Modulus Engineering Ltd. O3 2023 Document: ME-TC02 V 03-2017



Document Outlines the Objectives, Restrictions and Limitations of EWP Component Seals

MHP 23019

Sealed Engineered Wood Product (EWP) Components:

(Includes, but not limited to: I-Joists, LVL, LSL, Dimensional Lumber)

Modulus Engineering Ltd. (MEL) provides EWP component review among a variety of other engineering services to our clients. The scope of the work provided by EWP component review is governed by the arrangement between MEL and our client and not intended to extend or imply to extend beyond this scope. If further review or other engineering work beyond this scope is required, MEL may be retained by the client at the discretion of MEL.

It is becoming more common that component review is requested by project engineers or building designers as a means of additional verification of proprietary EWP components they have specified on their drawings or by building departments for verification of components not covered explicitly in the building code. The intent of this document is to clarify the objectives of the review of the EWP components to ensure the project engineer or building designer and the building official understand the limitations of the component seals. Of particular importance, it should not be implied that the EWP component review and subsequent component seals provided are to be considered as review of the overall structure. This remains the responsibility of the project engineer of record (EOR) or building designer (Architect or other).

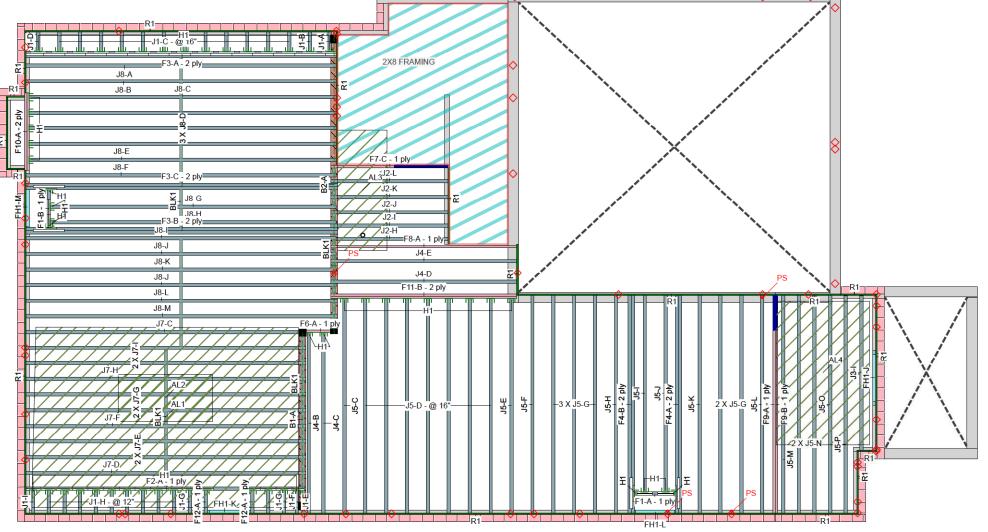
MEL reviews components produced by various proprietary design software programs, including: Mitek Sapphire (or Sapphire Supply), CSD iStruct, Simpson Strong-Tie Component Solutions, EZCad Wood-I and WoodWorks Sizer. Each program produces design notes that similarly attempt to clarify the limitations of the individual component design, with slight differences in language. The <u>General Notes</u> provided here are intended to supplement the notes on the sealed components and replace them where contradictions exist. If the intent is unclear, please contact MEL directly to ensure there are no unanswered questions.

General Notes:

- Although MEL reviews components submitted together with layouts provided by our clients (in order to review how the components frame to one another to check member to member loading in the structural model and to review the suitability of hangers noted as well as to determine lamination details provided on the component drawing), the individual component seal stands alone as a unique individual member design/review and the loading noted should also be verified by the building designer or EOR.
- Reactions shown are for gravity loads (vertical) or out of plane wind loads (horizontal, when reviewing wall components) on the member as noted only and does not include any additional loads that may be imposed by other factors such as overturning of shearwalls due to wind or seismic loads.
- Minimum bearing length noted is based on the EWP material itself unless the bearing capacity for the support is specifically noted (most often the case), for the full width of the member supported. If the capacity of the bearing material is less than the capacity of the member itself, a larger bearing length may be required, to be determined by building designer or EOR.
- Adequate bearing surface for large point loads from above must be provided, to be determined by building designer or EOR unless noted on the component design.
- Where bracing or lateral support assumptions are not noted on the component design, continuous bracing for any compression edge, point load location and bearing or support location is assumed.
- Design is based on Dry service condition, defined as an EMC average over the year of 15% or less and never over 19%.
- Dimensions and location of supports as provided and as noted on component design, to be verified by others.
- Lamination details provided on the component drawing have been designed as per CSA-086.
- Any hangers specified have been reviewed in accordance with manufacturers published capacity for gravity and uplift loads only.
- Any flat roof applications must have adequate drainage to avoid ponding and potential overloading of the structure as designed
- Building designer or EOR must ensure the structure is adequate to support the reactions shown (may include uplift at a bearing)
- Building designer or EOR is responsible for the overall structural design including the lateral stability of the structure.
- Products to be installed as per manufacturers instructions and/or as per details provided by the building designer or EOR
- Products should be stored on site and handled as per manufacturers recommendations.
- Damaged products or those modified outside the scope of the manufacturers recommendations should not be installed unless approved by an engineer or building official



MHP 23019



Label	L (Flus		Wid	lth	De	oth	(Qty	Plies		Pcs	Length
F9	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375					2	14-0-0
F11	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375		1	2		2	12-0-0
F8	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375					1	12-0-0
F7	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375					1	8-0-0
F10	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375		1	2		2	6-0-0
F6	Forex 2.0E-3	000Fb LVL	1.	75	11.8	375					1	4-0-0
I Joist (Flush)											
Label	Descr	iption	Wid	lth	De	pth	(Qty	Plies		Pcs	Length
J8	AJS 14		2	2.5	11.8	375					16	20-0-0
J7	AJS 14	10	2	2.5	11.8	375					10	18-0-0
J5	AJS 14	10	2	2.5	11.8	375					25	14-0-0
J4	AJS 14	10	2	2.5	11.8	375					4	12-0-0
J3	AJS 14		2	2.5	11.8						1	10-0-0
J2	AJS 14			2.5	11.8	_					5	8-0-0
J1	AJS 14		2	2.5	11.8	_					29	2-0-0
F3	AJS 14			2.5	11.8			3	2		6	20-0-0
F2	AJS 14			2.5	11.8	$\overline{}$					1	18-0-0
F4	AJS 14			2.5	11.8	$\overline{}$		2	2		4	14-0-0
F1	AJS 14			2.5	11.8	_					2	4-0-0
F12	AJS 14	10	2	2.5	11.8	375					2	2-0-0
Rim Bo												
Label	Descr	•	Wid		De		(Qty	Plies		Pcs	Length
R1	Norbor Plus 1. 11.875		1.1	25	11.8	375					14	12-0-0
Blockin	g											
Label	Descr	iption	Wid	lth	De	pth	(Qty	Plies		Pcs	Length
BLK1	AJS 14	10	2	2.5	11.8	375	L	inFt			Varies	36-0-0
Hanger								Bea	am/Gird	ler		ported ember
Label	Pcs	Descriptio	n	SI	kew	Slo	pe	fa	steners	5	fas	teners
H1	55	LF2511					-		12 10d	-	4 440	1 1/4WS

	JOB INFORMATION	
ngth	Builder	
1-0-0	GREENPARK	
+-0-0	Project	
2-0-0	ZADORRA ESTATES	
	Shipping	
2-0-0	PENROSE 2 EL 3	
3-0-0	OSHAWA,ON	
	Sales Rep	
6-0-0	RALPH MIRIGELLO	
1-0-0	- Designer	
+-0-0	wc	
	Plotted	
ngth	June 07, 2022	
0-0-0	Layout Name	
3-0-0	PENROSE 2- EL 3	
1-0-0	Job Path	
2-0-0 0-0-0	DESIGN CRITERIA	
3-0-0	Ground Floor	
2-0-0	Design Method LSD (Cana	da)
0-0-0	Building Code NBCC 2015 / OBC 20	,
3-0-0	Floor	
1-0-0	Loads	
1-0-0	Live	40
2-0-0	Dead	15
41-	Deflection Joist	
ngth 2-0-0	LL Span L/	180
2-0-0	TL Span L/	240
	Deflection Flush Girder	
	LL Span L/	180
ngth	1 oba	240
6-0-0	Deflection Dropped Girder	
	LE Opan E	180
ted	. z opa z	240
er ers	Deflection Header	
4WS		180
-110	- TE Spail E	240
	Decking Control of the Control of th	SB
		3/4"
	11110111000)/ 4
	CCMC References	

CCMC References

Boise - 12472-R , 12787-R LP - 12412-R Forex - 14056-R

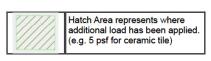
Kott Inc.

3228 Moodie Dr, Ottawa

613-838-2775 / 905-642-4400

14 Anderson Blvd, Uxbridge KOTT

- 1. All blocking to be cut from 12' joists
- 2. 2' & 4' Lengths to be cut from 8' Length, 6' lengths to be cut from 12' Length
- 3. Ends of joists to be laterally supported
- 4. Packing of Steel beams and attachment by others
- 5. Shower and water closet flange locations are approximate only, consult architectural drawing for exact locations
- 6. Beams identified as "B" are dropped and supplied by others
- 7. Install 2x4 blocking @ 24" o/c under parallel non-loadbearing walls
- 8. Load transfer blocks to be installed under all point loads
- 9. Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements
- 10. Hangers and Fasteners to be installed as per manufacturer
- 11. Framing shown on this layout may deviate from architectural drawings. Arch / Eng to review and approve the deviation prior to construction.
- 12. Multi ply beams with side loading to have all fasteners installed with the head



AJS140 I-Joists can be substituted with LP20 I-Joists for 9.5" and 11.875" depths shown on this layout.

on the side of the applied load

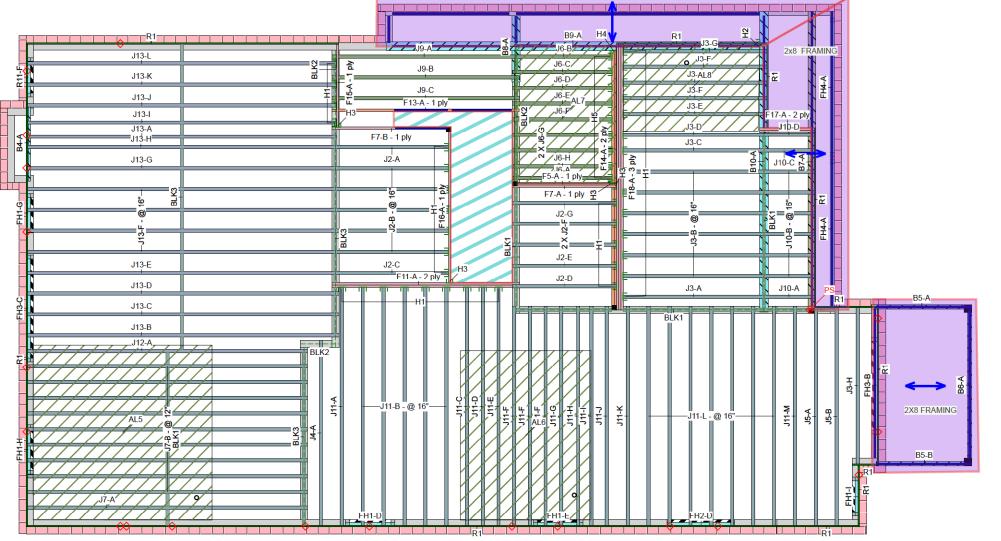
Legend

Load from Above Wall Wall Opening Norbord Rimboard Plus 1.125 X 11.875 AJS 140 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875 1.75 X 9.5 (Dropped) 5.25 X 8 (Dropped)

Point Load Support



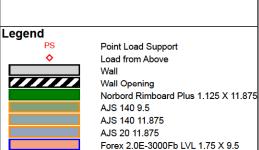
MHP 23019



Second											JOB INFORMATION
LVL/LS								D	-		Builder
	Descr	iption	Wid	-	Depth	100	Qty	Plies	Pcs	Length	GREENPARK
F14		000Fb LVL	1.7	100-01	9.	-2	1	2	2	18-0-0	Project
F13	Forex 2.0E-3	000Fb LVL	1.7	75	9.	5			1	12-0-0	ZADORRA ESTATES
F5	Forex 2.0E-3	000Fb LVL	1.7	75	9.	5			1	8-0-0	Shipping PENROSE 2 EL 3 OSHAWA,ON
F18		000Fb LVL	1.7		11.87		1	3	3	18-0-0	Sales Rep
F11	Forex 2.0E-3	000Fb LVL	1.7	75	11.87	5	1	2	2	12-0-0	RALPH MIRIGELLO Designer
F16	Forex 2.0E-3	000Fb LVL	1.7	75	11.87	5			1	10-0-0	wc
F7	Forex 2.0E-3	000Fb LVL	1.7	75	11.87	5			2	8-0-0	Plotted June 07, 2022
F15	Forex 2.0E-3	000Fb LVL	1.7	75	11.87	5			1	6-0-0	Layout Name
F17	Forex 2.0E-3	000Fb LVL	1.7	75	11.87	5	1	2	2	4-0-0	PENROSE 2- EL 3 Job Path
I Joist (Flush)										DESIGN CRITERIA
	Descr		Wid	th	Depth	1	Qty	Plies	Pcs	Length	
J9	AJS 14	•	2	.5	9.				3	12-0-0	Second Floor
J6	AJS 14			.5	9.				9	8-0-0	Design Method
J7	AJS 14	10	2	.5	11.87	5			10	18-0-0	Building Code NBCC
J11	AJS 14	10	2	.5	11.87	5			25	16-0-0	
J5	AJS 14		2		11.87	_			2	14-0-0	Floor
J4	AJS 14	10	2	.5	11.87	5			1	12-0-0	Loads
J3	AJS 14	10			11.87	_			15	10-0-0	Live
J2	AJS 14		2		11.87	_			12	8-0-0	Dead
J10	AJS 14	10	2	.5	11.87	5			9	4-0-0	Deflection Joist
J13	AJS 20)	2	.5	11.87	5			15	20-0-0	LL Span L/
J12	AJS 20)	2	.5	11.87	5			1	18-0-0	TL Span L/
Rim Bo	ard							1			Deflection Flush Girder
Label	Descr	iption	Wid	th	Depth	n (Qty	Plies	Pcs	Length	LL Span L/
R1		d Rimboard	1.12		11.87	_			13	12-0-0	TL Span L/
	11.875										Deflection Dropped Girder
Blockin	g										LL Span L/
Label	Descr	iption	Wid	th	Depth	1	Qty	Plies	Pcs	Length	TL Span L/
BLK2	AJS 14	10	2	.5	9.	5 L	_inFt		Varies	7-0-0	Deflection Header
BLK3	AJS 14	10	2	.5	11.87	5 L	inFt		Varies	9-0-0	LL Span L/
BLK1	AJS 14	10	2	.5	11.87	5 L	inFt		Varies	42-0-0	TL Span L/
BLK3	AJS 20)	2	.5	11.87	5 L	inFt		Varies	28-0-0	Decking
BLK2	AJS 20)	2	.5	11.87	5 L	inFt		Varies	2-0-0	Decking
Hanger	•										Thickness
							Bea	am/Girde		ported ember	CCMC References
Label	Pcs	Descriptio	n	Ske	w S	Slope	fa	steners	fas	teners	Boise - 12472-R , 12787-R
H1	39	LF2511				•		10dx1 1/2		1 1/4WS	LP - 12412-R Forex - 14056-R
H2	1	Hanger by Others									Kott Inc.
Н3	4	HUS1.81/1	0		+			30 16d	10	0 16d	3228 Moodie Dr. Ottawa
H4	1	LF359	-					10 10d		1 1/4WS	14 Anderson Blvd, Uxbridge
H5	9	LF259						10 10d		1 1/4WS	Outside 1
											Ontario

1	. All	blocking	to be	cut from	12' joists

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- 12. Multi ply beams with side loading to have all fasteners installed with the head on the side of the applied load



613-838-2775 / 905-642-4400

LSD (Canada)

15

480

240

480

240

480

240

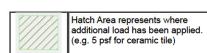
480

240

OSB

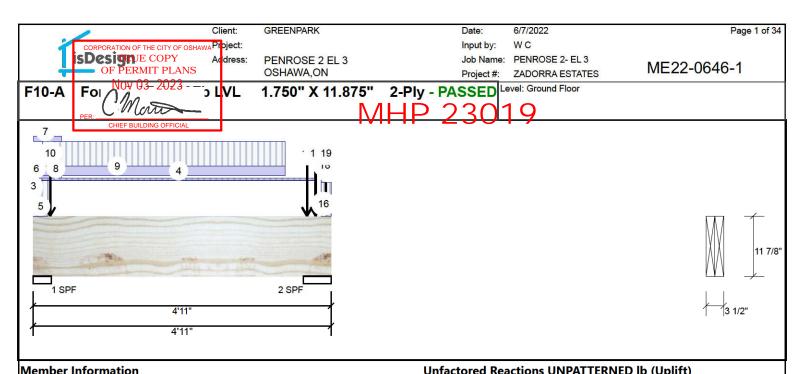
5/8"

NBCC 2015 / OBC 2012



AJS140 I-Joists can be substituted with LP20 I-Joists for 9.5" and 11.875" depths shown on this layout.

This placement plan is to be used as an installation guide only. It is meant to be used in conjunction with the manufacturers installation guide, the architectural and structural drawings, and not to replace them.



Application: Type: Floor (Residential) Plies 2 Design Method: Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 Vertical Deflection LL: 480 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal - II Vibration: Not Checked General Load 40 PSF Floor Live: 15 PSF Dead: **Analysis Results** Location Allowed Canacity

Oili	omactored Reactions ONFAFTERINED ID (Opint)									
Brg	Direction	Live	Dead	Snow	Wind					
1	Vertical	1864	1207	597	0					

1257

Bearings and Factored Reactions Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.625" 1509 / 3394 4902 L Vert 63% 1.25D+1.5L +S 1.25D+1.5L 2 - SPF 5.502" Vert 44% 1571 / 3596 5167 L +S

Analysis	Actual	Location	Allowed	Сараспу	Comb.	Case
Moment	1867 ft-lb	2'4 3/8"	34261 ft-lb	0.054 (5%)	1.25D+1.5L	L
Unbraced	1867 ft-lb	2'4 3/8"	34261 ft-lb	0.054 (5%)	1.25D+1.5L	L
Shear	1677 lb	3'5 5/8"	11596 lb	0.145 (14%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/22447)	2'4 7/16"	0.143 (L/360)	0.016 (2%)	D	Uniform
LL Defl inch	0.006 (L/8957)	2'4 7/16"	0.107 (L/480)	0.054 (5%)	L+0.5S	L
TL Defl inch	0.008 (L/6402)	2'4 7/16"	0.214 (L/240)	0.037 (4%)	D+L+0.5S	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 3.625.
- 2 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 3 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Multiple plies must be fastened together as per manufacturer's details.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be continuously laterally braced.
- 8 Bottom must have sheathing attached or be continuously braced.
- 9 Lateral slenderness ratio based on full section width.

LAMINATE WITH:

2143

2 ROWS OF 3.25" AIR NAILS FACE SPACING AT 12 IN O/C. NAIL FROM LOADED FACE MIN HANGER NAILS: (JOIST/BEAM) 3 IN

MODULUS ENGINEERING LTD.

382

0



DOC: ME-TC02 V 03-2017 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

landling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

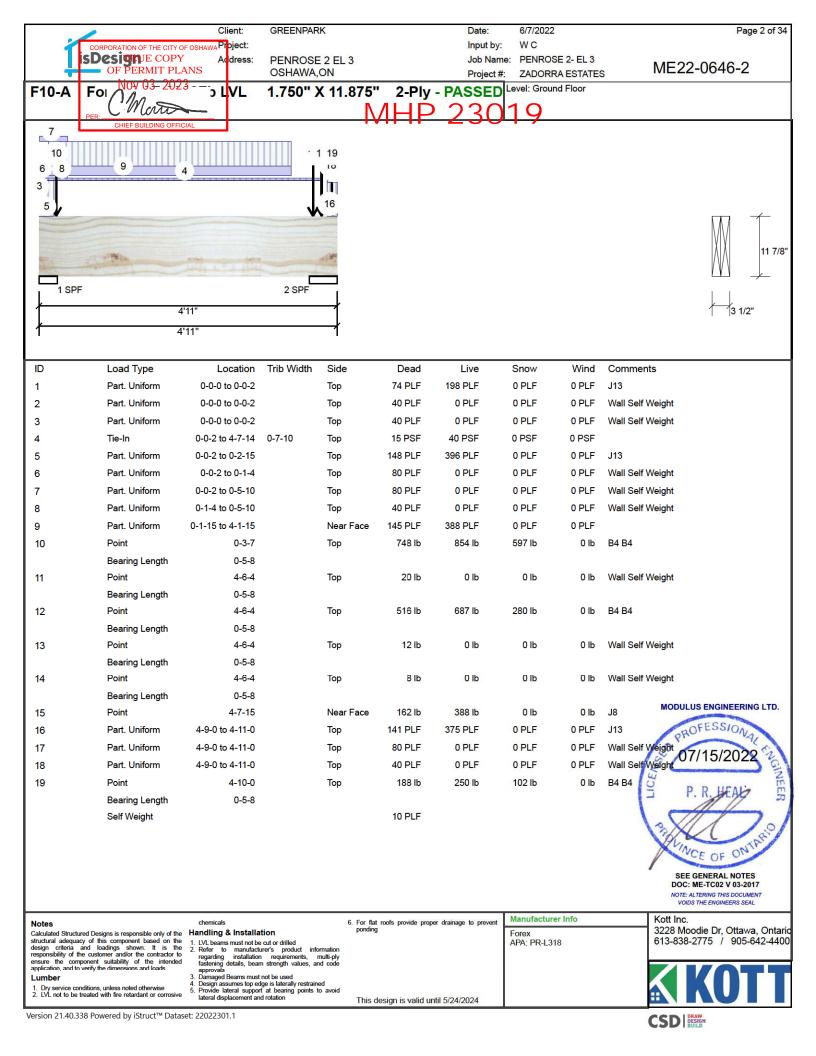
For flat roofs provide proper drainage to prevent ponding

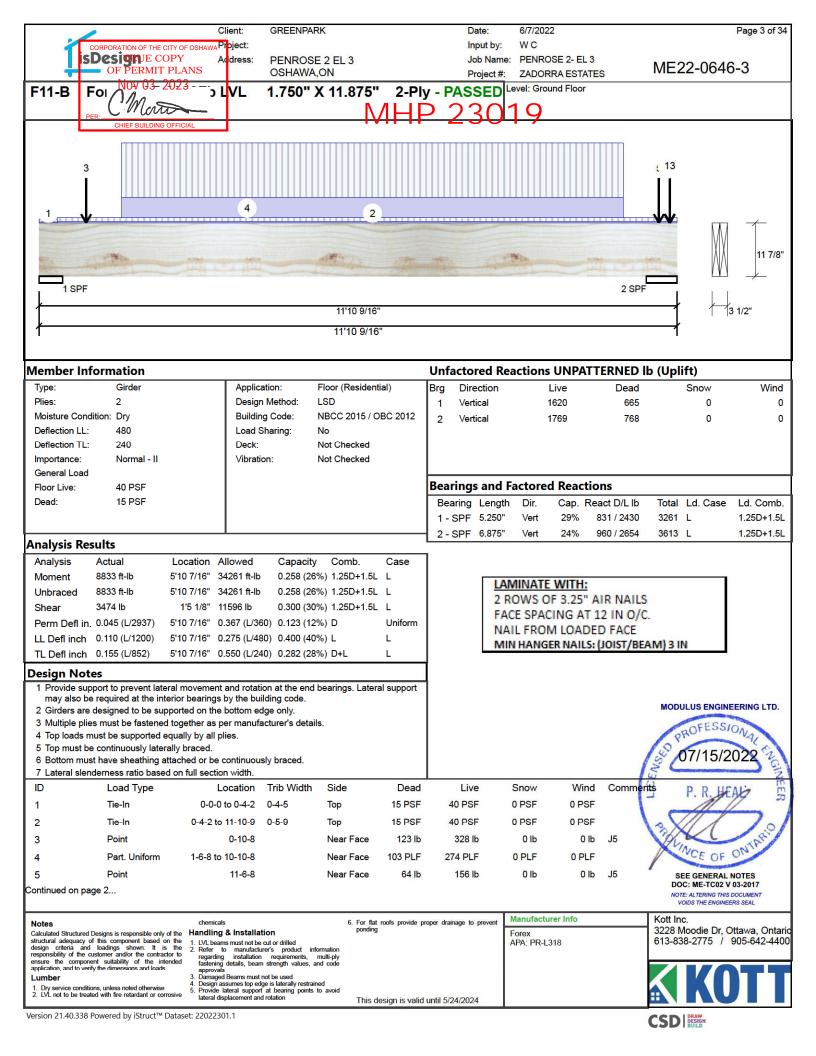
Manufacturer Info Forex APA: PR-L318

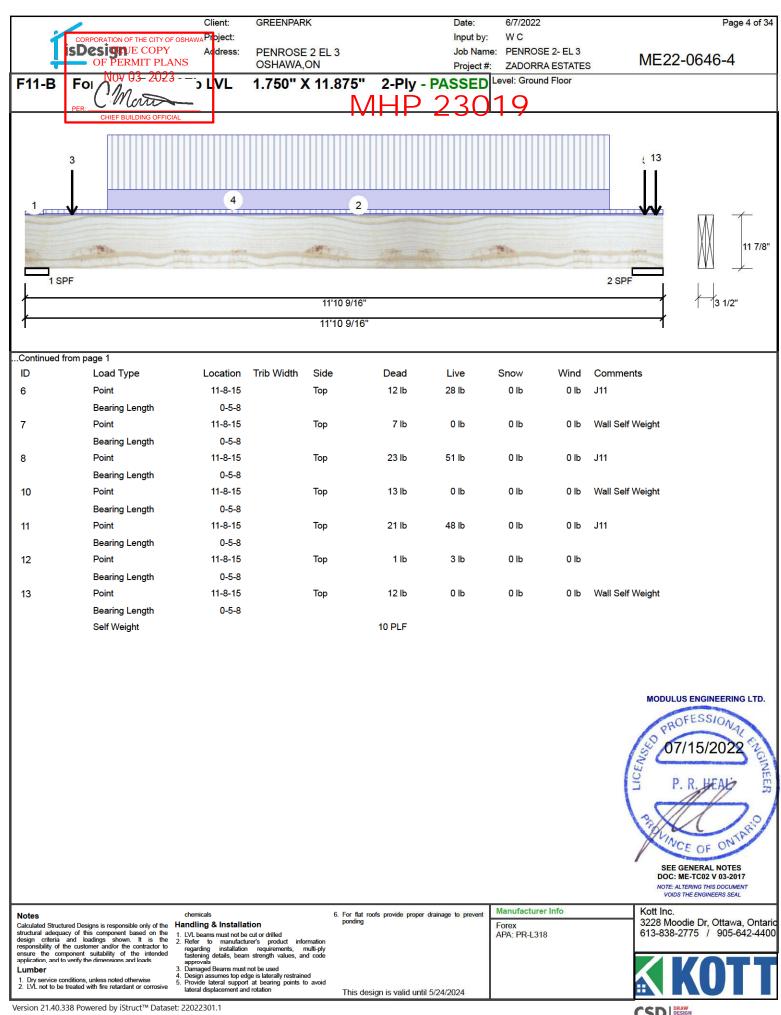
3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400

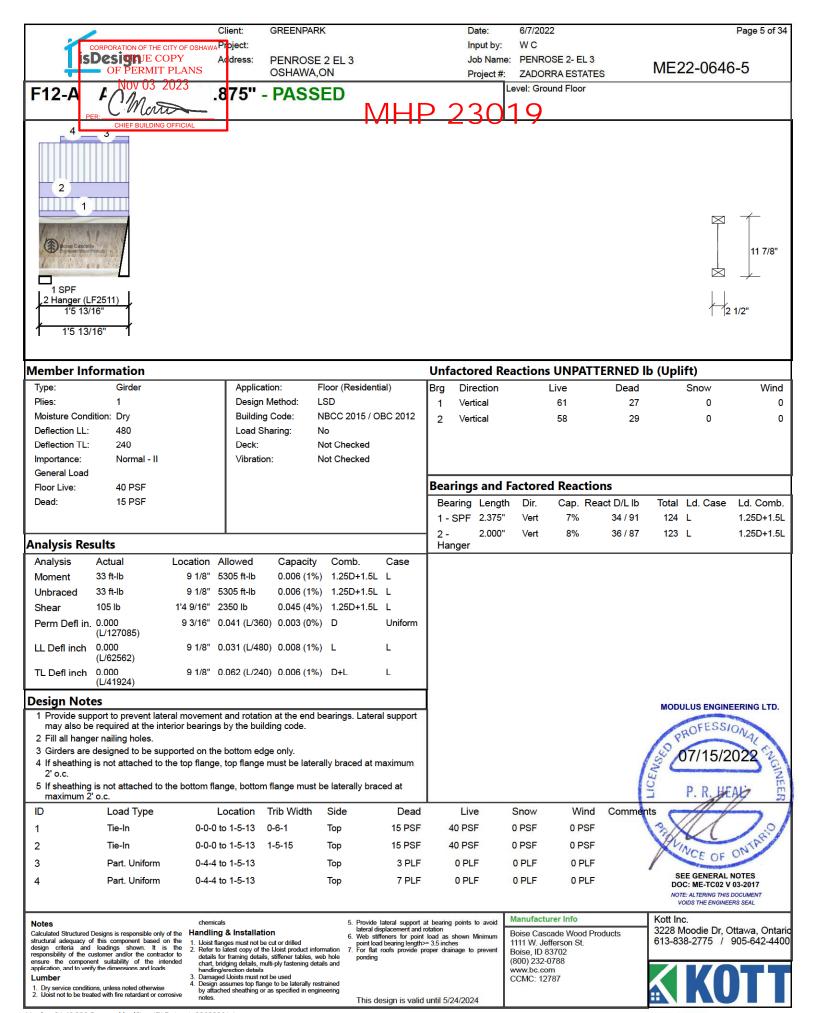


This design is valid until 5/24/2024









Client: **GREENPARK** Date: 6/7/2022 Page 6 of 34 CORPORATION OF THE CITY OF OSHAWA Project: Input by: WC SDesignue COPY

PENROSE 2 EL 3 OSHAWA, ON

Job Name: PENROSE 2- EL 3 Project #: ZADORRA ESTATES

Level: Ground Floor

ME22-0646-6

875" - PASSED F1-A

OF PERMIT PLANS

Address:

1

Hanger

Hanger

2 -

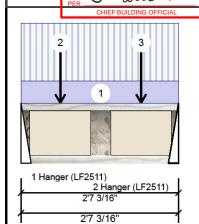
Vertical Vertical

2.000"

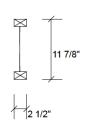
2.000"

Vert

Vert



15 PSF



0

0

586 L

596 L

Wind

1.25D+1.5L

1.25D+1.5L

0

0

Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift) Brg Direction Live Dead Snow

298

303

36%

37%

Bearings and Fa	actore	ed Reactions		
Bearing Length	Dir	Can React D/L lb	Total Ld Case	Ld Comb

139 / 447

142 / 454

111

113

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	314 ft-lb	1'1 5/8"	5305 ft-lb	0.059 (6%)	1.25D+1.5L	L
Unbraced	314 ft-lb	1'1 5/8"	5305 ft-lb	0.059 (6%)	1.25D+1.5L	L
Shear	590 lb	2'5 15/16"	2350 lb	0.251 (25%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/26402)	1'3"	0.080 (L/360)	0.014 (1%)	D	Uniform
LL Defl inch	0.003 (L/9877)	1'3"	0.060 (L/480)	0.049 (5%)	L	L
TL Defl inch	0.004 (L/7188)	1'3"	0.120 (L/240)	0.033 (3%)	D+L	L

Design Notes

	Alalysis	Actual	Location	Allowed	Capacity	COITID.	Casc
	Moment	314 ft-lb	1'1 5/8"	5305 ft-lb	0.059 (6%)	1.25D+1.5L	L
	Unbraced	314 ft-lb	1'1 5/8"	5305 ft-lb	0.059 (6%)	1.25D+1.5L	L
	Shear	590 lb	2'5 15/16"	2350 lb	0.251 (25%)	1.25D+1.5L	L
	Perm Defl in.	0.001 (L/26402)	1'3"	0.080 (L/360)	0.014 (1%)	D	Uniform
	LL Defl inch	0.003 (L/9877)	1'3"	0.060 (L/480)	0.049 (5%)	L	L
	TL Defl inch	0.004 (L/7188)	1'3"	0.120 (L/240)	0.033 (3%)	D+L	L
-							

MODULUS ENGINEERING LTD.

P. R.

VINCE OF

2 Fill all hanger nailing holes. 3 Girders are designed to be supported on the bottom edge only.

may also be required at the interior bearings by the building code.

4 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support

5 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 2-7-3	0-7-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-7-12		Far Face	100 lb	268 lb	0 lb	0 lb	J5 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
3	Point	1-11-12		Far Face	99 lb	264 lb	0 lb	0 lb	J5

SEE GENERAL NOTES DOC: ME-TC02 V 03-2017 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

ON

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoint not to be treated with fire retardant or corrosive

Handling & Installation

- and ling & installation
 Loist flanges must not be cut or drilled
 Refer to latest copy of the Lloist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handlingferection details
 Damaged Lloists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 Tor flat roofs provide proper drainage to prevent poorling.

This design is valid until 5/24/2024

Manufacturer Info

Boise Cascade Wood Products 1111 W. Jefferson St.

Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.





Client: **GREENPARK** Date: 6/7/2022 Page 7 of 34 CORPORATION OF THE CITY OF OSHAWA Project: Input by: WC

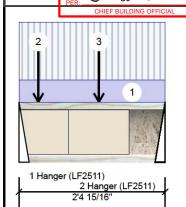
SDesignue COPY Address: OF PERMIT PLANS

PENROSE 2 EL 3 OSHAWA, ON

Job Name: PENROSE 2- EL 3 Project #:

ME22-0646-7 ZADORRA ESTATES

Level: Ground Floor 875" -**PASSED**



2'4 15/16

15 PSF

 \times 11 7/8"

Member Information

F1-B

Type:	Girder	Application:	Floor (Residential)
Plies:	1	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind
1	Vertical	471	177	0	0
2	Vertical	291	109	0	0

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	533 ft-lb	1'3 13/16"	5305 ft-lb	0.100 (10%)	1.25D+1.5L	L
Unbraced	533 ft-lb	1'3 13/16"	5305 ft-lb	0.100 (10%)	1.25D+1.5L	L
Shear	921 lb	1 1/4"	2350 lb	0.392 (39%)	1.25D+1.5L	L
Perm Defl in.	0.002 (L/15356)	1'3 13/16"	0.073 (L/360)	0.023 (2%)	D	Uniform
LL Defl inch	0.005 (L/5769)	1'3 13/16"	0.055 (L/480)	0.083 (8%)	L	L
TL Defl inch	0.006 (L/4194)	1'3 13/16"	0.110 (L/240)	0.057 (6%)	D+L	L

Bearings and Factored Reactions

-								
	Bearing	Length	Dir.	Cap. Re	eact D/L lb	Total	Ld. Case	Ld. Comb.
	•	2.000"	Vert	58%	221 / 706	927	L	1.25D+1.5L
	Hanger							
	2 -	2.000"	Vert	36%	137 / 437	574	L	1.25D+1.5L
	Hanger							

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fill all hanger nailing holes.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 5 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.

ID Load Type Location Trib Width Side Dead Live Wind Comments Snow 0-0-0 to 2-4-15 0-9-0 15 PSF 40 PSF 0 PSF 0 PSF Tie-In Top 1 Point 0-3-13 Near Face 103 lb 275 lb 0 lb 0 lb J8 2 3 **Point** 1-3-13 Near Face 156 lb 415 lb 0 lb 0 lb J8

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P. R. VINCE OF ON

> SEE GENERAL NOTES DOC: ME-TC02 V 03-2017 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoint not to be treated with fire retardant or corrosive

Handling & Installation

- and ling & installation
 Loist flanges must not be cut or drilled
 Refer to latest copy of the Lloist product information
 details for framing details, stiffener tables, web hole
 chart, bridging details, multi-ply fastening details and
 handlingferection details
 Damaged Lloists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 Tor flat roofs provide proper drainage to prevent poorling.

This design is valid until 5/24/2024

Manufacturer Info

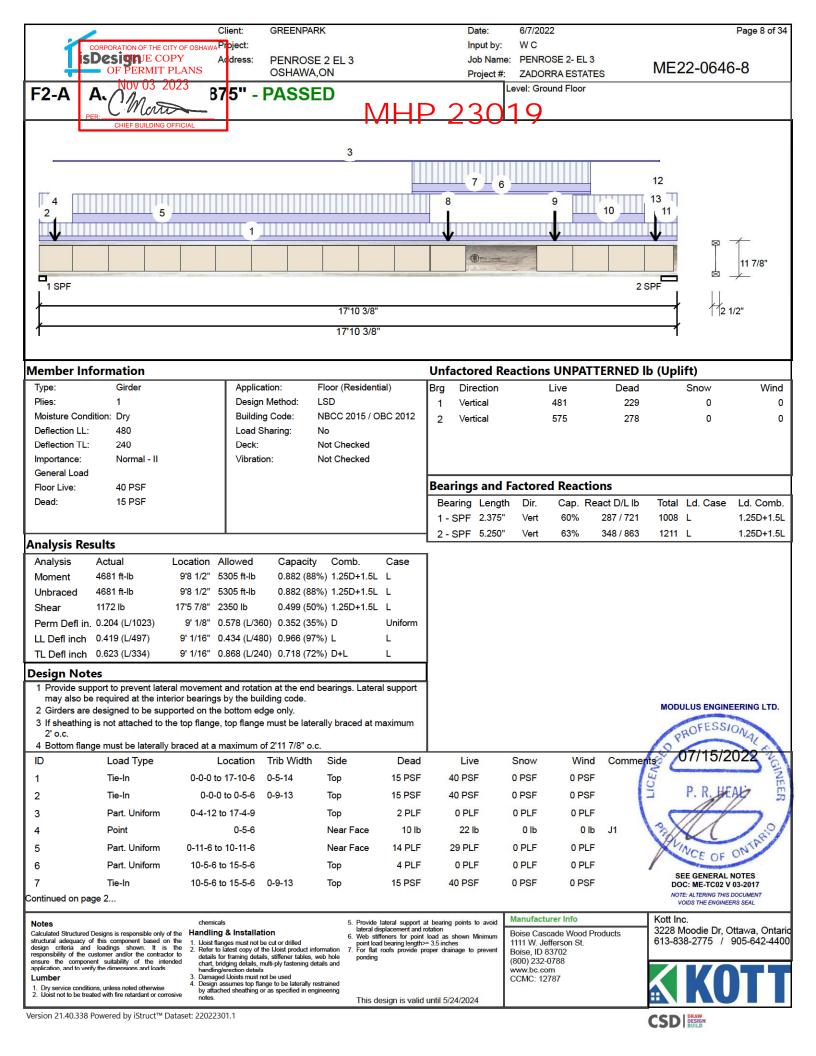
Boise Cascade Wood Products 1111 W. Jefferson St.

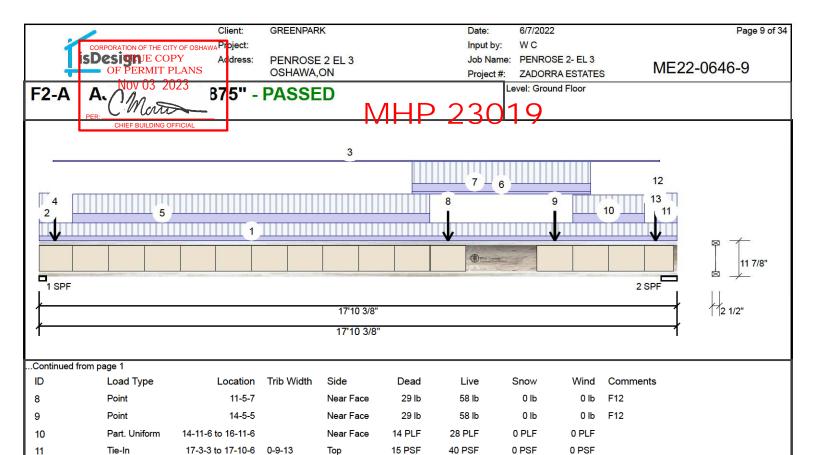
Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.









4 PLF

11 lb

0 PLF

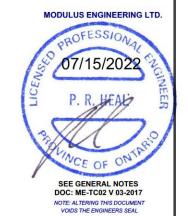
23 lb

0 PLF

0 lb

0 PLF

0 lb J1



Notes

12

13

Part. Uniform

Point

17-3-3 to 17-4-9

17-3-3

Тор

Near Face

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upist not to be treated with fire retardant or corrosive

Handling & Installation

- Lorist flanges must not be cut or drilled

 Refer to latest copy of the Loist product information details for farming details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handlinglerection details

 Damaged Lloists must not be used

 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length="3.5" inches
 For flat roofs provide proper drainage to prevent

This design is valid until 5/24/2024

Manufacturer Info

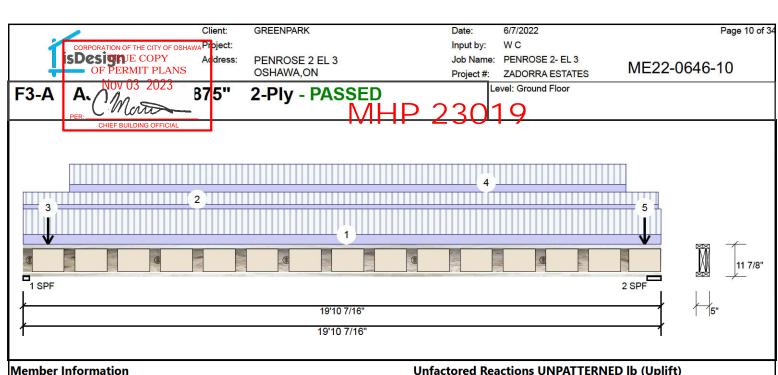
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(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.







Unfactored Reactions UNPATTERNED lb (Uplift) Type: Application: Floor (Residential) Brg Direction Live Dead Snow Wind Plies 2 Design Method: LSD 659 247 Vertical 0 0 Moisture Condition: Dry **Building Code:** NBCC 2015 / OBC 2012 Vertical 678 254 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal - II Vibration: Not Checked General Load Bearings and Factored Reactions 40 PSF Floor Live: 15 PSF Dead: Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 39% 309 / 989 1298 L 1.25D+1.5L 1 - SPF 2.375" Vert 2 - SPF 5.250" Vert 35% 318 / 1017 1335 L 1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6269 ft-lb	9'9 3/4"	10610 ft-lb	0.591 (59%)	1.25D+1.5L	L
Unbraced	6269 ft-lb	9'9 3/4"	10610 ft-lb	0.591 (59%)	1.25D+1.5L	L
Shear	1305 lb	19'5 15/16"	4700 lb	0.278 (28%)	1.25D+1.5L	L
Perm Defl in.	0.138 (L/1681)	9'9 13/16"	0.645 (L/360)	0.214 (21%)	D	Uniform
LL Defl inch	0.368 (L/631)	9'9 13/16"	0.484 (L/480)	0.761 (76%)	L	L
TL Defl inch	0.507 (L/459)	9'9 13/16"	0.968 (L/240)	0.523 (52%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.
- 6 If sheathing is not attached to the bottom flange, bottom flange must be laterally braced at maximum 2' o.c.

ı	111674111161111 2 6									
	ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
	1	Tie-In	0-0-0 to 19-10-7	0-9-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	2	Tie-In	0-0-0 to 19-9-5	0-4-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	13
	3	Point	0-9-6		Far Face	10 lb	26 lb	0 lb	0 lb	J1
	4	Part. Uniform	1-5-6 to 18-9-6		Far Face	9 PLF	24 PLF	0 PLF	0 PLF	
	5	Point	19-4-5		Far Face	8 lb	21 lb	0 lb	0 lb	J1

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Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Upoint not to be treated with fire retardant or corrosive

Handling & Installation

- Libit flanges must not be cut or drilled Refer to latest copy of the Lloist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastening details and handling-ferection details Damaged Lloists must not be used Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 Tor flat roofs provide proper drainage to prevent poorling.

This design is valid until 5/24/2024

Manufacturer Info

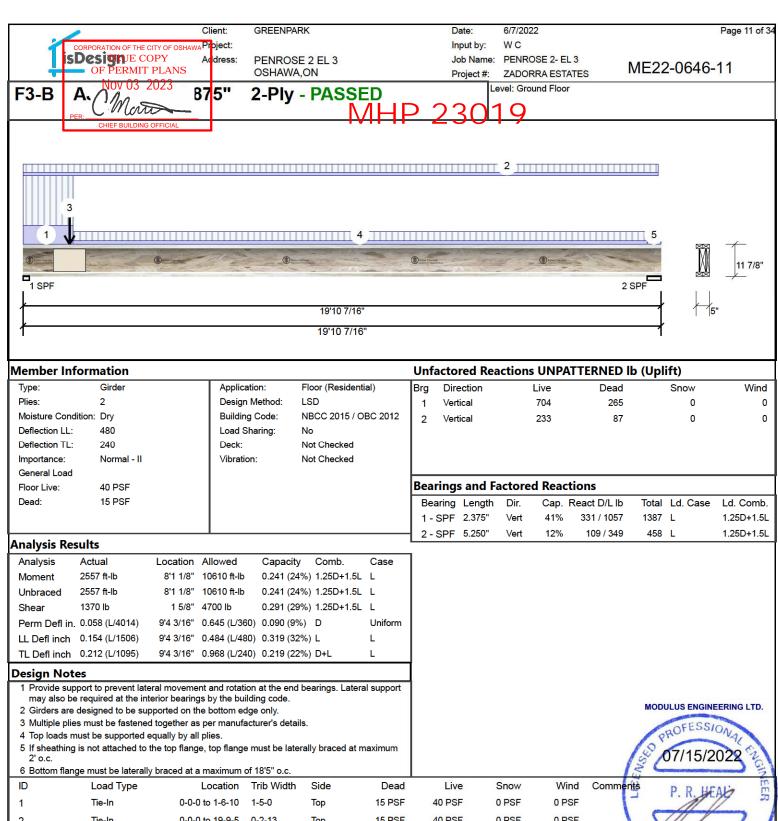
Boise Cascade Wood Products 1111 W. Jefferson St.

Boise, ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.







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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comme	P. R. HEAD EE
1	Tie-In	0-0-0 to 1-6-10	1-5-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		2
2	Tie-In	0-0-0 to 19-9-5	0-2-13	Тор	15 PSF	40 PSF	0 PSF	0 PSF	1	\a\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
3	Point	1-5-6		Far Face	177 lb	471 lb	0 lb	0 lb	F1	The Court of the C
4	Tie-In	1-6-10 to 19-5-3	0-3-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF		NCE OF ON
5	Tie-In	19-5-3 to 19-10-7	0-2-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF		SEE GENERAL NOTES DOC: ME-TC02 V 03-2017
										NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

Notes

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Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 5/24/2024

Manufacturer Info

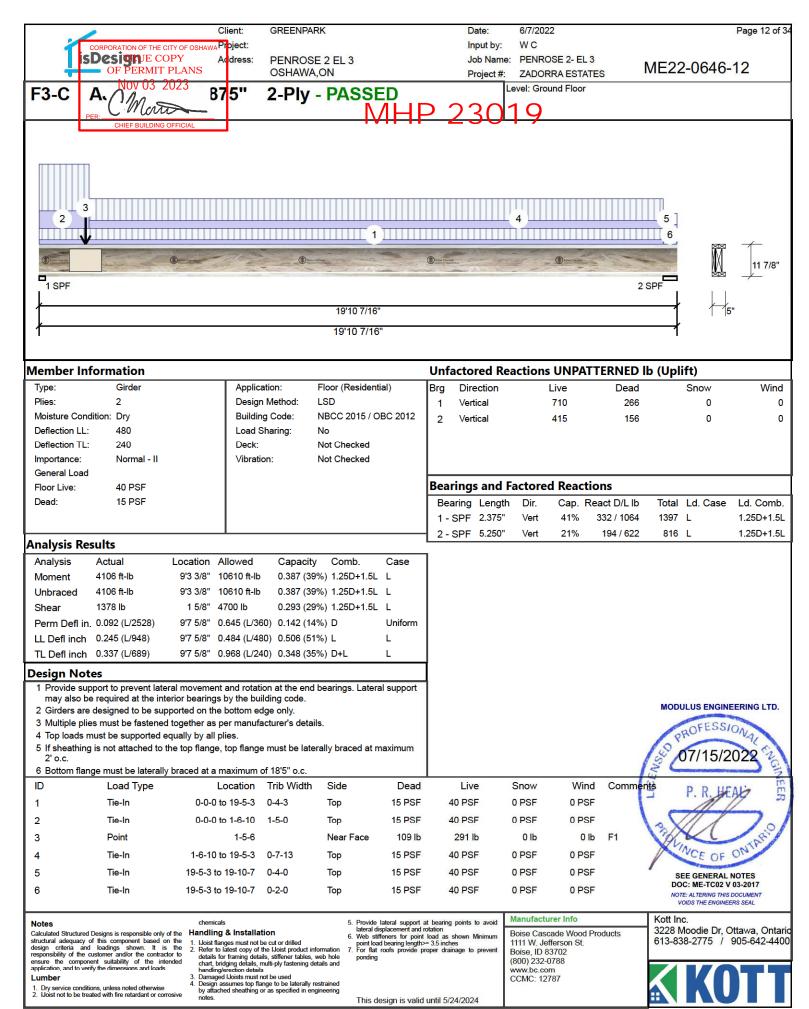
Boise Cascade Wood Products 1111 W. Jefferson St. Boise ID 83702

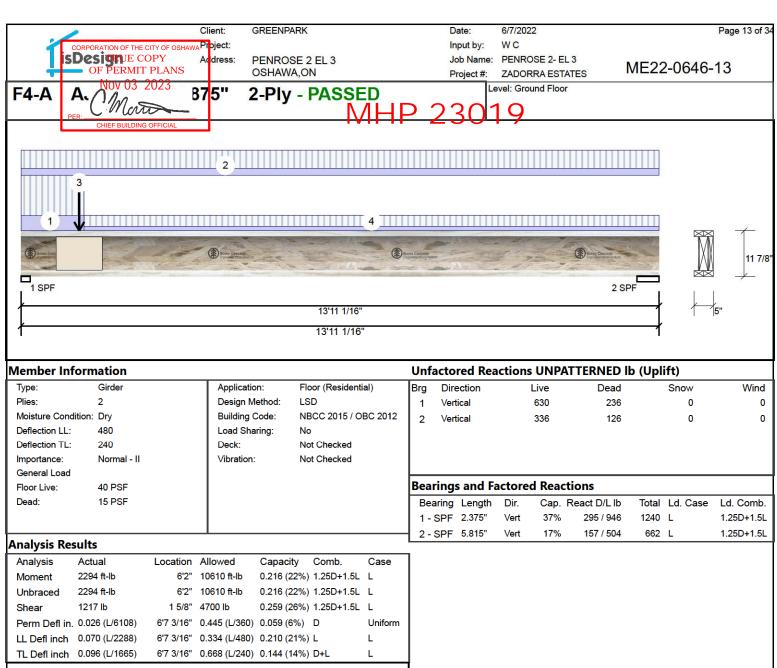
(800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.









Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 If sheathing is not attached to the top flange, top flange must be laterally braced at maximum 2' o.c.

6 Bottom flange must be laterally braced at a maximum of 12'7 13/16" o.c.

ID Location Trib Width Side Dead Live Snow Wind Comments Load Type 0-0-0 to 1-4-8 15 PSF 40 PSF 0 PSF 0 PSF 1 Tie-In 1-6-1 Top Tie-In 0-0-0 to 13-11-1 15 PSF 40 PSF 0 PSF 0 PSF 2 Top 3 Point 1-3-4 Far Face 113 lb 303 lb 0 lb 0 lb F1 Tie-In 1-4-8 to 13-11-1 0-4-15 Top 15 PSF 40 PSF 0 PSF 0 PSF



07/15/2022

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 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 5/24/2024

Manufacturer Info

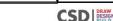
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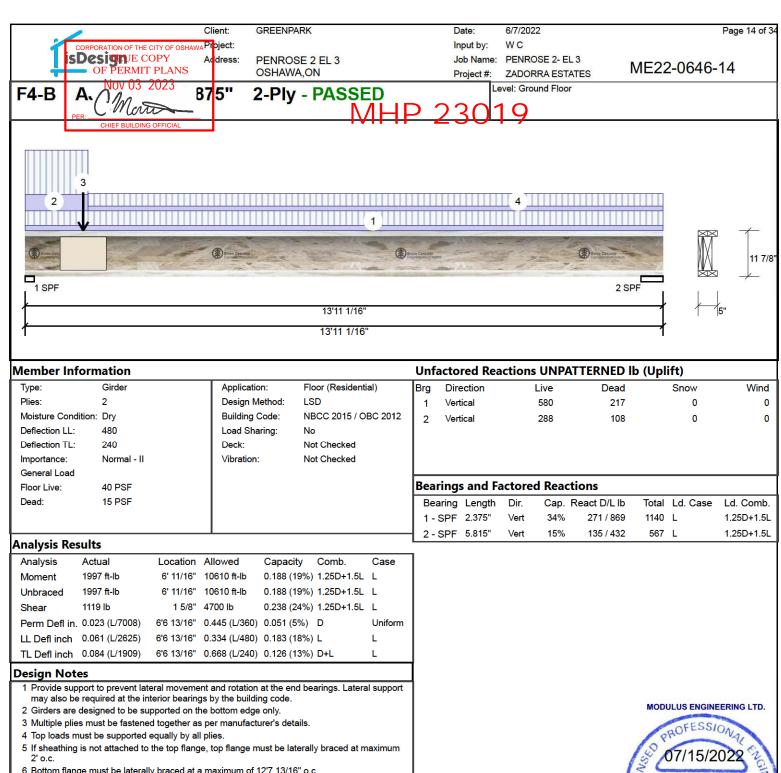
Boise ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

Kott Inc.

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6 Bottom flange must be laterally braced at a maximum of 12'7 13/16" o.c.

ID Trib Width Side Dead Live Snow Wind Comments Load Type Location 0-0-0 to 13-11-1 15 PSF 40 PSF 0 PSF 0 PSF 1 Tie-In 0-5-14 Top Tie-In 0-0-0 to 1-4-8 15 PSF 40 PSF 0 PSF 0 PSF 2 Top 3 Point 1-3-4 Near Face 111 lb 298 lb 0 lb 0 lb F1 Tie-In 1-4-8 to 13-11-1 0-5-2 Top 15 PSF 40 PSF 0 PSF 0 PSF

P. R.

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- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length=3.5 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 5/24/2024

Manufacturer Info

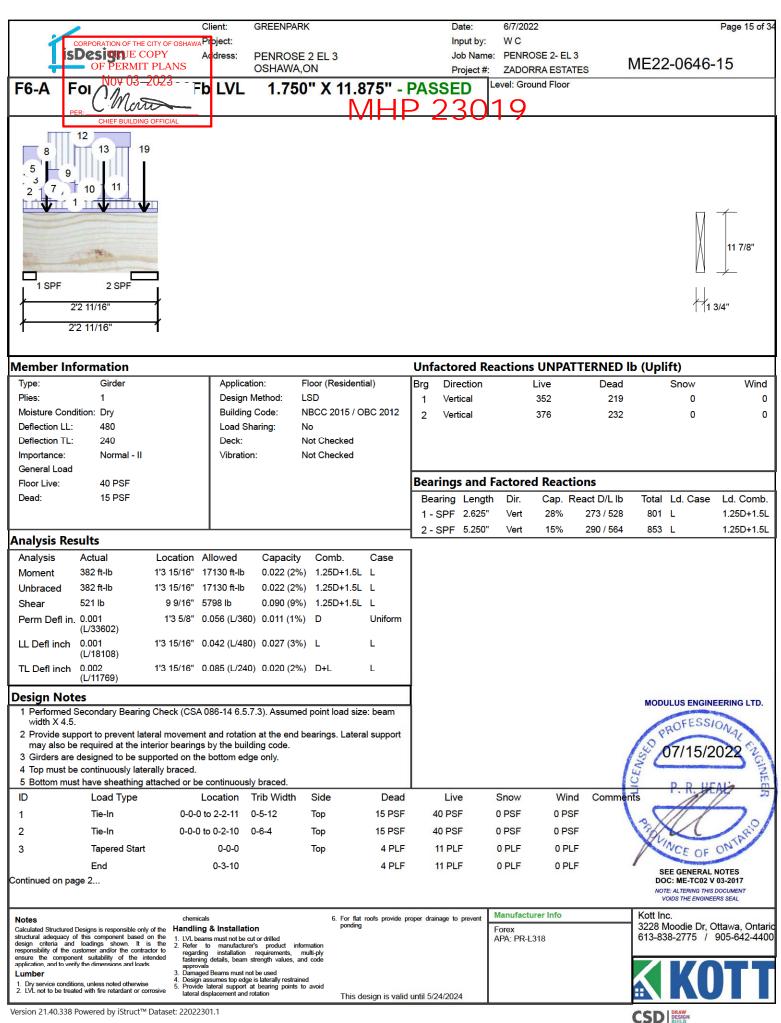
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Boise ID 83702 (800) 232-0788 www.bc.com CCMC: 12787

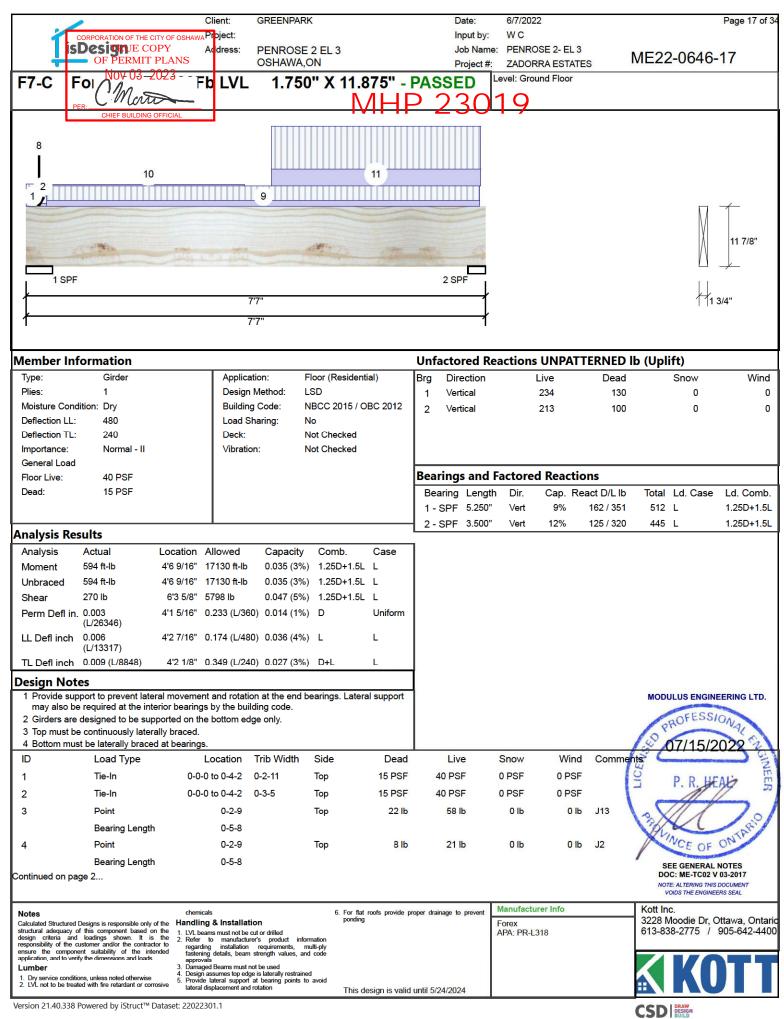
Kott Inc.

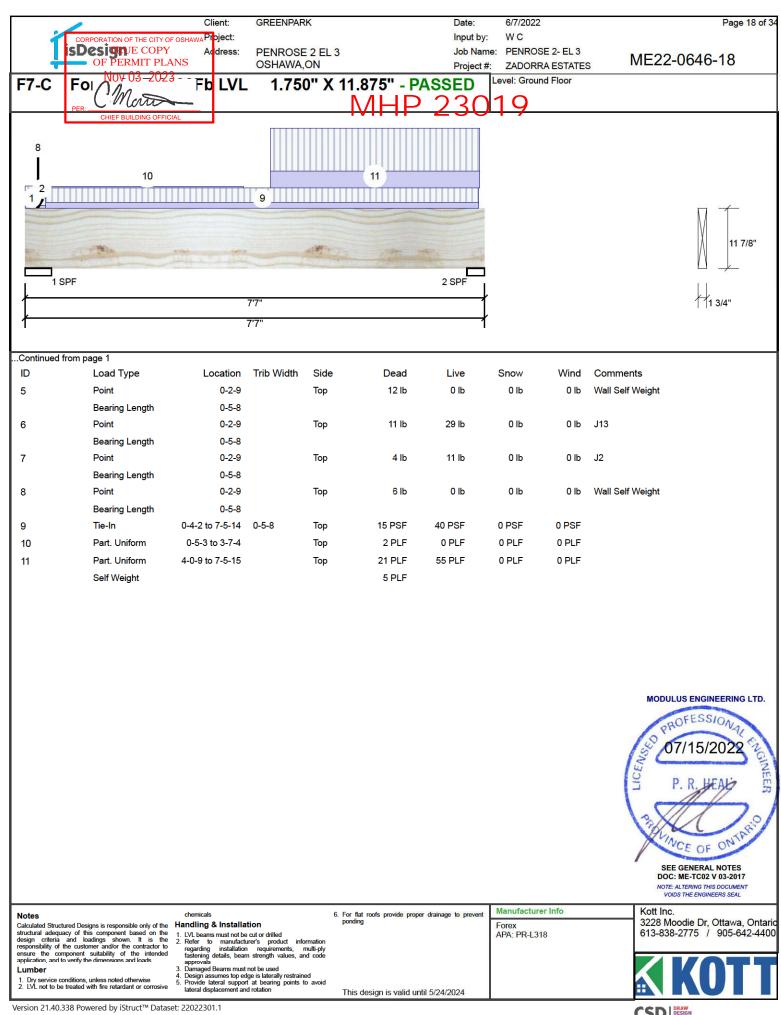


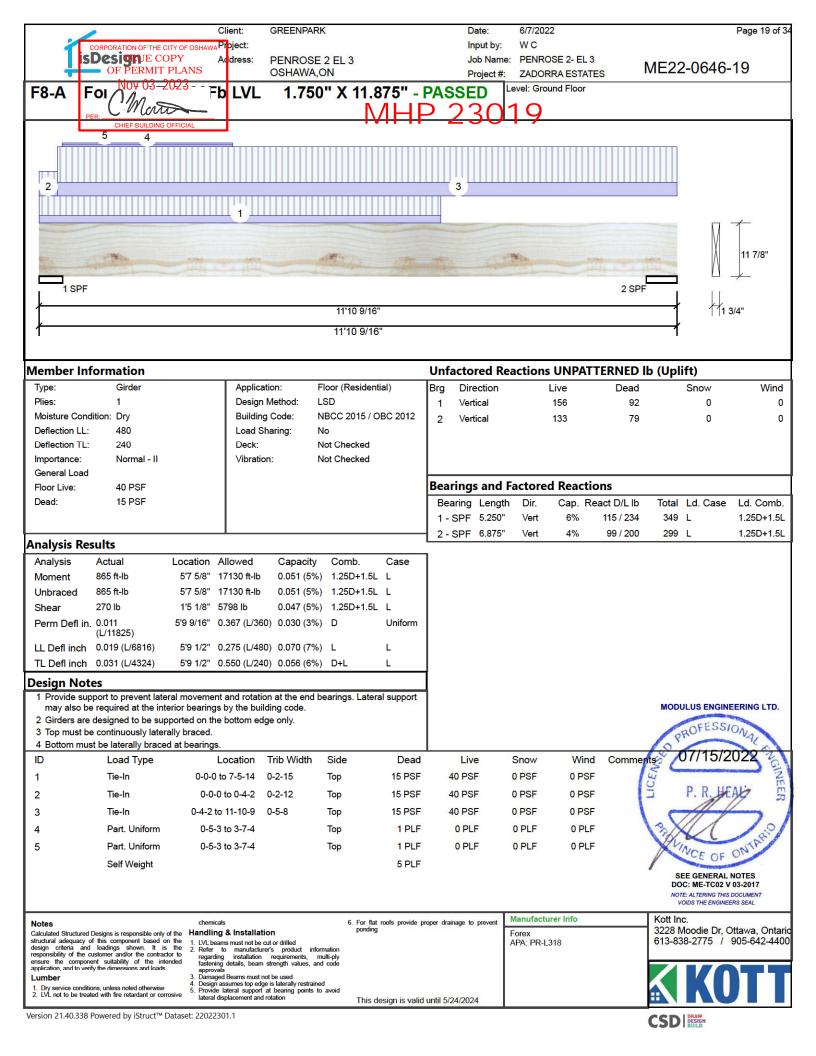


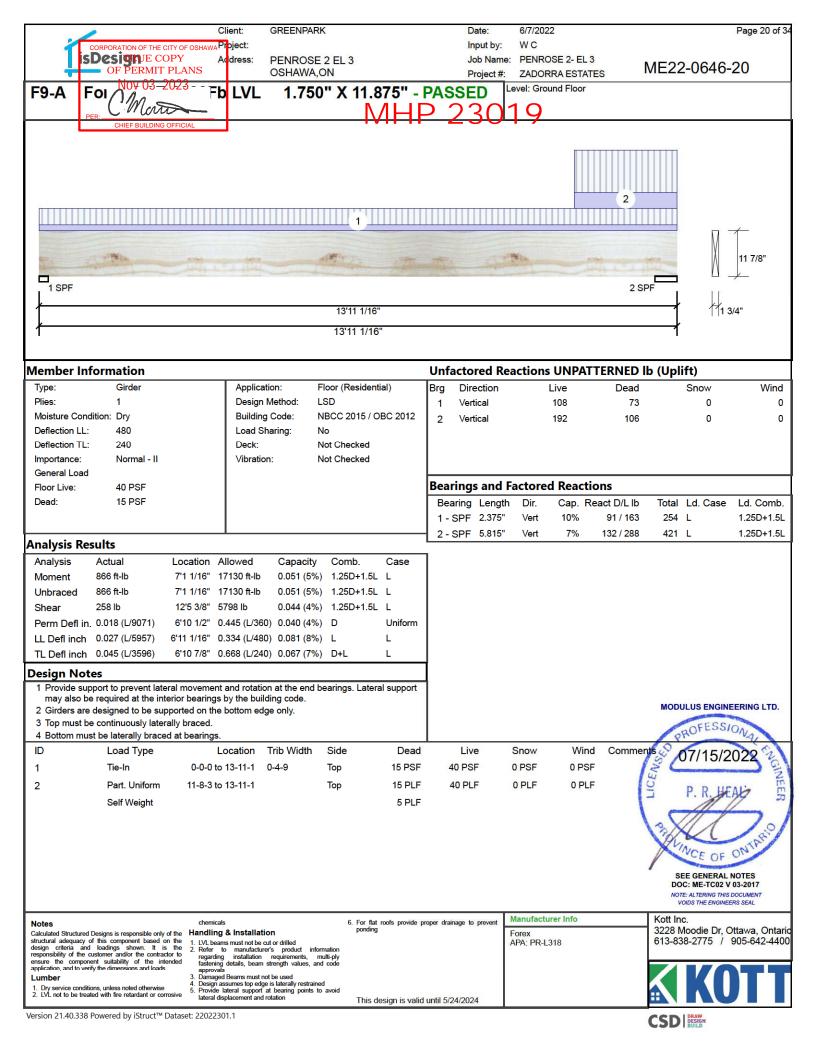


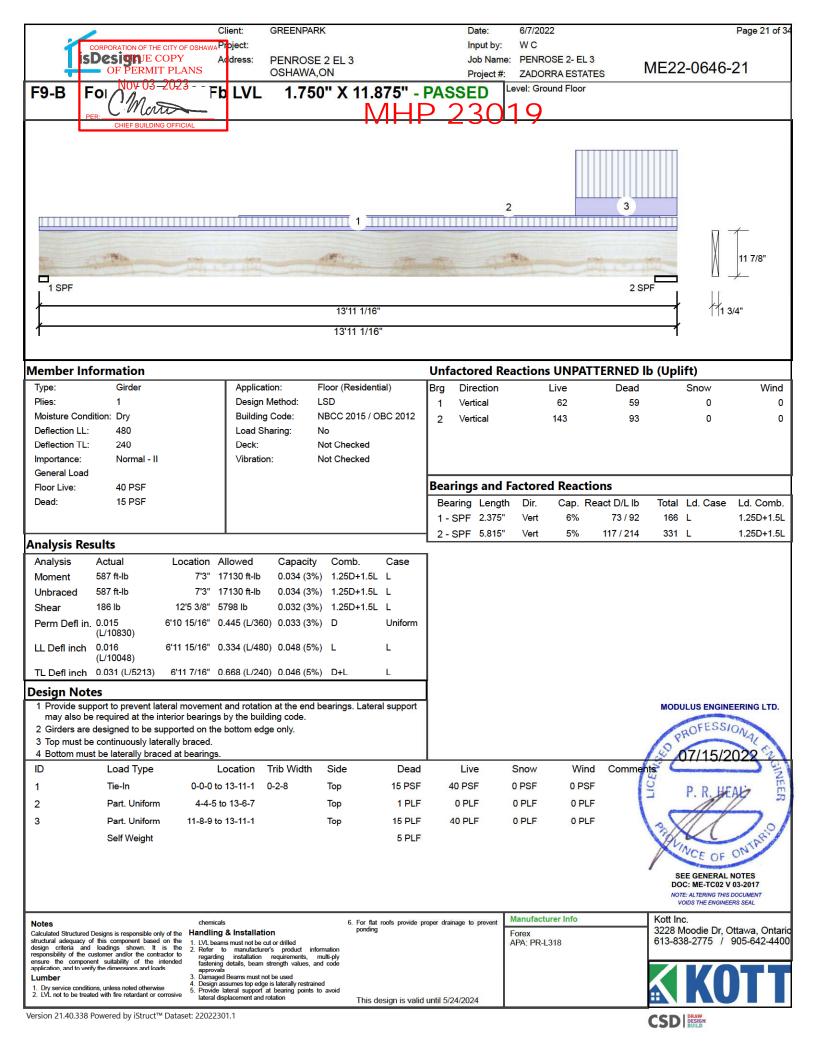
Client: **GREENPARK** Date: 6/7/2022 Page 16 of 34 Input by: WC CORPORATION OF THE CITY OF OSHAWA Project: SDesignue COPY PENROSE 2 EL 3 Job Name: PENROSE 2- EL 3 Address: ME22-0646-16 OF PERMIT PLANS OSHAWA, ON Project #: ZADORRA ESTATES Level: Ground Floor Foi Fb LVL 1.750" X 11.875" - PASSED F6-A 12 13 19 8 9 11 10 1 11 7/8' 1 SPF 2 SPF 2'2 11/16' 2'2 11/16 .Continued from page 1 Trib Width Side ID Load Type Location Dead Live Snow Wind Comments 0-0-0 3 PLF 9 PLF 0 PLF 0 PLF Tapered Start Top End 0-2-12 3 PLF 9 PLF 0 PLF 0 PLF 5 Part. Uniform 0-0-0 to 0-3-10 Top 80 PLF 0 PLF 0 PLF 0 PLF Wall Self Weight 2 PLF 6 Tapered Start 0-3-10 Top 5 PLF 0 PLF 0 PLF 2 PLF 5 PLF End 0 - 10 - 50 PLF 0 PLF 38 PLF Part. Uniform 0-3-10 to 0-10-5 0 PLF 0 PLF 0 PLF Wall Self Weight 7 Top **Point** 0-4-11 90 lb 178 lb 0 lb 0 lb J4 8 Near Face 34 PLF 92 PLF 0 PLF 0 PLF 9 Part. Uniform 0-4-15 to 0-10-5 Top J4 2 PLF 5 PLF 0 PLF 0 PLF Tapered Start 0-10-5 10 Top 2 PLF 5 PLF 0 PLF 0 PLF End 1-9-4 11 Part. Uniform 0-10-5 to 1-8-15 Тор 34 PLF 92 PLF 0 PLF 0 PLF 12 Part. Uniform 0-10-5 to 1-9-4 Тор **38 PLF** 0 PLF 0 PLF 0 PLF Wall Self Weight 13 Point 1-3-15 Near Face 173 lb 359 lb 0 lb 0 lb 14 **Point** 2-0-0 Top 2 lb 4 lb 0 lb 0 lb Bearing Length 0-5-8 2-0-0 0 lb 0 lb Point Top 1 lb 2 lb 15 0-5-8 Bearing Length 16 2-0-0 19 lb 0 lb 0 lb 0 lb Wall Self Weight Top Bearing Length 0-5-8 MODULUS ENGINEERING LTD. 17 Point 2-0-0 Top 1 lb 1 lb 0 lb 0 lb PROFESSIONAL SPO 0-5-8 Bearing Length 07/15/2022 18 Point 2-0-0 0 lb 0 lb 0 lb Top 1 lb Bearing Length 0-5-8 19 **Point** 2-0-0 Тор 6 lb 0 lb 0 lb 0 lb Wall Self Weight Bearing Length 0-5-8 Self Weight 5 PLF VINCE OF ON SEE GENERAL NOTES DOC: ME-TC02 V 03-2017 NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL Manufacturer Info Kott Inc. For flat roofs provide proper drainage to prevent ponding 3228 Moodie Dr, Ottawa, Ontario Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads Handling & Installation Forex LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code 613-838-2775 / 905-642-4400 APA: PR-L318 Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive This design is valid until 5/24/2024 Version 21.40.338 Powered by iStruct™ Dataset: 22022301.1 CSD DESIGN

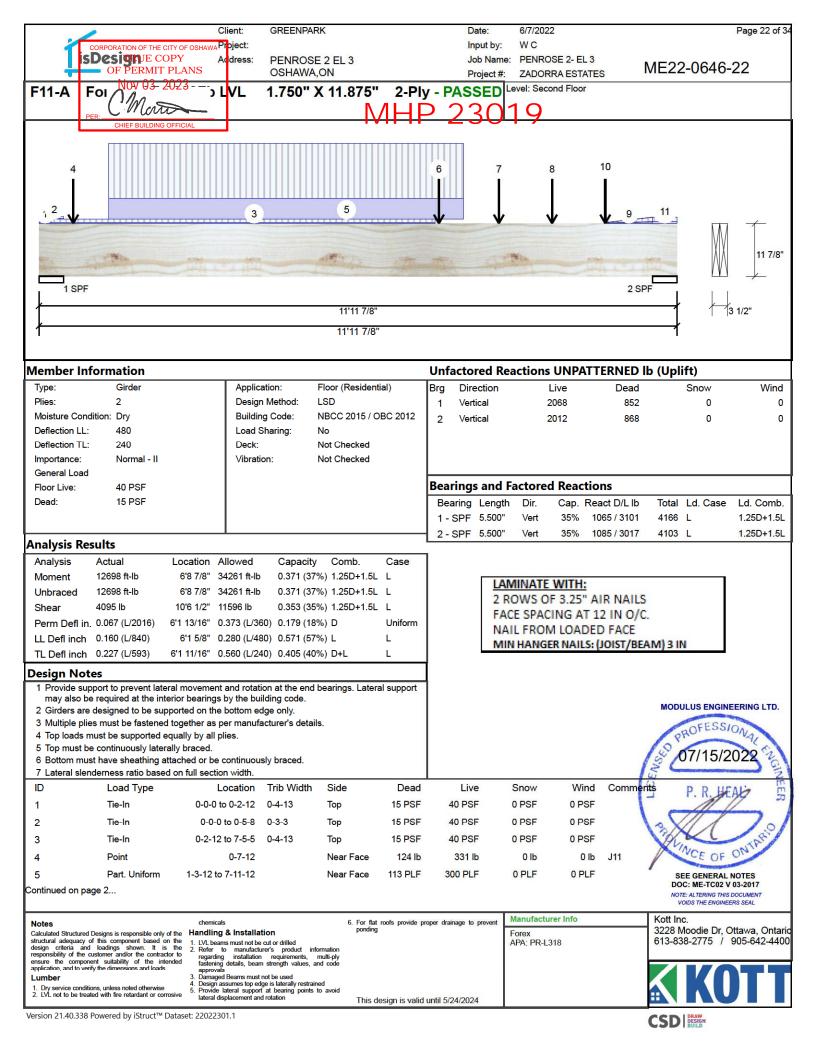


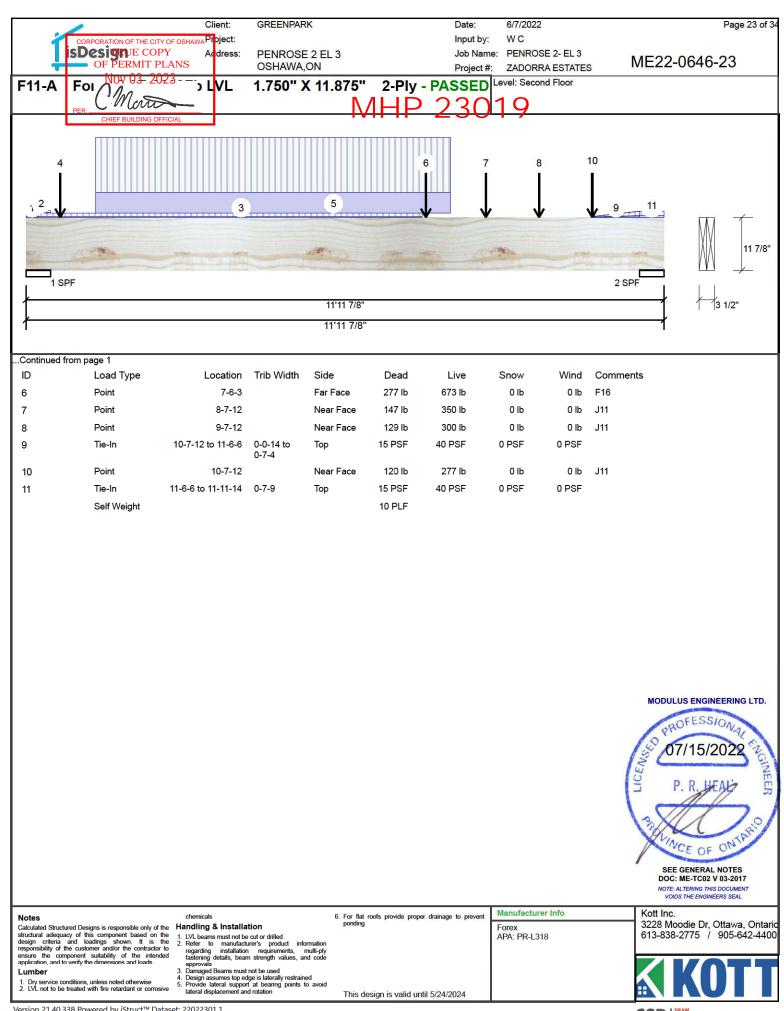


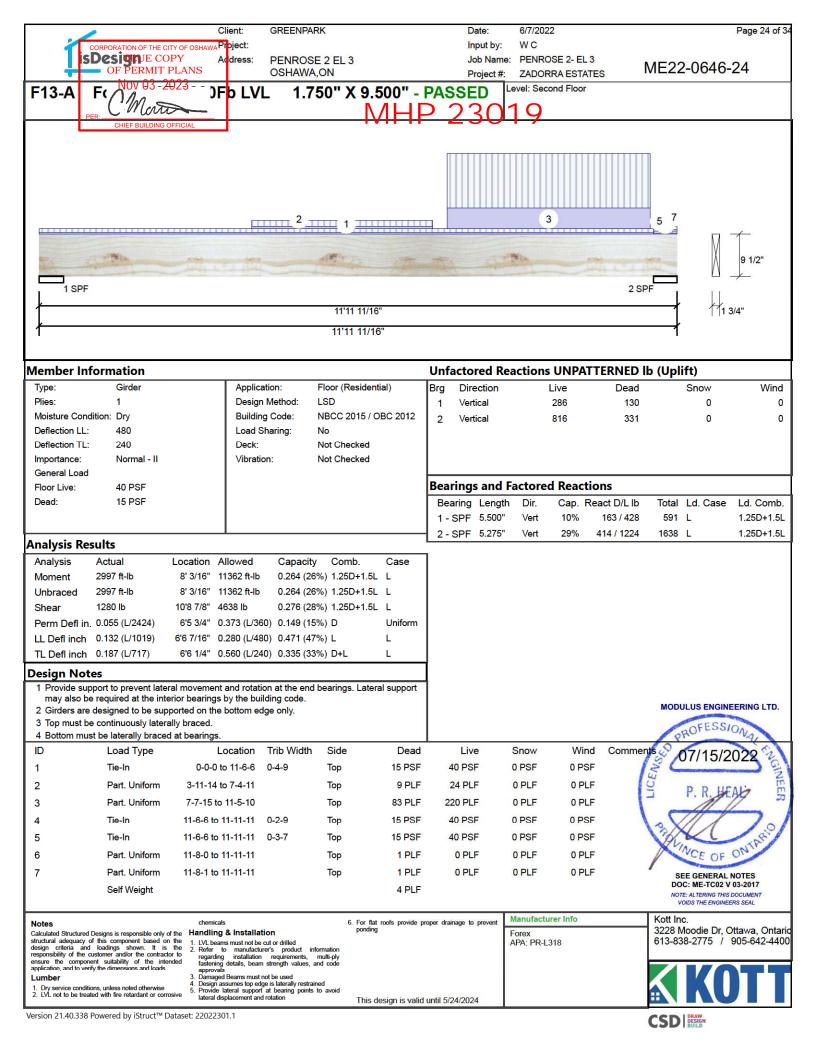


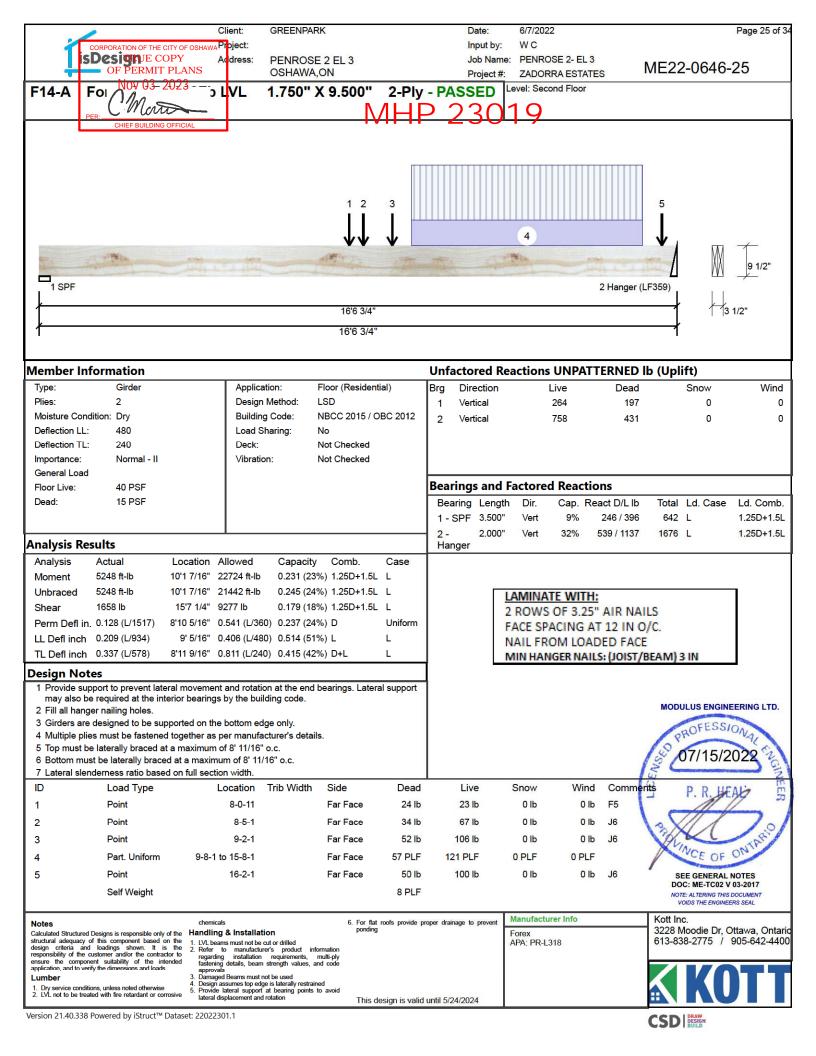


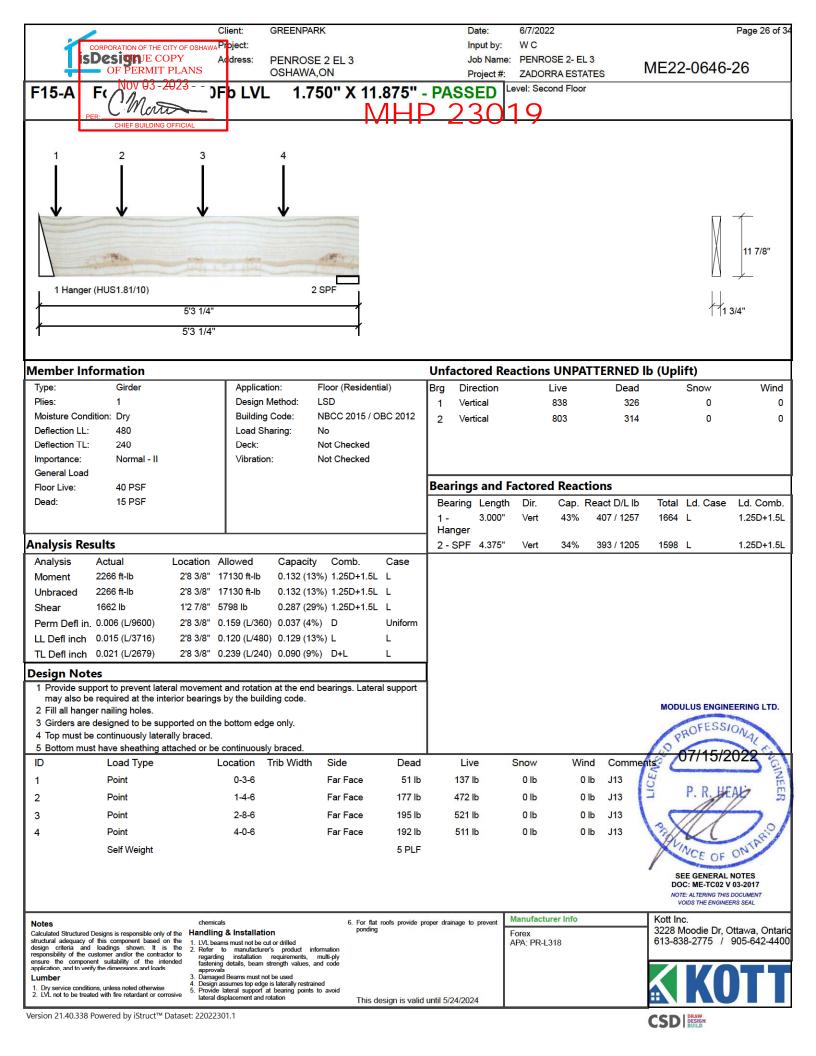


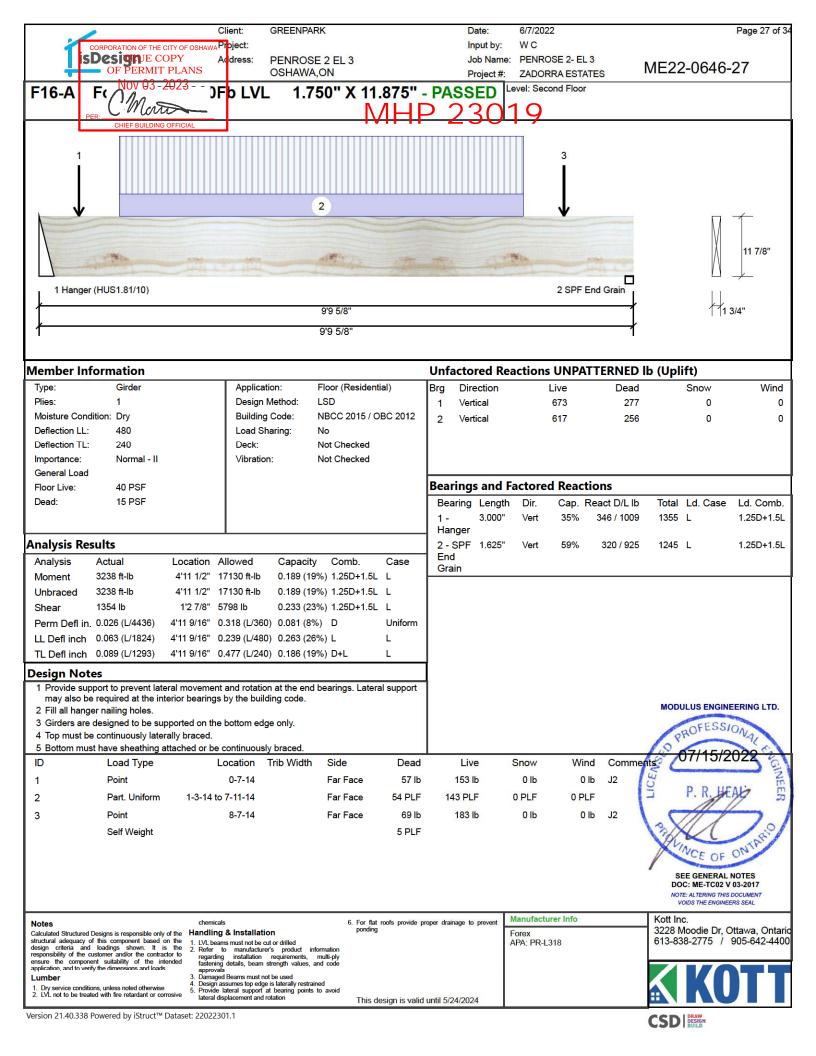


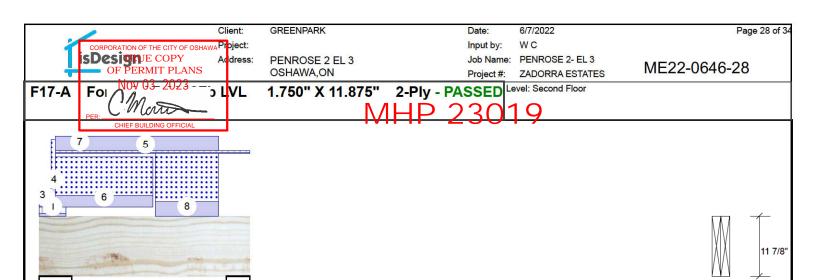












Member Infor	lember Information						Unfactored Reactions UNPATTERNED lb (Uplift)								
Type:	Girder		Application:	Floor (Residential)	Brg	Dire	ction		Live	Dead		Snow	Wind		
Plies:	2		Design Method:	LSD	1	Verti	cal		20	168		197	0		
Moisture Conditio	n: Dry		Building Code:	NBCC 2015 / OBC 2012	1 2	Verti	cal		10	121		158	0		
Deflection LL:	480		Load Sharing:	No	-										
Deflection TL:	240		Deck:	Not Checked	1										
Importance:	Normal - II		Vibration:	Not Checked	1										
General Load					_										
Floor Live:	40 PSF				Bear	rings	and Fa	actore	d Read	tions					
Dead:	15 PSF				Bea	aring	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.		
					1 - :	SPF	6.500"	Vert	4%	210 / 316	526	L	1.25D+1.5S +L		
Analysis Resul					2 -	SPF	4.625"	Vert	4%	151 / 247	398	L	1.25D+1.5S +L		
,	tual	Location													
Moment 28	3 ft_lh	1'10 3/16"	3/261 ft_lb 0.008 /	1%) 1 25D±1 5S I	1										

Moment 1'10 3/16" 34261 ft-lb 0.008 (1%) 1.25D+1.5S L Unbraced 283 ft-lb 1'10 3/16" 34261 ft-lb 0.008 (1%) 1.25D+1.5S L +L Shear 108 lb 1'6 3/8" 11596 lb 0.009 (1%) 1.25D+1.5S L Perm Defl in. 0.000 1'9 15/16" 0.089 (L/360) 0.004 (0%) D Uniform (L/91635) LL Defl inch 0.000 1'10 3/16" 0.067 (L/480) 0.007 (1%) S+0.5L (L/67289) TL Defl inch 0.001 1'10 1/16" 0.134 (L/240) 0.006 (1%) D+S+0.5L

2 SPF

Design Notes

1 SPF

3'5 11/16' 3'5 11/16'

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be continuously laterally braced.

(L/38801)

- 6 Bottom must be laterally braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

TOP LOADED LAMINATE WITH:

2 ROWS OF 3.25" AIR NAILS FACE SPACING AT 12 IN O/C. MAY NAIL FROM ONE FACE

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NOTE: ALTERING THIS DOCUMENT VOIDS THE ENGINEERS SEAL

Manufacturer Info 6. For flat roofs provide proper drainage to prevent ponding

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

I. UV. beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvate
 Damaged Beams must not be used

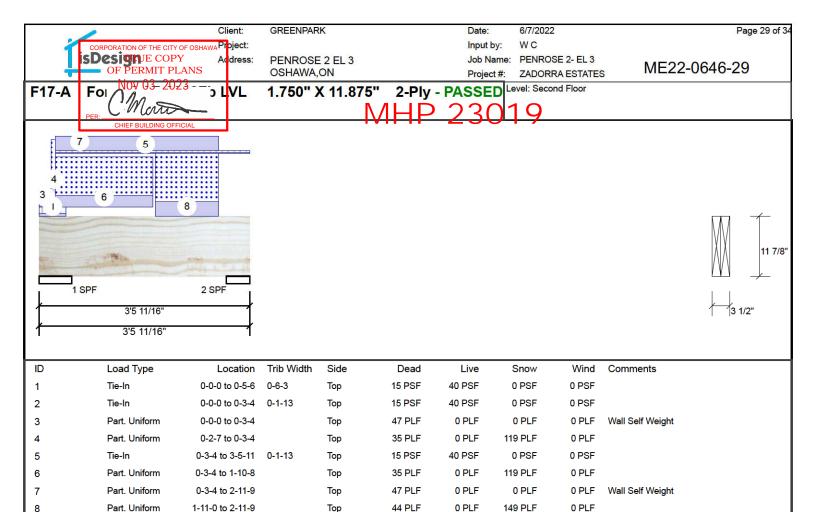
Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is valid until 5/24/2024

Forex APA: PR-L318







10 PLF



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For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex

APA: PR-L318

Kott Inc.

3228 Moodie Dr, Ottawa, Ontario 613-838-2775 / 905-642-4400



This design is valid until 5/24/2024



Self Weight

