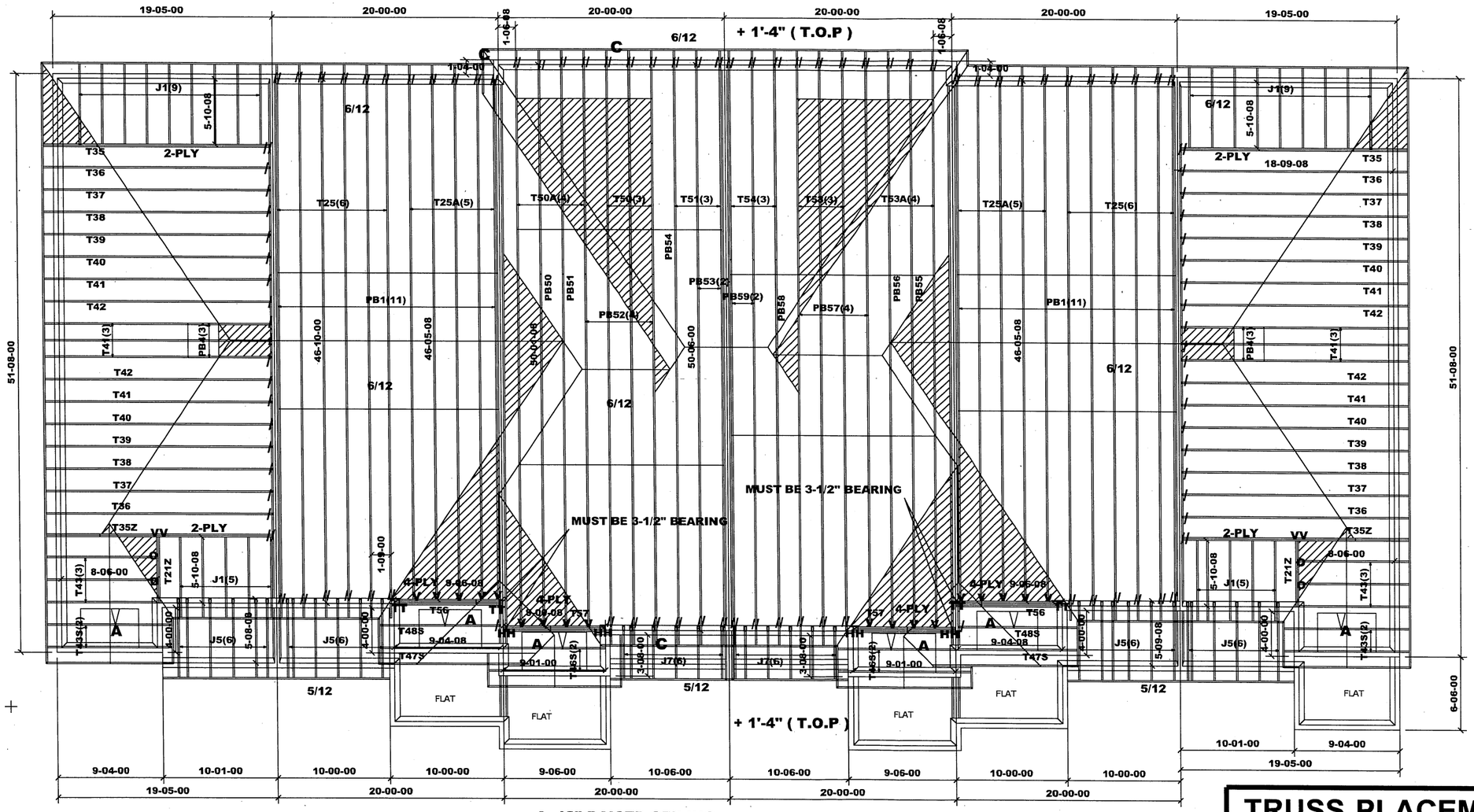
TH-43	TH-44	TH-45	TH-46	TH-47	TH-48
WILLOW 3E	WILLOW 1	WILLOW 2	WILLOW 2	WILLOW 1	WILLOW 3E
EL-2	EL-2	EL-2	EL-2	EL-2	EL-2
428212	428213	428214	428215	428213	428212

NOT THE GRANTING OF A PERMIT NOR REVIEWING OF SPECS & DRAWINGS NOR INSPECTIONS MADE DURING INSTALLATION BY THE OFFICIAL HAVING JURISDICTION SHALL RELIEVE THE OWNER FROM REQUIREMENTS OF THE ONTARIO BUILDING CODE AND ANY OTHER REFERENCED REQUIREMENTS.



9/12 ROOF PITCH UNLESS NOTED OTHERWISE

A- 12" RAISED CEILING

C -1'-4" RAISED FASCIA

ASPHALT SHINGLES
FINISHED OVERHANG: 12"
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012 (2019 amendment). Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DESIGN CONFORMS WITH OBC 2012 (2019 amendment) OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 33.4 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 26.7 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

HARDWARE:
LUS24 - (**O**)
LJS26DS - (V)
HGUS26-2 - (**XX**)
H2.5A- (/)
HTS16 -(T)
H8-(H)

DENOTES:
CONVENTIONAL FRAMING

E22085411 - E22085450

M15086

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

TRUSS PLACEMENT PLAN.

This is a truss placement plan only, NOT a final roof framing plan. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss drawings for each component identified on this placement plan.

Please verify that all dimensions match the dimensions found on the job. The Building designer is responsible for the temporary/ permanent bracing of the roof and floor system and its integration into the bracing of the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsability of the building designer unless otherwise noted in this plan. Building designer to review and aprove this plan to acertain comformity to his overall structural plan.

TAMARACK ROOF TRUSSES INC.

Job Track: 52956	Builder / Location: GREEN PARK HOMES / CAMBRIDGE		Model / Elevation: BLOCK 122 -WILLOW / TH43 TO TH48 (EL.2) — Mitek ver 8.5.3.233
Layout ID: 428162	Project: BARLASSINA		
Plan Log: 206472	Date: 2022-07-27	Designer: YPG	

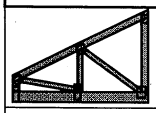
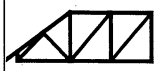
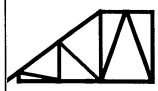
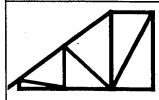
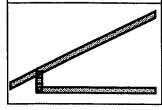
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
Builder:	GREEN PARK HOMES	PlanLog:	206472
Project:	BARLASSINA	Layout ID:	428212
Location:	CAMBRIDGE	Ref #	
Model:	BLOCK 122	Page:	1 of 2
Lot #:		Date:	08-16-2022
Elevation:	TH-43,TH48 (WILLOW 3 E , EL2)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

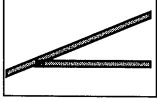
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T21Z Jack-Closed Girder	6 /12	**5-10-08**	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		
	1 2-ply	T35 Half Hip Girder	9 /12	**18-09-08**	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	176.49 109.33		
	1 2-ply	T35Z Half Hip Girder	9 /12	**18-09-08**	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	176.49 109.33		
	2	T36 Half Hip	9 /12	**18-09-08**	5-01-04	2 x 4	1-03-08	1-06-04 5-01-04	153.85 98.33		
	2	T37 Half Hip	9 /12	**18-09-08**	6-01-04	2 x 4	1-03-08	1-06-04 6-01-04	166.98 106.67		
	2	T38 Half Hip	9 /12	**18-09-08**	7-01-04	2 x 4	1-03-08	1-06-04 7-01-04	176.84 113.33		
	2	T39 Half Hip	9 /12	**18-09-08**	8-01-04	2 x 4	1-03-08	1-06-04 8-01-04	191.55 121.67		
	2	T40 Half Hip	9 /12	**18-09-08**	9-01-04	2 x 4	1-03-08	1-06-04 9-01-04	202.89 127.00		
	5	T41 Half Hip	9 /12	**18-09-08**	10-01-04	2 x 4	1-03-08	1-06-04 10-01-04	531.01 335.00		
	2	T42 Half Hip	9 /12	**18-09-08**	11-01-04	2 x 4	1-03-08	1-06-04 11-01-04	208.02 128.67		
	3	T43 Common	9 /12	**8-06-00**	4-08-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	114.84 77.50		
	2	T43S Roof Special	9 /12	**8-06-00**	4-08-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	90.89 67.33		
	3	PB4 Piggyback	9 /12	**7-04-03**	2-00-00	2 x 4		2-00-00	68.41 46.50		
	14	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
Builder:	GREEN PARK HOMES	PlanLog:	206472
Project:	BARLASSINA	Layout ID:	428212
Location:	CAMBRIDGE	Ref #	
Model:	BLOCK 122	Page:	2 of 2
Lot #:		Date:	08-16-2022
Elevation:	TH-43,TH48 (WILLOW 3 E , EL2)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	**6**	**J5 Jack-Open**	5 /12	**5-08-08**	3-02-13	2 x 4	1-03-08	4-01 2-08-09	91.77 56.00		

TOTAL # TRUSS= **51** TOTAL BFT OF ALL TRUSSES= **1682.99** BFT. TOTAL WEIGHT OF ALL TRSSES **2642.77** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
21	Hardware	H2.5A	
2	Hardware	LUS24	
1	Hardware	LUS26-2	

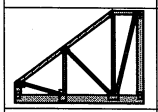
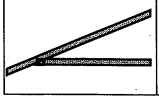
TOTAL NUMBER OF ITEMS= **24**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
Builder:	GREEN PARK HOMES	PlanLog:	206472
Project:	BARLASSINA	Layout ID:	428213
Location:	CAMBRIDGE	Ref #	
Model:	BLOCK 122	Page:	1 of 1
Lot #:		Date:	08-16-2022
Elevation:	TH-44,TH-47 (WILLOW 1 ,EL.2)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	6	T25 Piggyback Base	6 /12	**46-10-00**	9-10-00	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	1926.24 1160.00		
	5	T25A Piggyback Base	6 /12	**46-05-08**	9-10-00	2 x 6	1-03-08	1-02-00 1-04-04	1543.07 935.00		
	1	T47S Half Hip	9 /12	**9-04-08**	4-00-04	2 x 4	1-03-08	1-06-04 4-00-04	51.13 34.83		
	1	T48S Half Hip	9 /12	**9-04-08**	5-06-04	2 x 4	1-03-08	1-06-04 5-06-04	58.55 39.33		
	1 4-ply	T56 Half Hip Girder	9 /12	**9-06-08**	6-11-02	2 x 4 2 x 8		1-06-04 6-11-02	270.02 174.00		
	11	PB1 Piggyback	6 /12	**12-02-00**	3-00-08	2 x 4			319.22 199.83		
	6	J5 Jack-Open	5 /12	**5-08-08**	3-02-13	2 x 4	1-03-08	4-01 2-08-09	91.77 56.00		

TOTAL # TRUSS= **34** TOTAL BFT OF ALL TRUSSES= **2598.99** BFT. TOTAL WEIGHT OF ALL TRSSES **4260** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
35	Hardware	H2.5A	
4		HTS16	
5	Hardware	LJS26DS	

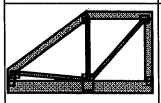
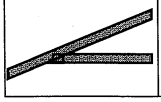
TOTAL NUMBER OF ITEMS= **44**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
Builder:	GREEN PARK HOMES	PlanLog:	206472
Project:	BARLASSINA	Layout ID:	428214
Location:	CAMBRIDGE	Ref #	
Model:	BLOCK 122	Page:	1 of 1
Lot #:		Date:	08-16-2022
Elevation:	TH-45 (WILLOW 2, EL.2)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T46S Roof Special	9 /12	**9-01-00**	4-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	95.12 67.33		
	3	T50 Piggyback Base	6 /12	**50-06-00**	9-10-06	2 x 6	1-03-08 1-03-08	2-06-00 2-06-00	949.89 575.50		
	4	T50A Roof Special	6 /12	**50-01-08**	9-10-06	2 x 6	1-03-08	2-06-00 2-08-04	1251.29 763.33		
	3	T51 Piggyback Base	6 /12	**50-06-00**	9-10-06	2 x 6	1-03-08 1-03-08	2-06-00 2-06-00	919.22 548.00		
	1 4-ply	T57 Half Hip Girder	9 /12	**9-00-08**	5-02-02	2 x 6 2 x 10		1-06-04 5-02-02	261.57 164.67		
	1	PB50 Piggyback	6 /12	**17-00-08**	1-06-00	2 x 4			47.94 31.50		
	1	PB51 Piggyback	6 /12	**17-00-08**	3-00-00	2 x 4			46.18 30.00		
	4	PB52 Piggyback	6 /12	**17-00-08**	4-03-02	2 x 4			197.24 128.00		
	2	PB53 Piggyback	6 /12	**21-00-08**	5-03-02	2 x 4			123.81 76.67		
	1	PB54 Piggyback	6 /12	**21-00-08**	4-07-12	2 x 4			68.57 43.50		
	6	J7 Jack-Open	5 /12	**3-04-08**	2-03-02	2 x 4	1-03-08	4-01 1-08-15	58.51 40.00		

TOTAL # TRUSS= **31** TOTAL BFT OF ALL TRUSSES= **2468.5** BFT. TOTAL WEIGHT OF ALL TRSSES **4019.34** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
32	Hardware	H2.5A	
4	Hardware	H8	
4	Hardware	LJS26DS	

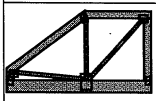
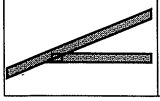
TOTAL NUMBER OF ITEMS= **40**

DELIVERY SHIPLIST

TAMARACK ROOF TRUSSES INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	52956
Builder:	GREEN PARK HOMES	PlanLog:	206472
Project:	BARLASSINA	Layout ID:	428215
Location:	CAMBRIDGE	Ref #	
Model:	BLOCK 122	Page:	1 of 1
Lot #:		Date:	08-16-2022
Elevation:	TH-46 (WILLOW 2 ,EL.2)	Designer:	
		Sales Rep:	Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T46S Roof Special	9 /12	**9-01-00**	4-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	95.12 67.33		
	3	T53 Piggyback Base	6 /12	**50-06-00**	9-10-06	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	938.4 579.50		
	4	T53A Roof Special	6 /12	**50-01-08**	9-10-06	2 x 6	1-03-08	1-02-00 1-04-04	1231.99 759.33		
	3	T54 Piggyback Base	6 /12	**50-06-00**	9-10-06	2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	916.88 544.50		
	1 4-ply	T57 Half Hip Girder	9 /12	**9-00-08**	5-02-02	2 x 6 2 x 10		1-06-04 5-02-02	261.57 164.67		
	1	PB55 Piggyback	6 /12	**14-04-08**	1-06-00	2 x 4			39.72 25.50		
	1	PB56 Piggyback	6 /12	**14-04-08**	3-00-00	2 x 4			38.35 25.00		
	4	PB57 Piggyback	6 /12	**14-04-08**	3-07-02	2 x 4			139.21 83.33		
	1	PB58 Piggyback	6 /12	**15-08-08**	3-03-12	2 x 4			42.31 27.00		
	2	PB59 Piggyback	6 /12	**15-08-08**	3-11-02	2 x 4			76.59 46.67		
	6	J7 Jack-Open	5 /12	**3-04-08**	2-03-02	2 x 4	1-03-08	4-01 1-08-15	58.51 40.00		

TOTAL # TRUSS= **31** TOTAL BFT OF ALL TRUSSES= **2362.83** BFT. TOTAL WEIGHT OF ALL TRSSES **3838.65** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
32	Hardware	H2.5A	
4	Hardware	H8	
4	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **40**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T21Z		2	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085426

Alpa Roof Truss, Maple

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Fri Aug 12 10:33:06 2022 Page 1
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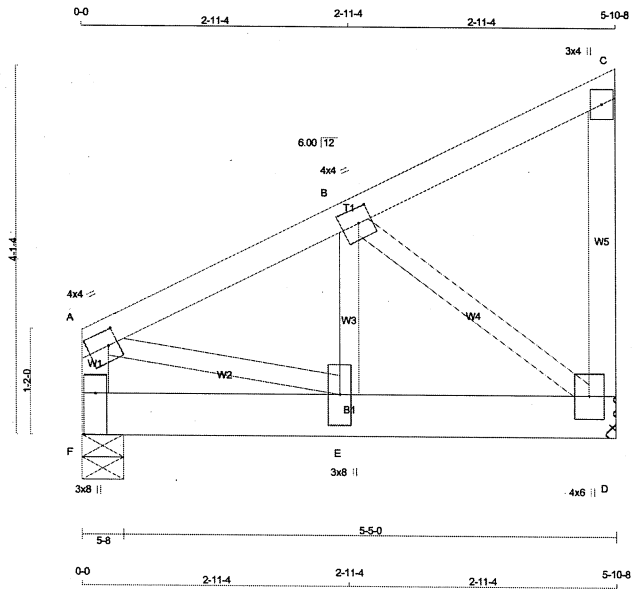

TOTAL WEIGHT = 4 X 29 = 115 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
F- A 1	12	TOP
A- C 1	12	TOP
C- D 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
F- D 2	12	SIDE(92.4)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	3.0	8.0		
F	BMV1+p	MT20	3.0	8.0	Edge	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	877	0	877	0	0	5-8	1-8
D	877	0	877	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	619	412 / 0	0 / 0	0 / 0	0 / 0	207 / 0	0 / 0
D	619	412 / 0	0 / 0	0 / 0	0 / 0	207 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-621 / 0	0.0	0.0	0.03 (1)	7.81
A- B	-735 / 0	-95.2	-95.2	0.07 (1)	6.25
B- C	-13 / 0	-95.2	-95.2	0.07 (1)	6.25
D- C	-110 / 0	0.0	0.0	0.01 (1)	7.81
F- E	0 / 0	-203.3	-203.3	0.06 (1)	10.00
E- D	0 / 669	-203.3	-203.3	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 692	0.09 (1)
E- B	0 / 507	0.06 (1)
B- D	-842 / 0	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 8-6-0
END DISTANCE = 5-10-8
END SPAN CARRIED = 8-6-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.07/1.00 (A-B:1) , BC=0.10/1.00 (D-E:1) , WB=0.10/1.00 (B-D:1) , SSI=0.11/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (A) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T25		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085427

Alpa Roof Truss, Maple

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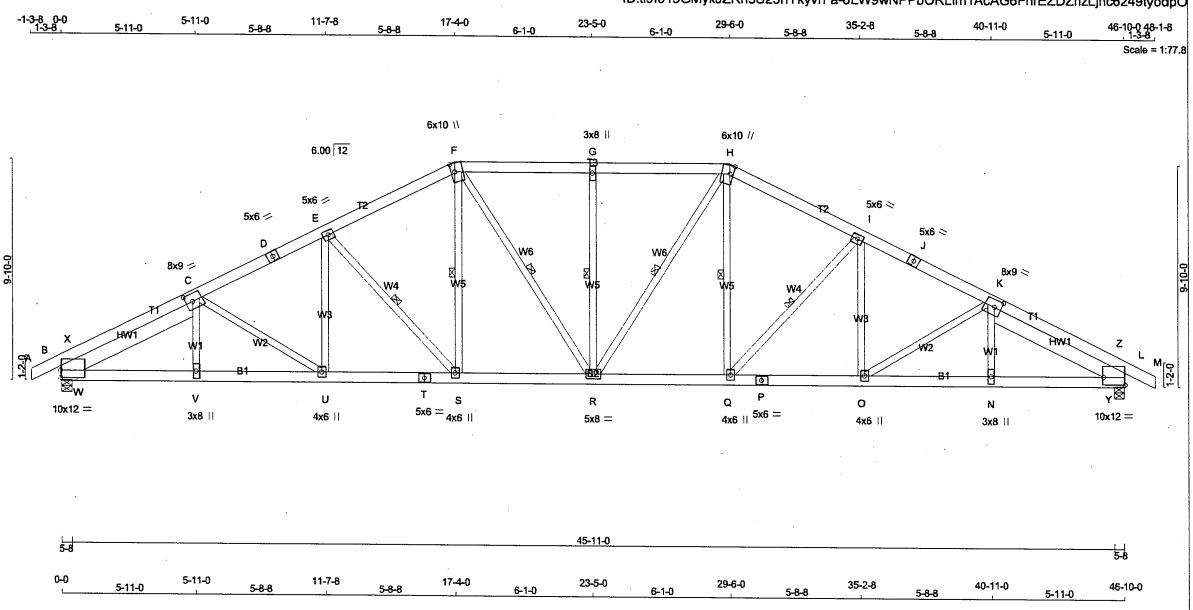

TOTAL WEIGHT = 12 X 321 = 3852 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
B - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	4.00	
C	TMWWW-t	MT20	8.0	9.0	4.00	3.50
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	10.0	4.50	1.75
G	TMW+w	MT20	3.0	8.0		
H	TTWW+m	MT20	6.0	10.0	4.50	1.75
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWWW-t	MT20	8.0	9.0	4.00	3.50
L	TMBMW1-I	MT20	10.0	12.0	4.00	Edge
N	BMW+w	MT20	3.0	8.0		
O, Q, S, U						
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		
V	BMW+w	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	3985	0	3985	138	-204	5-8	4-5
L	3985	0	3985	0	-204	5-8	4-5

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 204 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 204 LBS FACTORED UPLIFT

PROVIDE FOR 138 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2938	1741 / 0	492 / 0	0 / 0	75 / -599	705 / 0	0 / 0
L	2938	1741 / 0	492 / 0	0 / 0	75 / -599	705 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	99 / -99	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q.

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-120.3	-120.3	0.09 (2)	10.00
B- X	-4645 / 176	-120.3	-120.3	0.26 (2)	3.85
X- C	-3122 / 232	-120.3	-120.3	0.23 (2)	4.61
C- D	-5841 / 280	-120.3	-120.3	0.48 (1)	3.30
D- E	-5841 / 280	-120.3	-120.3	0.48 (1)	3.30
E- F	-5067 / 255	-120.3	-120.3	0.43 (1)	3.57
F- G	-4905 / 226	-132.8	-132.8	0.52 (1)	2.00
G- H	-4905 / 226	-132.8	-132.8	0.52 (1)	2.00
H- I	-5067 / 255	-120.3	-120.3	0.43 (1)	3.57
I- J	-5841 / 280	-120.3	-120.3	0.48 (1)	3.30
J- K	-5841 / 280	-120.3	-120.3	0.48 (1)	3.30
K- Z	-3122 / 232	-120.3	-120.3	0.23 (3)	4.61
Z- L	-4645 / 177	-120.3	-120.3	0.26 (3)	3.85
L- M	0 / 1	-120.3	-120.3	0.09 (3)	10.00
B- W	-257 / 2789	-39.5	-39.5	0.35 (1)	6.25
W- V	-300 / 5501	-39.5	-39.5	0.74 (1)	6.25
V- U	-302 / 5496	-39.5	-39.5	0.73 (1)	6.25
U- T	-196 / 5238	-39.5	-39.5	0.69 (1)	6.25
T- S	-196 / 5238	-39.5	-39.5	0.69 (1)	6.25
S- R	-55 / 4518	-39.5	-39.5	0.61 (1)	6.25
R- Q	0 / 4518	-39.5	-39.5	0.61 (1)	10.00
Q- P	-58 / 5238	-39.5	-39.5	0.69 (1)	6.25
P- O	-58 / 5238	-39.5	-39.5	0.69 (1)	6.25
O- N	-165 / 5496	-39.5	-39.5	0.73 (1)	6.25
N- Y	-163 / 5501	-39.5	-39.5	0.74 (1)	6.25
Y- L	-119 / 2790	-39.5	-39.5	0.35 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
V- C	0 / 279	0.05 (17)
C- U	-499 / 126	0.29 (2)
U- E	0 / 501	0.08 (2)
E- S	-1315 / 213	0.47 (2)
S- F	-81 / 1216	0.20 (2)
F- R	-169 / 1172	0.19 (3)
R- G	-975 / 83	0.41 (1)
R- H	-168 / 1172	0.19 (2)
Q- H	-81 / 1216	0.20 (3)
Q- I	-1315 / 213	0.47 (3)
O- I	0 / 501	0.08 (3)
O- K	-499 / 127	0.29 (3)
N- K	0 / 279	0.05 (17)
W- X	0 / 1956	0.00 (1)
W- C	-3306 / 53	0.96 (1)
K- Y	-3305 / 54	0.96 (1)
Y- Z	0 / 1956	0.00 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/180 (3.12")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.52/1.00 (F-G:1) , BC=0.74/1.00 (N-Y:1) , WB=0.96/1.00 (C-W:1) , SSI=0.32/1.00 (F-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.91 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T25A		1	GREEN PARK HOMES JT 53080	E22085428

Alpa Roof Truss, Maple

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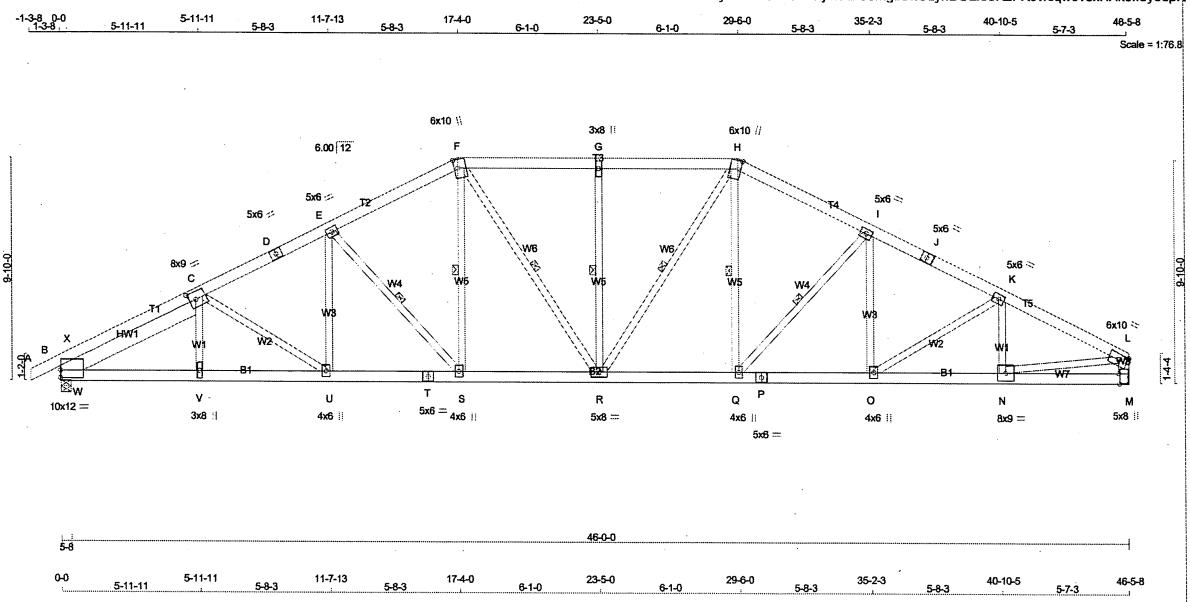

TOTAL WEIGHT = 10 X 309 = 3086 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
B - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

REINFORCING MEMBERS

HW1	2x8	DRY	No.2	SPF

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	4.00	
C	TMWWW-t	MT20	8.0	9.0	4.00	3.50
D	TS-t	MT20	5.0	6.0		
E, I, K						
E	TMWW-t	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	10.0	4.50	1.75
G	TMW+w	MT20	3.0	8.0		
H	TTWW+m	MT20	6.0	10.0	4.50	1.75
J	TS-t	MT20	5.0	6.0		
L	TMVW-t	MT20	6.0	10.0		Edge
M	BMV1+t	MT20	5.0	8.0	Edge	0.50
N	BMWW-t	MT20	8.0	9.0		
O, Q, S, U						
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	5.0	6.0		
V	BMW+w	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
B	3954	0	3954	140	-203	5-8	4-5
M	3788	0	3788	0	-174	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 203 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT M FOR 174 LBS FACTORED UPLIFT

PROVIDE FOR 140 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2915	1728 / 0	488 / 0	0 / 0	75 / -595	700 / 0	0 / 0
M	2802	1630 / 0	488 / 0	0 / 0	78 / -564	684 / 0	0 / 0

HORIZONTAL REACTIONS

B	—	0 / 0	0 / 0	0 / 0	100 / -88	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-S, F-S, F-R, G-R, H-R, H-Q, I-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 1	-120.3	-120.3	0.09 (2)	10.00
B- X	-4615 / 175	-120.3	-120.3	0.27 (2)	3.86
X- C	-3107 / 234	-120.3	-120.3	0.23 (2)	4.62
C- D	-5777 / 280	-120.3	-120.3	0.47 (2)	3.33
D- E	-5777 / 280	-120.3	-120.3	0.47 (2)	3.33
E- F	-5003 / 255	-120.3	-120.3	0.42 (1)	3.59
F- G	-4829 / 226	-132.8	-132.8	0.52 (1)	2.00
G- H	-4829 / 226	-132.8	-132.8	0.52 (1)	2.00
H- I	-4959 / 250	-120.3	-120.3	0.41 (1)	3.62
I- J	-5659 / 273	-120.3	-120.3	0.46 (1)	3.37
J- K	-5659 / 273	-120.3	-120.3	0.46 (1)	3.37
K- L	-5754 / 255	-120.3	-120.3	0.46 (1)	3.34
M- L	-3676 / 202	0.0	0.0	0.24 (1)	5.58
B- W	-259 / 2776	-39.5	-39.5	0.35 (1)	6.25
W- V	-301 / 5454	-39.5	-39.5	0.74 (1)	6.25
V- U	-302 / 5449	-39.5	-39.5	0.72 (1)	6.25
U- T	-197 / 5179	-39.5	-39.5	0.69 (1)	6.25
T- S	-197 / 5179	-39.5	-39.5	0.69 (1)	6.25
S- R	-56 / 4461	-39.5	-39.5	0.60 (1)	6.25
R- Q	0 / 4421	-39.5	-39.5	0.60 (1)	10.00
Q- P	-63 / 5069	-39.5	-39.5	0.67 (1)	6.25
P- O	-63 / 5069	-39.5	-39.5	0.67 (1)	6.25
O- N	-163 / 5165	-39.5	-39.5	0.70 (1)	6.25
N- M	-5 / 10	-39.5	-39.5	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	0 / 280	0.05 (17)
C- U	-511 / 127	0.29 (2)
U- E	0 / 508	0.08 (2)
E- S	-1315 / 213	0.47 (2)
S- F	-81 / 1219	0.20 (2)
F- R	-184 / 1139	0.18 (3)
R- G	-975 / 83	0.41 (1)
R- H	-139 / 1193	0.19 (2)
Q- H	-76 / 1143	0.18 (3)
Q- I	-1215 / 208	0.43 (3)
O- I	0 / 420	0.07 (6)
O- K	-326 / 174	0.19 (3)
N- K	-624 / 113	0.10 (1)
N- L	-175 / 5227	0.84 (1)
W- X	0 / 1932	0.00 (1)
W- C	-3262 / 50	0.97 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.55")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/180 (3.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.52/1.00 (F-G:1) , BC=0.74/1.00 (V-W:1) , WB=0.97/1.00 (C-W:1) , SSI=0.32/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.90 (T) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T35		2	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085429

Alpa Roof Truss, Maple

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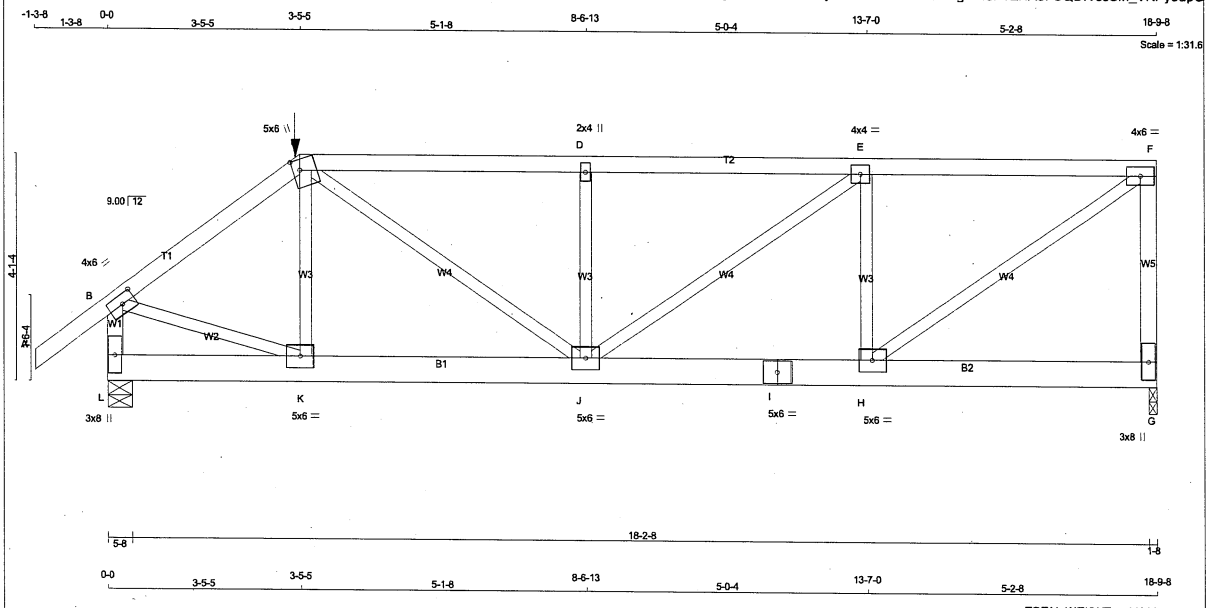


TOTAL WEIGHT = 4 X 88 = 353 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(46.1)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	2	12	SIDE(10.0)
I- G	2	12	SIDE(10.0)
WEBS : (0.122"X3") SPIRAL NAILS			
K- C	1	6	SIDE(16.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

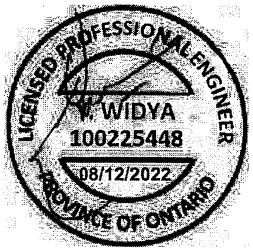
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.75
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	8.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	8.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2118	0	2118	0	0	1-8	1-8
L	2141	0	2141	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1495	997 / 0	0 / 0	0 / 0	0 / 0	498 / 0	0 / 0
L	1511	1010 / 0	0 / 0	0 / 0	0 / 0	501 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.07 (1)	10.00
B- C	-2115 / 0	-95.2	-95.2	0.13 (1)	5.96
C- D	-2780 / 0	-187.4	-187.4	0.40 (1)	5.00
D- E	-2780 / 0	-187.4	-187.4	0.41 (1)	4.97
E- F	-2281 / 0	-187.4	-187.4	0.40 (1)	5.38
G- F	-2030 / 0	0.0	0.0	0.25 (1)	7.74
L- B	-2097 / 0	0.0	0.0	0.12 (1)	7.64
L- K	0 / 0	-36.4	-36.4	0.04 (4)	10.00
K- J	0 / 1678	-36.4	-36.4	0.15 (1)	10.00
J- I	0 / 2281	-36.4	-36.4	0.19 (1)	10.00
I- H	0 / 2281	-36.4	-36.4	0.19 (1)	10.00
H- G	0 / 0	-36.4	-36.4	0.05 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-329 / 26	0.04 (1)
B- K	0 / 1768	0.22 (1)
H- F	0 / 2805	0.35 (1)
C- J	0 / 1361	0.17 (1)
H- E	-1426 / 0	0.18 (1)
J- D	-1024 / 0	0.13 (1)
J- E	0 / 623	0.08 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-239	-239	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	26.7	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-5-5
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.19/1.00 (H-J:1) , WB=0.35/1.00 (F-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.31 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T35Z		2	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085430

Alpa Roof Truss, Maple

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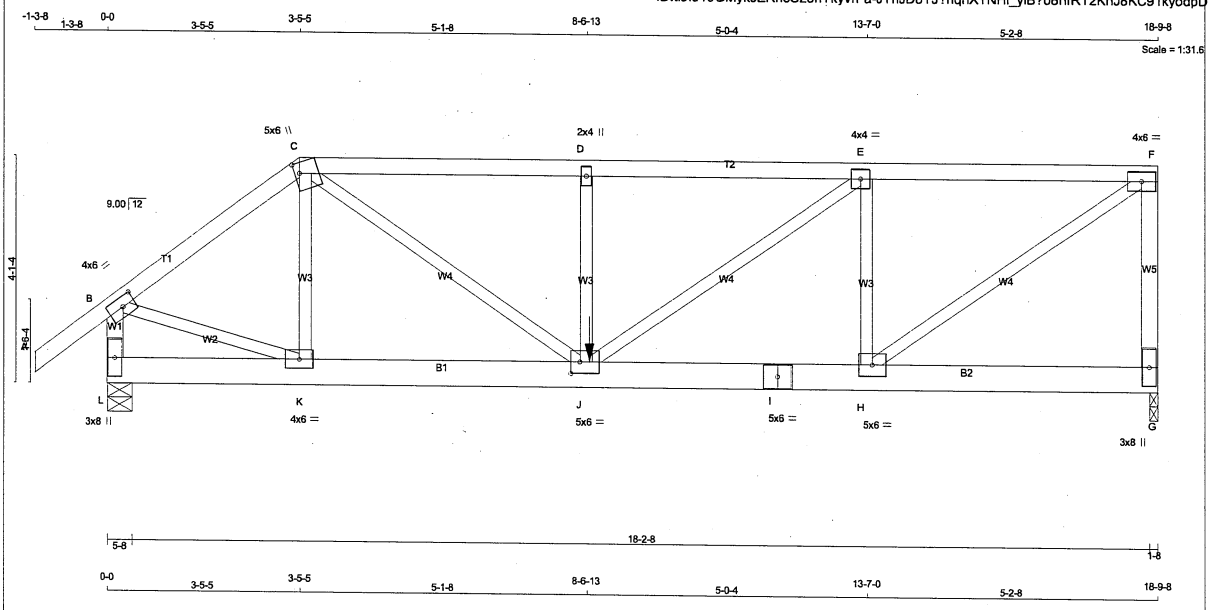

TOTAL WEIGHT = 4 X 88 = 353 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(46.1)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	2	12	SIDE(174.2)
I- G	2	12	SIDE(10.0)
WEBS : (0.122"X3") SPIRAL NAILS			
J- D	1	6	SIDE(54.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.75
C	TTWW+m	MT20	5.0	6.0	2.25	1.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	8.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0	2.50	2.00
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	8.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2288	0	2288	0	0	1-8	1-8
L	1983	0	1983	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1615	1076 / 0	0 / 0	0 / 0	0 / 0	539 / 0	0 / 0
L	1398	945 / 0	0 / 0	0 / 0	0 / 0	453 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.07 (1)	10.00
B- C	-1966 / 0	-95.2	-95.2	0.12 (1)	6.13
C- D	-3185 / 0	-95.2	-95.2	0.27 (1)	4.92
D- E	-3185 / 0	-187.4	-187.4	0.46 (1)	4.64
E- F	-2530 / 0	-187.4	-187.4	0.44 (1)	5.11
G- F	-2201 / 0	0.0	0.0	0.27 (1)	7.51
L- B	-1970 / 0	0.0	0.0	0.11 (1)	7.81
L- K	0 / 0	-18.5	-18.5	0.03 (1)	10.00
K- J	0 / 1559	-18.5	-18.5	0.14 (1)	10.00
J- I	0 / 2530	-36.4	-36.4	0.22 (1)	10.00
I- H	0 / 2530	-36.4	-36.4	0.22 (1)	10.00
H- G	0 / 0	-36.4	-36.4	0.05 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-357 / 0	0.05 (1)
B- K	0 / 1643	0.20 (1)
H- F	0 / 3112	0.39 (1)
C- J	0 / 2008	0.25 (1)
H- E	-1590 / 0	0.20 (1)
J- D	-712 / 0	0.09 (1)
J- E	0 / 815	0.10 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	8-7-8	-877	-877	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-5-5
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-2-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.46/1.00 (D-E:1) , BC=0.22/1.00 (H-J:1) , WB=0.39/1.00 (F-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.34 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T36		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085431

Alpa Roof Truss, Maple

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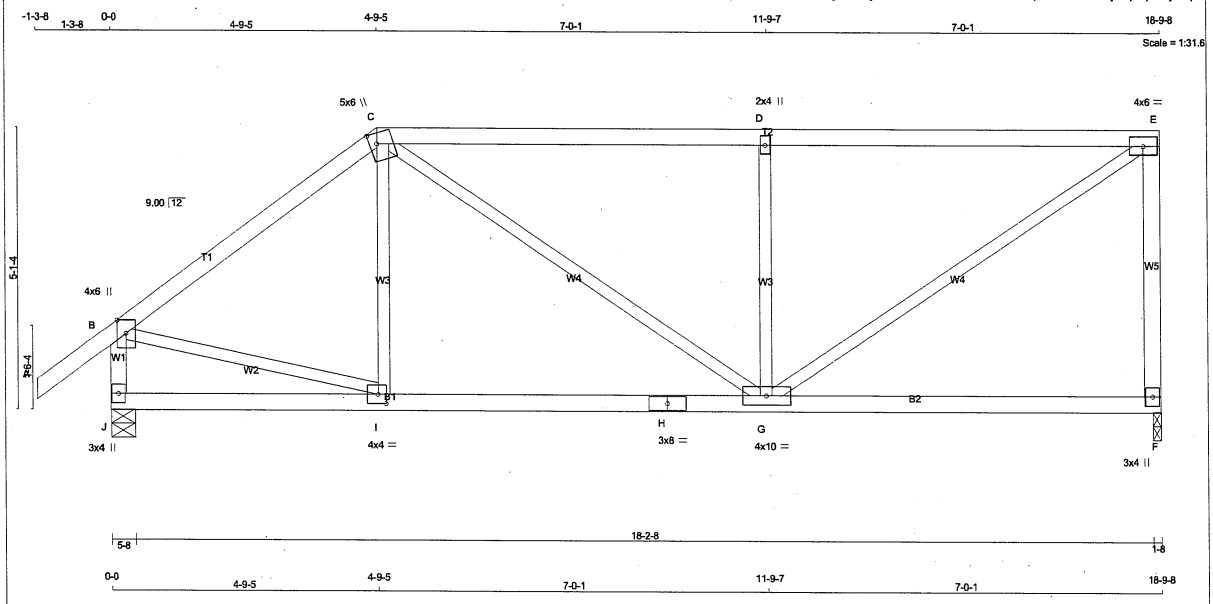

TOTAL WEIGHT = 4 X 77 = 308 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	10.0		
H	BS-t	MT20	3.0	8.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1068	0	1068	0	0	1-8	1-8
J	1200	0	1200	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	754	502 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
J	844	576 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13	(1)	10.00	
B- C	-1013 / 0	-95.2	-95.2	0.41	(1)	5.65	
C- D	-1113 / 0	-95.2	-95.2	0.89	(1)	4.26	
D- E	-1114 / 0	-95.2	-95.2	0.89	(1)	4.26	
F- E	-1015 / 0	0.0	0.0	0.45	(1)	7.77	
J- B	-1166 / 0	0.0	0.0	0.12	(1)	7.39	
J- I	0 / 0	-18.5	-18.5	0.13	(4)	10.00	
I- H	0 / 807	-18.5	-18.5	0.29	(4)	10.00	
H- G	0 / 807	-18.5	-18.5	0.29	(4)	10.00	
G- F	0 / 0	-18.5	-18.5	0.22	(4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-81 / 61	0.03 (1)
C- G	0 / 372	0.08 (1)
G- D	-827 / 0	0.32 (1)
G- E	0 / 1348	0.30 (1)
B- I	0 / 833	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.89/1.00 (D-E:1) , BC=0.29/1.00 (G-I:4) , WB=0.32/1.00 (D-G:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T37		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085432

Alpa Roof Truss, Maple

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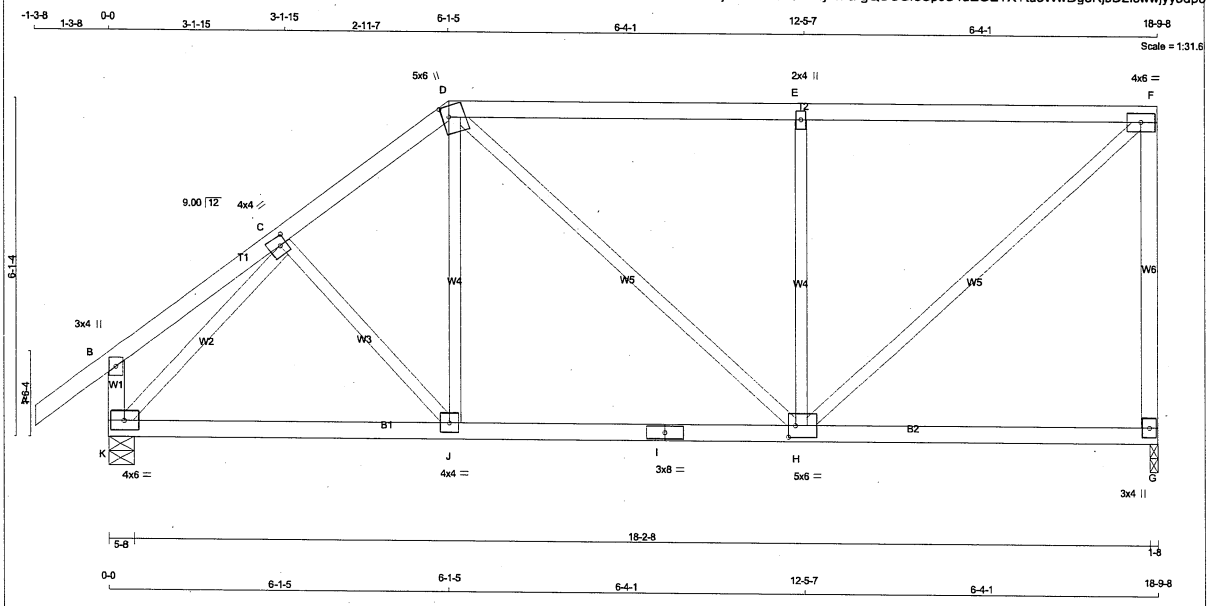

TOTAL WEIGHT = 4 X 83 = 334 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1068	0	1068	0	0	1-8	1-8
K	1200	0	1200	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	754	502 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	844	576 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	0 / 19	-95.2	-95.2	0.13 (1)	10.00
C- D	-984 / 0	-95.2	-95.2	0.14 (1)	6.12
D- E	-866 / 0	-95.2	-95.2	0.69 (1)	5.32
E- F	-866 / 0	-95.2	-95.2	0.69 (1)	5.32
G- F	-1019 / 0	0.0	0.0	0.74 (1)	7.76
K- B	-245 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 795	-18.5	-18.5	0.22 (4)	10.00
J- I	0 / 773	-18.5	-18.5	0.23 (4)	10.00
I- H	0 / 773	-18.5	-18.5	0.23 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-36 / 27	0.01 (1)
J- D	0 / 152	0.04 (4)
D- H	0 / 127	0.03 (1)
H- E	-748 / 0	0.44 (1)
H- F	0 / 1171	0.26 (1)
K- C	-1207 / 0	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD .

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.74/1.00 (F-G:1) , BC=0.23/1.00 (H-J:4) , WB=0.44/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T38		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085433

Alpa Roof Truss, Maple

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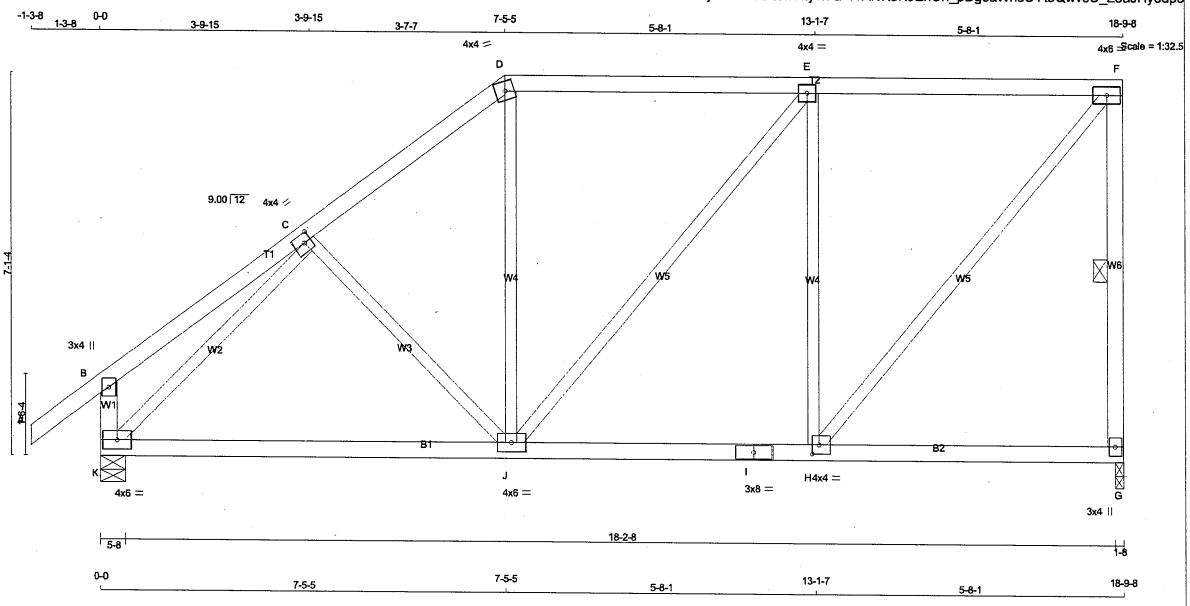

TOTAL WEIGHT = 4 X 88 = 354 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.50
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1068	0	1068	0	0	1-8	1-8
K	1200	0	1200	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	754	502 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	844	576 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	
B- C	0 / 24	-95.2	-95.2	0.21 (1)	10.00	
C- D	-923 / 0	-95.2	-95.2	0.22 (1)	6.15	
D- E	-720 / 0	-95.2	-95.2	0.54 (1)	6.10	
E- F	-685 / 0	-95.2	-95.2	0.54 (1)	6.21	
G- F	-1023 / 0	0.0	0.0	0.23 (1)	6.20	
K- B	-267 / 0	0.0	0.0	0.03 (1)	7.81	
K- J	0 / 816	-18.5	-18.5	0.28 (4)	10.00	
J- I	0 / 685	-18.5	-18.5	0.27 (4)	10.00	
I- H	0 / 685	-18.5	-18.5	0.27 (4)	10.00	
H- G	0 / 0	-18.5	-18.5	0.13 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-142 / 2	0.07 (1)
J- D	0 / 199	0.04 (1)
J- E	0 / 56	0.01 (4)
H- E	-713 / 0	0.62 (1)
H- F	0 / 1066	0.24 (1)
K- C	-1196 / 0	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.28/1.00 (J-K:4) , WB=0.62/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428162	T39		1	TRUSS DESC.	JT 53080	E22085434

Alpa Roof Truss, Maple

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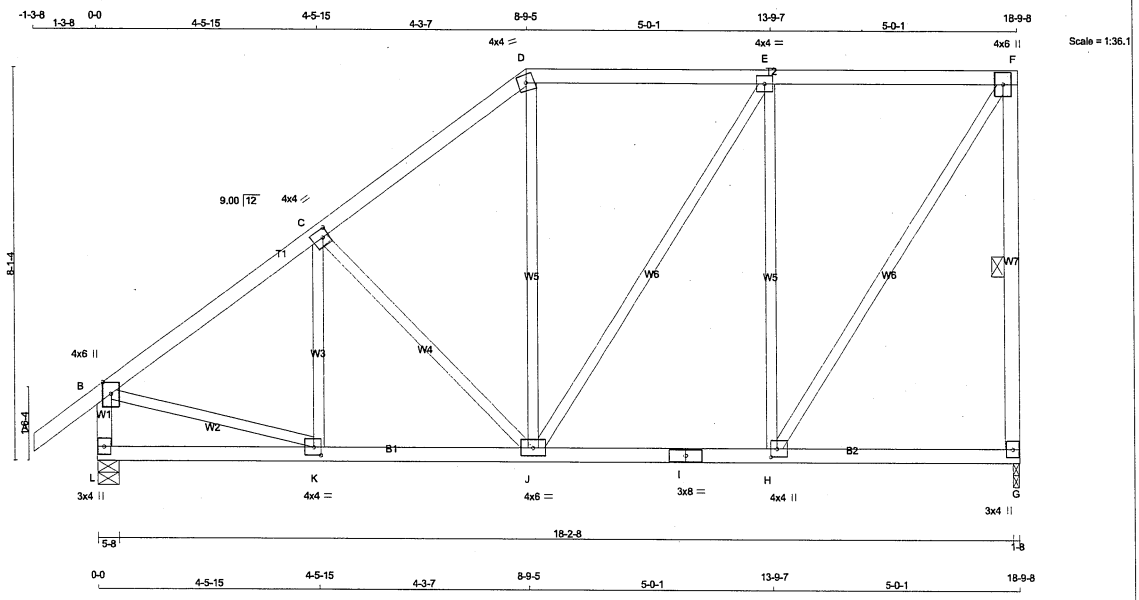

TOTAL WEIGHT = 4 X 96 = 383 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	4.0	2.00	1.50
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1068	0	1068	0	0	1-8	1-8
L	1200	0	1200	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	754	502 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
L	844	576 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00	
B- C	-1039 / 0	-95.2	-95.2	0.34 (1)	5.72	
C- D	-833 / 0	-95.2	-95.2	0.32 (1)	6.21	
D- E	-643 / 0	-95.2	-95.2	0.41 (1)	6.25	
E- F	-547 / 0	-95.2	-95.2	0.41 (1)	6.25	
G- F	-1030 / 0	0.0	0.0	0.31 (1)	6.18	
L- B	-1164 / 0	0.0	0.0	0.12 (1)	7.39	
L- K	0 / 0	-18.5	-18.5	0.08 (4)	10.00	
K- J	0 / 855	-18.5	-18.5	0.18 (1)	10.00	
J- I	0 / 547	-18.5	-18.5	0.16 (4)	10.00	
I- H	0 / 547	-18.5	-18.5	0.16 (4)	10.00	
H- G	0 / 0	-18.5	-18.5	0.11 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-137 / 28	0.05 (1)
C- J	-309 / 0	0.22 (1)
J- D	0 / 150	0.04 (4)
J- E	0 / 181	0.04 (1)
H- E	-742 / 0	0.94 (1)
H- F	0 / 1008	0.23 (1)
B- K	0 / 885	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.41/1.00 (D-E:1) , BC=0.18/1.00 (J-K:1) , WB=0.94/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T40		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085435

Alpa Roof Truss, Maple

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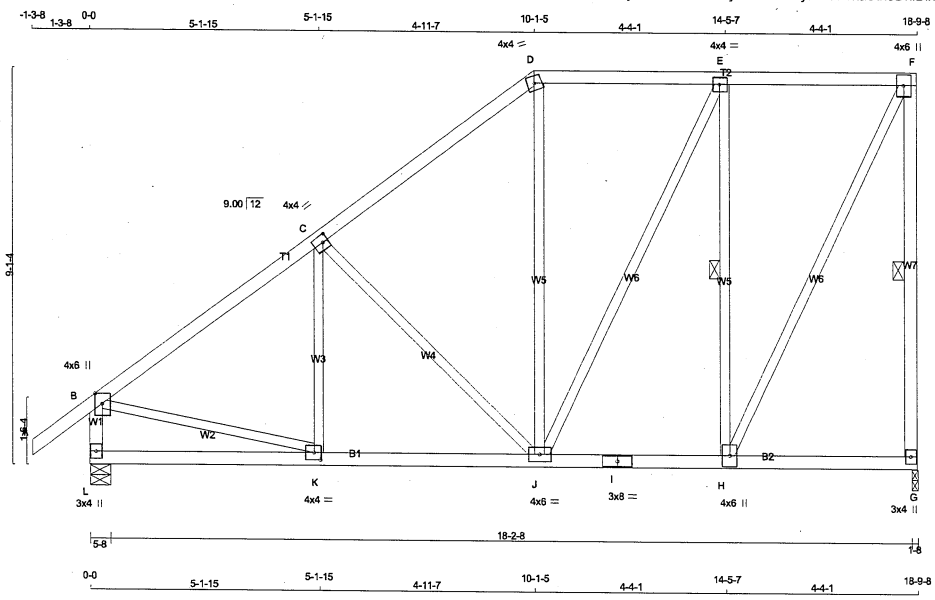

TOTAL WEIGHT = 4 X 101 = 406 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1068	0	1068	0	0	1-8	1-8
L	1200	0	1200	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	754	502 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
L	844	576 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	-1037 / 0	-95.2	-95.2	0.33 (1)	5.76
C- D	-745 / 0	-95.2	-95.2	0.32 (1)	6.25
D- E	-567 / 0	-95.2	-95.2	0.23 (1)	6.25
E- F	-435 / 0	-95.2	-95.2	0.22 (1)	6.25
G- F	-1035 / 0	0.0	0.0	0.40 (1)	6.18
L- B	-1160 / 0	0.0	0.0	0.12 (1)	7.39
L- K	0 / 0	-18.5	-18.5	0.12 (4)	10.00
K- J	0 / 857	-18.5	-18.5	0.20 (1)	10.00
J- I	0 / 435	-18.5	-18.5	0.11 (4)	10.00
I- H	0 / 435	-18.5	-18.5	0.11 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-91 / 57	0.04 (1)
C- J	-412 / 0	0.41 (1)
J- D	0 / 93	0.03 (4)
J- E	0 / 308	0.07 (1)
H- E	-786 / 0	0.42 (1)
H- F	0 / 977	0.22 (1)
B- K	0 / 880	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.40/1.00 (F-G:1) , BC=0.20/1.00 (J-K:1) , WB=0.42/1.00 (E-H:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T41		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085436

Alpa Roof Truss, Maple

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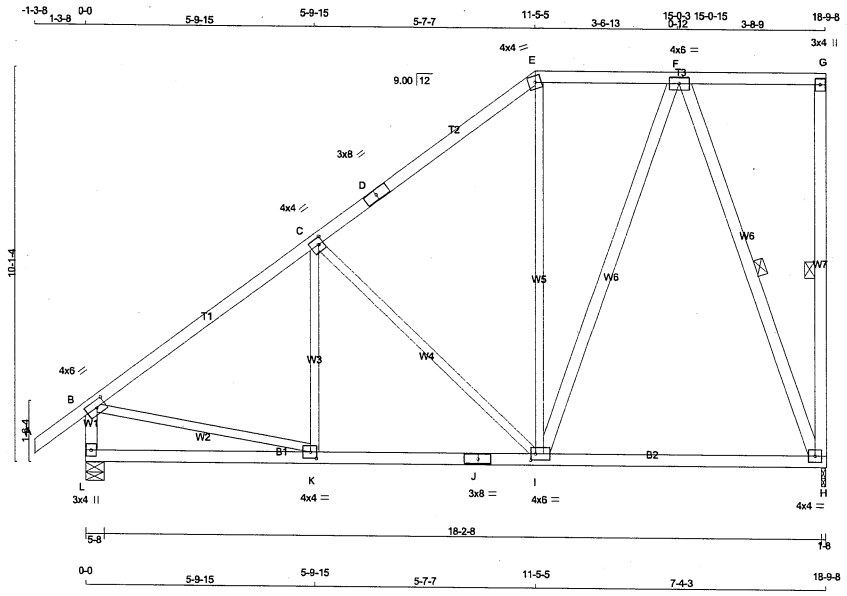

TOTAL WEIGHT = 10 X 106 = 1062 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0	2.00	1.50
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1113	0	1113	0	0	1-8	1-8
L	1210	0	1210	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	790	502 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
L	853	576 / 0	0 / 0	0 / 0	0 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

FOR SECTION E-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	-1036 / 0	-95.2	-95.2	0.43 (1)	5.60
C- D	-673 / 0	-95.2	-95.2	0.41 (1)	6.25
D- E	-673 / 0	-95.2	-95.2	0.41 (1)	6.25
E- F	-505 / 0	-102.7	-102.7	0.18 (1)	2.00
F- G	0 / 0	-102.7	-102.7	0.23 (1)	10.00
H- G	-145 / 0	0.0	0.0	0.07 (1)	6.25
L- B	-1164 / 0	0.0	0.0	0.12 (1)	7.39
L- K	0 / 0	-18.5	-18.5	0.13 (4)	10.00
K- J	0 / 860	-18.5	-18.5	0.28 (4)	10.00
J- I	0 / 860	-18.5	-18.5	0.28 (4)	10.00
I- H	0 / 345	-18.5	-18.5	0.24 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-69 / 63	0.04 (1)
C- I	-495 / 0	0.66 (1)
I- E	0 / 67	0.02 (4)
I- F	0 / 474	0.08 (1)
F- H	-976 / 0	0.56 (1)
B- K	0 / 878	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.43/1.00 (B-C:1) , BC=0.28/1.00 (I-K:4) , WB=0.66/1.00 (C-I:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.90)
JSI METAL= 0.29 (K) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T42		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085437

Alpa Roof Truss, Maple

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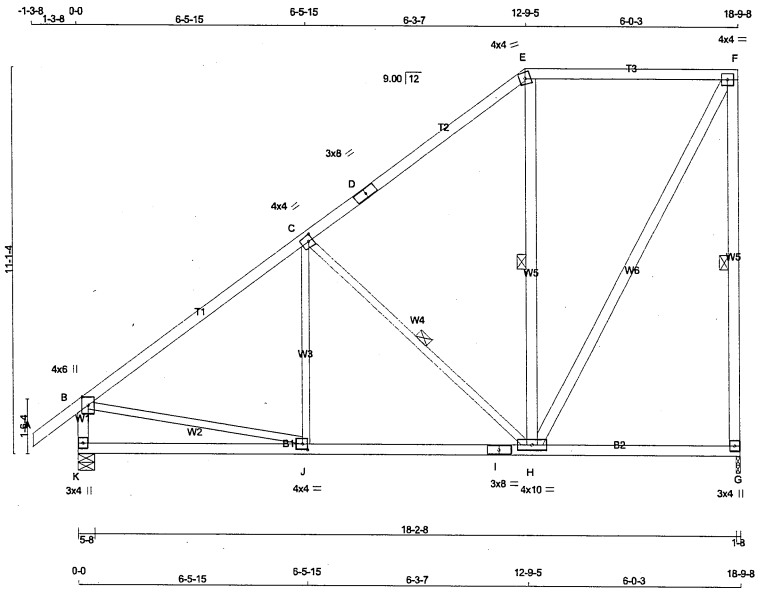

TOTAL WEIGHT = 4 X 104 = 416 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	10.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1068	0	1068	0	0	1-8	1-8
K	1200	0	1200	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	754	502 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
K	844	576 / 0	0 / 0	0 / 0	0 / 0	268 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	-1005 / 0	-95.2	-95.2	0.55 (1)	5.44
C- D	-552 / 0	-95.2	-95.2	0.52 (1)	6.25
D- E	-552 / 0	-95.2	-95.2	0.52 (1)	6.25
E- F	-401 / 0	-95.2	-95.2	0.44 (1)	6.25
G- F	-1025 / 0	0.0	0.0	0.65 (1)	6.20
K- B	-1151 / 0	0.0	0.0	0.12 (1)	7.42
K- J	0 / 0	-18.5	-18.5	0.18 (4)	10.00
J- I	0 / 840	-18.5	-18.5	0.25 (4)	10.00
I- H	0 / 840	-18.5	-18.5	0.25 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-22 / 99	0.03 (4)
C- H	-599 / 0	0.32 (1)
H- E	-206 / 13	0.13 (1)
H- F	0 / 842	0.14 (1)
B- J	0 / 854	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.65/1.00 (F-G:1) , BC=0.25/1.00 (H-J:4) , WB=0.32/1.00 (C-H:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T43		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085438

Alpa Roof Truss, Maple

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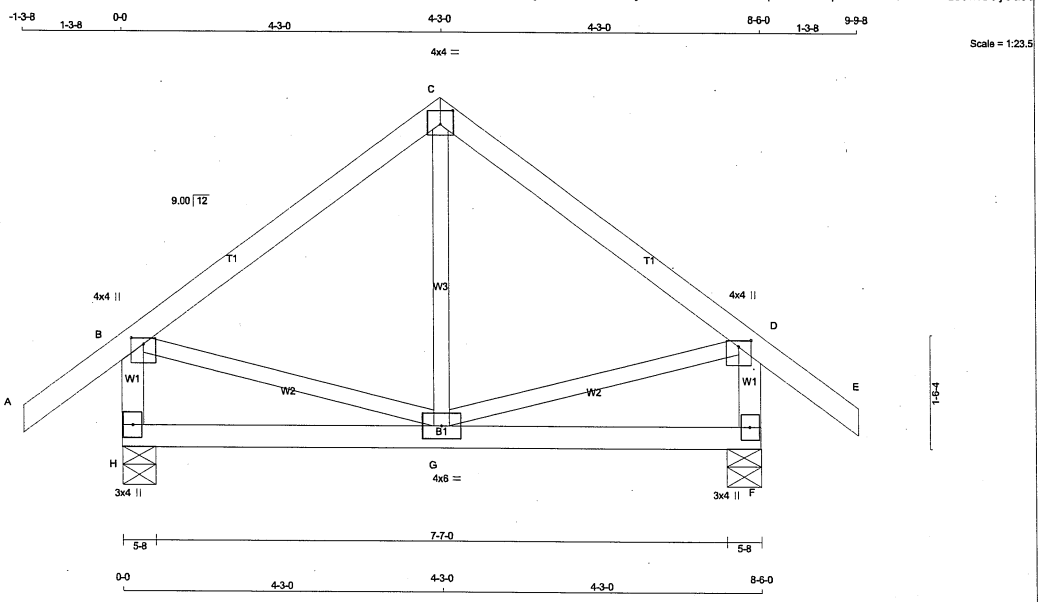

TOTAL WEIGHT = 6 X 38 = 230 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	615	0	615	0	0	5-8	1-8
F	615	0	615	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	431	301 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
F	431	301 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	-304 / 0	-95.2	-95.2	0.22 (1)	6.25
C- D	-304 / 0	-95.2	-95.2	0.22 (1)	6.25
D- E	0 / 39	-95.2	-95.2	0.13 (1)	10.00
H- B	-584 / 0	0.0	0.0	0.06 (1)	7.81
F- D	-584 / 0	0.0	0.0	0.06 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-40 / 60	0.02 (4)
B- G	0 / 252	0.06 (1)
G- D	0 / 252	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.09/1.00 (G-H:4) , WB=0.06/1.00 (D-G:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (D) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T43S		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085439

Alpa Roof Truss, Maple

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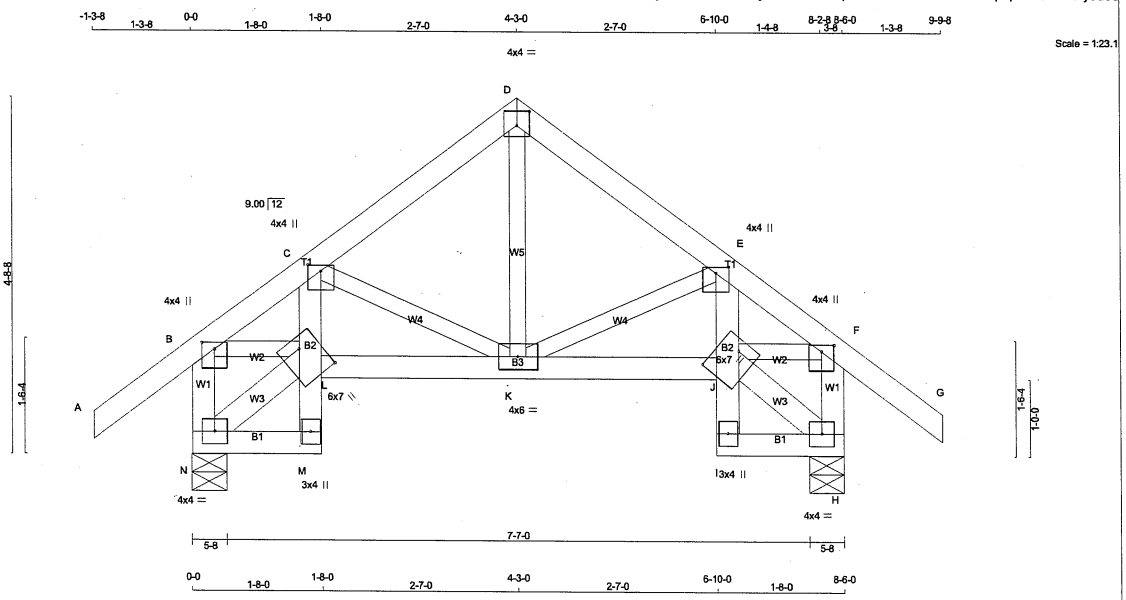

TOTAL WEIGHT = 4 X 45 = 182 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B, C, E, F					
B TMVW+p	MT20	4.0	4.0	1.00	2.00
D TTW-p	MT20	4.0	4.0	2.25	2.00
H BMVW1-t	MT20	4.0	4.0		
I BMV+p	MT20	3.0	4.0		
J BVMWW-w	MT20	6.0	7.0	3.00	5.25
K BMWWW-t	MT20	4.0	6.0		
L BVMWW-w	MT20	6.0	7.0	3.00	5.25
M BMV+p	MT20	3.0	4.0		
N BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	615	0	615	0	0	5-8	1-8
H	615	0	615	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	431	301 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0
H	431	301 / 0	0 / 0	0 / 0	0 / 0	130 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	-584 / 0	-95.2	-95.2	0.10 (1)	6.25
C- D	-359 / 0	-95.2	-95.2	0.09 (1)	6.25
D- E	-359 / 0	-95.2	-95.2	0.09 (1)	6.25
E- F	-584 / 0	-95.2	-95.2	0.10 (1)	6.25
F- G	0 / 39	-95.2	-95.2	0.13 (1)	10.00
N- B	-586 / 0	0.0	0.0	0.06 (1)	7.81
H- F	-586 / 0	0.0	0.0	0.06 (1)	7.81
N- M	0 / 19	-18.5	-18.5	0.01 (4)	10.00
M- L	0 / 16	0.0	0.0	0.03 (1)	10.00
L- C	0 / 39	0.0	0.0	0.04 (1)	10.00
L- K	0 / 468	-18.5	-18.5	0.10 (1)	10.00
K- J	0 / 469	-18.5	-18.5	0.10 (1)	10.00
I- J	0 / 16	0.0	0.0	0.03 (1)	10.00
J- E	0 / 39	0.0	0.0	0.04 (1)	10.00
I- H	0 / 19	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 187	0.04 (1)
K- E	-208 / 0	0.04 (1)
C- K	-208 / 0	0.04 (1)
N- L	-24 / 0	0.00 (1)
B- L	0 / 449	0.10 (1)
J- H	-24 / 0	0.00 (1)
J- F	0 / 449	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.10/1.00 (J-K:1) , WB=0.10/1.00 (F-J:1) , SSI=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T46S		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085440

Alpa Roof Truss, Maple

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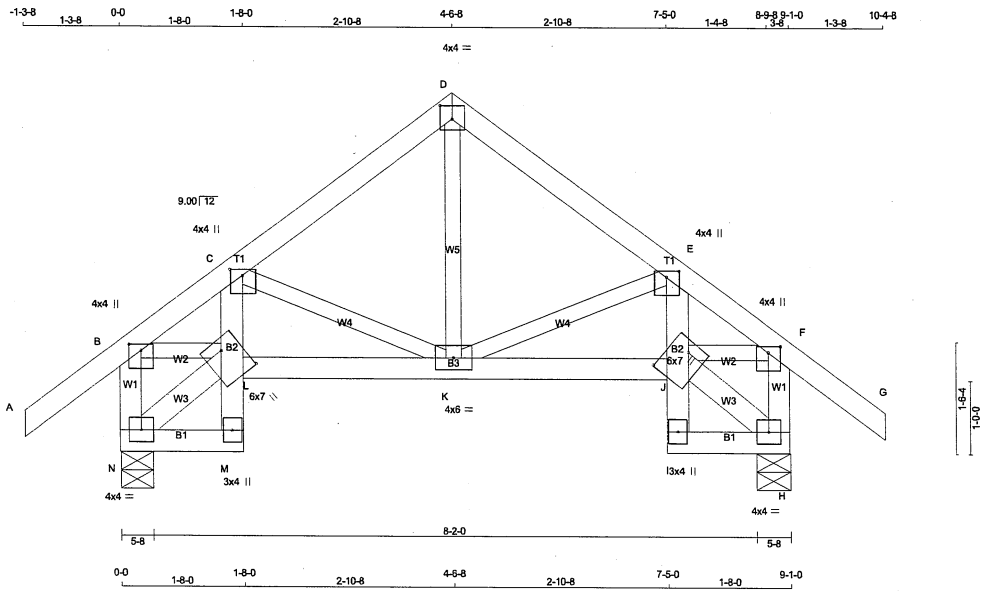

TOTAL WEIGHT = 4 X 48 = 190 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, C, E, F						
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW-p	MT20	4.0	4.0	2.25	2.00
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-w	MT20	6.0	7.0	3.00	5.25
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-w	MT20	6.0	7.0	3.00	5.25
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	648	0	648	0	0	5-8	1-8
H	648	0	648	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	455	317 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0
H	455	317 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	-645 / 0	-95.2	-95.2	0.10 (1)	6.25
C- D	-392 / 0	-95.2	-95.2	0.11 (1)	6.25
D- E	-392 / 0	-95.2	-95.2	0.11 (1)	6.25
E- F	-645 / 0	-95.2	-95.2	0.10 (1)	6.25
F- G	0 / 39	-95.2	-95.2	0.13 (1)	10.00
N- B	-618 / 0	0.0	0.0	0.06 (1)	7.81
H- F	-618 / 0	0.0	0.0	0.06 (1)	7.81
N- M	0 / 21	-18.5	-18.5	0.01 (4)	10.00
M- L	0 / 16	0.0	0.0	0.04 (1)	10.00
L- C	0 / 41	0.0	0.0	0.04 (1)	10.00
L- K	0 / 524	-18.5	-18.5	0.11 (1)	10.00
K- J	0 / 524	-18.5	-18.5	0.11 (1)	10.00
I- J	0 / 16	0.0	0.0	0.04 (1)	10.00
J- E	0 / 41	0.0	0.0	0.04 (1)	10.00
I- H	0 / 21	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 203	0.05 (1)
K- E	-238 / 0	0.05 (1)
C- K	-238 / 0	0.05 (1)
N- L	-26 / 0	0.00 (1)
B- L	0 / 503	0.11 (1)
J- H	-26 / 0	0.00 (1)
J- F	0 / 503	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (F-G:1) , BC=0.11/1.00 (K-L:1) , WB=0.11/1.00 (B-L:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (B) (INPUT = 0.90)
JSI METAL= 0.19 (F) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
Y. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T47S		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085441

Alpa Roof Truss, Maple

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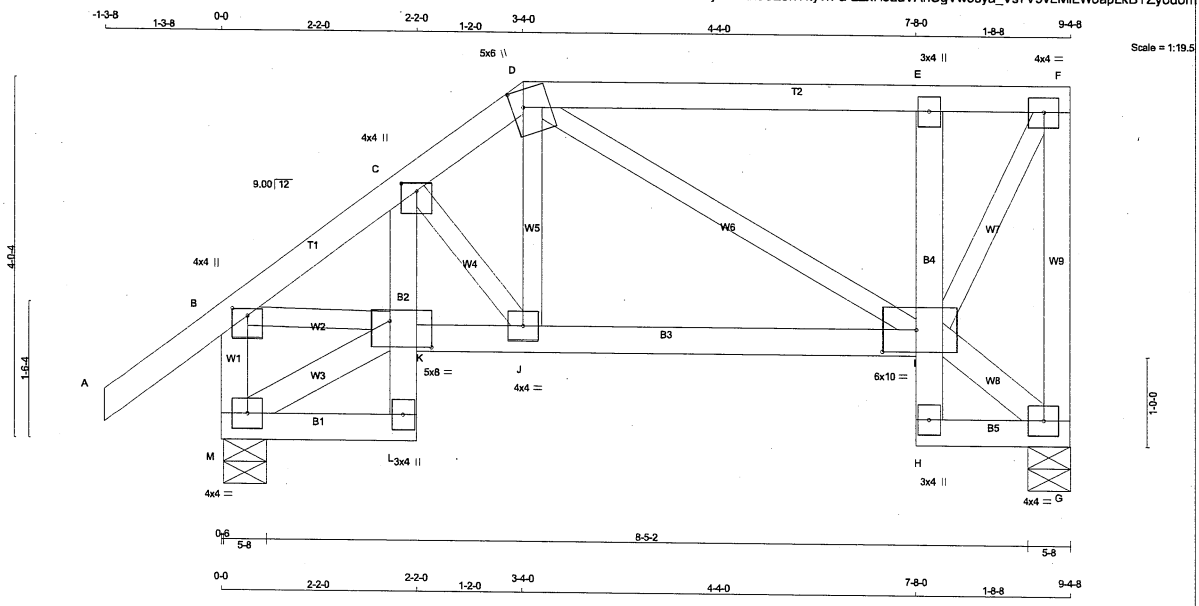

TOTAL WEIGHT = 2 X 51 = 102 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+p	MT20	3.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMV+p	MT20	3.0	4.0		
I	BVMWWW-I	MT20	6.0	10.0	3.00	4.50
J	BMWW-t	MT20	4.0	4.0		
K	BVMWW-I	MT20	5.0	8.0	3.50	5.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	533	0	533	0	0	5-8	1-8
M	664	0	664	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	376	251 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
M	467	324 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	-651 / 0	-95.2	-95.2	0.06 (1)	6.25
C- D	-548 / 0	-95.2	-95.2	0.03 (1)	6.25
D- E	-317 / 0	-95.2	-95.2	0.24 (1)	6.25
E- F	-311 / 0	-95.2	-95.2	0.24 (1)	6.25
G- F	-506 / 0	0.0	0.0	0.13 (1)	7.81
M- B	-636 / 0	0.0	0.0	0.07 (1)	7.81
M- L	0 / 18	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 21	0.0	0.0	0.03 (1)	10.00
K- C	0 / 16	0.0	0.0	0.03 (1)	10.00
K- J	0 / 538	-18.5	-18.5	0.13 (1)	10.00
J- I	0 / 435	-18.5	-18.5	0.11 (4)	10.00
H- I	0 / 16	0.0	0.0	0.03 (1)	10.00
I- E	-435 / 0	0.0	0.0	0.03 (1)	7.81
H- G	0 / 19	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-172 / 0	0.03 (1)
J- D	0 / 174	0.04 (1)
D- I	-137 / 0	0.06 (1)
I- G	-23 / 0	0.00 (1)
I- F	0 / 625	0.14 (1)
M- K	-20 / 0	0.00 (1)
B- K	0 / 526	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	26.7	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.24/1.00 (D-E:1) , BC=0.13/1.00 (J-K:1) , WB=0.14/1.00 (F-I:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
Y. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T48S		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085442

Alpa Roof Truss, Maple

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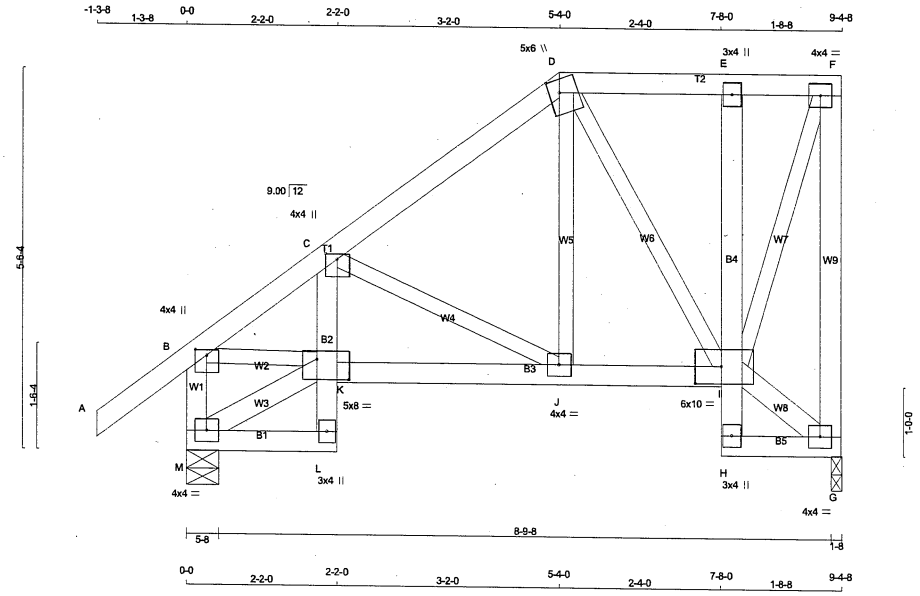

TOTAL WEIGHT = 2 X 59 = 117 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.75
E	TMV+p	MT20	3.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMV+p	MT20	3.0	4.0		
I	BVMWWW-I	MT20	6.0	10.0	3.00	4.50
J	BMWW-t	MT20	4.0	4.0		
K	BVMWW-I	MT20	5.0	8.0	3.50	5.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	533	0	533	0	0	1-8	1-8
M	664	0	664	0	0	5-8	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	376	251 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
M	467	324 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-95.2	-95.2	0.13 (1)	10.00
B- C	-681 / 0	-95.2	-95.2	0.09 (1)	6.25
C- D	-380 / 0	-95.2	-95.2	0.12 (1)	6.25
D- E	-174 / 0	-95.2	-95.2	0.06 (1)	6.25
E- F	-170 / 0	-95.2	-95.2	0.04 (1)	6.25
G- F	-509 / 0	0.0	0.0	0.28 (1)	7.81
M- B	-635 / 0	0.0	0.0	0.07 (1)	7.81
M- L	0 / 19	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 21	0.0	0.0	0.03 (1)	10.00
K- C	0 / 44	0.0	0.0	0.03 (1)	10.00
K- J	0 / 578	-18.5	-18.5	0.12 (1)	10.00
J- I	0 / 295	-18.5	-18.5	0.07 (4)	10.00
H- I	0 / 16	0.0	0.0	0.02 (1)	10.00
I- E	-236 / 0	0.0	0.0	0.03 (1)	7.81
H- G	0 / 16	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-321 / 0	0.08 (1)
J- D	0 / 191	0.04 (1)
D- I	-239 / 0	0.09 (1)
I- G	-18 / 0	0.00 (1)
I- F	0 / 492	0.11 (1)
M- K	-21 / 0	0.00 (1)
B- K	0 / 566	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.28/1.00 (F-G:1) , BC=0.12/1.00 (J-K:1) , WB=0.13/1.00 (B-K:1) , SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T50		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085443

Alpa Roof Truss, Maple

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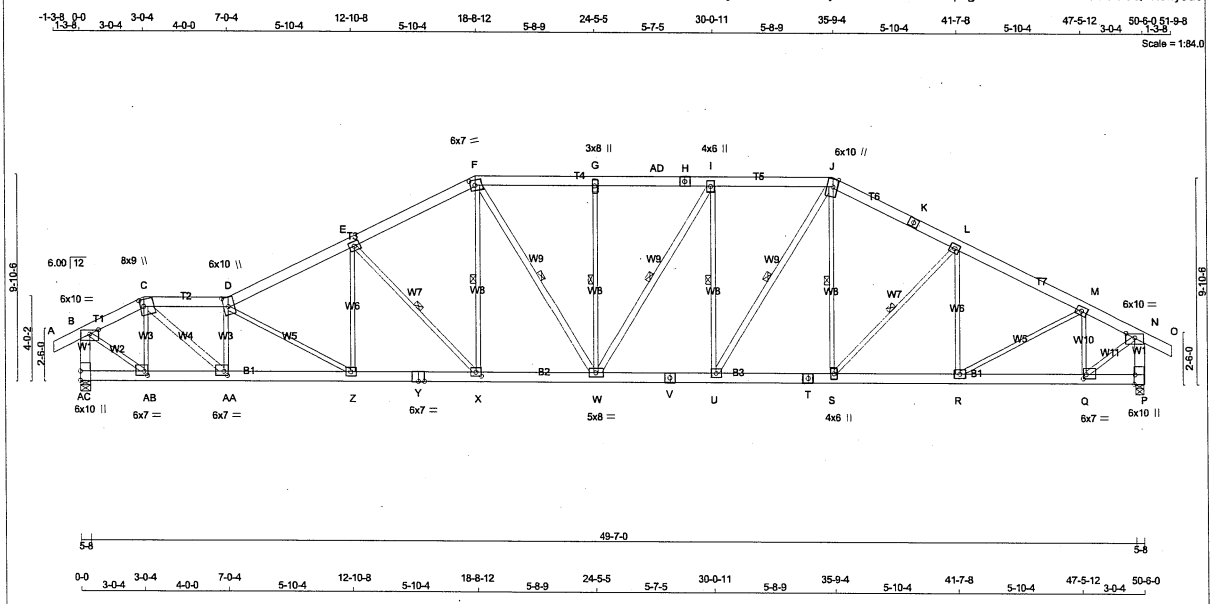

TOTAL WEIGHT = 3 X 317 = 950 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - K	2x6	DRY	No.2	SPF
K - O	2x6	DRY	No.2	SPF
AC- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AC- Y	2x6	DRY	No.2	SPF
Y - V	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - AA	2x4	DRY	No.2	SPF
U - J	2x4	DRY	No.2	SPF
F - W	2x4	DRY	No.2	SPF
W - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	10.0	Edge	
C	TTWW+m	MT20	8.0	9.0	3.75	2.00
D	TTWW+m	MT20	6.0	10.0	5.25	2.50
E, L, M						
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	7.0	3.00	2.75
G	TMW+w	MT20	3.0	8.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	10.0	Edge	2.25
K	TS-t	MT20	5.0	6.0		
N	TMVW-p	MT20	6.0	10.0	Edge	
P	BMV1+t	MT20	6.0	10.0	Edge	0.50
Q	BMWW-t	MT20	6.0	7.0	2.50	1.75
R, U, Z						
R	BMWW-t	MT20	5.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BS-t	MT20	5.0	6.0		
V	BS-t	MT20	5.0	6.0		
W	BMWWW-t	MT20	5.0	8.0		
X	BMWW-t	MT20	6.0	7.0	2.25	3.00
Y	BS-t	MT20	6.0	7.0		
AA	BMWW-t	MT20	6.0	7.0	2.50	2.00
AB	BMWW-t	MT20	6.0	7.0	2.50	2.00
AC	BMV1+t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AC	4300	0	4300	-159	-222	5-8	5-8
P	4317	0	4317	0	-181	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT AC FOR 222 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT P FOR 181 LBS FACTORED UPLIFT

PROVIDE FOR 159 LBS FACTORED HORIZONTAL REACTION AT JOINT AC

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
AC	3172	1870 / 0	530 / 0	0 / 0	89 / -655	772 / 0	0 / 0
P	3185	1870 / 0	530 / 0	0 / 0	73 / -634	785 / 0	0 / 0
HORIZONTAL REACTIONS							
AC	—	0 / 0	0 / 0	0 / 0	114 / -114	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AC, P

BRACING

FOR SECTION F-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-X, F-X, J-S, L-S, J-U, F-W, I-U, G-W, I-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-120.3	-120.3	0.10 (2)	10.00
B- C	-3821 / 213	-120.3	-120.3	0.16 (2)	4.29
C- D	-6957 / 345	-120.3	-120.3	0.38 (2)	3.07
D- E	-6971 / 318	-120.3	-120.3	0.62 (2)	2.89
E- F	-5913 / 286	-120.3	-120.3	0.53 (2)	3.22
F- G	-5686 / 257	-132.8	-132.8	0.45 (1)	2.00
G-AD	-5686 / 257	-132.8	-132.8	0.46 (1)	2.00
AD- H	-5686 / 257	-132.8	-132.8	0.46 (1)	2.00
H- I	-5686 / 257	-132.8	-132.8	0.46 (1)	2.00
I- J	-5497 / 220	-132.8	-132.8	0.45 (1)	2.00
J- K	-5257 / 232	-120.3	-120.3	0.47 (1)	3.46
K- L	-5257 / 232	-120.3	-120.3	0.47 (1)	3.46
L- M	-5355 / 228	-120.3	-120.3	0.49 (3)	3.42
M- N	-3724 / 173	-120.3	-120.3	0.21 (3)	4.31
N- O	0 / 44	-120.3	-120.3	0.10 (3)	10.00
AC- B	-4269 / 240	0.0	0.0	0.33 (1)	5.22
P- N	-4276 / 189	0.0	0.0	0.33 (1)	5.22
AC-AB	-135 / 148	-39.5	-39.5	0.07 (1)	6.25
AB-AA	-212 / 3353	-39.5	-39.5	0.48 (1)	6.25
AA- Z	-346 / 7050	-39.5	-39.5	0.97 (1)	6.25
Z- Y	-235 / 6270	-39.5	-39.5	0.81 (1)	6.25
Y- X	-235 / 6270	-39.5	-39.5	0.81 (1)	6.25
X- W	-74 / 5269	-39.5	-39.5	0.70 (1)	6.25
W- V	0 / 5497	-39.5	-39.5	0.71 (1)	10.00
V- U	0 / 5497	-39.5	-39.5	0.71 (1)	10.00
U- T	0 / 4674	-39.5	-39.5	0.63 (1)	10.00
T- S	0 / 4674	-39.5	-39.5	0.63 (1)	10.00
S- R	-30 / 4806	-39.5	-39.5	0.64 (1)	6.25
R- Q	-72 / 3356	-39.5	-39.5	0.48 (1)	6.25
Q- P	-11 / 24	-39.5	-39.5	0.07 (17)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AB- C	-1975 / 117	0.43 (1)
C-AA	-174 / 4868	0.78 (1)
AA- D	-3170 / 189	0.69 (1)
D- Z	-1038 / 130	0.90 (2)
Z- E	0 / 797	0.18 (2)
E- X	-1694 / 238	0.87 (2)
X- F	-100 / 1486	0.33 (2)
S- J	-65 / 608	0.14 (3)
S- L	-503 / 243	0.26 (3)
R- L	-644 / 72	0.45 (7)
R- M	0 / 1679	0.38 (1)
Q- M	-2127 / 133	0.46 (1)
B-AB	-141 / 4058	0.91 (1)
Q- N	-114 / 4078	0.92 (1)
U- J	-74 / 1660	0.27 (2)
F- W	-55 / 1086	0.17 (3)
U- I	-1192 / 136	0.72 (2)
W- G	-896 / 110	0.54 (2)
W- I	-640 / 1167	0.39 (10)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/180 (3.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.45")

CSI: TC=0.62/1.00 (D-E:2) , BC=0.97/1.00 (Z-AA:1) , WB=0.92/1.00 (N-Q:1) , SSI=0.31/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.99 (V) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T50A		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085444

Alpa Roof Truss, Maple

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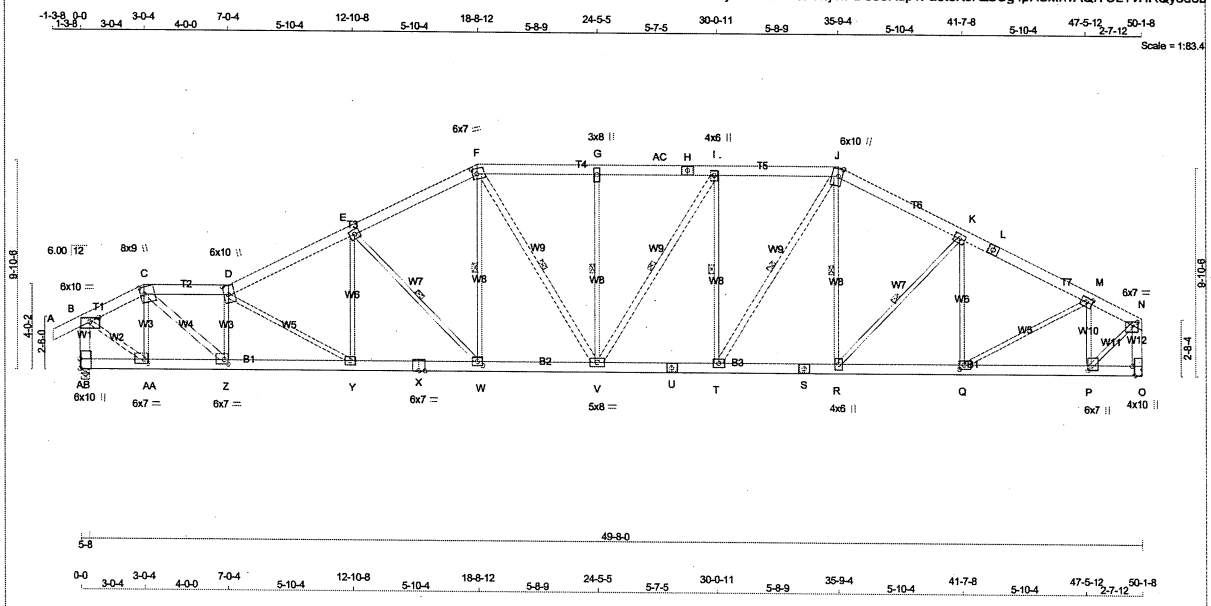

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
L - N	2x6	DRY	No.2	SPF
AB- B	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
AB- X	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - Z	2x4	DRY	No.2	SPF
T - J	2x4	DRY	No.2	SPF
F - V	2x4	DRY	No.2	SPF
V - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	10.0	Edge	
C	TTWW+m	MT20	8.0	9.0	4.00	2.00
D	TTWW+m	MT20	6.0	10.0	5.25	2.50
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	7.0	3.00	2.75
G	TMW+w	MT20	3.0	8.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW+m	MT20	6.0	10.0	Edge	2.25
K	TMWW-t	MT20	5.0	6.0		
L	TS-t	MT20	5.0	6.0		
M	TMWW-t	MT20	5.0	6.0	2.25	3.00
N	TMVW-p	MT20	6.0	7.0	1.75	3.00
O	BMV1+t	MT20	4.0	10.0	Edge	1.50
P	BMWW+t	MT20	6.0	7.0	2.75	1.75
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMWWW-t	MT20	5.0	8.0		
W	BMWW-t	MT20	5.0	6.0	2.25	3.00
X	BS-t	MT20	6.0	7.0		
Y	BMWW-t	MT20	5.0	6.0		
Z	BMWW-t	MT20	6.0	7.0	2.50	2.25
AA	BMWW-t	MT20	6.0	7.0	2.50	2.00
AB	BMV1+t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
Y. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AB	4269	0	4269	168	-222	5-8	5-8
O	4120	0	4120	0	-151	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O. MINIMUM BEARING LENGTH AT JOINT O = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT AB FOR 222 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 151 LBS FACTORED UPLIFT

PROVIDE FOR 168 LBS FACTORED HORIZONTAL REACTION AT JOINT AB

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	3149	1857 / 0	526 / 0	0 / 0	89 / -651	766 / 0	0 / 0
O	3049	1759 / 0	526 / 0	0 / 0	76 / -599	764 / 0	0 / 0
HORIZONTAL REACTIONS							
AB	—	0 / 0	0 / 0	0 / 0	120 / -105	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB

BRACING
FOR SECTION F-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-W, F-W, J-R, K-R, J-T, F-V, I-T, G-V, I-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-120.3	-120.3	0.10 (2)	10.00
B- C	-3790 / 213	-120.3	-120.3	0.16 (2)	4.31
C- D	-6897 / 343	-120.3	-120.3	0.37 (2)	3.09
D- E	-6903 / 316	-120.3	-120.3	0.61 (2)	2.91
E- F	-5844 / 285	-120.3	-120.3	0.53 (2)	3.24
F- G	-5606 / 255	-132.8	-132.8	0.45 (1)	2.00
G-AC	-5606 / 256	-132.8	-132.8	0.45 (1)	2.00
AC- H	-5606 / 256	-132.8	-132.8	0.45 (1)	2.00
H- I	-5606 / 256	-132.8	-132.8	0.45 (1)	2.00
I- J	-5398 / 213	-132.8	-132.8	0.44 (1)	2.00
J- K	-5127 / 223	-120.3	-120.3	0.45 (3)	3.52
K- L	-5132 / 213	-120.3	-120.3	0.47 (3)	3.52
L- M	-5132 / 213	-120.3	-120.3	0.47 (3)	3.52
M- N	-3254 / 144	-120.3	-120.3	0.20 (3)	4.57
AB- B	-4238 / 239	0.0	0.0	0.32 (1)	5.24
O- N	-4096 / 156	0.0	0.0	0.33 (1)	5.33
AB-AA	-144 / 136	-39.5	-39.5	0.06 (1)	6.25
AA- Z	-222 / 3326	-39.5	-39.5	0.47 (1)	6.25
Z- Y	-355 / 6989	-39.5	-39.5	0.97 (1)	6.25
Y- X	-244 / 6209	-39.5	-39.5	0.81 (1)	6.25
X- W	-244 / 6209	-39.5	-39.5	0.81 (1)	6.25
W- V	-84 / 5208	-39.5	-39.5	0.69 (1)	6.25
V- U	0 / 5399	-39.5	-39.5	0.70 (1)	10.00
U- T	0 / 5399	-39.5	-39.5	0.70 (1)	10.00
T- S	0 / 4557	-39.5	-39.5	0.61 (1)	10.00
S- R	0 / 4557	-39.5	-39.5	0.61 (1)	10.00
R- Q	-40 / 4605	-39.5	-39.5	0.62 (1)	6.25
Q- P	-67 / 2940	-39.5	-39.5	0.42 (1)	6.25
P- O	-12 / 26	-39.5	-39.5	0.07 (17)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- C	-1959 / 117	0.43 (1)
C- Z	-173 / 4823	0.78 (1)
Z- D	-3139 / 188	0.68 (1)
D- Y	-1038 / 130	0.90 (2)
Y- E	0 / 797	0.18 (2)
E- W	-1694 / 238	0.87 (2)
W- F	-100 / 1486	0.33 (2)
R- J	-107 / 527	0.12 (3)
R- K	-391 / 311	0.20 (3)
Q- K	-757 / 74	0.53 (1)
Q- M	0 / 1927	0.43 (1)
P- M	-2352 / 146	0.50 (1)
B-AA	-140 / 4026	0.91 (1)
P- N	-120 / 3891	0.88 (1)
T- J	-75 / 1681	0.27 (2)
F- V	-53 / 1054	0.17 (3)
T- I	-1210 / 137	0.73 (2)
V- G	-897 / 110	0.54 (2)
V- I	-612 / 1188	0.38 (10)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

TOTAL WEIGHT = 4 X 313 = 1251 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	35.1	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(60 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/180 (3.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.44")

CSI: TC=0.61/1.00 (D-E:2) , BC=0.97/1.00 (Y-Z:1) , WB=0.91/1.00 (B-AA:1) , SSI=0.31/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Z) (INPUT = 0.90)
JSI METAL= 0.97 (U) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T51		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085445

Alpa Roof Truss, Maple

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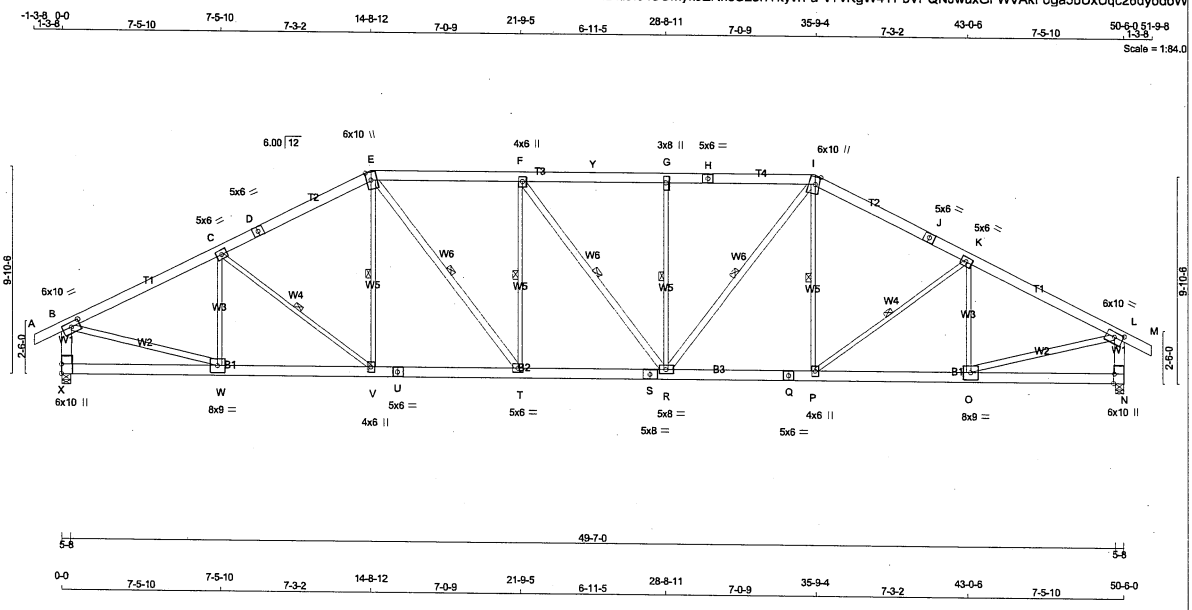

TOTAL WEIGHT = 3 X 306 = 919 lb
[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
I - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - T	2x4	DRY	No.2	SPF
F - R	2x4	DRY	No.2	SPF
R - I	2x4	DRY	No.2	SPF
B - W	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	10.0	2.75	5.00
C	TMWW-t	MT20	5.0	6.0		
D, H, J						
D	TS-t	MT20	5.0	6.0		
E	TTWW+m	MT20	6.0	10.0	Edge	2.25
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	8.0		
I	TTWW+m	MT20	6.0	10.0	Edge	2.25
K	TMWW-t	MT20	5.0	6.0		
L	TMVW-t	MT20	6.0	10.0	2.75	5.00
N	BMV1+t	MT20	6.0	10.0	Edge	0.50
O	BMWW-t	MT20	8.0	9.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	8.0		
T	BMWW-t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW+t	MT20	4.0	6.0		
W	BMWW-t	MT20	8.0	9.0		
X	BMV1+t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	4333	0	4333	-159	-184	5-8	5-8
N	4333	0	4333	0	-184	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT X FOR 184 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT N FOR 184 LBS FACTORED UPLIFT

PROVIDE FOR 159 LBS FACTORED HORIZONTAL REACTION AT JOINT X

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	3199	1870 / 0	530 / 0	0 / 0	81 / -645	799 / 0	0 / 0
N	3199	1870 / 0	530 / 0	0 / 0	81 / -645	799 / 0	0 / 0
HORIZONTAL REACTIONS							
X	—	0 / 0	0 / 0	0 / 0	114 / -114	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

FOR SECTION E-I, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-V, E-V, E-T, F-T, F-R, G-R, I-R, I-P, K-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-120.3	-120.3	0.10 (2)	10.00
B- C	-5311 / 214	-120.3	-120.3	0.85 (2)	3.08
C- D	-5306 / 226	-120.3	-120.3	0.82 (2)	3.09
D- E	-5306 / 226	-120.3	-120.3	0.82 (2)	3.09
E- F	-5663 / 216	-132.8	-132.8	0.68 (1)	2.00
F- Y	-5656 / 216	-132.8	-132.8	0.68 (1)	2.00
Y- G	-5656 / 216	-132.8	-132.8	0.68 (1)	2.00
G- H	-5656 / 215	-132.8	-132.8	0.65 (1)	2.00
H- I	-5656 / 215	-132.8	-132.8	0.65 (1)	2.00
I- J	-5307 / 226	-120.3	-120.3	0.82 (3)	3.09
J- K	-5307 / 226	-120.3	-120.3	0.82 (3)	3.09
K- L	-5311 / 214	-120.3	-120.3	0.85 (3)	3.08
L- M	0 / 44	-120.3	-120.3	0.10 (3)	10.00
X- B	-4205 / 223	0.0	0.0	0.32 (1)	5.26
N- L	-4205 / 223	0.0	0.0	0.32 (1)	5.26
X- W	-135 / 148	-39.5	-39.5	0.19 (17)	6.25
W- V	-187 / 4790	-39.5	-39.5	0.67 (1)	6.25
V- U	-30 / 4706	-39.5	-39.5	0.66 (1)	6.25
U- T	-30 / 4706	-39.5	-39.5	0.66 (1)	6.25
T- S	0 / 5664	-39.5	-39.5	0.76 (1)	10.00
S- R	0 / 5664	-39.5	-39.5	0.76 (1)	10.00
R- Q	0 / 4707	-39.5	-39.5	0.67 (1)	10.00
Q- P	0 / 4707	-39.5	-39.5	0.67 (1)	10.00
P- O	-68 / 4789	-39.5	-39.5	0.68 (1)	6.25
O- N	-11 / 24	-39.5	-39.5	0.19 (17)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-1001 / 139	0.53 (1)
C- V	-413 / 289	0.25 (2)
V- E	-39 / 542	0.12 (2)
E- T	-53 / 1596	0.26 (1)
T- F	-1019 / 134	0.60 (1)
F- R	-950 / 933	0.71 (2)
R- G	-1120 / 137	0.68 (3)
R- I	-52 / 1581	0.25 (1)
P- I	-35 / 546	0.12 (3)
P- K	-412 / 290	0.25 (3)
O- K	-1003 / 139	0.53 (1)
B- W	-94 / 4961	0.80 (1)
O- L	-94 / 4960	0.80 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/180 (3.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.85/1.00 (B-C:2) , BC=0.76/1.00 (R-T:1) , WB=0.80/1.00 (B-W:1) , SSI=0.38/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.96 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T53		1	GREEN PARK HOMES JT 53080	E22085446

Alpa Roof Truss, Maple

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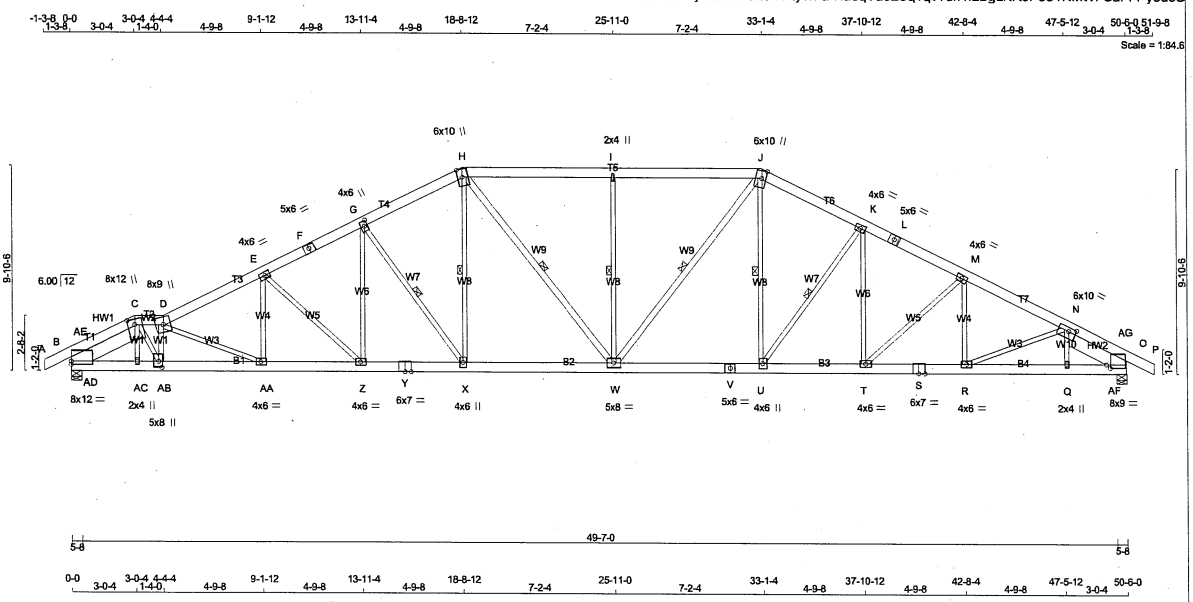

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
L - P	2x6	DRY	No.2	SPF
B - Y	2x6	DRY	2100F 1.8E	SPF
Y - V	2x6	DRY	2100F 1.8E	SPF
V - S	2x6	DRY	2100F 1.8E	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - W	2x4	DRY	No.2	SPF
W - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	8.0	12.0	2.00	
C	TTWWW+m	MT20	8.0	12.0	3.25	3.75
D	TTWW+m	MT20	8.0	9.0		
E, K, M						
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0	3.00	1.75
H	TTWW+m	MT20	6.0	10.0	Edge	2.25
I	TMW+w	MT20	2.0	4.0		
J	TTWW+m	MT20	6.0	10.0	Edge	2.25
L	TS-t	MT20	5.0	6.0		
N	TMWWW-t	MT20	6.0	10.0	2.25	4.00
O	TMBMW1-I	MT20	8.0	9.0	2.00	2.25
Q	BMW+w	MT20	2.0	4.0		
R, T, Z, AA						
R	BMWW-t	MT20	4.0	6.0		
S	BS-t	MT20	6.0	7.0		
U	BMWW+t	MT20	4.0	6.0		
V	BS-t	MT20	5.0	6.0		
W	BMWWW-t	MT20	5.0	8.0		
X	BMWW+t	MT20	4.0	6.0		
Y	BS-t	MT20	6.0	7.0		
AB	BMWW+t	MT20	5.0	8.0	3.50	1.75
AC	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
B	4289	0	4289	139	-220	5-8	4-10
O	4294	0	4294	0	-207	5-8	4-10

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 220 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 207 LBS FACTORED UPLIFT

PROVIDE FOR 139 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3163	1870 / 0	530 / 0	0 / 0	85 / -648	763 / 0	0 / 0
O	3167	1870 / 0	530 / 0	0 / 0	79 / -641	767 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	99 / -99	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, O

BRACING

FOR SECTION H-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-X, H-X, H-W, I-W, J-W, J-U, K-U.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-120.3	-120.3	0.09 (2)	10.00
B-AE	-4540 / 198	-120.3	-120.3	0.13 (1)	4.01
AE- C	-2961 / 186	-120.3	-120.3	0.09 (1)	4.84
C- D	-7146 / 340	-120.3	-120.3	0.22 (1)	3.18
D- E	-7586 / 340	-120.3	-120.3	0.49 (1)	2.88
E- F	-6739 / 319	-120.3	-120.3	0.40 (1)	3.15
F- G	-6739 / 319	-120.3	-120.3	0.40 (1)	3.15
G- H	-5882 / 288	-120.3	-120.3	0.36 (1)	3.39
H- I	-5715 / 234	-132.8	-132.8	0.80 (1)	2.00
I- J	-5715 / 234	-132.8	-132.8	0.80 (1)	2.00
J- K	-5699 / 271	-120.3	-120.3	0.35 (1)	3.45
K- L	-6323 / 291	-120.3	-120.3	0.39 (1)	3.27
L- M	-6323 / 291	-120.3	-120.3	0.39 (1)	3.27
M- N	-6704 / 293	-120.3	-120.3	0.42 (1)	3.16
N-AG	-2965 / 158	-120.3	-120.3	0.11 (3)	4.83
AG- O	-4621 / 193	-120.3	-120.3	0.14 (1)	4.00
O- P	0 / 1	-120.3	-120.3	0.09 (3)	10.00
B-AD	-240 / 2629	-39.5	-39.5	0.22 (1)	6.25
AD-AC	-335 / 5402	-39.5	-39.5	0.35 (1)	6.25
AC-AB	-337 / 5437	-39.5	-39.5	0.37 (1)	6.25
AB-AA	-402 / 7310	-39.5	-39.5	0.46 (1)	6.25
AA- Z	-319 / 6809	-39.5	-39.5	0.37 (1)	6.25
Z- Y	-202 / 6030	-39.5	-39.5	0.34 (1)	6.25
Y- X	-202 / 6030	-39.5	-39.5	0.34 (1)	6.25
X- W	-76 / 5252	-39.5	-39.5	0.30 (1)	6.25
W- V	0 / 5085	-39.5	-39.5	0.29 (1)	10.00
V- U	0 / 5085	-39.5	-39.5	0.29 (1)	10.00
U- T	-39 / 5659	-39.5	-39.5	0.32 (1)	6.25
T- S	-134 / 6005	-39.5	-39.5	0.33 (1)	6.25
S- R	-134 / 6005	-39.5	-39.5	0.33 (1)	6.25
R- Q	-190 / 5353	-39.5	-39.5	0.30 (1)	6.25
Q-AF	-188 / 5359	-39.5	-39.5	0.33 (1)	6.25
AF- O	-89 / 2657	-39.5	-39.5	0.20 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AC- C	-29 / 735	0.17 (1)
C-AB	-117 / 3510	0.79 (1)
AB- D	-3487 / 184	0.54 (1)
D-AA	-662 / 91	0.31 (2)
AA- E	0 / 547	0.12 (2)
E- Z	-1218 / 162	0.93 (2)
Z- G	-50 / 986	0.22 (2)
G- X	-1566 / 222	0.75 (2)
X- H	-103 / 1549	0.35 (2)
H- W	-199 / 1279	0.21 (3)
W- I	-1160 / 98	0.69 (1)
W- J	-102 / 1482	0.24 (2)
U- J	-90 / 1273	0.29 (3)
U- K	-1233 / 207	0.59 (3)
T- K	-29 / 611	0.14 (3)
T- M	-660 / 131	0.51 (3)
R- M	-159 / 127	0.05 (7)
R- N	0 / 715	0.16 (1)
Q- N	0 / 245	0.06 (4)
C-AD	-3840 / 133	0.30 (1)
AD-AE	-27 / 2157	0.00 (1)
N-AF	-3746 / 138	0.29 (1)
AF-AG	-66 / 2287	0.00 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/180 (3.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.80/1.00 (I-J:1) , BC=0.46/1.00 (AA-AB:1) , WB=0.93/1.00 (E-Z:2) , SSI=0.38/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.93 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T53A		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085447

Alpa Roof Truss, Maple

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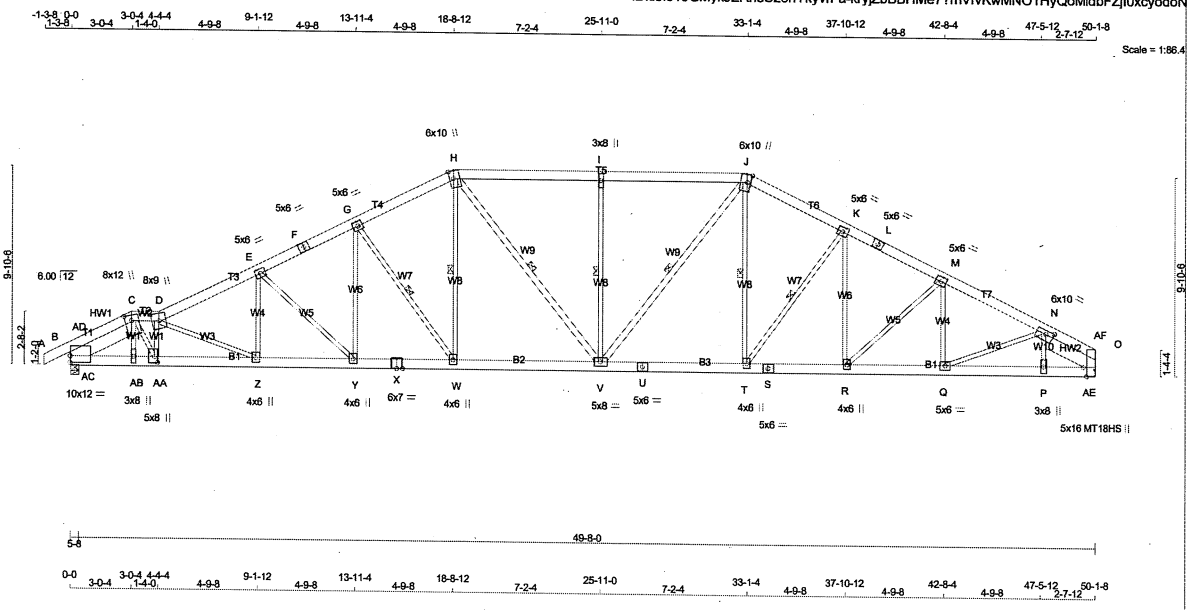

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
L - O	2x6	DRY	No.2	SPF
B - X	2x6	DRY	2100F 1.8E	SPF
X - U	2x6	DRY	2100F 1.8E	SPF
U - S	2x6	DRY	2100F 1.8E	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x8	DRY	No.2	SPF
HW2	2x8	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - V	2x4	DRY	No.2	SPF
V - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-I	MT20	10.0	12.0	4.00	
C	TTWWW+m	MT20	8.0	12.0	3.25	3.75
D	TTWW+m	MT20	8.0	9.0		
E, G, K, M						
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	10.0	Edge	2.25
I	TMW+w	MT20	3.0	8.0		
J	TTWW+m	MT20	6.0	10.0	Edge	2.25
L	TS-t	MT20	5.0	6.0		
N	TMWWW-t	MT20	6.0	10.0	2.25	4.25
O	TMBMW1+t	MT18HS	5.0	16.0	Edge	2.00
P	BMW+w	MT20	3.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R, T, W, Y, Z						
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	5.0	6.0		
U	BS-t	MT20	5.0	6.0		
V	BMWWW-t	MT20	5.0	8.0		
X	BS-t	MT20	6.0	7.0		
AA	BMWW+t	MT20	5.0	8.0	3.50	1.75
AB	BMW+w	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
Y. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4259	0	4259	139	-220	5-8	4-10
O	4097	0	4097	0	-177	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O. MINIMUM BEARING LENGTH AT JOINT O = 3-8.

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 220 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT O FOR 177 LBS FACTORED UPLIFT

PROVIDE FOR 139 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3141	1857 / 0	526 / 0	0 / 0	85 / -644	758 / 0	0 / 0
O	3031	1759 / 0	526 / 0	0 / 0	83 / -606	746 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	99 / -99	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
FOR SECTION H-J, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-W, H-W, H-V, I-V, J-V, J-T, K-T.

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-120.3	-120.3	0.09 (2)	10.00	
B-AD	-4506 / 197	-120.3	-120.3	0.13 (1)	4.02	
AD- C	-2939 / 185	-120.3	-120.3	0.09 (1)	4.85	
C- D	-7088 / 339	-120.3	-120.3	0.22 (1)	3.19	
D- E	-7518 / 339	-120.3	-120.3	0.49 (1)	2.90	
E- F	-6671 / 319	-120.3	-120.3	0.39 (1)	3.17	
F- G	-6671 / 319	-120.3	-120.3	0.39 (1)	3.17	
G- H	-5813 / 287	-120.3	-120.3	0.36 (1)	3.41	
H- I	-5631 / 233	-132.8	-132.8	0.79 (1)	2.00	
I- J	-5631 / 233	-132.8	-132.8	0.79 (1)	2.00	
J- K	-5579 / 265	-120.3	-120.3	0.35 (1)	3.50	
K- L	-6138 / 283	-120.3	-120.3	0.37 (1)	3.33	
L- M	-6138 / 283	-120.3	-120.3	0.37 (1)	3.33	
M- N	-6384 / 279	-120.3	-120.3	0.39 (1)	3.25	
N-AF	-1779 / 106	-120.3	-120.3	0.13 (1)	5.83	
AF- O	-3848 / 160	-120.3	-120.3	0.19 (1)	4.25	
B-AC	-240 / 2610	-39.5	-39.5	0.22 (1)	6.25	
AC-AB	-335 / 5359	-39.5	-39.5	0.35 (1)	6.25	
AB-AA	-337 / 5394	-39.5	-39.5	0.37 (1)	6.25	
AA- Z	-402 / 7251	-39.5	-39.5	0.46 (1)	6.25	
Z- Y	-319 / 6748	-39.5	-39.5	0.36 (1)	6.25	
Y- X	-202 / 5969	-39.5	-39.5	0.34 (1)	6.25	
X- W	-202 / 5969	-39.5	-39.5	0.34 (1)	6.25	
W- V	-76 / 5191	-39.5	-39.5	0.30 (1)	6.25	
V- U	0 / 4977	-39.5	-39.5	0.29 (1)	10.00	
U- T	0 / 4977	-39.5	-39.5	0.29 (1)	10.00	
T- S	-32 / 5494	-39.5	-39.5	0.31 (1)	6.25	
S- R	-32 / 5494	-39.5	-39.5	0.31 (1)	6.25	
R- Q	-122 / 5720	-39.5	-39.5	0.32 (1)	6.25	
Q- P	-162 / 4695	-39.5	-39.5	0.27 (1)	6.25	
P-AE	-160 / 4703	-39.5	-39.5	0.31 (1)	6.25	
AE- O	-41 / 1573	-39.5	-39.5	0.16 (1)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AB- C	-29 / 730	0.16 (1)
C-AA	-116 / 3480	0.78 (1)
AA- D	-3456 / 184	0.53 (1)
D- Z	-663 / 91	0.31 (2)
Z- E	0 / 548	0.12 (2)
E- Y	-1218 / 162	0.93 (2)
Y- G	-50 / 986	0.22 (2)
G- W	-1566 / 222	0.75 (2)
W- H	-103 / 1549	0.35 (2)
H- V	-216 / 1244	0.20 (3)
V- I	-1161 / 98	0.69 (1)
V- J	-102 / 1504	0.24 (2)
T- J	-86 / 1195	0.27 (3)
T- K	-1139 / 202	0.54 (3)
R- K	-24 / 508	0.11 (3)
R- M	-508 / 124	0.39 (3)
Q- M	-313 / 60	0.10 (7)
Q- N	0 / 1123	0.25 (1)
P- N	0 / 279	0.06 (4)
C-AC	-3808 / 133	0.30 (1)
AC-AD	-27 / 2140	0.00 (1)
N-AE	-4430 / 168	0.33 (1)
AE-AF	-81 / 2526	0.00 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

TOTAL WEIGHT = 4 X 308 = 1232 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN HIGHEST FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

LOADING IN OTHER FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/180 (3.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.40")

CSI: TC=0.79/1.00 (I-J:1) , BC=0.46/1.00 (Z-AA:1) , WB=0.93/1.00 (E-Y:2) , SSI=0.38/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.94 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T54		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085448

Alpa Roof Truss, Maple

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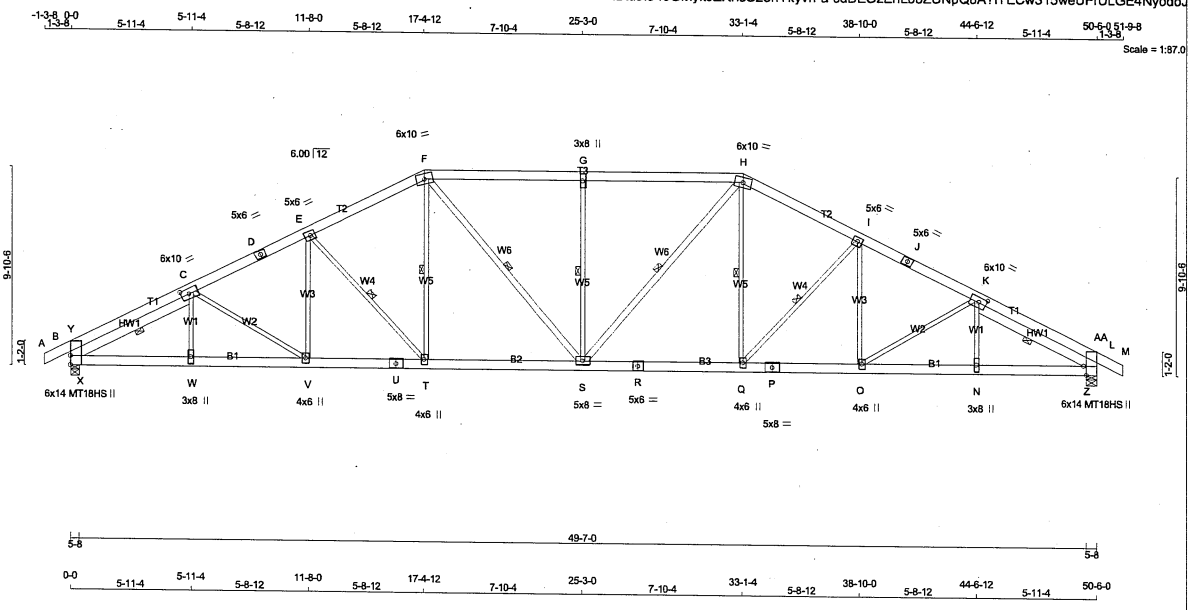

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
B - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - S	2x4	DRY	No.2	SPF
S - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1+t	MT18HS	6.0	14.0	Edge	
C	TMWWW-t	MT20	6.0	10.0	3.00	4.50
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TTWW-m	MT20	6.0	10.0		
G	TMW+w	MT20	3.0	8.0		
H	TTWW-m	MT20	6.0	10.0		
I	TMWW-t	MT20	5.0	6.0		
J	TS-t	MT20	5.0	6.0		
K	TMWWW-t	MT20	6.0	10.0	3.00	4.50
L	TMBMW1+t	MT18HS	6.0	14.0	Edge	1.50
N	BMW+w	MT20	3.0	8.0		
O, Q, T, V						
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	8.0		
U	BS-t	MT20	5.0	8.0		
W	BMW+w	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	4300	0	4300	139	-208	5-8	5-8
L	4300	0	4300	0	-208	5-8	5-8

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 208 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT L FOR 208 LBS FACTORED UPLIFT

PROVIDE FOR 139 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	3172	1870 / 0	530 / 0	0 / 0	82 / -645	772 / 0	0 / 0
L	3172	1870 / 0	530 / 0	0 / 0	82 / -645	772 / 0	0 / 0
HORIZONTAL REACTIONS							
B	—	0 / 0	0 / 0	0 / 0	99 / -99	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, L

BRACING
FOR SECTION F-H, MAX. UNBRACED TOP CHORD LENGTH = 2.00 FT.
FOR OTHER SECTIONS, MAX. UNBRACED TOP CHORD LENGTH = 3.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-T, F-T, F-S, G-S, H-S, H-Q, I-Q, C-X, K-Z.

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 1	-120.3	-120.3	0.09 (2)	10.00
B- Y	-5011 / 142	-120.3	-120.3	0.39 (1)	3.56
Y- C	-2978 / 221	-120.3	-120.3	0.34 (2)	4.56
C- D	-6462 / 289	-120.3	-120.3	0.53 (1)	3.10
D- E	-6462 / 289	-120.3	-120.3	0.53 (1)	3.10
E- F	-5721 / 263	-120.3	-120.3	0.48 (1)	3.34
F- G	-5757 / 217	-132.8	-132.8	0.98 (1)	2.00
G- H	-5757 / 217	-132.8	-132.8	0.98 (1)	2.00
H- I	-5721 / 263	-120.3	-120.3	0.48 (1)	3.34
I- J	-6462 / 289	-120.3	-120.3	0.53 (1)	3.10
J- K	-6462 / 289	-120.3	-120.3	0.53 (1)	3.10
K-AA	-2978 / 220	-120.3	-120.3	0.34 (3)	4.56
AA- L	-5011 / 142	-120.3	-120.3	0.39 (1)	3.56
L- M	0 / 1	-120.3	-120.3	0.09 (3)	10.00
B- X	-249 / 2645	-39.5	-39.5	0.37 (1)	6.25
X- W	-308 / 5991	-39.5	-39.5	0.78 (1)	6.25
W- V	-309 / 5988	-39.5	-39.5	0.80 (1)	6.25
V- U	-204 / 5796	-39.5	-39.5	0.79 (1)	6.25
U- T	-204 / 5796	-39.5	-39.5	0.79 (1)	6.25
T- S	-60 / 5098	-39.5	-39.5	0.72 (1)	6.25
S- R	0 / 5098	-39.5	-39.5	0.72 (1)	10.00
R- Q	0 / 5098	-39.5	-39.5	0.72 (1)	10.00
Q- P	-65 / 5796	-39.5	-39.5	0.79 (1)	6.25
P- O	-65 / 5796	-39.5	-39.5	0.79 (1)	6.25
O- N	-171 / 5988	-39.5	-39.5	0.80 (1)	6.25
N- Z	-170 / 5991	-39.5	-39.5	0.78 (1)	6.25
Z- L	-110 / 2645	-39.5	-39.5	0.37 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	0 / 272	0.07 (17)
C- V	-428 / 125	0.36 (2)
V- E	0 / 456	0.10 (5)
E- T	-1281 / 216	0.65 (2)
T- F	-71 / 1234	0.28 (2)
F- S	-99 / 1473	0.24 (3)
S- G	-1275 / 108	0.76 (1)
S- H	-99 / 1473	0.24 (2)
Q- H	-71 / 1234	0.28 (3)
Q- I	-1281 / 216	0.65 (3)
O- I	0 / 456	0.10 (6)
O- K	-428 / 126	0.36 (3)
N- K	0 / 272	0.07 (17)
X- Y	0 / 2544	0.00 (1)
X- C	-4021 / 71	0.60 (1)
K- Z	-4021 / 72	0.60 (1)
Z-AA	0 / 2544	0.00 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

TOTAL WEIGHT = 3 X 306 = 917 lb
[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE HEEL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 5.0 P.S.F.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.28")
ALLOWABLE DEFL.(TL)= L/180 (3.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.41")

CSI: TC=0.98/1.00 (G-H:1) , BC=0.80/1.00 (V-W:1) , WB=0.76/1.00 (G-S:1) , SSI=0.41/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MT18HS	586	403	2455	1382	3163	3004

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.97 (U) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T56		4	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085449

Alpa Roof Truss, Maple

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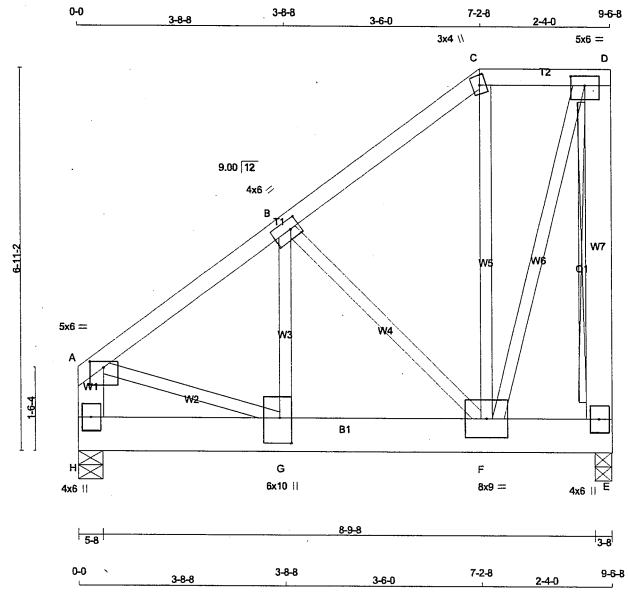


TOTAL WEIGHT = 8 X 68 = 540 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
H - E	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- E	2	12	TOP
H- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	4	4	SIDE(1356.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSAO86 3.3.2.

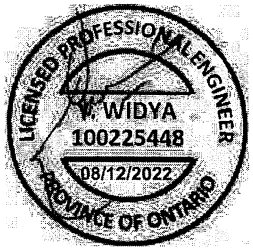
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.25	3.00
B	TMWW-t	MT20	4.0	6.0	2.00	2.00
C	TTW+m	MT20	3.0	4.0		
D	TMVW-t	MT20	5.0	6.0	2.00	3.00
E	BMV1+p	MT20	4.0	6.0		
F	BMWWW-t	MT20	8.0	9.0		
G	BMWW+t	MT20	6.0	10.0	5.50	2.50
H	BMV1+p	MT20	4.0	6.0		

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	9393	0	9393	0	-386	3-8	2-9
H	9393	0	9393	204	-317	5-8	2-9

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 386 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 317 LBS FACTORED UPLIFT

PROVIDE FOR 204 LBS FACTORED HORIZONTAL REACTION AT JOINT H

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	6925	4162 / 0	1214 / 0	0 / 0	1329 / -1272	1549 / 0	0 / 0
H	6925	4162 / 0	1214 / 0	0 / 0	1273 / -1222	1549 / 0	0 / 0
HORIZONTAL REACTIONS							
H	—	0 / 0	0 / 0	0 / 0	146 / -97	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING
MAX. UNBRACED TOP CHORD LENGTH = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x8 DRY SPF No.2 T-BRACE AT D-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)
A- B	-7507 / 263	-120.3	-120.3	0.16 (2)
B- C	-3205 / 168	-120.3	-120.3	0.08 (2)
C- D	-2568 / 175	-120.3	-120.3	0.05 (3)
E- D	-7585 / 333	0.0	0.0	0.19 (1)
H- A	-6390 / 239	0.0	0.0	0.12 (1)
H- G	-192 / 131	-1848.6	-1848.6	0.32 (2)
G- F	-280 / 6020	-1848.6	-1848.6	0.42 (1)
F- E	-28 / 72	-1848.6	-1848.6	0.12 (1)

WEBS

MEMB. FR-TO	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	4.46	G- B	-164 / 5878	0.33 (3)
B- F	6.25	B- F	-5073 / 291	0.55 (2)
F- C	6.25	F- C	-97 / 1680	0.09 (2)
A- G	7.81	A- G	-161 / 6250	0.35 (1)
F- D	7.46	F- D	-320 / 7894	0.44 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 46-5-8
END DISTANCE = 9-6-8
END SPAN CARRIED = 46-5-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 83 % OF GSL.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/180 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.19/1.00 (D-E:1) , BC=0.42/1.00 (F-G:1) , WB=0.55/1.00 (B-F:2) , SSI=0.56/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.45 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	T57		4	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085450

Alpa Roof Truss, Maple

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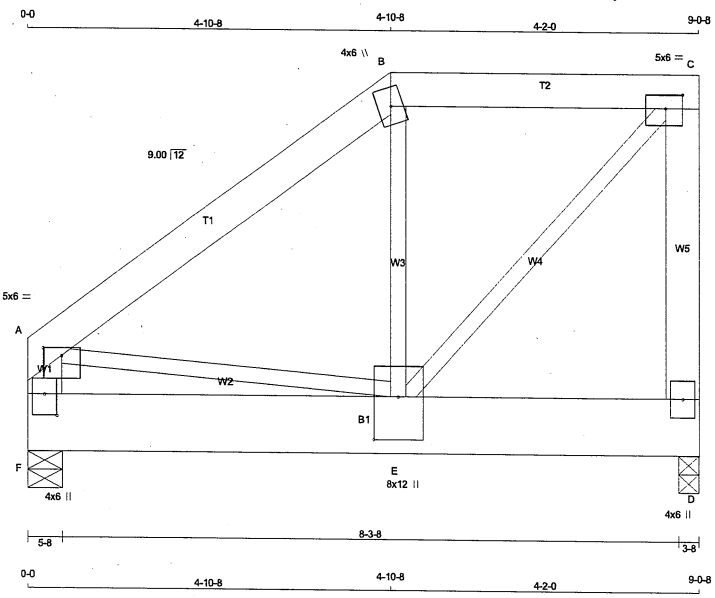

TOTAL WEIGHT = 8 X 65 = 523 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x10	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- B 2	12	TOP
B- C 2	12	TOP
C- D 2	12	TOP
F- A 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
F- D 5	4	SIDE(1468.7)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSA086 3.3.2.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.25	3.00
B	TTW+m	MT20	4.0	6.0		
C	TMVW-t	MT20	5.0	6.0	2.25	2.75
D	BMV1+p	MT20	4.0	6.0		
E	BMWWW+t	MT20	8.0	12.0	7.00	4.00
F	BMV1+p	MT20	4.0	6.0	3.50	2.00

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	9576	0	9576	0	-373	3-8	2-10
F	9576	0	9576	143	-337	5-8	2-10

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 373 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 337 LBS FACTORED UPLIFT

PROVIDE FOR 143 LBS FACTORED HORIZONTAL REACTION AT JOINT F

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	7059	4243 / 0	1237 / 0	0 / 0	1342 / -1282	1579 / 0	0 / 0
F	7059	4243 / 0	1237 / 0	0 / 0	1310 / -1256	1579 / 0	0 / 0

HORIZONTAL REACTIONS

F	—	0 / 0	0 / 0	0 / 0	102 / -70	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 5.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (18)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-6592 / 227	-120.3	-120.3	0.10 (2)	5.81	
B- C	-5389 / 253	-120.3	-120.3	0.07 (3)	6.25	
D- C	-6371 / 287	0.0	0.0	0.37 (1)	7.47	
F- A	-5152 / 221	0.0	0.0	0.09 (1)	7.81	
F- E	-134 / 93	-1997.8	-1997.8	0.42 (1)	6.25	
E- D	-20 / 50	-1997.8	-1997.8	0.27 (3)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-141 / 3560	0.20 (3)
E- C	-308 / 8183	0.46 (1)
A- E	-136 / 5333	0.30 (1)

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF { 7.5} PSF AT {30-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {ROUGH TERRAIN} AT MINIMUM {1.000} MILES DISTANCE FROM OPEN TERRAIN., AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 6.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	59.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 50-1-8
END DISTANCE = 9-0-8
END SPAN CARRIED = 50-1-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 83 % OF GSL.

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018 , ABC 2019
- PART 4 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED
- PERCENTAGE OF GROUND SNOW LOAD IS USER-DEFINED.

(80 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 35.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/180 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.42/1.00 (E-F:1) , WB=0.46/1.00 (C-E:1) , SSI=0.69/1.00 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL: = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
428162	PB1		1	TRUSS DESC.	JT 53080	E22085414

Alpa Roof Truss, Maple

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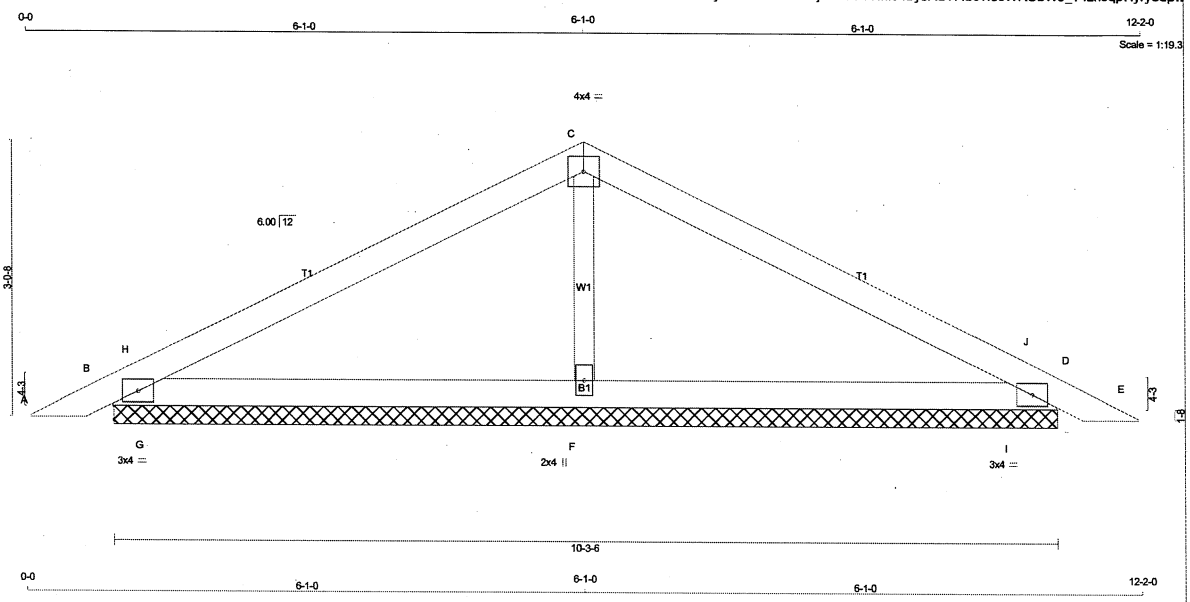

TOTAL WEIGHT = 22 X 29 = 638 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0		
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	418	0	418	0	0	10-3-6	1-8
D	418	0	418	0	0	10-3-6	1-8
F	493	0	493	0	0	10-3-6	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	293	210 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
D	293	210 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
F	351	219 / 0	0 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- H	-53 / 0	-95.2	-95.2	0.13 (1)	6.25
H- C	-179 / 0	-95.2	-95.2	0.30 (1)	6.25
C- J	-179 / 0	-95.2	-95.2	0.30 (1)	6.25
J- D	-53 / 0	-95.2	-95.2	0.13 (1)	6.25
D- E	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- G	0 / 152	-18.5	-18.5	0.28 (1)	10.00
G- F	0 / 152	-18.5	-18.5	0.28 (1)	10.00
F- I	0 / 152	-18.5	-18.5	0.28 (1)	10.00
I- D	0 / 152	-18.5	-18.5	0.28 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-257 / 0	0.04 (1)
G- H	-494 / 0	0.00 (1)
I- J	-494 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-J:1) , BC=0.28/1.00 (F-I:1) , WB=0.04/1.00 (C-F:1) , SSI=0.37/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (B) (INPUT = 0.90)
JSI METAL= 0.09 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB4		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085415

Alpa Roof Truss, Maple

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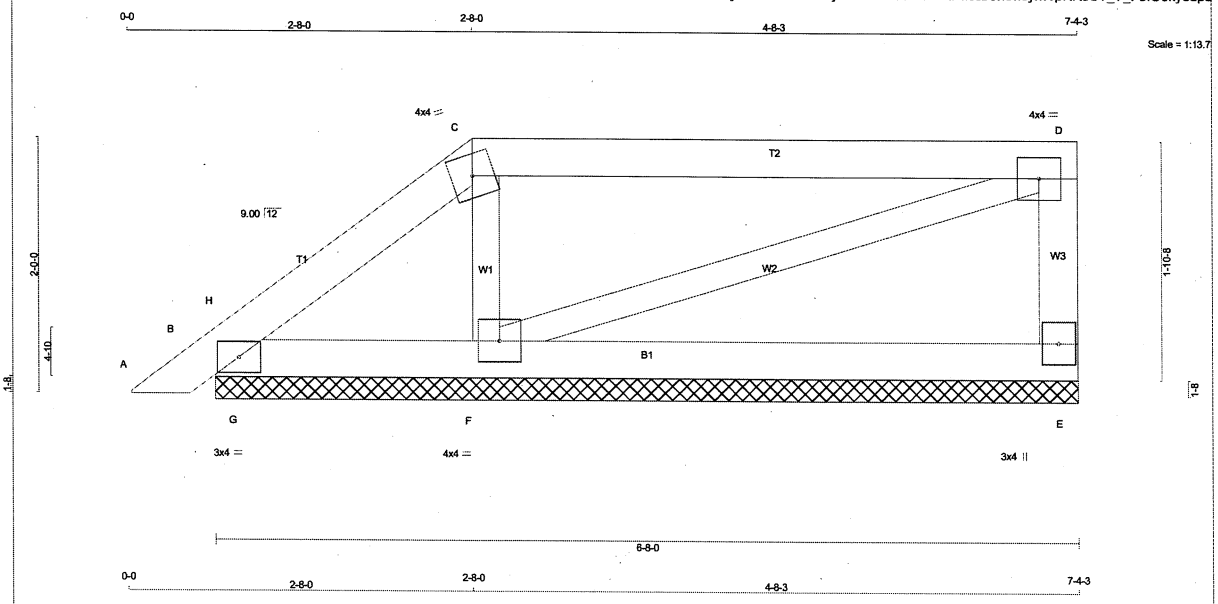

TOTAL WEIGHT = 6 X 23 = 137 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-l	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	262	0	262	0	0	6-8-0	1-8
B	177	0	177	0	0	6-8-0	1-8
F	374	0	374	0	0	6-8-0	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	184	128 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
B	122	99 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
F	267	160 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 15	-95.2	-95.2	0.02 (1)	10.00
B- H	-14 / 10	-95.2	-95.2	0.02 (1)	6.25
H- C	-48 / 0	-95.2	-95.2	0.04 (1)	6.25
C- D	-17 / 0	-95.2	-95.2	0.35 (1)	6.25
E- D	-229 / 0	0.0	0.0	0.02 (1)	7.81
B- G	0 / 35	-18.5	-18.5	0.05 (1)	10.00
G- F	0 / 35	-18.5	-18.5	0.09 (4)	10.00
F- E	0 / 0	-18.5	-18.5	0.09 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-271 / 0	0.04 (1)
F- D	0 / 18	0.00 (1)
G- H	-145 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35/1.00 (C-D:1) , BC=0.09/1.00 (F-G:4) , WB=0.04/1.00 (C-F:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
W. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB50		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085416

Alpa Roof Truss, Maple

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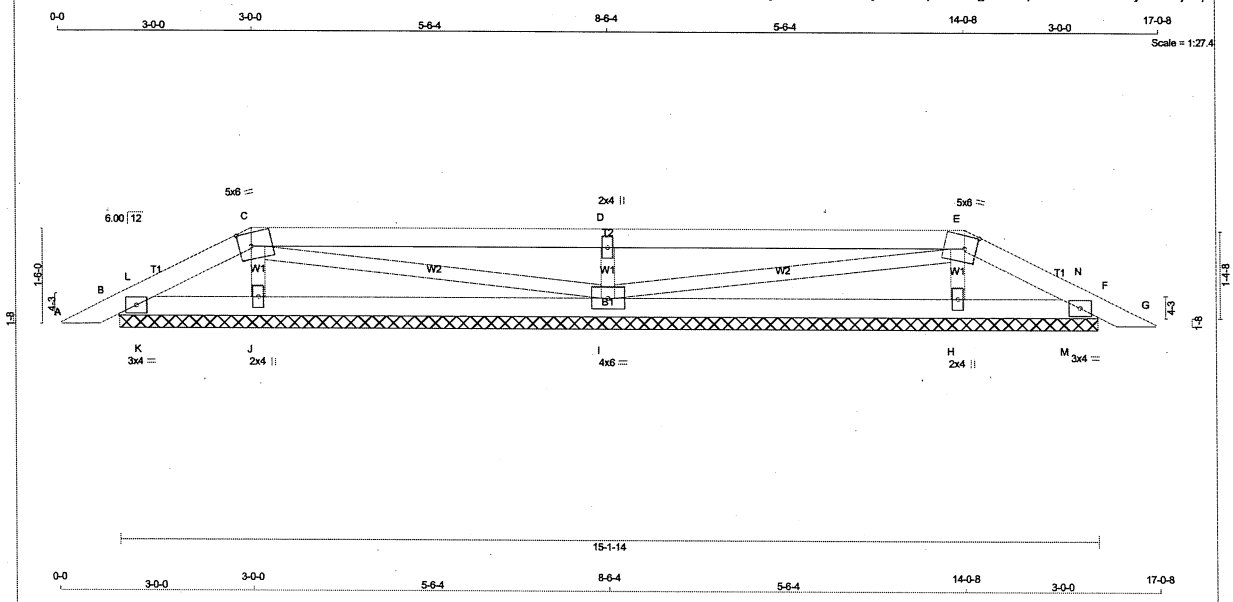

TOTAL WEIGHT = 48 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.50	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.25
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	200	0	200	0	0	15-1-14	1-8
F	200	0	200	0	0	15-1-14	1-8
J	360	0	360	0	0	15-1-14	1-8
I	764	0	764	0	0	15-1-14	1-8
H	360	0	360	0	0	15-1-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	138	110 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
F	138	110 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
J	257	156 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
I	537	369 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
H	257	156 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- L	-37 / 0	-95.2	-95.2	0.01 (4)	6.25
L- C	-43 / 0	-95.2	-95.2	0.04 (1)	6.25
C- D	-2 / 0	-95.2	-95.2	0.49 (1)	10.00
D- E	-2 / 0	-95.2	-95.2	0.49 (1)	10.00
E- N	-43 / 0	-95.2	-95.2	0.04 (1)	6.25
N- F	-37 / 0	-95.2	-95.2	0.01 (4)	6.25
F- G	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- K	0 / 37	-18.5	-18.5	0.05 (1)	10.00
K- J	0 / 37	-18.5	-18.5	0.08 (4)	10.00
J- I	0 / 12	-18.5	-18.5	0.12 (4)	10.00
I- H	0 / 12	-18.5	-18.5	0.12 (4)	10.00
H- M	0 / 37	-18.5	-18.5	0.08 (4)	10.00
M- F	0 / 37	-18.5	-18.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-253 / 0	0.04 (1)
C- I	-10 / 0	0.01 (1)
I- D	-657 / 0	0.09 (1)
I- E	-10 / 0	0.01 (1)
H- E	-253 / 0	0.04 (1)
K- L	-109 / 0	0.00 (1)
M- N	-109 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.49/1.00 (D-E:1) , BC=0.12/1.00 (H-I:4) , WB=0.09/1.00 (D-I:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (D) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
Y. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB51		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085417

Alpa Roof Truss, Maple

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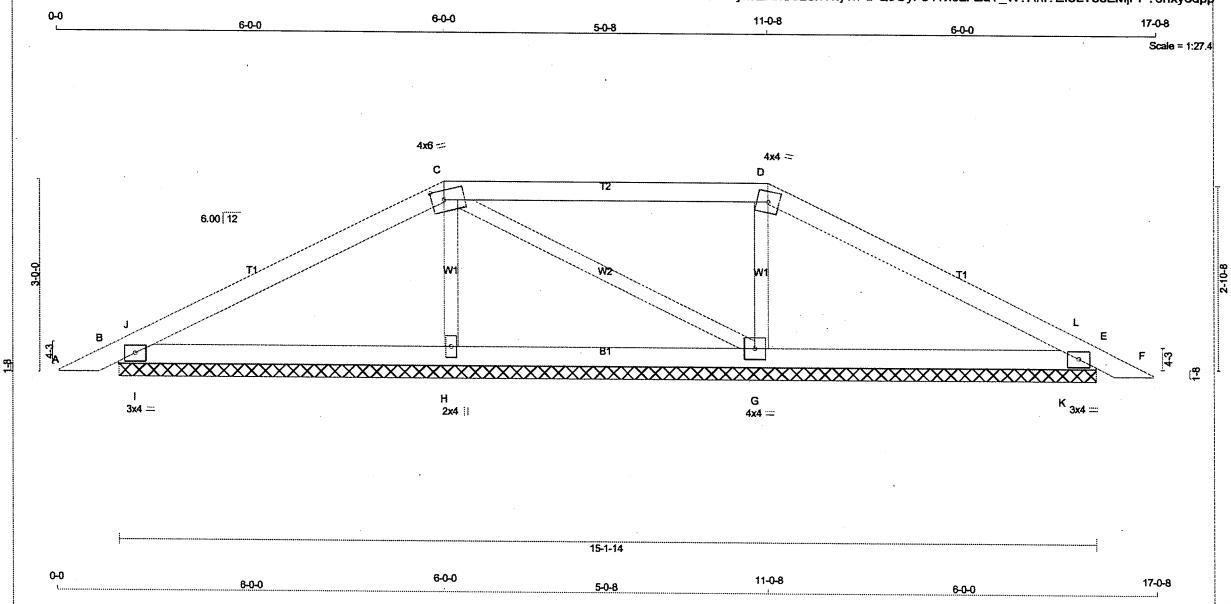

TOTAL WEIGHT = 46 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	429	0	429	0	0	15-1-14	1-8
E	412	0	412	0	0	15-1-14	1-8
H	495	0	495	0	0	15-1-14	1-8
G	549	0	549	0	0	15-1-14	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	300	216 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
E	288	206 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
H	351	224 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
G	388	255 / 0	0 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- J	-81 / 0	-95.2	-95.2	0.10 (1)	6.25
J- C	-172 / 0	-95.2	-95.2	0.31 (1)	6.25
C- D	-98 / 0	-95.2	-95.2	0.41 (1)	6.25
D- L	-134 / 0	-95.2	-95.2	0.31 (1)	6.25
L- E	-43 / 0	-95.2	-95.2	0.10 (1)	6.25
E- F	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- I	0 / 148	-18.5	-18.5	0.25 (1)	10.00
I- H	0 / 148	-18.5	-18.5	0.25 (1)	10.00
H- G	0 / 133	-18.5	-18.5	0.13 (1)	10.00
G- K	0 / 114	-18.5	-18.5	0.25 (1)	10.00
K- E	0 / 114	-18.5	-18.5	0.25 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
H- C	-354 / 0	0.06 (1)
C- G	-40 / 0	0.02 (1)
G- D	-389 / 0	0.07 (1)
I- J	-421 / 0	0.00 (1)
K- L	-423 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.41/1.00 (C-D:1) , BC=0.25/1.00 (B-I:1) , WB=0.07/1.00 (D-G:1) , SSI=0.33/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB52		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085418

Alpa Roof Truss, Maple

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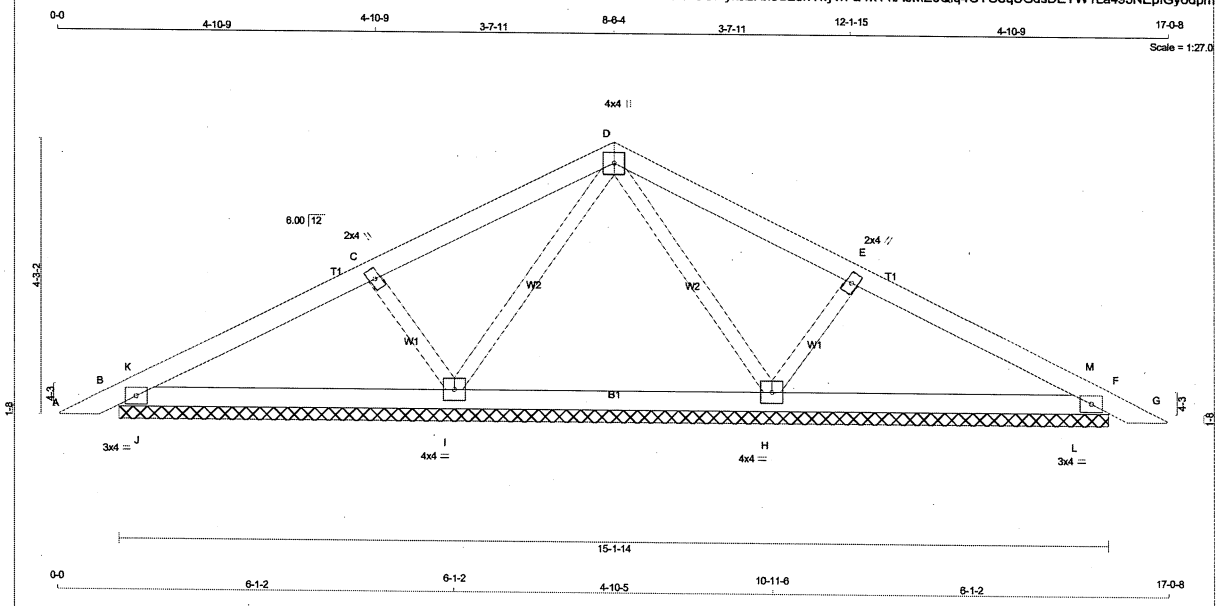

TOTAL WEIGHT = 4 X 49 = 197 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TTWW+p	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMWW1-t	MT20	4.0	4.0	
I	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	337	0	337	0	0	15-1-14	1-8
F	337	0	337	0	0	15-1-14	1-8
H	605	0	605	0	0	15-1-14	1-8
I	605	0	605	0	0	15-1-14	1-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	236	166 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0
F	236	166 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0
H	427	284 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0
I	427	284 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- K	-196 / 0	-95.2	-95.2	0.06 (4)	6.25
K- C	-141 / 0	-95.2	-95.2	0.22 (1)	6.25
C- D	0 / 56	-95.2	-95.2	0.23 (1)	10.00
D- E	0 / 56	-95.2	-95.2	0.23 (1)	10.00
E- M	-141 / 0	-95.2	-95.2	0.22 (1)	6.25
M- F	-196 / 0	-95.2	-95.2	0.06 (4)	6.25
F- G	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- J	0 / 147	-18.5	-18.5	0.08 (1)	10.00
J- I	0 / 147	-18.5	-18.5	0.11 (4)	10.00
I- H	0 / 38	-18.5	-18.5	0.10 (4)	10.00
H- L	0 / 147	-18.5	-18.5	0.11 (4)	10.00
L- F	0 / 147	-18.5	-18.5	0.08 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- H	-197 / 0	0.07 (1)
H- E	-394 / 0	0.06 (1)
I- D	-197 / 0	0.07 (1)
C- I	-394 / 0	0.06 (1)
J- K	-69 / 54	0.00 (1)
L- M	-69 / 54	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (D-E:1) , BC=0.11/1.00 (H-L:4) , WB=0.07/1.00 (D-H:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (F) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB53		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085419

Alpa Roof Truss, Maple

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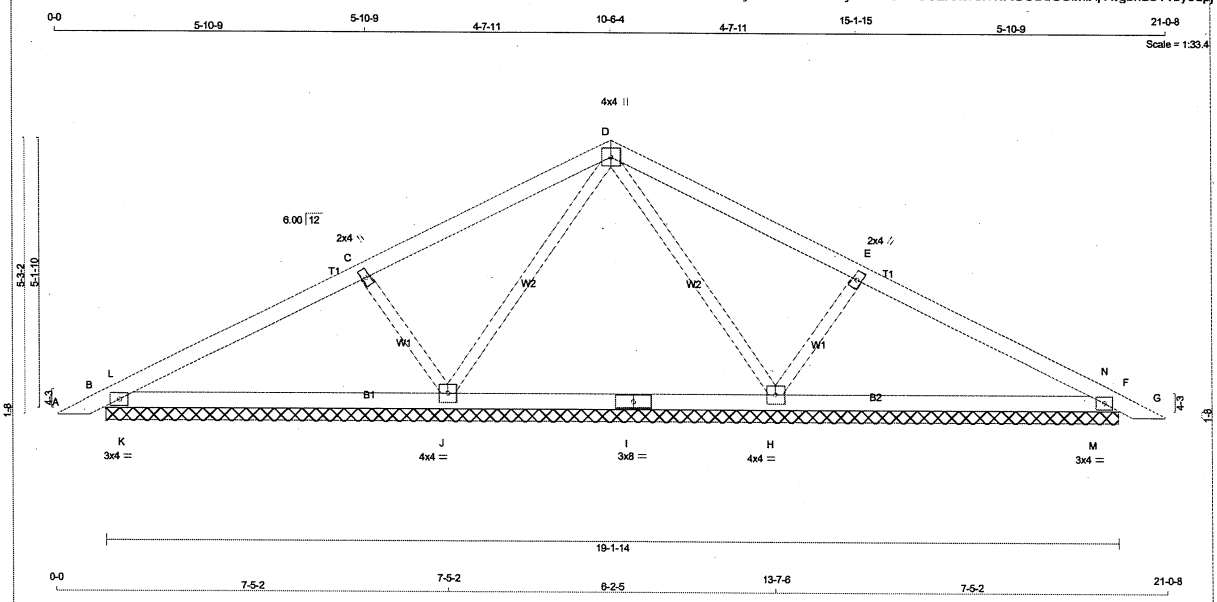


TOTAL WEIGHT = 2 X 62 = 124 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TTWW+p	MT20	4.0	4.0	
E	TMW+w	MT20	2.0	4.0	
F	TMB1-I	MT20	3.0	4.0	
H	BMWW1-t	MT20	4.0	4.0	
I	BS-t	MT20	3.0	8.0	
J	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	408	0	408	0	0	19-1-14 (9-11-8)	
H	762	0	762	0	0	19-1-14 (9-11-8)	
J	762	0	762	0	0	19-1-14 (9-11-8)	
F	408	0	408	0	0	19-1-14 (9-11-8)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	286	199 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
H	538	358 / 0	0 / 0	0 / 0	0 / 0	180 / 0	0 / 0
J	538	358 / 0	0 / 0	0 / 0	0 / 0	180 / 0	0 / 0
F	286	199 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- L	-252 / 0	-95.2	-95.2	0.08 (4)	6.25
L- C	-192 / 0	-95.2	-95.2	0.36 (1)	6.25
C- D	0 / 61	-95.2	-95.2	0.36 (1)	10.00
D- E	0 / 61	-95.2	-95.2	0.36 (1)	10.00
E- N	-192 / 0	-95.2	-95.2	0.36 (1)	6.25
N- F	-252 / 0	-95.2	-95.2	0.08 (4)	6.25
F- G	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- K	0 / 196	-18.5	-18.5	0.11 (1)	10.00
K- J	0 / 196	-18.5	-18.5	0.17 (4)	10.00
J- I	0 / 57	-18.5	-18.5	0.16 (4)	10.00
I- H	0 / 57	-18.5	-18.5	0.16 (4)	10.00
H- M	0 / 196	-18.5	-18.5	0.17 (4)	10.00
M- F	0 / 196	-18.5	-18.5	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- H	-246 / 0	0.14 (1)
H- E	-499 / 0	0.10 (1)
J- D	-246 / 0	0.14 (1)
C- J	-499 / 0	0.10 (1)
K- L	-104 / 81	0.00 (1)
M- N	-104 / 81	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.36/1.00 (C-D:1) , BC=0.17/1.00 (J-K:4) , WB=0.14/1.00 (D-J:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

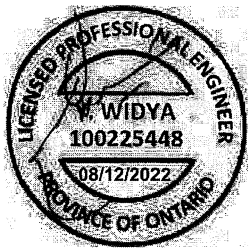
PLATE	GRIP(DRY) MAX	MIN	SHEAR (PSI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20.	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (J) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB54		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085420

Alpa Roof Truss, Maple

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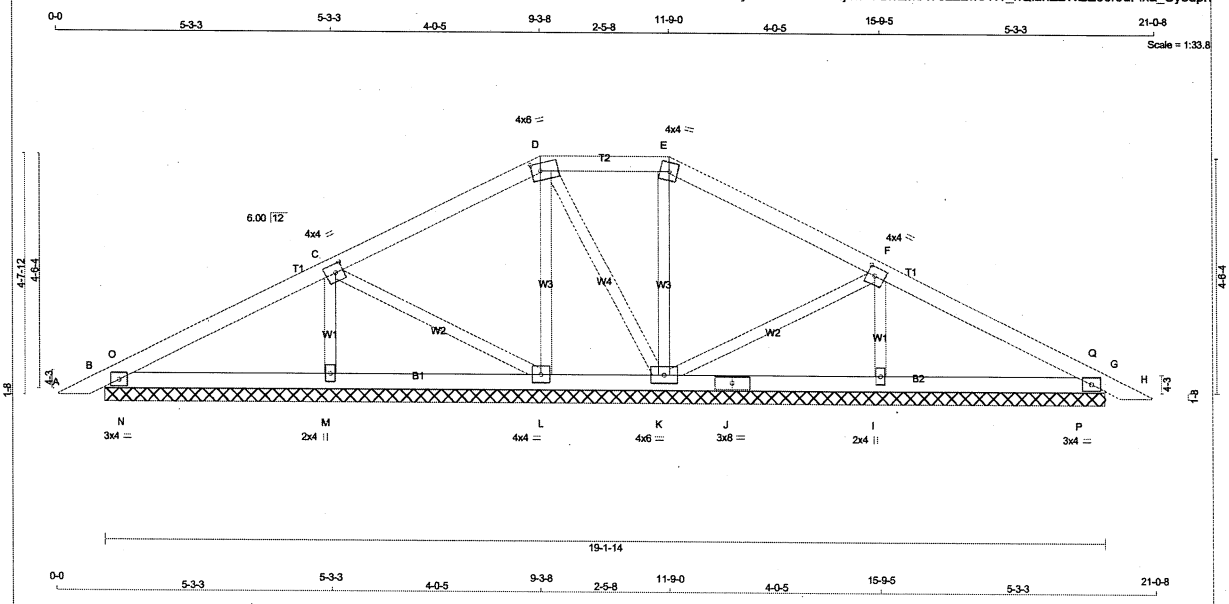

0-0 5-3-3 5-3-3 4-0-5 9-3-8 2-5-8 11-9-0 4-0-5 15-9-5 5-3-3 21-0-8

TOTAL WEIGHT = 69 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
B - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-l	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	4.0	6.0	1.75	2.00
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMB1-l	MT20	3.0	4.0		
I	BMW1+w	MT20	2.0	4.0		
J	BS-t	MT20	3.0	8.0		
K	BMWWW1-t	MT20	4.0	6.0		
L	BMWW1-t	MT20	4.0	4.0		
M	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	317	0	317	0	0	19-1-14 (7-2-2)	3/8
M	518	0	518	0	0	19-1-14 (7-2-2)	3/8
L	251	0	251	0	0	19-1-14 (7-2-2)	3/8
K	452	0	452	0	0	19-1-14 (7-2-2)	3/8
I	494	0	494	0	0	19-1-14 (7-2-2)	3/8
G	307	0	307	0	0	19-1-14 (7-2-2)	3/8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	222	159 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
M	366	241 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
L	179	110 / 0	0 / 0	0 / 0	0 / 0	69 / 0	0 / 0
K	317	224 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
I	349	227 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0
G	215	153 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, L, K, I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- O	-96 / 0	-95.2	-95.2	0.02 (4)	6.25
O- C	-84 / 0	-95.2	-95.2	0.26 (1)	6.25
C- D	-48 / 0	-95.2	-95.2	0.26 (1)	6.25
D- E	0 / 27	-95.2	-95.2	0.10 (1)	10.00
E- F	-2 / 1	-95.2	-95.2	0.26 (1)	10.00
F- Q	-62 / 0	-95.2	-95.2	0.26 (1)	6.25
Q- G	-74 / 0	-95.2	-95.2	0.02 (4)	6.25
G- H	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- N	0 / 93	-18.5	-18.5	0.12 (1)	10.00
N- M	0 / 93	-18.5	-18.5	0.12 (1)	10.00
M- L	0 / 93	-18.5	-18.5	0.09 (1)	10.00
L- K	0 / 20	-18.5	-18.5	0.04 (4)	10.00
K- J	0 / 74	-18.5	-18.5	0.09 (4)	10.00
J- I	0 / 74	-18.5	-18.5	0.09 (4)	10.00
I- P	0 / 74	-18.5	-18.5	0.11 (1)	10.00
P- G	0 / 74	-18.5	-18.5	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-399 / 0	0.06 (1)
C- L	-78 / 0	0.03 (1)
L- D	-167 / 0	0.05 (1)
D- K	-95 / 0	0.04 (1)
K- E	-272 / 0	0.08 (1)
K- F	-106 / 0	0.04 (1)
I- F	-374 / 0	0.06 (1)
N- O	-175 / 7	0.00 (1)
P- Q	-175 / 7	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.26/1.00 (F-Q:1) , BC=0.12/1.00 (M-N:1) , WB=0.08/1.00 (E-K:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB55		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085421

Alpa Roof Truss, Maple

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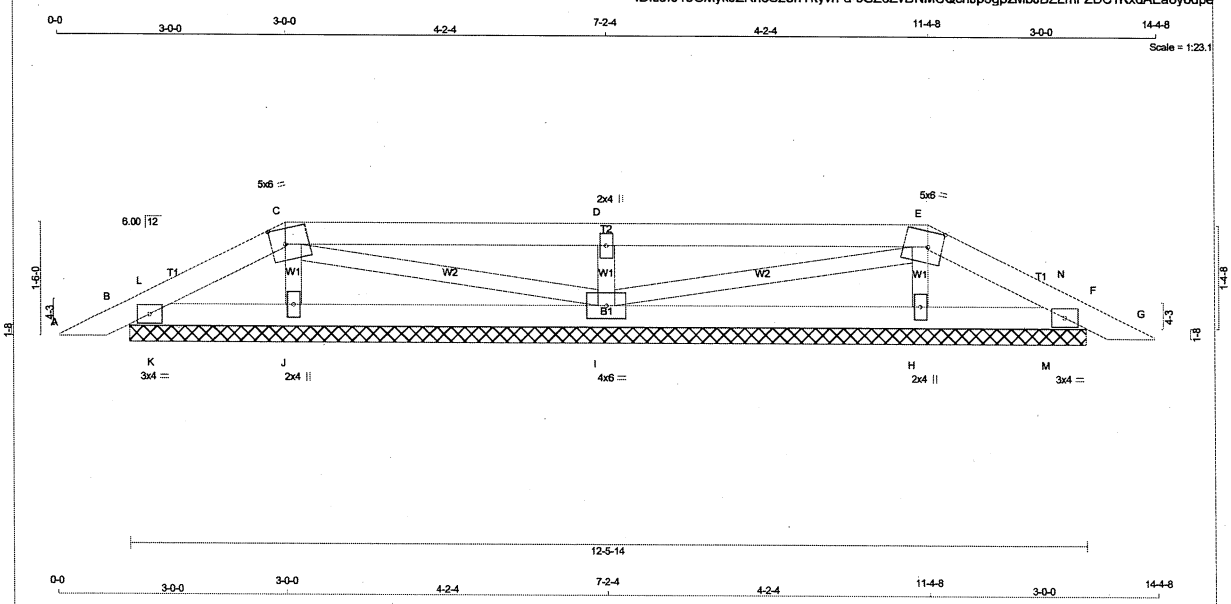

TOTAL WEIGHT = 40 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.50	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.50	2.25
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	205	0	205	0	0	12-5-14	1-8
F	205	0	205	0	0	12-5-14	1-8
J	296	0	296	0	0	12-5-14	1-8
I	579	0	579	0	0	12-5-14	1-8
H	296	0	296	0	0	12-5-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	142	109 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
F	142	109 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0
J	211	130 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
I	407	280 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
H	211	130 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- L	-39 / 0	-95.2	-95.2	0.00 (1)	6.25
L- C	-39 / 0	-95.2	-95.2	0.04 (1)	6.25
C- D	-1 / 0	-95.2	-95.2	0.28 (1)	10.00
D- E	-1 / 0	-95.2	-95.2	0.28 (1)	10.00
E- N	-39 / 0	-95.2	-95.2	0.04 (1)	6.25
N- F	-39 / 0	-95.2	-95.2	0.00 (1)	6.25
F- G	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- K	0 / 35	-18.5	-18.5	0.05 (1)	10.00
K- J	0 / 35	-18.5	-18.5	0.05 (1)	10.00
J- I	0 / 15	-18.5	-18.5	0.07 (4)	10.00
I- H	0 / 15	-18.5	-18.5	0.07 (4)	10.00
H- M	0 / 35	-18.5	-18.5	0.05 (1)	10.00
M- F	0 / 35	-18.5	-18.5	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-208 / 0	0.03 (1)
C- I	-14 / 0	0.00 (1)
I- D	-498 / 0	0.07 (1)
I- E	-14 / 0	0.00 (1)
H- E	-208 / 0	0.03 (1)
K- L	-97 / 0	0.00 (1)
M- N	-97 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (C-D:1) , BC=0.07/1.00 (I-J:4) , WB=0.07/1.00 (D-I:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB56		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085422

Alpa Roof Truss, Maple

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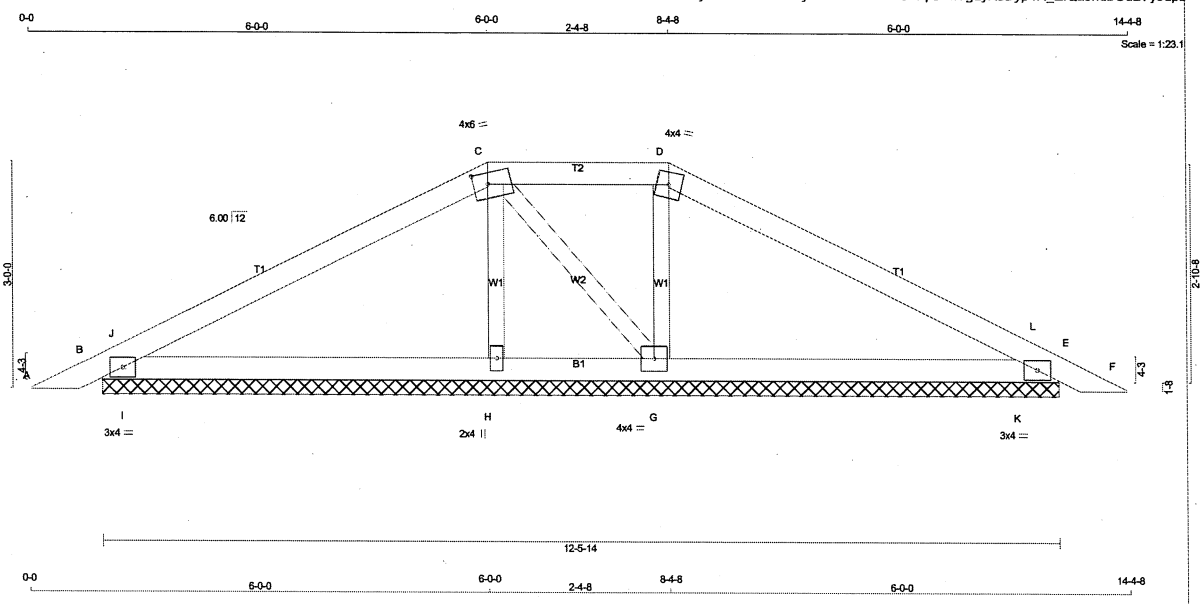

TOTAL WEIGHT = 38 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	428	0	428	0	0	12-5-14	1-8
E	408	0	408	0	0	12-5-14	1-8
H	314	0	314	0	0	12-5-14	1-8
G	430	0	430	0	0	12-5-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	300	214 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0
E	286	202 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
H	224	138 / 0	0 / 0	0 / 0	0 / 0	85 / 0	0 / 0
G	303	204 / 0	0 / 0	0 / 0	0 / 0	99 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- J	-81 / 0	-95.2	-95.2	0.10 (1)	6.25
J- C	-177 / 0	-95.2	-95.2	0.30 (1)	6.25
C- D	-101 / 0	-95.2	-95.2	0.09 (1)	6.25
D- L	-132 / 0	-95.2	-95.2	0.30 (1)	6.25
L- E	-49 / 0	-95.2	-95.2	0.10 (1)	6.25
E- F	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- I	0 / 151	-18.5	-18.5	0.26 (1)	10.00
I- H	0 / 151	-18.5	-18.5	0.26 (1)	10.00
H- G	0 / 143	-18.5	-18.5	0.15 (1)	10.00
G- K	0 / 112	-18.5	-18.5	0.25 (1)	10.00
K- E	0 / 112	-18.5	-18.5	0.25 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-195 / 0	0.03 (1)
G- G	-64 / 0	0.01 (1)
G- D	-262 / 0	0.05 (1)
I- J	-428 / 0	0.00 (1)
K- L	-431 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-J:1) , BC=0.26/1.00 (B-I:1) , WB=0.05/1.00 (D-G:1) , SSI=0.33/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB57		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085423

Alpa Roof Truss, Maple

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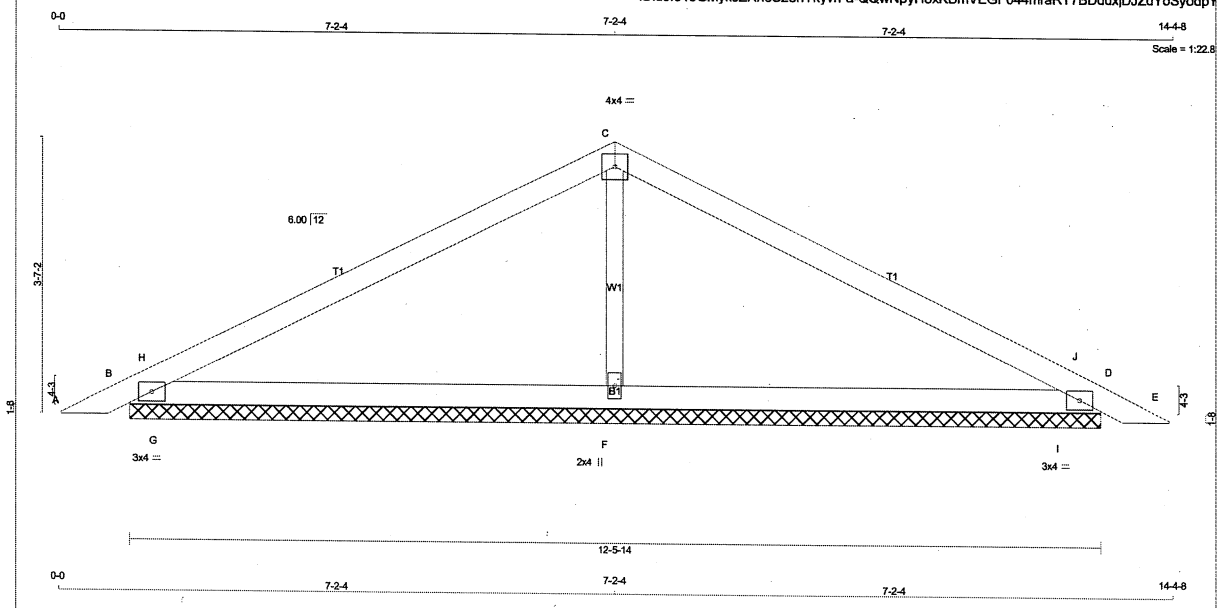

TOTAL WEIGHT = 4 X 35 = 139 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0		
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	505	0	505	0	0	12-5-14	1-8
D	505	0	505	0	0	12-5-14	1-8
F	570	0	570	0	0	12-5-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	354	253 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
D	354	253 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
F	406	252 / 0	0 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)
A- B	0 / 17	-95.2	-95.2	0.05 (1)
B- H	-58 / 0	-95.2	-95.2	0.22 (1)
H- C	-249 / 0	-95.2	-95.2	0.46 (1)
C- J	-249 / 0	-95.2	-95.2	0.46 (1)
J- D	-58 / 0	-95.2	-95.2	0.22 (1)
D- E	0 / 17	-95.2	-95.2	0.05 (1)
B- G	0 / 212	-18.5	-18.5	0.41 (1)
G- F	0 / 212	-18.5	-18.5	0.41 (1)
F- I	0 / 212	-18.5	-18.5	0.41 (1)
I- D	0 / 212	-18.5	-18.5	0.41 (1)

WEBS

MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
10.00	F- C	-288 / 0	0.06 (1)
6.25	G- H	-707 / 0	0.00 (1)
6.25	I- J	-707 / 0	0.00 (1)
6.25			
6.25			
10.00			
10.00			
10.00			
10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46/1.00 (C-H:1) , BC=0.41/1.00 (F-G:1) , WB=0.06/1.00 (C-F:1) , SSI=0.53/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (D) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
W. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.		DRWG NO.
428162	PB58		1	TRUSS DESC.	GREEN PARK HOMES JT 53080	E22085424

Alpa Roof Truss, Maple

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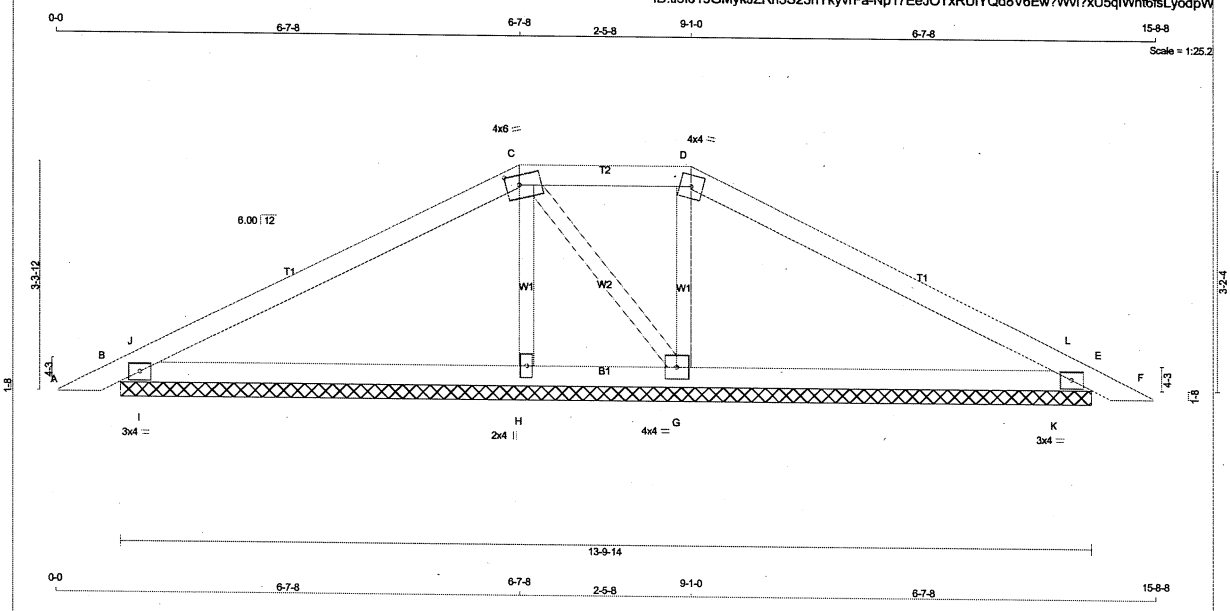

TOTAL WEIGHT = 42 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	6.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	477	0	477	0	0	13-9-14	1-8
E	456	0	456	0	0	13-9-14	1-8
H	333	0	333	0	0	13-9-14	1-8
G	466	0	466	0	0	13-9-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	334	237 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
E	320	225 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
H	237	146 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
G	329	221 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- J	-80 / 0	-95.2	-95.2	0.14 (1)	6.25
J- C	-214 / 0	-95.2	-95.2	0.39 (1)	6.25
C- D	-130 / 0	-95.2	-95.2	0.10 (1)	6.25
D- L	-166 / 0	-95.2	-95.2	0.39 (1)	6.25
L- E	-58 / 0	-95.2	-95.2	0.14 (1)	6.25
E- F	0 / 17	-95.2	-95.2	0.05 (1)	10.00
B- I	0 / 183	-18.5	-18.5	0.32 (1)	10.00
I- H	0 / 183	-18.5	-18.5	0.32 (1)	10.00
H- G	0 / 176	-18.5	-18.5	0.19 (1)	10.00
G- K	0 / 140	-18.5	-18.5	0.31 (1)	10.00
K- E	0 / 140	-18.5	-18.5	0.31 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-201 / 0	0.04 (1)
C- G	-72 / 0	0.02 (1)
G- D	-278 / 0	0.05 (1)
I- J	-534 / 0	0.00 (1)
K- L	-537 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (C-J:1) , BC=0.32/1.00 (B-I:1) , WB=0.05/1.00 (D-G:1) , SSI=0.41/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
W. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	PB59		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085425

Alpa Roof Truss, Maple

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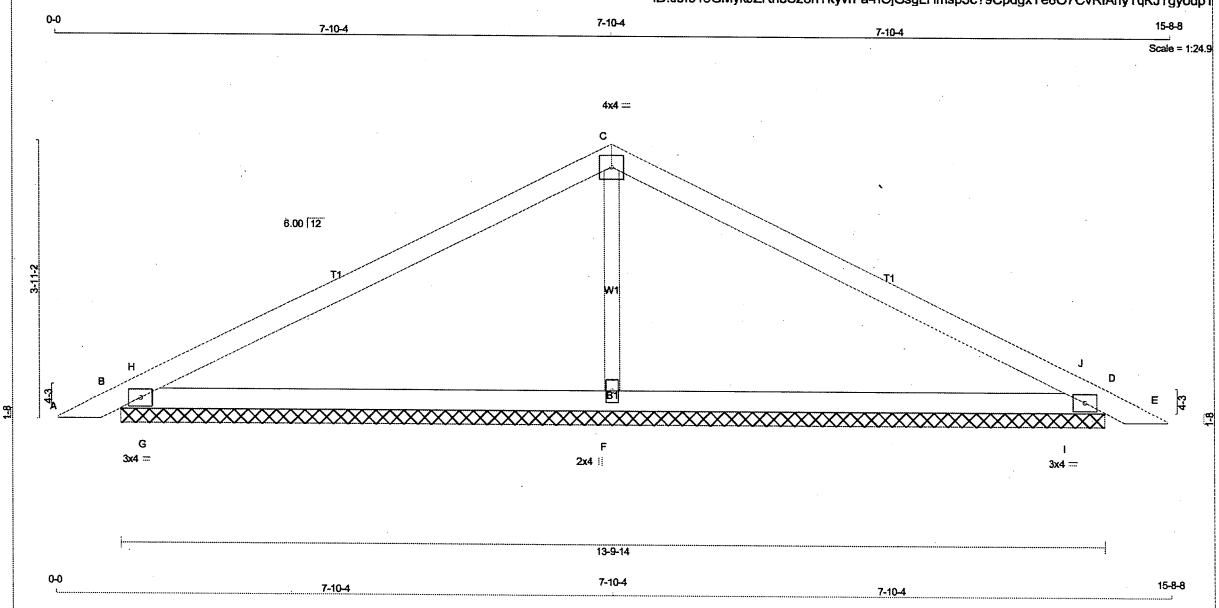

TOTAL WEIGHT = 2 X 38 = 77 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	560	0	560	0	0	13-9-14	1-8
D	560	0	560	0	0	13-9-14	1-8
F	613	0	613	0	0	13-9-14	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	392	280 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
D	392	280 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	436	270 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-95.2	-95.2	0.05	(1)	10.00
B- H	-67 / 0	-95.2	-95.2	0.29	(1)	6.25
H- C	-297 / 0	-95.2	-95.2	0.57	(1)	6.25
C- J	-297 / 0	-95.2	-95.2	0.57	(1)	6.25
J- D	-67 / 0	-95.2	-95.2	0.29	(1)	6.25
D- E	0 / 17	-95.2	-95.2	0.05	(1)	10.00
B- G	0 / 252	-18.5	-18.5	0.50	(1)	10.00
G- F	0 / 252	-18.5	-18.5	0.50	(1)	10.00
F- I	0 / 252	-18.5	-18.5	0.50	(1)	10.00
I- D	0 / 252	-18.5	-18.5	0.50	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
F- C	-302 / 0	0.07	(1)
G- H	-854 / 0	0.00	(1)
I- J	-854 / 0	0.00	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.57/1.00 (C-H:1) , BC=0.50/1.00 (B-G:1) , WB=0.07/1.00 (C-F:1) , SSI=0.65/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
W. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	J1		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085411

Alpa Roof Truss, Maple

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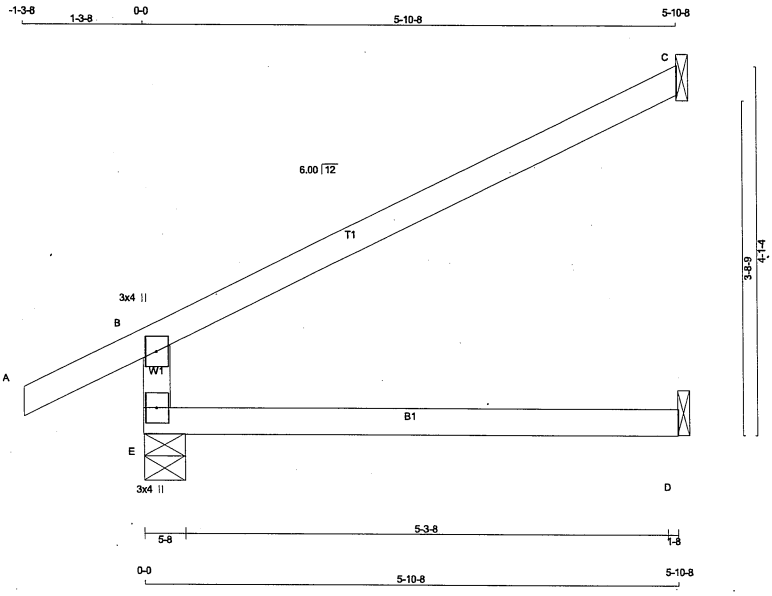

TOTAL WEIGHT = 28 X 17 = 470 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	542	0	542	0	0	5-8	1-8
C	210	0	210	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	380	269 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	144	118 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
E- B	-479 / 0	0.0	0.0	0.13 (4)	7.81
A- B	0 / 29	-95.2	-95.2	0.12 (1)	10.00
B- C	-31 / 0	-95.2	-95.2	0.56 (1)	6.25
E- D	0 / 0	-18.5	-18.5	0.13 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	J5		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085412

Alpa Roof Truss, Maple

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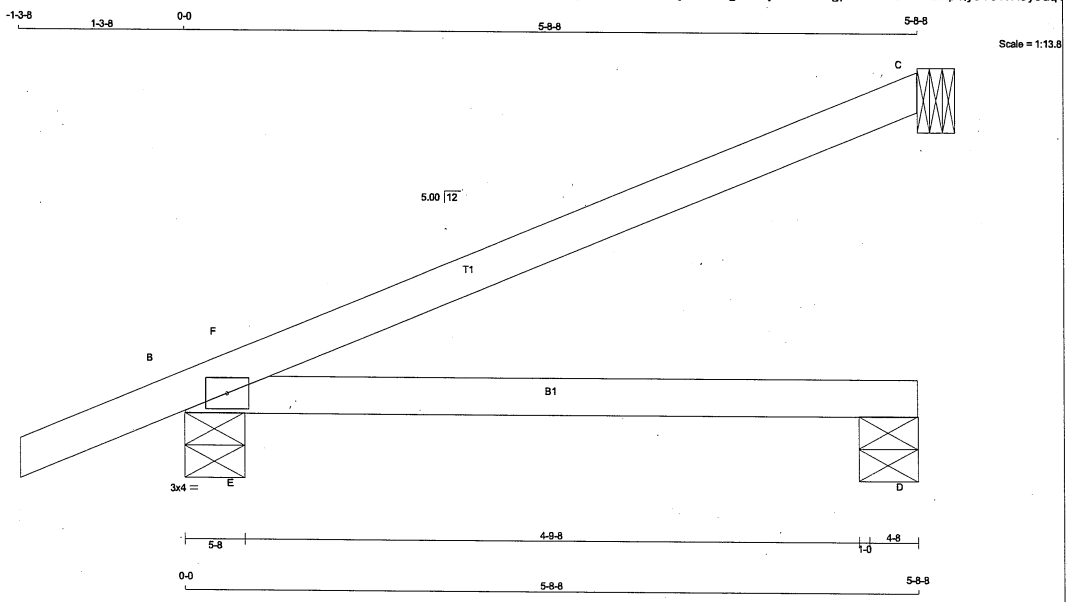

TOTAL WEIGHT = 24 X 15 = 367 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	238	0	238	0	0	4-8	1-8
B	453	0	453	0	0	5-8	1-8
D	87	0	87	0	0	5-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	164	129 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
B	317	225 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
D	65	23 / 0	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-95.2	-95.2	0.12	(1)	10.00
B- F	-24 / 51	-95.2	-95.2	0.09	(1)	6.25
F- C	-4 / 2	-95.2	-95.2	0.40	(1)	10.00
B- E	0 / 0	-18.5	-18.5	0.27	(1)	10.00
E- D	0 / 0	-18.5	-18.5	0.28	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-399 / 12	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 523 (0.13")

CSI: TC=0.40/1.00 (C-F:1) , BC=0.28/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.33/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
Y. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
428162	J7		1	GREEN PARK HOMES TRUSS DESC. JT 53080	E22085413

Alpa Roof Truss, Maple

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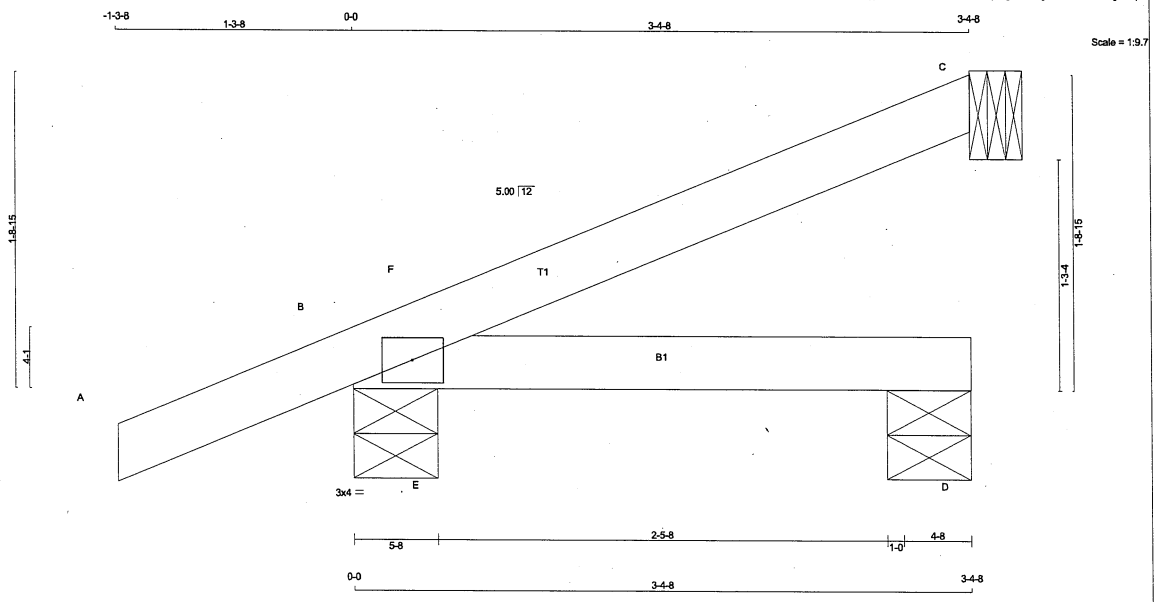

TOTAL WEIGHT = 12 X 10 = 117 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	138	0	138	0	0	4-8	1-8
B	320	0	320	0	0	5-8	1-8
D	54	0	54	0	0	5-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	96	75 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
B	224	162 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
D	40	15 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 23	-95.2	-95.2	0.13 (5)	10.00
B- F	-12 / 0	-95.2	-95.2	0.03 (4)	6.25
F- C	0 / 2	-95.2	-95.2	0.14 (1)	10.00
B- E	0 / 0	-18.5	-18.5	0.11 (1)	10.00
E- D	0 / 0	-18.5	-18.5	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-155 / 5	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	26.7	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	40.1	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 33.4 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.14/1.00 (C-F:1) , BC=0.11/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.13/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LATERAL BRACE(S) SHOWN SHALL BE 2X4 SPF#2

LICENSED PROFESSIONAL ENGINEER
V. WIDYA
100225448
08/12/2022
PROVINCE OF ONTARIO

EWP DESIGN INC.

(905) 832-2250 FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

THESE DRAWINGS CONSTITUTE THE PROPERTY OF EWP DESIGN INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY THE ALPA LUMBER GROUP, AND WILL BE RETRACTED BY EWP DESIGN INC. IF UTILIZED FOR ANY OTHER PURPOSE.

February 1, 2019

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	**Common wire**	**Common spiral**	**Common wire**	**Common spiral**	**Gun Nail**
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

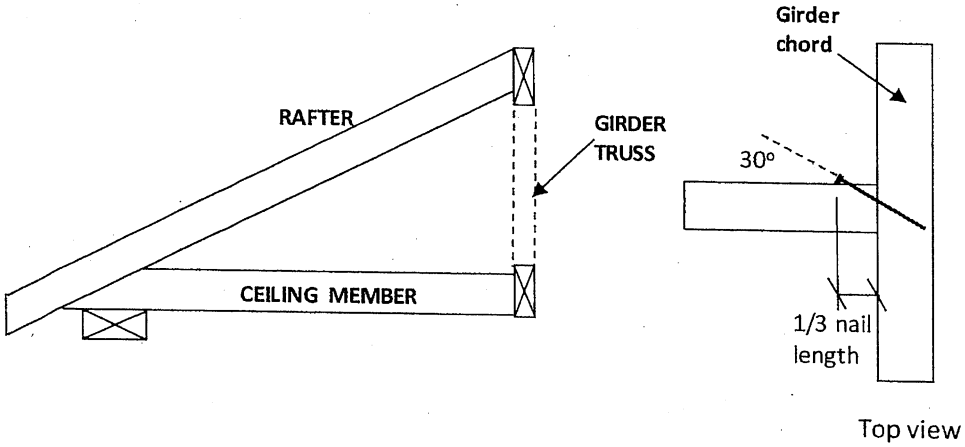

Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

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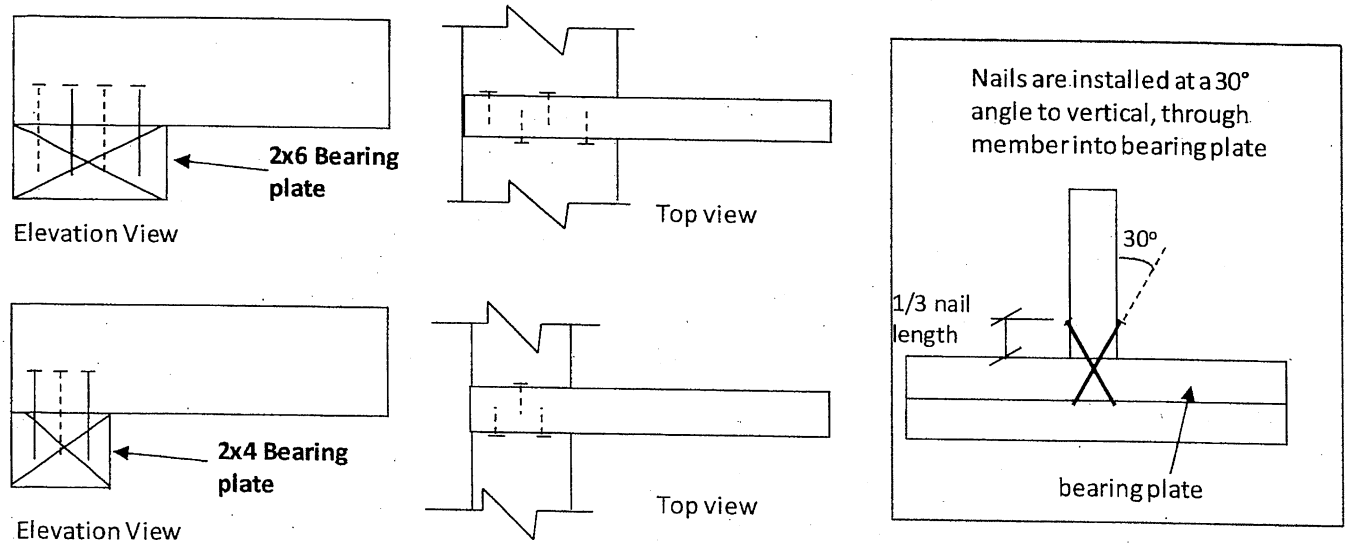
TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).

2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).

3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.

4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).

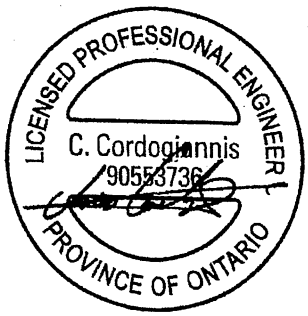
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.

6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.

7. Lumber must be dry (< 19% moisture content) at the time of nail installation.

8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



December 21, 2020

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LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie®

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

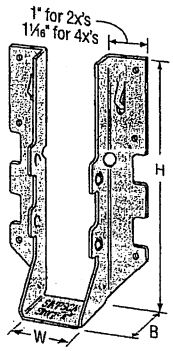
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

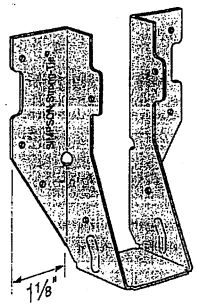
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



LUS28



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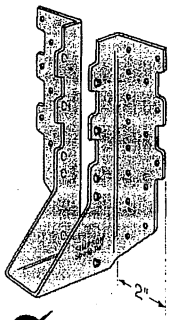
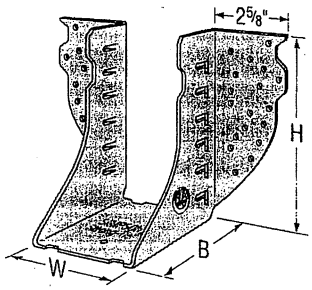
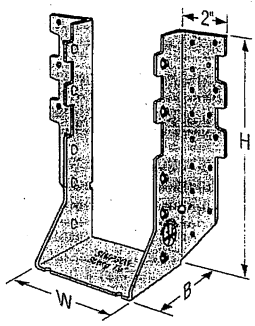


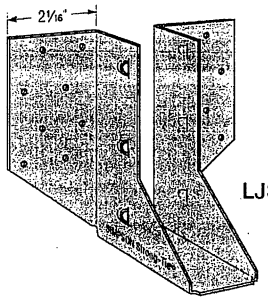
HUS210
(HUS26, HUS28, and HHUS similar)



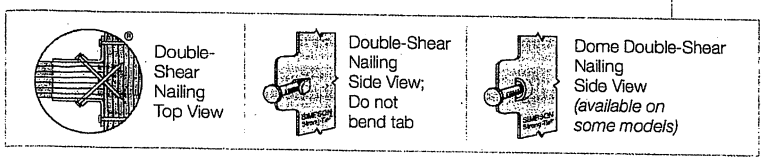
HGUS28-2

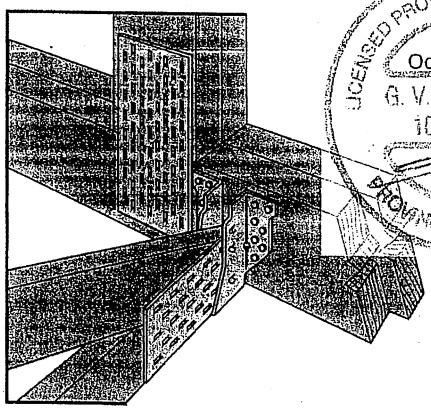


HHUS210-2



LJS26DS





Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)

LICENSED PROFESSIONAL ENGINEER
Oct. 14, 2020
G. V. LUBARSKY
100044463
PROVINCE OF ONTARIO

Plated Truss Connectors

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TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

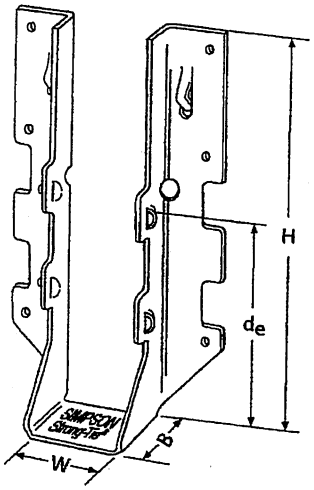
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

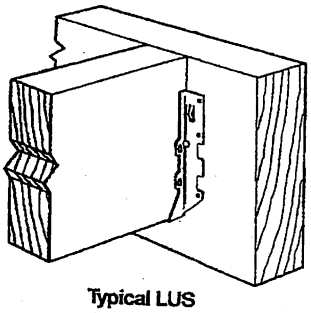
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



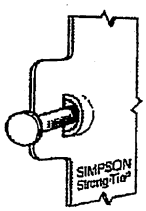
LUS28



Typical LUS Installation

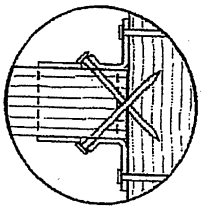
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

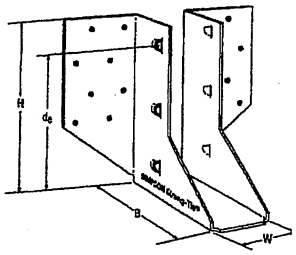
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

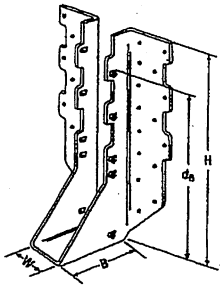
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

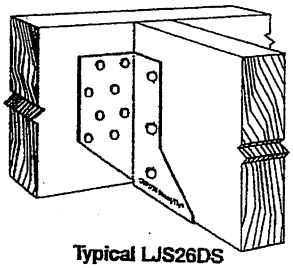
- See current catalogue for options



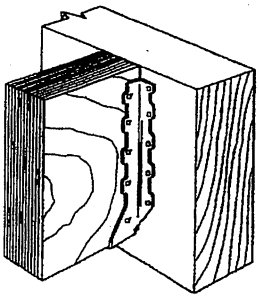
LJS26DS



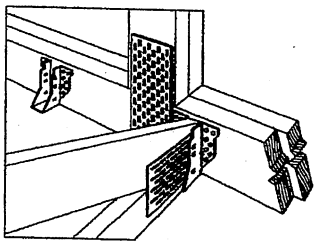
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



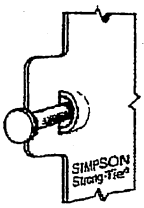
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

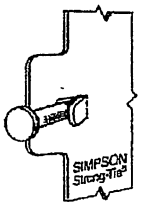
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L.		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

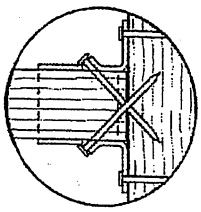


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

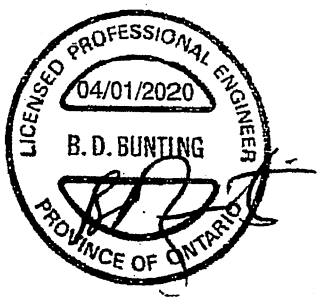
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

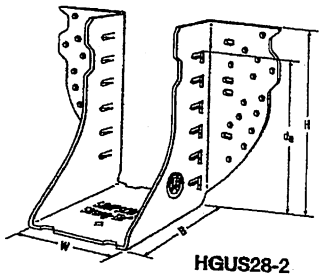
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

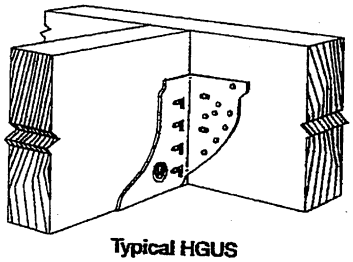
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

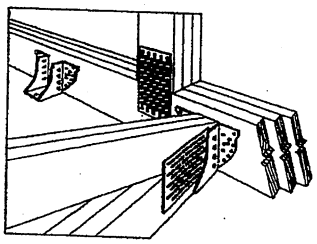
- See current catalogue for options



HGUS28-2



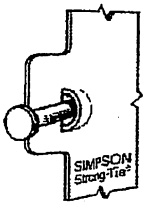
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

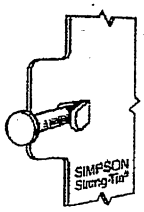
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1⅝	9⅛	5	7⅞	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

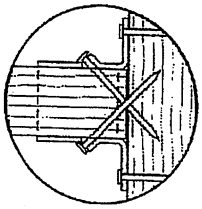


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

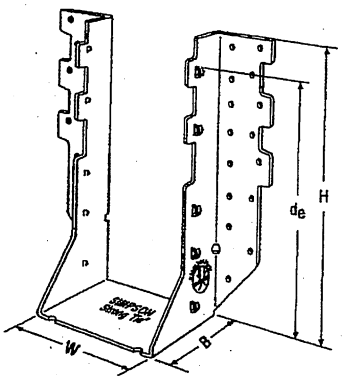
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

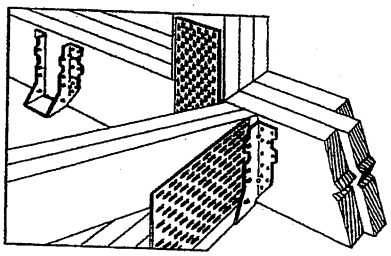
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



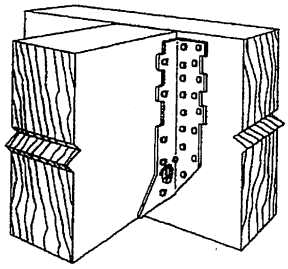
HHUS410



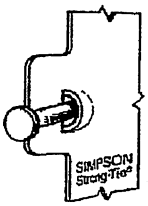
Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4670	10155	3370	7210

1. d_e is the distance from the seat of the hanger to the highest joist nail.

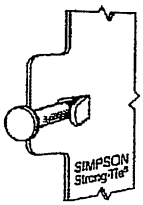


Typical HHUS Installation

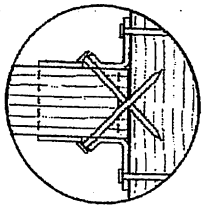


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

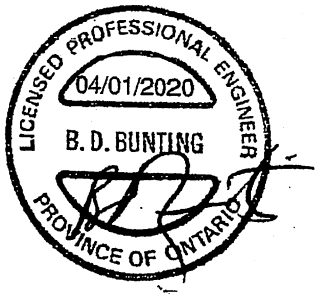
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHHUS20 3/20 exp. 6/22

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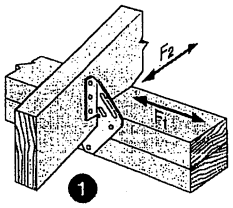
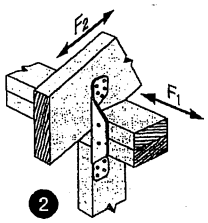
strongtie.com

H/TSP

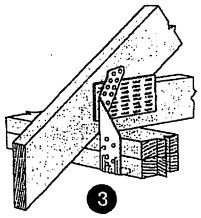
Seismic and Hurricane Ties (cont.)



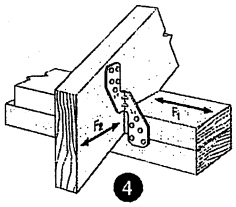
1 H1 Installation



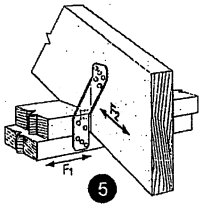
2 H2A Installation



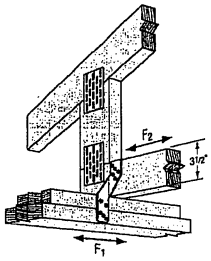
3 TSP Installation



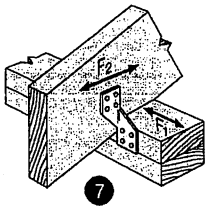
4 H2.5A Installation
(Nails into both top plates)



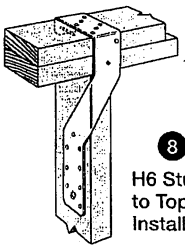
5 H2.5T Installation
(Nails into both top plates)



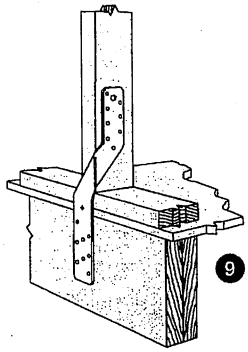
6 H2.5T Installation



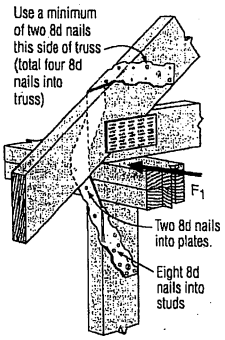
7 H3 Installation
(Nails into upper top plate)



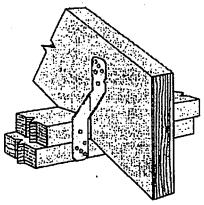
8 H6 Stud to Top Plate Installation



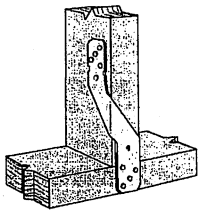
9 H6 Stud to Band Joist Installation



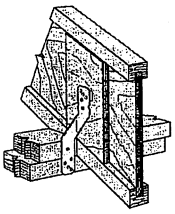
10 H7Z Installation



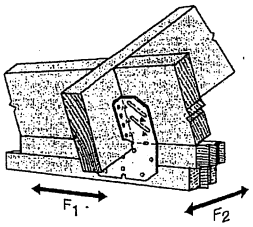
11 H8 Attaching Rafter to Double Top Plates



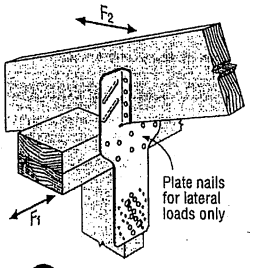
12 H8 attaching Stud to Sill
((4) 8d into plate, (5) 8d into stud)



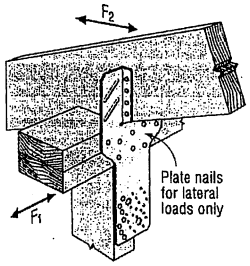
13 H8 attaching I-Joist to Double Top Plates



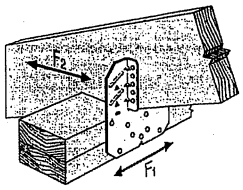
14 H10A Field-Bent Installation



15 H10S Installation



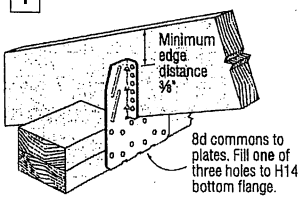
16 H10S Installation with Stud Offset



17 H10A Installation

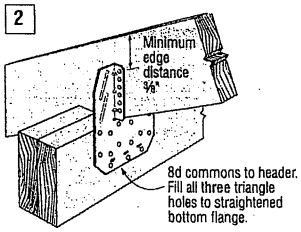
H10A optional positive angle nailing connects shear blocking to rafter. Use 8d common nails. Slot allows maximum field-bending up to a pitch of 6/12, use 75% of the table uplift value; bend one time only.

1



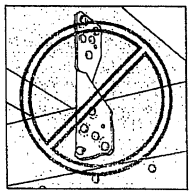
18 H14 Installation to Double Top Plates

2



19 H14 Installation to Double 2x Header

Avoid a Misinstallation



Do not make new holes or overdrive nails.

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H/TSP

Seismic and Hurricane Ties

Simpson Strong-Tie® hurricane ties provide a positive connection between truss/rafter and the wall of the structure to resist wind and seismic forces. New additions to the line provide even more options.

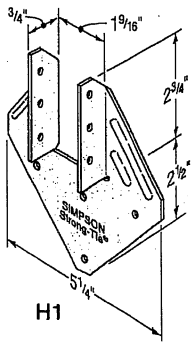
- H10AR — The heavy-duty design of the H10A available with a 2" wide throat to accommodate rough lumber
- H10A-2 — The H10A design with a 3" throat for double 2x members
- H2ASS, H2.5ASS and H10ASS — Popular ties now available in stainless steel

Material: See table

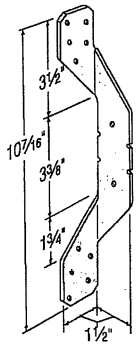
Finish: Galvanized. H7Z and H11Z — ZMAX® coating. Some models available in stainless steel or ZMAX; see Corrosion Information, pp. 20–24 or visit **strongtie.com**.

Installation:

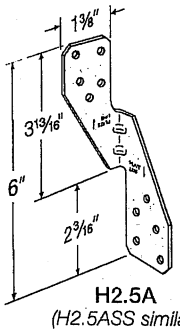
- Use all specified fasteners; see General Notes.
- H1 can be installed with flanges facing inward (reverse of H1 installation drawing; number 1).
- H2.5T, H3 and H6 ties are shipped in equal quantities of right and left versions (right versions shown).
- Hurricane ties do not replace solid blocking.
- When installing ties on plated trusses (on the side opposite the truss plate) do not fasten through the truss plate from behind. This can force the truss plate off of the truss and compromise truss performance.
- H10A optional nailing to connect shear blocking, use 8d nails. Slots allow maximum field bending up to a pitch of 6:12, use H10A sloped loads for field bent installation.



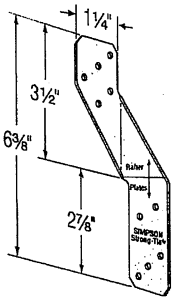
H1



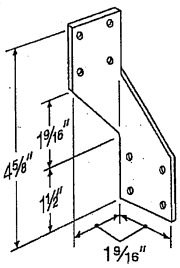
H2A
(H2ASS similar)



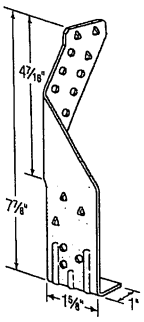
H2.5A
(H2.5ASS similar)



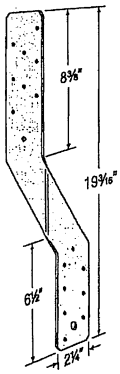
H2.5T



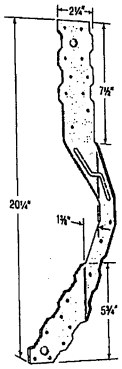
H3



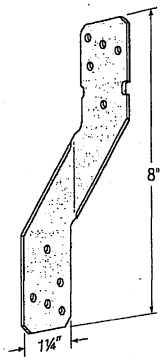
TSP



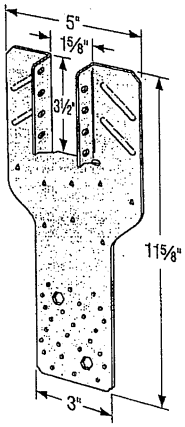
H6



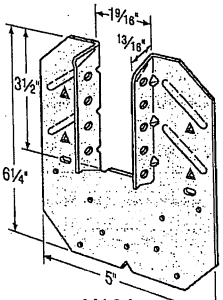
H7Z



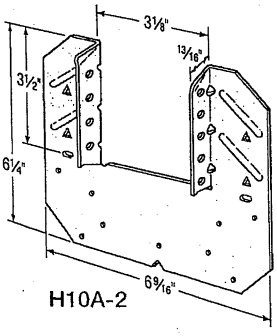
H8



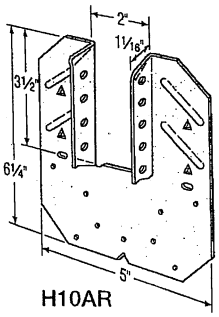
H10S



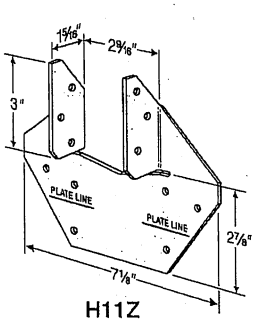
H10A
(H10ASS similar)



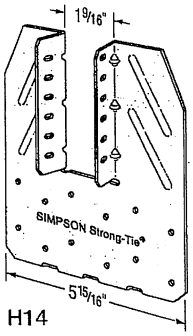
H10A-2



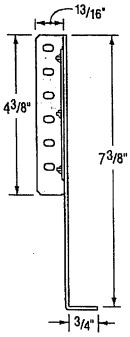
H10AR



H11Z



H14



H14
Profile

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TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON Strong-Tie®

The H connector series provides wind and seismic ties for trusses and rafters.

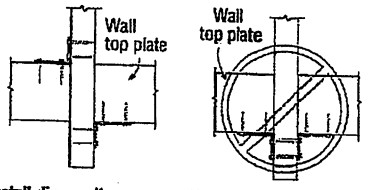
Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14
• Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners
• Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½ long, 10d x 1½" = 0.146" x 1½" long
• H1 can be installed with flanges facing outwards
• Hurricane ties do not replace solid blocking

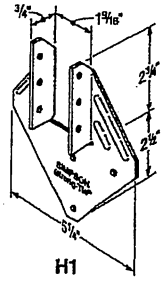
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

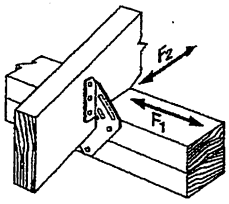


Install diagonally across from each other for minimum 2x truss.

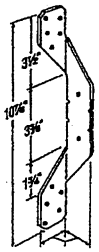
Nailing into both sides of a single ply 2x truss may cause the wood to split.



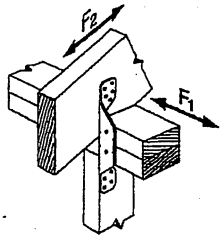
H1



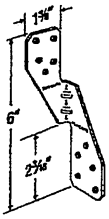
H1 Installation



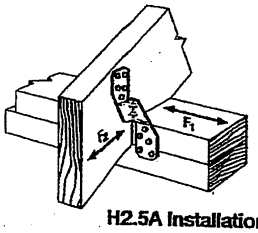
H2A



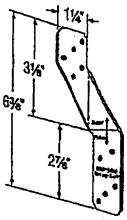
H2A Installation



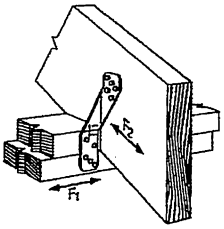
H2.5A



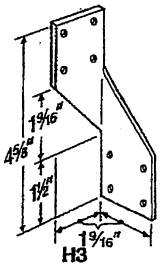
H2.5A Installation



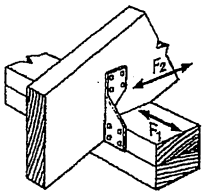
H2.5T



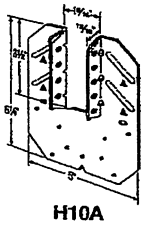
H2.5T Installation (Nails into both top plates)



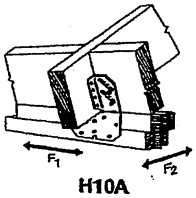
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal F_1	Normal F_2	Uplift	Normal F_1	Normal F_2
					(K_D=1.15)			(K_D=1.15)		
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

TC – Truss Connectors

SIMPSON Strong-Tie®

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

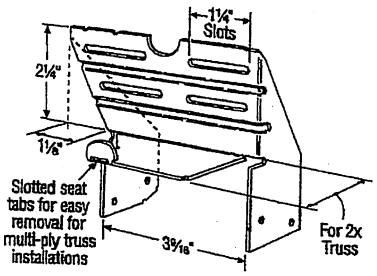
Design: Factored resistances are in accordance with CSA 086-14

Installation:

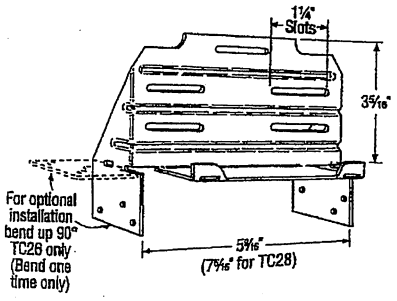
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

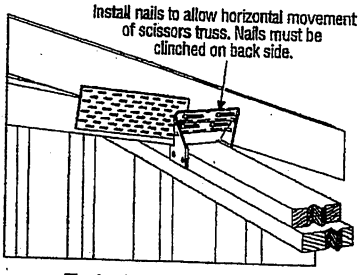
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



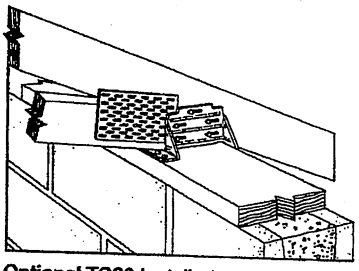
TC24
U.S. Patent 4,932,173



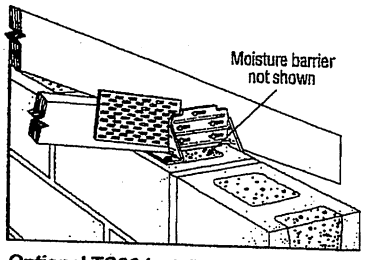
TC26
(TC28 Similiar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⁹⁄₁₆" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
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HTU

SIMPSON Strong-Tie®

Face-Mount Truss Hanger (cont.)

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Alternate Installation for (2) 2x4 and (2) 2x6 Headers

Model No.	Min. Heel Height (in.)	Minimum Header Size	Fasteners: Header	Fasteners: Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
HTU26 (Min.)	3⅞	(2) 2x4	(10) 16d	(14) 10d x 1½"	1740 / 7.74	3340 / 14.86	1235 / 5.49	2370 / 10.54
HTU26 (Max.)	5½	(2) 2x4	(10) 16d	(20) 10d x 1½"	2470 / 10.99	4015 / 17.86	1755 / 7.81	2850 / 12.68
HTU28 (Max.)	3⅞	(2) 2x6	(20) 16d	(26) 10d x 1½"	4150 / 18.46	6395 / 28.45	2945 / 13.10	4540 / 20.19
HTU210 (Max.)	7¼	(2) 2x6	(20) 16d	(32) 10d x 1½"	4150 / 18.46	6395 / 28.45	2945 / 13.10	4540 / 20.19

See table footnotes on p. 260.

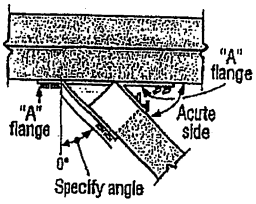
LICENSED PROFESSIONAL ENGINEER 5/29/19 B. D. BUNTING PROVINCE OF ONTARIO

Hanger Options

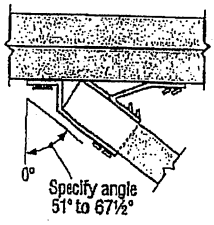
See Hanger Options information on pp. 125–127.

Skewed Seat

- Skewable up to 67½°
- Available in single and 2-ply size
- No bevel cut required



Top View HTU Hanger Skewed Right < 51°



Top View HTU Hanger Skewed Right ≥ 51°

Factored Resistances for Skewed HTU Hangers

Model No.	Skew Angle (Degrees)	Fasteners: Header	Fasteners: Joist	Factored Resistance D.Fir-L Uplift (KD=1.15) lbs / kN	D.Fir-L Normal (KD=1.00) lbs / kN	S-P-F Uplift (KD=1.15) lbs / kN	S-P-F Normal (KD=1.00) lbs / kN
HTU26	< 51	(20) 16d	(14) 10d x 1½"	1835 / 8.16	4110 / 18.28	1300 / 5.78	2905 / 12.92
	51–67½	(20) 16d	(12) 10d x 1½"	1350 / 6.01	3620 / 16.10	955 / 4.25	2560 / 11.39
HTU28	< 51	(26) 16d	(20) 10d x 1½"	2810 / 12.50	4270 / 18.99	1985 / 8.83	3030 / 13.48
	51–67½	(26) 16d	(17) 10d x 1½"	2075 / 9.23	3930 / 17.48	1465 / 6.52	2780 / 12.37
HTU210	< 51	(32) 16d	(26) 10d x 1½"	3785 / 16.84	4430 / 19.71	2675 / 11.90	3135 / 13.95
	51–67½	(32) 16d	(22) 10d x 1½"	2795 / 12.43	4240 / 18.86	1980 / 8.81	3000 / 13.35
HTU26-2	< 51	(20) 16d	(14) 10d	2140 / 9.52	3715 / 16.53	1515 / 6.74	2625 / 11.68
	51–67½	(20) 16d	(12) 10d	1610 / 7.16	3920 / 17.44	1140 / 5.07	2785 / 12.39
HTU28-2	< 51	(26) 16d	(20) 10d	3960 / 17.62	5425 / 24.13	2815 / 12.52	3855 / 17.15
	51–67½	(26) 16d	(17) 10d	2385 / 10.61	5425 / 24.13	1695 / 7.54	3855 / 17.15
HTU210-2	< 51	(32) 16d	(26) 10d	5025 / 22.35	6890 / 30.65	3570 / 15.88	4890 / 21.75
	51–67½	(36) 16d	(22) 10d	3145 / 13.99	6680 / 29.72	2225 / 9.90	4745 / 21.10

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Reduced heel heights are not permitted for skewed HTU's.
3. Nails: 16d = 0.162" dia. x 3½" long, 10d x 1½" = 0.148" dia. x 1½" long, 10d = 0.148" dia. x 3" long. See pp. 27–28 for other nail sizes and information.

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ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

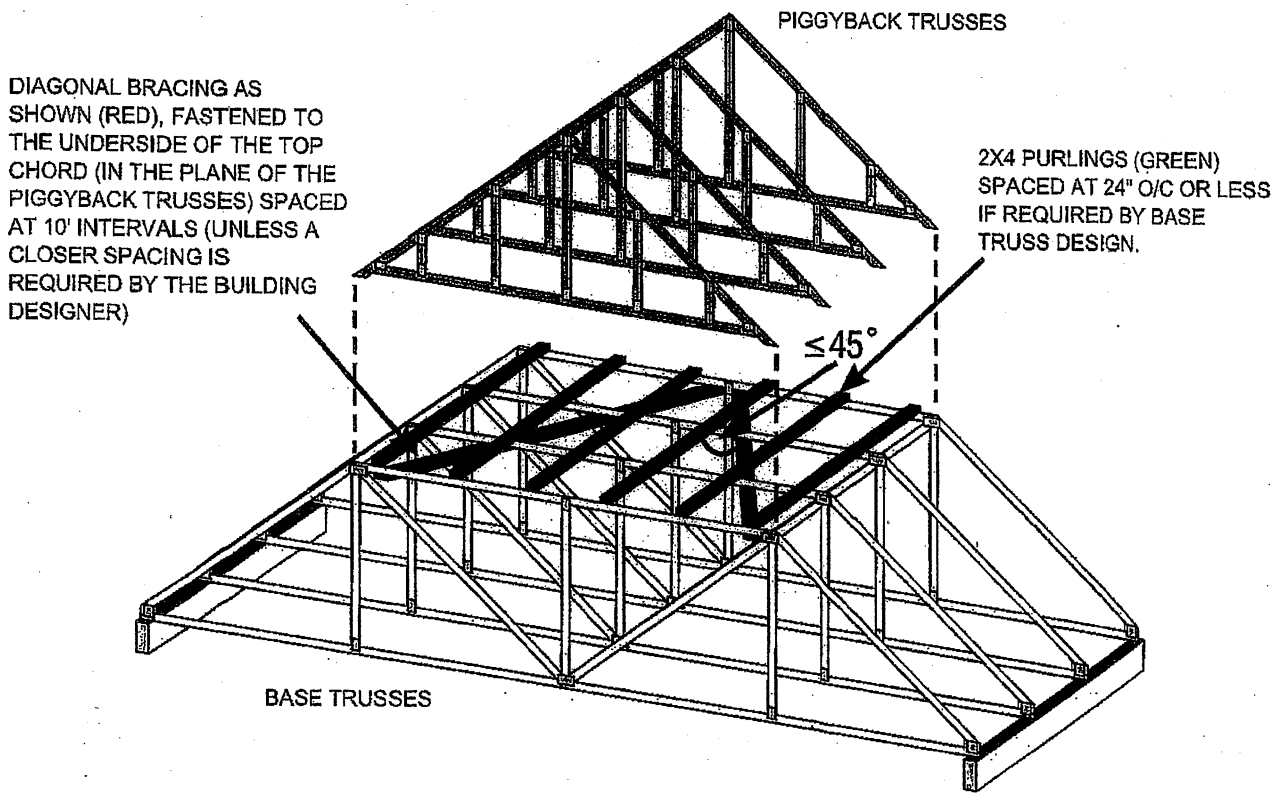
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

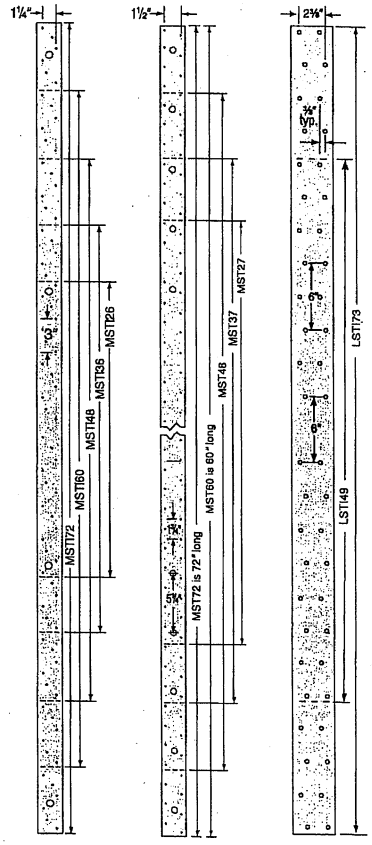
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

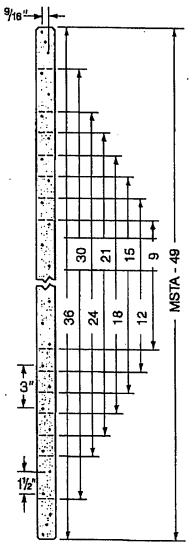
Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

Installation: Use all specified fasteners; see General Notes

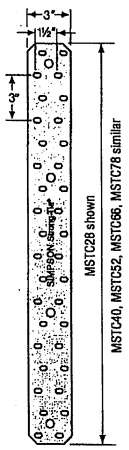
Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths



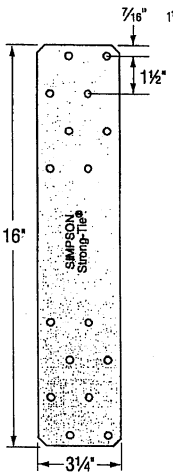
MSTI MST LSTI



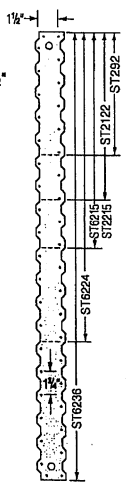
LSTA and MSTA
(Pilot holes not shown)



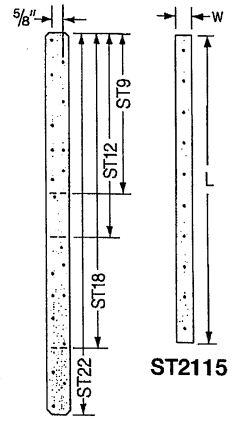
MSTC



HRS416Z

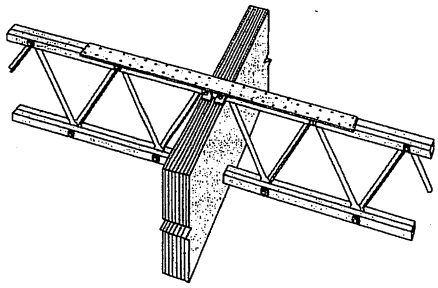


ST

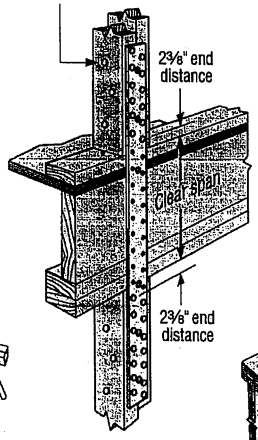


ST9, ST12, ST18, ST22

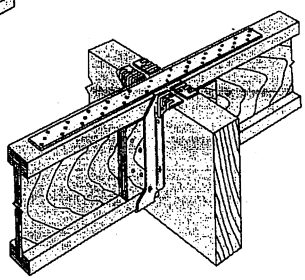
ST2115



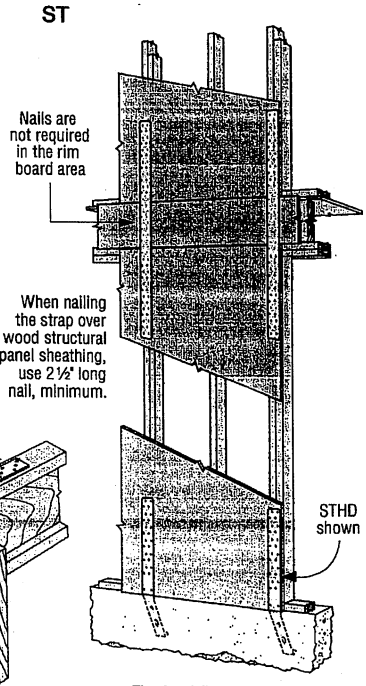
Typical LSTI Installation



Floor-to-Floor Tie Installation Showing a Clear Span



Typical MSTI Installation
(MIT hanger shown)
LSTI similar



Typical Detail with Strap Installed over Wood Structural Panel Sheathing

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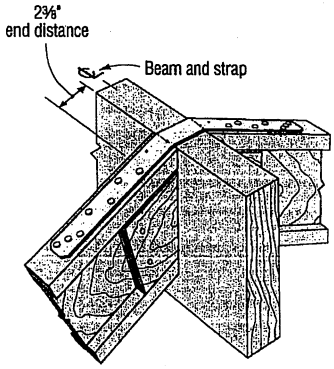
HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties (cont.)

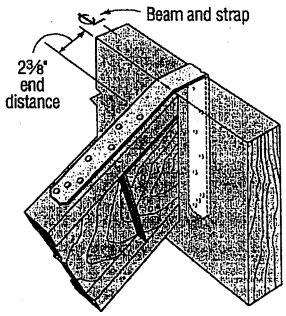
These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.) W	Dimensions (in.) L	Fasteners (Total)	Factored Tensile Resistance D.Fir–L (K_D = 1.00) lb. / kN	D.Fir–L (K_D = 1.15) lb. / kN	S-P-F (K_D = 1.00) lb. / kN	S-P-F (K_D = 1.15) lb. / kN
LSTA9	20	1¼	9	(6) 10d	600 / 2.67	690 / 3.07	555 / 2.47	635 / 2.82
LSTA12	20	1¼	12	(8) 10d	800 / 3.56	920 / 4.09	735 / 3.27	845 / 3.76
LSTA15	20	1¼	15	(10) 10d	1000 / 4.45	1150 / 5.12	920 / 4.09	1060 / 4.72
LSTA18	20	1¼	18	(12) 10d	1200 / 5.34	1380 / 6.14	1105 / 4.92	1270 / 5.65
LSTA21	20	1¼	21	(14) 10d	1400 / 6.23	1610 / 7.16	1290 / 5.74	1485 / 6.61
LSTA24	20	1¼	24	(16) 10d	1600 / 7.12	1840 / 8.19	1475 / 6.56	1695 / 7.54
ST292	20	2 1/16	9 5/16	(8) 8d	585 / 2.60	675 / 3.00	535 / 2.38	615 / 2.74
ST2122	20	2 1/16	12 13/16	(12) 8d	940 / 4.18	1085 / 4.83	865 / 3.85	995 / 4.43
ST2115	20	¾	16 5/16	(8) 8d	670 / 2.98	770 / 3.43	615 / 2.74	710 / 3.16
ST2215	20	2 1/16	16 5/16	(16) 8d	1335 / 5.94	1540 / 6.85	1235 / 5.49	1420 / 6.32
LSTA30	18	1¼	30	(20) 10d	2235 / 9.94	2465 / 10.97	2075 / 9.23	2385 / 10.61
LSTA36	18	1¼	36	(24) 10d	2465 / 10.97	2465 / 10.97	2465 / 10.97	2465 / 10.97
LSTI49	18	3¾	49	(32) 10d x 1½"	3115 / 13.86	3580 / 15.93	2852 / 12.69	3280 / 14.59
LSTI73	18	3¾	73	(48) 10d x 1½"	4670 / 20.77	5370 / 23.89	4280 / 19.04	4920 / 21.89
MSTA9	18	1¼	9	(6) 10d	670 / 2.98	770 / 3.43	625 / 2.78	715 / 3.18
SS MSTA12	18	1¼	12	(8) 10d	895 / 3.98	1030 / 4.58	830 / 3.69	955 / 4.25
MSTA15	18	1¼	15	(10) 10d	1120 / 4.98	1285 / 5.72	1040 / 4.63	1195 / 5.32
SS MSTA18	18	1¼	18	(12) 10d	1340 / 5.96	1545 / 6.87	1245 / 5.54	1430 / 6.36
MSTA21	18	1¼	21	(14) 10d	1565 / 6.96	1800 / 8.01	1455 / 6.47	1670 / 7.43
SS MSTA24	18	1¼	24	(16) 10d	1790 / 7.96	2060 / 9.16	1660 / 7.38	1910 / 8.50
MSTA30	16	1¼	30	(20) 10d	2470 / 10.99	2840 / 12.63	2260 / 10.05	2595 / 11.54
SS MSTA36	16	1¼	36	(24) 10d	2965 / 13.19	3070 / 13.66	2710 / 12.06	3070 / 13.66
MSTA49	16	1¼	49	(28) 8d	2725 / 12.12	2725 / 12.12	2545 / 11.32	2725 / 12.12
ST6215	16	2 1/16	16 5/16	(16) 8d	1405 / 6.25	1615 / 7.18	1300 / 5.78	1500 / 6.67
ST6224	16	2 1/16	23 5/16	(24) 8d	2305 / 10.25	2650 / 11.79	2155 / 9.59	2475 / 11.01
ST9	16	1¼	9	(6) 8d	525 / 2.34	605 / 2.69	490 / 2.18	560 / 2.49
ST12	16	1¼	11⅝	(8) 8d	700 / 3.11	805 / 3.58	650 / 2.89	750 / 3.34
ST18	16	1¼	17¾	(12) 8d	1050 / 4.67	1210 / 5.38	975 / 4.34	1125 / 5.00
ST22	16	1¼	21⅝	(18) 8d	1580 / 7.03	1790 / 7.96	1465 / 6.52	1685 / 7.50



Typical LSTA Installation
(hanger not shown)
Bend strap one time only



Typical LSTA Installation
(hanger not shown)
Bend strap one time only

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.

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SIMPSON Strong-Tie®

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

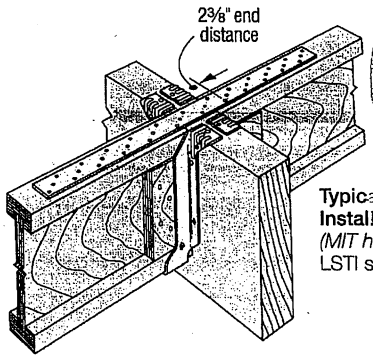
Strap Ties (cont.)

These products are available with additional corrosion protection. For more information, see p. 20.

SD Many of these products are approved for installation with Strong-Drive® SD Connector screws. See pp. 366–370 for more information.

Model No.	Ga.	Dimensions (in.) W	Dimensions (in.) L	Fasteners (Total)	Factored Tensile Resistance D.Fir–L (K_D = 1.00) lb. / kN	D.Fir–L (K_D = 1.15) lb. / kN	S-P-F (K_D = 1.00) lb. / kN	S-P-F (K_D = 1.15) lb. / kN
MSTC28	16	3	28¼	(32) 10d	3955	4545	3615	4155
					17.59	20.22	16.08	18.48
MSTC40		3	40¼	(48) 10d	5930	6820	5420	6235
					26.38	30.34	24.11	27.74
MSTC52		3	52¼	(54) 10d	6670	6940	6100	6940
					29.67	30.87	27.14	30.87
MSTC66	14	3	65¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
MSTC78		3	77¾	(66) 10d	8515	8565	7455	8565
					37.88	38.10	33.16	38.10
ST6236		2 1/16	33 13/16	(36) 8d	3735	4295	3270	3760
					16.61	19.11	14.55	16.73
MSTI26	12	2 1/16	26	(22) 10d x 1½"	2825	3250	2475	2850
					12.57	14.46	11.01	12.68
MSTI36		2 1/16	36	(32) 10d x 1½"	4110	4725	3600	4140
					18.28	21.02	16.01	18.42
MSTI48		2 1/16	48	(44) 10d x 1½"	5650	6500	4955	5695
					25.13	28.91	22.04	25.33
MSTI60		2 1/16	60	(56) 10d x 1½"	7195	7360	6305	7250
					32.01	32.74	28.05	32.25
MSTI72		2 1/16	72	(68) 10d x 1½"	7360	7360	7240	7360
					32.74	32.74	32.21	32.74
MST27		2 1/16	27	(26) 8d	2685	3090	2355	2710
					11.94	13.75	10.48	12.06
MST37		2 1/16	37½	(38) 8d	3930	4515	3440	3960
					17.48	20.08	15.30	17.62
MST48		2 1/16	48	(50) 8d	5170	5945	4530	5210
					23.00	26.45	20.15	23.18
HRS416Z		3¼	16	(16) ¼" x 1½" SDS	2400	2760	2120	2440
					10.68	12.28	9.43	10.85
MST60	10	2 1/16	60	(64) 8d	6620	7610	5800	6670
					29.45	33.85	25.80	29.67
MST72		2 1/16	72	(78) 8d	8065	9135	7065	8125
					35.88	40.64	31.43	36.14

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Use half of the nails in each member being connected to achieve the listed resistances.
3. **Nails:** 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long. See pp. 22–23 for other nail sizes and information.



Typical MSTI Installation
(MIT hanger shown)
LSTI similar

LICENSED PROFESSIONAL ENGINEER
Jan. 5, 2021
G. V. LUBARSKY
100044463
PROVINCE OF ONTARIO

Straps and Ties

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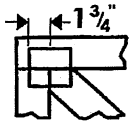
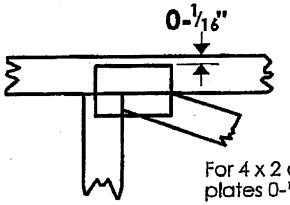
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

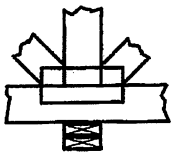
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
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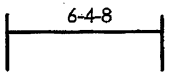
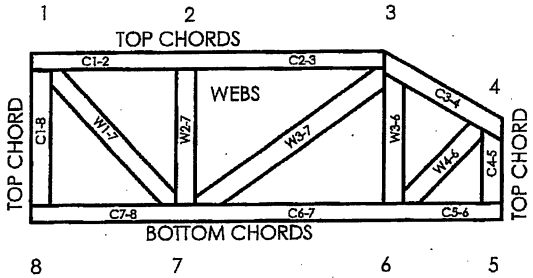
Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
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20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Symbols

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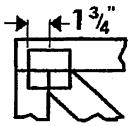

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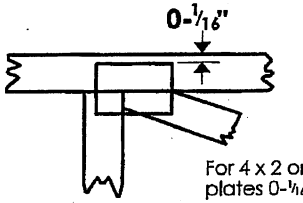

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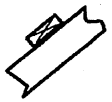

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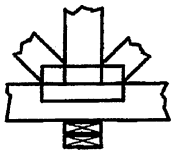

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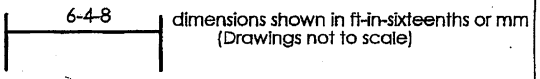

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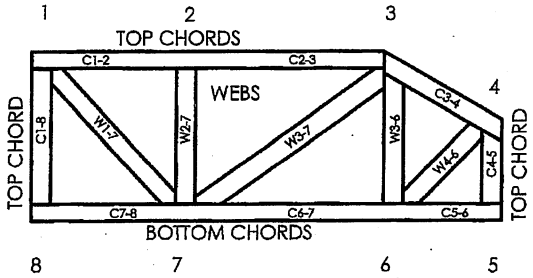

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