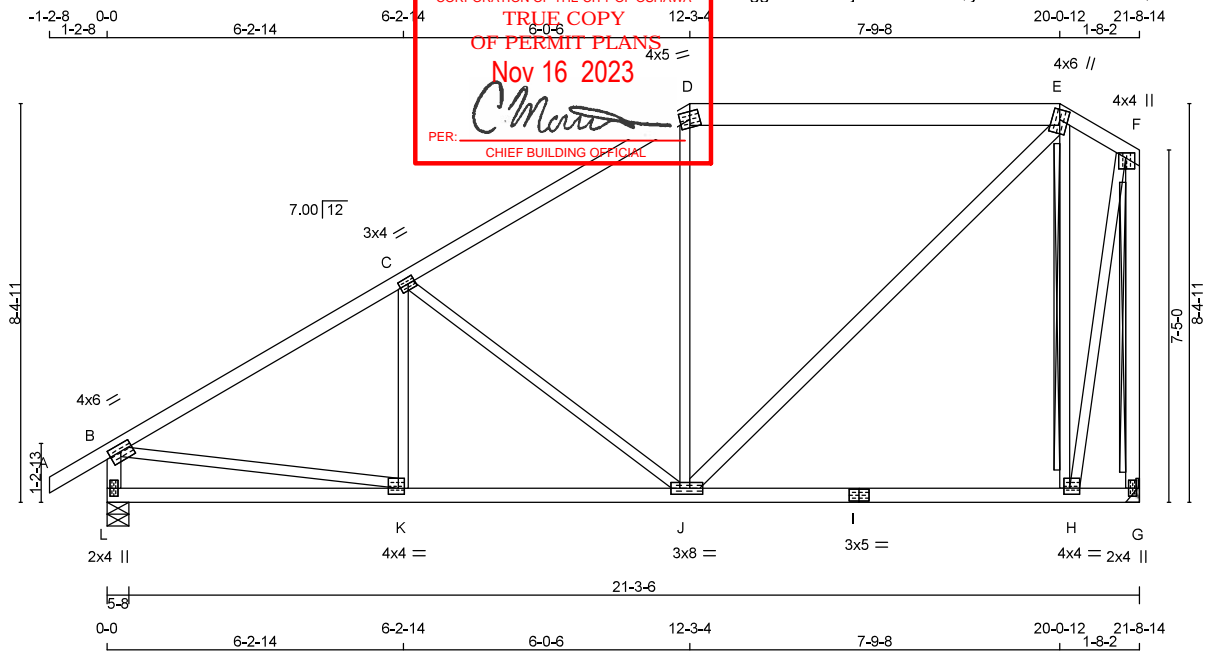


JOB NAME NE0723-062	TRUSS NAME T07	QUANTITY 1	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - CAROL 12-2	DRWG NO.
------------------------	-------------------	---------------	----------	---	----------

Version 8.630 S Mar 22 2023 MitTek Industries, Inc. Wed Jul 12 13:38:27 2023 Page 1



Scale = 1:48.5

TOTAL WEIGHT = 113 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x6	DRY	No.2
E - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT J - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	6.0	1.75 3.00
C	TMVW4	MT20	3.0	4.0	1.50 1.75
D	TTW-m	MT20	4.0	5.0	
E	TTVW+m	MT20	4.0	6.0	2.50 2.00
F	TMVW+p	MT20	4.0	4.0	1.25 1.75
G	BMV1+p	MT20	2.0	4.0	
H	BMVW4	MT20	4.0	4.0	1.50 1.50
I	BS-t	MT20	3.0	5.0	
J	BMVW4	MT20	3.0	8.0	1.50 2.25
K	BMVW4	MT20	4.0	4.0	1.50 1.50
L	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
L	1650	0	1650	0
G	1497	0	1497	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-10.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1150	846 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
G	1046	757 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-H, F-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX	MAX
FR-TO	FROM	TO	FROM	TO	FR-TO	FROM	TO	FROM	TO
A-B	0 / 39	-119.4	-119.4	0.14 (1)	10.00	K-C	-123 / 68	0.04 (1)	
B-C	-1797 / 0	-119.4	-119.4	0.66 (1)	4.14	C-J	-730 / 0	0.81 (1)	
C-D	-1214 / 0	-119.4	-119.4	0.61 (1)	4.93	J-D	-150 / 60	0.21 (1)	
D-E	-1013 / 0	-119.4	-119.4	0.48 (1)	6.25	J-E	0 / 1028	0.17 (1)	
E-F	-289 / 0	-119.4	-119.4	0.04 (1)	6.25	H-E	-1158 / 0	0.57 (1)	
L-B	-1600 / 0	0.0	0.0	0.18 (1)	6.53	B-K	0 / 1607	0.36 (1)	
G-F	-1532 / 0	0.0	0.0	0.48 (1)	7.81	H-F	0 / 1314	0.30 (1)	
L-K	0 / 0	-18.2	-18.2	0.14 (4)	10.00				
K-J	0 / 1590	-18.2	-18.2	0.37 (1)	10.00				
J-I	0 / 294	-18.2	-18.2	0.23 (4)	10.00				
I-H	0 / 294	-18.2	-18.2	0.23 (4)	10.00				
H-G	0 / 0	-18.2	-18.2	0.21 (4)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.72")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.66/1.00 (B-C:1), BC=0.37/1.00 (J-K:1), WB=0.81/1.00 (C-J:1), SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

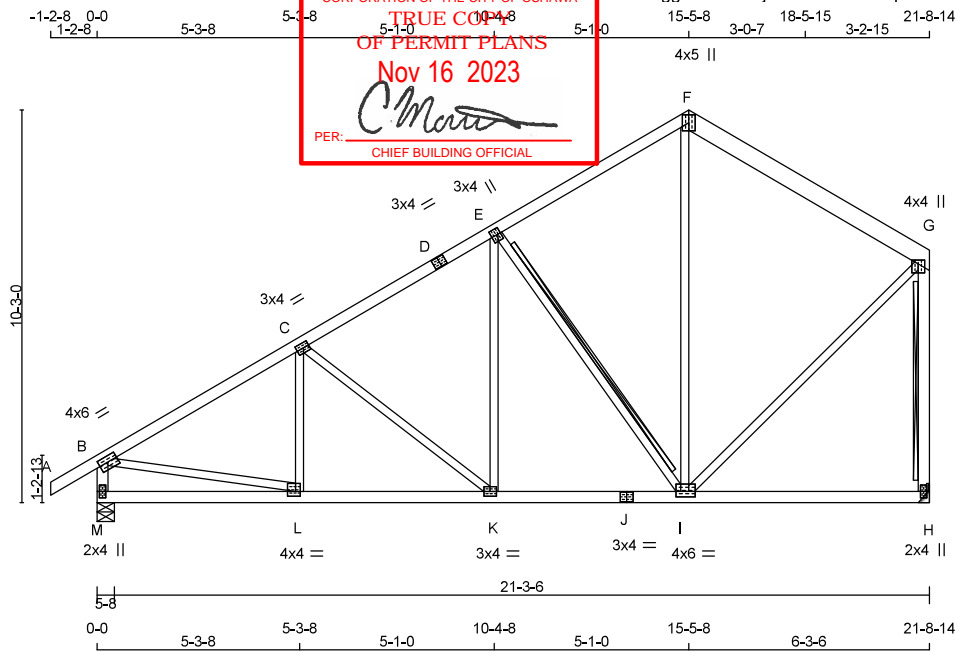
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL = 0.54 (K) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:28 2023 Page 1



Scale = 1:60.2

TOTAL WEIGHT = $2 \times 107 = 214 \text{ lb}$

LUMBER

N. L. G. A. RULES		
CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - G	2x6	DRY No.2
M - B	2x4	DRY No.2
H - G	2x4	DRY No.2
M - J	2x4	DRY No.2
J - H	2x4	DRY No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
--------------------	-----	-----	------

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-t	MT20	4.0	6.0	1.75	3.00
C	TMWV-t	MT20	3.0	4.0	1.50	1.75
D	TS-t	MT20	3.0	4.0		
E	TMWV+t	MT20	3.0	4.0	2.00	0.75
F	TTV+p	MT20	4.0	5.0		
G	TMWV+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWVW-t	MT20	4.0	6.0	1.75	1.50
J	BS-t	MT20	3.0	4.0		
K	BMWV-t	MT20	3.0	4.0		
L	BMWV-t	MT20	4.0	4.0	1.50	1.50
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1650	0	1650	0	0	5-8	1-14
H	1497	0	1497	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1150	846 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1046	757 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-I, G-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 39	-119.4	-119.4	0.14 (1)	10.00	L-C	-170 / 48
B-C	-1802 / 0	-119.4	-119.4	0.44 (1)	4.52	K-E	-474 / 0
C-D	-1402 / 0	-119.4	-119.4	0.36 (1)	5.10	C-K	0 / 372
D-E	-1402 / 0	-119.4	-119.4	0.36 (1)	5.10	E-I	-893 / 0
E-F	-326 / 0	-119.4	-119.4	0.38 (1)	6.21	I-F	0 / 185
F-G	-801 / 0	-119.4	-119.4	0.30 (1)	6.25	I-G	0 / 959
M-B	-1608 / 0	0.0	0.0	0.16 (1)	6.52	B-L	0 / 1606
H-G	-1449 / 0	0.0	0.0	0.34 (1)	7.81		
M-L	0 / 0	-18.2	-18.2	0.12 (4)	10.00		
L-K	0 / 1582	-18.2	-18.2	0.30 (1)	10.00		
K-J	0 / 1210	-18.2	-18.2	0.29 (1)	10.00		
J-I	0 / 1210	-18.2	-18.2	0.29 (1)	10.00		
I-H	0 / 0	-18.2	-18.2	0.17 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	34.8	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.3	PSF
TOTAL LOAD				=	48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.44/1.00 (B-C:1) , BC=0.30/1.00 (K-L:1) ,
WB=0.50/1.00 (E-I:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.54 (L) (INPUT = 1.00)

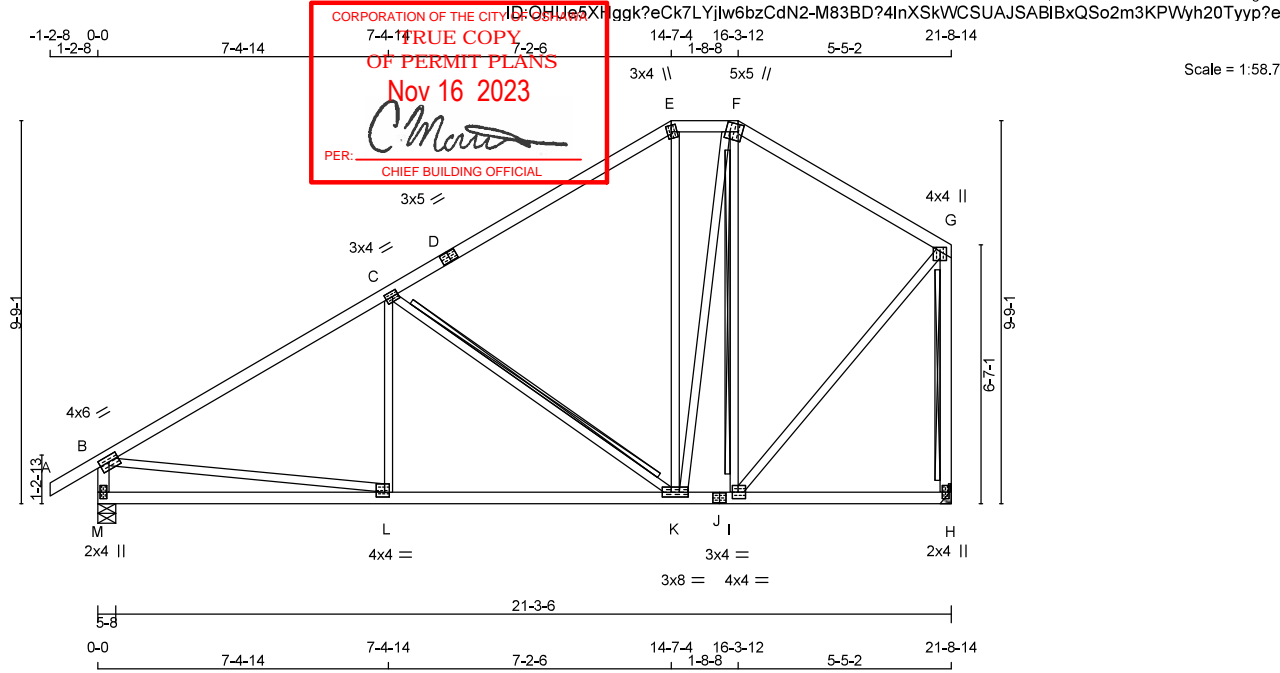


**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES - CAROL 12-2	DRWG NO.
NE0723-062	T09	1	1	TRUSS DESC.		

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:29 2023 Page 1



TOTAL WEIGHT = 109 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-H	MT20	4.0	6.0	1.75	3.00
C	TMVW-H	MT20	3.0	4.0	1.50	1.75
D	TS4	MT20	3.0	5.0		
E	TTW+m	MT20	3.0	4.0	2.00	1.25
F	TTWV+m	MT20	5.0	5.0	2.25	1.25
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMVW-H	MT20	4.0	4.0	2.00	1.75
J	BS4	MT20	3.0	4.0		
K	BMVWV-H	MT20	3.0	8.0		
L	BMVW-H	MT20	4.0	4.0	1.50	1.50
M	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1650	0	1650	0	0	5-8	1-14
H	1497	0	1497	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1150	846 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1046	757 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-K, F-I, G-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO							
A-B	0 / 39	-119.4	-119.4	0.14 (1)	10.00	L-C	-37 / 123
B-C	-1756 / 0	-119.4	-119.4	0.97 (1)	3.22	C-K	-964 / 0
C-D	-929 / 0	-119.4	-119.4	0.87 (1)	4.54	K-E	0 / 62
D-E	-929 / 0	-119.4	-119.4	0.87 (1)	4.54	K-F	0 / 609
E-F	-755 / 0	-119.4	-119.4	0.05 (1)	6.25	L-F	-640 / 0
F-G	-785 / 0	-119.4	-119.4	0.48 (1)	6.19	B-L	0 / 1575
M-B	-1595 / 0	0.0	0.0	0.18 (1)	6.54	I-G	0 / 999
H-G	-1460 / 0	0.0	0.0	0.36 (1)	7.81		
M-L	0 / 0	-18.2	-18.2	0.25 (4)	10.00		
L-K	0 / 1563	-18.2	-18.2	0.39 (1)	10.00		
K-J	0 / 653	-18.2	-18.2	0.21 (1)	10.00		
J-I	0 / 653	-18.2	-18.2	0.21 (1)	10.00		
I-H	0 / 0	-18.2	-18.2	0.13 (4)	10.00		

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.72")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.97/1.00 (B-C:1) , BC=0.39/1.00 (K-L:1) , WB=0.63/1.00 (C-K:1) , SS=0.37/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PL)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

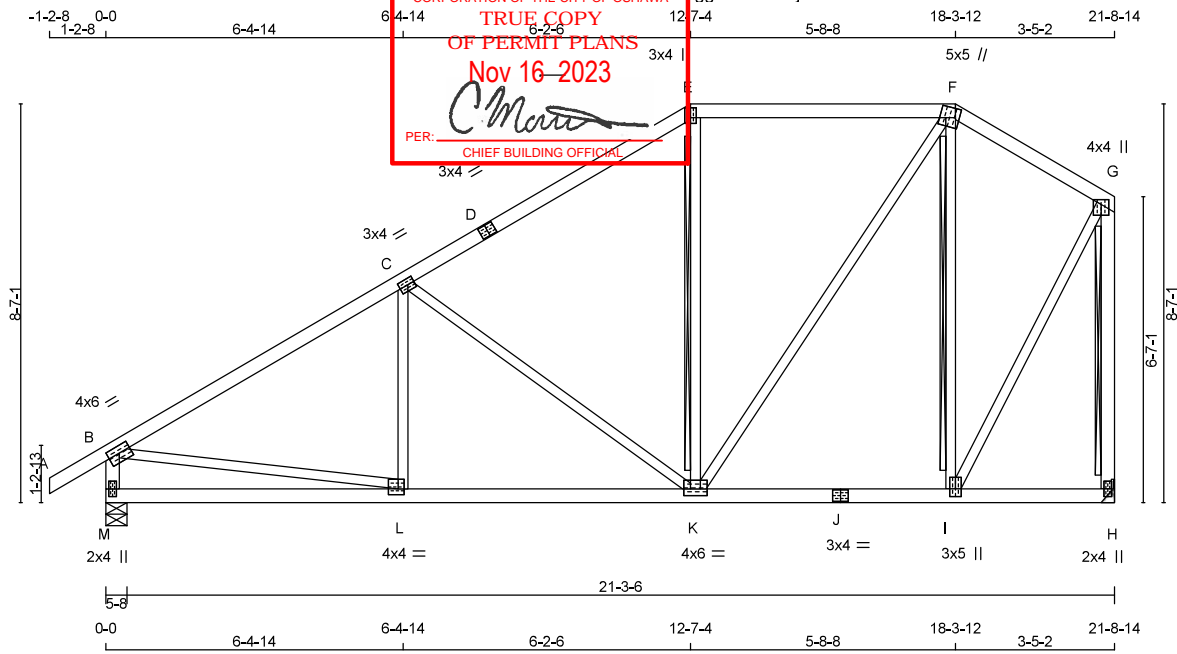
JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.53 (L) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-062	T10	1	1	TRUSS DESC.	CAROL 12-2	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:29 2023 Page 1



Scale = 1:49.6

TOTAL WEIGHT = 103 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW4	MT20	4.0	6.0	1.75	3.00
C TMVW4	MT20	3.0	4.0	1.50	1.75
D TS4	MT20	3.0	4.0		
E TTW+p	MT20	3.0	4.0	2.50	1.50
F TTWV+m	MT20	5.0	5.0	2.25	1.00
G TMVW+p	MT20	4.0	4.0	1.25	2.00
H BMV1+p	MT20	2.0	4.0		
I BMVW+t	MT20	3.0	5.0	2.00	1.50
J BS4	MT20	3.0	4.0		
K BMVWV4	MT20	4.0	6.0	1.75	3.00
L BMVW4	MT20	4.0	4.0	1.50	1.50
M BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
M	1650	0	1650	0
H	1497	0	1497	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1150	846 / 0	0 / 0	0 / 0	0 / 0	305 / 0	0 / 0
H	1046	757 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-K, F-I, G-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE	(PLF)	FROM	TO	CSI (LC)	UNBRAC	LENGTH	FORCE	MAX
FR-TO	(LBS)						FR-TO	(LBS)	CSI (LC)
A-B	0 / 39	-119.4	-119.4	0.14 (1)	10.00	L-C	-100 / 86	0.04 (1)	
B-C	-1795 / 0	-119.4	-119.4	0.70 (1)	4.06	C-K	-788 / 0	1.00 (1)	
C-D	-1155 / 0	-119.4	-119.4	0.84 (1)	4.95	K-E	-58 / 67	0.03 (1)	
D-E	-1155 / 0	-119.4	-119.4	0.84 (1)	4.95	K-F	0 / 765	0.17 (1)	
E-F	-955 / 0	-119.4	-119.4	0.51 (1)	5.60	I-F	-864 / 0	0.46 (1)	
F-G	-623 / 0	-119.4	-119.4	0.18 (1)	6.25	B-L	0 / 1606	0.36 (1)	
M-B	-1601 / 0	0.0	0.0	0.16 (1)	6.53	I-G	0 / 1095	0.25 (1)	
H-G	-1475 / 0	0.0	0.0	0.36 (1)	7.81				
M-L	0 / 0	-18.2	-18.2	0.18 (4)	10.00				
L-K	0 / 1590	-18.2	-18.2	0.34 (1)	10.00				
K-J	0 / 527	-18.2	-18.2	0.15 (4)	10.00				
J-I	0 / 527	-18.2	-18.2	0.15 (4)	10.00				
I-H	0 / 0	-18.2	-18.2	0.09 (4)	10.00				

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL)= L/999 (0.11")CSI: TC=0.70/1.00 (B-C:1), BC=0.34/1.00 (K-L:1),
WB=1.00/1.00 (C-K:1), SS=0.32/1.00 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PSI)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

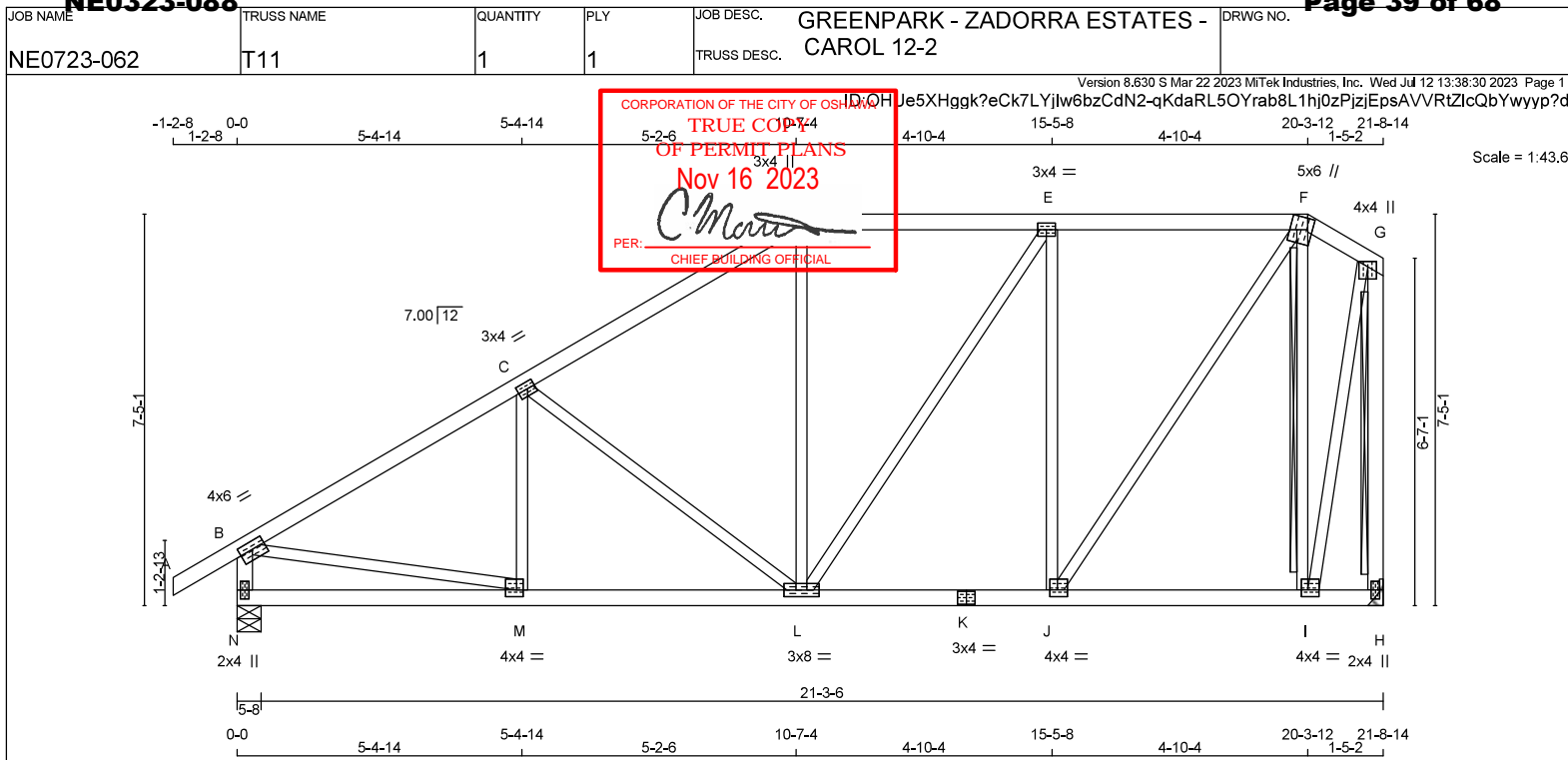
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (G) (INPUT = 0.90)
JSI METAL = 0.54 (L) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - G	2x4	DRY No.2
N - B	2x4	DRY No.2
H - G	2x4	DRY No.2
N - K	2x4	DRY No.2
K - H	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	6.0	1.75	3.00
C	TMVW4	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0	2.50	1.50
E	TMVW4	MT20	3.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.50	1.00
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMVW4	MT20	4.0	4.0	1.50	1.50
J	BMVW4	MT20	4.0	4.0	1.50	1.75
K	BS4	MT20	3.0	4.0		
L	BMVW4	MT20	3.0	8.0		
M	BMVW4	MT20	4.0	4.0	1.50	1.50
N	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
JT	1650	0	1650	0
N	1497	0	1497	0
H				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
JT	1150	846 / 0 0 / 0 0 / 0 305 / 0 0 / 0
N	1046	757 / 0 0 / 0 0 / 0 289 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-I, G-H

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO		FROM TO			FR-TO		
A-B	0 / 39	-119.4 -119.4	0.14 (1)	10.00	M-C	-166 / 51	0.05 (1)
B-C	-1815 / 0	-119.4 -119.4	0.48 (1)	4.43	C-L	-580 / 0	0.48 (1)
C-D	-1361 / 0	-119.4 -119.4	0.45 (1)	5.00	L-D	0 / 212	0.05 (1)
D-E	-1144 / 0	-119.4 -119.4	0.37 (1)	5.48	L-E	0 / 278	0.06 (1)
E-F	-990 / 0	-119.4 -119.4	0.37 (1)	5.79	J-E	-945 / 0	0.93 (1)
F-G	-310 / 0	-119.4 -119.4	0.03 (1)	6.25	J-F	0 / 1259	0.28 (1)
N-B	-1607 / 0	0.0 0.0	0.16 (1)	6.52	I-F	-1190 / 0	0.46 (1)
H-G	-1488 / 0	0.0 0.0	0.37 (1)	7.81	B-M	0 / 1623	0.37 (1)
					I-G	0 / 1273	0.29 (1)
N-M	0 / 0	-18.2 -18.2	0.12 (4)	10.00			
M-L	0 / 1600	-18.2 -18.2	0.31 (1)	10.00			
L-K	0 / 990	-18.2 -18.2	0.21 (1)	10.00			
K-J	0 / 990	-18.2 -18.2	0.21 (1)	10.00			
J-I	0 / 281	-18.2 -18.2	0.11 (4)	10.00			
I-H	0 / 0	-18.2 -18.2	0.05 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")

CALCULATED VERT. DEFL.(LL) = L/999 (0.06")

ALLOWABLE DEFL.(TL) = L/360 (0.72")

CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.48/1.00 (B-C-1), BC=0.31/1.00 (L-M-1), WB=0.93/1.00 (E-J-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PL)	(PL)	(PS)	(PL)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

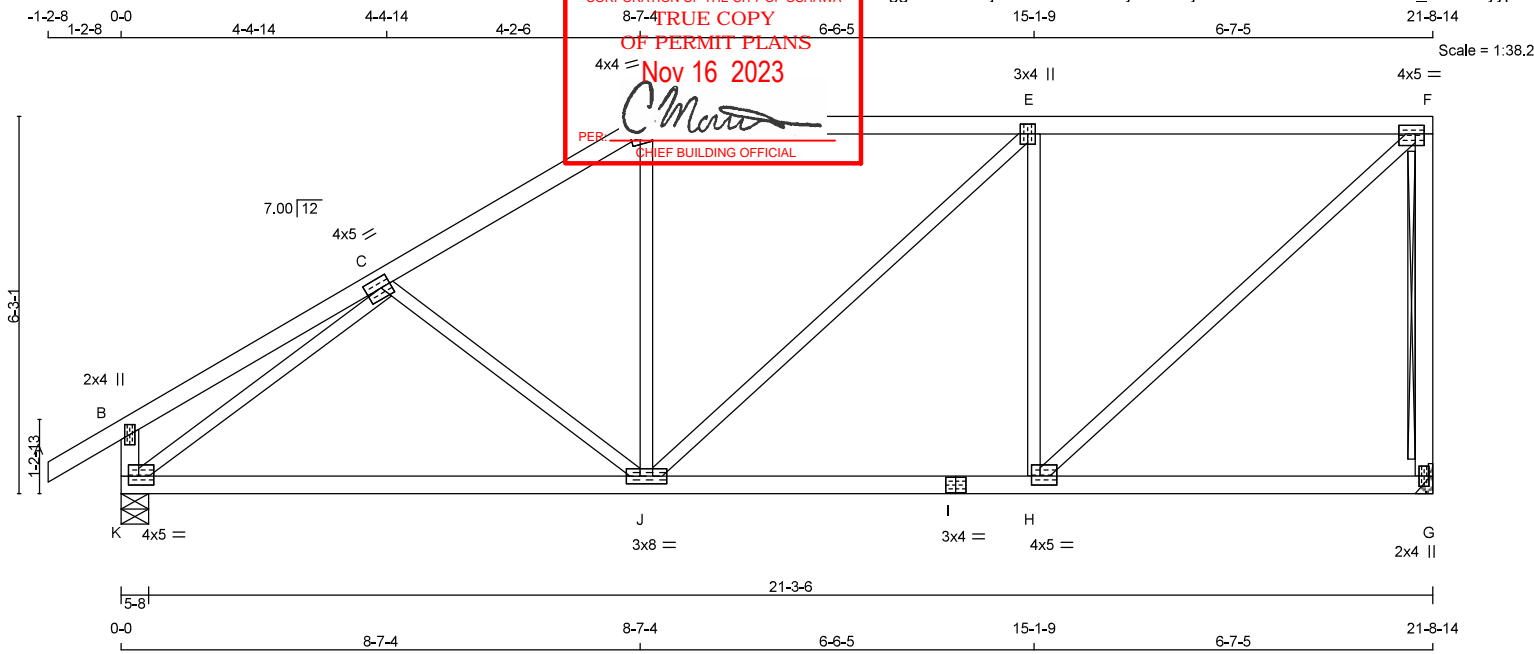
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (E) (INPUT = 0.90)
JSI METAL = 0.55 (M) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

JOB NAME NE0723-062	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - CAROL 12-2	DRWG NO.
------------------------	-------------------	---------------	----------	---	----------

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:31 2023 Page 1



TOTAL WEIGHT = 92 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW+I	MT20	4.0	5.0	2.00	2.00
D	TTM+M	MT20	4.0	4.0		
E	TMWW+I	MT20	3.0	4.0		
F	TMWW+I	MT20	4.0	5.0	1.75	1.75
G	BMV1+p	MT20	2.0	4.0		
H	BMWW+I	MT20	4.0	5.0	1.75	1.75
I	BS+I	MT20	3.0	4.0		
J	BMWWWW+I	MT20	3.0	8.0		
K	BMVW1+I	MT20	4.0	5.0	1.75	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1497	0	1497	0
K	1650	0	1650	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-10.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS
G	COMBINED	SNOW
K	1150	846 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-B	0 / 39	-119.4	-119.4	0.14 (1)	10.00	C-J	-283 / 0	0.15 (1)	
B-C	0 / 29	-119.4	-119.4	0.35 (1)	10.00	J-D	0 / 281	0.06 (1)	
C-D	-1562 / 0	-119.4	-119.4	0.40 (1)	4.79	J-E	0 / 78	0.02 (4)	
D-E	-1327 / 0	-119.4	-119.4	0.89 (1)	3.44	H-E	-1027 / 0	0.64 (1)	
E-F	-1270 / 0	-119.4	-119.4	0.99 (1)	3.52	H-F	0 / 1710	0.38 (1)	
G-F	-1445 / 0	0.0	0.0	0.31 (1)	7.81	K-C	-1976 / 0	0.99 (1)	
K-B	-352 / 0	0.0	0.0	0.04 (1)	7.81				
K-J	0 / 1545	-18.2	-18.2	0.41 (1)	10.00				
J-I	0 / 1270	-18.2	-18.2	0.37 (4)	10.00				
I-H	0 / 1270	-18.2	-18.2	0.37 (4)	10.00				
H-G	0 / 0	-18.2	-18.2	0.16 (4)	10.00				

DESIGN CRITERIASPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF**SPACING = 24.0 IN./C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.72")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.72")
CALCULATED VERT. DEFL.(TL)= L/999 (0.21")CSI: TC=0.99/1.00 (D-E:1) , BC=0.41/1.00 (J-K:1) ,
WB=0.99/1.00 (C-K:1) , SS=0.38/1.00 (E-F:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

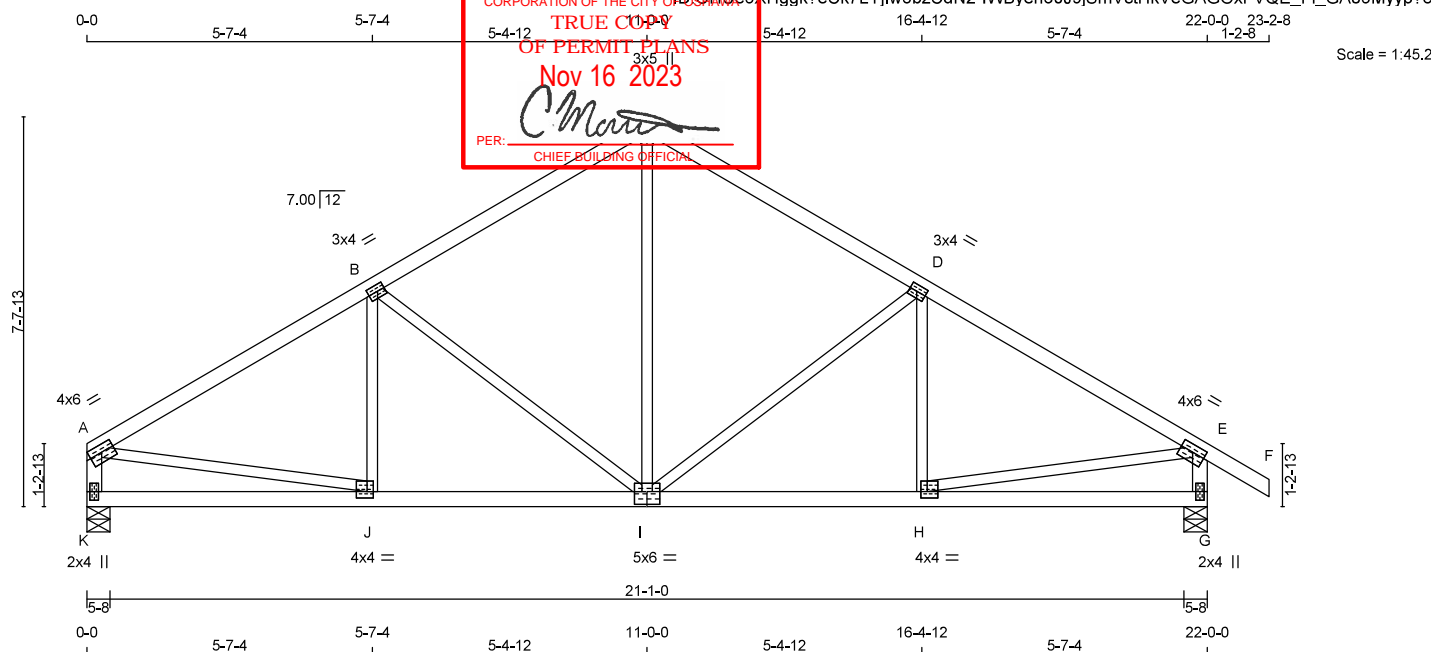
JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL = 0.50 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-062	T13	1	1	GREENPARK - ZADORRA ESTATES - CAROL 12-2	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:31 2023 Page 1



TOTAL WEIGHT = 89 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY - SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	4.0	6.0	1.75	Edge
B	TMVW-I	MT20	3.0	4.0	1.50	1.75
C	TTW+p	MT20	3.0	5.0		
D	TMVW-I	MT20	3.0	4.0	1.50	1.75
E	TMVW-I	MT20	4.0	6.0	1.75	3.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-I	MT20	4.0	4.0	1.50	1.50
I	BSWWWW-I	MT20	5.0	6.0	3.00	3.00
J	BMWW-I	MT20	4.0	4.0	1.50	1.50
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER							
BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQD
	GROSS	REACTION	GROSS	REACTION	UPLIFT	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1514	0	1514	0	0	5-8	1-10
G	1668	0	1668	0	0	5-8	1-15

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1058	766 / 0	0 / 0	0 / 0	0 / 0	293 / 0	0 / 0
G	1163	855 / 0	0 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOCAL CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	-1842 / 0	-119.4	-119.4, 0.52 (1)	4.35	I-C	0 / 840	0.19 (1)
B-C	-1355 / 0	-119.4	-119.4, 0.49 (1)	4.96	I-D	-620 / 0	0.54 (1)
C-D	-1355 / 0	-119.4	-119.4, 0.49 (1)	4.96	H-D	-148 / 66	0.04 (1)
D-E	-1842 / 0	-119.4	-119.4, 0.52 (1)	4.35	B-I	-620 / 0	0.54 (1)
E-F	0 / 39	-119.4	-119.4, 0.14 (1)	10.00	J-B	-148 / 66	0.04 (1)
K-A	-1472 / 0	0.0	0.0, 0.15 (1)	6.74	A-J	0 / 1647	0.37 (1)
G-E	-1625 / 0	0.0	0.0, 0.17 (1)	6.49	H-E	0 / 1647	0.37 (1)
K-J	0 / 0	-18.2	-18.2, 0.15 (4)	10.00			
J-I	0 / 1625	-18.2	-18.2, 0.33 (1)	10.00			
I-H	0 / 1625	-18.2	-18.2, 0.33 (1)	10.00			
H-G	0 / 0	-18.2	-18.2, 0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL
BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.52/1.00 (A-B:1) , BC=0.33/1.00 (I-J:1) ,
WB=0.54/1.00 (B-I:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.55 (H) (INPUT = 1.00)

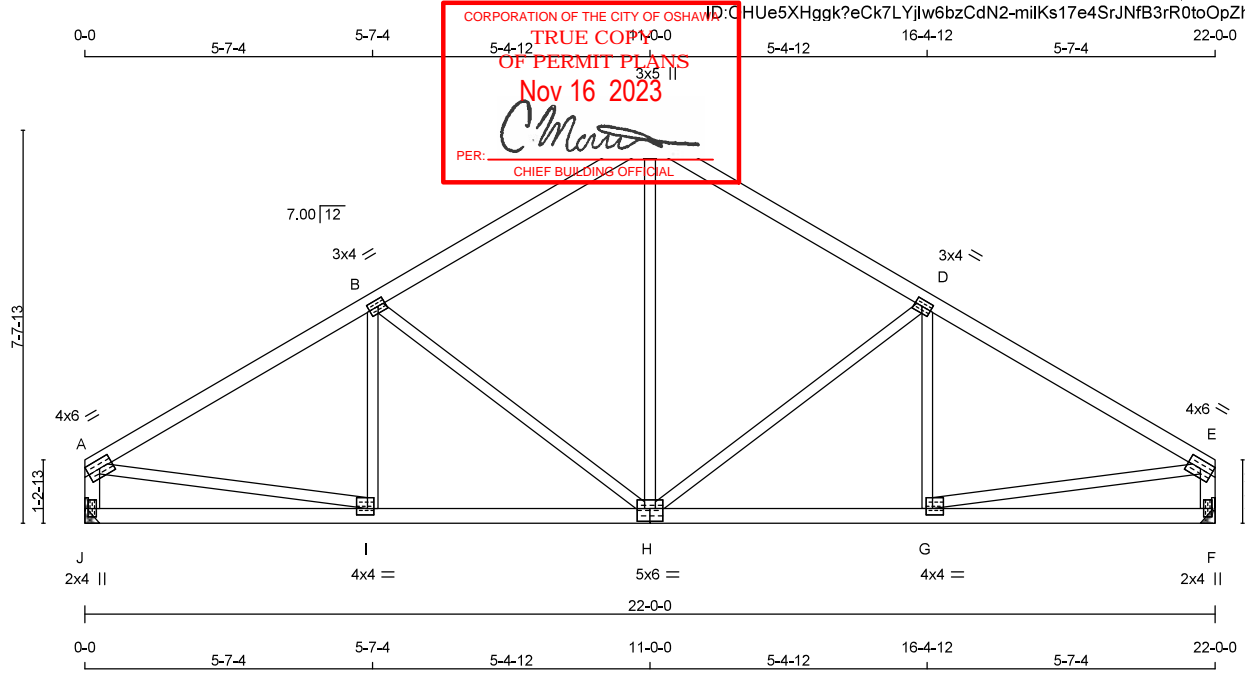


**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



JOB NAME NE0723-062	TRUSS NAME T14	QUANTITY 4	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - CAROL 12-2	DRWG NO.
------------------------	-------------------	---------------	----------	---	----------

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:32 2023 Page 1



Scale = 1:44.8

TOTAL WEIGHT = 4 X 88 = 350 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-H	MT20	4.0	6.0		Edge
B	TMVW-H	MT20	3.0	4.0	1.50	1.75
C	TTVW-p	MT20	3.0	5.0		
D	TMVW-H	MT20	3.0	4.0	1.50	1.75
E	TMVW-H	MT20	4.0	6.0		Edge
F	BMV1+p	MT20	2.0	4.0		
G	BMVW-H	MT20	4.0	4.0	1.50	1.50
H	BSVW-H	MT20	5.0	6.0	3.00	3.00
I	BMVW-H	MT20	4.0	4.0	1.50	1.50
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING****DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	1514	0	1514	0
F	1514	0	1514	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT
JOINT J = 1-10, JOINT F = 1-10.**UNFACTORED REACTIONS**

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1058	766 / 0	0 / 0	0 / 0	0 / 0	293 / 0	0 / 0
F	1058	766 / 0	0 / 0	0 / 0	0 / 0	293 / 0	0 / 0

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	-1842 / 0	-119.4	-119.4 0.52 (1)	4.35	H-C	0 / 841	0.19 (1)
B-C	-1355 / 0	-119.4	-119.4 0.49 (1)	4.96	H-D	-620 / 0	0.54 (1)
C-D	-1355 / 0	-119.4	-119.4 0.49 (1)	4.96	G-D	-148 / 66	0.04 (1)
D-E	-1842 / 0	-119.4	-119.4 0.52 (1)	4.35	B-H	-620 / 0	0.54 (1)
J-A	-1472 / 0	0.0	0.0 0.15 (1)	6.74	I-B	-148 / 66	0.04 (1)
F-E	-1472 / 0	0.0	0.0 0.15 (1)	6.74	A-I	0 / 1647	0.37 (1)
					G-E	0 / 1647	0.37 (1)
J-I	0 / 0	-18.2	-18.2 0.15 (4)	10.00			
I-H	0 / 1625	-18.2	-18.2 0.33 (1)	10.00			
H-G	0 / 1625	-18.2	-18.2 0.33 (1)	10.00			
G-F	0 / 0	-18.2	-18.2 0.15 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN./CTHIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL
BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")CSI: TC=0.52/1.00 (D-E:1), BC=0.33/1.00 (G-H:1),
WB=0.54/1.00 (D-H:1), SSI=0.28/1.00 (D-E:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

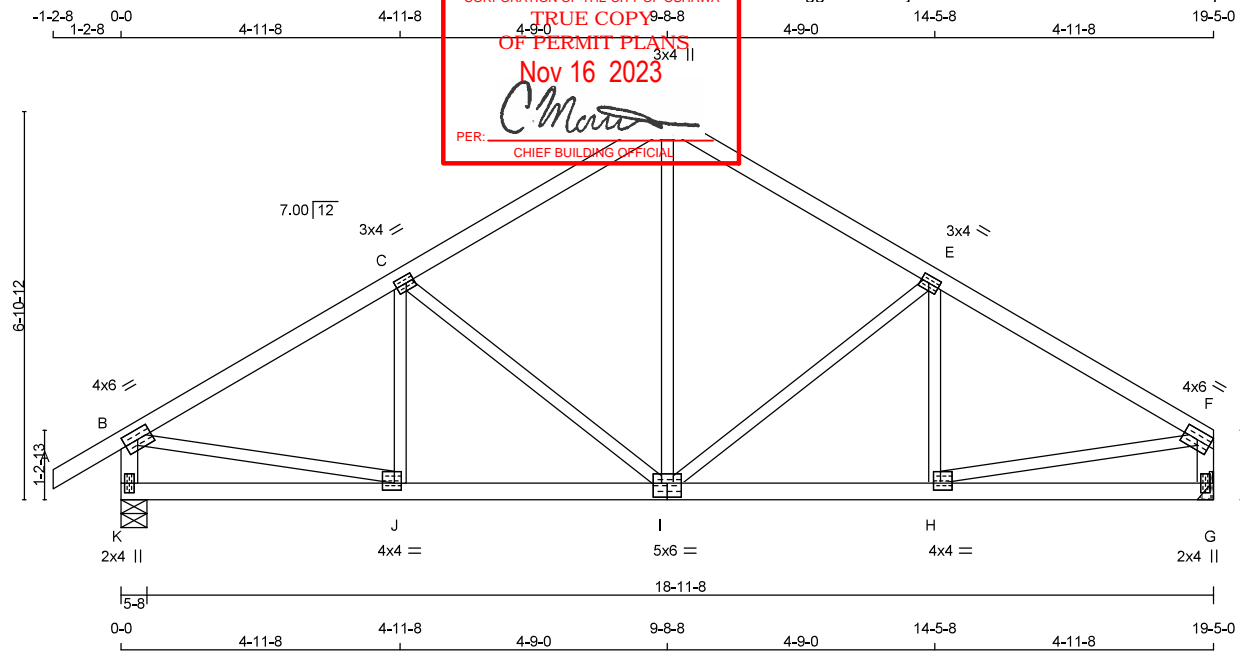
JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.55 (G) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.



JOB NAME NE0723-062	TRUSS NAME T15	QUANTITY 2	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - CAROL 12-2	DRWG NO.
------------------------	-------------------	---------------	----------	---	----------

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:32 2023 Page 1



Scale = 1:41.0

TOTAL WEIGHT = 2 X 79 = 159 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW4	MT20	4.0	6.0	2.00	3.00
C	TMWW4	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMWW4	MT20	3.0	4.0	1.50	1.75
F	TMWW4	MT20	4.0	6.0		Edge
G	BMV1+p	MT20	2.0	4.0		
H	BMWW4	MT20	4.0	4.0	1.50	1.50
I	BSWW4	MT20	5.0	6.0	3.00	3.00
J	BMWW4	MT20	4.0	4.0	1.50	1.50
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1490	0	1490	0	0	5-8	1-10
G	1337	0	1337	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1039	765 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
G	934	676 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 39	-119.4	-119.4	0.14 (1)	10.00	I-D	0 / 723	0.16 (1)
B-C	-1577 / 0	-119.4	-119.4	0.39 (1)	4.82	I-E	-520 / 0	0.34 (1)
C-D	-1176 / 0	-119.4	-119.4	0.37 (1)	5.43	H-E	-155 / 52	0.04 (1)
D-E	-1176 / 0	-119.4	-119.4	0.37 (1)	5.43	C-I	-520 / 0	0.34 (1)
E-F	-1577 / 0	-119.4	-119.4	0.39 (1)	4.82	J-C	-155 / 52	0.04 (1)
K-B	-1452 / 0	0.0	0.0	0.15 (1)	6.79	B-J	0 / 1415	0.32 (1)
G-F	-1298 / 0	0.0	0.0	0.13 (1)	7.08	H-F	0 / 1415	0.32 (1)
K-J	0 / 0	-18.2	-18.2	0.11 (4)	10.00			
J-I	0 / 1391	-18.2	-18.2	0.27 (1)	10.00			
I-H	0 / 1391	-18.2	-18.2	0.27 (1)	10.00			
H-G	0 / 0	-18.2	-18.2	0.11 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.65")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.09")CSI: TC=0.39/1.00 (E-F:1), BC=0.27/1.00 (I-J:1),
WB=0.34/1.00 (E-I:1), SSI=0.24/1.00 (E-F:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

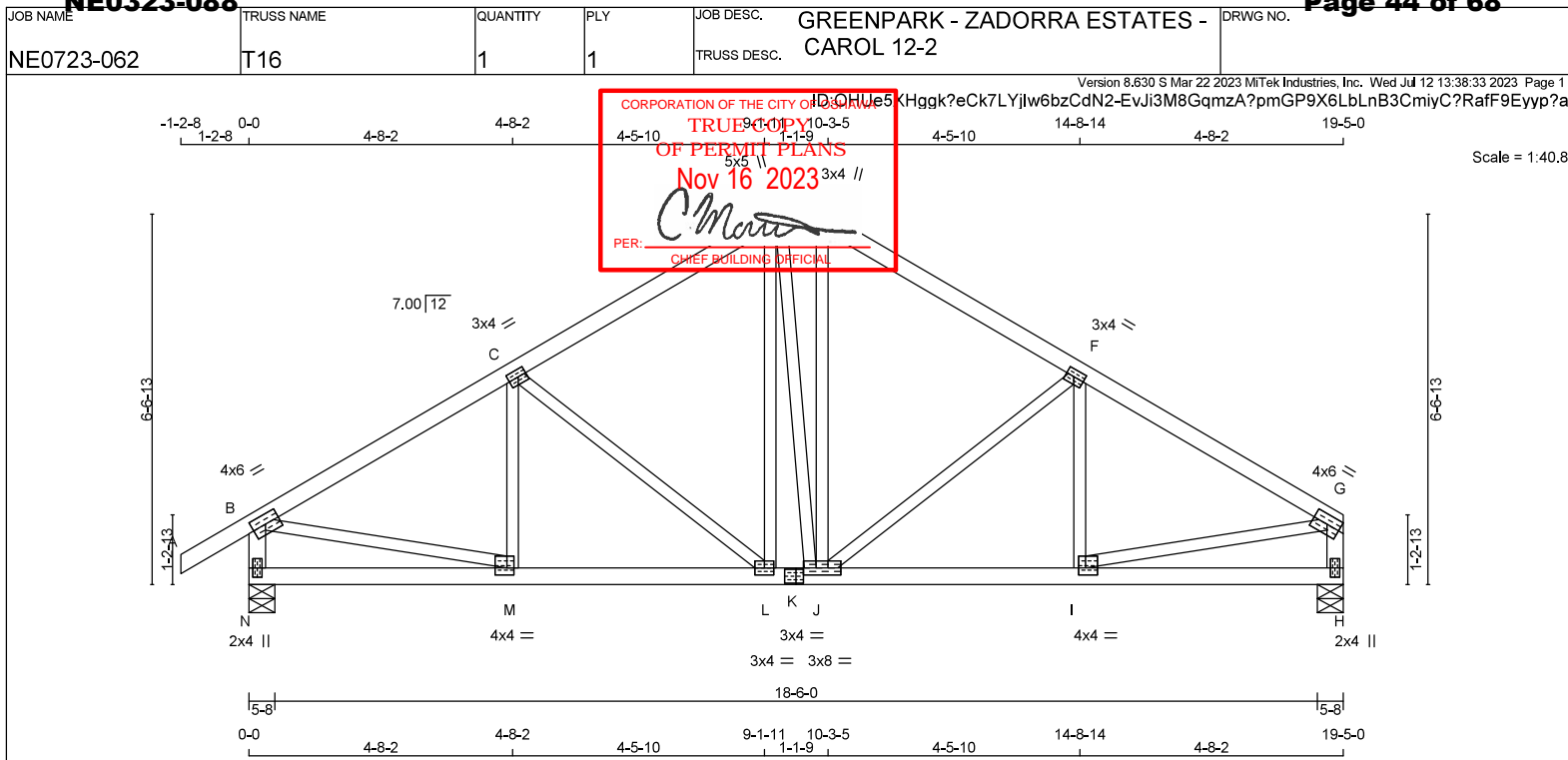
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.47 (J) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 88 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - G	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW4	MT20	4.0	6.0	2.00	3.00
C	TMVW4	MT20	3.0	4.0	1.50	1.75
D	TTWV+m	MT20	5.0	5.0	2.50	1.25
E	TTWV+m	MT20	3.0	4.0	2.00	1.25
F	TMVW4	MT20	3.0	4.0	1.50	1.75
G	TMVW4	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	2.0	4.0		
I	BMVW4	MT20	4.0	4.0	1.50	1.50
J	BMVW4	MT20	3.0	8.0		
K	BS-t	MT20	3.0	4.0	1.50	1.50
L	BMVW4	MT20	3.0	4.0		
M	BMVW4	MT20	4.0	4.0	1.50	1.50
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING****DESIGNER****BEARINGS**

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	1490	0	1490	0
H	1337	0	1337	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1039	765 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
H	934	676 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0 / 39	-119.4	-119.4 0.14 (1)	10.00	M-C	-180 / 36	0.04 (1)
B-C	-1576 / 0	-119.4	-119.4 0.35 (1)	4.88	C-L	-461 / 0	0.27 (1)
C-D	-1228 / 0	-119.4	-119.4 0.33 (1)	5.40	L-D	0 / 307	0.07 (1)
D-E	-1038 / 0	-119.4	-119.4 0.03 (1)	6.15	D-J	0 / 27	0.01 (1)
E-F	-1230 / 0	-119.4	-119.4 0.33 (1)	5.40	J-E	0 / 337	0.08 (1)
F-G	-1574 / 0	-119.4	-119.4 0.35 (1)	4.89	J-F	-454 / 0	0.27 (1)
G-H	-1453 / 0	0.0	0.0 0.15 (1)	6.78	F-I	-188 / 33	0.05 (1)
H-G	-1298 / 0	0.0	0.0 0.13 (1)	7.08	I-F	0 / 1416	0.32 (1)
					I-G	0 / 1414	0.32 (1)
N-M	0 / 0	-18.2	-18.2 0.09 (4)	10.00			
M-L	0 / 1389	-18.2	-18.2 0.26 (1)	10.00			
L-K	0 / 1034	-18.2	-18.2 0.22 (1)	10.00			
K-J	0 / 1034	-18.2	-18.2 0.22 (1)	10.00			
J-I	0 / 1387	-18.2	-18.2 0.27 (1)	10.00			
I-H	0 / 0	-18.2	-18.2 0.09 (4)	10.00			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL)= L/999 (0.08")CSI: TC=0.35/1.00 (B-C:1) , BC=0.27/1.00 (I-J:1) ,
WB=0.32/1.00 (B-M:1) , SSI=0.23/1.00 (B-C:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PL)
(PSI) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

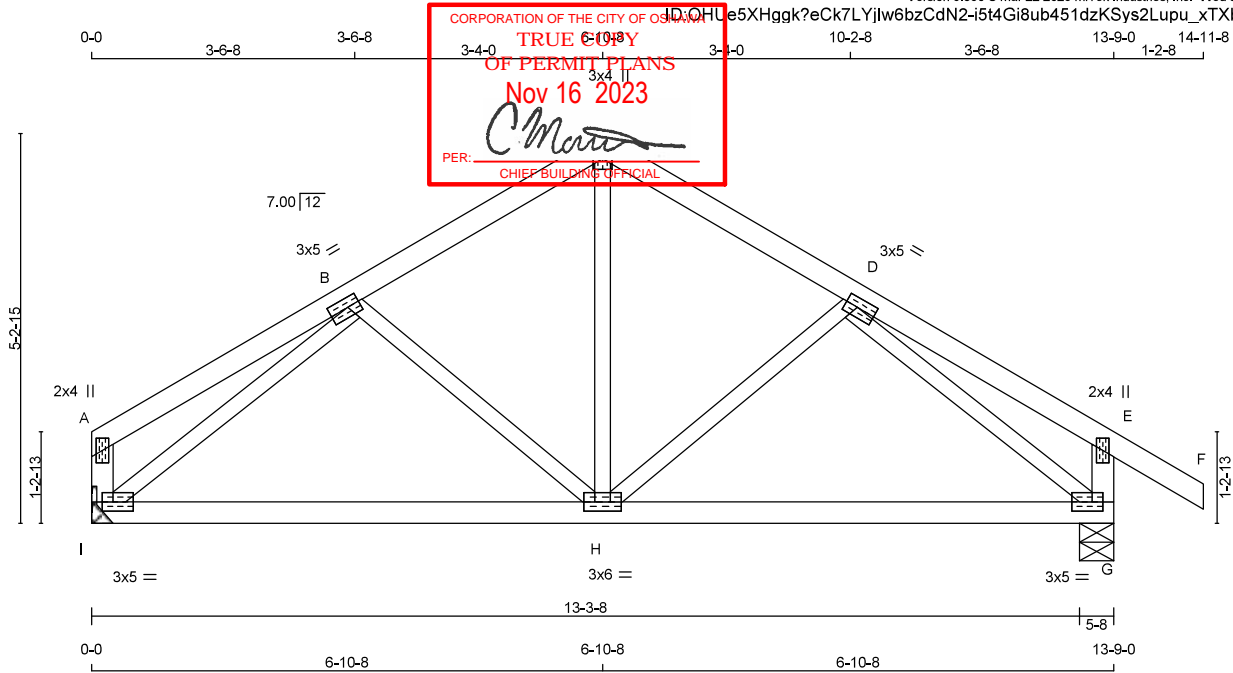
JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL = 0.47 (M) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-062	T17	2	1	GREENPARK - ZADORRA ESTATES - CAROL 12-2	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:34 2023 Page 1



Scale = 1:31.0

TOTAL WEIGHT = 2 X 55 = 110 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - A	2x4	DRY	No.2
G - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
--------------------	-----	-----	------

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMVWW-t	MT20	3.0	5.0	1.50	2.00
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVWW-t	MT20	3.0	5.0	1.50	2.00
E	TMV+p	MT20	2.0	4.0		
G	BMVWV1-t	MT20	3.0	5.0	1.50	1.75
H	BMVWWWW-t	MT20	3.0	6.0		
I	BMVWV1-t	MT20	3.0	5.0	1.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS						
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX MECHANICAL
I	947	0	947	0	0	
G	1100	0	1100	0	0	5-8 1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	661	479 / 0	0 / 0	0 / 0	0 / 0	183 / 0	0 / 0
G	766	568 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 CSI (LC)	MAX. UNBRAC.	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 23	-119.4	-119.4	0.22 (1)	10.00	H-C	0 / 474
B-C	-810 / 0	-119.4	-119.4	0.17 (1)	6.25	H-D	-247 / 0
C-D	-810 / 0	-119.4	-119.4	0.17 (1)	6.25	B-H	-247 / 0
D-E	0 / 23	-119.4	-119.4	0.22 (1)	10.00	I-B	-1139 / 0
E-F	0 / 39	-119.4	-119.4	0.14 (1)	10.00	D-G	-1139 / 0
I-A	-160 / 0	0.0	0.0	0.02 (1)	7.81		0.37 (1)
G-E	-313 / 0	0.0	0.0	0.03 (1)	7.81		
I-H	0 / 867	-18.2	-18.2	0.30 (4)	10.00		
H-G	0 / 868	-18.2	-18.2	0.30 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:			
TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.22/1.00 (D-E:1) , BC=0.30/1.00 (G-H:4) ,
WB=0.37/1.00 (D-G:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

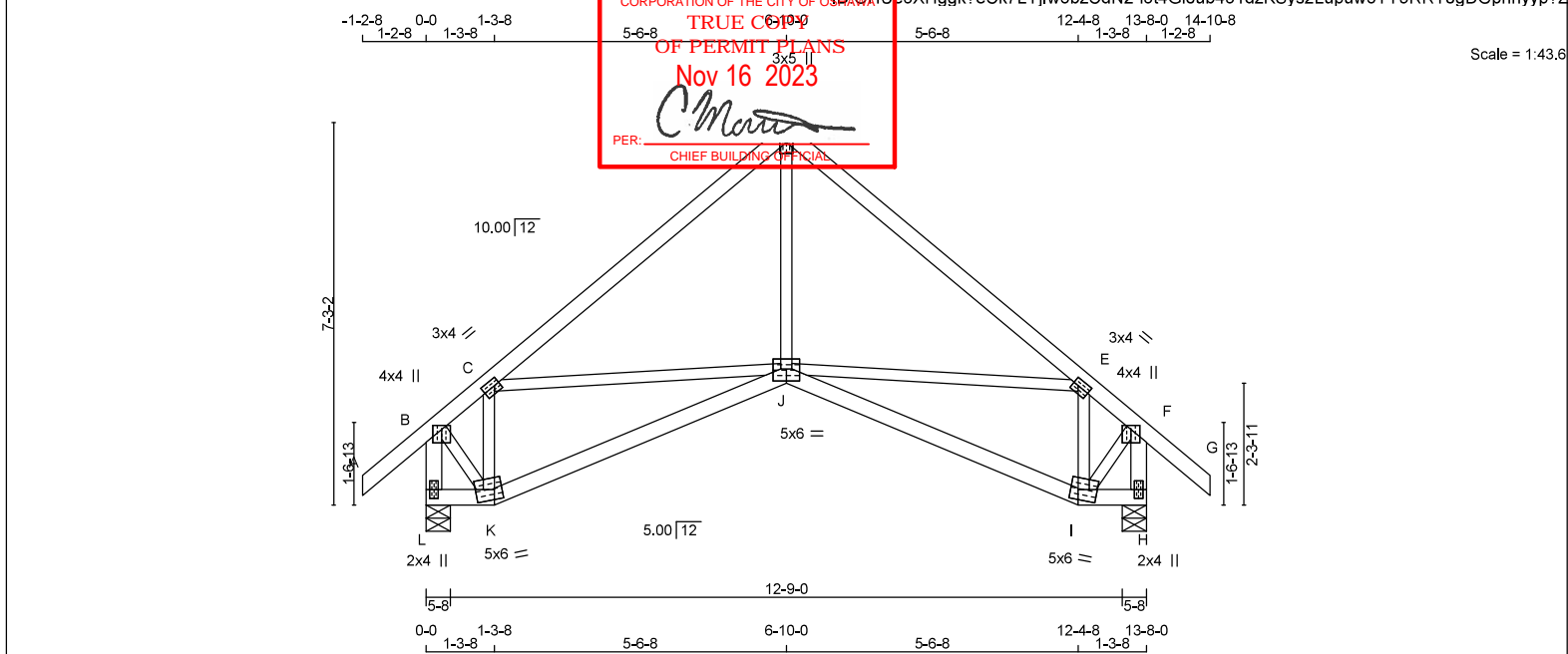


**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



JOB NAME NE0723-062	TRUSS NAME T18	QUANTITY 2	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - CAROL 12-2	DRWG NO.
------------------------	-------------------	---------------	----------	---	----------

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:34 2023 Page 1



Scale = 1:43.6

TOTAL WEIGHT = 2 X 62 = 124 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-H	MT20	3.0	4.0	1.50	1.25
D	TTW+p	MT20	3.0	5.0		
E	TMVW-H	MT20	3.0	4.0	1.50	1.25
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BBVW-m	MT20	5.0	6.0	2.25	1.75
J	BBVW-m	MT20	5.0	6.0	2.75	3.00
K	BBVW-m	MT20	5.0	6.0	2.25	1.75
L	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
L 1096	0	0	5-8
H 1096	0	0	1-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
L 764	566 / 0 0 / 0 0 / 0 0 / 0 197 / 0 0 / 0
H 764	566 / 0 0 / 0 0 / 0 0 / 0 197 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		
A-B	0 / 50	-119.4 -119.4	0.15 (1)	J-D	0 / 585	0.13 (1)
B-C	-647 / 0	-119.4 -119.4	0.40 (1)	J-E	0 / 101	0.02 (1)
C-D	-822 / 0	-119.4 -119.4	0.48 (1)	I-E	-758 / 0	0.12 (1)
D-E	-822 / 0	-119.4 -119.4	0.48 (1)	C-J	0 / 101	0.02 (1)
E-F	-647 / 0	-119.4 -119.4	0.40 (1)	K-C	-758 / 0	0.12 (1)
F-G	0 / 50	-119.4 -119.4	0.15 (1)	B-K	0 / 842	0.19 (1)
L-B	-1084 / 0	0.0 0.0	0.11 (1)	I-F	0 / 842	0.19 (1)
H-F	-1084 / 0	0.0 0.0	0.11 (1)			
L-K	0 / 0	-18.2 -18.2	0.01 (4)			
K-J	0 / 623	-18.3 -18.3	0.20 (4)			
J-I	0 / 623	-18.2 -18.2	0.20 (4)			
I-H	0 / 0	-18.2 -18.2	0.01 (4)			

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")CSI: TC=0.46/1.00 (C-D:1) , BC=0.20/1.00 (J-K:4) ,
WB=0.19/1.00 (B-K:1) , SSI=0.24/1.00 (C-D:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.30 (K) (INPUT = 1.00)

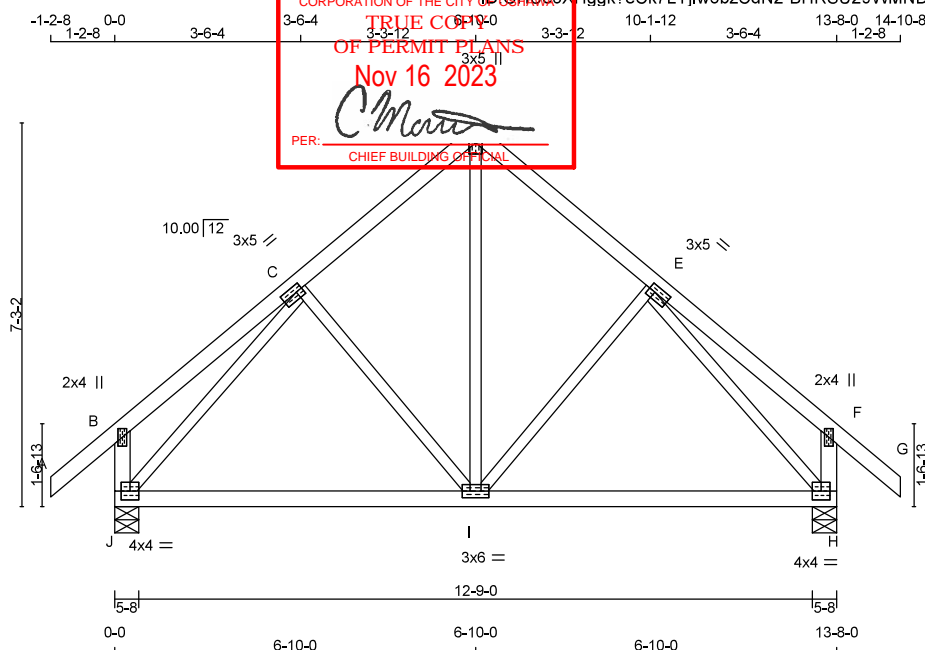
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-062	T19	2	1	GREENPARK - ZADORRA ESTATES - CAROL 12-2	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:35 2023 Page 1

Scale = 1:43.6



TOTAL WEIGHT = $2 \times 64 = 128$ lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
--------------------	-----	-----	------

DRY- SEASONED LUMBER

PLATES (table is in inches)		W	LEN	Y	X
JT	TMV+p	MT20	2.0	4.0	
B	TMVW+t	MT20	3.0	5.0	1.50
D	TTW+p	MT20	3.0	5.0	
E	TMVW+t	MT20	3.0	5.0	1.50
F	TMV+p	MT20	2.0	4.0	
H	BMVW1+	MT20	4.0	4.0	
I	BMVWVW+	MT20	3.0	6.0	
J	RMVW1+	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS, AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER							
BEARINGS							
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	1096	0	1096	0	0	5-8	1-8
H	1096	0	1096	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	764	566 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0
H	764	566 / 0	0 / 0	0 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (L)	MAX. UNBRAC TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (L)
FR-TO		FROM			FR-TO		
A-B	0 / 50	-119.4	-119.4 0.15 (1)	10.00	I-D	0 / 478	0.11 (1)
B-C	0 / 30	-119.4	-119.4 0.22 (1)	10.00	I-E	-213 / 0	0.10 (1)
C-D	-652 / 0	-119.4	-119.4 0.17 (1)	6.25	C-I	-213 / 0	0.10 (1)
D-E	-652 / 0	-119.4	-119.4 0.17 (1)	6.25	J-C	-961 / 0	0.43 (1)
E-F	0 / 30	-119.4	-119.4 0.22 (1)	10.00	E-H	-961 / 0	0.43 (1)
F-G	0 / 50	-119.4	-119.4 0.15 (1)	10.00			
J-B	-311 / 0	0.0	0.0 0.03 (1)	7.81			
H-F	-311 / 0	0.0	0.0 0.03 (1)	7.81			
J-I	0 / 616	-18.2	-18.2 0.28 (4)	10.00			
I-H	0 / 616	-18.2	-18.2 0.28 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	34.8	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.22/1.00 (E-F:1) , BC=0.28/1.00 (I-J:4) ,
WB=0.43/1.00 (E-H:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

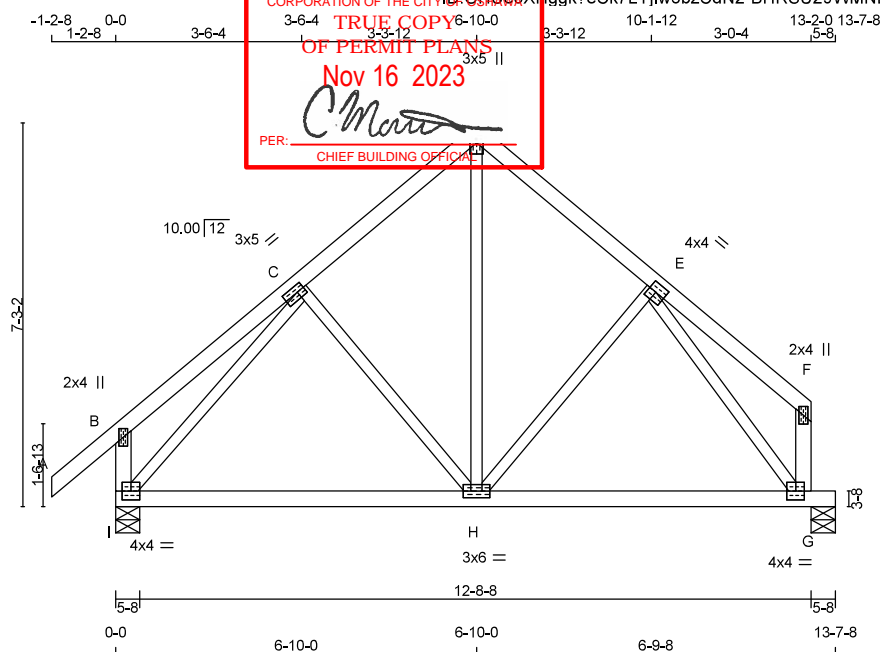
JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:35 2023 Page 1



Scale = 1:43.6

TOTAL WEIGHT = $3 \times 62 = 185$ lb

LUMBER

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
--------------------	-----	-----	------

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWV-t	MT20	3.0	5.0	1.50	1.75
D	TTW+p	MT20	3.0	5.0		
E	TMWV-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	2.0	4.0		
G	BMWV1-t	MT20	4.0	4.0		
H	BMWVWV-t	MT20	3.0	6.0		
I	BMVWV1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS, AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DIMENSION
DESIGNER

BEARINGS		FACTORED			MAXIMUM FACTORED		INPUT	REQRD
		GROSS REACTION		GROSS REACTION		BRG		BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
I	1062	0	1062	0	0	5-8	1-8	
G	906	0	906	0	0	5-8	5-0	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	740	549/0	0/0	0/0	0/0	191/0	0/0
G	633	458/0	0/0	0/0	0/0	175/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I. G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 50	-119.4	-119.4	0.15 (1)	10.00	C-H	-222 / 0
B-C	0 / 30	-119.4	-119.4	0.22 (1)	10.00	H-D	0 / 414
C-D	-607 / 0	-119.4	-119.4	0.17 (1)	6.25	E-E	-128 / 7
D-E	-603 / 0	-119.4	-119.4	0.16 (1)	6.25	I-C	-915 / 0
E-F	0 / 31	-119.4	-119.4	0.19 (1)	10.00	E-G	-907 / 0
I-B	-314 / 0	0.0	0.0	0.03 (1)	7.81		
G-F	-1210 / 0	0.0	0.0	0.01 (1)	7.81		
I-H	0 / 587	-18.2	-18.2	0.26 (4)	10.00		
H-G	0 / 526	-18.2	-18.2	0.26 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	34.8	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL
BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.26/1.00 (H-I:4) ,
WB=0.41/1.00 (C-I:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

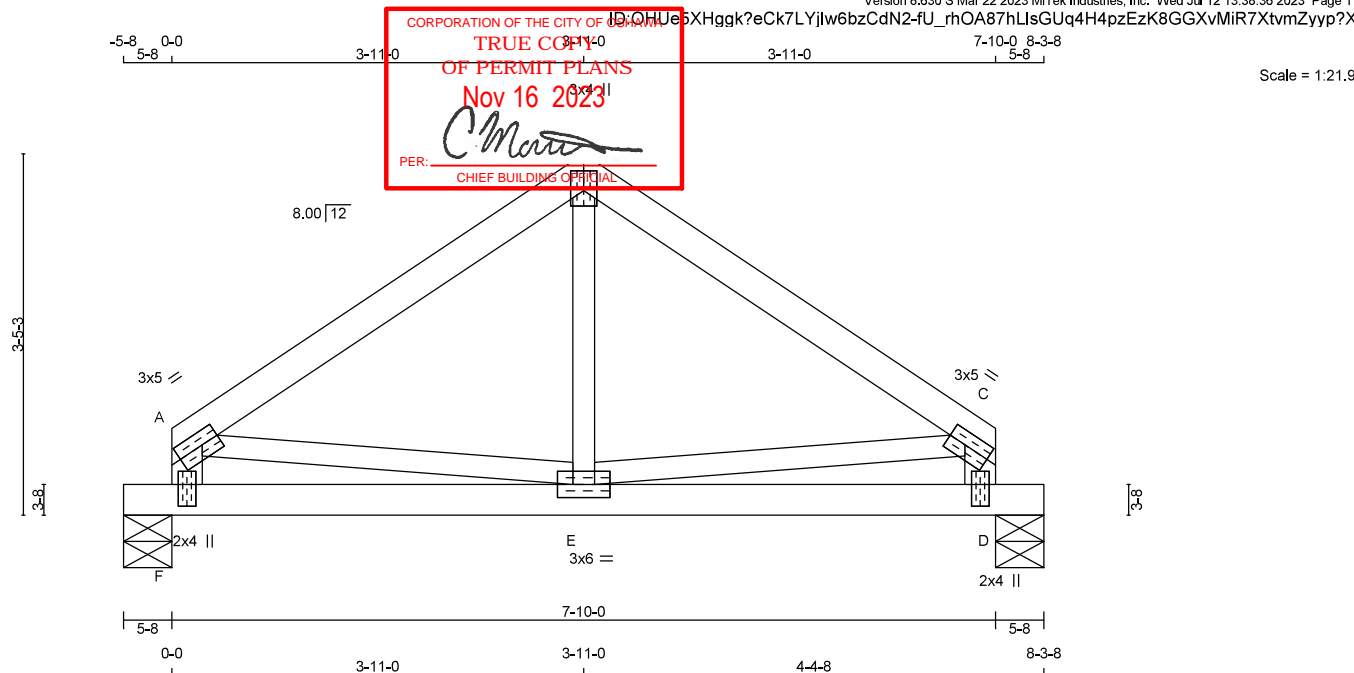


**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-062	T21	1	1	TRUSS DESC.	CAROL 12-2	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:36 2023 Page 1



TOTAL WEIGHT = 30 lb

LUMBER		
N. L. G. A. RULES		
CHORDS	SIZE	LUMBER
A - B	2x4	NO.2
B - C	2x4	NO.2
F - A	2x4	NO.2
D - C	2x4	NO.2
E - D	2x4	NO.2

ALL WEBS EXCEPT	2x3	DRY	No.2
--------------------	-----	-----	------

DRY- SEASONED LUMBER

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	MTWw4	MT20	3.0	5.0	1.50	2.00
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	MTWw4	MT20	3.0	5.0	1.50	2.00
D	BMV1+p	MT20	2.0	4.0	2.50	1.00
E	BMWWWWw4	MT20	3.0	6.0		
F	BMV1+p	MT20	2.0	4.0	2.50	1.00

PLATES (table is in inches)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER						
BEARINGS						
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
F	539	0	539	0	0	5-8
D	539	0	539	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	377	273 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0
D	377	273 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F. D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD	LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	PLF	LENGTH	FR-TO			
A-B	-418 / 0	-119.4	-119.4	0.24 (1)	6.25	E-B	-4 / 72	0.02 (4)	
B-C	-418 / 0	-119.4	-119.4	0.24 (1)	6.25	A-E	0 / 351	0.08 (1)	
F-A	-510 / 0	0.0	0.0	0.05 (1)	7.81	E-C	0 / 351	0.08 (1)	
D-C	-510 / 0	0.0	0.0	0.05 (1)	7.81				
F-E	0 / 0	-18.2	-18.2	0.08 (4)	10.00				
E-D	0 / 0	-18.2	-18.2	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS.			
TOP	CH.	LL =	34.8 PSF
		DL =	6.0 PSF
BOT	CH.	LL =	0.0 PSF
		DL =	7.3 PSF
TOTAL LOAD		=	48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL
BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.08/1.00 (D-E:4) ,
WB=0.08/1.00 (A-E:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (E) (INPUT = 0.90)
JSI METAL= 0.16 (C) (INPUT = 1.00)

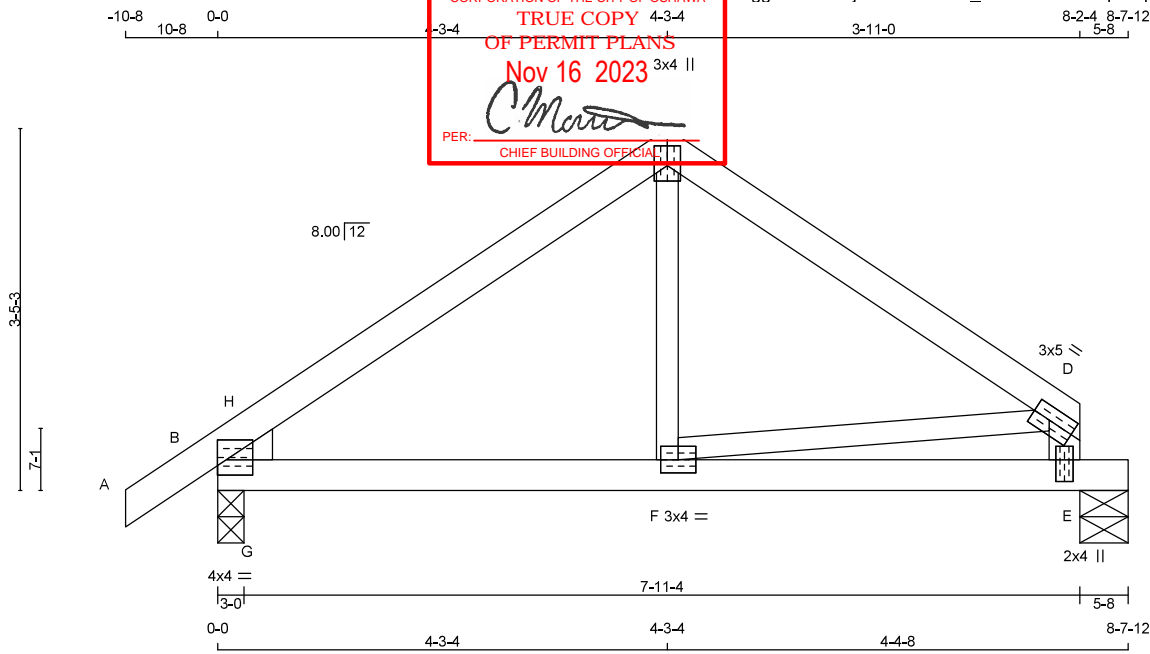


**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-062	T22	1	1	GREENPARK - ZADORRA ESTATES - CAROL 12-2	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:36 2023 Page 1



Scale = 1:21.9

TOTAL WEIGHT = 29 lb

LUMBER			
N. L. G. A.	RULES		
CHORDS	SIZE		LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)			W	LEN	Y	X
JT	TYPE	PLATES				
B	TMBH1-I	MT20	4.0	4.0	1.75	Edge
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMVW-t	MT20	3.0	5.0	1.50	2.00
E	BMV1+p	MT20	2.0	4.0	2.50	1.00
F	BMWW-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER		FACTORED		MAXIMUM FACTORED		INPUT	REQRD	
BEARINGS		GROSS REACTION		GROSS REACTION		BRG	BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	678	0	678	0	0	3-0	1-8	2x4 L
E	564	0	564	0	0	5-8	5-0	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	472	352 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0
E	394	285 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED L1 (LC)	MAX. UNBRAC (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED L1 (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 21	-119.4	-119.4	0.08 (1)	10.00	F-C	0 / 87
B-H	-437 / 0	-119.4	-119.4	0.11 (1)	6.25	F-D	0 / 393
H-C	-484 / 0	-119.4	-119.4	0.23 (1)	6.25	G-H	-288 / 15
C-D	-469 / 0	-119.4	-119.4	0.24 (1)	6.25		0.00 (1)
E-D	-544 / 0	0.0	0.0	0.05 (1)	7.81		
B-G	0 / 390	-18.2	-18.2	0.21 (1)	10.00		
G-F	0 / 390	-18.2	-18.2	0.21 (1)	10.00		
F-E	0 / 0	-18.2	-18.2	0.09 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.24/1.00 (C-D:1), BC=0.21/1.00 (F-G:1),
WB=0.09/1.00 (D-F:1), SSI=0.22/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

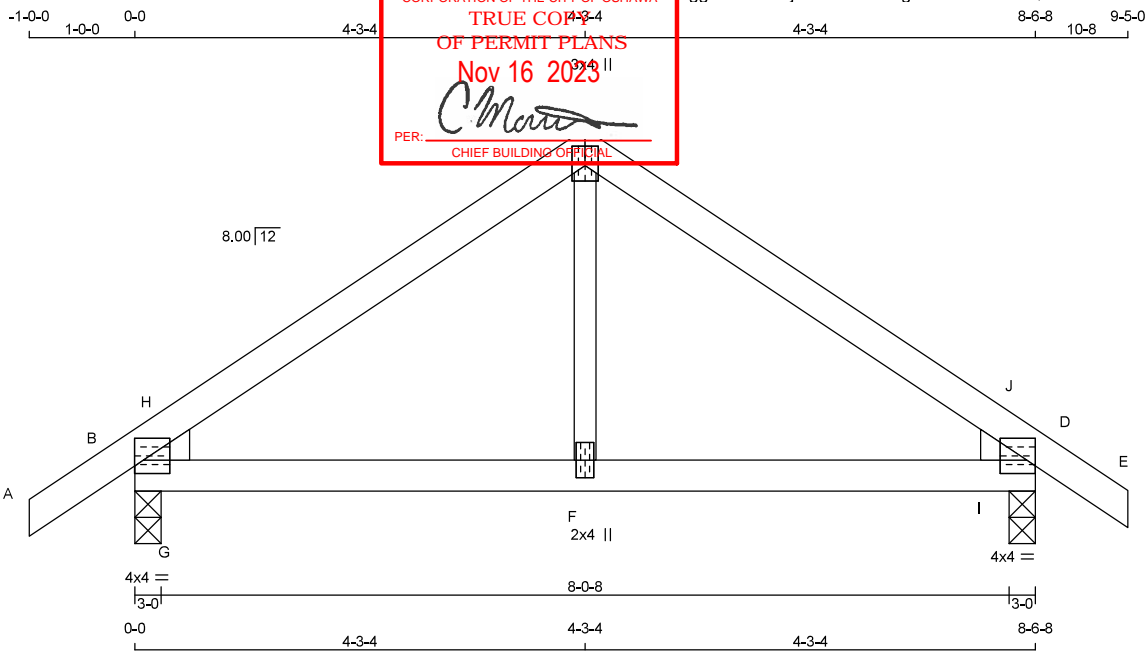


**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



JOB NAME NE0723-062	TRUSS NAME T23	QUANTITY 2	PLY 1	JOB DESC. GREENPARK - ZADORRA ESTATES - CAROL 12-2	DRWG NO.
------------------------	-------------------	---------------	----------	---	----------

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:37 2023 Page 1



Scale = 1:21.8

TOTAL WEIGHT = 2 X 28 = 55 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	4.0	1.50	Edge
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMBH1-I	MT20	4.0	4.0	1.50	Edge
F	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	HEEL
B	717	0	717	0	0	3-0	1-8	2x4 L
D	702	0	702	0	0	3-0	1-8	2x4 R

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN.	COMPONENT REACTIONS
B	COMBINED	SNOW	LIVE
B	499	373 / 0	0 / 0
D	489	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							FR-TO			
A-B	0 / 25	-119.4	-119.4	0.10 (1)	10.00	F-C	0 / 177	0.04 (1)		
B-H	-485 / 0	-119.4	-119.4	0.13 (1)	6.25	G-H	-294 / 17	0.00 (1)		
H-C	-539 / 0	-119.4	-119.4	0.22 (1)	6.25	I-J	-294 / 17	0.00 (1)		
C-J	-539 / 0	-119.4	-119.4	0.22 (1)	6.25					
J-D	-485 / 0	-119.4	-119.4	0.13 (1)	6.25					
D-E	0 / 21	-119.4	-119.4	0.08 (1)	10.00					
B-G	0 / 434	-18.2	-18.2	0.22 (1)	10.00					
G-F	0 / 434	-18.2	-18.2	0.22 (1)	10.00					
F-I	0 / 434	-18.2	-18.2	0.22 (1)	10.00					
I-D	0 / 434	-18.2	-18.2	0.22 (1)	10.00					

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22/1.00 (C-J:1) , BC=0.22/1.00 (D-I:1) ,
WB=0.04/1.00 (C-F:1) , SSI=0.22/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

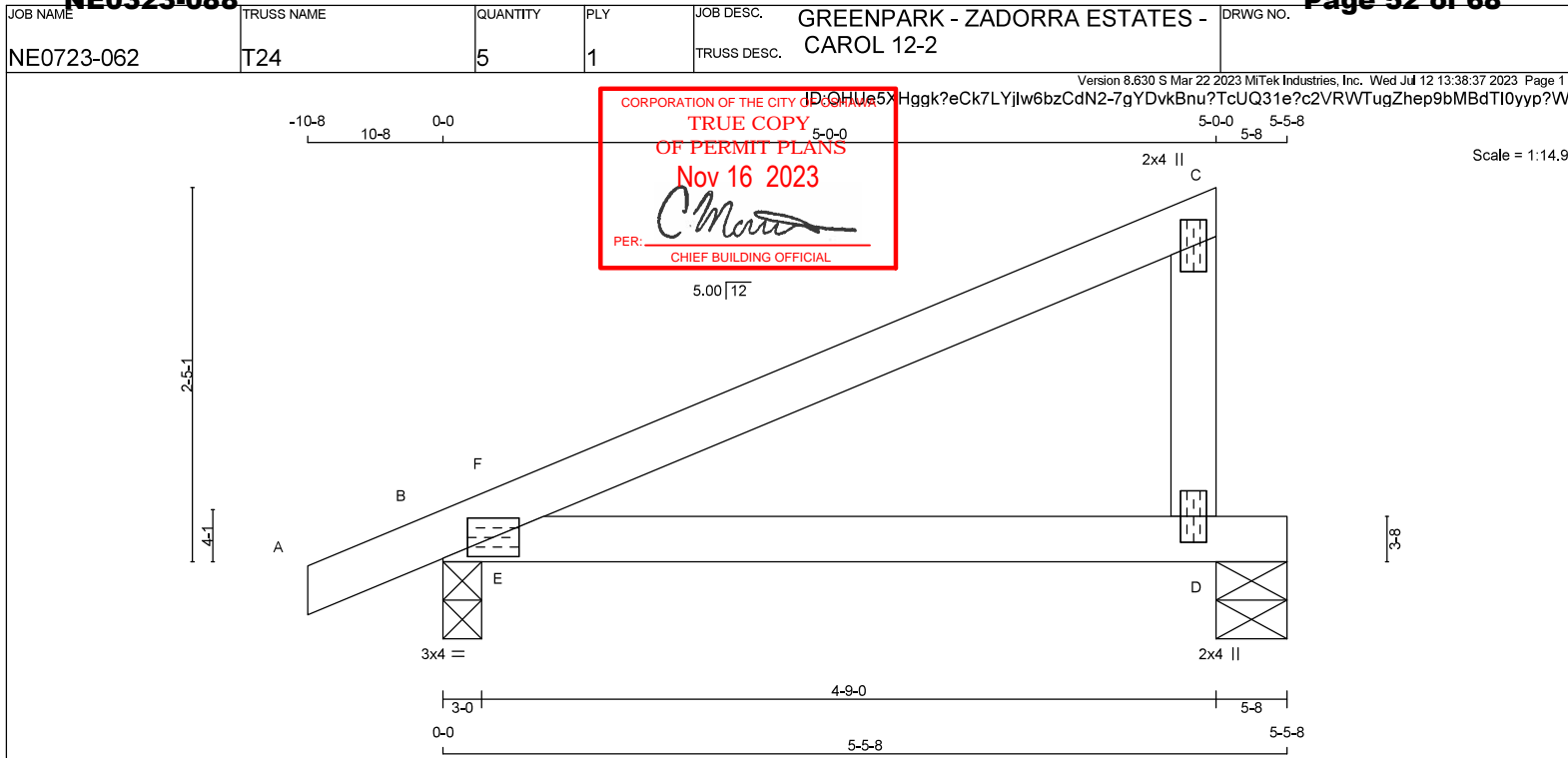
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.90)
JSI METAL = 0.18 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 5 X 16 = 78 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
D	344	344	0	5-8
B	455	455	0	3-0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	241	174 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
B	317	239 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-B	0 / 20		-119.4	-119.4	0.07 (1)	10.00							
B-F	-20 / 44		-119.4	-119.4	0.07 (1)	6.25							
F-C	-4 / 2		-119.4	-119.4	0.36 (1)	10.00							
D-C	-258 / 0		0.0	0.0	0.03 (1)	7.81							
B-E	0 / 0		-18.2	-18.2	0.27 (1)	10.00							
E-D	0 / 0		-18.2	-18.2	0.27 (1)	10.00							

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH. LL	=	34.8	PSF
DL	=	6.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/643 (0.09")CSI: TC=0.36/1.00 (C-F:1), BC=0.27/1.00 (B-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.33/1.00 (B-E:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

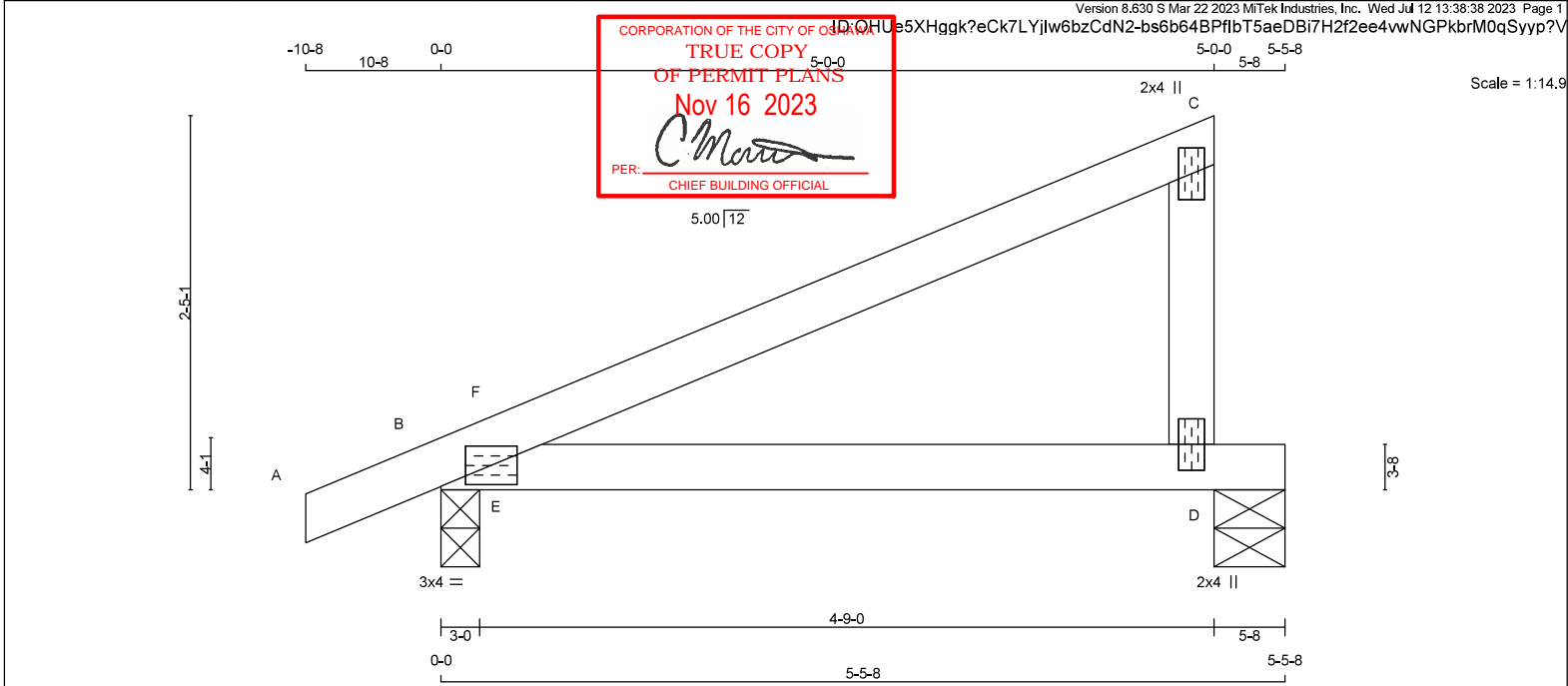
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL = 0.09 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



LUMBER				DESIGNER			
N. L. G. A. RULES				SPF			
CHORDS	SIZE	LUMBER		SPF	SPF	SPF	
A - C	2x4	DRY	No.2				
D - C	2x4	DRY	No.2				
B - D	2x4	DRY	No.2				

DRY: SEASONED LUMBER.

PLATES (table is in inches)				DESIGNER			
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMB1-I	MT20	3.0	4.0			
C	TMV+p	MT20	2.0	4.0			
D	BMV1+p	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING									
DESIGNER									
BEARINGS									
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
D	344	0	344	0	0	5-8	5-0	5-0	5-0
B	455	0	455	0	0	3-0	1-8		

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	241	174 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0
B	317	239 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 20	-119.4	-119.4 0.07 (1)	E-F	-412 / 8	0.00 (1)	
B-F	-20 / 44	-119.4	-119.4 0.07 (1)				
F-C	-4 / 2	-119.4	-119.4 0.36 (1)				
D-C	-258 / 0	0.0	0.0 0.03 (1)				
B-E	0 / 0	-18.2	-18.2 0.27 (1)				
E-D	0 / 0	-18.2	-18.2 0.27 (1)				

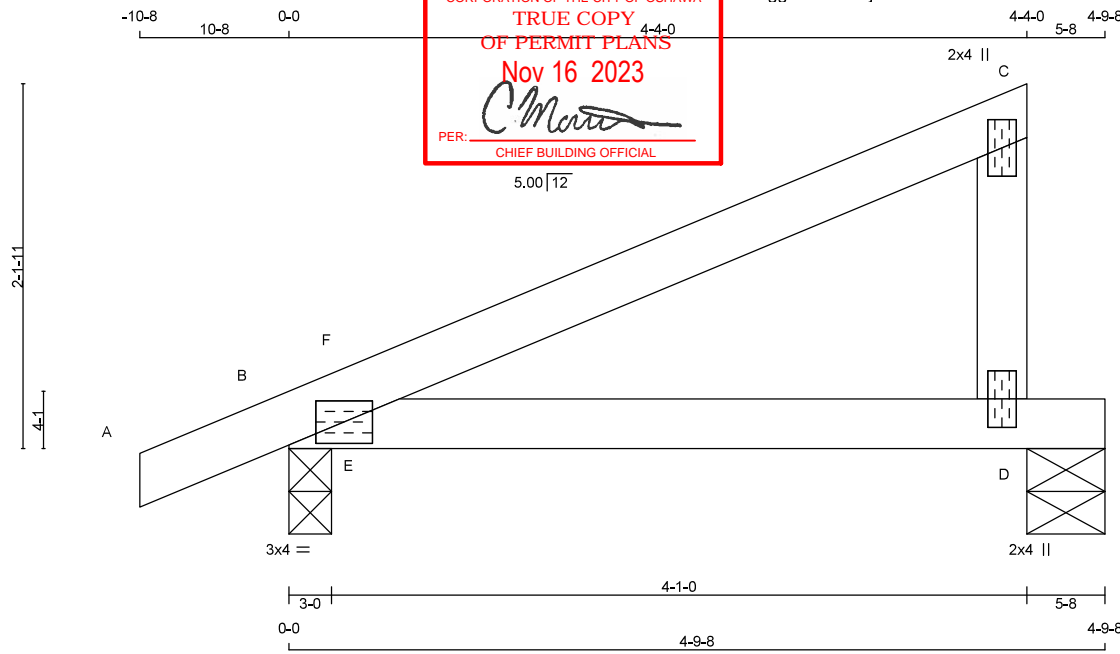


READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
NE0723-062	T26	2	1	GREENPARK - ZADORRA ESTATES - CAROL 12-2	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:38 2023 Page 1



Scale = 1:13.5

TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	
D - C	2x4	DRY	No.2	
B - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REQD
D	298	0	298	0	0	5-8	5-0
B	409	0	409	0	0	3-0	1-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	208	151 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
B	284	216 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACINGTOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1
FR-TO													
A-B	0 / 20		-119.4	-119.4	0.07 (1)	10.00							
B-F	-16 / 23		-119.4	-119.4	0.04 (1)	6.25							
F-C	-3 / 2		-119.4	-119.4	0.26 (1)	10.00							
D-C	-223 / 0		0.0	0.0	0.03 (1)	7.81							
B-E	0 / 0		-18.2	-18.2	0.21 (1)	10.00							
E-D	0 / 0		-18.2	-18.2	0.21 (1)	10.00							

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/971 (0.05")CSI: TC=0.26/1.00 (C-F:1), BC=0.21/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.25/1.00 (B-E:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

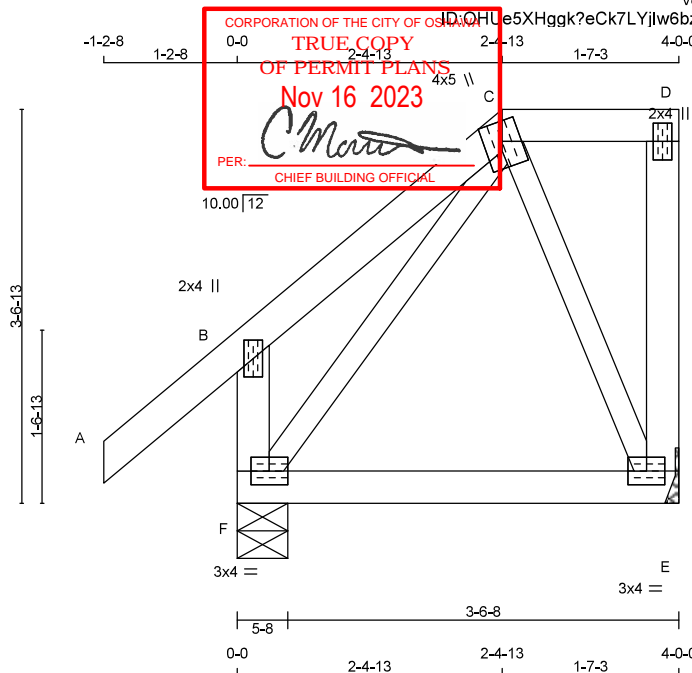
JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL = 0.08 (C) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES - CAROL 12-2	DRWG NO.
NE0723-062	T27	1	1	TRUSS DESC.		

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:39 2023 Page 1



Scale = 1:20.9

TOTAL WEIGHT = 22 lb

LUMBER		
N. L. G. A. RULES		
CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - D	2x4	DRY No.2
E - D	2x4	DRY No.2
F - B	2x4	DRY No.2
F - E	2x4	DRY No.2

ALL WEBS EXCEPT	2x3	DRY	No.2
--------------------	-----	-----	------

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TTWW+m	MT20	4.0	5.0	Edge	
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

<u>BEARINGS</u>							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	275	0	275	0	0	MECHANICAL	
F	431	0	431	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1'-8".

UNFACTORED REACTIONS

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	192	139 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	299	230 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH (F-T)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO					
A-B	0/50	-119.4	-119.4	0.15 (1)	10.00	F-E	-160 / 0
B-C	0/0	-119.4	-119.4	0.12 (1)	10.00	F-C	-119 / 0
C-D	0/0	-119.4	-119.4	0.05 (1)	10.00		0.04 (1)
E-D	-96 / 0	0.0	0.0	0.02 (1)	7.81		0.03 (1)
F-B	-299 / 0	0.0	0.0	0.03 (1)	7.81		
F-E	0/70	-18.2	-18.2	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP	CH.	LL =	34.8	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.3	PSF
TOTAL LOAD		=	48.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL
BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.09/1.00 (E-F:4) ,
WB=0.04/1.00 (C-E:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

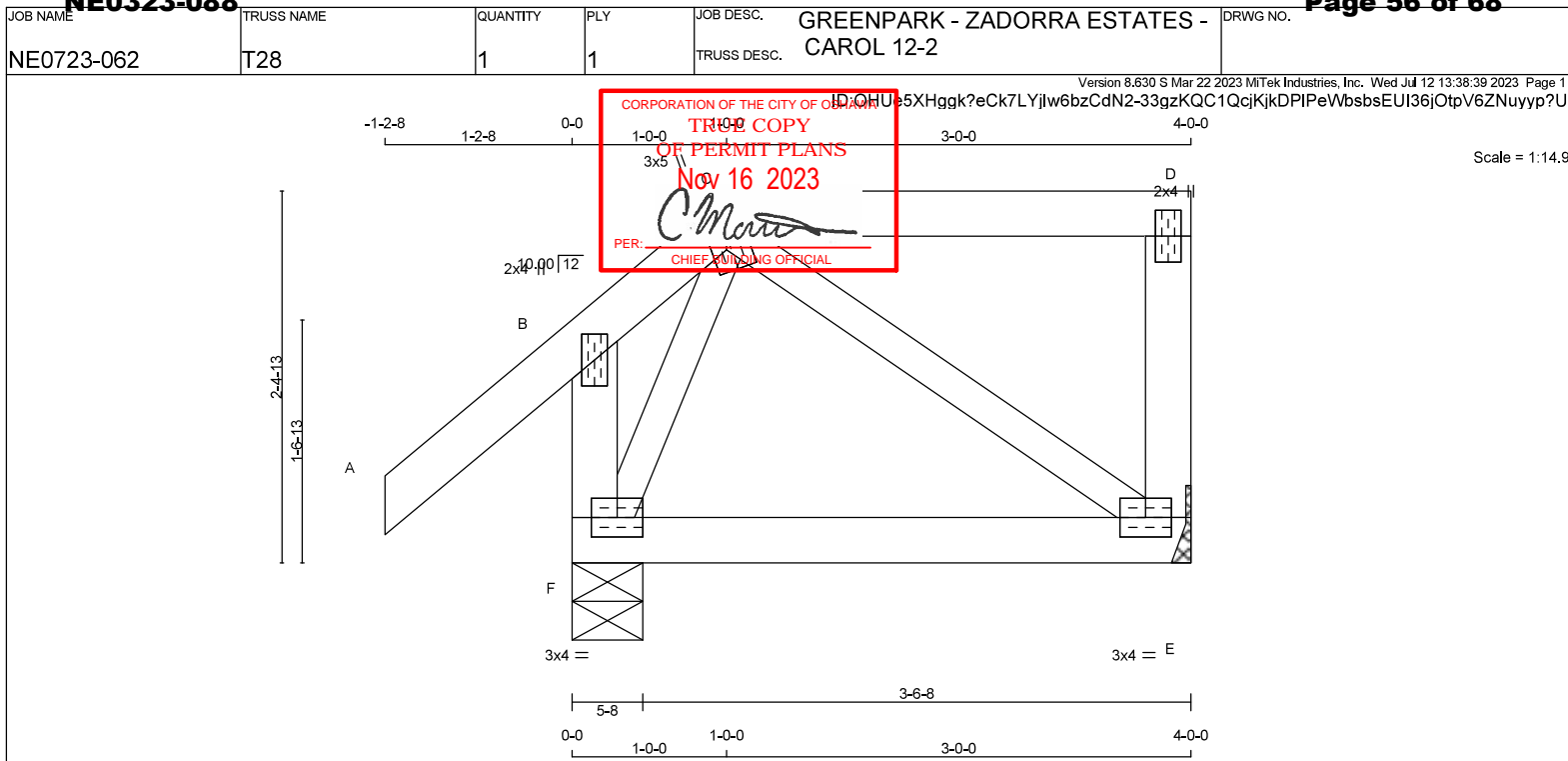
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (F) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
F - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TTWW+m	MT20	3.0	5.0	Edge	
D	TMV+p	MT20	2.0	4.0		
E	BMVW14	MT20	3.0	4.0		
F	BMVW14	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES
EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	250	0	250	0	0	MECHANICAL	
F	456	0	456	0	0	5-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
E	175	124 / 0
F	316	245 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (PL)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PL)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 50	-119.4	-119.4	0.15 (1)	10.00	C-E	-60 / 0
B-C	-66 / 0	-119.4	-119.4	0.14 (1)	6.25	F-C	-114 / 0
C-D	0 / 0	-119.4	-119.4	0.18 (1)	10.00		
E-D	-179 / 0	0.0	0.0	0.02 (1)	7.81		
F-B	-316 / 0	0.0	0.0	0.03 (1)	7.81		
F-E	0 / 49	-18.2	-18.2	0.09 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.18/1.00 (C-D:1), BC=0.09/1.00 (E-F:4),
WB=0.02/1.00 (C-F:1), SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.25 inches

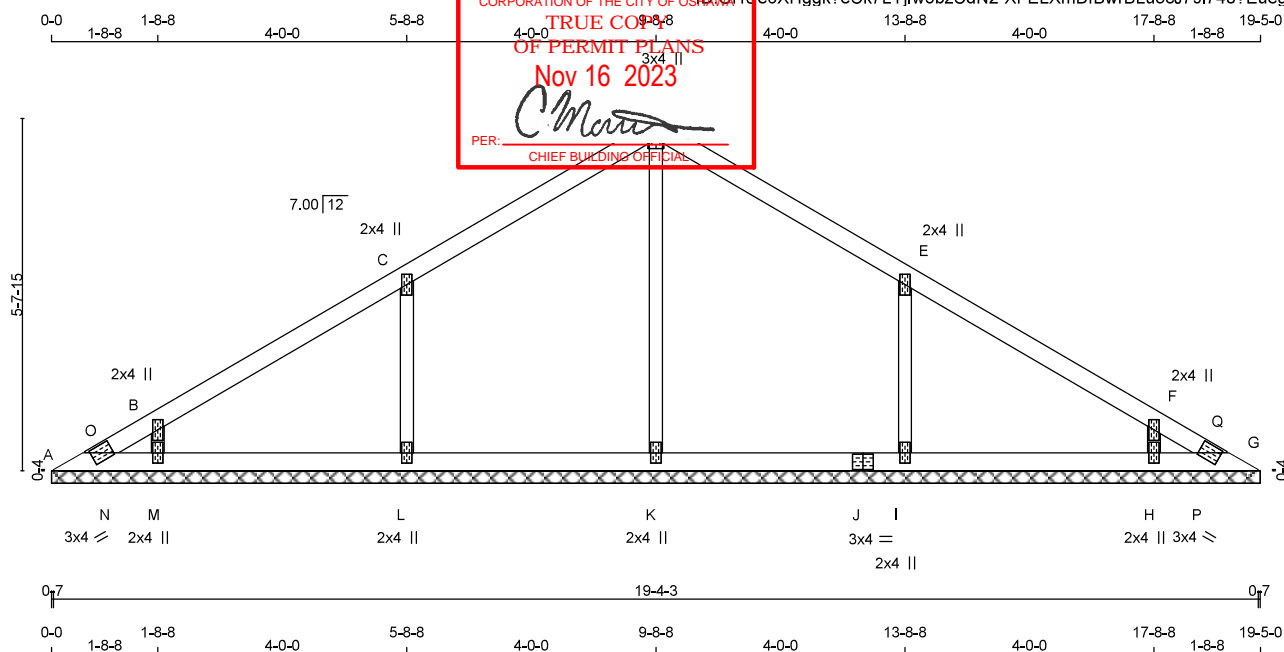
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.35 (C) (INPUT = 0.90)
JSI METAL = 0.17 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:37.0

TOTAL WEIGHT = 56 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
A - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)		W	LEN	Y	X
JT	TYPE	PLATES			
A	TBM1-h	MT20	3.0	4.0	
B	C, E, F				
B	TMW+w	MT20	2.0	4.0	
D	TTW+p	MT20	3.0	4.0	2.25
G	TBM1-h	MT20	3.0	4.0	1.50
H, I, K, L, M					
H	BMW1+w	MT20	2.0	4.0	
J	BS-t	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER		MAXIMUM FACTORED		INPUT	REQ'D
BEARINGS		GROSS REACTION		BRG	BRG
	VERT	HORZ	DOWN	UP/LIFT	HSX
JT	74	0	74	0	19-50 (6-43) JI-8
A	74	0	74	0	19-50 (6-43) JI-8
G	74	0	74	0	19-50 (6-43) JI-8
K	396	0	396	0	19-50 (6-43) JI-8
L	629	0	629	0	19-50 (6-43) JI-8
M	431	0	431	0	19-50 (6-43) JI-8
I	629	0	629	0	19-50 (6-43) JI-8
H	431	0	431	0	19-50 (6-43) JI-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFAVORABLE REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM./LIVE	WIND	DEAD	SOIL
A	51	40/0	0/0	0/0	0/0	11/0	0/0
G	40	51/0	0/0	0/0	0/0	11/0	0/0
K	279	190/0	0/0	0/0	0/0	89/0	0/0
L	439	323/0	0/0	0/0	0/0	116/0	0/0
M	301	215/0	0/0	0/0	0/0	86/0	0/0
I	439	323/0	0/0	0/0	0/0	116/0	0/0
H	301	215/0	0/0	0/0	0/0	86/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, K, L, M, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOCAL CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-O	-113 / 0	-119.4	-119.4	0.01 (1)	6.25	K-D	-325 / 0
O-B	-46 / 0	-119.4	-119.4	0.15 (1)	6.25	L-C	-553 / 0
B-C	-59 / 0	-119.4	-119.4	0.29 (1)	6.25	M-B	-384 / 0
C-D	-83 / 0	-119.4	-119.4	0.29 (1)	6.25	I-E	-553 / 0
D-E	-83 / 0	-119.4	-119.4	0.29 (1)	6.25	H-F	-384 / 0
E-F	-59 / 0	-119.4	-119.4	0.29 (1)	6.25	N-O	-4 / 35
F-Q	-46 / 0	-119.4	-119.4	0.15 (1)	6.25	P-Q	-4 / 35
Q-G	-113 / 0	-119.4	-119.4	0.01 (1)	6.25		
A-N	0 / 109	-18.2	-18.2	0.03 (1)	10.00		
N-M	0 / 106	-18.2	-18.2	0.05 (4)	10.00		
M-L	0 / 62	-18.2	-18.2	0.06 (4)	10.00		
L-K	0 / 48	-18.2	-18.2	0.06 (4)	10.00		
K-J	0 / 48	-18.2	-18.2	0.06 (4)	10.00		
J-I	0 / 48	-18.2	-18.2	0.06 (4)	10.00		
I-H	0 / 62	-18.2	-18.2	0.06 (4)	10.00		
H-P	0 / 106	-18.2	-18.2	0.04 (1)	10.00		
P-G	0 / 109	-18.3	-18.3	0.03 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.3	PSF
TOTAL LOAD =		48.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL
BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1) , BC=0.06/1.00 (L-M:4) ,
WB=0.15/1.00 (D-K:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (D) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)

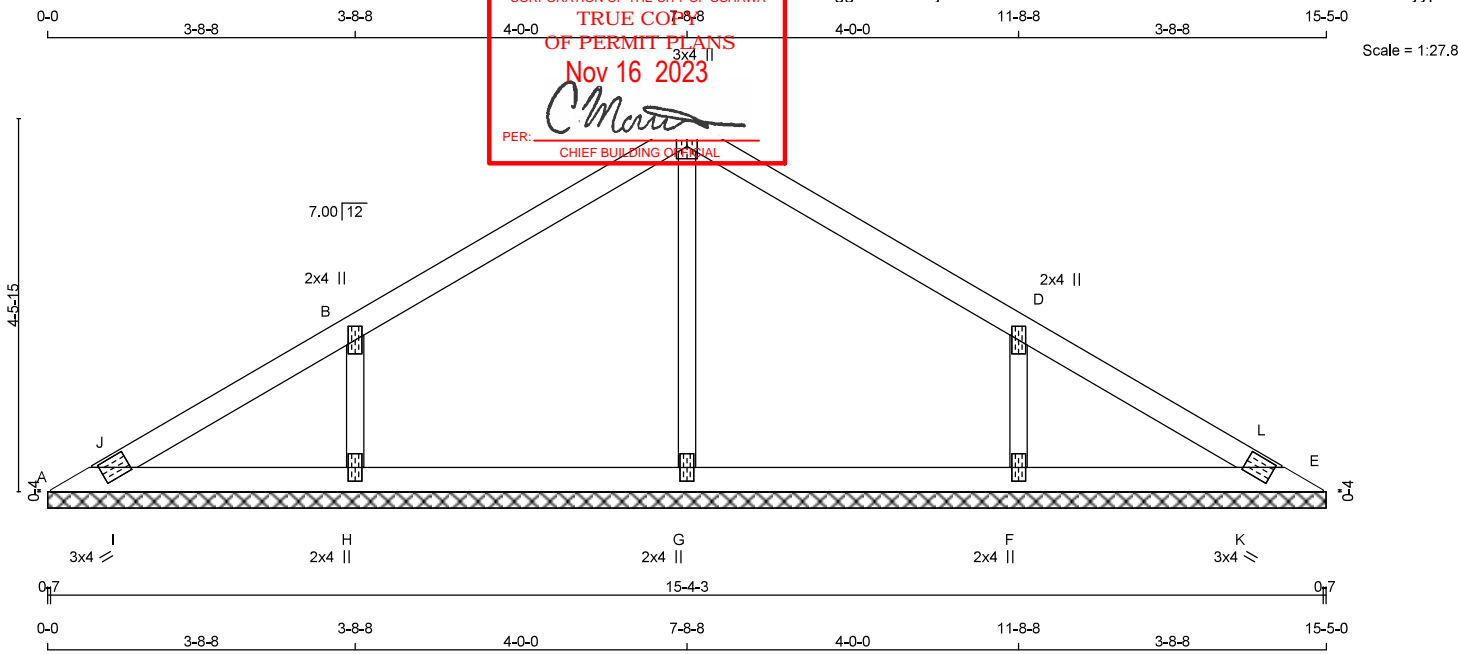


**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK - ZADORRA ESTATES -	DRWG NO.
NE0723-062	V02	1	1	TRUSS DESC.	CAROL 12-2	

Version 8.630 S Mar 22 2023 MiTek Industries, Inc. Wed Jul 12 13:38:40 2023 Page 1



TOTAL WEIGHT = 42 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING**DESIGNER****BEARINGS**

JT	VERT	HORZ	FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG UPLIFT	REQRD BRG IN-SX
A	168	0	168	0	0	15-5-0 (15-4-3)8
E	168	0	168	0	0	15-5-0 (15-4-3)8
G	495	0	495	0	0	15-5-0 (15-4-3)8
H	641	0	641	0	0	15-5-0 (15-4-3)8
F	641	0	641	0	0	15-5-0 (15-4-3)8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	117	87 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	117	87 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
G	349	236 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
H	447	329 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
F	447	329 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	0 / 13	-119.4 -119.4	0.06 (1)	G-C	-437 / 0	0.13 (1)	
J-B	0 / 73	-119.4 -119.4	0.29 (1)	H-B	-535 / 0	0.08 (1)	
B-C	0 / 26	-119.4 -119.4	0.29 (1)	F-D	-535 / 0	0.08 (1)	
C-D	0 / 26	-119.4 -119.4	0.29 (1)	I-J	-74 / 6	0.00 (1)	
D-L	0 / 73	-119.4 -119.4	0.29 (1)	K-L	-74 / 6	0.00 (1)	
L-E	0 / 13	-119.4 -119.4	0.06 (1)				
A-I	-34 / 0	-18.2 -18.2	0.08 (1)				6.25
I-H	-23 / 0	-18.2 -18.2	0.08 (1)				6.25
H-G	-44 / 0	-18.2 -18.2	0.08 (4)				6.25
G-F	-44 / 0	-18.2 -18.2	0.08 (4)				6.25
F-K	-23 / 0	-18.2 -18.2	0.08 (1)				6.25
K-E	-34 / 0	-18.2 -18.2	0.08 (1)				6.25

DESIGN CRITERIA**SPECIFIED LOADS:**

TOP CH.	LL	=	34.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.3	PSF
TOTAL LOAD	=	48.1	PSF	

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
 EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (B-J-1), BC=0.08/1.00 (F-K-1),
 WB=0.13/1.00 (C-G-1), SSI=0.20/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
 FOR QUALITY CONTROL IN THE TRUSS
 MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

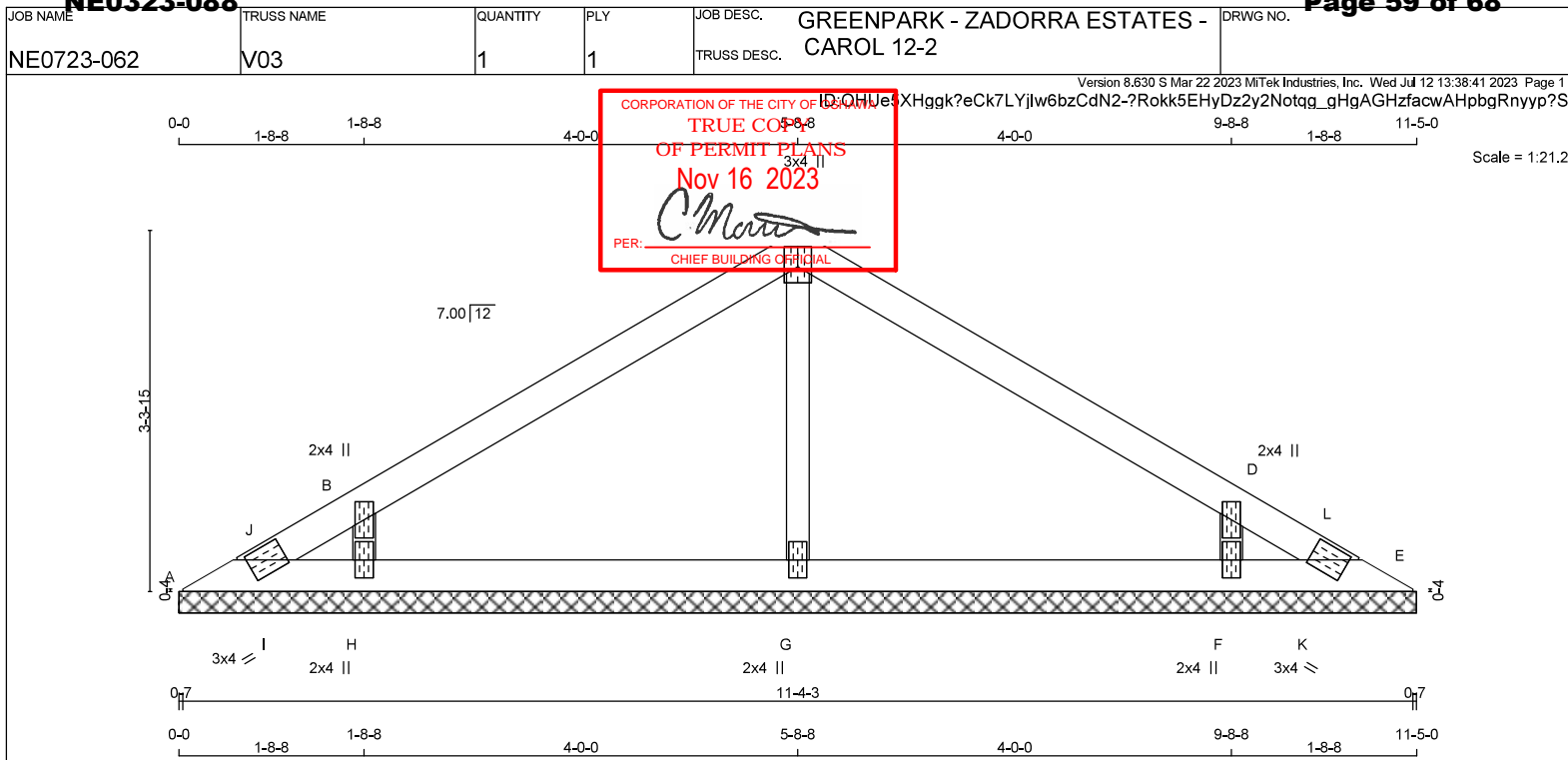
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
 JSI METAL= 0.25 (D) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
 ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
 IS AN INTEGRAL PART OF THIS DRAWING AS IT
 CONTAINS SPECIFICATIONS AND CRITERIA USED
 IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW+p	MT20	3.0	4.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG	REQRD BRG
A	32	0	32	0	11-5-0 (11-4-3)8	
E	32	0	32	0	11-5-0 (11-4-3)8	
G	397	0	397	0	11-5-0 (11-4-3)8	
H	551	0	551	0	11-5-0 (11-4-3)8	
F	551	0	551	0	11-5-0 (11-4-3)8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	22	15 / 0	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
E	22	15 / 0	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
G	280	188 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
H	384	286 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
F	384	286 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-J	-136 / 0	-119.4 -119.4	0.02 (1)	G-C	-309 / 0	0.06 (1)	
J-B	-19 / 0	-119.4 -119.4	0.27 (1)	H-B	-566 / 0	0.08 (1)	
B-C	-102 / 0	-119.4 -119.4	0.28 (1)	F-D	-566 / 0	0.08 (1)	
C-D	-102 / 0	-119.4 -119.4	0.28 (1)	I-J	0 / 131	0.00 (1)	
D-L	-19 / 0	-119.4 -119.4	0.27 (1)	K-L	0 / 131	0.00 (1)	
L-E	-136 / 0	-119.4 -119.4	0.02 (1)				
A-I	0 / 141	-18.2 -18.2	0.08 (1)				
I-H	0 / 132	-18.2 -18.2	0.08 (1)				
H-G	0 / 67	-18.2 -18.2	0.07 (4)				
G-F	0 / 67	-18.2 -18.2	0.07 (4)				
F-K	0 / 132	-18.2 -18.2	0.08 (1)				
K-E	0 / 141	-18.3 -18.3	0.08 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 48.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (B-C-1), BC=0.08/1.00 (A-I-1),
WB=0.08/1.00 (B-H-1), SS=0.21/1.00 (B-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE
FOR QUALITY CONTROL IN THE TRUSS
MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

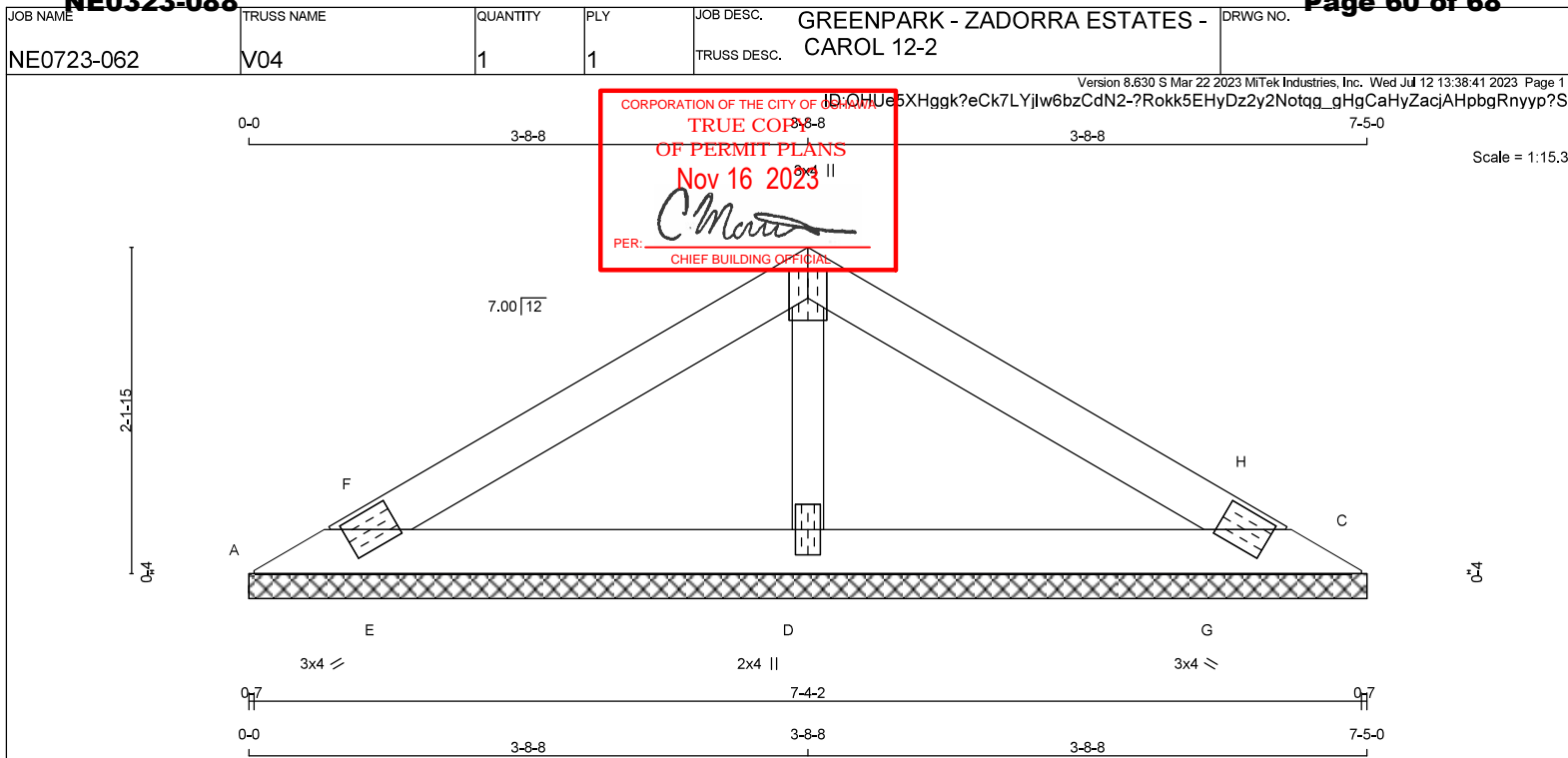
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTES: TRUSSES. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
A - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
A	97	0	97	0	7-5-0 (7-4-2) 1-8	1-8
C	97	0	97	0	7-5-0 (7-4-2) 1-8	1-8
D	818	0	818	0	7-5-0 (7-4-2) 1-8	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	68	49 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0	0 / 0
C	68	49 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0	0 / 0
D	572	413 / 0	0 / 0	0 / 0	0 / 0	158 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LC)
FR-TO	0 / 286	-119.4	-119.4	0.07 (1)	10.00	D-B	-617 / 0	0.09 (1)	
F-B	0 / 293	-119.4	-119.4	0.19 (1)	10.00	E-F	-205 / 0	0.00 (1)	
B-H	0 / 293	-119.4	-119.4	0.19 (1)	10.00	G-H	-205 / 0	0.00 (1)	
H-C	0 / 286	-119.4	-119.4	0.07 (1)	10.00				
A-E	-279 / 0	-18.2	-18.2	0.15 (1)	6.25				
E-D	-256 / 0	-18.2	-18.2	0.15 (1)	6.25				
D-G	-256 / 0	-18.2	-18.2	0.15 (1)	6.25				
G-C	-279 / 0	-18.3	-18.3	0.15 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH. LL	=	34.8 PSF
DL	=	6.0 PSF
BOT CH. LL	=	0.0 PSF
DL	=	7.3 PSF
TOTAL LOAD	=	48.1 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, NBC-2019AE
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD)
EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOADCSI: TC=0.19/0.97 (B-F:1), BC=0.15/0.97 (D-E:1),
WB=0.09/0.97 (B-D:1), SSI=0.12/1.00 (B-F:1)DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10
SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: TRUSSES. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



KOTT TRUSS BEARING CAPACITY TABLE

(INTERNAL USE ONLY)

TRUSS BEARING CAPACITIES [LBS.], BY TRUSS LUMBER TYPE (SUPPORTED ON SP

CORPORATION OF THE CITY OF OSHAWA

TRUE COPY

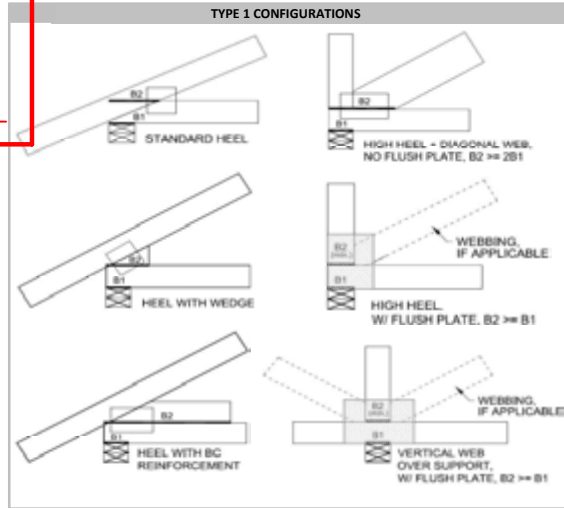
OF PERMIT PLANS

Nov 16 2023

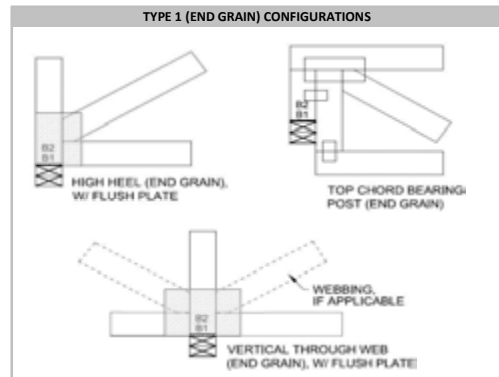
15230 23337 20293

15230 23337 20293

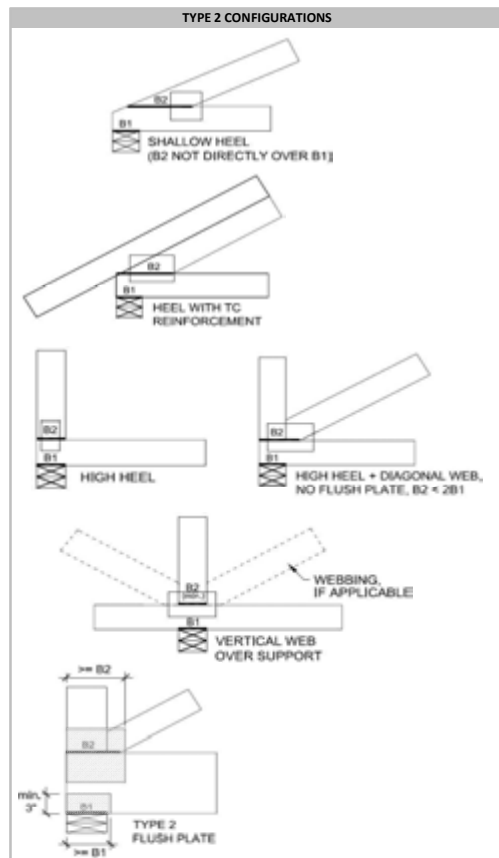
NO BEARING ENHANCER	BEARING PLATE (B1)	TYPE 1, NO FLUSH PLATE, B2 >= 2B1							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
1 1/2"	2x4	1383		2767		4151		5534	
	2x6	5834	5073	11668	10146	17503	15230	23337	20293
	2x8	7690	6687	15381	13375	23072	20005	30703	26750
	2x10								
FLUSH PLATE	BEARING PLATE (B1)	TYPE 1, FLUSH PLATE							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
		1 1/2"	1383	2767	4151	5534			
		2x4	3712	7425	11138	14851			
BEARING ENHANCER	BEARING PLATE (B1)	TYPE 1, FLUSH PLATE + BEARING ENHANCER							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
		CPn-4 (Simpson)	2x4	4515	9030	13545	18065		
		CPn-6 (Simpson)	2x6	7095	14190	21285	28390		
END GRAIN	BEARING PLATE (B1)	TYPE 1 (END GRAIN)							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)
		2x4	2x4	3712	7425	11138	14851		
		2x6	2x6	5834	11668	17503	23337		
END GRAIN, BEARING ENHANCER	BEARING PLATE (B1)	TYPE 1 (END GRAIN), FLUSH PLATE + BEARING ENHANCER							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)
		CPn-4 (Simpson)	2x4	4515	9030	13545	18065		
		CPn-6 (Simpson)	2x6	7095	14190	21285	28390		
FLUSH PLATE	BEARING PLATE (B1)	TYPE 2, NO FLUSH PLATE, B2 < 2B1							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
		2x4	2x4	2639	2152	5279	4304	7919	6457
		2x6	2x6	3393	2767	6787	5534	10181	8302
FLUSH PLATE	BEARING PLATE (B1)	TYPE 2, FLUSH PLATE							
		1-PLY		2-PLY		3-PLY		4-PLY	
		MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
		2x4	2x4	4672	3809	9344	7619	14016	11429
		2x6	2x6	7342	5986	14684	11973	22026	17959



END GRAIN	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 1 (END GRAIN)							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)
CPn-4 (Simpson)	2x4	2x4	3712	7425	11138	14851				
	2x6	2x6	5834	11668	17503	23337				
	2x8	2x8	7690	15381	23072	30763				
	2x10	2x10								
END GRAIN, BEARING ENHANCER	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 1 (END GRAIN), FLUSH PLATE + BEARING ENHANCER							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)	MSR2100	SPF No.2 (EG)
			CPn-4 (Simpson)	2x4	4515	9030	13545	18065		
			CPn-6 (Simpson)	2x6	7095	14190	21285	28390		
FLUSH PLATE	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, FLUSH PLATE							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
			CPn-4 (Simpson)	2x4	4515	9030	13545	18065		
			CPn-6 (Simpson)	2x6	7095	14190	21285	28390		



NO BEARING ENHANCER	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, NO FLUSH PLATE, B2 < 2B1							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
2x4	2x4	2x4	2639	2152	5279	4304	7919	6457	10588	8609
	2x6	2x6	3393	2767	6787	5534	10181	8302	13575	11069
	2x8	2x8	4147	3382	8296	6764	12444	10146	16592	13529
	2x10	2x10	4808	3920	9616	7840	14424	11761	19232	15681
FLUSH PLATE	BEARING PLATE (B1)	POST (B2) ABOVE BEARING	TYPE 2, FLUSH PLATE							
			1-PLY		2-PLY		3-PLY		4-PLY	
			MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950	MSR2100	SPF No.2 MSR1950
			2x4	2x4	4672	3809	9344	7619	14016	11429
			2x6	2x6	7342	5986	14684	11973	22026	17959



NOTES:

- Factored truss reaction shall not exceed bearing capacity corresponding to: configuration type, size of bearing surfaces, truss lumber, # of plies, and applicable enhancers.
- Values in table are in conformance with CSA O86-14 Cl. 6.5.7 and TPIC 2014-Update 2, and may be used for residential or commercial designs.
- Values in table are in conformance with MiTek Canada Detail B37821Q "SPF Bearing Capacities".
- Values in table are in conformance with Simpson Catalogue C-C-CAN2020.
- Conditions for use of table values include: standard duration ($K_{d1}=1$), dry lumber ($K_{d2}=1$), untreated lumber ($K_{d3}=1$), length of bearing factor not applied ($K_{d4}=1$).
- Size factor (K_{d5}) applied to support material calculation when acceptable. Flush plate factor (K_p) applied to truss material calculation when acceptable (ie. excludes end grain).
- Flat roof factor (K_r) must applied for trusses making up a flat roof system; to do so, multiply bearing capacity values by 0.75 for this application.
- Bearing plate is to be specified by the project engineer; values in table assume a bearing material of SPF #2 (or better).
- When required, flush plate must not be located further than 1/4" away from bearing surface, and must cover the entire bearing plate length (B1).
- When required, bearing enhancer must be installed as per manufacturer's guidelines.
- Type 2 bearing configurations can be converted to use Type 1 table values as outlined in TPIC 2014-Update 1 Cl. 7.5.9.
- This table is not valid after April 30, 2022.

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	Length (in)	Diameter (in)	LATERAL Resistance per nail (Lbs.)		WITHDRAWAL Resistance per nail (Lbs.)	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
LUMBER	MAXIMUM NUMBER OF TOE-NAILS				
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

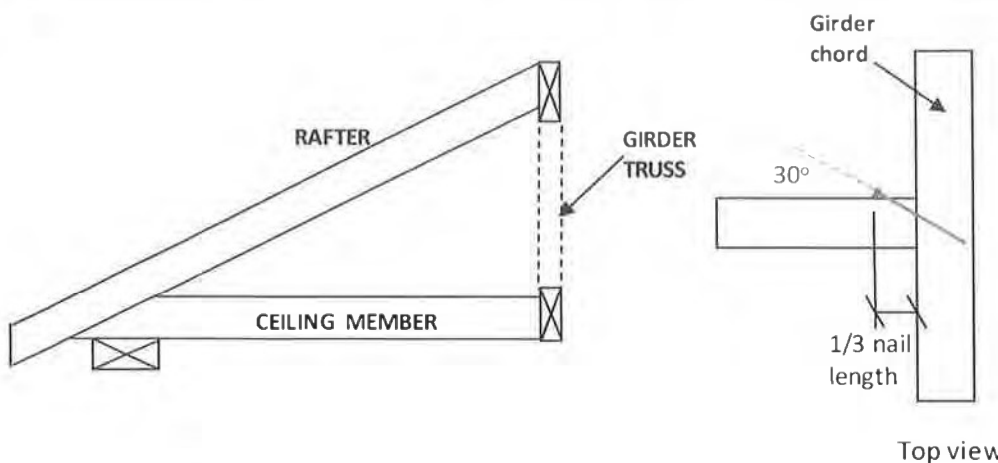


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



Issued: MARCH 1, 2022

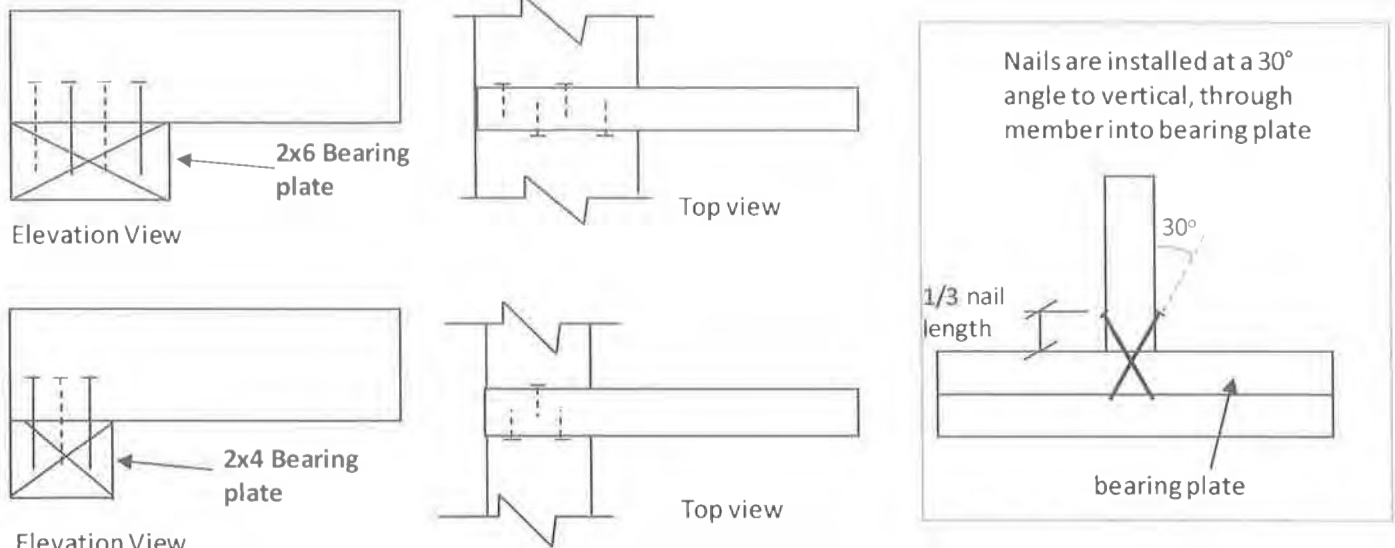
Expiry: APRIL 30, 2024

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 16 2023

PER: *C. Mart*
CITY BUILDING OFFICIAL

TOE-NAIL ANCHORAGE DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



MiTek[®]

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 16 2023

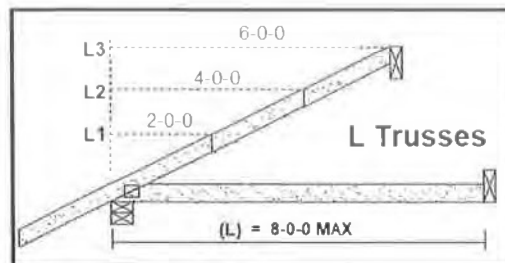
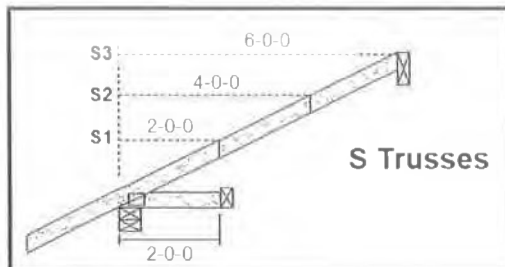
PER: *C. Cordogianis*
ENGINEERING OFFICIAL

STANDARD DETAIL MSD2015-J

Issued: MARCH 17, 2021

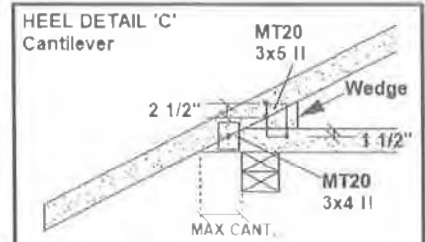
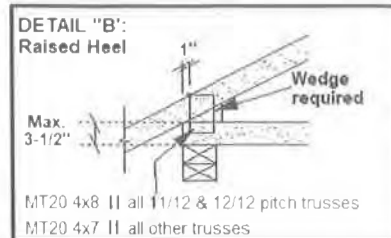
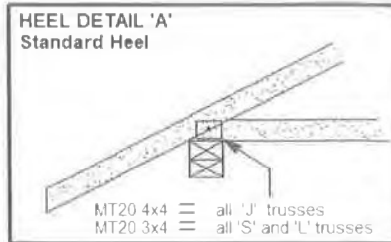
Expiry: APRIL 30, 2023

STANDARD HIP AND FRAMING



Specified Load Rating:

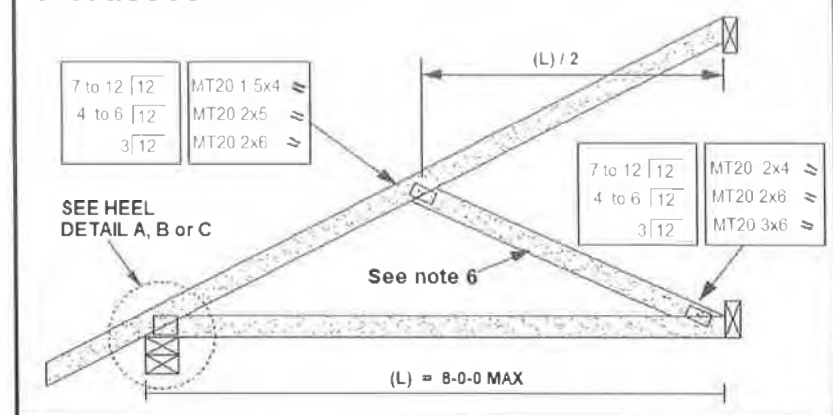
Top chord Live:	51.0 PSF or less
Top chord Dead:	6.0 PSF or less
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF or less



CANTILEVER DETAIL 'C'

SLOPE	MAX CANT.	WEDGE PLATE	WEDGE SIZE
3/12	17"	3 X 5	2 X 3
4/12	14"	3 X 5	2 X 3
5/12	12"	3 X 5	2 X 4
6/12	10"	3 X 5	2 X 4
7/12	9"	3 X 5	2 X 6
8/12	8 1/2"	3 X 5	2 X 6
9/12	8"	3 X 5	2 X 6
10/12	7 1/2"	3 X 5	2 X 6

J Trusses



PEO

Certificate No. 10889485

NOTES:

1. This detail is valid only for projects conforming to **PART 9 NBCC 2015** that do not require a wind analysis to be incorporated into the design of the trusses.
2. Overhang length shall not exceed 24 inches.
3. All lumber shall be 2x4 SPF (or D-Fir) DRY No. 2 grade or better.
4. All plates specified are MITEK MT20, pressed into both faces of each truss. Heel plates of all trusses shall conform to heel details 'A', 'B' or 'C'.
5. Diagonal hip rafter design shall conform to section 9.23.14.6 of NBCC 2015.
6. For 6.0 ft. or less span, diagonal web on truss 'J' is optional. Girder design must reflect choice of partial jack ('J' with diagonal web) or open jack ('J' without diagonal web)
7. All truss-to-rafter and truss-to-truss connections shall be specified as per MITEK standard detail 'MSD2015-H: Toe-Nail Capacity Details'



MiTek

CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 16 2023

PER: *C. M...*
CHIEF BUILDING OFFICIAL

STANDARD DETAIL MSD2015-K

Issued: MARCH 1, 2022

Expiry: APRIL 30, 2024

STANDARD GABLE END DETAIL

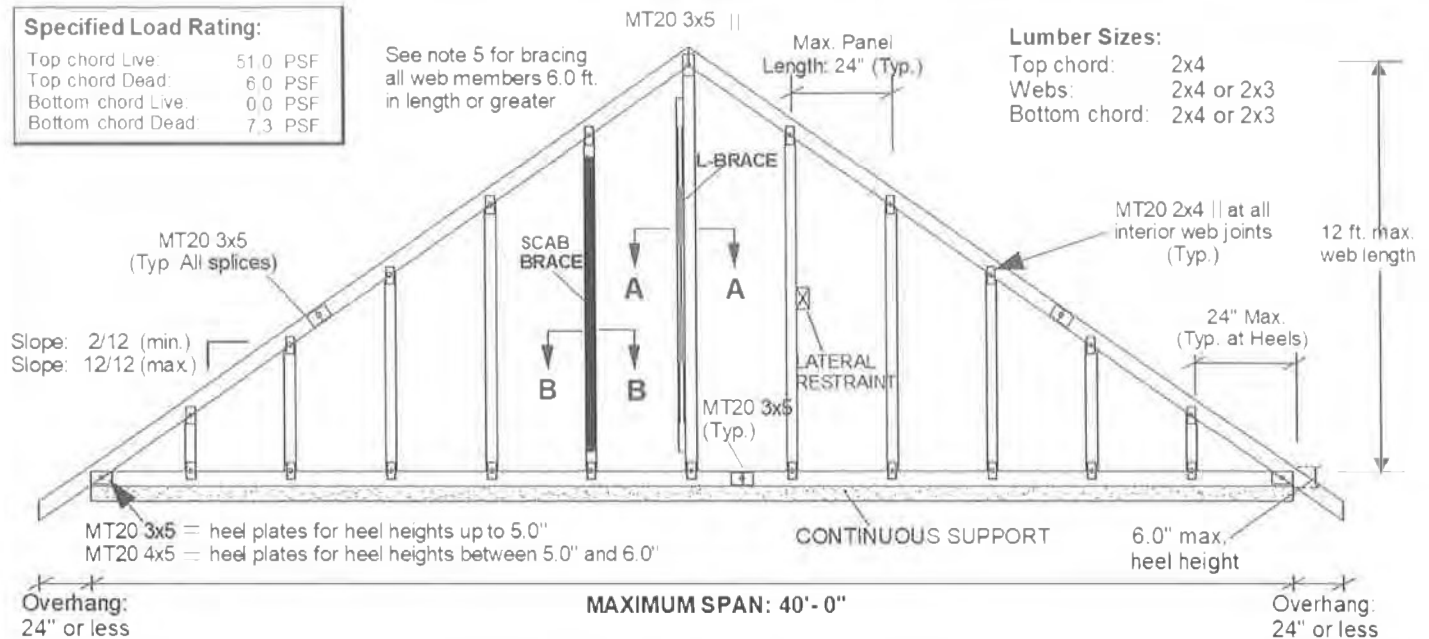
Specified Load Rating:

Top chord Live:	51.0 PSF
Top chord Dead:	6.0 PSF
Bottom chord Live:	0.0 PSF
Bottom chord Dead:	7.3 PSF

See note 5 for bracing
all web members 6.0 ft.
in length or greater

Lumber Sizes:

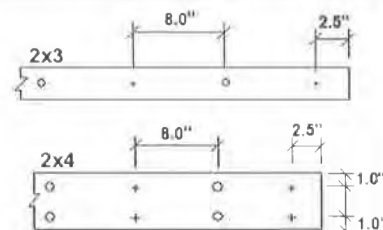
Top chord:	2x4
Webs:	2x4 or 2x3
Bottom chord:	2x4 or 2x3



SCAB BRACE DETAIL (Section B-B)

Gable Web

SPF No. 2 DRY Scab, same size as web. Scab brace must cover 90% of web length



- + 0.122"x3.0" nail driven from front face
- o 0.122"x3.0" nail driven from back face

L BRACE DETAIL (Section A-A)

2x4 SPF No. 2 DRY L-Brace

Gable web

3" NAIL @ 6" C/C

0.75" (TYP)

Fasten L-Brace to narrow edge of web with one row of 0.122" x 3.0" nails spaced at 6.0" c/c along entire length of web. Brace must cover 90% of the web length. Respect a 2.5" minimum end distance.

Notes:

1. This detail is only valid for projects conforming to **Part 9, NBCC 2015** that do not require a wind analysis to be incorporated into the design of the truss.
2. This detail is for vertical (gravity) load rating of the truss only. Truss must be continuously supported over the entire length of bottom chord.
3. Maximum web length not to exceed 12.0 ft. Spacing of gable stud webs in the truss not to exceed 24 inches cc.
4. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint.
5. Lateral restraint required at half-length of all webs over 6.0 ft. long. Alternatively install an L-Brace or scab brace as shown above. Scab braces shall be limited to 10 ft. long webs or less.
6. All plates are MITTEK MT20 pressed into both faces of truss.
7. All lumber to be SPF (or D-Fir) DRY and of No.2 grade or better.
8. Additional building bracing is typically installed to brace the face of the end wall assembly. See BCSI Canada 'Building Designer Responsibilities for Gable End Frame Bracing' for additional information on building bracing for gable-end assemblies.

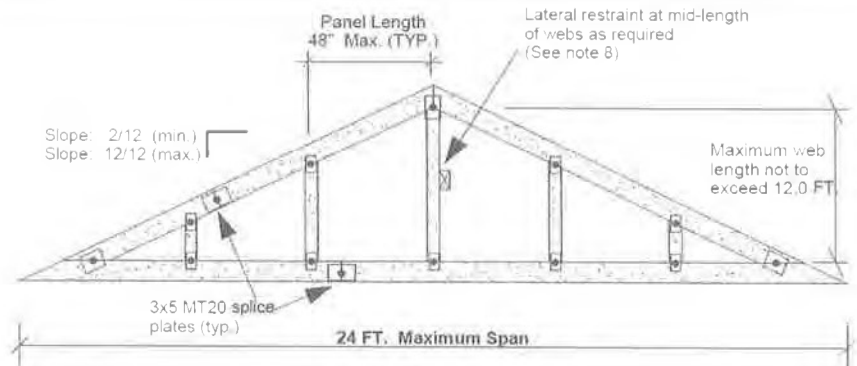
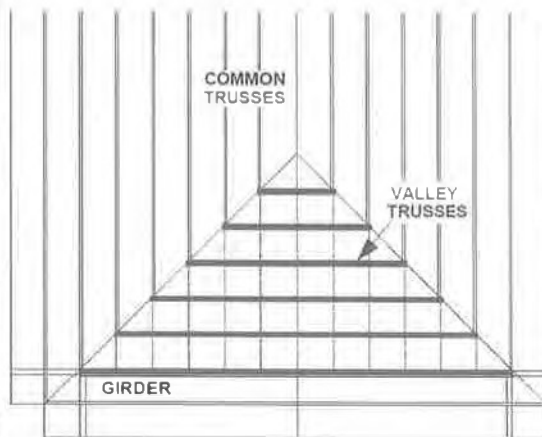
PEO
Certificate No. 10869485



CORPORATION OF THE CITY OF OSHAWA
TRUE COPY
OF PERMIT PLANS
Nov 16 2023

PERMIT
CHIEF BUILDING OFFICIAL

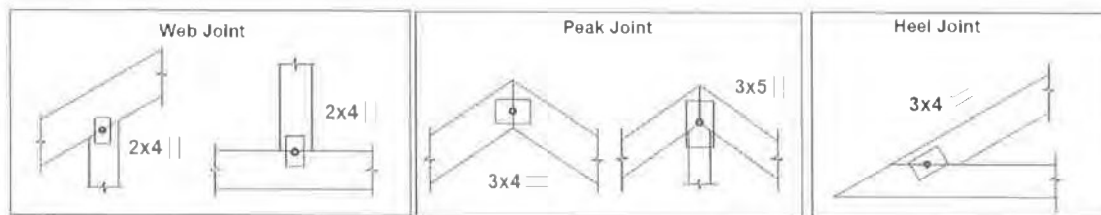
VALLEY SET ROOF FRAMING PLAN



VALLEY TRUSS LUMBER:

Top Chord	2x4
Bottom Chord	2x3 or 2x4
Web	2x3 or 2x4

Splices (if required) shall be located at 1/4 point of a panel. Splice joints shall not be located in the first panel adjacent to the heel joint or peak joint. If beveling the bottom chord, splice plates shall be flush with top edge of bottom chord.

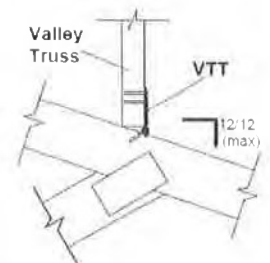
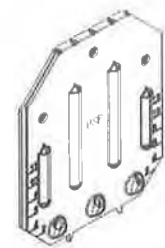


NOTES:

- Specified roof loads shall not exceed the maximum limits shown below:

Top chord:	Live =	70.0 PSF
Top Chord:	Dead =	6.0 PSF
Bottom Chord:	Live =	0.0 PSF
Bottom Chord:	Dead =	7.3 PSF
- Valley truss design assumes continuous support by the truss system underneath. Spacing of all valley trusses as well as the underlying trusses shall not exceed **24 in. c/c**.
- Vertical web spacing in valley trusses not to exceed **48 in. c/c**. Web lengths not to exceed 12 ft.
- All lumber to be DRY No. 2 grade or better, SPF or D-Fir.
- Bottom chord may be beveled to match the slope of the intersecting roof. If beveling, a minimum 2x4 bottom chord is required, with a maximum bevel slope of 4/12 (spliced chord) or 8/12 (non-spliced chord). For bevel slopes exceeding the 2x4 limits, use a 2x6 bottom chord.
- Truss plates are MITEK MT20 pressed into both faces of the truss and centered at each joint.
- Use MiTek VTT Valley Truss Ties to attach each valley truss at the intersection point between valley chord and underlying truss chord. Install clips as per product installation instructions. Alternatively, toe-nail valley truss bottom chord at all intersection points with each underlying truss, using two 0.122"x3.25" nails per connection.
- One continuous lateral brace is required at **1/2 length** of all webs that exceed 6.0 ft. in length.
- This detail is only valid for residential projects conforming to **PART 9 - NBCC2015**, that do not require a wind analysis to be incorporated into the design of the trusses.

MITEK VTT (Valley Truss Tie)



Install as per MiTek product installation instructions

PEO
Certificate No. 10889485



Australat & Bracing for 3x2 and 4x2 Parallel Chord Trusses
Réforme et contreventement pour les fermes à membres parallèle de 3x2 et de 4x2

TRUE COPY
OF PERMIT PLANS
Nov 16 2023PER: 
CHIEF BUILDING OFFICIAL

NE0323-088

B3C

BCSI-B3C SUMMARY SHEET - PERMANENT RESTRAINT/BRACING OF CHORDS & WEB MEMBERS Page 68 of 68

Truss clear spans of 60' or greater may require complex permanent bracing. Please always consult a registered design professional.

NEED TO KNOW Deregarding permanent restraint/bracing is a major cause of truss field performance problems and has been known to lead to roof or floor system collapse.

Le code du bâtiment exige que le fait de négliger la retenue et le contreventement permanents est une cause majeure des problèmes visant la performance des fermes sur le chapitre et cela est reconnu pour avoir entraîné l'effondrement des systèmes de toit et de plancher.

Les fermes avec des portées libres de 18,3 m (60 pi) ou plus peuvent exiger un contreventement permanent complexe. Veuillez toujours consulter un concepteur professionnel agréé.

Permanent Bracing Materials & Fasteners

Matériaux et attaches de retenue et de contreventement

Commonly used restraint/bracing materials include wood structural panels, gypsom board sheathing, stress-graded lumber, proprietary metal products, and metal purlins and straps.

Les matériaux de retenue et de contreventement communs incluent les panneaux structuraux de bois, le revêtement de panneaux de gypse, le bois classé par résistance mécanique, les produits métalliques exclusifs, les pannes et les feuillards métalliques.

MINIMUM ATTACHMENT REQUIREMENTS FOR LUMBER RESTRAINT/BRACING

LES EXIGENCES MINIMALES DES ATTACHEMENTS POUR LE CONTREVENTEMENT DU BOIS

Lumber Size Taille du bois	Minimum Nail Size Minimum des clous	Minimum Number of Nails per Connection Minimum du nombre de clous par connexion
2x4 stress-graded lumber or stress-graded metal purlins	7 common (19122) or 3x4 common (19122) or 3x4 common (19122)	2
2x6 stress-graded lumber or stress-graded metal purlins	7 common (19122) or 3x4 common (19122) or 3x4 common (19122)	3

Permanent Bracing for the Various Planes of a Truss

Contreventement permanent pour les divers plans d'une ferme

Permanent bracing is important because it:

- prevents out-of-plane buckling of truss members,
- helps maintain proper truss spacing,
- resists and transfers lateral loads from wind and seismic forces.

Le contreventement permanent est important parce qu'il:

- prévoient le flambement hors plan des membres de fermes,
- aide à maintenir l'espacement approprié des fermes,
- résiste aux charges latérales des forces de vent et des forces sismiques et les transfère.

Trusses require permanent bracing within ALL of the following planes:

- Top chord plane
- Bottom chord plane
- Web member plane

Les fermes exigent un contreventement permanents TOUS les plans suivants:

- Le plan de la membrure supérieure
- Le plan de la membrure inférieure
- Le plan de la membrure d'âme

CAUTION: Without permanent bracing the truss, or a portion of its members, will buckle (i.e., fail) at loads far less than design.

ATTENTION: Sans un contreventement permanent, la ferme ou une partie de ses membres, flambera (c'est-à-dire fera défaut) à des charges grandement inférieures à celles indiquées dans le conception.

1. Permanent Bracing for the Top Chord Plane

Contreventement permanent pour le plan de membrure supérieure

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Fastener size and spacing requirements and grade for the sheathing, purlins and bracing are provided in the building code and/or by the building designer. Le code du bâtiment et (ou) le concepteur du bâtiment fournissent les exigences relatives à la taille des attaches, à l'espacement et au classement du revêtement, des pannes et du contreventement.

2. Permanent Bracing for the Bottom Chord Plane

Contreventement permanent pour le plan de membrure inférieure

Use rows of continuous lateral restraint with diagonal bracing. gypsom board sheathing or some other ceiling material (capable of functioning as a diaphragm). Utilisez des rangées de retenue latérales continues avec un contreventement diagonal, un revêtement de panneaux de gypse ou un autre matériau en mesure de fonctionner comme diaphragme.

The TDD provides information on the assumed support for the bottom chord.

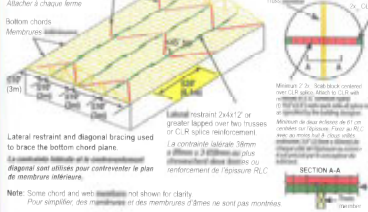
Le DCF fournit des renseignements sur le support prévu pour la membrure inférieure.

Install bottom chord permanent lateral restraint at the spacing indicated on the TDD and/or by the building designer with a maximum of 10 (3 m) on center.

Installez la retenue latérale permanente de la membrure inférieure à l'espacement indiqué sur le DCF et (ou) par le concepteur du bâtiment avec un maximum de 3m (10 pi) entraxes.

Diagonal bracing Attach to each truss, Contreventement et diaphragme Attache à chaque ferme

Bottom chords Membres



3. Permanent Bracing for the Web Member Plane

Contreventement permanent pour le plan de membrure d'âme

Web member permanent bracing collects and transfers bracing resistant forces and/or lateral loads from wind and seismic forces. The same bracing can often be used for both functions. Le contreventement permanent de la membrure d'âme recueille et transfère les forces de retenue de flambement et (ou) les charges latérales des forces de vent et des forces sismiques. Le même contreventement peut souvent être utilisé pour les deux fonctions.

Individual Web Member Permanent Restraint & Bracing

Retenue et contreventement permanents des membrures d'âme individuelles

Check the TDD to determine which web members (if any) require restraint to resist buckling.

Vérifiez le DCF pour déterminer quelles membrures (le cas échéant) nécessitent une retenue pour résister au flambement.

Verify the DCF for determine which web members (if any) require restraint to resist buckling.

Vérifiez le DCF pour déterminer quelles membrures (le cas échéant) nécessitent une retenue pour résister au flambement.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Use plywood, oriented strand board (OSB), or wood or metal structural purlins that are properly braced. Attach to each truss.

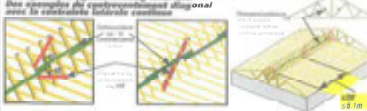
Utilisez du contreventement des panneaux OSB ou des pannes structurales de bois ou de métal correctement contreventées. Fixez à chaque ferme.

The truss design drawing (TDD) provides information on the assumed support for the top chord.

Le dessin de conception de ferme (DCF) fournit des renseignements sur le support prévu pour la membrure supérieure.

Examples of Diagonal Bracing with Continuous Restraint

Exemples de contreventement diagonal continu



Group of 3 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Group of 2 trusses

Cells End Frames and Slag Bottom Chords

Cellules et cadres finaux et cordons inférieurs en pente

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.

The gable and frame should always match the profile of the adjacent trusses to permit installation of proper bottom chord plane restraint & bracing unless special bracing is designed to support the end wall.

La ferme et le cadre doivent toujours correspondre au profil des fermes adjacentes pour permettre l'installation de la retenue et du contreventement appropriés du plan de membrure inférieure, à moins qu'un contreventement spécial soit conçu pour supporter le mur d'extrémité.