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TRUE COPY
OF PERMIT PLANS
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Noverging ering Notes: EWP-Floors



# MHP 23029

PLEASE READ ALL NOTES PRIOR TO INSTALLATION OF THE COMPONENT

# **RESPONSIBILTIES**

THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS ONLY LIMITED TO THE CALCULATION OF THIS BUILDING COMPONENT FOR THE LOADS AND CONDITIONS SHOWN ON THIS DRAWING.

THE RESPONSIBILITY OF THE UNDERSIGNED IS LIMITED TO THE VERIFICATION OF THE STRUCTURAL CAPACITY OF THE FLOOR JOISTS AND LVL BEAMS BASED ON PLACEMENT AS SHOWN ON THE LAYOUT. THE LOADS APPLIED ARE LIMITED TO THE GRAVITY EFFECTS OF THE SPECIFIED LOADS. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE EFFECT OF WIND, UPLIFT, SEISMIC, LATERAL OR OTHER FORCES, CALCULATION OF ADEQUATE SUPPORT AND ANCHORAGE OF COMPONENTS, AS WELL AS THE DIMENSIONS AND DESIGN LOADS USED TO CALCULATE COMPONENTS ARE THE RESPONSIBILITY OF THE OVERALL BUILDING DESIGNER. FLOOR JOISTS AND OSB RIM BOARD ARE DESIGNED TO CARRY UNIFORMLY DISTRIBUTED LOADS ONLY. POINT LOADS SHOULD BE TRANSFERRED THROUGH THE FLOOR CAVITY WITH TRANSFER BLOCKS. STRUCTURAL ELEMENTS SUCH AS WALLS, POSTS, CONNECTORS, AND TRANSFER BLOCKS ARE THE RESPONSIBILITY OF THE OVERALL BUILDING DESIGNER.

THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF BEING FURNISHED FAULTY OR INCORRECT INFORMATION, SPECIFICATIONS AND/OR DESIGNS.

# COMPONENT DESIGN INFORMATION

- 1. THIS BUILDING COMPONENT IS CERTIFIED AS AN INDIVIDUAL COMPONENT FOR THE LOADS AND CONDITIONS SHOWN ON THE CALCULATION PAGE BASED ON INFORMATION PROVIDED BY KOTT DESIGN.
- 2. THE BUILDING COMPONENT USED IN CONSTRUCTION MUST BE THE SAME AS INDICATED ON THE DRAWINGS.
- 3. UNLESS NOTED OTHERWISE ON THE LAYOUT OR BEAM CALCULATION SHEET, MEMBERS CONSISTING OF MULTIPLE PLIES MUST BE CONNECTED AS PER THE DOCUMENT "MULTIPLE MEMBER CONNECTION DETAILS" SHOWN ON PAGE 2 OF THIS DOCUMENT.
- 4. PASS-THRU TRANSFER BLOCK FRAMING IS REQUIRED AT ALL POINT LOADS OVER BEARINGS.
- 5. IT IS ASSUMED THAT EACH LVL BEAM WHERE NOT SEATED IN A HANGER IS ATTACHED USING (4) FOUR 3-1/4" COMMON SPIRAL NAILS FOR UP TO 5.5" LONG BEARINGS AND USING (6) SIX 3-1/4" COMMON SPIRAL NAILS FOR BEARINGS EQUAL TO OR LONGER THAN 5.5", UNLESS INDICATED OTHERWISE.

## CODE

THIS BUILDING COMPONENT IS DESIGNED IN ACCORDANCE WITH THE NATIONAL BUILDING CODE OF CANADA, THE ONTARIO BUILDING CODE, CCMC AND CANADIAN STANDARDS ASSOCIATION GUIDELINES.

# HANDLING AND INSTALLATION

- 1. DO NOT DRILL ANY HOLE, CUT OR NOTCH A CERTIFIED BUILDING COMPONENT WITHOUT A WRITTEN PRE-AUTHORIZATION.
- 2. INSTALLATION AND ASSEMBLY OF FLOOR JOISTS AND LVL BEAMS IS TO BE CARRIED OUT IN ACCORDANCE WITH THE CURRENT EDITION OF THE MANUFACTURER'S LITERATURE.



1

# MHP 23029

ENG-IM0723-070-KTF-GREENPARK-ZADORRA ESTATES-ROSE 3-3

MULTIPLE MEMBER CONNECTIONS FOR UNIFORMLY DISTRIBUTED TOP & SIDE LOADED LVL BEAMS SHOWN ON KOTT LAYOUTS

Page 2 of 57

# KOTT

# MULTIPLE MEMBER CONNECTIONS FOR BEAMS SHOWN ON KOTT LAYOUTS

4-ply 9 1/2"-18" 16"-18" LVL 11 7/8" - 14" LVL 9 1/2" LVL 3 1/4" mln 6 3/4° SDW 3" mln 2-ply 3 1/4" mln FOR 4 PLY BEAMS\*, ATTACH PLIES 3-ply TOGETHER USING 5-3/4" SDW SCREWS IN 3 ROWS @ 16" C/C USE AN ADDITIONAL THREE (3) 6-3/4" SDW SCREWS ON EACH SIDE (OF EACH FACE) AT POINT LOAD LVL Connection Notes: -LVL ply width is 1-3/4" LOCATIONS @ 1/2 SPACING, -Nalls to be 3 1/2" common wire nalls WHERE APPLICABLE. -Nalls to be located 2 1/2" mln. from the top and bottom of the member. Start all nalls 2 1/2" mln. from ends.
-Minimum 3 1/4" spacing between rows,

-Number of rows and spacing as per details shown, unless noted otherwise. - "X" represents nall driven from the opposite side.

- Head of all specified screws must be on the loaded side.

UNLESS NOTED OTHERWISE ON LAYOUT OR CALCULATION SHEET OF BEAM IN THE FLOOR PACKAGE

FOR MULTIPLE MEMBER CONNECTION OF BOISE ALLJOISTS REFER TO THE BOISE CASCADE INSTALLATION GUIDE

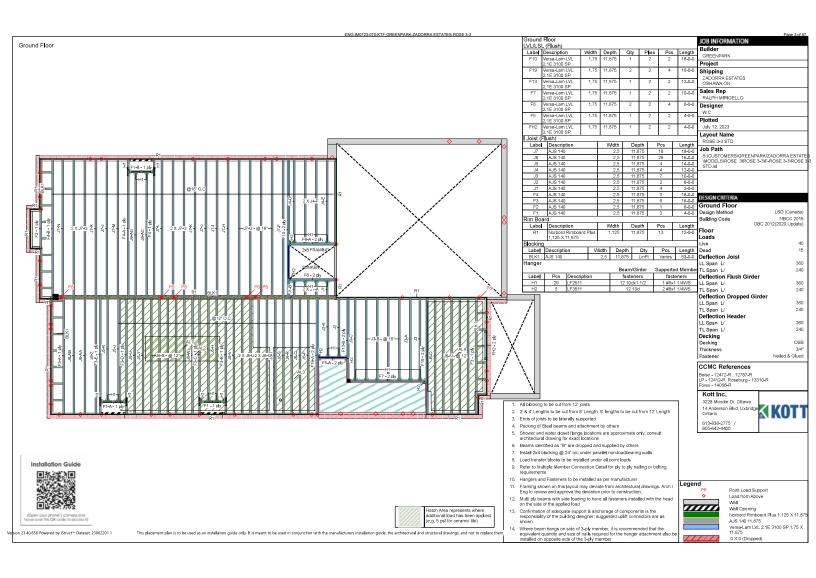
Installation Guide



Last Revised January 13, 2023

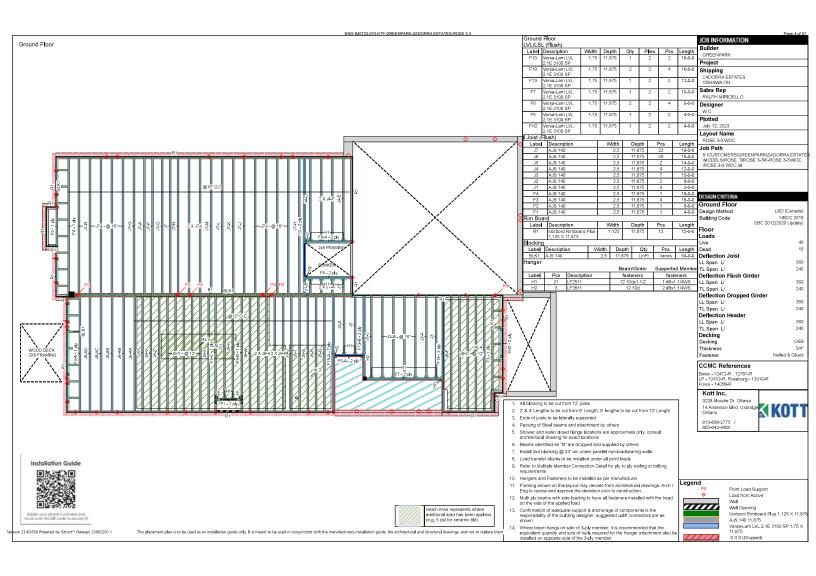


# MHP 23029





# MHP 23029



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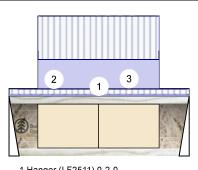
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lient: OF PERMIT PLANS ES 11.875L

Date: 7/12/2023 WС Input by: Job Name: ROSE 3-3 STD

Project #

Level: Ground Floor



1 Hanger (LF2511) 0-2-0

2 Hanger (LF2511) 0-2-0 2'11 11/16'

2'11 11/16'

11 7/8"

## Member Information

	Type:	Girder	Application:	Floor (Residential)
	Plies:	1	Design Method:	LSD
	Moisture Condition:	Dry	Building Code:	NBCC 2015
ı	Deflection LL:	360		OBC 2012(2020 Update)
	Deflection TL:	240	Load Sharing:	No
	Importance:	Normal - II	Deck:	Not Checked
	General Load		Vibration:	Not Checked
	Floor Live:	40 PSF		
	Dead:	15 PSF		

# **Unfactored Reactions UNPATTERNED lb (Uplift)**

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	308	196	0	0
2	Vertical	305	194	0	0

### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	597 ft-lb	1'5 13/16"	5305 ft-lb	0.113 (11%)	1.25D+1.5L	L
Unbraced	597 ft-lb	1'5 13/16"	5305 ft-lb	0.113 (11%)	1.25D+1.5L	L
Shear	701 <b>l</b> b	1 1/4"	2350 lb	0.298 (30%)	1.25D+1.5L	L
Perm Defl in.	0.003 (L/10538)	1'5 13/16"	0.092 (L/360)	0.034 (3%)	D	Uniform
LL Defl inch	0.005 (L/6761)	1'5 13/16"	0.092 (L/360)	0.053 (5%)	L	L
TL Defl inch	0.008 (L/4119)	1'5 13/16"	0.138 (L/240)	0.058 (6%)	D+L	L

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	597 ft-lb	1'5 13/16"	5305 ft-lb	0.113 (11%)	1.25D+1.5L	L
Unbraced	597 ft-lb	1'5 13/16"	5305 ft-lb	0.113 (11%)	1.25D+1.5L	L
Shear	701 <b>l</b> b	1 1/4"	2350 lb	0.298 (30%)	1.25D+1.5L	L
Perm Defl in.	0.003 (L/10538)	1'5 13/16"	0.092 (L/360)	0.034 (3%)	D	Uniform
LL Defl inch	0.005 (L/6761)	1'5 13/16"	0.092 (L/360)	0.053 (5%)	L	L
TL Defl inch	0.008 (L/4119)	1'5 13/16"	0.138 (L/240)	0.058 (6%)	D+L	L

# **Bearings and Factored Reactions**

Bearing	Length	Dir.	Cap. R	eact D/L <b>I</b> b	Total	Ld. Case	Ld. Comb.
1 -	2.000"	Vert	44%	245 / 463	708	L	1.25D+1.5L
Hanger							
2 -	2.000"	Vert	44%	242 / 458	700	L	1.25D+1.5L
Hanger							

# **Design Notes**

- Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: SPF, Thickness: 2 1/2"
- 4 Right Header: SPF, Thickness: 2 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 7 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.



JULY 13, 2023

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

<b>I</b> D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-11-11	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-0-0 to 2-11-11		Тор	4 PLF	0 PLF	0 PLF	0 PLF	
3	Part. Uniform	0-5-12 to 2-5-12		Far Face	172 PLF	261 PLF	0 PLF	0 PLF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs provide proper drainage to prevent ponding.

This design is valid until 4/17/2026

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.





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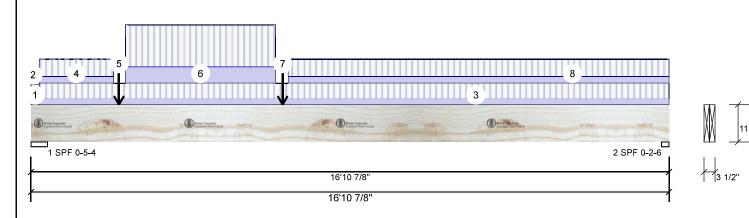
7/12/2023 Input by: WC

Job Name: ROSE 3-3 STD

Project #

Versa-Lam LVL 2.1E 31 00 SPCHIEF 61 27 50 FF X 11.87 5'





Member Info	rmation			Unfa	actored Rea	actions (	UNP	ATTERNED II	b (Upl	ift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Li	ve	Dead		Snow	Wind
Plies:	2	Design Method:	LSD	1	Vertical	6	87	409		0	0
Moisture Condition	on: Dry	Building Code:	NBCC 2015	2	Vertical	4	51	285		0	0
Deflection LL:	360		OBC 2012(2020 Update)								
Deflection TL:	240	Load Sharing:	No								
Importance:	Normal - II	Deck:	Not Checked								
General Load		Vibration:	Not Checked								
Floor Live:	40 PSF			Bear	ings and F	actored	Rea	ctions			
Dead:	15 PSF			Bea	ring Length	Dir.	Сар.	React D/L Ib	Total	Ld. Case	Ld. Comb.
				1 - 9	SPF 5.250"	Vert	14%	512 / 1031	1543	L	1.25D+1.5L
				J 2-9	SPF 2.375"	Vert	20%	356 / 677	1033	L	1.25D+1.5L

## **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6906 ft-lb	6'8"	35392 ft-lb	0.195 (20%)	1.25D+1.5L	L
Unbraced	6906 ft-lb	6'8"	35392 ft-lb	0.195 (20%)	1.25D+1.5L	L
Shear	1464 <b>l</b> b	1'5 1/8"	13217 <b>l</b> b	0.111 (11%)	1.25D+1.5L	L
Perm Defl in.	0.074 (L/2641)	8'2 11/16"	0.546 (L/360)	0.136 (14%)	D	Uniform
LL Defl inch	0.131 (L/1506)	8'1 11/16"	0.546 (L/360)	0.239 (24%)	L	L
TL Defl inch	0.205 (L/959)	8'2 1/16"	0.820 (L/240)	0.250 (25%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be continuously laterally braced.
- 6 Bottom must be laterally braced at a maximum of 10'2 13/16" o.c.
- 7 Lateral slenderness ratio based on full section width



PROFESSIONAL

I.MATIJEVIC 100528832

**ENGINEERING NOTES: EWP-FLOORS. THE NOTE** PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

7 Edicidi olondo	illoss latio basca	on fall ocollon width.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 0-2-10	0-3-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 0-2-10	0-2-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Tie-In	0-2-10 to 16-10-14	0-3-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
4	Tie-In	0-2-10 to 2-2-3	0-4-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
5	Point	2-3-15		Near Face	70 <b>l</b> b	99 lb	0 lb	0 <b>l</b> b	F6	

Continued on page 2...

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

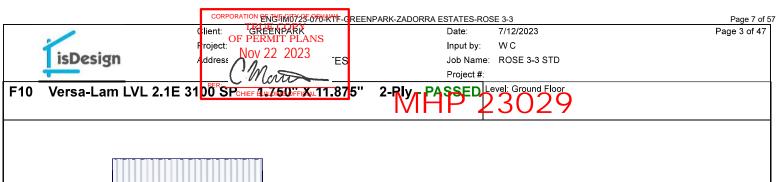
(800) 232-0788 www.bc.com CCMC: 12472

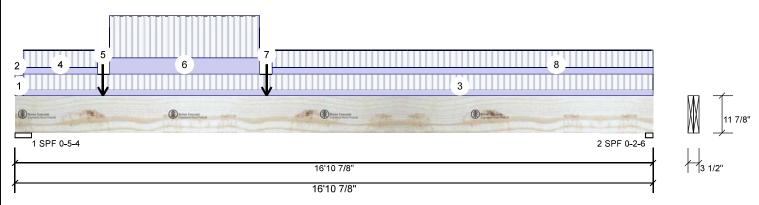
Manufacturer Info

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400









Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
6	Part. Uniform	2-6-1 to 6-5-13		Тор	13 PLF	34 PLF	0 PLF	0 PLF	
7	Point	6-8-0		Near Face	228 <b>l</b> b	520 <b>l</b> b	0 lb	0 <b>l</b> b	F6
8	Tie-In	6-9-12 to 16-10-14	0-4-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				12 PLF				



JULY 13, 2023

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### Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

# Handling & Installation

Handling & Installation

1. IVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

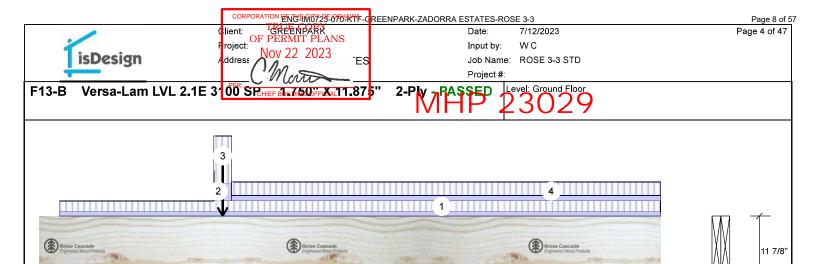
(800) 232-0788 www.bc.com CCMC: 12472

Kott Inc.

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400







Member Information Application: Floor (Residential) Type: Plies 2 Design Method: LSD Moisture Condition: Dry Building Code: **NBCC 2015** OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load Floor Live: 40 PSF 15 PSF Dead:

Capacity

0.054 (5%)

0.054 (5%)

Comb.

0.057 (6%) 1.25D+1.5L L

1.25D+1.5L L

1.25D+1.5L L

Case

Uniform

L

10'3 1/16' 10'3 1/16"

# **Unfactored Reactions UNPATTERNED lb (Uplift)**

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	354	210	0	0
2	Vertical	267	165	0	0

2 SPF 0-2-6

# **Bearings and Factored Reactions**

Bearing Length	ı Dir.	Cap.	React D/L Ib	Total	Ld. Case	Ld. Comb.
1 - SPF 5.500" End Grain	Vert	4%	263 / 531	794	L	1.25D+1.5L
2 - SPF 2.375"	Vert	12%	206 / 401	607	L	1.25D+1.5L



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# TL Defl inch 0.022 (L/5242)

Analysis Results

Perm Defl in 0.008

LL Defl inch 0.014 (L/8271)

Actual

1918 ft-lb

1918 ft-lb

(L/14317)

749 lb

Analysis

Moment

Shear

Unbraced

Design Notes 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.

Location Allowed

1'5 3/8" 13217 lb

3'8" 35392 ft-lb

3'8" 35392 ft-lb

5' 3/4" 0.324 (L/360) 0.025 (3%)

5' 1/8" 0.324 (L/360) 0.044 (4%) L

5' 3/8" 0.486 (L/240) 0.046 (5%) D+L

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be continuously laterally braced.

1 SPF End Grain 0-5-8

- 6 Bottom must be laterally braced at a maximum of 7'2 11/16" o.c.
- 7 Lateral slenderness ratio based on full section width.

<b>I</b> D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-4-1 to 10-3-1	0-5-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	2-10-10 to 3-2-2	1-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	3-0-6		Far Face	123 lb	272 <b>l</b> b	0 lb	0 lb	F5
4	Tie-In	3-2-2 to 10-3-1	0-6-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				12 PLF				

### Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

### Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code
approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

# 6. For flat roofs provide proper drainage to prevent ponding

Boise Cascade Wood Products 1111 W. Jefferson St. Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12472

### Manufacturer Info Kott Inc.

3228 Moodie Dr. Ottawa, Ontario 613-838-2775 / 905-642-4400





lient: GREENPARK OF PERMIT PLANS

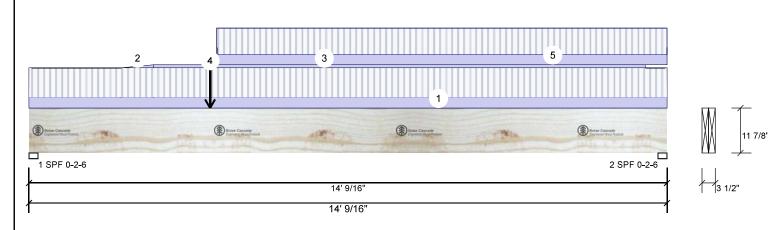
Date: 7/12/2023 Input by: WC

Job Name: ROSE 3-3 STD

Project #

Versa-Lam LVL 2.1E 3100 SPCHIEF 4 1/7/50 FF 1X 11.87 5





Member Infori	mation			Unfactored Reactions UNPATTERNED lb (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Live	Dead		Snow	Wind
Plies:	2	Design Method:	LSD	1	Vertical	1074	535		0	0
Moisture Condition	: Dry	Building Code:	NBCC 2015	2	Vertical	588	332		0	0
Deflection LL:	360		OBC 2012(2020 Update)							
Deflection TL:	240	Load Sharing:	No							
Importance:	Normal - II	Deck:	Not Checked							
General Load		Vibration:	Not Checked							
Floor Live:	40 PSF			Bear	ings and F	actored R	eactions			
Dead:	15 PSF			Bea	ring Length	Dir. C	ap. React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - 8	SPF 2.375"	Vert 4	5% 669 / 1611	2280	L	1.25D+1.5L
				2 - 5	SPF 2.375"	Vert 2	5% 415 / 883	1297	L	1.25D+1.5L

### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8361 ft-lb	3'11 7/8"	35392 ft-lb	0.236 (24%)	1.25D+1.5L	L
Unbraced	8361 ft-lb	3'11 7/8"	35392 ft-lb	0.236 (24%)	1.25D+1.5L	L
Shear	2232 lb	1'2 1/4"	13217 <b>l</b> b	0.169 (17%)	1.25D+1.5L	L
Perm Defl in	0.057 (L/2914)	6'6 11/16"	0.459 (L/360)	0.124 (12%)	D	Uniform
LL Defl inch	0.112 (L/1474)	6'5 1/2"	0.459 (L/360)	0.244 (24%)	L	L
TL Defl inch	0.169 (L/979)	6'5 7/8"	0.689 (L/240)	0.245 (25%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be continuously laterally braced.
- 6 Bottom must be laterally braced at a maximum of 10' 11/16" o.c.
- 7 Lateral slenderness ratio based on full section width.



JULY 13, 2023

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 14-0-9	0-5-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tapered Start	2-0-7		Тор	0 PLF	0 PLF	0 PLF	0 PLF	
	End	2-8-15			2 PLF	0 PLF	0 PLF	0 PLF	
3	Part. Uniform	2-8-15 to 13-6-13		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Point	3-11-14		Far Face	512 lb	1220 lb	0 lb	0 lb	F7

Continued on page 2...

### Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

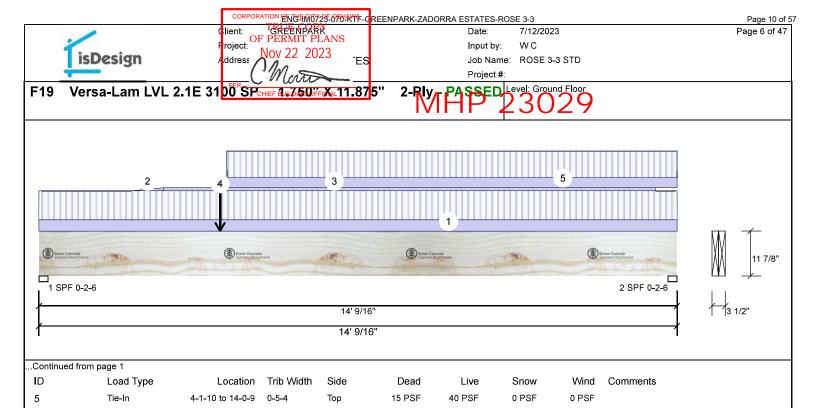
Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12472

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400







12 PLF



READ ALL NOTES ON THIS PAGE AND ON THE

**ENGINEERING NOTES: EWP-FLOORS. THE NOTE** PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

### Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

# Handling & Installation

Handling & Installation

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Kott Inc.

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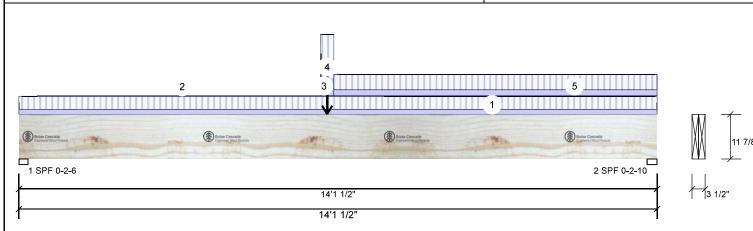
This design is valid until 4/17/2026

Self Weight

Project #

Versa-Lam LVL 2.1E 3100 SR-HEF EUL/1500FFXA11.875"

SED Level: Ground Floor



Member Inform	nation			Unfa	actored Rea	ctions l	JNP	ATTERNED II	o (Uplit	ft)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Li	ve	Dead		Snow	Wind
Plies:	2	Design Method:	LSD	1	Vertical	3	43	228		0	0
Moisture Condition:	: Dry	Building Code:	NBCC 2015	2	Vertical	4	28	257		0	0
Deflection LL:	360		OBC 2012(2020 Update)								
Deflection TL:	240	Load Sharing:	No								
Importance:	Normal - II	Deck:	Not Checked								
General Load		Vibration:	Not Checked								
Floor Live:	40 PSF			Bear	rings and Fa	actored	Rea	ctions			
Dead:	15 PSF			Bea	aring Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 -	SPF 2.375"	Vert	16%	285 / 515	800	L	1.25D+1.5L
		1		2 -	SPF 2.625"	Vert	17%	321 / 642	962	L	1.25D+1.5L

### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3979 ft-lb	6'9 7/8"	35392 ft-lb	0.112 (11%)	1.25D+1.5L	L
Unbraced	3979 ft-lb	6'9 7/8"	35392 ft-lb	0.112 (11%)	1.25D+1.5L	L
Shear	846 <b>l</b> b	12'11"	13217 <b>l</b> b	0.064 (6%)	1.25D+1.5L	L
Perm Defl in.	0.032 (L/5120)	7' 15/16"	0.461 (L/360)	0.070 (7%)	D	Uniform
LL Defl inch	0.054 (L/3055)	7'1 1/4"	0.461 (L/360)	0.118 (12%)	L	L
TL Defl inch	0.087 (L/1913)	7'1 1/8"	0.692 (L/240)	0.125 (13%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be continuously laterally braced.
- 6 Bottom must be laterally braced at a maximum of 7'3 5/8" o.c.
- 7 Lateral slenderness ratio based on full section width.



READ ALL NOTES ON THIS PAGE AND ON THE **ENGINEERING NOTES: EWP-FLOORS. THE NOTE** PAGE IS AN INTEGRAL PART OF THIS DRAWING

AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
Tie-In	0-0-0 to 14-1-8	0-6-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Part. Uniform	0-4-14 to 8-0-15		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
Tie-In	6-8-2 to 6-11-10	1-10-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Point	6-9-14		Near Face	117 <b>l</b> b	256 lb	0 lb	0 lb	F5
Tie-In	6-11-10 to 14-1-8	0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Tie-In Part. Uniform Tie-In Point	Tie-In 0-0-0 to 14-1-8 Part. Uniform 0-4-14 to 8-0-15 Tie-In 6-8-2 to 6-11-10 Point 6-9-14	Tie-In 0-0-0 to 14-1-8 0-6-9 Part. Uniform 0-4-14 to 8-0-15 Tie-In 6-8-2 to 6-11-10 1-10-5 Point 6-9-14	Tie-In         0-0-0 to 14-1-8         0-6-9         Top           Part. Uniform         0-4-14 to 8-0-15         Top           Tie-In         6-8-2 to 6-11-10         1-10-5         Top           Point         6-9-14         Near Face	Tie-In         0-0-0 to 14-1-8         0-6-9         Top         15 PSF           Part. Uniform         0-4-14 to 8-0-15         Top         1 PLF           Tie-In         6-8-2 to 6-11-10         1-10-5         Top         15 PSF           Point         6-9-14         Near Face         117 lb	Tie-In         0-0-0 to 14-1-8         0-6-9         Top         15 PSF         40 PSF           Part. Uniform         0-4-14 to 8-0-15         Top         1 PLF         0 PLF           Tie-In         6-8-2 to 6-11-10         1-10-5         Top         15 PSF         40 PSF           Point         6-9-14         Near Face         117 lb         256 lb	Tie-In         0-0-0 to 14-1-8         0-6-9         Top         15 PSF         40 PSF         0 PSF           Part. Uniform         0-4-14 to 8-0-15         Top         1 PLF         0 PLF         0 PLF           Tie-In         6-8-2 to 6-11-10         1-10-5         Top         15 PSF         40 PSF         0 PSF           Point         6-9-14         Near Face         117 lb         256 lb         0 lb	Tie-In         0-0-0 to 14-1-8         0-6-9         Top         15 PSF         40 PSF         0 PSF         0 PSF           Part. Uniform         0-4-14 to 8-0-15         Top         1 PLF         0 PLF         0 PLF         0 PLF           Tie-In         6-8-2 to 6-11-10         1-10-5         Top         15 PSF         40 PSF         0 PSF         0 PSF           Point         6-9-14         Near Face         117 lb         256 lb         0 lb         0 lb

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and badings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

# Handling & Installation

Handling & Installation

1. IVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

12 PLF

# Manufacturer Info

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(800) 232-0788 www.bc.com CCMC: 12472

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400



This design is valid until 4/17/2026

Self Weight

lient: GREENPARK OF PERMIT PLANS Nov 22 2023 ES

7/12/2023 WС Input by:

Job Name: ROSE 3-3 STD

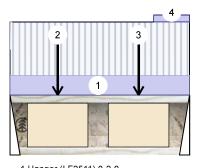
Project #

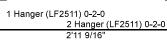
Brg Direction

**AJS 140** 

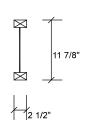
11.825







2'11 9/16'



## Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015
Deflection LL:	360		OBC 2012(2020 Update)
Deflection TL:	240	Load Sharing:	No
Importance:	Normal - II	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	40 PSF		
Dead:	15 PSF		

# **Unfactored Reactions UNPATTERNED Ib (Uplift)** Live

26	0 0	
28	0 0	

### Analysis Results

Analysis	Actua <b>l</b>	Location	Allowed	Capacity	Comb.	Case
Moment	457 ft-lb	1'9 3/16"	5305 ft-lb	0.086 (9%)	1.25D+1.5L	L
Unbraced	457 ft-lb	1'9 3/16"	5305 ft-lb	0.086 (9%)	1.25D+1.5L	L
Shear	647 <b>l</b> b	1 1/4"	2350 lb	0.275 (28%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/19337)	1'7 1/16"	0.092 (L/360)	0.019 (2%)	D	Uniform
LL Defl inch	0.004 (L/7467)	1'6 9/16"	0.092 (L/360)	0.048 (5%)	L	L
TI D. 0.1.	0.000 (1.45007)	410.07411	0.400 (1.0040)	0.045 (40()	D	

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	457 ft-lb	1'9 3/16"	5305 ft-lb	0.086 (9%)	1.25D+1.5L	L
Unbraced	457 ft-lb	1'9 3/16"	5305 ft-lb	0.086 (9%)	1.25D+1.5L	L
Shear	647 <b>l</b> b	1 1/4"	2350 lb	0.275 (28%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/19337)	1'7 1/16"	0.092 (L/360)	0.019 (2%)	D	Uniform
LL Defl inch	0.004 (L/7467)	1'6 9/16"	0.092 (L/360)	0.048 (5%)	L	L
TL Defl inch	0.006 (L/5387)	1'6 3/4"	0.138 (L/240)	0.045 (4%)	D+L	L

# **Bearings and Factored Reactions**

Bearing	Length	Dir.	Cap. F	React D/L <b>I</b> b	Total	Ld. Case	Ld. Comb.
1 -	2.000"	Vert	41%	157 / 497	654	L	1.25D+1.5L
Hanger							
2 -	2.000"	Vert	40%	160 / 484	644	L	1.25D+1.5L
Hanger							

## **Design Notes** Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.

- 2 Fill all hanger nailing holes.
- 3 Left Header: SPF, Thickness: 2 1/2"
- 4 Right Header: SPF, Thickness: 2 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 7 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.



JULY 13, 2023

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ID	Load Type	Location 7	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-11-9	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-7		Far Face	104 lb	278 <b> </b> b	0 <b>l</b> b	0 <b>l</b> b	J5
3	Point	2-1-7		Far Face	113 lb	285 lb	0 <b>l</b> b	0 lb	J5
4	Part. Uniform	2-4-7 to 2-11-9		Тор	4 PLF	0 PLF	0 PLF	0 PLF	

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. Noist flanges must not be cut or drilled

  2. Refer to latest copy of the Juoist product information details for framing details, stifferer tables, web hole chart, bridging details, multi-ray fastening details and handling/erection details

  3. Damaged Juoists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
7. For flat roofs provide proper drainage to prevent ponding.

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400



OF PERMIT PLANS roject: Nov 22 2023 ES

Date: 7/12/2023 WС Input by:

Job Name: ROSE 3-3 STD

Project #

Brg

2

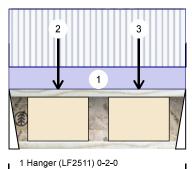
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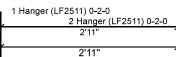
Vertical

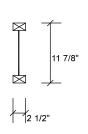
Vertica

**AJS 140** F1-B









Snow

0

0

Wind

0

0

# Member Information Application: Floor (Residential) Type: Plies: Design Method: LSD Moisture Condition: Dry **Building Code: NBCC 2015**

Analysis Resu	ults		
Dead:	15 PSF		
Floor Live:	40 PSF		
General Load		Vibration:	Not Checked
Importance:	Normal - II	Deck:	Not Checked
Deflection TL:	240	Load Sharing:	No
Deflection LL:	360		OBC 2012(2020 Update)

# **Bearings and Factored Reactions**

**Unfactored Reactions UNPATTERNED lb (Uplift)** 

Live

377

383

Bearing	Length	Dir.	Cap.	React D/L <b>I</b> b	Total	Ld. Case	Ld. Comb.
1 - Hanger	2.000"	Vert	46%	177 / 565	742	L	1.25D+1.5L
2 - Hanger	2.000"	Vert	47%	180 / 575	755	L	1.25D+1.5L

Dead

142

144

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	510 ft-lb	1'3 1/16"	5305 ft-lb	0.096 (10%)	1.25D+1.5L	L
Unbraced	510 ft-lb	1'3 1/16"	5305 ft-lb	0.096 (10%)	1.25D+1.5L	L
Shear	748 lb	2'9 3/4"	2350 lb	0.318 (32%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/17466)	1'4 7/8"	0.090 (L/360)	0.021 (2%)	D	Uniform
LL Defl inch	0.005 (L/6567)	1'4 7/8"	0.090 (L/360)	0.055 (5%)	L	L
TL Defl inch	0.007 (L/4773)	1'4 7/8"	0.135 (L/240)	0.050 (5%)	D+L	L

# **Design Notes**

- Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Left Header: SPF, Thickness: 2 1/2"
- 4 Right Header: SPF, Thickness: 2 1/2"
- 5 Girders are designed to be supported on the bottom edge only.
- 6 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 7 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.



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<b>I</b> D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-11-0	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-11		Near Face	127 <b> </b> b	338 lb	0 lb	0 <b>l</b> b	J6
3	Point	2-1-11		Near Face	125 <b>l</b> b	332 lb	0 lb	0 lb	J6

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. Noist flanges must not be cut or drilled

  2. Refer to latest copy of the Lioist product information details for framing details. stifferer tables, web hole chart, bridging details. multi-qly fastening details and handling/erection details

  3. Damaged Lioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
   Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
   For flat roofs provide proper drainage to prevent

This design is valid until 4/17/2026

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise. ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

### Kott Inc.







F2

18

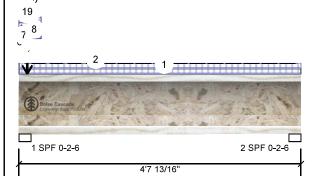
OF PERMIT PLANS roject: Nov 22 2023

7/12/2023 Date: W C Input by:

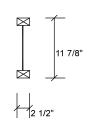
Job Name: ROSE 3-3 STD

Project #

Level: Ground Floor **AJS 140** 11.875



4'7 13/16'



### Member Information **Unfactored Reactions UNPATTERNED lb (Uplift)** Application: Floor (Residential) Wind Type: Brg Direction Live Dead Snow Plies: Design Method: LSD 489 382 Vertical 188 1 0 Moisture Condition: Dry **Building Code: NBCC 2015** 2 Vertical 126 48 0 0 OBC 2012(2020 Update) Deflection LL: 360 Load Sharing: Deflection TL: 240 Not Checked Deck: Importance: Normal - II Vibration: Not Checked General Load **Bearings and Factored Reactions** Floor Live: 40 PSF 15 PSF Dead: Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 2.375" Vert 82% 611 / 762 1373 L 1.25D+1.5S +L 2 - SPF 2.375" Vert 17% 60 / 189 249 L 1 25D+1 5L

### Analysis Results

Analysis	Actua <b>l</b>	Location	Allowed	Capacity	Comb.	Case
Moment	259 ft-lb	2'3 3/4"	4668 ft-lb	0.055 (6%)	1.25D+1.5L	L
Unbraced	259 ft-lb	2'3 3/4"	4668 ft-lb	0.055 (6%)	1.25D+1.5L	L
Shear	282 lb	1 5/8"	2068 lb	0.136 (14%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/40201)	2'3 9/16"	0.146 (L/360)	0.009 (1%)	D	Uniform
LL Defl inch	0.003 (L/15680)	2'3 7/8"	0.146 (L/360)	0.023 (2%)	L+0.5S	L
TL Defl inch	0.005 (L/11281)	2'3 3/4"	0.219 (L/240)	0.021 (2%)	D+L+0.5S	L

## **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 Rottom flange must be laterally braced at bearings



JULY 13, 2023

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4 Bottom hange must be laterally braced at bearings.										
	<b>I</b> D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 4-7-13	0-7-9	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 4-7-13	0-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	3	Part. Uniform	0-0-0 to 0-1-2		Тор	15 PLF	0 PLF	40 PLF	0 PLF	
	4	Part. Uniform	0-0-0 to 0-1-2		Тор	20 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
	5	Tapered Start	0-0-0		Тор	2 PLF	6 PLF	0 PLF	0 PLF	

Continued on page 2...

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the IJoist product information
  details for framing details, stiffener tables, web hole
  chart, bridging details, multi-rily fastening details and
  handling/erection details

  3. Damaged IJoists must not be used
  4. Design assumes top flange to be laterally restrained
  by attached sheathing or as specified in engineering
  notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
   Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
   For flat roofs provide proper drainage to prevent

(800) 232-0788

www.bc.com CCMC: 12787

# Manufacturer Info

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**AJS 140** 

F2

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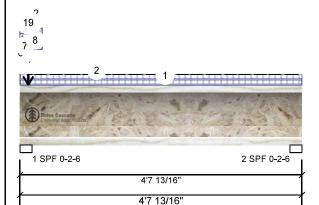
lient: OF PERMIT PLANS Nov 22 2023 ddress ES PAHES BOUND OFFICIA 11.875

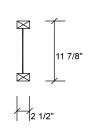
Date: 7/12/2023 Input by: WC

Job Name: ROSE 3-3 STD

Project #

Level: Ground Floor





Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	End	0-1-2			2 PLF	6 PLF	0 PLF	0 PLF	
6	Part. Uniform	0-0-0 to 0-1-2		Тор	1 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
7	Part. Uniform	0-0-0 to 0-1-2		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
8	Part. Uniform	0-0-0 to 0-4-6		Тор	30 PLF	0 PLF	80 PLF	0 PLF	
9	Part. Uniform	0-0-0 to 0-4-6		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
10	Tapered Start	0-0-0		Тор	5 PLF	13 PLF	0 PLF	0 PLF	
	End	0-4-6			5 PLF	13 PLF	0 PLF	0 PLF	
11	Part. Uniform	0-0-0 to 0-4-6		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
12	Part. Uniform	0-0-0 to 0-4-6		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
13	Tapered Start	0-1-2		Тор	5 PLF	13 PLF	0 PLF	0 PLF	
	End	0-1-2			5 PLF	13 PLF	0 PLF	0 PLF	
14	Part. Uniform	0-1-2 to 0-1-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
15	Part. Uniform	0-1-2 to 0-1-2		Тор	30 PLF	0 PLF	80 PLF	0 PLF	
16	Part. Uniform	0-1-2 to 0-1-2		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
17	Tapered Start	0-1-2		Тор	5 PLF	13 PLF	0 PLF	0 PLF	
	End	0-1-2			5 PLF	13 PLF	0 PLF	0 PLF	
18	Part. Uniform	0-1-2 to 0-1-2		Тор	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight
19	Point	0-1-10		Тор	378 lb	57 <b>l</b> b	350 lb	0 <b>l</b> b	B2
	Bearing Length	0-1-8							

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. Uloist flanges must not be cut or drilled

  2. Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

  3. Damaged Lioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

# Kott Inc.





OF PERMIT PLANS roject: Nov 22 2023 ES

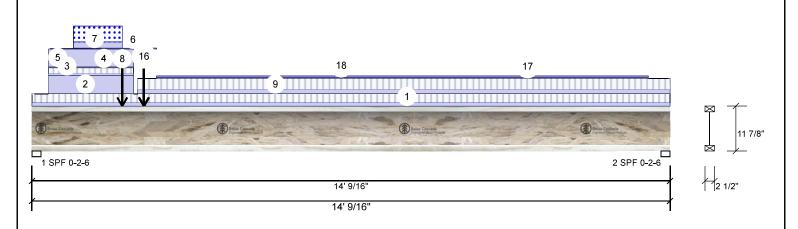
WС Input by:

Job Name: ROSE 3-3 STD

Project #

F3 **AJS 140**  11.875L





Member Inforn	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction		Live	Dead		Snow
Plies:	1	Design Method:	LSD	1	Vertical		388	418		110
Moisture Condition:	Dry	Building Code:	NBCC 2015	2	Vertical		334	188		15
Deflection LL:	360		OBC 2012(2020 Update)							
Deflection TL:	240	Load Sharing:	No							
Importance:	Normal - II	Deck:	Not Checked							
General Load		Vibration:	Not Checked							
Floor Live:	40 PSF			Bea	rings and F	actore	d Read	tions		
Dead:	15 PSF			Ве	aring Length	Dir.	Cap.	React D/L Ib	Total	Ld. Cas
				1 -	SPF 2.375"	Vert	72%	523 / 692	1215	L
Analysis Result	s	,		2 -	SPF 2.375"	Vert	45%	236 / 516	752	L

Ana <b>l</b> ysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2914 ft-lb	6' 11/16"	5305 ft-lb	0.549 (55%)	1.25D+1.5L +S	L
Unbraced	2914 ft-lb	6' 11/16"	5305 ft-lb	0.549 (55%)	1.25D+1.5L +S	L
Shear	1210 lb	1 5/8"	2350 lb	0.515 (51%)	1.25D+1.5L +S	L
Perm Defl in.	0.102 (L/1623)	6'6 15/16"	0.459 (L/360)	0.222 (22%)	D	Uniform
LL Defl inch	0.159 (L/1042)	6'10"	0.459 (L/360)	0.345 (35%)	L+0.5S	L
TL Defl inch	0.260 (L/635)	6'8 13/16"	0.689 (L/240)	0.378 (38%)	D+L+0.5S	L

## **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 bollom nange	must be laterally brace	ed at bearings.				_				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 14-0-9	0-6-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Part. Uniform	0-4-6 to 2-2-13		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
3	Tapered Start	0-4-6		Тор	5 PLF	12 PLF	0 PLF	0 PLF		
	End	2-2-13			5 PLF	12 PLF	0 PLF	0 PLF		
4	Part. Uniform	0-4-6 to 2-2-13		Top	2 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight	

Continued on page 2...

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info Kott Inc.

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702 (800) 232-0788

www.bc.com CCMC: 12787

613-838-2775 / 905-642-4400



3228 Moodie Dr, Ottawa, Ontario

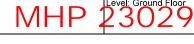
Page 16 of 57

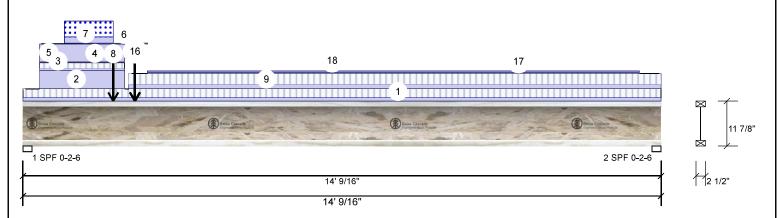
Wind 0 0

Ld. Comb. 1.25D+1.5L +S 1 25D+1 5L +S

Page 12 of 47







<u> </u>									
	from page 1								
<b>I</b> D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
5	Part. Uniform	0-4-6 to 2-2-13		Тор	40 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
6	Tapered Start	0-4-14		Тор	1 PLF	0 PLF	0 PLF	0 PLF	
	End	2-8-15			2 PLF	0 PLF	0 PLF	0 PLF	
7	Part. Uniform	0-11-0 to 1-11-14		Тор	14 PLF	0 PLF	35 PLF	0 PLF	
8	Point	1-11-14		Тор	45 lb	0 lb	88 lb	0 <b>l</b> b	F14
	Bearing Length	0-3-8							
9	Tie-In	2-3-15 to 14-0-9	0-7-7	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
11	Point	2-5-9		Тор	1 lb	0 lb	0 <b>l</b> b	0 <b>l</b> b	Rim Board Self Weight
	Bearing Length	0-3-8							
12	Point	2-5-9		Тор	18 <b>l</b> b	47 lb	0 lb	0 <b>l</b> b	J4
	Bearing Length	0-3-8							
13	Point	2-5-9		Тор	17 <b>l</b> b	0 lb	0 <b>l</b> b	0 <b>l</b> b	Wall Self Weight
	Bearing Length	0-3-8							
14	Point	2-5-9		Тор	17 <b>l</b> b	0 lb	0 <b> </b> b	0 <b>l</b> b	Wall Self Weight
	Bearing Length	0-3-8							
15	Point	2-5-9		Тор	30 lb	81 <b>l</b> b	0 <b>l</b> b	0 <b>l</b> b	J4
	Bearing Length	0-3-8							
16	Point	2-5-9		Тор	29 lb	0 lb	0 <b>l</b> b	0 <b>l</b> b	Wall Self Weight
	Bearing Length	0-3-8							
17	Part. Uniform	2-8-15 to 13-6-13		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
18	Part. Uniform	2-8-15 to 13-6-13		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
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READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



JULY 13, 2023

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. Julist flanges must not be cut or drilled

  2. Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-qly fastening details and handling/erection details

  3. Damaged IJoists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

### Kott Inc.

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400

Page 17 of 57

Page 13 of 47





OF PERMIT PLANS roject: Nov 22 2023 ES

Date: 7/12/2023 Input by: WC

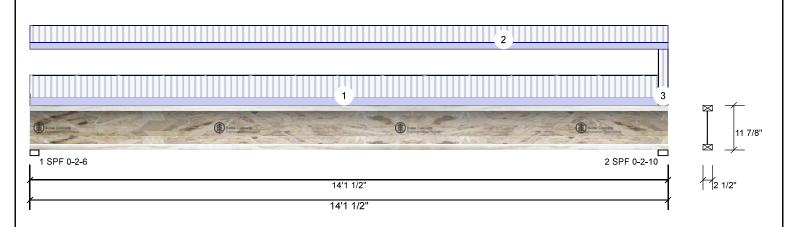
Job Name: ROSE 3-3 STD

Project #

**AJS 140** F<sub>3</sub>-A

11.825





Member Inforn	nation			Unfactored Reactions UNPATTERNED lb (Uplift)									
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Li	/e	Dead		Snow	Wind		
Plies:	1	Design Method:	LSD	1	Vertical	3	93	147		0	0		
Moisture Condition:	: Dry	Building Code:	NBCC 2015	2	Vertical	4	00	150		0	0		
Deflection LL:	360		OBC 2012(2020 Update)										
Deflection TL:	240	Load Sharing:	No										
Importance:	Normal - II	Deck:	Not Checked										
General Load		Vibration:	Not Checked										
Floor Live:	40 PSF			Bea	rings and Fa	actored	Read	ctions					
Dead:	15 PSF			Bea	aring Length	Dir.	Сар.	React D/L Ib	Total	Ld. Case	Ld. Comb.		
				1 -	SPF 2.375"	Vert	46%	184 / 589	773	L	1.25D+1.5L		
				2 -	SPF 2.625"	Vert	45%	187 / 600	787	L	1.25D+1.5L		

### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2623 ft-lb	7' 5/8"	5305 ft-lb	0.495 (49%)	1.25D+1.5L	L
Unbraced	2623 ft-lb	7' 5/8"	5305 ft-lb	0.495 (49%)	1.25D+1.5L	L
Shear	762 <b>l</b> b	13'11 5/8"	2350 lb	0.324 (32%)	1.25D+1.5L	L
Perm Defl in	0.063 (L/2651)	7' 5/8"	0.461 (L/360)	0.136 (14%)	D	Uniform
LL Defl inch	0.167 (L/994)	7' 5/8"	0.461 (L/360)	0.362 (36%)	L	L
TL Defl inch	0.230 (L/723)	7' 5/8"	0.692 (L/240)	0.332 (33%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 4 Bottom flange must be laterally braced at bearings.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

	,									
<b>I</b> D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Tie-In	0-0-0 to 13-10-14	0-9-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
2	Tie-In	0-0-0 to 14-1-8	0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF		
3	Tie-In	13-10-14 to 14-1-8	1-5-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF		

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
   Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
   For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

# Kott Inc.



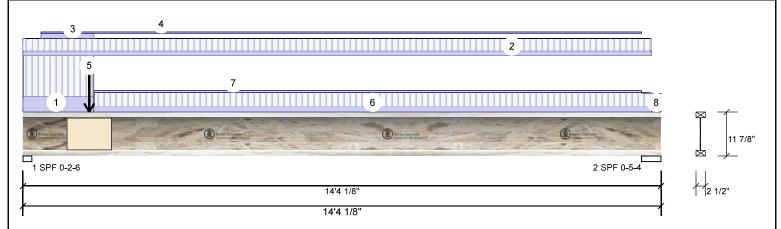
Date: 7/12/2023 Input by: W C

Job Name: ROSE 3-3 STD

Project #

11.875 PASSE **AJS 140** F<sub>3</sub>-B

# Level: Ground Floor



Member Info	rmation			Unfactored Reactions UNPATTERNED <b>l</b> b (Uplift)									
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction		_ive	Dead		Snow	Wind		
Plies:	1	Design Method:	LSD	1	Vertical		632	348		0	0		
Moisture Condition	on: Dry	Building Code:	NBCC 2015	2	Vertical		332	167		0	0		
Deflection LL:	360		OBC 2012(2020 Update)										
Deflection TL:	240	Load Sharing:	No										
Importance:	Normal - II	Deck:	Not Checked										
General Load		Vibration:	Not Checked										
Floor Live:	40 PSF			Bear	rings and F	actored	l Rea	ctions					
Dead:	15 PSF			Bea	aring Length	Dir.	Сар.	React D/L Ib	Total	Ld. Case	Ld. Comb.		
				1 -	SPF 2.375"	Vert	82%	435 / 948	1383	L	1.25D+1.5L		
				2 -	SPF 5.250"	Vert	37%	209 / 498	707	L	1.25D+1.5L		

## **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2664 ft-lb	6'2 3/8"	5305 ft-lb	0.502 (50%)	1.25D+1.5L	L
Unbraced	2664 ft-lb	6'2 3/8"	5305 ft-lb	0.502 (50%)	1.25D+1.5L	L
Shear	1361 lb	1 5/8"	2350 lb	0.579 (58%)	1.25D+1.5L	L
Perm Defl in.	0.083 (L/2007)	6'8 15/16"	0.461 (L/360)	0.179 (18%)	D	Uniform
LL Defl inch	0.158 (L/1048)	6'9 5/8"	0.461 (L/360)	0.344 (34%)	L	L
TL Defl inch	0.241 (L/688)	6'9 3/8"	0.692 (L/240)	0.349 (35%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 Bottom flange must be laterally braced at a maximum of 12'10 1/4" o.c.



ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING USED IN THE DESIGN OF THIS COMPONENT.

		2.0000 0.0 0.110/1110/11							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-7-2	1-7-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 14-1-8	0-5-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-4-14 to 1-7-2		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-4-14 to 13-10-14		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-5-14		Far Face	194 lb	305 lb	0 <b>l</b> b	0 lb	F1
6	Tie-In	1-7-2 to 14-1-8	0-6-10	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-7-2 to 13-10-14		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	14-1-8 to 14-4-2	0-6-5	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

www.bc.com CCMC: 12787

This design is valid until 4/17/2026

# READ ALL NOTES ON THIS PAGE AND ON THE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

Manufacturer Info Kott Inc. 3228 Moodie Dr, Ottawa, Ontario Boise Cascade Wood Products 613-838-2775 / 905-642-4400 1111 W. Jefferson St. Boise, ID 83702 (800) 232-0788



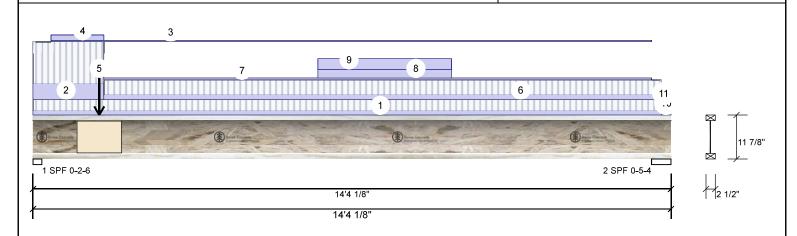
Page 19 of 57

Page 15 of 47

11.8<mark>75</mark>" F3-C **AJS 140** 

Level: Ground Floor

Project #



Member Inforr	nation			Unfactored Reactions UNPATTERNED <b>l</b> b (Uplift)									
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	L	ive	Dead	Sı	now	Wind		
Plies:	1	Design Method:	LSD	1	Vertical	(	612	379		0	0		
Moisture Condition	: Dry	Building Code:	NBCC 2015	2	Vertical		310	208		0	0		
Deflection LL:	360		OBC 2012(2020 Update)										
Deflection TL:	240	Load Sharing:	No										
Importance:	Normal - II	Deck:	Not Checked										
General Load		Vibration:	Not Checked										
Floor Live:	40 PSF			Bear	rings and Fa	actored	l Rea	ctions					
Dead:	15 PSF			Bea	aring Length	Dir.	Сар.	React D/L lb	Total Lo	d. Case	Ld. Comb.		
				1 -	SPF 2.375"	Vert	83%	474 / 918	1392 L		1.25D+1.5L		
				2 -	SPF 5.250"	Vert	38%	260 / 465	725 L		1.25D+1.5L		
Amalusia Dasult	-				011								

### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2814 ft-lb	6'7 3/8"	5305 ft-lb	0.530 (53%)	1.25D+1.5L	L
Unbraced	2814 ft-lb	6'7 3/8"	5305 ft-lb	0.530 (53%)	1.25D+1.5L	L
Shear	1370 <b>l</b> b	1 5/8"	2350 lb	0.583 (58%)	1.25D+1.5L	L
Perm Defl in	0.108 (L/1532)	6'11 3/8"	0.461 (L/360)	0.235 (23%)	D	Uniform
LL Defl inch	0.149 (L/1115)	6'9 3/8"	0.461 (L/360)	0.323 (32%)	L	L
TL Defl inch	0.257 (L/646)	6'10 1/4"	0.692 (L/240)	0.372 (37%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 Bottom flange must be laterally braced at a maximum of 12'10 1/4" o.c.



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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 14-1-8	0-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-7-2	1-7-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-4-14 to 13-10-14		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-4-14 to 1-7-2		Тор	8 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-5-14		Near Face	196 lb	308 lb	0 lb	0 <b>l</b> b	F1
6	Tie-In	1-7-2 to 14-1-8	0-6-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-7-2 to 13-10-14		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
Continued on pa	ge 2								

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# Manufacturer Info

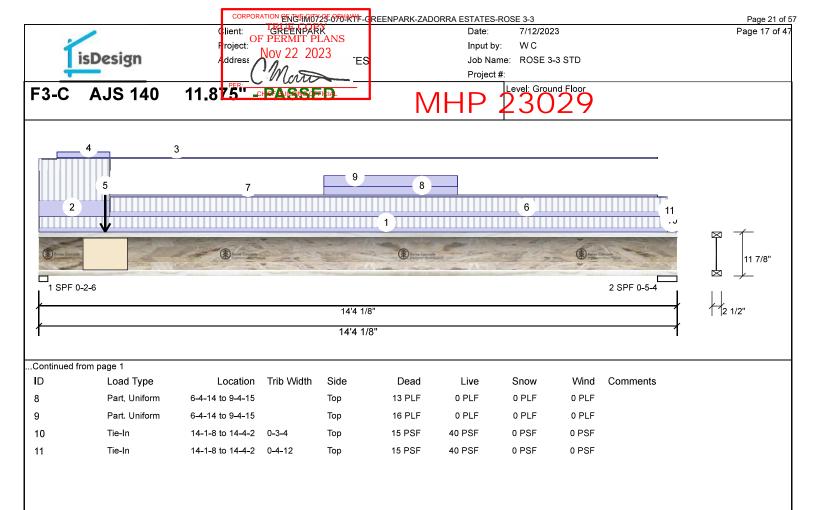
Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

# Kott Inc.









READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

### Notes

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the IJoist product information
  details for framing details, stiffener tables, web hole
  chart, bridging details, multi-rily fastening details and
  handling/erection details

  3. Damaged IJoists must not be used
  4. Design assumes top flange to be laterally restrained
  by attached sheathing or as specified in engineering
  notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

### Kott Inc.

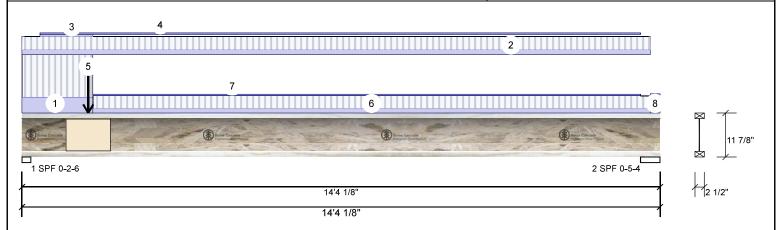




Page 22 of 57 Page 18 of 47

11.8<mark>75</mark>" **AJS 140** F3-D

# Level: Ground Floor



Member Inform	nation			Unfactored Reactions UNPATTERNED lb (Uplift)								
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Live	Dead	Snow	Wind			
Plies:	1	Design Method:	LSD	1	Vertical	633	271	0	0			
Moisture Condition:	Dry	Building Code:	NBCC 2015	2	Vertical	315	146	0	0			
Deflection LL:	360		OBC 2012(2020 Update)									
Deflection TL:	240	Load Sharing:	No									
Importance:	Normal - II	Deck:	Not Checked									
General Load		Vibration:	Not Checked									
Floor Live:	40 PSF			Bear	rings and F	actored Read	ctions					
Dead:	15 PSF			Bea	aring Length	Dir. Cap.	React D/L Ib	Total Ld. Case	Ld. Comb.			
				1 - 3	SPF 2.375"	Vert 77%	339 / 950	1289 L	1.25D+1.5L			
				2 - 3	SPF 5.250"	Vert 34%	183 / 472	655 L	1.25D+1.5L			

## **Analysis Results**

Analysis	Actua <b>l</b>	Location	Allowed	Capacity	Comb.	Case
Moment	2466 ft-lb	6'2 3/8"	5305 ft-lb	0.465 (46%)	1.25D+1.5L	L
Unbraced	2466 ft-lb	6'2 3/8"	5305 ft-lb	0.465 (46%)	1.25D+1.5L	L
Shear	1267 <b>l</b> b	1 5/8"	2350 lb	0.539 (54%)	1.25D+1.5L	L
Perm Defl in.	0.070 (L/2377)	6'9 3/4"	0.461 (L/360)	0.151 (15%)	D	Uniform
LL Defl inch	0.152 (L/1090)	6'9 1/4"	0.461 (L/360)	0.330 (33%)	L	L
TL Defl inch	0.222 (L/747)	6'9 7/16"	0.692 (L/240)	0.321 (32%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 Bottom flange must be laterally braced at a maximum of 12'10 1/4" o.c.



ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING USED IN THE DESIGN OF THIS COMPONENT.

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		2.0000 0.0 0.110/1110/11							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-7-2	1-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 14-1-8	0-6-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-4-14 to 1-7-2		Тор	3 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-4-14 to 13-10-14		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
5	Point	1-5-14		Far Face	128 lb	323 lb	0 <b>l</b> b	0 lb	F1
6	Tie-In	1-7-2 to 14-1-8	0-5-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
7	Part. Uniform	1-7-2 to 13-10-14		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	14-1-8 to 14-4-2	0-6-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# READ ALL NOTES ON THIS PAGE AND ON THE AS IT CONTAINS SPECIFICATIONS AND CRITERIA

Manufacturer Info Kott Inc. 3228 Moodie Dr, Ottawa, Ontario Boise Cascade Wood Products 613-838-2775 / 905-642-4400 1111 W. Jefferson St. Boise, ID 83702



OF PERMIT PLANS roject: Nov 22 2023 ES

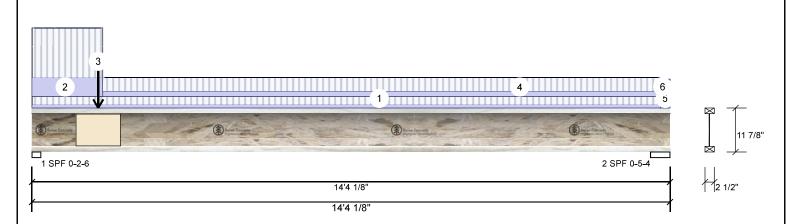
Date: 7/12/2023 Input by: W C

Job Name: ROSE 3-3 STD

Project #

11.825 **AJS 140** F3-E





Member Infoi	mation			Unfa	ctored Rea	actions	UNP	ATTERNED <b>i</b> i	o (Upli	ft)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	L	ive	Dead		Snow	Wind
Plies:	1	Design Method:	LSD	1	Vertical	į	68	215		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2015	2	Vertical	2	242	91		0	0
Deflection LL:	360		OBC 2012(2020 Update)								
Deflection TL:	240	Load Sharing:	No								
Importance:	Normal - II	Deck:	Not Checked								
General Load		Vibration:	Not Checked								
Floor Live:	40 PSF			Bear	ings and F	actored	Rea	ctions			
Dead:	15 PSF			Bea	ring Length	Dir.	Сар.	React D/L Ib	Total	Ld. Case	Ld. Comb.
				1 - 5	SPF 2.375"	Vert	66%	268 / 852	1120	L	1.25D+1.5L
				2 - 5	SPF 5.250"	Vert	25%	114 / 363	477	L	1.25D+1.5L

### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1870 ft-lb	5'9 3/8"	5305 ft-lb	0.352 (35%)	1.25D+1.5L	L
Unbraced	1870 ft-lb	5'9 3/8"	5305 ft-lb	0.352 (35%)	1.25D+1.5L	L
Shear	1100 lb	1 5/8"	2350 lb	0.468 (47%)	1.25D+1.5L	L
Perm Defl in	0.046 (L/3628)	6'8 3/16"	0.461 (L/360)	0.099 (10%)	D	Uniform
LL Defl inch	0.122 (L/1366)	6'8 1/4"	0.461 (L/360)	0.264 (26%)	L	L
TL Defl inch	0.167 (L/992)	6'8 3/16"	0.692 (L/240)	0.242 (24%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 Bottom flange must be laterally braced at a maximum of 12'10 1/4" o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

<b>I</b> D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 14-1-8	0-3-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-7-2	1-7-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-5-14		Near Face	126 lb	331 lb	0 <b>l</b> b	0 <b>l</b> b	F1
4	Tie-In	1-7-2 to 14-1-8	0-5-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	14-1-8 to 14-4-2	0-3-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Tie-In	14-1-8 to 14-4-2	0-4-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

# Kott Inc.





**AJS 140** 

F4

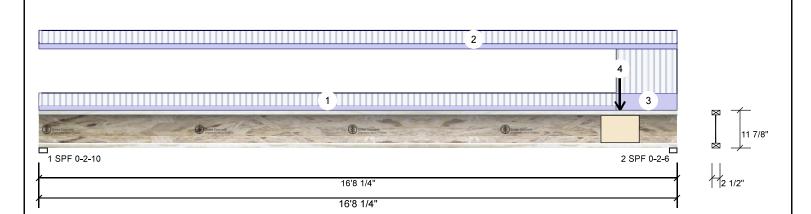
OF PERMIT PLANS roject: Nov 22 2023 ES PAHESBOUMD OFFICIA 11.875

Date: 7/12/2023 Input by: WC

Job Name: ROSE 3-3 STD

Project #

Level: Ground Floor



Member Inforn	nation			Unfactored Reactions UNPATTERNED lb (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Li	ve	Dead		Snow	Wind
Plies:	1	Design Method:	LSD	1	Vertical	3	41	128		0	0
Moisture Condition:	: Dry	Building Code:	NBCC 2015	2	Vertical	7	26	272		0	0
Deflection LL:	360		OBC 2012(2020 Update)								
Deflection TL:	240	Load Sharing:	No								
Importance:	Normal - II	Deck:	Not Checked								
General Load		Vibration:	Not Checked								
Floor Live:	40 PSF			Bea	rings and F	actored	Rea	ctions			
Dead:	15 PSF			Be	aring Length	Dir.	Cap.	React D/L Ib	Total	Ld. Case	Ld. Comb.
				1 -	SPF 2.625"	Vert	39%	160 / 511	671	L	1.25D+1.5L
						Vert	85%	341 / 1088	1429	L	1.25D+1.5L

### Analysis Results

Ana <b>l</b> ysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3015 ft-lb	9'3 9/16"	5305 ft-lb	0.568 (57%)	1.25D+1.5L	L
Unbraced	3015 ft-lb	9'3 9/16"	5305 ft-lb	0.568 (57%)	1.25D+1.5L	L
Shear	1407 <b>l</b> b	16'6 5/8"	2350 lb	0.599 (60%)	1.25D+1.5L	L
Perm Defl in	0.100 (L/1968)	8'7 3/4"	0.546 (L/360)	0.183 (18%)	D	Uniform
LL Defl inch	0.266 (L/738)	8'7 3/4"	0.546 (L/360)	0.488 (49%)	L	L
TL Defl inch	0.366 (L/537)	8'7 3/4"	0.820 (L/240)	0.447 (45%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 4 Bottom flange must be laterally braced at a maximum of 15'2 3/8" o.c.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

		, ,								
	<b>I</b> D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 15-1-2	0-5-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	2	Tie-In	0-0-0 to 16-8-4	0-5-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	3	Tie-In	15-1-2 to 16-8-4	1-6-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
ı	4	Point	15-2-6		Far Face	144 lb	383 lb	0 <b>l</b> b	0 lb	F1
ı										

### Notes

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Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

## Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

### Kott Inc.





OF PERMIT PLANS roject: Nov 22 2023 ES

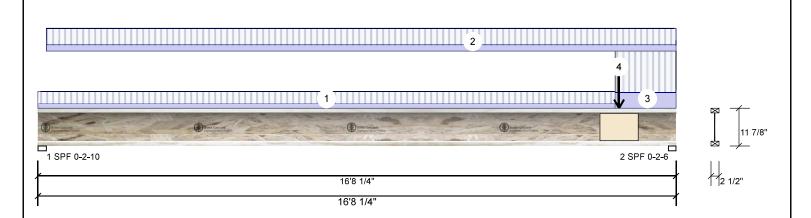
Date: 7/12/2023 Input by: WC

Job Name: ROSE 3-3 STD

Project #

11.875" **AJS 140** 

# Level: Ground Floor



Member Infor	mation			Unfactored Reactions UNPATTERNED lb (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	Live	Dead	Snow	Wind	
Plies:	1	Design Method:	LSD	1	Vertical	390	146	0	0	
Moisture Condition	n: Dry	Building Code:	NBCC 2015	2	Vertical	774	291	0	0	
Deflection LL:	360		OBC 2012(2020 Update)							
Deflection TL:	240	Load Sharing:	No							
Importance:	Normal - II	Deck:	Not Checked							
General Load		Vibration:	Not Checked							
Floor Live:	40 PSF			Bear	rings and Fa	actored Rea	ctions			
Dead:	15 PSF			Bea	aring Length	Dir. Cap.	React D/L lb	Total Ld. Case	Ld. Comb.	
				1 - 3	SPF 2.625"	Vert 44%	183 / 586	769 L	1.25D+1.5L	
A l						Vert 91%	364 / 1161	1525 L	1.25D+1.5L	

### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3442 ft-lb	9'1 11/16"	5305 ft-lb	0.649 (65%)	1.25D+1.5L	L
Unbraced	3442 ft-lb	9'1 11/16"	5305 ft-lb	0.649 (65%)	1.25D+1.5L	L
Shear	1502 lb	16'6 5/8"	2350 lb	0.639 (64%)	1.25D+1.5L	L
Perm Defl in.	0.114 (L/1727)	8'7 1/4"	0.546 (L/360)	0.208 (21%)	D	Uniform
LL Defl inch	0.304 (L/648)	8'7 3/16"	0.546 (L/360)	0.556 (56%)	L	L
TL Defl inch	0.417 (L/471)	8'7 3/16"	0.820 (L/240)	0.509 (51%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 4 Bottom flange must be laterally braced at a maximum of 15'2 3/8" o.c.

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•	JULY 13, 2023	

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 15-1-2	0-5-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-2-10 to 16-8-4	0-7-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Tie-In	15-1-2 to 16-8-4	1-6-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
4	Point	15-2-6		Near Face	142 <b>l</b> b	377 <b>l</b> b	0 lb	0 lb	F1

### Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

# Kott Inc.





OF PERMIT PLANS roject: Nov 22 2023 isDesign

Date: 7/12/2023 Input by: W C

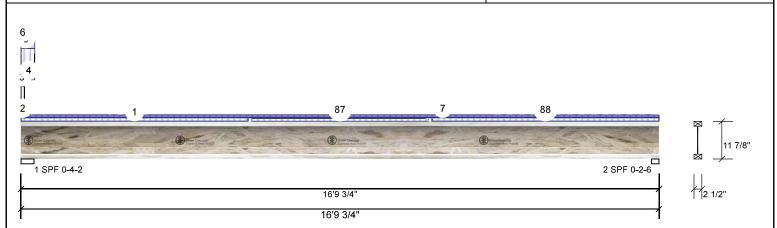
Job Name: ROSE 3-3 STD

Project #

**AJS 140** F4-B

11.8<mark>75</mark>"

Level: Ground Floor



ES

<b>Member Infor</b>	mation			Unfactored Reactions UNPATTERNED Ib (Uplift)							
Туре:	Girder	Application:	Floor (Residential)	Brg	Direction	L	ive	Dead	Snow	Wind	
Plies:	1	Design Method:	LSD	1	Vertical	:	577	263	0	0	
Moisture Condition	n: Dry	Building Code:	NBCC 2015	2	Vertical		455	171	0	0	
Deflection LL:	360		OBC 2012(2020 Update)								
Deflection TL:	240	Load Sharing:	No								
Importance:	Normal - II	Deck:	Not Checked								
General Load		Vibration:	Not Checked								
Floor Live:	40 PSF			Bea	rings and F	actored	Read	ctions			
Dead:	15 PSF			Bea	aring Length	Dir.	Сар.	React D/L Ib	Total Ld. Cas	E Ld. Comb.	
				1 -	SPF 4.125"	Vert	62%	329 / 866	1195 L	1.25D+1.5L	
				2 -	SPF 2.375"	Vert	53%	213 / 683	896 L	1.25D+1.5L	

### Analysis Results

Ana <b>l</b> ysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3574 ft-lb	8'5 3/4"	5305 ft-lb	0.674 (67%)	1.25D+1.5L	L
Unbraced	3574 ft-lb	8'5 3/4"	5305 ft-lb	0.674 (67%)	1.25D+1.5L	L
Shear	933 lb	3 3/8"	2350 lb	0.397 (40%)	1.25D+1.5L	L
Perm Defl in	0.116 (L/1695)	8'5 3/4"	0.546 (L/360)	0.212 (21%)	D	Uniform
LL Defl inch	0.309 (L/636)	8'5 3/4"	0.546 (L/360)	0.566 (57%)	L	L
TL Defl inch	0.425 (L/463)	8'5 3/4"	0.820 (L/240)	0.519 (52%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

4 Bottom flange must be laterally braced at bearings.



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

4 Bottom flange must be laterally braced at bearings.											
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments		
1	Tie-In	0-0-0 to 5-11-13	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF			
2	Part. Uniform	0-0-0 to 0-1-2		Тор	118 PLF	257 PLF	0 PLF	0 PLF	J7		
3	Part. Uniform	0-0-0 to 0-1-2		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight		
4	Part. Uniform	0-0-0 to 0-4-6		Тор	118 PLF	257 PLF	0 PLF	0 PLF	J7		
5	Part. Uniform	0-0-0 to 0-4-6		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight		
6	Part. Uniform	0-0-9 to 0-0-9		Тор	4 PLF	0 PLF	0 PLF	0 PLF	Rim Board Self Weight		
7	Tie-In	0-1-8 to 16-9-12	0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF			
Continued on page 2											

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Jioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-rjly fastening details and handling/erection details

  3. Damaged Jioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

# Manufacturer Info

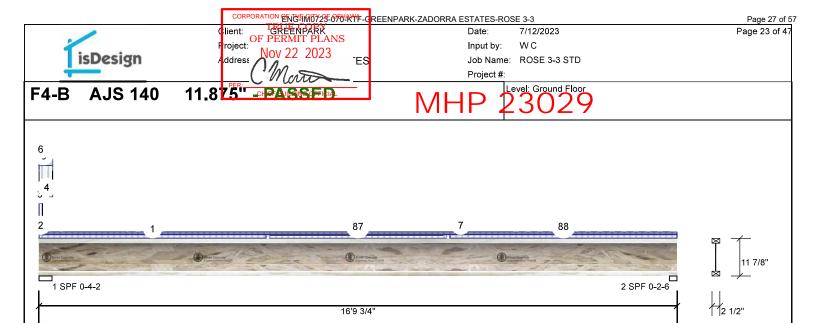
Boise Cascade Wood Products 1111 W. Jefferson St. Boise, ID 83702

(800) 232-0788 www.bc.com CCMC: 12787

### Kott Inc.







..Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind C	Comments
87	Tie-In	6-0-15 to 10-8-12	0-8-11	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
88	Tie-In	10-9-14 to 16-9-12	0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

16'9 3/4'



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTES: EWP-FLOORS. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Notice Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 IJoist not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. Noist flanges must not be out or drilled

  2. Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-pty fastening details and handling/erection details

  3. Damaged Lioists must not be used

  4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- 5. Provide lateral support at bearing points to avoid lateral displacement and rotation 6. Web stiffeners for point load as shown Minimum point load bearing length>= 3,5 inches 7. For flat roofs provide proper drainage to prevent ponding

This design is valid until 4/17/2026

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