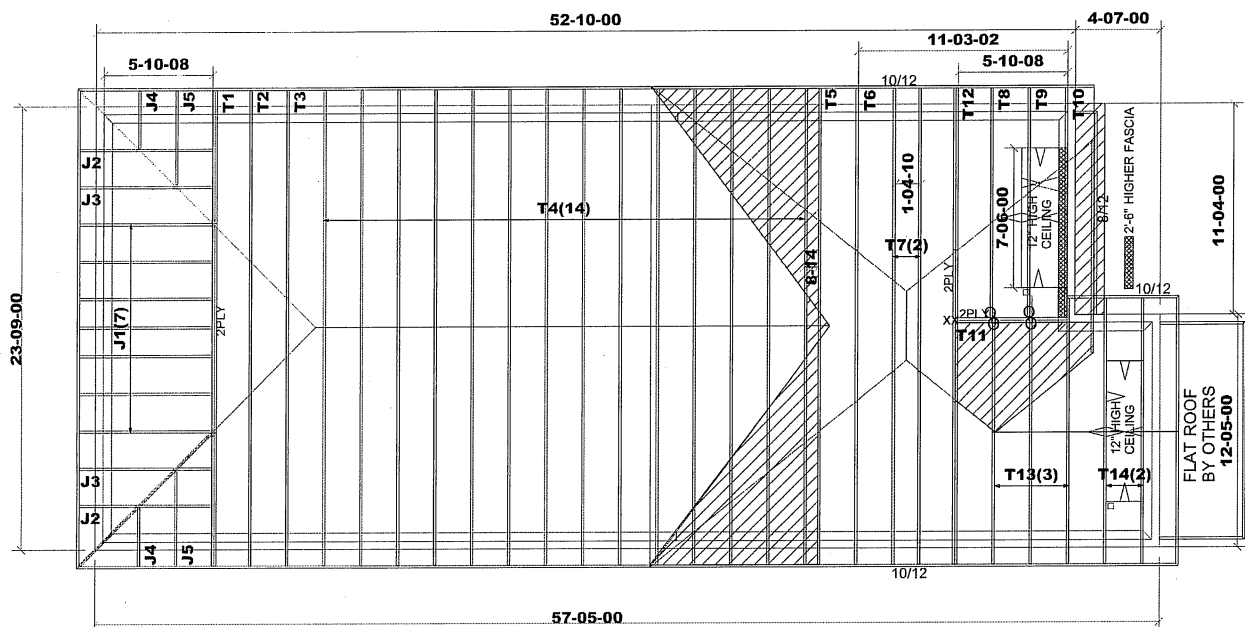




FINISH O.H: 12"
 Heel: R.T.M.C
 CLADDING ALLOWANCE: 5"
 2X6 EXTERIOR WALLS
 ASPHALT SHINGLES
 2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
 ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c. WITH A 2"x4" SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance between end points and between rows of bracing does not exceed 6'.

TRUSSES DESIGNED CONFORM TO THE RELEVANT SECTION OF THE LATEST EDITION OF THE ONTARIO BUILDING CODE (RESIDENTIAL PART 9)

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
 OCCUPANCY: RESIDENTIAL | PART: 9
 Ss = 33.4 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCsL = 26.7 psf
 TCdL = 6.0 psf
 BCdL = 0.0 psf
 BCDL = 7.4 psf

HARDWARE
 LUS24(O)
 LJS26DS(V)
 LUS26-2(VV)
 HGUS26-2(XX)

ALL ROOF PITCHES ARE 6/12 UNLESS OTHERWISE NOTED

 DENOTES:
 CONVENTIONAL FRAMING

DWG# TR22080111 TO TR22080129



STRUCTURAL COMPONENTS ONLY
 DWG# TR22080157

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer unless otherwise noted in this plan. Building designer to review and approve this plan to ascertain conformity to his overall structural plan.



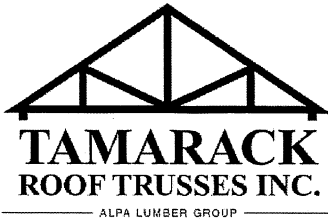
Job Track: **52956**
 Layout ID: **427503**
 Plan Log: **206390**

Builder / Location: **GREENPARK HOMES / CAMBRIDGE**
 Project: **BARLASSINA**
 Date: 6/02/22 Designer: AT

Model / Elevation: **GARDEN 3 / 1**
 THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
 Milek ver 8.5.3.233

M15037

DELIVERY SHIPLIST



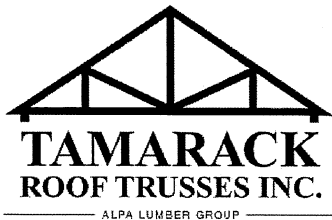
Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: GARDEN 3
 Lot #:
 Elevation: 1

Job Track: 52956
 PlanLog: 206390
 Layout ID: 427503
 Ref #
 Page: 1 of 2
 Date: 08-03-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	22-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	203.69 128.00		
	1	T2 Hip	6 /12	22-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	94.14 59.33		
	1	T3 Hip	6 /12	22-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	96.52 60.17		
	14	T4 Common	6 /12	22-11-00	6-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1280.52 807.33		
	1	T5 Hip	10 /12	22-11-00	6-05-13	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	99.98 62.33		
	1	T6 Hip	10 /12	22-11-00	7-09-13	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	113.07 73.00		
	2	T7 Hip	10 /12	22-11-00	9-01-13	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	225.15 140.67		
	1	T8 Half Hip	10 /12	11-01-00	6-06-13	2 x 4	1-03-08	1-07-10 6-06-13	56.02 36.00		
	1	T9 Half Hip	10 /12	11-01-00	5-02-13	2 x 4	1-03-08	1-07-10 5-02-13	62.11 40.67		
	1	T10 GABLE	10 /12	11-01-00	3-10-13	2 x 4	1-03-08	1-07-10 3-10-13	58.77 39.83		
	1 2-ply	T11 Jack-Open Girder	8 /12	5-10-08	7-09-13	2 x 4 2 x 6		3-10-13 7-09-13	85.52 55.67		
	1 2-ply	T12 Hip Girder	10 /12	22-11-00	7-09-13	2 x 4 2 x 6	1-03-08 1-03-08	1-07-10 1-07-10	253.16 161.33		
	3	T13 Common	10 /12	11-07-00	6-05-09	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	153.8 99.00		
	2	T14 Roof Special	10 /12	11-07-00	6-05-09	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	121.05 83.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: BARLASSINA
 Location: CAMBRIDGE
 Model: GARDEN 3
 Lot #:
 Elevation: 1

Job Track: 52956
 PlanLog: 206390
 Layout ID: 427503
 Ref #
 Page: 2 of 2
 Date: 08-03-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	7	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	117.56 74.67		
	2	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		

TOTAL # TRUSS= 49

TOTAL BFT OF ALL TRUSSES= 1974.33

BFT.

TOTAL WEIGHT OF ALL TRSSES 3105.68 LBS

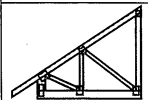
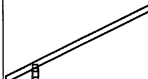
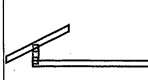
HARDWARE

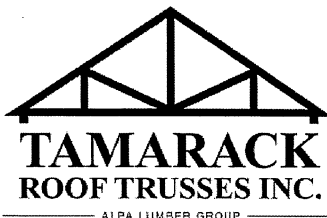
QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 5

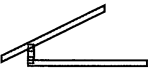
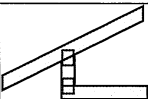
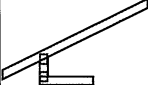
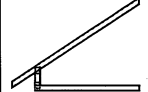
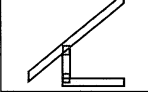
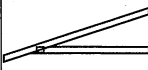
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
	Builder:	GREENPARK HOMES	PlanLog:	206390
	Project:	BARLASSINA	Layout ID:	427510
	Location:	CAMBRIDGE	Ref #	
Model:	GARDEN 3	Page:	1 of 2	
Lot #:		Date:	08-03-2022	
Elevation:	2	Designer:		
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	22-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	203.69 128.00		
	1	T2 Hip	6 /12	22-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	94.14 59.33		
	1	T3 Hip	6 /12	22-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	96.52 60.17		
	13	T4 Common	6 /12	22-11-00	6-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1189.06 749.67		
	1 2-ply	T15 Hip Girder	10 /12	22-11-00	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-07-10 1-07-10	219.98 138.33		
	1	T15Z Hip	10 /12	22-11-00	5-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-07-10 1-07-10	109.99 69.17		
	2	T16 Hip	10 /12	22-11-00	6-07-13	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	212.06 134.33		
	2	T17 Hip	10 /12	22-11-00	7-11-13	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	228.27 144.67		
	1 2-ply	T18 Jack-Open Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6	1-05-00	1-04-13 5-03-13	68.4 42.67		
	3	T19 Common	10 /12	11-07-00	7-11-09	2 x 4	1-03-08 1-03-08	3-01-10 3-01-10	170.21 109.00		
	1	T20 Hip	10 /12	11-07-00	6-08-15	2 x 4	1-03-08 1-03-08	3-01-10 3-01-10	63.34 40.17		
	1	T21 Hip Girder	10 /12	11-07-00	3-06-15	2 x 4	1-03-08 1-03-08	1-07-10 1-07-10	53.43 34.50		
	7	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	117.56 74.67		
	2	J2 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: BARLASSINA Location: CAMBRIDGE Model: GARDEN 3 Lot #: Elevation: 2	Job Track: 52956 PlanLog: 206390 Layout ID: 427510 Ref # Page: 2 of 2 Date: 08-03-2022 Designer: Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	5	J6 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	89.15 53.33		
	5	J7 Jack-Open	10 /12	2-04-00	3-06-15	2 x 4	1-03-08	1-07-10 3-06-15	48.48 33.33		
	6	J10 Monopitch	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	86.69 56.00		

TOTAL # TRUSS= 62 TOTAL BFT OF ALL TRUSSES= 1980.67 BFT. TOTAL WEIGHT OF ALL TRSSES 3135.58 LBS

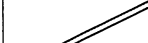
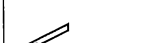
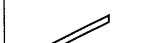
HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 4

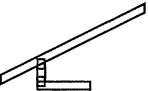
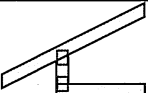
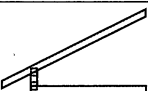
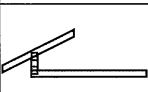
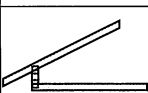
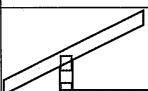
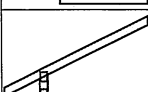
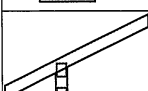
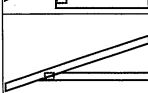
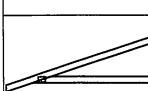
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
	Builder:	GREENPARK HOMES	PlanLog:	206390
	Project:	BARLASSINA	Layout ID:	427527
	Location:	CAMBRIDGE	Ref #	
Model:	GARDEN 3	Page:	1 of 2	
Lot #:		Date:	08-03-2022	
Elevation:	3	Designer:		
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T2 Hip	6 /12	22-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	188.29 118.67		
	2	T3 Hip	6 /12	22-11-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	193.05 120.33		
	16	T4 Common	6 /12	22-11-00	6-10-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1463.45 922.67		
	1 2-ply	T22 Hip Girder	6 /12	22-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	209.31 132.00		
	1 2-ply	T22Z Hip Girder	6 /12	22-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	209.31 132.00		
	1 2-ply	T23 Half Hip Girder	6 /12	5-10-08	2-02-08	2 x 4 2 x 6	1-03-08	1-02-00 2-02-08	55.23 35.33		
	1 2-ply	T24 Hip Girder	6 /12	20-07-00	3-08-12	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	180.25 110.33		
	1 2-ply	T25 Jack-Open Girder	6 /12	5-01-08	3-08-12	2 x 4 2 x 6		1-02-00 3-08-12	47.87 31.00		
	3	T26 Common	6 /12	11-07-00	4-00-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	134.56 88.00		
	1	T27 Hip Girder	6 /12	11-07-00	3-03-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	52.64 37.00		
	7	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	117.56 74.67		
	2	J2 Jack-Open Girder	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 52956	
	Builder: GREENPARK HOMES						PlanLog: 206390	
	Project: BARLASSINA						Layout ID: 427527	
	Location: CAMBRIDGE						Ref #	
	Model: GARDEN 3						Page: 2 of 2	
Lot #:						Date: 08-03-2022		
Elevation: 3						Designer:		
						Sales Rep: Rick DiCiano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J21 Jack-Open	6 /12	2-01-00	2-02-08	2 x 4	1-03-08	1-02-00 2-02-08	15.27 10.67		
	2	J22 Jack-Open	6 /12	5-01-08	3-08-12	2 x 4	1-03-08	1-02-00 3-08-12	29.97 20.00		
	1	J23 Jack-Open	6 /12	1-10-15	2-01-08	2 x 4	1-03-08 3-02-09	1-02-00 2-01-08	10.89 7.33		
	1	J24 Jack-Open	6 /12	3-10-15	3-01-08	2 x 4	1-03-08 1-02-09	1-02-00 3-01-08	13.44 8.67		
	1	J25 Jack-Open	6 /12	2-00-00	2-01-08	2 x 4	1-03-08	1-02-00 2-02-00	7.32 4.67		
	1	J26 Jack-Open	6 /12	2-00-00	3-01-08	2 x 4	1-03-08 1-10-15	1-02-00 2-02-00	9.87 6.00		
	4	J27 Jack-Open	6 /12	2-04-00	2-04-00	2 x 4	1-03-08	1-02-00 2-04-00	32.96 24.00		
	5	J28 Jack-Open	4 /12	3-04-00	2-01-11	2 x 4	1-03-08 10-08	3-15 1-05-04	57.6 36.67		
	2	J29 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	28.9 18.67		

TOTAL # TRUSS= 68 TOTAL BFT OF ALL TRUSSES= 1992.01 BFT. TOTAL WEIGHT OF ALL TRSSES 3142.35 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
9	Hardware	LUS24	

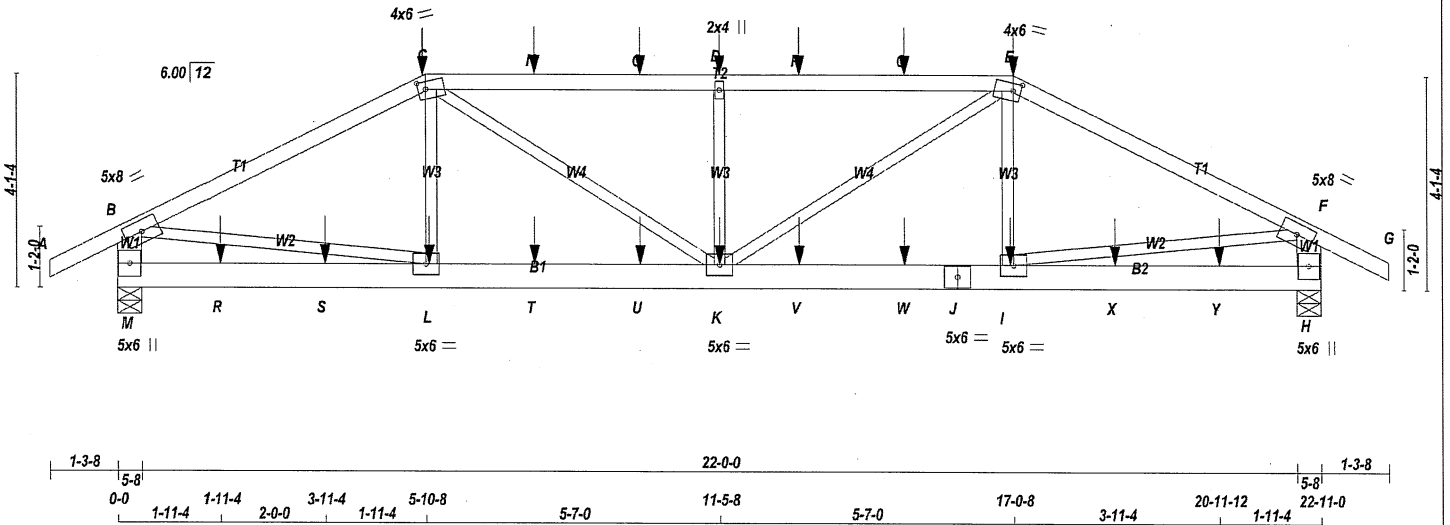
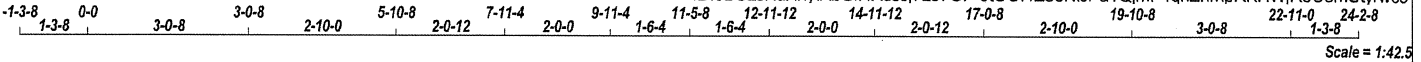
TOTAL NUMBER OF ITEMS= 12

JOB NAME 427503	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:9BS2JKdXhYlAbOiHRsssi7z37OF-6tOOI4Z3eNiJPa1QimP4qhZnmpRKHWjR0C5mUtyrwc9



TOTAL WEIGHT = 2 X 102 = 204 lb [M]

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
M - J	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF

ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
M - B	2x6	DRY No.2	SPF
H - F	2x6	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-E	12	SIDE(61.0)
E-G	12	SIDE(61.0)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M-J	12	SIDE(183.1)
J-H	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
L-C	6	SIDE(2.3)
I-E	6	SIDE(2.3)
2x3	6	
2x6	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORIZ	DOWN	HORIZ
M	2334	0	2334	0
H	2334	0	2334	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN.	COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE			
M	1846	1105 / 0	0 / 0	0 / 0	541 / 0	0 / 0
H	1846	1105 / 0	0 / 0	0 / 0	541 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 34	-95.2 -95.2	0.09 (1)	10.00	B-L	0 / 2842	0.35 (1)
B-C	-3142 / 0	-95.2 -95.2	0.37 (1)	4.84	L-C	-230 / 85	0.03 (1)
C-N	-3726 / 0	-95.2 -95.2	0.52 (1)	4.28	C-K	0 / 1110	0.14 (1)
N-O	-3726 / 0	-95.2 -95.2	0.52 (1)	4.28	K-D	-1081 / 0	0.14 (1)
O-D	-3726 / 0	-95.2 -95.2	0.52 (1)	4.28	K-E	0 / 1110	0.14 (1)
D-P	-3726 / 0	-95.2 -95.2	0.52 (1)	4.28	I-E	-230 / 85	0.03 (1)
P-Q	-3726 / 0	-95.2 -95.2	0.52 (1)	4.28	I-F	0 / 2842	0.35 (1)
Q-E	-3726 / 0	-95.2 -95.2	0.52 (1)	4.28	M-B	-2263 / 0	0.08 (1)
E-F	-3142 / 0	-95.2 -95.2	0.37 (1)	4.84	H-F	-2263 / 0	0.08 (1)
F-G	0 / 34	-95.2 -95.2	0.09 (1)	10.00			
M-R	0 / 0	-18.5 -18.5	0.06 (4)	10.00			
R-S	0 / 0	-18.5 -18.5	0.06 (4)	10.00			
S-L	0 / 0	-18.5 -18.5	0.06 (4)	10.00			
L-T	0 / 2802	-18.5 -18.5	0.23 (1)	10.00			
T-U	0 / 2802	-18.5 -18.5	0.23 (1)	10.00			
U-K	0 / 2802	-18.5 -18.5	0.23 (1)	10.00			
K-V	0 / 2802	-18.5 -18.5	0.23 (1)	10.00			
V-W	0 / 2802	-18.5 -18.5	0.23 (1)	10.00			
W-J	0 / 2802	-18.5 -18.5	0.23 (1)	10.00			
J-I	0 / 2802	-18.5 -18.5	0.23 (1)	10.00			
I-X	0 / 0	-18.5 -18.5	0.06 (4)	10.00			
X-Y	0 / 0	-18.5 -18.5	0.06 (4)	10.00			
Y-H	0 / 0	-18.5 -18.5	0.06 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-329	-329	---	FRONT	VERT	TOTAL	---	C1
D	11-5-8	-79	-79	---	FRONT	VERT	TOTAL	---	C1
E	17-0-8	-329	-329	---	FRONT	VERT	TOTAL	---	C1
I	16-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
K	11-5-8	-21	-21	---	FRONT	VERT	TOTAL	---	C1
L	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	7-11-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
O	9-11-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
P	12-11-12	-79	-79	---	FRONT	VERT	TOTAL	---	C1
Q	14-11-12	-79	-79	---	FRONT	VERT	TOTAL	---	C1
R	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
S	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.52/1.00 (D-E-1), BC=0.23/1.00 (I-K-1), WB=0.35/1.00 (F-I-1), SS=0.21/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080119 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																																																																																																																																										
427503	T1	1	2	GREENPARK HOMES																																																																																																																																																											
Tamarack Roof Truss, Burlington				TRUSS DESC.																																																																																																																																																											
Version 8.530 S Feb 23 2022 MITek Industries, Inc. Tue Aug 2 10:43:00 2022 Page 2																																																																																																																																																															
ID:9BS2JKdXhylAbOiHRsssi7z37OF-6tOOI4Z3eNiJPa1QjmP4qhZnmpRKHwJfR0C5mUtyrwc9																																																																																																																																																															
PLATES (table is in inches) <table><tr><td>JT</td><td>TYPE</td><td>PLATES</td><td>W</td><td>LEN</td><td>Y</td><td>X</td></tr><tr><td>B</td><td>TMWW-t</td><td>MT20</td><td>5.0</td><td>8.0</td><td></td><td></td></tr><tr><td>C</td><td>TTWW-m</td><td>MT20</td><td>4.0</td><td>6.0</td><td>1.75</td><td>1.75</td></tr><tr><td>D</td><td>TMWW-w</td><td>MT20</td><td>2.0</td><td>4.0</td><td></td><td></td></tr><tr><td>E</td><td>TTWW-m</td><td>MT20</td><td>4.0</td><td>6.0</td><td>1.75</td><td>1.75</td></tr><tr><td>F</td><td>TMWW-t</td><td>MT20</td><td>5.0</td><td>8.0</td><td></td><td></td></tr><tr><td>H</td><td>BMW1-w</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>I</td><td>BMWW-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>J</td><td>BS-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>K</td><td>BMWWW-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>L</td><td>BMWW-t</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>M</td><td>BMW1-w</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr></table> NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2. SPECIFIED CONCENTRATED LOADS (LBS) <table><tr><td>JT</td><td>LOC.</td><td>LC1</td><td>MAX-</td><td>MAX+</td><td>FACE</td><td>DIR.</td><td>TYPE</td><td>HEEL</td><td>CONN.</td></tr><tr><td>T</td><td>7-11-4</td><td>-21</td><td>-21</td><td>—</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>—</td><td>C1</td></tr><tr><td>U</td><td>9-11-4</td><td>-21</td><td>-21</td><td>—</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>—</td><td>C1</td></tr><tr><td>V</td><td>12-11-12</td><td>-21</td><td>-21</td><td>—</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>—</td><td>C1</td></tr><tr><td>W</td><td>14-11-12</td><td>-21</td><td>-21</td><td>—</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>—</td><td>C1</td></tr><tr><td>X</td><td>18-11-12</td><td>-21</td><td>-21</td><td>—</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>—</td><td>C1</td></tr><tr><td>Y</td><td>20-11-12</td><td>-20</td><td>-20</td><td>—</td><td>FRONT</td><td>VERT</td><td>TOTAL</td><td>—</td><td>C1</td></tr></table> CONNECTION REQUIREMENTS 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED. LICENSED PROFESSIONAL ENGINEER 8/02/22 C. M. HEYENS 100505065  PROVINCE OF ONTARIO STRUCTURAL COMPONENT ONLY DWG # TR22080119 PG 2/2						JT	TYPE	PLATES	W	LEN	Y	X	B	TMWW-t	MT20	5.0	8.0			C	TTWW-m	MT20	4.0	6.0	1.75	1.75	D	TMWW-w	MT20	2.0	4.0			E	TTWW-m	MT20	4.0	6.0	1.75	1.75	F	TMWW-t	MT20	5.0	8.0			H	BMW1-w	MT20	5.0	6.0			I	BMWW-t	MT20	5.0	6.0			J	BS-t	MT20	5.0	6.0			K	BMWWW-t	MT20	5.0	6.0			L	BMWW-t	MT20	5.0	6.0			M	BMW1-w	MT20	5.0	6.0			JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	T	7-11-4	-21	-21	—	FRONT	VERT	TOTAL	—	C1	U	9-11-4	-21	-21	—	FRONT	VERT	TOTAL	—	C1	V	12-11-12	-21	-21	—	FRONT	VERT	TOTAL	—	C1	W	14-11-12	-21	-21	—	FRONT	VERT	TOTAL	—	C1	X	18-11-12	-21	-21	—	FRONT	VERT	TOTAL	—	C1	Y	20-11-12	-20	-20	—	FRONT	VERT	TOTAL	—	C1
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J	BS-t	MT20	5.0	6.0																																																																																																																																																											
K	BMWWW-t	MT20	5.0	6.0																																																																																																																																																											
L	BMWW-t	MT20	5.0	6.0																																																																																																																																																											
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JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																																																																																																																																																						
T	7-11-4	-21	-21	—	FRONT	VERT	TOTAL	—	C1																																																																																																																																																						
U	9-11-4	-21	-21	—	FRONT	VERT	TOTAL	—	C1																																																																																																																																																						
V	12-11-12	-21	-21	—	FRONT	VERT	TOTAL	—	C1																																																																																																																																																						
W	14-11-12	-21	-21	—	FRONT	VERT	TOTAL	—	C1																																																																																																																																																						
X	18-11-12	-21	-21	—	FRONT	VERT	TOTAL	—	C1																																																																																																																																																						
Y	20-11-12	-20	-20	—	FRONT	VERT	TOTAL	—	C1																																																																																																																																																						

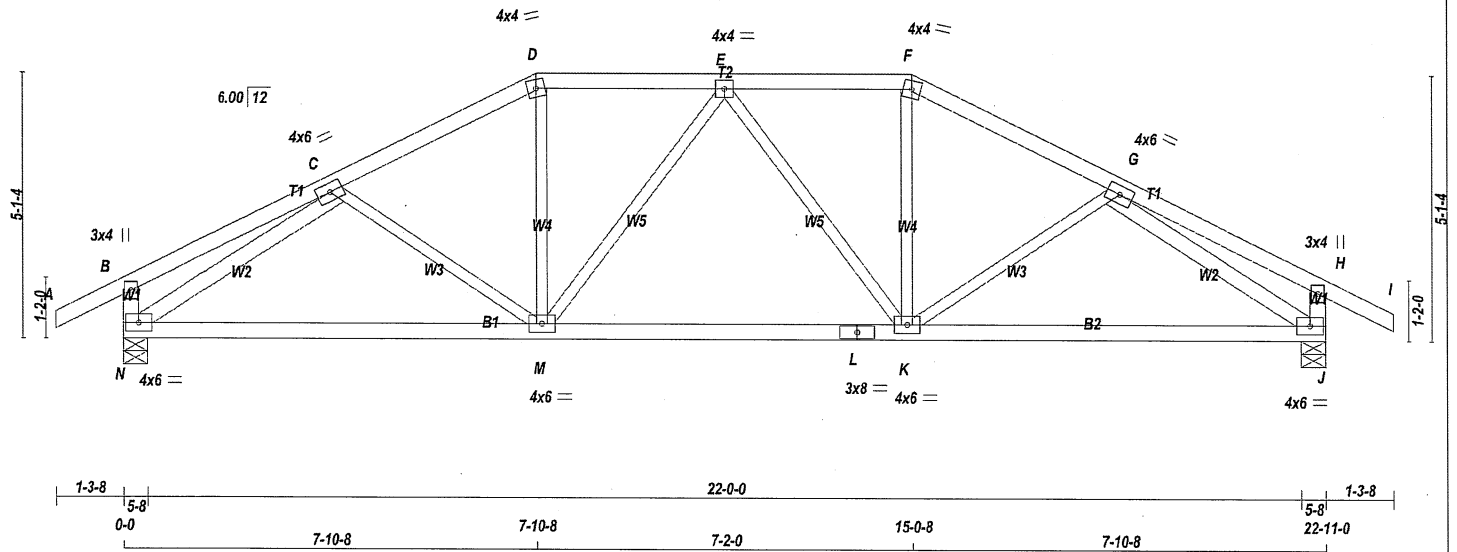
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427503	T2	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:9BS2JKdXhYlAbOihRsssi7z37OF-SH8QCADtTS2QQ7JsWAOG9Zsvm7U3 LuALCakz2yrwcc

1-3-8 0-0 4-0-8 4-0-8 3-10-0 7-10-8 3-7-0 11-5-8 3-7-0 15-0-8 3-10-0 18-10-8 4-0-8 22-11-0 24-2-8
Scale = 1:42.5



TOTAL WEIGHT = 94 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF		GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	26.7 PSF
D - F	2x4	DRY	No.2	SPF		VERT	DOWN	IN-SX	IN-SX	DL =	6.0 PSF
F - I	2x4	DRY	No.2	SPF		N	1432 0	1432 0	5-8	BOT CH. LL =	0.0 PSF
N - B	2x4	DRY	No.2	SPF		J	1432 0	1432 0	5-8	DL =	7.4 PSF
J - H	2x4	DRY	No.2	SPF						TOTAL LOAD =	40.1 PSF
N - L	2x4	DRY	No.2	SPF							
L - J	2x4	DRY	No.2	SPF							

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	4.0	
G	TMWW-t	MT20	4.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW1-t	MT20	4.0	6.0	
K	BMVWW-t	MT20	4.0	6.0	
L	BS-t	MT20	3.0	8.0	
M	BMVWW-t	MT20	4.0	6.0	
N	BMVW1-t	MT20	4.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
N	1009	685 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0
J	1009	685 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 29	-95.2	-95.2 0.12 (1)	10.00	C-M	-131 / 21	0.05 (1)
B-C	0 / 18	-95.2	-95.2 0.22 (1)	10.00	M-D	0 / 431	0.10 (1)
C-D	-1606 / 0	-95.2	-95.2 0.20 (1)	5.03	M-E	-253 / 0	0.16 (1)
D-E	-1428 / 0	-95.2	-95.2 0.17 (1)	5.30	E-K	-253 / 0	0.16 (1)
E-F	-1428 / 0	-95.2	-95.2 0.17 (1)	5.30	K-F	0 / 431	0.10 (1)
F-G	-1606 / 0	-95.2	-95.2 0.20 (1)	5.03	K-G	-131 / 21	0.05 (1)
G-H	0 / 18	-95.2	-95.2 0.22 (1)	10.00	N-C	-1877 / 0	0.52 (1)
H-I	0 / 29	-95.2	-95.2 0.12 (1)	10.00	G-J	-1877 / 0	0.52 (1)
N-B	-277 / 0	0.0	0.0 0.03 (1)	7.81			
J-H	-277 / 0	0.0	0.0 0.03 (1)	7.81			
N-M	0 / 1525	-18.5	-18.5 0.39 (1)	10.00			
M-L	0 / 1576	-18.5	-18.5 0.38 (1)	10.00			
L-K	0 / 1576	-18.5	-18.5 0.38 (1)	10.00			
K-J	0 / 1525	-18.5	-18.5 0.39 (1)	10.00			

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.39/1.00 (M-N:1)
WB=0.52/1.00 (C-N:1) , SS=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

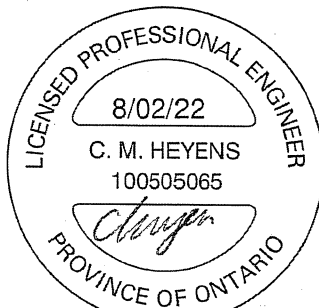
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.56 (L) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080116

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427503	T3	1	1	TRUSS DESC.		

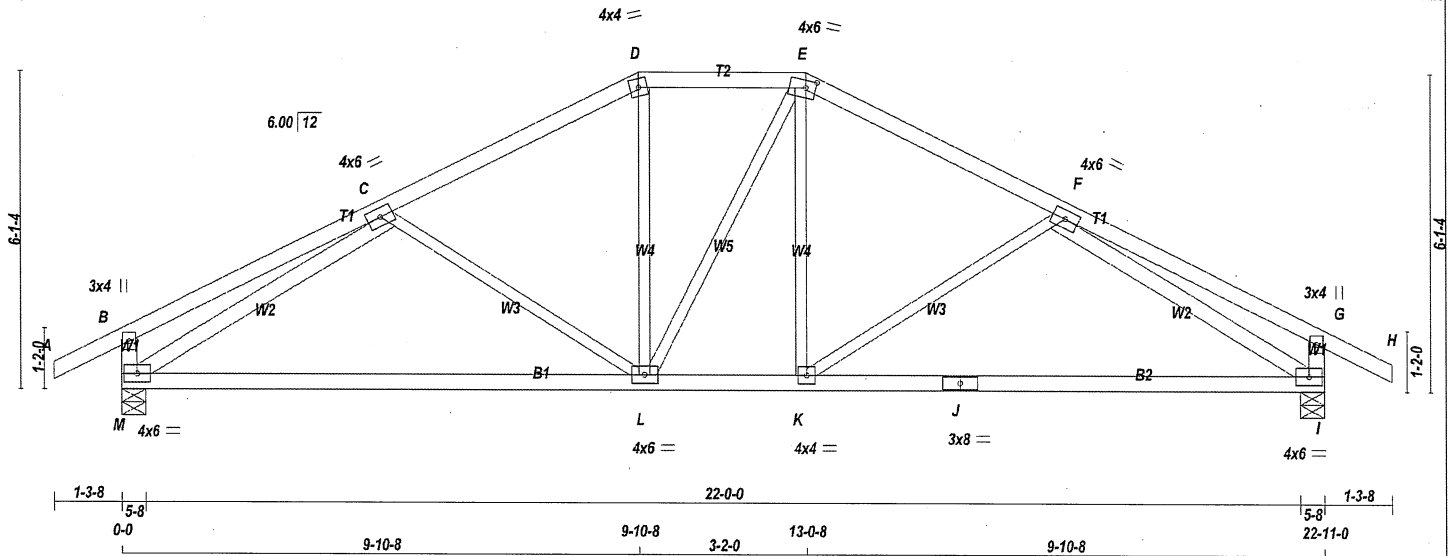
Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 10:42:32 2022 Page 1

ID:9BS2JKdXhYlAbOIHRsssj7z37OF-wUioQWVDVEmAH2Hu24uvVinP2AWpcjFJasJIVVyrwcb

-1-3-8 0-0 5-0-8 5-0-8 4-10-0 9-10-8 3-2-0 13-0-8 4-10-0 17-10-8 5-0-8 22-11-0 24-2-8 1-3-8

Scale = 1:42.5



TOTAL WEIGHT = 97 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.25
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
M	1432	0	1432	0
I	1432	0	1432	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN.	COMPONENT REACTIONS	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
M	1009	685 / 0	0 / 0	0 / 0	323 / 0
I	1009	685 / 0	0 / 0	0 / 0	323 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S									
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		MAX. FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	MEMB.	FORCE	MAX		
	(LBS)	(PLF)		CS1 (LC)	UNBRAC		(LBS)				(LC)		
FR-TO		FROM	TO		LENGTH	FR-TO							
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00	C-L	-323 / 0		0.21 (1)				
B-C	0 / 24	-95.2	-95.2	0.37 (1)	10.00	L-D	0 / 314		0.07 (1)				
C-D	-1460 / 0	-95.2	-95.2	0.31 (1)	5.08	L-E	0 / 1		0.00 (1)				
D-E	-1289 / 0	-95.2	-95.2	0.14 (1)	5.55	K-E	0 / 313		0.07 (1)				
E-F	-1460 / 0	-95.2	-95.2	0.31 (1)	5.08	K-F	-323 / 0		0.21 (1)				
F-G	0 / 24	-95.2	-95.2	0.37 (1)	10.00	M-C	-1870 / 0		0.83 (1)				
G-H	0 / 29	-95.2	-95.2	0.12 (1)	10.00	F-I	-1870 / 0		0.83 (1)				
M-B	-311 / 0	0.0	0.0	0.03 (1)	7.81								
I-G	-311 / 0	0.0	0.0	0.03 (1)	7.81								
M-L	0 / 1552	-18.5	-18.5	0.49 (4)	10.00								
L-K	0 / 1288	-18.5	-18.5	0.49 (4)	10.00								
K-J	0 / 1551	-18.5	-18.5	0.49 (4)	10.00								
J-I	0 / 1551	-18.5	-18.5	0.49 (4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/861 (0.32")

CS1: TC=0.37/1.00 (F-G:1), BC=0.49/1.00 (L-M:4), WB=0.83/1.00 (C-M:1), SS1=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.67 (J) (INPUT = 1.00)



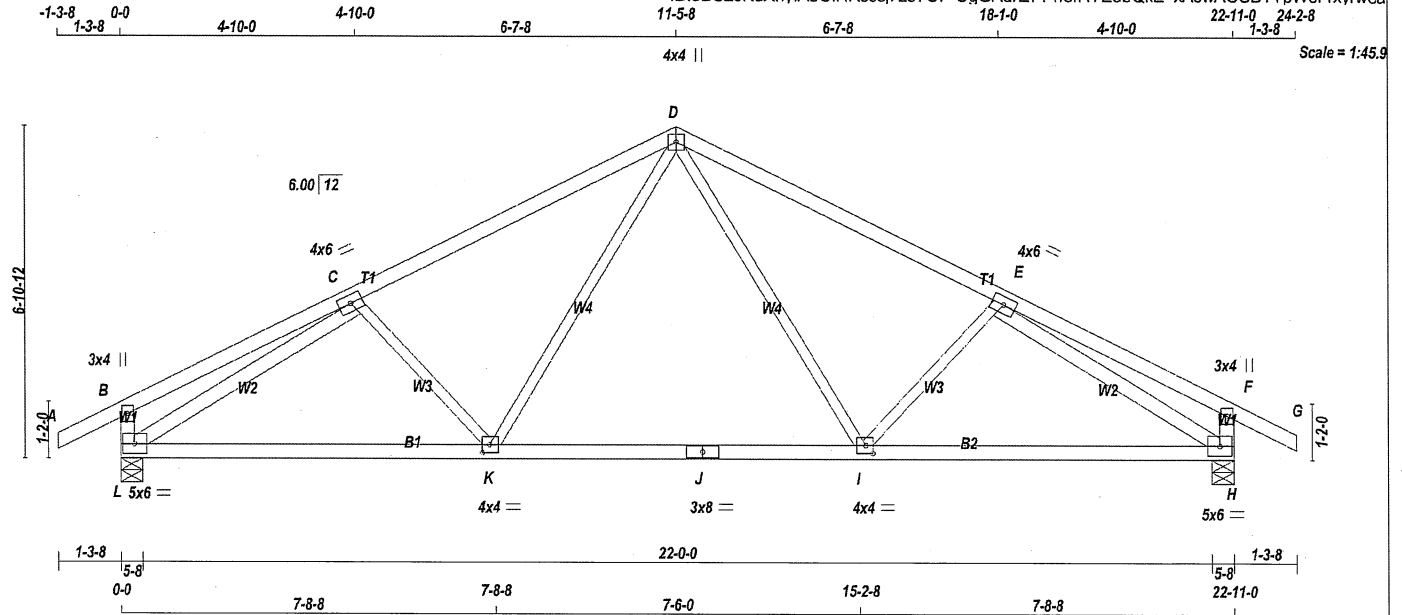
STRUCTURAL COMPONENT ONLY
DWG # TR22080117

JOB NAME 427503	TRUSS NAME T4	QUANTITY 14	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:9BS2JKdXhYlAbOIHRsssj7z37OF-OgGAdrE774I8fRTedbQkE xAswAOSBTTPW3r1xynwca



TOTAL WEIGHT = 14 X 91 = 1281 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BMVW-t	MT20	4.0	4.0	2.00	1.75
J	BS-t	MT20	3.0	8.0		
K	BMVW-t	MT20	4.0	4.0	2.00	1.75
L	BMVW1-t	MT20	5.0	6.0		

NOTES-

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
	VERT	HORIZ	DOWN	HORIZ	UPLIFT
JT	1432	0	1432	0	0
L	1432	0	1432	0	0
H	1432	0	1432	0	0

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN.	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1009	685 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0
L	1009	685 / 0	0 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 29	-95.2 -95.2	0.12 (1)	10.00	D-I	0 / 496	0.11 (1)
B-C	0 / 38	-95.2 -95.2	0.56 (1)	10.00	I-E	-368 / 0	0.12 (1)
C-D	-1566 / 0	-95.2 -95.2	0.57 (1)	4.58	K-D	0 / 496	0.11 (1)
D-E	-1566 / 0	-95.2 -95.2	0.57 (1)	4.58	C-K	-368 / 0	0.12 (1)
E-F	0 / 38	-95.2 -95.2	0.56 (1)	10.00	L-C	-1964 / 0	0.77 (1)
F-G	0 / 29	-95.2 -95.2	0.12 (1)	10.00	E-H	-1964 / 0	0.77 (1)
L-B	-268 / 0	0.0 0.0	0.03 (1)	7.81			
H-F	-268 / 0	0.0 0.0	0.03 (1)	7.81			
L-K	0 / 1623	-18.5 -18.5	0.39 (1)	10.00			
K-J	0 / 1127	-18.5 -18.5	0.34 (4)	10.00			
J-I	0 / 1127	-18.5 -18.5	0.34 (4)	10.00			
I-H	0 / 1623	-18.5 -18.5	0.39 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.14")

CSI: TC=0.57/1.00 (D-E:1), BC=0.39/1.00 (H-I:1), WB=0.77/1.00 (E-H:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.42 (C) (INPUT = 1.00)



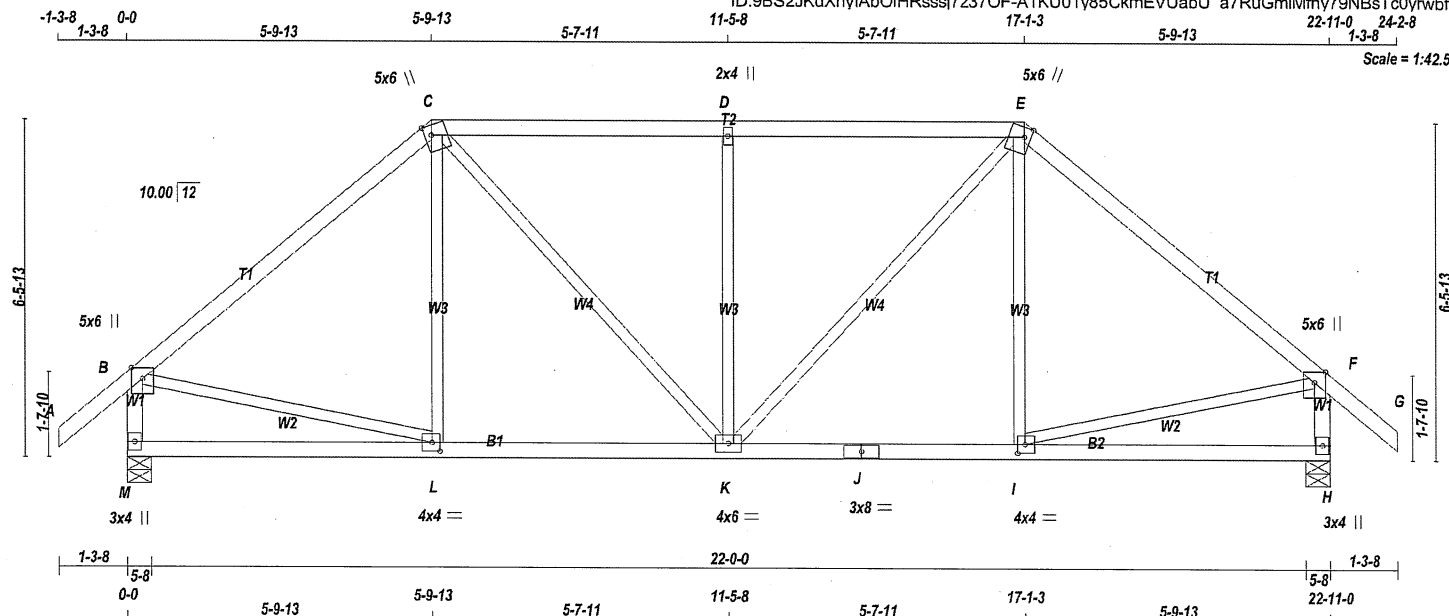
STRUCTURAL COMPONENT ONLY
DWG # TR22080118

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427503	T5	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:9BS2JKdXhYlAbOihRsssi7z37OF-A1KU01y85CkmEVUabU a7RuGmIMfny79NBsTc0YnwbF



TOTAL WEIGHT = 100 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - C	2x4	DRY	No.2	SPF	VERT	HORZ	DOWN	HORZ	UPLIFT	TOP CH. LL = 26.7 PSF	
C - E	2x4	DRY	No.2	SPF						DL = 8.0 PSF	
E - G	2x4	DRY	No.2	SPF						BOT CH. LL = 0.0 PSF	
M - B	2x4	DRY	No.2	SPF						DL = 7.4 PSF	
H - F	2x4	DRY	No.2	SPF						TOTAL LOAD = 40.1 PSF	
M - J	2x4	DRY	No.2	SPF							
J - H	2x4	DRY	No.2	SPF							

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS							
JT	1ST CASE	MAX	MIN	COMPONENT	REACTIONS		
M	1010	687	0	0	0	0	0
H	1010	687	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 42	-95.2	-95.2 0.13 (1)	10.00	L-C	-81 / 68	0.06 (1)
B-C	-1212 / 0	-95.2	-95.2 0.45 (1)	5.26	C-K	0 / 506	0.11 (1)
C-D	-1269 / 0	-95.2	-95.2 0.41 (1)	5.21	K-D	-657 / 0	0.45 (1)
D-E	-1269 / 0	-95.2	-95.2 0.41 (1)	5.21	K-E	0 / 506	0.11 (1)
E-F	-1212 / 0	-95.2	-95.2 0.45 (1)	5.26	I-E	-81 / 68	0.06 (1)
F-G	0 / 42	-95.2	-95.2 0.13 (1)	10.00	B-L	0 / 952	0.21 (1)
M-B	-1391 / 0	0.0	0.0 0.15 (1)	6.90	I-F	0 / 952	0.21 (1)
H-F	-1391 / 0	0.0	0.0 0.15 (1)	6.90			
M-L	0 / 0	-18.5	-18.5 0.15 (4)	10.00			
L-K	0 / 928	-18.5	-18.5 0.24 (1)	10.00			
K-J	0 / 928	-18.5	-18.5 0.24 (1)	10.00			
J-I	0 / 928	-18.5	-18.5 0.24 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.15 (4)	10.00			

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.45/1.00 (B-C:1), BC=0.24/1.00 (K-L:1), WB=0.45/1.00 (D-K:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

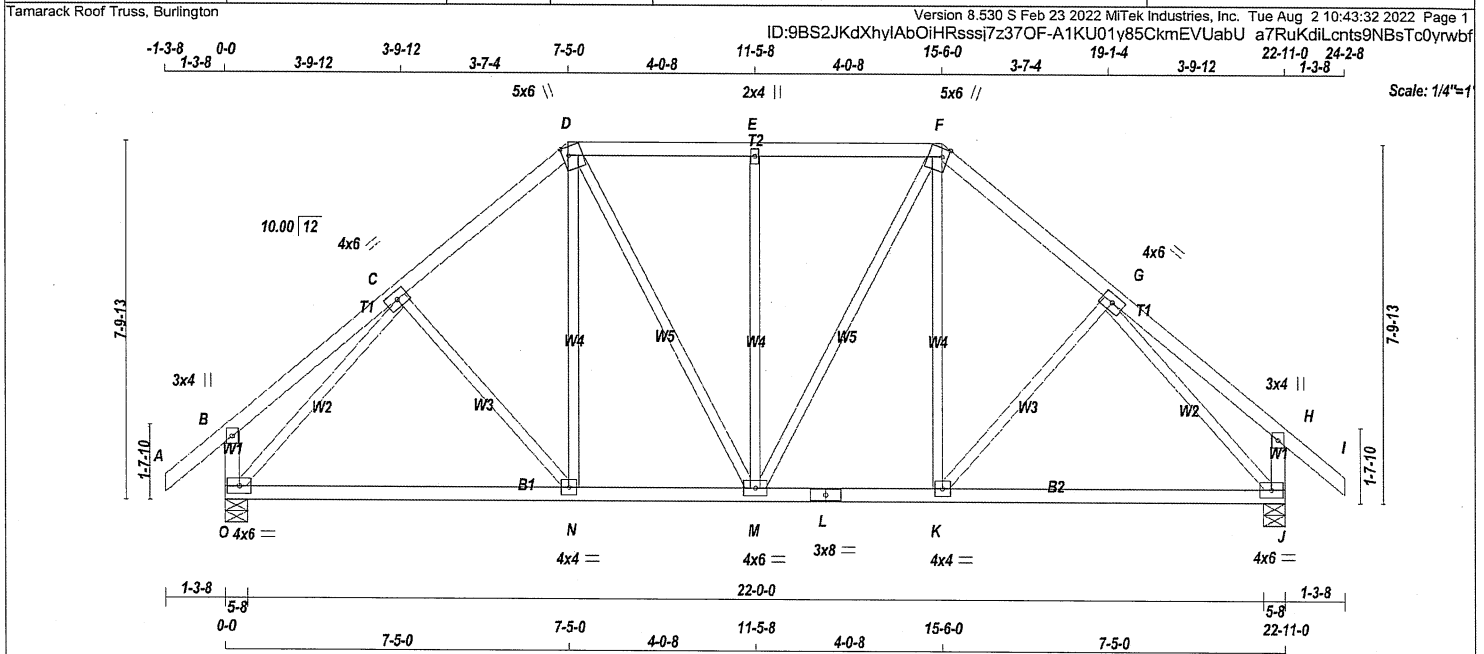
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080120

JOB NAME 427503	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 113 lb
[M][F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY No.2	SPF	
D - F	2x4	DRY No.2	SPF	
F - I	2x4	DRY No.2	SPF	
O - B	2x4	DRY No.2	SPF	
J - H	2x4	DRY No.2	SPF	
O - L	2x4	DRY No.2	SPF	
L - J	2x4	DRY No.2	SPF	
ALL WEBS	2x3	DRY No.2	SPF	
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	6.0	
D	TTWW+m	MT20	5.0	6.0	2.25 1.50
E	TMV+w	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	6.0	2.25 1.50
G	TMVW-t	MT20	4.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW1-t	MT20	4.0	6.0	
K	BMVW-t	MT20	4.0	4.0	
L	BS-t	MT20	3.0	8.0	
M	BMVW1-t	MT20	4.0	6.0	
N	BMVW-t	MT20	4.0	4.0	
O	BMVW1-t	MT20	4.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
O	1435	0	1435	0	5-8
J	1435	0	1435	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX / MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
O	1010	687 / 0	0 / 0	0 / 0	0 / 0	324 / 0
J	1010	687 / 0	0 / 0	0 / 0	0 / 0	324 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-95.2 -95.2	0.13 (1)	10.00	C-N	-82 / 19	0.05 (1)
B-C	0 / 26	-95.2 -95.2	0.20 (1)	10.00	N-D	0 / 208	0.05 (4)
C-D	-1176 / 0	-95.2 -95.2	0.17 (1)	5.70	D-M	0 / 273	0.06 (1)
D-E	-1014 / 0	-95.2 -95.2	0.20 (1)	5.99	M-E	-463 / 0	0.53 (1)
E-F	-1014 / 0	-95.2 -95.2	0.20 (1)	5.99	M-F	0 / 273	0.06 (1)
F-G	-1176 / 0	-95.2 -95.2	0.17 (1)	5.70	K-F	0 / 208	0.05 (4)
G-H	0 / 26	-95.2 -95.2	0.20 (1)	10.00	K-G	-82 / 19	0.05 (1)
H-I	0 / 42	-95.2 -95.2	0.13 (1)	10.00	O-C	-1452 / 0	0.79 (1)
O-B	-267 / 0	0.0 0.0	0.03 (1)	7.81	G-J	-1452 / 0	0.79 (1)
J-H	-267 / 0	0.0 0.0	0.03 (1)	7.81			
O-N	0 / 936	-18.5 -18.5	0.30 (4)	10.00			
N-M	0 / 884	-18.5 -18.5	0.30 (4)	10.00			
M-L	0 / 884	-18.5 -18.5	0.30 (4)	10.00			
L-K	0 / 884	-18.5 -18.5	0.30 (4)	10.00			
K-J	0 / 936	-18.5 -18.5	0.30 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.11")

CSI: TC=0.20/1.00 (B-C:1), BC=0.30/1.00 (M-N:4), WB=0.79/1.00 (G-J:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

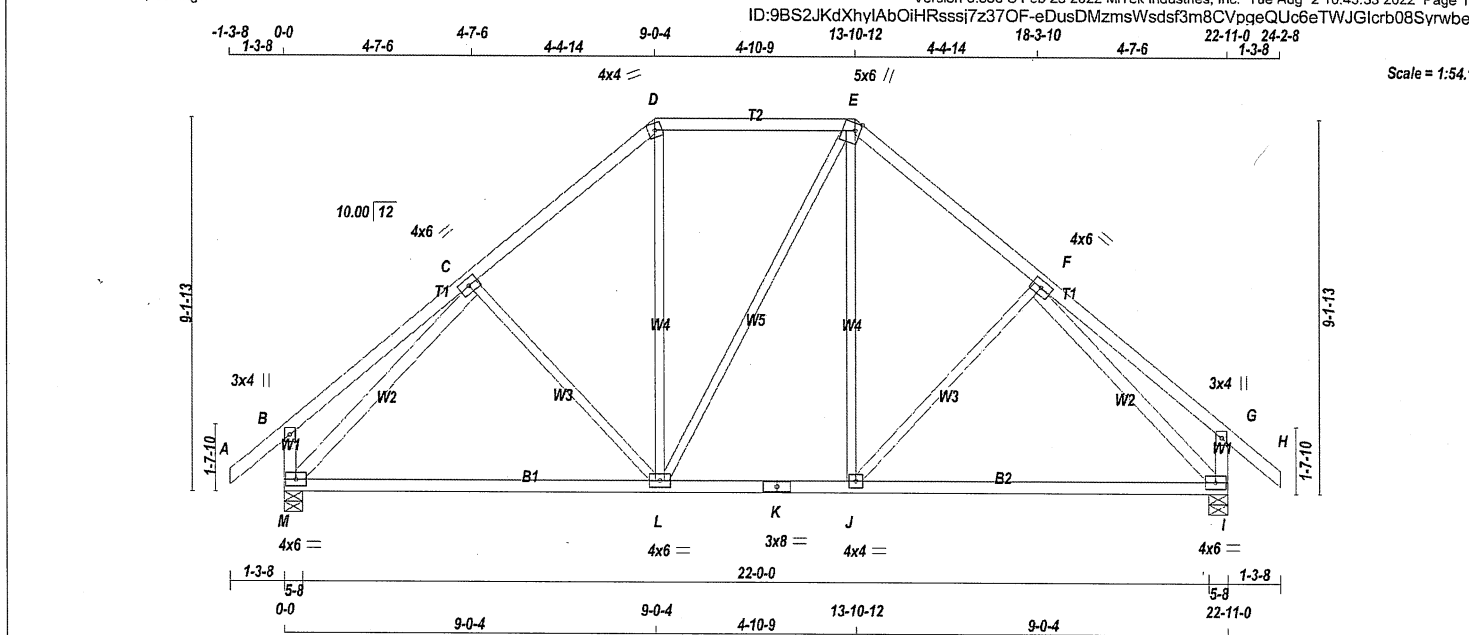
JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080121

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427503	T7	2	1	TRUSS DESC.		

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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
M - C	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW-m	MT20	4.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMWW-t	MT20	4.0	6.0	
G	TMV+p	MT20	3.0	4.0	
I	BMVW1-t	MT20	4.0	6.0	
J	BMWW-t	MT20	4.0	4.0	
K	BS-t	MT20	3.0	8.0	
L	BMVWW-t	MT20	4.0	6.0	
M	BMVW1-t	MT20	4.0	6.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	1435	0	1435	0	0	5-8	5-8		
I	1435	0	1435	0	0	5-8	5-8		

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	324 / 0	0 / 0	324 / 0	0 / 0
M	1010	687 / 0	0 / 0	0 / 0	0 / 0				
I	1010	687 / 0	0 / 0	0 / 0	0 / 0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)		CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	LC1	MAX	MEMB.	FORCE (LBS)	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	UNBRAC
FR-TO		FROM	TO			FR-TO				FR-TO			
A-B	0 / 42	-95.2	-95.2	0.13 (1)	10.00	C-L	-199 / 0	0.17 (1)		C-L	-199 / 0	0.17 (1)	
B-C	0 / 33	-95.2	-95.2	0.32 (1)	10.00	L-D	0 / 287	0.06 (1)		L-D	0 / 287	0.06 (1)	
C-D	-1099 / 0	-95.2	-95.2	0.26 (1)	5.74	L-E	0 / 0	0.00 (1)		L-E	0 / 0	0.00 (1)	
D-E	-820 / 0	-95.2	-95.2	0.30 (1)	6.25	J-E	0 / 286	0.06 (1)		J-E	0 / 286	0.06 (1)	
E-F	-1098 / 0	-95.2	-95.2	0.26 (1)	5.74	J-F	-199 / 0	0.17 (1)		J-F	-199 / 0	0.17 (1)	
F-G	0 / 33	-95.2	-95.2	0.32 (1)	10.00	M-C	-1432 / 0	0.84 (1)		M-C	-1432 / 0	0.84 (1)	
G-H	0 / 42	-95.2	-95.2	0.13 (1)	10.00	F-I	-1432 / 0	0.84 (1)		F-I	-1432 / 0	0.84 (1)	
M-B	-294 / 0	0.0	0.0	0.03 (1)	7.81								
I-G	-294 / 0	0.0	0.0	0.03 (1)	7.81								
M-L	0 / 952	-18.5	-18.5	0.39 (4)	10.00								
L-K	0 / 820	-18.5	-18.5	0.36 (4)	10.00								
K-J	0 / 820	-18.5	-18.5	0.36 (4)	10.00								
J-I	0 / 952	-18.5	-18.5	0.39 (4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL)= L/999 (0.23")

CSI: TC=0.32/1.00 (F-G:1), BC=0.39/1.00 (I-J:4), WB=0.84/1.00 (C-M:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

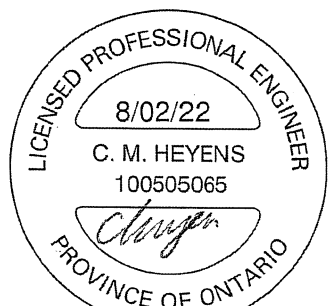
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

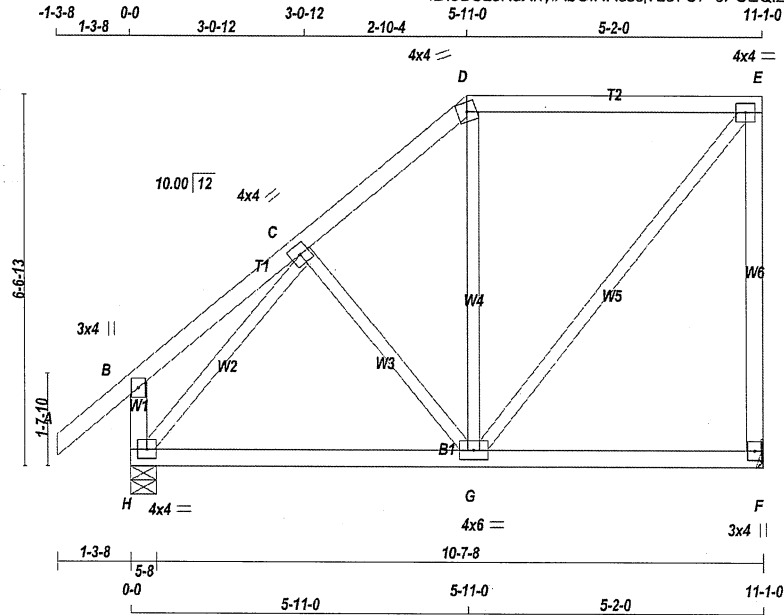
JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.46 (K) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080122

JOB NAME 427503	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:9BS2JKdXhYlAbOihRsssj7z37OF-6PSEQizOdp UUpeyiv02Cszb0W1pFwCRrVLagvyrwbd



TOTAL WEIGHT = 56 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES-

(1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	630	0	630	0
H	762	0	762	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
F	445	298 / 0
H	535	370 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX
MEMB.	(LBS)	(PLF)	(LBS)	(LBS)	(LBS)	(LBS)	FR-TO	(LBS)	(LBS)
A-B	0 / 42	-95.2	-95.2	0.13 (1)	10.00	C-G	-141 / 0	0.05 (1)	
B-C	0 / 21	-95.2	-95.2	0.14 (1)	10.00	G-D	-117 / 23	0.08 (1)	
C-D	-394 / 0	-95.2	-95.2	0.10 (1)	6.25	G-E	0 / 452	0.10 (1)	
D-E	-284 / 0	-95.2	-95.2	0.33 (1)	6.25	H-C	-608 / 0	0.22 (1)	
F-E	-596 / 0	0.0	0.0	0.53 (1)	7.81				
H-B	-238 / 0	0.0	0.0	0.03 (1)	7.81				
H-G	0 / 375	-18.5	-18.5	0.19 (4)	10.00				
G-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.53/1.00 (E-F:1), BC=0.19/1.00 (G-H:4), WB=0.22/1.00 (C-H:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (C) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

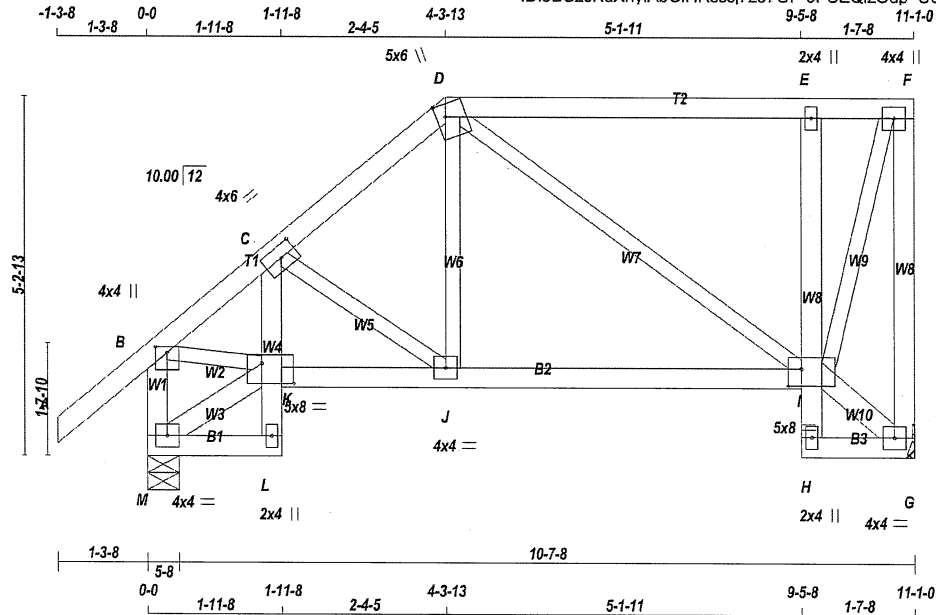


STRUCTURAL COMPONENT ONLY
DWG # TR22080123

JOB NAME 427503	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:9BS2JKdXhYlAbOIHRsss7z37OF-6PSEQizOdp UUpayiv02CszevW2VFX1RrVLagvyrwbd



TOTAL WEIGHT = 62 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF
D - F	2x4 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF
M - B	2x4 DRY	No.2	SPF
M - L	2x4 DRY	No.2	SPF
K - I	2x4 DRY	No.2	SPF
H - G	2x4 DRY	No.2	SPF

ALL WEBS EXCEPT	SIZE	LUMBER	DESCR.
C - J	2x3 DRY	No.2	SPF
J - D	2x3 DRY	No.2	SPF
D - I	2x3 DRY	No.2	SPF
B - K	2x3 DRY	No.2	SPF
I - F	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0		
G	BMVW-t	MT20	4.0	4.0		
H	BMVW+w	MT20	2.0	4.0		
I	BMVWVW*-i	MT20	5.0	8.0	3.00	2.25
J	BMVW-t	MT20	4.0	4.0		
K	BMVWVW*-i	MT20	5.0	8.0	3.50	5.50
L	BMVW+w	MT20	2.0	4.0		
M	BMVW-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
G	622	0	622	0
M	770	0	770	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
G	439	292 / 0	0 / 0
M	541	375 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX
FR-TO							FR-TO		
A-B	0 / 42	-95.2	-95.2	0.13 (1)	10.00	L-K	0 / 19	0.03 (1)	
B-C	-706 / 0	-95.2	-95.2	0.13 (1)	6.25	K-C	-13 / 14	0.03 (1)	
C-D	-553 / 0	-95.2	-95.2	0.09 (1)	6.25	H-I	0 / 15	0.03 (1)	
D-E	-265 / 0	-95.2	-95.2	0.35 (1)	6.25	I-E	-537 / 0	0.04 (1)	
E-F	-259 / 0	-95.2	-95.2	0.34 (1)	6.25	C-J	-145 / 0	0.03 (1)	
G-F	-593 / 0	0.0	0.0	0.28 (1)	7.81	J-D	0 / 154	0.04 (1)	
M-B	-743 / 0	0.0	0.0	0.08 (1)	7.81	D-I	-195 / 0	0.15 (1)	
						M-K	-20 / 0	0.00 (1)	
M-L	0 / 17	-18.5	-18.5	0.02 (4)	10.00	B-K	0 / 528	0.12 (1)	
K-J	0 / 536	-18.5	-18.5	0.15 (4)	10.00	I-G	-27 / 0	0.00 (1)	
J-I	0 / 421	-18.5	-18.5	0.14 (4)	10.00	I-F	0 / 739	0.17 (1)	
H-G	0 / 22	-18.5	-18.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.35/1.00 (D-E:1), BC=0.15/1.00 (J-K:4), WB=0.17/1.00 (F-I:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)

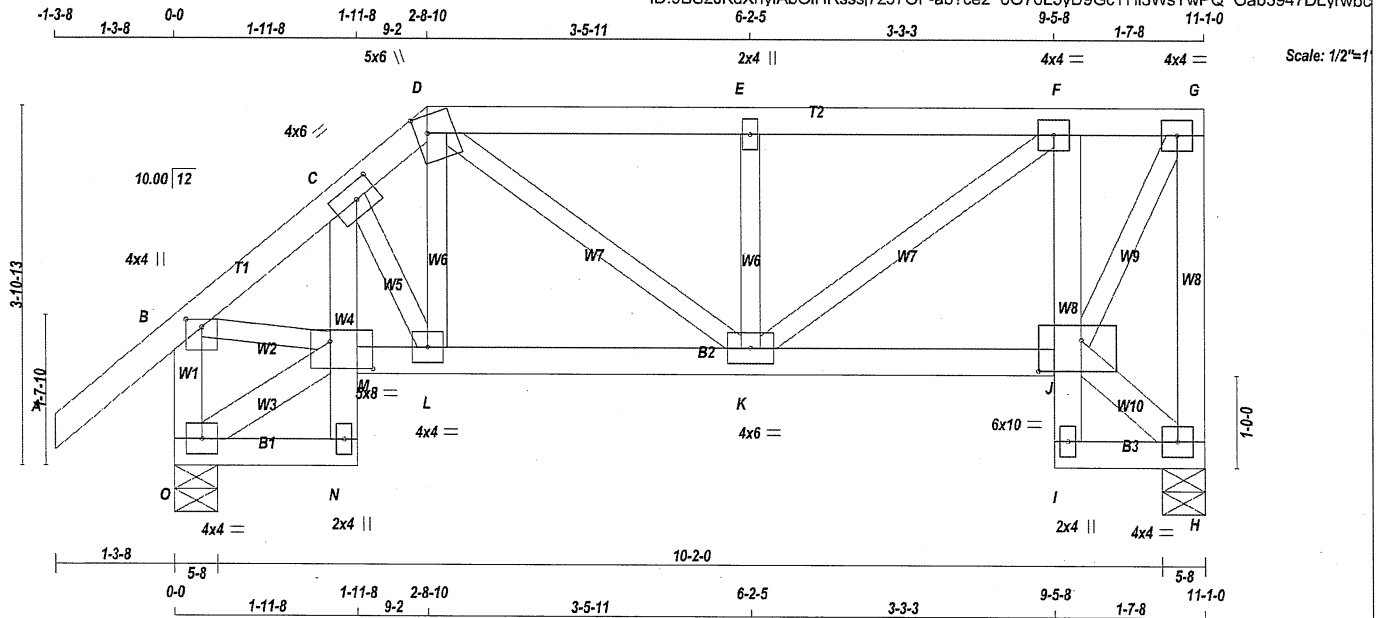
JSI METAL= 0.27 (I) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080124

JOB NAME 427503	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
O - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
O - N	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF

ALL WEBS EXCEPT	2x3	DRY No.2	SPF
N - C	2x4	DRY No.2	SPF
I - F	2x4	DRY No.2	SPF
O - M	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW-t	MT20	4.0	6.0	2.00	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMVW+w	MT20	2.0	4.0		
J	BMVWW-i	MT20	6.0	10.0	4.00	5.50
K	BMVWW-t	MT20	4.0	6.0		
L	BMVWW-t	MT20	4.0	4.0		
M	BMVWW-i	MT20	5.0	8.0	3.50	5.50
N	BMVW+w	MT20	2.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
O-B	-743 / 0	0.0	0.0 0.08 (1)	7.81	N-M	0 / 19	0.03 (1)
A-B	0 / 42	-95.2	-95.2 0.13 (1)	10.00	M-C	0 / 16	0.03 (1)
B-C	-688 / 0	-95.2	-95.2 0.13 (1)	6.25	I-J	0 / 15	0.03 (1)
C-D	-618 / 0	-95.2	-95.2 0.03 (1)	6.25	J-F	-516 / 0	0.03 (1)
D-E	-691 / 0	-95.2	-95.2 0.16 (1)	6.25	L-D	0 / 54	0.02 (4)
E-F	-691 / 0	-95.2	-95.2 0.16 (1)	6.25	C-L	-65 / 0	0.01 (1)
F-G	-341 / 0	-95.2	-95.2 0.08 (1)	6.25	O-M	-20 / 0	0.00 (1)
H-G	-594 / 0	0.0	0.0 0.14 (1)	7.81	B-M	0 / 501	0.11 (1)
					J-H	-23 / 0	0.00 (1)
O-N	0 / 17	-18.5	-18.5 0.02 (4)	10.00	J-G	0 / 657	0.15 (1)
M-L	0 / 512	-18.5	-18.5 0.10 (1)	10.00	K-F	0 / 430	0.10 (1)
L-K	0 / 481	-18.5	-18.5 0.10 (1)	10.00	D-K	0 / 262	0.06 (1)
K-J	0 / 350	-18.5	-18.5 0.09 (1)	10.00	K-E	-372 / 0	0.06 (1)
I-H	0 / 18	-18.5	-18.5 0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.02")

CSI: TC=0.16/1.00 (D-E:1), BC=0.10/1.00 (L-M:1), WB=0.15/1.00 (G-J:1), SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

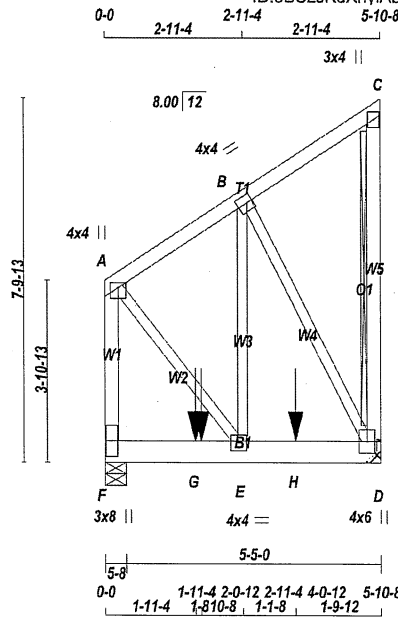


STRUCTURAL COMPONENT ONLY
DWG # TR22080125

JOB NAME 427503	TRUSS NAME T11	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:47.5

TOTAL WEIGHT = 2 X 43 = 86 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00



STRUCTURAL COMPONENT ONLY
DWG # TR22080126 PG 1/2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
JT	1211	0	1211	0	0	0
D	1444	0	1444	0	5-8	5-8
F	1444	0	1444	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST CASE		MAX/MIN. COMPONENT REACTIONS		LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	GROSS	REACT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT	852	585 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	267 / 0	0 / 0
D	1015	701 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-D

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH (FT)		MAX. FACTORED FORCE (LBS)		MAX. UNBRACED LENGTH (FT)	
	FR-TO	LOC.	LC1	MAX	FR-TO	LOC.	FR-TO	LOC.	FR-TO	LOC.
A-B	-619 / 0	-95.2	-95.2	0.07 (1)	6.25	E-B	0 / 993	0.12 (1)	6.25	E-B
B-C	-17 / 0	-95.2	-95.2	0.07 (1)	6.25	B-D	-1116 / 0	0.36 (1)	6.25	B-D
D-C	-109 / 0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 823	0.10 (1)	7.81	A-E
F-A	-1091 / 0	0.0	0.0	0.13 (1)	7.81					
F-G	0 / 0	-18.5	-18.5	0.24 (1)	10.00					
G-E	0 / 0	-18.5	-18.5	0.24 (1)	10.00					
E-H	0 / 529	-18.5	-18.5	0.19 (1)	10.00					
H-D	0 / 529	-18.5	-18.5	0.19 (1)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-541	-541	---	BACK	VERT	TOTAL	---	C1
G	2-0-12	-424	-424	---	FRONT	VERT	TOTAL	---	C1
H	4-0-12	-430	-430	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.13/1.00 (A-F:1), BC=0.24/1.00 (E-F:1), WB=0.36/1.00 (B-D:1), SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)

JSI METAL= 0.18 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427503	T11	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	4.0	4.0	2.50	1.75
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

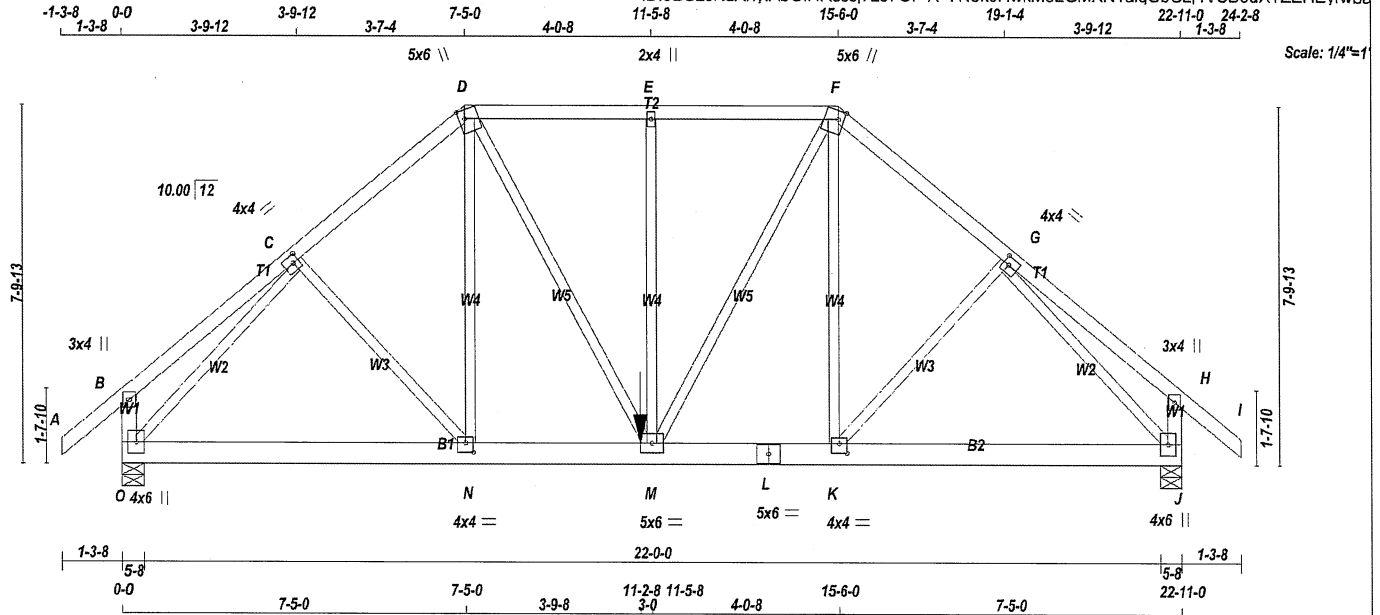
1) Lateral braces to be a minimum of 2X4 SPF #2.



STRUCTURAL COMPONENT ONLY
DWG # TR22080126 PG 2/2

JOB NAME 427503	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 127 = 253 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	TOP
D-F	12	TOP
F-I	12	TOP
O-B	12	TOP
J-H	12	TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O-L	12	SIDE(183.1)
L-J	12	TOP

WEBS : (0.122"x3") SPIRAL NAILS		
E-M	1	SIDE(127.9)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	2031	0	2031	0
J	2031	0	2031	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1429	979 / 0	0 / 0	0 / 0	0 / 0	450 / 0	0 / 0
J	1429	979 / 0	0 / 0	0 / 0	0 / 0	450 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX.
MEMB.	FORCE	VERT.	LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX.
(LBS)	(LBS)	(PLF)	(PLF)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO		FROM	TO				FR-TO		
A-B	0 / 42	-95.2	-95.2	0.07 (1)	10.00	C-N	0 / 54	0.01 (4)	
B-C	0 / 25	-95.2	-95.2	0.11 (1)	10.00	N-D	0 / 137	0.02 (4)	
C-D	-1967 / 0	-95.2	-95.2	0.13 (1)	6.12	D-M	0 / 923	0.11 (1)	
D-E	-1931 / 0	-95.2	-95.2	0.14 (1)	6.14	M-E	-446 / 0	0.24 (1)	
E-F	-1931 / 0	-95.2	-95.2	0.14 (1)	6.14	M-F	0 / 923	0.11 (1)	
F-G	-1967 / 0	-95.2	-95.2	0.13 (1)	6.12	K-F	0 / 137	0.02 (4)	
G-H	0 / 25	-95.2	-95.2	0.11 (1)	10.00	K-G	0 / 54	0.01 (4)	
H-I	0 / 42	-95.2	-95.2	0.07 (1)	10.00	O-C	-2247 / 0	0.59 (1)	
O-B	-269 / 0	0.0	0.0	0.02 (1)	7.81	G-J	-2247 / 0	0.59 (1)	
J-H	-269 / 0	0.0	0.0	0.02 (1)	7.81				
O-N	0 / 1465	-18.5	-18.5	0.13 (1)	10.00				
N-M	0 / 1490	-18.5	-18.5	0.13 (1)	10.00				
M-L	0 / 1490	-18.5	-18.5	0.13 (1)	10.00				
L-K	0 / 1490	-18.5	-18.5	0.13 (1)	10.00				
K-J	0 / 1465	-18.5	-18.5	0.13 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	11-2-8	-837	-837	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.14/1.00 (D-E:1), BC=0.13/1.00 (K-M:1), WB=0.59/1.00 (G-J:1), SSI=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.38 (C) (INPUT = 1.00)



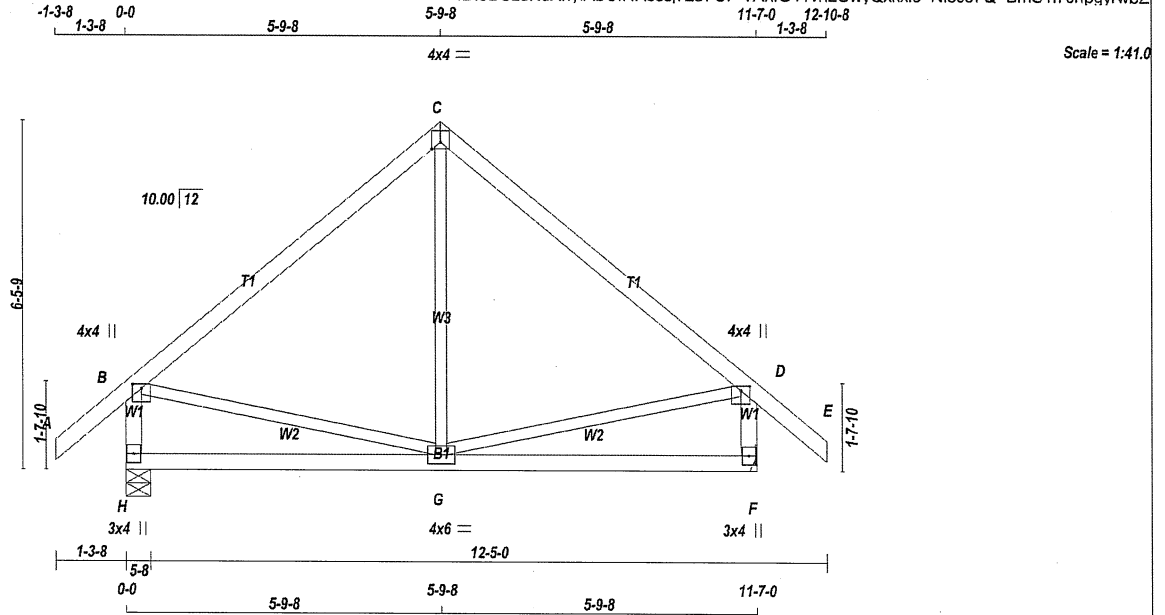
STRUCTURAL COMPONENT ONLY
DWG # TR22080127 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427503	T13	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:9BS2JKdXhyIAbOiHRsssj7z37OF-7AhlG41vh2UwyQxkd5 Ni8Js7Q BmS117JnpgyrwbZ



TOTAL WEIGHT = 3 X 51 = 154 lb

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TTW-p	MT20	4.0	4.0	1.50	2.00
D TMVW+p	MT20	4.0	4.0	1.00	2.00
F BMV1+p	MT20	3.0	4.0		
G BMVWW-t	MT20	4.0	6.0		
H BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	790	0	790	0	5-8	5-8
F	790	0	790	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND		
H	556	384 / 0	0 / 0	0 / 0	172 / 0	0 / 0
F	556	384 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 42	-95.2	-95.2	0.13 (1)	10.00	G-C	-13 / 97
B-C	-421 / 0	-95.2	-95.2	0.41 (1)	6.25	B-G	0 / 331
C-D	-421 / 0	-95.2	-95.2	0.41 (1)	6.25	G-D	0 / 331
D-E	0 / 42	-95.2	-95.2	0.13 (1)	10.00		
H-B	-749 / 0	0.0	0.0	0.08 (1)	7.81		
F-D	-749 / 0	0.0	0.0	0.08 (1)	7.81		
H-G	0 / 0	-18.5	-18.5	0.18 (4)	10.00		
G-F	0 / 0	-18.5	-18.5	0.18 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.41/1.00 (B-C:1), BC=0.18/1.00 (F-G:4), WB=0.07/1.00 (D-G:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

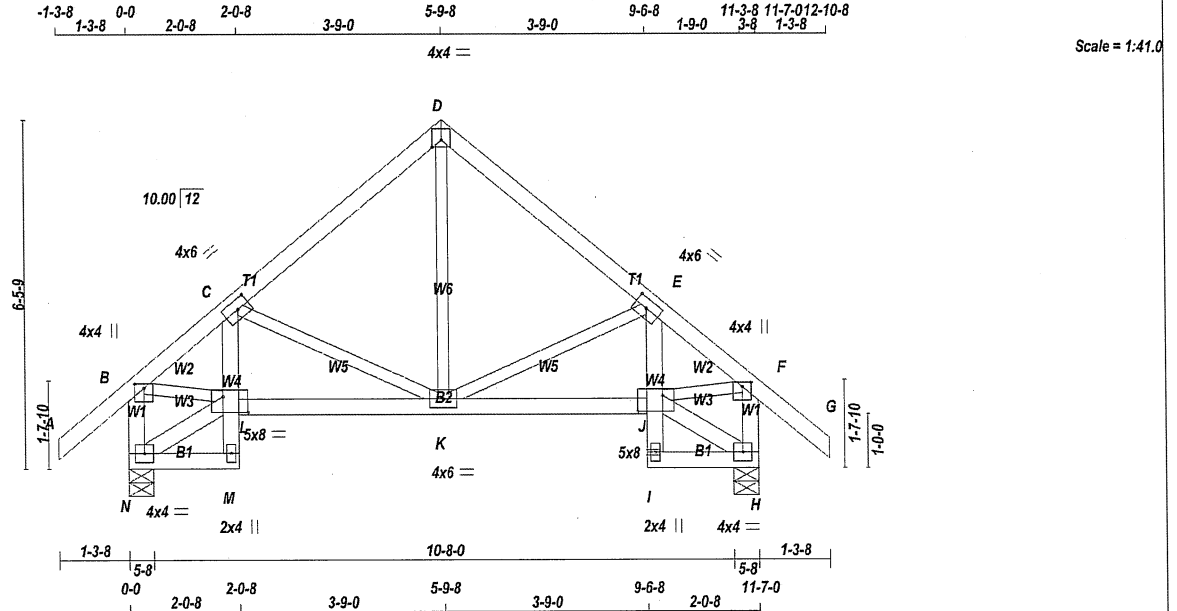


STRUCTURAL COMPONENT ONLY
DWG # TR22080128

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427503	T14	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:9BS2JKdXhYlAbOiHRssj7z37OF-?AhIG41vh2UwyQxkl5 Ni8NZ7QaBIY117JnpgyrbwZ



TOTAL WEIGHT = 2 X 61 = 121 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
K - D	2x3	DRY	No.2
K - E	2x3	DRY	No.2
C - K	2x3	DRY	No.2
B - L	2x3	DRY	No.2
J - F	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMWW-t	MT20	4.0	6.0	2.00	2.75
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMVW1-t	MT20	4.0	4.0		
I	BMW+w	MT20	2.0	4.0		
J	BMVW1-t	MT20	5.0	8.0	3.50	5.50
K	BMVW1-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	5.0	8.0	3.50	5.50
M	BMW+w	MT20	2.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
N	790	0	5-8	5-8
H	790	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM. LIVE	MAX./MIN. WIND	MAX./MIN. DEAD	MAX./MIN. SOIL
N	556	384 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0
H	556	384 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-B	0 / 42	-95.2	-95.2	0.13 (1)	10.00	M-L	0 / 20	0.03 (1)	
B-C	-760 / 0	-95.2	-95.2	0.12 (1)	6.25	L-C	-6 / 34	0.02 (4)	
C-D	-487 / 0	-95.2	-95.2	0.18 (1)	6.25	I-J	0 / 20	0.03 (1)	
D-E	-487 / 0	-95.2	-95.2	0.18 (1)	6.25	J-E	-6 / 34	0.02 (4)	
E-F	-760 / 0	-95.2	-95.2	0.12 (1)	6.25	K-D	0 / 285	0.06 (1)	
F-G	0 / 42	-95.2	-95.2	0.13 (1)	10.00	K-E	-265 / 0	0.08 (1)	
N-B	-762 / 0	0.0	0.0	0.08 (1)	7.81	C-K	-265 / 0	0.08 (1)	
H-F	-762 / 0	0.0	0.0	0.08 (1)	7.81	N-L	-21 / 0	0.00 (1)	
N-M	0 / 18	-18.5	-18.5	0.02 (4)	10.00	B-L	0 / 591	0.13 (1)	
L-K	0 / 600	-18.5	-18.5	0.14 (1)	10.00	J-H	-21 / 0	0.00 (1)	
K-J	0 / 600	-18.5	-18.5	0.14 (1)	10.00	J-F	0 / 591	0.13 (1)	
I-H	0 / 18	-18.5	-18.5	0.02 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.18/1.00 (C-D:1), BC=0.14/1.00 (K-L:1), WB=0.13/1.00 (B-L:1), SS=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.23 (F) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080129

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427510	T15	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 10:53:43 2022 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMVW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	8.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW+t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.50 (I) (INPUT = 1.00)



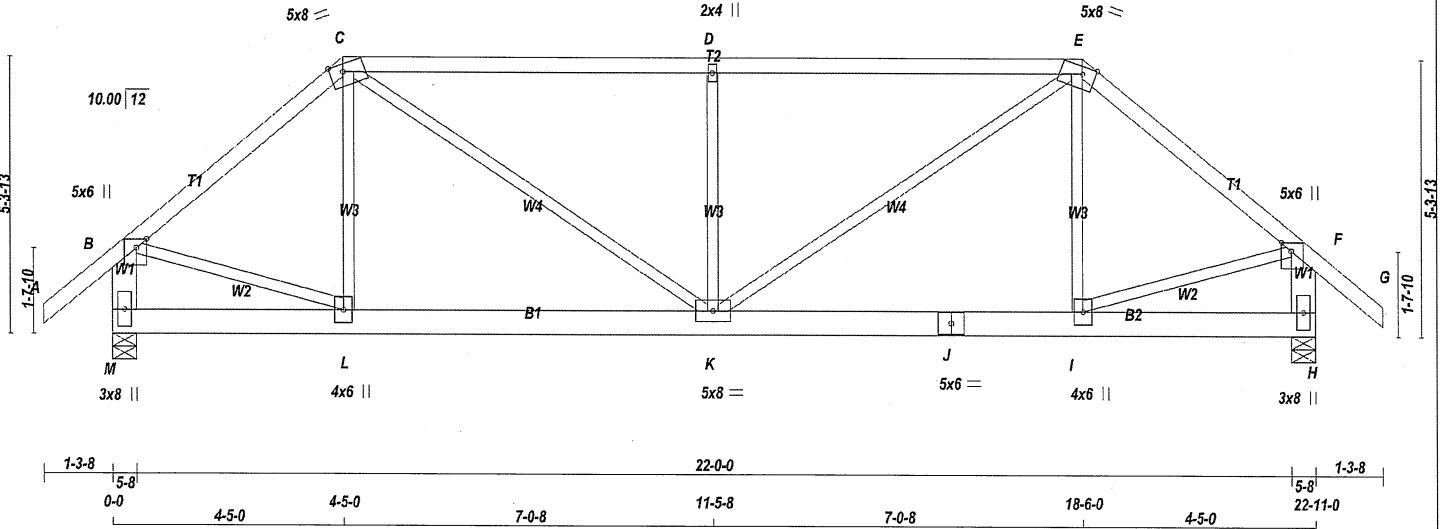
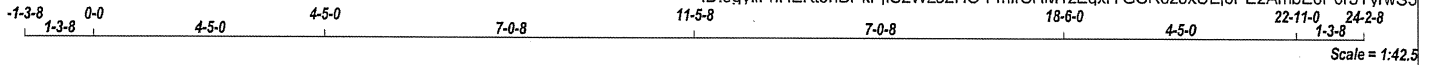
STRUCTURAL COMPONENT ONLY
DWG # TR22080133 PG 2/2

JOB NAME 427510	TRUSS NAME T15Z	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 110 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMVW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMVW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	8.0		
I	BMVW+t	MT20	4.0	6.0		
J	BS-I	MT20	5.0	6.0		
K	BMVW+t	MT20	5.0	8.0		
L	BMVW+t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
M	1435	0	1435	0
H	1435	0	1435	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE
M	1010	687 / 0	0 / 0	0 / 0
H	1010	687 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-95.2 -95.2	0.13 (1)	10.00	L-C	-130 / 46	0.05 (1)
B-C	-1256 / 0	-95.2 -95.2	0.37 (1)	5.28	C-K	0 / 817	0.18 (1)
C-D	-1630 / 0	-95.2 -95.2	0.98 (1)	3.52	K-D	-823 / 0	0.32 (1)
D-E	-1630 / 0	-95.2 -95.2	0.98 (1)	3.52	K-E	0 / 817	0.18 (1)
E-F	-1256 / 0	-95.2 -95.2	0.37 (1)	5.28	I-E	-130 / 46	0.05 (1)
F-G	0 / 42	-95.2 -95.2	0.13 (1)	10.00	B-L	0 / 997	0.22 (1)
M-B	-1408 / 0	0.0 0.0	0.09 (1)	7.81	I-F	0 / 997	0.22 (1)
H-F	-1408 / 0	0.0 0.0	0.09 (1)	7.81			
M-L	0 / 0	-18.5 -18.5	0.07 (4)	10.00			
L-K	0 / 960	-18.5 -18.5	0.16 (1)	10.00			
K-J	0 / 960	-18.5 -18.5	0.16 (1)	10.00			
J-I	0 / 960	-18.5 -18.5	0.16 (1)	10.00			
I-H	0 / 0	-18.5 -18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.98/1.00 (C-D:1), BC=0.16/1.00 (K-L:1), WB=0.32/1.00 (D-K:1), SSI=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (L) (INPUT = 0.90)
JSI METAL= 0.43 (I) (INPUT = 1.00)



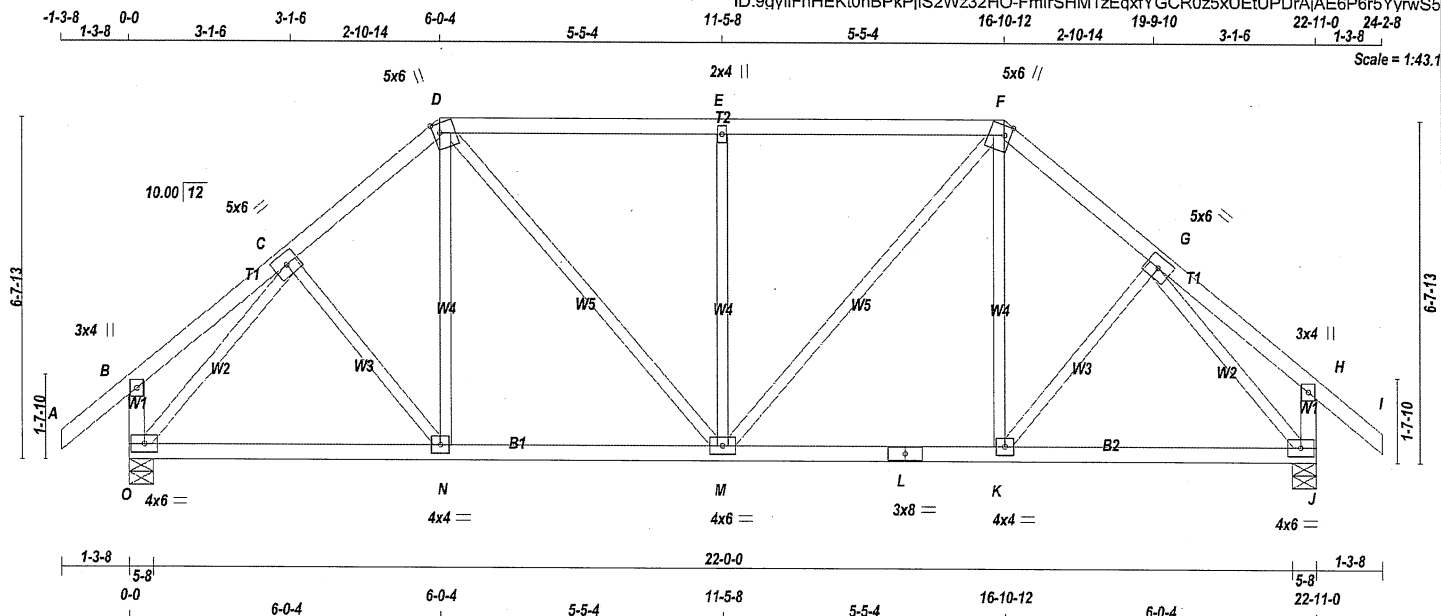
STRUCTURAL COMPONENT ONLY
DWG # TR22080134

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427510	T16	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:9qyIfnHEKt0nBPkPjS2Wz32HO-FmlrSHM1zEqxYGCROz5xUetUPDrAE6P6r5YyvwSS



TOTAL WEIGHT = 2 X 106 = 212 lb

LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 2 X 106 = 212 lb				
N. L. G. A. RULES															[M/F]				
CHORDS		SIZE	LUMBER	DESCR.	BEARINGS										DESIGN CRITERIA				
A - D	2x4	DRY	No.2	SPF	FACTORED					MAXIMUM FACTORED					INPUT		REQRD		
D - F	2x4	DRY	No.2	SPF	GROSS REACTION					GROSS REACTION					BRG				
F - I	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX							
O - B	2x4	DRY	No.2	SPF	O	1435	0	1435	0	0	5-8	5-8							
J - H	2x4	DRY	No.2	SPF	J	1435	0	1435	0	0	5-8	5-8							
O - L	2x4	DRY	No.2	SPF											SPECIFIED LOADS:				
L - J	2x4	DRY	No.2	SPF											TOP CH. LL = 26.7 PSF				
												DL = 6.0 PSF							
												BOT CH. LL = 0.0 PSF							
												DL = 7.4 PSF							
												TOTAL LOAD = 40.1 PSF							

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 42	-95.2	-95.2 0.13 (1)	10.00	C-N	0 / 47	0.02 (4)
B-C	0 / 19	-95.2	-95.2 0.12 (1)	10.00	N-D	0 / 103	0.04 (4)
C-D	-1223 / 0	-95.2	-95.2 0.11 (1)	5.69	K-F	0 / 103	0.04 (4)
D-E	-1231 / 0	-95.2	-95.2 0.38 (1)	5.32	K-G	0 / 47	0.02 (4)
E-F	-1231 / 0	-95.2	-95.2 0.38 (1)	5.32	O-C	-1458 / 0	0.54 (1)
F-G	-1223 / 0	-95.2	-95.2 0.11 (1)	5.69	G-J	-1458 / 0	0.54 (1)
G-H	0 / 19	-95.2	-95.2 0.12 (1)	10.00	M-F	0 / 472	0.11 (1)
H-I	0 / 42	-95.2	-95.2 0.13 (1)	10.00	D-M	0 / 472	0.11 (1)
O-B	-244 / 0	0.0	0.0 0.03 (1)	7.81	M-E	-632 / 0	0.46 (1)
J-H	-244 / 0	0.0	0.0 0.03 (1)	7.81			
O-N	0 / 902	-18.5	-18.5 0.23 (1)	10.00			
N-M	0 / 924	-18.5	-18.5 0.24 (1)	10.00			
M-L	0 / 924	-18.5	-18.5 0.24 (1)	10.00			
L-K	0 / 924	-18.5	-18.5 0.24 (1)	10.00			
K-J	0 / 902	-18.5	-18.5 0.23 (1)	10.00			

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.24/1.00 (K-M:1) , WB=0.54/1.00 (G-J:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (O) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

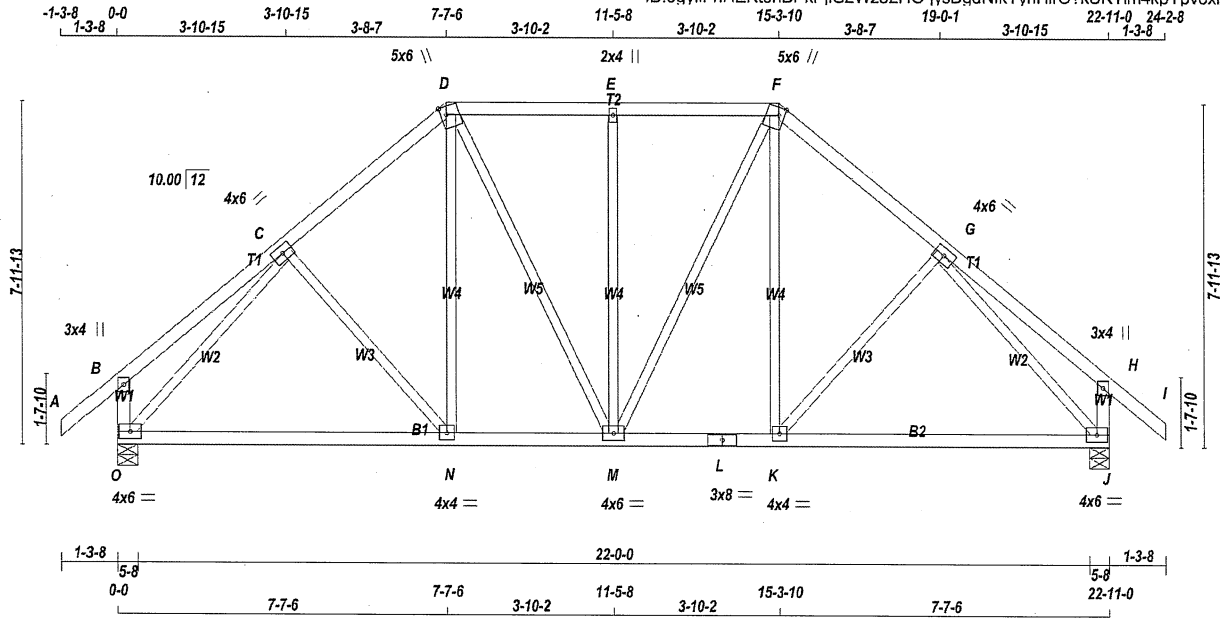


STRUCTURAL COMPONENT ONLY
DWG # TR22080135

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427510	T17	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 114 = 228 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	6.0		
D TTWW+m	MT20	5.0	6.0	2.25	1.50
E TMW+w	MT20	2.0	4.0		
F TTWW+m	MT20	5.0	6.0	2.25	1.50
G TMWW-t	MT20	4.0	6.0		
H TMV+p	MT20	3.0	4.0		
J BMVW1-t	MT20	4.0	6.0		
K BMWW-t	MT20	4.0	4.0		
L BS-t	MT20	3.0	8.0		
M BMWW-t	MT20	4.0	6.0		
N BMWW-t	MT20	4.0	4.0		
O BMVW1-t	MT20	4.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT				
O 1435	0	1435	0	5-8
J 1435	0	1435	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX / MIN	COMPONENT REACTIONS				
JT COMBINED							
O 1010	687 / 0	0 / 0	PERM LIVE	WIND	DEAD	SOIL	
J 1010	687 / 0	0 / 0	0 / 0	0 / 0	324 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.
FR-TO				FR-TO			
A-B	0 / 42	-95.2	-95.2 0.13 (1)	10.00	C-N	-97 / 16	0.06 (1)
B-C	0 / 27	-95.2	-95.2 0.22 (1)	10.00	N-D	0 / 224	0.05 (4)
C-D	-1168 / 0	-95.2	-95.2 0.18 (1)	5.71	K-F	0 / 224	0.05 (4)
D-E	-988 / 0	-95.2	-95.2 0.18 (1)	6.07	K-G	-97 / 16	0.06 (1)
E-F	-988 / 0	-95.2	-95.2 0.18 (1)	6.07	O-C	-1451 / 0	0.83 (1)
F-G	-1168 / 0	-95.2	-95.2 0.18 (1)	5.71	G-J	-1451 / 0	0.83 (1)
G-H	0 / 27	-95.2	-95.2 0.22 (1)	10.00	M-F	0 / 248	0.06 (1)
H-I	0 / 42	-95.2	-95.2 0.13 (1)	10.00	D-M	0 / 248	0.06 (1)
O-B	-270 / 0	0.0	0.0 0.03 (1)	7.81	M-E	-438 / 0	0.53 (1)
J-H	-270 / 0	0.0	0.0 0.03 (1)	7.81			
O-N	0 / 939	-18.5	-18.5 0.31 (4)	10.00			
N-M	0 / 878	-18.5	-18.5 0.32 (4)	10.00			
M-L	0 / 878	-18.5	-18.5 0.32 (4)	10.00			
L-K	0 / 878	-18.5	-18.5 0.32 (4)	10.00			
K-J	0 / 939	-18.5	-18.5 0.31 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 26.7 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.22/1.00 (G-H-1), BC=0.32/1.00 (K-M-4), WB=0.83/1.00 (C-O-1), SSI=0.18/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.33 (L) (INPUT = 1.00)

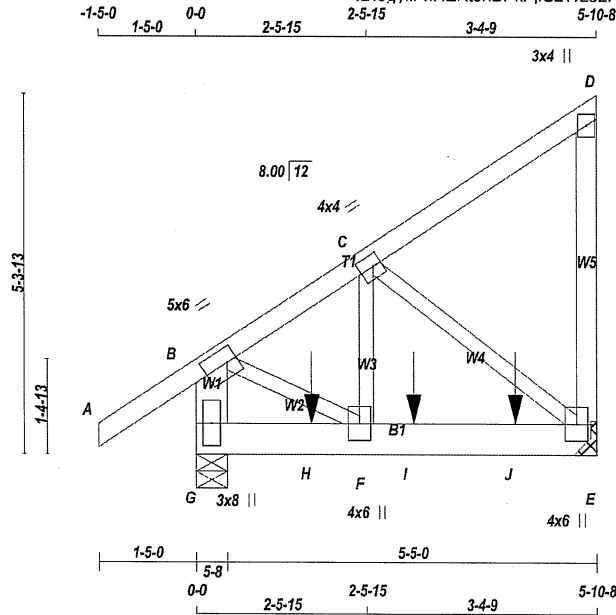


STRUCTURAL COMPONENT ONLY
DWG # TR22080136

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427510	T18	1	2	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1:32.7

TOTAL WEIGHT = 2 X 34 = 68 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 1	12	TOP
E - D 1	12	TOP
G - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - E 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION		GROSS REACTION			
JT	VERT	HORZ	DOWN	HORZ	UP/LT	IN-SX
E	1590	0	1590	0	0	5-8
G	1536	0	1536	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED		SNOW	LIVE		
JT	1116		782 / 0	0 / 0	0 / 0	0 / 0
E	1076		764 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 40	-95.2 -95.2	0.08 (1)	F-C	0 / 1321
B-C	-1356 / 0	-95.2 -95.2	0.07 (1)	C-E	-1456 / 0
C-D	-15 / 0	-95.2 -95.2	0.07 (1)	B-F	0 / 1232
E-D	-135 / 0	0.0 0.0	0.03 (1)		
G-B	-1446 / 0	0.0 0.0	0.05 (1)		
G-H	0 / 0	-18.5 -18.5	0.17 (1)		
H-F	0 / 0	-18.5 -18.5	0.17 (1)		
F-I	0 / 1144	-18.5 -18.5	0.31 (1)		
I-J	0 / 1144	-18.5 -18.5	0.31 (1)		
J-E	0 / 1144	-18.5 -18.5	0.31 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	1-8-4	-541	-541	---	BACK	VERT	TOTAL	---	C1
I	3-2-4	-541	-541	---	BACK	VERT	TOTAL	---	C1
J	4-8-4	-541	-541	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.08/1.00 (A-B:1), BC=0.31/1.00 (E-F:1)
WB=0.22/1.00 (C-E:1), SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)

JSI METAL= 0.28 (E) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080137 PG 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427510	T18	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW1+p	MT20	4.0	6.0		
F	BMWW+t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

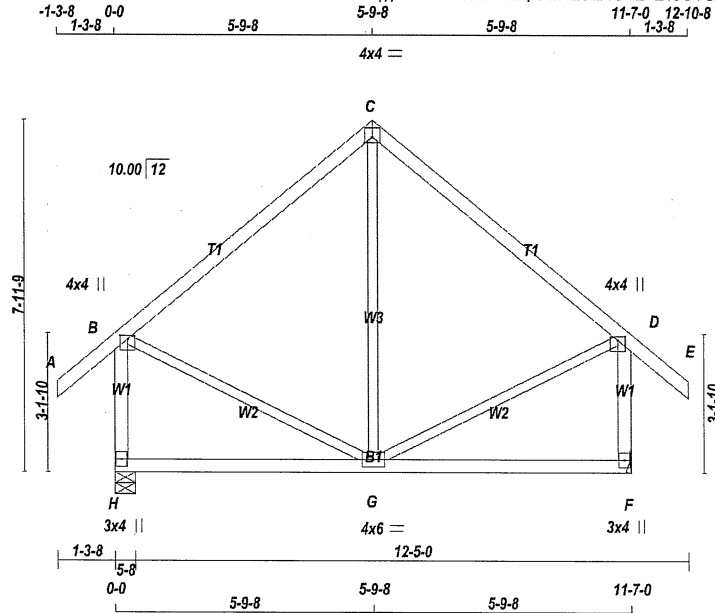


STRUCTURAL COMPONENT ONLY
DWG # TR22080137 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427510	T19	3	1	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

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Scale = 1:50.1

TOTAL WEIGHT = 3 X 57 = 170 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION		GROSS REACTION			
H	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
H	790	0	790	0	5-8	5-8
F	790	0	790	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED		SNOW	LIVE			
H	556	384 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0
F	556	384 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-95.2 -95.2	0.13 (1)	10.00	G-C	-118 / 59	0.14 (1)
B-C	-338 / 0	-95.2 -95.2	0.41 (1)	6.25	B-G	0 / 288	0.06 (1)
C-D	-338 / 0	-95.2 -95.2	0.41 (1)	6.25	G-D	0 / 288	0.06 (1)
D-E	0 / 42	-95.2 -95.2	0.13 (1)	10.00			
H-B	-750 / 0	0.0 0.0	0.13 (1)	7.81			
F-D	-750 / 0	0.0 0.0	0.13 (1)	7.81			
H-G	0 / 0	-18.5 -18.5	0.18 (4)	10.00			
G-F	0 / 0	-18.5 -18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.41/1.00 (B-C:1), BC=0.18/1.00 (G-H:4), WB=0.14/1.00 (C-G:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)

JSI METAL= 0.15 (B) (INPUT = 1.00)



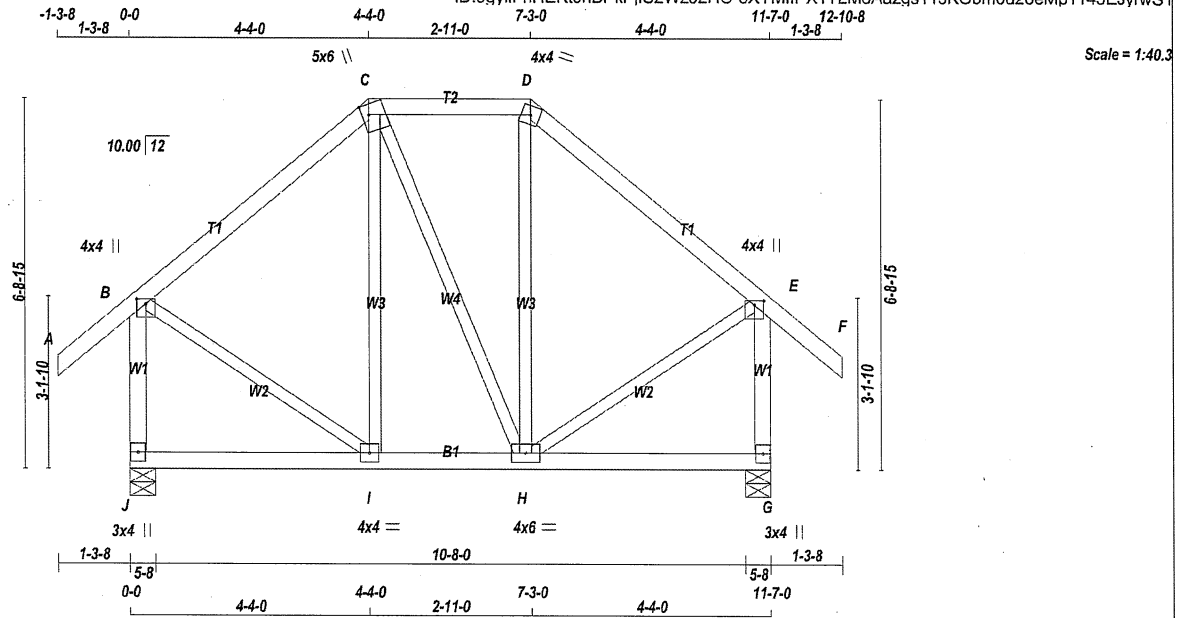
STRUCTURAL COMPONENT ONLY
DWG # TR22080138

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427510	T20	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 63 lb

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TTWW+m	MT20	5.0	6.0	2.25	1.50
D TTW-m	MT20	4.0	4.0		
E TMVW+p	MT20	4.0	4.0	1.00	2.00
G BMV1+p	MT20	3.0	4.0		
H BMVWW-t	MT20	4.0	6.0		
I BMVWW-t	MT20	4.0	4.0		
J BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	790	0	790	0
G	790	0	790	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
J	COMBINED	SNOW	LIVE	PERM.LIVE	
J	556	384 / 0	0 / 0	0 / 0	172 / 0
G	556	384 / 0	0 / 0	0 / 0	172 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-95.2 -95.2 0.13 (1)	10.00	I-C	-106 / 20	0.08 (1)	
B-C	-371 / 0	-95.2 -95.2 0.23 (1)	6.25	C-H	-1 / 0	0.00 (1)	
C-D	-282 / 0	-95.2 -95.2 0.10 (1)	6.25	H-D	-107 / 20	0.08 (1)	
D-E	-370 / 0	-95.2 -95.2 0.23 (1)	6.25	B-I	0 / 336	0.08 (1)	
E-F	0 / 42	-95.2 -95.2 0.13 (1)	10.00	H-E	0 / 335	0.08 (1)	
J-B	-755 / 0	0.0 0.0 0.13 (1)	7.81				
G-E	-755 / 0	0.0 0.0 0.13 (1)	7.81				
J-I	0 / 0	-18.5 -18.5 0.08 (4)	10.00				
I-H	0 / 282	-18.5 -18.5 0.09 (4)	10.00				
H-G	0 / 0	-18.5 -18.5 0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.23/1.00 (B-C:1), BC=0.09/1.00 (H-I:4), WB=0.08/1.00 (D-H:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)



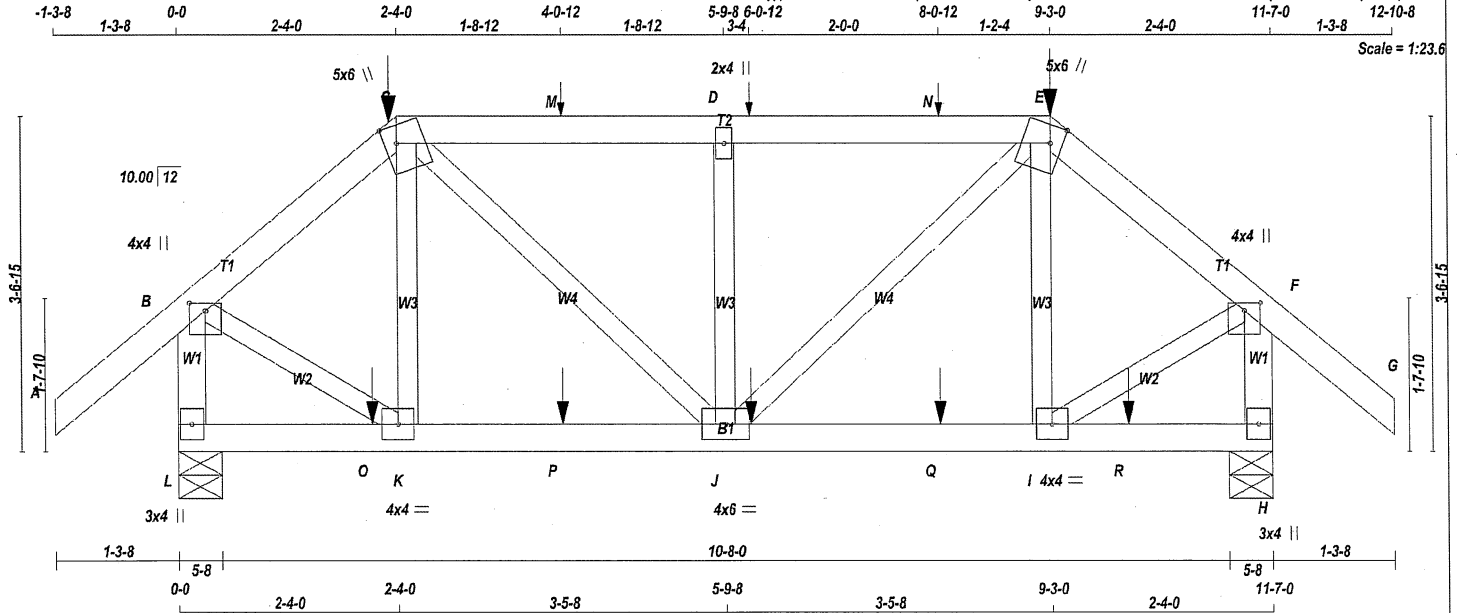
STRUCTURAL COMPONENT ONLY
DWG # TR22080139

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427510	T21	1	1	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 53 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
L - B	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		
L - H	2x4 DRY	No.2	SPF		

ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-i	MT20	4.0	4.0		
J	BMVW-w	MT20	4.0	6.0		
K	BMVW-i	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
L	842	0	842	0	5-8
H	843	0	843	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
L	592	412 / 0	0 / 0	0 / 0	180 / 0	0 / 0
H	592	412 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MEMB.	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-95.2	-95.2 0.14 (1)	10.00	K-C	-162 / 0	0.04 (1)
B-C	-555 / 0	-95.2	-95.2 0.10 (1)	6.25	C-J	0 / 330	0.08 (1)
C-M	-659 / 0	-95.2	-95.2 0.20 (1)	6.25	J-D	-396 / 0	0.09 (1)
M-D	-659 / 0	-95.2	-95.2 0.20 (1)	6.25	J-E	0 / 330	0.08 (1)
D-N	-659 / 0	-95.2	-95.2 0.20 (1)	6.25	I-E	-162 / 0	0.04 (1)
N-E	-659 / 0	-95.2	-95.2 0.20 (1)	6.25	B-K	0 / 481	0.12 (1)
E-F	-555 / 0	-95.2	-95.2 0.10 (1)	6.25	I-F	0 / 481	0.12 (1)
F-G	0 / 42	-95.2	-95.2 0.14 (1)	10.00			
L-B	-826 / 0	0.0	0.0 0.10 (1)	7.81			
H-F	-826 / 0	0.0	0.0 0.10 (1)	7.81			
L-O	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
O-K	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
K-P	0 / 419	-18.5	-18.5 0.10 (1)	10.00			
P-J	0 / 419	-18.5	-18.5 0.10 (1)	10.00			
J-Q	0 / 419	-18.5	-18.5 0.10 (1)	10.00			
Q-I	0 / 419	-18.5	-18.5 0.10 (1)	10.00			
I-R	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
R-H	0 / 0	-18.5	-18.5 0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-4-0	-6	-6	---	FRONT	VERT	DEAD	---	C1
C	2-4-0	-28	-28	---	FRONT	VERT	SNOW	---	C1
D	6-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
E	9-3-0	-6	-6	---	FRONT	VERT	DEAD	---	C1
E	9-3-0	-28	-28	---	FRONT	VERT	SNOW	---	C1
J	6-0-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
M	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
N	8-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
O	2-0-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
P	4-0-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
Q	8-0-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1
R	10-0-12	-1	-1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.20/1.00 (C-D:1), BC=0.10/1.00 (I-J:1), WB=0.12/1.00 (B-K:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (F) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22080140 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427510	T21	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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CONNECTION REQUIREMENTS

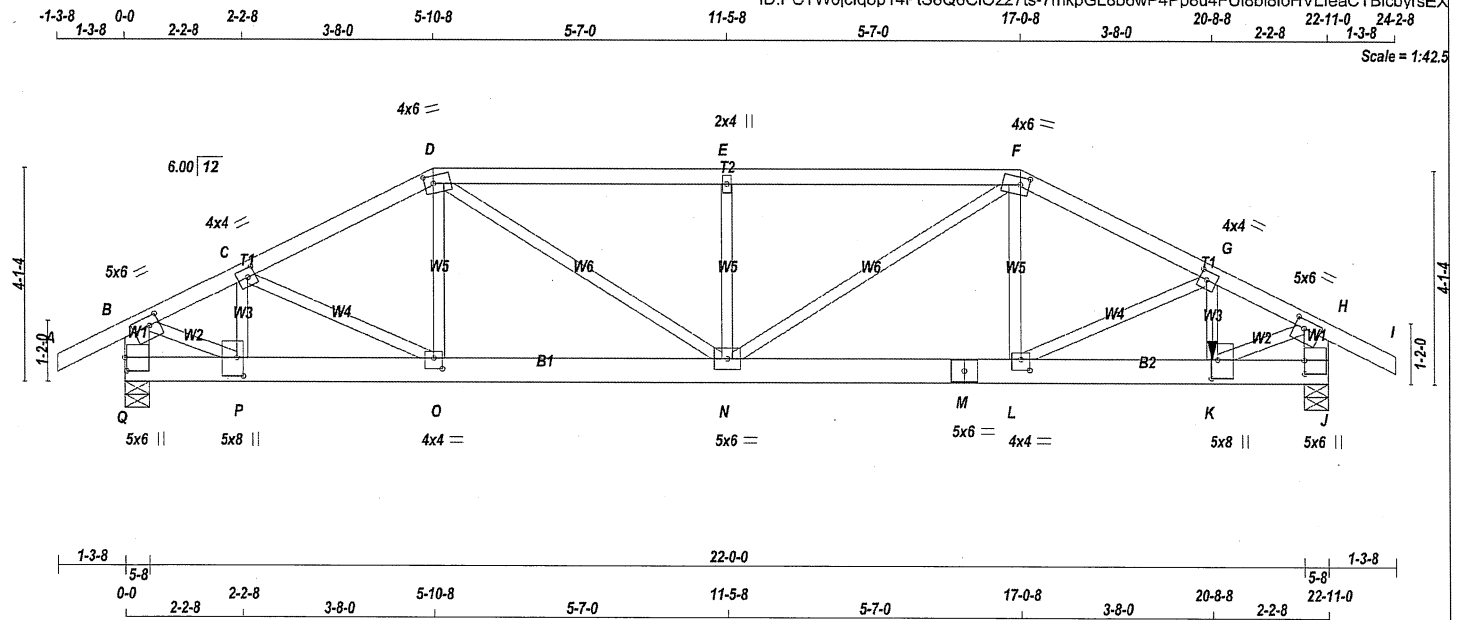
- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427527	T22	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
Q - B	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	12	TOP
D-F	12	TOP
F-I	12	TOP
Q-B	2	TOP
J-H	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q-M	11	TOP
M-J	11	SIDE(244.1)
WEBS : (0.122"x3") SPIRAL NAILS		
G-K	2	SIDE(277.5)
2x3	2	
O-C	6	
O-D	6	
N-D	6	
E-N	6	
N-F	6	
L-F	6	
L-G	6	
P-B	6	
K-H	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
Q	1625	0	1625	0	0	5-8	5-8	5-8	5-8
J	3239	0	3239	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SO
Q	1144	780 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 /
J	2276	1574 / 0	0 / 0	0 / 0	0 / 0	702 / 0	0 /

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.81 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX.		MEMB.		FORCE		MAX.		WEBS		FACTORED		MAX.	
MEMB.	FORCE (LBS)	VERT.	LOAD LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)	MEMB.	FORCE (LBS)	MAX	CS1 (LC)	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO			LENGTH	FR-TO				FR-TO				FR-TO		
A-B	0 / 29	-95.2	-95.2	0.07 (1)	10.00	P-C	-592 / 0	0.05 (1)			P-C	0 / 265	0.03 (1)		P-C	0 / 265	0.03 (1)
B-C	-1788 / 0	-95.2	-95.2	0.08 (1)	6.25	C-O	0 / 75	0.01 (4)			C-O	0 / 75	0.01 (4)		C-O	0 / 75	0.01 (4)
C-D	-2087 / 0	-95.2	-95.2	0.10 (1)	6.02	O-D	0 / 981	0.12 (1)			O-D	0 / 981	0.12 (1)		O-D	0 / 981	0.12 (1)
D-E	-2671 / 0	-95.2	-95.2	0.31 (1)	5.22	N-E	-647 / 0	0.08 (1)			N-E	-647 / 0	0.08 (1)		N-E	-647 / 0	0.08 (1)
E-F	-2671 / 0	-95.2	-95.2	0.31 (1)	5.22	N-F	0 / 275	0.03 (1)			N-F	0 / 275	0.03 (1)		N-F	0 / 275	0.03 (1)
F-G	-2718 / 0	-95.2	-95.2	0.13 (1)	5.41	L-F	0 / 697	0.09 (1)			L-F	0 / 697	0.09 (1)		L-F	0 / 697	0.09 (1)
G-H	-3761 / 0	-95.2	-95.2	0.09 (1)	4.81	L-G	-1063 / 0	0.16 (1)			L-G	-1063 / 0	0.16 (1)		L-G	-1063 / 0	0.16 (1)
H-I	0 / 29	-95.2	-95.2	0.07 (1)	10.00	K-G	0 / 624	0.08 (1)			K-G	0 / 624	0.08 (1)		K-G	0 / 624	0.08 (1)
Q-B	-1565 / 0	0.0	0.0	0.06 (1)	7.81	B-P	0 / 1713	0.21 (1)			B-P	0 / 1713	0.21 (1)		B-P	0 / 1713	0.21 (1)
J-H	-3079 / 0	0.0	0.0	0.11 (1)	7.81	K-H	0 / 3576	0.44 (1)			K-H	0 / 3576	0.44 (1)		K-H	0 / 3576	0.44 (1)
Q-P	0 / 0	-18.5	-18.5	0.03 (1)	10.00												
P-O	0 / 1617	-18.5	-18.5	0.14 (1)	10.00												
O-N	0 / 1855	-18.5	-18.5	0.15 (1)	10.00												
N-M	0 / 2442	-18.5	-18.5	0.20 (1)	10.00												
M-L	0 / 2442	-18.5	-18.5	0.20 (1)	10.00												
L-K	0 / 3374	-18.5	-18.5	0.33 (1)	10.00												
K-J	0 / 0	-18.5	-18.5	0.11 (1)	10.00												

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	20-8-8	-1403	-1403	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD = 40.1 PSF				

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.31/1.00 (D-E:1), BC=0.33/1.00 (K-L:1), WB=0.44/1.00 (H-K:1), SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.61 (P) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080150 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427527	T22	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:PCTW0jclg8p14PtS8Q6CfOz27ts-7mkpGL8b6wP4Pp8u4FUI8b18ioHVLleaCTBIcbyrsEX

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.00	2.25
J	BMV1+p	MT20	5.0	6.0	3.00	
K	BMWW+t	MT20	5.0	8.0	4.25	1.50
L	BMWW-t	MT20	4.0	4.0	2.50	2.00
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.50	2.00
P	BMWW+t	MT20	5.0	8.0	4.25	1.50
Q	BMV1+p	MT20	5.0	6.0	3.00	0.50

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



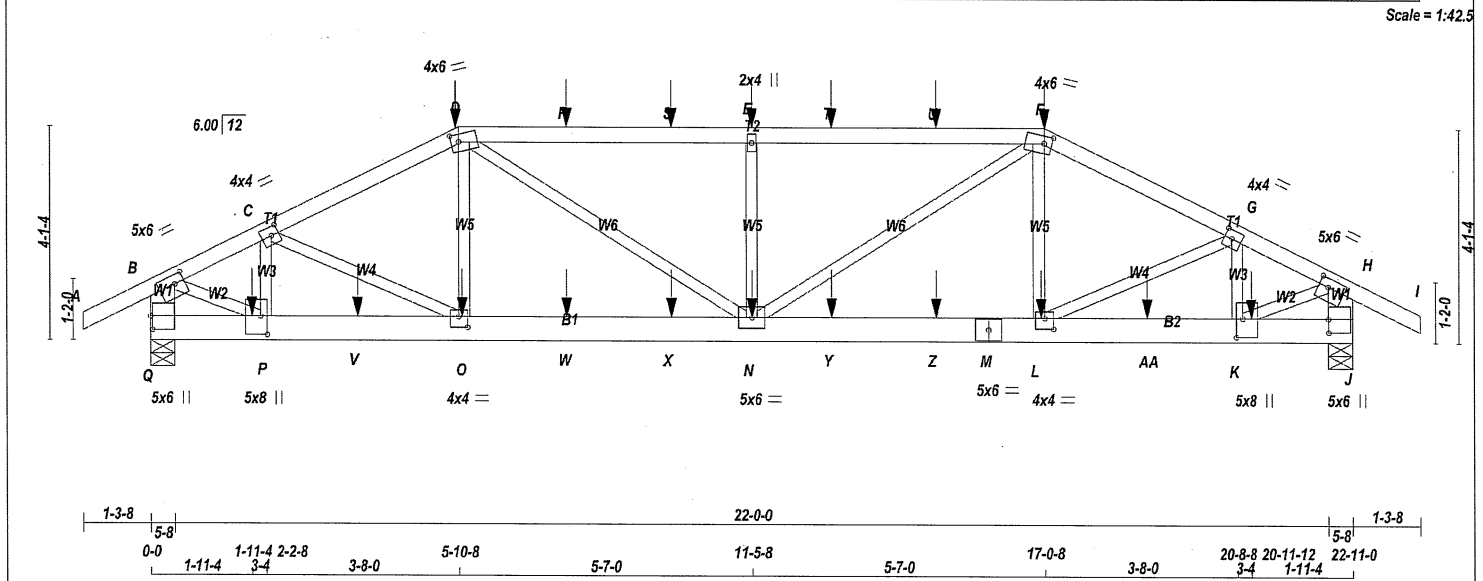
STRUCTURAL COMPONENT ONLY
DWG # TR22080150 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427527	T22Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: PCTW0jclq8p14PIS8Q6CioZ7ts-39sZh1AsdXfne7IHCgWDD0rQsb TpE3tngsqUyrsEV
 1-3-8 0-0 2-2-8 3-8-0 5-10-8 2-0-12 2-0-0 9-11-4 11-5-8 12-11-12 14-11-12 17-0-8 3-8-0 20-8-8 22-11-0 24-2-8
 1-3-8 2-2-8 3-8-0 5-10-8 2-0-12 2-0-0 9-11-4 11-5-8 12-11-12 14-11-12 17-0-8 3-8-0 20-8-8 22-11-0 24-2-8
 Scale = 1:42.5



TOTAL WEIGHT = 2 X 105 = 209 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
Q - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
Q - M	2x6	DRY No.2	SPF
M - J	2x6	DRY No.2	SPF

ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	SIDE(61.0)
D-F 1	12	SIDE(61.0)
F-I 1	12	SIDE(61.0)
Q-B 2	12	TOP
J-H 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q-M 2	12	SIDE(183.1)
M-J 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
O-D 1	6	SIDE(2.3)
L-F 1	6	SIDE(2.3)
2x3 1	6	SIDE(2.3)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT 2339	DOWN 2339	0	5-8
Q VERT 2339	HORIZ 0	0	5-8
J VERT 2339	HORIZ 0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX / MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
Q	1650	1105 / 0
J	1650	1105 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	FR-TO
A-B 0 / 29	P-C -866 / 0
B-C -2704 / 0	C-O 0 / 577
C-D -3313 / 0	O-D -28 / 119
D-R -3858 / 0	R-S -1081 / 0
R-S -3858 / 0	S-E -1081 / 0
S-E -3858 / 0	E-T -28 / 119
E-T -3858 / 0	T-U -1081 / 0
T-U -3858 / 0	U-F -1081 / 0
U-F -3858 / 0	F-G -28 / 119
F-G -3313 / 0	G-H -1081 / 0
G-H -2704 / 0	H-I -1081 / 0
H-I 0 / 29	I-J -1081 / 0
Q-B -2265 / 0	J-K -1081 / 0
J-H -2265 / 0	K-L -1081 / 0

CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	FR-TO
Q-P 0 / 0	P-Q -18.5 / 0.04 (1)
P-V 0 / 2434	V-P -18.5 / 0.20 (1)
V-O 0 / 2434	O-V -18.5 / 0.20 (1)
O-W 0 / 2950	W-O -18.5 / 0.24 (1)
W-X 0 / 2950	X-W -18.5 / 0.24 (1)
X-N 0 / 2950	N-X -18.5 / 0.24 (1)
N-Y 0 / 2950	Y-N -18.5 / 0.24 (1)
Y-Z 0 / 2950	Z-Y -18.5 / 0.24 (1)
Z-M 0 / 2950	M-Z -18.5 / 0.24 (1)
M-L 0 / 2950	L-M -18.5 / 0.24 (1)
L-AA 0 / 2434	AA-L -18.5 / 0.20 (1)
AA-K 0 / 2434	K-AA -18.5 / 0.20 (1)
K-J 0 / 0	J-K -18.5 / 0.04 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-329	-329	---	FRONT	VERT	TOTAL	---	C1
E	11-5-8	-79	-79	---	FRONT	VERT	TOTAL	---	C1
F	17-0-8	-329	-329	---	FRONT	VERT	TOTAL	---	C1
K	20-11-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1
L	16-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	11-5-8	-21	-21	---	FRONT	VERT	TOTAL	---	C1
O	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
P	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 26.7 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 086-14
 - TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL)= L/360 (0.76")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.53/1.00 (D-E:1), BC=0.24/1.00 (N-O:1), WB=0.32/1.00 (B-P:1), SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (D) (INPUT = 0.90)
 JSI METAL= 0.44 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
 DWG # TR22080151

PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427527	T22Z	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 15:41:18 2022 Page 2			

ID: PCTW0jclq8p14PiS8Q6CioZ27ts-39sZh1AsdXfne7IHCgWDD0rQsb_TpE3tfngsqUyrsEV

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-t	MT20	5.0	6.0	2.00	2.25
J	BMV1+p	MT20	5.0	6.0	3.00	
K	BMWW+t	MT20	5.0	8.0	4.25	1.50
L	BMWW-t	MT20	4.0	4.0	2.50	2.00
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.50	2.00
P	BMWW+t	MT20	5.0	8.0	4.25	1.50
Q	BMV1+p	MT20	5.0	6.0	3.00	0.50

NOTES- (1)

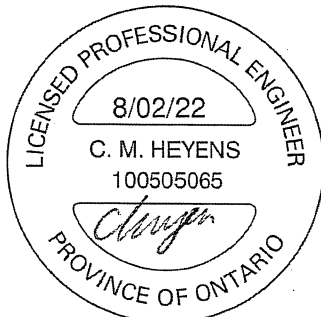
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	7-11-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
S	9-11-4	-79	-79	---	FRONT	VERT	TOTAL	---	C1
T	12-11-12	-79	-79	---	FRONT	VERT	TOTAL	---	C1
U	14-11-12	-79	-79	---	FRONT	VERT	TOTAL	---	C1
V	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
W	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
X	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Y	12-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
Z	14-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AA	18-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

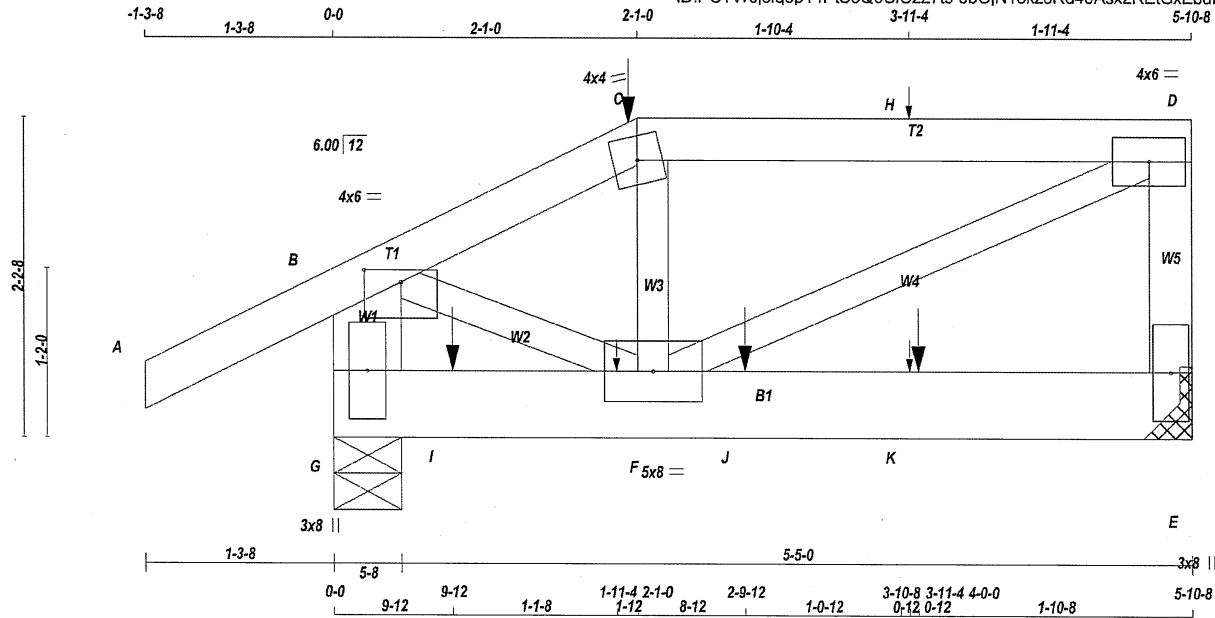
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



STRUCTURAL COMPONENT ONLY
DWG # TR22080151 PG 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427527	T23	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:PCTW0jclq8p14PiS8Q6ClOz27ts-0b0jN1okz5Rd40Asx2REtGxEbuKM8ex6KWvTLayrWEd



Scale = 1:15.3

TOTAL WEIGHT = 2 X 28 = 55 lb
[M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(61.0)
C - D 1	12	SIDE(61.0)
D - E 1	12	TOP
G - B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - E 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	2020	0	2020	0	0
G	1612	0	1612	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 4-0.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	1419	985 / 0	0 / 0	0 / 0	0 / 0	434 / 0	0 / 0
G	1127	813 / 0	0 / 0	0 / 0	0 / 0	315 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 29	F-C	0 / 617
B-C	-2086 / 0	B-F	0 / 1976
C-H	-1898 / 0	F-D	0 / 2170
H-D	-1898 / 0		
E-D	-1120 / 0		
G-B	-1827 / 0		
G-I	0 / 0		
I-F	0 / 0		
F-J	0 / 0		
J-K	0 / 0		
K-E	0 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-1-0	-5	-5	---	FRONT	VERT	DEAD	---	C1
C	2-1-0	-3	-3	---	FRONT	VERT	TOTAL	---	C1
C	2-1-0	-20	-20	---	FRONT	VERT	SNOW	---	C1
F	1-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
I	9-12	-170	-170	---	BACK	VERT	TOTAL	---	C1
J	2-9-12	-251	-251	---	BACK	VERT	TOTAL	---	C1
K	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1
K	4-0-0	-1536	-1536	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.14/1.00 (C-D:1), BC=0.63/1.00 (E-F:1), WB=0.26/1.00 (D-F:1), SSI=0.57/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

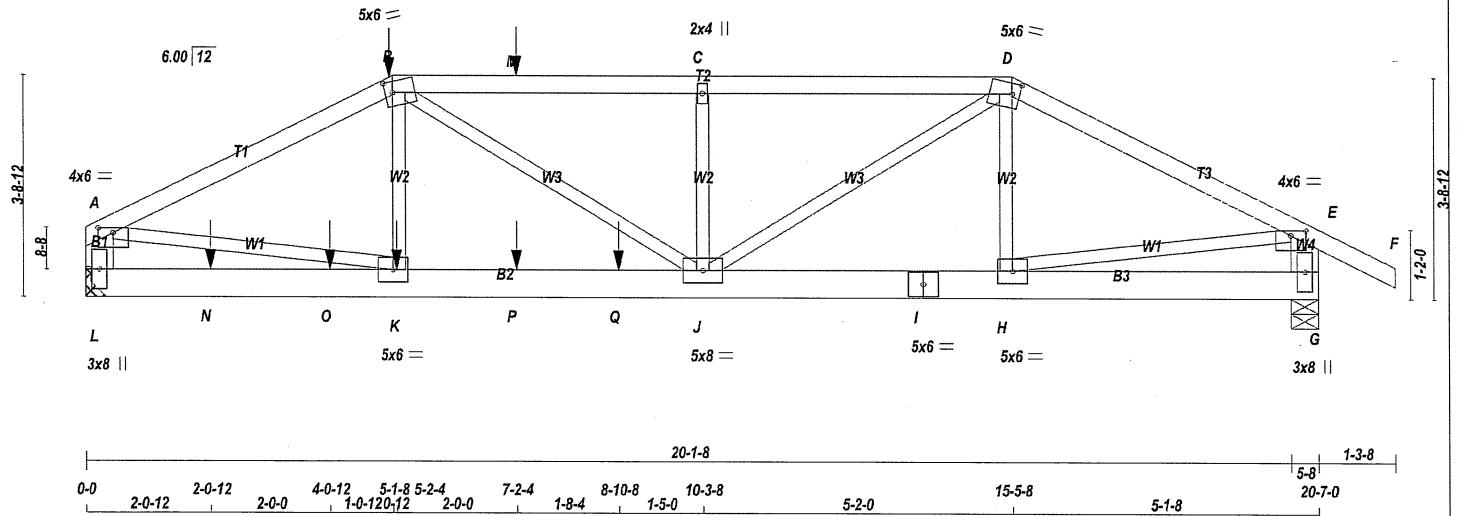
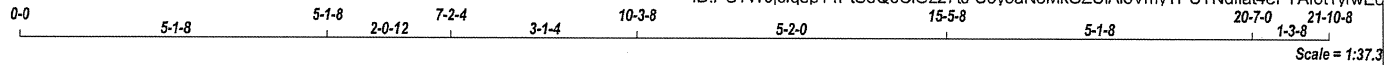
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STRUCTURAL COMPONENT ONLY
DWG # TR22080152 PG 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427527	T24	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 90 = 180 lb [M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
G - E	2x6	DRY	No.2	SPF	
L - I	2x6	DRY	No.2	SPF	
I - G	2x6	DRY	No.2	SPF	
L - A	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B	1	12
B-D	1	12
D-F	1	12
G-E	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-I	2	12
I-G	2	12
A-L	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	2202	0	2202	0	0	5-8	5-8
G	1888	0	1888	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 4-0.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1551	1051 / 0	0 / 0	0 / 0	0 / 0	500 / 0	0 / 0
G	1328	914 / 0	0 / 0	0 / 0	0 / 0	413 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-3222 / 0	K-B	-17 / 123
B-M	-3789 / 0	J-C	0 / 1085
M-C	-3789 / 0	J-D	-635 / 0
C-D	-3789 / 0	J-E	0 / 1860
D-E	-2504 / 0	H-D	-324 / 0
E-F	0 / 29	H-E	0 / 2263
G-E	-1828 / 0	A-K	0 / 2925
L-N	0 / 0		
N-O	0 / 0		
O-K	0 / 0		
K-P	0 / 2879		
P-Q	0 / 2879		
Q-J	0 / 2879		
J-I	0 / 2228		
I-H	0 / 2228		
H-G	0 / 0		
L-A	-2190 / 0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-1-8	-272	-272	---	BACK	VERT	TOTAL	---	C1
K	5-2-4	-17	-17	---	BACK	VERT	TOTAL	---	C1
M	7-2-4	-60	-60	---	BACK	VERT	TOTAL	---	C1
N	2-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
O	4-0-12	-17	-17	---	BACK	VERT	TOTAL	---	C1
P	7-2-4	-17	-17	---	BACK	VERT	TOTAL	---	C1
Q	8-10-8	-756	-756	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(5% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.32/1.00 (B-C-1), BC=0.50/1.00 (J-K-1), WB=0.36/1.00 (A-K-1), SSI=0.28/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MIN	MIN
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.35 (E) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080153

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427527	T24	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

T24

1

2

TRUSS DESC.

GREENPARK HOMES

DRWG NO.	
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	BTW-vp	MT20	4.0	6.0	1.00	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	1.50
C	TMW-vp	MT20	2.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	1.50
E	TMW-vp	MT20	4.0	6.0	1.00	3.00
G	BMV1+p	MT20	3.0	8.0		
H	BMWW-vt	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWWW-vt	MT20	5.0	8.0		
K	BMWW-vt	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



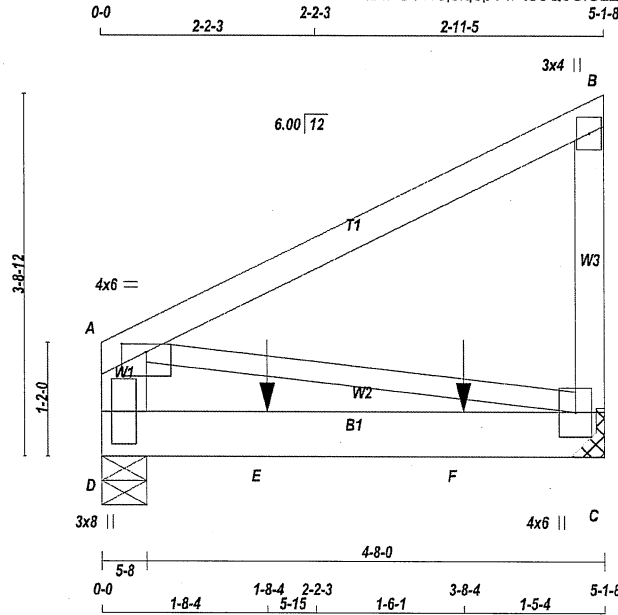
STRUCTURAL COMPONENT ONLY
DWG # TR22080153 PG 2/2

DWG # TR22080153 PG 2/2

JOB NAME 427527	TRUSS NAME T25	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:22.8

TOTAL WEIGHT = 2 X 24 = 48 lb [M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - A	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	TOP
C-B 1	12	TOP
D-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
C	1098	0	1098	0
D	1023	0	1023	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

	1ST CASE COMBINED	MAX./MIN. SNOW	MIN. LIVE	MAX. PERM. LIVE	WIND	DEAD	SOIL
JT	771	538 / 0	0 / 0	0 / 0	0 / 0	233 / 0	0 / 0
C	718	501 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 0	-95.2	-95.2 0.23 (1)	A-C	0 / 0	0.00	0.00 (1)
C-B	-244 / 0	0.0	0.0 0.02 (1)				
D-A	-244 / 0	0.0	0.0 0.01 (1)				
D-E	0 / 0	-18.5	-18.5 0.47 (1)				
E-F	0 / 0	-18.5	-18.5 0.47 (1)				
F-C	0 / 0	-18.5	-18.5 0.47 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-8-4	-539	-539	---	BACK	VERT	TOTAL	---	C1
F	3-8-4	-539	-539	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/654 (0.09")

CSI: TC=0.23/1.00 (A-B:1), BC=0.47/1.00 (C-D:1)
WB=0.00/1.00 (A-C:1), SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

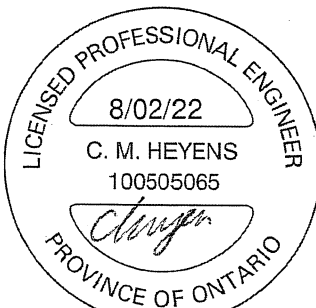
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (A) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080154 PG 1/2

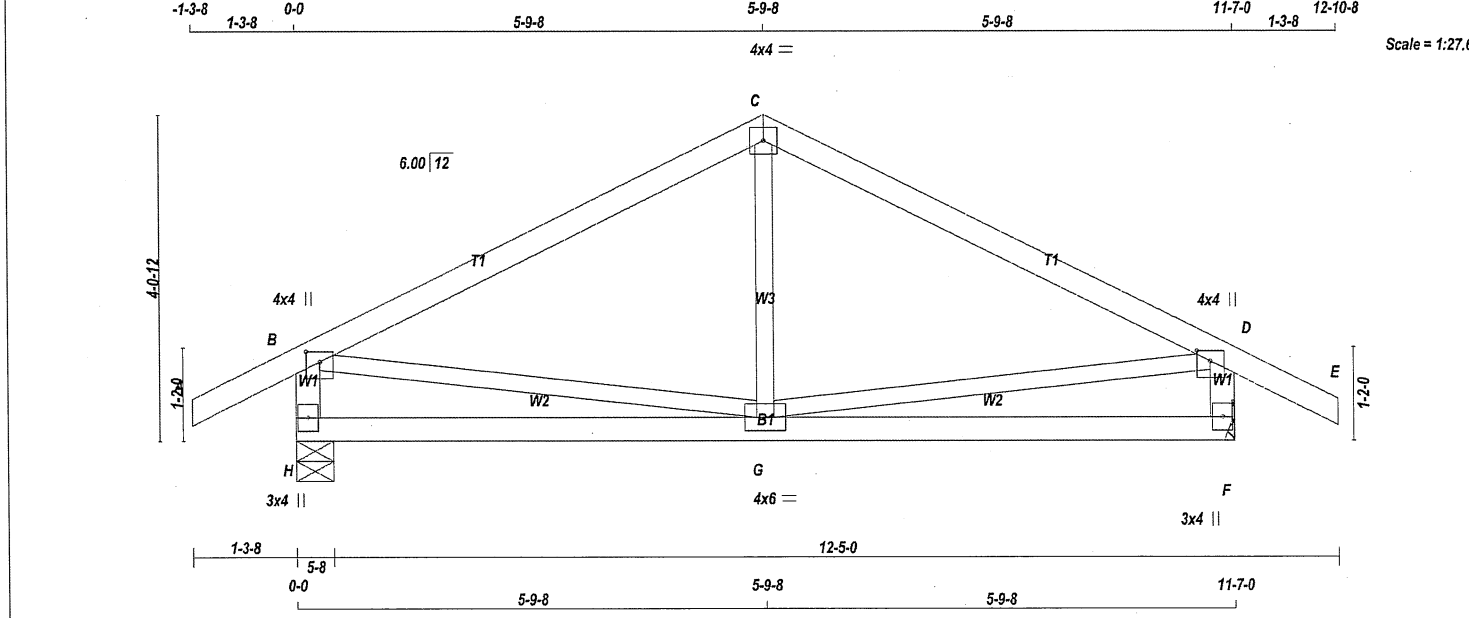
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427527	T25	1	2	GREENPARK HOMES	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches) JT TYPE PLATES W LEN Y X D BMV1+p MT20 3.0 8.0 NOTES- (1) 1) Lateral braces to be a minimum of 2X4 SPF #2.  STRUCTURAL COMPONENT ONLY DWG # TR22080154 PG 2/2		
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TOTAL WEIGHT = 3 X 45 = 135 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.50	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED GROSS REACTION				MAXIMUM FACTORED GROSS REACTION			
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	
H	788	0		788	0	0	5-8
F	788	0		788	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
H	554	382 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0	
F	554	382 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 29	-95.2 -95.2	0.12 (1)	10.00	G-C	-25 / 93	0.03 (4)
B-C	-588 / 0	-95.2 -95.2	0.41 (1)	6.25	B-G	0 / 532	0.12 (1)
C-D	-588 / 0	-95.2 -95.2	0.41 (1)	6.25	G-D	0 / 532	0.12 (1)
D-E	0 / 29	-95.2 -95.2	0.12 (1)	10.00			
H-B	-746 / 0	0.0 0.0	0.08 (1)	7.81			
F-D	-746 / 0	0.0 0.0	0.08 (1)	7.81			
H-G	0 / 0	-18.5 -18.5	0.17 (4)	10.00			
G-F	0 / 0	-18.5 -18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.17/1.00 (F-G:4)
, WB=0.12/1.00 (B-G:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (G) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

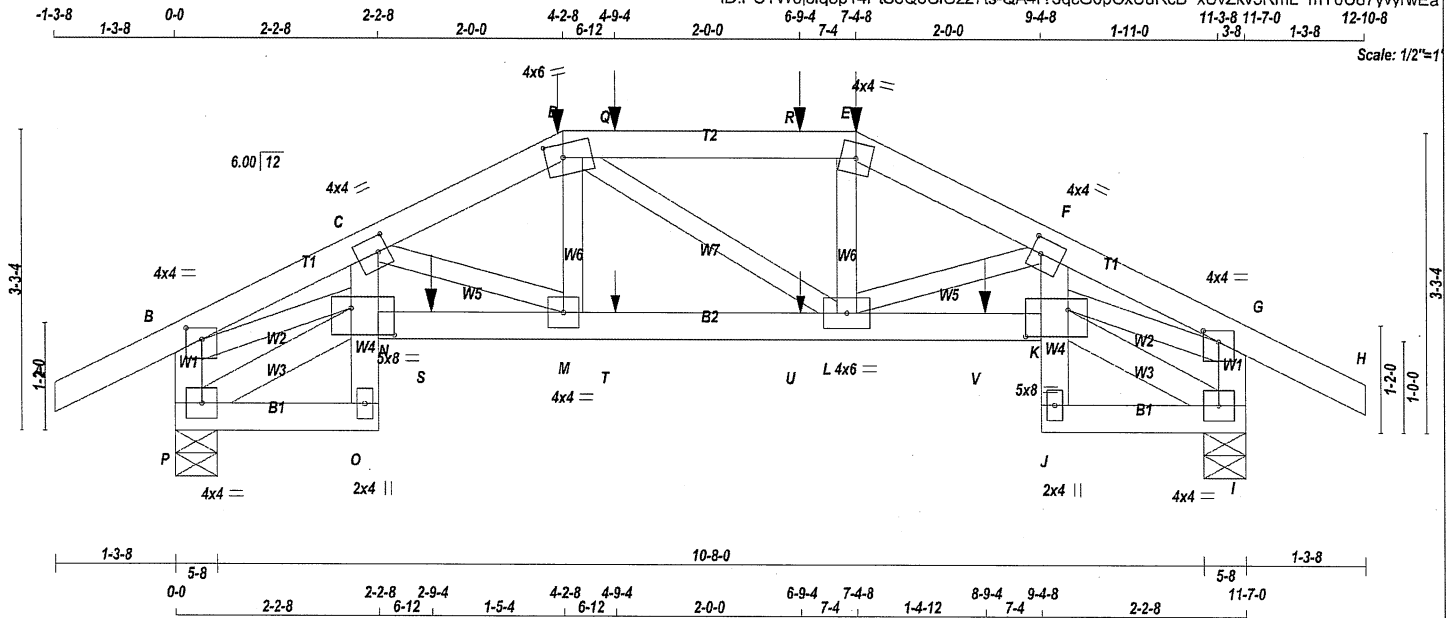


STRUCTURAL COMPONENT ONLY
DWG # TR22080155

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427527	T27	1	1	GREENPARK HOMES	

Tamarack Roof Truss, Burlington

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ID:PCTW0iclg8p14PtS8Q6CioZ27ts-QA4r?3qcG0pCxUuRcB xUvZkv5RmL mY0U87yvvwEa



TOTAL WEIGHT = 53 lb
(M)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
P - O	2x4	DRY	No.2
N - K	2x4	DRY	No.2
J - I	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
C - M	2x3	DRY	No.2
M - D	2x3	DRY	No.2
D - L	2x3	DRY	No.2
L - E	2x3	DRY	No.2
L - F	2x3	DRY	No.2
B - N	2x3	DRY	No.2
K - G	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW-p	MT20	4.0	4.0	1.50	2.00
I	BMVW1-t	MT20	4.0	4.0		
J	BMVW-w	MT20	2.0	4.0		
K	BMVWVW-i	MT20	5.0	8.0	3.50	5.50
L	BMVWVW-t	MT20	4.0	6.0		
M	BMVWVW-t	MT20	4.0	4.0		
N	BMVWVW-i	MT20	5.0	8.0	3.50	5.50
O	BMVW-w	MT20	2.0	4.0		
P	BMVW1-t	MT20	4.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REORD
	VERT	HORZ	DOWN	HORZ
JT	850	0	850	0
P	851	0	851	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
JT	597	415/0	0/0
P	598	415/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
FR-TO		FROM TO	CSI (LC)	FR-TO			
A-B	0 / 29	-95.2	-95.2 0.14 (1)	O-N	0 / 17	0.15 (1)	
B-C	-1447 / 0	-95.2	-95.2 0.11 (1)	N-C	0 / 71	0.16 (1)	
C-D	-1139 / 0	-95.2	-95.2 0.11 (1)	J-K	0 / 17	0.15 (1)	
D-Q	-1031 / 0	-95.2	-95.2 0.20 (1)	K-F	0 / 70	0.16 (1)	
Q-R	-1031 / 0	-95.2	-95.2 0.20 (1)	C-M	-398 / 0	0.07 (1)	
R-E	-1031 / 0	-95.2	-95.2 0.20 (1)	M-D	0 / 197	0.05 (1)	
E-F	-1130 / 0	-95.2	-95.2 0.11 (1)	D-L	0 / 3	0.00 (4)	
F-G	-1445 / 0	-95.2	-95.2 0.11 (1)	L-E	0 / 196	0.05 (1)	
G-H	0 / 29	-95.2	-95.2 0.14 (1)	L-F	-402 / 0	0.07 (1)	
P-B	-782 / 0	0.0	0.0 0.09 (1)	K-I	-107 / 0	0.01 (1)	
I-G	-782 / 0	0.0	0.0 0.09 (1)	P-N	-107 / 0	0.01 (1)	
				B-N	0 / 1296	0.32 (1)	
				K-G	0 / 1294	0.32 (1)	
P-O	0 / 95	-18.5	-18.5 0.03 (4)				
N-S	0 / 1389	-18.5	-18.5 0.26 (1)				
S-M	0 / 1389	-18.5	-18.5 0.26 (1)				
M-T	0 / 1029	-18.5	-18.5 0.20 (1)				
T-U	0 / 1029	-18.5	-18.5 0.20 (1)				
U-L	0 / 1029	-18.5	-18.5 0.20 (1)				
L-V	0 / 1386	-18.5	-18.5 0.26 (1)				
V-K	0 / 1386	-18.5	-18.5 0.26 (1)				
J-I	0 / 95	-18.5	-18.5 0.03 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	4-2-8	-6	-6	---	FRONT	VERT	DEAD	---	C1
D	4-2-8	-28	-28	---	FRONT	VERT	SNOW	---	C1
E	7-4-8	-6	-6	---	FRONT	VERT	DEAD	---	C1
E	7-4-8	-28	-28	---	FRONT	VERT	SNOW	---	C1
Q	4-9-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
R	6-9-4	-5	-5	---	FRONT	VERT	TOTAL	---	C1
S	2-9-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1
T	4-9-4	-0	-0	---	FRONT	VERT	TOTAL	---	C1
U	6-9-4	-0	-0	---	FRONT	VERT	TOTAL	---	C1
V	8-9-4	-1	-1	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.20/1.00 (D-E:1), BC=0.26/1.00 (M-N:1), WB=0.32/1.00 (B-N:1), SI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2



STRUCTURAL COMPONENT ONLY
DWG # TR22080156 PG 1/2

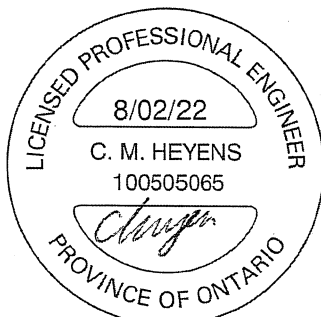
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
427527	T27	1	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080156 PG 2/2



TOTAL WEIGHT = $7 \times 17 = 118 \text{ lb}$

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

1) Lateral braces to be a minimum of 2X4 SPF #2.

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	GROSS REACTION		GROSS REACTION		BRG	BRG	
	VERT	HORZ	DOWN	HORZ			UPLIFT
E	542	0	542	0	0	5-8	5-8
C	210	0	210	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	380	269 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	144	118 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED	FACTORED		MAX. UNBRAC	MEMB.	MAX. FACTORED	MAX
	FORCE	VERT. LOAD	LC1			FORCE	
	(LBS)	(PLF)	CSI (LC)			(LBS)	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
E-B	-479 / 0	0.0	0.0	0.13 (4)	7.81		
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00		
B-C	-31 / 0	-95.2	-95.2	0.56 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

SPECIFIED LOADS:				
TOP	CH.	LL =	26.7	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	40.1	PSF

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4)
 , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	650	371	1747	788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

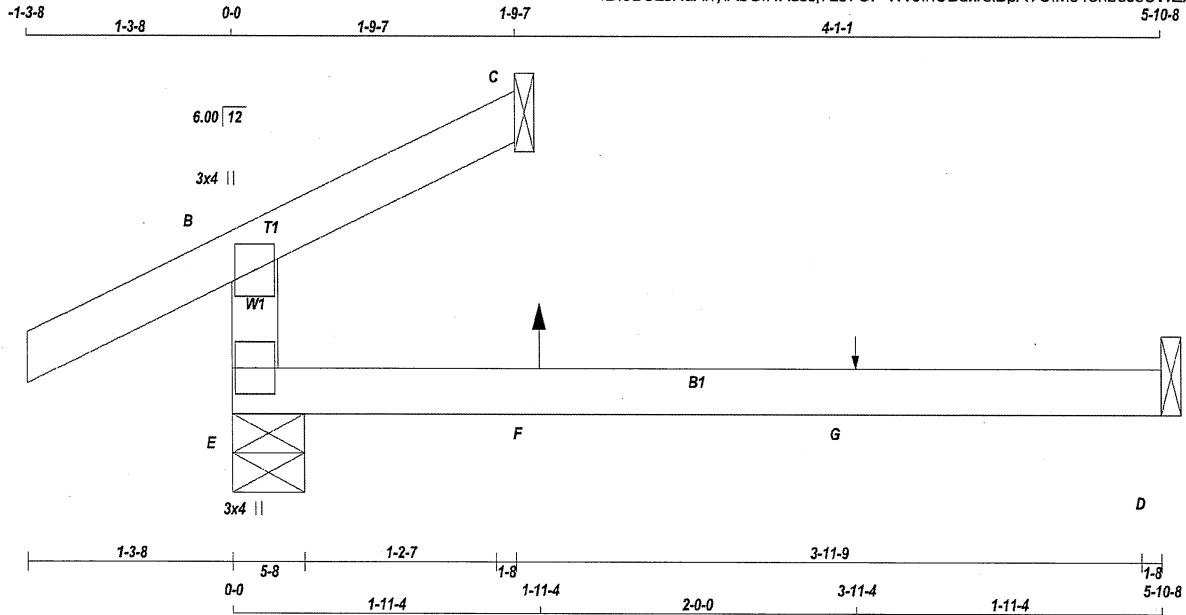
JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080111

JOB NAME 427503	TRUSS NAME J2	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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5-10-8



Scale = 1:14.1

TOTAL WEIGHT = 2 X 12 = 23 lb

LUMBER				DESCR.	SPF
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
E	293	0	293	0	0	5-8	5-8		
C	64	0	64	0	1-8	1-8	1-8		
D	43	0	52	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	206	144 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	47	22 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	35	0 / -3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX	MAX. UNBRAC
FR-TO		FROM	TO		LENGTH	FR-TO			
E-B	-237 / 0	0.0	0.0	0.11 (4)	7.81				
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00				
B-C	-10 / 9	-95.2	-95.2	0.08 (4)	6.25				
E-F	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
F-G	0 / 0	-18.5	-18.5	0.14 (4)	10.00				
G-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	BACK	VERT	TOTAL	—	C1
G	3-11-4	1	1	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	(PSI)	(PL)	SECTION
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

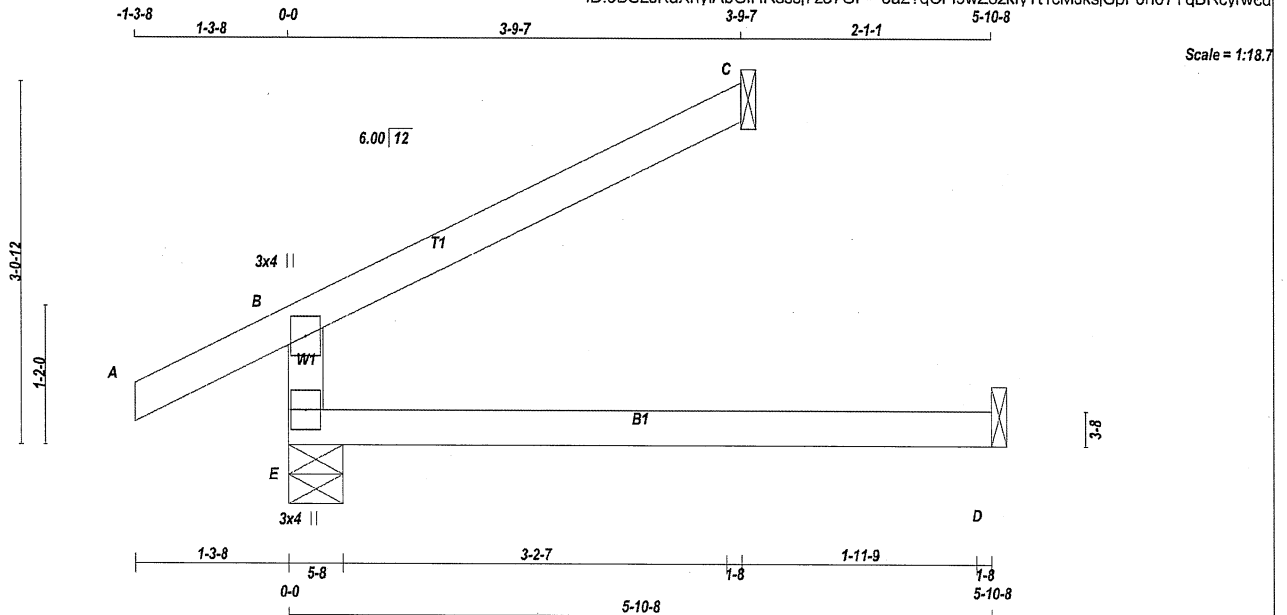
JSI GRIP= 0.09 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080112

JOB NAME 427503	TRUSS NAME J3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 14 = 28 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQRD BRG IN-SX
VERT	HORZ	DOWN	HORZ	UPLIFT
E	418	0	418	0
C	135	0	135	0
D	45	0	50	0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	295	199 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	93	76 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
E-B	-354 / 0	0.0	0.0	0.13 (4)	7.81	E-B	-354 / 0	0.0	0.13 (4)
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00	A-B	0 / 29	-95.2	-95.2
B-C	-20 / 0	-95.2	-95.2	0.23 (1)	6.25	B-C	-20 / 0	-95.2	-95.2
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00	E-D	0 / 0	-18.5	-18.5

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	40.1	PSF	

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.23/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)

JSI METAL= 0.10 (B) (INPUT = 1.00)

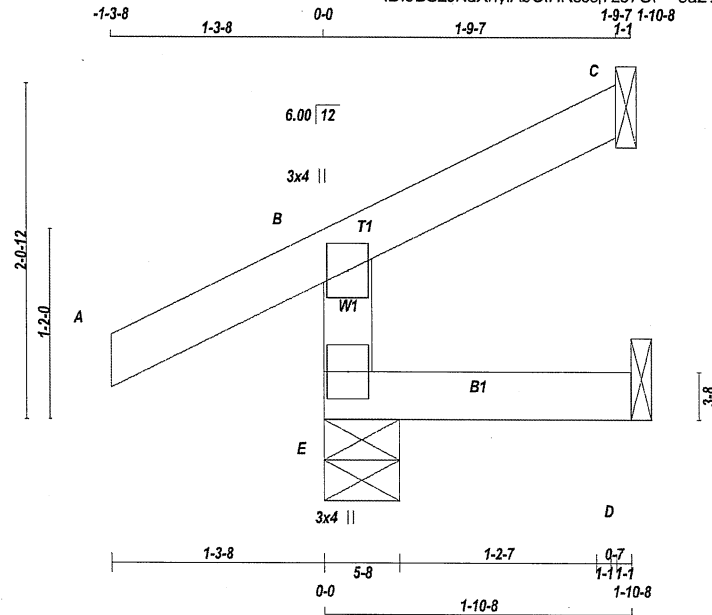


STRUCTURAL COMPONENT ONLY
DWG # TR22080113

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427503	J4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:13.6

TOTAL WEIGHT = 2 X 7 = 14 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	280	0	280	0	0	5-8	5-8
C	46	0	46	0	-24	1-8	1-8
D	8	0	17	0	-2	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	195	148 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	32	25 / -19	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
E-B	-253 / 0	0.0	0.0	0.04 (5)	7.81		
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00		
B-C	-17 / 0	-95.2	-95.2	0.09 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5)
, WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

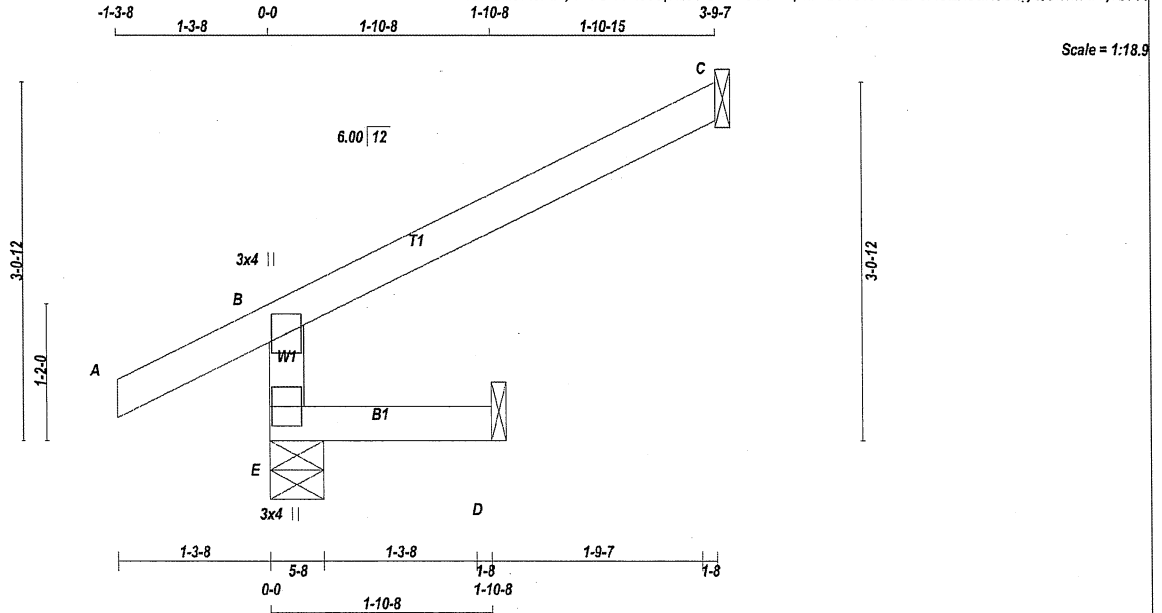
JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080114

JOB NAME 427503	TRUSS NAME J5	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 10 = 19 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	374	0	374	0	5-8	5-8
C	135	0	135	0	1-8	1-8
D	16	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	259	199 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	93	76 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-354 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 29	-95.2	-95.2	0.13 (5)	10.00		
B-C	-20 / 0	-95.2	-95.2	0.23 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.23/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX	MIN	MAX
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

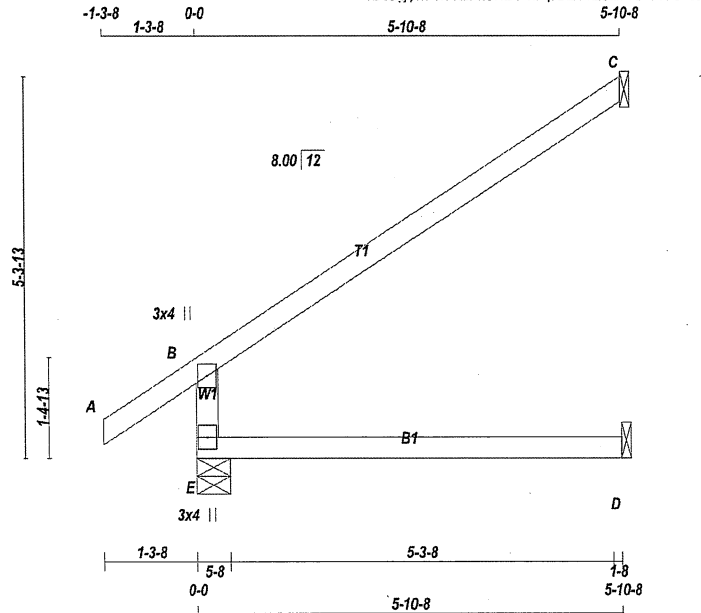
JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080115

JOB NAME 427510	TRUSS NAME J6	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:30.8

TOTAL WEIGHT = 5 X 18 = 89 lb

LUMBER				DESCR.	SPF
N. L. G. A. RULES	SIZE	LUMBER	No.2		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
E	543	0	543	0	0	5-8	5-8	5-8	5-8
C	210	0	210	0	0	1-8	1-8	1-8	1-8
D	46	0	51	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	381	270 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	144	118 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEB S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	UNBRAC LENGTH FR-TO	MEMB.	FORCE (LBS)	MAX (LC)
FR-TO		FROM TO			FR-TO		
E-B	-480 / 0	0.0	0.0	0.12 (4)	7.81		
A-B	0 / 36	-95.2	-95.2	0.13 (1)	10.00		
B-C	-39 / 0	-95.2	-95.2	0.56 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.03")

CSI: TC=0.56/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

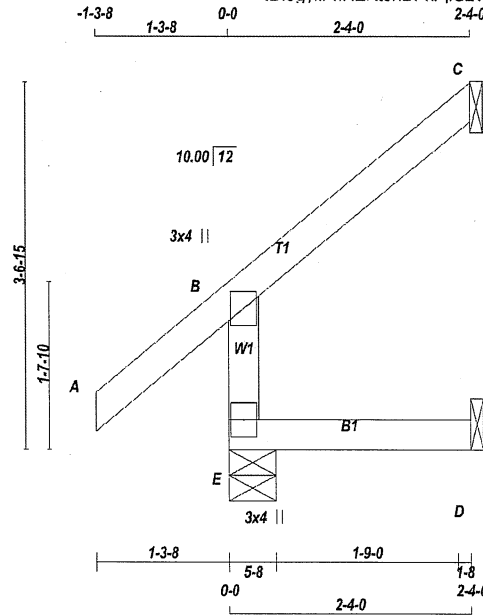
JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080130

JOB NAME 427510	TRUSS NAME J7	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:21.6

TOTAL WEIGHT = 5 X 10 = 48 lb [M]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	294	0	294	0	5-8	5-8
C	84	0	84	0	1-8	1-8
D	20	0	22	0	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	205	152 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	58	47 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	16	0 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
E-B	-270 / 0	0.0	0.0 0.01 (4)	7.81	
A-B	0 / 42	-95.2	-95.2 0.13 (5)	10.00	
B-C	-17 / 0	-95.2	-95.2 0.09 (1)	6.25	
E-D	0 / 0	-18.5	-18.5 0.02 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 26.7 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

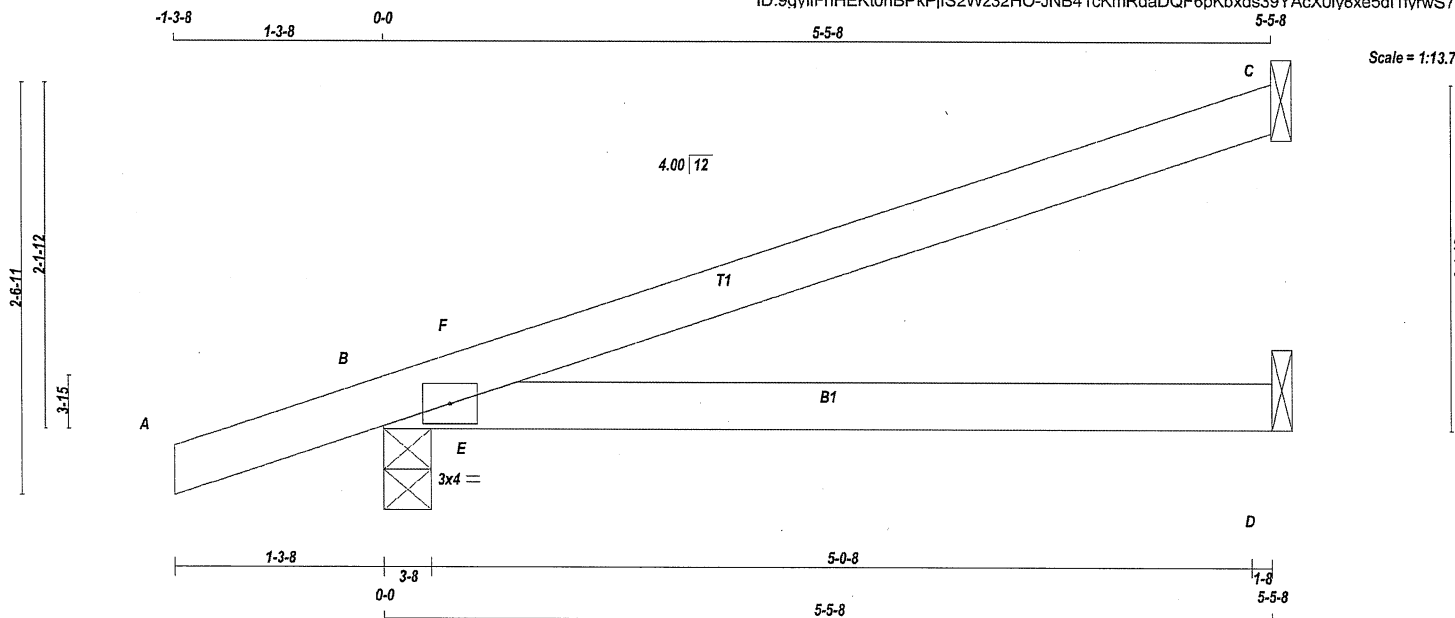


STRUCTURAL COMPONENT ONLY
DWG # TR22080131

JOB NAME 427510	TRUSS NAME J10	QUANTITY 6	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Tue Aug 2 10:53:42 2022 Page 1

ID:9gyllFnHEK10nBPkPjS2Wz32HO-JNB41cKmRdaDQF6pKbxdS39YAcX0iy8xe5dl1fyrwS7



TOTAL WEIGHT = 6 X 14 = 87 lb [M]

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	TOP CH.	LL = 26.7 PSF
A - C	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG	DL = 6.0 PSF	BOT CH.	LL = 0.0 PSF
B - D	2x4	DRY	No.2	SPF	VERT	HORZ	DOWN	HORZ	DL = 7.4 PSF	TOTAL LOAD	= 40.1 PSF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
C	156	123 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
B	307	217 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	63	23 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 19	-95.2	-95.2	0.12 (1)	10.00	E-F	-301 / 11
B-F	-18 / 13	-95.2	-95.2	0.06 (4)	6.25		
F-C	-1 / 2	-95.2	-95.2	0.37 (1)	10.00		
B-E	0 / 0	-18.5	-18.5	0.26 (1)	10.00		
E-D	0 / 0	-18.5	-18.5	0.26 (1)	10.00		

SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/589 (0.11")

CSI: TC=0.37/1.00 (C-F:1), BC=0.26/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

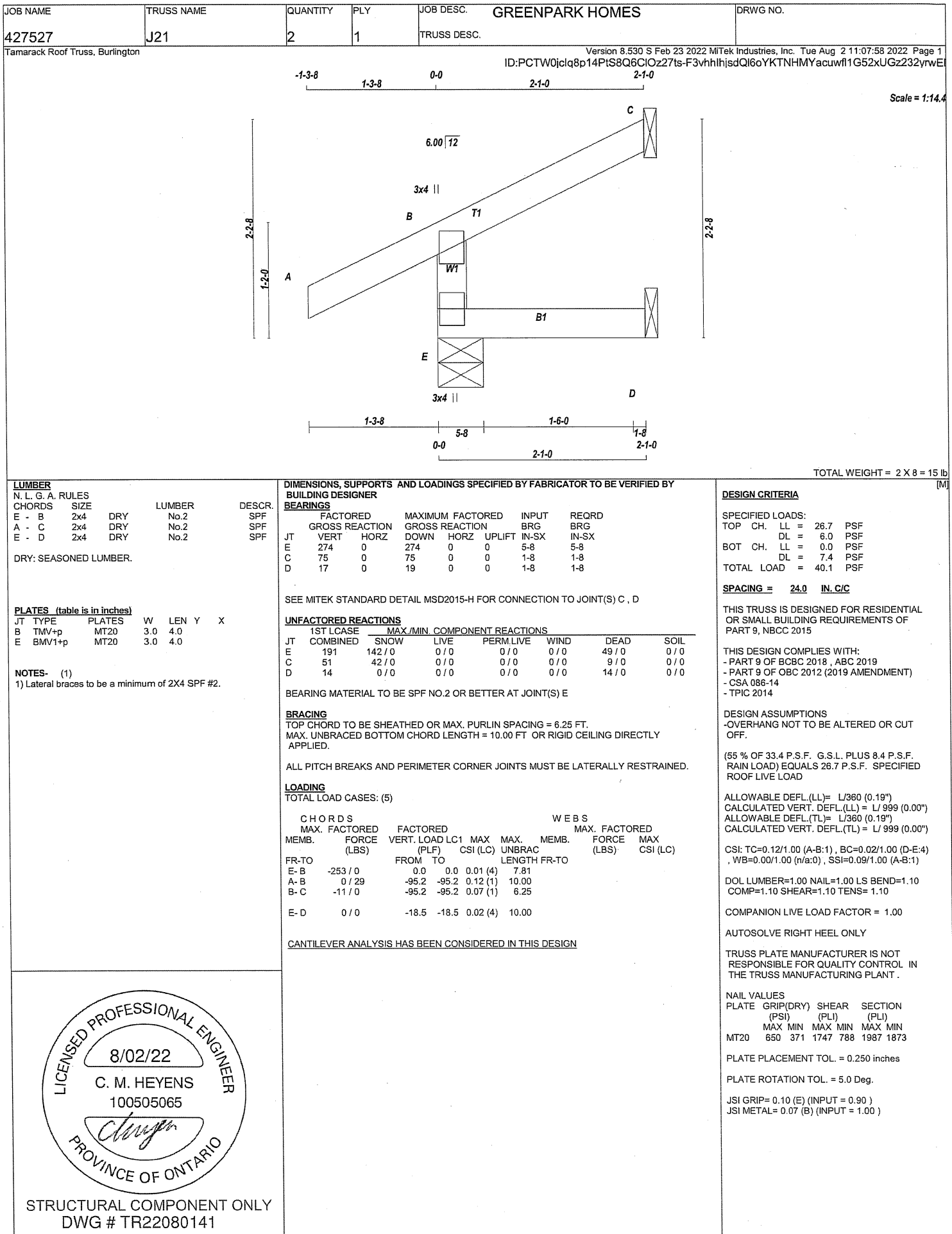
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

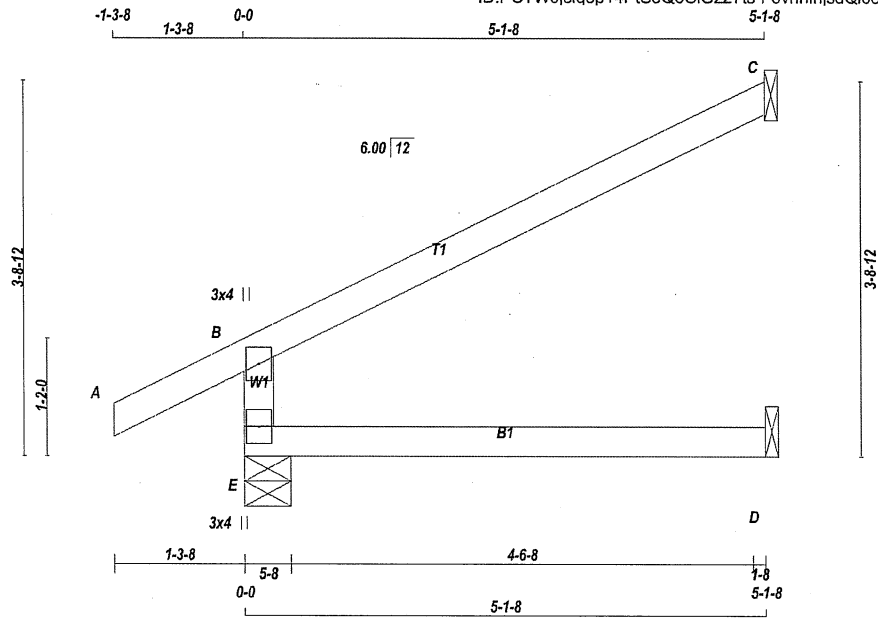


STRUCTURAL COMPONENT ONLY
DWG # TR22080132



JOB NAME 427527	TRUSS NAME J22	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: PCTW0jclq8p14PtS8Q6CIZ27ts-F3vhhjisdQl6oYKTNHMYacpCfknG52xUGz232yrwE



Scale = 1:22.0

TOTAL WEIGHT = 2 X 15 = 30 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
E	489	489	5-8	5-8
C	183	183	1-8	1-8
D	40	44	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	343	244 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0
C	126	103 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	32	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO		
E-B	-434 / 0	0.0	0.0	0.09 (4)	7.81	E-B	-434 / 0	0.0
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00	A-B	0 / 29	-95.2
B-C	-27 / 0	-95.2	-95.2	0.43 (1)	6.25	B-C	-27 / 0	-95.2
E-D	0 / 0	-18.5	-18.5	0.10 (4)	10.00	E-D	0 / 0	-18.5

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.43/1.00 (B-C:1) , BC=0.10/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

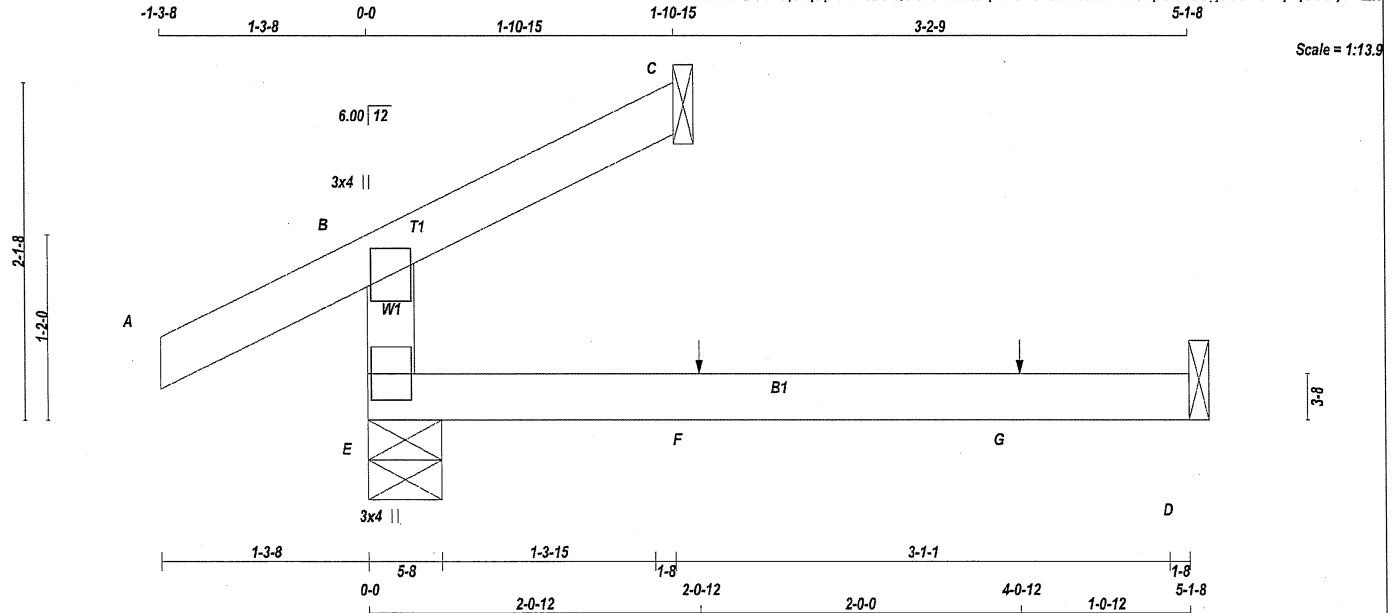
JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080142

JOB NAME 427527	TRUSS NAME J23	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:PCTW0jclq8p14PtS8Q6ClOz27ts-jFT3veiLdxZckx7W14pb4o83q330?Y14jwbbUyrwEK



TOTAL WEIGHT = 11 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	298	0	298	0	5-8	5-8
C	69	0	69	0	1-8	1-8
D	40	0	44	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	211	136 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
C	47	39 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	32	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-242 / 0	0.0	0.0	0.09 (4)	7.81		
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00		
B-C	-10 / 0	-95.2	-95.2	0.06 (1)	10.00		
E-F	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
F-G	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
G-D	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	2-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1
G	4-0-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.10/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

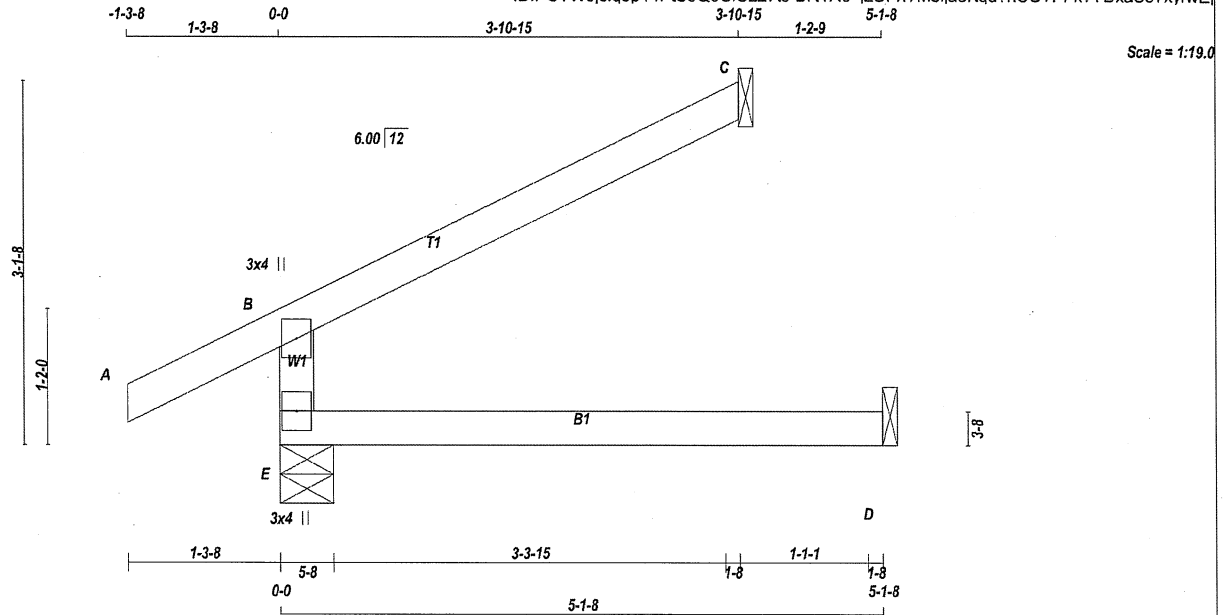
JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080143

JOB NAME 427527	TRUSS NAME J24	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: PCTW0jclq8p14PtS8Q6CioZ27ts-BR1R6 izOFhTM5iaoKqd?hCUTPFk?YDxaS97xyrWEI



TOTAL WEIGHT = 13 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS SIZE

E - B 2x4 DRY No.2

A - C 2x4 DRY No.2

E - D 2x4 DRY No.2

LUMBER

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0		
E	BMV1+P	MT20	3.0	4.0		

NOTES-

(1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	417	0	417	0	5-8	5-8
C	140	0	140	0	1-8	1-8
D	40	0	44	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	293	203 / 0	0 / 0	0 / 0	0 / 0	90 / 0	0 / 0
C	96	78 / 0	0 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	32	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX (LC)
FR-TO		FROM TO			FR-TO		
E-B	-362 / 0	0.0	0.0	0.09 (4)	7.81		
A-B	0 / 29	-95.2	-95.2	0.12 (1)	10.00		
B-C	-21 / 0	-95.2	-95.2	0.25 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.25/1.00 (B-C-1), BC=0.10/1.00 (D-E-4), WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

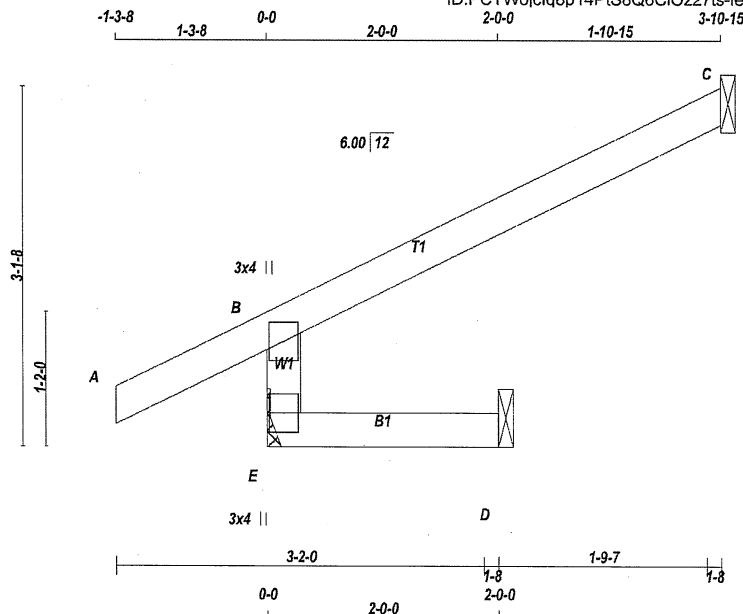
JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080144

JOB NAME 427527	TRUSS NAME J26	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:PCTW0jclq8p14PtS8Q6ClOz27ts-febqKKkb9YpKzFHv8Vr3ADDNEtmITRoNAECigNyrwEi



Scale = 1:19.2

TOTAL WEIGHT = 10 lb [M]

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
E - B	2x4	DRY	No.2		SPF
A - C	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	382	0	382	0
C	140	0	140	0
D	16	0	16	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	265	203 / 0	0 / 0	0 / 0	0 / 0	62 / 0
C	96	78 / 0	0 / 0	0 / 0	0 / 0	18 / 0
D	13	0 / 0	0 / 0	0 / 0	0 / 0	13 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. FACTORED UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
E-B	-362 / 0	0.0	0.0	0.01 (4)	7.81		
A-B	0 / 29	-95.2	-95.2	0.13 (5)	10.00		
B-C	-21 / 0	-95.2	-95.2	0.25 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.25/1.00 (B-C:1), BC=0.02/1.00 (D-E:4)
, WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

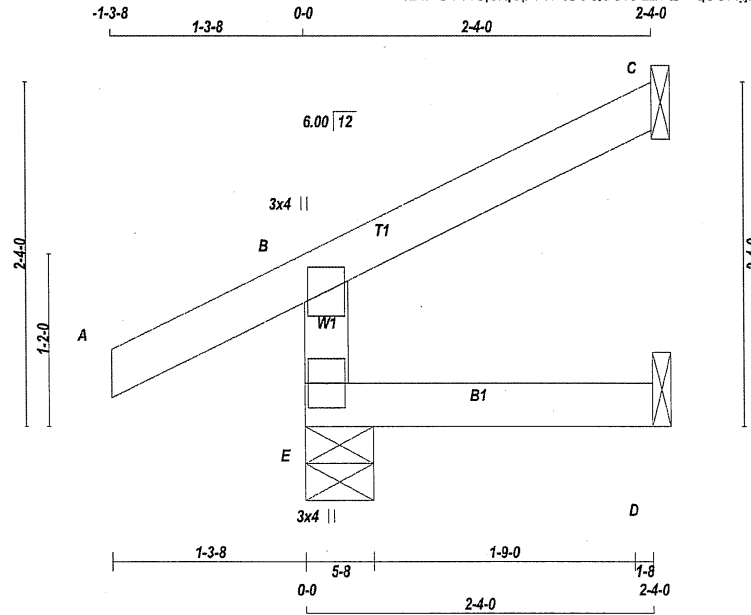
JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080146

JOB NAME 427527	TRUSS NAME J27	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:PCTW0jclq8p14PtS8Q6CioZ27ts-7q8CXgkDvsxBbPs5iDMliQmatH6vCu2WPuxFCpyrwh



Scale = 1:15.0

TOTAL WEIGHT = 4 X 8 = 33 lb

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
E	292	0	292	0	0	5-8	5-8		
C	84	0	84	0	0	1-8	1-8		
D	19	0	21	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	203	150 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
C	58	47 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FORCE (LBS)	MAX	MAX.
FR-TO		FROM TO				FR-TO			
E-B	-268 / 0	0.0	0.0	0.01 (4)	7.81				
A-B	0 / 29	-95.2	-95.2	0.13 (5)	10.00				
B-C	-12 / 0	-95.2	-95.2	0.09 (1)	6.25				
E-D	0 / 0	-18.5	-18.5	0.02 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:5), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080147

JOB NAME

427527

TRUSS NAME

J28

QUANTITY

5

PLY

1

JOB DESC.

GREENPARK HOMES

DRWG NO.

TRUSS DESC.

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

B - D 2x4 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

DESCR.

SPF

SPF

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
C	173	0	173	0	1-8	1-8
B	367	0	367	0	3-8	3-8
D	66	0	66	0	5-0	5-0

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
C	120	94 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
B	256	184 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
D	49	19 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	
FR-TO					FR-TO				
A-B	0 / 19	-95.2	-95.2	0.12 (1)	10.00	E-F	-190 / 7	0.00 (1)	
B-F	-13 / 0	-95.2	-95.2	0.05 (4)	6.25				
F-C	0 / 2	-95.2	-95.2	0.21 (1)	10.00				
B-E	0 / 0	-18.5	-18.5	0.16 (1)	10.00				
E-D	0 / 0	-18.5	-18.5	0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 26.7 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")

ALLOWABLE DEFL.(TL)= L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.21/1.00 (C-F:1) , BC=0.16/1.00 (D-E:1)

, WB=0.00/1.00 (E-F:1) , SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)

JSI METAL= 0.06 (B) (INPUT = 1.00)

8/02/22

C. M. HEYENS

100505065

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

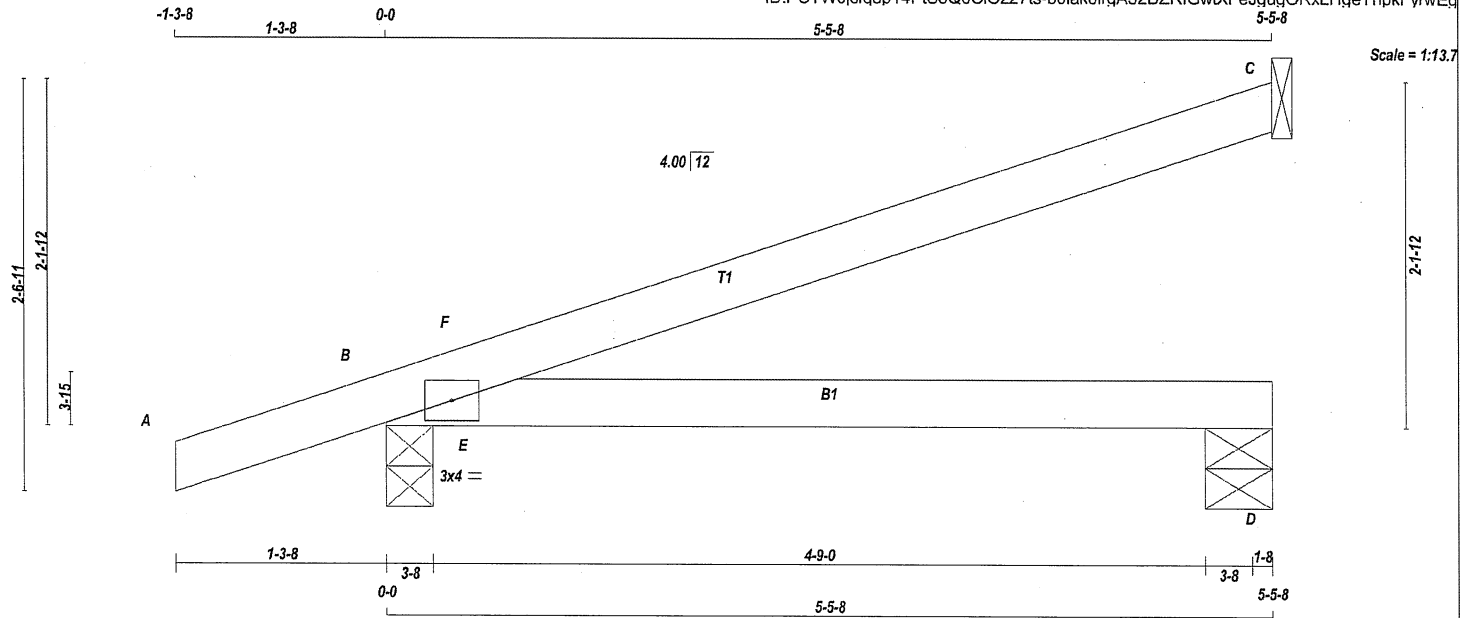
STRUCTURAL COMPONENT ONLY

DWG # TR22080148

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
427527	J29	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 14 = 29 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

B - D 2x4 DRY No.2

LUMBER

No.2

DESCR.

SPF

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
C	226	0	226	0	1-8	1-8
B	438	0	438	0	3-8	3-8
D	84	0	84	0	5-0	5-0

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE	SNOW	LIVE				
C	156	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0
B	307	217 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	63	23 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 19	-95.2	-95.2 0.12 (1)	10.00	E-F	-301 / 11	0.00 (1)
B-F	-18 / 13	-95.2	-95.2 0.06 (4)	6.25			
F-C	-1 / 2	-95.2	-95.2 0.37 (1)	10.00			
B-E	0 / 0	-18.5	-18.5 0.26 (1)	10.00			
E-D	0 / 0	-18.5	-18.5 0.26 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 26.7 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 40.1 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/589 (0.11")

CSI: TC=0.37/1.00 (C-F:1), BC=0.26/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.90)

JSI METAL= 0.07 (B) (INPUT = 1.00)



STRUCTURAL COMPONENT ONLY
DWG # TR22080149

EWP DESIGN INC.

(905) 832-2250

FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

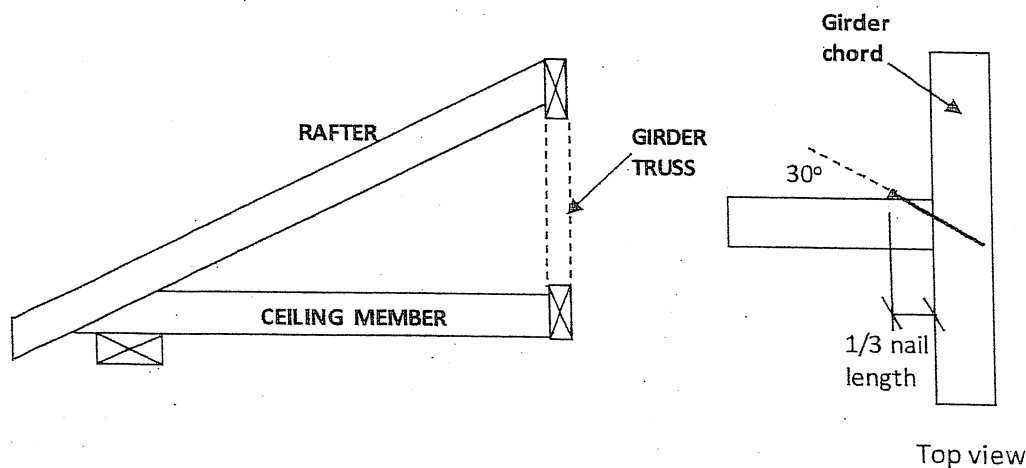
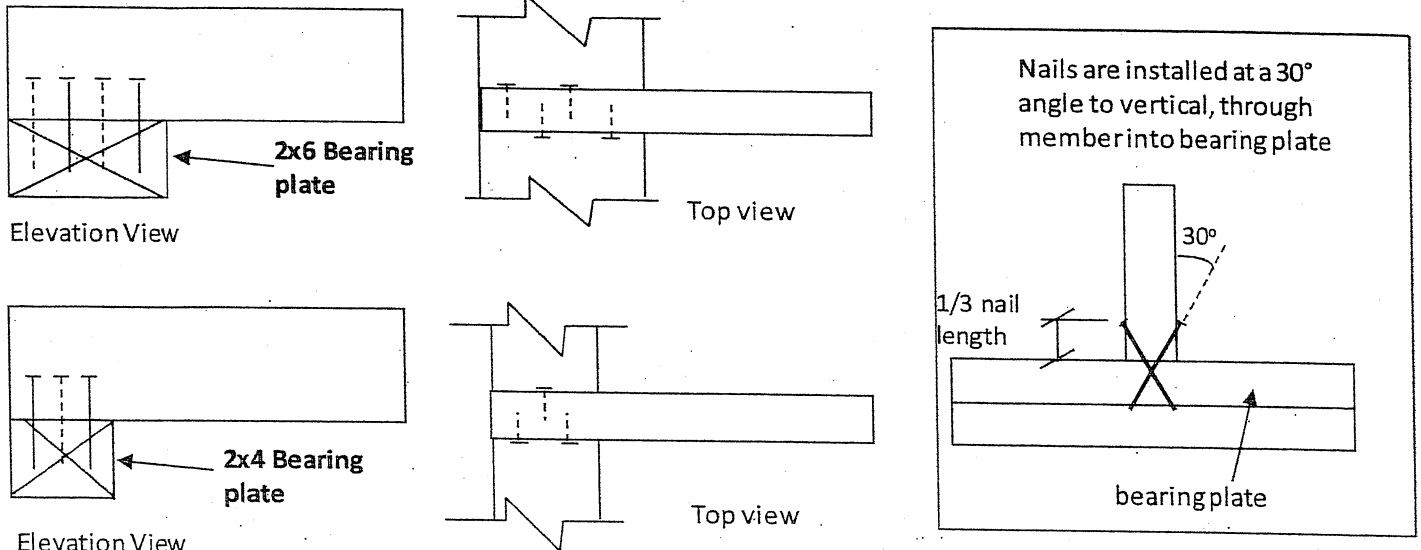


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



TOE-NAIL CAPACITY DETAILS

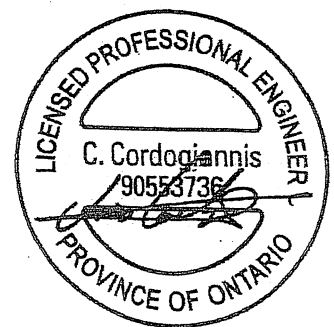
Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear
Nailing
Top View

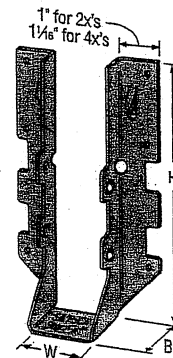
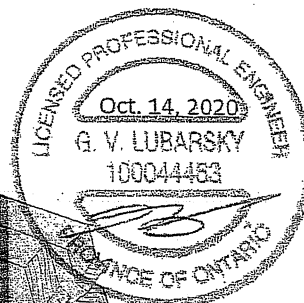
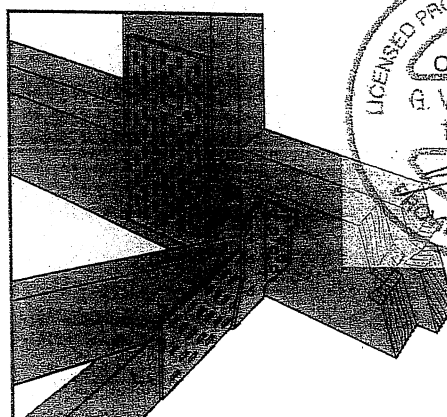


Double-Shear
Nailing
Side View;
Do not
bend tab

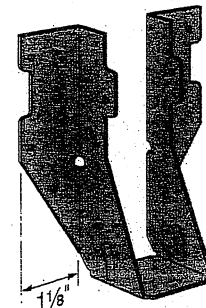


Dome Double-Shear
Nailing
Side View
(available on
some models)

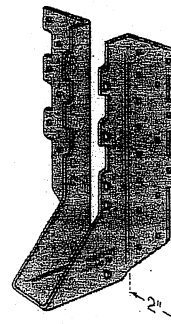
Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



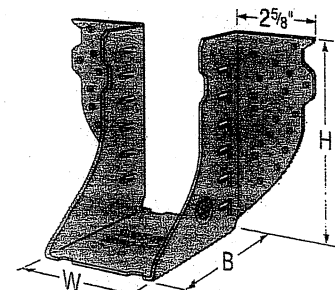
✓ LUS28



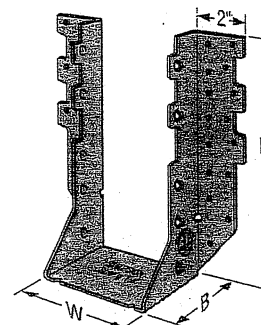
LU26L



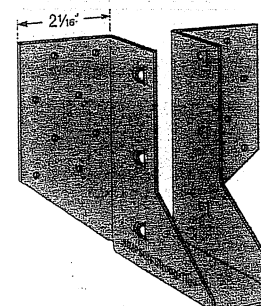
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



LJS26DS

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

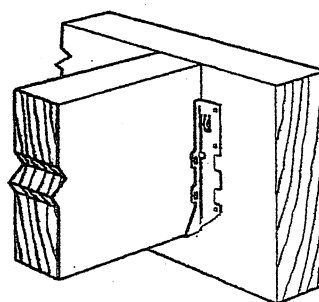
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

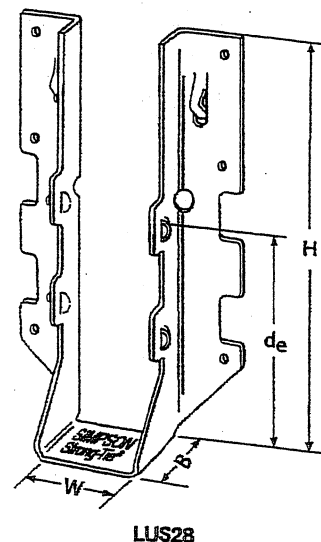
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

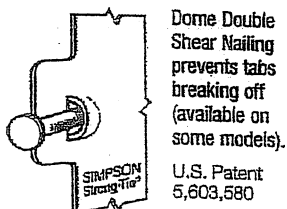


Typical LUS
Installation

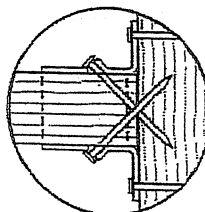


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LUS24	18	1½	3½	1¾	1½	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8½	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

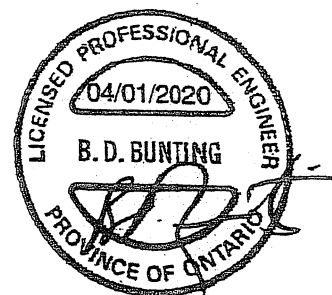
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

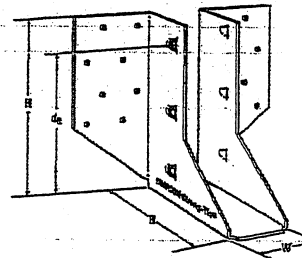
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

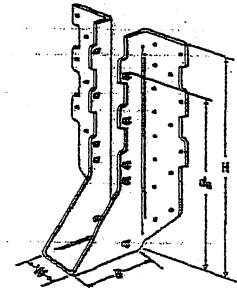
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

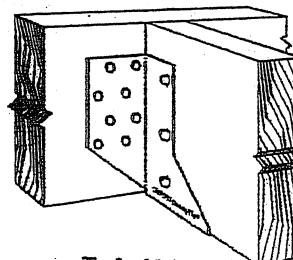
- See current catalogue for options



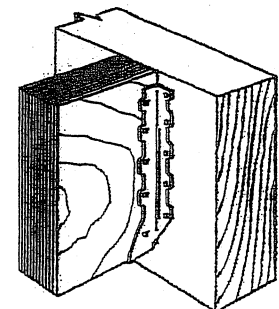
LJS26DS



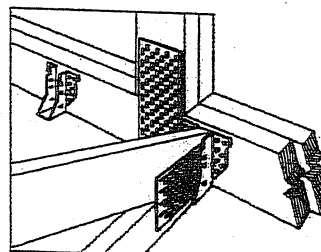
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



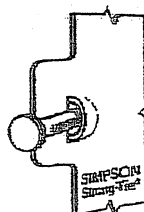
Typical HUS
Installation



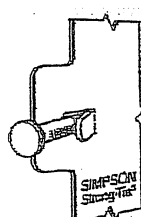
Typical HUS installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist		D.F.-I		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d		2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 15/16	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1 1/8	7 1/32	3	6 1/32	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1 1/8	9 1/32	3	7 1/32	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1 7/16	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

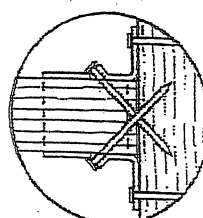
1. d_g is the distance from the seat of the hanger to the highest joist nail.



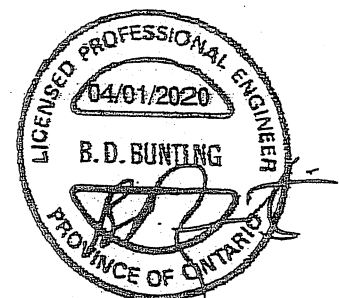
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

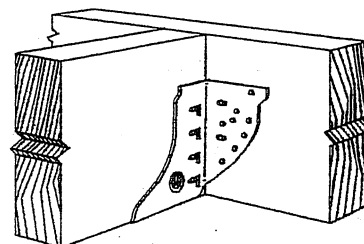
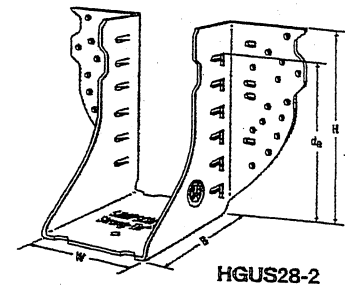
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

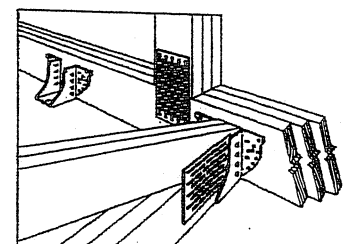
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



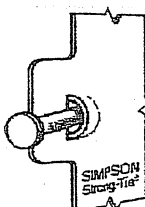
Typical HGUS Installation



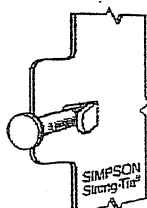
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹		Face	Joist	D.Fir-L		S-P-F	
									Uplift	Normal	Uplift	Normal
HGUS26	12	1⅞	5⅜	5	4⅝	(20) 16d	(8) 16d		2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅞	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅞	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅞	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅞	(36) 16d	(12) 16d		3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅞	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅞	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅞	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS210	12	1⅞	9⅞	5	7⅞	(46) 16d	(16) 16d		3535	11070	2510	8090
HGUS210-2	12	3⅞	9⅞	4	8⅞	(46) 16d	(16) 16d		6840	14015	4855	10270
HGUS210-3	12	4⅞	9⅞	4	8⅞	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-4	12	6⅞	9⅞	4	8⅞	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅞	(56) 16d	(20) 16d		7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅞	(66) 16d	(22) 16d		10130	16400	7195	11645

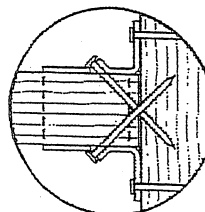
1. d_g is the distance from the seat of the hanger to the highest joist nail.



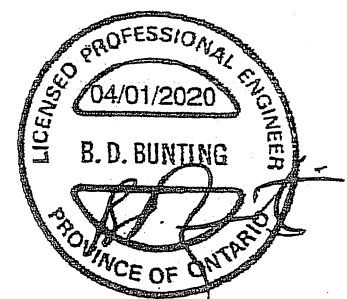
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

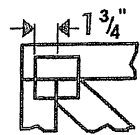
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T-SPECHGUS20 3/20 exp. 6/22

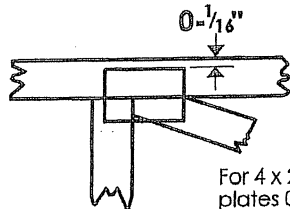
(800) 999-5099
strongtie.com

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

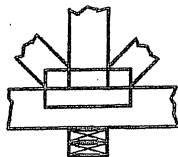
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

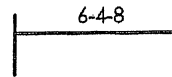


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

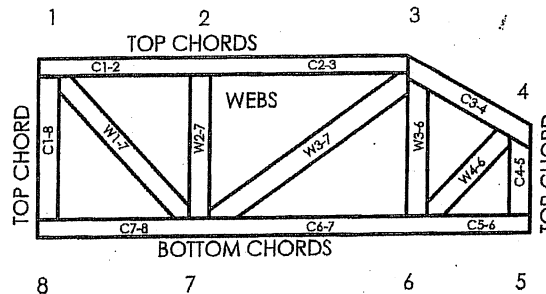
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MIL-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.