shall be specified as per MITEK standard detail 'MSD2015-H: Toe-Nail Capacity Details'



STANDARD GABLE END DETAIL

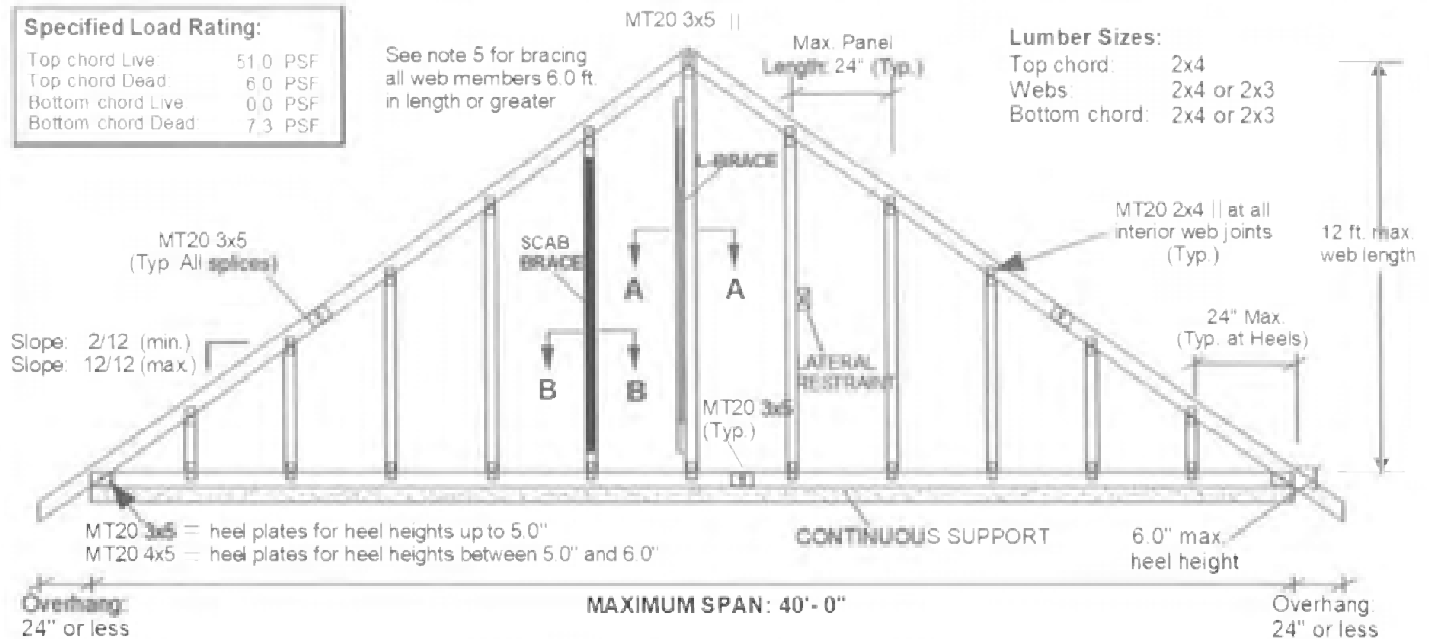
Specified Load Rating:

Top chord Live	51.0 PSF
Top chord Dead	6.0 PSF
Bottom chord Live	0.0 PSF
Bottom chord Dead	7.3 PSF

See note 5 for bracing
all web members 6.0 ft
in length or greater

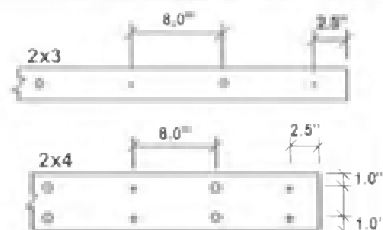
Lumber Sizes:

Top chord:	2x4
Webs:	2x4 or 2x3
Bottom chord:	2x4 or 2x3

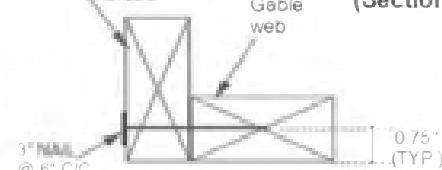
SCAB BRACE DETAIL
(Section B-B)

Gable Web

SPF No. 2 DRY Scab, same size as web. Scab brace must cover 90% of web length



+ 0.122"x3.0" nail driven from front face
o 0.122"x3.0" nail driven from back face

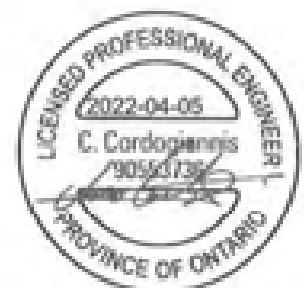
2x4 SPF No. 2
DRY L-BraceL BRACE DETAIL
(Section A-A)

Fasten L-Brace to narrow edge of web with one row of 0.122" x 3.0" nails spaced at 6.0" c/c along entire length of web. Brace must cover 90% of the web length. Respect a 2.5" minimum end distance.

Notes:

1. This detail is only valid for projects conforming to **Part 9, NBCC 2015** that do not require a wind analysis to be incorporated into the design of the truss.
2. This detail is for vertical (gravity) load rating of the truss only. Truss must be continuously supported over the entire length of bottom chord.
3. Maximum web length not to exceed 12.0 ft. Spacing of gable stud webs in the truss not to exceed 24 inches cc.
4. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint.
5. Lateral restraint required at half-length of all webs over 6.0 ft. long. Alternatively install an L-Brace or scab brace as shown above. Scab braces shall be limited to 10 ft. long webs or less.
6. All plates are MITTEK MT20 pressed into both faces of truss.
7. All lumber to be SPF (or D-Fir) DRY and of No.2 grade or better.
8. Additional building bracing is typically installed to brace the face of the end wall assembly. See BCSI Canada 'Building Designer Responsibilities for Gable End Frame Bracing' for additional information on building bracing for gable-end assemblies.

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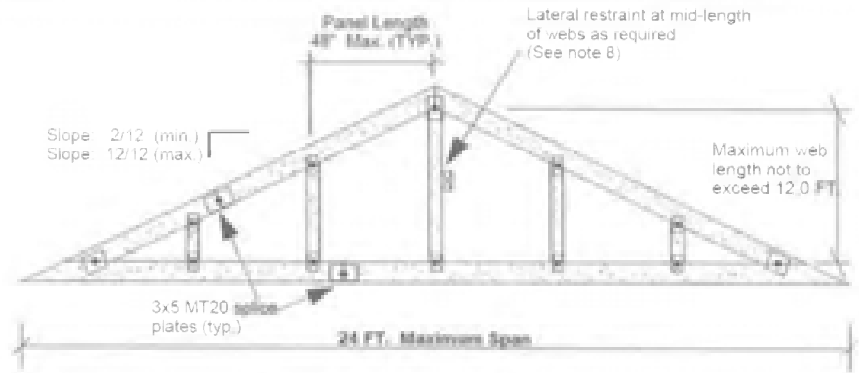
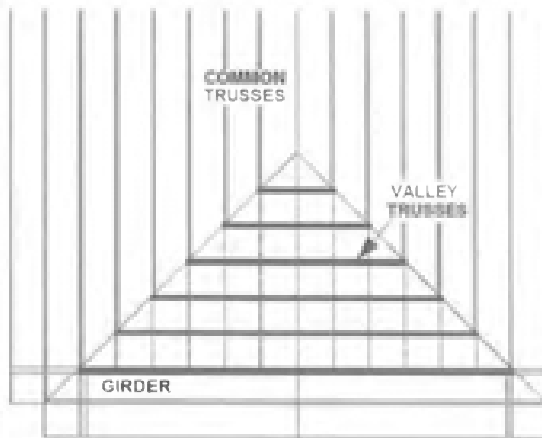


Issued: MARCH 1, 2022

Expiry: APRIL 30, 2024



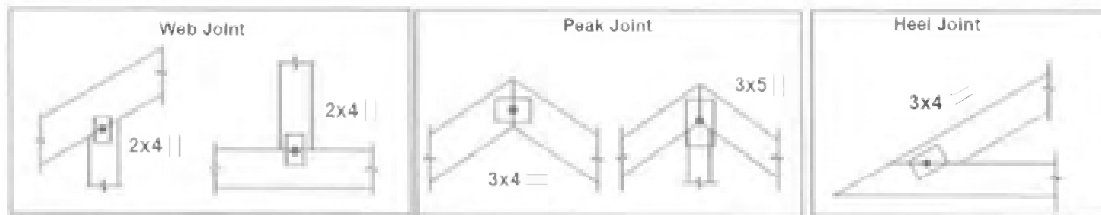
VALLEY SET ROOF FRAMING PLAN



VALLEY TRUSS LUMBER:

Top Chord	2x4
Bottom Chord	2x3 or 2x4
Web	2x3 or 2x4

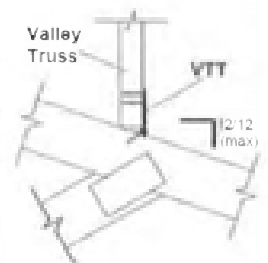
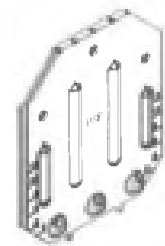
Splices (if required) shall be located at 1/4 point of a panel. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint. If beveling the bottom chord, splice plates shall be flush with top edge of bottom chord.



NOTES:

- Specified roof loads shall not exceed the maximum limits shown below:

Top chord:	Live =	70.0 PSF
Top Chord:	Dead =	6.0 PSF
Bottom Chord:	Live =	0.0 PSF
Bottom Chord:	Dead =	7.3 PSF
- Valley truss design assumes continuous support by the truss system underneath. Spacing of all valley trusses as well as the underlying trusses shall not exceed **24 in. c/c**.
- Vertical web spacing in valley trusses not to exceed **48 in. c/c**. Web lengths not to exceed 12 ft.
- All lumber to be DRY No. 2 grade or better, SPF or D-Fir.
- Bottom chord may be beveled to match the slope of the intersecting roof. If beveling, a minimum 2x4 bottom chord is required, with a maximum bevel slope of 4/12 (spliced chord) or 8/12 (non-spliced chord). For bevel slopes exceeding the 2x4 limits, use a 2x6 bottom chord.
- Truss plates are MITEK MT20 pressed into both faces of the truss and centered at each joint.
- Use MiTek VTT Valley Truss Ties to attach each valley truss at the intersection point between valley chord and underlying truss chord. Install clips as per product installation instructions. Alternatively, toe-nail valley truss bottom chord at all intersection points with each underlying truss, using two 0.122"x3.25" nails per connection.
- One continuous lateral brace is required at $\frac{1}{4}$ length of all webs that exceed 6.0 ft. in length.
- This detail is only valid for residential projects conforming to **PART 9 - NBCC2015**, that do not require a wind analysis to be incorporated into the design of the trusses.

MITEK VTT
(Valley Truss Tie)

Install as per MiTek product installation instructions

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Certificate No. 10889485



