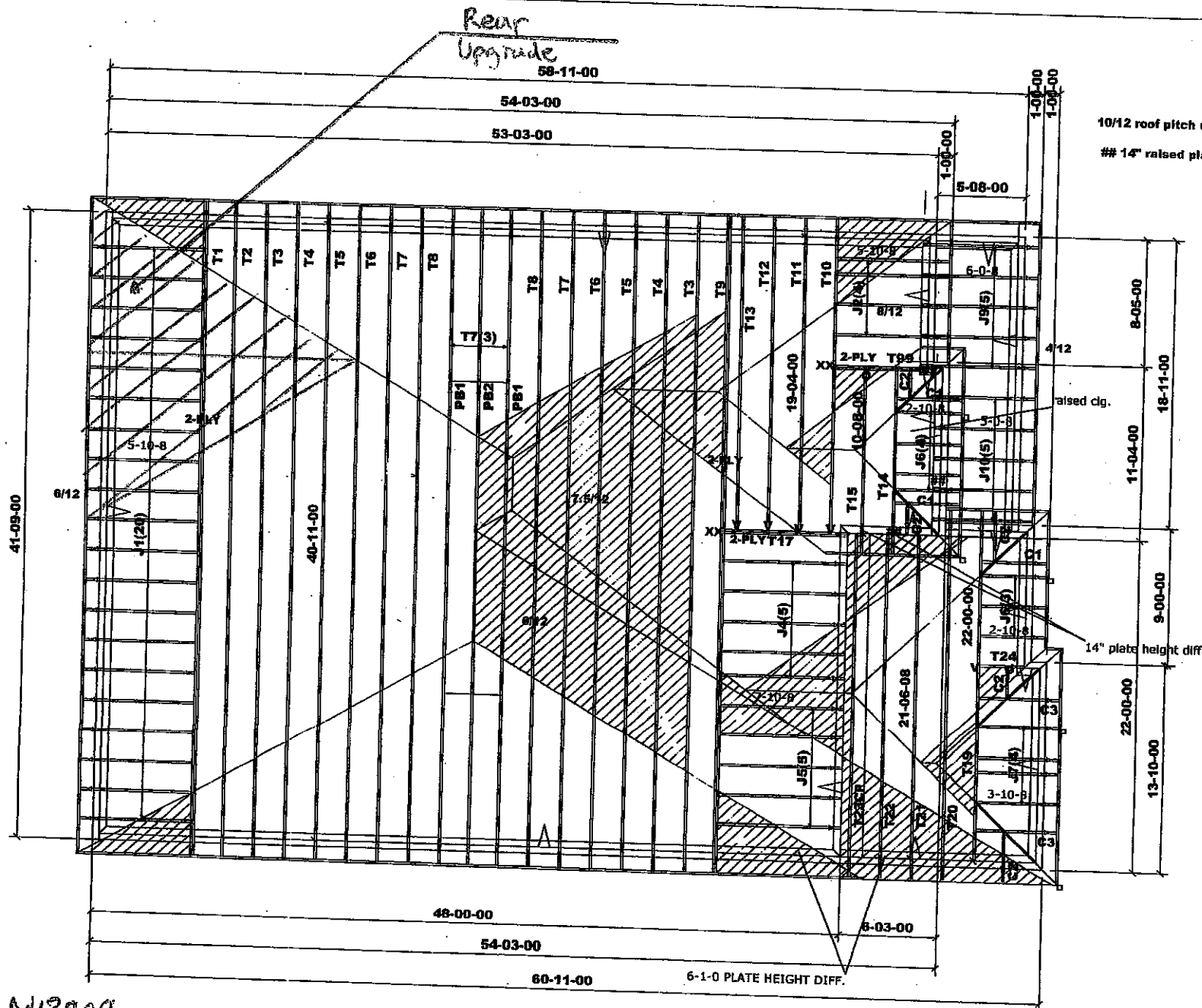
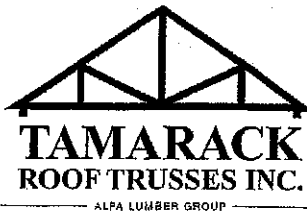








DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 2
 Lot #:
 Elevation: 1

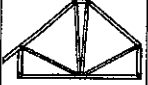
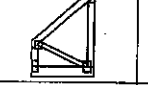
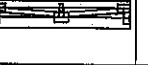
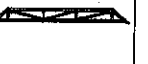
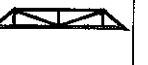
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 PlanLog: 204461
 Layout ID: 417277
 Ref #
 Page: 1 of 3
 Date: 03-22-2021
 Designer:
 Sales Rep: William Garcia

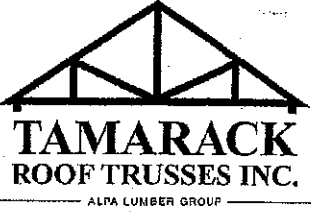
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 /12	40-11-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	457.67 281.33		
	1	T2 Hip	10 /12	40-11-00	5-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	210.31 129.67		
	2	T3 Hip	10 /12	40-11-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	421.09 260.00		
	2	T4 Hip	10 /12	40-11-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	449.04 275.67		
	2	T5 Hip	10 /12	40-11-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	432.52 289.33		
	2	T6 Hip	10 /12	40-11-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	478.5 295.33		
	5	T7 Hip	10 /12	40-11-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	1165.49 721.67		
	2	T8 Hip	10 /12	40-11-00	11-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	509.92 309.33		
	1 2-ply	T9 Roof Special Girder	10 /12	40-11-00	5-08-05	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	487.27 296.00		
	1	T10 Hip Girder	10 /12	19-04-00	5-03-14	2 x 4 2 x 6	1-03-08	1-07-11 2-09-11	94.75 60.00		
	1	T11 Hip	10 /12	19-04-00	6-07-14	2 x 4	1-03-08	1-07-11 2-09-11	92.41 60.00		
	1	T12 Hip	10 /12	19-04-00	7-11-14	2 x 4	1-03-08	1-07-11 2-09-11	92.22 58.67		
	1	T13 Hip	10 /12	19-04-00	9-03-14	2 x 4	1-03-08	1-07-11 2-09-11	103.69 65.33		
	1	T14 Hip Girder	10 /12	10-11-00	4-00-07	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	53.44 35.33		

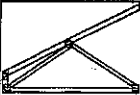
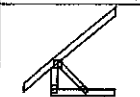
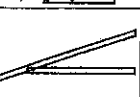
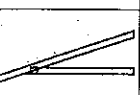
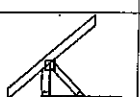
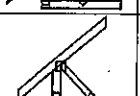
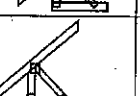
 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: GRANDVILLE 2 Lot #: Elevation: 1	Job Track: 52166 PlanLog: 204461 Layout ID: 417277 Ref # Page: 2 of 3 Date: 03-22-2021 Designer: Sales Rep: William Garcia		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T15 Hip	10 /12	10-08-00	6-10-07	2 x 4	1-03-08	2-09-11 3-00-03	54.86 36.33		
	1 2-ply	T17 Jack-Closed Girder	7.5 /12	7-10-08	5-08-05	2 x 6		9-04 5-08-05	84.93 52.00		
	1	T19 Roof Special Girder	10 /12	22-00-00	4-10-07	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	110.7 72.33		
	1	T20 Hip	10 /12	22-00-00	5-08-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	93.53 60.67		
	1	T21 Hip	10 /12	21-06-08	7-04-07	2 x 4		1-07-11 2-00-04	104.05 66.83		
	1	T22 Hip	10 /12	21-06-08	9-00-07	2 x 4		1-07-11 2-00-04	106.35 69.00		
	1	T23CP Hip	10 /12	21-06-08	10-08-07	2 x 4		1-07-11 2-00-04	102.87 64.83		
	1	T24 Jack-Closed Girder	10 /12	2-10-08	4-00-07	2 x 4 2 x 6		1-07-11 4-00-07	16.59 11.17		
	1 2-ply	T99 Flat Girder	0 /12	5-10-08	1-02-00	2 x 4 2 x 6		1-02-00 1-02-00	45.4 29.67		
	2	PB1 Piggyback	10 /12	20-07-03	2-00-00	2 x 4			127.9 84.67		
	1	PB2 Piggyback	10 /12	20-07-03	2-11-02	2 x 4			64.49 40.83		
	20	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	335.93 213.33		
	4	J2 Jack-Open	8 /12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	76.78 46.67		
	5	J4 Jack-Partial	7.5 /12	7-10-08	5-08-05	2 x 4		9-04 5-08-05	139.63 90.00		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	52166
	Builder:	GREENPARK HOMES		PlanLog:	204461
	Project:	RUSSELL GARDENS PH.4		Layout ID:	417277
	Location:	HAMILTON		Ref #	
	Model:	GRANDVILLE 2		Page:	3 of 3
	Lot #:			Date:	03-22-2021
Elevation:	1		Designer:		
			Sales Rep:	William Garcia	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	J5 Jack-Partial	6 /12	7-10-08	5-01-04	2 x 4		1-02-00 5-01-04	137.12 85.00		
	7	J6 Jack-Open	10 /12	2-10-08	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	88.21 58.33		
	4	J7 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	60.9 38.67		
	5	J9 Jack-Open	4 /12	6-00-08	2-09-00	2 x 4	1-03-08	3-15 2-04-02	79.07 50.00		
	5	J10 Jack-Open	4 /12	5-00-08	2-05-00	2 x 4	1-03-08	3-15 2-00-02	67.36 43.33		
	3	C1 Jack-Open	10 /12	1-10-15	3-02-13	2 x 4	1-03-08 11-09	1-07-11 3-02-13	33.42 23.00		
	5	C2 Jack-Open	10 /12	1-10-15	3-02-13	2 x 4	1-03-08 1-01	1-07-11 3-02-13	50.72 35.00		
	2	C3 Jack-Open	10 /12	1-10-15	3-02-13	2 x 4	1-03-08 1-11-09	1-07-11 3-02-13	24.56 16.67		

TOTAL # TRUSS= 104

TOTAL BFT OF ALL TRUSSES= 4405.99

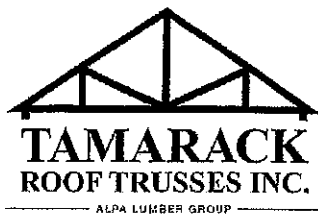
BFT.

TOTAL WEIGHT OF ALL TRSSES 7053.74 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
5	Hardware	LJS26DS	
2	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 9



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 2
 Lot #:
 Elevation: 2

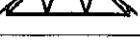
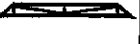
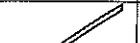
Job Track: 52166
 PlanLog: 204461
 Layout ID: 417278
 Ref #
 Page: 1 of 3
 Date: 03-22-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	10 /12	40-11-00	4-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	415.43 256.00		
	1	T2 Hip	10 /12	40-11-00	5-01-04	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	210.31 129.67		
	2	T3 Hip	10 /12	40-11-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	421.09 260.00		
	2	T4 Hip	10 /12	40-11-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	449.04 275.67		
	2	T5 Hip	10 /12	40-11-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	425.59 265.33		
	2	T6 Hip	10 /12	40-11-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	478.5 295.33		
	5	T7 Hip	10 /12	40-11-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	1165.49 721.67		
	2	T8 Hip	10 /12	40-11-00	11-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	509.92 309.33		
	1 2-ply	T9Z Roof Special Girder	10 /12	40-11-00	5-08-05	2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	487.27 296.00		
	1 2-ply	T17Z Jack-Closed Girder	7.5 /12	7-10-08	5-08-05	2 x 6		9-04 5-08-05	84.93 52.00		
	3	T23 Common	10 /12	21-06-08	10-09-11	2 x 4		1-07-11 2-00-04	309.61 196.00		
	1	T30 Hip Girder	10 /12	8-08-00	4-00-07	2 x 4 2 x 6		1-07-11 1-07-11	43.59 29.50		
	2	T31 Common	10 /12	8-02-00	6-09-00	2 x 4		3-04-03 3-04-03	82.53 53.00		
	2 2-ply	T32 Flat Girder	0 /12	5-10-08	1-06-00	2 x 6		1-06-00 1-06-00	108.38 67.33		

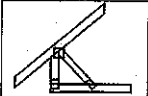
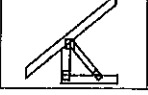
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST						
	Lumber Yard: TAMARACK LUMBER						Job Track: 52166
	Builder: GREENPARK HOMES						PlanLog: 204461
	Project: RUSSELL GARDENS PH.4						Layout ID: 417278
	Location: HAMILTON						Ref #
	Model: GRANDVILLE 2						Page: 2 of 3
Lot #:						Date: 03-22-2021	
Elevation: 2						Designer:	
						Sales Rep: William Garcia	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T33 Hip Girder	10 /12	19-04-00	5-03-14	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	97.22 61.50		
	1	T34 Hip	10 /12	19-04-00	6-07-14	2 x 4	1-03-08	1-07-11 1-07-11	89.45 58.00		
	1	T35 Hip	10 /12	19-04-00	7-11-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	93.35 59.83		
	1	T36 Hip	10 /12	19-04-00	9-03-14	2 x 4	1-03-08	1-07-11 1-07-11	98.43 60.67		
	4	T38 Common	10 /12	22-00-00	10-09-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	466.19 288.00		
	1	G39 GABLE	10 /12	13-00-00	7-00-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	63.37 41.50		
	2	PB1 Piggyback	10 /12	20-07-03	2-00-00	2 x 4			125.68 80.67		
	1	PB2 Piggyback	10 /12	20-07-03	2-11-02	2 x 4			64.49 40.83		
	20	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	335.93 213.33		
	6	J2 Jack-Open	8 /12	5-10-08	5-03-14	2 x 4	1-03-08	1-04-14 5-03-14	115.18 70.00		
	5	J4 Jack-Partial	7.5 /12	7-10-08	5-08-05	2 x 4		9-04 5-08-05	138.22 88.33		
	5	J5 Jack-Partial	6 /12	7-10-08	5-01-04	2 x 4		1-02-00 5-01-04	137.12 85.00		
	3	J6 Jack-Open	10 /12	2-10-08	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	37.8 25.00		
	6	J9 Jack-Open	4 /12	6-00-08	2-09-00	2 x 4	1-03-08	3-15 2-04-02	94.89 60.00		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52166
	Builder:	GREENPARK HOMES	PlanLog:	204461
	Project:	RUSSELL GARDENS PH.4	Layout ID:	417278
	Location:	HAMILTON	Ref #	
	Model:	GRANDVILLE 2	Page:	3 of 3
Lot #:		Date:	03-22-2021	
Elevation:	2	Designer:		
		Sales Rep:	William Garcia	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J10 Jack-Open	4 /12	5-00-08	2-05-00	2 x 4	1-03-08	3-15 2-00-02	53.89 34.87		
	2	C1 Jack-Open	10 /12	1-10-15	3-02-13	2 x 4	1-03-08 11-09	1-07-11 3-02-13	22.28 15.33		
	2	C2 Jack-Open	10 /12	2-00-00	3-02-13	2 x 4	1-03-08	1-07-11 3-03-11	20.09 14.00		

TOTAL # TRUSS= 97

TOTAL BFT OF ALL TRUSSES= 4503.49

BFT.

TOTAL WEIGHT OF ALL TRSSES 7243.27 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
3	Hardware	LJS26DS	
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 10



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: GRANDVILLE 2
 Lot #:
 Elevation: 3

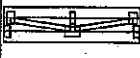
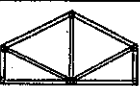
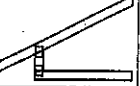
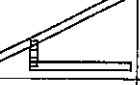
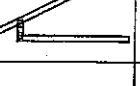
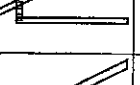
Job Track: 52166
 PlanLog: 204461
 Layout ID: 417279
 Ref #
 Page: 1 of 3
 Date: 03-22-2021
 Designer:
 Sales Rep: William Garcia

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T50 Hip Girder	6 /12	40-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	424.04 260.00		
	1 2-ply	T50Z Hip Girder	6 /12	40-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	424.04 260.00		
	2	T51 Hip	6 /12	40-11-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	405.66 249.67		
	2	T52 Hip	6 /12	40-11-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	402.92 245.67		
	2	T53 Hip	6 /12	40-11-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	391.29 241.00		
	2	T54 Hip	6 /12	40-11-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	386 236.67		
	2	T55 Hip	6 /12	40-11-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	416.92 257.67		
	2	T56 Hip	6 /12	40-11-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	423.17 259.33		
	2	T57 Hip	6 /12	40-11-00	11-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	429.55 262.67		
	3	T58 Common	6 /12	40-11-00	11-04-12	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	613.87 372.00		
	1 2-ply	T59 Roof Special Girder	6 /12	28-02-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	252.92 163.67		
	1	T60 Hip	6 /12	28-02-00	4-00-12	2 x 4	1-03-08	1-02-00 1-02-00	105.11 67.17		
	1	T61 Hip	6 /12	28-02-00	5-00-12	2 x 4	1-03-08	1-02-00 1-02-00	107.83 68.17		
	1 2-ply	T62 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	57.64 37.00		

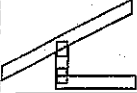
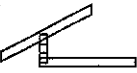
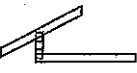
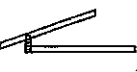
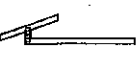
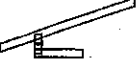
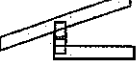
 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST					
	Lumber Yard: TAMARACK LUMBER				Job Track: 52166	
	Builder: GREENPARK HOMES				PlanLog: 204461	
	Project: RUSSELL GARDENS PH.4				Layout ID: 417279	
	Location: HAMILTON				Ref #	
	Model: GRANDVILLE 2				Page: 2 of 3	
Lot #:				Date: 03-22-2021		
Elevation: 3				Designer:		
				Sales Rep: William Garcia		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T63 Flat Girder	0 / 12	5-10-08	1-06-00	2 x 6		1-06-00 1-06-00	54.19 33.67		
	1 2-ply	T64 Jack-Closed Girder	6 / 12	3-09-08	3-00-12	2 x 4 2 x 6		1-02-00 3-00-12	35.28 24.00		
	1	T65 Hip Girder	6 / 12	9-10-00	2-09-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	40.1 27.67		
	1	T66 Common	6 / 12	9-07-00	5-01-08	2 x 4		2-08-00 2-09-08	40.35 26.67		
	1	T67 Hip Girder	4 / 12	13-00-00	2-10-11	2 x 4	1-03-08 1-03-08	11-03 11-03	50.06 33.17		
	1	T68 Common	4 / 12	13-00-00	3-01-03	2 x 4	1-03-08 1-03-08	11-03 11-03	48.03 31.50		
	2	T69 Common	4 / 12	12-03-08	3-01-03	2 x 4	1-03-08	11-03 1-02-00	93.09 62.00		
	23	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	386.27 245.33		
	3	J50 Jack-Open	6 / 12	3-03-08	2-09-12	2 x 4	1-03-08	1-02-00 2-09-12	31.67 22.00		
	3	J51 Jack-Open	6 / 12	3-09-08	3-00-12	2 x 4	1-03-08	1-02-00 3-00-12	35.29 22.00		
	2	J52 Jack-Open	4 / 12	5-10-08	2-10-11	2 x 4	1-03-08	11-03 2-10-11	31.83 20.00		
	4	C50 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	56.53 34.67		
	4	C51 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	46.33 29.33		
	4	C52 Jack-Open	6 / 12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	38.28 24.00		

DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52166
	Builder:	GREENPARK HOMES	PlanLog:	204461
	Project:	RUSSELL GARDENS PH.4	Layout ID:	417279
	Location:	HAMILTON	Ref #	
	Model:	GRANDVILLE 2	Page:	3 of 3
	Lot #:		Date:	03-22-2021
	Elevation:	3	Designer:	
		Sales Rep:	William Garcia	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	7	C53 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	49.15 32.67		
	2	C54 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-06-01	1-02-00 2-00-12	17.27 12.00		
	1	C55 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 2-00-01	1-02-00 2-00-12	9.21 6.00		
	2	C56 Jack-Open	4 /12	3-09-07	2-02-05	2 x 4	1-03-08 2-01-01	11-03 2-02-05	26.81 17.33		
	2	C57 Jack-Open	4 /12	1-09-07	1-06-05	2 x 4	1-03-08 4-01-01	11-03 1-06-05	22 14.67		
	2	C58 Jack-Open	4 /12	1-10-08	2-02-05	2 x 4	1-03-08 1-10-15	11-03 1-06-11	17.89 12.00		
	2	C59 Jack-Open	4 /12	1-09-07	1-06-05	2 x 4	1-03-08 1-01	11-03 1-06-05	12.88 9.33		

TOTAL # TRUSS= 98 TOTAL BFT OF ALL TRUSSES= 3720.7 BFT. TOTAL WEIGHT OF ALL TRSSES 5983.24 LBS

HARDWARE

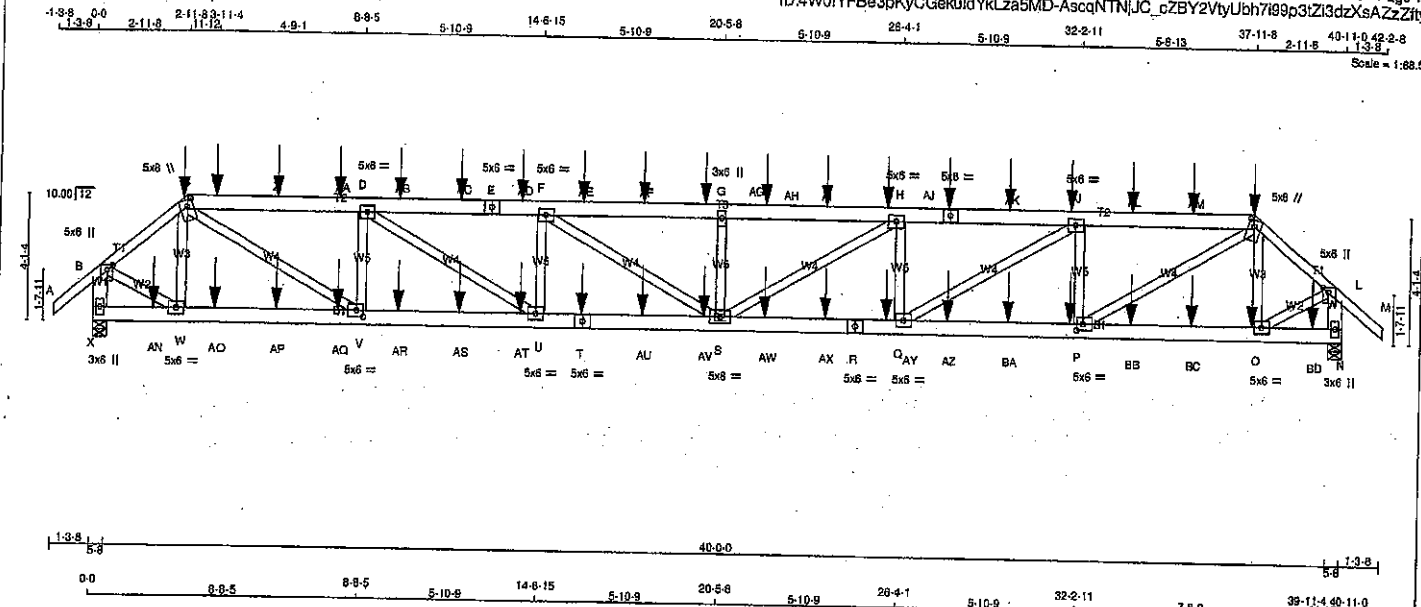
QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
2	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 6

JOB NAME 417277	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Mar 18 16:07:29 2021 Page 1
ID:4W0IYFB3pkyCGekuldYkLza5MD-AscqNTNJC_cZBY2VtyUbb7H99p3IZI3dzXsAZzZity



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
E - I	2x6	DRY	No.2	SPF	
I - K	2x6	DRY	No.2	SPF	
K - M	2x4	DRY	No.2	SPF	
M - B	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	
L - T	2x6	DRY	No.2	SPF	
T - R	2x6	DRY	No.2	SPF	
R - N	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
K-M	12	SIDE(61.0)
C-E	12	SIDE(61.0)
E-I	12	SIDE(61.0)
I-K	12	SIDE(183.1)
X-B	12	TOP
N-L	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-T	12	SIDE(0.0)
T-R	12	SIDE(0.0)
R-N	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
X	3836	0	0	3836	0	0	5-8	5-8	5-8
N	3903	0	0	3903	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
X	2712	1784 / 0	0 / 0
N	2781	1811 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX		MAX		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	UNBRAC
FR-TO		FROM	TO	CS1 (LC)	LENGTH	FR-TO		CS1 (LC)	
A-B	0 / 41	-91.8	-91.8	0.07 (1)	10.00	W-C	-1039 / 0	0.09 (1)	
B-C	-3592 / 0	-91.8	-91.8	0.13 (1)	4.81	O-K	-1059 / 0	0.09 (1)	
C-Y	-7098 / 0	-91.8	-91.8	0.26 (1)	4.32	B-W	0 / 3094	0.27 (1)	
Y-Z	-7098 / 0	-91.8	-91.8	0.26 (1)	4.32	O-L	0 / 3142	0.28 (1)	
Z-AA	-7098 / 0	-91.8	-91.8	0.26 (1)	4.32	P-K	0 / 4980	0.44 (1)	
AA-D	-7098 / 0	-91.8	-91.8	0.26 (1)	4.32	C-V	0 / 5007	0.44 (1)	
D-AB	-9605 / 0	-91.8	-91.8	0.35 (1)	3.72	P-J	-2513 / 0	0.22 (1)	
AB-AC	-9605 / 0	-91.8	-91.8	0.35 (1)	3.72	V-D	-2528 / 0	0.22 (1)	
AC-E	-9605 / 0	-91.8	-91.8	0.35 (1)	3.72	Q-J	0 / 2935	0.26 (1)	
E-AD	-9605 / 0	-91.8	-91.8	0.35 (1)	3.72	D-U	0 / 2951	0.26 (1)	
AD-F	-9605 / 0	-91.8	-91.8	0.35 (1)	3.72	Q-H	-1360 / 0	0.12 (1)	
F-AE	-10456 / 0	-91.8	-91.8	0.35 (1)	3.59	U-F	-1368 / 0	0.12 (1)	
AE-AF	-10456 / 0	-91.8	-91.8	0.35 (1)	3.59	S-H	0 / 992	0.09 (1)	
AF-AG	-10456 / 0	-91.8	-91.8	0.35 (1)	3.59	F-S	0 / 1001	0.09 (1)	
AG-G	-10456 / 0	-91.8	-91.8	0.35 (1)	3.59	S-G	-871 / 0	0.07 (1)	
G-AH	-10456 / 0	-91.8	-91.8	0.35 (1)	3.59				
AH-AI	-10456 / 0	-91.8	-91.8	0.35 (1)	3.59				
AI-AJ	-10456 / 0	-91.8	-91.8	0.35 (1)	3.59				
AJ-H	-10456 / 0	-91.8	-91.8	0.35 (1)	3.59				
H-I	-8613 / 0	-91.8	-91.8	0.34 (1)	3.74				
I-AK	-8613 / 0	-91.8	-91.8	0.34 (1)	3.74				
AK-J	-8613 / 0	-91.8	-91.8	0.34 (1)	3.74				
J-L	-7119 / 0	-91.8	-91.8	0.28 (1)	4.31				
AL-AM	-7119 / 0	-91.8	-91.8	0.28 (1)	4.31				
AM-K	-7119 / 0	-91.8	-91.8	0.28 (1)	4.31				
K-L	-3739 / 0	-91.8	-91.8	0.13 (1)	4.78				
L-M	0 / 41	-91.8	-91.8	0.07 (1)	10.00				
X-B	-3841 / 0	0.0	0.0	0.14 (1)	7.22				
N-L	-3897 / 0	0.0	0.0	0.14 (1)	7.17				
X-AN	0 / 0	-18.5	-18.5	0.05 (4)	10.00				
AN-W	0 / 0	-18.5	-18.5	0.05 (4)	10.00				
W-AO	0 / 2879	-18.5	-18.5	0.23 (1)	10.00				
AO-AP	0 / 2879	-18.5	-18.5	0.23 (1)	10.00				
AP-AQ	0 / 2879	-18.5	-18.5	0.23 (1)	10.00				
AQ-V	0 / 2879	-18.5	-18.5	0.23 (1)	10.00				
V-AR	0 / 7087	-18.5	-18.5	0.52 (1)	10.00				
AR-AS	0 / 7087	-18.5	-18.5	0.52 (1)	10.00				
AS-AT	0 / 7087	-18.5	-18.5	0.52 (1)	10.00				
AT-U	0 / 7087	-18.5	-18.5	0.52 (1)	10.00				
U-T	0 / 9605	-18.5	-18.5	0.89 (1)	10.00				
T-AU	0 / 9605	-18.5	-18.5	0.89 (1)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = -7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.31")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/861 (0.57")

CSI: TC=0.35/1.00 (F-G:1), BC=0.89/1.00 (S-U:1), WB=0.44/1.00 (C-V:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

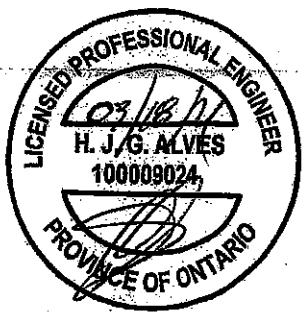
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (C) (INPUT = 0.90)

JSI METAL = 0.87 (T) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2107821

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	8.0	2.50	2.25
D	TMVW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
G	TMVW+w	MT20	5.0	6.0		
I	TS-t	MT20	5.0	6.0		
K	TTWW+m	MT20	5.0	8.0	2.50	2.25
L	TMVW+p	MT20	5.0	6.0	2.00	2.25
N	BMV1+p	MT20	5.0	6.0		
O	Q, U, W					
P	BMVW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0	2.50	2.50
S	BMVW/t	MT20	5.0	6.0		
T	BS-t	MT20	5.0	6.0		
V	BMVW-t	MT20	5.0	6.0	2.50	2.50
X	BMV1+p	MT20	5.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. UNBRACED LENGTH FR-TO
FR-TO		FROM TO		FR-TO			
AU-AV	0/9605	-18.5 -18.5	0.69 (1) 10.00				
AV-S	0/9605	-18.5 -18.5	0.69 (1) 10.00				
S-AW	0/9613	-18.5 -18.5	0.69 (1) 10.00				
AW-AX	0/9613	-18.5 -18.5	0.69 (1) 10.00				
AX-R	0/9618	-18.5 -18.5	0.69 (1) 10.00				
R-AY	0/9613	-18.5 -18.5	0.69 (1) 10.00				
AY-Q	0/9613	-18.5 -18.5	0.69 (1) 10.00				
Q-AZ	0/7119	-18.5 -18.5	0.52 (1) 10.00				
AZ-BA	0/7119	-18.5 -18.5	0.52 (1) 10.00				
BA-P	0/7119	-18.5 -18.5	0.52 (1) 10.00				
P-BB	0/2924	-18.5 -18.5	0.23 (1) 10.00				
BB-BC	0/2924	-18.5 -18.5	0.23 (1) 10.00				
BC-C	0/2924	-18.5 -18.5	0.23 (1) 10.00				
C-BD	0/0	-18.5 -18.5	0.05 (4) 10.00				
BD-N	0/0	-18.5 -18.5	0.05 (4) 10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-30	-30	FRONT	VERT	DEAD		C1
C	2-11-8	-125	-125	FRONT	VERT	SNOW		C1
J	27-11-4	-76	-76	FRONT	VERT	TOTAL		C1
J	31-11-4	-76	-76	FRONT	VERT	TOTAL		C1
K	37-11-8	-30	-30	FRONT	VERT	DEAD		C1
K	37-11-8	-82	-82	FRONT	VERT	TOTAL		C1
K	37-11-8	-125	-125	FRONT	VERT	SNOW		C1
O	37-11-4	-21	-21	FRONT	VERT	TOTAL		C1
P	31-11-4	-21	-21	FRONT	VERT	TOTAL		C1
T	15-11-4	-21	-21	FRONT	VERT	TOTAL		C1
Y	3-11-4	-77	-77	FRONT	VERT	TOTAL		C1
Z	5-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AA	7-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AB	9-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AC	11-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AD	13-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AE	15-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AF	17-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AG	19-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AH	21-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AI	23-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AJ	25-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AK	29-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AL	33-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AM	35-11-4	-76	-76	FRONT	VERT	TOTAL		C1
AN	1-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AO	3-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AP	5-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AQ	7-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AR	9-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AS	11-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AT	13-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AU	17-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AV	19-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AW	21-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AX	23-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AY	25-11-4	-21	-21	FRONT	VERT	TOTAL		C1
AZ	27-11-4	-21	-21	FRONT	VERT	TOTAL		C1
BA	29-11-4	-21	-21	FRONT	VERT	TOTAL		C1
BB	33-11-4	-21	-21	FRONT	VERT	TOTAL		C1
BC	35-11-4	-21	-21	FRONT	VERT	TOTAL		C1
BD	39-11-4	-21	-21	FRONT	VERT	TOTAL		C1

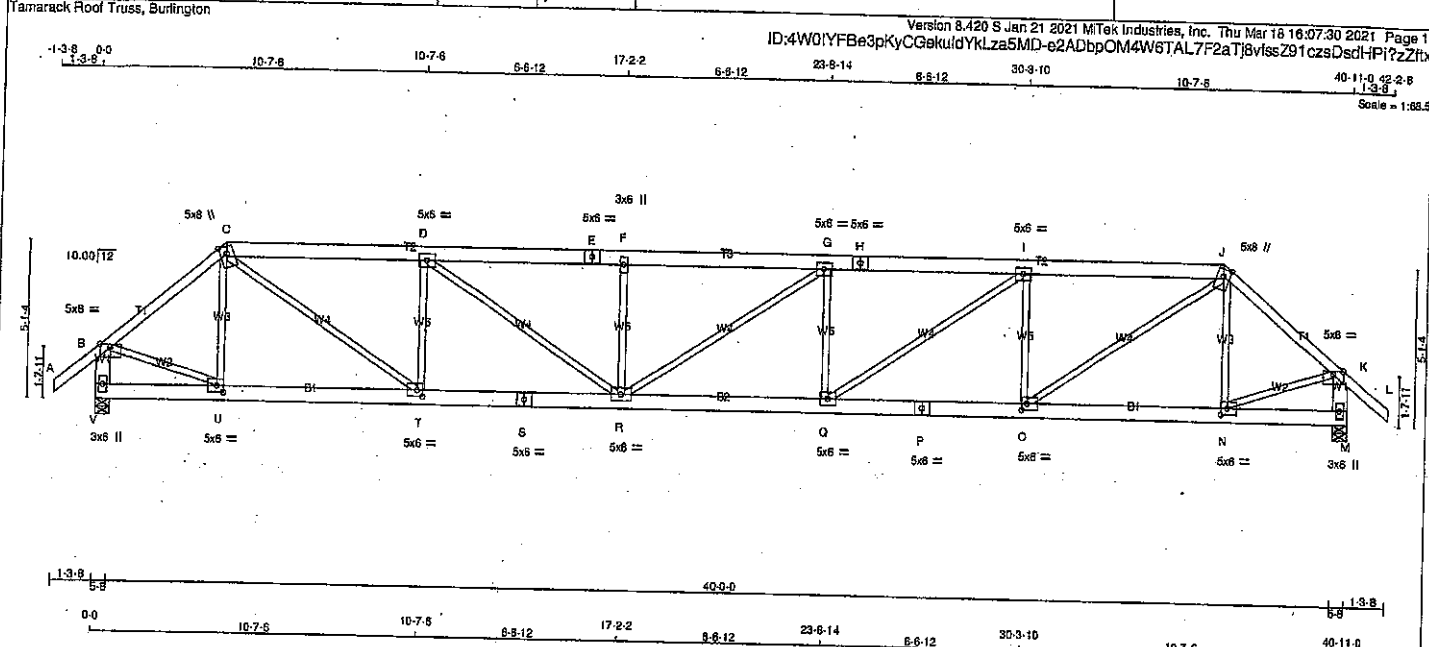
CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2107821

JOB NAME 417277	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x6 DRY	No.2	SPF		
E - H	2x6 DRY	No.2	SPF		
H - J	2x6 DRY	No.2	SPF		
J - L	2x4 DRY	No.2	SPF		
V - B	2x6 DRY	No.2	SPF		
M - K	2x6 DRY	No.2	SPF		
V - S	2x6 DRY	No.2	SPF		
S - P	2x6 DRY	No.2	SPF		
P - M	2x6 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TTWW+m	MT20	5.0	8.0	Edge	
D, G, I					
D TMVW-t	MT20	5.0	6.0		
E TS-t	MT20	5.0	6.0		
F TMVW-w	MT20	3.0	8.0		
H TS-t	MT20	5.0	6.0		
J TTWW+m	MT20	5.0	8.0	Edge	
K TMVW-p	MT20	5.0	8.0	Edge	
M BMV1-p	MT20	3.0	8.0		
N BMVW-t	MT20	5.0	6.0	2.50	2.50
O BMVW-t	MT20	5.0	6.0	2.50	2.00
P BS-t	MT20	5.0	6.0		
Q BMVW-t	MT20	5.0	6.0		
R BMVW-t	MT20	5.0	6.0		
S BS-t	MT20	5.0	6.0		
T BMVW-t	MT20	5.0	6.0	2.50	2.00
U BMVW-t	MT20	5.0	6.0	2.50	2.50
V BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)



Structural component only
DWG# T-2107822

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT
V	2383	0	2383	0	0
M	2383	0	2383	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
V	1663	1118 / 0	0 / 0	0 / 0	0 / 0	565 / 0	0 / 0	
M	1663	1118 / 0	0 / 0	0 / 0	0 / 0	565 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL TRUSS BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. UNBRACED LENGTH	W E B S	MEMB. MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO	FROM TO				FR-TO		
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	U-C	-422 / 0
B-C	-2350 / 0	-91.8	-91.8	0.39 (1)	4.11	N-J	-421 / 0
C-D	-3895 / 0	-91.8	-91.8	0.35 (1)	4.09	B-U	0 / 1890
D-E	-4892 / 0	-91.8	-91.8	0.42 (1)	3.68	N-K	0 / 1890
E-F	-4892 / 0	-91.8	-91.8	0.42 (1)	3.68	O-J	0 / 2559
F-G	-4892 / 0	-91.8	-91.8	0.36 (1)	3.72	C-T	0 / 2561
G-H	-4896 / 0	-91.8	-91.8	0.42 (1)	3.66	O-I	-1385 / 0
H-I	-4896 / 0	-91.8	-91.8	0.42 (1)	3.66	T-D	-1384 / 0
I-J	-3893 / 0	-91.8	-91.8	0.35 (1)	4.09	Q-I	0 / 1229
J-K	-2350 / 0	-91.8	-91.8	0.39 (1)	4.11	D-R	0 / 1229
K-L	0 / 41	-91.8	-91.8	0.13 (1)	10.00	Q-G	-588 / 0
V-B	-2363 / 0	0.0	0.0	0.16 (1)	6.69	R-F	-588 / 0
M-K	-2363 / 0	0.0	0.0	0.16 (1)	6.69	R-G	-5 / 0
V-U	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
U-T	0 / 1819	-18.5	-18.5	0.26 (1)	10.00		
T-S	0 / 3894	-18.5	-18.5	0.52 (1)	10.00		
S-R	0 / 3894	-18.5	-18.5	0.52 (1)	10.00		
R-Q	0 / 4896	-18.5	-18.5	0.64 (1)	10.00		
Q-P	0 / 3893	-18.5	-18.5	0.52 (1)	10.00		
P-O	0 / 3893	-18.5	-18.5	0.52 (1)	10.00		
O-N	0 / 1819	-18.5	-18.5	0.26 (1)	10.00		
N-M	0 / 0	-18.5	-18.5	0.07 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/998 (0.27")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/965 (0.51")

CSI: TC=0.42/1.00 (D-F:1), BC=0.64/1.00 (Q-R:1), WB=0.58/1.00 (C-T:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1987 1873

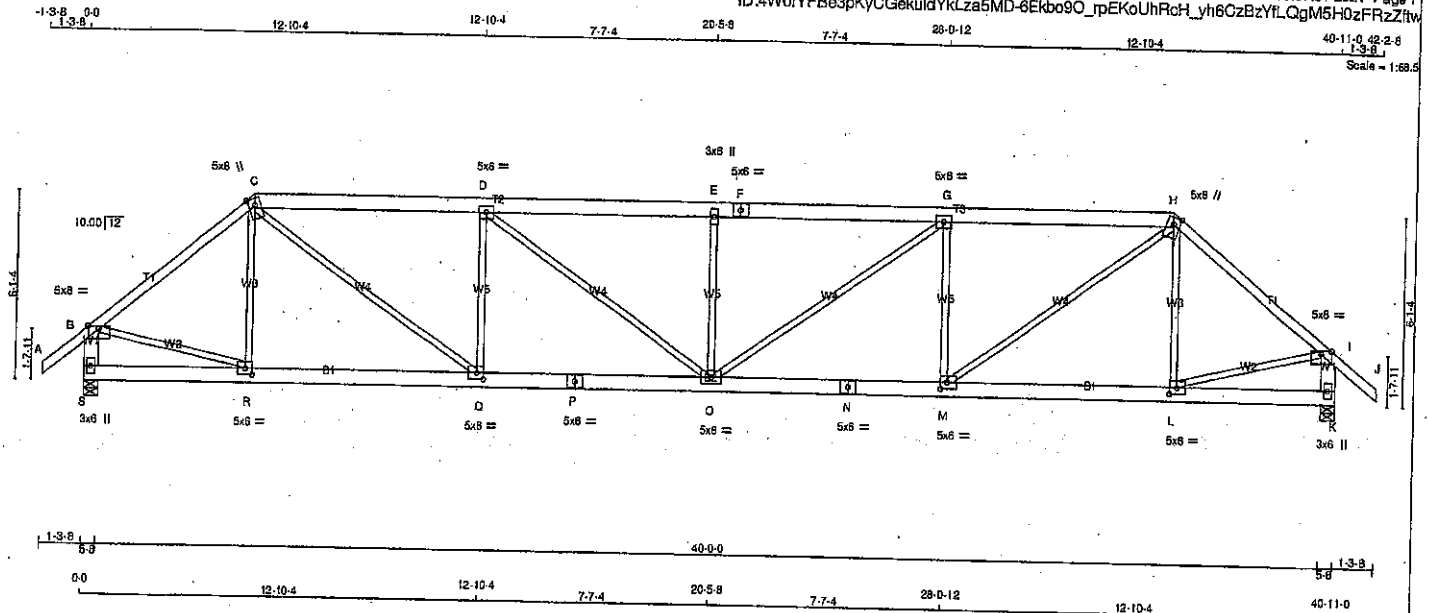
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (T) (INPUT = 0.90)
JSI METAL = 0.74 (S) (INPUT = 1.00)

JOB NAME 417277	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MITeX Industries, Inc. Thu Mar 18 16:07:31 2021 Page 1
ID:4W01YFB3pKyCGekuldYkLza5MD-6Eko9O_rpeKoUhRch_yh6CzBzYfLqGM5H0zFRzZfhw
40-11-0 42-2-8
Scale = 1:68.8



TOTAL WEIGHT = 2 X 211 = 422 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMVW-t	MT20	5.0	8.0		
E	TMVW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	8.0		
G	TMVW-t	MT20	5.0	8.0		
H	TTWW+m	MT20	5.0	8.0	Edge	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	8.0	2.50	2.75
M	BMVW-t	MT20	5.0	8.0	2.50	2.50
N	BS-t	MT20	5.0	8.0		
O	BMVW-w	MT20	5.0	8.0		
P	BS-t	MT20	5.0	8.0		
Q	BMVW-t	MT20	5.0	8.0	2.50	2.50
R	BMVW-t	MT20	5.0	8.0	2.50	2.75
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	JT	DOWN	IN-SX	IN-SX	JT	DOWN
S	2383	0	2383	0	5-8	5-8	0
K	2383	0	2383	0	5-8	5-8	0

UNFACTORED REACTIONS

JT	1ST LOSE	MAX. MIN. COMPONENT REACTIONS	JT	1ST LOSE	MAX. MIN. COMPONENT REACTIONS
S	1883	1118 / 0	0 / 0	PERM LIVE	WIND
K	1883	1118 / 0	0 / 0	DEAD	SOIL

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	A-B	-295 / 0
B-C	-2418 / 0	B-C	-295 / 0
C-D	-3821 / 0	C-D	0 / 1910
D-E	-4152 / 0	D-E	0 / 1910
E-F	-4152 / 0	E-F	0 / 2199
F-G	-4152 / 0	F-G	0 / 2199
G-H	-3821 / 0	G-H	-1186 / 0
H-I	-2418 / 0	H-I	-1186 / 0
I-J	0 / 41	I-J	0 / 883
S-B	-2349 / 0	S-B	0 / 883
K-I	-2349 / 0	K-I	-654 / 0
S-R	0 / 0	S-R	-18.5 -18.5 0.09 (4)
R-Q	0 / 1866	R-Q	-18.5 -18.5 0.27 (1)
Q-P	0 / 3920	Q-P	-18.5 -18.5 0.49 (1)
P-O	0 / 3920	P-O	-18.5 -18.5 0.49 (1)
O-N	0 / 3920	O-N	-18.5 -18.5 0.49 (1)
N-M	0 / 3920	N-M	-18.5 -18.5 0.49 (1)
M-L	0 / 1866	M-L	-18.5 -18.5 0.27 (1)
L-K	0 / 0	L-K	-18.5 -18.5 0.09 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
BOT CH.	LL = 6.0 PSF
	DL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.39")

CSI: TO=0.70/1.00 (B-C:1), BC=0.49/1.00 (O-Q:1), WB=0.60/1.00 (D-Q:1), SS=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

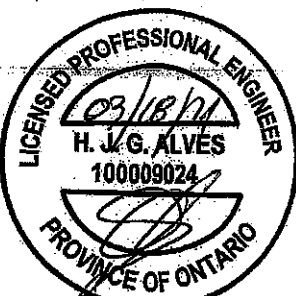
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1887 1873

PLATE PLACEMENT TOL. = 0.250 inches

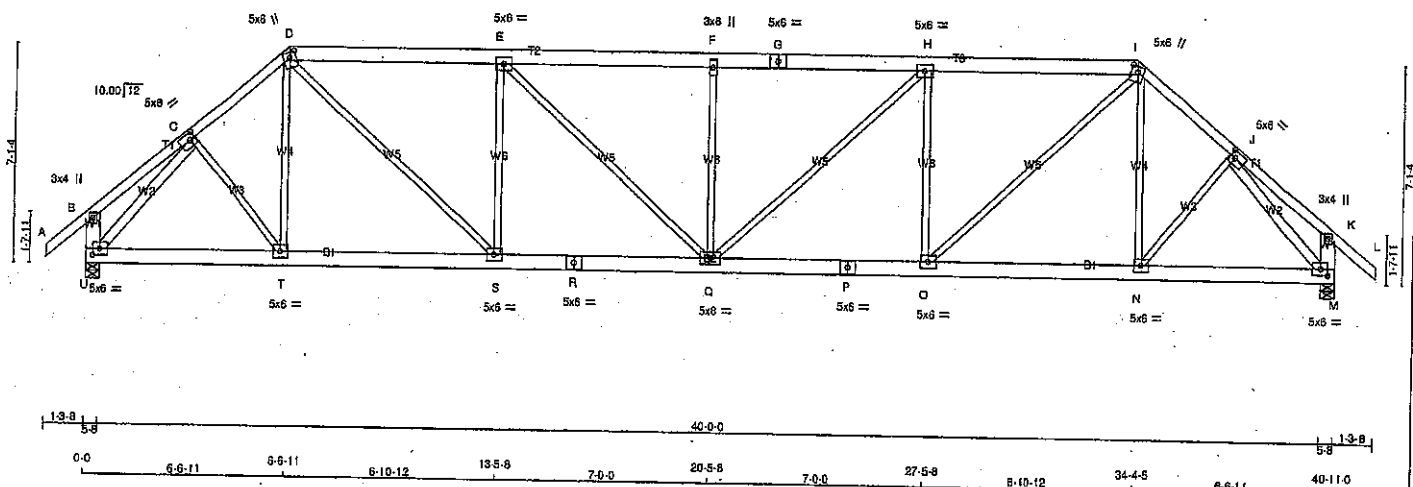
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.69 (P) (INPUT = 1.00)



Structural component only
DWG# T-2107823

JOB NAME 417277	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:07:32 2021 Page 1 ID:W01YfBe3pKyCGekuidYkLza5MD-aRiz?VPoc7MBQeGdA?WBDKICMx4pGVKxmwWnuZzRtv	



TOTAL WEIGHT = 2 X 225 = 449 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
U - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	2.50 2.00
D	TTWW+m	MT20	5.0	6.0	2.25 2.50
E	TMWW-t	MT20	5.0	6.0	
F	TMWW-w	MT20	3.0	6.0	
G	TS-t	MT20	6.0	6.0	
H	TMWW-t	MT20	5.0	6.0	
I	TTWW+m	MT20	5.0	6.0	2.25 2.50
J	TMWW-t	MT20	5.0	6.0	2.50 2.00
K	TMV+p	MT20	3.0	4.0	
M	BMVW-t	MT20	5.0	6.0	2.50 2.75
N	O.S.T.				
N	BMWW-t	MT20	5.0	6.0	
P	BS-t	MT20	5.0	6.0	
Q	BMWW-t	MT20	5.0	6.0	
R	BS-t	MT20	5.0	6.0	
U	BMVW-t	MT20	5.0	6.0	2.50 2.75

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD. BRG
JT	VERT	DOWN	BRG
U	2383	0	5-8
M	2383	0	5-8

UNFACTORED REACTIONS			
	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
U	1883	1118 / 0	0 / 0
M	1883	1118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 41	-91.8	10.00
B-C	0 / 20	-91.8	10.00
C-D	-2451 / 0	-91.8	4.21
D-E	-3187 / 0	-91.8	4.43
E-F	-3519 / 0	-91.8	4.21
F-G	-3519 / 0	-91.8	4.21
G-H	-3519 / 0	-91.8	4.21
H-I	-3187 / 0	-91.8	4.43
I-J	-2451 / 0	-91.8	4.21
J-K	0 / 20	-91.8	10.00
K-L	0 / 41	-91.8	10.00
U-B	-244 / 0	0.0	7.81
M-K	-244 / 0	0.0	7.81
U-T	0 / 1728	-18.5	10.00
T-S	0 / 1874	-18.5	10.00
S-R	0 / 3137	-18.5	10.00
R-Q	0 / 3137	-18.5	10.00
Q-P	0 / 3137	-18.5	10.00
P-O	0 / 3137	-18.5	10.00
O-N	0 / 1874	-18.5	10.00
N-M	0 / 1728	-18.5	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 81.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.38")
CALCULATED VERT. DEFL. (LL) = L/999 (0.16")
ALLOWABLE DEFL. (TL) = L/800 (1.38")
CALCULATED VERT. DEFL. (TL) = L/999 (0.29")

CSI: TC=0.40/1.00 (E-F:1), BC=0.42/1.00 (Q-S:1), WB=0.84/1.00 (H-O:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (D) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

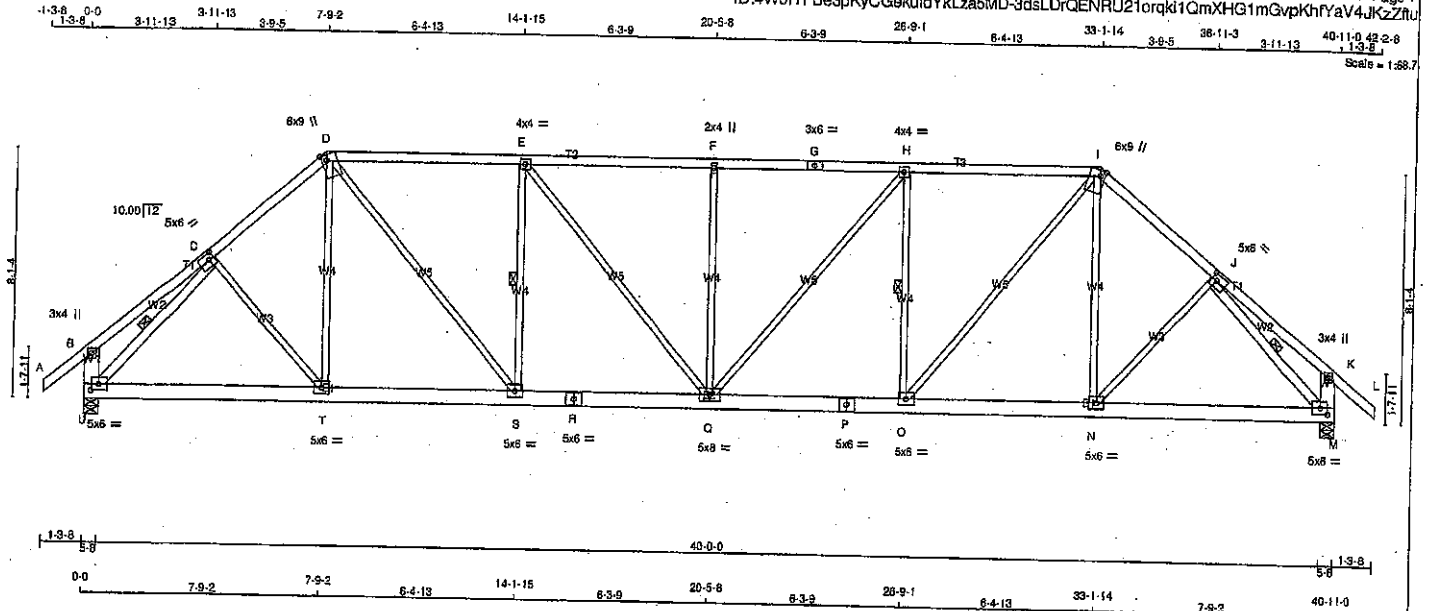
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (J) (INPUT = 0.80)
JSI METAL = 0.61 (C) (INPUT = 1.00)



Structural component only
DWG# T-2107824

JOB NAME 417277	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Mar 18 16:07:33 2021 Page 1 ID:4W01YFBe3pKyCGekuidYkLza5MD-3dsLRQENRU21orqki1QmXHG1mGvpKhYv4JKZ7Ru		



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
L - M	2x6	DRY	No.2
M - K	2x6	DRY	No.2
K - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
U - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMV+P	MT20	3.0	4.0
C TMVW+I	MT20	5.0	6.0 2.50 2.00
D TMVW+M	MT20	5.0	9.0 Edge 1.75
E TMVW+I	MT20	4.0	4.0
F TMVW+M	MT20	2.0	4.0
G TS+I	MT20	3.0	6.0
H TMVW+I	MT20	4.0	4.0
I TMVW+M	MT20	6.0	9.0 Edge 1.75
J TMVW+I	MT20	5.0	6.0 2.50 2.00
K TMV+P	MT20	3.0	4.0
M BMVW+I	MT20	5.0	6.0 2.50 2.75
N, O, S, T			
N BMVW+I	MT20	5.0	6.0
P BS+I	MT20	5.0	6.0
Q BMVW+I	MT20	5.0	6.0
R BS+I	MT20	5.0	6.0
U BMVW+I	MT20	5.0	6.0 2.50 2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP	IN-SX	IN-SX
U	2383	0	2383	0	5-8
M	2383	0	2383	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1883	1118 / 0	0 / 0	0 / 0	0 / 0	565 / 0	0 / 0
M	1883	1118 / 0	0 / 0	0 / 0	0 / 0	565 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, H-O, C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	C-T	0 / 94
B-C	0 / 25	T-D	0 / 112
C-D	-2442 / 0	D-S	0 / 1415
D-E	-3017 / 0	S-E	-976 / 0
E-F	-3017 / 0	F-Q	0 / 412
F-G	-3017 / 0	Q-F	-532 / 0
G-H	-3017 / 0	H-Q	0 / 412
H-I	-2758 / 0	I-J	-976 / 0
I-J	-2442 / 0	J-I	0 / 1415
J-K	0 / 25	K-I	0 / 112
K-L	0 / 41	L-N	0 / 94
L-M	-264 / 0	N-J	0 / 94
M-K	-264 / 0	J-M	-2727 / 0
U-T	0 / 1788		
T-S	0 / 1853		
S-R	0 / 2757		
R-Q	0 / 2757		
Q-P	0 / 2757		
P-O	0 / 2757		
O-N	0 / 1853		
N-M	0 / 1788		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = U/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = U/999 (0.29")

CSI: TC=0.87/1.00 (F-H:1), BC=0.37/1.00 (Q-S:1), WS=0.63/1.00 (F-Q:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

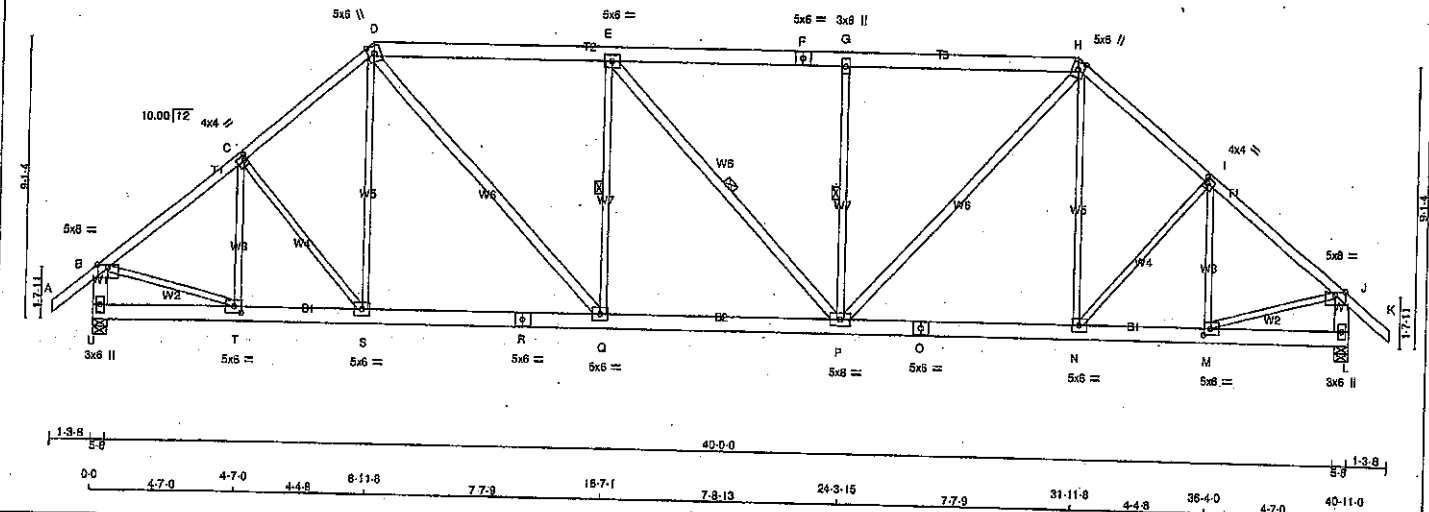
JSI GRIP = 0.87 (D) (INPUT = 0.90)

JSI METAL = 0.61 (C) (INPUT = 1.00)



Structural component only
DWG# T-2107825

JOB NAME 417277	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Mar 18 16:07:34 2021 Page 1	
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LUMBER	N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
CHORDS					
A - D	2x4	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
U - J	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
P - H	2x4	DRY	No.2	SPF	
D - Q	2x4	DRY	No.2	SPF	
E - P	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B - TMVW-p	MT20	5.0	8.0	Edge	
C - TMVW-t	MT20	4.0	4.0	2.00	1.25
D - TTWW+m	MT20	5.0	6.0	Edge	
E - TMVW-t	MT20	5.0	6.0		
F - TS-t	MT20	5.0	6.0		
G - TMVW-w	MT20	3.0	6.0		
H - TTWW+m	MT20	5.0	6.0	Edge	
I - TMVW-t	MT20	4.0	4.0	2.00	1.25
J - TMVW-p	MT20	5.0	8.0	Edge	
L - BMV1+p	MT20	3.0	8.0		
M - BMVW-t	MT20	5.0	6.0	2.50	2.75
N, Q, S					
N - BMVW-t	MT20	5.0	6.0		
O - BS-t	MT20	5.0	6.0		
P - BMVW-t	MT20	5.0	8.0		
R - BS-t	MT20	5.0	6.0		
T - BMVW-t	MT20	5.0	6.0	2.50	2.75
U - BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
VERT	HORZ	DOWN	HORZ
JT	2383	0	0
U	2383	0	0
L	2383	0	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS	DEAD	SOIL
COMBINED	SNOW	LIVE	PERM.LIVE	WIND
JT	1883	1118 / 0	0 / 0	0 / 0
U	1883	1118 / 0	0 / 0	0 / 0
L	1883	1118 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P, E-Q, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 41	-91.8	-91.8	0.13 (1)	10.00	T-C	-445 / 0	0.19 (1)
B-C	-2388 / 0	-91.8	-91.8	0.45 (1)	4.05	C-S	-42 / 0	0.03 (1)
C-D	-2408 / 0	-91.8	-91.8	0.44 (1)	4.03	S-D	0 / 168	0.05 (4)
D-E	-2643 / 0	-91.8	-91.8	0.42 (1)	4.68	N-H	0 / 170	0.05 (4)
E-F	-2638 / 0	-91.8	-91.8	0.43 (1)	4.68	N-I	-40 / 0	0.03 (1)
F-G	-2638 / 0	-91.8	-91.8	0.43 (1)	4.68	M-I	-447 / 0	0.19 (1)
G-H	-2639 / 0	-91.8	-91.8	0.41 (1)	4.69	B-T	0 / 1922	0.43 (1)
H-I	-2410 / 0	-91.8	-91.8	0.44 (1)	4.03	M-J	0 / 1922	0.43 (1)
I-J	-2387 / 0	-91.8	-91.8	0.45 (1)	4.05	P-H	0 / 1225	0.20 (1)
J-K	0 / 41	-91.8	-91.8	0.13 (1)	10.00	D-Q	0 / 1232	0.20 (1)
U-B	-2333 / 0	0.0	0.0	0.16 (1)	6.73	P-G	-778 / 0	0.38 (1)
L-J	-2333 / 0	0.0	0.0	0.16 (1)	6.73	Q-E	-778 / 0	0.38 (1)
						E-P	-6 / 0	0.00 (1)
U-T	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
T-S	0 / 1857	-18.5	-18.5	0.27 (1)	10.00			
S-R	0 / 1828	-18.5	-18.5	0.27 (1)	10.00			
R-Q	0 / 1828	-18.5	-18.5	0.27 (1)	10.00			
Q-P	0 / 2543	-18.5	-18.5	0.37 (1)	10.00			
P-O	0 / 1829	-18.5	-18.5	0.27 (1)	10.00			
O-N	0 / 1829	-18.5	-18.5	0.27 (1)	10.00			
N-M	0 / 1857	-18.5	-18.5	0.27 (1)	10.00			
M-L	0 / 0	-18.5	-18.5	0.05 (4)	10.00			

TOTAL WEIGHT = 2 X 239 = 478 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.35")
CALCULATED VERT. DEFL. (LL) = L/999 (0.11")
ALLOWABLE DEFL. (TL) = L/350 (1.35")
CALCULATED VERT. DEFL. (TL) = L/999 (0.20")

CSI: TC=0.45/1.00 (B-C:1), BC=0.37/1.00 (P-Q:1), WB=0.43/1.00 (S-T:1), SI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

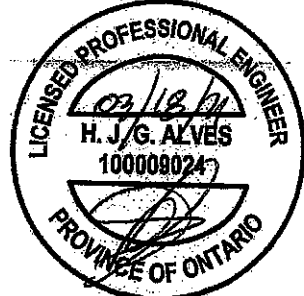
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

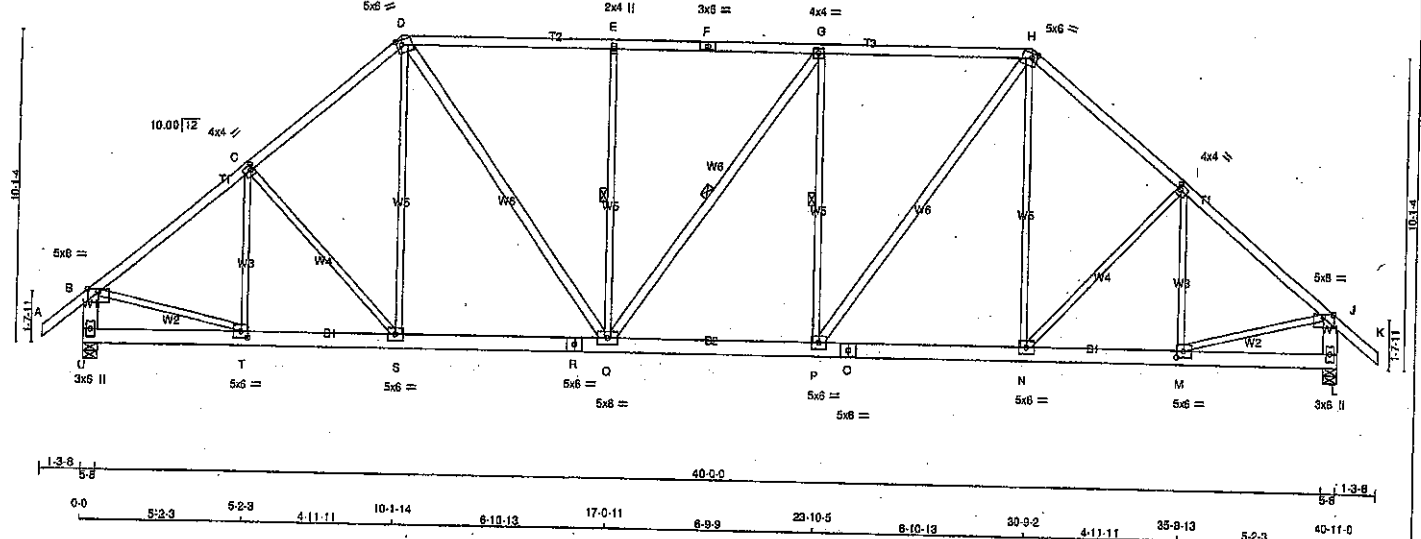
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.42 (T) (INPUT = 1.00)



Structural component only
DWG# T-2107826

JOB NAME 417277	TRUSS NAME T7	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 9 Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:07:35 2021 Page 1 ID:4W01YFBa3pKyCGekuldYkLza5MD-70z6eWRUv2kmH6?C73uryNcfaxBHG_you_AOCzZ7ts	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - Q	2x4	DRY	No.2
Q - G	2x4	DRY	No.2
P - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	5.0	8.0	Edge
C TMVW-t	MT20	4.0	4.0	2.00 1.25
D TTWW-m	MT20	5.0	5.0	1.75 2.00
E TMVW-w	MT20	2.0	4.0	
F TS-t	MT20	3.0	8.0	
G TMVW-t	MT20	4.0	4.0	
H TTWW-m	MT20	5.0	8.0	1.75 2.00
I TMVW-t	MT20	4.0	4.0	2.00 1.25
J TMVW-p	MT20	5.0	8.0	Edge
L BMV1+p	MT20	3.0	8.0	
M BMVW-t	MT20	5.0	6.0	2.50 2.75
N, P, S				
N BMVW-t	MT20	5.0	6.0	
O BS-t	MT20	5.0	6.0	
Q BMVW-t	MT20	5.0	8.0	
R BS-t	MT20	5.0	6.0	
T BMVW-t	MT20	5.0	8.0	2.50 2.75
U BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
U	2383	0	2383	0	5-8
L	2383	0	2383	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
U	1683	1118/0	0/0
L	1683	1118/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 3.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, G-Q, G-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)
MEMB.	FR-TO					MEMB.		
A-B	0/41	-91.8	-91.8	0.13 (1)	10.00	T-C	-370/0	0.19 (1)
B-C	-2426/0	-91.8	-91.8	0.81 (1)	3.87	C-S	-165/0	0.18 (1)
C-D	-2357/0	-91.8	-91.8	0.58 (1)	3.92	S-D	0/253	0.08 (1)
D-E	-2352/0	-91.8	-91.8	0.85 (1)	3.53	D-Q	0/985	0.16 (1)
E-F	-2352/0	-91.8	-91.8	0.85 (1)	3.52	Q-E	-681/0	0.46 (1)
F-G	-2352/0	-91.8	-91.8	0.85 (1)	3.52	G-Q	-6/0	0.01 (1)
G-H	-2352/0	-91.8	-91.8	0.85 (1)	3.51	P-G	-678/0	0.45 (1)
H-I	-2352/0	-91.8	-91.8	0.85 (1)	3.92	P-H	0/993	0.16 (1)
I-J	-2427/0	-91.8	-91.8	0.81 (1)	3.86	N-H	0/246	0.08 (4)
J-K	0/41	-91.8	-91.8	0.13 (1)	10.00	N-I	-167/0	0.18 (1)
U-B	-2332/0	0.0	0.0	0.16 (1)	8.73	M-I	-368/0	0.19 (1)
L-J	-2332/0	0.0	0.0	0.16 (1)	6.73	B-T	0/1943	0.44 (1)
						M-J	0/1943	0.44 (1)
U-T	0/0	-18.5	-18.5	0.06 (4)	10.00			
T-S	0/1891	-18.5	-18.5	0.27 (1)	10.00			
S-R	0/1782	-18.5	-18.5	0.25 (1)	10.00			
R-Q	0/1782	-18.5	-18.5	0.25 (1)	10.00			
Q-P	0/2356	-18.5	-18.5	0.32 (1)	10.00			
P-O	0/1781	-18.5	-18.5	0.25 (1)	10.00			
O-N	0/1781	-18.5	-18.5	0.25 (1)	10.00			
N-M	0/1891	-18.5	-18.5	0.26 (1)	10.00			
M-L	0/0	-18.5	-18.5	0.06 (4)	10.00			

TOTAL WEIGHT = 5 X 233 = 1165 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 33.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.88/1.00 (G-H:1), BC=0.32/1.00 (P-Q:1), WB=0.48/1.00 (E-Q:1), SS=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

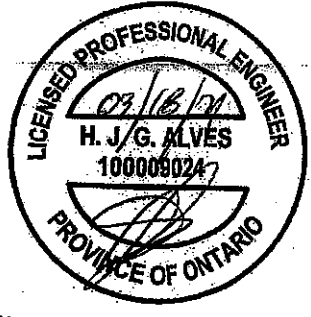
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

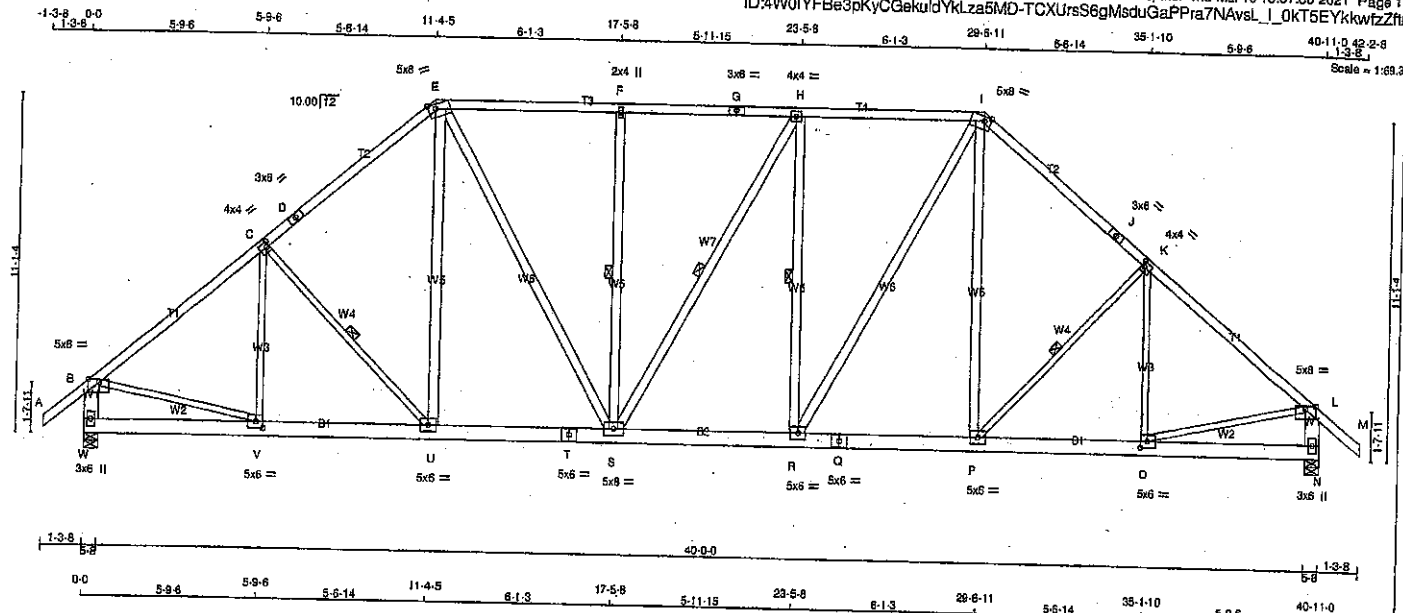
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.67 (H) (INPUT = 0.90)
JSI METAL=0.47 (C) (INPUT = 1.00)



Structural component only
DWG# T-2107827

JOB NAME 417277	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:07:38 2021 Page 1 ID:W01YFBe3pKyCGekuldYKza5MD-TCXUrsS6gMsdGPPra7NAvsL_1_0kT5EYkwtzZftr	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
G - I	2x4 DRY	No.2	SPF		
I - J	2x4 DRY	No.2	SPF		
J - M	2x4 DRY	No.2	SPF		
W - B	2x6 DRY	No.2	SPF		
N - L	2x6 DRY	No.2	SPF		
W - T	2x6 DRY	No.2	SPF		
T - Q	2x6 DRY	No.2	SPF		
Q - N	2x6 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x4 DRY	No.2	SPF		
V - C	2x3 DRY	No.2	SPF		
C - U	2x3 DRY	No.2	SPF		
P - K	2x3 DRY	No.2	SPF		
O - K	2x3 DRY	No.2	SPF		
B - V	2x3 DRY	No.2	SPF		
O - L	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT TYPE	MT20	5.0	8.0	Edge	
B TMVW-p	MT20	4.0	4.0	2.00	1.25
C TMVW-l	MT20	4.0	4.0		
D, G, J					
E TS-I	MT20	3.0	6.0		
F TTWW-m	MT20	5.0	8.0	1.75	2.75
G TTWW-w	MT20	2.0	4.0		
H TTWW-l	MT20	4.0	4.0		
I TTWW-m	MT20	5.0	8.0	1.75	2.75
K TTWW-l	MT20	4.0	4.0	2.00	1.25
L TMVW-p	MT20	5.0	8.0	Edge	
N BMV1-p	MT20	3.0	6.0		
O BMVW-l	MT20	5.0	6.0	2.50	2.75
P, R, U					
Q BMVW-l	MT20	5.0	8.0		
R BS-I	MT20	5.0	8.0		
S BMVW-l	MT20	5.0	8.0		
T BS-I	MT20	5.0	8.0		
V BMVW-l	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
W	2383	0	5-8	5-8
N	2383	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
W	1683	1118 / 0	0 / 0	0 / 0	565 / 0	0 / 0
N	1683	1118 / 0	0 / 0	0 / 0	565 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-U, F-S, H-S, H-R, K-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO		
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	V-C	-308 / 0	0.19 (1)
B-C	-2452 / 0	-91.8 -91.8 0.55 (1)	3.91	C-U	-267 / 0	0.12 (1)
C-D	-2298 / 0	-91.8 -91.8 0.51 (1)	4.05	U-E	0 / 319	0.05 (1)
D-E	-2298 / 0	-91.8 -91.8 0.51 (1)	4.05	E-S	0 / 811	0.13 (1)
E-F	-2136 / 0	-91.8 -91.8 0.47 (1)	4.18	S-F	-600 / 0	0.37 (1)
F-G	-2136 / 0	-91.8 -91.8 0.43 (1)	4.24	S-H	-6 / 0	0.01 (1)
G-H	-2136 / 0	-91.8 -91.8 0.43 (1)	4.24	H-R	-598 / 0	0.37 (1)
H-I	-2136 / 0	-91.8 -91.8 0.47 (1)	4.18	R-I	0 / 818	0.13 (1)
I-J	-2297 / 0	-91.8 -91.8 0.51 (1)	4.05	P-I	0 / 313	0.05 (1)
J-K	-2297 / 0	-91.8 -91.8 0.51 (1)	4.05	P-K	-269 / 0	0.12 (1)
K-L	-2452 / 0	-91.8 -91.8 0.55 (1)	3.91	O-K	-308 / 0	0.19 (1)
L-M	0 / 41	-91.8 -91.8 0.13 (1)	10.00	B-V	0 / 1957	0.44 (1)
W-B	-2330 / 0	0.0 0.0 0.16 (1)	6.73	O-L	0 / 1957	0.44 (1)
N-L	-2330 / 0	0.0 0.0 0.16 (1)	6.73			
W-V	0 / 0	-18.5 -18.5 0.07 (4)	10.00			
U-U	0 / 1914	-18.5 -18.5 0.27 (1)	10.00			
V-U	0 / 1735	-18.5 -18.5 0.25 (1)	10.00			
T-S	0 / 1735	-18.5 -18.5 0.25 (1)	10.00			
S-R	0 / 2139	-18.5 -18.5 0.29 (1)	10.00			
R-Q	0 / 1734	-18.5 -18.5 0.24 (1)	10.00			
Q-P	0 / 1734	-18.5 -18.5 0.24 (1)	10.00			
P-O	0 / 1915	-18.5 -18.5 0.26 (1)	10.00			
O-N	0 / 0	-18.5 -18.5 0.07 (4)	10.00			

TOTAL WEIGHT = 2 X 256 = 510 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 21.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.19")

CSI: TC=0.55/1.00 (K-L-1), BC=0.29/1.00 (R-S-1), WB=0.44/1.00 (L-C-1), SS=0.26/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.80)
JSI METAL = 0.44 (O) (INPUT = 1.00)



Structural component only
DWG# T-2107828

CONTINUED ON PAGE 2

JOB NAME 417277	TRUSS NAME T8	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Thu Mar 18 16:07:36 2021 Page 2
ID:4W0IYFB83pKvCGekIdYKLza5MD-TCXUrsS6qMsduGaPPra7NAvsl I 0KTSEYkkwfzZty

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
W BMV1+p MT20 3.0 6.0

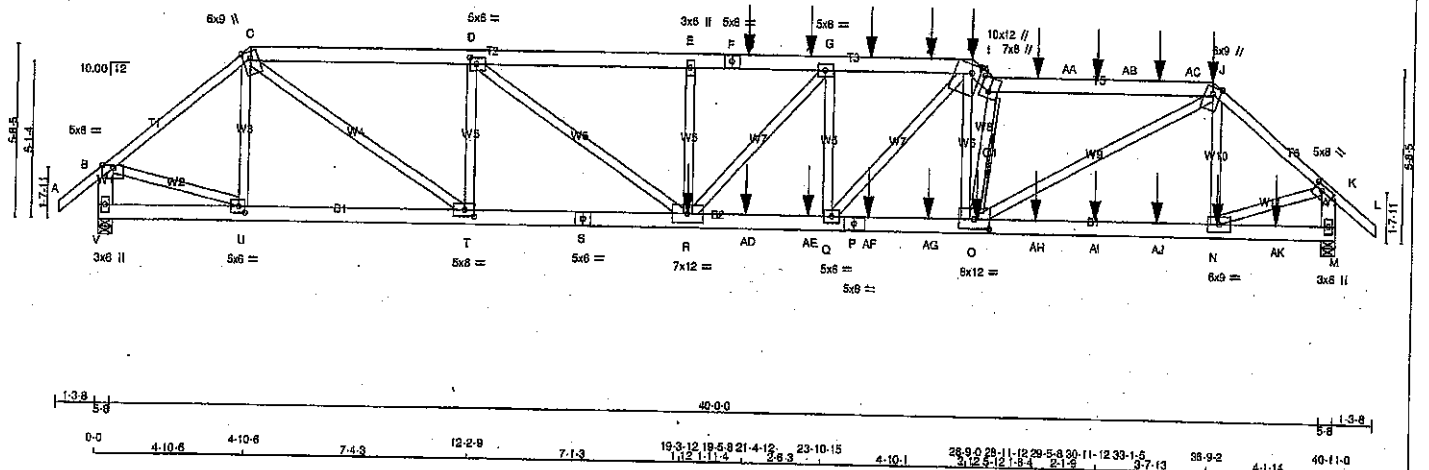
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2107828 *ML*

JOB NAME 417277	TRUSS NAME T9	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 M Tek Industries, Inc. Thu Mar 18 16:07:38 2021 Page 1	
ID: 4W0IYFBe3pKyCGekuldYkLza5MD-PbIEGYUNCz6L8ZknWGcbTb?AKnwBUVMOIsDr_XzZt					



TOTAL WEIGHT = 2 X 244 = 487 b

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - I	2x6	DRY	No.2
I - J	2x6	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	1650F 1.5E
S - P	2x6	DRY	1650F 1.5E
P - M	2x6	DRY	1650F 1.5E
ALL WEBS	2x4	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	TOP SIDE(61.0)
J-L	12	TOP
C-F	12	TOP
F-H	12	TOP
H-I	12	TOP
I-J	12	TOP
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
V-S	12	TOP
S-P	12	TOP
P-M	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x4	8	TOP
E-R	5	TOP
		SIDE(518.8)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
	VERT		DOWN		UP		IN-SX
V	4898	0	4898	0	0	5-8	5-8
M	6750	0	6750	0	0	5-8	5-8

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	1ST CASE	JT	1ST CASE	WIND	DEAD	WIND	DEAD	SOIL	SOIL
V	3412	2288 / 0	0 / 0	0 / 0	0 / 0	1124 / 0	0 / 0	0 / 0	0 / 0
M	4756	3221 / 0	0 / 0	0 / 0	0 / 0	1536 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 41	U-C	-928 / 0
B-C	-5325 / 0	R-E	-523 / 0
C-D	-9484 / 0	R-G	0 / 609
D-E	-13515 / 0	Q-G	-987 / 0
E-F	-13515 / 0	O-H	0 / 7589
F-W	-13515 / 0	O-I	-10336 / 0
W-X	-13515 / 0	N-J	-527 / 0
X-G	-13515 / 0	B-U	0 / 4234
G-Y	-13075 / 0	N-K	0 / 5946
Y-Z	-13075 / 0	O-J	0 / 8191
Z-H	-13075 / 0	Q-H	0 / 2477
H-I	-15495 / 0	D-R	0 / 4964
I-AA	-12754 / 0	C-T	0 / 6602
AA-AB	-12754 / 0	T-D	-3705 / 0
AB-AC	-12754 / 0		
AC-J	-12754 / 0		
J-K	-7397 / 0		
K-L	0 / 41		
V-B	-4810 / 0		
M-K	-6763 / 0		
V-U	0 / 0		
U-T	0 / 4124		
T-S	0 / 9484		
S-R	0 / 9484		
R-AD	0 / 13075		
AD-AE	0 / 13075		
AE-Q	0 / 13075		
Q-P	0 / 11410		
P-AF	0 / 11410		
AF-AG	0 / 11410		
AG-O	0 / 11410		
O-AH	0 / 5704		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 5.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.39")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/755 (0.65")

CSI: TO=0.65/1.00 (I-J), BC=0.62/1.00 (Q-R), WB=0.59/1.00 (I-O), SS=0.19/1.00 (N-O)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (T) (INPUT = 0.90)

JSI METAL= 0.87 (F) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2107829 1/2

JOB NAME 417277	TRUSS NAME T9	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 18:07:58 2021 Page 2 ID:4W0IYFBe3pKyCGekuldyKLza5MD-PbIEGYUNCz6L8ZknWGbTb?AKnvBLUVMQIsDr XzZfnc		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	10.0	12.0	Edge	3.25
I	TTW+m	MT20	7.0	8.0	5.50	3.25
J	TTWW+m	MT20	6.0	8.0	Edge	
K	TMVW-t	MT20	5.0	8.0	2.00	3.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	6.0	9.0		
O	BMWW-t	MT20	8.0	12.0	3.75	6.00
P	BS-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	7.0	12.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	8.0	2.50	3.75
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
AH-AI	0/5704	-18.5	-18.5	0.43 (1)			
AI-AJ	0/5704	-18.5	-18.5	0.43 (1)			
AJ-N	0/5704	-18.5	-18.5	0.43 (1)			
N-AK	0/0	-18.5	-18.5	0.20 (1)			
AK-M	0/0	-18.5	-18.5	0.20 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	28-9-0	-58	-58	---	FRONT	VERT	DEAD	C1
H	28-9-0	-59	-59	---	BACK	VERT	TOTAL	C1
H	28-9-0	-247	-247	---	FRONT	VERT	SNOW	C1
J	36-9-2	-58	-58	---	FRONT	VERT	DEAD	C1
J	36-9-2	-40	-40	---	BACK	VERT	TOTAL	C1
J	36-9-2	-247	-247	---	FRONT	VERT	SNOW	C1
N	36-11-12	-195	-195	---	BACK	VERT	TOTAL	C1
O	28-11-12	-195	-195	---	BACK	VERT	TOTAL	C1
R	19-5-6	-1898	-1898	---	BACK	VERT	TOTAL	C1
W	21-4-12	-35	-35	---	BACK	VERT	TOTAL	C1
X	23-4-12	-35	-35	---	BACK	VERT	TOTAL	C1
Y	25-4-12	-35	-35	---	BACK	VERT	TOTAL	C1
Z	27-4-12	-35	-35	---	BACK	VERT	TOTAL	C1
AA	30-11-12	-35	-35	---	BACK	VERT	TOTAL	C1
AB	32-11-12	-35	-35	---	BACK	VERT	TOTAL	C1
AC	34-11-12	-35	-35	---	BACK	VERT	TOTAL	C1
AD	21-4-12	-195	-195	---	BACK	VERT	TOTAL	C1
AE	23-4-12	-195	-195	---	BACK	VERT	TOTAL	C1
AF	25-4-12	-195	-195	---	BACK	VERT	TOTAL	C1
AG	27-4-12	-195	-195	---	BACK	VERT	TOTAL	C1
AH	30-11-12	-195	-195	---	BACK	VERT	TOTAL	C1
AI	32-11-12	-195	-195	---	BACK	VERT	TOTAL	C1
AJ	34-11-12	-195	-195	---	BACK	VERT	TOTAL	C1
AK	36-11-12	-195	-195	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

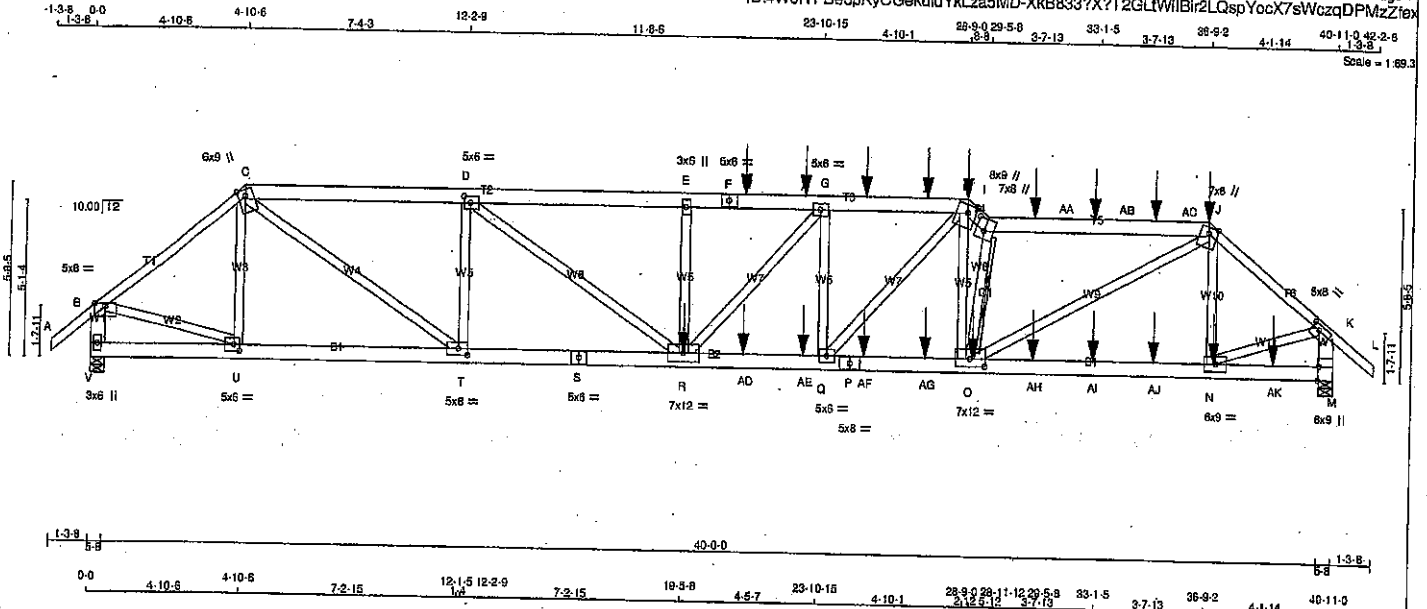


Structural component only
DWG# T-2107829 *7/2*

JOB NAME 417278	TRUSS NAME T9Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:23:30 2021 Page 1
ID:4W0YFBa3pKyCGekuldYkLza5MD-Xk8B33?X?T2GLtWIBir2LQspYocX7sWczqDPMzZfex
Scale = 1/8"=1'-0"



TOTAL WEIGHT = 2 X 244 = 487 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - I	2x6	DRY	No.2
I - J	2x6	DRY	No.2
J - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	1650F 1.5E
S - P	2x6	DRY	1650F 1.5E
P - M	2x6	DRY	1650F 1.5E
ALL WEBS EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
J-L	12	SIDE(61.0)
C-F	12	TOP
F-H	12	SIDE(61.0)
H-I	12	SIDE(61.0)
I-J	12	SIDE(61.0)
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S	12	TOP
S-P	12	SIDE(183.1)
P-M	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	SIDE(591.7)
E-R	5	SIDE(591.7)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	4984	0	4984	0	5-8	5-8
V	6882	0	6882	0	5-8	5-8
M						

UNFACTORED REACTIONS

JT	1ST LOOSE		MAX/MIN. COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
V	3515	2362 / 0	0 / 0	0 / 0	0 / 0	1153 / 0
M	4848	3287 / 0	0 / 0	0 / 0	0 / 0	1561 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT I-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO (LBS)
FR-TO			FR-TO		
A-B	0 / 41	-91.8 -91.8 0.07 (1)	U-C	-961 / 0	0.16 (1)
B-C	-5499 / 0	-91.8 -91.8 0.42 (1)	R-E	-521 / 0	0.08 (1)
C-D	-9825 / 0	-91.8 -91.8 0.39 (1)	R-G	0 / 837	0.07 (1)
D-E	-14055 / 0	-91.8 -91.8 0.60 (1)	Q-G	-1112 / 0	0.18 (1)
E-F	-14055 / 0	-91.8 -91.8 0.41 (1)	O-H	0 / 7710	0.68 (1)
F-W	-14055 / 0	-91.8 -91.8 0.41 (1)	O-I	-1118 / 0	0.71 (1)
W-X	-14055 / 0	-91.8 -91.8 0.41 (1)	N-J	-580 / 0	0.07 (1)
X-G	-14055 / 0	-91.8 -91.8 0.41 (1)	B-U	0 / 4373	0.39 (1)
G-Y	-13504 / 0	-91.8 -91.8 0.41 (1)	N-K	0 / 8089	0.54 (1)
Y-Z	-13504 / 0	-91.8 -91.8 0.41 (1)	O-J	0 / 8449	0.75 (1)
Z-H	-13504 / 0	-91.8 -91.8 0.41 (1)	Q-H	0 / 2656	0.23 (1)
H-I	-13504 / 0	-91.8 -91.8 0.41 (1)	D-R	0 / 5209	0.46 (1)
I-AA	-13096 / 0	-91.8 -91.8 0.66 (1)	C-T	0 / 6854	0.61 (1)
AA-AB	-13096 / 0	-91.8 -91.8 0.66 (1)	T-D	-3850 / 0	0.61 (1)
AB-AC	-13096 / 0	-91.8 -91.8 0.66 (1)			
AC-J	-13096 / 0	-91.8 -91.8 0.66 (1)			
J-K	-7541 / 0	-91.8 -91.8 0.44 (1)			
K-L	0 / 41	-91.8 -91.8 0.07 (1)			
V-B	-4957 / 0	0.0 0.0 0.18 (1)			
M-K	-6897 / 0	0.0 0.0 0.25 (1)			
V-U	0 / 0	-18.5 -18.5 0.03 (4)			
U-T	0 / 4260	-18.5 -18.5 0.19 (1)			
T-S	0 / 9825	-18.5 -18.5 0.45 (1)			
S-R	0 / 9825	-18.5 -18.5 0.45 (1)			
R-AD	0 / 13504	-18.5 -18.5 0.84 (1)			
AD-AE	0 / 13504	-18.5 -18.5 0.84 (1)			
AE-Q	0 / 13504	-18.5 -18.5 0.84 (1)			
Q-P	0 / 11719	-18.5 -18.5 0.56 (1)			
P-AF	0 / 11719	-18.5 -18.5 0.56 (1)			
AF-AG	0 / 11719	-18.5 -18.5 0.56 (1)			
AG-O	0 / 11719	-18.5 -18.5 0.56 (1)			
O-AH	0 / 5824	-18.5 -18.5 0.43 (1)			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
DL	6.0 PSF
BOT CH.	LL = 0.0 PSF
DL	7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.37")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = 1/730 (0.67")

CSI: TC=0.66/1.00 (N-I:1), BC=0.64/1.00 (Q-R:1),
WB=0.75/1.00 (J-O:1), SS=0.19/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY)	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	850	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (T) (INPUT = 0.90)

JSI METAL = 0.99 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2107845

JOB NAME 417278	TRUSS NAME T9Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:4W0YFB9pKyCGekuldYkLza5MD-Xk8833?X?T2GLWIIBir2LQspYocX7sWczqDPMzZlex

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	8.0	2.50	2.50
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	3.25	3.25
I	TTW+m	MT20	7.0	8.0	5.25	3.50
J	TTWW+m	MT20	7.0	8.0	Edge	
K	TMVW-t	MT20	5.0	8.0	2.00	3.25
M	BMV1-t	MT20	6.0	9.0	Edge	0.50
N	BMWW-t	MT20	6.0	9.0		
O	BMWW-t	MT20	7.0	12.0	3.00	6.00
P	BS-t	MT20	5.0	8.0		
Q	BMWW-t	MT20	5.0	8.0		
R	BMWW-t	MT20	7.0	12.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	8.0	2.50	3.50
U	BMWW-t	MT20	5.0	6.0	2.50	2.25
V	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

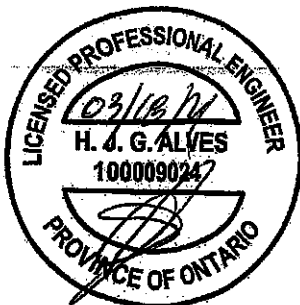
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC. LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
AF-AI	0/5824	-18.5	-18.5 0.43 (1)	10.00			
AI-AJ	0/5824	-18.5	-18.5 0.43 (1)	10.00			
AJ-N	0/5824	-18.5	-18.5 0.43 (1)	10.00			
N-AK	0/0	-18.5	-18.5 0.20 (1)	10.00			
AK-M	0/0	-18.5	-18.5 0.20 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	26-8-0	-58	-58	---	FRONT	VERT	DEAD	---	C1
H	26-8-0	-59	-59	---	BACK	VERT	TOTAL	---	C1
H	26-8-0	-247	-247	---	FRONT	VERT	SNOW	---	C1
J	38-8-2	-58	-58	---	FRONT	VERT	DEAD	---	C1
J	38-8-2	-40	-40	---	BACK	VERT	TOTAL	---	C1
J	38-8-2	-247	-247	---	FRONT	VERT	SNOW	---	C1
N	35-11-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
O	28-11-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
R	19-5-8	-2093	-2093	---	BACK	VERT	TOTAL	---	C1
W	21-4-12	-35	-35	---	BACK	VERT	TOTAL	---	C1
X	23-4-12	-35	-35	---	BACK	VERT	TOTAL	---	C1
Y	25-4-12	-35	-35	---	BACK	VERT	TOTAL	---	C1
Z	27-4-12	-35	-35	---	BACK	VERT	TOTAL	---	C1
AA	30-11-12	-35	-35	---	BACK	VERT	TOTAL	---	C1
AB	32-11-12	-35	-35	---	BACK	VERT	TOTAL	---	C1
AC	34-11-12	-35	-35	---	BACK	VERT	TOTAL	---	C1
AD	21-4-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
AE	23-4-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
AF	25-4-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
AG	27-4-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
AH	30-11-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
AI	32-11-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
AJ	34-11-12	-195	-195	---	BACK	VERT	TOTAL	---	C1
AK	38-11-12	-195	-195	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2107845 912

JOB NAME 417277	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:4W01YFB83pKyCGekuldYKLza5MD-InDcTuV?zHECijJz4z8q?oXGkBAzD4fXxWyOX_zzftc

Scale = 1:32.7

Dimensions (Top): 1-3-8, 1-3-8, 0-0, 4-5-0, 4-5-0, 1-11-4, 5-4-4, 4-0-2, 10-4-6, 5-11-6, 15-3-13, 3-0-3, 19-4-0

Dimensions (Bottom): 1-3-8, 1-5-8, 16-10-8, 0-0, 4-5-0, 4-5-0, 4-1-8, 8-5-0, 1-9-14, 10-4-6, 5-11-6, 15-3-13, 3-0-3, 16-4-0

Member Labels: T1, T2, T3, W1, W2, W3, W4, W5, W6, W7, W8, W9, W10, W11, W12, W13, W14, W15, W16, W17, W18, W19, W20, W21, W22, W23, W24, W25, W26, W27, W28, W29, W30, W31, W32, W33, W34, W35, W36, W37, W38, W39, W40, W41, W42, W43, W44, W45, W46, W47, W48, W49, W50, W51, W52, W53, W54, W55, W56, W57, W58, W59, W60, W61, W62, W63, W64, W65, W66, W67, W68, W69, W70, W71, W72, W73, W74, W75, W76, W77, W78, W79, W80, W81, W82, W83, W84, W85, W86, W87, W88, W89, W90, W91, W92, W93, W94, W95, W96, W97, W98, W99, W100, W101, W102, W103, W104, W105, W106, W107, W108, W109, W110, W111, W112, W113, W114, W115, W116, W117, W118, W119, W120, W121, W122, W123, W124, W125, W126, W127, W128, W129, W130, W131, W132, W133, W134, W135, W136, W137, W138, W139, W140, W141, W142, W143, W144, W145, W146, W147, W148, W149, W150, W151, W152, W153, W154, W155, W156, W157, W158, W159, W160, W161, W162, W163, W164, W165, W166, W167, W168, W169, W170, W171, W172, W173, W174, W175, W176, W177, W178, W179, W180, W181, W182, W183, W184, W185, W186, W187, W188, W189, W190, W191, W192, W193, W194, W195, W196, W197, W198, W199, W200, W201, W202, W203, W204, W205, W206, W207, W208, W209, W210, W211, W212, W213, W214, W215, W216, W217, W218, W219, W220, W221, W222, W223, W224, W225, W226, W227, W228, W229, W230, W231, W232, W233, W234, W235, W236, W237, W238, W239, W240, W241, W242, W243, W244, W245, W246, W247, W248, W249, W250, W251, W252, W253, W254, W255, W256, W257, W258, W259, W260, W261, W262, W263, W264, W265, W266, W267, W268, W269, W270, W271, W272, W273, W274, W275, W276, W277, W278, W279, W280, W281, W282, W283, W284, W285, W286, W287, W288, W289, W290, W291, W292, W293, W294, W295, W296, W297, W298, W299, W300, W301, W302, W303, W304, W305, W306, W307, W308, W309, W310, W311, W312, W313, W314, W315, W316, W317, W318, W319, W320, W321, W322, W323, W324, W325, W326, W327, W328, W329, W330, W331, W332, W333, W334, W335, W336, W337, W338, W339, W340, W341, W342, W343, W344, W345, W346, W347, W348, W349, W350, W351, W352, W353, W354, W355, W356, W357, W358, W359, W360, W361, W362, W363, W364, W365, W366, W367, W368, W369, W370, W371, W372, W373, W374, W375, W376, W377, W378, W379, W380, W381, W382, W383, W384, W385, W386, W387, W388, W389, W390, W391, W392, W393, W394, W395, W396, W397, W398, W399, W400, W401, W402, W403, W404, W405, W406, W407, W408, W409, W410, W411, W412, W413, W414, W415, W416, W417, W418, W419, W420, W421, W422, W423, W424, W425, W426, W427, W428, W429, W430, W431, W432, W433, W434, W435, W436, W437, W438, W439, W440, W441, W442, W443, W444, W445, W446, W447, W448, W449, W450, W451, W452, W453, W454, W455, W456, W457, W458, W459, W460, W461, W462, W463, W464, W465, W466, W467, W468, W469, W470, W471, W472, W473, W474, W475, W476, W477, W478, W479, W480, W481, W482, W483, W484, W485, W486, W487, W488, W489, W490, W491, W492, W493, W494, W495, W496, W497, W498, W499, W500, W501, W502, W503, W504, W505, W506, W507, W508, W509, W510, W511, W512, W513, W514, W515, W516, W517, W518, W519, W520, W521, W522, W523, W524, W525, W526, W527, W528, W529, W530, W531, W532, W533, W534, W535, W536, W537, W538, W539, W540, W541, W542, W543, W544, W545, W546, W547, W548, W549, W550, W551, W552, W553, W554, W555, W556, W557, W558, W559, W560, W561, W562, W563, W564, W565, W566, W567, W568, W569, W570, W571, W572, W573, W574, W575, W576, W577, W578, W579, W580, W581, W582, W583, W584, W585, W586, W587, W588, W589, W590, W591, W592, W593, W594, W595, W596, W597, W598, W599, W600, W601, W602, W603, W604, W605, W606, W607, W608, W609, W610, W611, W612, W613, W614, W615, W616, W617, W618, W619, W620, W621, W622, W623, W624, W625, W626, W627, W628, W629, W630, W631, W632, W633, W634, W635, W636, W637, W638, W639, W640, W641, W642, W643, W644, W645, W646, W647, W648, W649, W650, W651, W652, W653, W654, W655, W656, W657, W658, W659, W660, W661, W662, W663, W664, W665, W666, W667, W668, W669, W670, W671, W672, W673, W674, W675, W676, W677, W678, W679, W680, W681, W682, W683, W684, W685, W686, W687, W688, W689, W690, W691, W692, W693, W694, W695, W696, W697, W698, W699, W700, W701, W702, W703, W704, W705, W706, W707, W708, W709, W710, W711, W712, W713, W714, W715, W716, W717, W718, W719, W720, W721, W722, W723, W724, W725, W726, W727, W728, W729, W730, W731, W732, W733, W734, W735, W736, W737, W738, W739, W740, W741, W742, W743

LUMBER				
N.L.G.A. RULES				
CHORDS	SIZE		LUMBER	DESC
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-1	MT20	5.0	6.0	2.00	2.25
C	TTWV-w	MT20	5.0	6.0	2.00	2.25
D	TMWV-m	MT20	2.0	4.0		
E	TTWV+w	MT20	6.0	9.0	Edge	1.75
F	TWVW+p	MT20	5.0	6.0	Edge	
G	BMV1+ ^a	MT20	3.0	6.0		
H	GMWV-w	MT20	5.0	6.0		
I	BS ^a	MT20	5.0	6.0		
J	BMWVW-1	MT20	5.0	8.0	2.50	2.50
K	BMVW-1	MT20	5.0	6.0	2.50	2.75
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	2383	0	2383	0	0	5-8	5-8
G	1707	0	1707	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	1685	1105 / 0	0 / 0	0 / 0	0 / 0	580 / 0	0 / 0
G	1207	792 / 0	0 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: {4}

CHORDS				WEBBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (FLF)	MAX. CSF (LC)	MAX. UNSTRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 41	-81.8	-91.8	0.14 (1)	10.00	K-C	0 / 191
B-C	-2420 / 0	-91.8	-91.8	0.51 (7)	3.80	C-J	0 / 879
C-M	-2379 / 0	-91.8	-91.8	0.92 (1)	3.10	J-D	-740 / 0
D-E	-2379 / 0	-91.8	-91.8	0.92 (1)	3.10	J-E	0 / 1880
M-D	-2379 / 0	-91.8	-91.8	0.92 (1)	3.10	H-E	-747 / 0
E-F	-1237 / 0	-91.8	-91.8	0.18 (1)	5.54	B-K	0 / 1921
L-B	-2388 / 0	0.0	0.0	0.27 (1)	5.48	H-F	0 / 1192
G-F	-1659 / 0	0.0	0.0	0.25 (1)	6.39		
L-N	0 / 0	-18.5	-18.5	0.29 (1)	10.00		
N-O	0 / 0	-18.5	-18.5	0.29 (1)	10.00		
O-K	0 / 0	-18.5	-18.5	0.25 (1)	10.00		
K-P	0 / 1856	-18.5	-18.5	0.91 (1)	10.00		
P-Q	0 / 1856	-18.5	-18.5	0.91 (1)	10.00		
Q-J	0 / 1856	-18.5	-18.5	0.91 (1)	10.00		
J-I	0 / 931	-18.5	-18.5	0.44 (7)	10.00		
I-H	0 / 931	-18.5	-18.5	0.44 (1)	10.00		
H-G	0 / 0	-18.5	-18.5	0.05 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONIN.
C	4-5-0	-44	-44	--	FRONT	VERT	DEAD	--	C1
C	4-5-0	-128	-128	--	BACK	VERT	TOTAL	--	C1
C	4-5-0	-188	-188	--	FRONT	VERT	SNOW	--	C1
K	4-4-4	-29	-29	--	BACK	VERT	TOTAL	--	C1
M	8-4-4	-122	-122	--	BACK	VERT	TOTAL	--	C1
N	1-4-4	-29	-29	--	BACK	VERT	TOTAL	--	C1
O	2-4-4	-29	-29	--	BACK	VERT	TOTAL	--	C1
P	6-4-4	-29	-29	--	BACK	VERT	TOTAL	--	C1
Q	8-6-8	-701	-701	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **Ct:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD			=	39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 8.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.64")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.64")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.20")

CSI: TC=0.92/1.00 (C-D:1), BC=0.91/1.00 (J-K:1),
WB=0.48/1.00 (B-K:1), SSI=0.80/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	850	371	1747	785	1987 1873

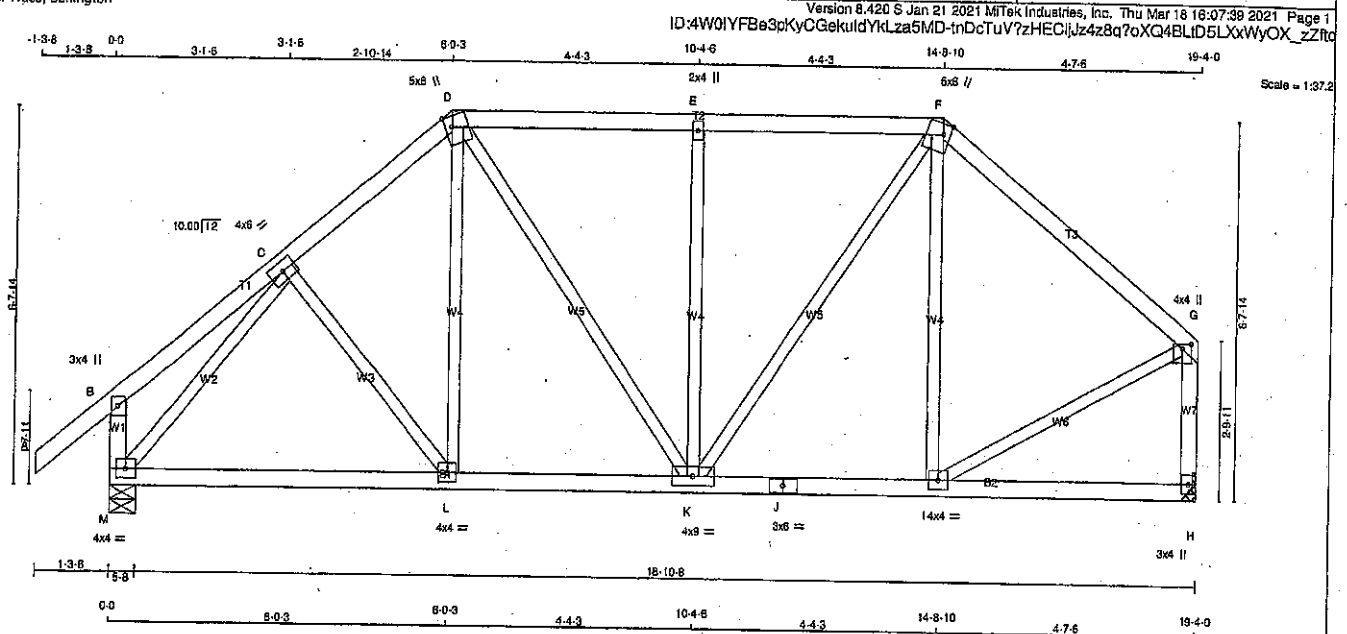
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.88 (B) (INPUT = 0.90)
ISI METAL= 0.60 (B) (INPUT = 1.00)

Structural component only
DWG# T-2107830

JOB NAME 417277	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 92 lb [M/F]

LUMBER	N.C.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
BT TMV+p MT20	3.0	4.0		
CT TMWW-t MT20	4.0	6.0		
DT TMWW+m MT20	5.0	6.0	2.25	1.50
ET TMWW-w MT20	2.0	4.0		
FT TMWW+m MT20	5.0	6.0	2.25	1.50
GT TMVV+p MT20	4.0	4.0	1.00	2.00
H BMV1+p MT20	3.0	4.0		
I BMWW-t MT20	4.0	4.0		
J BS-t MT20	3.0	6.0		
K BMWW-w MT20	4.0	9.0		
L BMWW-t MT20	4.0	4.0		
M BMVV1-t MT20	4.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
M	VERT	DOWN	IN-SX	IN-SX
H	HORZ	UPLIFT	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
M	841	586 / 0 0 / 0 0 / 0 0 / 0 278 / 0 0 / 0
H	754	495 / 0 0 / 0 0 / 0 0 / 0 259 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0 / 141	-91.8	-91.8	0.13 (1)	10.00	C-L	-20 / 30	0.01 (4)	
B-C	0 / 119	-91.8	-91.8	0.12 (1)	10.00	L-D	0 / 133	0.04 (4)	
C-D	-938 / 0	-91.8	-91.8	0.10 (1)	6.25	D-K	0 / 233	0.05 (1)	
D-E	-938 / 0	-91.8	-91.8	0.22 (1)	6.25	K-E	-405 / 0	0.35 (1)	
E-F	-938 / 0	-91.8	-91.8	0.22 (1)	6.25	K-F	0 / 425	0.10 (1)	
F-G	-785 / 0	-91.8	-91.8	0.26 (1)	6.25	I-F	-215 / 0	0.16 (1)	
M-B	-234 / 0	0.0	0.0	0.02 (1)	7.81	M-C	-1160 / 0	0.43 (1)	
H-G	-1031 / 0	0.0	0.0	0.15 (1)	7.73	I-G	0 / 678	0.15 (1)	
M-L	0 / 717	-18.5	-18.5	0.20 (4)	10.00				
L-K	0 / 707	-18.5	-18.5	0.21 (4)	10.00				
K-J	0 / 596	-18.5	-18.5	0.15 (1)	10.00				
J-I	0 / 596	-18.5	-18.5	0.15 (1)	10.00				
I-H	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ASB 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.64")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.64")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSL: TC=0.26/1.00 (F-G:1), BC=0.21/1.00 (K-L:4), WB=0.43/1.00 (C-M:1), SSL=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

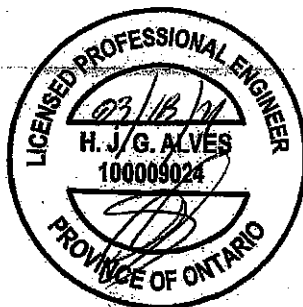
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

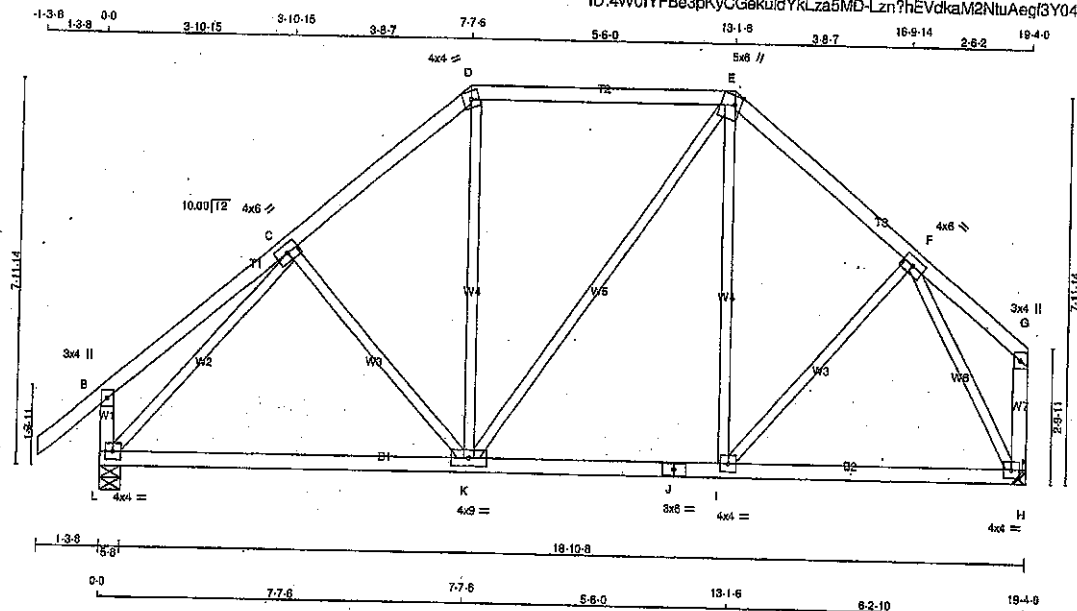
JSI GRIP = 0.87 (M) (INPUT = 0.90)
JSI METAL = 0.28 (C) (INPUT = 1.00)



Structural component only
DWG# T-2107831

JOB NAME 417277	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:43.7

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	6.0		
D TTW-m	MT20	4.0	4.0		
E TTWW+m	MT20	5.0	6.0	2.25	1.50
F TMVW-t	MT20	4.0	6.0		
G TMV+p	MT20	3.0	4.0		
H BMVW-t	MT20	4.0	4.0		
I BMVW-t	MT20	4.0	4.0		
J BS-t	MT20	3.0	6.0		
K BMVW-t	MT20	4.0	6.0		
L BMVW-t	MT20	4.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	1193	0	1193	0
H	1066	0	1066	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
L	841	566 / 0	0 / 0	0 / 0	278 / 0	0 / 0
H	754	485 / 0	0 / 0	0 / 0	259 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	WEBS	MAX. FACTORED	MAX
		FORCE						FORCE	
		(LBS)						(LBS)	
A-B	0 / 41						C-K	-137 / 0	0.08 (1)
B-C	0 / 26						K-D	0 / 157	0.04 (4)
C-D	-875 / 0						K-E	0 / 103	0.02 (1)
D-E	-853 / 0						E-F	0 / 87	0.02 (4)
E-F	-794 / 0						F-G	0 / 122	0.03 (1)
F-G	0 / 31						L-C	-1145 / 0	0.85 (1)
L-B	-260 / 0						F-H	-1082 / 0	0.45 (1)
H-G	-83 / 0								
L-K	0 / 741								
K-J	0 / 593								
J-I	0 / 593								
I-H	0 / 513								

TOTAL WEIGHT = 92 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TRG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.84")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.84")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.38/1.00 (D-E-1), BC=0.28/1.00 (K-L-4),
WB=0.55/1.00 (C-L-1), SS=0.20/1.00 (D-E-1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	850	371	1747
	789	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (L) (INPUT = 0.90)
JSI METAL = 0.26 (C) (INPUT = 1.00)



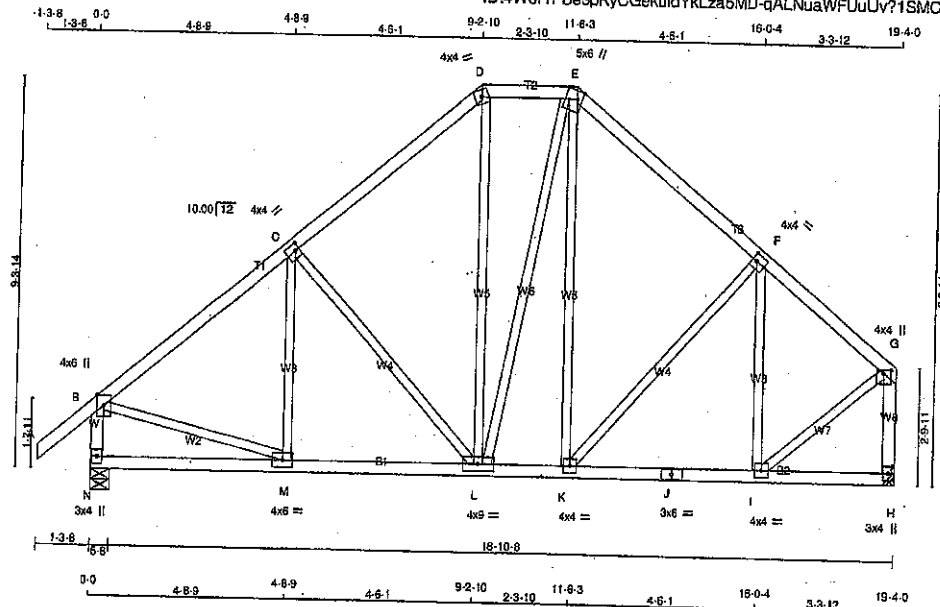
Structural component only
DWG# T-2107832

JOB NAME 417277	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:50.4



TOTAL WEIGHT = 104 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4 DRY	No.2		
D - E	2x4 DRY	No.2		
E - G	2x4 DRY	No.2		
N - B	2x4 DRY	No.2		
H - G	2x4 DRY	No.2		
N - J	2x4 DRY	No.2		
J - H	2x4 DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	Edge		
C TMVW-t	MT20	4.0	4.0	2.00	1.25	
D TJW-m	MT20	4.0	4.0			
E TTWV+m	MT20	5.0	6.0	2.25	1.50	
F TMVW-t	MT20	4.0	4.0	2.00	1.25	
G TMVW+p	MT20	4.0	4.0	1.00	2.00	
H BMV1+p	MT20	3.0	4.0			
I BMVW-t	MT20	4.0	4.0			
J BS-t	MT20	3.0	6.0			
K BMVW-t	MT20	4.0	4.0			
L BMVW-t	MT20	4.0	8.0			
M BMVW-t	MT20	4.0	6.0			
N BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	1193	0	1193	0	5-8	5-8
H	1088	0	1088	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	841	566 / 0	0 / 0	0 / 0	0 / 0	278 / 0	0 / 0
H	754	495 / 0	0 / 0	0 / 0	0 / 0	259 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 41	-91.8 -91.8 0.13 (1)	10.00	M-C	-118 / 39	0.06 (1)	
B-C	-981 / 0	-91.8 -91.8 0.26 (1)	5.98	C-L	-315 / 0	0.29 (1)	
C-D	-777 / 0	-91.8 -91.8 0.26 (1)	6.25	L-D	0 / 218	0.05 (1)	
D-E	-572 / 0	-91.8 -91.8 0.07 (1)	6.25	K-E	0 / 113	0.03 (4)	
E-F	-746 / 0	-91.8 -91.8 0.22 (1)	6.25	K-F	-71 / 0	0.07 (1)	
F-G	-746 / 0	-91.8 -91.8 0.20 (1)	6.25	I-F	-364 / 0	0.17 (1)	
N-B	-1157 / 0	0.0 0.0 0.12 (1)	7.40	B-M	0 / 807	0.18 (1)	
H-G	-1043 / 0	0.0 0.0 0.16 (1)	7.89	I-G	0 / 747	0.17 (1)	
				L-E	0 / 80	0.02 (1)	
N-M	0 / 0	-18.5 -18.5 0.10 (4)	10.00				
M-L	0 / 778	-18.5 -18.5 0.17 (1)	10.00				
L-K	0 / 553	-18.5 -18.5 0.12 (1)	10.00				
K-J	0 / 589	-18.5 -18.5 0.13 (1)	10.00				
J-I	0 / 599	-18.5 -18.5 0.13 (1)	10.00				
I-H	0 / 0	-18.5 -18.5 0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.9 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.28/1.00 (B-C:1), BC=0.17/1.00 (L-M:1), WB=0.29/1.00 (C-L:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

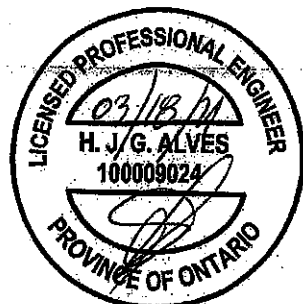
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.78 (B) (INPUT = 0.90)
JSI METAL = 0.50 (B) (INPUT = 1.00)

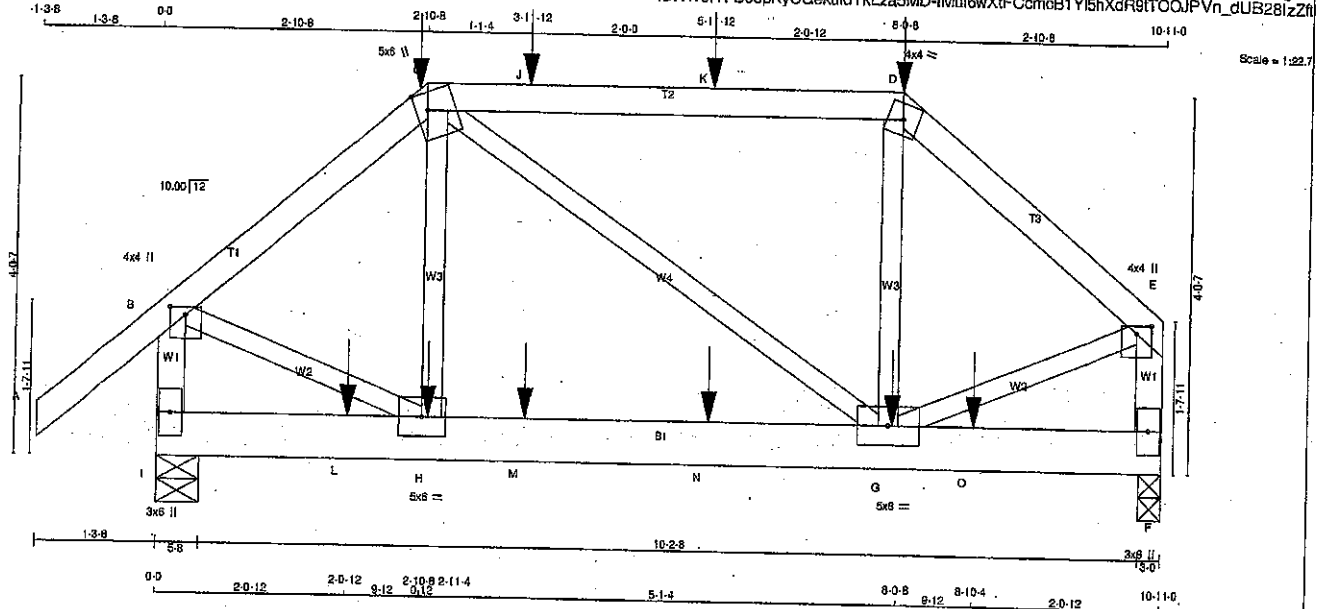


Structural component only
DWG# T-2107833

JOB NAME 417277	TRUSS NAME T14	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:07:42 2021 Page 1
ID:4W01YFBe3pKyCGekuldYkLza5MD-IMut6wXtFComeB1Y15hXdr9tTCQJPVn_dUB28Iz2f



Scale = 1:22.7

TOTAL WEIGHT = 53 lb

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - A	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	6.0		
G	BMVWV-t	MT20	5.0	8.0		
H	BMVWV-t	MT20	5.0	8.0		
I	BMV1+p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
I	897	0	5-8	5-8
F	761	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	COMBINED	SNOW	0.0	0.0	0.0	207.0	0.0
F	539	351.0	0.0	0.0	0.0	188.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO			
A-B	0/41	-91.8	-91.8	0.14 (1)	10.00	H-C	-95/50	0.02 (1)
B-C	-643/0	-91.8	-91.8	0.15 (1)	6.25	C-G	-10/0	0.01 (1)
C-D	-481/0	-91.8	-91.8	0.57 (1)	6.25	G-D	-93/49	0.02 (1)
D-E	-481/0	-91.8	-91.8	0.57 (1)	6.25	B-H	0/532	0.13 (1)
E-F	-632/0	-91.8	-91.8	0.15 (1)	6.25	G-E	0/523	0.13 (1)
F-G	-875/0	0.0	0.0	0.10 (1)	7.81			
G-H	-737/0	0.0	0.0	0.08 (1)	7.81			
H-I	0/0	-18.5	-18.5	0.04 (4)	10.00			
I-J	0/0	-18.5	-18.5	0.04 (4)	10.00			
J-K	0/480	-18.5	-18.5	0.10 (1)	10.00			
K-L	0/480	-18.5	-18.5	0.10 (1)	10.00			
L-M	0/480	-18.5	-18.5	0.10 (1)	10.00			
M-N	0/0	-18.5	-18.5	0.04 (4)	10.00			
N-O	0/0	-18.5	-18.5	0.04 (4)	10.00			
O-P	0/0	-18.5	-18.5	0.04 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-10-8	-68	-68	94	FRONT	VERT	TOTAL	---	C1
D	8-0-8	-68	-68	94	FRONT	VERT	TOTAL	---	C1
G	7-11-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
H	2-11-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
J	3-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
K	5-11-12	-28	-28	---	FRONT	VERT	TOTAL	---	C1
L	2-0-12	-7	-7	---	FRONT	VERT	TOTAL	---	C1
M	3-11-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
N	5-11-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
O	8-10-4	-7	-7	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD		= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TC=0.57/1.00 (C-D:1), BC=0.10/1.00 (G-H:1), WB=0.13/1.00 (B-H:1), SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

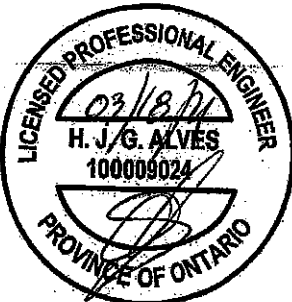
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

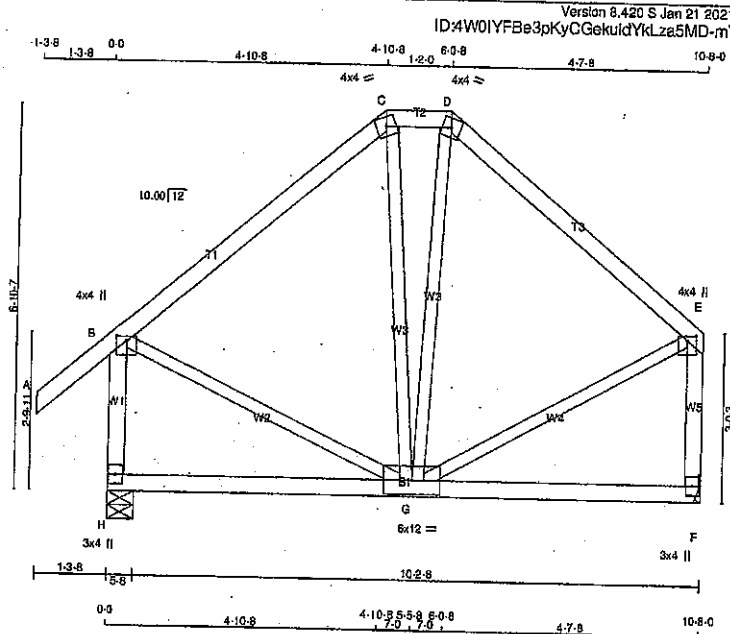
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.73 (B) (INPUT = 0.90)
JSI METAL = 0.20 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107834

JOB NAME 417277	TRUSS NAME T15	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMVW+p	MT20	4.0	4.0	1.00 2.00
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW+i	MT20	6.0	12.0	
H	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
H	715	0	715	0	0	5-8	5-8
F	588	0	588	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	503	344 / 0	0 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0
F	418	273 / 0	0 / 0	0 / 0	0 / 0	0 / 0	143 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

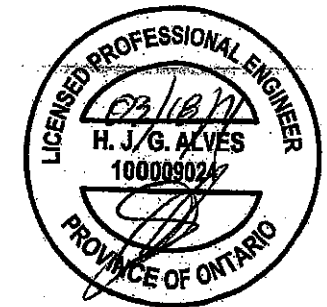
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 41	-91.8	-91.8 0.13 (1)	10.00	B-G	0 / 277	0.06 (1)
B-C	-329 / 0	-91.8	-91.8 0.28 (1)	6.25	G-E	0 / 282	0.06 (1)
C-D	-246 / 0	-91.8	-91.8 0.02 (1)	8.25	C-G	-66 / 25	0.05 (1)
D-E	-328 / 0	-91.8	-91.8 0.25 (1)	6.25	G-D	-55 / 27	0.04 (1)
E-B	-676 / 0	0.0	0.0 0.10 (1)	7.81			
F-E	-552 / 0	0.0	0.0 0.09 (1)	7.81			
H-G	0 / 0	-18.5	-18.5 0.15 (4)	10.00			
G-F	0 / 0	-18.5	-18.5 0.15 (4)	10.00			



Structural component only
DWG# T-2107835

TOTAL WEIGHT = 55 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.8 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TC=0.28/1.00 (B-C:1), BC=0.15/1.00 (G-H:4), WB=0.06/1.00 (E-G:1), SSI=0.13/1.00 (B-C:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI)	PLI	PLI
MAX MIN MAX MIN		
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

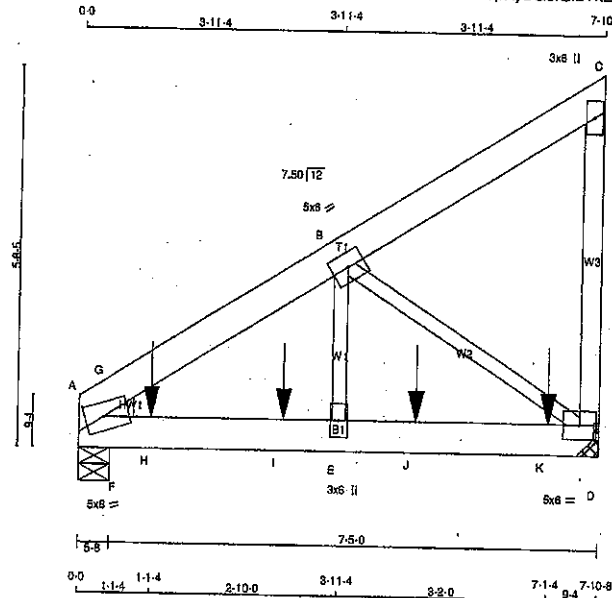
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.51 (B) (INPUT = 0.90)
JSI METAL=0.14 (B) (INPUT = 1.00)

JOB NAME 417277	TRUSS NAME T17	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:31.7

TOTAL WEIGHT = 2 X 42 = 84 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	TOP
C - D 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMBH1-m	MT20	5.0	8.0	2.50	3.25
B TMBW1	MT20	5.0	6.0		
C TMBW1	MT20	3.0	6.0		
D BMBW1-t	MT20	5.0	6.0		
E BMBW-w	MT20	3.0	6.0		

NOTES - (1)
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	HEEL
VERT					
HORZ					
UP/LIFT					
IN-SX					
OUT-SX					
MECHANICAL					
5-8					
2x4 L					

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS
COMBINED		
SNOW		
LIVE		
PERM. LIVE		
WIND		
DEAD		
SOIL		
1912	1274 / 0	0 / 0
2113	1404 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS-FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS-FT)
FR-TO						FR-TO			
A-G	-3755 / 0	-91.8	-91.8	0.14 (1)	5.88	E-B	0 / 2534	0.31 (1)	
G-B	-2645 / 0	-91.8	-91.8	0.13 (1)	6.25	B-D	-2807 / 0	0.50 (1)	
B-C	-18 / 0	-91.8	-91.8	0.05 (1)	6.25	F-G	0 / 1505	0.00 (1)	
D-C	-146 / 0	0.0	0.0	0.04 (1)	7.81				
A-F	0 / 2306	-18.5	-18.5	0.16 (1)	10.00				
F-H	0 / 2306	-18.5	-18.5	0.48 (1)	10.00				
H-I	0 / 2306	-18.5	-18.5	0.48 (1)	10.00				
I-E	0 / 2306	-18.5	-18.5	0.48 (1)	10.00				
E-J	0 / 2306	-18.5	-18.5	0.48 (1)	10.00				
J-K	0 / 2306	-18.5	-18.5	0.48 (1)	10.00				
K-D	0 / 2306	-18.5	-18.5	0.48 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	1-1-4	-1193	-1193		FRONT	VERT	TOTAL		C1
I	3-1-4	-739	-739		FRONT	VERT	TOTAL		C1
J	5-1-4	-739	-739		FRONT	VERT	TOTAL		C1
K	7-1-4	-741	-741		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ABC 2015
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = U/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = U/999 (0.04")

CSI: TC=0.14/1.00 (A-G:1), BC=0.48/1.00 (E-F:1), WB=0.50/1.00 (B-D:1), SS=0.49/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1887 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT= 0.90)
JSI METAL= 0.36 (E) (INPUT= 1.00)



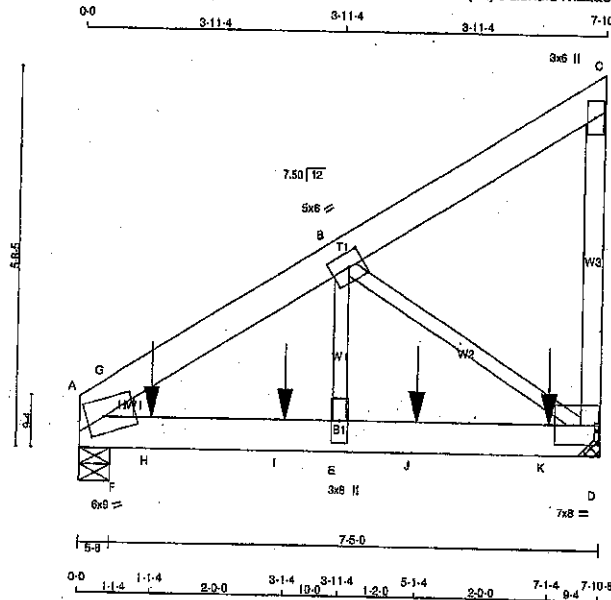
Structural component only
DWG# T-2107836

JOB NAME 417278	TRUSS NAME T17Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Rod Truss, Burlington

Version 8.420 S Jan 21 2021 M Tek Industries, Inc. Thu Mar 18 16:23:31 2021 Page 1
ID:4W01YFB3pkyCGekuldYkLza5MD-7wKHP09mnA7z14xrvD4bZz7Px8TGbSfdZmxozZfew

Scale = 1/31.7



LUMBER	N. L. B. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
A - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2 12	TOP	
C - D 1 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 2 12	SIDE (0.0)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - TMBH1-m	MT20	6.0	9.0	3.00	3.00	
B - TMWV1-t	MT20	5.0	6.0	2.50	2.75	
C - TMV1-p	MT20	3.0	6.0			
D - BMVW1-t	MT20	7.0	8.0	3.50	3.25	
E - BMW1-w	MT20	3.0	8.0			

NOTES: (1)



Structural component only
DWG# T-2107846

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL
JT	VERT	DOWN	HORIZ	UPLIFT	WEDGE
D	2988	0	0	MECHANICAL	
A	4244	0	0	5-8	2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	2108	1415 / 0	0 / 0	0 / 0	0 / 0	693 / 0	0 / 0
A	2993	2008 / 0	0 / 0	0 / 0	0 / 0	985 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-G	-4873 / 0	-91.8 -91.8	0.29 (1)	4.94	E-B	0 / 2986	0.37 (1)
G-B	-3123 / 0	-91.8 -91.8	0.27 (1)	5.89	B-D	-3378 / 0	0.60 (1)
B-C	-32 / 0	-91.8 -91.8	0.09 (1)	6.25	F-G	0 / 2447	0.00 (1)
D-C	-121 / 0	0.0 0.0	0.03 (1)	7.81			
A-F	0 / 2775	-18.5 -18.5	0.21 (1)	10.00			
F-H	0 / 2775	-18.5 -18.5	0.67 (1)	10.00			
H-I	0 / 2775	-18.5 -18.5	0.67 (1)	10.00			
I-E	0 / 2775	-18.5 -18.5	0.67 (1)	10.00			
E-J	0 / 2775	-18.5 -18.5	0.60 (1)	10.00			
J-K	0 / 2775	-18.5 -18.5	0.60 (1)	10.00			
K-D	0 / 2775	-18.5 -18.5	0.60 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	1-1-4	-2181	-2181		FRONT	VERT	TOTAL		C1
I	3-1-4	-739	-739		FRONT	VERT	TOTAL		C1
J	5-1-4	-828	-828		FRONT	VERT	TOTAL		C1
K	7-1-4	-741	-741		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 42 = 85 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.28")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: TC=0.29/1.00 (A-G:1), BC=0.87/1.00 (E-F:1), WB=0.60/1.00 (B-D:1), SSI=0.85/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

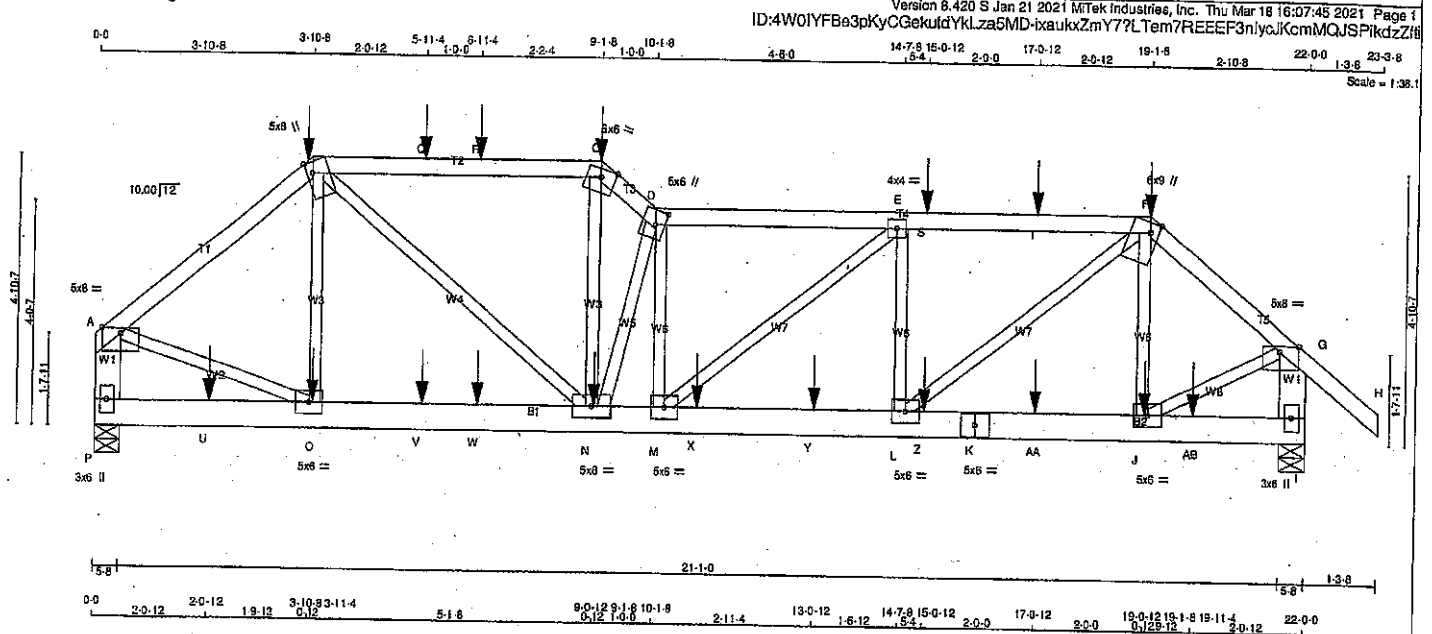
NAIL VALUES	PLATE GRIP (DRY)	SHEAR	SECTION
	(FSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (9) (INPUT = 0.90)
JSI METAL = 0.42 (E) (INPUT = 1.00)

JOB NAME 417277	TRUSS NAME T19	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



N. L. G. A. RULES	CHOROS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
P - A	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
P - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TTWW-m	MT20	5.0	8.0	Edge 1.25	
C	TTW-m	MT20	5.0	6.0	Edge	
D	TTWW-m	MT20	5.0	6.0	3.00 2.00	
E	TMVW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	6.0	3.0	Edge 1.75	
G	TMVW-p	MT20	5.0	8.0	Edge	
I	BMV1+p	MT20	3.0	6.0		
J, L, M, O						
K	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	8.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT.	HORZ.	DOWN	HORZ.		
P	1851	0	1851	0	5-8	5-8
I	1864	0	1864	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		DEAD	SOIL
	SNOW	LIVE	PERM LIVE	WIND		
P	1307	886 / 0	0 / 0	0 / 0	441 / 0	0 / 0
I	1314	885 / 0	0 / 0	0 / 0	429 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	CHORDS	FROM	TO	CS1 (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CS1 (LC)
A-B	-1868 / 0	-91.8	-91.8	0.34 (1)	4.53	Q-B	-295 / 13	0.10 (1)	
B-C	-2321 / 0	-91.8	-91.8	0.94 (1)	2.91	B-N	0 / 1189	0.29 (1)	
Q-R	-2321 / 0	-91.8	-91.8	0.94 (1)	2.91	N-C	0 / 1311	0.32 (1)	
R-C	-2321 / 0	-91.8	-91.8	0.94 (1)	2.91	N-D	-2005 / 0	0.53 (1)	
C-D	-2982 / 0	-91.8	-91.8	0.13 (1)	3.92	M-D	-205 / 0	0.05 (1)	
D-E	-2657 / 0	-91.8	-91.8	0.52 (1)	3.54	M-E	0 / 367	0.09 (1)	
E-S	-2535 / 0	-91.8	-91.8	0.49 (1)	3.75	L-E	-780 / 0	0.19 (1)	
S-T	-2585 / 0	-91.8	-91.8	0.49 (1)	3.75	L-F	0 / 1713	0.42 (1)	
T-F	-2585 / 0	-91.8	-91.8	0.49 (1)	3.75	J-F	-436 / 0	0.11 (1)	
F-G	-1653 / 0	-91.8	-91.8	0.19 (1)	4.94	A-O	0 / 1497	0.37 (1)	
G-H	0 / 41	-91.8	-91.8	0.14 (1)	10.00	J-G	0 / 1388	0.34 (1)	
P-A	-1816 / 0	0.0	0.0	0.13 (1)	7.37				
I-G	-1842 / 0	0.0	0.0	0.14 (1)	7.33				
P-U	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
U-O	0 / 0	-18.5	-18.5	0.07 (4)	10.00				
Q-V	0 / 1424	-18.5	-18.5	0.23 (1)	10.00				
V-W	0 / 1424	-18.5	-18.5	0.23 (1)	10.00				
W-N	0 / 1424	-18.5	-18.5	0.23 (1)	10.00				
N-M	0 / 2873	-18.5	-18.5	0.43 (1)	10.00				
M-X	0 / 2585	-18.5	-18.5	0.53 (1)	10.00				
X-Y	0 / 2585	-18.5	-18.5	0.53 (1)	10.00				
Y-L	0 / 2585	-18.5	-18.5	0.53 (1)	10.00				
L-Z	0 / 1253	-18.5	-18.5	0.22 (1)	10.00				
Z-K	0 / 1253	-18.5	-18.5	0.22 (1)	10.00				
K-AA	0 / 1253	-18.5	-18.5	0.22 (1)	10.00				
AA-J	0 / 1253	-18.5	-18.5	0.22 (1)	10.00				
J-AB	0 / 0	-18.5	-18.5	0.04 (4)	10.00				
AB-I	0 / 0	-18.5	-18.5	0.04 (4)	10.00				

JT	LOC.		MAX.	FACE	DIR.	TYPE	HEEL	CONN.
	LC1	MAX+						
B	3-10-8	-145	-145	FRONT	VERT	TOTAL	---	C1
C	9-1-8	-153	-153	FRONT	VERT	TOTAL	---	C1
F	19-1-8	-68	-68	FRONT	VERT	TOTAL	---	C1
J	19-0-12	-6	-6	FRONT	VERT	TOTAL	---	C1
N	9-0-12	-14	-14	FRONT	VERT	TOTAL	---	C1
O	3-11-4	-14	-14	FRONT	VERT	TOTAL	---	C1
Q	5-11-4	-59	-59	FRONT	VERT	TOTAL	---	C1
R	8-11-4	-59	-59	FRONT	VERT	TOTAL	---	C1
S	15-0-12	-28	-28	FRONT	VERT	TOTAL	---	C1
T	17-0-12	-28	-28	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TO=0.94/1.00 (B-C:1), BC=0.53/1.00 (L-M:1), WB=0.53/1.00 (D-N:1), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSQLVE HEELS OFF

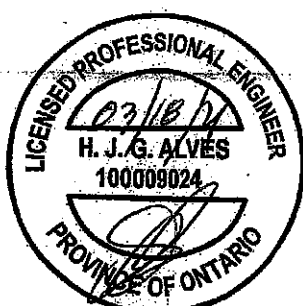
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.51 (C) (INPUT = 1.00)



Structural component only
DWG# T-2107837 1/12

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417277	T19	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Mar 18 18:07:45 2021 Page 2
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SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	2-0-12	-14	-14	---	FRONT	VERT	TOTAL	---	C1
V	5-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
W	6-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
X	10-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
Y	13-0-12	-164	-164	---	FRONT	VERT	TOTAL	---	C1
Z	15-0-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
AA	17-0-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1
AB	19-11-4	-7	-7	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

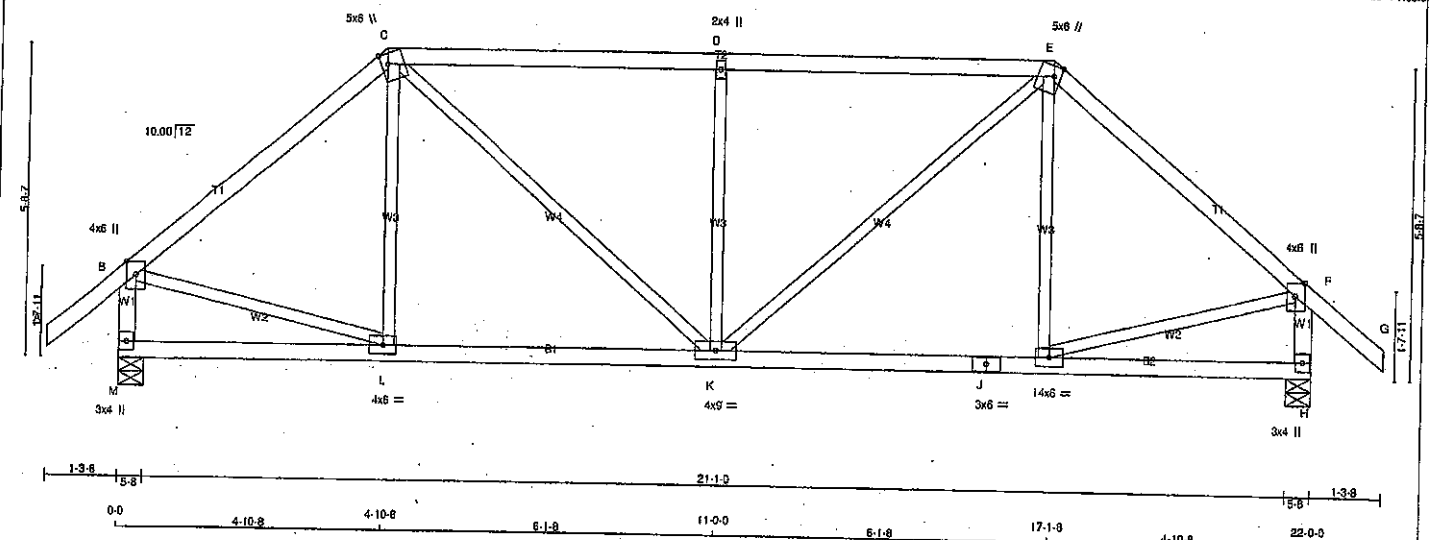


Structural component only
DWG# T-2107837 *in*

JOB NAME 417277	TRUSS NAME T20	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 6.420 S Jan 21 2021 MiTek Industries, Inc. Thu Mar 18 16:07:46 2021 Page 1
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Scale = 1:36.0



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF

A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW-w	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
M	1340	0	1340	0	5-8	5-8
H	1340	0	1340	0	5-8	5-8

JT	UNFACTORED REACTIONS						
	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	945	634 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0
H	945	634 / 0	0 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 41	-91.8	-91.8 0.13 (1)	L-C	-113 / 48	0.06 (1)	0.06 (1)
B-C	-1132 / 0	-91.8	-91.8 0.43 (1)	C-K	0 / 603	0.14 (1)	0.14 (1)
C-D	-1318 / 0	-91.8	-91.8 0.65 (1)	K-D	-698 / 0	0.34 (1)	0.34 (1)
D-E	-1318 / 0	-91.8	-91.8 0.65 (1)	E-K	0 / 803	0.14 (1)	0.14 (1)
E-F	-1132 / 0	-91.8	-91.8 0.43 (1)	I-E	-113 / 48	0.06 (1)	0.06 (1)
F-G	0 / 41	-91.8	-91.8 0.13 (1)	B-L	0 / 887	0.20 (1)	0.20 (1)
M-B	-1305 / 0	0.0	0.0 0.14 (1)	I-F	0 / 887	0.20 (1)	0.20 (1)
H-F	-1305 / 0	0.0	0.0 0.14 (1)				
M-L	0 / 0	-18.5	-18.5 0.13 (4)				
L-K	0 / 865	-18.5	-18.5 0.21 (1)				
K-J	0 / 865	-18.5	-18.5 0.21 (1)				
J-I	0 / 865	-18.5	-18.5 0.21 (1)				
I-H	0 / 0	-18.5	-18.5 0.13 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TO=0.65/1.00 (C-D:1), BC=0.21/1.00 (K-L:1), WS=0.34/1.00 (D-K:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1673

PLATE PLACEMENT TOL. = 0.260 inches

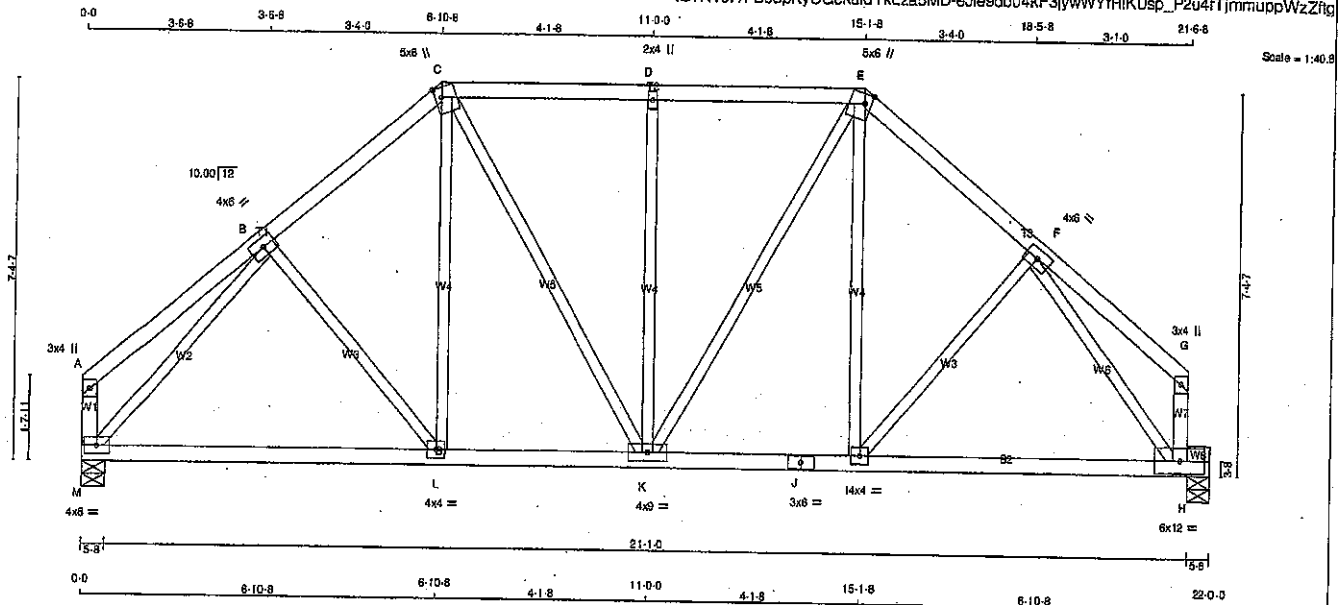
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.80)
JSI METAL = 0.56 (F) (INPUT = 1.00)



Structural component only
DWG# T-2107838

JOB NAME 417277	TRUSS NAME T21	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
M - A	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0			
B	TMWW-1	MT20	4.0	6.0			
C	TTWW+m	MT20	5.0	6.0	2.25	1.50	
D	TMW+w	MT20	2.0	4.0			
E	TTWW+m	MT20	5.0	6.0	2.25	1.50	
F	TMW+p	MT20	4.0	6.0			
G	BMVWW-1	MT20	3.0	4.0			
H	BMVWW-1	MT20	6.0	12.0			
I	BMWW-1	MT20	4.0	4.0			
J	BS-1	MT20	3.0	6.0			
K	BMVWW-1	MT20	4.0	9.0			
L	BMWW-1	MT20	4.0	4.0			
M	BMVW-1	MT20	4.0	6.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
M	1188	0	1188	0
H	1188	0	1188	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
M	840	551.0	0.0	0.0	289.0
H	840	551.0	0.0	0.0	289.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO			FROM TO			FR-TO			
A-B	0/23	-91.8	-91.8	0.17 (1)	10.00	B-L	-55/23	0.03 (1)	
B-C	-1063/0	-91.8	-91.8	0.14 (1)	5.98	L-C	0/176	0.05 (4)	
C-D	-928/0	-91.8	-91.8	0.20 (1)	6.18	C-K	0/254	0.06 (1)	
D-E	-928/0	-91.8	-91.8	0.20 (1)	6.18	K-D	-457/0	0.44 (1)	
E-F	-1022/0	-91.8	-91.8	0.13 (1)	8.06	K-E	0/315	0.07 (1)	
F-G	0/23	-91.8	-91.8	0.15 (1)	10.00	E-F	0/103	0.04 (4)	
M-A	-121/0	0.0	0.0	0.01 (1)	7.81	I-F	0/52	0.02 (4)	
H-G	-100/0	0.0	0.0	0.01 (1)	7.81	M-B	-1312/0	0.61 (1)	
						F-H	-1275/0	0.53 (1)	
M-L	0/833	-18.5	-18.5	0.26 (4)	10.00				
L-K	0/800	-18.5	-18.5	0.26 (4)	10.00				
K-J	0/769	-18.5	-18.5	0.24 (4)	10.00				
J-I	0/769	-18.5	-18.5	0.24 (4)	10.00				
I-H	0/739	-18.5	-18.5	0.23 (4)	10.00				

TOTAL WEIGHT = 104 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPO 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.72")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.72")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.06")

CS1: TO=0.20/1.00 (C-D:1), BC=0.26/1.00 (K-L:4), WB=0.61/1.00 (B-M:1), SS=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (F) (INPUT = 0.90)
JSI METAL = 0.29 (B) (INPUT = 1.00)

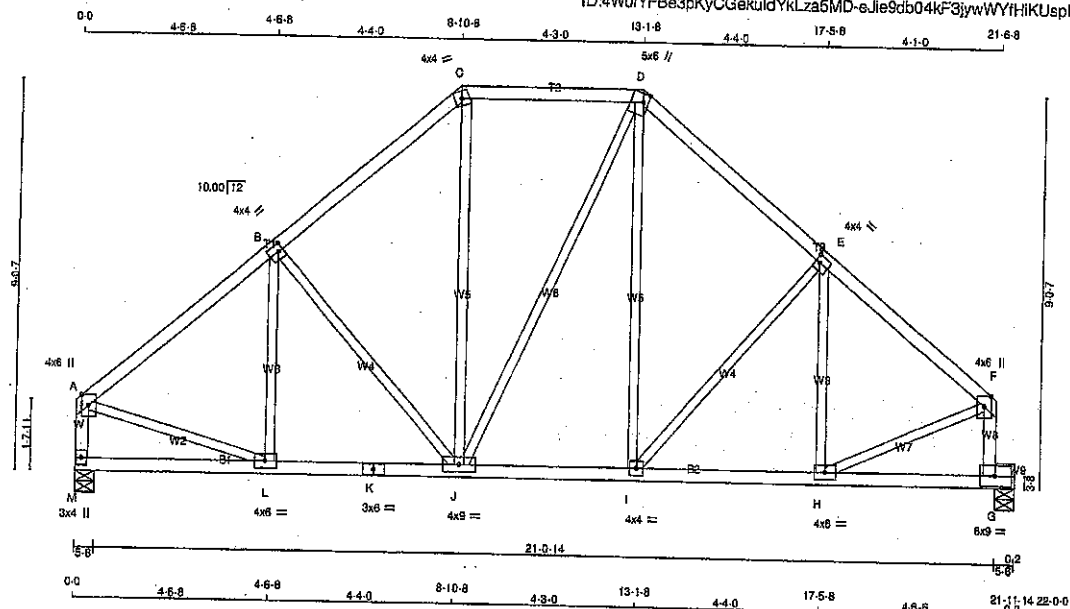


Structural component only
DWG# T-2107839

JOB NAME 417277	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:48.8

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
M - A	2x4	DRY	No.2
G - F	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMVW+p	MT20	4.0	6.0	Edge	
B - TMVW-i	MT20	4.0	4.0	2.00	1.25
C - TTW-m	MT20	4.0	4.0		
D - TTW+m	MT20	5.0	6.0	2.25	1.50
E - TMVW-i	MT20	4.0	4.0	2.00	1.25
F - TMVW+p	MT20	4.0	6.0	Edge	
G - BMVW-i	MT20	6.0	9.0		
H - BMVW-i	MT20	4.0	6.0		
I - BMVW-i	MT20	4.0	4.0		
J - BMVW-i	MT20	4.0	9.0		
K - BS-i	MT20	3.0	6.0		
L - BMVW-i	MT20	4.0	6.0		
M - BMVW-i	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
M	1188	0	1188	0
G	1188	0	1188	0

UNFACTORED REACTIONS

JT	1ST LOSE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
M	840	551 / 0	0 / 0	0 / 0	289 / 0	0 / 0
G	840	551 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	-1121 / 0	-91.8	-91.8 0.25 (1)	L-B	-184 / 18	0.07 (1)	
B-C	-960 / 0	-91.8	-91.8 0.24 (1)	B-J	-265 / 0	0.22 (1)	
C-D	-714 / 0	-91.8	-91.8 0.22 (1)	J-C	0 / 252	0.06 (1)	
D-E	-941 / 0	-91.8	-91.8 0.22 (1)	J-D	0 / 229	0.01 (1)	
E-F	-1031 / 0	-91.8	-91.8 0.22 (1)	I-D	0 / 211	0.05 (1)	
M-A	-1152 / 0	0.0	0.0 0.12 (1)	I-E	-178 / 0	0.13 (1)	
G-F	-1156 / 0	0.0	0.0 0.13 (1)	H-E	-251 / 0	0.11 (1)	
				A-L	0 / 920	0.21 (1)	
M-L	0 / 0	-18.5	-18.5 0.09 (4)	H-F	0 / 893	0.20 (1)	
L-K	0 / 885	-18.5	-18.5 0.18 (1)				
K-J	0 / 885	-18.5	-18.5 0.18 (1)				
J-I	0 / 701	-18.5	-18.5 0.15 (1)				
I-H	0 / 818	-18.5	-18.5 0.17 (1)				
H-G	0 / 0	-18.5	-18.5 0.08 (4)				

TOTAL WEIGHT = 108 lb

[MIF]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA C86-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.72")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.72")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.25/1.00 (A-B:1), BC=0.18/1.00 (J-L:1), WB=0.22/1.00 (B-J:1), SI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 L8 BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 789

PLATE PLACEMENT TO L = 0.250 inches

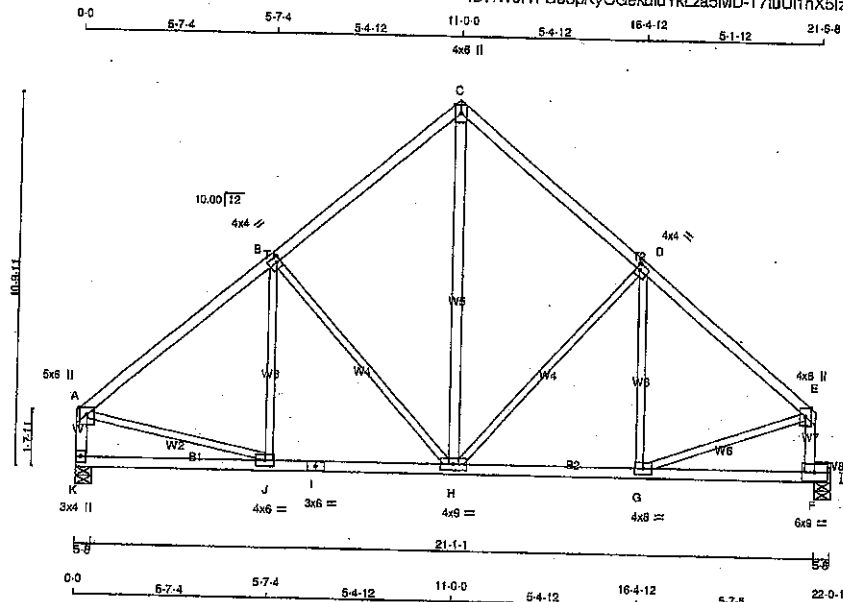
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (A) (INPUT = 0.80)
JSI METAL = 0.52 (A) (INPUT = 1.00)



Structural component only
DWG# T-2107840

JOB NAME 417278	TRUSS NAME T23	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Mar 18 18:23:32 2021 Page 1			
		ID:4W0IYFBa3pKyCGekldYkLza5MD-T7uUl1nX5lzbA8PckJ7mVGWLB0?3Op4HJKTEzZlev			
		Scale = 1/81.2			



TOTAL WEIGHT = 3 X 103 = 310 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - E	2x4	DRY	No.2	
K - A	2x4	DRY	No.2	
F - E	2x4	DRY	No.2	
I - F	2x4	DRY	No.2	

ALL WEBS	2x3	DRY	No.2
EXCEPT			
H - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	TMVW+p	MT20	4.0	6.0	Edge	
F	BMVW-t	MT20	6.0	9.0		
G	BMVW-t	MT20	4.0	6.0		
H	BMVW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMV-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
K	1188	0	1188	0
F	1188	0	1188	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	840	551 / 0	0 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
F	840	551 / 0	0 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. OSI (LC)	MAX. UNBRAC LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. OSI (LC)
FR-TO						FR-TO			
A-B	-1119 / 0	-91.8	-91.8	0.39 (1)	5.51	J-B	-94 / 59	0.06 (1)	
B-C	-840 / 0	-91.8	-91.8	0.37 (1)	6.16	B-H	-410 / 0	0.55 (1)	
C-D	-836 / 0	-91.8	-91.8	0.35 (1)	6.21	H-C	0 / 649	0.10 (1)	
D-E	-1052 / 0	-91.8	-91.8	0.35 (1)	5.70	H-D	-334 / 0	0.45 (1)	
K-A	-1145 / 0	0.0	0.0	0.12 (1)	7.43	G-D	-167 / 30	0.11 (1)	
F-E	-1149 / 0	0.0	0.0	0.13 (1)	7.42	A-J	0 / 913	0.21 (1)	
						G-E	0 / 882	0.20 (1)	
K-J	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
J-I	0 / 889	-18.5	-18.5	0.21 (1)	10.00				
I-H	0 / 889	-18.5	-18.5	0.21 (1)	10.00				
H-G	0 / 838	-18.5	-18.5	0.20 (1)	10.00				
G-F	0 / 0	-18.5	-18.5	0.12 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.72")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.72")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.39/1.00 (A-B:1), BC=0.21/1.00 (H-J:1), WB=0.55/1.00 (B-H:1), SS=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (E) (INPUT = 0.90)
JSI METAL = 0.50 (E) (INPUT = 1.00)

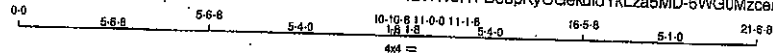


Structural component only
DWG# T-2107847

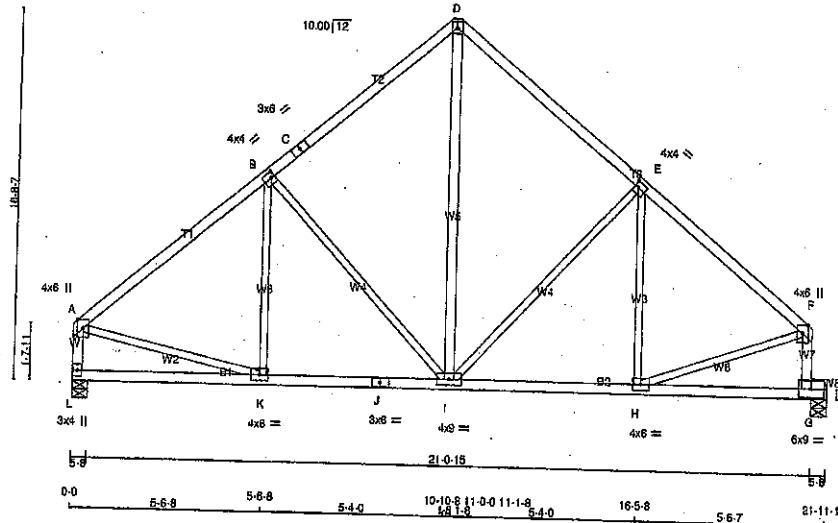
JOB NAME 417277	TRUSS NAME T23CP	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Mar 18 16:07:48 2021 Page 1
ID:4W01YFB3pKyCGekuldYkLza5MD-6W3GMzcer2Nwk6Vi6MoxdiPxpqP7js?QeMLyzZn



Scale = 1/8"=1'



LUMBER	SIZE	DRY	LUMBER
N. L. G. A. RULES			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
L - A	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
I - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	6.0	Edge	
B TMVW-i	MT20	4.0	4.0	2.00	1.25
C TS-i	MT20	3.0	6.0		
D TTW-p	MT20	4.0	4.0		
E TMVW-i	MT20	4.0	4.0	2.00	1.25
F TMVW+p	MT20	4.0	6.0	Edge	
G BMVW-i	MT20	6.0	9.0		
H BMVW-i	MT20	4.0	6.0		
I BMVW-i	MT20	4.0	9.0		
J BS-i	MT20	3.0	6.0		
K BMVW-i	MT20	4.0	6.0		
L BMVW+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
L	1188	0	1188	0
G	1188	0	1188	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
L	840 551 / 0 0 / 0 0 / 0 0 / 0 289 / 0 0 / 0
G	840 551 / 0 0 / 0 0 / 0 0 / 0 289 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)
FR-TO		FROM TO		FR-TO
A-B	-1120 / 0	-91.8 -91.8 0.39 (1)	5.52	K-B -97 / 57 0.06 (1)
B-C	-839 / 0	-91.8 -91.8 0.37 (1)	8.16	B-I -408 / 0 0.55 (1)
C-D	-839 / 0	-91.8 -91.8 0.37 (1)	8.16	I-E -331 / 0 0.45 (1)
D-E	-838 / 0	-91.8 -91.8 0.35 (1)	6.20	H-E -171 / 28 0.11 (1)
E-F	-1053 / 0	-91.8 -91.8 0.35 (1)	5.70	A-K 0 / 915 0.21 (1)
L-A	-1145 / 0	0.0 0.0 0.12 (1)	7.43	H-F 0 / 884 0.20 (1)
G-F	-1149 / 0	0.0 0.0 0.13 (1)	7.42	I-D 0 / 842 0.10 (1)
L-K	0 / 0	-18.5 -18.5 0.13 (4)	10.00	
K-J	0 / 891	-18.5 -18.5 0.21 (1)	10.00	
J-I	0 / 891	-18.5 -18.5 0.21 (1)	10.00	
I-H	0 / 839	-18.5 -18.5 0.20 (1)	10.00	
H-G	0 / 0	-18.5 -18.5 0.12 (4)	10.00	

TOTAL WEIGHT = 103 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
CALCULATED VERT. DEFL.(LL) = L/998 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.72")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.39/1.00 (A-B:1), BC=0.21/1.00 (I-K:1),
WB=0.55/1.00 (B-I:1), SS=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

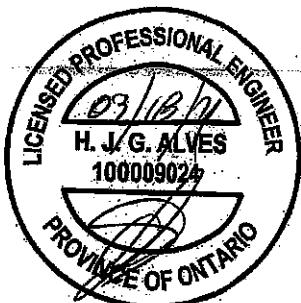
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650 371 1747 785 1887 1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (A) (INPUT = 0.90)
JSI METAL = 0.53 (A) (INPUT = 1.00)

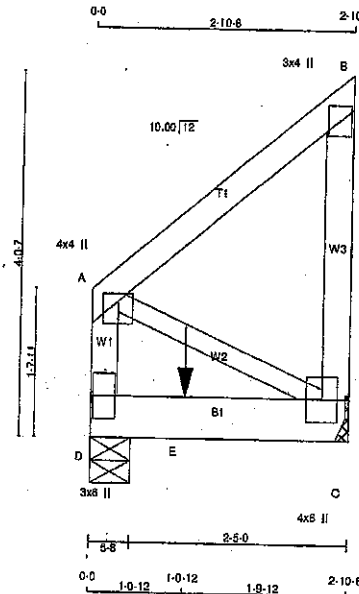


Structural component only
DWG# T-2107841

JOB NAME 417277	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

ID:4W01YFBa3pKyCGekuldYkLza5MD-biqQaJcGcLVnyG4ug3JAPvyAQDmFYIZ0E4NwtPzZfte
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Scale = 1:25.4

TOTAL WEIGHT = 17 Lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
D - A	2x4	DRY No.2
A - B	2x4	DRY No.2
C - B	2x4	DRY No.2
D - C	2x6	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
D	325	0	0	5-8
C	256	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-12.

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
D	227	165 / 0 0 / 0 0 / 0 0 / 0 61 / 0 0 / 0
C	179	127 / 0 0 / 0 0 / 0 0 / 0 52 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CS1 (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO		
D-A	-132 / 0	0.0	0.0	0.02 (1)	7.81	A-C	0 / 0 0.00 (1)
A-B	0 / 0	-91.8	-91.8	0.14 (1)	10.00		
C-B	-132 / 0	0.0	0.0	0.03 (1)	7.81		
D-E	0 / 0	-18.5	-18.5	0.14 (1)	10.00		
E-C	0 / 0	-18.5	-18.5	0.14 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-0-12	-182	-182	MAX+	TOP	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 066-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $1/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $1/999$ (0.01")

CSI: TC=0.14/1.00 (A-B:1), BC=0.14/1.00 (C-D:1),
WB=0.09/1.00 (A-C:1), SS=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 768	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

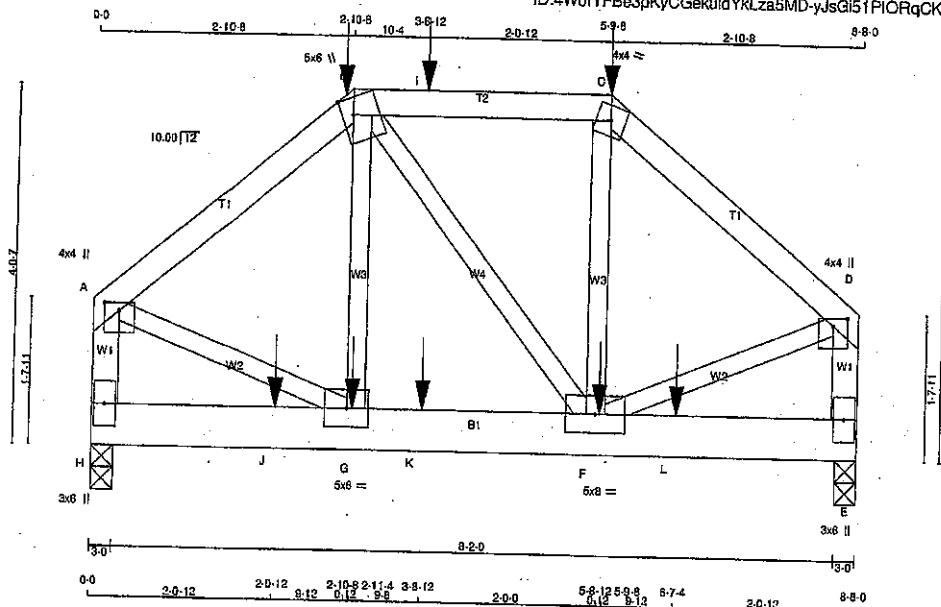
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.10 (A) (INPUT= 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107842

JOB NAME 417278	TRUSS NAME T30	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
H - A	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x6	DRY	No.2

ALL WEBS 2x8 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A - TMVW+p	MT20	4.0	4.0	1.00	2.00
B - TTWW+m	MT20	5.0	5.0	2.25	1.50
C - TTW-m	MT20	4.0	4.0		
D - TMVW+p	MT20	4.0	4.0	1.00	2.00
E - BMV1+p	MT20	3.0	6.0		
F - BMVWW-t	MT20	5.0	8.0		
G - BMVWW-t	MT20	5.0	6.0		
H - BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
H	625	0	625	0	0	3-0	3-0	3-0	3-0
E	617	0	617	0	0	3-0	3-0	3-0	3-0

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
H	442	287/0	0/0	0/0	0/0
E	437	284/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD	LC1	MAX.	MEMB.	FORCE (LBS)	MAX.	CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	-484/0	-91.8	-91.8	0.15 (1)	6.25	G-B	-78/32	0.02 (1)	
B-I	-361/0	-91.8	-91.8	0.18 (1)	6.25	B-F	-13/0	0.00 (1)	
I-C	-361/0	-91.8	-91.8	0.18 (1)	6.25	F-C	-73/32	0.02 (1)	
C-D	-474/0	-91.8	-91.8	0.15 (1)	6.25	A-G	0/400	0.10 (1)	
H-A	-595/0	0.0	0.0	0.07 (1)	7.81	F-D	0/392	0.10 (1)	
E-D	-586/0	0.0	0.0	0.07 (1)	7.81				
H-J	0/0	-18.5	-18.5	0.02 (4)	10.00				
J-G	0/0	-18.5	-18.5	0.02 (4)	10.00				
G-K	0/368	-18.5	-18.5	0.08 (1)	10.00				
K-F	0/368	-18.5	-18.5	0.08 (1)	10.00				
F-L	0/0	-18.5	-18.5	0.02 (4)	10.00				
L-E	0/0	-18.5	-18.5	0.02 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
B	2-10-8	-68	-68	94	FRONT	VERT	TOTAL	---	C1
C	5-9-8	-68	-68	94	FRONT	VERT	TOTAL	---	C1
F	5-8-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
G	2-11-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
I	3-8-12	-32	-32	---	FRONT	VERT	TOTAL	---	C1
J	2-0-12	-7	-7	---	FRONT	VERT	TOTAL	---	C1
K	3-8-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
L	6-7-4	-7	-7	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 44 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF ORC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.01")

CSI: TC=0.18/1.00 (B-C:1), BC=0.08/1.00 (F-G:1), WB=0.10/1.00 (A-G:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.51 (A) (INPUT=0.90)
JSI METAL=0.14 (A) (INPUT=1.00)



Structural component only
DWG# T-2107848

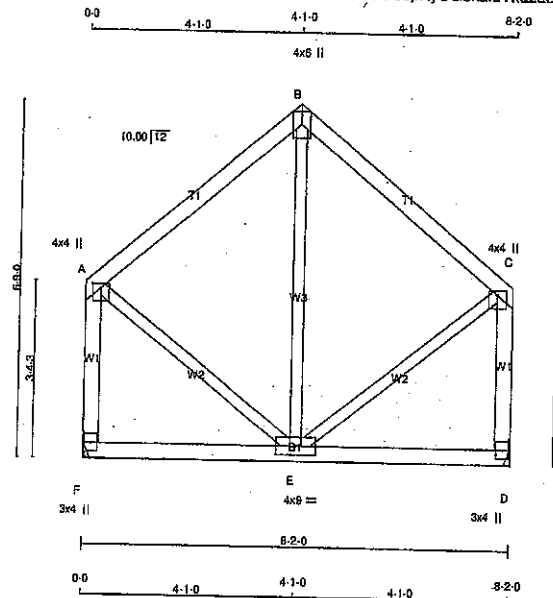
JOB NAME 417278	TRUSS NAME T31	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

ID:4W0IYFBe3pKyCGekuldYkLza5MD-QVQevR213IZhqUpWX1mnCBbe59JC7428XboQY7zZle

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Scale = 1/40.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW+p	MT20	4.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW+1	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
F	480	0	480	0
D	480	0	480	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
F	318 209 / 0
D	318 209 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	-194 / 0	-91.8 -91.8 0.20 (1)	E-B	-127 / 23
B-C	-194 / 0	-91.8 -91.8 0.20 (1)	A-E	0 / 185
F-A	-421 / 0	0.0 0.0 0.08 (1)	E-C	0 / 185
D-C	-421 / 0	0.0 0.0 0.08 (1)		
F-E	0 / 0	-18.5 -18.5 0.09 (4)		
E-D	0 / 0	-18.5 -18.5 0.09 (4)		

TOTAL WEIGHT = 2 X 41 = 82 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (A-B:1), BC=0.09/1.00 (E-F:4), WB=0.09/1.00 (B-E:1), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

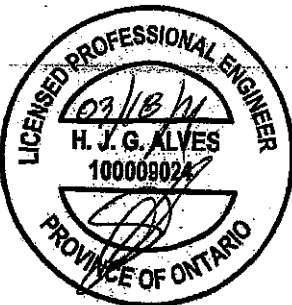
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

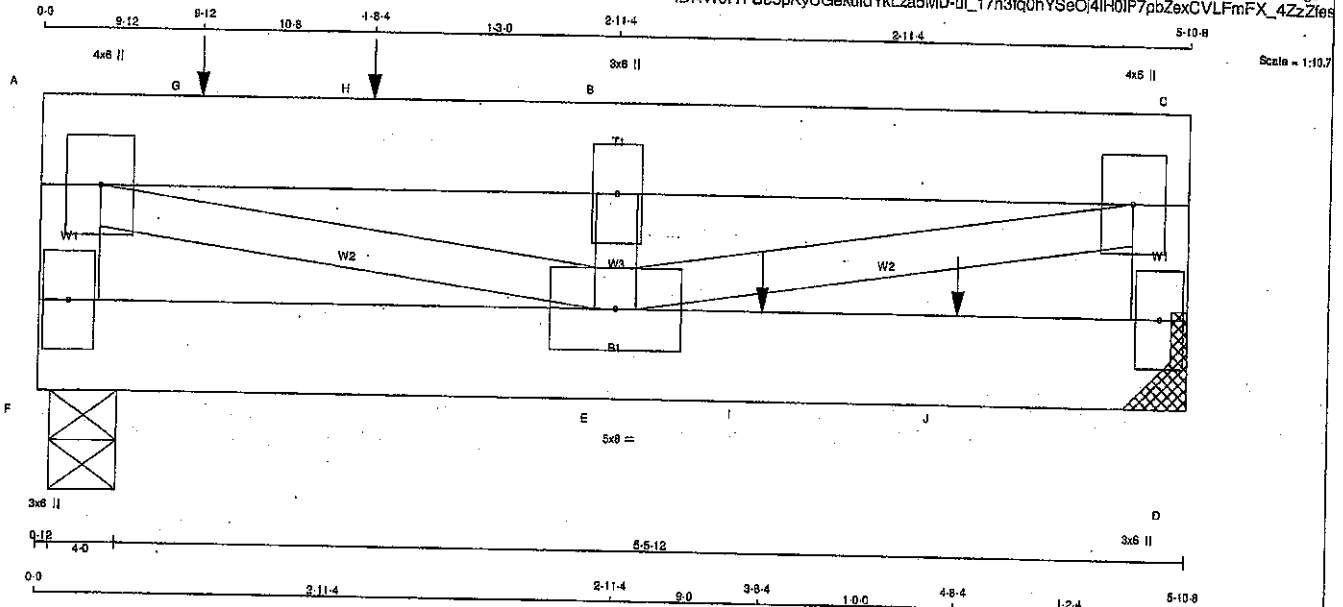
JSI GRIP= 0.29 (C) (INPUT = 0.60)
JSI METAL= 0.06 (C) (INPUT = 1.00)



Structural component only
DWG# T-2107849

JOB NAME 417278	TRUSS NAME T32	QUANTITY 2	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Thu Mar 18 16:23:35 2021 Page 1
ID:4W0YFBe3pKyCGekuldYkLza5MD-ul_17n3fqhYSeOj4IH0IP7pZexCVLFmFX_4ZzZies



Scale = 1:10.7

TOTAL WEIGHT = 4 X 27 = 108 lb

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
F - A	2x4	DRY	No.2		SPF
A - C	2x6	DRY	No.2		SPF
D - C	2x4	DRY	No.2		SPF
F - D	2x6	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
C-D 1	12	TOP
A-C 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE (10.5)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0			
B	TMVW+w	MT20	3.0	6.0			
C	TMVW+p	MT20	4.0	6.0			
D	BMV1+p	MT20	3.0	6.0			
E	BMVWV-t	MT20	5.0	6.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	1183	0	1183	0
D	1175	0	1175	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	841	528 / 0	0 / 0	0 / 0	0 / 0	319 / 0	0 / 0
D	834	530 / 0	0 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO					FR-TO		
F-A	-1107 / 0	0.0	0.0	0.08 (1)	A-E	0 / 1764	0.22 (1)
A-G	-1663 / 0	-91.8	-91.8	0.21 (1)	E-B	-615 / 0	0.05 (1)
G-H	-1663 / 0	-91.8	-91.8	0.21 (1)	B-C	0 / 1764	0.22 (1)
H-B	-1663 / 0	-91.8	-91.8	0.21 (1)			
B-C	-1663 / 0	-91.8	-91.8	0.21 (1)			
D-C	-733 / 0	0.0	0.0	0.04 (1)			
F-E	0 / 0	-37.3	-37.3	0.03 (1)			
E-I	0 / 0	-37.3	-37.3	0.18 (1)			
I-J	0 / 0	-37.3	-37.3	0.18 (1)			
J-D	0 / 0	-37.3	-37.3	0.18 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX. MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	9-12	-142	-142	---	TOP	VERT	---	C1
H	1-8-4	-379	-379	---	TOP	VERT	---	C1
I	3-8-4	-304	-304	---	FRONT	VERT	---	C1
J	4-8-4	-304	-304	---	FRONT	VERT	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.6	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 39.0	PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.03")

CSI: TC=0.21/1.00 (A-B:1), BC=0.18/1.00 (D-E:1), WB=0.22/1.00 (A-E:1), SS=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

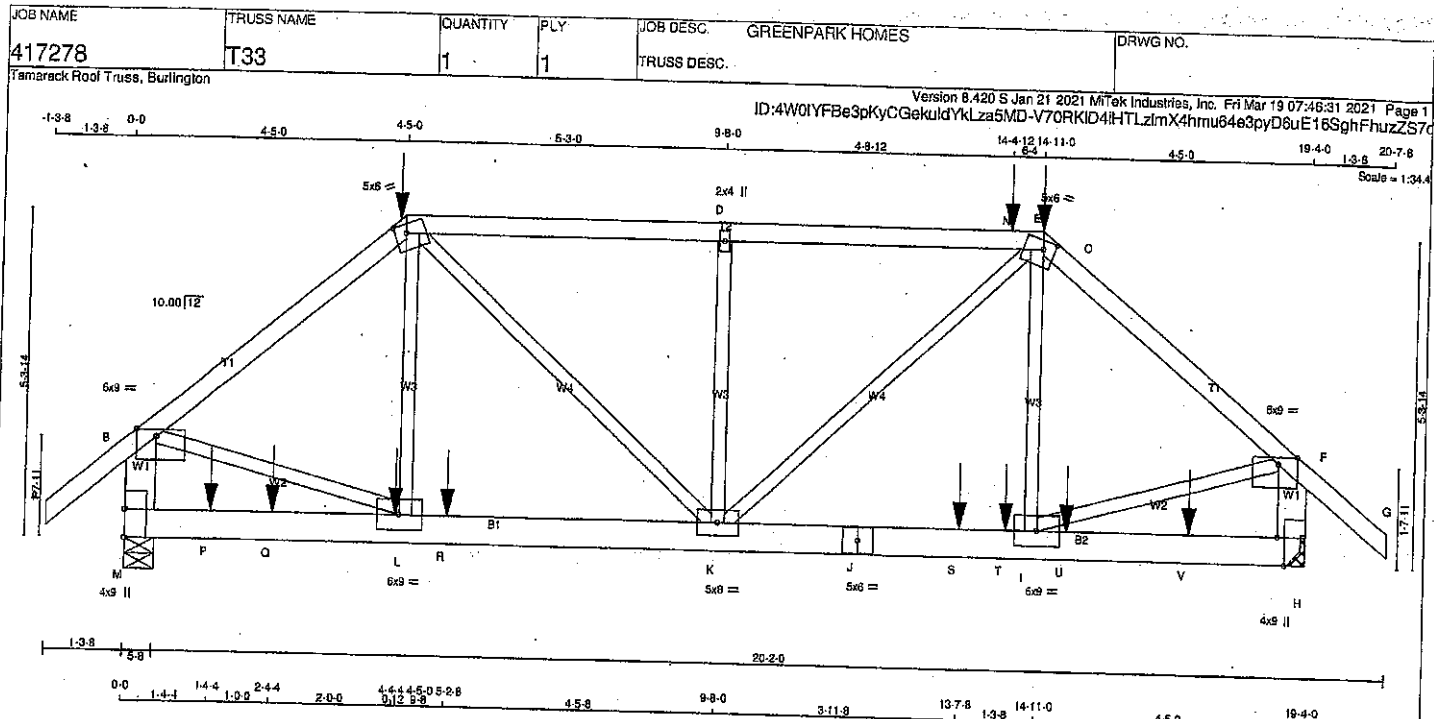
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (A) (INPUT = 0.80)

JSI METAL= 0.37 (C) (INPUT = 1.00)



Structural component only
DWG# T-2107850.



CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
E - G	2x4	DRY No.2
M - B	2x6	DRY No.2
H - F	2x6	DRY No.2
M - J	2x6	DRY No.2
J - H	2x6	DRY No.2
ALL WEBS	2x3	DRY No.2
EXCEPT		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	6.0	9.0	Edge	3.75
C TTWV-m	MT20	5.0	6.0	1.75	2.25
D TMWV-w	MT20	2.0	4.0		
E TTWV-m	MT20	5.0	6.0	1.75	2.25
F TMWV-p	MT20	8.0	9.0	Edge	3.75
H BMW1-t	MT20	4.0	8.0	Edge	1.50
J BMWV-t	MT20	6.0	9.0		
K BMWV-w	MT20	5.0	6.0		
L BMWV-w	MT20	6.0	9.0		
M BMW1-t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
M	3023	0	3023	0
H	2951	0	2951	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
M	2138 1402.0 0.0 0.0 0.0 735.0 0.0
H	2085 1376.0 0.0 0.0 0.0 710.0 0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0/41	-91.8	-91.8	0.14 (1)	L-C	0/545	0.14 (1)	
B-C	-3136/0	-91.8	-91.8	0.60 (1)	C-K	0/607	0.20 (1)	
C-D	-3003/0	-91.8	-91.8	0.71 (1)	K-D	-597/0	0.24 (1)	
D-N	-3004/0	-91.8	-91.8	0.72 (1)	K-E	0/832	0.21 (1)	
N-E	-3004/0	-91.8	-91.8	0.72 (1)	E-I	0/519	0.13 (1)	
E-O	-3114/0	-91.8	-91.8	0.60 (1)	B-L	0/2490	0.62 (1)	
O-F	-3114/0	-91.8	-91.8	0.60 (1)	I-F	0/2472	0.61 (1)	
F-G	0/41	-91.8	-91.8	0.14 (1)				
M-B	-2994/0	0.0	0.0	0.22 (1)				
H-F	-2975/0	0.0	0.0	0.22 (1)				
M-P	0/0	-18.5	-18.5	0.18 (1)				
P-Q	0/0	-18.5	-18.5	0.18 (1)				
Q-L	0/0	-18.5	-18.5	0.18 (1)				
L-R	0/2415	-18.5	-18.5	0.85 (1)				
R-K	0/2415	-18.5	-18.5	0.85 (1)				
K-J	0/2397	-18.5	-18.5	0.84 (1)				
J-S	0/2397	-18.5	-18.5	0.84 (1)				
S-T	0/2397	-18.5	-18.5	0.84 (1)				
T-U	0/2397	-18.5	-18.5	0.84 (1)				
U-V	0/0	-18.5	-18.5	0.30 (1)				
V-H	0/0	-18.5	-18.5	0.30 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-5-0	-44	-44	-	FRONT	VERT	DEAD	-	C1
C	4-5-0	-128	-128	-	BACK	VERT	TOTAL	-	C1
C	4-5-0	-188	-188	-	FRONT	VERT	SNOW	-	C1
E	14-11-0	-44	-44	-	FRONT	VERT	DEAD	-	C1
E	14-11-0	-188	-188	-	FRONT	VERT	SNOW	-	C1
L	4-4-4	-29	-29	-	BACK	VERT	TOTAL	-	C1
N	14-4-12	-138	-138	-	BACK	VERT	TOTAL	-	C1
P	1-4-4	-29	-29	-	BACK	VERT	TOTAL	-	C1
Q	2-4-4	-29	-29	-	BACK	VERT	TOTAL	-	C1
R	5-2-8	-820	-820	-	BACK	VERT	TOTAL	-	C1
S	13-7-8	-820	-820	-	BACK	VERT	TOTAL	-	C1
T	14-4-12	-29	-29	-	BACK	VERT	TOTAL	-	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.72/1.00 (D-E:1), BC=0.84/1.00 (I-K:1), WB=0.62/1.00 (B-L:1), SSI=0.68/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

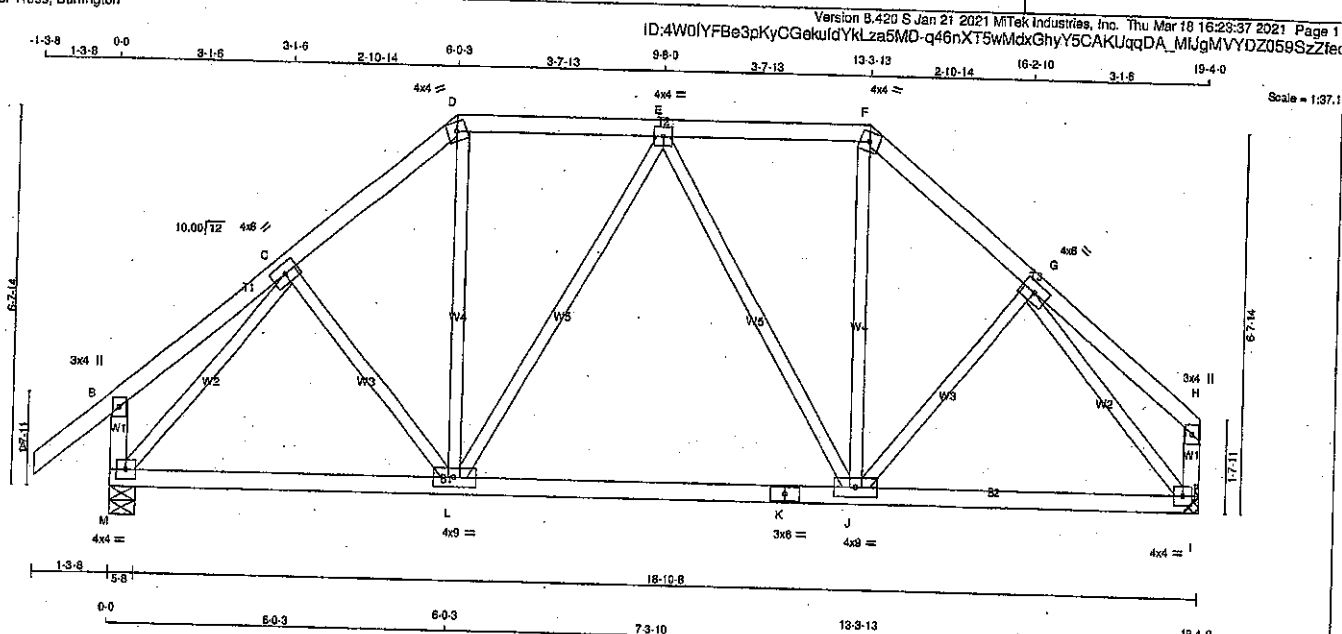
JSI GRIP=0.89 (C) (INPUT = 0.90)
JSI METAL=0.70 (J) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2107851

JOB NAME 417278	TRUSS NAME T34	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
M - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV-p	MT20	3.0	4.0	
C TMWW-I	MT20	4.0	6.0	
D TTW-m	MT20	4.0	4.0	
E TMWW-I	MT20	4.0	4.0	
F TTW-m	MT20	4.0	4.0	
G TMWW-I	MT20	4.0	6.0	
H TMV-p	MT20	3.0	4.0	
I BMWW-I	MT20	4.0	4.0	
J BMWW-I	MT20	4.0	9.0	
K BS-I	MT20	3.0	6.0	
L BMWW-I	MT20	4.0	9.0	
M BMWW-I	MT20	4.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
M	1193	0	1193	0	5-8	5-8
I	1086	0	1086	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	841	568 / 0	0 / 0	0 / 0	0 / 0	276 / 0	0 / 0
I	754	495 / 0	0 / 0	0 / 0	0 / 0	259 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 41	-91.8 -91.8 0.13 (1)	C-L	-18 / 31
B-C	0 / 19	-91.8 -91.8 0.12 (1)	L-D	0 / 350
C-D	-942 / 0	-91.8 -91.8 0.10 (1)	E-J	-231 / 0
D-E	-713 / 0	-91.8 -91.8 0.15 (1)	J-F	-231 / 0
E-F	-713 / 0	-91.8 -91.8 0.15 (1)	F-G	0 / 350
F-G	-942 / 0	-91.8 -91.8 0.10 (1)	G-I	-18 / 31
G-H	0 / 19	-91.8 -91.8 0.12 (1)	M-C	-1193 / 0
M-B	-235 / 0	0.0 0.0 0.02 (1)	G-I	-1193 / 0
I-H	-108 / 0	0.0 0.0 0.01 (1)		
M-L	0 / 719	-18.5 -18.5 0.24 (4)		
L-K	0 / 825	-18.5 -18.5 0.25 (4)		
K-J	0 / 825	-18.5 -18.5 0.25 (4)		
J-I	0 / 719	-18.5 -18.5 0.24 (4)		

TOTAL WEIGHT = 89 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 988-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.15/1.00 (D-E-1), BC=0.25/1.00 (J-L-4), WB=0.43/1.00 (C-M-1), SS=0.16/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

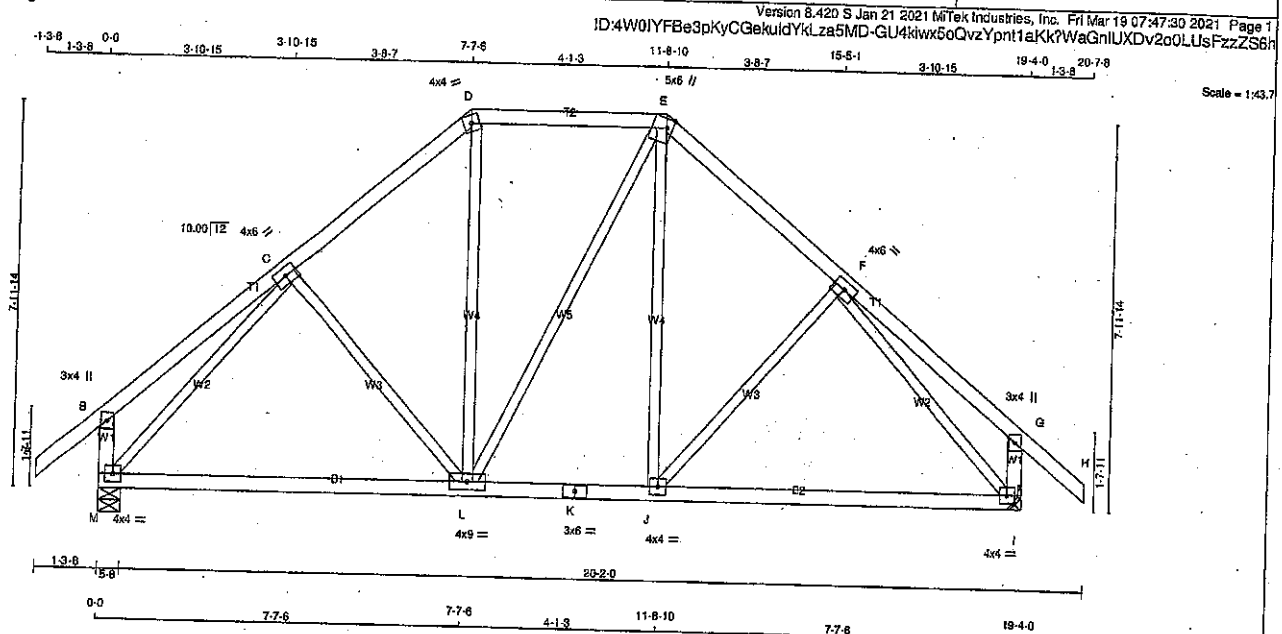
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (I) (INPUT = 0.80)
JSI METAL = 0.26 (C) (INPUT = 1.00)



Structural component only
DWG# T-2107852

JOB NAME 417278	TRUSS NAME T35	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - A	2x4	DRY	No.2

ALL WEBS EXCEPT

ALL WEBS EXCEPT	SIZE	LUMBER
ALL WEBS EXCEPT	2x3	DRY No.2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	0	0
M	HORIZ	UP	0	0
I	HORIZ	DOWN	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
TOP CH.	LL	25.6
BOT CH.	LL	5.0
BOT CH.	LL	0.0
BOT CH.	LL	7.4
BOT CH.	LL	39.0

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+o	MT20	3.0	4.0		
C	TMVW+o	MT20	4.0	6.0		
D	TTW+m	MT20	4.0	4.0		
E	TTW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+o	MT20	4.0	6.0		
G	TMV+o	MT20	3.0	4.0		
H	BMVW+o	MT20	4.0	4.0		
I	BMVW+o	MT20	3.0	4.0		
J	BMVW+o	MT20	3.0	4.0		
K	BS+o	MT20	3.0	4.0		
L	BMVW+o	MT20	4.0	4.0		
M	BMVW+o	MT20	4.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
M	COMBINED	SNOW	0.0	0.0	276.0	0.0
I	COMBINED	SNOW	0.0	0.0	276.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0/41	-91.8	-91.8 0.13 (1)	C-L	-137.0	0.08 (1)	
B-C	0/26	-91.8	-91.8 0.21 (1)	L-D	0/221	0.05 (1)	
C-D	-874.0	-91.8	-91.8 0.17 (1)	D-E	0/0	0.00 (1)	
D-E	-853.0	-91.8	-91.8 0.20 (1)	E-F	0/220	0.05 (1)	
E-F	-873.0	-91.8	-91.8 0.17 (1)	F-G	-137.0	0.08 (1)	
F-G	0/26	-91.8	-91.8 0.21 (1)	G-H	-1144.0	0.65 (1)	
G-H	0/41	-91.8	-91.8 0.13 (1)	H-I	-1144.0	0.65 (1)	
H-I	-260.0	0.0	0.0 0.03 (1)	I-J			
I-J	-260.0	0.0	0.0 0.03 (1)				
J-K	0/740	-18.5	-18.5 0.29 (4)				
K-L	0/652	-18.5	-18.5 0.26 (4)				
L-M	0/652	-18.5	-18.5 0.26 (4)				
M-N	0/740	-18.5	-18.5 0.29 (4)				

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 3.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.12")

CSI: TC=0.21/1.00 (F-G:1), BC=0.29/1.00 (I-J:4), WB=0.65/1.00 (C-M:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

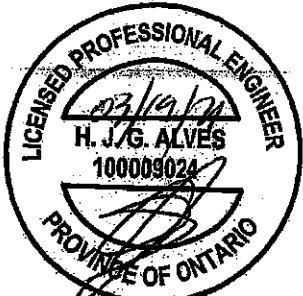
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (M) (INPUT = 0.90)
JSI METAL = 0.84 (K) (INPUT = 1.00)

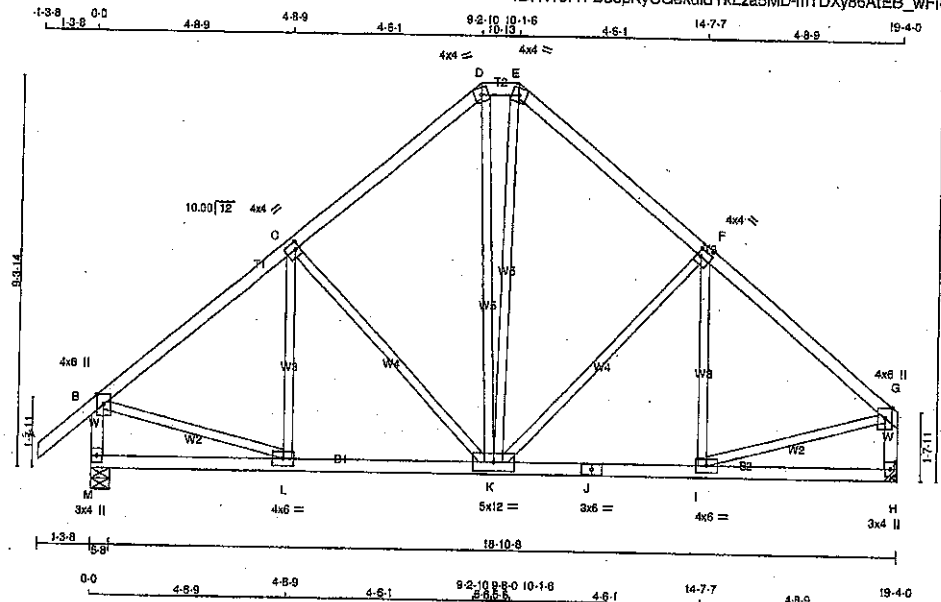


Structural component only
DWG# T-2107853

JOB NAME 417278	TRUSS NAME T36	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:23:39 2021 Page 1
ID:4W0YFB3pKyCGakuldyKLa5MD-mTDxY86AtEB_wFIUJaMyrFIUmA?08lyrhtVBKzZfio



Scale = 1/20.4

TOTAL WEIGHT = 96 lb

LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
N - O	2x4	DRY	No.2	SPF
P - Q	2x4	DRY	No.2	SPF
R - S	2x4	DRY	No.2	SPF
T - U	2x4	DRY	No.2	SPF
V - W	2x4	DRY	No.2	SPF
X - Y	2x4	DRY	No.2	SPF
Z - AA	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE				
B TMVW+p	MT20	4.0	6.0	Edge
C TMVW-t	MT20	4.0	4.0	2.00 1.25
D TTW-m	MT20	4.0	4.0	
E TTW-m	MT20	4.0	4.0	
F TMVW-t	MT20	4.0	4.0	2.00 1.25
G TMVW+p	MT20	4.0	6.0	Edge
H BMV+p	MT20	3.0	4.0	
I BMV-t	MT20	4.0	6.0	
J BS-t	MT20	3.0	6.0	
K BMVWVW-t	MT20	5.0	12.0	
L BMVW-t	MT20	4.0	6.0	
M BMV+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
M	1193	0	1193	0
H	1066	0	1066	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
M	841 566 / 0 0 / 0 0 / 0 0 / 0 276 / 0 0 / 0
H	754 495 / 0 0 / 0 0 / 0 0 / 0 259 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNSHEATHED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. UNBRACED LENGTH (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
MEMB.				MEMB.		
FR-TO				FR-TO		
A-B	0 / 41	-91.8	-91.8 0.13 (1)	L-C	-119 / 99	0.08 (1)
B-C	-980 / 0	-91.8	-91.8 0.26 (1)	C-K	-315 / 0	0.31 (1)
C-D	-768 / 0	-91.8	-91.8 0.26 (1)	K-F	-315 / 0	0.31 (1)
D-E	-575 / 0	-91.8	-91.8 0.26 (1)	F-I	-119 / 39	0.08 (1)
E-F	-765 / 0	-91.8	-91.8 0.26 (1)	I-L	0 / 807	0.18 (1)
F-G	-980 / 0	-91.8	-91.8 0.26 (1)	G-L	0 / 807	0.18 (1)
M-B	-1157 / 0	0.0	0.0 0.12 (1)	D-K	0 / 275	0.06 (1)
H-G	-1030 / 0	0.0	0.0 0.11 (1)	K-E	0 / 275	0.06 (1)
M-L	0 / 0	-18.5	-18.5 0.10 (4)			
L-K	0 / 778	-18.5	-18.5 0.17 (1)			
K-J	0 / 778	-18.5	-18.5 0.17 (1)			
J-I	0 / 778	-18.5	-18.5 0.17 (1)			
I-H	0 / 0	-18.5	-18.5 0.10 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(56% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.64")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.64")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.26/1.00 (B-C:1), BC=0.17/1.00 (H-K:1), WS=0.31/1.00 (F-K:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

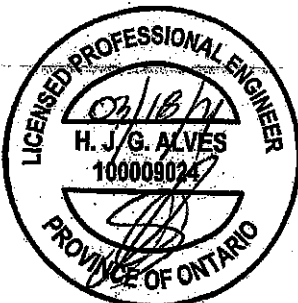
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

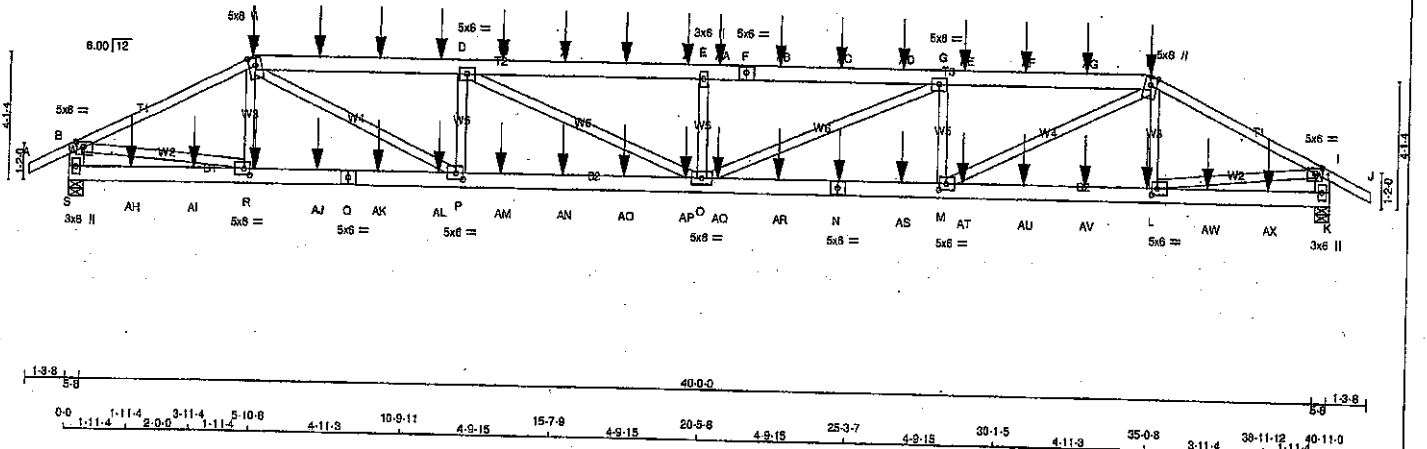
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.78 (G) (INPUT = 0.90)
JSI METAL = 0.50 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107854

JOB NAME 417279	TRUSS NAME T50	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Thu Mar 18 16:36:08 2021 Page 1			
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Scale = 1:88.2					



LUMBER	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x6	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE (61.0)
H - J 1	12	SIDE (61.0)
C - F 2	12	SIDE (61.0)
F - H 2	12	SIDE (61.0)
S - B 2	12	TOP
K - I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - Q 2	12	SIDE (183.1)
Q - N 2	12	SIDE (183.1)
N - K 2	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
S	3783	0	0	5-8
K	3783	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL
S	2878 1745 / 0 0 / 0 0 / 0 933 / 0 0 / 0
K	2878 1745 / 0 0 / 0 0 / 0 933 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRAIDED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	R-C	-505 / 0	0.05 (1)	
B-C	-5880 / 0	-91.8	-91.8	0.59 (1)	3.58	C-P	0 / 4548	0.40 (1)	
C-T	-9284 / 0	-91.8	-91.8	0.44 (1)	3.57	P-D	-1940 / 0	0.17 (1)	
T-U	-9284 / 0	-91.8	-91.8	0.44 (1)	3.57	D-O	0 / 1709	0.15 (1)	
U-V	-9284 / 0	-91.8	-91.8	0.44 (1)	3.57	O-E	-1188 / 0	0.10 (1)	
V-D	-9284 / 0	-91.8	-91.8	0.44 (1)	3.57	E-G	0 / 1709	0.15 (1)	
D-W	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58	G-H	-1940 / 0	0.17 (1)	
W-X	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58	H-I	0 / 4548	0.40 (1)	
X-Y	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58	I-H	-505 / 0	0.05 (1)	
Y-Z	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58	H-R	0 / 5319	0.47 (1)	
Z-E	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58	E-L	0 / 5319	0.47 (1)	
E-AA	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58				
AA-F	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58				
F-AB	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58				
AB-AC	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58				
AC-AD	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58				
AD-G	-10832 / 0	-91.8	-91.8	0.55 (1)	3.58				
G-AE	-9283 / 0	-91.8	-91.8	0.44 (1)	3.57				
AE-AF	-9283 / 0	-91.8	-91.8	0.44 (1)	3.57				
AF-AG	-9283 / 0	-91.8	-91.8	0.44 (1)	3.57				
AG-H	-9283 / 0	-91.8	-91.8	0.44 (1)	3.57				
H-I	-5880 / 0	-91.8	-91.8	0.59 (1)	3.58				
I-J	0 / 28	-91.8	-91.8	-0.07 (1)	10.00				
S-B	-3713 / 0	0.0	0.0	0.13 (1)	7.31				
K-I	-3712 / 0	0.0	0.0	0.13 (1)	7.31				
S-AH	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
AH-AI	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
AI-R	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
R-AJ	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AJ-Q	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
Q-AK	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AK-AL	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AL-P	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
P-AM	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AM-AN	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AN-AO	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AO-AP	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AP-O	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
O-AQ	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AQ-AR	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				
AR-N	0 / 5291	-18.5	-18.5	0.40 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2015 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.38")
CALCULATED VERT. DEFL. (LL) = L/999 (0.39")
ALLOWABLE DEFL. (TL) = L/360 (1.38")
CALCULATED VERT. DEFL. (TL) = L/786 (0.82")
CSI: TC=0.59/1.00 (B-C:1), BC=0.68/1.00 (M-O:1),
WB=0.47/1.00 (B-R:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

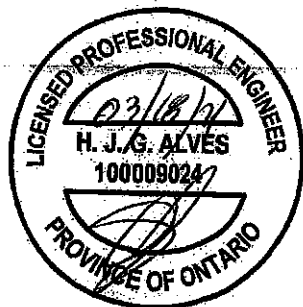
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.84 (R) (INPUT = 0.90)
JSI METAL=0.88 (N) (INPUT = 1.00)



Structural component only
DWG# T-2107870 1/12

CONTINUED ON PAGE 2

JOB NAME 417279	TRUSS NAME T50	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:38:08 2021 Page 2	
				ID:K77hYrav7ro?MDbCT?VixHza_o6-LZ_h9FA?UNYIHMSRhoIbmBpPISWJ7?g5wvRKQzZIT5	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	2.00	2.75
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	8.0		
E	TMWW-w	MT20	3.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	
I	TMVW-p	MT20	5.0	8.0	2.00	2.75
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	2.75
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	2.25
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
FR-TO								
N-AS	0/9283	-18.5	-18.5	0.88 (1)	10.00			
AS-M	0/9283	-18.5	-18.5	0.88 (1)	10.00			
M-AT	0/5291	-18.5	-18.5	0.40 (1)	10.00			
AT-AU	0/5291	-18.5	-18.5	0.40 (1)	10.00			
AU-AV	0/5291	-18.5	-18.5	0.40 (1)	10.00			
AV-L	0/5291	-18.5	-18.5	0.40 (1)	10.00			
L-AW	0/0	-18.5	-18.5	0.06 (4)	10.00			
AW-AX	0/0	-18.5	-18.5	0.06 (4)	10.00			
AX-K	0/0	-18.5	-18.5	0.06 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-254	-254	---	FRONT	VERT	TOTAL	---	C1
H	35-0-8	-254	-254	---	FRONT	VERT	TOTAL	---	C1
L	34-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
N	24-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
R	5-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
U	9-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
V	11-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
W	13-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
X	15-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Y	17-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
Z	19-11-4	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AA	20-11-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AB	22-11-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AC	24-11-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AD	26-11-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AE	28-11-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AF	30-11-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AG	32-11-12	-76	-76	---	FRONT	VERT	TOTAL	---	C1
AH	1-11-4	-20	-20	---	FRONT	VERT	TOTAL	---	C1
AI	3-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AJ	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AK	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AL	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AM	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AN	15-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AO	17-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AP	19-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AQ	20-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AR	22-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AS	24-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AT	26-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AU	30-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AV	32-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AW	36-11-12	-21	-21	---	FRONT	VERT	TOTAL	---	C1
AX	38-11-12	-20	-20	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

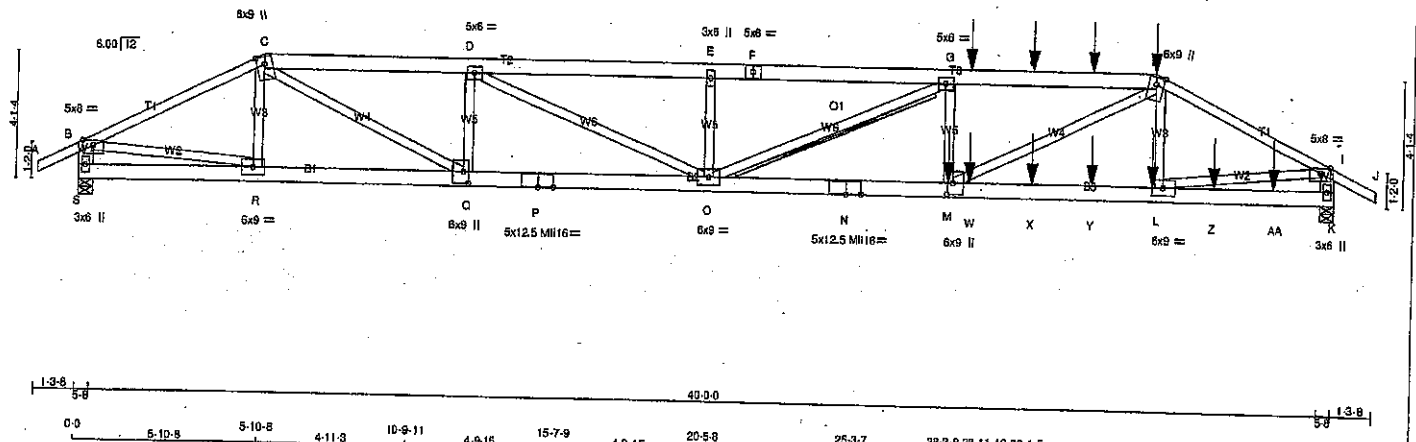


Structural component only
DWG# T-2107870 *212*

JOB NAME 417279	TRUSS NAME T50Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:36:10 2021 Page 1
ID:K77hYrav7ro7MDbCT7V6xHza_o6-Hy6RaxCF0_oQXgcpPpK3rcvRv6AbsPNNdWR5JzZfT3

1-3-8 0-0 6-10-8 5-10-8 4-11-3 10-9-11 4-9-15 15-7-9 4-9-15 20-5-8 4-9-15 25-3-7 3-8-5 28-11-12 30-1-12 32-11-12 35-0-8 40-11-0 42-2-8 1-3-8 1-11-4
Scale = 1:88.2



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

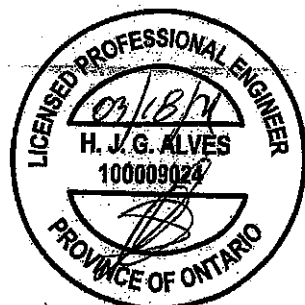
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
H-J	12	SIDE(61.0)
C-F	12	TOP
F-H	2	SIDE(61.0)
S-B	2	TOP
K-I	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-P	2	TOP
P-N	2	TOP
N-K	2	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	SIDE(577.9)
G-M	3	
D-Q	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



Structural component only
DWG# T-2107871

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
S	3440	0	0	3440	0	0	5-8	5-8	5-8
K	5092	0	0	5092	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
S	2429	1814	0	0/0	0/0	815	0	0/0	0/0	0/0	0/0
K	3599	2375	0	0/0	0/0	1223	0	0/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0/28	-91.8 -91.8	0.07 (1)	10.00	R-C	-519/0	0.05 (1)
B-C	-5308/0	-91.8 -91.8	0.54 (1)	3.78	C-Q	0/5003	0.44 (1)
C-D	-9172/0	-91.8 -91.8	0.32 (1)	3.83	Q-D	-2280/0	0.19 (1)
D-E	-12351/0	-91.8 -91.8	0.51 (1)	3.24	D-O	0/3505	0.31 (1)
E-F	-12351/0	-91.8 -91.8	0.50 (1)	3.25	O-E	-701/0	0.05 (1)
F-G	-12351/0	-91.8 -91.8	0.50 (1)	3.25	O-G	-1468/0	0.46 (1)
G-T	-13680/0	-91.8 -91.8	0.65 (1)	2.90	M-G	-379/15	0.03 (1)
T-U	-13680/0	-91.8 -91.8	0.65 (1)	2.90	M-H	0/7166	0.63 (1)
U-V	-13680/0	-91.8 -91.8	0.65 (1)	2.90	L-H	-750/0	0.07 (1)
V-H	-13680/0	-91.8 -91.8	0.85 (1)	2.90	B-R	0/4802	0.42 (1)
H-I	-8208/0	-91.8 -91.8	0.83 (1)	2.91	L-I	0/7425	0.66 (1)
I-J	0/28	-91.8 -91.8	0.07 (1)	10.00			
S-B	-3390/0	0.0 0.0	0.12 (1)	7.57			
K-L	-5026/0	0.0 0.0	0.18 (1)	6.49			
S-R	0/0	-18.5 -18.5	0.03 (4)	10.00			
R-Q	0/4779	-18.5 -18.5	0.35 (1)	10.00			
Q-P	0/9172	-18.5 -18.5	0.66 (1)	10.00			
P-O	0/9172	-18.5 -18.5	0.66 (1)	10.00			
O-N	0/13680	-18.5 -18.5	0.98 (1)	10.00			
N-M	0/13680	-18.5 -18.5	0.98 (1)	10.00			
M-W	0/7387	-18.5 -18.5	0.57 (1)	10.00			
W-X	0/7387	-18.5 -18.5	0.57 (1)	10.00			
X-Y	0/7387	-18.5 -18.5	0.57 (1)	10.00			
Y-L	0/7387	-18.5 -18.5	0.57 (1)	10.00			
L-Z	0/0	-18.5 -18.5	0.07 (4)	10.00			
Z-AA	0/0	-18.5 -18.5	0.07 (4)	10.00			
AA-K	0/0	-18.5 -18.5	0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	35-0-8	-254	-254	---	BACK	VERT	TOTAL	---	C1
L	34-11-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.40")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/558 (0.75")

CSI: TC=0.83/1.00 (H-I), BC=0.98/1.00 (M-O),
WB=0.66/1.00 (L-L), SS=0.19/1.00 (G-H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	(PSI)	(PL)	SECTION	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788
MH16	438	302	2547	1256

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (C) (INPUT = 0.90)
JSI METAL = 0.95 (M) (INPUT = 1.00)

JOB NAME 417279	TRUSS NAME T50Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 5 Jan 21 2021 M-Tek Industries, Inc. Thu Mar 18 16:38:10 2021 Page 2
ID:K77hYrav7ro?MDbCT?VfxHza o6-Hy6RaxCF0 oQXqcdppK3roviRv6AbsPNNDwR5JzZTT3

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	8.0		
E	TMWW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	6.0	9.0		
M	BMVW-t	MT20	6.0	9.0	4.50	2.00
N	BS-t	MIH6	5.0	12.5	0.25	6.00
O	BMVW-t	MT20	6.0	9.0		
P	BS-t	MIH6	5.0	12.5	0.25	5.75
Q	BMVW-t	MT20	6.0	9.0	4.50	2.00
R	BMVW-t	MT20	6.0	9.0		
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	28-9-8	-2058	-2058	---	BACK	VERT	TOTAL	---	C1
T	28-11-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
U	30-11-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
V	32-11-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
W	28-11-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
X	30-11-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Y	32-11-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
Z	35-11-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AA	38-11-12	-20	-20	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

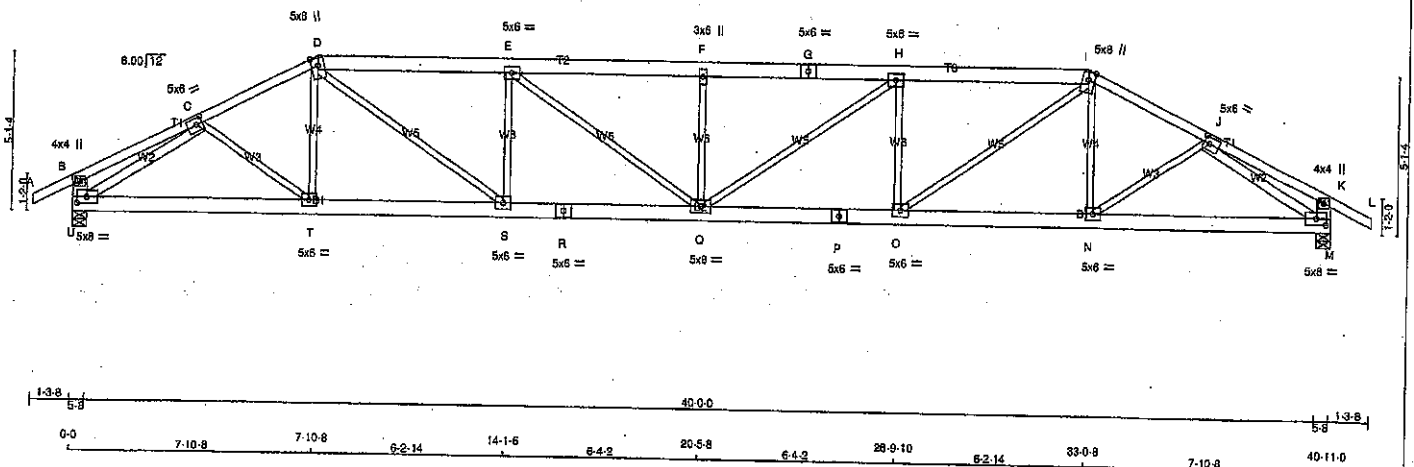
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2107871 *212*

JOB NAME 417279	TRUSS NAME T51	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:36:11 2021 Page 1	

ID:K77hyrav7ro?MDbCT?VixHza_o6I8fqhIDnIxfH8pBONWriOqRzyJYKfKfXctg_diz2fT2
Scale = 1:882



TOTAL WEIGHT = 2 X 203 = 406 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x6	DRY	No.2	SPF	
G - I	2x6	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - P	2x6	DRY	No.2	SPF	
P - M	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
U - C	2x4	DRY	No.2	SPF	
J - M	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	4.0	4.0		
C TMWW+/-	MT20	5.0	6.0	2.50	2.25
D TTWW+m	MT20	5.0	8.0	Edge	
E TMWW+/-	MT20	5.0	6.0		
F TMWW+/-	MT20	3.0	6.0		
G TS-t	MT20	5.0	6.0		
H TMWW+/-	MT20	5.0	6.0		
I TTWW+m	MT20	5.0	8.0	Edge	
J TMWW+/-	MT20	5.0	6.0	2.50	2.25
K TMV+p	MT20	4.0	4.0		
M BMVW1-t	MT20	5.0	8.0	2.50	3.75
N, O, S, T					
N BMVW-t	MT20	5.0	6.0		
P BS-t	MT20	5.0	6.0		
Q BMVW-t	MT20	5.0	8.0		
R BS-t	MT20	5.0	6.0		
U BMVW1-t	MT20	5.0	8.0	2.50	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
U	2380	0	2380	0	0	5-8	5-8	5-8
M	2380	0	2380	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1881	1116 : 0	0 : 0	0 : 0	0 : 0	565 : 0	0 : 0
M	1881	1116 : 0	0 : 0	0 : 0	0 : 0	565 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-T	0 / 169	0.04 (4)
B-C	0 / 16	-91.8	-91.8 0.20 (1)	10.00	T-D	0 / 99	0.03 (4)
C-D	-3449 / 0	-91.8	-91.8 0.37 (1)	3.49	N-I	0 / 99	0.03 (4)
D-E	-4580 / 0	-91.8	-91.8 0.39 (1)	3.81	N-J	0 / 169	0.04 (4)
E-F	-5012 / 0	-91.8	-91.8 0.39 (1)	3.83	U-C	-3588 / 0	0.91 (1)
F-G	-5012 / 0	-91.8	-91.8 0.39 (1)	3.83	J-M	-3588 / 0	0.91 (1)
G-H	-5012 / 0	-91.8	-91.8 0.39 (1)	3.83	O-I	0 / 1849	0.42 (1)
H-I	-4580 / 0	-91.8	-91.8 0.39 (1)	3.81	D-S	0 / 1849	0.42 (1)
I-J	-3449 / 0	-91.8	-91.8 0.37 (1)	3.49	O-H	-1001 / 0	0.33 (1)
J-K	0 / 16	-91.8	-91.8 0.20 (1)	10.00	S-E	-1001 / 0	0.33 (1)
K-L	0 / 28	-91.8	-91.8 0.12 (1)	10.00	Q-H	0 / 562	0.13 (1)
U-B	-270 / 0	0.0	0.0 0.02 (1)	7.81	E-Q	0 / 562	0.13 (1)
M-K	-270 / 0	0.0	0.0 0.02 (1)	7.81	Q-F	-549 / 0	0.18 (1)
U-T	0 / 2944	-18.5	-18.5 0.42 (1)	10.00			
T-S	0 / 3080	-18.5	-18.5 0.41 (1)	10.00			
S-R	0 / 4559	-18.5	-18.5 0.60 (1)	10.00			
R-Q	0 / 4559	-18.5	-18.5 0.60 (1)	10.00			
Q-P	0 / 4559	-18.5	-18.5 0.60 (1)	10.00			
P-O	0 / 4559	-18.5	-18.5 0.60 (1)	10.00			
O-N	0 / 3080	-18.5	-18.5 0.41 (1)	10.00			
N-M	0 / 2944	-18.5	-18.5 0.42 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = U/999 (0.27")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = U/978 (0.50")

CS: TC=0.39/1.00 (F:H:1), BC=0.60/1.00 (D:S:1),
WB=0.91/1.00 (U:M:1), SS=0.20/1.00 (H:I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

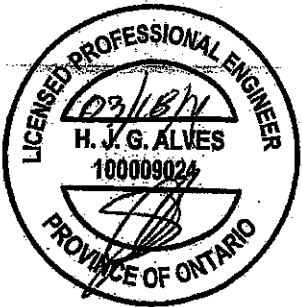
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (C) (INPUT = 0.90)
JSI METAL=0.83 (R) (INPUT = 1.00)



Structural component only
DWG# T-2107872

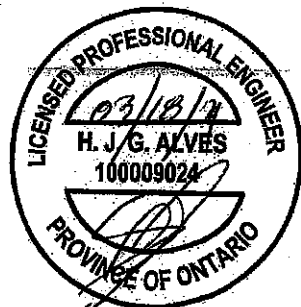
Tamarack Roof Truss, Burlington



ALL WEBS	2x9	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

NOTES- (1)
(1) Lateral braces to be a minimum of 2X4 SPF #2.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

U-T	0/0	-18.5	-18.5	0.08 (1)	10.00	E-P	-8/0	0.00 (1)
T-S	0/3071	-18.5	-18.5	0.43 (1)	10.00			
R-R	0/2989	-18.5	-18.5	0.41 (1)	10.00			
S-Q	0/2989	-18.5	-18.5	0.41 (1)	10.00			
Q-P	0/4042	-18.5	-18.5	0.54 (1)	10.00			
P-O	0/2989	-18.5	-18.5	0.41 (1)	10.00			
O-N	0/2989	-18.5	-18.5	0.41 (1)	10.00			
N-M	0/3071	-18.5	-18.5	0.43 (1)	10.00			
M-L	0/0	-18.5	-18.5	0.08 (1)	10.00			

DESIGN CRITERIA

RECEIVED 1988

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.22")
ALLOWABLE DEFL.(TL)= $L/360$ (1.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.42")

CSI: TC=0.61/1.00 (B-C:1), BC=0.54/1.00 (P-Q:1),
WB=0.70/1.00 (B-T:1), SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

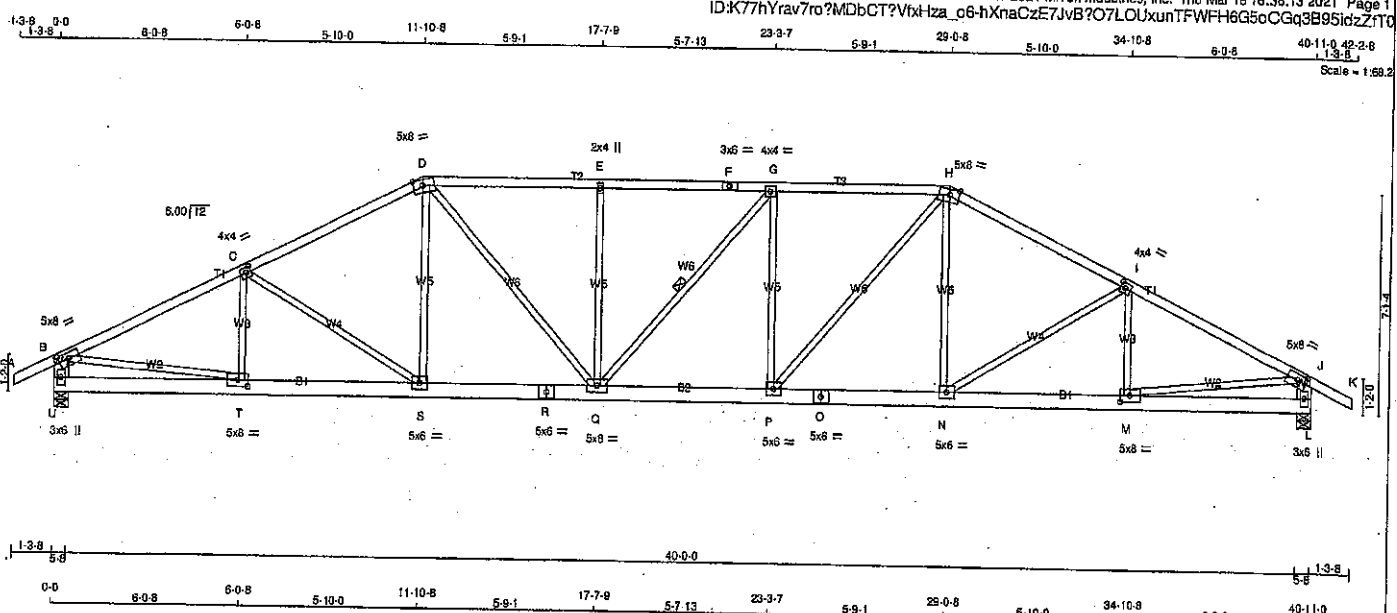
NAIL VALUES	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.89 (H) (INPUT = 0.90)
ISI METAL= 0.57 (B) (INPUT = 1.00)

JOB NAME 417279	TRUSS NAME T53	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:38:13 2021 Page 1	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
E - F	2x4	DRY	No.2	SPF		
G - H	2x4	DRY	No.2	SPF		
I - K	2x4	DRY	No.2	SPF		
L - S	2x6	DRY	No.2	SPF		
T - U	2x6	DRY	No.2	SPF		
V - W	2x6	DRY	No.2	SPF		
X - Y	2x6	DRY	No.2	SPF		
Z - L	2x6	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.75
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	5.0	8.0	2.25	3.75
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	5.0	8.0	2.25	3.75
L	BMV1-p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	8.0	2.50	3.75
N, P, S						
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	8.0	2.50	3.75
U	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT		
U	2380	0	2380	0	5-8	5-8
L	2380	0	2380	0	5-8	5-8

UNFACTORED REACTIONS						
1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
U	1681	1116 / 0	0 / 0	0 / 0	0 / 0	565 / 0
L	1681	1116 / 0	0 / 0	0 / 0	0 / 0	565 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO	MAX. UNBRAC LENGTH	FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	T-C	-325 / 0 0.08 (1)	
B-C	-3487 / 0	-91.8 -91.8	0.66 (1)	3.22	C-S	-365 / 0 0.34 (1)	
C-D	-3201 / 0	-91.8 -91.8	0.80 (1)	3.40	S-D	0 / 342 0.08 (1)	
D-E	-3404 / 0	-91.8 -91.8	0.54 (1)	3.38	D-Q	0 / 865 0.19 (1)	
E-F	-3404 / 0	-91.8 -91.8	0.50 (1)	3.41	Q-E	-568 / 0 0.48 (1)	
F-G	-3404 / 0	-91.8 -91.8	0.80 (1)	3.41	C-G	-9 / 0 0.00 (1)	
G-H	-3409 / 0	-91.8 -91.8	0.54 (1)	3.36	P-G	-563 / 0 0.45 (1)	
H-I	-3200 / 0	-91.8 -91.8	0.80 (1)	3.40	P-H	0 / 875 0.20 (1)	
I-J	-3487 / 0	-91.8 -91.8	0.66 (1)	3.22	N-H	0 / 335 0.08 (1)	
J-K	0 / 28	-91.8 -91.8	0.12 (1)	10.00	N-I	-367 / 0 0.34 (1)	
U-B	-2314 / 0	0.0 0.0	0.15 (1)	6.74	M-I	-323 / 0 0.08 (1)	
L-J	-2315 / 0	0.0 0.0	0.15 (1)	6.74	B-T	0 / 3169 0.71 (1)	
					M-J	0 / 3170 0.71 (1)	
U-T	0 / 0	-18.5 -18.5	0.08 (4)	10.00			
T-S	0 / 3143	-18.5 -18.5	0.43 (1)	10.00			
S-R	0 / 2841	-18.5 -18.5	0.39 (1)	10.00			
R-Q	0 / 2841	-18.5 -18.5	0.39 (1)	10.00			
Q-P	0 / 3409	-18.5 -18.5	0.45 (1)	10.00			
P-O	0 / 2840	-18.5 -18.5	0.39 (1)	10.00			
O-N	0 / 2840	-18.5 -18.5	0.38 (1)	10.00			
N-M	0 / 3144	-18.5 -18.5	0.43 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 068-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.40")

CSI: TC=0.66/1.00 (I-J:1), BC=0.45/1.00 (P-Q:1), WB=0.71/1.00 (J-M:1), SS=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

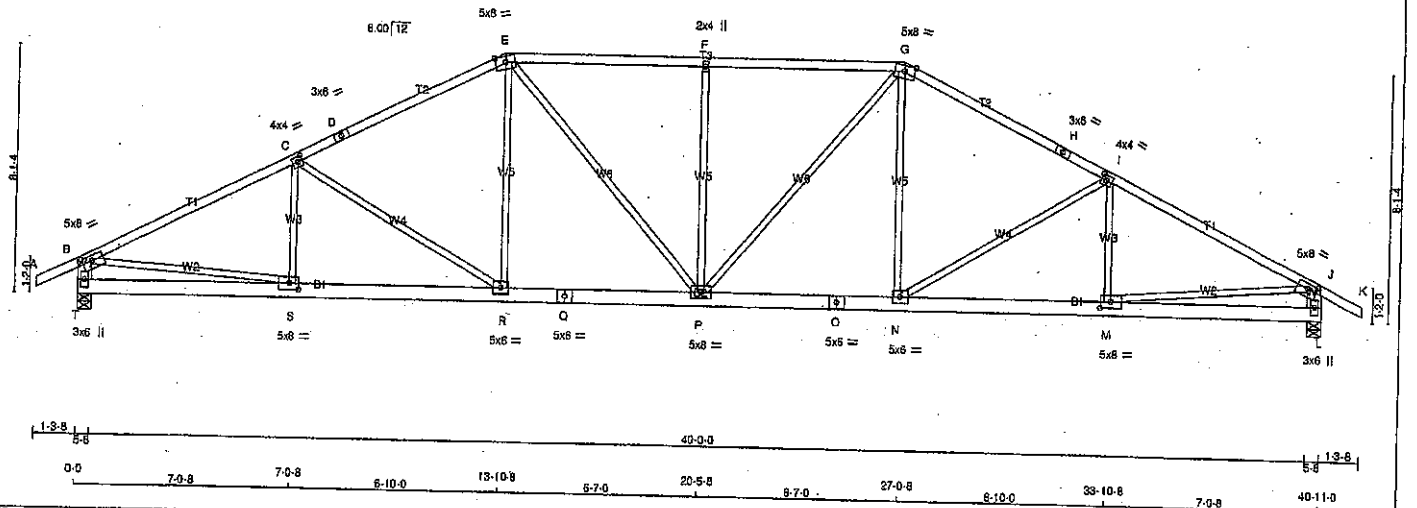
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.65 (J) (INPUT = 1.00)



Structural component only
DWG# T-2107874

JOB NAME 417279	TRUSS NAME T54	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 M Tek Industries, Inc. Thu Mar 18 16:36:14 2021 Page 1					
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Scale = 1:80.0					



TOTAL WEIGHT = 2 X 193 = 386 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
T - Q	2x6	DRY	No.2
Q - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQ'D
	GROSS REACTION	VERT	GROSS REACTION	DOWN	BRG	BRG
JT						
T	2380	0	2380	0	5-8	5-8
L	2380	0	2380	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	PERM. LIVE		
JT	1681	1116 / 0	0 / 0	585 / 0	0 / 0
T	1681	1116 / 0	0 / 0	585 / 0	0 / 0
L	1681	1116 / 0	0 / 0	585 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	S-C	-239 / 34	0.07 (1)	0.07 (1)
B-C	-3520 / 0	-91.8	-91.8 0.95 (1)	C-R	-581 / 0	0.81 (1)	0.81 (1)
C-D	-3038 / 0	-91.8	-91.8 0.82 (1)	E-P	0 / 457	0.10 (1)	0.10 (1)
D-E	-3038 / 0	-91.8	-91.8 0.82 (1)	P-G	-742 / 0	0.89 (1)	0.89 (1)
E-F	-3049 / 0	-91.8	-91.8 0.59 (1)	G-H	0 / 551	0.12 (1)	0.12 (1)
F-G	-3049 / 0	-91.8	-91.8 0.69 (1)	H-I	-3038 / 0	0.10 (1)	0.10 (1)
G-H	-3038 / 0	-91.8	-91.8 0.82 (1)	I-J	-3038 / 0	0.81 (1)	0.81 (1)
H-I	-3038 / 0	-91.8	-91.8 0.82 (1)	J-K	-3520 / 0	0.07 (1)	0.07 (1)
I-J	-3520 / 0	-91.8	-91.8 0.95 (1)	K-L	0 / 28	0.72 (1)	0.72 (1)
J-K	0 / 28	-91.8	-91.8 0.12 (1)	L-J	-2312 / 0	0.00	0.00
K-L	-2312 / 0	0.00	0.00 0.15 (1)				
L-J	-2312 / 0	0.00	0.00 0.15 (1)				
T-S	0 / 0	-18.5	-18.5 0.10 (4)				
S-R	0 / 3179	-18.5	-18.5 0.43 (1)				
R-Q	0 / 2691	-18.5	-18.5 0.38 (1)				
Q-P	0 / 2691	-18.5	-18.5 0.38 (1)				
P-O	0 / 2691	-18.5	-18.5 0.38 (1)				
O-N	0 / 2691	-18.5	-18.5 0.38 (1)				
N-M	0 / 3179	-18.5	-18.5 0.43 (1)				
M-L	0 / 0	-18.5	-18.5 0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	DL = 25.6 PSF
	LL = 6.0 PSF
BOT CH.	DL = 0.0 PSF
	LL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.98")
CALCULATED VERT. DEFL. (LL) = 1/998 (0.20")
ALLOWABLE DEFL. (TL) = L/360 (1.98")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.37")

CSI: TC=0.95/1.00 (B-C:1), BC=0.43/1.00 (M-N:1), WB=0.89/1.00 (F-P:1), SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN MAX MIN MAX MIN		
MT20	650 371 1747 768 1987 1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (M) (INPUT = 0.90)
JSI METAL = 0.68 (B) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	
F	TMVW-w	MT20	5.0	8.0		
G	TTWW-m	MT20	5.0	8.0	Edge	
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.75
J	TMVW-t	MT20	5.0	8.0	2.00	3.75
L	BMV1-p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	8.0	2.50	3.75
N	BMVW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	8.0		
P	BMVW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	8.0		
R	BMVW-t	MT20	5.0	8.0		
S	BMVW-t	MT20	5.0	8.0	2.50	3.75
T	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

- 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2107875

[illegible]

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.							
<u>BEARINGS</u>							
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
JT							
V	2380	0	2380	0	0	5-8	5-8
N	2380	0	2380	0	0	5-8	5-8

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

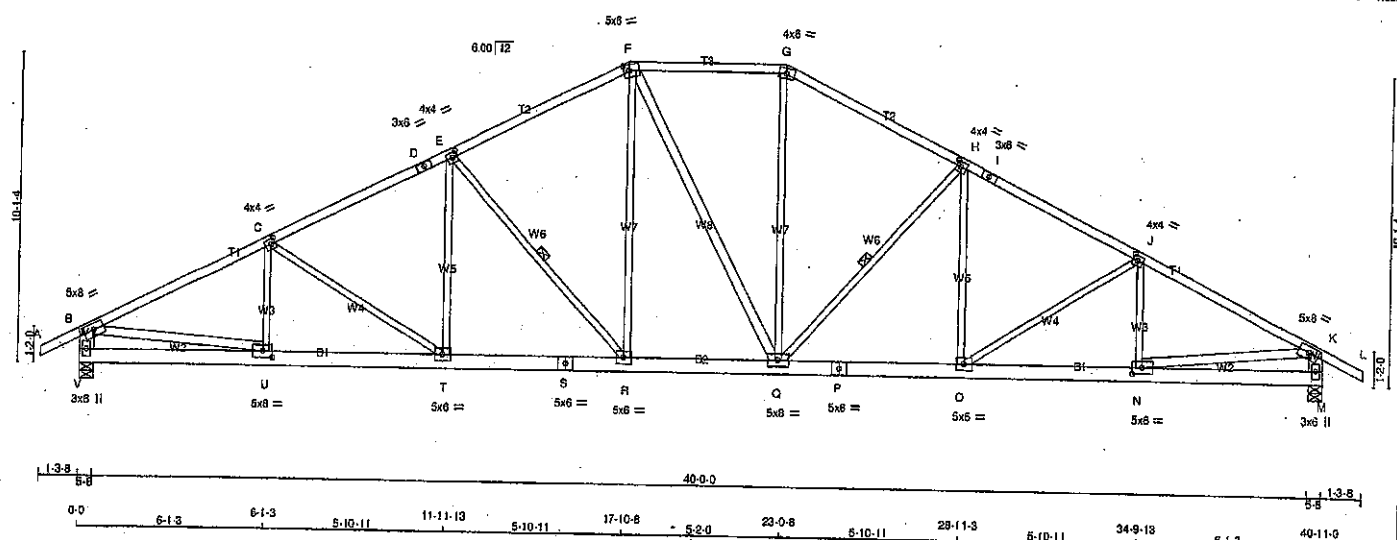
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

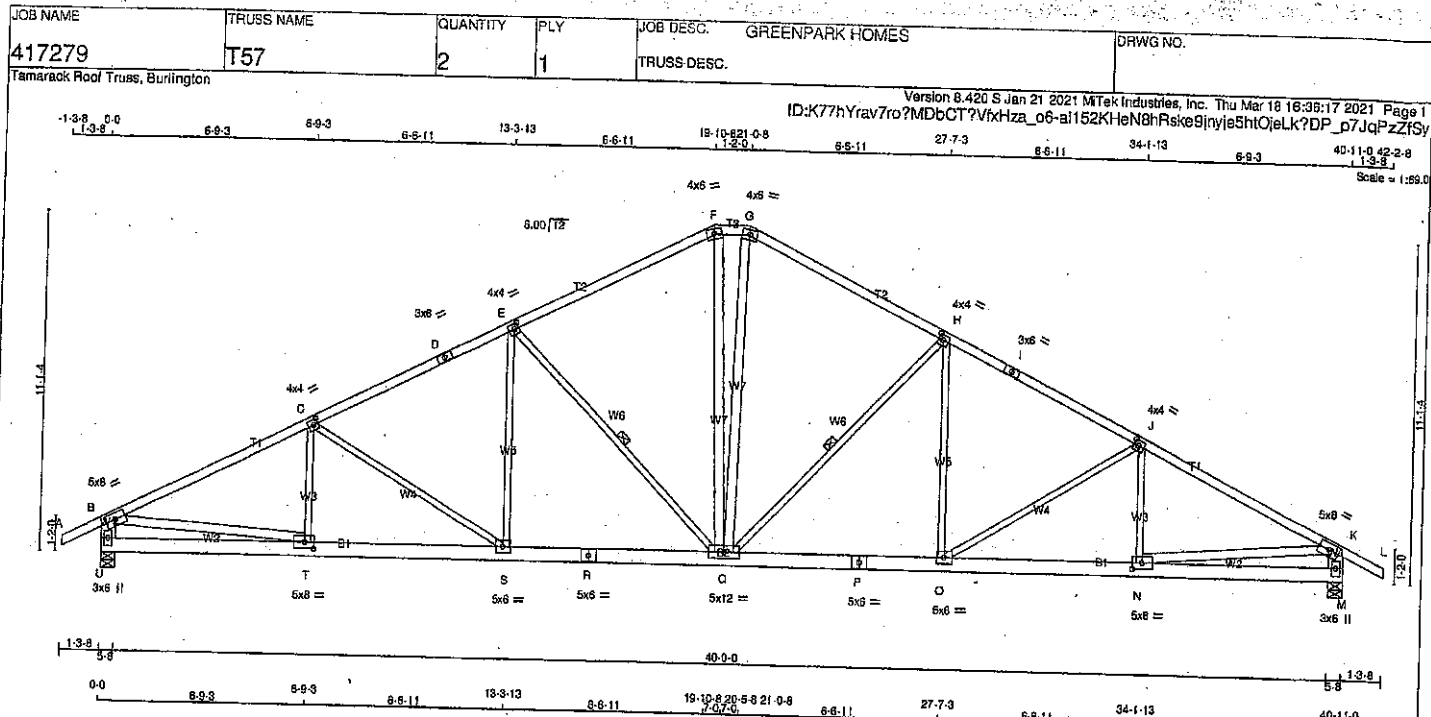
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-R, C-V, K-N.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 26	-91.8	-91.8	0.12 (1)	10.00	C-U	-75 / 49	0.02 (1)	
B-C	0 / 20	-91.8	-91.8	0.32 (1)	10.00	E-U	0 / 227	0.05 (4)	
C-D	-3387 / 0	-91.8	-91.8	0.47 (1)	3.48	E-S	-649 / 0	0.89 (1)	
D-E	-3387 / 0	-91.8	-91.8	0.47 (1)	3.48	S-F	0 / 624	0.14 (1)	
E-F	-2834 / 0	-91.8	-91.8	0.42 (1)	3.79	F-R	0 / 315	0.07 (1)	
F-G	-2668 / 0	-91.8	-91.8	0.33 (1)	3.96	R-G	-508 / 0	0.26 (1)	
G-H	-2668 / 0	-91.8	-91.8	0.33 (1)	3.96	R-H	0 / 315	0.07 (1)	
H-I	-2834 / 0	-91.8	-91.8	0.42 (1)	3.79	Q-H	0 / 624	0.14 (1)	
I-J	-3387 / 0	-91.8	-91.8	0.47 (1)	3.48	I-O	-649 / 0	0.89 (1)	
J-K	-3387 / 0	-91.8	-91.8	0.47 (1)	3.48	I-O	0 / 227	0.06 (4)	
K-L	0 / 20	-91.8	-91.8	0.32 (1)	10.00	O-K	-75 / 49	0.02 (1)	
L-M	0 / 28	-91.8	-91.8	0.12 (1)	10.00	V-C	-3883 / 0	0.70 (1)	
M-N	-325 / 0	0.0	0.0	0.02 (1)	7.81	K-N	-3863 / 0	0.70 (1)	
N-L	-325 / 0	0.0	0.0	0.02 (1)	7.81				
V-U	0 / 3077	-18.5	-18.5	0.45 (1)	10.00				
U-T	0 / 2941	-18.5	-18.5	0.41 (1)	10.00				
T-S	0 / 2941	-18.5	-18.5	0.41 (1)	10.00				
S-R	0 / 2522	-18.5	-18.5	0.34 (1)	10.00				
R-Q	0 / 2522	-18.5	-18.5	0.34 (1)	10.00				
Q-P	0 / 2941	-18.5	-18.5	0.41 (1)	10.00				
P-O	0 / 2941	-18.5	-18.5	0.41 (1)	10.00				
O-N	0 / 3077	-18.5	-18.5	0.45 (1)	10.00				



JOB NAME 417279	TRUSS NAME T56	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 M Tek Industries, Inc. Thu Mar 18 16:36:16 2021 Page 1 ID:K77hYrav7ro?MDbCT?VfxHza_o6-66Tjq_G0ccZaFb3z93RU5t8mNK18?dDGI9NIHyZISZ	





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
B - T	2x4	DRY	No.2
N - K	2x4	DRY	No.2
F - Q	2x4	DRY	No.2
Q - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-I	MT20	5.0	8.0		
C, E, H, J					
O TMVW-I	MT20	4.0	4.0	2.00	1.75
D TS-I	MT20	3.0	6.0		
F TTW-m	MT20	4.0	6.0		
G TTW-m	MT20	4.0	6.0		
I TS-I	MT20	3.0	6.0		
K TMVW-I	MT20	5.0	8.0		
M BMV1-p	MT20	3.0	6.0		
N BMVW-I	MT20	5.0	8.0	2.50	3.75
O BMVW-I	MT20	5.0	8.0		
P BS-I	MT20	5.0	6.0		
Q BMVWW-I	MT20	5.0	12.0		
R BS-I	MT20	5.0	6.0		
S BMVW-I	MT20	5.0	6.0		
T BMVW-I	MT20	5.0	8.0	2.50	3.75
U BMV1-p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral brace to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT		
U	2380	0	2380	0	5-8	5-8
M	2380	0	2380	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1681	1116 / 0	0 / 0	0 / 0	0 / 0	565 / 0	0 / 0
M	1681	1116 / 0	0 / 0	0 / 0	0 / 0	565 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 28			T-C	-247 / 24	0.07 (1)	
B-C	-3502 / 0	-91.8	-91.8 0.12 (1)	C-S	-442 / 0	0.54 (1)	
C-D	-3111 / 0	-91.8	-91.8 0.84 (1)	S-E	0 / 361	0.08 (1)	
D-E	-3111 / 0	-91.8	-91.8 0.87 (1)	E-Q	-959 / 0	0.72 (1)	
E-F	-2398 / 0	-91.8	-91.8 0.81 (1)	Q-H	-959 / 0	0.72 (1)	
F-G	-2160 / 0	-91.8	-91.8 0.81 (1)	H-I	-442 / 0	0.54 (1)	
G-H	-2398 / 0	-91.8	-91.8 0.08 (1)	I-J	-247 / 24	0.07 (1)	
H-I	-3111 / 0	-91.8	-91.8 0.87 (1)	J-K	0 / 3176	0.51 (1)	
I-J	-3111 / 0	-91.8	-91.8 0.87 (1)	K-L	0 / 3176	0.51 (1)	
J-K	-3502 / 0	-91.8	-91.8 0.12 (1)	L-M	0 / 768	0.12 (1)	
K-L	0 / 28			M-N	0 / 767	0.12 (1)	
L-M	-2316 / 0	0.0	0.0 0.15 (1)				
M-N	-2316 / 0	0.0	0.0 0.15 (1)				
N-O	0 / 0						
O-P	0 / 3155	-18.5	-18.5 0.09 (4)				
P-Q	0 / 2782	-18.5	-18.5 0.43 (1)				
Q-R	0 / 2782	-18.5	-18.5 0.40 (1)				
R-S	0 / 2782	-18.5	-18.5 0.40 (1)				
S-T	0 / 2782	-18.5	-18.5 0.40 (1)				
T-U	0 / 2782	-18.5	-18.5 0.40 (1)				
U-V	0 / 2782	-18.5	-18.5 0.40 (1)				
V-W	0 / 3155	-18.5	-18.5 0.43 (1)				
W-X	0 / 0						

TOTAL WEIGHT = 2 X 215 = 430 LB

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")

CALCULATED VERT. DEFL.(LL) = L/999 (0.17")

ALLOWABLE DEFL.(TL) = L/360 (1.36")

CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.84/1.00 (B-C:1), BC=0.43/1.00 (S-T:1), W6=0.72/1.00 (E-Q:1), SSI=0.26/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

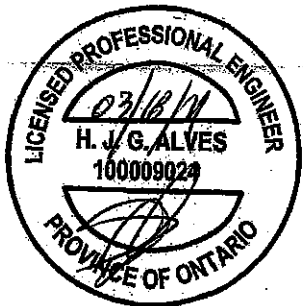
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (N) (INPUT = 0.90)

JSI METAL = 0.69 (B) (INPUT = 1.00)



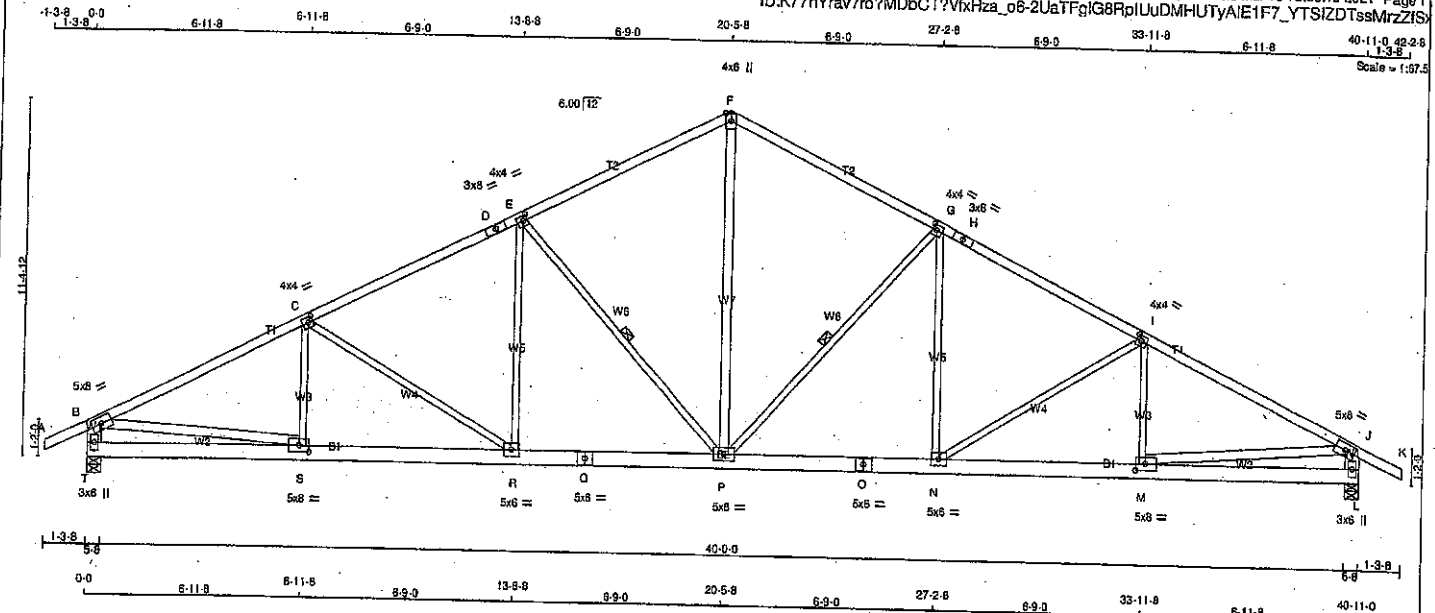
Structural component only

DWG# T-2107878

JOB NAME 417279	TRUSS NAME T58	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 5.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:36:18 2021 Page 1
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LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - L	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
T - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
P - F	2x4	DRY	No.2	SPF	
B - S	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-1	MT20	5.0	8.0		
C, E, G, I					
C TMVW-1	MT20	4.0	4.0	2.00	1.75
D TS-1	MT20	3.0	8.0		
F TTW-p	MT20	4.0	8.0	Edge	
H TS-1	MT20	3.0	8.0		
J TMVW-1	MT20	5.0	8.0		
L BMV1-p	MT20	3.0	6.0		
M BMVW-1	MT20	5.0	8.0	2.50	3.75
N BMVW-1	MT20	5.0	6.0		
O BS-1	MT20	5.0	6.0		
P BMVW-1	MT20	5.0	8.0		
Q BS-1	MT20	5.0	6.0		
R BMVW-1	MT20	5.0	8.0	2.50	3.75
S BMVW-1	MT20	5.0	8.0	2.50	3.75
T BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT		
JT	2380	0	2380	0	5-8	5-8
T	2380	0	2380	0	5-8	5-8
L	2380	0	2380	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	1681	1116 / 0	0 / 0	0 / 0	0 / 0	565 / 0
T	1681	1116 / 0	0 / 0	0 / 0	0 / 0	565 / 0
L	1681	1116 / 0	0 / 0	0 / 0	0 / 0	565 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-P, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
		FROM	TO				
FR-TO							
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	P-F	0 / 1808
B-C	-3506 / 0	-91.8	-91.8	0.90 (1)	3.00	P-G	-979 / 0
C-D	-3079 / 0	-91.8	-91.8	0.71 (1)	3.41	N-G	0 / 379
D-E	-3079 / 0	-91.8	-91.8	0.71 (1)	3.41	N-I	-480 / 0
E-F	-2380 / 0	-91.8	-91.8	0.65 (1)	3.81	M-I	-231 / 34
F-G	-2380 / 0	-91.8	-91.8	0.65 (1)	3.81	E-P	-979 / 0
G-H	-3079 / 0	-91.8	-91.8	0.71 (1)	3.41	R-E	0 / 379
H-I	-3079 / 0	-91.8	-91.8	0.71 (1)	3.41	C-R	-480 / 0
I-J	-3506 / 0	-91.8	-91.8	0.80 (1)	3.00	S-C	-231 / 34
J-K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	B-S	0 / 3179
T-B	-2315 / 0	0.0	0.0	0.15 (1)	6.74	M-J	0 / 3179
L-J	-2315 / 0	0.0	0.0	0.15 (1)	6.74		
T-S	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
S-R	0 / 3159	-18.5	-18.5	0.43 (1)	10.00		
R-Q	0 / 2753	-18.5	-18.5	0.38 (1)	10.00		
Q-P	0 / 2753	-18.5	-18.5	0.38 (1)	10.00		
P-O	0 / 2753	-18.5	-18.5	0.38 (1)	10.00		
O-N	0 / 2763	-18.5	-18.5	0.38 (1)	10.00		
N-M	0 / 3159	-18.5	-18.5	0.43 (1)	10.00		
M-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00		

TOTAL WEIGHT = 3 X 205 = 614 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.36")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.18")
ALLOWABLE DEFL. (TL) = L/360 (1.36")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.33")

CSI: TC=0.50/1.00 (B-C:1), BC=0.43/1.00 (R-S:1), WB=0.73/1.00 (E-P:1), SS=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

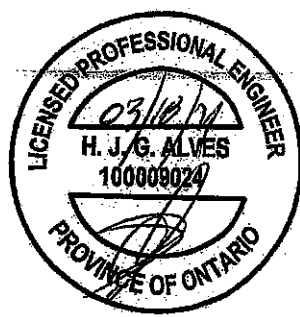
MAX MIN MAX MIN MAX MIN

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (M) (INPUT = 0.90)

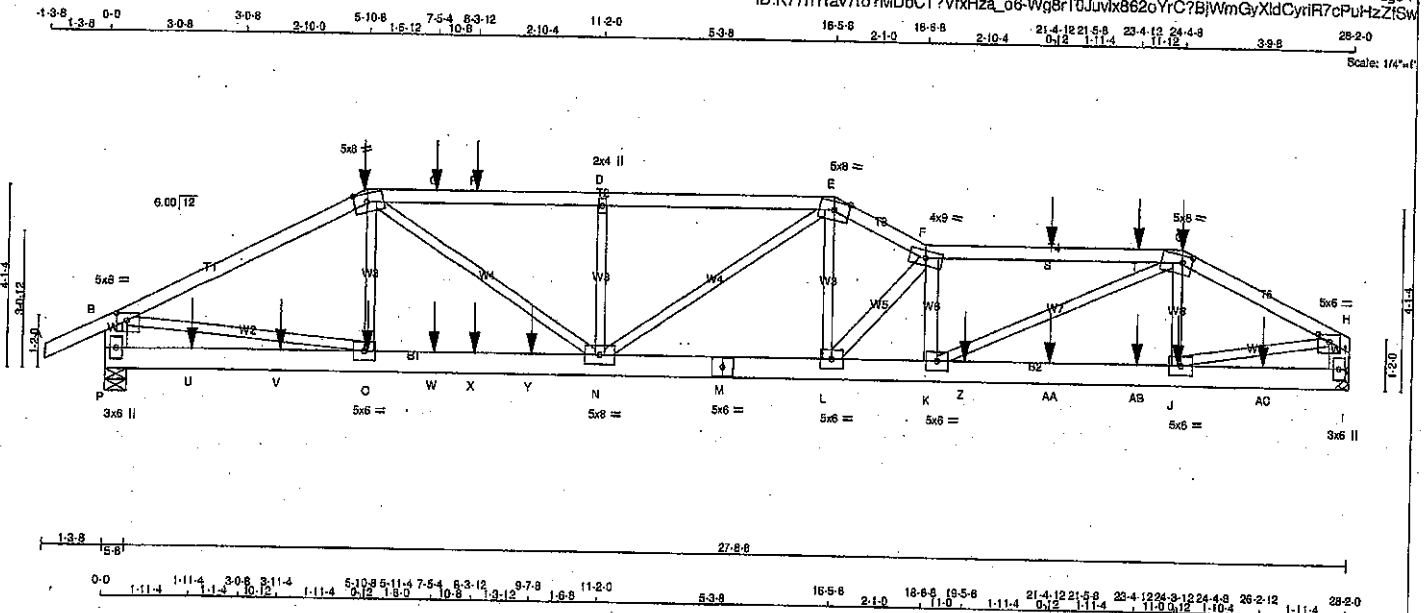
JSI METAL = 0.88 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107879

JOB NAME 417279	TRUSS NAME T59	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:K77hYrav7ro?MdbCT?VfxHza_o6-Wg8rT0Juvk862oYrC7BjWmGyXldCyrIR7cPuHzZfSW



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - F	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(81.0)
C-E	12	SIDE(81.0)
E-F	12	TOP
F-G	12	SIDE(81.0)
G-H	12	SIDE(81.0)
P-B	12	TOP
I-H	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
P-M	12	SIDE(183.1)
M-I	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	8	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	3040	0	5-8
P	2580	0	MECHANICAL
I	2580	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 4-0.

UNFACTORED REACTIONS

1ST LOOSE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
P	2151 1408 / 0 0 / 0 0 / 0 744 / 0 0 / 0
I	1824 1198 / 0 0 / 0 0 / 0 626 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	O-C	-38 / 160	0.03 (4)	
B-C	-4619 / 0	-91.8	-91.8	0.49 (1)	4.05	C-N	0 / 1996	0.25 (1)	
C-Q	-5761 / 0	-91.8	-91.8	0.49 (1)	3.63	N-D	-687 / 0	0.09 (1)	
Q-R	-5761 / 0	-91.8	-91.8	0.49 (1)	3.63	D-E	0 / 736	0.23 (1)	
R-D	-5761 / 0	-91.8	-91.8	0.49 (1)	3.63	E-F	0 / 1851	0.23 (1)	
D-E	-5761 / 0	-91.8	-91.8	0.46 (1)	3.66	F-S	-2403 / 0	0.20 (1)	
E-F	-5710 / 0	-91.8	-91.8	0.14 (1)	3.99	K-F	-1083 / 0	0.07 (1)	
F-S	-8568 / 0	-91.8	-91.8	0.85 (1)	3.28	K-G	0 / 3482	0.43 (1)	
S-T	-8568 / 0	-91.8	-91.8	0.85 (1)	3.28	J-G	-410 / 0	0.04 (1)	
T-G	-8568 / 0	-91.8	-91.8	0.65 (1)	3.28	B-O	0 / 4163	0.52 (1)	
G-H	-3842 / 0	-91.8	-91.8	0.19 (1)	4.68	J-H	0 / 3501	0.43 (1)	
P-S	-2998 / 0	0.0	0.0	0.11 (1)	7.81				
I-H	-2581 / 0	0.0	0.0	0.09 (1)	7.81				
P-U	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
U-V	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
V-O	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
O-W	0 / 4128	-18.5	-18.5	0.57 (1)	10.00				
W-X	0 / 4128	-18.5	-18.5	0.57 (1)	10.00				
X-Y	0 / 4128	-18.5	-18.5	0.57 (1)	10.00				
Y-N	0 / 4128	-18.5	-18.5	0.57 (1)	10.00				
N-M	0 / 5159	-18.5	-18.5	0.45 (1)	10.00				
M-L	0 / 5159	-18.5	-18.5	0.45 (1)	10.00				
L-K	0 / 8627	-18.5	-18.5	0.49 (1)	10.00				
K-Z	0 / 3418	-18.5	-18.5	0.38 (1)	10.00				
Z-AA	0 / 3418	-18.5	-18.5	0.38 (1)	10.00				
AA-AB	0 / 3418	-18.5	-18.5	0.38 (1)	10.00				
AB-J	0 / 3418	-18.5	-18.5	0.38 (1)	10.00				
J-AC	0 / 0	-18.5	-18.5	0.08 (1)	10.00				
AC-I	0 / 0	-18.5	-18.5	0.06 (1)	10.00				

LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT	5-10-8	-254	-254	---	BACK	VERT	C1
C	24-4-8	-108	-108	---	BACK	VERT	C1
G	24-3-12	-9	-9	---	BACK	VERT	C1
J	5-11-4	-21	-21	---	BACK	VERT	C1
O	7-5-4	-76	-76	---	BACK	VERT	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TO=0.85/1.00 (F-G:1), BC=0.57/1.00 (N-O:1), WB=0.52/1.00 (B-O:1), SSI=0.25/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

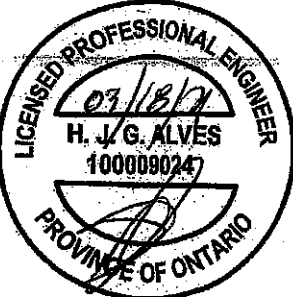
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (G) (INPUT = 0.90)
JSI METAL= 0.47 (O) (INPUT = 1.00)



Structural component only
DWG# T-2107880

JOB NAME 417279	TRUSS NAME T59	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	2.75
C	TTWW-m	MT20	5.0	8.0	2.25	3.50
D	TMW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	Edge	
F	TTWW-m	MT20	4.0	9.0		
G	TTWW-m	MT20	5.0	8.0	2.00	2.50
H	TMVW-p	MT20	5.0	8.0	2.00	3.00
I	BMV1-p	MT20	3.0	6.0		
J, K, L, O						
J	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	8.0		
P	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

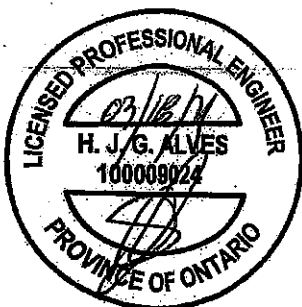
1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	8-3-12	-76	-76		BACK	VERT	TOTAL		C1
S	21-4-12	-27	-27		BACK	VERT	TOTAL		C1
T	23-4-12	-27	-27		BACK	VERT	TOTAL		C1
U	1-11-4	-20	-20		BACK	VERT	TOTAL		C1
V	3-11-4	-21	-21		BACK	VERT	TOTAL		C1
W	7-5-4	-21	-21		BACK	VERT	TOTAL		C1
X	8-3-12	-21	-21		BACK	VERT	TOTAL		C1
Y	9-7-8	-677	-677		BACK	VERT	TOTAL		C1
Z	19-5-8	-308	-308		BACK	VERT	TOTAL		C1
AA	21-4-12	-9	-9		BACK	VERT	TOTAL		C1
AB	23-4-12	-9	-9		BACK	VERT	TOTAL		C1
AC	26-2-12	-7	-7		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

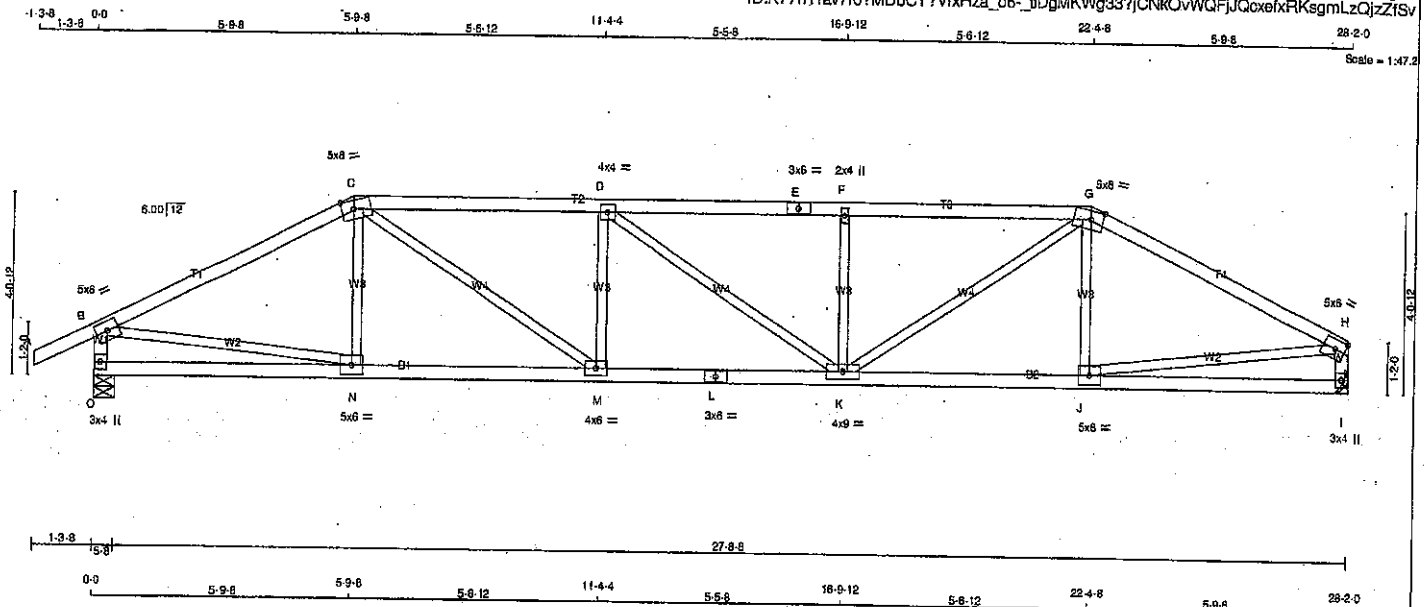
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2107880

JOB NAME 417279	TRUSS NAME T60	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER			
N.L.G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF
G - H	2x4 DRY	No.2	SPF
H - I	2x4 DRY	No.2	SPF
I - J	2x4 DRY	No.2	SPF
J - K	2x4 DRY	No.2	SPF
K - L	2x4 DRY	No.2	SPF
L - M	2x4 DRY	No.2	SPF
M - N	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-t	MT20	5.0	6.0	
C TTWW-m	MT20	5.0	8.0	2.25 3.25
D TMWW-t	MT20	4.0	4.0	
E TS-t	MT20	3.0	6.0	
F TMW-w	MT20	2.0	4.0	
G TTWW-m	MT20	5.0	8.0	2.25 3.25
H TMVW-t	MT20	5.0	6.0	Edge
I BMV1-p	MT20	3.0	4.0	
J BMWW-t	MT20	5.0	6.0	
K BMWW-m	MT20	4.0	9.0	
L BS-t	MT20	3.0	6.0	
M BMWW-t	MT20	4.0	6.0	
N BMWW-m	MT20	5.0	6.0	
O BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
 1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
O	1877	0	1877	0	5-8	
I	1853	0	1553	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LOOSE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
O	1184	790 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0
I	1098	721 / 0	0 / 0	0 / 0	0 / 0	377 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	UNBRACED LENGTH
FR-TO		FROM	TO	FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	N-C	-160 / 37	0.04 (1)
B-C	-2147 / 0	-91.8	-91.8 0.72 (1)	3.82	C-M	0 / 1150	0.26 (1)
C-D	-2867 / 0	-91.8	-91.8 0.56 (1)	3.60	M-D	-548 / 0	0.14 (1)
D-E	-2866 / 0	-91.8	-91.8 0.55 (1)	3.60	D-K	-2 / 0	0.00 (1)
E-F	-2866 / 0	-91.8	-91.8 0.55 (1)	3.60	K-F	-547 / 0	0.14 (1)
F-G	-2866 / 0	-91.8	-91.8 0.56 (1)	3.61	K-G	0 / 1148	0.26 (1)
G-H	-2147 / 0	-91.8	-91.8 0.72 (1)	3.82	J-G	-159 / 37	0.04 (1)
H-I	-1632 / 0	0.0	0.0 0.16 (1)	6.48	B-N	0 / 1939	0.44 (1)
I-H	-1507 / 0	0.0	0.0 0.15 (1)	6.68	J-H	0 / 1940	0.44 (1)
O-N	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
N-M	0 / 1915	-18.5	-18.5 0.38 (1)	10.00			
M-L	0 / 2867	-18.5	-18.5 0.52 (1)	10.00			
L-K	0 / 2867	-18.5	-18.5 0.52 (1)	10.00			
K-J	0 / 1915	-18.5	-18.5 0.38 (1)	10.00			
J-I	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 33.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2015, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 886-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.94")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
 ALLOWABLE DEFL.(TL) = L/360 (0.94")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.72/1.00 (G-H:1), BC=0.52/1.00 (K-M:1), WB=0.44/1.00 (H-I:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 850 371 1747 786 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

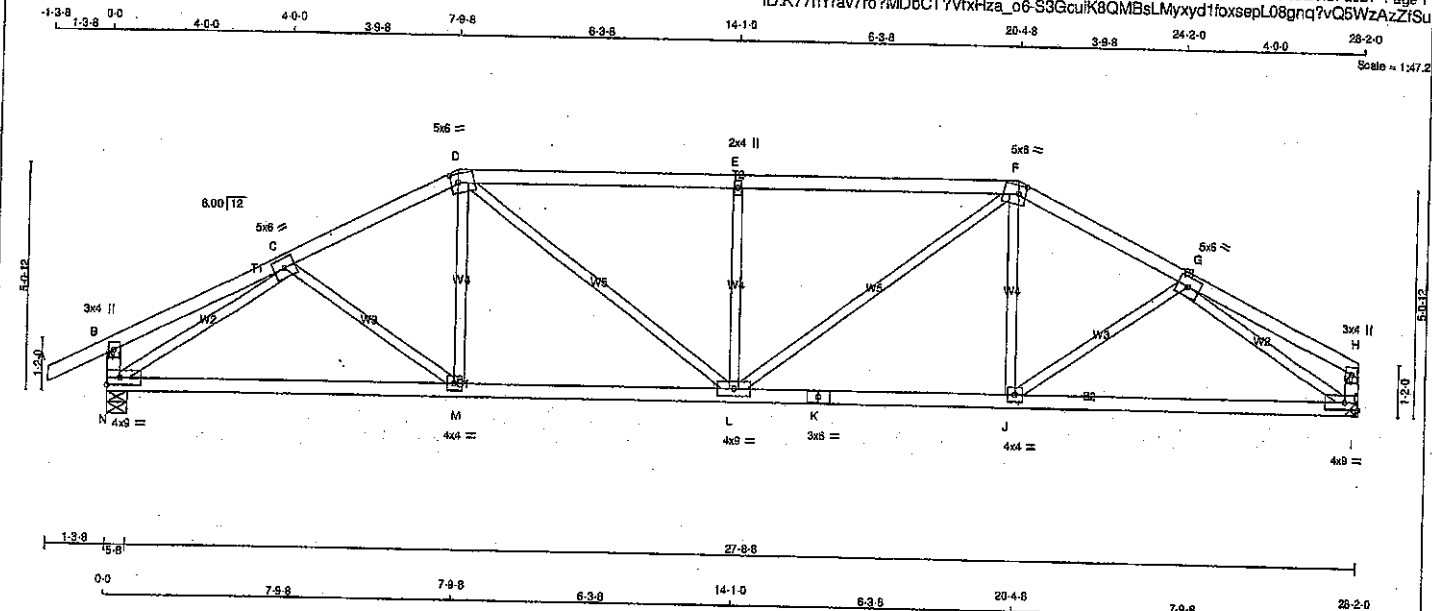
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (C) (INPUT = 0.90)
 JSI METAL = 0.92 (L) (INPUT = 1.00)



Structural component only
 DWG# T-2107881

JOB NAME 417279	TRUSS NAME T61	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
N - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	6.0		
D	TMVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-1	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW-1	MT20	4.0	9.0		Edge
J	BMVW-1	MT20	4.0	9.0		
K	BS-1	MT20	3.0	6.0		
L	BMVW-1	MT20	4.0	9.0		
M	BMVW-1	MT20	4.0	9.0		
N	BMVW-1	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
N	1677	0	1677	0	5-8
I	1553	0	1553	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3'-8".

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1164	780 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0
I	1098	721 / 0	0 / 0	0 / 0	0 / 0	377 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 28	C-M	-23 / 56
B-C	0 / 16	M-D	0 / 169
C-D	-2094 / 0	D-L	0 / 660
D-E	-2367 / 0	L-E	-709 / 0
E-F	-2367 / 0	F-G	0 / 660
F-G	-2094 / 0	G-H	0 / 169
G-H	0 / 16	H-I	-23 / 56
H-I	-269 / 0	I-J	-2311 / 0
I-J	-143 / 0	J-K	0 / 169
		K-L	-2311 / 0
		L-M	0 / 169
		M-N	-23 / 56
		N-O	0 / 169
		O-P	-23 / 56
		P-Q	0 / 169
		Q-R	-23 / 56
		R-S	0 / 169

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.94")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.11")
ALLOWABLE DEFL. (TL) = L/360 (0.94")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.20")

CSI: TC=0.56/1.00 (D-E:1), BC=0.44/1.00 (M-N:1), WB=0.87/1.00 (C-N:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

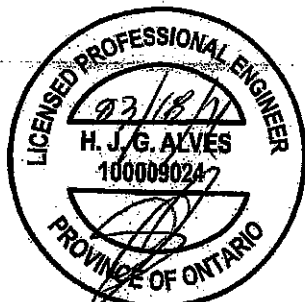
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1967 1873

PLATE PLACEMENT TOL = 0.250 inches

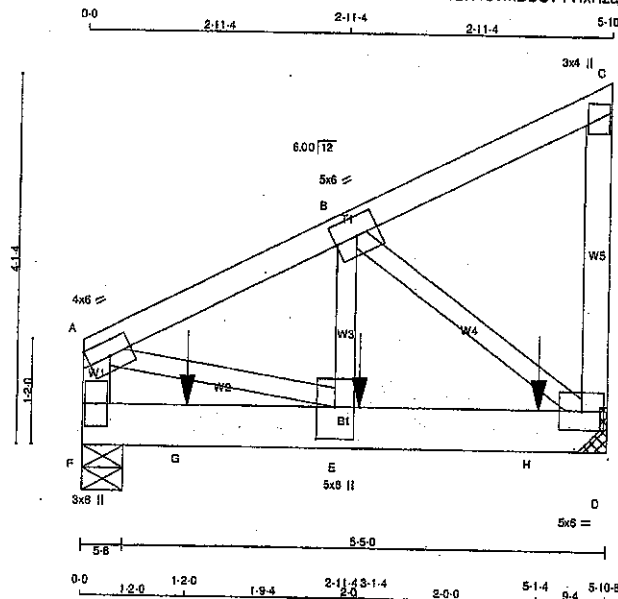
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.80 (K) (INPUT = 1.00)



Structural component only
DWG# T-2107882

JOB NAME 417279	TRUSS NAME T62	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
A - C	12	TOP
C - D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
B - E	3	SIDE (217.2)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-H	MT20	4.0	6.0		Edge
B	TMVW-H	MT20	5.0	6.0		
C	TMVW-p	MT20	3.0	4.0		
D	BMVW-H	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	3345	0	3345	0
D	2934	0	2934	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	2363	1565 / 0	0 / 0	0 / 0	0 / 0	797 / 0	0 / 0
D	2073	1375 / 0	0 / 0	0 / 0	0 / 0	698 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO		FROM TO					
F - A	-1967 / 0	0.0	0.0	0.11 (1)	7.81	A - E	0 / 2500
A - B	-2697 / 0	-91.8	-91.8	0.08 (1)	5.48	E - B	0 / 2757
B - C	-8 / 0	-91.8	-91.8	0.08 (1)	10.00	B - D	-3042 / 0
D - C	-121 / 0	0.0	0.0	0.01 (1)	7.81		
F - G	0 / 0	-18.5	-18.5	0.59 (1)	10.00		
G - E	0 / 0	-18.5	-18.5	0.59 (1)	10.00		
E - H	0 / 2418	-18.5	-18.5	0.43 (1)	10.00		
H - D	0 / 2418	-18.5	-18.5	0.43 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	3-1-4	-1083	-1083	---	FRONT	VERT	TOTAL	---	C1
G	1-2-0	-1809	-1809	---	FRONT	VERT	TOTAL	---	C1
H	5-1-4	-1085	-1085	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.11/1.00 (A-F:1), BC=0.59/1.00 (E-F:1),
WB=0.36/1.00 (B-D:1), SS=0.46/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MT20	850	371	1747
	788	1807	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.85 (B) (INPUT = 0.90)
JSI METAL = 0.44 (E) (INPUT = 1.00)



Structural component only
DWG# T-2107883

CONTINUED ON PAGE 2

JOB NAME 417279	TRUSS NAME T62	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

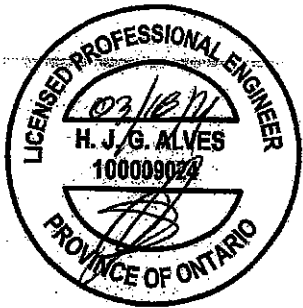
Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Thu Mar 18 18:36:22 2021 Page 2
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PLATES (table in inches)

PLATE TYPE	PLATES	W	LEN	Y	X
BMWV-t	MT20	5.0	8.0	4.25	2.50
BMV1+p	MT20	3.0	8.0		

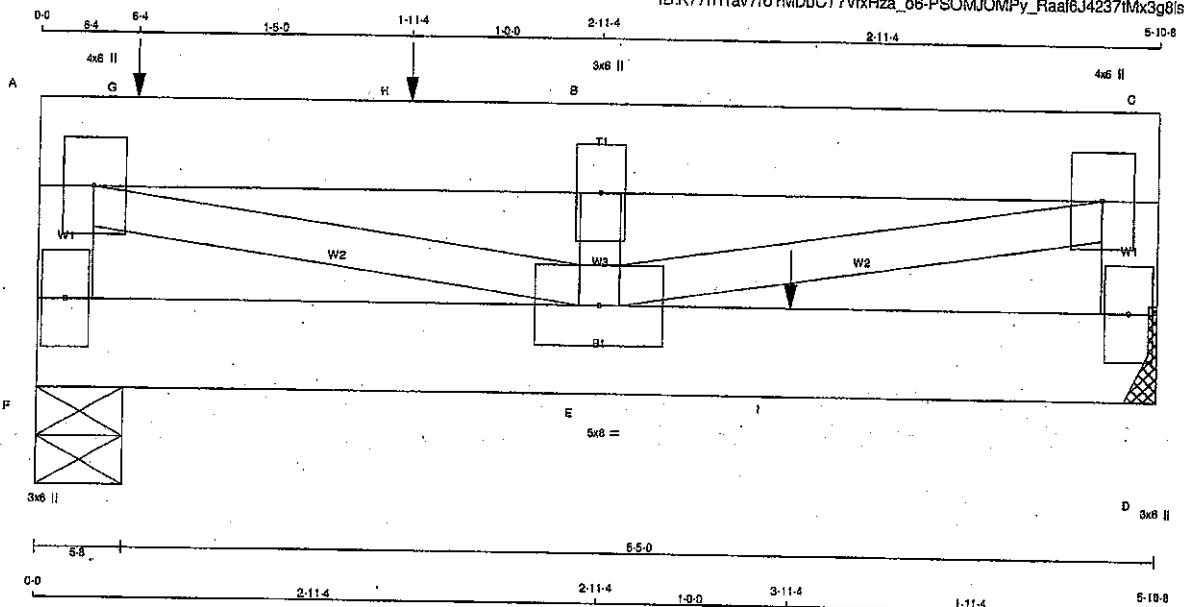
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2107883

JOB NAME 417279	TRUSS NAME T63	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 5 Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:36:23 2021 Page 1 ID:K77hYrav7ro7MDbCT7VfxHza_o6-PSOMJOMPy_Raaf6J4237IMx3g8is8ralMkad12zzZ18s	



TOTAL WEIGHT = 2 X 27 = 54 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF	
A - C	2x6	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - D	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
	DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A	12	TOP
C - D	12	TOP
A - C	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(10.5)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	8.0		
B	TMVW-w	MT20	3.0	8.0		
C	TMVW+p	MT20	4.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMVWW-w	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	1217	0	1217	0	5-8	5-8
D	973	0	973	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 2-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	863	552 / 0	0 / 0	0 / 0	0 / 0	312 / 0	0 / 0
D	692	433 / 0	0 / 0	0 / 0	0 / 0	259 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH
FR-TO		FROM TO				FR-TO		FROM TO			
F-A	-1120 / 0	0.0	0.0	0.06 (1)	7.81	A-E	0 / 1711	0.21 (1)			
A-G	-1613 / 0	-91.8	-91.8	0.22 (1)	6.25	E-B	-783 / 0	0.08 (1)			
G-H	-1613 / 0	-91.8	-91.8	0.22 (1)	6.25	E-C	0 / 1711	0.21 (1)			
H-B	-1613 / 0	-91.8	-91.8	0.22 (1)	8.25						
B-C	-1613 / 0	-91.8	-91.8	0.04 (1)	6.25						
D-C	-702 / 0	0.0	0.0	0.04 (1)	7.81						
F-E	0 / 0	-37.3	-37.3	0.05 (1)	10.00						
E-I	0 / 0	-37.3	-37.3	0.17 (1)	10.00						
I-D	0 / 0	-37.3	-37.3	0.17 (1)	10.00						

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	5-4	-150	-150		TOP	VERT	TOTAL		C1
H	1-11-4	-500	-500		TOP	VERT	TOTAL		C1
I	3-11-4	-359	-359		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABO 2016
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.22/1.00 (A-B:1), BC=0.17/1.00 (D-E:1), WB=0.21/1.00 (A-E:1), SS=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (A) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

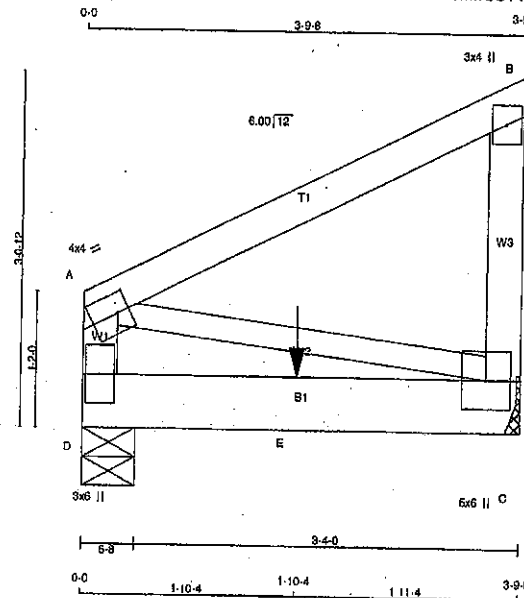


Structural component only
DWG# T-2107884

JOB NAME 417279	TRUSS NAME T64	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 5 Jan 21 2021 MiTek Industries, Inc. Thu Mar 18 16:36:24 2021 Page 1
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Scale = 1:18.2

TOTAL WEIGHT = 2 X 18 = 35 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
D - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
D - C	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1	12	TOP
A-B 1	12	TOP
B-C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMWV-1	MT20	4.0	4.0	2.00	1.25
B TMV-p	MT20	3.0	4.0		
C 8MWV1+p	MT20	5.0	6.0		
D 8MW1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT				
D 470	0	470	0	5-8
C 458	0	458	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
D	331	222 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	323	217 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO									
D-A	-174 / 0	0.0	0.0	0.01 (1)	7.81				
A-B	0 / 0	-91.8	-91.8	0.12 (1)	10.00				
C-B	-174 / 0	0.0	0.0	0.01 (1)	7.81				
D-E	0 / 0	-18.5	-18.5	0.19 (1)	10.00				
E-C	0 / 0	-18.5	-18.5	0.19 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-10-4	-359	-359		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/699 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.12/1.00 (A-B:1), BC=0.18/1.00 (C-D:1), WB=0.00/1.00 (A-C:1), SS=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

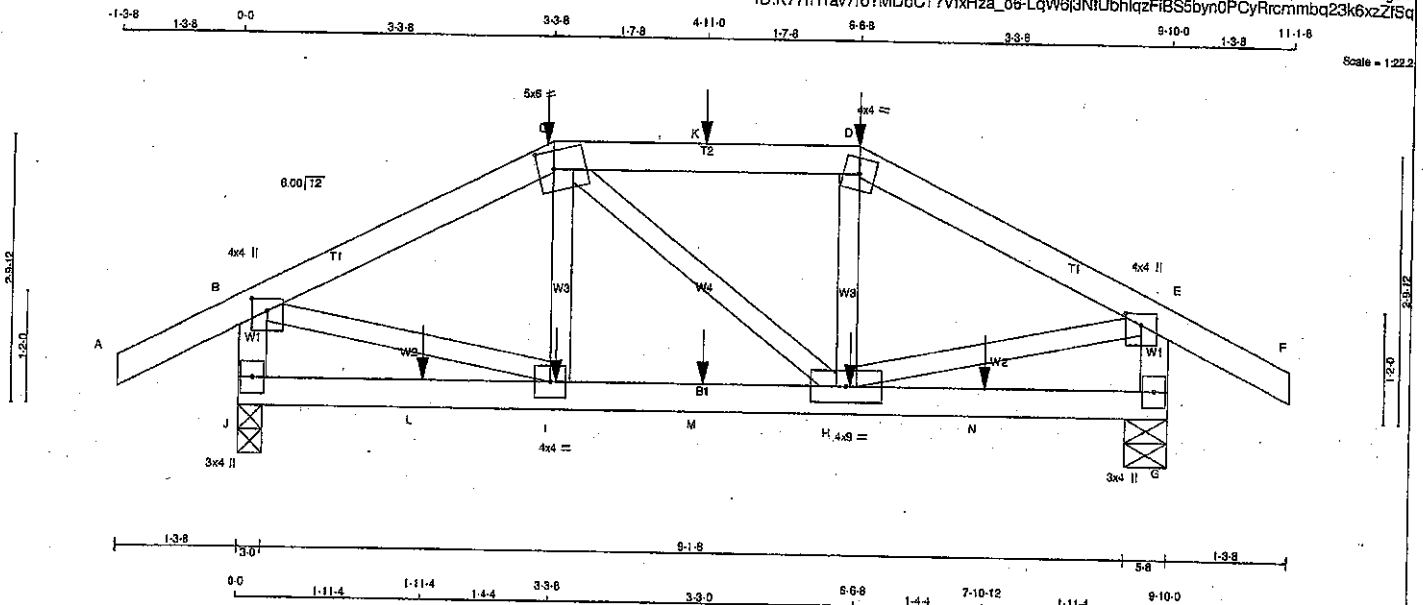
JSI GRIP = 0.05 (A) (INPUT = 0.90)
JSI METAL = 0.02 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107885

JOB NAME 417279	TRUSS NAME T65	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:K77hYrav7ro?MDbCT?VfxHza_06-LqW6j3NfUblhgzFIBS5byn0PCyRrcmmbq23k6xzZISq



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - B	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B - TMVW+p	MT20	4.0	4.0	1.50	2.00
C - TTWW-m	MT20	5.0	6.0	2.25	2.00
D - TTWW-m	MT20	4.0	4.0		
E - TMVW+p	MT20	4.0	4.0	1.50	2.00
G - BMV1+p	MT20	3.0	4.0		
H - BMVWW-t	MT20	4.0	9.0		
I - BMVWW-t	MT20	4.0	4.0		
J - BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
J	798	0	798	0	0	3-0	5-8	3-0	5-8
G	798	0	798	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	SNOW	LIVE
J	563	379 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0	563	379 / 0	0 / 0	0 / 0	184 / 0	0 / 0	563	379 / 0
G	563	379 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0	563	379 / 0	0 / 0	0 / 0	184 / 0	0 / 0	563	379 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (FLF)	MAX CSI (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (FLF)	MAX CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 28	-91.8	-91.8 0.13 (1)	I-C	-90 / 31	0.02 (1)	
B-C	-732 / 0	-91.8	-91.8 0.20 (1)	C-H	0 / 3	0.00 (4)	
C-K	-652 / 0	-91.8	-91.8 0.22 (1)	H-D	-87 / 34	0.02 (1)	
K-D	-652 / 0	-91.8	-91.8 0.22 (1)	H-E	0 / 678	0.17 (1)	
D-E	-737 / 0	-91.8	-91.8 0.20 (1)	B-I	0 / 675	0.17 (1)	
E-F	0 / 28	-91.8	-91.8 0.13 (1)				
G-E	-769 / 0	0.0	0.0 0.09 (1)				
J-B	-770 / 0	0.0	0.0 0.09 (1)				
J-L	0 / 0	-18.5	-18.5 0.06 (4)				
L-I	0 / 0	-18.5	-18.5 0.06 (4)				
I-M	0 / 660	-18.5	-18.5 0.13 (1)				
M-H	0 / 660	-18.5	-18.5 0.13 (1)				
H-N	0 / 0	-18.5	-18.5 0.06 (4)				
N-G	0 / 0	-18.5	-18.5 0.06 (4)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-3-8	-74	-74	---	BACK	VERT	TOTAL	---	C1
D	6-6-8	-74	-74	---	BACK	VERT	TOTAL	---	C1
H	6-5-12	-6	-6	---	BACK	VERT	TOTAL	---	C1
I	3-4-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
K	4-11-0	-15	-15	---	BACK	VERT	TOTAL	---	C1
L	1-11-4	-3	-3	---	BACK	VERT	TOTAL	---	C1
M	4-11-0	-6	-6	---	BACK	VERT	TOTAL	---	C1
N	7-10-12	-3	-3	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT. CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CSI: TC=0.22/1.00 (C-D:1), BC=0.13/1.00 (H-I:1), WB=0.17/1.00 (E-H:1), SB=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

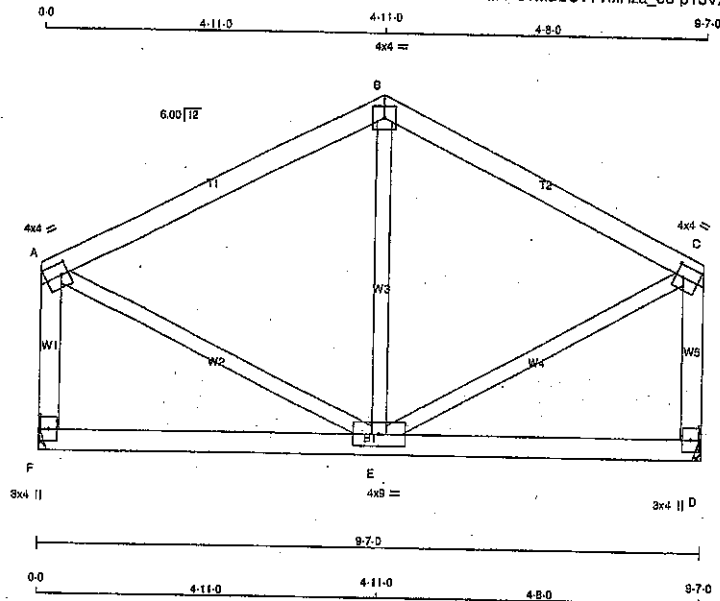
JSI GRIP= 0.76 (E) (INPUT = 0.90)

JSI METAL= 0.22 (I) (INPUT = 1.00)



Structural component only
DWG# T-2107886

JOB NAME 417279	TRUSS NAME T66	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



Scale = 1:30.2

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TTW-p	MT20	4.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

(1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	528	0	528	0	0
D	528	0	528	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	374	245 / 0	0 / 0	0 / 0	128 / 0	0 / 0
D	374	245 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. VERT. LOAD (LC)
FR-TO			FROM	TO		FR-TO		
A-B	-305 / 0	-91.8	-91.8	0.28 (1)	6.25	E-B	-167 / 21	0.06 (1)
B-C	-305 / 0	-91.8	-91.8	0.28 (1)	6.25	A-E	0 / 302	0.07 (1)
F-A	-493 / 0	0.0	0.0	0.07 (1)	7.81	E-C	0 / 309	0.07 (1)
D-C	-496 / 0	0.0	0.0	0.07 (1)	7.81			
F-E	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
E-D	0 / 0	-18.5	-18.5	0.12 (4)	10.00			

TOTAL WEIGHT = 40 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA C88-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/860 (0.32")
CALCULATED VERT. DEFL.(LL) = L/899 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/998 (0.02")

CSI: TC=0.28/1.00 (A-B:1), BC=0.12/1.00 (D-E:4), WB=0.07/1.00 (C-E:1), SS=0.18/1.00 (A-B:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

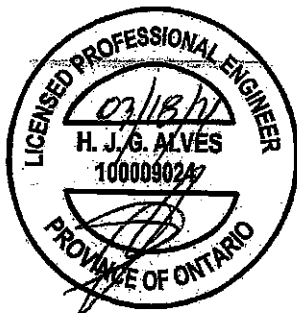
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 789 1887 1873

PLATE PLACEMENT TOL. = 0.250 inches

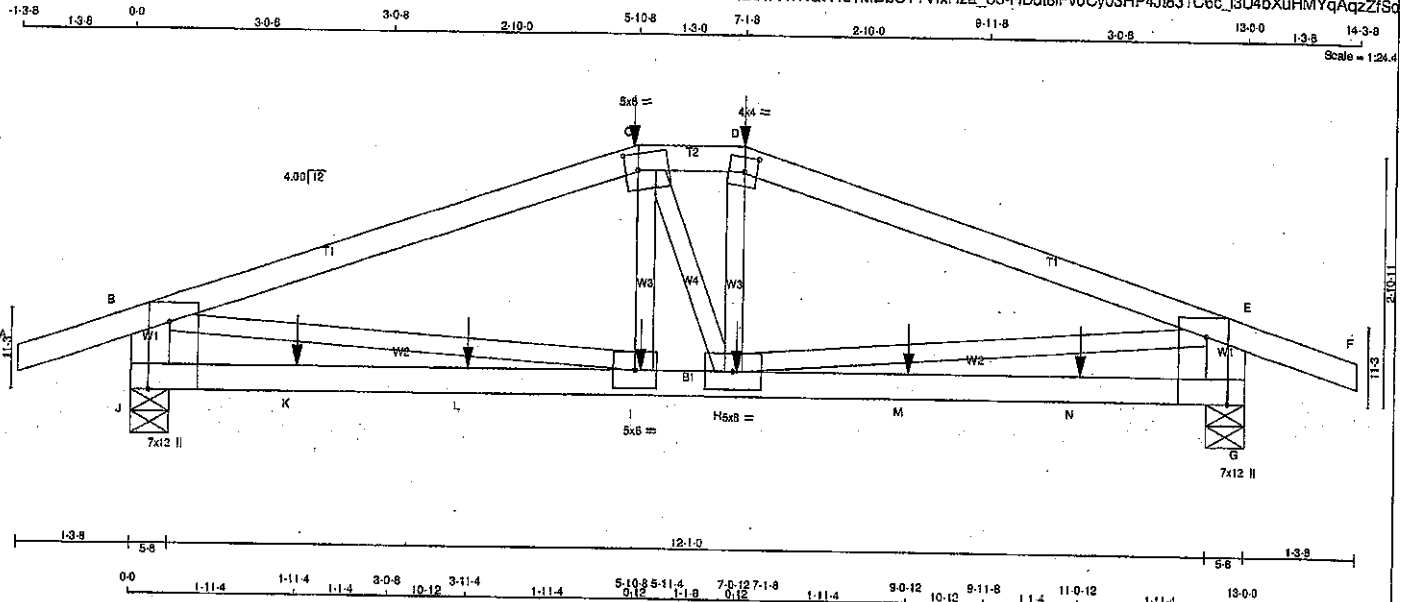
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (C) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)



Structural component only
DWG# T-2107887

JOB NAME 417279	TRUSS NAME T67	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Thu Mar 18 16:36:27 2021 Page 1	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
D - D	2x4	DRY	No.2	SPF	
E - E	2x4	DRY	No.2	SPF	
F - F	2x4	DRY	No.2	SPF	
G - G	2x4	DRY	No.2	SPF	
H - H	2x4	DRY	No.2	SPF	
I - I	2x4	DRY	No.2	SPF	
J - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (tables in inches)

JT TYPE	PLATES	W	LEN	Y	X
B, E, G, J					
C	TTW-m	MT20	5.0	6.0	2.25 1.75
D	TTW-m	MT20	4.0	4.0	2.00 1.75
G	TMBMWW1*-pMT20	7.0	12.0	Edge	3.00
H	BMWW1	MT20	5.0	6.0	
I	BMWW1	MT20	5.0	6.0	
J	TMBMWW1*-pMT20	7.0	12.0	Edge	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
J	1291	0	0	5-8
G	1290	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE
J	912	602 / 0	0 / 0	0 / 0
G	912	602 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO						FR-TO			
A-B	0 / 19	-91.8	-91.8	0.13 (1)	10.00	B-I	0 / 1898	0.47 (1)	
B-C	-1990 / 0	-91.8	-91.8	0.77 (1)	3.74	I-O	-73 / 75	0.03 (4)	
C-D	-1885 / 0	-91.8	-91.8	0.97 (1)	4.82	C-H	0 / 5	0.00 (4)	
D-E	-1992 / 0	-91.8	-91.8	0.77 (1)	3.74	H-D	-72 / 79	0.03 (4)	
E-F	0 / 19	-91.8	-91.8	0.13 (1)	10.00	H-E	0 / 1900	0.47 (1)	
J-B	-1222 / 0	0.0	0.0	0.09 (1)	7.81				
G-E	-1221 / 0	0.0	0.0	0.09 (1)	7.81				
J-K	0 / 0	-18.5	-18.5	0.25 (4)	10.00				
K-L	0 / 0	-18.5	-18.5	0.25 (4)	10.00				
L-I	0 / 0	-18.5	-18.5	0.25 (4)	10.00				
I-H	0 / 1884	-18.5	-18.5	0.44 (1)	10.00				
H-M	0 / 0	-18.5	-18.5	0.26 (4)	10.00				
M-N	0 / 0	-18.5	-18.5	0.26 (4)	10.00				
N-G	0 / 0	-18.5	-18.5	0.26 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX-	MAX+	FACE	DIRL	TYPE	HEEL	CONN.
C	5-10-8	-260	-260	---	BACK	VERT	TOTAL	---	C1
D	7-1-8	-260	-260	---	BACK	VERT	TOTAL	---	C1
H	7-0-12	-20	-20	---	BACK	VERT	TOTAL	---	C1
I	5-11-4	-20	-20	---	BACK	VERT	TOTAL	---	C1
K	1-11-4	-20	-20	---	BACK	VERT	TOTAL	---	C1
L	3-11-4	-20	-20	---	BACK	VERT	TOTAL	---	C1
M	9-0-12	-20	-20	---	BACK	VERT	TOTAL	---	C1
N	11-0-12	-20	-20	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2015
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(85% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.43")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.43")
CALCULATED VERT. DEFL. (TL) = L/999 (0.12")

CSI: TC=0.77/1.00 (D-E:1), BC=0.44/1.00 (H-I:1), WB=0.47/1.00 (E-H:1), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

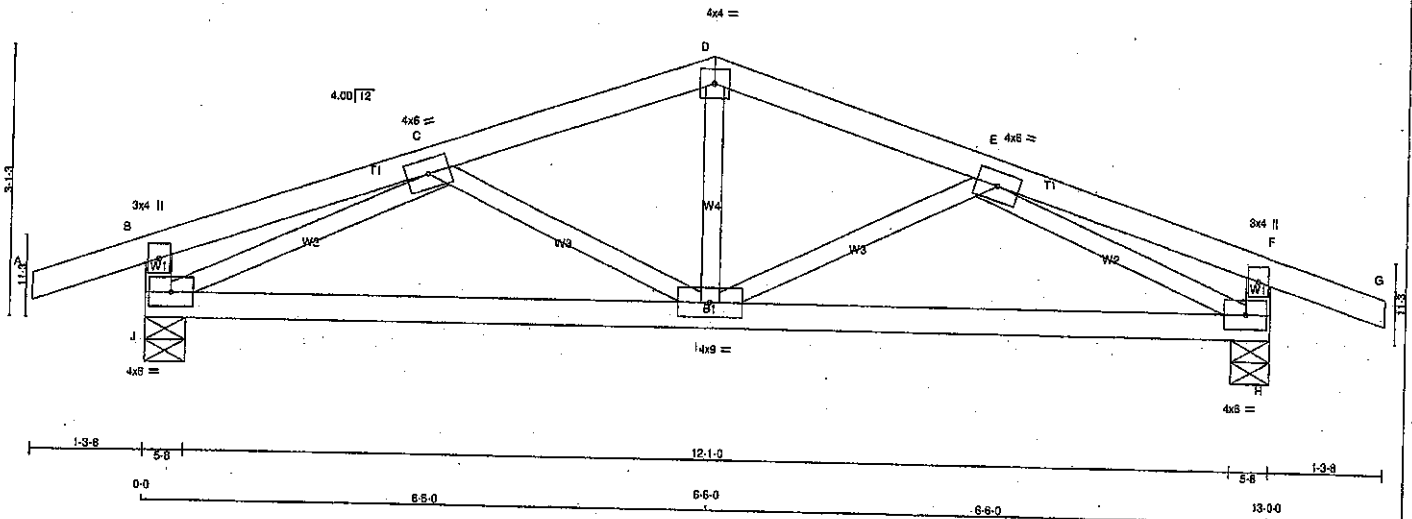
JSI GRIP = 0.86 (H) (INPUT = 0.80)
JSI METAL = 0.43 (H) (INPUT = 1.00)



Structural component only
DWG# T-2107888

JOB NAME 417279	TRUSS NAME T68	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:K77hYrav7ro?MDbCT?VfxHza_c6-HDdt8IPv0Cy03HP4Jt831C6n4f5n4eNulHMVqAqzZfSc
Scale: 1/2"=1'



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
J - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	4.0	6.0			
D	TTW-p	MT20	4.0	4.0			
E	TMWW-t	MT20	4.0	6.0			
F	TMV+p	MT20	3.0	4.0			
H	BMVW1-t	MT20	4.0	6.0			
I	BMVW1-t	MT20	4.0	6.0			
J	BMVW1-t	MT20	4.0	6.0			

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
J	839	0	5-8	5-8
H	839	0	5-8	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J	581	401 / 0	0 / 0
H	581	401 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MEMB.	FORCE	MAX
FR-TO	FROM	TO	CS1	LC1	UNBRAC	LENGTH	FR-TO
A-B	0 / 19	-91.8	-91.8	0.11 (1)	10.00	I-D	0 / 337
B-C	0 / 9	-91.8	-91.8	0.13 (1)	10.00	I-E	-202 / 0
C-D	-916 / 0	-91.8	-91.8	0.11 (1)	6.25	C-I	-202 / 0
D-E	-916 / 0	-91.8	-91.8	0.11 (1)	6.25	J-C	-1174 / 0
E-F	0 / 9	-91.8	-91.8	0.13 (1)	10.00	E-H	-1174 / 0
F-G	0 / 19	-91.8	-91.8	0.11 (1)	10.00		
J-B	-246 / 0	0.0	0.0	0.02 (1)	7.81		
H-F	-246 / 0	0.0	0.0	0.02 (1)	7.81		
J-I	0 / 1039	-18.5	-18.5	0.30 (4)	10.00		
I-H	0 / 1039	-18.5	-18.5	0.30 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA C86-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/850 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CS1: TC=0.13/1.00 (E-F:1), BC=0.30/1.00 (H-I:4), WB=0.29/1.00 (E-H:1), SS1=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

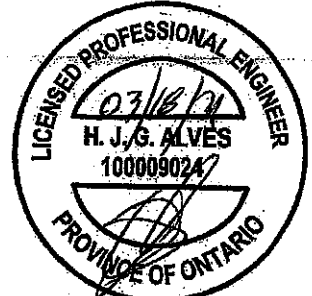
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

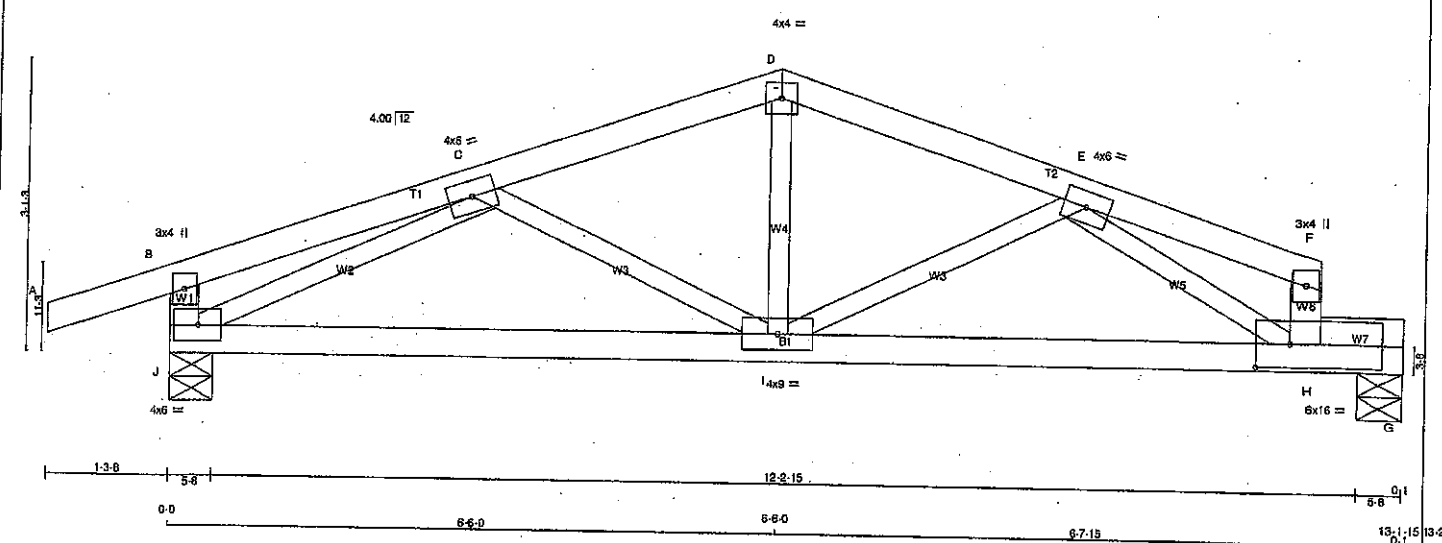
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT=0.90)
JSI METAL= 0.25 (H) (INPUT=1.00)



Structural component only
DWG# T-2107889

JOB NAME 417279	TRUSS NAME T69	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:36:28 2021 Page 1 ID:K77hYrav7ro?MDbCT?VfxHza_c6-lPBfM5QXnW4thR_HsblfaPexg9GUp5P1W0HOjGzZfS		



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		
J - G	2x4 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2
EXCEPT
SPF

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
C	TMWW-t	MT20	4.0	6.0			
D	TTW-p	MT20	4.0	4.0			
E	TMWW-t	MT20	4.0	6.0			
F	TMV+p	MT20	3.0	4.0			
H	BMVWW-t	MT20	9.0	16.0	3.00	4.50	
I	BMVWW-t	MT20	4.0	9.0			
J	BMVWW-t	MT20	4.0	6.0			

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
G	648	0	648	0	5-8	5-8
J	846	0	846	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	460	294 / 0	0 / 0	0 / 0	0 / 0	0 / 0	186 / 0	0 / 0
J	596	404 / 0	0 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 19	-91.8	10.00	C-I	-190 / 3	0.05 (1)	
B-C	0 / 9	-91.8	10.00	I-D	0 / 370	0.08 (1)	
C-D	-977 / 0	-91.8	8.13	I-E	8 / 54	0.02 (4)	
D-E	-976 / 0	-91.8	6.15	J-C	-1227 / 0	0.30 (1)	
E-F	0 / 9	-91.8	10.00	E-H	-1114 / 0	0.22 (1)	
J-B	-246 / 0	0.0	7.81				
H-F	-90 / 0	0.0	7.81				
J-I	0 / 1086	-18.5	10.00				
I-H	0 / 926	-18.5	10.00				
H-G	0 / 0	-18.5	10.00				

TOTAL WEIGHT = 2 X 47 = 93 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT. CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.44")
CALCULATED VERT. DEFL. (LL) = L/839 (0.19")
ALLOWABLE DEFL. (TL) = L/360 (0.44")
CALCULATED VERT. DEFL. (TL) = L/418 (0.38")

CSI: TC=0.14/1.00 (C-D:1), BC=0.91/1.00 (H-I:1), WB=0.30/1.00 (C-J:1), SS=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

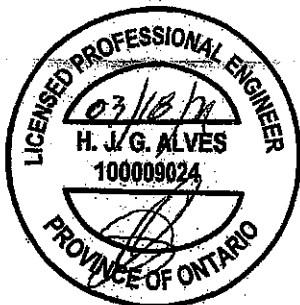
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 571 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

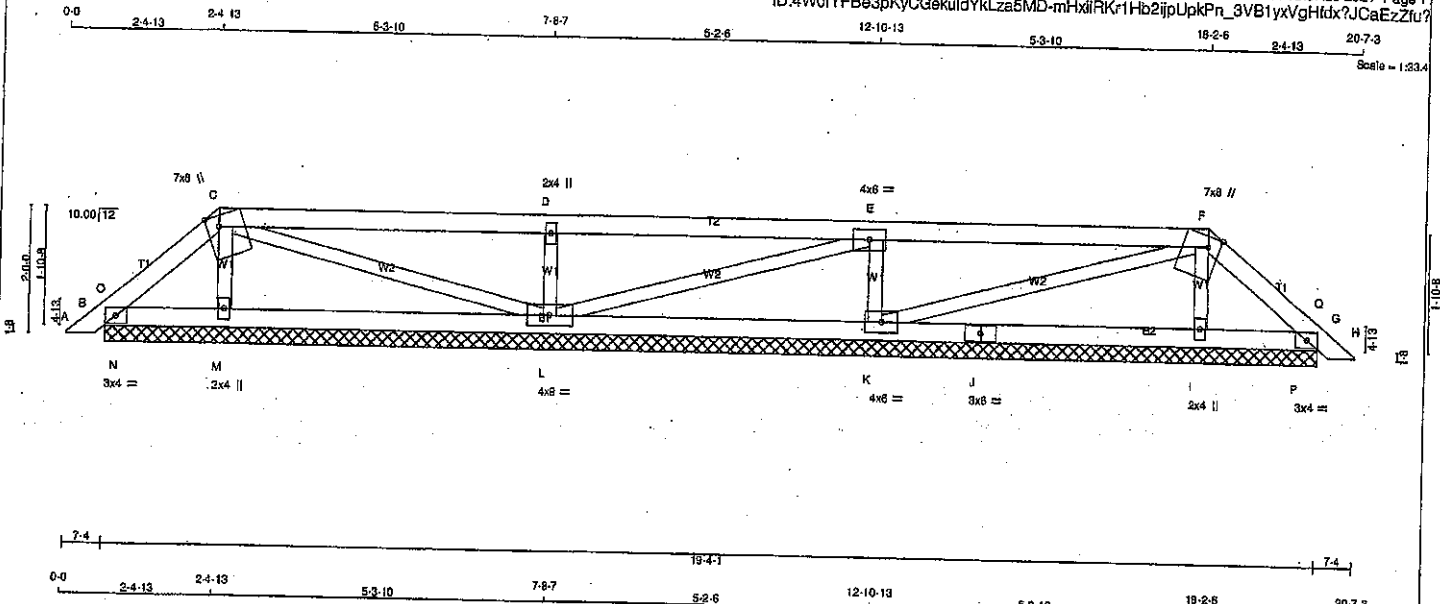
JSI GRIP = 0.85 (J) (INPUT = 0.90)
JSI METAL = 0.28 (J) (INPUT = 1.00)



Structural component only
DWG# T-2107890

JOB NAME 417277	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER			
N. L. G. A. RULES	SIZE	DRY	NO.2
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
B - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TM81-I	MT20	3.0	4.0	
C	TTWW+m	MT20	7.0	8.0	Edge 2.25
D	TMW+w	MT20	2.0	4.0	
E	TMW+w	MT20	4.0	6.0	
F	TTWW+m	MT20	7.0	8.0	Edge 2.25
G	TM81-I	MT20	3.0	4.0	
I	BMW1+w	MT20	2.0	4.0	
J	BS-I	MT20	3.0	6.0	
K	BMW1-I	MT20	4.0	6.0	
L	BMW1-I	MT20	4.0	6.0	
M	BMW1+w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
B	162	0	162	0	19-4-1 (13-11) 484-1	
M	316	0	316	0	19-4-1 (13-11) 484-1	
I	312	0	312	0	19-4-1 (13-11) 484-1	
G	170	0	170	0	19-4-1 (13-11) 484-1	
K	625	0	625	0	19-4-1 (13-11) 484-1	
L	642	0	642	0	19-4-1 (13-11) 484-1	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	FACTORED MAX/MIN. COMPONENT REACTIONS		PERM LIVE	WIND	DEAD	SOIL
	SNOW	LIVE				
B	112	91/0	0/0	0/0	20/0	0/0
M	227	131/0	0/0	0/0	96/0	0/0
I	224	128/0	0/0	0/0	96/0	0/0
G	117	96/0	0/0	0/0	21/0	0/0
K	441	284/0	0/0	0/0	147/0	0/0
L	453	303/0	0/0	0/0	150/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, M, I, G, K, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0/14	-91.8	-91.8 0.02 (1)	M-C	-217/0
B-C	-23/9	-91.8	-91.8 0.02 (1)	I-F	-213/0
O-C	-56/0	-91.8	-91.8 0.03 (1)	K-F	-17/0
C-D	0/5	-91.8	-91.8 0.35 (1)	C-L	-31/0
D-E	0/5	-91.8	-91.8 0.35 (1)	K-E	-523/0
E-F	-17/0	-91.8	-91.8 0.34 (1)	L-D	-529/0
F-Q	-68/0	-91.8	-91.8 0.03 (1)	L-E	-23/0
Q-G	-35/7	-91.8	-91.8 0.02 (1)	N-O	-125/0
G-H	0/14	-91.8	-91.8 0.02 (1)	P-Q	-125/0
B-N	0/39	-18.5	-18.5 0.04 (1)		
N-M	0/39	-18.5	-18.5 0.08 (4)		
M-L	0/24	-18.5	-18.5 0.10 (4)		
L-K	0/17	-18.5	-18.5 0.10 (4)		
K-J	0/34	-18.5	-18.5 0.10 (4)		
J-I	0/34	-18.5	-18.5 0.10 (4)		
I-P	0/48	-18.5	-18.5 0.08 (4)		
P-G	0/48	-18.5	-18.5 0.04 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.35/1.00 (C-D:1), BC=0.10/1.00 (H-K:4), WB=0.08/1.00 (D-L:1), SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

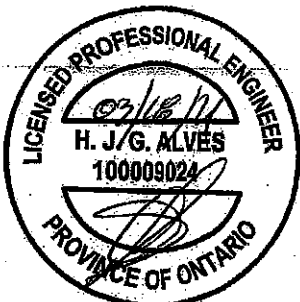
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

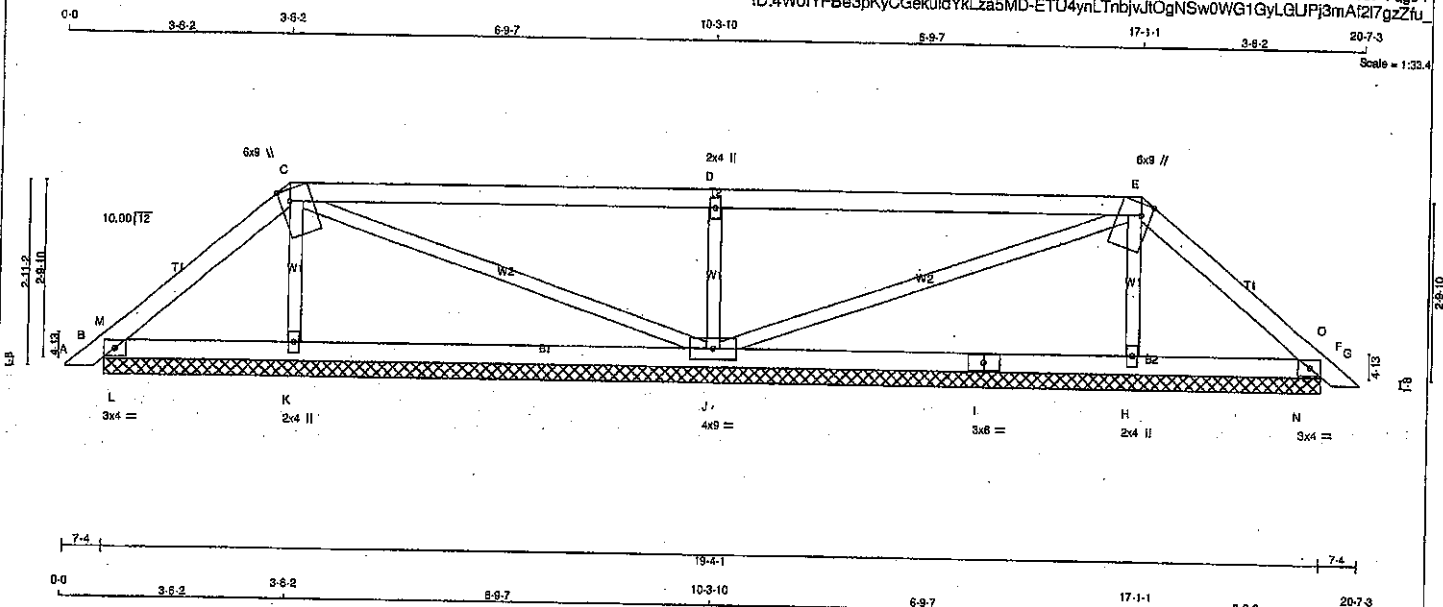
JSI GRIP=0.44 (C) (INPUT=0.50)
JSI METAL=0.11 (D) (INPUT=1.00)



Structural component only
DWG# T-2107819

JOB NAME 417277	TRUSS NAME PB2	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 64 lb [M]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+M	MT20	6.0	9.0	Edge 1.75
D	TMW+W	MT20	2.0	4.0	
E	TTWW+M	MT20	6.0	9.0	Edge 1.75
F	TMB1-I	MT20	3.0	4.0	
H	BMW1+W	MT20	2.0	4.0	
I	BS-I	MT20	3.0	6.0	
J	BMW1+W	MT20	4.0	9.0	
K	BMW1+W	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG	BRG
B	248	0	248	0	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1
K	399	0	399	0	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1
J	934	0	934	0	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1
H	399	0	399	0	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1
F	248	0	248	0	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1	19-4-1 (5-4-5) 19-4-1

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS		1ST LC CASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE
B	172	133 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0	0 / 0	0 / 0
K	286	165 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0	0 / 0	0 / 0
J	657	447 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0	0 / 0	0 / 0
H	286	165 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0	0 / 0	0 / 0
F	172	133 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, K, J, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (LBS)	MEMB.	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 14	-91.8	-91.8 0.02 (1)	10.00	K-C	-268 / 0	0.05 (1)
B-M	0 / 8	-91.8	-91.8 0.05 (1)	10.00	C-J	-42 / 0	0.04 (1)
M-C	-100 / 0	-91.8	-91.8 0.09 (1)	6.25	J-D	-778 / 0	0.13 (1)
C-D	-19 / 0	-91.8	-91.8 0.72 (1)	6.25	J-E	-42 / 0	0.04 (1)
D-E	-18 / 0	-91.8	-91.8 0.72 (1)	6.25	H-E	-268 / 0	0.05 (1)
E-O	-100 / 0	-91.8	-91.8 0.09 (1)	6.25	L-M	-268 / 0	0.00 (1)
O-F	0 / 8	-91.8	-91.8 0.05 (1)	10.00	N-O	-268 / 0	0.00 (1)
F-G	0 / 14	-91.8	-91.8 0.02 (1)	10.00			
B-L	0 / 70	-18.5	-18.5 0.09 (1)	10.00			
L-K	0 / 70	-18.5	-18.5 0.13 (4)	10.00			
K-J	0 / 59	-18.5	-18.5 0.18 (4)	10.00			
J-I	0 / 59	-18.5	-18.5 0.18 (4)	10.00			
I-H	0 / 59	-18.5	-18.5 0.18 (4)	10.00			
H-N	0 / 70	-18.5	-18.5 0.13 (4)	10.00			
N-F	0 / 70	-18.5	-18.5 0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.72/1.00 (C-D:1), BC=0.18/1.00 (J-K:4), WB=0.13/1.00 (D-J:1), SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.54 (C) (INPUT = 0.90)
JSI METAL = 0.16 (D) (INPUT = 1.00)



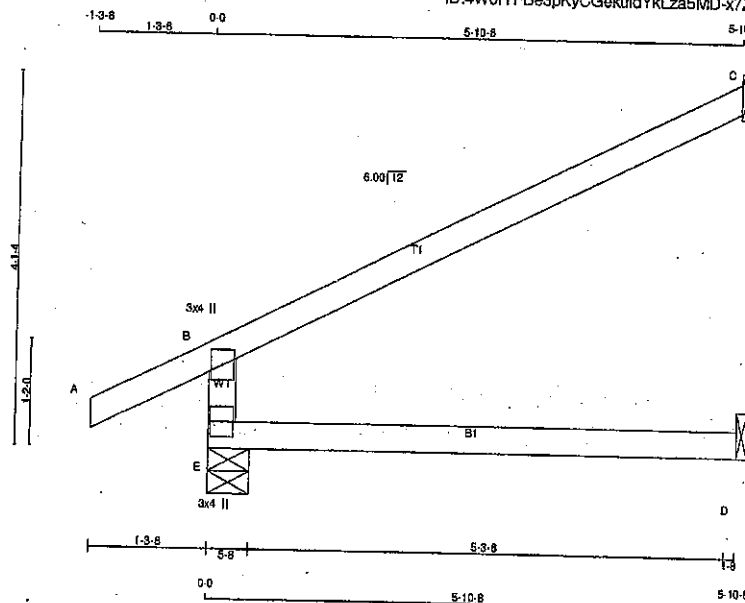
Structural component only
DWG# T-2107820

JOB NAME 417277	TRUSS NAME J1	QUANTITY 20	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamrack Roof Truss, Burlington

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ID:4W01YFB3pKyCGekuldYkLza5MD-x72RUOG4RRruzoMKTUINkoF7WXHGbNkZ3suNazZtu5

Scale = 1:20.3



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	525	0	525	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	45	0	50	0	1-8	1-8

SEE MTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	369	257 / 0	0 / 0	0 / 0	0 / 0	111 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	38 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1	MAX	MEMB. MAX. FORCE (LBS)	MAX
FR-TO						
E-B	-461 / 0	0.0	0.0	0.13 (4)	7.81	
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-30 / 0	-91.8	-91.8	0.54 (1)	6.25	
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00	

TOTAL WEIGHT = 20 X 17 = 336 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/959 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/959 (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

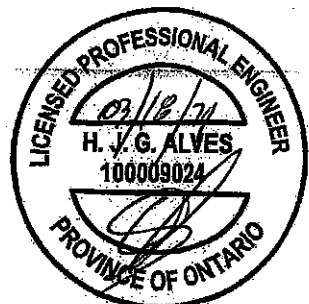
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

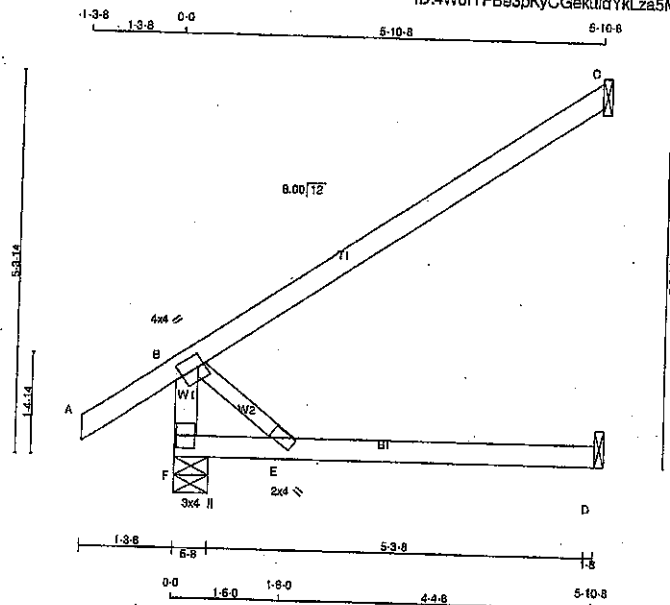
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.13 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107811

JOB NAME 417277	TRUSS NAME J2	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.00
E	BMV-w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	450	0	450	0	5-8	5-8
C	270	0	270	0	1-8	1-8
D	54	0	61	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
F	316	221 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	186	150 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 MAX (PLF)		MAX. CSI (LC) UNBRAC LENGTH FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CSI (LC)
	FR-TO	TO	FROM	TO			FR-TO	TO	
F-B	-395 / 0		0.0	0.0	0.04 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 35		-91.8	-91.8	0.12 (1)	10.00			
B-C	0 / 0		-91.8	-91.8	0.54 (1)	10.00			
F-E	0 / 0		-18.5	-18.5	0.14 (4)	10.00			
E-D	0 / 0		-18.5	-18.5	0.19 (4)	10.00			

TOTAL WEIGHT = 4 X 19 = 77 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT. CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TO=0.54/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

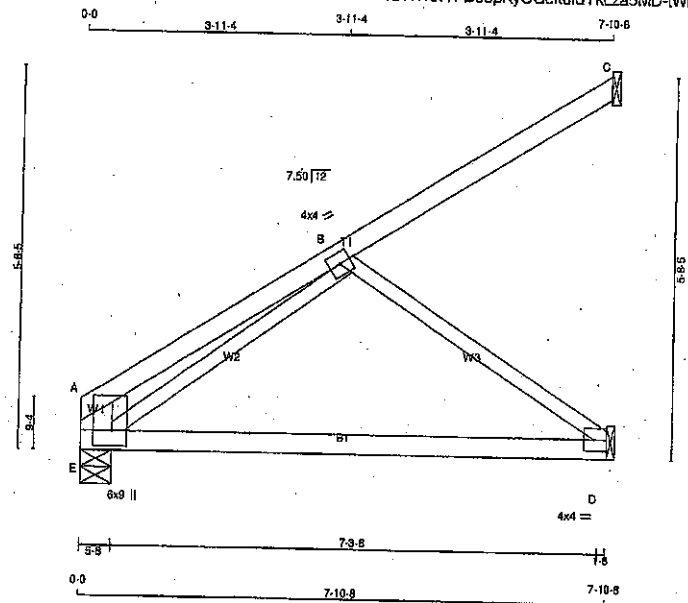
JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107812

JOB NAME 417277	TRUSS NAME J4	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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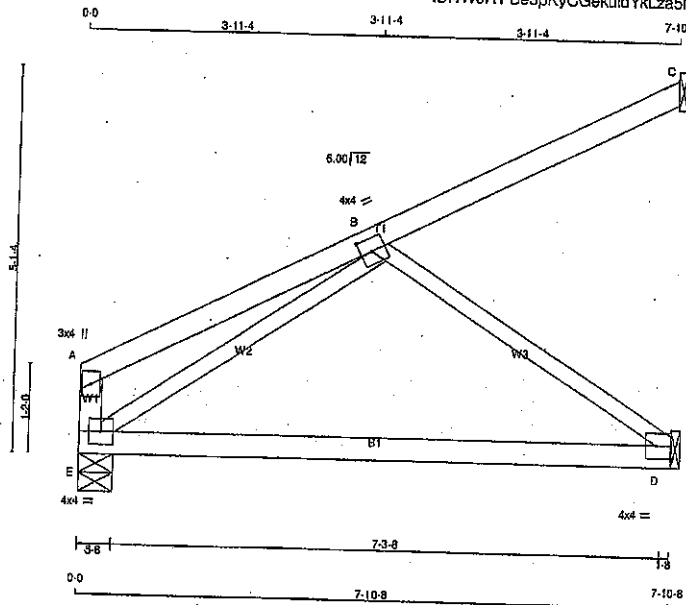


JOB NAME 417277	TRUSS NAME J5	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:4W0YFB3pKyCGekuldYkLza5MD-LIFZ6QlykMDTqG4v8cr4MQ8Wkr2TwcBF14Y_vzZfu2

Scale = 1:27.0



LUMBER			
N. L. G. A. RULES	SIZE	DRY	DESCR.
CHORDS	2x4	DRY	No.2
E - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY SEASONED LUMBER			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0	
B	TMWW-t	MT20	4.0	4.0	2.00 1.75
D	BMW1-t	MT20	4.0	4.0	2.00 Edge
E	BMWV1-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	431	0	431	0	5-8	5-8
C	142	0	142	0	1-8	1-8
D	294	0	294	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
		SNOW	LIVE	PERM. LIVE	WIND		
E	304	200 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0
C	98	79 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
D	210	124 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
E-A	-132 / 0	0.0 0.0	0.01 (1)	7.81	E-B	-398 / 0	0.14 (1)		
A-B	0 / 20	-91.8 -91.8	0.24 (1)	10.00	B-D	-383 / 0	0.18 (1)		
B-C	-19 / 0	-91.8 -91.8	0.18 (1)	6.25					
E-D	0 / 313	-18.5 -18.5	0.34 (4)	10.00					

TOTAL WEIGHT = 5 X 27 = 137 LB

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TFC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = 1/565 (0.17")

CSI: TO=0.24/1.00 (A-B:1), BC=0.34/1.00 (D-E:4), WB=0.16/1.00 (B-D:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 768 1987 1873

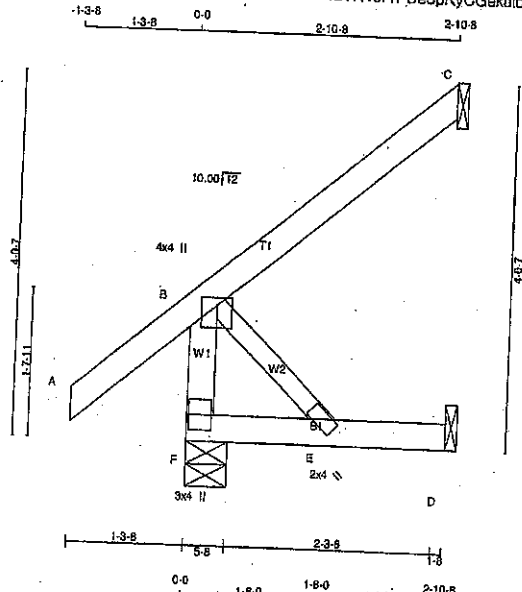
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.37 (B) (INPUT = 0.80)
JSI METAL = 0.11 (E) (INPUT = 1.00)



Structural component only
DWG# T-2107814



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMV+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
F	288	0	288	0	5-8	5-8
C	132	0	132	0	1-8	1-8
D	27	0	30	0	1-8	1-8

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	200	144.0	0.0				
C	91	74.0	0.0	0.0	0.0	55.0	0.0
D	21	0.0	0.0	0.0	0.0	17.0	0.0
				0.0	0.0	21.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		FR-TO	FROM-TO	FR-TO	FROM-TO
F-B	-259.0	0.0	0.0	0.0	0.0
A-B	0.41	-91.8	-91.8	0.13	0.13
B-C	0.0	-91.8	-91.8	0.13	0.13
F-E	0.0	-18.5	-18.5	0.04	0.04
E-D	0.0	-18.5	-18.5	0.04	0.04

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 7 X 13 = 88 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.04/1.00 (E-F-4), WB=0.00/1.00 (B-E-1), SS=0.08/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

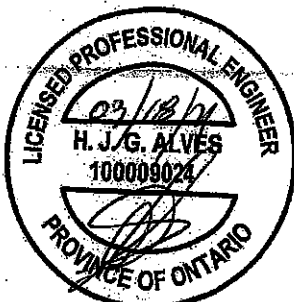
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	650	371	1747
	788	1887	1873

PLATE PLACEMENT TOL. = 0.250 inches

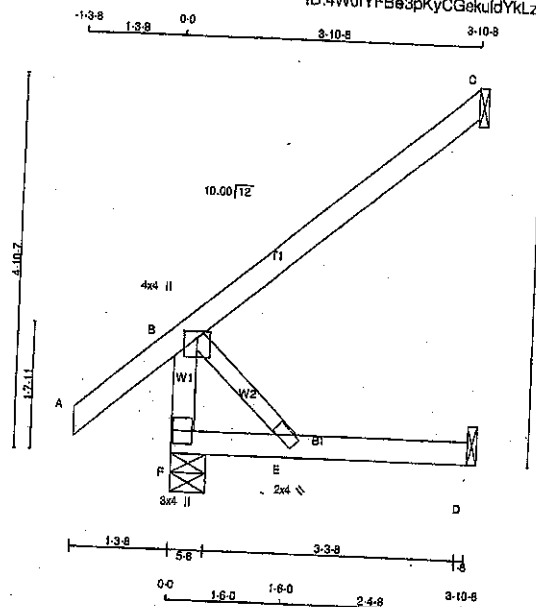
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.80)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107815

JOB NAME 417277	TRUSS NAME J7	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:07:24 2021 Page 1 ID:4W0IYFB83pKyCGekuldYkLza5MD-pupyKmJbVgLKSQI5UjMjuePiH8FMCONKUhg5WLzZfu1		



Scale = 1/27.5

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMVW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	341	0	341	0	5-8	5-8
C	178	0	178	0	1-8	1-8
D	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	238	170/0	0/0	0/0	0/0	69/0	0/0
C	122	99/0	0/0	0/0	0/0	23/0	0/0
D	28	0/0	0/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 (LC)
FR-TO									
F-B		-305/0	0.0	0.0	0.03 (1)	7.81	B-E	0/0	0.00 (1)
A-B		0/41	-91.8	-91.8	0.14 (5)	10.00			
B-C		0/0	-91.8	-91.8	0.23 (1)	10.00			
F-E		0/0	-18.5	-18.5	0.08 (4)	10.00			
E-D		0/0	-18.5	-18.5	0.08 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 15 = 61 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA C88-14
- TPIC 2014

(85% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.23/1.00 (B-C:1), BC=0.09/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

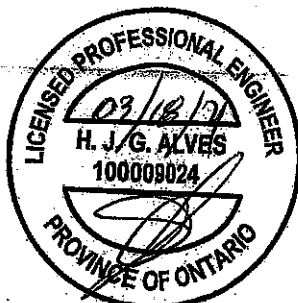
PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

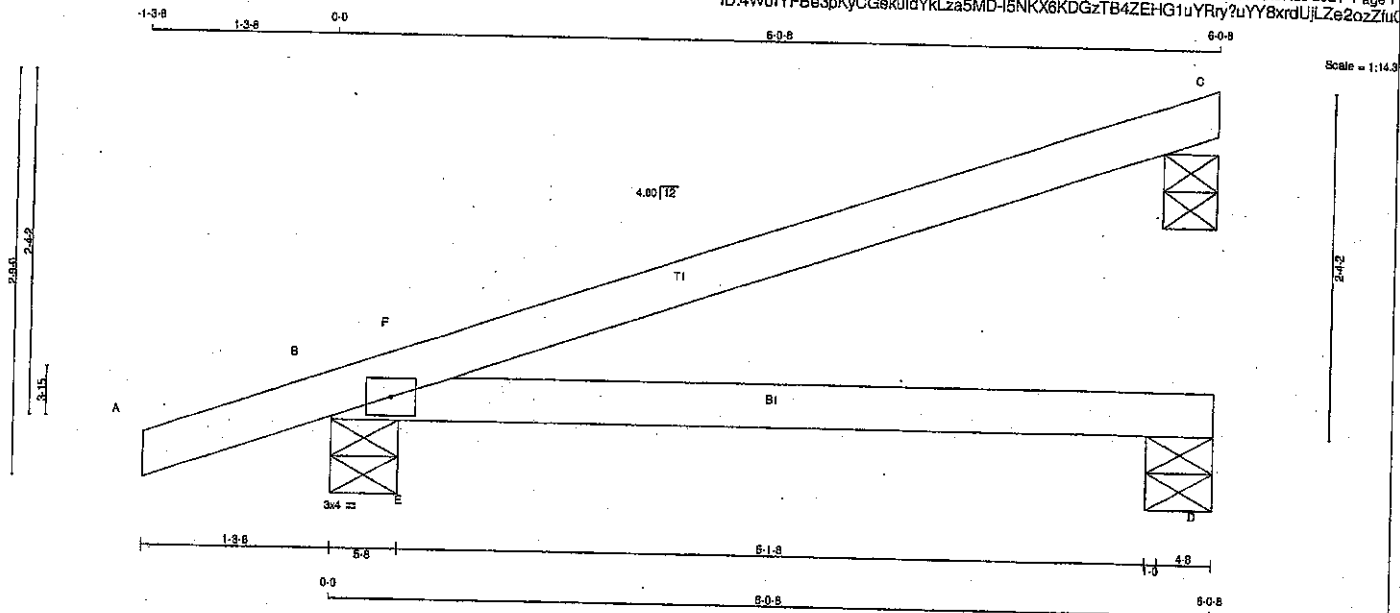
JSI GRIP = 0.22 (B) (INPUT = 0.90)

JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107816

JOB NAME 417277	TRUSS NAME J9	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:07:25 2021 Page 1	
				ID:4W01YFB63pKyCGskuldYkLza5MD-15NKX6KDGzTB4ZEHG1uYRry?uYY8xrdUjLZe2ozZfu	



Scale = 1:14.3

TOTAL WEIGHT = 5 X 16 = 79 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESOR.
A - C	2x4	DRY	No.2	SPF		
B - D	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-J	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	242	0	242	0	4-8	4-8
B	456	0	456	0	5-8	5-8
D	91	0	91	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	188	131 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	320	223 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	68	24 / 0	0 / 0	0 / 0	0 / 0	44 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD (LC)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO					FR-TO			
A-B	0 / 18	-91.8	-91.8	0.11 (1)	10.00	E-F	-345 / 13	0.00 (1)
B-F	-20 / 22	-91.8	-91.8	0.07 (4)	6.25			
F-C	-2 / 2	-91.8	-91.8	0.44 (1)	10.00			
B-E	0 / 0	-18.5	-18.5	0.30 (1)	10.00			
E-D	0 / 0	-18.5	-18.5	0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/928 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/452 (0.16")

CSI: TC=0.44/1.00 (C-F:1), BC=0.30/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SI=0.29/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

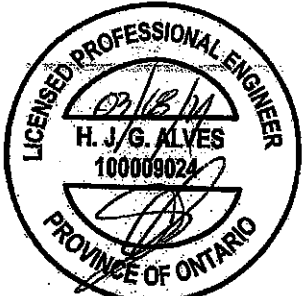
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

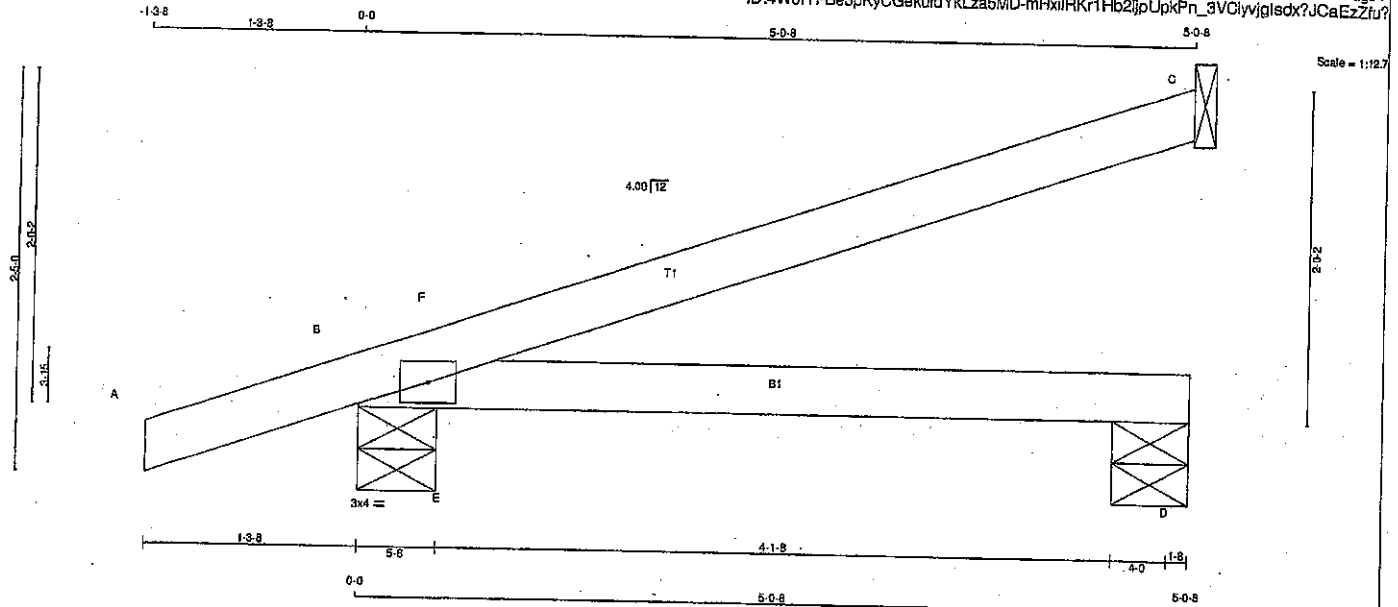
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107817

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417277	J10	5	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						



TOTAL WEIGHT = 5 X 13 = 67 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN Y	X
B	TMB1-H	MT20	3.0	4.0

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
C	201	201	0	0
B	401	401	0	0
D	77	77	0	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	139	108 / 0	0 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
B	281	197 / 0	0 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
D	57	21 / 0	0 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 18	-91.8	-91.8 0.11 (1)	10.00	E-F	249 / 10	0.00 (1)
B-F	-16 / 6	-91.8	-91.8 0.06 (4)	8.25			
F-C	0 / 2	-91.8	-91.8 0.30 (1)	10.00			
B-E	0 / 0	-18.5	-18.5 0.22 (1)	10.00			
E-D	0 / 0	-18.5	-18.5 0.22 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/763$ (0.09")

CSI: TC=0.30/1.00 (C-F:1), BC=0.22/1.00 (D-E:1),

WB=0.00/1.00 (E-F:1), SSI=0.21/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MIN	MAX	MIN
MT20	660	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (B) (INPUT = 0.80)
JSI METAL = 0.07 (B) (INPUT = 1.00)

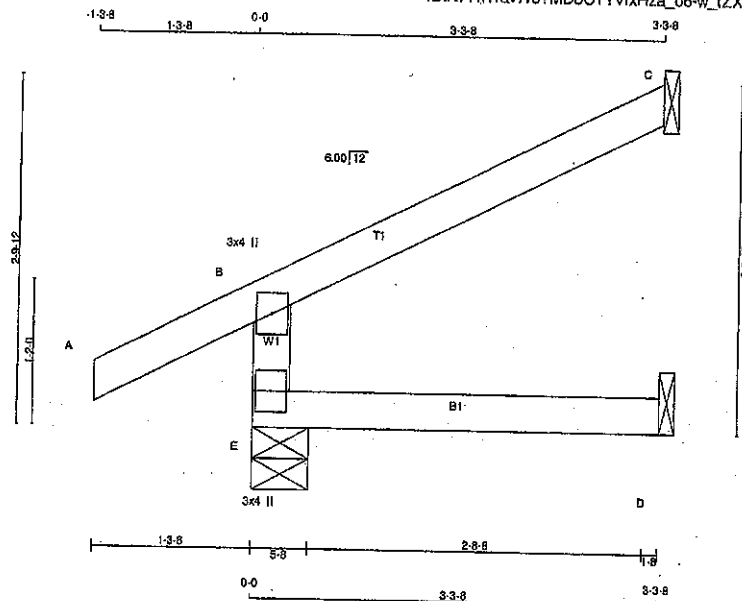


Structural component only
DWG# T-2107818

JOB NAME 417279	TRUSS NAME J50	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:17.0

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	348	0	348	0	5-8	5-8
C	113	0	113	0	1-8	1-8
D	28	0	28	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	243	175.0	0.0	0.0	0.0	0.0	89.0
C	78	63.0	0.0	0.0	0.0	0.0	15.0
D	21	0.0	0.0	0.0	0.0	0.0	21.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS				MEMB.	MAX. FACTORED FORCE (LBS)	WEBS	
		VERT. LOAD	LC1	MAX. (PLF)	MAX. CSI (LC)			MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			FR-TO			
E-B	-313.0	0.0	0.0	0.03 (4)	7.81				
A-B	0.28	-91.8	-91.8	0.13 (5)	10.00				
B-C	-17.0	-91.8	-91.8	0.17 (1)	6.25				
E-D	0.0	-18.5	-18.5	0.04 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 11 = 32 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIO 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TC=0.17/1.00 (B-C:1), BC=0.04/1.00 (D-E:4), WB=0.00/1.00 (W=0), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

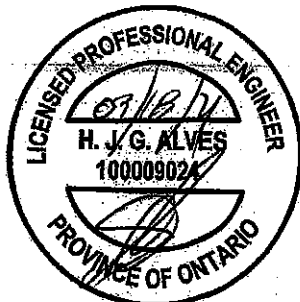
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 708 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.12 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



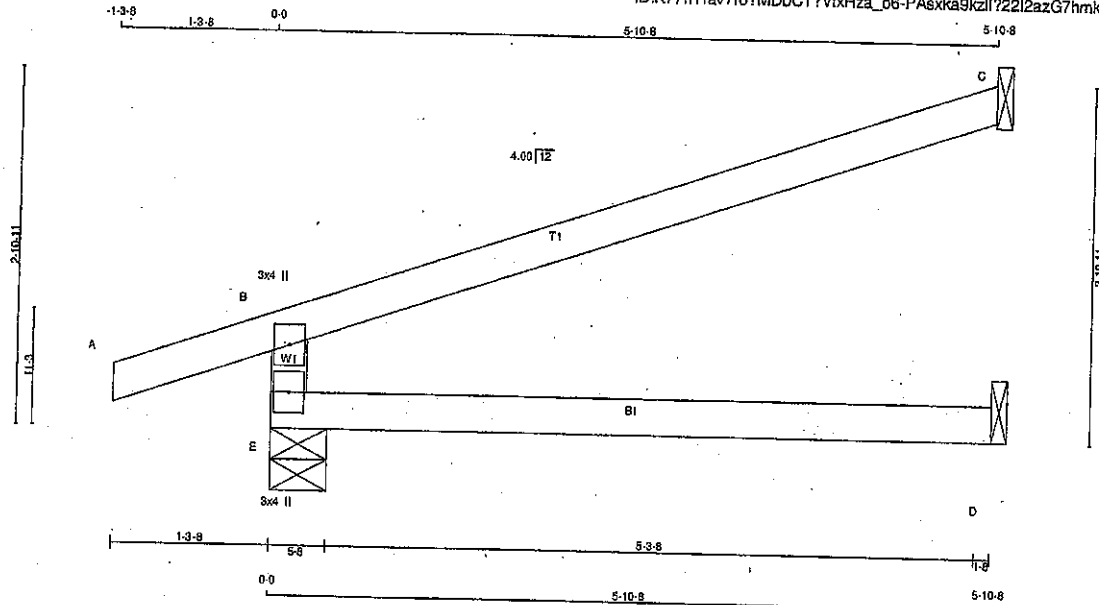
Structural component only
DWG# T-2107867

JOB NAME 417279	TRUSS NAME J52	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/12



TOTAL WEIGHT = 2 X 16 = 32 lb

LUMBER

N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B 7MV+p	MT20	3.0	4.0		
E 8MV1+p	MT20	3.0	4.0		

NOTES

(1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	524	0	524	0	5-8	5-8
C	202	0	202	0	1-8	1-8
D	44	0	49	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97781H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	388	256 / 0	0 / 0	0 / 0	0 / 0	112 / 0	0 / 0
C	139	113 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
		FROM	TO				
E-B	-460 / 0	0.0	0.0	0.14 (4)	7.81		
A-B	0 / 19	-91.8	-91.8	0.11 (1)	10.00		
B-C	-21 / 0	-91.8	-91.8	0.54 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.14 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
BOT CH. LL = 6.0 PSF
DL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NSCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.03")

CS1: TC=0.54/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS1=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (E) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)

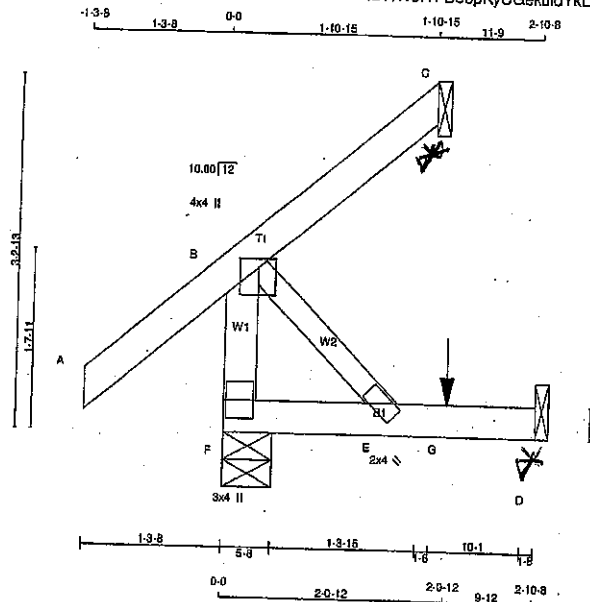


Structural component only
DWG# T-2107869

JOB NAME 417277	TRUSS NAME C1	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:19.3

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMV+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	SPF	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	SPF	288	0	288	0	0	5-8	5-8	
C	SPF	42	0	42	0	-36	1-8	1-8	
D	SPF	28	0	31	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	201	145 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
C	29	23 / -26	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	22	0 / 0	0 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

FR-TO	CHORDS		FACTORED		WEBS	
	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
F-B		-261 / 0	0.0	0.0	0.03 (1)	7.81
A-B		0 / 41	-91.8	-91.8	0.14 (5)	10.00
B-C		-29 / 0	-91.8	-91.8	0.13 (5)	6.25
F-E		0 / 0	-18.5	-18.5	0.05 (4)	10.00
E-G		0 / 0	-18.5	-18.5	0.05 (4)	10.00
G-D		0 / 0	-18.5	-18.5	0.05 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-1	-1		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 11 = 33 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS.

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.14/1.00 (A-B), BC=0.05/1.00 (D-E), WB=0.00/1.00 (B-E), SS=0.09/1.00 (A-B-S)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371	1747
	788	1987	1873

PLATE PLACEMENT-TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.80)
JSI METAL = 0.05 (B) (INPUT = 1.00)



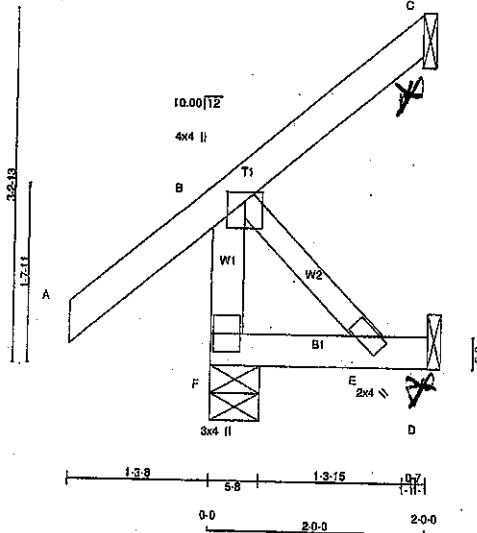
Structural component only
DWG# T-2107808

JOB NAME 417277	TRUSS NAME C2	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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1-3-8 1-3-8 0-0 1-10-15 1-10-15 2-0-0

Scale = 1:19.2



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMVW+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
F	279	0	279	0	0	5-8	5-8	5-8	5-8
C	42	0	42	0	-36	1-8	1-8	1-8	1-8
D	19	0	21	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	194	145 / 0	0 / 0	0 / 0	0 / 0	49 / 0	0 / 0
C	29	23 / -26	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	15	0 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
F-B	-261 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 41	-91.8	-91.8	0.13 (1)			
B-C	-29 / 0	-91.8	-91.8	0.12 (1)			
F-E	0 / 0	-18.5	-18.5	0.02 (4)			
E-D	0 / 0	-18.5	-18.5	0.02 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 10 = 51 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA D88-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

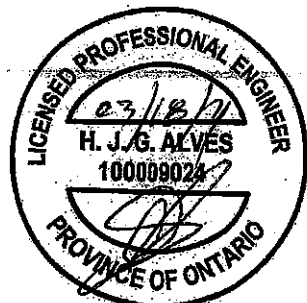
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

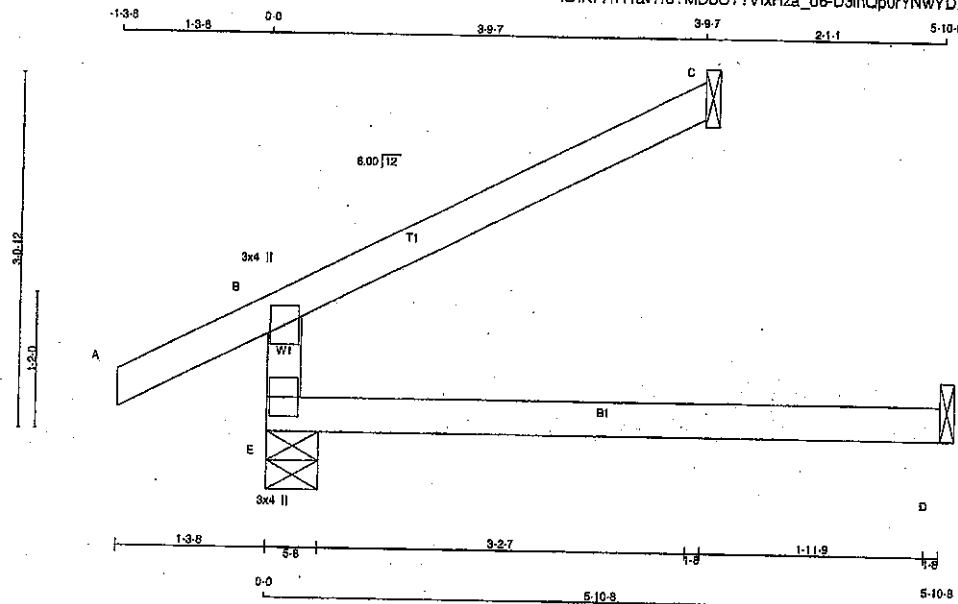
JSI GRIP=0.19 (B) (INPUT = 0.90)
JSI METAL=0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107809

JOB NAME 417279	TRUSS NAME C50	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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TOTAL WEIGHT = 4 X 14 = 57 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4	DRY No.2
A - C	2x4	DRY No.2
E - D	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	RECORD BRG IN-SX
E	405	405	5-8	5-8
C	130	130	1-8	1-8
D	45	50	1-8	1-8

SEE MITEK STANDARD DETAIL B97781H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. PERM. LIVE	WIND	DEAD	SOIL
E	288	190 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO					FR-TO		
E-B	-342 / 0	0.0	0.0	0.13 (4)	7.61		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		
B-C	-19 / 0	-91.8	-91.8	0.22 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.5	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 38.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 8 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TO=0.22/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

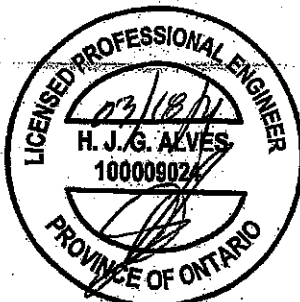
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

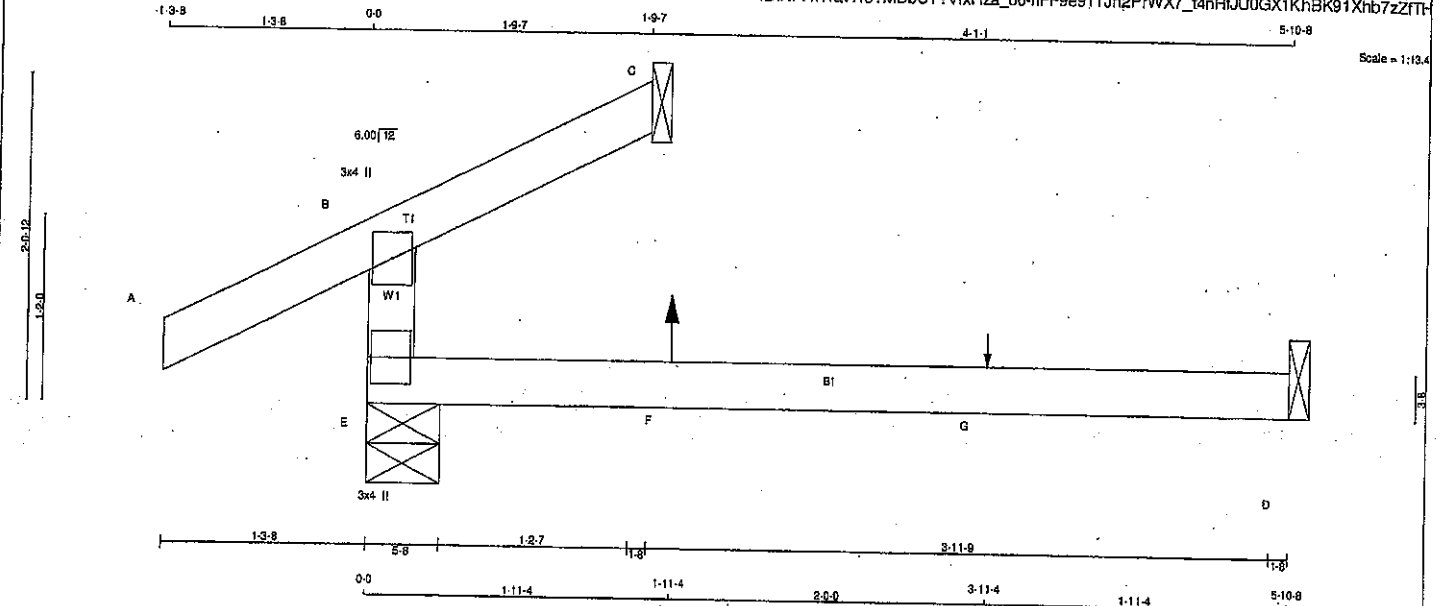
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.30)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107856



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	83	0	83	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	137/0	0/0	0/0	0/0	62/0	0/0
C	45	21/0	0/0	0/0	0/0	25/0	0/0
D	35	0/3	0/0	0/0	0/0	37/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRACED LENGTH (LC)	MEMB.
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-227/0	0.0	0.0 0.11 (4)	7.81			
A-B	0/28	-91.8	-91.8 0.12 (1)	10.00			
B-C	-9/9	-91.8	-91.8 0.08 (4)	10.00			
E-F	0/0	-18.5	-18.5 0.14 (4)	10.00			
F-G	0/0	-18.5	-18.5 0.14 (4)	10.00			
G-D	0/0	-18.5	-18.5 0.14 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	BACK	VERT	TOTAL	--	C1
G	3-11-4	1	1	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 12 = 48 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (F9)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.09 (E) (INPUT = 0.80)
JSI METAL = 0.06 (B) (INPUT = 1.00)



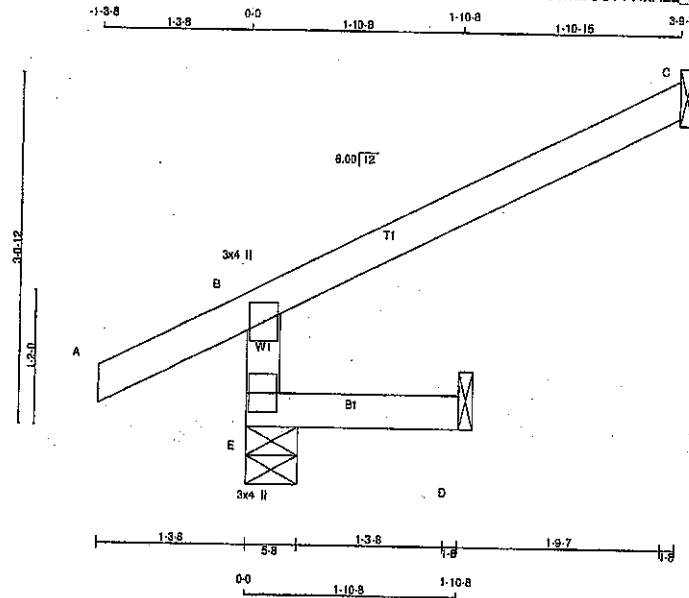
Structural component only
DWG# T-2107857

JOB NAME 417279	TRUSS NAME C52	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:18.4



TOTAL WEIGHT = 4 X 10 = 38 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4 DRY	No.2
A - C	2x4 DRY	No.2
E - D	2x4 DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	361	361	5-8	5-8
C	130	130	1-8	1-8
D	16	17	1-8	1-8

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	250	190/0	0/0	0/0	0/0	0/0	60/0	0/0
C	90	73/0	0/0	0/0	0/0	0/0	17/0	0/0
D	12	0/0	0/0	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							
E-B	-342/0	0.0	0.0	0.01 (4)	7.81		
A-B	0/28	-91.8	-91.8	0.13 (5)	10.00		
B-C	-19/0	-91.8	-91.8	0.22 (1)	8.25		
E-D	0/0	-18.5	-18.5	0.02 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TO=0.22/1.00 (B-C:1), BC=0.02/1.00 (D-E:4), WB=0.00/1.00 (v.a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

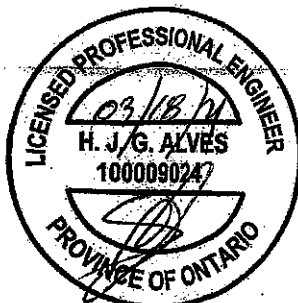
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



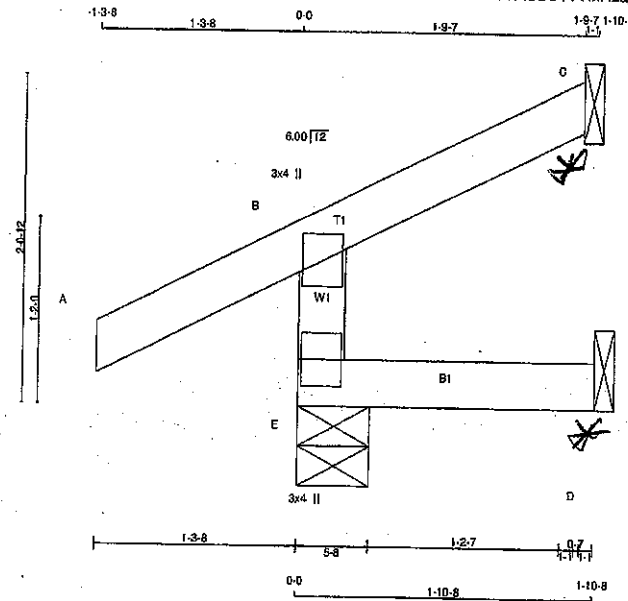
Structural component only
DWG# T-2107858

JOB NAME 417279	TRUSS NAME C53	QUANTITY 7	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:13.3



LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT		
E	271	0	271	0	5-8	5-8
C	45	0	45	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT E C D	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	188	141/0	0/0	0/0	0/0	47/0	0/0
	31	24/-18	0/0	0/0	0/0	7/0	0/0
	7	0/-8	0/0	0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)		MAX. FACTORED PERM. LIVE (LBS)	MAX. FACTORED WIND (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
			FROM	TO					
E-B		-244/0	0.0	0.0	0.04 (5)	7.81			
A-B		0/28	-91.8	-91.8	0.12 (1)	10.00			
B-C		-17/0	-91.8	-91.8	0.09 (1)	6.25			
E-D		0/0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 7 X 7 = 49 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPK 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (A-B:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 L3 BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

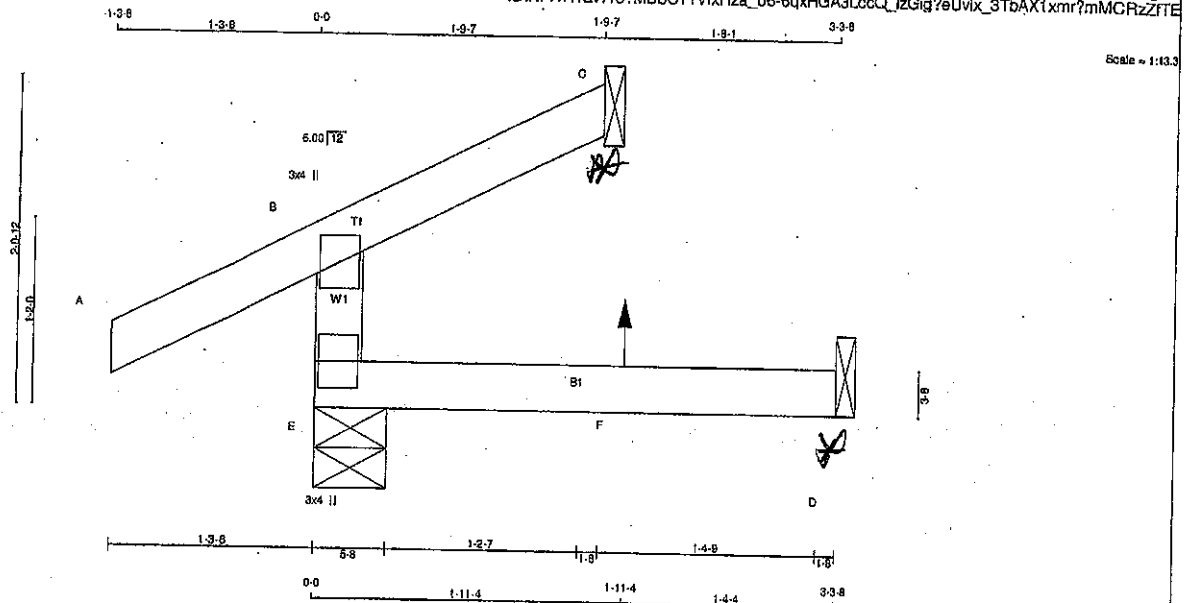
JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107859

JOB NAME 417279	TRUSS NAME C54	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 2 X 9 = 17 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	BMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	SPF	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT			VERT	HORZ	UP/LIFT	IN-SX
E			276	0	0	5-8
C			47	0	-22	1-8
D			20	0	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	193	138 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	33	23 / -20	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	17	0 / -6	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRACED LENGTH	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							FR-TO			
E-B		-242 / 0	0.0	0.0	0.04 (5)	7.81				
A-B		0 / 28	-91.8	-91.8	0.13 (5)	10.00				
B-C		-18 / 0	-91.8	-91.8	0.09 (6)	8.25				
E-F		0 / 0	-18.5	-18.5	0.04 (4)	10.00				
F-D		0 / 0	-18.5	-18.5	0.04 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015, ABC 2015
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA D86-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B-5), BC=0.04/1.00 (D-E-4),
WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	850	371	1747
	799	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.80)

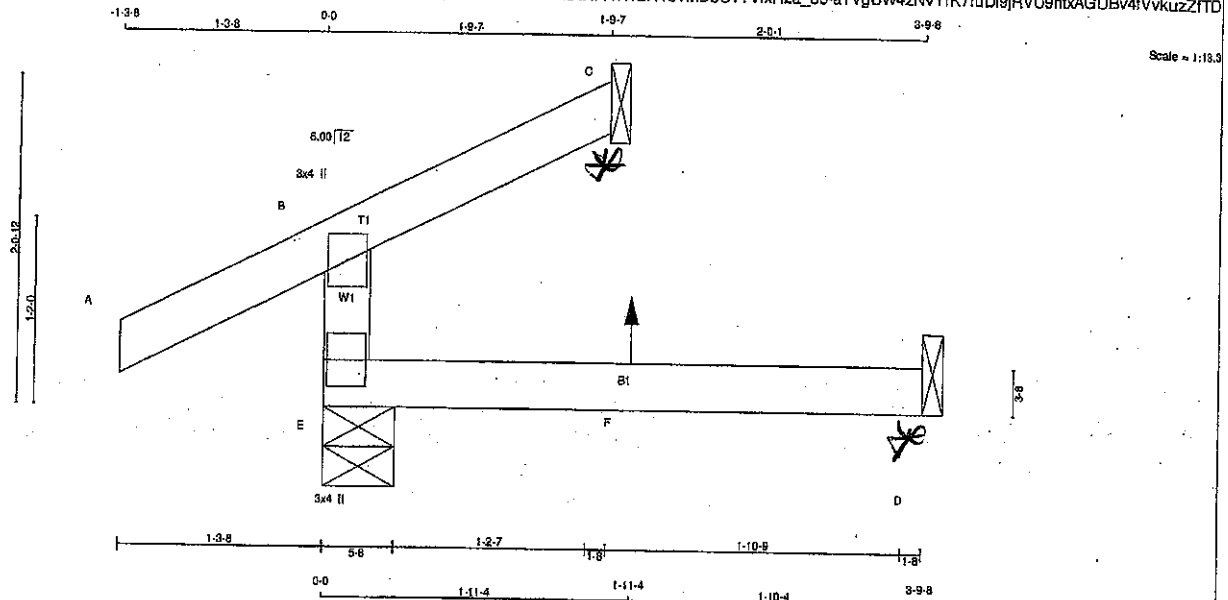
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107860

JOB NAME 417279	TRUSS NAME C55	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MitTek Industries, Inc. Thu Mar 18 16:38:00 2021 Page 1
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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	278	0	278	0	5.8	5.8
C	48	0	48	0	-21	1.8
D	25	0	34	0	1.8	1.8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	195	138 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0
C	34	22 / -20	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	21	0 / -5	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (9)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. FACTORED UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
E-B	-241 / 0	0.0	0.0 (5) (5)	7.81		
A-B	0 / 28	-91.8	-91.8 (13) (5)	10.00		
B-C	-15 / 0	-91.8	-91.8 (8) (6)	6.25		
E-F	0 / 0	-18.5	-18.5 (4) (4)	10.00		
F-D	0 / 0	-18.5	-18.5 (4) (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.13/1.00 (A-B-5), BC=0.08/1.00 (D-E-4), WB=0.00/1.00 (NA=0), SS=0.10/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT-TOL. = 0.250 inches

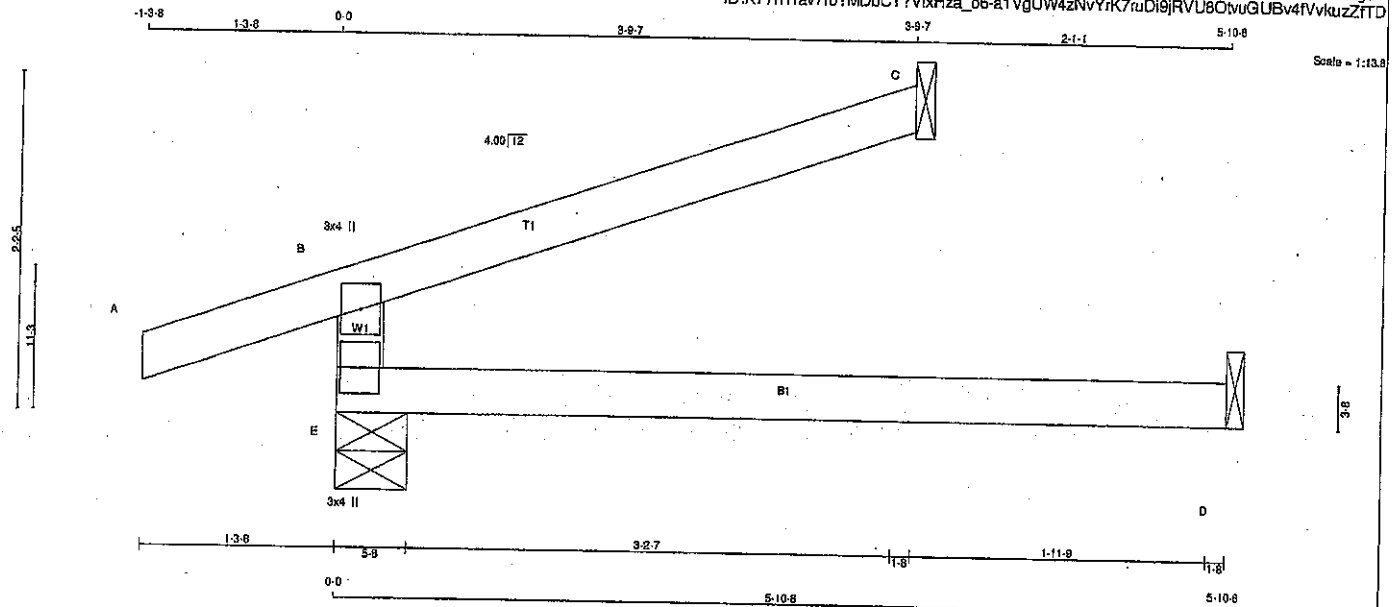
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107861

JOB NAME 417279	TRUSS NAME C56	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 8 Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:36:00 2021 Page 1 ID:K77hYrav7ro?MDbCT?VbHza_c6-a1VgUW4zNvYrK7ruD19jRVU8OnuGU8v4IVvkuzZTTD	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	SPF
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	404	0	404	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	44	0	49	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
E	286	190 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
C	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO		FROM TO
E-B	-340 / 0	0.0 0.0 0.14 (4)	7.81	
A-B	0 / 19	-91.8 -91.8 0.11 (1)	10.00	
B-C	-14 / 0	-91.8 -91.8 0.22 (1)	6.25	
E-D	0 / 0	-18.5 -18.5 0.14 (4)	10.00	

TOTAL WEIGHT = 2 X 13 = 27 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (A-B:0), SI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

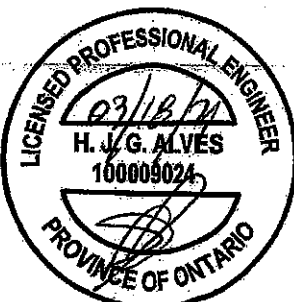
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1937 1873

PLATE PLACEMENT TOL = 0.250 inches

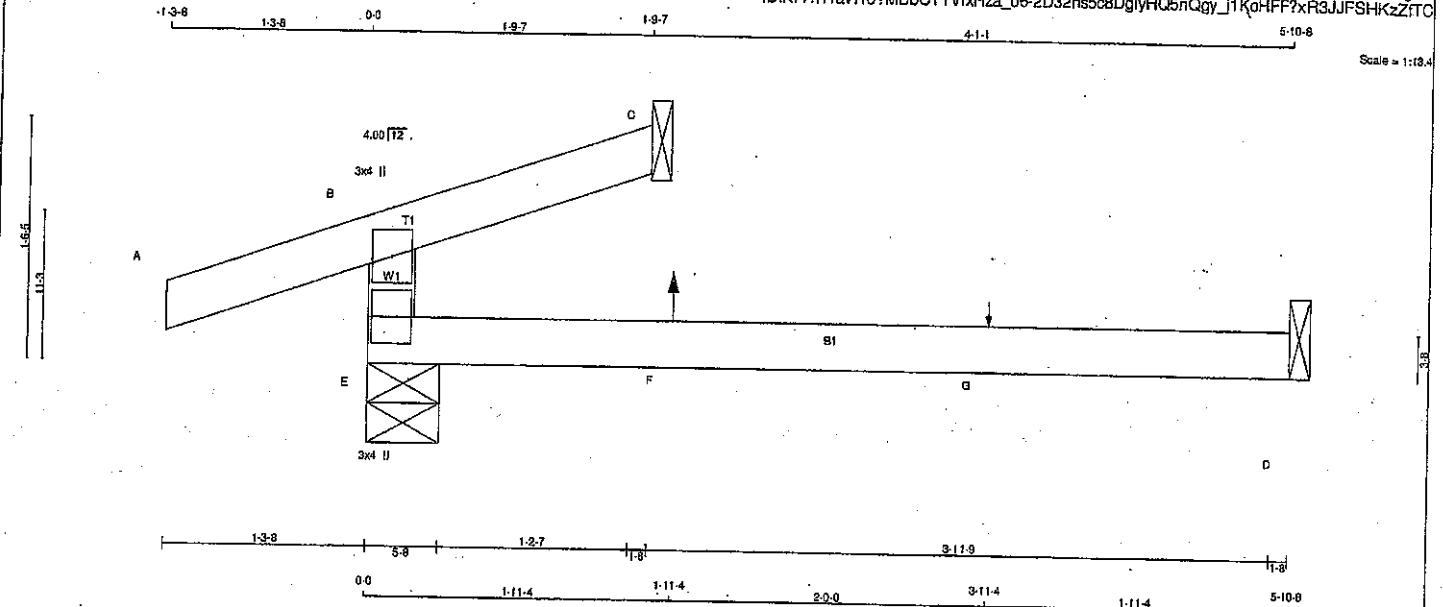
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107862

JOB NAME 417279	TRUSS NAME C57	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Thu Mar 18 16:36:01 2021 Page 1					
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TOTAL WEIGHT = 2 X 11 = 22 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	E - B	2x4	DRY	No.2
	A - C	2x4	DRY	No.2
	E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
	B	TMV+p	MT20	3.0	4.0		
	E	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	280	0	280	0	5-8	5-8
C	68	0	68	0	1-8	1-8
D	48	0	51	0	1-8	1-8

SEE MITEK STANDARD DETAIL B9779:1H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS				DEAD	SCIL
		SNOW	LIVE	PERM.LIVE	WIND		
E	197	136 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
C	48	21 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	35	0 / 3	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-223 / 0	0.0	0.0	0.12 (4)	7.81		
A-B	0 / 19	-91.8	-91.8	0.12 (1)	10.00		
B-C	-6 / 7	-91.8	-91.8	0.09 (4)	10.00		
E-F	0 / 0	-18.5	-18.5	0.13 (4)	10.00		
F-G	0 / 0	-18.5	-18.5	0.13 (4)	10.00		
G-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	5	1	8	FRONT	VERT	TOTAL	---	C1
G	3-11-4	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 89.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.12/1.00 (B-E:4), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

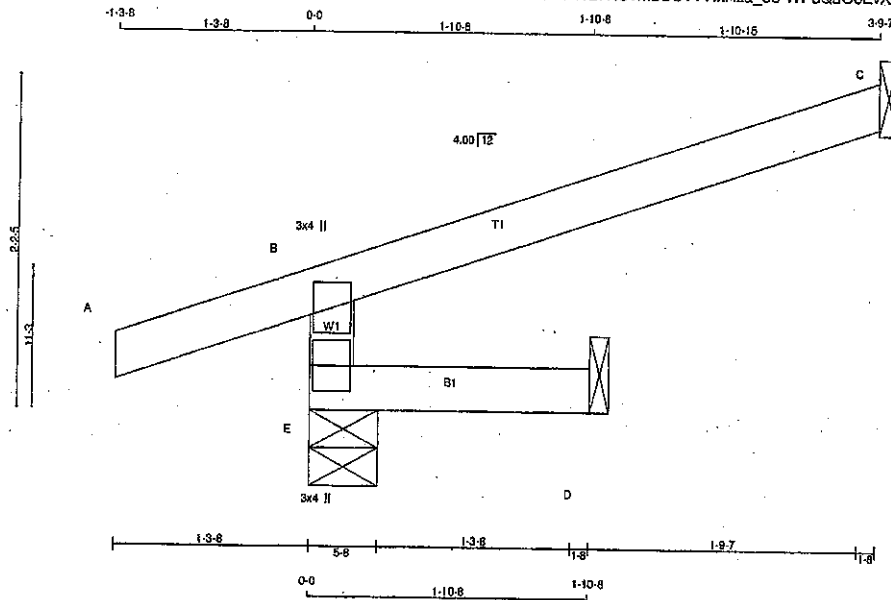
JSI GRIP = 0.09 (E) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107863

JOB NAME 417279	TRUSS NAME C58	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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TOTAL WEIGHT = 2 X 9 = 18 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
E - D	2x4	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	359	0	359	0	5-8	5-8
C	130	0	130	0	1-8	1-8
D	15	0	17	0	1-8	1-8

SEE MI TEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT E C D	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	250	190 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
	90	73 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH FR-TO
E-B	-340 / 0	0.0	0.0 (1) (4)	7.81
A-B	0 / 19	-91.8	-91.8 (1) (5)	10.00
B-C	-14 / 0	-91.8	-91.8 (1) (1)	6.25
E-D	0 / 0	-18.5	-18.5 (1) (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA C88-14
- TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CS1: TO=0.22/1.00 (B-C:1), BC=0.01/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS1=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

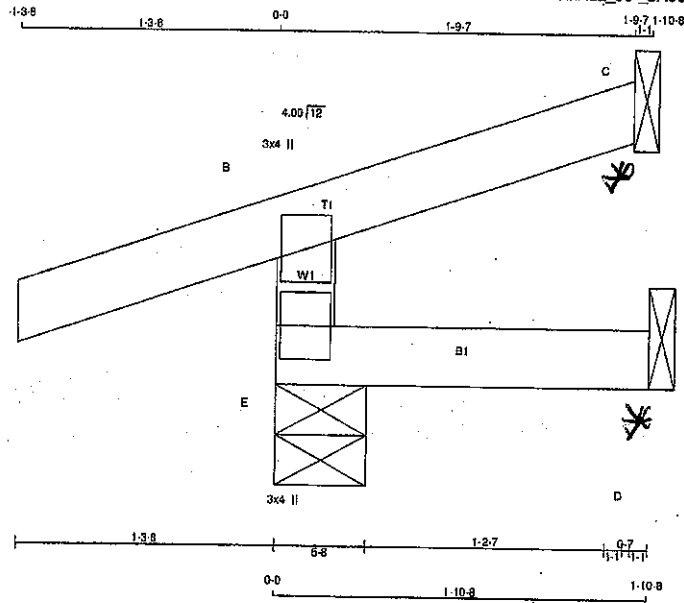
JSI GRIP = 0.14 (E) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107864

JOB NAME 417279	TRUSS NAME C59	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:10.5

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMV+p	MT20	3.0	4.0
E BMV1+p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
E	268	0	268	0	5-8	5-8
C	46	0	46	0	1-8	1-8
D	8	0	16	0	1-8	1-8

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
E	186	140/0	0/0		0/0	0/0	47/0	0/0
C	32	25/-17	0/0		0/0	0/0	7/0	0/0
D	7	0/-8	0/0		0/0	0/0	12/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	PLF	LC1 (LBS)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	LC1 (LBS)
FR-TO								
E-B	-240/0	0.0	0.0	0.04 (5)	7.81			
A-B	0/19	-91.8	-91.8	0.11 (1)	10.00			
B-C	-11/0	-91.8	-91.8	0.09 (1)	6.25			
E-D	0/0	-18.5	-18.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 6 = 13 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.11/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.08/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

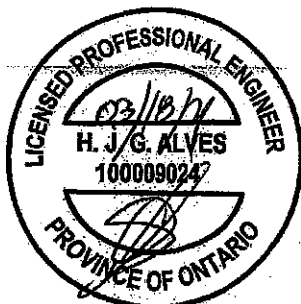
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.10 (E) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2107865



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018

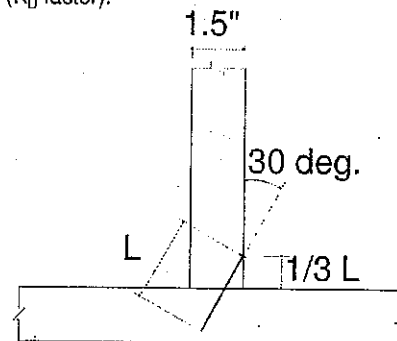
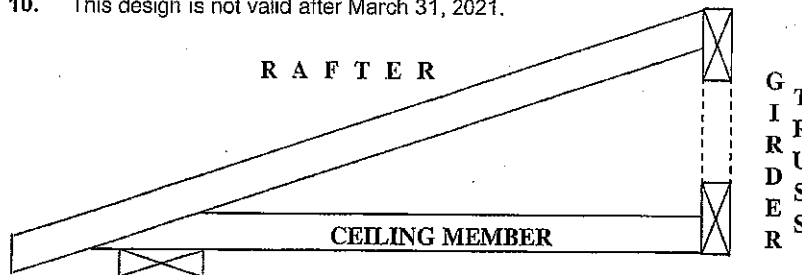
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: G = 0.42 (SPF), G = 0.49 (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

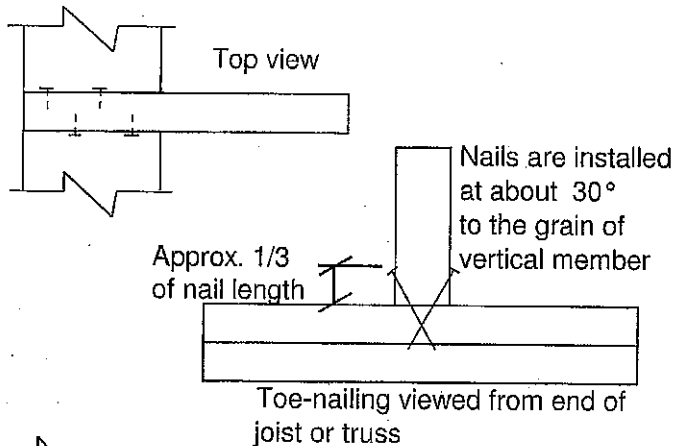
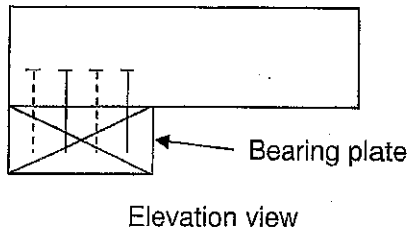
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

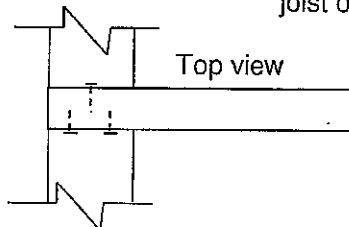
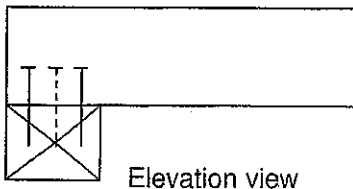
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889465



December 2, 2019

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

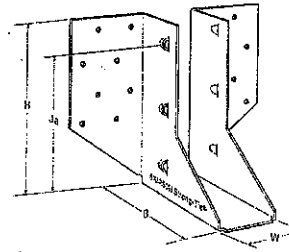
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

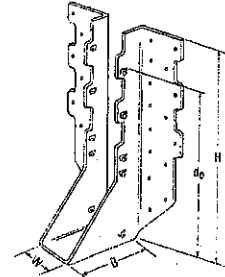
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

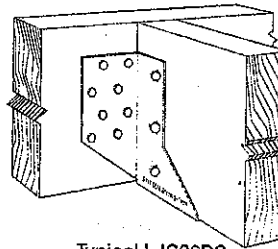
- See current catalogue for options



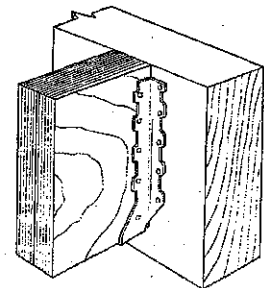
LJS26DS



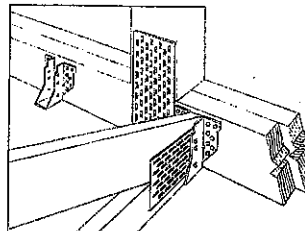
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



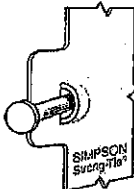
Typical HUS
Installation



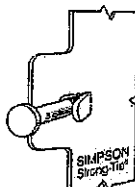
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15) lb.	Normal (K ₀ =1.00) lb.	Uplift (K ₀ =1.15) lb.	Normal (K ₀ =1.00) lb.
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 5/8	5 3/8	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 5/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 5/8	9 3/8	3	7 1/4	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	18	1 3/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

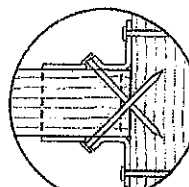
1. d_s is the distance from the seat of the hanger to the highest joist nail.



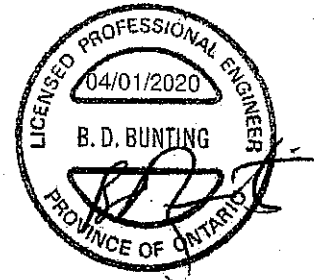
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,590



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

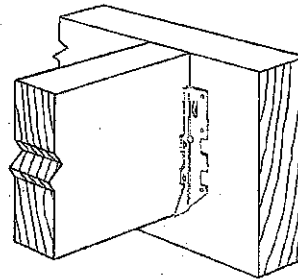
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

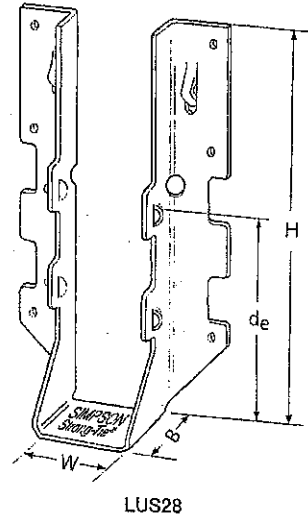
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



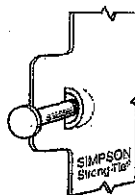
Typical LUS
Installation



LUS28

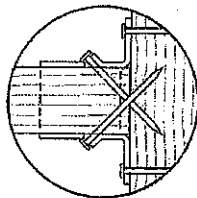
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 13/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/16	4 3/8	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 3/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 3/4	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 13/16	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020.
This information is updated periodically and should not be relied upon after June 30, 2022.
Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

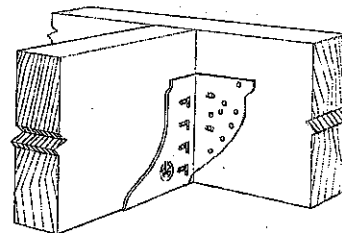
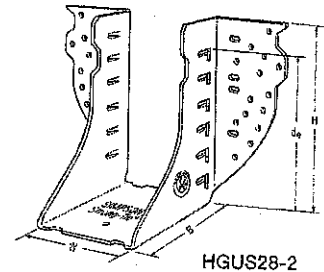
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

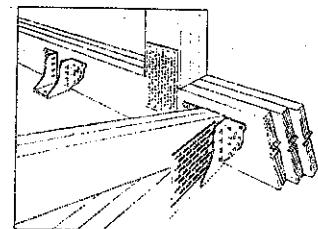
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



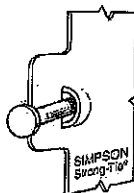
Typical HGUS Installation



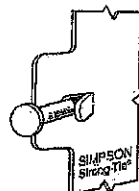
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
HGUS26	12	1 5/8	5 3/8	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 9/16	5 3/8	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 1/2	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 1/8	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/2	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 7/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

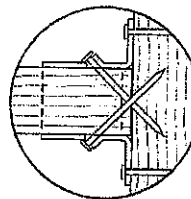
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



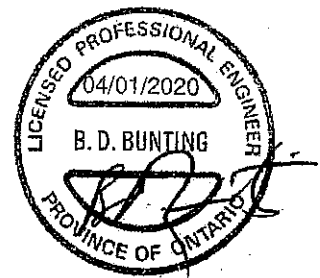
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14

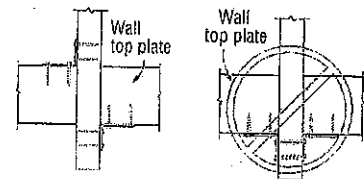
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

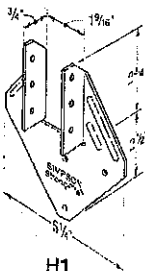
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

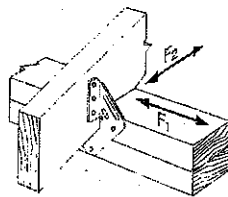


Install diagonally across from each other for minimum 2x truss.

Nailing into both sides of a single ply 2x truss may cause the wood to split.



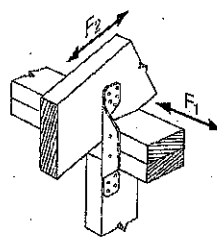
H1



H1 Installation



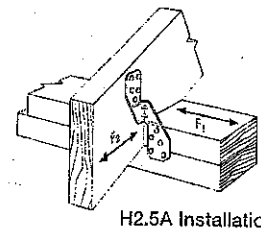
H2A



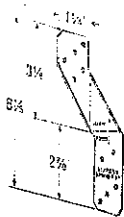
H2A Installation



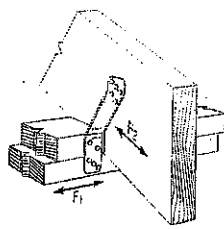
H2.5A



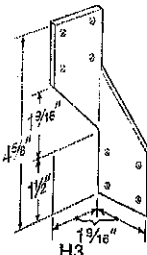
H2.5A Installation



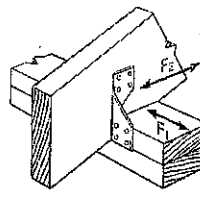
H2.5T



H2.5T Installation
(Nails into both top plates)



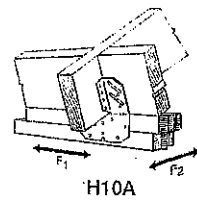
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D-Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	585	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



**LIMIT
STATES
DESIGN**

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TECHNICAL BULLETIN

TC – Truss Connectors



The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

Material: 16 gauge

Finish: G90 galvanized

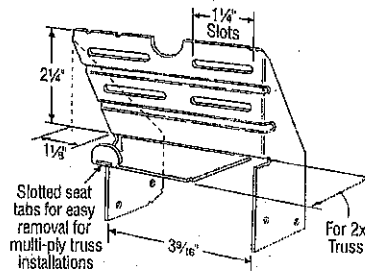
Design: Factored resistances are in accordance with CSA 086-14

Installation:

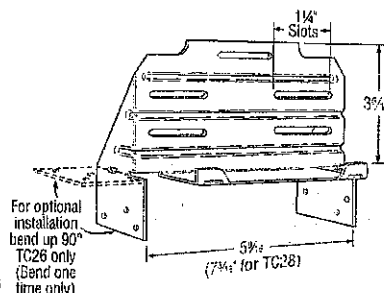
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss—allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

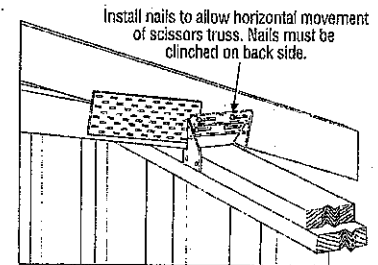
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



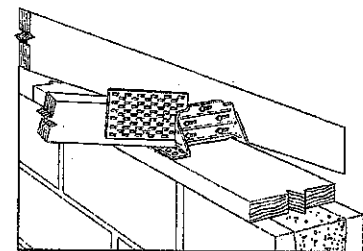
TC24
U.S. Patent 4,932,173



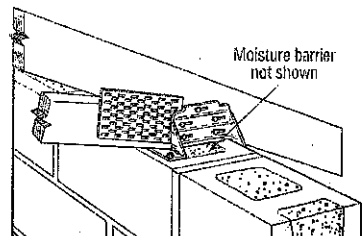
TC26
(TC28 Similar)



Typical TC24 Installation



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer (8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L Uplift (K _u =1.15)	S-P-F Uplift (K _u =1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L Uplift (K _u =1.15)	S-P-F Uplift (K _u =1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	680
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ½" x 2¼" Titen screws has a factored uplift resistance of 275 lb.



LIMIT
STATES
DESIGN

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TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers



All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

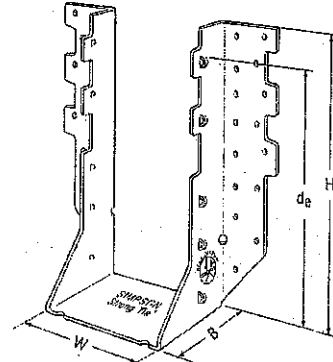
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

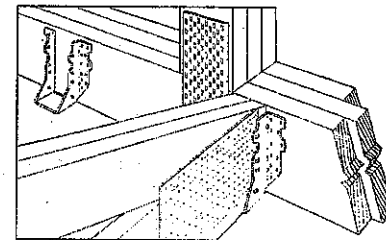
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications

Options:

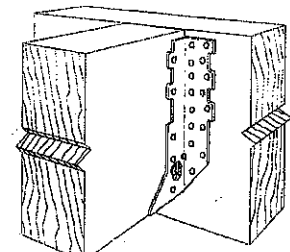
- See current catalogue for options



HHUS410



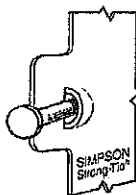
Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)



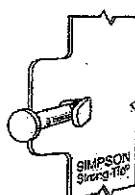
Typical HHUS Installation

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K ₂ =1.15)	Normal (K ₃ =1.00)	Uplift (K ₂ =1.15)	Normal (K ₃ =1.00)
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 3/8	9 3/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
HHUS210-4	14	6 1/8	8 2/8	3	7 2/8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS46	14	3 3/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS410	14	3 3/8	9	3	8	(30) 16d	(10) 16d	4670	9855	4235	7000
HHUS5.50/10	14	5 1/8	9	3	8	(30) 16d	(10) 16d	4670	10155	4235	7210
HHUS7.25/10	14	7 1/8	9	3 3/8	7 2/8	(30) 16d	(10) 16d	4670	10155	3370	7210

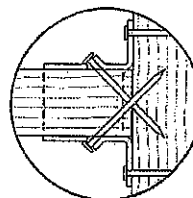
1. d_g is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

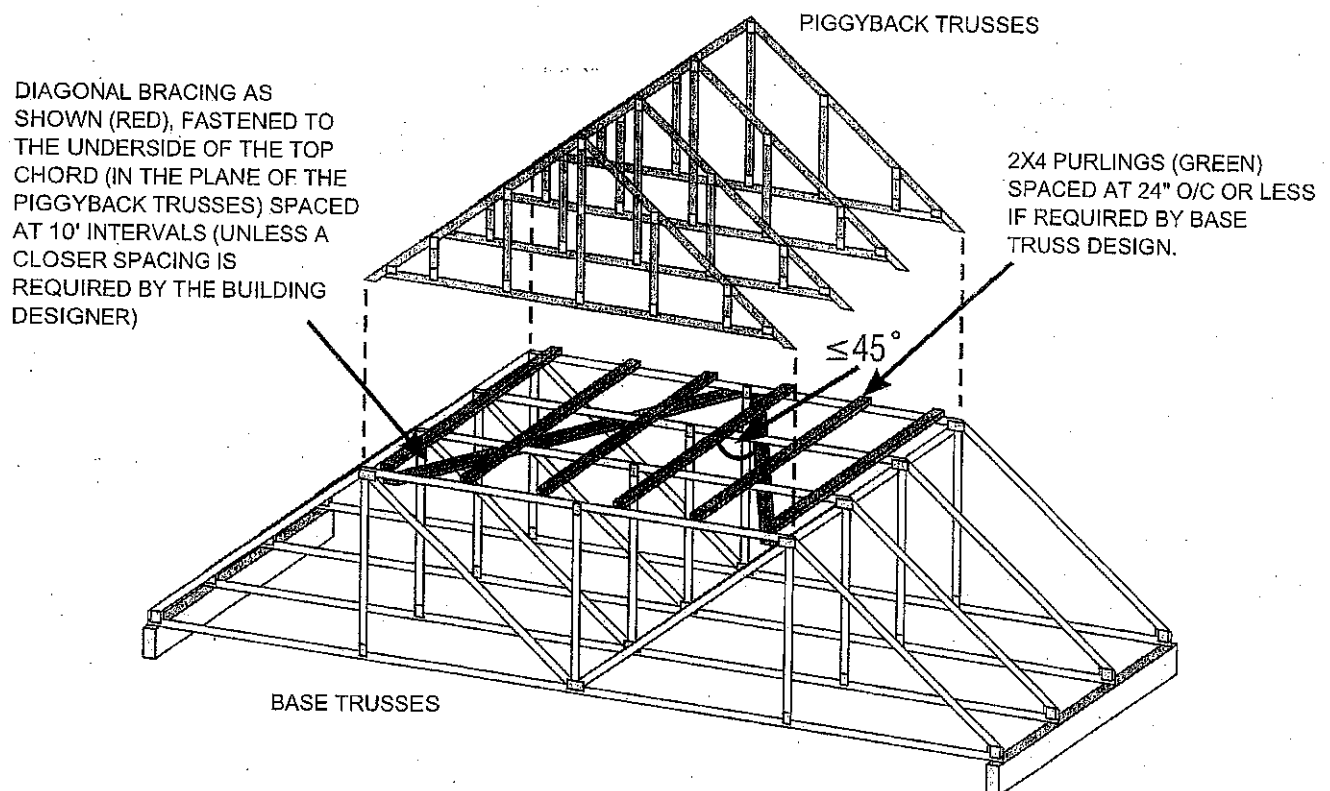
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

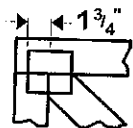
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

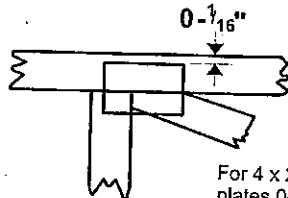
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0 - 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

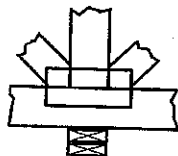
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

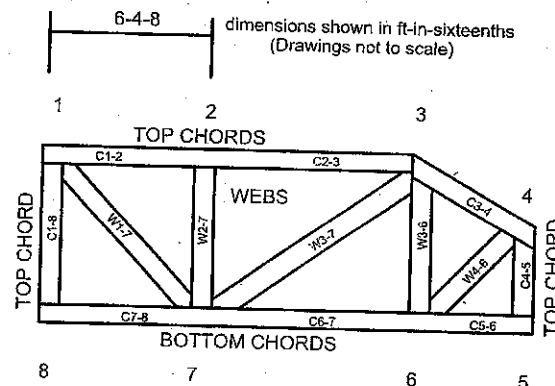


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.