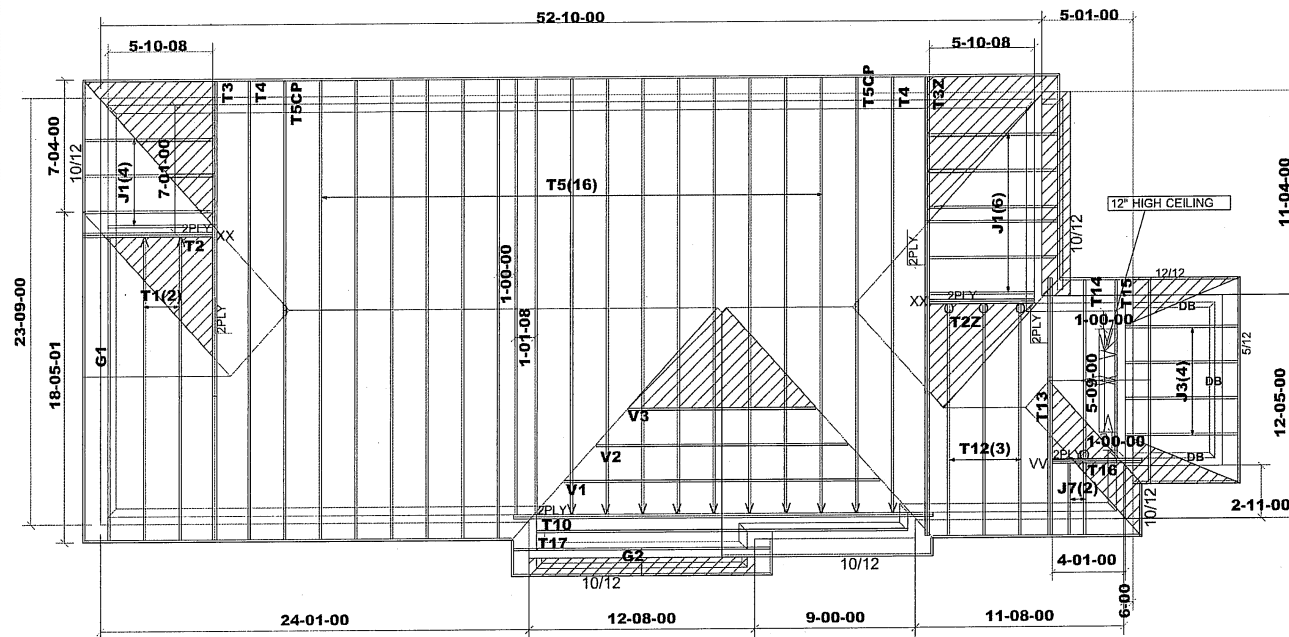




FINISH O.H: 12"
 Heel: R.T.M.C
 CLADDING ALLOWANCE: 5"
 2X6 EXTERIOR WALLS
 ASPHALT SHINGLES
 2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
 ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4" SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance between end points and between rows of bracing does not exceed 6'.

TRUSSES DESIGNED CONFORM TO THE RELEVANT SECTION OF THE LATEST EDITION OF THE ONTARIO BUILDING CODE (RESIDENTIAL PART 9)

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
 OCCUPANCY: RESIDENTIAL | PART: 9
 S_s = 33.4 psf | S_r = 8.4 psf

DESIGN LOADS:
 TC_{SL} = 26.7 psf
 TC_{DL} = 6.0 psf
 BC_{LL} = 0.0 psf
 BC_{DL} = 7.4 psf

HARDWARE
 LUS24(O)
 LIS26DS(V)
 LUS26-2(VV)
 HGUS26-2(XX)

DROPPED BEAM (DB)

ALL ROOF PITCHES ARE 9/12 UNLESS OTHERWISE NOTED

 **DENOTES:**
 CONVENTIONAL
 FRAMING

DWG# TR22080207 TO TR22080229



STRUCTURAL COMPONENTS ONLY
 DWG# TR22080230

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

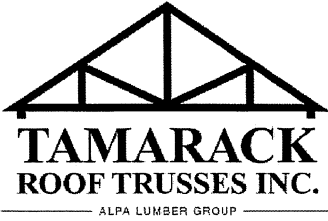
Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



Job Track: 52956	Builder / Location: GREEN PARK HOMES / CAMBRIDGE	Model / Elevation: GARDEN 9 / 1
Layout ID: 427579	Project: BARLASSINA	These drawings constitute the property of Tamarack Roof Trusses Inc. and shall not be reproduced, published, or redistributed in any manner or utilized for any purpose other than the manufacture of trusses by Tamarack Roof Trusses Inc. and will be retracted by Tamarack Roof Trusses Inc. if utilized for any other purpose.
Plan Log: 206405	Date: 8/02/22	Designer: AT

M15039

DELIVERY SHIPLIST

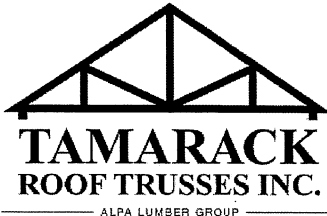


Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: GARDEN 9
 Lot #:
 Elevation: 1

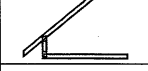
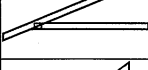
Job Track: 52956
 PlanLog: 206405
 Layout ID: 427579
 Ref #
 Page: 1 of 2
 Date: 08-03-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T1 Common	9 /12	15-07-00	7-04-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	138.75 89.67		
	1 2-ply	T2 Jack-Open Girder	10 /12	5-10-08	6-06-06	2 x 4 2 x 6	1-05-00	1-07-10 6-06-06	69.03 43.33		
	1 2-ply	T2Z Jack-Open Girder	10 /12	5-10-08	6-06-06	2 x 4 2 x 6	1-05-00	1-07-10 6-06-06	69.03 43.33		
	1 2-ply	T3 Hip Girder	9 /12	22-11-00	6-06-06	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	230.44 146.67		
	1 2-ply	T3Z Hip Girder	9 /12	22-11-00	6-06-06	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	230.44 146.67		
	2	T4 Hip	9 /12	22-11-00	8-02-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	214.41 133.67		
	16	T5 Common	9 /12	22-11-00	10-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1786.8 1130.67		
	2	T5CP Common	9 /12	22-11-00	9-10-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	222.67 141.33		
	1 2-ply	T10 Common Girder	10 /12	20-10-00	10-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-07-10 1-07-10	265.58 166.67		
	3	T12 Common	9 /12	11-07-00	5-10-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	148.73 99.00		
	1 2-ply	T13 Hip Girder	9 /12	11-07-00	4-07-07	2 x 4 2 x 6	1-03-08 1-05-00	1-06-04 1-06-04	121.76 78.33		
	1	T14 Common	9 /12	8-08-00	4-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	38.89 26.83		
	1	T15 Roof Special	9 /12	8-08-00	4-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	45.42 31.33		
	1 2-ply	T16 Half Hip Girder	10 /12	3-07-00	3-06-04	2 x 4 2 x 6	1-05-00	1-07-10 3-06-04	48.87 34.33		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: GARDEN 9 Lot #: Elevation: 1	Job Track: 52956 PlanLog: 206405 Layout ID: 427579 Ref # Page: 2 of 2 Date: 08-03-2022 Designer: Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T17 Common	10 / 12	20-10-00	10-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-07-10 1-07-10	112.98 70.33		
	1	G1 GABLE	9 / 12	15-07-00	7-04-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	73.48 47.83		
	1	G2 GABLE	10 / 12	11-10-00	6-06-13	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	57.51 37.00		
	1	V1 Valley	10 / 12	17-08-13	7-04-11	2 x 4			67.82 43.50		
	1	V2 Valley	10 / 12	14-01-10	5-10-11	2 x 4			49.33 31.00		
	1	V3 Valley	10 / 12	10-06-06	4-04-11	2 x 4			33 21.67		
	10	J1 Jack-Open	10 / 12	5-10-08	6-06-06	2 x 4	1-03-08	1-07-10 6-06-06	189.91 120.00		
	4	J3 Monopitch	5 / 12	5-05-08	3-01-09	2 x 4	1-03-08	4-01 2-05-01	58.8 37.33		
	2	J7 Jack-Open	9 / 12	2-08-00	3-06-04	2 x 4	1-03-08	1-06-04 3-06-04	20.34 14.67		

TOTAL # TRUSS= 63 TOTAL BFT OF ALL TRUSSES= 2735.16 BFT. TOTAL WEIGHT OF ALL TRSSES 4293.99 LBS

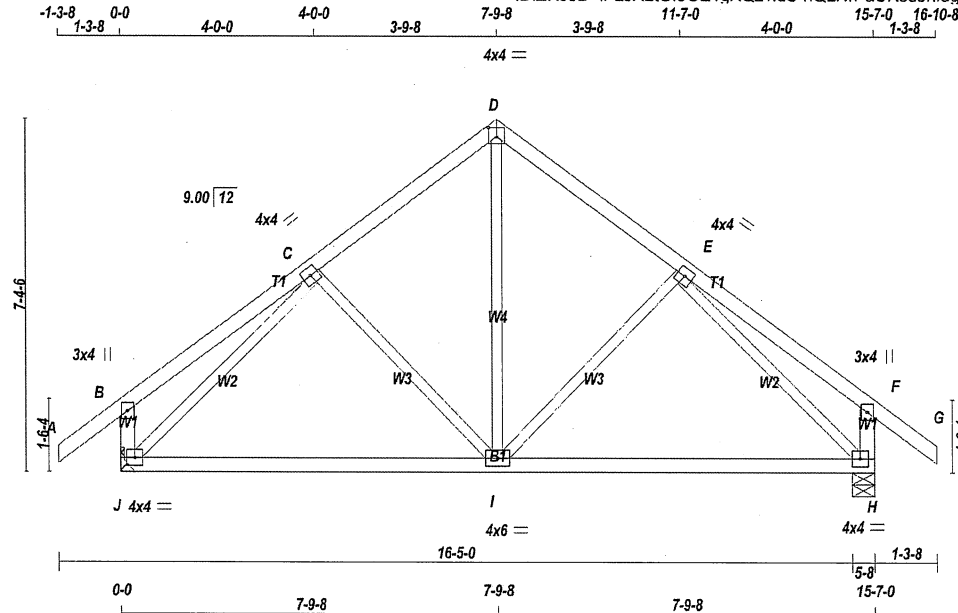
HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	
12	Hardware	LJS26DS	
1	Hardware	LUS26-2	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 19

JOB NAME 427579	TRUSS NAME T1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:46.2



TOTAL WEIGHT = 2 X 69 = 139 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
JT	1017	0	1017	0
J	1017	0	1017	0
H	1017	0	1017	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
JT	716	490 / 0	0 / 0
J	716	490 / 0	0 / 0
H	716	490 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	I-D	0 / 477
B-C	0 / 26	-95.2	-95.2	0.23 (1)	10.00	I-E	-206 / 0
C-D	-662 / 0	-95.2	-95.2	0.18 (1)	6.25	C-I	-206 / 0
D-E	-662 / 0	-95.2	-95.2	0.18 (1)	6.25	J-C	-949 / 0
E-F	0 / 26	-95.2	-95.2	0.23 (1)	10.00	E-H	-949 / 0
F-G	0 / 39	-95.2	-95.2	0.13 (1)	10.00		
J-B	-273 / 0	0.0	0.0	0.03 (1)	7.81		
H-F	-273 / 0	0.0	0.0	0.03 (1)	7.81		
J-I	0 / 652	-18.5	-18.5	0.37 (4)	10.00		
I-H	0 / 652	-18.5	-18.5	0.37 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.23/1.00 (B-C-1), BC=0.37/1.00 (H-I-4), WB=0.50/1.00 (C-J-1), SSI=0.14/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(Psi) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

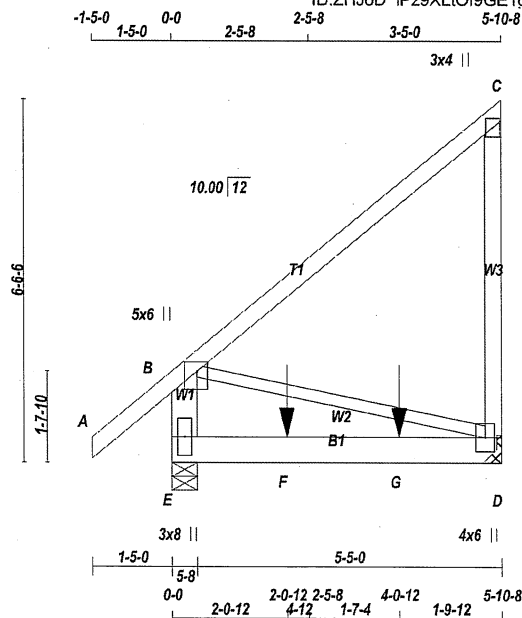
JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080212

JOB NAME 427579	TRUSS NAME T2	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:39.8

TOTAL WEIGHT = 2 X 35 = 69 lb [M]

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY				SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
D - C 1	12	TOP
E - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
E - D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
D	1375	0	1375	0
E	1434	0	1434	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
D	967	668 / 0 0 / 0 0 / 0 298 / 0 0 / 0
E	1006	707 / 0 0 / 0 0 / 0 0 / 0 298 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	0 / 46	-95.2	-95.2	0.09 (1)	B-D	0 / 0	0.00 (1)
B-C	0 / 0	-95.2	-95.2	0.31 (1)			
D-C	-280 / 0	0.0	0.0	0.11 (1)			
E-B	-423 / 0	0.0	0.0	0.02 (1)			
E-F	0 / 0	-18.5	-18.5	0.75 (1)			
F-G	0 / 0	-18.5	-18.5	0.75 (1)			
G-D	0 / 0	-18.5	-18.5	0.75 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	-701	-701	---	FRONT	VERT	TOTAL	---	C1
G	4-0-12	-701	-701	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 669 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 366 (0.19")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.75/1.00 (D-E:1) , WB=0.00/1.00 (B-D:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080213 PG 1/2

CONTINUED ON PAGE 2

JOB NAME 427579	TRUSS NAME T2	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMV1+p	MT20	3.0	8.0		

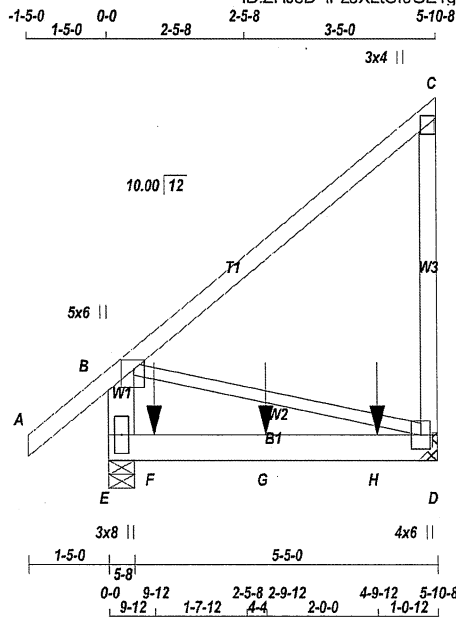
NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22080213 PG 2/2

JOB NAME 427579	TRUSS NAME T2Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



Scale = 1:39.8

TOTAL WEIGHT = 2 X 35 = 69 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x6	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
D - C 1	12	TOP
E - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
E - D 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	1442	1442	0	0
D 1442	0	0	0	0
E 1685	0	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1012	708 / 0	0 / 0	0 / 0	0 / 0	304 / 0	0 / 0
E	1181	837 / 0	0 / 0	0 / 0	0 / 0	343 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (PLF)	MAX. FACTORED UNBRAC (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LBS)
FR-TO					FR-TO		
A-B	0 / 46	-95.2	-95.2 0.09 (1)	10.00	B-D	0 / 0	0.00 (1)
B-C	0 / 0	-95.2	-95.2 0.31 (1)	10.00			
D-C	-280 / 0	0.0	0.0 0.11 (1)	7.81			
E-B	-423 / 0	0.0	0.0 0.02 (1)	7.81			
E-F	0 / 0	-18.5	-18.5 0.71 (1)	10.00			
F-G	0 / 0	-18.5	-18.5 0.71 (1)	10.00			
G-H	0 / 0	-18.5	-18.5 0.71 (1)	10.00			
H-D	0 / 0	-18.5	-18.5 0.71 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	9-12	-542	-542	---	BACK	VERT	TOTAL	---	C1
G	2-9-12	-540	-540	---	BACK	VERT	TOTAL	---	C1
H	4-9-12	-540	-540	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	26.7 PSF
DL	=	6.0 PSF
BOT CH. LL	=	0.0 PSF
DL	=	7.4 PSF
TOTAL LOAD	=	40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/752 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/414 (0.17")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.71/1.00 (D-E:1)
WB=0.00/1.00 (B-D:1) , SSI=0.42/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (B) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080214 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427579	T2Z	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	BMV1+p	MT20	3.0	8.0		

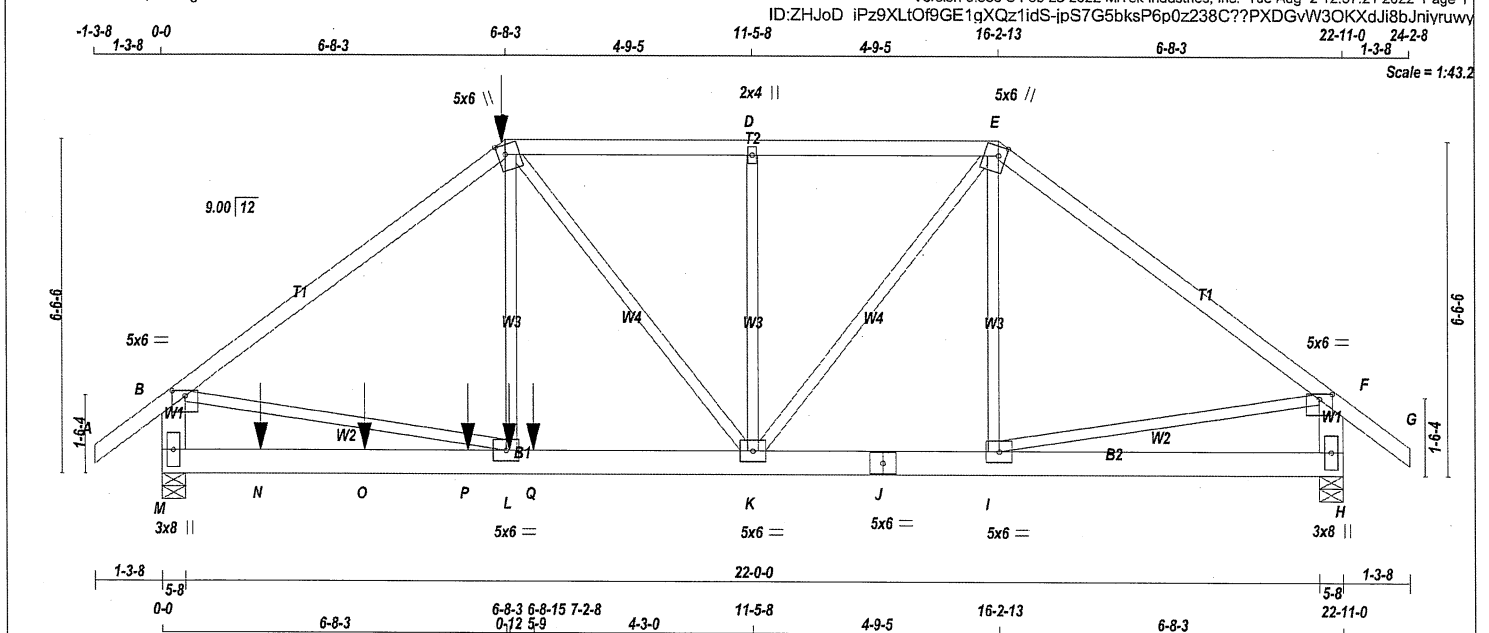
NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
427579	T3	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 2 X 115 = 230 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
M - B	2x6	DRY	No.2	SPF	
H - F	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - E	12	SIDE(61.0)
E - G	12	TOP
M - B	2	TOP
H - F	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - J	2	SIDE(183.1)
J - H	2	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
L - C	1	SIDE(44.5)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
M	2905	0	5-8	5-8
H	2069	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
M	2045	1396 / 0	0 / 0	0 / 0	0 / 0	649 / 0
H	1456	1000 / 0	0 / 0	0 / 0	0 / 0	456 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO				FR-TO		
A-B	0 / 39	-95.2 -95.2	0.07 (1)	L-C	0 / 1046	0.13 (1)
B-C	-3263 / 0	-95.2 -95.2	0.81 (1)	I-E	-176 / 51	0.06 (1)
C-D	-2450 / 0	-95.2 -95.2	0.22 (1)	B-L	0 / 2641	0.33 (1)
D-E	-2450 / 0	-95.2 -95.2	0.22 (1)	I-F	0 / 1729	0.21 (1)
E-F	-2136 / 0	-95.2 -95.2	0.51 (1)	K-D	-542 / 0	0.18 (1)
F-G	0 / 39	-95.2 -95.2	0.07 (1)	C-K	-284 / 0	0.17 (1)
M-B	-2838 / 0	0.0 0.0	0.10 (1)	K-E	0 / 1215	0.15 (1)
H-F	-2013 / 0	0.0 0.0	0.07 (1)			
M-N	0 / 0	-18.5 -18.5	0.09 (4)			
N-O	0 / 0	-18.5 -18.5	0.09 (4)			
O-P	0 / 0	-18.5 -18.5	0.09 (4)			
P-L	0 / 0	-18.5 -18.5	0.09 (4)			
L-Q	0 / 2624	-18.5 -18.5	0.30 (1)			
Q-K	0 / 2624	-18.5 -18.5	0.30 (1)			
K-J	0 / 1704	-18.5 -18.5	0.14 (1)			
J-I	0 / 1704	-18.5 -18.5	0.14 (1)			
I-H	0 / 0	-18.5 -18.5	0.05 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	6-8-3	-65	-65	---	FRONT	VERT	DEAD	---	C1
C	6-8-3	-84	-84	---	FRONT	VERT	TOTAL	---	C1
C	6-8-3	-290	-290	---	FRONT	VERT	SNOW	---	C1
L	6-8-15	-22	-22	---	FRONT	VERT	TOTAL	---	C1
N	1-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
P	5-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
Q	7-2-8	-952	-952	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	40.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.08")

CSI: TC=0.61/1.00 (B-C:1), BC=0.30/1.00 (K-L:1), WB=0.33/1.00 (B-L:1), SSI=0.43/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22080215 PG 1/2

JOB NAME 427579	TRUSS NAME T3	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.75
F	TMVW-p	MT20	5.0	6.0	1.25	3.00
H	BMV1+p	MT20	3.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

JSI GRIP= 0.66 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)



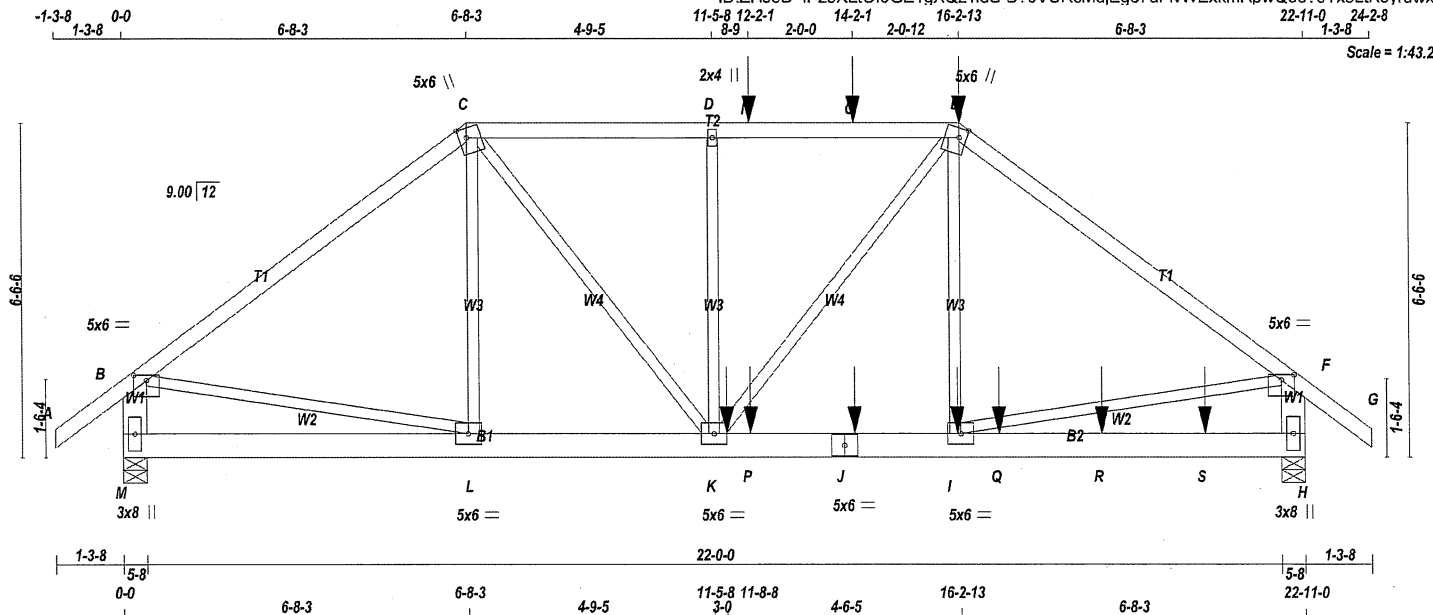
STRUCTURAL COMPONENT ONLY
DWG # TR22080215 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
427579	T3Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 115 = 230 lb [M]

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2	SPF
	C - E	2x4	DRY	No.2	SPF
	E - G	2x4	DRY	No.2	SPF
	M - B	2x6	DRY	No.2	SPF
	H - F	2x6	DRY	No.2	SPF
	M - J	2x6	DRY	No.2	SPF
	J - H	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	SIDE(61.0)
E - G	12	SIDE(61.0)
M - B	2	TOP
H - F	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M - J	12	SIDE(183.1)
J - H	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
I - E	6	SIDE(44.5)
D - K	6	SIDE(188.3)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
M	2476	0	2476	0
H	2850	0	2850	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. / MIN. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1741	1198 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0
H	2007	1365 / 0	0 / 0	0 / 0	0 / 0	641 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO							
A - B	0 / 39	-95.2	0.07 (1)	10.00	L - C	-198 / 40	0.07 (1)
B - C	-2698 / 0	-95.2	0.55 (1)	4.95	I - E	-151 / 148	0.05 (1)
C - D	-3186 / 0	-95.2	0.30 (1)	4.88	B - L	0 / 2184	0.27 (1)
D - N	-3186 / 0	-95.2	0.30 (1)	4.88	I - F	0 / 2565	0.32 (1)
N - O	-3186 / 0	-95.2	0.30 (1)	4.88	K - D	-710 / 0	0.24 (1)
O - E	-3186 / 0	-95.2	0.30 (1)	4.88	C - K	0 / 1684	0.21 (1)
E - F	-3168 / 0	-95.2	0.60 (1)	4.63	K - E	0 / 1071	0.13 (1)
F - G	0 / 39	-95.2	0.07 (1)	10.00			
M - B	-2424 / 0	0.0	0.09 (1)	7.81			
H - F	-2769 / 0	0.0	0.10 (1)	7.81			
M - L	0 / 0	-18.5	-18.5	0.05 (4)	10.00		
L - K	0 / 2152	-18.5	-18.5	0.18 (1)	10.00		
K - P	0 / 2529	-18.5	-18.5	0.21 (1)	10.00		
P - J	0 / 2529	-18.5	-18.5	0.21 (1)	10.00		
J - I	0 / 2529	-18.5	-18.5	0.21 (1)	10.00		
I - Q	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
Q - R	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
R - S	0 / 0	-18.5	-18.5	0.08 (4)	10.00		
S - H	0 / 0	-18.5	-18.5	0.08 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
E	16-2-13	-65	-65	---	FRONT	VERT	DEAD	C1
E	16-2-13	-84	-84	---	FRONT	VERT	TOTAL	C1
E	16-2-13	-290	-290	---	FRONT	VERT	SNOW	C1
I	16-2-1	-22	-22	---	FRONT	VERT	TOTAL	C1
J	14-2-1	-22	-22	---	FRONT	VERT	TOTAL	C1
K	11-8-8	-997	-997	---	FRONT	VERT	TOTAL	C1
N	12-2-1	-79	-79	---	FRONT	VERT	TOTAL	C1
O	14-2-1	-79	-79	---	FRONT	VERT	TOTAL	C1
P	12-2-1	-22	-22	---	FRONT	VERT	TOTAL	C1
Q	16-11-12	-22	-22	---	FRONT	VERT	TOTAL	C1
R	18-11-12	-22	-22	---	FRONT	VERT	TOTAL	C1
S	20-11-12	-22	-22	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	40.1	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.60/1.00 (E-F:1), BC=0.21/1.00 (I-K:1),
WB=0.32/1.00 (F-I:1), SS=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22080216 PG 1/2

JOB NAME 427579	TRUSS NAME T3Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:37:22 2022 Page 2 ID:ZHJoD iPz9XLtOf9GE1gXQz1dS-B?0VURcMdiEge7dFivWExkmRpwQ03?0TxoLtK8yruwx			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.75
F	TMVW-p	MT20	5.0	6.0	1.25	3.00
H	BMV1+p	MT20	3.0	8.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

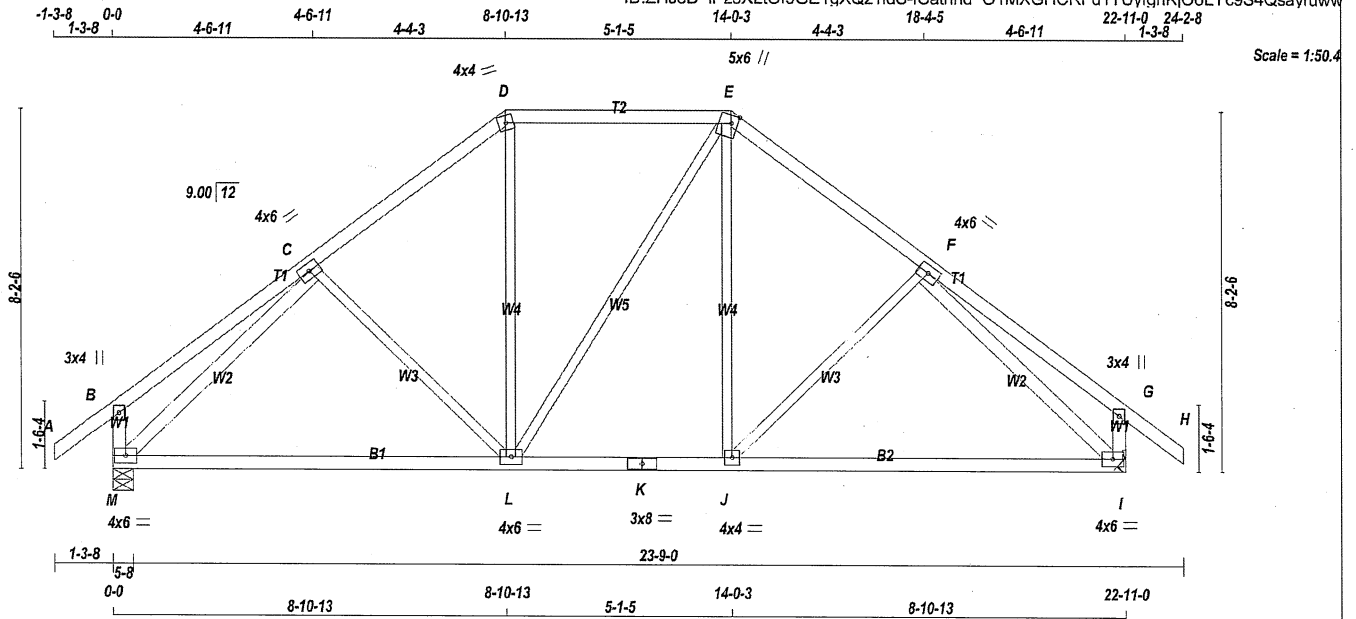
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.32 (B) (INPUT = 1.00)



JOB NAME 427579	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:ZHJoD iPz9XLtOf9GE1qXQz1IdS-fCathnd O1MXGHCRFd1TUylgnKjOoLYc9S4Qsaynuww



TOTAL WEIGHT = 2 X 107 = 214 lb [M/JF]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - C	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.75
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMVW1-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
M 1434	0	1434	0
I 1434	0	1434	0
		5-8	5-8
		MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1010	686 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0
I	1010	686 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 39	-95.2	-95.2 0.13 (1)	10.00	C-L	-198 / 0	0.15 (1)
B-C	0 / 30	-95.2	-95.2 0.31 (1)	10.00	L-D	0 / 280	0.06 (1)
C-D	-1171 / 0	-95.2	-95.2 0.25 (1)	5.61	L-E	0 / 0	0.00 (1)
D-E	-914 / 0	-95.2	-95.2 0.33 (1)	6.02	J-E	0 / 280	0.06 (1)
E-F	-1170 / 0	-95.2	-95.2 0.25 (1)	5.61	J-F	-198 / 0	0.15 (1)
F-G	0 / 30	-95.2	-95.2 0.31 (1)	10.00	M-C	-1501 / 0	0.75 (1)
G-H	0 / 39	-95.2	-95.2 0.13 (1)	10.00	F-I	-1501 / 0	0.75 (1)
M-B	-292 / 0	0.0	0.0 0.03 (1)	7.81			
I-G	-292 / 0	0.0	0.0 0.03 (1)	7.81			
M-L	0 / 1051	-18.5	-18.5 0.39 (4)	10.00			
L-K	0 / 914	-18.5	-18.5 0.36 (4)	10.00			
K-J	0 / 914	-18.5	-18.5 0.36 (4)	10.00			
J-I	0 / 1051	-18.5	-18.5 0.39 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.33/1.00 (D-E:1), BC=0.39/1.00 (I-J:4), WB=0.75/1.00 (C-M:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.90)
JSI METAL= 0.43 (K) (INPUT = 1.00)



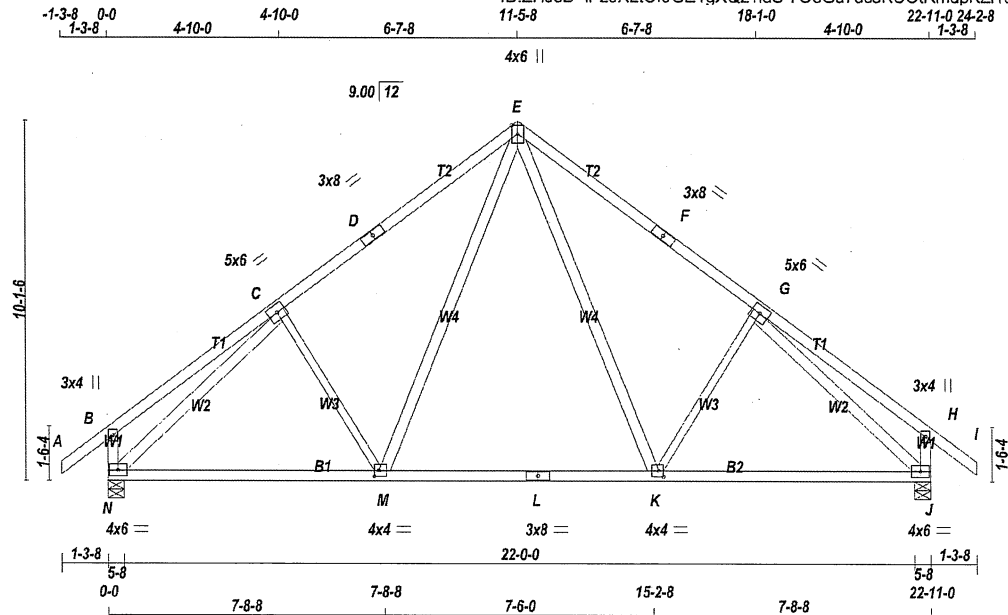
STRUCTURAL COMPONENT ONLY
DWG # TR22080217

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
427579	T5	16	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:ZHJoD iPz9XLtOf9GE1gXQz1idS-7O8Gu7dc9KUOtRmdpKZi19rohK4XXmXmO6q O1yruwv



Scale = 1:62.1

TOTAL WEIGHT = 16 X 112 = 1787 lb

LUMBER			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			DESIGN CRITERIA		
N. L. G. A. RULES			BEARINGS			SPECIFIED LOADS:		
CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
A - D	2x4	DRY	No.2	SPF	GROSS REACTION	BRG	BRG	
D - E	2x4	DRY	No.2	SPF	VERT	DOWN	IN-SX	
E - F	2x4	DRY	No.2	SPF	HORIZ	HORIZ	IN-SX	
F - I	2x4	DRY	No.2	SPF	JT	1434	0	5-8
N - B	2x4	DRY	No.2	SPF	J	1434	0	5-8
J - H	2x4	DRY	No.2	SPF				
N - L	2x4	DRY	No.2	SPF				
L - J	2x4	DRY	No.2	SPF				
ALL WEBS	2x4	DRY	No.2	SPF	UNFACTORED REACTIONS			
EXCEPT					1ST LCASE	MAX/MIN. COMPONENT REACTIONS		
K - G	2x3	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE
C - M	2x3	DRY	No.2	SPF	N	1010	686 / 0	0 / 0
					J	1010	686 / 0	0 / 0

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	3.0	8.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 39	-95.2 -95.2 0.13 (1)	10.00	E-K	0 / 468
B-C	0 / 53	-95.2 -95.2 0.58 (1)	10.00	K-G	-333 / 0
C-D	-1198 / 0	-95.2 -95.2 0.57 (1)	5.08	M-E	0 / 468
D-E	-1198 / 0	-95.2 -95.2 0.57 (1)	5.08	C-M	-333 / 0
E-F	-1198 / 0	-95.2 -95.2 0.57 (1)	5.08	N-C	-1568 / 0
F-G	-1198 / 0	-95.2 -95.2 0.57 (1)	5.08	G-J	-1568 / 0
G-H	0 / 53	-95.2 -95.2 0.13 (1)	10.00		
H-I	0 / 39	-95.2 -95.2 0.13 (1)	10.00		
N-B	-265 / 0	0.0 0.0 0.03 (1)	7.81		
J-H	-265 / 0	0.0 0.0 0.03 (1)	7.81		
N-M	0 / 1106	-18.5 -18.5 0.33 (4)	10.00		
M-L	0 / 759	-18.5 -18.5 0.31 (4)	10.00		
L-K	0 / 759	-18.5 -18.5 0.31 (4)	10.00		
K-J	0 / 1106	-18.5 -18.5 0.33 (4)	10.00		

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.12")

CSI: TC=0.58/1.00 (G-H:1), BC=0.33/1.00 (J-K:4), WB=0.89/1.00 (G-J:1), SSI=0.24/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

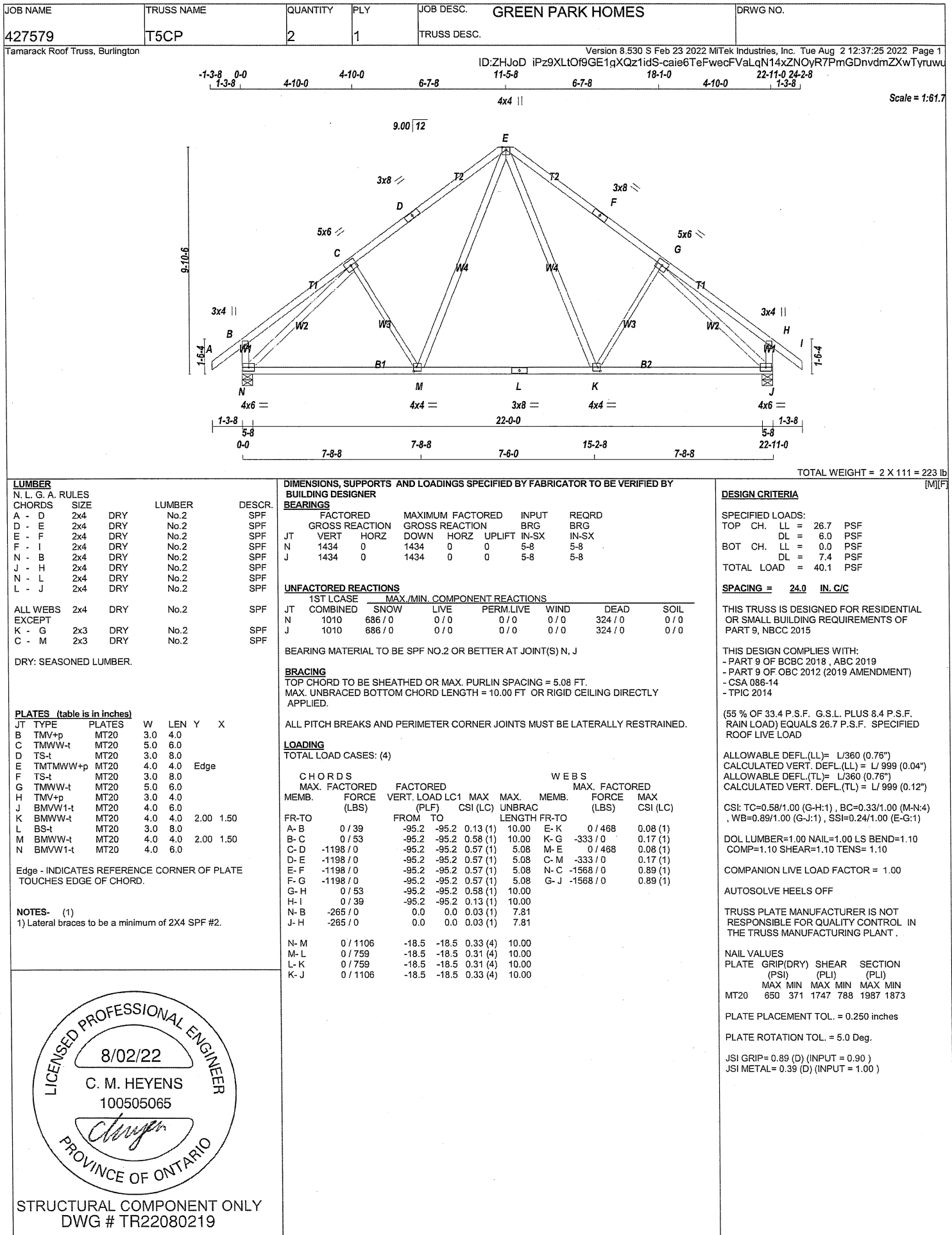
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.39 (D) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080218



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
427579	T10	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TMWW-t	MT20	4.0	6.0	2.00	2.25
E	TTW+p	MT20	5.0	8.0	Edge	
F	TMWW-t	MT20	4.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW-t	MT20	6.0	7.0	2.25	2.50
J	BMV1+t	MT20	6.0	10.0	Edge	0.50
K	BMWW+t	MT20	6.0	7.0	4.25	1.50
L	BS-t	MT20	5.0	6.0		
M	BMWW+t	MT20	3.0	8.0		
N	BMWW+t	MT20	8.0	9.0		
O	BMWW+t	MT20	3.0	8.0		
P	BMWW+t	MT20	6.0	7.0	4.25	1.50
Q	BMV1+t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

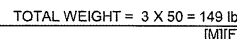
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	15-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
M	13-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
R	1-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
S	3-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
T	5-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
U	7-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
V	9-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
W	11-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
X	17-11-12	-995	-995	---	TOP	VERT	TOTAL	---	C1
Y	19-11-12	-996	-996	---	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.





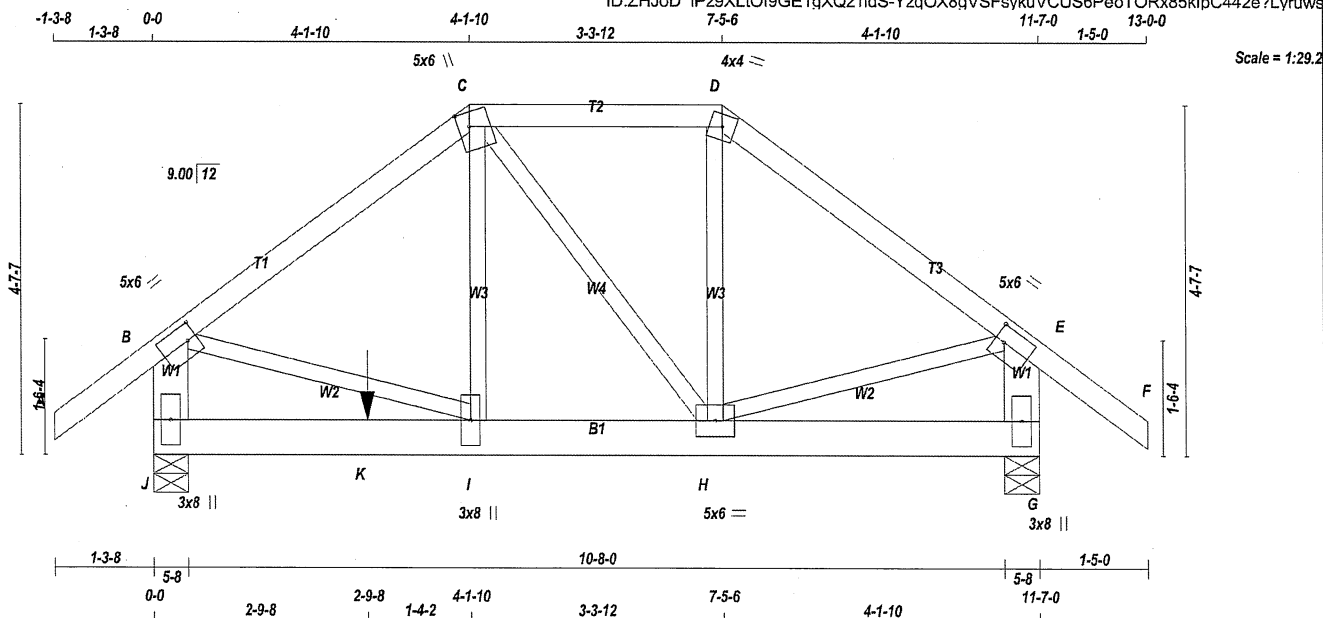
JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



JOB NAME 427579	TRUSS NAME T13	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 61 = 122 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x6	DRY	No.2	SPF	
G - E	2x6	DRY	No.2	SPF	
J - G	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
D - F	12	TOP
J - B	12	TOP
G - E	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - G	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1208	0	1208	0	5-8
G	934	0	934	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
J	847	597 / 0	0 / 0	0 / 0	0 / 0	250 / 0
G	656	458 / 0	0 / 0	0 / 0	0 / 0	198 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX.		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 39	-95.2	-95.2	0.07 (1)	10.00	I-C	0 / 389	0.05 (1)	
B-C	-873 / 0	-95.2	-95.2	0.16 (1)	6.25	C-H	-272 / 0	0.06 (1)	
C-D	-544 / 0	-95.2	-95.2	0.10 (1)	6.25	H-D	0 / 69	0.01 (4)	
D-E	-679 / 0	-95.2	-95.2	0.16 (1)	6.25	B-I	0 / 721	0.09 (1)	
E-F	0 / 43	-95.2	-95.2	0.08 (1)	10.00	H-E	0 / 560	0.07 (1)	
J-B	-1036 / 0	0.0	0.0	0.04 (1)	7.81				
G-E	-889 / 0	0.0	0.0	0.03 (1)	7.81				
J-K	0 / 0	-18.5	-18.5	0.15 (1)	10.00				
K-I	0 / 0	-18.5	-18.5	0.15 (1)	10.00				
I-H	0 / 707	-18.5	-18.5	0.13 (1)	10.00				
H-G	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	2-9-8	-384	-384	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.16/1.00 (B-C:1), BC=0.15/1.00 (I-J:1), WB=0.09/1.00 (B-I:1), SSI=0.15/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (I) (INPUT = 0.90)
JSI METAL= 0.21 (I) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080222 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427579	T13	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MITek Industries, Inc. Tue Aug 2 12:37:27 2022 Page 2
ID:ZHJoD iPz9XLtOf9GE1gXQz1dS-YzqOX8gVSFSykuVCUS6PcoTORx85klpC442e?Lyrws

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.75
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	5.0	6.0	2.50	1.50
G	BMV1+p	MT20	3.0	8.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW+t	MT20	3.0	8.0		
J	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

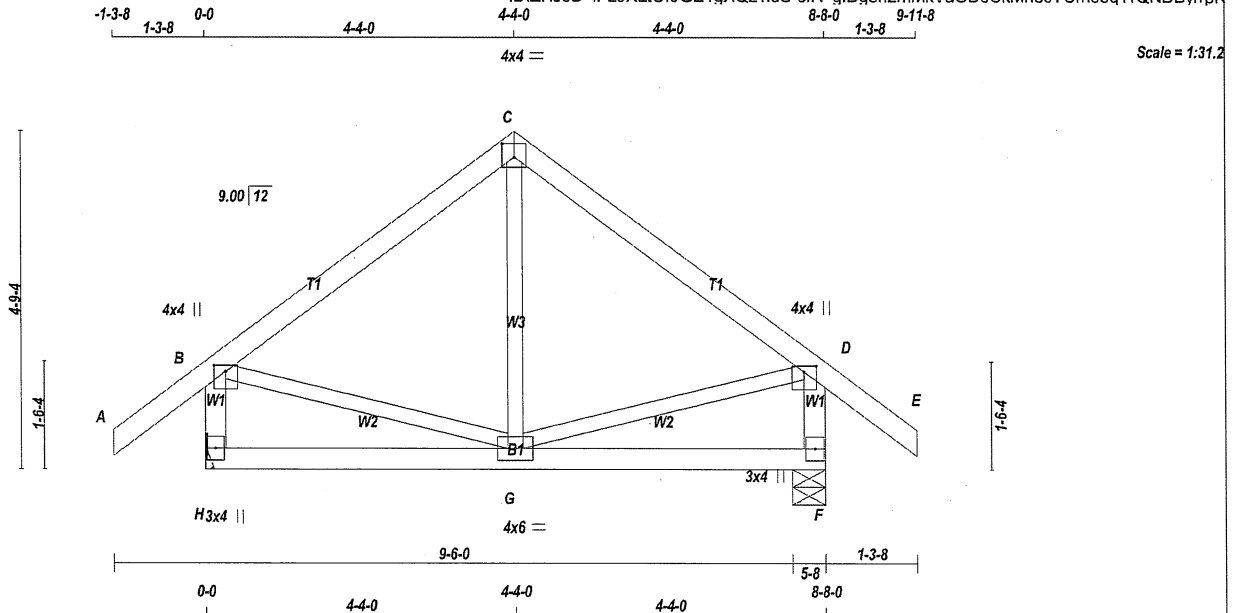
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22080222

JOB NAME 427579	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:ZHJoD iPz9XLtOf9GE1gXQz1idS-eiR_gfDg5ri2mMkVaOD0CkMhecYUmoeqYrQNDByrrpK



TOTAL WEIGHT = 39 lb

LUMBER				DESCR.	
N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	SPF
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	624	0	624	0	0	MECHANICAL
F	624	0	624	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
H	438	305 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
F	438	305 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 39	-95.2	-95.2 0.13 (1)	10.00	G-C	-39 / 62	0.02 (4)
B-C	-311 / 0	-95.2	-95.2 0.23 (1)	6.25	B-G	0 / 258	0.06 (1)
C-D	-311 / 0	-95.2	-95.2 0.23 (1)	6.25	G-D	0 / 258	0.06 (1)
D-E	0 / 39	-95.2	-95.2 0.13 (1)	10.00			
H-B	-593 / 0	0.0	0.0 0.06 (1)	7.81			
F-D	-593 / 0	0.0	0.0 0.06 (1)	7.81			
H-G	0 / 0	-18.5	-18.5 0.10 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (C-D:1), BC=0.10/1.00 (F-G:4), WB=0.06/1.00 (D-G:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

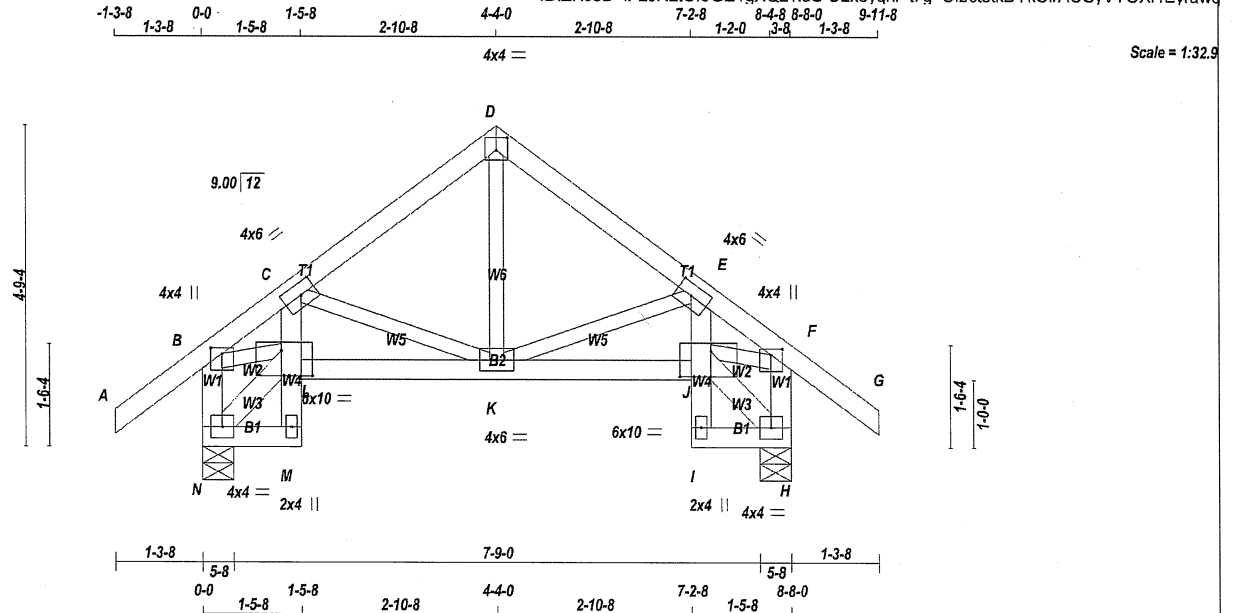
JSI GRIP= 0.50 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080223

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427579	T15	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 45 lb
(M/F)

LUMBER				LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY	No.2	DESCR.	SPF	
A - D	2x4	DRY	No.2	SPF			
D - G	2x4	DRY	No.2	SPF			
N - B	2x4	DRY	No.2	SPF			
H - F	2x4	DRY	No.2	SPF			
N - M	2x4	DRY	No.2	SPF			
L - J	2x4	DRY	No.2	SPF			
I - H	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF			
K - D	2x3	DRY	No.2	SPF			
K - E	2x3	DRY	No.2	SPF			
C - K	2x3	DRY	No.2	SPF			
B - L	2x3	DRY	No.2	SPF			
J - F	2x3	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	4.0	6.0	2.00	2.75
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	6.0	2.00	2.75
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW-w	MT20	2.0	4.0		
J	BMVWVW-j	MT20	6.0	10.0	4.50	5.50
K	BMVWVW-t	MT20	4.0	6.0		
L	BMVWVW-j	MT20	6.0	10.0	4.50	5.50
M	BMVW-w	MT20	2.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS				FACTORED				MAXIMUM FACTORED				INPUT				REQRD			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG	IN-SX	IN-SX	BRG	BRG	IN-SX	IN-SX	BRG	BRG	IN-SX	IN-SX
N	624	0	624	0	0	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8
H	624	0	624	0	0	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	438	305 / 0	0 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0
H	438	305 / 0	0 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 39	-95.2 -95.2	0.13 (1)	10.00	M-L	0 / 14	0.04 (1)
B-C	-643 / 0	-95.2 -95.2	0.10 (1)	6.25	L-C	0 / 61	0.05 (1)
C-D	-371 / 0	-95.2 -95.2	0.11 (1)	6.25	I-J	0 / 14	0.04 (1)
D-E	-371 / 0	-95.2 -95.2	0.11 (1)	6.25	J-E	0 / 61	0.05 (1)
E-F	-643 / 0	-95.2 -95.2	0.10 (1)	6.25	K-D	0 / 178	0.04 (1)
F-G	0 / 39	-95.2 -95.2	0.13 (1)	10.00	K-E	-255 / 0	0.05 (1)
N-B	-591 / 0	0.0 0.0	0.06 (1)	7.81	C-K	-255 / 0	0.05 (1)
H-F	-591 / 0	0.0 0.0	0.06 (1)	7.81	N-L	-31 / 0	0.00 (1)
N-M	0 / 23	-18.5 -18.5	0.01 (4)	10.00	J-H	-31 / 0	0.00 (1)
L-K	0 / 528	-18.5 -18.5	0.11 (1)	10.00	B-L	0 / 499	0.11 (1)
K-J	0 / 528	-18.5 -18.5	0.11 (1)	10.00	J-F	0 / 499	0.11 (1)
I-H	0 / 23	-18.5 -18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	40.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (F-G:1), BC=0.11/1.00 (J-K:1), WB=0.11/1.00 (F-J:1), SS=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)

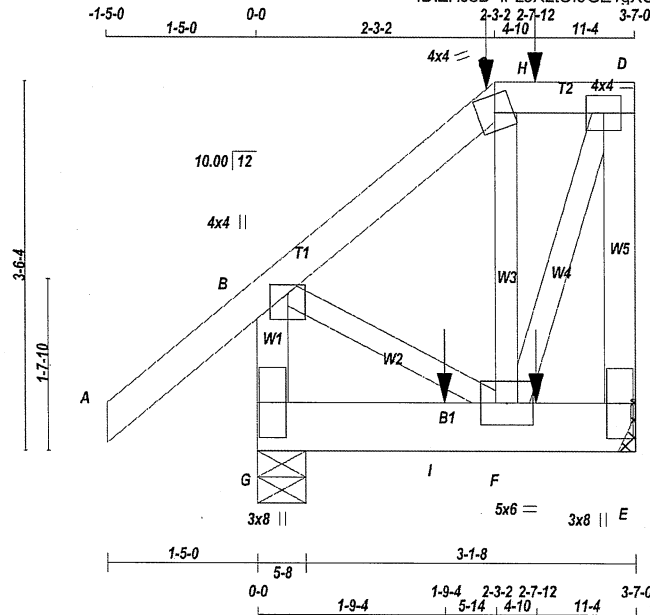
JSI METAL= 0.19 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080224

JOB NAME 427579	TRUSS NAME T16	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:21.1

TOTAL WEIGHT = 2 X 24 = 49 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - D	12	SIDE(61.0)
D - E	12	TOP
G - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - E	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	569	0	569	0	0
G	687	0	687	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 2-0.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN	COMPONENT REACTIONS
COMBINED	SNOW	LIVE	PERM.LIVE
E	399	281/0	0/0
G	480	348/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0/46	-95.2 -95.2	0.13 (5)	F-C	-81/6
B-C	-274/0	-95.2 -95.2	0.05 (1)	F-D	0/572
C-H	-207/0	-95.2 -95.2	0.02 (1)	B-F	0/235
H-D	-207/0	-95.2 -95.2	0.02 (1)		
E-D	-603/0	0.0 0.0	0.06 (1)		
G-B	-535/0	0.0 0.0	0.03 (1)		
G-I	0/0	-18.5 -18.5	0.09 (1)		
I-F	0/0	-18.5 -18.5	0.09 (1)		
F-E	0/0	-18.5 -18.5	0.02 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
C	2-3-2	-8	-8	FRONT	VERT	DEAD	---	C1
C	2-3-2	-34	-34	FRONT	VERT	SNOW	---	C1
F	2-7-12	-3	-3	BACK	VERT	TOTAL	---	C1
H	2-7-12	-22	-22	BACK	VERT	TOTAL	---	C1
I	1-9-4	-423	-423	FRONT	VERT	TOTAL	---	C1
I	1-9-4	-3	-3	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

SPECIFIED LOADS:				
TOP	CH.	LL =	26.7	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.09/1.00 (F-G:1), WB=0.07/1.00 (D-F:1), SSI=0.17/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.90)
JSI METAL= 0.08 (F) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22080225 PG 1/2

JOB NAME 427579	TRUSS NAME T16	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.530 5 Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:37:29 2022 Page 2 ID:ZHJoD iPz9XLtOf9GE1gXQz1idS-ULx8yqhl t7g Cfbc8tkDYISlrWCCcVVOXI4Eyrw			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	8.0		
F	BMWWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

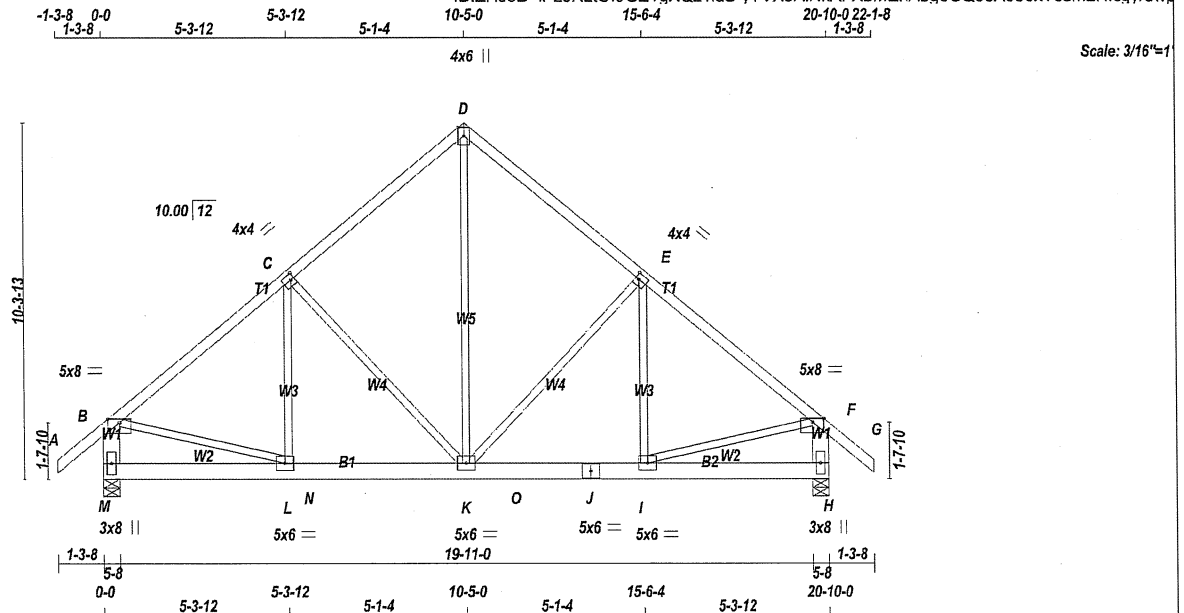
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22080225 PG 2/2

JOB NAME 427579	TRUSS NAME T17	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:37:30 2022 Page 1
ID:ZHJdD iPz9XLtOf9GE1gXQz1dS-yYVX9AikAFxMBEnAbq6GQ5oA856xVcem2HlCgyruwp



TOTAL WEIGHT = 113 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge
C	TMVW-t	MT20	4.0	4.0	2.00 1.25
D	TTW+p	MT20	4.0	6.0	Edge
E	TMVW-t	MT20	4.0	4.0	2.00 1.25
F	TMVW-p	MT20	5.0	8.0	Edge
H	BMV1+p	MT20	3.0	8.0	
I	BMVW-t	MT20	5.0	6.0	
J	BS-t	MT20	5.0	6.0	
K	BMVW-t	MT20	5.0	6.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMV1+p	MT20	3.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
M	1662	0	0	1662	0	5-8	5-8	5-8	5-8
H	1933	0	0	1933	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1204	631 / 0	0 / 0	0 / 0	0 / 0	573 / 0	0 / 0
H	1420	631 / 0	0 / 0	0 / 0	0 / 0	789 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	MAX. FORCE (LBS)	MAX	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 42	-95.2	-95.2	0.15 (1)	10.00	K-D	0 / 1263	0.36 (4)	
B-C	-1545 / 0	-95.2	-95.2	0.61 (1)	4.52	K-E	-628 / 0	0.73 (1)	
C-D	-1310 / 0	-95.2	-95.2	0.57 (1)	4.85	I-E	0 / 398	0.15 (4)	
D-E	-1310 / 0	-95.2	-95.2	0.57 (1)	4.86	C-K	-356 / 0	0.41 (1)	
E-F	-1783 / 0	-95.2	-95.2	0.64 (1)	4.24	L-C	-151 / 2	0.08 (1)	
F-G	0 / 42	-95.2	-95.2	0.15 (1)	10.00	B-L	0 / 1247	0.32 (1)	
M-B	-1618 / 0	0.0	0.0	0.12 (1)	7.69	I-F	0 / 1434	0.36 (1)	
H-F	-1812 / 0	0.0	0.0	0.14 (1)	7.37				
M-L	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
L-N	0 / 1216	-18.5	-18.5	0.40 (4)	10.00				
N-K	0 / 1216	-18.5	-112.2	0.40 (4)	10.00				
K-O	0 / 1398	-112.2	-112.2	0.45 (4)	10.00				
O-J	0 / 1398	-133.5	-111.4	0.45 (4)	10.00				
J-I	0 / 1398	-111.4	-95.6	0.45 (4)	10.00				
I-H	0 / 0	-95.6	-41.0	0.28 (4)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.64/1.00 (E-F:1), BC=0.45/1.00 (I-K:4), WB=0.73/1.00 (E-K:1), SSI=0.34/1.00 (I-K:4)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

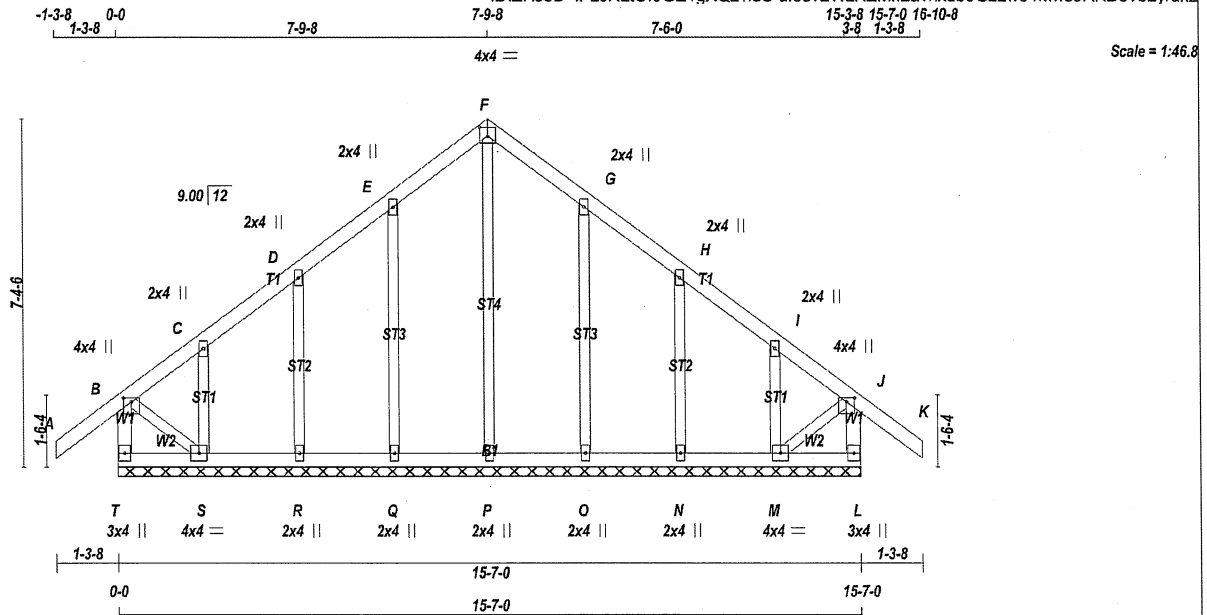
JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.39 (J) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080226

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427579	G1	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 12:37:15 2022 Page 1
ID:ZHJoD iPz9XLtOf9GE1gXQz1idS-uf5s?2WzHZMf12avnub9GzLw54Mws9RKD8?b2yruX2



TOTAL WEIGHT = 73 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
ALL GABLE WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMVW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMV1+w	MT20	2.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
T-B	-292 / 0	0.0	0.0 0.03 (1)	7.81	P-F	-142 / 0	0.13 (1)
A-B	0 / 39	-95.2	-95.2 0.13 (1)	10.00	Q-E	-214 / 0	0.11 (1)
B-C	-58 / 0	-95.2	-95.2 0.12 (1)	6.25	R-D	-195 / 0	0.06 (1)
C-D	-19 / 0	-95.2	-95.2 0.05 (1)	6.25	S-C	-117 / 0	0.02 (1)
D-E	-18 / 0	-95.2	-95.2 0.05 (1)	6.25	O-G	-214 / 0	0.11 (1)
E-F	-27 / 0	-95.2	-95.2 0.05 (1)	6.25	N-H	-195 / 0	0.06 (1)
F-G	-27 / 0	-95.2	-95.2 0.05 (1)	6.25	M-I	-117 / 0	0.02 (1)
G-H	-18 / 0	-95.2	-95.2 0.05 (1)	6.25	B-S	0 / 29	0.01 (1)
H-I	-19 / 0	-95.2	-95.2 0.05 (1)	6.25	M-J	0 / 29	0.01 (1)
I-J	-58 / 0	-95.2	-95.2 0.12 (1)	6.25			
J-K	0 / 39	-95.2	-95.2 0.13 (1)	10.00			
L-J	-292 / 0	0.0	0.0 0.03 (1)	7.81			
T-S	0 / 0	-18.5	-18.5 0.02 (4)	10.00			
S-R	0 / 20	-18.5	-18.5 0.02 (4)	10.00			
R-Q	0 / 16	-18.5	-18.5 0.02 (4)	10.00			
Q-P	0 / 12	-18.5	-18.5 0.02 (4)	10.00			
P-O	0 / 12	-18.5	-18.5 0.02 (4)	10.00			
O-N	0 / 16	-18.5	-18.5 0.02 (4)	10.00			
N-M	0 / 20	-18.5	-18.5 0.02 (4)	10.00			
M-L	0 / 0	-18.5	-18.5 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 33.4 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (J-K:1) , BC=0.02/1.00 (R-S:4) , WB=0.13/1.00 (F-P:1) , SSI=0.08/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

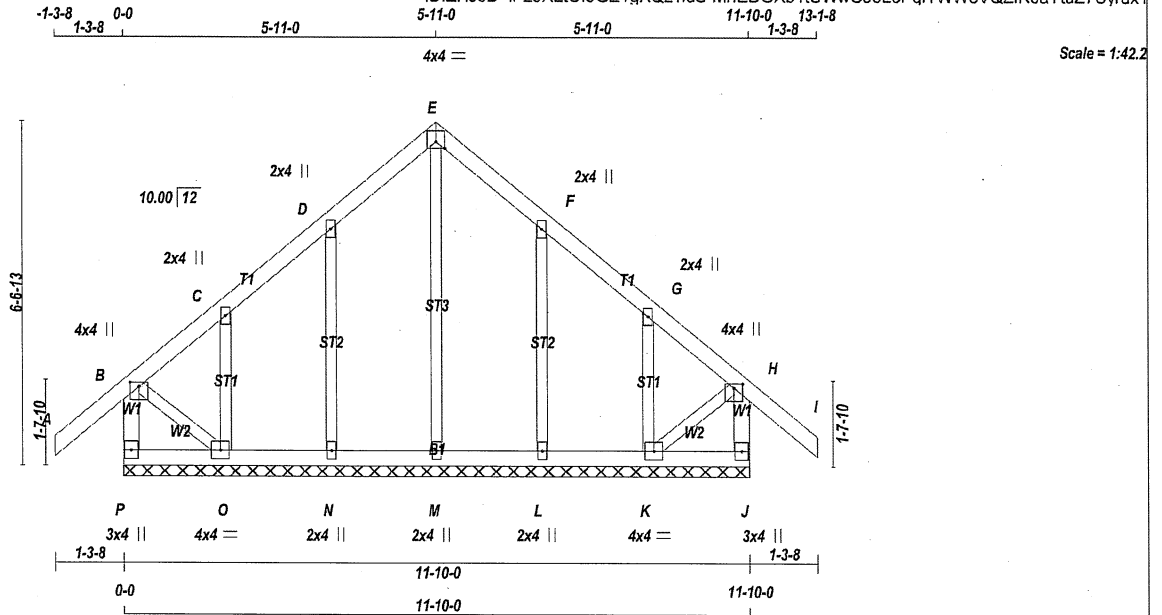
JSI GRIP= 0.22 (J) (INPUT = 0.90)
JSI METAL = 0.11 (E) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080207

JOB NAME 427579	TRUSS NAME G2	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:ZHoD iPz9XLtO9GE1gXQz1idS-MrEDOXb1tUWwC95LePqiTWWeVQZfK0aYtuZ7UyruX1



TOTAL WEIGHT = 58 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L, M, N						
L	BMV1+w	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-291 / 0	0.0	0.03 (1)	M-E	-139 / 0	0.09 (1)	
A-B	0 / 42	-95.2	-95.2 0.13 (1)	N-D	-228 / 0	0.08 (1)	
B-C	-53 / 0	-95.2	-95.2 0.12 (1)	O-C	-119 / 0	0.02 (1)	
C-D	-10 / 0	-95.2	-95.2 0.06 (1)	L-F	-228 / 0	0.08 (1)	
D-E	-28 / 0	-95.2	-95.2 0.06 (1)	K-G	-119 / 0	0.02 (1)	
E-F	-28 / 0	-95.2	-95.2 0.06 (1)	B-O	0 / 23	0.01 (1)	
F-G	-10 / 0	-95.2	-95.2 0.06 (1)	K-H	0 / 23	0.01 (1)	
G-H	-53 / 0	-95.2	-95.2 0.12 (1)				
H-I	0 / 42	-95.2	-95.2 0.13 (1)				
J-H	-291 / 0	0.0	0.03 (1)				
P-O	0 / 0	-18.5	-18.5 0.02 (4)				
O-N	0 / 15	-18.5	-18.5 0.02 (4)				
N-M	0 / 11	-18.5	-18.5 0.02 (4)				
M-L	0 / 11	-18.5	-18.5 0.02 (4)				
L-K	0 / 15	-18.5	-18.5 0.02 (4)				
K-J	0 / 0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (N-O:4), WB=0.09/1.00 (E-M:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

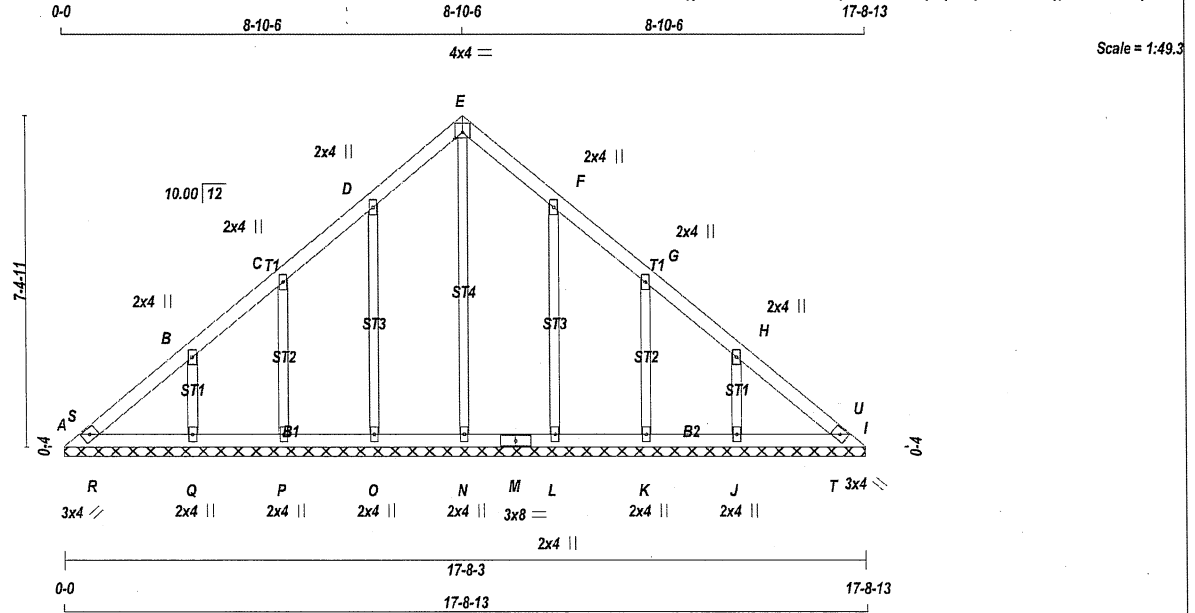
JSI GRIP= 0.22 (E) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080208

JOB NAME 427579	TRUSS NAME V1	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:ZHJoD iPz9XLtOf9GE1qXQz1IdS-Qk3vMWj?VUNODVp jIBLpee5nYXJg5do?i0r87yruwo



TOTAL WEIGHT = 68 lb [M]

LUMBER	DESCR.
N.L.G.A. RULES	
CHORDS	
A - E 2x4 DRY No.2 SPF	
E - I 2x4 DRY No.2 SPF	
A - M 2x4 DRY No.2 SPF	
M - I 2x4 DRY No.2 SPF	
ALL WEBS 2x3 DRY No.2 SPF	
DRY: SEASONED LUMBER.	

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TBM1-h MT20	3.0	4.0		
B, C, D, F, G, H				
B TMW+w MT20	2.0	4.0		
E TTW-p MT20	4.0	4.0	1.50	2.00
I TBM1-h MT20	3.0	4.0		
J, K, L, N, O, P, Q				
J BMW1+w MT20	2.0	4.0		
M BS-t MT20	3.0	8.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	135	0	135	0	17-8-3 (7-8-7)	17-8-3
I	135	0	135	0	17-8-3 (7-8-7)	17-8-3
N	208	0	208	0	17-8-3 (7-8-7)	17-8-3
O	263	0	263	0	17-8-3 (7-8-7)	17-8-3
P	183	0	183	0	17-8-3 (7-8-7)	17-8-3
Q	320	0	320	0	17-8-3 (7-8-7)	17-8-3
L	263	0	263	0	17-8-3 (7-8-7)	17-8-3
K	183	0	183	0	17-8-3 (7-8-7)	17-8-3
J	320	0	320	0	17-8-3 (7-8-7)	17-8-3

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	95	66 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
I	95	66 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
N	148	92 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0
O	185	126 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
P	129	85 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
Q	226	150 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0
L	185	126 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
K	129	85 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0
J	226	150 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, I, N, O, P, Q, L, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FR-TO	PLF			FR-TO			
A-S	-22 / 0	-95.2	-95.2	0.02 (1)	6.25	N-E	-174 / 0	0.16 (1)	
S-B	0 / 10	-95.2	-95.2	0.07 (1)	10.00	O-D	-220 / 0	0.11 (1)	
B-C	-7 / 4	-95.2	-95.2	0.07 (1)	10.00	P-C	-165 / 0	0.04 (1)	
C-D	0 / 12	-95.2	-95.2	0.06 (1)	10.00	Q-B	-231 / 0	0.04 (1)	
D-E	0 / 6	-95.2	-95.2	0.06 (1)	10.00	L-F	-220 / 0	0.11 (1)	
E-F	0 / 6	-95.2	-95.2	0.06 (1)	10.00	K-G	-165 / 0	0.04 (1)	
F-G	0 / 12	-95.2	-95.2	0.06 (1)	10.00	J-H	-231 / 0	0.04 (1)	
G-H	-7 / 4	-95.2	-95.2	0.07 (1)	10.00	R-S	-79 / 0	0.00 (1)	
H-U	0 / 10	-95.2	-95.2	0.07 (1)	10.00	T-U	-79 / 0	0.00 (1)	
U-I	-22 / 0	-95.2	-95.2	0.02 (1)	6.25				
A-R	-7 / 0	-18.5	-18.5	0.07 (1)	10.00				
R-Q	-3 / 10	-18.5	-18.5	0.07 (1)	10.00				
Q-P	-5 / 0	-18.5	-18.5	0.04 (1)	10.00				
P-O	-6 / 0	-18.5	-18.5	0.02 (4)	10.00				
O-N	-9 / 0	-18.5	-18.5	0.02 (4)	10.00				
N-M	-9 / 0	-18.5	-18.5	0.02 (4)	10.00				
M-L	-9 / 0	-18.5	-18.5	0.02 (4)	10.00				
L-K	-6 / 0	-18.5	-18.5	0.02 (4)	10.00				
K-J	-5 / 0	-18.5	-18.5	0.04 (1)	10.00				
J-T	-3 / 10	-18.5	-18.5	0.07 (1)	10.00				
T-I	-7 / 0	-18.5	-18.5	0.07 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (H-U:1), BC=0.07/1.00 (J-T:1), WB=0.16/1.00 (E-N:1), SSI=0.08/1.00 (H-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080227 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427579	V1	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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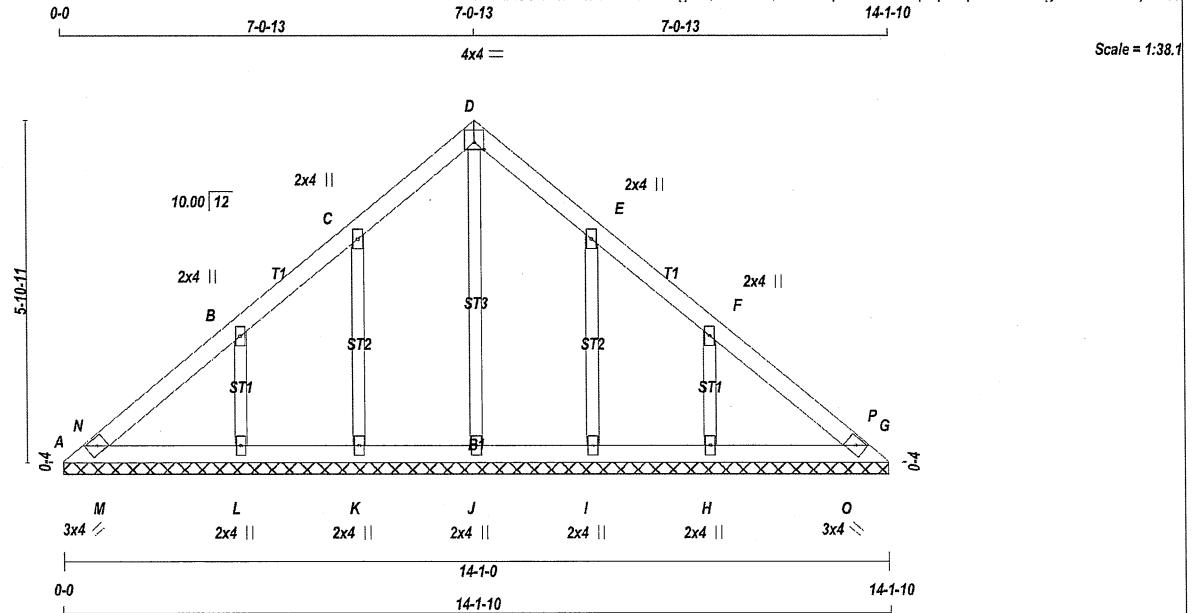


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
427579	V2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 49 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2	SPF
	D - G	2x4	DRY	No.2	SPF
	A - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	3.0	4.0				
B, C, E, F							
B TMW+w	MT20	2.0	4.0				
D TTW-p	MT20	4.0	4.0	1.50	2.00		
G TBM1-h	MT20	3.0	4.0				
H, I, J, K, L							
H BMW1+w	MT20	2.0	4.0				

NOTES- (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	131	0	131	0	0	14-1-0	14-1-0
A	131	0	131	0	0	14-1-0	14-1-0
G	257	0	257	0	0	14-1-0	14-1-0
K	212	0	212	0	0	14-1-0	14-1-0
L	329	0	329	0	0	14-1-0	14-1-0
I	212	0	212	0	0	14-1-0	14-1-0
H	329	0	329	0	0	14-1-0	14-1-0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	92	64 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
G	92	64 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
J	183	115 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
K	149	102 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
L	232	153 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0
I	149	102 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
H	232	153 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, K, L, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED				MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
			VERT. LOAD	LC1	MAX	UNBRAC			
FR-TO			FROM	TO	CSI (LC)	LENGTH	FR-TO		
A-N	0 / 5	-95.2	-95.2	0.02 (4)	10.00	J-D	-208 / 0	0.11 (1)	
N-B	0 / 34	-95.2	-95.2	0.08 (1)	10.00	K-C	-199 / 0	0.05 (1)	
B-C	0 / 22	-95.2	-95.2	0.08 (1)	10.00	L-B	-234 / 0	0.04 (1)	
C-D	0 / 23	-95.2	-95.2	0.05 (1)	10.00	I-E	-199 / 0	0.05 (1)	
D-E	0 / 23	-95.2	-95.2	0.05 (1)	10.00	H-F	-234 / 0	0.04 (1)	
E-F	0 / 22	-95.2	-95.2	0.08 (1)	10.00	M-N	-91 / 0	0.00 (1)	
F-P	0 / 34	-95.2	-95.2	0.08 (1)	10.00	O-P	-91 / 0	0.00 (1)	
P-G	0 / 5	-95.2	-95.2	0.02 (4)	10.00				
A-M	-29 / 0	-18.5	-18.5	0.07 (1)	6.25				
M-L	-11 / 0	-18.5	-18.5	0.07 (1)	6.25				
L-K	-22 / 0	-18.5	-18.5	0.05 (1)	6.25				
K-J	-27 / 0	-18.5	-18.5	0.02 (4)	6.25				
J-I	-27 / 0	-18.5	-18.5	0.02 (4)	6.25				
I-H	-22 / 0	-18.5	-18.5	0.05 (1)	6.25				
H-O	-11 / 0	-18.5	-18.5	0.07 (1)	6.25				
O-G	-29 / 0	-18.5	-18.5	0.07 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	40.1	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (B-N:1), BC=0.07/1.00 (G-O:1), WB=0.11/1.00 (D-J:1), SSI=0.08/1.00 (B-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)



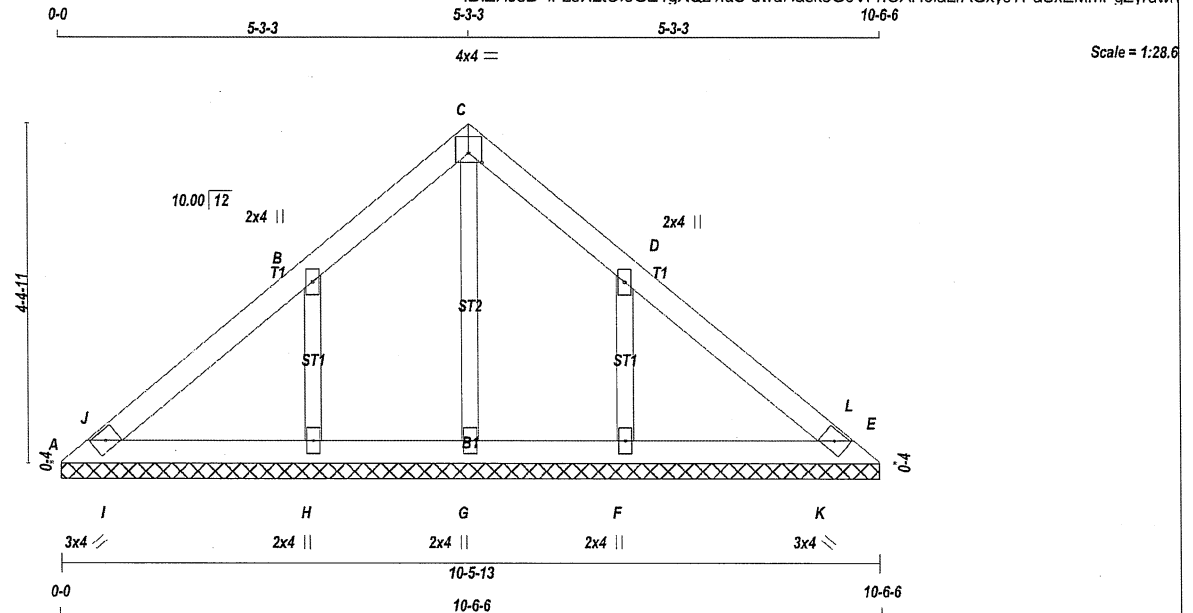
STRUCTURAL COMPONENT ONLY
DWG # TR22080228

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
427579	V3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 33 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMW-w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	112	0	112	0	10-5-13	10-5-13
E	112	0	112	0	10-5-13	10-5-13
G	219	0	219	0	10-5-13	10-5-13
H	374	0	374	0	10-5-13	10-5-13
F	374	0	374	0	10-5-13	10-5-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	79	54 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
E	79	54 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
G	156	98 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
H	264	177 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
F	264	177 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED	FACTORED			MAX. UNBRAC	MEMB.	MAX. FACTORED	MAX	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	FORCE (LBS)			FORCE (LBS)		
FR-TO		FROM	TO		LENGTH	FR-TO			
A- J	0 / 48	-95.2	-95.2	0.02 (4)	10.00	G- C	-228 / 0	0.06 (1)	
J- B	0 / 82	-95.2	-95.2	0.11 (1)	10.00	H- B	-274 / 0	0.05 (1)	
B- C	0 / 54	-95.2	-95.2	0.11 (1)	10.00	F- D	-274 / 0	0.05 (1)	
C- D	0 / 54	-95.2	-95.2	0.11 (1)	10.00	I- J	-94 / 2	0.00 (1)	
D- L	0 / 82	-95.2	-95.2	0.11 (1)	10.00	K- L	-94 / 2	0.00 (1)	
L- E	0 / 48	-95.2	-95.2	0.02 (4)	10.00				

A - I	-65 / 0	-18.5	-18.5	0.07 (1)	6.25
I - H	-47 / 0	-18.5	-18.5	0.07 (1)	6.25
H - G	-58 / 0	-18.5	-18.5	0.05 (1)	6.25
G - F	-58 / 0	-18.5	-18.5	0.05 (1)	6.25
F - K	-47 / 0	-18.5	-18.5	0.07 (1)	6.25
K - E	-65 / 0	-18.5	-18.5	0.07 (1)	6.25

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (B-J:1), BC=0.07/1.00 (H-I:1), WB=0.06/1.00 (C-G:1), SSI=0.09/1.00 (D-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (C) (INPUT = 0.90)

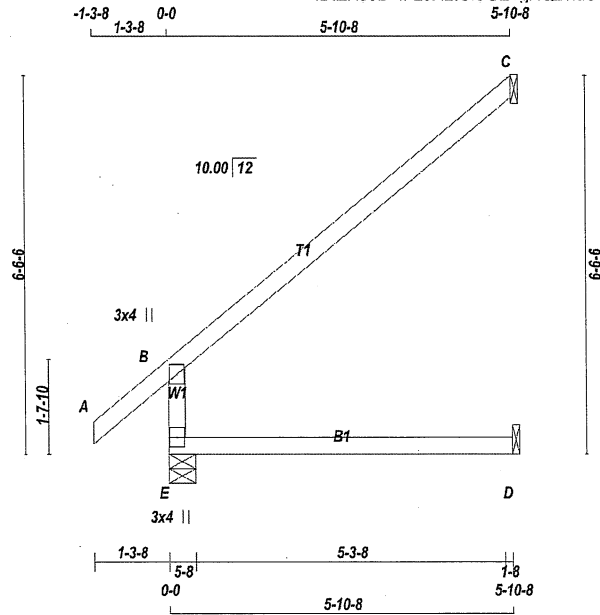
JSI METAL= 0.15 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080229

JOB NAME 427579	TRUSS NAME J1	QUANTITY 10	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.530 S Feb 23 2022 Mitek Industries, Inc. Tue Aug 2 12:37:16 2022 Page 1
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Scale = 1:38.2

TOTAL WEIGHT = 10 X 19 = 190 lb

LUMBER				DESCR.	SPF
N. L. G. A. RULES	SIZE	LUMBER	No.2		
CHORDS					
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	544	0	544	0	5-8	5-8
C	210	0	210	0	1-8	1-8
D	46	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	381	270 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	144	118 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	37	0 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
		(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO			FROM TO		LENGTH	FR-TO		
E-B	-481 / 0		0.0	0.0	0.11 (4)	7.81		
A-B	0 / 42		-95.2	-95.2	0.13 (1)	10.00		
B-C	-45 / 0		-95.2	-95.2	0.56 (1)	6.25		
E-D	0 / 0		-18.5	-18.5	0.14 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.56/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)

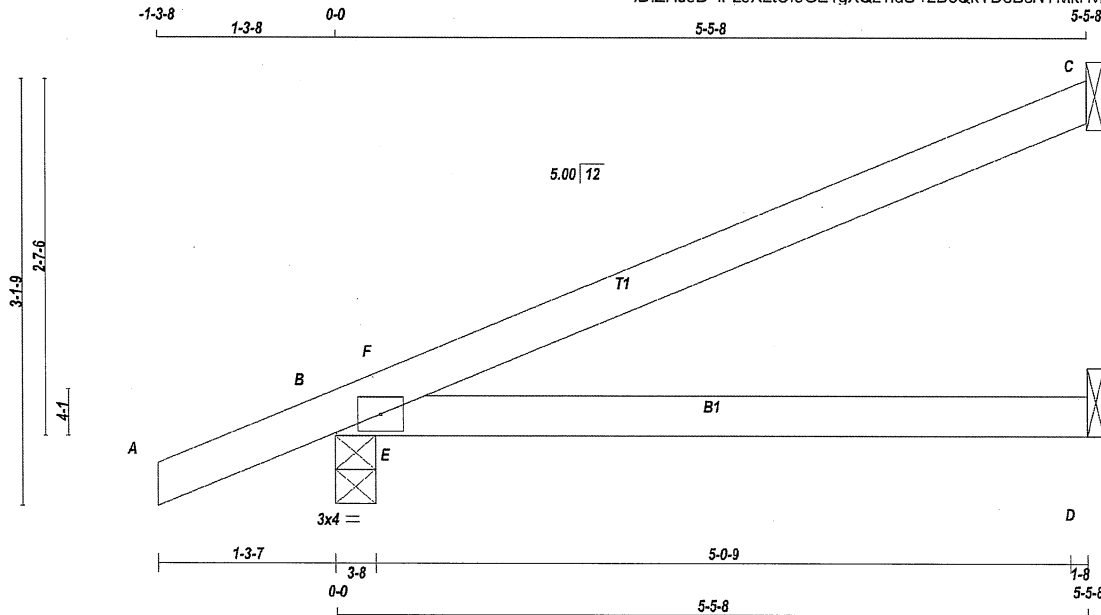
JSI METAL= 0.17 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080209

JOB NAME 427579	TRUSS NAME J3	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:ZHJoD iPz9XLtOf9GE1gXQz1idS-r2DcQkYDoBcNYMkHvMx3Eh3ehv6OojnXd6fxyrux0



TOTAL WEIGHT = 4 X 15 = 59 lb [M]

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
C	227	0	227	0	1-8	1-8
B	439	0	439	0	3-8 (3-7)	3-8
D	83	0	83	0	1-8	1-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
C	157	123 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	307	218 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	62	22 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX (LC)	
FR-TO				FR-TO			
A-B	0 / 23	-95.2	-95.2 0.12 (1)	E-F	-369 / 11	0.00 (1)	
B-F	-23 / 44	-95.2	-95.2 0.07 (1)				
F-C	-4 / 2	-95.2	-95.2 0.37 (1)				
B-E	0 / 0	-18.5	-18.5 0.25 (1)				
E-D	0 / 0	-18.5	-18.5 0.25 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/596 (0.11")

CSI: TC=0.37/1.00 (C-F:1), BC=0.25/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.30/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

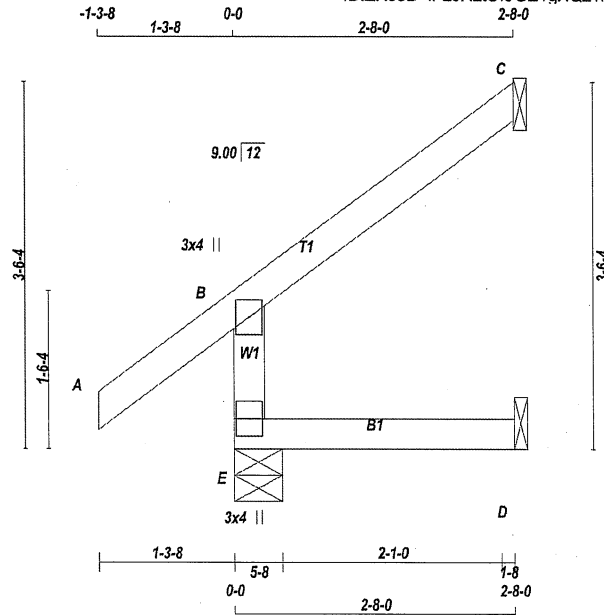
JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080210

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427579	J7	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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Scale = 1:21.2

TOTAL WEIGHT = 2 X 10 = 20 lb [M]

LUMBER

N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.	SPF	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
E	317	0	317	0
C	96	0	96	0
D	22	0	25	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED						
E	221	163 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
C	66	54 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	18	0 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED	CS1 (LC)
FR-TO		FROM TO		FR-TO			
E-B	-290 / 0	0.0	0.02 (4)	7.81			
A-B	0 / 39	-95.2	-95.2	0.13 (5)	10.00		
B-C	-19 / 0	-95.2	-95.2	0.11 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.03 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.03/1.00 (D-E:4)
WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080211

EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

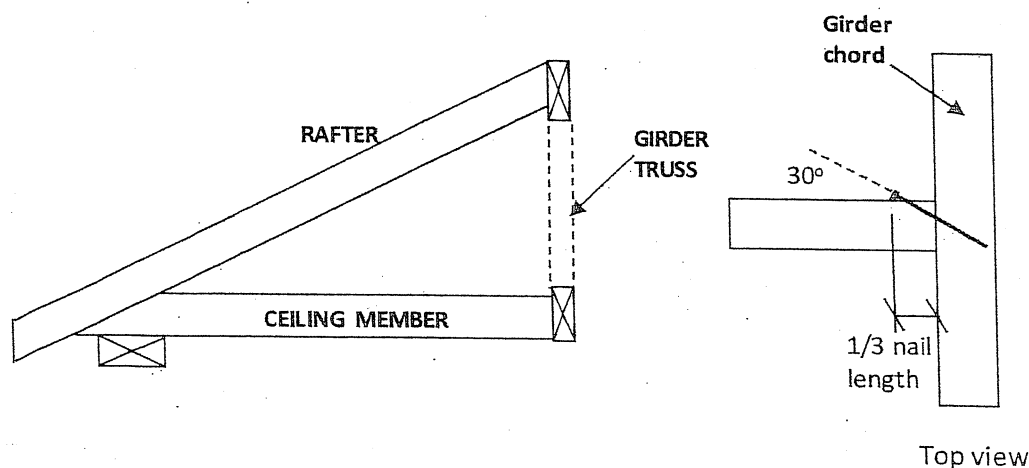
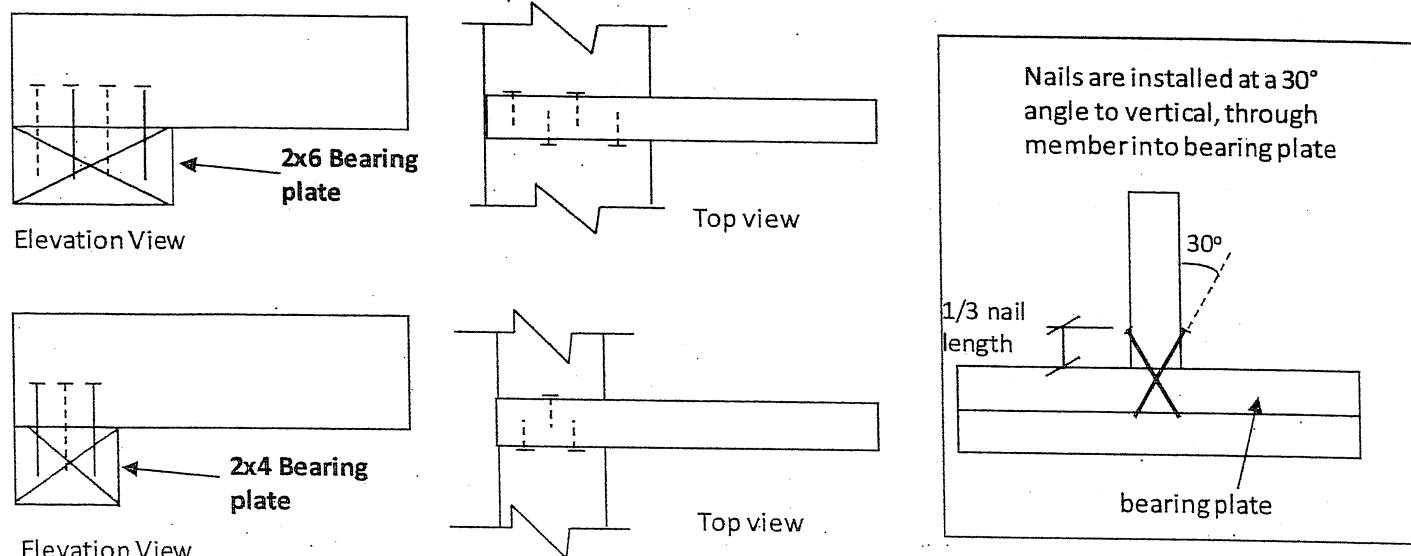


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_b factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

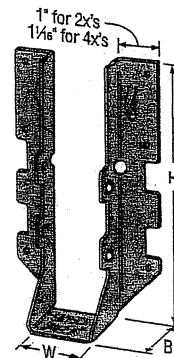
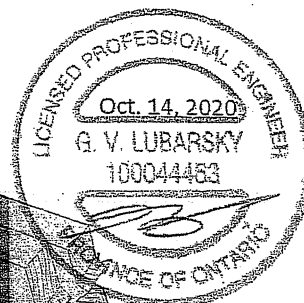
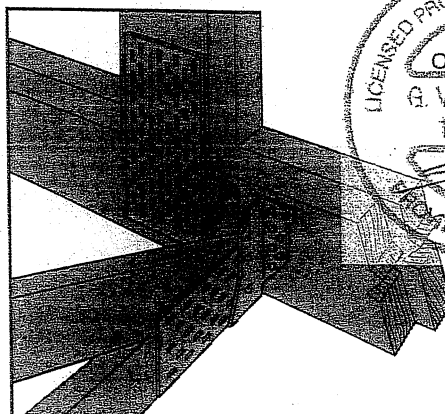


Double-Shear Nailing Side View; Do not bend tab

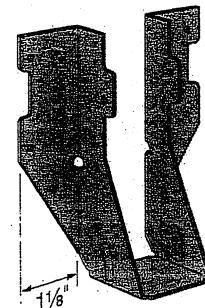


Dome Double-Shear Nailing Side View (available on some models)

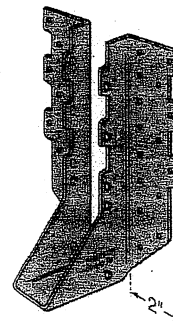
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



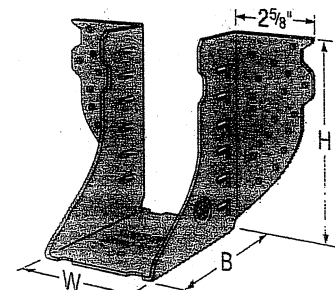
✓ LUS28



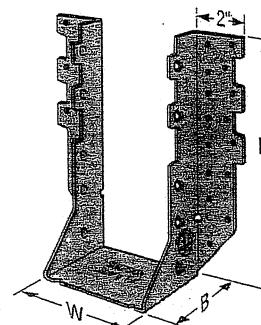
LU26L



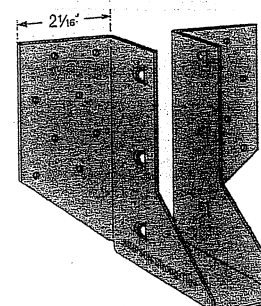
✓ HUS210
(HUS26, HUS28, and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



LJS26DS

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

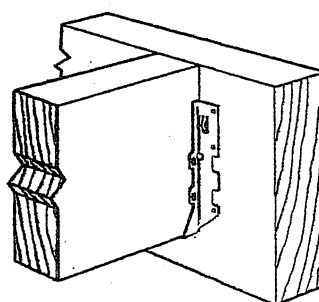
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

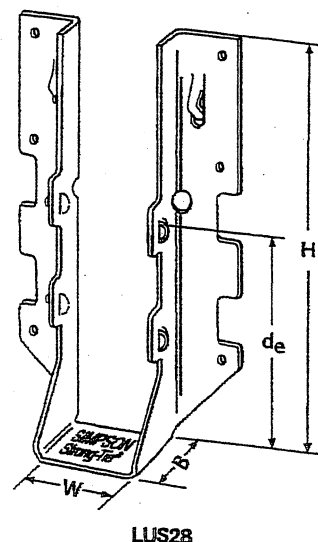
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

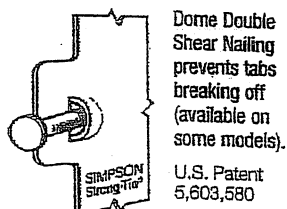


Typical LUS Installation

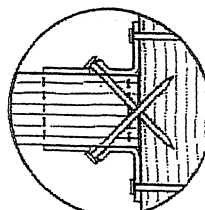


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1½	3½	1¾	1½	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7¾	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

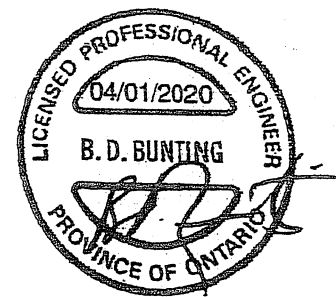
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

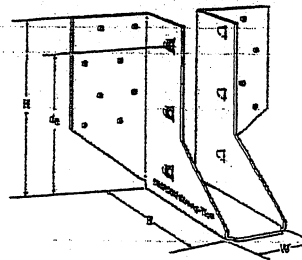
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

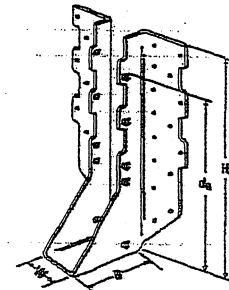
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

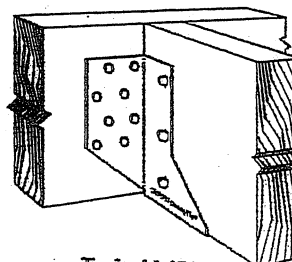
- See current catalogue for options



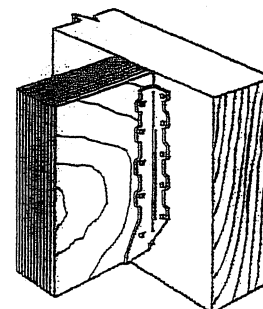
LJS26DS



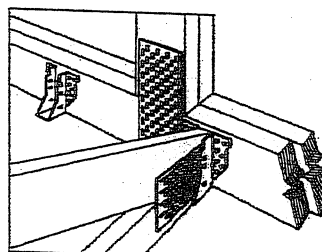
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



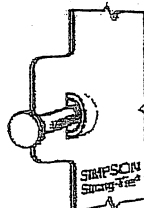
Typical HUS
Installation



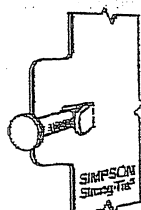
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

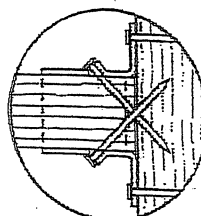
1. d_s is the distance from the seat of the hanger to the highest joist nail.



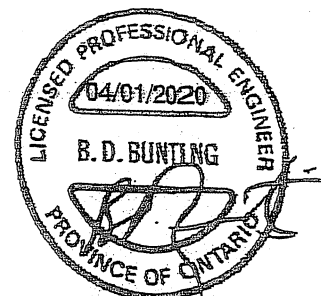
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

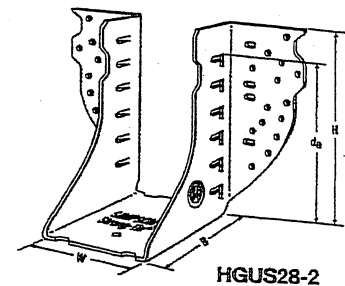
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

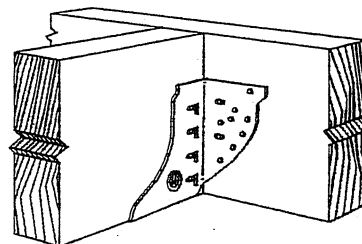
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

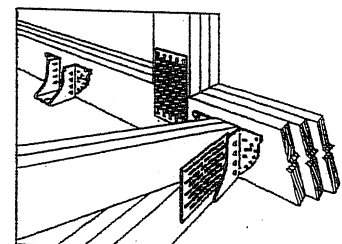
- See current catalogue for options



HGUS28-2



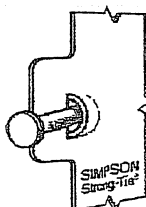
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

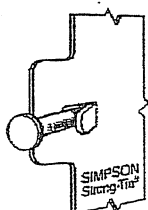
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift	Normal	Uplift	Normal
HGUS26	12	1 1/8	5 3/8	5	4 3/32	(20) 16d	(8) 16d		2685	6625	2685	5700
HGUS26-2	12	3 1/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-3	12	4 1/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-4	12	6 1/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d		3310	7675	3100	6900
HGUS28-2	12	3 1/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-3	12	4 1/16	7 1/4	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-4	12	6 1/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 7/8	(46) 16d	(16) 16d		3535	11070	2510	8090
HGUS210-2	12	3 1/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d		6840	14015	4855	10270
HGUS210-3	12	4 1/16	9 1/4	4	8 3/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-4	12	6 1/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS212-4	12	6 1/16	10 3/8	4	10 1/8	(56) 16d	(20) 16d		7640	14995	5425	10645
HGUS214-4	12	6 1/16	12 3/8	4	11 1/8	(66) 16d	(22) 16d		10130	16400	7195	11645

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

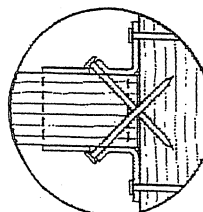


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

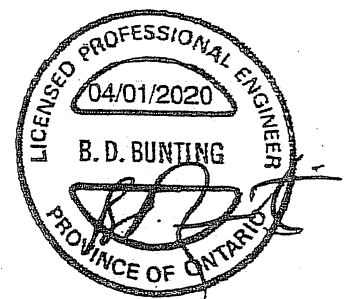
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

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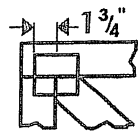
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T-SPECHGUS20 3/20 exp. 6/22

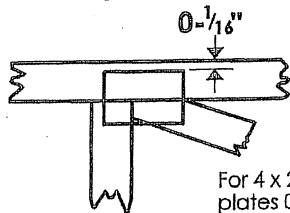
(800) 999-5099
strongtie.com

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

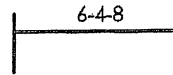


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

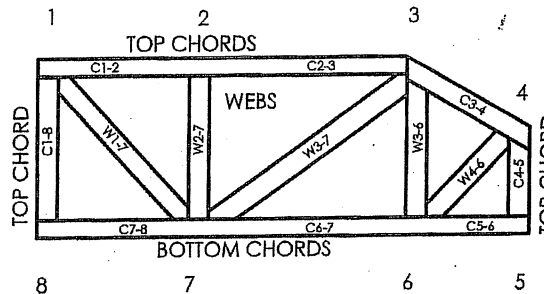
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: M11-7473C rev. 10-'08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.