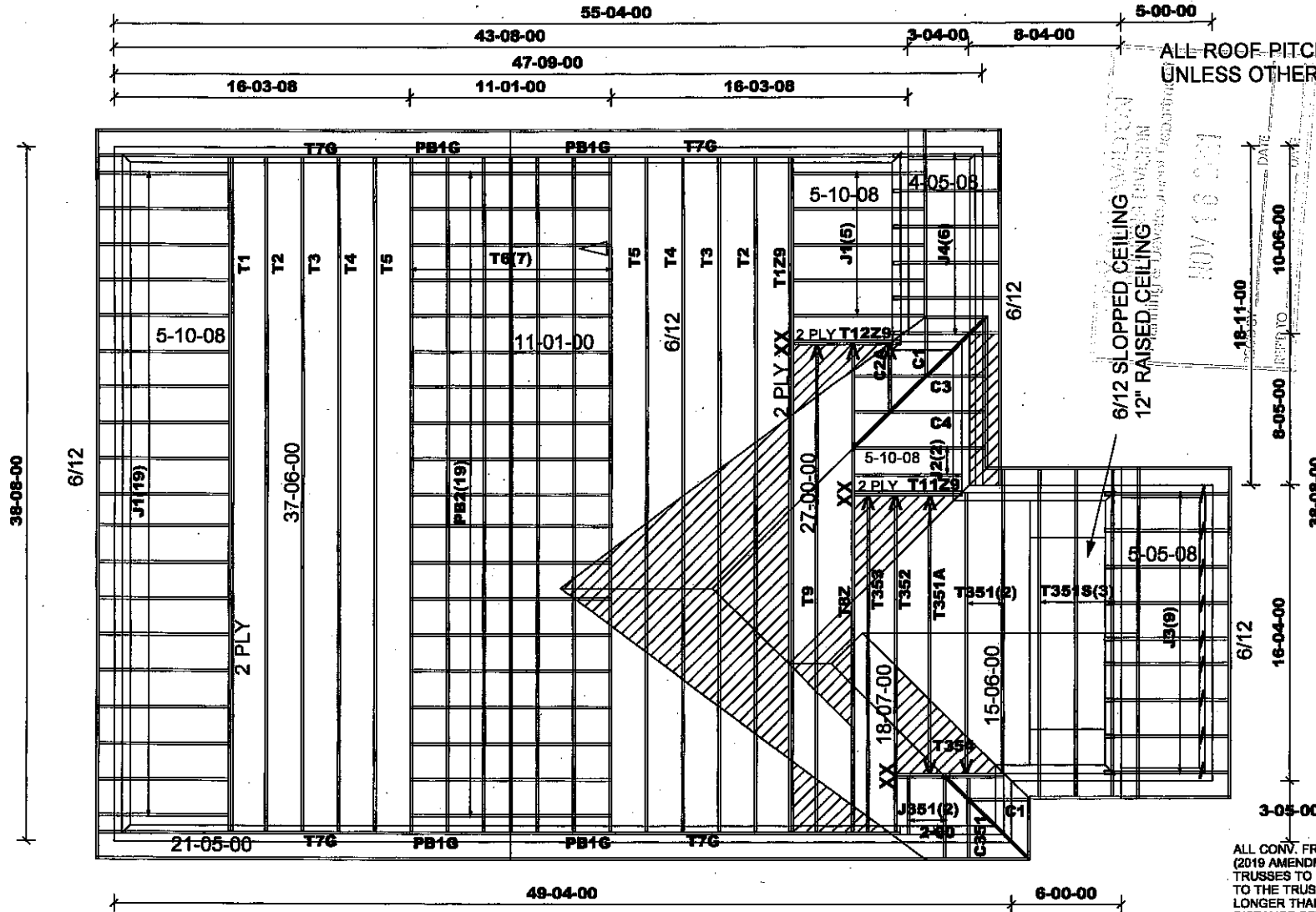




ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
H2.5T - (I)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS WITH
OBC 2012
(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL
PART: 9
Ss = 31.3 psf Sr = 8.4 psf

DESIGN LOADS:
TCSL = 25.6 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
(2019 AMENDMENT) ROOF RAFTERS THAT CROSS MEET OVER
TRUSSES TO BE 2X4 SPF @ 24"O.C. WITH A 2X4 VERT. POST
TO THE TRUSS UNDER NEATH AT EACH CROSS PT. VERT. POST
LONGER THAN 8" TO HAVE LATERAL BRACING SO THAT THE
DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING
DOES NOT EXCEED 8"

M13790



Job Track: **52166**
Plan Log: **204478**
Layout ID: **421219**

Builder / Location:
GREENPARK HOMES / HAMILTON

Project: **RUSSELL GARDENS PH.4**

Date: 2021-08-26 Sales: Rick DiCiano Designer: LC

Model / Elevation:
SPRINGFIELD 2 / EL.1 LOT 562

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED,
OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE.

Mitek ver 8.4.2.286



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 2
 Lot #:
 Elevation: EL.1

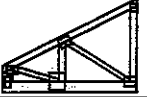
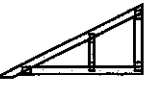
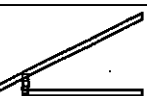
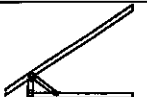
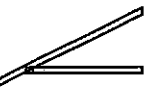
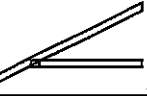
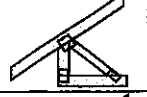
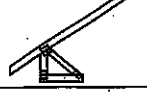
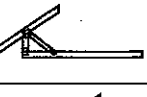
Job Track: 52166
 PlanLog: 204478
 Layout ID: 417422
 Ref #
 Page: 1 of 2
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Flat Girder	0 /12	37-06-00	4-01-04	2 x 6		4-01-04 4-01-04	416.51 250.00		
	1 2-ply	T1Z Flat Girder	0 /12	37-06-00	4-01-04	2 x 6		4-01-04 4-01-04	416.51 250.00		
	2	T2 Flat	0 /12	37-06-00	5-01-04	2 x 4 2 x 6		5-01-04 5-01-04	369.7 225.67		
	2	T3 Flat	0 /12	37-06-00	6-01-04	2 x 4 2 x 6		6-01-04 6-01-04	379.94 228.67		
	2	T4 Flat	0 /12	37-06-00	7-01-04	2 x 4 2 x 6		7-01-04 7-01-04	402.12 243.67		
	2	T5 Flat	0 /12	37-06-00	8-01-04	2 x 4 2 x 6		8-01-04 8-01-04	424.82 258.67		
	7	T6 Piggyback Base	0 /12	37-06-00	9-00-00	2 x 4 2 x 6		9-00-00 9-00-00	1698.27 1048.83		
	4	T7G GABLE	6 /12	21-05-00	9-00-00	2 x 4	1-03-08	1-02-00 9-00-00	467.53 296.67		
	1	T8 Hip Girder	8 /12	27-00-00	5-03-13	2 x 4 2 x 6		1-06-02 1-06-02	138.2 88.17		
	1	T9 Hip	8 /12	27-00-00	6-07-13	2 x 4		1-06-02 1-06-02	115.93 74.83		
	2	T10 Common	8 /12	18-11-00	7-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	173.46 107.33		
	3	T10A Common	8 /12	18-07-00	7-08-08	2 x 4		1-06-02 1-06-02	247.75 157.00		
	3	T10S Roof Special	8 /12 6 /12	18-11-00	7-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	257.01 171.50		
	1 2-ply	T11 Monopitch Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	65.45 41.33		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	52166
	Builder:	GREENPARK HOMES		PlanLog:	204478
	Project:	RUSSELL GARDENS PH.4		Layout ID:	417422
	Location:	HAMILTON		Ref #	
	Model:	SPRINGFIELD 2		Page:	2 of 2
Lot #:			Date:	03-22-2021	
Elevation:	EL.1		Designer:	Leo Chen	
			Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Monopitch Girder	6 / 12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.98 38.33		
	4	PB1G GABLE	6 / 12	5-09-00	2-10-08	2 x 4		2-10-08	64.66 44.00		
	19	PB2 Piggyback	6 / 12	11-01-00	2-10-08	2 x 4		1-04 1-04	525.37 332.50		
	24	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	403.07 258.00		
	2	J2 Jack-Open	8 / 12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	38.38 23.33		
	11	J3 Jack-Open	6 / 12	5-05-08	3-08-07	2 x 4	1-03-08	4-03 3-00-15	164.96 102.67		
	6	J4 Jack-Open	6 / 12	4-05-08	3-02-07	2 x 4	1-03-08	4-03 2-06-15	75.48 48.00		
	1	C1 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	9.03 5.67		
	1	C2A Jack-Open	8 / 12	1-08-08	3-11-02	2 x 4	1-03-08 1-10-15	1-06-02 2-07-13	11.52 8.33		
	1	C3 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	13.59 8.33		
	1	C4 Jack-Open	8 / 12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	16.33 10.33		

TOTAL # TRUSS= 107

TOTAL BFT OF ALL TRUSSES= 4319.83

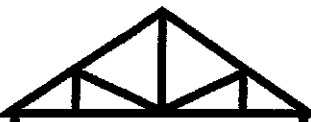
BFT.

TOTAL WEIGHT OF ALL TRSSES 6954.55 LBS

HARDWARE

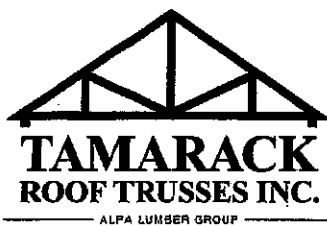
QTY	TYPE	MODEL	LENGTH
11	Hardware	H2.5T	
2	Hardware	HGUS26-2	
6	Hardware	LJS28DS	

TOTAL NUMBER OF ITEMS= 19

DELIVERY SHIPLIST											
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP			Lumber Yard: TAMARACK LUMBER				Job Track: 52166				
			Builder: GREENPARK HOMES				PlanLog: 204478				
			Project: RUSSELL GARDENS PH.4				Layout ID: 421219				
			Location: HAMILTON				Ref #				
			Model: SPRINGFIELD 2				Page: 1 of 3				
			Lot #:				Date: 08-25-2021				
			Elevation: EL.1 LOT 562				Designer: Leo Chen				
							Sales Rep: Rick DiCiano				

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Flat Girder	0 /12	37-06-00	4-01-04	2 x 6		4-01-04 4-01-04	416.51 250.00		
	1 2-ply	T129 Flat Girder	0 /12	37-06-00	4-01-04	2 x 6		4-01-04 4-01-04	422.45 253.33		
	2	T2 Flat	0 /12	37-06-00	5-01-04	2 x 4 2 x 6		5-01-04 5-01-04	369.7 225.67		
	2	T3 Flat	0 /12	37-06-00	6-01-04	2 x 4 2 x 6		6-01-04 6-01-04	379.94 228.67		
	2	T4 Flat	0 /12	37-06-00	7-01-04	2 x 4 2 x 6		7-01-04 7-01-04	402.12 243.67		
	2	T5 Flat	0 /12	37-06-00	8-01-04	2 x 4 2 x 6		8-01-04 8-01-04	424.82 258.67		
	7	T6 Piggyback Base	0 /12	37-06-00	9-00-00	2 x 4 2 x 6		9-00-00 9-00-00	1698.27 1048.83		
	4	T7G GABLE	6 /12	21-05-00	9-00-00	2 x 4	1-03-08	1-02-00 9-00-00	467.53 296.67		
	1	T8Z Hip Girder	8 /12	27-00-00	5-03-13	2 x 4 2 x 6		1-06-02 1-06-02	137.95 88.17		
	1	T9 Hip	8 /12	27-00-00	6-07-13	2 x 4		1-06-02 1-06-02	115.93 74.83		
	1 2-ply	T1129 Monopitch Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	65.45 41.33		
	1 2-ply	T1229 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	60.33 39.33		
	2	T351 Common	8 /12	15-06-00	6-06-13	2 x 4	1-03-08	1-04-13 1-04-13	128.74 81.33		
	1	T351A Common	8 /12	15-04-00	6-06-13	2 x 4		1-06-02 1-04-13	62.21 40.00		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 2
 Lot #:
 Elevation: EL.1 LOT 562

Job Track: 52166
 PlanLog: 204478
 Layout ID: 421219
 Ref #
 Page: 2 of 3
 Date: 08-25-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	T351S Roof Special	8 /12 6 /12	15-06-00	6-06-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	199.44 134.50		
	1	T352 Hip Girder	8 /12	18-07-00	5-03-13	2 x 4 2 x 6		1-06-02 1-06-02	98.64 64.50		
	1	T353 Hip	8 /12	18-07-00	6-03-13	2 x 4		1-06-02 1-06-02	79.04 50.17		
	1 2-ply	T354 Half Hip Girder	8 /12	5-10-08	3-06-02	2 x 4 2 x 6		1-04-13 3-06-02	80.78 37.33		
	4	PB1G GABLE	6 /12	5-09-00	2-10-08	2 x 4		2-10-08	64.66 44.00		
	19	PB2 Piggyback	6 /12	11-01-00	2-10-08	2 x 4		1-04 1-04	525.37 332.50		
	24	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	403.07 256.00		
	2	J2 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	38.38 23.33		
	9	J3 Jack-Open	6 /12	5-05-08	3-08-07	2 x 4	1-03-08	4-03 3-00-15	134.96 84.00		
	6	J4 Jack-Open	6 /12	4-05-08	3-02-07	2 x 4	1-03-08	4-03 2-06-15	75.48 48.00		
	2	J351 Jack-Open	8 /12	3-02-00	3-06-02	2 x 4	1-03-08	1-04-13 3-06-02	24.78 16.67		
	2	C1 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	18.05 11.33		
	1	C2A Jack-Open	8 /12	1-08-08	3-11-02	2 x 4	1-03-08 1-10-15	1-06-02 2-07-13	11.52 8.33		
	1	C3 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	13.59 8.33		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREENPARK HOMES Project: RUSSELL GARDENS PH.4 Location: HAMILTON Model: SPRINGFIELD 2 Lot #: Elevation: EL.1 LOT 562	Job Track: 52166 PlanLog: 204478 Layout ID: 421219 Ref # Page: 3 of 3 Date: 08-25-2021 Designer: Leo Chen Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	C4 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	16.33 10.33		
	1	C351 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-04-09	1-04-13 2-07-02	10.5 7.00		

TOTAL # TRUSS= 111 TOTAL BFT OF ALL TRUSSES= 4306.82 BFT. TOTAL WEIGHT OF ALL TRSSES 6926.54 LBS

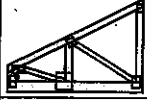
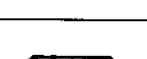
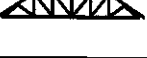
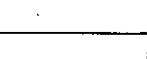
HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	H2.5T	
3	Hardware	HGUS26-2	
8	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 20

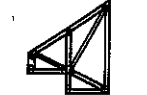
DELIVERY SHIPLIST				
 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track:	52166
	Builder:	GREENPARK HOMES	PlanLog:	204478
	Project:	RUSSELL GARDENS PH.4	Layout ID:	417441
	Location:	HAMILTON	Ref #	
	Model:	SPRINGFIELD 2	Page:	1 of 2
	Lot #:		Date:	03-22-2021
	Elevation:	EL.2	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T122 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.98 38.33		
	1 2-ply	T122Z Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.98 38.33		
	1 2-ply	T15 Hip Girder	8 /12	37-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	414.58 256.00		
	1 2-ply	T15Z Hip Girder	8 /12	37-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	414.58 256.00		
	2	T16 Hip	8 /12	37-10-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	355.39 217.00		
	2	T17 Hip	8 /12	37-10-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	371.77 226.67		
	2	T18 Hip	8 /12	37-10-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	388.4 239.00		
	2	T19 Hip	8 /12	37-10-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	387.76 237.33		
	5	T20 Hip	8 /12	37-10-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	983.98 592.50		
	2	T21 Hip	8 /12	37-10-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	406.58 247.33		
	1	T22 Hip Girder	8 /12	27-07-00	4-01-04	2 x 4 2 x 6	1-03-08	2-08-13 1-04-13	157.03 97.33		
	1	T23 Hip	8 /12	27-07-00	5-01-04	2 x 4	1-03-08	2-08-13 1-04-13	111.37 70.33		
	1	T24 Hip Girder	8 /12	18-11-00	6-04-08	2 x 4 2 x 6	1-03-08 1-03-08	2-08-13 2-08-13	107.46 67.33		
	3	T25 Hip	8 /12	18-11-00	7-08-08	2 x 4	1-03-08 1-03-08	2-08-13 2-08-13	278.53 174.00		

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST					
	Lumber Yard:	TAMARACK LUMBER			Job Track:	52166
	Builder:	GREENPARK HOMES			PlanLog:	204478
	Project:	RUSSELL GARDENS PH.4			Layout ID:	417441
	Location:	HAMILTON			Ref #	
	Model:	SPRINGFIELD 2			Page:	2 of 2
Lot #:				Date:	03-22-2021	
Elevation:	EL.2			Designer:	Leo Chen	
				Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T26 Common	8 / 12	10-06-00	4-10-13	2 x 4	1-03-08	1-04-13 1-04-13	42.28 26.83		
	1	T26G GABLE	8 / 12	10-06-00	4-10-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	43.58 28.17		
	2	T27 Common	8 / 12	10-06-00	6-02-13	2 x 4	1-03-08	2-08-13 2-08-13	94.17 62.00		
	1 2-ply	T28 Monopitch Girder	8 / 12	5-05-08	6-04-08	2 x 4		1-04-13 6-04-08	71.56 46.67		
	3	PB11 Piggyback	8 / 12	14-08-11	2-00-00	2 x 4			129.85 86.00		
	30	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	503.84 320.00		
	3	J11 Jack-Open	8 / 12	5-05-08	6-04-08	2 x 4	1-03-08	1-04-13 6-04-08	84.51 56.50		
	6	J12 Jack-Open	8 / 12	4-05-08	4-02-03	2 x 4	1-03-08	4-07 3-04-02	79.11 52.00		
	1	C11 Jack-Open	8 / 12	1-09-07	3-11-02	2 x 4	1-03-08 1-01	2-08-13 3-11-02	11.35 7.50		
	1	C12 Jack-Open	8 / 12	1-10-08	5-03-02	2 x 4	1-03-08 1-10-15	2-08-13 3-11-13	14.09 9.50		
	1	C13 Jack-Open	8 / 12	1-09-07	3-11-02	2 x 4	1-03-08 3-08-01	2-08-13 3-11-02	15.43 10.17		

TOTAL # TRUSS= 80

TOTAL BFT OF ALL TRUSSES= 3462.82

BFT. TOTAL WEIGHT OF ALL TRSSES 5585.13 LBS

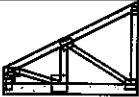
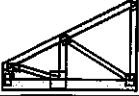
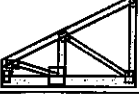
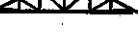
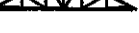
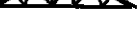
HARDWARE

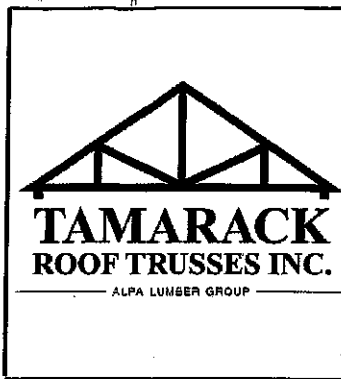
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
3	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 10

DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER	Job Track:	52166
	Builder:	GREENPARK HOMES	PlanLog:	204478
	Project:	RUSSELL GARDENS PH.4	Layout ID:	417443
	Location:	HAMILTON	Ref #	
	Model:	SPRINGFIELD 2	Page:	1 of 3
	Lot #:		Date:	03-22-2021
	Elevation:	EL.3	Designer:	Leo Chen
		Sales Rep:	Rick DiCiano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12Z3 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.98 38.33		
	1 2-ply	T12Z4 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.98 38.33		
	1 2-ply	T12Z5 Monopitch Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.98 38.33		
	1 2-ply	T31 Hip Girder	6 /12	37-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	352.76 220.67		
	1 2-ply	T31Z Hip Girder	6 /12	37-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	352.76 220.67		
	2	T32 Hip	6 /12	37-10-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	352.41 216.33		
	2	T33 Hip	6 /12	37-10-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	354.63 216.00		
	2	T34 Hip	6 /12	37-10-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	352.12 215.00		
	2	T35 Hip	6 /12	37-10-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	364.92 222.67		
	5	T36 Hip	6 /12	37-10-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	969.41 591.67		
	2	T37 Hip	6 /12	37-10-00	10-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	389.39 238.67		
	1	T38 Hip Girder	6 /12	28-02-00	4-01-04	2 x 4 2 x 6	1-03-08	1-02-00 1-02-00	154.92 97.00		
	1	T39 Hip	6 /12	28-02-00	5-01-04	2 x 4	1-03-08	1-02-00 1-02-00	111.06 69.83		
	1	T40 Hip Girder	6 /12	12-04-00	4-05-04	2 x 4 2 x 6	1-03-08	2-06-00 2-06-00	63.1 39.67		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENPARK HOMES
 Project: RUSSELL GARDENS PH.4
 Location: HAMILTON
 Model: SPRINGFIELD 2
 Lot #:
 Elevation: EL.3

Job Track: 52166
 PlanLog: 204478
 Layout ID: 417443
 Ref #
 Page: 2 of 3
 Date: 03-22-2021
 Designer: Leo Chen
 Sales Rep: Rick DiCiano

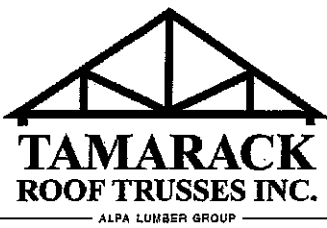
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T41 Hip Girder	6 / 12	18-11-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 2-06-00	90.53 56.67		
	1	T42 Hip	6 / 12	18-11-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 2-06-00	76.56 49.00		
	1	T43 Hip	6 / 12	18-11-00	6-01-04	2 x 4	1-03-08	1-02-00 2-06-00	83.38 53.33		
	2	T44 Common	6 / 12	18-11-00	6-06-12	2 x 4	1-03-08	1-02-00 2-06-00	156.79 101.00		
	3	PB31 Piggyback	6 / 12	6-01-00	1-06-04	2 x 4			37.36 26.00		
	19	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	319.1 202.67		
	4	J31 Jack-Open	6 / 12	3-10-08	4-05-04	2 x 4	1-03-08	1-02-00 4-05-04	78.98 54.00		
	5	C31 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	35.1 23.33		
	5	C32 Jack-Open	6 / 12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	47.85 30.00		
	5	C33 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	57.91 36.67		
	5	C34 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	70.66 43.33		
	2	C35 Jack-Open	6 / 12	1-10-08	3-04-12	2 x 4	1-03-08	2-06-00 3-05-04	21.03 15.00		
	2	C36 Jack-Open	6 / 12	1-09-07	3-04-12	2 x 4	1-03-08 2-01-01	2-06-00 3-04-12	25.59 17.67		

TOTAL # TRUSS= 83

TOTAL BFT OF ALL TRUSSES= 3171.84

BFT. TOTAL WEIGHT OF ALL TRSSES 5095.23 LBS

DELIVERY SHIPLIST		
	Lumber Yard:	TAMARACK LUMBER
	Builder:	GREENPARK HOMES
	Project:	RUSSELL GARDENS PH.4
	Location:	HAMILTON
	Model:	SPRINGFIELD 2
	Lot #:	
	Elevation:	EL.3
	Job Track:	52166
	PlanLog:	204478
	Layout ID:	417443
	Ref #	
	Page:	3 of 3
	Date:	03-22-2021
	Designer:	Leo Chen
	Sales Rep:	Rick DiCiano

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
8	Hardware	LJS26DS	

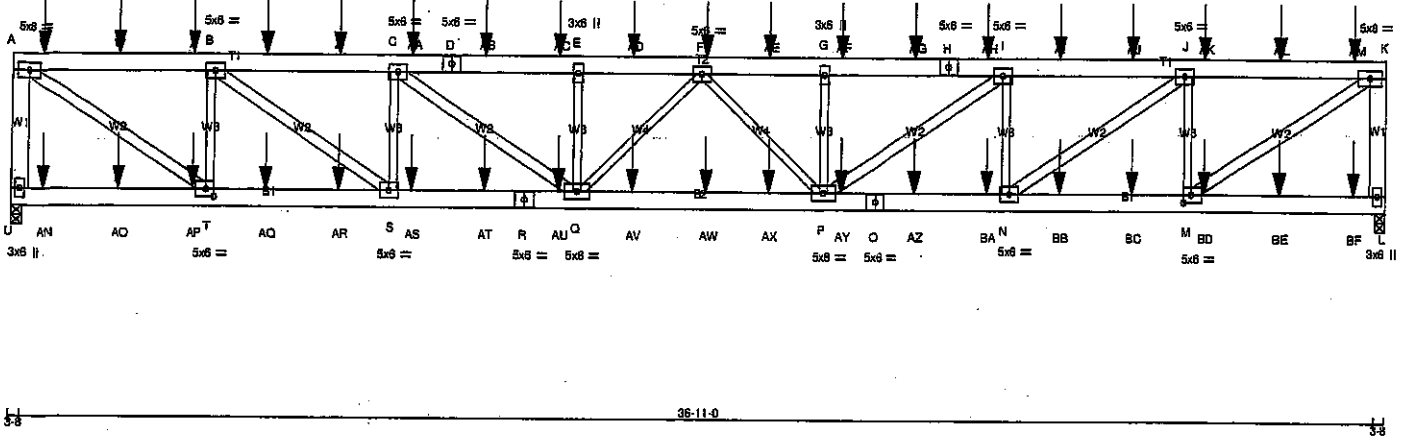
TOTAL NUMBER OF ITEMS= 11

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417422	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:GV6EnpZB3IFFw?AU1cPMODzZmnp-SU8fL233m6PXLtn_cYlooUeFaqRIT6QqW6Yr82zZ6av

0-0 10-12 4-5-10 5-4-6 5-0-2 10-4-6 10-5-12 5-5-12 15-11-8 2-9-8 18-9-0 2-9-8 21-6-8 5-5-12 27-0-4 27-1-8 5-0-2 32-1-10 4-5-10 38-7-4 37-6-0 5-4-6
Scale = 1:59.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
U - A	2x6	DRY	No.2	SPF	
A - D	2x6	DRY	No.2	SPF	
D - H	2x6	DRY	No.2	SPF	
H - K	2x6	DRY	No.2	SPF	
L - K	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
B - S	2x4	DRY	No.2	SPF	
A - T	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	
P - I	2x4	DRY	No.2	SPF	
C - Q	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
U-A	2	12
A-D	2	12
D-H	2	12
H-K	2	12
K-L	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-R	2	12
R-O	2	12
O-L	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

<u>BEARINGS</u>		FACTORED		MAXIMUM FACTORED		INPUT	REQRD
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG
U		3373	0	3373	0	0	BRG
L		3373	0	3373	0	0	IN-SX
						3-8	3-8
						3-8	3-8

<u>UNFACTORED REACTIONS</u>		<u>MAX./MIN. COMPONENT REACTIONS</u>						
JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	2389	1547 / 0	0 / 0	0 / 0	0 / 0	0 / 0	842 / 0	0 / 0
L	2389	1547 / 0	0 / 0	0 / 0	0 / 0	0 / 0	842 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
FORCE (LBS)	VERT. LOAD (PLF)	FORCE (LBS)	FORCE (LBS)
FROM	TO	FROM	TO
FR-TO		FR-TO	
U-A	-3292 / 0	0.0	0.0
A-V	-4346 / 0	-91.8	-91.8
V-W	-4346 / 0	-91.8	-91.8
W-X	-4346 / 0	-91.8	-91.8
X-B	-4346 / 0	-91.8	-91.8
B-Y	-6974 / 0	-91.8	-91.8
Y-Z	-6974 / 0	-91.8	-91.8
Z-C	-6974 / 0	-91.8	-91.8
C-AA	-8417 / 0	-91.8	-91.8
AA-D	-8417 / 0	-91.8	-91.8
D-AB	-8417 / 0	-91.8	-91.8
AB-AC	-8417 / 0	-91.8	-91.8
AC-E	-8417 / 0	-91.8	-91.8
E-AD	-8417 / 0	-91.8	-91.8
AD-F	-8417 / 0	-91.8	-91.8
FAE	-8418 / 0	-91.8	-91.8
AE-G	-8418 / 0	-91.8	-91.8
G-AF	-8418 / 0	-91.8	-91.8
AF-AG	-8418 / 0	-91.8	-91.8
AG-H	-8418 / 0	-91.8	-91.8
H-AH	-8418 / 0	-91.8	-91.8
AH-I	-8418 / 0	-91.8	-91.8
IAI	-6974 / 0	-91.8	-91.8
IAJ	-6974 / 0	-91.8	-91.8
AJ-J	-6974 / 0	-91.8	-91.8
J-AK	-4346 / 0	-91.8	-91.8
AK-AL	-4346 / 0	-91.8	-91.8
AL-AM	-4346 / 0	-91.8	-91.8
AM-K	-4346 / 0	-91.8	-91.8
L-K	-3292 / 0	0.0	0.0

U-AN	0 / 0	-18.5	-18.5
AN-AO	0 / 0	-18.5	-18.5
AO-AP	0 / 0	-18.5	-18.5
AP-T	0 / 0	-18.5	-18.5
T-AQ	0 / 4346	-18.5	-18.5
AQ-AR	0 / 4346	-18.5	-18.5
AR-S	0 / 4346	-18.5	-18.5
S-AS	0 / 6974	-18.5	-18.5
AS-AT	0 / 6974	-18.5	-18.5
AT-R	0 / 6974	-18.5	-18.5
R-AU	0 / 6974	-18.5	-18.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.44")

CSI: TC=0.28/1.00 (G-I), BC=0.63/1.00 (P-Q), WB=0.46/1.00 (K-M), SSI=0.16/1.00 (A-B-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.87 (M) (INPUT = 0.90)
JSI METAL = 0.63 (R) (INPUT = 1.00)



Structural component only
DWG# T-2108086 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417422	T1	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:GV6EnpZB3IFFw?AU1oPMODzZmp-SU8fl233m6PXLtn_cYlooUeFAqRIT6OqW6Yr82zZ6av

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B, C, F, I, J						
B	TMVW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
G	TMVW-w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
K	TMVW-t	MT20	5.0	6.0		
L	BMV1-p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0	2.50	2.50
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
S	BMVW-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)
FR-TO				FR-TO			
AU-Q	0 / 8974	-18.5	-18.5 0.50 (1)	10.00			
Q-AV	0 / 8624	-18.5	-18.5 0.63 (1)	10.00			
AV-AW	0 / 8624	-18.5	-18.5 0.63 (1)	10.00			
AW-AX	0 / 8624	-18.5	-18.5 0.63 (1)	10.00			
AX-P	0 / 8624	-18.5	-18.5 0.63 (1)	10.00			
P-AY	0 / 8974	-18.5	-18.5 0.50 (1)	10.00			
AY-O	0 / 8974	-18.5	-18.5 0.50 (1)	10.00			
O-AZ	0 / 8974	-18.5	-18.5 0.50 (1)	10.00			
AZ-BA	0 / 8974	-18.5	-18.5 0.50 (1)	10.00			
BA-N	0 / 8974	-18.5	-18.5 0.50 (1)	10.00			
N-BB	0 / 4346	-18.5	-18.5 0.32 (1)	10.00			
BB-BC	0 / 4346	-18.5	-18.5 0.32 (1)	10.00			
BC-M	0 / 4346	-18.5	-18.5 0.32 (1)	10.00			
M-BD	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
BD-BE	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
BE-BF	0 / 0	-18.5	-18.5 0.05 (4)	10.00			
BF-L	0 / 0	-18.5	-18.5 0.05 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	18-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
V	10-12	-79	-79	--	FRONT	VERT	TOTAL	--	C1
W	2-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
X	4-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
Y	6-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
Z	8-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AA	10-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AB	12-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AC	14-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AD	16-10-12	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AE	20-7-4	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AF	22-7-4	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AG	24-7-4	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AH	26-7-4	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AI	28-7-4	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AJ	30-7-4	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AK	32-7-4	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AL	34-7-4	-76	-76	--	FRONT	VERT	TOTAL	--	C1
AM	36-7-4	-79	-79	--	FRONT	VERT	TOTAL	--	C1
AN	10-12	-22	-22	--	FRONT	VERT	TOTAL	--	C1
AO	2-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AP	4-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AQ	6-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AR	8-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AS	10-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AT	12-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AU	14-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AV	16-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AW	18-10-12	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AX	20-7-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AY	22-7-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
AZ	24-7-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
BA	26-7-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
BB	28-7-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
BC	30-7-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
BD	32-7-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
BE	34-7-4	-21	-21	--	FRONT	VERT	TOTAL	--	C1
BF	36-7-4	-22	-22	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 417422	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:17:09 2021 Page 2	
				ID:GV6EnpZB3lFFw?AU1oPMODzZmnp-wqi2Y04hXQXOz1MAAFp1KhBO8Ek6CWvzImPhUzZ6au	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0		
B	TMWW-t	MT20	5.0	6.0	2.50	2.25
C, F, I						
C	TMWW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMW-w	MT20	3.0	6.0		
G	TMW-w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.25
K	TMVW-t	MT20	6.0	9.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	2.00
N	BMWW-t	MT20	6.0	9.0	4.50	2.75
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	6.0	9.0	4.50	2.75
T	BMWW-t	MT20	5.0	8.0	2.50	2.00
U	BMV1+p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AB	2-10-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AC	4-10-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AD	6-10-12	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	8-10-12	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

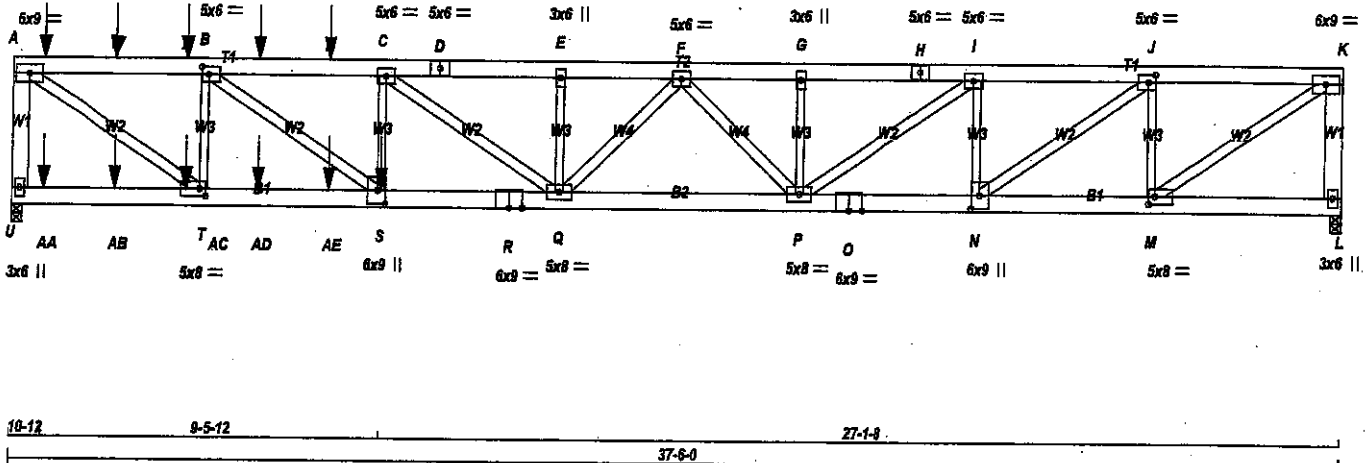
NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108087 *ML*

JOB NAME 421219	TRUSS NAME T1Z9	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 5 Jan 21 2021 Mitek Industries, Inc. Tue Aug 24 17:28 2021 Page 1 ID:GV6EnpZB3IFFw7AU1oPMDzZmnp-2BQKfig_ChEq7PoZg6naUbKQ3YzN2EITBSnYkaml	

10-12 36-7-4 Scale = 1:61.6



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
U - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
N - I	2x3	DRY	No.2	SPF
T - B	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF
P - G	2x3	DRY	No.2	SPF
Q - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
U-A 2	12	TOP
A-D 2	12	TOP
D-H 2	12	TOP
H-K 2	12	TOP
K-L 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U-R 2	12	SIDE (183.1)
R-O 2	12	TOP
O-L 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
C-S 1	2	SIDE (627.0)
2x3 1	2	
T-B 1	6	
M-J 1	6	
P-G 1	6	
Q-E 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	BRG	BRG
U	4914	0	0	3-8
L	3016	0	0	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	3473	2291 / 0	0 / 0	0 / 0	0 / 0	1182 / 0	0 / 0
L	2132	1405 / 0	0 / 0	0 / 0	0 / 0	726 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	WEBS	MEMB.	FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						FR-TO			
U-A	-4838 / 0	0.0	0.0	0.35 (1)	6.59	S-C	-250 / 0	0.03 (1)	0.10 (1)
A-V	-6814 / 0	-91.8	-91.8	0.23 (1)	4.47	N-I	-2030 / 0	0.24 (1)	0.24 (1)
V-W	-6814 / 0	-91.8	-91.8	0.23 (1)	4.47	B-S	0 / 5737	0.51 (1)	0.51 (1)
W-X	-6814 / 0	-91.8	-91.8	0.23 (1)	4.47	A-T	0 / 7997	0.71 (1)	0.71 (1)
X-B	-6814 / 0	-91.8	-91.8	0.23 (1)	4.47	T-B	-4266 / 0	0.51 (1)	0.51 (1)
B-Y	-11253 / 0	-91.8	-91.8	0.37 (1)	3.45	M-K	0 / 4908	0.43 (1)	0.43 (1)
Y-Z	-11253 / 0	-91.8	-91.8	0.37 (1)	3.45	N-J	0 / 3801	0.32 (1)	0.32 (1)
Z-C	-10907 / 0	-91.8	-91.8	0.33 (1)	3.55	M-J	-2652 / 0	0.32 (1)	0.32 (1)
C-D	-10907 / 0	-91.8	-91.8	0.33 (1)	3.55	P-I	0 / 2680	0.24 (1)	0.24 (1)
D-E	-10907 / 0	-91.8	-91.8	0.33 (1)	3.55	C-Q	-427 / 0	0.10 (1)	0.10 (1)
E-F	-10907 / 0	-91.8	-91.8	0.26 (1)	3.60	P-G	-390 / 0	0.05 (1)	0.05 (1)
F-G	-9137 / 0	-91.8	-91.8	0.20 (1)	3.94	Q-E	-413 / 0	0.05 (1)	0.05 (1)
G-H	-9137 / 0	-91.8	-91.8	0.25 (1)	3.91	Q-F	0 / 1124	0.10 (1)	0.10 (1)
H-I	-9137 / 0	-91.8	-91.8	0.25 (1)	3.91	F-P	-1488 / 0	0.22 (1)	0.22 (1)
I-J	-6969 / 0	-91.8	-91.8	0.17 (1)	4.44				
J-K	-4058 / 0	-91.8	-91.8	0.12 (1)	5.55				
L-K	-2987 / 0	0.0	0.0	0.22 (1)	7.81				

U-AA	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AA-AB	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AB-AC	0 / 0	-18.5	-18.5	0.05 (4)	10.00
AC-T	0 / 0	-18.5	-18.5	0.05 (4)	10.00
T-AD	0 / 6614	-18.5	-18.5	0.52 (1)	10.00
AD-AE	0 / 6614	-18.5	-18.5	0.52 (1)	10.00
AE-S	0 / 6614	-18.5	-18.5	0.52 (1)	10.00
S-R	0 / 11253	-18.5	-18.5	0.82 (1)	10.00
R-Q	0 / 11253	-18.5	-18.5	0.82 (1)	10.00
Q-P	0 / 10145	-18.5	-18.5	0.71 (1)	10.00
P-O	0 / 6969	-18.5	-18.5	0.50 (1)	10.00
O-N	0 / 6969	-18.5	-18.5	0.50 (1)	10.00
N-M	0 / 4058	-18.5	-18.5	0.29 (1)	10.00
M-L	0 / 0	-18.5	-18.5	0.03 (4)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
S	10-4-8	-2191	-2191	---	BACK	VERT	TOTAL	---	C1
V	10-12	-79	-79	---	BACK	VERT	TOTAL	---	C1
W	2-10-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
X	4-10-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Y	6-10-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
Z	8-10-12	-76	-76	---	BACK	VERT	TOTAL	---	C1
AA	10-12	-22	-22	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.28")
ALLOWABLE DEFL. (TL) = L/360 (1.25")
CALCULATED VERT. DEFL. (TL) = L/859 (0.52")

CSI: TC=0.37/1.00 (B-C:1), BC=0.82/1.00 (O-S:1), WB=0.71/1.00 (A-T:1), SS=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

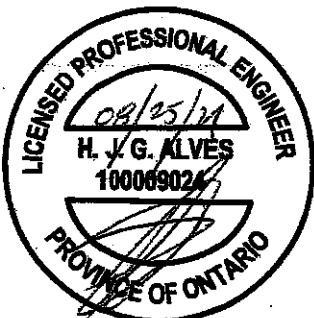
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1887 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (T) (INPUT = 0.90)
JSI METAL=0.83 (R) (INPUT = 1.00)



Structural component only
DWG# T-2129737

JOB NAME 421219	TRUSS NAME T1Z9	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0		
B	TMVW-t	MT20	5.0	6.0	2.50	2.25
C, F, I						
C	TMVW-t	MT20	5.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMVW-w	MT20	3.0	6.0		
G	TMVW-w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
J	TMVW-t	MT20	5.0	6.0	2.50	2.25
K	TMVW-t	MT20	6.0	9.0		
L	BMV1-p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	8.0	2.50	2.00
N	BMVW-t	MT20	6.0	9.0	4.50	2.75
O	BS-t	MT20	6.0	9.0		
P	BMVW-t	MT20	5.0	8.0		
Q	BMVW-t	MT20	5.0	8.0		
R	BS-t	MT20	6.0	9.0		
S	BMVW-t	MT20	6.0	9.0	4.50	2.75
T	BMVW-t	MT20	5.0	8.0	2.50	2.00
U	BMV1-p	MT20	3.0	6.0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AB	2-10-12	-21	-21	—	BACK	VERT	TOTAL	—	C1
AC	4-10-12	-21	-21	—	BACK	VERT	TOTAL	—	C1
AD	6-10-12	-21	-21	—	BACK	VERT	TOTAL	—	C1
AE	8-10-12	-21	-21	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

NOTES- (1)

- 1) Lateral braces to be a minimum of 2X4 SPF #2.

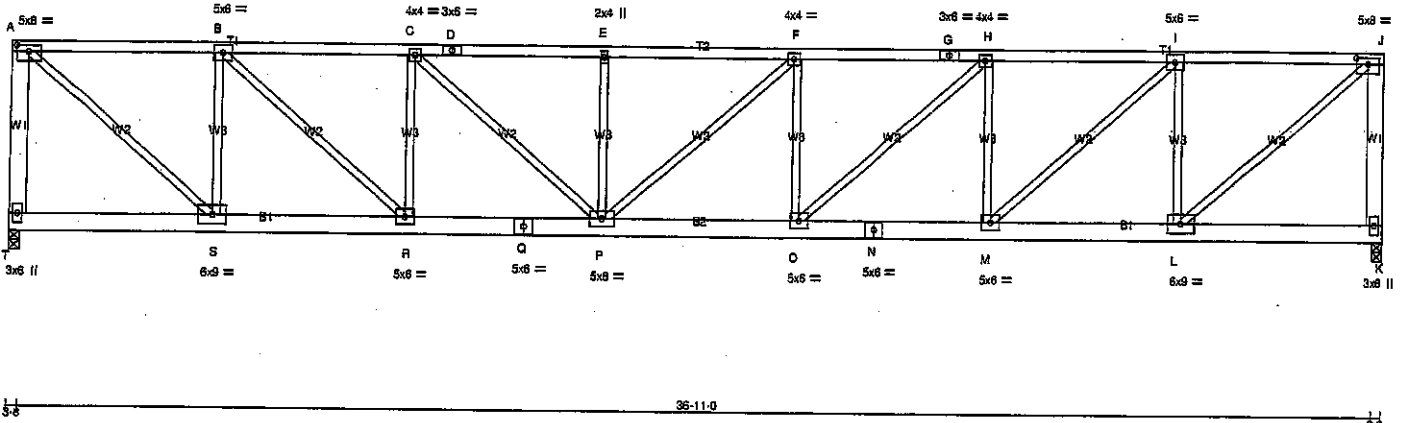


Structural component only
DWG# T-2129737

JOB NAME 417422	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:GV6EnpZB3IFFw?AU1oPMODzZmnp-OsGQmk5JHjFfaBxMjzKGtKvU6e8dx_G7_P1yDwzZ6at

0-0 5-7-5 5-7-5 5-3-1 10-10-6 5-3-1 16-1-7 5-3-1 21-4-9 5-3-1 26-7-10 5-3-1 31-10-11 5-7-5 37-6-0
Scale = 1:59.8



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
T - A	2x6	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
K - Q	2x6	DRY	No.2
T - J	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-I	MT20	5.0	8.0	2.00 4.00
B	TMVW-I	MT20	5.0	6.0	
C, F, H					
C	TMVW-I	MT20	4.0	4.0	
D	TS-I	MT20	3.0	6.0	
E	TMVW-I	MT20	2.0	4.0	
G	TS-I	MT20	3.0	6.0	
I	TMVW-I	MT20	5.0	6.0	
J	TMVW-I	MT20	5.0	8.0	2.00 4.00
K	BMV-I	MT20	3.0	6.0	
L	BMVW-I	MT20	6.0	9.0	
M, O, R					
M	BMVW-I	MT20	5.0	6.0	
N	BS-I	MT20	5.0	6.0	
P	BMVW-I	MT20	5.0	8.0	
Q	BS-I	MT20	5.0	6.0	
S	BMVW-I	MT20	8.0	9.0	
T	BMV-I	MT20	3.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
T	2067	0	2067	0	3-8
K	2067	0	2067	0	3-8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
T	1462	959.0	0.0	0.0	0.0	503.0
K	1462	959.0	0.0	0.0	0.0	503.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
T-A	-2019.0	0.0	0.0 0.52 (1)	L-J	0.2812	0.63 (1)	
A-B	-2150.0	-91.8	-91.8 0.53 (1)	A-S	0.2811	0.63 (1)	
B-C	-3410.0	-91.8	-91.8 0.68 (1)	L-I	-1700.0	0.61 (1)	
C-D	-4051.0	-91.8	-91.8 0.84 (1)	S-B	-1702.0	0.61 (1)	
D-E	-4051.0	-91.8	-91.8 0.84 (1)	M-I	0.1692	0.38 (1)	
E-F	-4051.0	-91.8	-91.8 0.85 (1)	B-R	0.1696	0.38 (1)	
F-G	-4059.0	-91.8	-91.8 0.65 (1)	M-H	-1043.0	0.37 (1)	
G-H	-4059.0	-91.8	-91.8 0.65 (1)	R-C	-1036.0	0.37 (1)	
H-I	-3408.0	-91.8	-91.8 0.68 (1)	O-H	0.876	0.20 (1)	
I-J	-2150.0	-91.8	-91.8 0.53 (1)	C-P	0.882	0.19 (1)	
K-J	-2020.0	0.0	0.0 0.52 (1)	O-F	-481.0	0.17 (1)	
				P-E	-484.0	0.17 (1)	
				P-F	-11.0	0.01 (1)	
T-S	0.0	-18.5	-18.5 0.06 (4)				
S-R	0.2150	-18.5	-18.5 0.29 (1)				
R-Q	0.3410	-18.5	-18.5 0.46 (1)				
Q-P	0.3410	-18.5	-18.5 0.46 (1)				
P-O	0.4059	-18.5	-18.5 0.54 (1)				
O-N	0.3408	-18.5	-18.5 0.45 (1)				
N-M	0.3408	-18.5	-18.5 0.45 (1)				
M-L	0.2150	-18.5	-18.5 0.29 (1)				
L-K	0.0	-18.5	-18.5 0.06 (4)				

TOTAL WEIGHT = 2 X 185 = 370 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.25")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.25")
ALLOWABLE DEFL. (TL) = $L/360$ (1.25")
CALCULATED VERT. DEFL. (TL) = $L/928$ (0.48")

CSI: TC=0.68/1.00 (H-I), BC=0.54/1.00 (O-P);
WB=0.83/1.00 (J-L); SS=0.24/1.00 (I-J)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

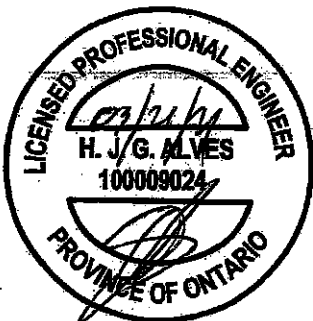
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.85 (Q) (INPUT = 1.00)

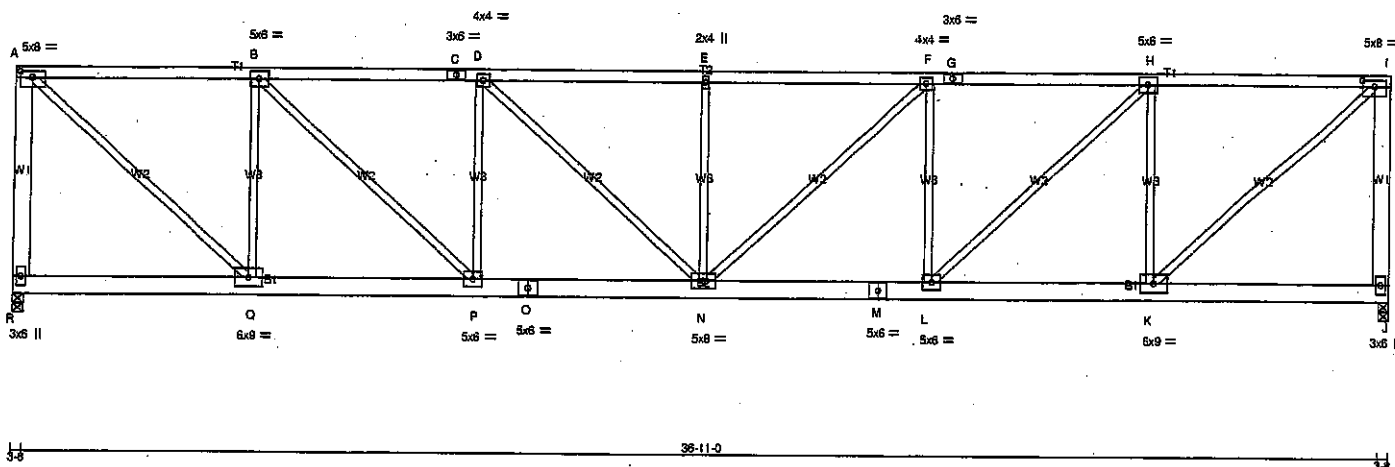
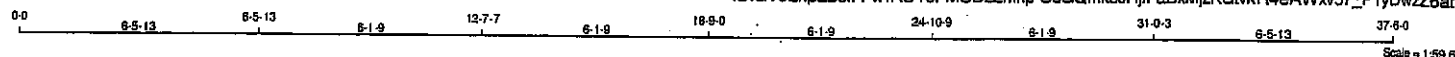


Structural component only
DWG# T-2108088

JOB NAME 417422	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:GV6EnpZB3IFFw?AU1oPM0dzZmnp-OsGQmk5JHjFfaBxmJzKGtkR4eAWxv57_P1yDwzZ6at



TOTAL WEIGHT = 2 X 190 = 380 lb

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	
R - A	2x6	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
J - L	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX
R	2067	0	2067	0	0	3-8
J	2067	0	2067	0	0	3-8

UNFACTORED REACTIONS

	MAX./MIN. COMPONENT REACTIONS						
	1ST CASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	1462	959	0	0	0	0	503
R	1462	959	0	0	0	0	503
J	1462	959	0	0	0	0	503

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE	VERT. LOAD	MAX. (PLF)	MAX. (LC)	MAX. FORCE	VERT. LOAD	MAX. (PLF)	MAX. (LC)
FR-TO								
R-A	-2015	0	0.0	0.0	0.86	(1)	7.12	K-I
A-B	-2011	0	-91.8	-91.8	0.71	(1)	3.95	A-Q
B-C	-3067	0	-91.8	-91.8	0.87	(1)	3.15	K-H
C-D	-3067	0	-91.8	-91.8	0.87	(1)	3.15	Q-B
D-E	-3432	0	-91.8	-91.8	0.77	(1)	3.20	L-H
E-F	-3432	0	-91.8	-91.8	0.77	(1)	3.20	B-P
F-G	-3067	0	-91.8	-91.8	0.87	(1)	3.15	L-F
G-H	-3067	0	-91.8	-91.8	0.87	(1)	3.15	P-D
H-I	-2011	0	-91.8	-91.8	0.71	(1)	3.95	N-F
J-I	-2015	0	0.0	0.0	0.86	(1)	7.12	D-N
R-Q	0	0	-18.5	-18.5	0.08	(4)	10.00	N-E
Q-P	0	2011	-18.5	-18.5	0.27	(1)	10.00	
P-O	0	3067	-18.5	-18.5	0.42	(1)	10.00	
O-N	0	3067	-18.5	-18.5	0.42	(1)	10.00	
N-M	0	3067	-18.5	-18.5	0.42	(1)	10.00	
M-L	0	3067	-18.5	-18.5	0.42	(1)	10.00	
L-K	0	2011	-18.5	-18.5	0.27	(1)	10.00	
K-J	0	0	-18.5	-18.5	0.08	(4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00.12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.39")

CSI: TC=0.87/1.00 (B-D:1), BC=0.42/1.00 (N-P:1),
WB=0.90/1.00 (H-K:1), SS=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

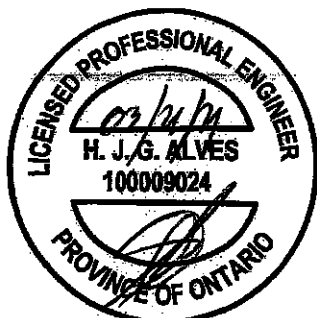
JSI GRIP = 0.85 (I) (INPUT = 0.90)
JSI METAL = 0.58 (O) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	8.0	2.00	4.00
B	TMVW-I	MT20	5.0	6.0		
C	TS-I	MT20	3.0	6.0		
D	TMVW-I	MT20	4.0	4.0		
E	TMVW-I	MT20	2.0	4.0		
F	TMVW-I	MT20	4.0	4.0		
G	TS-I	MT20	3.0	6.0		
H	TMVW-I	MT20	5.0	6.0		
I	TMVW-I	MT20	5.0	8.0	2.00	4.00
J	BMV1-p	MT20	3.0	6.0		
K	BMVW-I	MT20	6.0	9.0		
L	BMVW-I	MT20	5.0	6.0		
M	BS-I	MT20	5.0	6.0		
N	BMVW-I	MT20	5.0	8.0		
O	BS-I	MT20	5.0	6.0		
P	BMVW-I	MT20	5.0	6.0		
Q	BMVW-I	MT20	6.0	9.0		
R	BMV1-p	MT20	3.0	6.0		

NOTES:

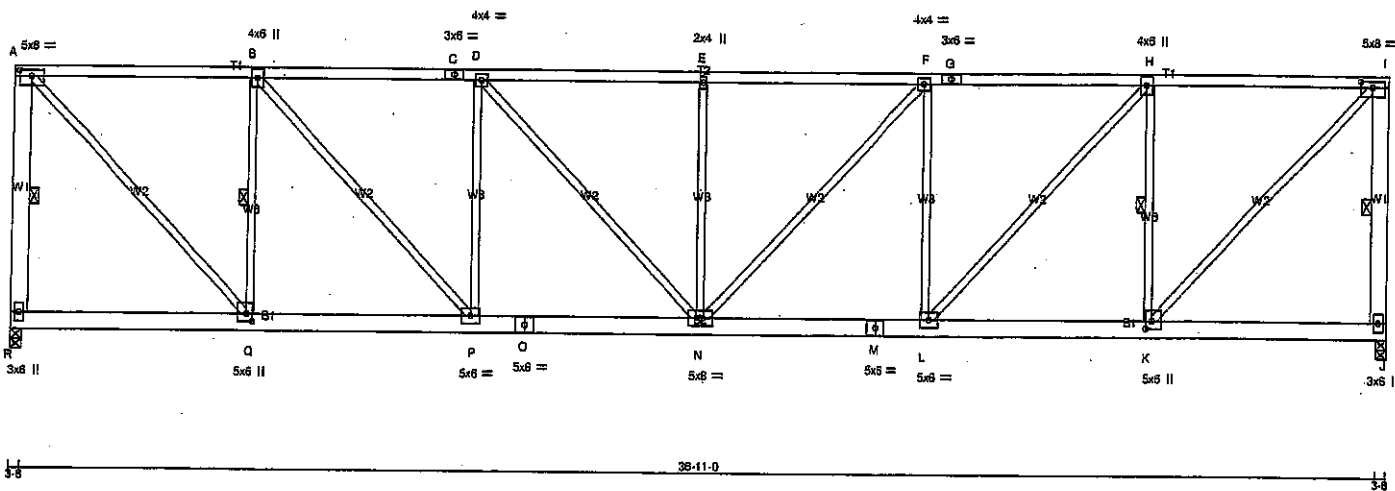
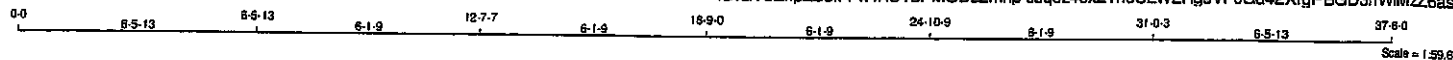
- 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108089

JOB NAME 417422	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:GV6EnpZB3IFFw?AU1oPMODzZmnp-s3qoz46x21n6CLWZHgsVP6Gd42XfgPBGD3nWIMzZ6as



TOTAL WEIGHT = 2 X 201 = 402 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
R - A	2x6	DRY	No.2
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
J - I	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - M	2x6	DRY	No.2
M - J	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	5.0	8.0	2.00 4.00
B	TMVW-t	MT20	4.0	6.0	
C	TS-t	MT20	3.0	6.0	
D	TMVW-t	MT20	4.0	4.0	
E	TMVW-t	MT20	2.0	4.0	
F	TMVW-t	MT20	4.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	6.0	
I	TMVW-t	MT20	5.0	8.0	2.00 4.00
J	BMV1-p	MT20	3.0	6.0	
K	BMVW-t	MT20	5.0	6.0	2.50 2.00
L	BMVW-t	MT20	5.0	6.0	
M	BS-t	MT20	5.0	6.0	
N	BMVW-t	MT20	5.0	8.0	
O	BS-t	MT20	5.0	6.0	
P	BMVW-t	MT20	5.0	6.0	
Q	BMVW-t	MT20	5.0	6.0	2.50 2.00
R	BMV1-p	MT20	3.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
R	2067	0	2067	0	0	3-8	3-8	
J	2067	0	2067	0	0	3-8	3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS					
	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
R	1462	959 / 0	0 / 0	0 / 0	0 / 0	503 / 0	0 / 0	
J	1462	959 / 0	0 / 0	0 / 0	0 / 0	503 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, H-K, B-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
R-A	-2016 / 0	0.0	0.0 0.28 (1)	5.69	K-I	0 / 2489	0.56 (1)
A-B	-1714 / 0	-91.8	-91.8 0.67 (1)	4.25	A-Q	0 / 2469	0.58 (1)
B-C	-2612 / 0	-91.8	-91.8 0.79 (1)	3.45	K-H	-1646 / 0	0.50 (1)
C-D	-2612 / 0	-91.8	-91.8 0.79 (1)	3.45	Q-B	-1646 / 0	0.50 (1)
D-E	-2924 / 0	-91.8	-91.8 0.68 (1)	3.50	L-H	0 / 1334	0.30 (1)
E-F	-2924 / 0	-91.8	-91.8 0.68 (1)	3.50	B-P	0 / 1334	0.30 (1)
F-G	-2612 / 0	-91.8	-91.8 0.79 (1)	3.45	L-F	-876 / 0	0.72 (1)
G-H	-2612 / 0	-91.8	-91.8 0.79 (1)	3.45	P-D	-876 / 0	0.72 (1)
H-I	-1714 / 0	-91.8	-91.8 0.67 (1)	4.25	N-F	0 / 482	0.10 (1)
J-I	-2016 / 0	0.0	0.0 0.28 (1)	5.69	D-N	0 / 482	0.10 (1)
					N-E	-575 / 0	0.47 (1)
R-Q	0 / 0	-18.5	-18.5 0.08 (4)	10.00			
Q-P	0 / 1714	-18.5	-18.5 0.24 (1)	10.00			
P-O	0 / 2612	-18.5	-18.5 0.36 (1)	10.00			
O-N	0 / 2612	-18.5	-18.5 0.36 (1)	10.00			
N-M	0 / 2612	-18.5	-18.5 0.36 (1)	10.00			
M-L	0 / 2612	-18.5	-18.5 0.36 (1)	10.00			
L-K	0 / 1714	-18.5	-18.5 0.24 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0" PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.79/1.00 (F-H:1), BC=0.36/1.00 (N-P:1),
WB=0.72/1.00 (F-L:1), SSI=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

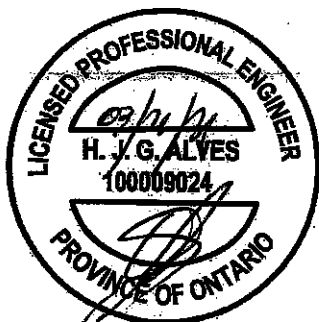
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

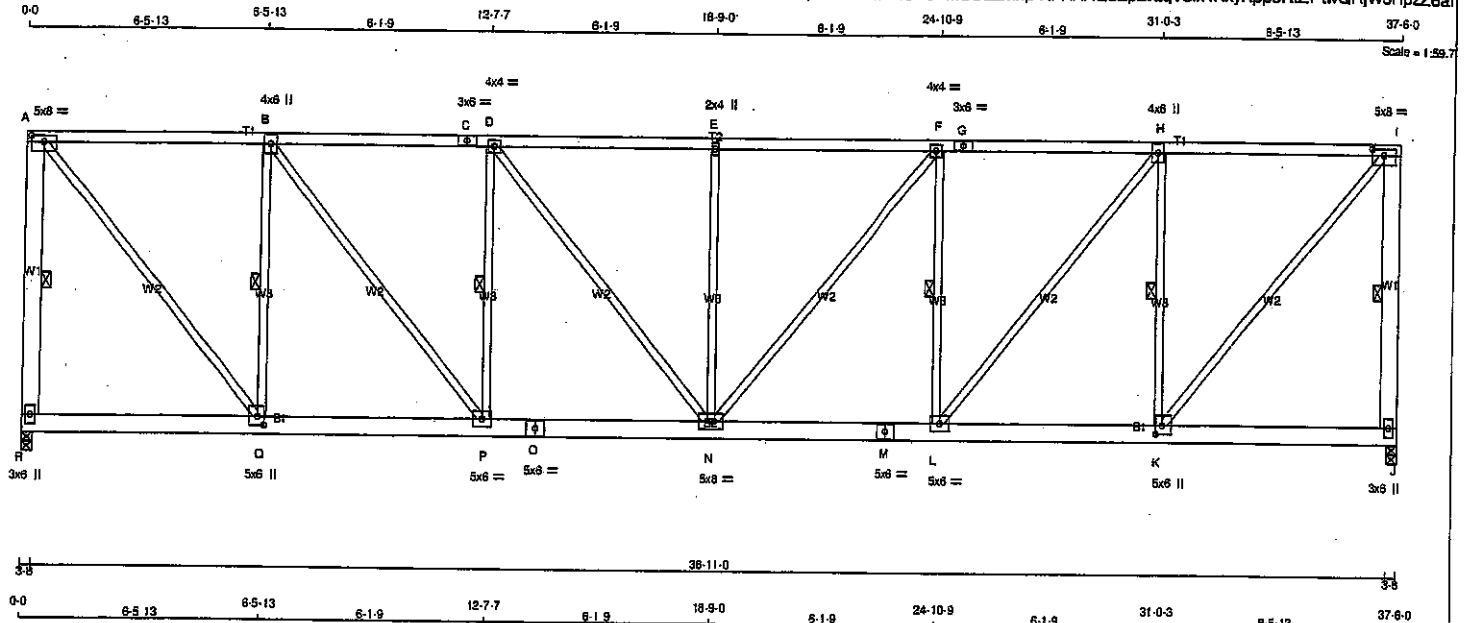
JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.60 (C) (INPUT = 1.00)



Structural component only
DWG# T-2108090

JOB NAME 417422	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
R - A	2x6 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
C - G	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
J - I	2x6 DRY	No.2	SPF
R - O	2x6 DRY	No.2	SPF
O - M	2x6 DRY	No.2	SPF
M - J	2x6 DRY	No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.00	4.00
B	TMVW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	8.0	2.00	4.00
J	BMV1-p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.75	2.00
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.75	2.00
R	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
R	2067	0	2067	0	3-8	3-8
J	2067	0	2067	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1482	959 / 0	0 / 0	0 / 0	0 / 0	503 / 0	0 / 0
J	1482	959 / 0	0 / 0	0 / 0	0 / 0	503 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, H-K, B-Q, F-L, D-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED		MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	FACTORED		
FR-TO		VERT. LOAD (PLF)	LC1 (PLF)	MAX. CSI (LC)	FR-TO		MAX. CSI (LC)		
R-A	-2017 / 0	0.0	0.0	0.36 (1)	5.69	K-I	0 / 2322	0.52 (1)	
A-B	-1493 / 0	-91.8	-91.8	0.65 (1)	4.52	A-Q	0 / 2322	0.52 (1)	
B-C	-2275 / 0	-91.8	-91.8	0.74 (1)	3.71	K-H	-1648 / 0	0.65 (1)	
C-D	-2275 / 0	-91.8	-91.8	0.74 (1)	3.71	Q-B	-1648 / 0	0.65 (1)	
D-E	-2546 / 0	-91.8	-91.8	0.62 (1)	3.75	L-H	0 / 1258	0.28 (1)	
E-F	-2546 / 0	-91.8	-91.8	0.62 (1)	3.75	B-P	0 / 1258	0.28 (1)	
F-G	-2275 / 0	-91.8	-91.8	0.74 (1)	3.71	L-F	-877 / 0	0.35 (1)	
G-H	-2275 / 0	-91.8	-91.8	0.74 (1)	3.71	P-D	-877 / 0	0.35 (1)	
H-I	-1493 / 0	-91.8	-91.8	0.65 (1)	4.52	N-F	0 / 437	0.10 (1)	
I-J	-2017 / 0	0.0	0.0	0.36 (1)	5.69	D-N	0 / 437	0.10 (1)	
						N-E	-575 / 0	0.69 (1)	
R-Q	0 / 0	-18.5	-18.5	0.08 (4)	10.00				
Q-P	0 / 1493	-18.5	-18.5	0.22 (1)	10.00				
P-O	0 / 2275	-18.5	-18.5	0.32 (1)	10.00				
O-N	0 / 2275	-18.5	-18.5	0.32 (1)	10.00				
N-M	0 / 2275	-18.5	-18.5	0.32 (1)	10.00				
M-L	0 / 2275	-18.5	-18.5	0.32 (1)	10.00				
L-K	0 / 1493	-18.5	-18.5	0.22 (1)	10.00				
K-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

TOTAL WEIGHT = 2 X 212 = 425 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.74/1.00 (B-D:1), BC=0.32/1.00 (N-P:1),
WB=0.69/1.00 (E-N:1), SS=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

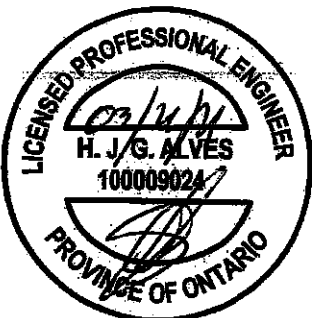
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

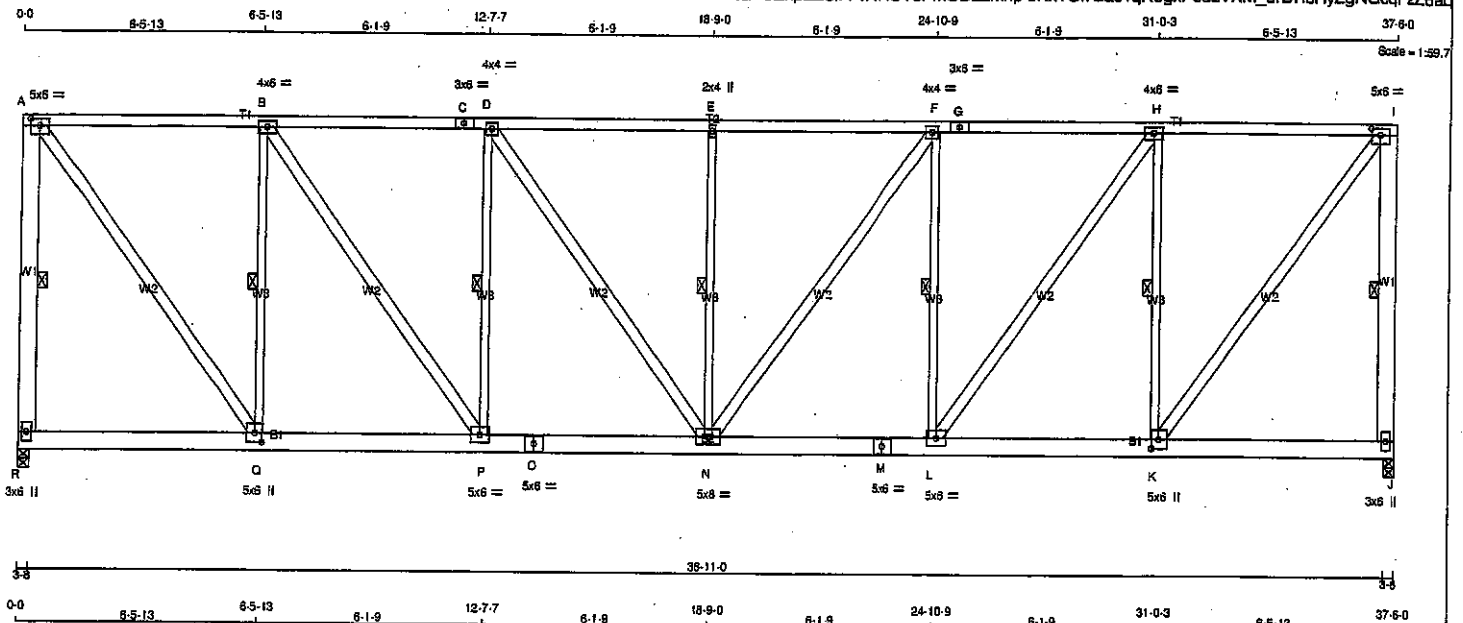
JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.52 (Q) (INPUT = 1.00)



JOB NAME 417422	TRUSS NAME T6	QUANTITY 7	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:GV6EnpZB3lFFw?AU1oPMODzZmnp-oRxYOI7Bae1qRegxP5uzVXM_urDH8HyZgNGcqFzZ6ac



TOTAL WEIGHT = 7 X 243 = 1698 lb

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
R - A	2x6	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
C - G	2x4	DRY	No.2	SPF	SPF
G - I	2x4	DRY	No.2	SPF	SPF
J - I	2x6	DRY	No.2	SPF	SPF
R - O	2x6	DRY	No.2	SPF	SPF
O - M	2x6	DRY	No.2	SPF	SPF
M - J	2x6	DRY	No.2	SPF	SPF
ALL WEBS	2x4	DRY	No.2	SPF	SPF
EXCEPT					
K - H	2x3	DRY	No.2	SPF	SPF
Q - B	2x3	DRY	No.2	SPF	SPF
L - F	2x3	DRY	No.2	SPF	SPF
P - D	2x3	DRY	No.2	SPF	SPF
N - E	2x3	DRY	No.2	SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	3.00
B	TMVW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	6.0		
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0		
I	TMVW-t	MT20	5.0	6.0	2.25	3.00
J	BMV1-p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	3.00	2.25
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	3.00	2.25
R	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	2067	0	2067	0	3-8	3-8
J	2067	0	2067	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
R	1462	959 / 0	0 / 0	0 / 0	503 / 0	0 / 0	0 / 0
J	1462	959 / 0	0 / 0	0 / 0	503 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-R, I-J, H-K, B-Q, F-L, D-P, E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	WEBS	FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LBS)	LC2 (LBS)			MAX. FORCE (LBS)	MAX. CSI (LC)
FR-TO								
R-A	-2018 / 0	0.0	0.0	0.46 (1)	5.89	K-I	0 / 2228	0.38 (1)
A-B	-1339 / 0	-91.8	-91.8	0.63 (1)	4.72	A-Q	0 / 2228	0.36 (1)
B-C	-2039 / 0	-91.8	-91.8	0.71 (1)	3.91	K-H	-1646 / 0	0.83 (1)
C-D	-2039 / 0	-91.8	-91.8	0.71 (1)	3.91	Q-B	-1646 / 0	0.83 (1)
D-E	-2283 / 0	-91.8	-91.8	0.58 (1)	3.96	L-H	0 / 1209	0.19 (1)
E-F	-2283 / 0	-91.8	-91.8	0.58 (1)	3.96	B-P	0 / 1209	0.19 (1)
F-G	-2039 / 0	-91.8	-91.8	0.71 (1)	3.91	L-F	-877 / 0	0.44 (1)
G-H	-2039 / 0	-91.8	-91.8	0.71 (1)	3.91	P-D	-877 / 0	0.44 (1)
H-I	-1339 / 0	-91.8	-91.8	0.63 (1)	4.72	N-F	0 / 420	0.07 (1)
I-J	-2018 / 0	0.0	0.0	0.46 (1)	5.89	D-N	0 / 420	0.07 (1)
						N-E	-575 / 0	0.29 (1)
R-Q	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
Q-P	0 / 1339	-18.5	-18.5	0.21 (1)	10.00			
P-O	0 / 2039	-18.5	-18.5	0.28 (1)	10.00			
O-N	0 / 2039	-18.5	-18.5	0.28 (1)	10.00			
N-M	0 / 2039	-18.5	-18.5	0.28 (1)	10.00			
M-L	0 / 2039	-18.5	-18.5	0.28 (1)	10.00			
L-K	0 / 1339	-18.5	-18.5	0.21 (1)	10.00			
K-J	0 / 0	-18.5	-18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.25")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.25")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.71/1.00 (B-D:1), BC=0.28/1.00 (N-P:1), WB=0.83/1.00 (H-K:1), SS=0.28/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (I) (INPUT = 0.90)
JSI METAL = 0.48 (Q) (INPUT = 1.00)

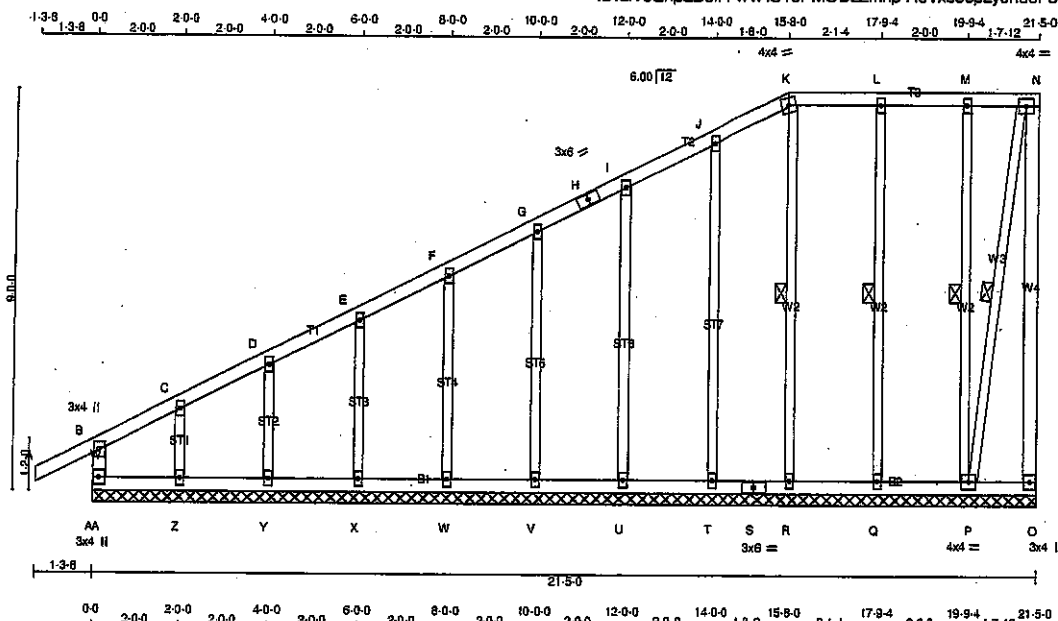


Structural component only
DWG# T-2108092

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417422	T7G	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 08:17:14 2021 Page 1
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TOTAL WEIGHT = 4 X 117 = 468 lb [M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
AA- B	2x4	DRY	No.2	SPF	
A - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
K - N	2x4	DRY	No.2	SPF	
O - N	2x4	DRY	No.2	SPF	
AA- S	2x4	DRY	No.2	SPF	
S - O	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I, J, L, M					
C TMW+w	MT20	2.0	4.0		
H TS-t	MT20	3.0	6.0		
K TTW-m	MT20	4.0	4.0		
N TMVW-t	MT20	4.0	4.0		
O BMV1+p	MT20	3.0	4.0		
P BMW1-t	MT20	4.0	4.0		
Q, R, T, U, V, W, X, Y, Z					
Q BMV1+w	MT20	2.0	4.0		
S BS-t	MT20	3.0	6.0		
AA BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

PROVIDE ANCHORAGE AT BEARING JOINT O FOR 150 LBS. FACTORED UPLIFT
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1:2 LENGTH OF K-R, L-Q, M-P, N-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
AA-B	-198 / 0	0.0 0.0	0.02 (1)	Z-C	-197 / 0	0.03 (1)	
A-B	0 / 28	-91.8 -91.8	0.12 (1)	Y-D	-179 / 0	0.03 (1)	
B-C	0 / 8	-91.8 -91.8	0.05 (1)	X-E	-182 / 0	0.05 (1)	
C-D	0 / 7	-91.8 -91.8	0.05 (1)	W-F	-182 / 0	0.07 (1)	
D-E	0 / 13	-91.8 -91.8	0.04 (1)	V-G	-182 / 0	0.11 (1)	
E-F	0 / 16	-91.8 -91.8	0.04 (1)	U-I	-182 / 0	0.16 (1)	
F-G	0 / 19	-91.8 -91.8	0.04 (1)	T-J	-188 / 0	0.25 (1)	
G-H	0 / 21	-91.8 -91.8	0.05 (1)	R-K	-149 / 0	0.08 (1)	
H-I	0 / 21	-91.8 -91.8	0.05 (1)	Q-L	-221 / 0	0.12 (1)	
I-J	0 / 23	-91.8 -91.8	0.05 (1)	P-M	-165 / 0	0.09 (1)	
J-K	0 / 17	-91.8 -91.8	0.04 (1)	P-N	-167 / 0	0.09 (1)	
K-L	0 / 24	-91.8 -91.8	0.06 (1)				
L-M	0 / 24	-91.8 -91.8	0.06 (1)				
M-N	0 / 28	-91.8 -91.8	0.04 (1)				
O-N	0 / 99	0.0 0.0	0.02 (1)				
AA-Z	0 / 0	-18.5 -18.5	0.02 (4)				
Z-Y	-7 / 0	-18.5 -18.5	0.02 (4)				
Y-X	-11 / 0	-18.5 -18.5	0.01 (4)				
X-W	-14 / 0	-18.5 -18.5	0.01 (4)				
W-V	-17 / 0	-18.5 -18.5	0.01 (4)				
V-U	-19 / 0	-18.5 -18.5	0.01 (4)				
U-T	-21 / 0	-18.5 -18.5	0.01 (4)				
T-S	-22 / 0	-18.5 -18.5	0.01 (4)				
S-R	-22 / 0	-18.5 -18.5	0.01 (4)				
R-Q	-24 / 0	-18.5 -18.5	0.02 (4)				
Q-P	-24 / 0	-18.5 -18.5	0.02 (4)				
P-O	0 / 0	-18.5 -18.5	0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (A-B-1), BC=0.02/1.00 (Y-Z-4), WB=0.25/1.00 (J-T-1), SSI=0.09/1.00 (K-L-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

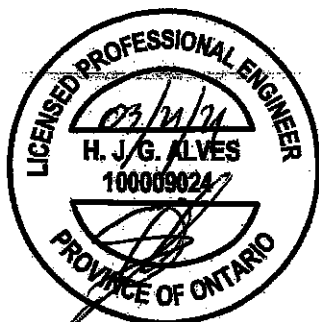
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.59 (K) (INPUT = 0.90)
JSI METAL = 0.08 (C) (INPUT = 1.00)



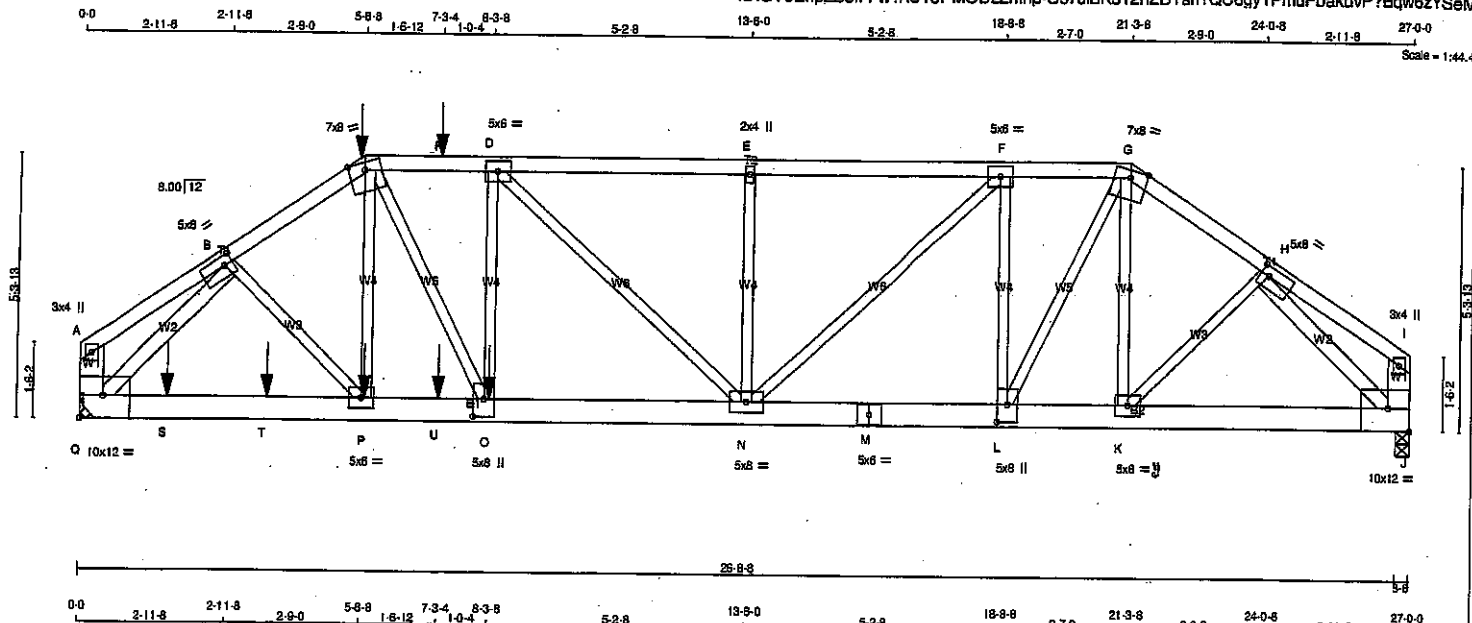
Structural component only
DWG# T-2108093

JOB NAME 417422	TRUSS NAME T8	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:44.4



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	No.2
A - C	2x4	DRY	No.2	SPF	No.2
C - G	2x4	DRY	No.2	SPF	No.2
G - I	2x4	DRY	No.2	SPF	No.2
Q - A	2x6	DRY	No.2	SPF	No.2
J - I	2x6	DRY	No.2	SPF	No.2
Q - M	2x6	DRY	No.2	SPF	No.2
M - J	2x6	DRY	No.2	SPF	No.2
ALL WEBS	2x3	DRY	No.2	SPF	No.2
EXCEPT					
Q - B	2x4	DRY	No.2	SPF	No.2
H - J	2x4	DRY	No.2	SPF	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	JT	GROSS REACTION	BRG	BRG	BRG	BRG
Q	3808	Q	3808	0	0	0	0
J	2322	J	2322	0	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT Q. MINIMUM BEARING LENGTH AT JOINT Q = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2550	1881 / 0	0 / 0	0 / 0	0 / 0	889 / 0	0 / 0
J	1640	1087 / 0	0 / 0	0 / 0	0 / 0	554 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. (PLF)	MAX. (LC)	MEMB.	FORCE (LBS)
FR-TO	0 / 10	FROM TO	LENGTH	FR-TO	0 / 10
A-B	-4592 / 0	-91.8 -91.8 0.10 (1)	10.00	P-C	-288 / 44
B-C	-4886 / 0	-91.8 -91.8 0.40 (1)	2.95	K-G	-182 / 0
C-R	-4886 / 0	-91.8 -91.8 0.55 (1)	2.64	O-D	0 / 74
R-D	-4886 / 0	-91.8 -91.8 0.55 (1)	2.64	C-O	0 / 2346
D-E	-4350 / 0	-91.8 -91.8 0.74 (1)	2.75	L-F	-1512 / 0
E-F	-4350 / 0	-91.8 -91.8 0.74 (1)	2.75	L-G	0 / 1765
F-G	-3158 / 0	-91.8 -91.8 0.35 (1)	3.57	N-F	0 / 1648
G-H	-2833 / 0	-91.8 -91.8 0.21 (1)	3.92	D-N	-739 / 0
H-I	0 / 13	-91.8 -91.8 0.10 (1)	10.00	N-E	-504 / 0
Q-A	-112 / 0	0.0 0.0 0.01 (1)	7.81	B-P	0 / 801
J-I	-107 / 0	0.0 0.0 0.01 (1)	7.81	Q-B	-4713 / 0
Q-S	0 / 3240	-18.5 -18.5 0.50 (1)	10.00	K-H	0 / 407
S-T	0 / 3240	-18.5 -18.5 0.50 (1)	10.00	H-J	-2889 / 0
T-P	0 / 3240	-18.5 -18.5 0.50 (1)	10.00		
P-U	0 / 3797	-18.5 -18.5 0.80 (1)	10.00		
U-O	0 / 3797	-18.5 -18.5 0.80 (1)	10.00		
O-N	0 / 4887	-18.5 -18.5 0.75 (1)	10.00		
N-M	0 / 3157	-18.5 -18.5 0.46 (1)	10.00		
M-L	0 / 3157	-18.5 -18.5 0.46 (1)	10.00		
L-K	0 / 2337	-18.5 -18.5 0.34 (1)	10.00		
K-J	0 / 2054	-18.5 -18.5 0.31 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	5-8-8	-341	-341	---	BACK	VERT	TOTAL	C1
O	8-3-8	-1507	-1507	---	BACK	VERT	TOTAL	C1
P	5-8-4	-29	-29	---	BACK	VERT	TOTAL	C1
R	7-3-4	-122	-122	---	BACK	VERT	TOTAL	C1
S	1-9-4	-29	-29	---	BACK	VERT	TOTAL	C1
T	3-9-4	-29	-29	---	BACK	VERT	TOTAL	C1
U	7-3-4	-29	-29	---	BACK	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.74/1.00 (E-F-1), BC=0.75/1.00 (N-O-1),
WB=0.92/1.00 (B-C-1), SSI=0.24/1.00 (O-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

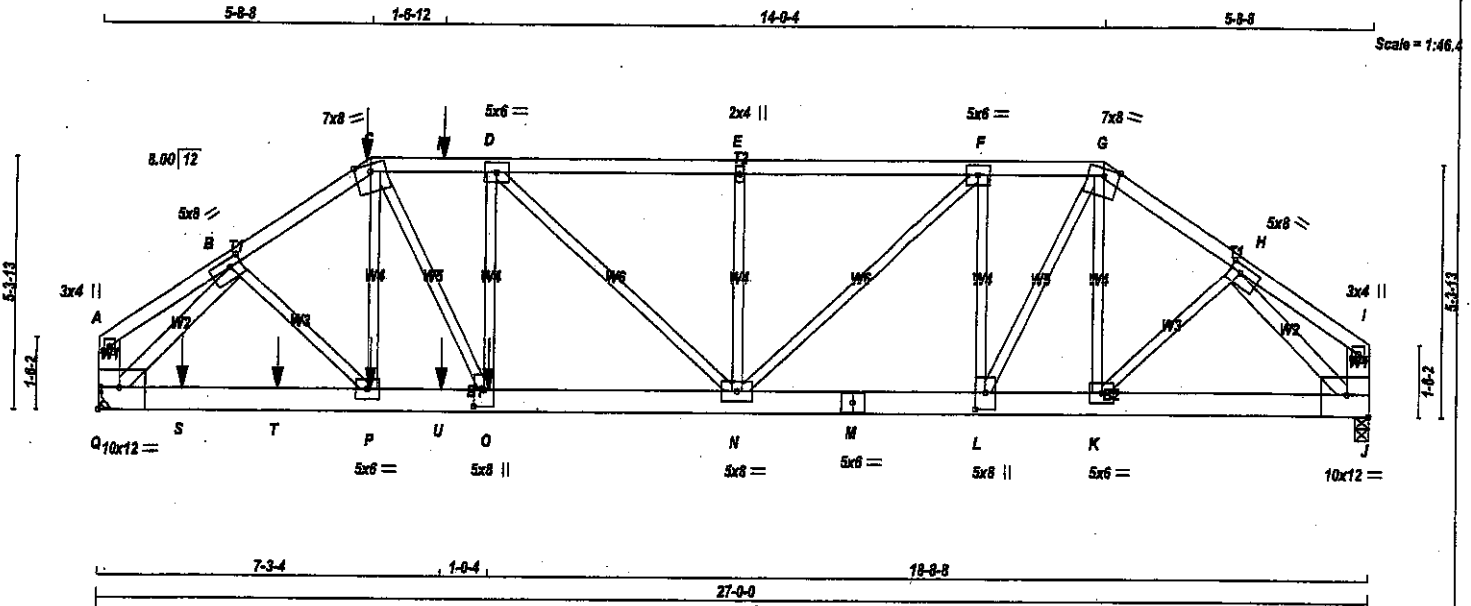
JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.95 (M) (INPUT = 1.00)



Structural component only
DWG# T-2108094

JOB NAME 421219	TRUSS NAME T8Z	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 138 lb [M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
Q - A	2x6	DRY	No.2	SPF	
J - I	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV-p	MT20	3.0	4.0		
B	TMVW-l	MT20	5.0	8.0	2.00	3.00
C	TTWW-m	MT20	7.0	8.0	Edge	
D	TMVW-l	MT20	5.0	6.0		
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-l	MT20	5.0	6.0		
G	TTWW-m	MT20	7.0	8.0	Edge	
H	TMVW-l	MT20	5.0	8.0	2.00	3.00
I	TMV-p	MT20	3.0	4.0		
J	BMVW-l	MT20	10.0	12.0	5.50	Edge
K	BMVW-l	MT20	5.0	6.0		
L	BMVW-l	MT20	5.0	8.0	4.25	2.50
M	BS-l	MT20	5.0	6.0		
N	BMVW-w	MT20	5.0	8.0		
O	BMVW-l	MT20	5.0	8.0	4.25	2.50
P	BMVW-l	MT20	5.0	6.0		
Q	BMVW-l	MT20	10.0	12.0	5.50	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	BRG	BRG
J	2334	0	2334	0
Q	3635	0	3635	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT Q. MINIMUM BEARING LENGTH AT JOINT Q = 3'-8".

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	1849	1093 / 0	0 / 0	0 / 0	0 / 0	598 / 0	0 / 0
Q	2569	1695 / 0	0 / 0	0 / 0	0 / 0	874 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 11	-91.8 -91.8 0.09 (1)	P-C	-299 / 42
B-C	-4630 / 0	-91.8 -91.8 0.42 (1)	K-G	-180 / 0
C-R	-4931 / 0	-91.8 -91.8 0.56 (1)	C-D	0 / 77
R-D	-4931 / 0	-91.8 -91.8 0.56 (1)	C-O	0 / 2374
D-E	-4383 / 0	-91.8 -91.8 0.74 (1)	L-F	-1524 / 0
E-F	-4383 / 0	-91.8 -91.8 0.74 (1)	L-G	0 / 1778
F-G	-3177 / 0	-91.8 -91.8 0.35 (1)	N-F	0 / 1663
G-H	-2850 / 0	-91.8 -91.8 0.23 (1)	D-N	-756 / 0
H-I	0 / 14	-91.8 -91.8 0.10 (1)	N-E	-504 / 0
Q-A	-105 / 0	0.0 0.0 0.01 (1)	B-P	0 / 848
J-I	-100 / 0	0.0 0.0 0.01 (1)	Q-B	-4724 / 0
			K-H	0 / 435
			H-J	-2992 / 0
Q-S	0 / 3219	-18.5 -18.5 0.50 (1)		
S-T	0 / 3219	-18.5 -18.5 0.50 (1)		
T-P	0 / 3219	-18.5 -18.5 0.50 (1)		
P-U	0 / 3829	-18.5 -18.5 0.61 (1)		
U-O	0 / 3829	-18.5 -18.5 0.61 (1)		
O-N	0 / 4932	-18.5 -18.5 0.76 (1)		
N-M	0 / 3177	-18.5 -18.5 0.46 (1)		
M-L	0 / 3177	-18.5 -18.5 0.46 (1)		
L-K	0 / 2351	-18.5 -18.5 0.35 (1)		
K-J	0 / 2039	-18.5 -18.5 0.31 (1)		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-8-8	-341	-341		BACK	VERT	TOTAL		C1
O	8-3-8	-1535	-1535		BACK	VERT	TOTAL		C1
P	5-9-4	-29	-29		BACK	VERT	TOTAL		C1
R	7-3-4	-122	-122		BACK	VERT	TOTAL		C1
S	1-9-4	-29	-29		BACK	VERT	TOTAL		C1
T	3-9-4	-29	-29		BACK	VERT	TOTAL		C1
U	7-3-4	-29	-29		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.74/1.00 (D-E-1), BC=0.78/1.00 (N-O-1),
WB=0.88/1.00 (B-C-1), SSI=0.24/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL = 0.250 inches

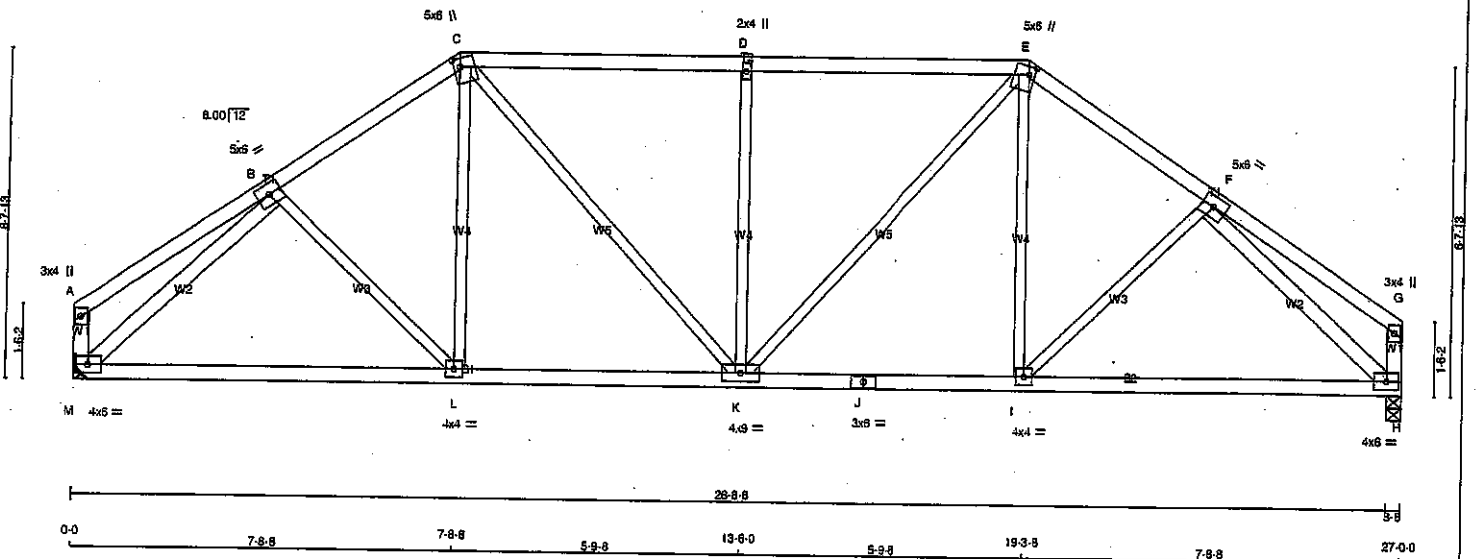
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.96 (M) (INPUT = 1.00)



Structural component only
DWG# T-2129738

JOB NAME 417422	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 08:17:16 2021 Page 1 ID:GV6EnpZB3IFFw?AU1oPMDzZmnp-D0dh0nA4tZQPi6PW4DRg6AzaO3Eilhy?MLUGRazZ6an	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - A	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
M - B	2x4	DRY	No.2
F - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y' X
A	TMV+P	MT20	3.0	4.0
B	TMVW+L	MT20	5.0	6.0
C	TMVW+m	MT20	5.0	6.0 2.00 1.50
D	TMVW-w	MT20	2.0	4.0
E	TMVW+m	MT20	5.0	6.0 2.00 1.50
F	TMVW+L	MT20	5.0	6.0
G	TMV+P	MT20	3.0	4.0
H	BMVW+L	MT20	4.0	6.0
I	BMVW+L	MT20	4.0	4.0
J	BS-L	MT20	3.0	6.0
K	BMVW+L	MT20	4.0	6.0
L	BMVW+L	MT20	4.0	4.0
M	BMVW+L	MT20	4.0	6.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD. BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1488	0	1488	0	0	MECHANICAL	MECHANICAL
H	1488	0	1488	0	0	3-8	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1053	691 / 0	0 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0
H	1053	691 / 0	0 / 0	0 / 0	0 / 0	0 / 0	362 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 22	-91.8 -91.8	0.21 (1)	10.00	B-L	-23 / 45	0.02 (4)
B-C	-1595 / 0	-91.8 -91.8	0.19 (1)	5.05	L-C	0 / 170	0.05 (4)
C-D	-1640 / 0	-91.8 -91.8	0.43 (1)	4.69	C-K	0 / 491	0.11 (1)
D-E	-1640 / 0	-91.8 -91.8	0.43 (1)	4.69	K-D	-651 / 0	0.48 (1)
E-F	-1595 / 0	-91.8 -91.8	0.19 (1)	5.05	K-E	0 / 491	0.11 (1)
F-G	0 / 22	-91.8 -91.8	0.21 (1)	10.00	E-I	0 / 170	0.05 (4)
M-A	-137 / 0	0.0 0.0	0.01 (1)	7.81	I-F	-23 / 45	0.02 (4)
H-G	-137 / 0	0.0 0.0	0.01 (1)	7.81	F-H	-1850 / 0	0.82 (1)
M-L	0 / 1324	-18.5 -18.5	0.35 (4)	10.00			
L-K	0 / 1309	-18.5 -18.5	0.36 (4)	10.00			
K-J	0 / 1309	-18.5 -18.5	0.36 (4)	10.00			
J-I	0 / 1309	-18.5 -18.5	0.36 (4)	10.00			
I-H	0 / 1324	-18.5 -18.5	0.35 (4)	10.00			

TOTAL WEIGHT = 116 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	38.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.43/1.00 (C-D:1), BC=0.36/1.00 (H-K:4), WB=0.82/1.00 (F-H:1), SS=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY) (PSI)	GRIP (WET) (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	850	371	1747	788	1987	1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

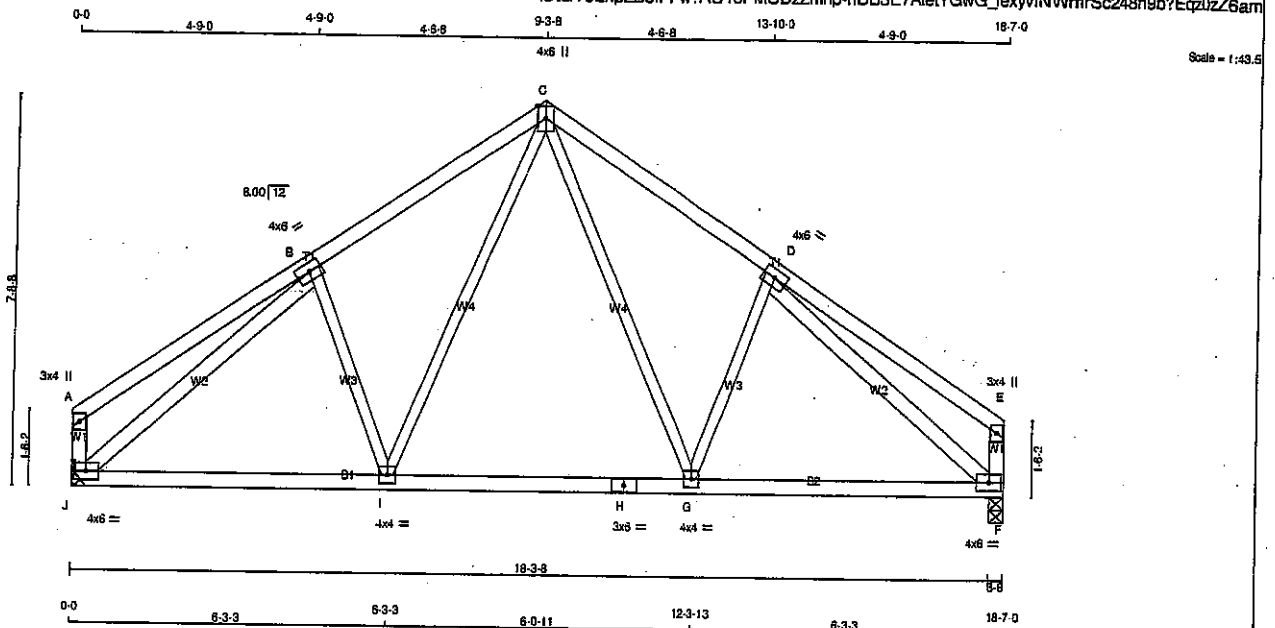
JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.42 (J) (INPUT = 1.00)



Structural component only
DWG# T-2108095

JOB NAME 417422	TRUSS NAME T10A	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 3 X 83 = 249 lb
(M/F)

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED INPUT		RECORD	
	DOWN	HORIZ	BRG	IN-SX
JT	1024	0	0	3-8
F	1024	0	0	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	724	475.0	0.0	0.0	0.0	249.0	0.0
F	724	475.0	0.0	0.0	0.0	249.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.32 (1)	C-G	0 / 390	0.09 (1)	
B-C	-977 / 0	-91.8 -91.8	0.26 (1)	G-D	-255 / 0	0.09 (1)	
C-D	-977 / 0	-91.8 -91.8	0.26 (1)	D-E	0 / 390	0.09 (1)	
D-E	0 / 28	-91.8 -91.8	0.32 (1)	E-F	-255 / 0	0.09 (1)	
J-A	-180 / 0	0.0 0.0	0.02 (1)	F-G	-1202 / 0	0.59 (1)	
F-E	-180 / 0	0.0 0.0	0.02 (1)	G-H	-1202 / 0	0.59 (1)	
J-I	0 / 882	-18.5 -18.5	0.23 (4)	H-I	0 / 882	-18.5 -18.5	0.21 (4)
I-H	0 / 882	-18.5 -18.5	0.21 (4)	H-G	0 / 882	-18.5 -18.5	0.21 (4)
H-G	0 / 882	-18.5 -18.5	0.21 (4)	G-F	0 / 882	-18.5 -18.5	0.23 (4)
G-F	0 / 882	-18.5 -18.5	0.23 (4)				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMVW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108097

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.32/1.00 (D-E-1), BC=0.23/1.00 (F-G-4),
WB=0.59/1.00 (B-J-1), SS=0.17/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

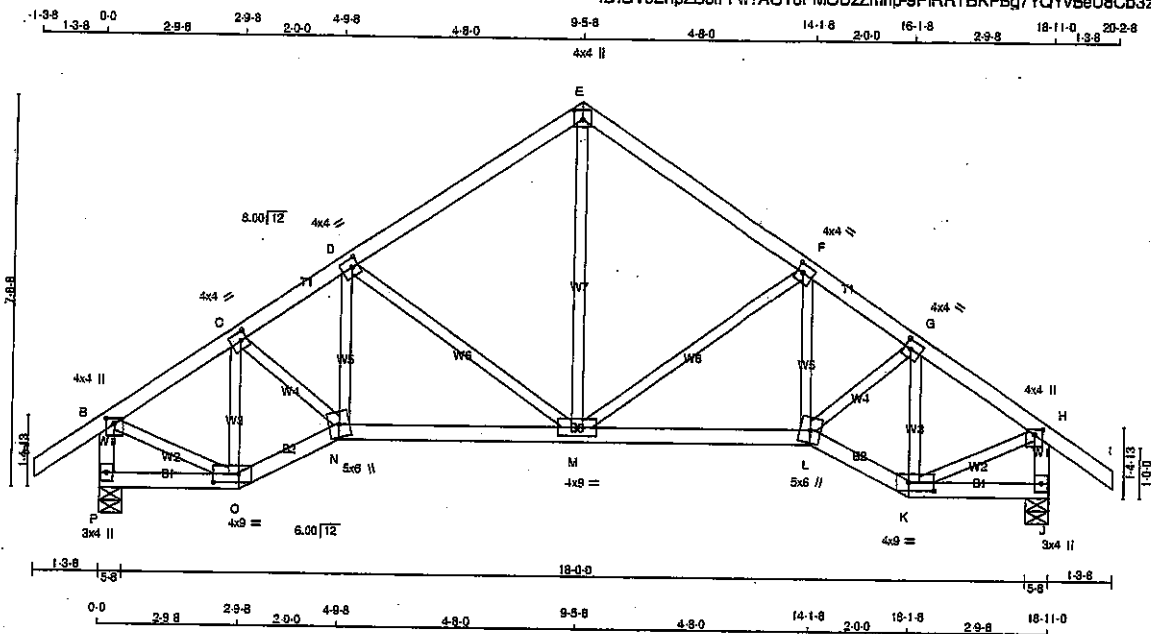
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)

JOB NAME 417422	TRUSS NAME T10S	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mittek Industries, Inc. Sat Mar 20 08:17:18 2021 Page 1	
				ID:GV6EnpZB3IFFw?AU1oPMODzZmnp-9PIRRTBKPg7YQYvBeUaCb3z?sxpgvlfzNVsZz6a	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TTW+p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	BMV1+p	MT20	3.0	4.0		
L	BBWW-p	MT20	4.0	9.0	2.00	6.00
M	BBWW+m	MT20	5.0	6.0		
N	BBWW-t	MT20	4.0	9.0		
O	BBWW+m	MT20	5.0	6.0		
P	BBWW-p	MT20	4.0	9.0	2.00	6.00
Q	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1169	0	1169	0	5-8	5-8
J	1169	0	1169	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	824	554 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
J	824	554 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	M-E	0 / 691	0.16 (1)
B-C	-1027 / 0	-91.8	-91.8 0.10 (1)	6.09	M-F	-498 / 0	0.27 (1)
C-D	-1390 / 0	-91.8	-91.8 0.19 (1)	5.33	L-F	0 / 156	0.04 (1)
D-E	-957 / 0	-91.8	-91.8 0.23 (1)	5.07	L-G	0 / 473	0.11 (1)
E-F	-957 / 0	-91.8	-91.8 0.23 (1)	5.07	K-G	-701 / 0	0.13 (1)
F-G	-1390 / 0	-91.8	-91.8 0.19 (1)	5.33	D-M	-498 / 0	0.27 (1)
G-H	-1027 / 0	-91.8	-91.8 0.10 (1)	6.09	N-D	0 / 156	0.04 (1)
H-I	0 / 35	-91.8	-91.8 0.12 (1)	10.00	C-N	0 / 473	0.11 (1)
P-B	-1143 / 0	0.0	0.0 0.12 (1)	7.43	C-C	-701 / 0	0.13 (1)
J-H	-1143 / 0	0.0	0.0 0.12 (1)	7.43	B-O	0 / 918	0.21 (1)
					K-H	0 / 918	0.21 (1)
P-O	0 / 0	-18.5	-18.5 0.04 (4)	10.00			
O-N	0 / 930	-18.5	-18.5 0.16 (1)	10.00			
N-M	0 / 1182	-18.5	-18.5 0.25 (1)	10.00			
M-L	0 / 1182	-18.5	-18.5 0.25 (1)	10.00			
L-K	0 / 930	-18.5	-18.5 0.16 (1)	10.00			
K-J	0 / 0	-18.5	-18.5 0.04 (4)	10.00			

TOTAL WEIGHT = 3 X 88 = 257 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.23/1.00 (E-F:1), BC=0.25/1.00 (M-N:1), WB=0.27/1.00 (F-M:1), SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

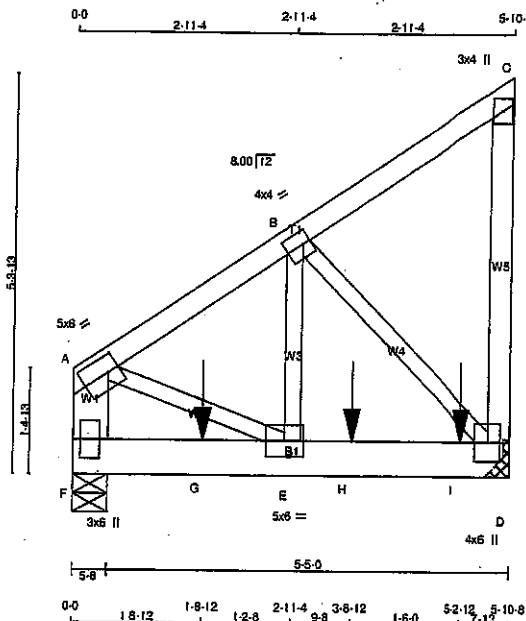
JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.26 (H) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417422	T11	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2\"/>

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - A	2x6	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122\"X3\") SPIRAL NAILS		
A-C 1 12		TOP
C-D 1 12		TOP
F-A 2 12		TOP
BOTTOM CHORDS : (0.122\"X3\") SPIRAL NAILS		
F-D 2 12		SIDE(0.0)
WEBS : (0.122\"X3\") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-t	MT20	5.0	6.0	2.50	1.75
B TMWW-t	MT20	4.0	4.0	2.00	1.25
C TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	DOWN	UP	IN-SX
D 2157 0	2157 0	0 0	0 0
F 1512 0	1512 0	0 0	0 0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE			
D 1522 1015 : 0	0 : 0	0 : 0	0 : 0	507 : 0	0 : 0
F 1067 712 : 0	0 : 0	0 : 0	0 : 0	350 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX. FACTORED	WEBS	MAX. FACTORED	MEMB.	FORCE	MAX
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX. FACTORED	MEMB.	FORCE	MAX			
FR-TO	(LBS)	(PLF)	(LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CSI (LC)			
A-B	-1350 : 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 : 1578	0.20 (1)			
B-C	-15 : 0	-91.8	-91.8	0.06 (1)	6.25	B-D	-1811 : 0	0.23 (1)			
D-C	-108 : 0	0.0	0.0	0.02 (1)	7.81	A-E	0 : 1199	0.15 (1)			
F-A	-1251 : 0	0.0	0.0	0.04 (1)	7.81						
F-G	0 : 0	-18.5	-18.5	0.19 (1)	10.00						
G-E	0 : 0	-18.5	-18.5	0.19 (1)	10.00						
E-H	0 : 1136	-18.5	-18.5	0.29 (1)	10.00						
H-I	0 : 1136	-18.5	-18.5	0.29 (1)	10.00						
I-D	0 : 1136	-18.5	-18.5	0.29 (1)	10.00						

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-8-12	-710	-710	---	BACK	VERT	TOTAL	---	C1
H	3-8-12	-710	-710	---	BACK	VERT	TOTAL	---	C1
I	5-2-12	-712	-712	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 33 = 65 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.06/1.00 (A-B:1), BC=0.29/1.00 (D-E:1), WB=0.23/1.00 (B-D:1), SSI=0.39/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.78 (B) (INPUT = 0.90)
JSI METAL = 0.26 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108099

CONTINUED ON PAGE 2

JOB NAME 417422	TRUSS NAME T11	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:17:19 2021 Page 2 ID:GV6EnpZB3IFFw?AU1oPMODzZmnp-dblqapCyAUo 9Z75IL?NkobAOGGeY8pS2Jix1vzZ6ak	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
D	BMVW1+p	MT20	4.0	6.0	
E	BMVW1-t	MT20	5.0	6.0	
F	BMV1+p	MT20	3.0	6.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

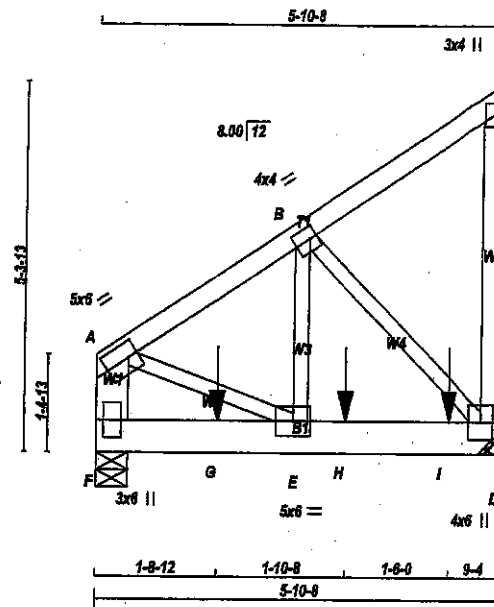


Structural component only
DWG# T-2108099 *2/1*

JOB NAME 421219	TRUSS NAME T1129	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Scale: 3/8"=1'

TOTAL WEIGHT = 2 X 33 = 65 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - A	2x6 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
F - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-t	MT20	5.0	6.0	2.50	1.75
B TMVW-t	MT20	4.0	4.0	2.00	1.25
C TMV-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UP
D	2196	0	2196	0
F	1514	0	1514	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1550	1035 / 0	0 / 0	0 / 0	0 / 0	514 / 0	0 / 0
F	1068	714 / 0	0 / 0	0 / 0	0 / 0	354 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO							
A-B	-1437 / 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 / 1899
B-C	-14 / 0	-91.8	-91.8	0.06 (1)	6.25	B-D	-1713 / 0
D-C	-109 / 0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 1275
F-A	-1324 / 0	0.0	0.0	0.05 (1)	7.81		
F-G	0 / 0	-18.5	-18.5	0.19 (1)	10.00		
G-E	0 / 0	-18.5	-18.5	0.19 (1)	10.00		
E-H	0 / 1208	-18.5	-18.5	0.32 (1)	10.00		
H-I	0 / 1208	-18.5	-18.5	0.32 (1)	10.00		
I-D	0 / 1208	-18.5	-18.5	0.32 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-8-12	-583	-583		BACK	VERT	TOTAL		C1
H	3-7-4	-866	-866		BACK	VERT	TOTAL		C1
I	5-1-4	-711	-711		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.06/1.00 (A-B:1), BC=0.32/1.00 (D-E:1),
WB=0.24/1.00 (B-D:1), SSI=0.47/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

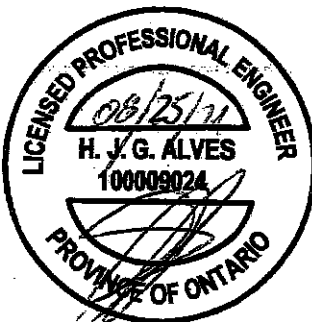
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.84 (B) (INPUT = 0.90)
JSI METAL=0.27 (D) (INPUT = 1.00)



Structural component only
DWG# T-2129739

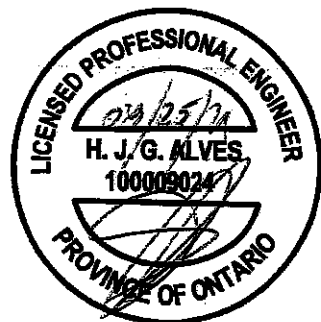
CONTINUED ON PAGE 2

JOB NAME 421219	TRUSS NAME T11Z9	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 5 Jan 21 2021 MITek Industries, Inc. Tue Aug 24 17:17:30 2021 Page 2 ID:GV6EnpZB3IFFw?AU1aPMODzZmnp- aY44M?TDr4re6i3DBcypH7NMF8r4?aDnqZGgvkam		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

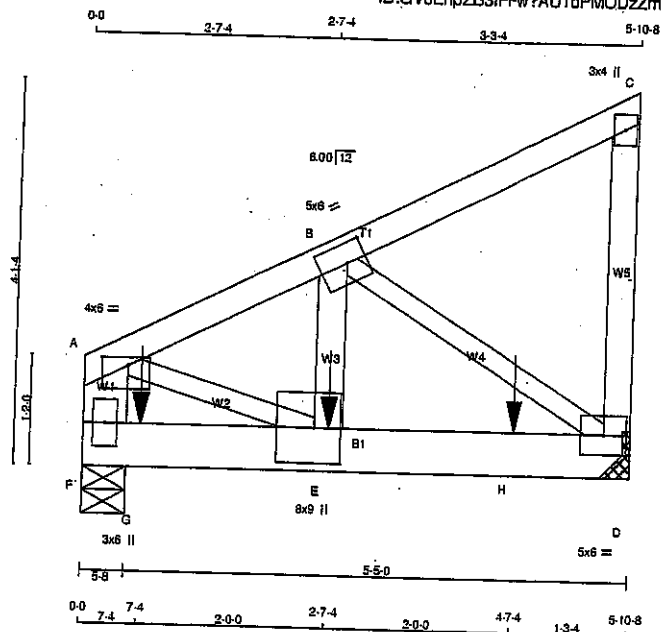
NOTES (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2129739 *SM*

JOB NAME 417422	TRUSS NAME T12	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:17:20 2021 Page 1
 ID:GV6EnpZB3IFFw?AU1oPMODzZmnp-5osCs9DaxowmjiHJ3VWcH08LggXXHIX6bHzSuaLzZ6a



Scale = 1/28.5

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x6	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - B	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
B - E	2	SIDE(753.3)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00 3.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	3097	0	3097	0	0	MECHANICAL	
F	2904	0	2904	0	0	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
D	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	2187	1453 / 0	0 / 0	0 / 0	0 / 0	734 / 0	0 / 0
F	2047	1382 / 0	0 / 0	0 / 0	0 / 0	665 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO		FR-TO			
A-B	-3594 / 0	-91.8 -91.8	0.09 (1)	4.89	E-B	0 / 3366	0.30 (1)
B-C	-3 / 0	-91.8 -91.8	0.08 (1)	10.00	B-D	-3812 / 0	0.48 (1)
D-C	-143 / 0	0.0 0.0	0.02 (1)	7.81	A-E	0 / 3358	0.42 (1)
F-A	-2676 / 0	0.0 0.0	0.09 (1)	7.81			
F-G	0 / 0	-18.5 -18.5	0.05 (1)	10.00			
G-E	0 / 0	-18.5 -18.5	0.05 (1)	10.00			
E-H	0 / 3219	-18.5 -18.5	0.63 (1)	10.00			
H-D	0 / 3219	-18.5 -18.5	0.63 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-7-4	-2535	-2535	---	BACK	VERT	TOTAL	---	C1
G	7-4	-203	-203	---	BACK	VERT	TOTAL	---	C1
H	4-7-4	-1038	-1038	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 59 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, ABC 2019
 - PART 9 OF OBC 2012 (2019 AMENDMENT)
 - CSA 085-14
 - TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.09/1.00 (A-F:1), BC=0.63/1.00 (D-E:1), WB=0.48/1.00 (B-D:1), SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.90)
 JSI METAL= 0.49 (E) (INPUT = 1.00)



Structural component only
 DWG# T-2108100

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417422	T12	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

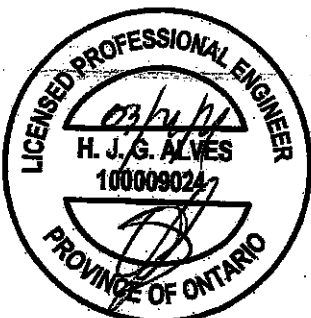
Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Sat Mar 20 08:17:20 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	2.50
C	TMV-p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMVW-t	MT20	8.0	9.0	4.50	3.25
F	BMV1-p	MT20	3.0	6.0		

NOTES- (1)

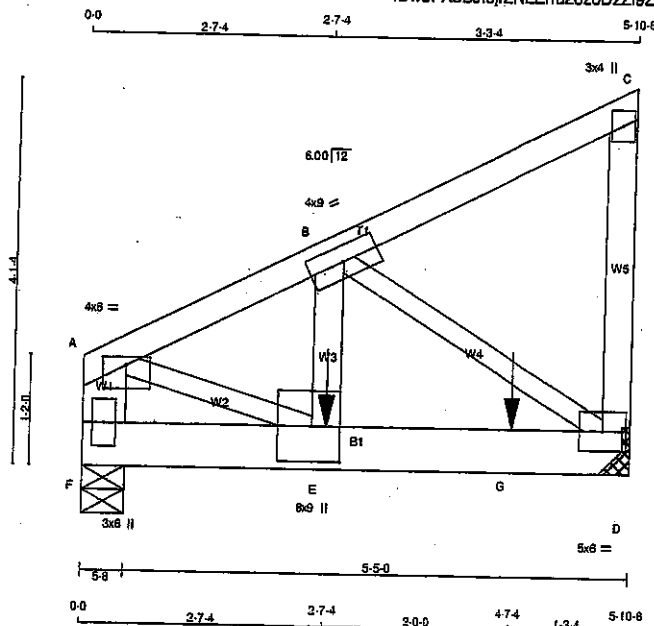
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108100 *m*

JOB NAME 417441	TRUSS NAME T12Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - A	2x6 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
E - B	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
F - A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
B - E 2	2	SIDE(851.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-p	MT20	4.0	6.0	1.00	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
D	2918	0	2918	0
F	2429	0	2429	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOOSE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
D	2060	1372.0	0.0	0.0
F	1715	1142.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO					FR-TO		
A-B	-3255.0	-91.8	-91.8	0.08 (1)	E-B	0.3018	0.27 (1)
B-C	-4.0	-91.8	-91.8	0.08 (1)	B-D	-3455.0	0.44 (1)
D-C	-140.0	0.0	0.0	0.02 (1)	A-E	0.3043	0.38 (1)
F-A	-2433.0	0.0	0.0	0.09 (1)			
F-E	0.0	-18.5	-18.5	0.03 (1)			
E-G	0.2917	-18.5	-18.5	0.02 (1)			
G-D	0.2917	-18.5	-18.5	0.02 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
E	2-7-4	-2256	-2256	---	BACK	VERT	TOTAL	---	C1
G	4-7-4	-1061	-1061	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 23 = 59 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.8	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.09/1.00 (A-F:1), BC=0.62/1.00 (D-E:1), WB=0.44/1.00 (B-D:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

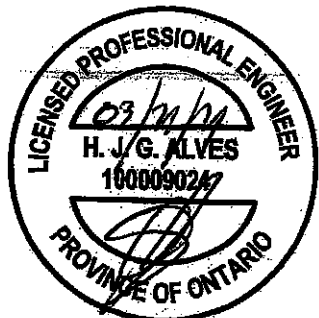
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788
		1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (B) (INPUT = 0.90)

JSI METAL = 0.41 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108108 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	T12Z	1	2	TRUSS DESC.		

Tamrack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	9.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMVW+t	MT20	6.0	9.0	4.50	3.50
F	BMV1+p	MT20	3.0	6.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

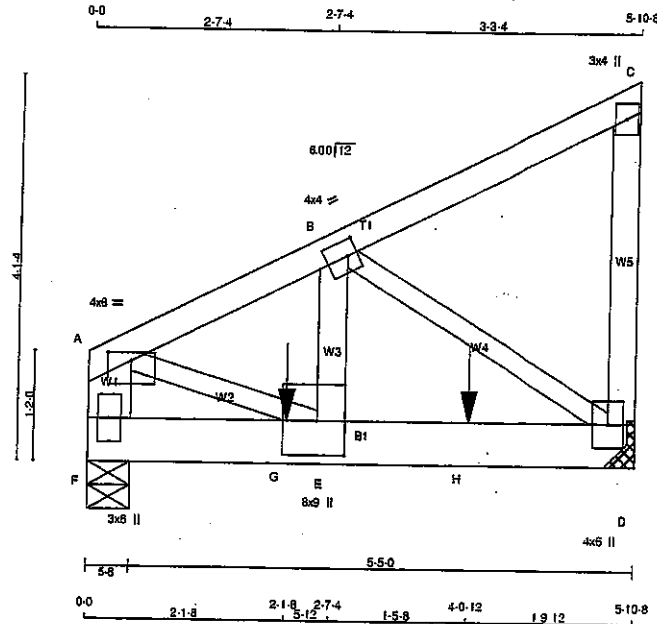


Structural component only
DWG# T-2108108 *W*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	T12Z2	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2\"/>

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122\"X3\") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	TOP
F-A	2	12	TOP
BOTTOM CHORDS: (0.122\"X3\") SPIRAL NAILS			
F-D	2	12	TOP
WEBS: (0.122\"X3\") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV-p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	8.0	9.0	4.50	3.50
F	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
D	1535	0	1535	0	MECHANICAL	5-8
F	1413	0	1413	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	1082	734 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0
F	995	675 / 0	0 / 0	0 / 0	0 / 0	320 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS MAX. FACTORED MEMB. FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	WEBS MAX. FACTORED MEMB. FORCE (LBS)		MAX. CSI (LC)
	FROM	TO	FROM	TO		FROM	TO	
A-B	-1719 / 0		-91.8	-91.8	0.06 (1)	6.25	E-B	0 / 1442
B-C	-9 / 0		-91.8	-91.8	0.07 (1)	10.00	B-D	-1833 / 0
D-C	-130 / 0		0.0	0.0	0.02 (1)	7.81	A-E	0 / 1615
F-A	-1329 / 0		0.0	0.0	0.05 (1)	7.81		
F-G	0 / 0		-18.5	-18.5	0.15 (1)	10.00		
G-E	0 / 0		-18.5	-18.5	0.15 (1)	10.00		
E-H	0 / 1548		-18.5	-18.5	0.37 (1)	10.00		
H-D	0 / 1548		-18.5	-18.5	0.37 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-1-8	-809	-809		TOP	VERT	TOTAL		C1
H	4-0-12	-809	-809		TOP	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 59 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.07/1.00 (B-C:1), BC=0.37/1.00 (D-E:1), WB=0.23/1.00 (B-D:1), SS=0.37/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850	371	1747
		788	1987
		1873	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT= 0.80)
JSI METAL= 0.35 (D) (INPUT= 1.00)

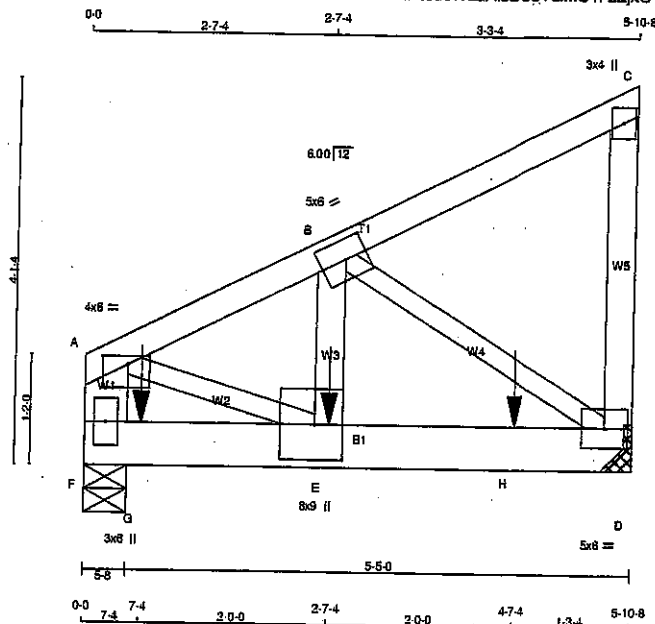


Structural component only
DWG# T-2108109

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T12Z3	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 29 = 59 lb

LUMBER				
N. L. & A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT E - B	2x3	DRY	No.2	SPF
	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C 1	12	TOP
C - D 1	12	TOP
F - A 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE (183.1)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	
B - E 2	2	SIDE (700.6)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-p	MT20	4.0	6.0	1.00	3.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	3064	0	3064	0	MECHANICAL	5-8
D	2855	0	2855	0	5-8	5-8
F	2855	0	2855	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LOOSE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	2164	1436	0	0	0	0	728	0
F	2012	1358	0	0	0	0	654	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED WEBS FORCE (LBS)
A-B	-3488	0	-91.8	0.08 (1)
B-C	-4	0	-91.8	0.08 (1)
D-C	-142	0	0.0	0.02 (1)
F-A	-2800	0	0.0	0.09 (1)
F-G	0	0	-18.5	0.08 (1)
G-E	0	0	-18.5	0.06 (1)
E-H	0	3124	-18.5	0.64 (1)
H-D	0	3124	-18.5	0.64 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-7-4	-2394	-2394	---	BACK	VERT	TOTAL	---	C1
G	7-4	-241	-241	---	TOP	VERT	TOTAL	---	C1
H	4-7-4	-1083	-1083	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.09/1.00 (A-F:1), BC=0.64/1.00 (D-E:1), WB=0.47/1.00 (B-D:1), SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

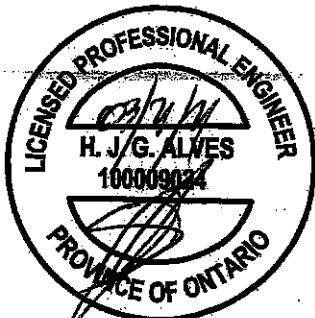
NAIL VALLES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.44 (E) (INPUT = 1.00)



Structural component only
DWG# T-2108135

CONTINUED ON PAGE 2

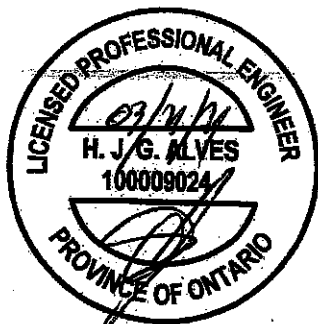
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417443	T12Z3	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW-t	MT20	8.0	9.0	4.50	3.50
F	BMV1+p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

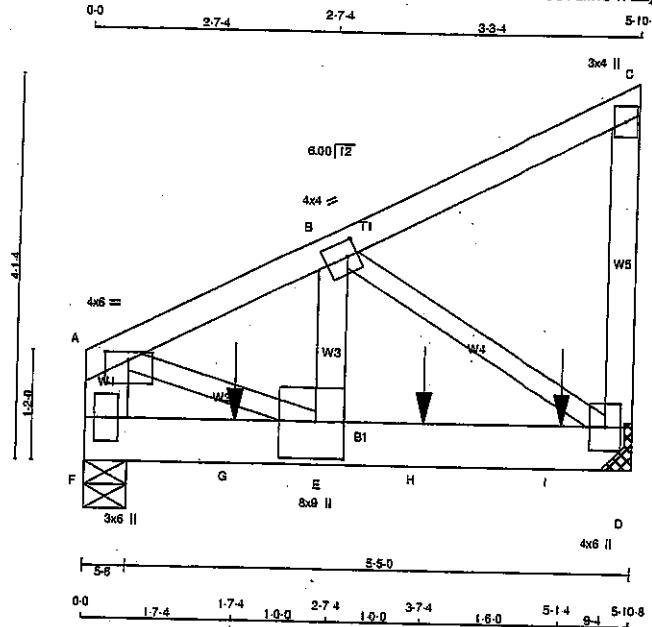


Structural component only
DWG# T-2108135 *m*

JOB NAME 417443	TRUSS NAME T12Z4	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - A	2x6 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS EXCEPT E - B	2x3 DRY	No.2	SPF		
	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	8	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	MTMV-p	MT20	4.0	6.0	1.00 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	2124	0	2124	0	0	MECHANICAL	
F	1599	0	1599	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
D	1499	1000 / 0	0 / 0	0 / 0	499 / 0	0 / 0
F	1129	783 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	FORCE	FACTORED	FACTORED		MEMB.	FORCE	FACTORED		
FR-TO	(LBS)	VERT. LOAD	LG1	MAX	MAX.	FR-TO	(LBS)	MAX	
		FROM	TO	CSI (LC)	UNBRACED LENGTH			CSI (LC)	
A-B	-1819 / 0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 / 1544	0.14 (1)	
B-C	-9 / 0	-91.8	-91.8	0.07 (1)	10.00	B-D	-1938 / 0	0.24 (1)	
D-C	-131 / 0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 1707	0.21 (1)	
F-A	-1400 / 0	0.0	0.0	0.05 (1)	7.81				
F-G	0 / 0	-18.5	-18.5	0.21 (1)	10.00				
G-E	0 / 0	-18.5	-18.5	0.21 (1)	10.00				
E-H	0 / 1636	-18.5	-18.5	0.37 (1)	10.00				
H-I	0 / 1636	-18.5	-18.5	0.37 (1)	10.00				
I-D	0 / 1636	-18.5	-18.5	0.37 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-7-4	-723	-723	---	BACK	VERT	TOTAL	---	C1
H	3-7-4	-723	-723	---	BACK	VERT	TOTAL	---	C1
I	5-1-4	-724	-724	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 29 = 59 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.07/1.00 (B-C:1), BC=0.37/1.00 (D-E:1), WB=0.24/1.00 (B-D:1), SSI=0.38/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSF) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (B) (INPUT = 0.80)
JSI METAL = 0.37 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108136 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T12Z4	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV-p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	8.0	9.0	4.75	3.50
F	BMV1+p	MT20	3.0	6.0		

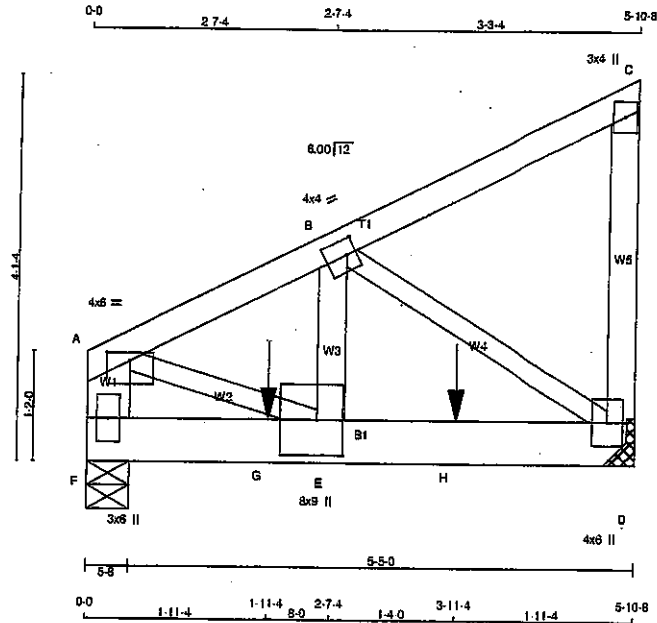
NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108136 *m*

JOB NAME 417443	TRUSS NAME T12Z5	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 08:52:39 2021 Page 1 ID:wa2t6usWauAi5BCJTGMC4FzjxO-ZSPGbislvauAVMCYxb1XisPTNrqWGz_VY8G6S1zZ63c			



TOTAL WEIGHT = 2 X 29 = 59 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - D	12	TOP
F - A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00 3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	1071	0	1071	0	0	MECHANICAL
F	822	0	822	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	DOWN			
D	754	514	0	0	0	240	0
F	577	401	0	0	0	178	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. CSI (LC)	UNBRAC LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)		MAX. CSI (LC)
	FR-TO	LOC	FROM	TO				FR-TO	LOC	
A-B	-1044	0	-91.8	-91.8	0.06 (1)	6.25	E-B	0 / 749	0.07 (1)	
B-C	-11	0	-91.8	-91.8	0.06 (1)	6.25	B-D	-1121	0	0.14 (1)
D-C	-126	0	0.0	0.0	0.02 (1)	7.81	A-E	0 / 987	0.12 (1)	
F-A	-843	0	0.0	0.0	0.03 (1)	7.81				
F-G	0 / 0		-18.5	-18.5	0.11 (1)	10.00				
G-E	0 / 0		-18.5	-18.5	0.11 (1)	10.00				
E-H	0 / 946		-18.5	-18.5	0.29 (1)	10.00				
H-D	0 / 946		-18.5	-18.5	0.29 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-176	-176		FRONT	VERT	TOTAL		C1
H	3-11-4	-697	-697		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	39.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.06/1.00 (B-C:1), BC=0.29/1.00 (D-E:1), WB=0.14/1.00 (B-D:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650 371 1747 788 1987 1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.50 (D) (INPUT = 0.90)

JSI METAL=0.21 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108137

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417443	T12Z5	1	2	GREENPARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

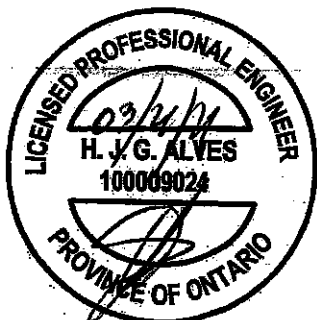
Version 8.420 S Jan 21 2021 MTEK Industries, Inc. Sat Mar 20 08:52:39 2021 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW-t	MT20	8.0	9.0	4.50	3.25
F	BMV1+p	MT20	3.0	6.0		

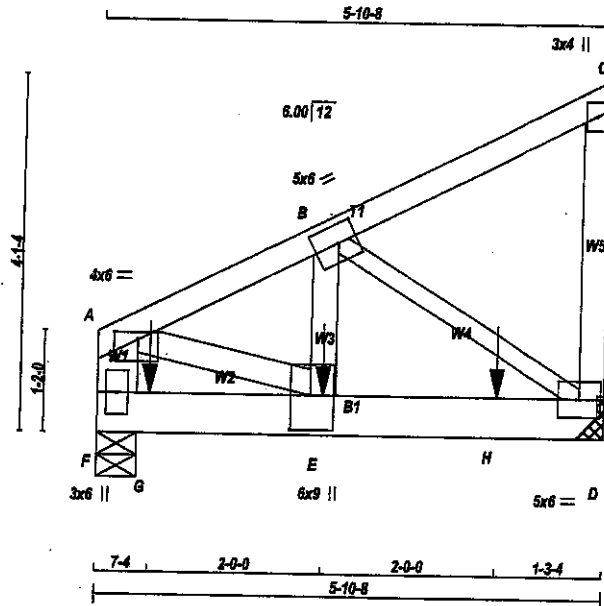
NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2108137 *ml*

JOB NAME 421219	TRUSS NAME T12Z9	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Tue Aug 24 17:30 2021 Page 1			
		ID:GV6EnpZB3IFFw?AU1oPMODzZmnp_aY44M7TDrdre6i3DBcygiH7uMBLr08aDngZGgykam			



Scale = 1:25.4

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
B - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	1 12	TOP
C-D	1 12	TOP
F-A	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	2 12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1 6	
B-E	2 2	SIDE (788.9)
2x4	1 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	BRG	BRG
JT	VERT	HORIZ	DOWN	HORIZ	IN-SX	IN-SX
D	3124	0	3124	0	0	0
F	2938	0	2938	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
JT	2208	1466 / 0	0 / 0	0 / 0	0 / 0	738 / 0	0 / 0
D	2070	1388 / 0	0 / 0	0 / 0	0 / 0	672 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNGRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-3653 / 0	-91.8 -91.8 0.09 (1)		4.86	E-B	0 / 3423	0.30 (1)
B-C	-4 / 0	-91.8 -91.8 0.08 (1)		10.00	B-D	-3878 / 0	0.49 (1)
D-C	-141 / 0	0.0 0.0 0.02 (1)		7.81	A-E	0 / 3414	0.30 (1)
F-A	-2717 / 0	0.0 0.0 0.10 (1)		7.81			
F-G	0 / 0	-18.5 -18.5 0.05 (1)		10.00			
G-E	0 / 0	-18.5 -18.5 0.05 (1)		10.00			
E-H	0 / 3272	-18.5 -18.5 0.83 (1)		10.00			
H-D	0 / 3272	-18.5 -18.5 0.83 (1)		10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-7-4	-2578	-2578	—	BACK	VERT	TOTAL	—	C1
G	7-4	-203	-203	—	BACK	VERT	TOTAL	—	C1
H	4-7-4	-1038	-1038	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 2 X 30 = 60 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2018
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/899 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.10/1.00 (A-F:1), BC=0.63/1.00 (D-E:1), WB=0.49/1.00 (B-D:1), SS=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1997	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.53 (E) (INPUT = 1.00)



Structural component only
DWG# T-2129740

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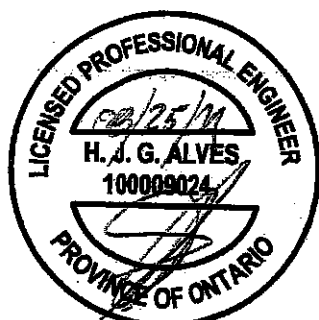
JOB NAME 421219	TRUSS NAME T12Z9	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 S Jan 21 2021 MITak Industries, Inc. Tue Aug 24 17:17:30 2021 Page 2
 ID:GV6EnpZB3lFFw7AU1cPMODzZmnp- aY44M71Dr4re6i3DBcvgIH7uMBLr08aDngZGovkam

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.25
B	TMVW-t	MT20	5.0	6.0	2.50	2.50
C	TMV-p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMVW-t	MT20	6.0	9.0	4.75	2.75
F	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
 1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
 DWG# T-2129740 *200*

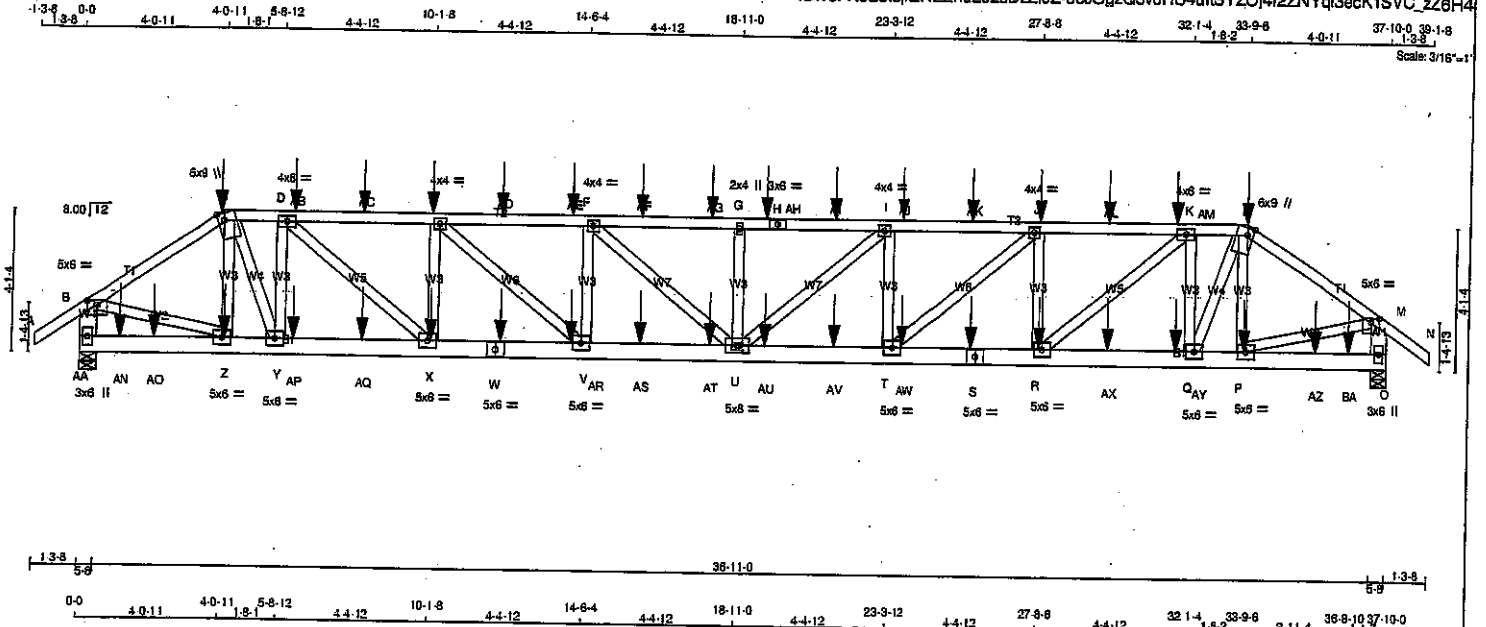
JOB NAME 417441	TRUSS NAME T15	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:v5FX5Bj0jrZNLenu26zoDzI9Z-ocJ0gzQ3v0HU4ultSYZOj4i2ZNYqI3ecK1SVC_z26H4

Scale: 3/16"=1'



CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
L - N	2x4	DRY	No.2	SPF
AA - B	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
AA - W	2x6	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
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DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(61.0)
C - H 1	12	SIDE(61.0)
H - L 1	12	SIDE(61.0)
L - N 1	12	SIDE(61.0)
AA - B 2	12	TOP
O - M 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
AA - W 2	12	SIDE(183.1)
W - S 2	12	SIDE(0.0)
S - O 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

MEMB.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT						
AA	3680	0	3680	0	5-8	5-8
O	3680	0	3680	0	5-8	5-8

UNFACTORED REACTIONS

MEMB.	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
JT							
AA	2603	1706 / 0	0 / 0	0 / 0	0 / 0	897 / 0	0 / 0
O	2603	1706 / 0	0 / 0	0 / 0	0 / 0	897 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED				FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 35	-91.8	-91.8	0.07 (1)	10.00	Z-C	-728 / 0	0.07 (1)
B-C	-4361 / 0	-91.8	-91.8	0.23 (1)	4.38	P-L	-728 / 0	0.07 (1)
C-D	-4746 / 0	-91.8	-91.8	0.20 (1)	4.25	B-Z	0 / 3740	0.33 (1)
D-AB	-7106 / 0	-91.8	-91.8	0.41 (1)	3.41	P-M	0 / 3740	0.33 (1)
AB-AC	-7106 / 0	-91.8	-91.8	0.41 (1)	3.41	Y-D	-2450 / 0	0.22 (1)
AC-E	-7106 / 0	-91.8	-91.8	0.41 (1)	3.41	Q-K	-2450 / 0	0.22 (1)
E-AD	-8495 / 0	-91.8	-91.8	0.51 (1)	3.05	R-K	0 / 3082	0.27 (1)
AD-AE	-8495 / 0	-91.8	-91.8	0.51 (1)	3.05	D-X	0 / 3082	0.27 (1)
AE-F	-8495 / 0	-91.8	-91.8	0.51 (1)	3.05	R-J	-1851 / 0	0.17 (1)
F-AF	-8943 / 0	-91.8	-91.8	0.54 (1)	2.95	X-E	-1851 / 0	0.17 (1)
AF-AG	-8943 / 0	-91.8	-91.8	0.54 (1)	2.95	T-J	0 / 1821	0.18 (1)
AG-G	-8943 / 0	-91.8	-91.8	0.54 (1)	2.95	E-V	0 / 1821	0.18 (1)
G-AH	-8943 / 0	-91.8	-91.8	0.54 (1)	2.95	T-I	-1024 / 0	0.09 (1)
AH-H	-8943 / 0	-91.8	-91.8	0.54 (1)	2.95	V-F	-1024 / 0	0.09 (1)
H-AI	-8943 / 0	-91.8	-91.8	0.54 (1)	2.95	U-I	0 / 590	0.05 (1)
AI-I	-8943 / 0	-91.8	-91.8	0.54 (1)	2.95	F-U	0 / 590	0.05 (1)
I-AJ	-8495 / 0	-91.8	-91.8	0.51 (1)	3.05	U-G	-657 / 0	0.08 (1)
AJ-AK	-8495 / 0	-91.8	-91.8	0.51 (1)	3.05	C-Y	0 / 2766	0.24 (1)
AK-J	-8495 / 0	-91.8	-91.8	0.51 (1)	3.05	Q-L	0 / 2766	0.24 (1)
J-L	-7106 / 0	-91.8	-91.8	0.41 (1)	3.41			
AL-AM	-7106 / 0	-91.8	-91.8	0.41 (1)	3.41			
AM-K	-7106 / 0	-91.8	-91.8	0.41 (1)	3.41			
K-L	-4746 / 0	-91.8	-91.8	0.20 (1)	4.25			
L-M	-4361 / 0	-91.8	-91.8	0.23 (1)	4.38			
M-N	0 / 35	-91.8	-91.8	0.07 (1)	10.00			
AA-B	-3615 / 0	0.0	0.0	0.13 (1)	7.39			
OO-M	-3615 / 0	0.0	0.0	0.13 (1)	7.39			

AA-AN	0 / 0	-18.5	-18.5	0.04 (4)	10.00
AN-AO	0 / 0	-18.5	-18.5	0.04 (4)	10.00
AO-Z	0 / 0	-18.5	-18.5	0.04 (4)	10.00
Z-Y	0 / 3610	-18.5	-18.5	0.24 (1)	10.00
Y-AP	0 / 4747	-18.5	-18.5	0.35 (1)	10.00
AP-AQ	0 / 4747	-18.5	-18.5	0.35 (1)	10.00
AQ-X	0 / 4747	-18.5	-18.5	0.35 (1)	10.00
X-W	0 / 7106	-18.5	-18.5	0.51 (1)	10.00
W-AR	0 / 7106	-18.5	-18.5	0.51 (1)	10.00
AR-V	0 / 7106	-18.5	-18.5	0.51 (1)	10.00
V-AS	0 / 8495	-18.5	-18.5	0.62 (1)	10.00
AS-AT	0 / 8495	-18.5	-18.5	0.62 (1)	10.00
AT-U	0 / 8495	-18.5	-18.5	0.62 (1)	10.00
U-AU	0 / 8495	-18.5	-18.5	0.62 (1)	10.00

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 25.6	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN/LAND) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.26")
CALCULATED VERT. DEFL. (LL) = L/599 (0.22")
ALLOWABLE DEFL. (TL) = L/360 (1.26")
CALCULATED VERT. DEFL. (TL) = L/855 (0.53")

CSI: TC=0.54/1.00 (F-G:1), BC=0.82/1.00 (U-V:1), WB=0.33/1.00 (B-Z:1), SS=0.15/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PL)	SECTION
	(PSI)	(PL)	(MIN)
MT20	650	371	1747 788 1887 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (Z) (INPUT = 0.90)
JSI METAL= 0.66 (W) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2108110

JOB NAME 417441	TRUSS NAME T15	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 08:38:17 2021 Page 2 ID:v5FX5BjicirZNLEnu26zoDzZi9Z-ocJOpzQ3v0HU4uliSYZOj42ZNYgl3ecK1SVC zZ8H4	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMVW-t	MT20	4.0	6.0		
E, F, I, J						
E	TMVW-t	MT20	4.0	4.0		
G	TMVW-w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
K	TMVW-t	MT20	4.0	6.0		
L	TTWW+m	MT20	6.0	9.0	Edge	2.00
M	TMVW-p	MT20	5.0	6.0	1.50	3.00
O	BMV1-p	MT20	3.0	6.0		
P, Q, R, T, V, X, Y, Z						
P	BMVW-t	MT20	5.0	6.0		
S	BS-t	MT20	5.0	6.0		
U	BMVW-t	MT20	5.0	8.0		
W	BS-t	MT20	5.0	6.0		
AA	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
AU-AV	0/8495	-18.5	-18.5	0.62 (1)	10.00		
AV-T	0/8495	-18.5	-18.5	0.62 (1)	10.00		
T-AW	0/7106	-18.5	-18.5	0.51 (1)	10.00		
AW-S	0/7106	-18.5	-18.5	0.51 (1)	10.00		
S-R	0/7106	-18.5	-18.5	0.51 (1)	10.00		
R-AX	0/4747	-18.5	-18.5	0.35 (1)	10.00		
AX-AY	0/4747	-18.5	-18.5	0.35 (1)	10.00		
AY-Q	0/4747	-18.5	-18.5	0.35 (1)	10.00		
Q-P	0/3610	-18.5	-18.5	0.24 (1)	10.00		
P-AZ	0/0	-18.5	-18.5	0.04 (4)	10.00		
AZ-BA	0/0	-18.5	-18.5	0.04 (4)	10.00		
BA-O	0/0	-18.5	-18.5	0.04 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-11	-41	-41	-	FRONT	VERT	DEAD	-	C1
C	4-0-11	-82	-82	-	FRONT	VERT	TOTAL	-	C1
C	4-0-11	-175	-175	-	FRONT	VERT	SNOW	-	C1
E	10-1-7	-78	-78	-	FRONT	VERT	TOTAL	-	C1
J	27-8-10	-76	-76	-	FRONT	VERT	TOTAL	-	C1
L	33-8-6	-41	-41	-	FRONT	VERT	DEAD	-	C1
L	33-8-6	-82	-82	-	FRONT	VERT	TOTAL	-	C1
L	33-8-6	-175	-175	-	FRONT	VERT	SNOW	-	C1
P	33-8-10	-21	-21	-	FRONT	VERT	TOTAL	-	C1
R	27-8-10	-21	-21	-	FRONT	VERT	TOTAL	-	C1
S	25-8-10	-21	-21	-	FRONT	VERT	TOTAL	-	C1
W	12-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
X	10-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
Z	4-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AB	6-1-7	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AC	8-1-7	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AD	12-1-7	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AE	14-1-7	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AF	18-1-7	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AG	18-1-7	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AH	19-8-9	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AI	21-8-10	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AJ	23-8-10	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AK	25-8-10	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AL	29-8-10	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AM	31-8-9	-76	-76	-	FRONT	VERT	TOTAL	-	C1
AN	1-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AO	2-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AP	6-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AQ	8-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AR	14-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AS	16-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AT	18-1-7	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AU	19-8-9	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AV	21-8-10	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AW	23-8-10	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AX	29-8-10	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AY	31-8-9	-21	-21	-	FRONT	VERT	TOTAL	-	C1
AZ	35-8-10	-21	-21	-	FRONT	VERT	TOTAL	-	C1
BA	38-8-10	-21	-21	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

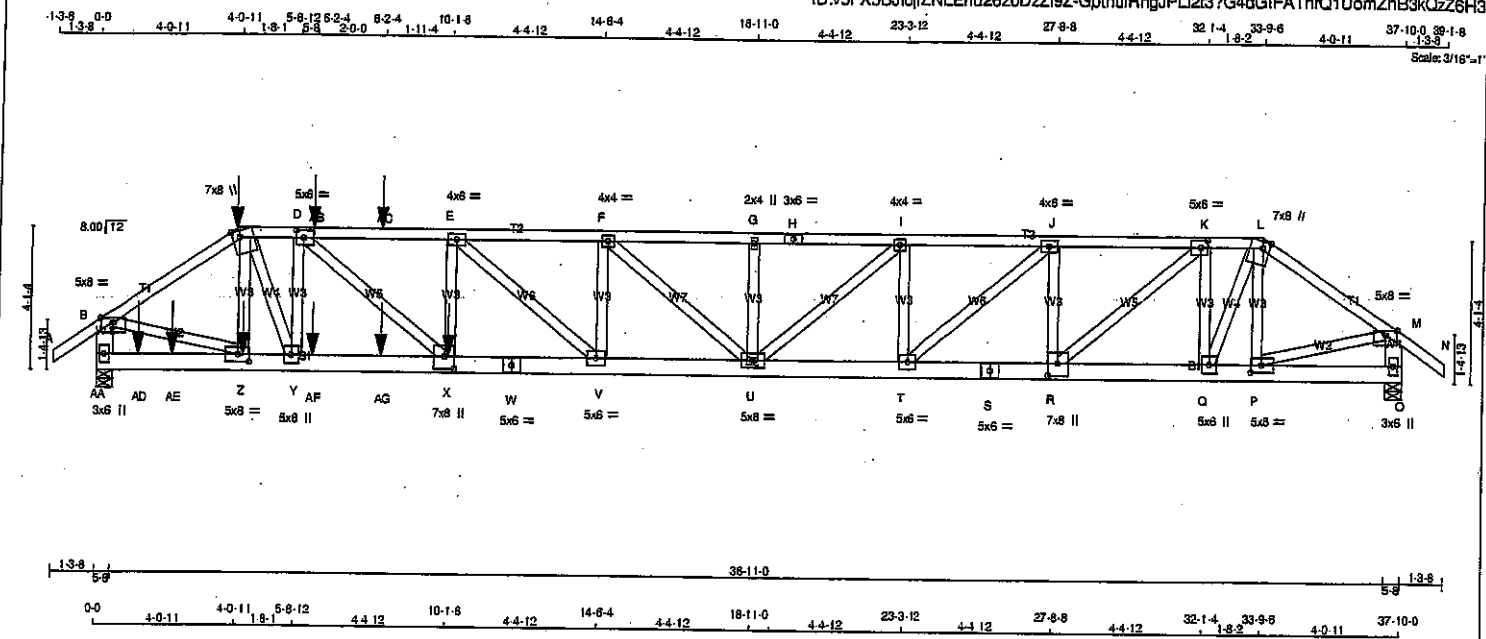
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108110 *2M*

JOB NAME 417441	TRUSS NAME T15Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:v5FX5BJoqrZNLenu26zoDzZ19Z-GptnuIRhgJPLJ2i37G4dGIFATnrQ1UomZhB3kQzZ6H3



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - H	2x4	DRY	No.2
H - L	2x4	DRY	No.2
L - N	2x4	DRY	No.2
AA - B	2x6	DRY	No.2
O - M	2x6	DRY	No.2
AA - W	2x6	DRY	No.2
W - S	2x6	DRY	No.2
S - O	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-H	12	SIDE(61.0)
H-L	12	TOP
L-N	12	TOP
AA-B	12	TOP
O-M	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
AA-W	12	SIDE(183.1)
W-S	12	TOP
S-O	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	SIDE(573.4)
X-E	1	
R-J	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	JT	HORZ	JT	IN-SX	JT	IN-SX
AA	5012	0	5012	0	5-8	5-8	5-8
O	3094	0	3094	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	JT	SNOW
AA	3539	2352	0
O	2184	1458	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 35	Z-C	-1097 / 0
B-C	-6150 / 0	P-L	-635 / 0
C-D	-8905 / 0	B-Z	0 / 5250
D-AB	-10878 / 0	P-M	0 / 3113
AB-AC	-10878 / 0	Y-D	-3720 / 0
AC-E	-10878 / 0	Q-K	-2380 / 0
E-F	-10529 / 0	R-K	0 / 3249
F-G	-9751 / 0	D-X	0 / 5058
G-H	-9751 / 0	R-J	-2020 / 0
H-I	-9751 / 0	X-E	-355 / 0
I-J	-8468 / 0	T-J	0 / 2473
J-K	-6552 / 0	E-V	-196 / 0
K-L	-4094 / 0	T-I	-1498 / 0
L-M	-3648 / 0	V-F	0 / 271
M-N	0 / 35	U-I	0 / 1690
AA-B	-4948 / 0	F-U	-1024 / 0
O-M	-3061 / 0	U-G	-395 / 0
		C-Y	0 / 4235
		Q-L	0 / 2655

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.33")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/758 (0.60")

CSI: TC=0.72/1.00 (D-E:1), BC=0.78/1.00 (V-X:1), WB=0.46/1.00 (B-Z:1), SS=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (Y) (INPUT=0.90)
JSI METAL= 1.00 (W) (INPUT=1.00)



Structural component only
DWG# T-2108111

SPECIFIED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX.	FACE	DIR.	JT	HEEL	JT	CONN.	JT	CONN.
C	4-0-11	-41	-41	FRONT	VERT	DEAD	---	---	C1	---	---
C	4-0-11	-81	-81	BACK	VERT	TOTAL	---	---	C1	---	---
C	4-0-11	-175	-175	FRONT	VERT	SNOW	---	---	C1	---	---
X	10-1-8	-2045	-2045	BACK	VERT	TOTAL	---	---	C1	---	---
Z	4-2-4	-21	-21	BACK	VERT	TOTAL	---	---	C1	---	---

JOB NAME 417441	TRUSS NAME T15Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	2.50
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMVW-t	MT20	5.0	6.0		2.50
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TMVW-w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0		
J	TMVW-t	MT20	4.0	6.0		
K	TMVW-t	MT20	5.0	6.0		2.50
L	TTWW+m	MT20	7.0	8.0	Edge	2.50
M	TMVW-p	MT20	5.0	8.0	Edge	
O	BMVW-t	MT20	3.0	6.0		
P	BMVW-t	MT20	5.0	8.0		2.50
Q	BMVW-t	MT20	5.0	6.0		
R	BMVW-t	MT20	7.0	8.0		4.25
S	BS-t	MT20	5.0	6.0		3.25
T	BMVW-t	MT20	5.0	6.0		
U	BMVW-t	MT20	5.0	8.0		
V	BMVW-t	MT20	5.0	6.0		
W	BS-t	MT20	5.0	6.0		
X	BMVW-t	MT20	7.0	8.0		4.25
Y	BMVW-t	MT20	5.0	6.0		
Z	BMVW-t	MT20	5.0	8.0		2.50
AA	BMVW-t	MT20	3.0	6.0		3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AB	8-2-4	-78	-76	---	BACK	VERT	TOTAL	---	C1
AC	8-2-4	-76	-76	---	BACK	VERT	TOTAL	---	C1
AD	1-2-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AE	2-2-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AF	6-2-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
AG	8-2-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

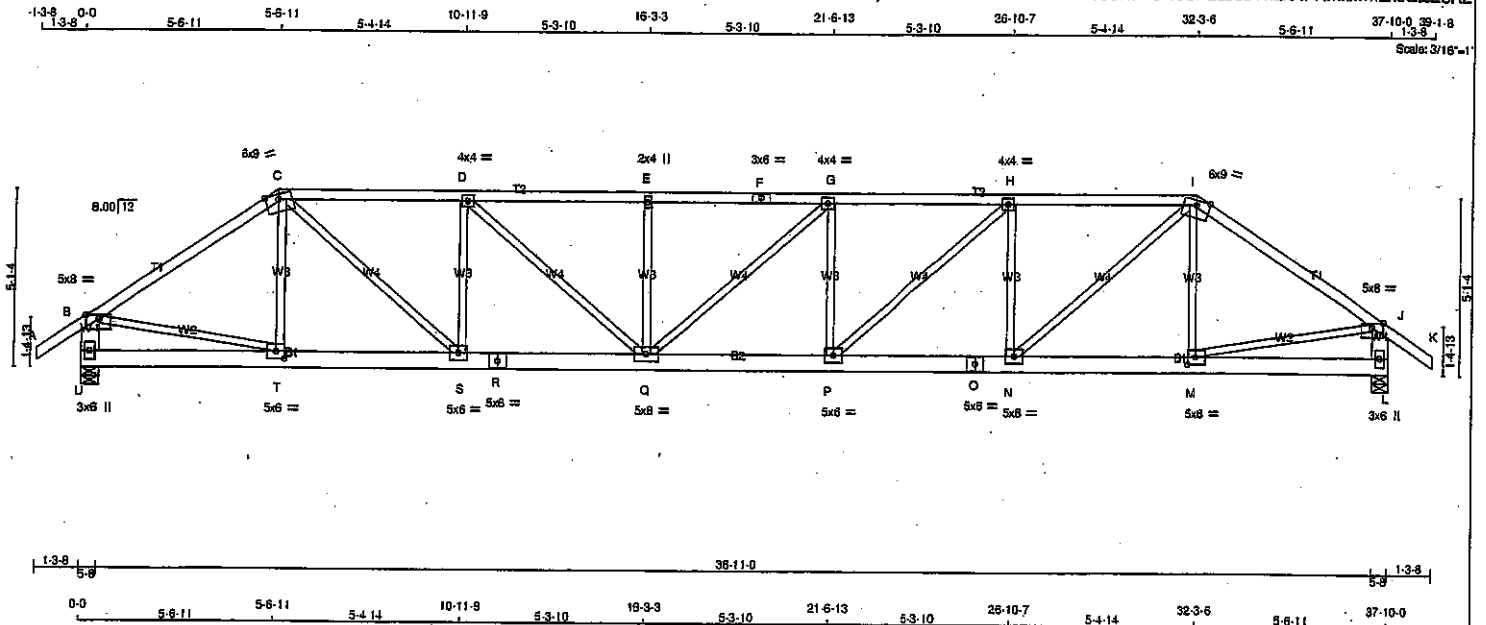


Structural component only
DWG# T-2108111

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	T16	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:v5FX5BJoJrZNLenu26zoDzZi9Z-k?R95eSJrdYCKCSFZzboVnLkAFNmXmVnLxcGtZG2H2



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TTVW-m	MT20	6.0	9.0	Edge	
D, G, H					
D TMVW-t	MT20	4.0	4.0		
E TMVW-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
I TTVW-m	MT20	6.0	9.0	Edge	
J TMVW-p	MT20	5.0	8.0	Edge	
L BMV1-p	MT20	3.0	6.0		
M BMVW-t	MT20	5.0	6.0	2.50	2.75
N, P, S					
N BMVW-t	MT20	5.0	6.0		
O BS-t	MT20	5.0	6.0		
Q BMVW-w	MT20	5.0	8.0		
R BS-t	MT20	5.0	6.0		
T BMVW-t	MT20	5.0	6.0	2.50	2.75
U BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES (1)

1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	DOWN	HORIZ	DOWN	HORIZ	BRG	IN-SX	BRG	IN-SX
U	2212	0	0	2212	0	0	5-8	5-8	5-8
L	2212	0	0	2212	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1562	1038.0	0.0	0.0	0.0	523.0	0.0
L	1562	1038.0	0.0	0.0	0.0	523.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX		W E B S		MAX	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00	T-C	-256 / 0	0.09 (1)	
B-C	-2541 / 0	-91.8	-91.8	0.74 (1)	3.59	M-I	-254 / 2	0.09 (1)	
C-D	-3489 / 0	-91.8	-91.8	0.68 (1)	3.22	B-T	0 / 2145	0.48 (1)	
D-E	-4120 / 0	-91.8	-91.8	0.75 (1)	2.89	M-J	0 / 2145	0.48 (1)	
E-F	-4120 / 0	-91.8	-91.8	0.62 (1)	3.05	N-I	0 / 1832	0.41 (1)	
F-G	-4120 / 0	-91.8	-91.8	0.62 (1)	3.05	C-S	0 / 1835	0.41 (1)	
G-H	-4128 / 0	-91.8	-91.8	0.75 (1)	2.89	N-H	-1115 / 0	0.40 (1)	
H-I	-3487 / 0	-91.8	-91.8	0.85 (1)	3.22	S-D	-1108 / 0	0.40 (1)	
I-J	-2542 / 0	-91.8	-91.8	0.74 (1)	3.59	P-H	0 / 858	0.19 (1)	
J-K	0 / 35	-91.8	-91.8	0.12 (1)	10.00	D-Q	0 / 845	0.19 (1)	
U-B	-2166 / 0	0.0	0.0	0.14 (1)	6.93	P-G	-467 / 0	0.17 (1)	
L-J	-2167 / 0	0.0	0.0	0.14 (1)	6.93	Q-E	-470 / 0	0.17 (1)	
						Q-G	-11 / 0	0.01 (1)	
U-T	0 / 0	-18.5	-18.5	0.06 (4)	10.00				
T-S	0 / 2108	-18.5	-18.5	0.28 (1)	10.00				
S-R	0 / 3489	-18.5	-18.5	0.47 (1)	10.00				
R-Q	0 / 3489	-18.5	-18.5	0.47 (1)	10.00				
Q-P	0 / 4128	-18.5	-18.5	0.55 (1)	10.00				
P-O	0 / 3487	-18.5	-18.5	0.46 (1)	10.00				
O-N	0 / 3487	-18.5	-18.5	0.46 (1)	10.00				
N-M	0 / 2107	-18.5	-18.5	0.28 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/990 (0.46")

CS1: TC=0.75/1.00 (D-E:1), BC=0.55/1.00 (P-Q:1), WB=0.48/1.00 (J-M:1), SS1=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

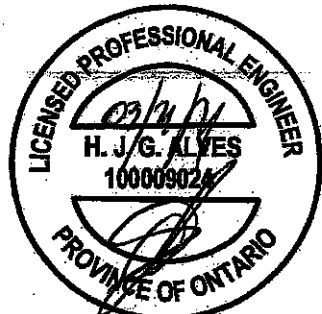
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.63 (R) (INPUT = 1.00)

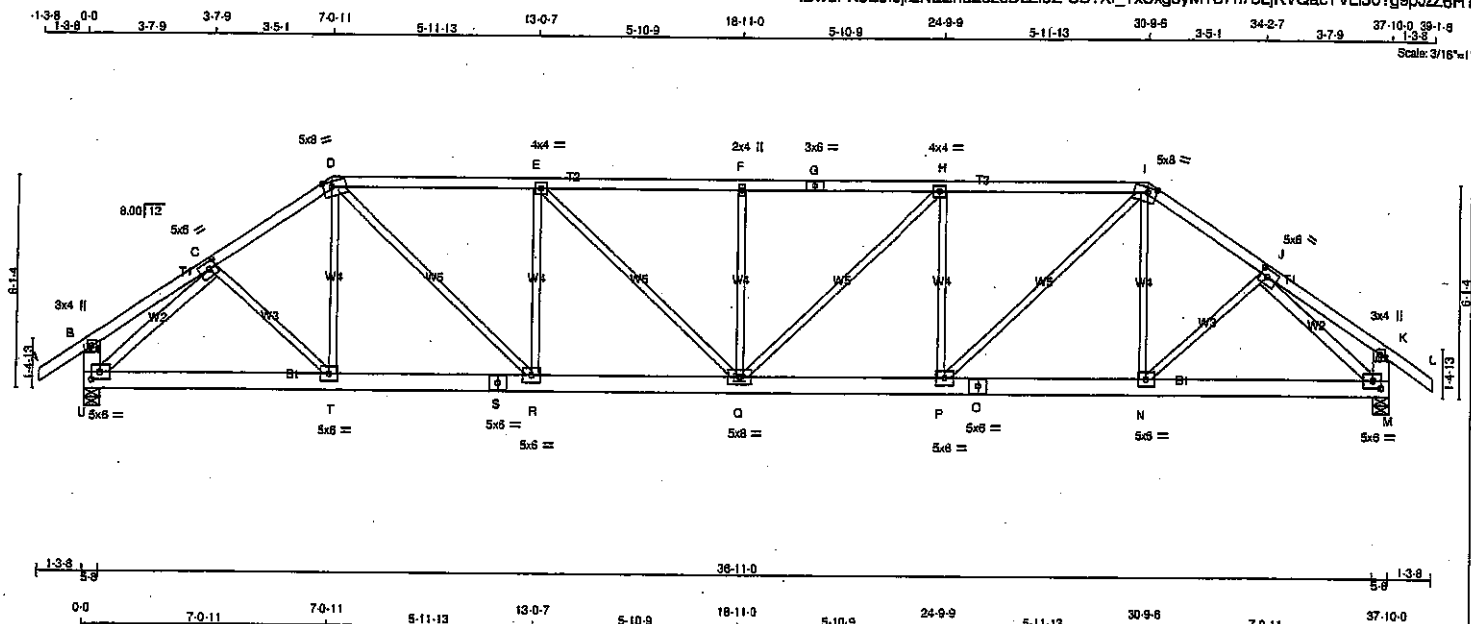


Structural component only
DWG# T-2108112

JOB NAME 417441	TRUSS NAME T17	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:v5FX5BJ0jrZNLenu26zoDzZl9Z-CB?Xl_TxCxg3yM1S7h75LjKVQacTVL30?g9pJzZ6H1



TOTAL WEIGHT = 2 X 186 = 372 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - L	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
U - S	2x6	DRY	No.2	SPF	
S - O	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
U - C	2x4	DRY	No.2	SPF	
J - M	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	JT	VERT	JT	DOWN	JT	HORZ	JT	BRG
U	2212	0	2212	0	0	5-8	5-8	5-8	5-8
M	2212	0	2212	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1562	1038 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0
M	1562	1038 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	C-T	0 / 143	0.03 (4)	
B-C	0 / 19	-91.8	-91.8 0.16 (1)	10.00	T-D	0 / 92	0.03 (4)	
C-D	-2557 / 0	-91.8	-91.8 0.26 (1)	4.10	N-I	0 / 92	0.03 (4)	
D-E	-3172 / 0	-91.8	-91.8 0.77 (1)	3.22	N-J	0 / 143	0.03 (4)	
E-F	-3478 / 0	-91.8	-91.8 0.82 (1)	3.04	U-C	-2762 / 0	0.89 (1)	
F-G	-3478 / 0	-91.8	-91.8 0.82 (1)	3.04	J-M	-2762 / 0	0.89 (1)	
G-H	-3478 / 0	-91.8	-91.8 0.82 (1)	3.04	P-I	0 / 1470	0.33 (1)	
H-I	-3172 / 0	-91.8	-91.8 0.77 (1)	3.22	D-R	0 / 1470	0.33 (1)	
I-J	-2557 / 0	-91.8	-91.8 0.26 (1)	4.10	P-H	-912 / 0	0.50 (1)	
J-K	0 / 19	-91.8	-91.8 0.16 (1)	10.00	R-E	-912 / 0	0.50 (1)	
K-L	0 / 35	-91.8	-91.8 0.12 (1)	10.00	Q-H	0 / 427	0.10 (1)	
U-B	-254 / 0	0.0	0.0 0.02 (1)	7.81	E-Q	0 / 427	0.10 (1)	
M-K	-254 / 0	0.0	0.0 0.02 (1)	7.81	Q-F	-496 / 0	0.27 (1)	
U-T	0 / 2003	-18.5	-18.5 0.29 (1)	10.00				
T-S	0 / 2109	-18.5	-18.5 0.30 (1)	10.00				
S-R	0 / 2109	-18.5	-18.5 0.30 (1)	10.00				
R-Q	0 / 3172	-18.5	-18.5 0.43 (1)	10.00				
Q-P	0 / 3172	-18.5	-18.5 0.43 (1)	10.00				
P-O	0 / 2109	-18.5	-18.5 0.30 (1)	10.00				
O-N	0 / 2109	-18.5	-18.5 0.30 (1)	10.00				
N-M	0 / 2003	-18.5	-18.5 0.29 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.33")

CSI: TC=0.82/1.00 (F-H:1), BC=0.43/1.00 (Q-R:1),
WB=0.69/1.00 (C-U:1), SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (I) (INPUT = 0.90)
JSI METAL = 0.63 (C) (INPUT = 1.00)

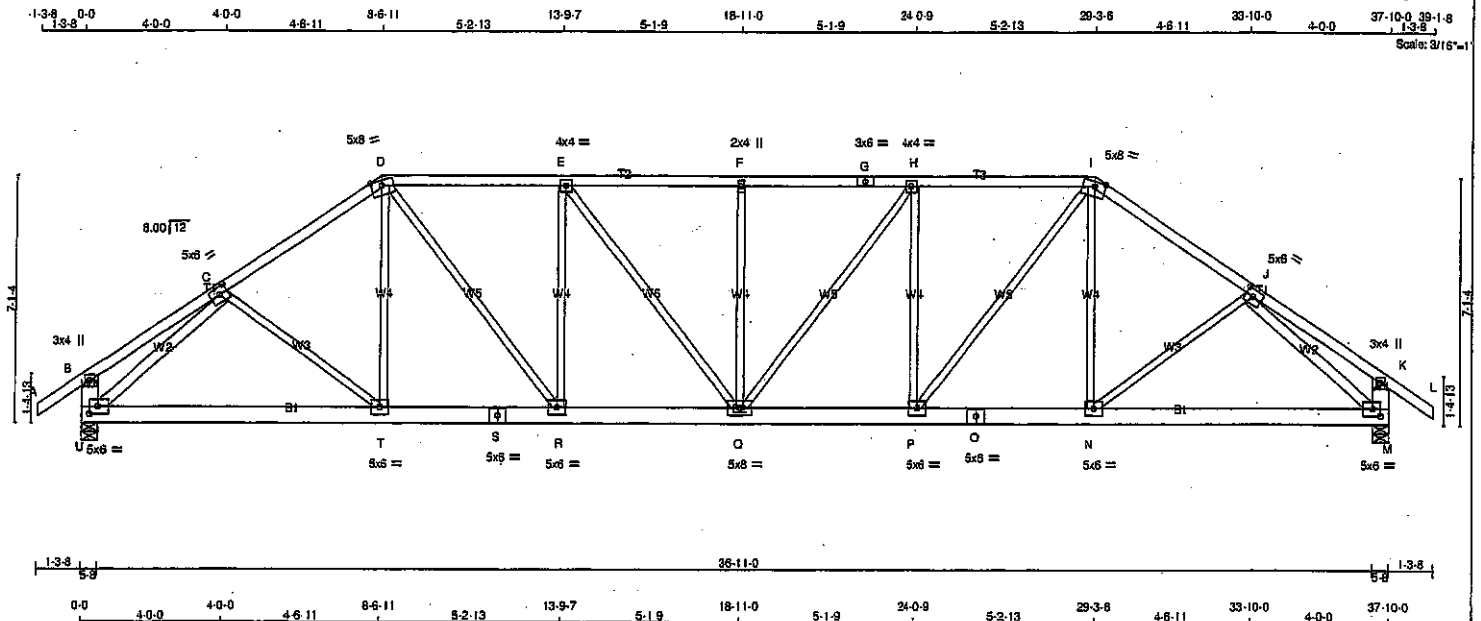


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	T18	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:v5FX5BjlojrZNLenu26zoDzZl9Z-gOZvWKTZzEowZWoeH0eKuwtkQ_zcEiWCFIQJLizZ6H0



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
U - B	2x6	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF
U - S	2x6	DRY No.2	SPF
S - O	2x6	DRY No.2	SPF
O - M	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
U - C	2x4	DRY No.2	SPF
J - M	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	5.0	6.0	2.50	2.75
D TTWW-m	MT20	5.0	8.0	1.75	3.75
E TMWW-t	MT20	4.0	4.0		
F TMWW-w	MT20	2.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMWW-t	MT20	4.0	4.0		
I TTWW-m	MT20	5.0	8.0	1.75	3.75
J TMWW-t	MT20	5.0	6.0	2.50	2.75
K TMV+p	MT20	3.0	4.0		
M BMVW1-t	MT20	5.0	6.0	2.50	2.75
N, P, R, T					
N BMVW1-t	MT20	5.0	6.0		
O BS-t	MT20	5.0	6.0		
Q BMVW1-t	MT20	5.0	8.0		
S BS-t	MT20	5.0	6.0		
U BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES

1) Lateral bracing to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	2212	0	0	0
M	2212	0	0	0
U	2212	0	0	0

UNFACTORED REACTIONS

	1ST LOOSE	MAX. MIN. COMPONENT REACTIONS					
JT COMBINED	1562	1038	0	0	0	0	0
M	1562	1038	0	0	0	0	0
U	1562	1038	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)
FR-TO				FR-TO		
A-B	0/35	-91.8	0.12 (1)	T-D	0/140	0.05 (4)
B-C	0/27	-91.8	0.27 (1)	M-I	0/140	0.05 (4)
C-D	-2515/0	-91.8	0.41 (1)	P-I	0/1110	0.25 (1)
D-E	-2755/0	-91.8	0.53 (1)	D-R	0/1110	0.25 (1)
E-F	-2952/0	-91.8	0.54 (1)	P-H	-795/0	0.65 (1)
F-G	-2952/0	-91.8	0.54 (1)	R-H	-795/0	0.65 (1)
G-H	-2952/0	-91.8	0.54 (1)	Q-H	0/325	0.07 (1)
H-I	-2755/0	-91.8	0.53 (1)	E-Q	0/325	0.07 (1)
I-J	-2515/0	-91.8	0.41 (1)	Q-F	-432/0	0.35 (1)
J-K	0/27	-91.8	0.27 (1)	C-T	0/71	0.02 (4)
K-L	0/35	-91.8	0.12 (1)	U-C	-2785/0	0.85 (1)
U-B	-255/0	0.0	0.02 (1)	N-J	0/71	0.02 (4)
M-K	-255/0	0.0	0.02 (1)	J-M	-2785/0	0.85 (1)
U-T	0/2047	-18.5	0.33 (1)			
T-S	0/2079	-18.5	0.31 (1)			
S-R	0/2073	-18.5	0.31 (1)			
R-Q	0/2755	-18.5	0.37 (1)			
Q-P	0/2755	-18.5	0.37 (1)			
P-O	0/2073	-18.5	0.31 (1)			
O-N	0/2073	-18.5	0.31 (1)			
N-M	0/2047	-18.5	0.33 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 8.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.54/1.00 (F-H:1), BC=0.37/1.00 (Q-R:1),
WB=0.85/1.00 (J-M:1), SS=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

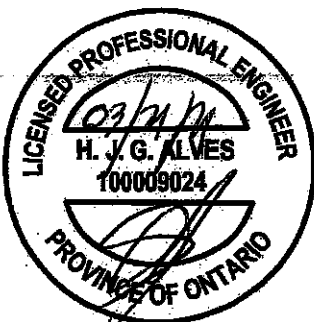
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	550	371	1747

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (U) (INPUT = 0.90)
JSI METAL = 0.83 (J) (INPUT = 1.00)

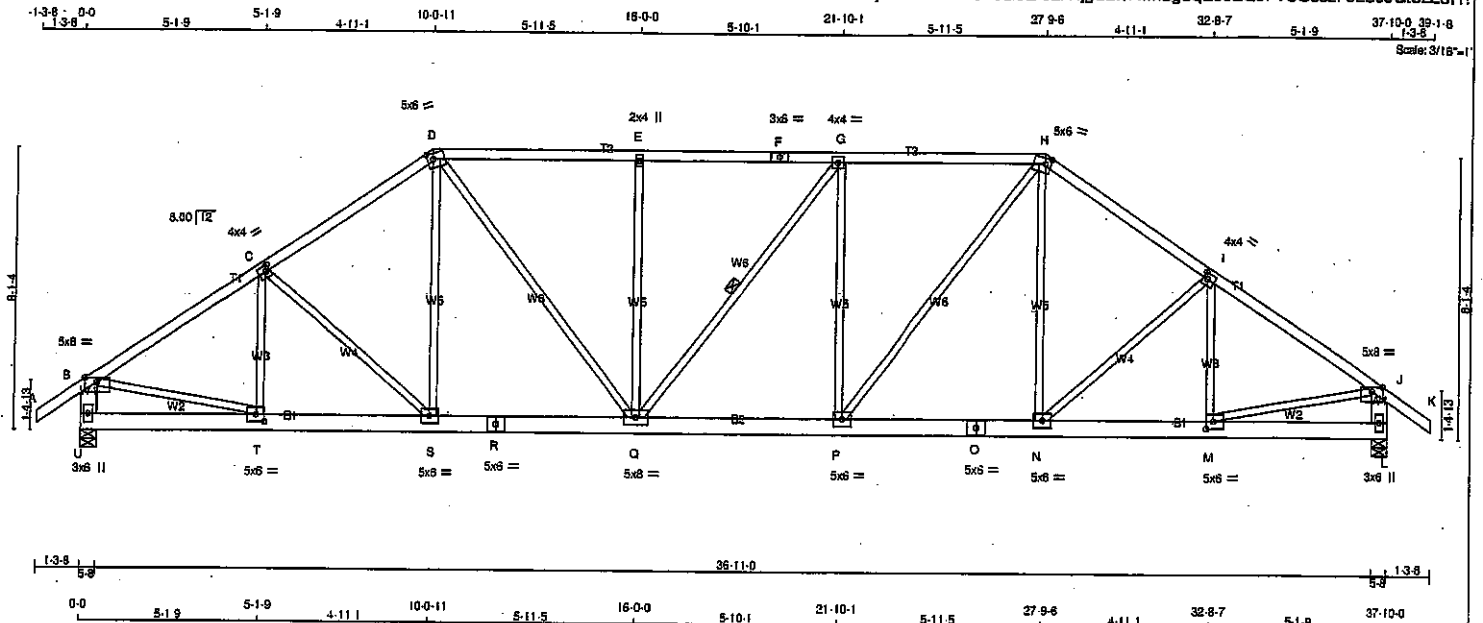


Structural component only
DWG# T-2108114

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	T19	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:v5FX5BJ0jrZNLenu28zoDzZl9Z-8a7HjgUBKymBgBqE69ZQ8PvCOJzF8LU9GICZ6H?



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	8.0	2.00	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTVW-m	MT20	5.0	8.0	2.00	2.00
I	TMVW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1-p	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	8.0	2.50	2.50
N, P, S						
N	BMVW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
Q	BMVW-w	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMVW-t	MT20	5.0	8.0	2.50	2.75
U	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
	FACTORED			MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2212	0		2212	0	0	5-8	5-8
L	2212	0		2212	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1562	1038 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0
L	1562	1038 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRAC LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX. UNBRAC LENGTH	FR-TO
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00	T-C	-349 / 0	0.11 (1)		
B-C	-2552 / 0	-91.8	-91.8 0.40 (1)	3.98	C-S	-195 / 0	0.16 (1)		
C-D	-2440 / 0	-91.8	-91.8 0.39 (1)	4.06	S-D	0 / 259	0.06 (1)		
D-E	-2531 / 0	-91.8	-91.8 0.48 (1)	3.89	D-Q	0 / 863	0.19 (1)		
E-F	-2532 / 0	-91.8	-91.8 0.44 (1)	3.94	Q-E	-584 / 0	0.70 (1)		
F-G	-2532 / 0	-91.8	-91.8 0.44 (1)	3.94	G-Q	-8 / 0	0.00 (1)		
G-H	-2536 / 0	-91.8	-91.8 0.48 (1)	3.94	P-G	-562 / 0	0.69 (1)		
H-I	-2438 / 0	-91.8	-91.8 0.39 (1)	4.07	P-H	0 / 872	0.20 (1)		
I-J	-2552 / 0	-91.8	-91.8 0.40 (1)	3.88	N-H	0 / 252	0.06 (1)		
J-K	0 / 35	-91.8	-91.8 0.12 (1)	10.00	N-I	-195 / 0	0.16 (1)		
U-B	-2157 / 0	0.0	0.0 0.14 (1)	6.94	M-I	-347 / 0	0.11 (1)		
L-J	-2158 / 0	0.0	0.0 0.14 (1)	6.94	B-T	0 / 2188	0.49 (1)		
					M-J	0 / 2188	0.49 (1)		
U-T	0 / 0	-18.5	-18.5 0.06 (4)	10.00					
T-S	0 / 2148	-18.5	-18.5 0.30 (1)	10.00					
S-R	0 / 2005	-18.5	-18.5 0.28 (1)	10.00					
R-Q	0 / 2005	-18.5	-18.5 0.28 (1)	10.00					
Q-P	0 / 2536	-18.5	-18.5 0.34 (1)	10.00					
P-O	0 / 2004	-18.5	-18.5 0.27 (1)	10.00					
O-N	0 / 2004	-18.5	-18.5 0.27 (1)	10.00					
N-M	0 / 2148	-18.5	-18.5 0.29 (1)	10.00					
M-L	0 / 0	-18.5	-18.5 0.06 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (1.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.23")

CSI: TC=0.48/1.00 (G-H-1), BC=0.34/1.00 (P-Q-1),
WB=0.70/1.00 (E-Q-1), SSI=0.25/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (T) (INPUT = 0.90)
JSI METAL = 0.49 (M) (INPUT = 1.00)

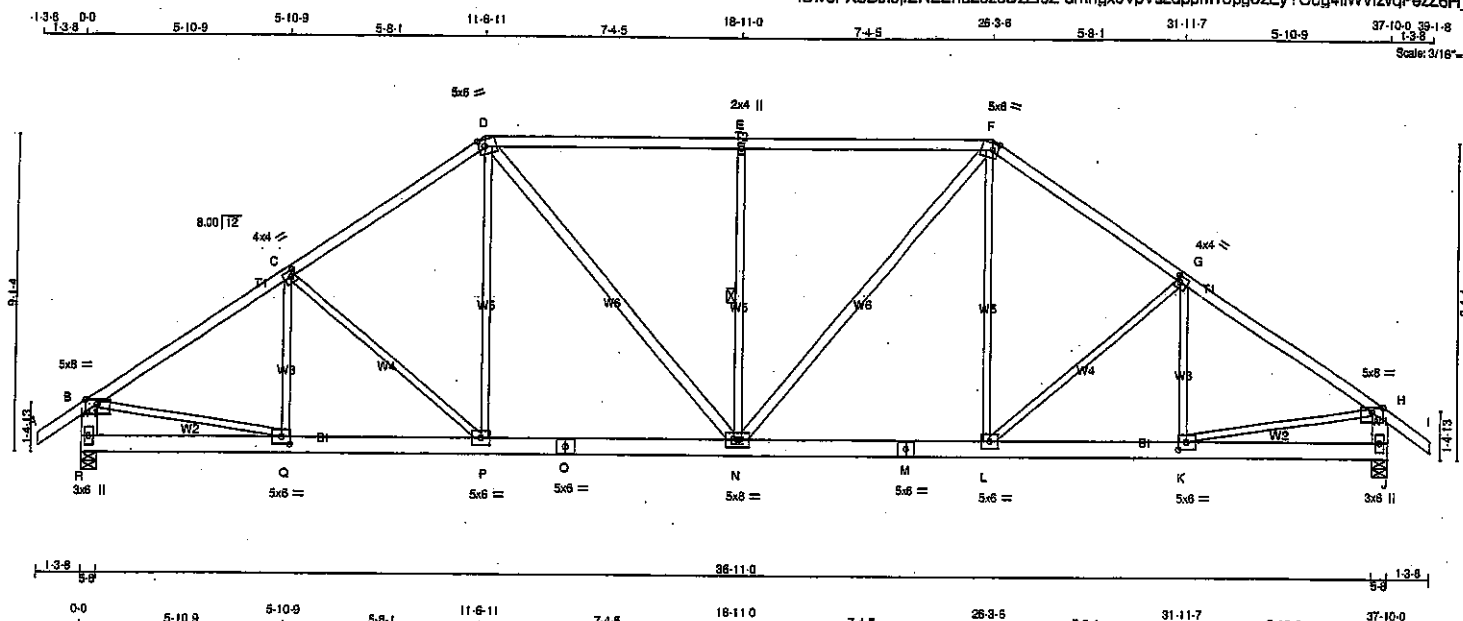


Structural component only
DWG# T-2108115

JOB NAME 417441	TRUSS NAME T20	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MTek Industries, Inc. Sat Mar 20 08:38:23 2021 Page 1
ID:v5FX5BJ0jrZNLenu26zoDzZl9Z-dmhp0VpVs20ppm10pgozLy?Cog4ilWVizvqPezZ6H



TOTAL WEIGHT = 5 X 197 = 984 lb (M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS		2x3	DRY	No.2
EXCEPT				SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	2212	0	2212	0	5-8	5-8
J	2212	0	2212	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1562	1038 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0
J	1562	1038 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
A-B	0 / 35	-91.8	-91.8 0.12 (1)	Q-C	-279 / 0	0.11 (1)
B-C	-2583 / 0	-91.8	-91.8 0.55 (1)	C-P	-332 / 0	0.38 (1)
C-D	-2353 / 0	-91.8	-91.8 0.50 (1)	P-D	0 / 353	0.08 (1)
D-E	-2336 / 0	-91.8	-91.8 0.78 (1)	D-N	0 / 629	0.10 (1)
E-F	-2336 / 0	-91.8	-91.8 0.78 (1)	N-E	-831 / 0	0.43 (1)
F-G	-2353 / 0	-91.8	-91.8 0.50 (1)	N-F	0 / 629	0.10 (1)
G-H	-2583 / 0	-91.8	-91.8 0.55 (1)	L-F	0 / 353	0.08 (1)
H-I	0 / 35	-91.8	-91.8 0.12 (1)	L-G	-332 / 0	0.38 (1)
R-B	-2155 / 0	0.0	0.0 0.14 (1)	K-G	-279 / 0	0.11 (1)
J-H	-2155 / 0	0.0	0.0 0.14 (1)	B-Q	0 / 2209	0.50 (1)
R-Q	0 / 0	-18.5	-18.5 0.07 (4)	K-H	0 / 2209	0.50 (1)
O-P	0 / 2178	-18.5	-18.5 0.31 (1)			
P-O	0 / 1990	-18.5	-18.5 0.28 (1)			
O-N	0 / 1990	-18.5	-18.5 0.28 (1)			
N-M	0 / 1990	-18.5	-18.5 0.28 (1)			
M-L	0 / 1990	-18.5	-18.5 0.28 (1)			
L-K	0 / 2178	-18.5	-18.5 0.31 (1)			
K-J	0 / 0	-18.5	-18.5 0.07 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	25.6	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.78/1.00 (D-E-1), BC=0.31/1.00 (P-Q-1),
WB=0.50/1.00 (B-Q-1), SSI=0.39/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 786 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (C) (INPUT = 0.90)
JSI METAL = 0.50 (C) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1-p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-w	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

(1) Lateral braces to be a minimum of 2X4 SPF #2.



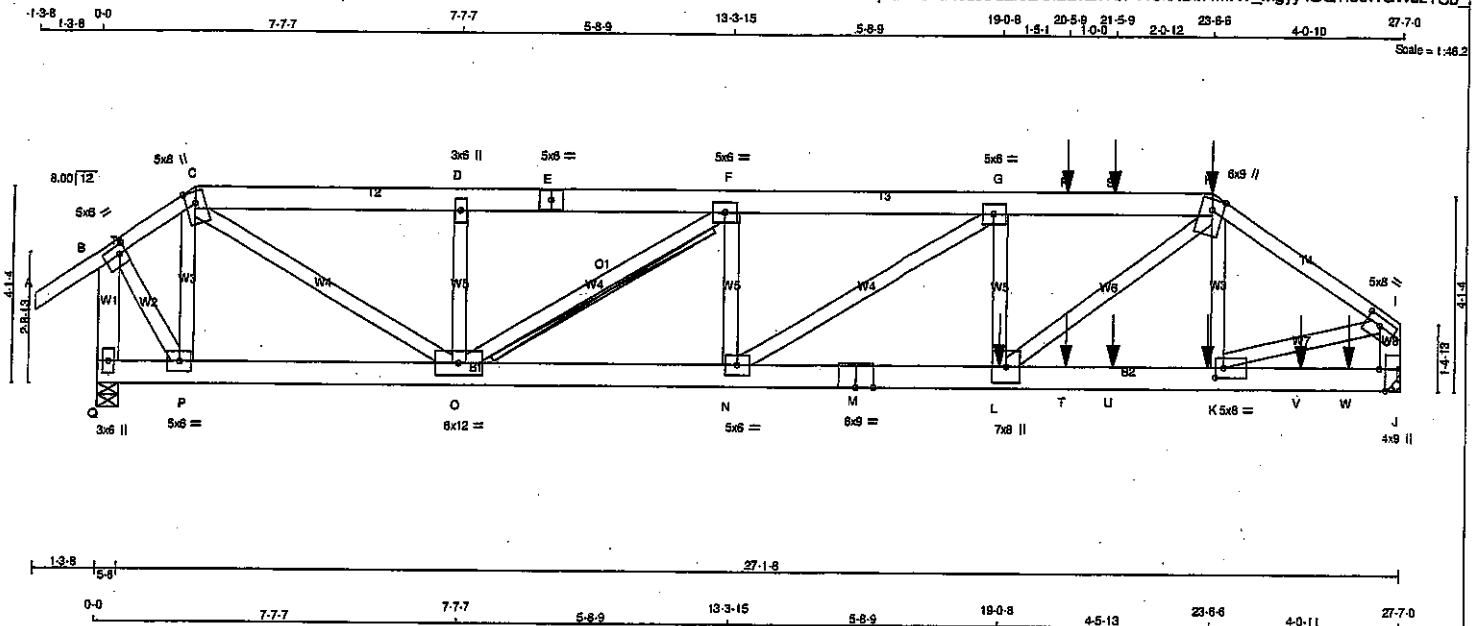
Structural component only
DWG# T-2108116

Structural component only DWG# T-2108117

JOB NAME 417441	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Mon Mar 22 08:04:15 2021 Page 1
ID:v5FX5BJlojZNLenu26zoDzZ9Z-3fBz12xY9P?v5kNBIn4mRV_Mgyy4BQT3cWGwBzYSb



LUMBER				
N. L. & A. RULES				
CHORDS	SIZE	LUMBER		DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.25	1.75
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMW-w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	
I	TMVW-t	MT20	5.0	8.0	2.00	3.75
J	BMV1-t	MT20	4.0	9.0	Edge	1.50
K	BMVW-t	MT20	5.0	8.0	2.50	2.25
L	BMVW-t	MT20	7.0	8.0		
M	BS-t	MT20	6.0	9.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	6.0	12.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
Q	2252	0	2252	0	5-8	5-8
J	3216	0	3216	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3'-8".

UNFACTORED REACTIONS

JT	1ST LOASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1588	1068	0	0	0	0	521	0
J	2271	1508	0	0	0	0	783	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
			LC1	LC2				
FR-TO						FR-TO		
A-B	0/35	-91.8	-91.8	0.14 (1)	10.00	P-C	-1209/0	0.21 (1)
B-C	-1312/0	-91.8	-91.8	0.10 (1)	5.52	K-H	-647/0	0.11 (1)
C-D	-3628/0	-91.8	-91.8	0.28 (1)	4.24	B-P	0/1733	0.31 (1)
D-E	-3627/0	-91.8	-91.8	0.28 (1)	4.24	K-I	0/3353	0.59 (1)
E-F	-3627/0	-91.8	-91.8	0.28 (1)	4.24	L-G	-341/0	0.08 (1)
F-G	-5136/0	-91.8	-91.8	0.34 (1)	3.61	L-H	0/3078	0.54 (1)
G-R	-5657/0	-91.8	-91.8	0.47 (1)	3.28	N-G	-617/0	0.36 (1)
R-S	-5657/0	-91.8	-91.8	0.47 (1)	3.28	C-O	0/2973	0.53 (1)
S-H	-5657/0	-91.8	-91.8	0.47 (1)	3.28	N-F	0/453	0.08 (1)
H-I	-3893/0	-91.8	-91.8	0.58 (1)	3.04	C-D	-890/0	0.10 (1)
Q-B	-2278/0	0.0	0.0	0.22 (1)	6.75	O-F	-1790/0	0.91 (1)
J-I	-3159/0	0.0	0.0	0.23 (1)	5.91			
Q-P	0/0	-18.5	-18.5	0.06 (4)	10.00			
P-O	0/1146	-18.5	-18.5	0.21 (1)	10.00			
O-N	0/5136	-18.5	-18.5	0.73 (1)	10.00			
N-M	0/5656	-18.5	-18.5	0.82 (1)	10.00			
M-L	0/5656	-18.5	-18.5	0.82 (1)	10.00			
L-T	0/3274	-18.5	-18.5	0.51 (1)	10.00			
T-U	0/3274	-18.5	-18.5	0.51 (1)	10.00			
U-K	0/3274	-18.5	-18.5	0.51 (1)	10.00			
K-V	0/0	-18.5	-18.5	0.07 (4)	10.00			
V-W	0/0	-18.5	-18.5	0.07 (4)	10.00			
W-J	0/0	-18.5	-18.5	0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	23-6-6	-41	-41	-	FRONT	VERT	DEAD	-	C1
H	23-6-6	-82	-82	-	FRONT	VERT	TOTAL	-	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OSC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL)= L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL)= L/999 (0.32")

CS: TC=0.58/1.00 (H-I:1), BC=0.82/1.00 (L-N:1), WB=0.91/1.00 (F-O:1), SS=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

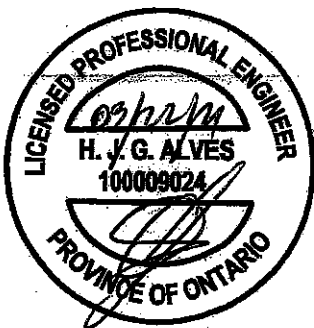
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 971 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (K) (INPUT = 0.90)
JSI METAL = 0.87 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2108118 1/2

JOB NAME 417441	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MTek Industries, Inc. Mon Mar 22 08:04:15 2021 Page 2 ID: y5FX5BJl0irZNLenu28zoDzZl9Z-3fBz12xY9P?v5kNBm4mRV_Mqwy4BQTl3cWGwbzYSb	

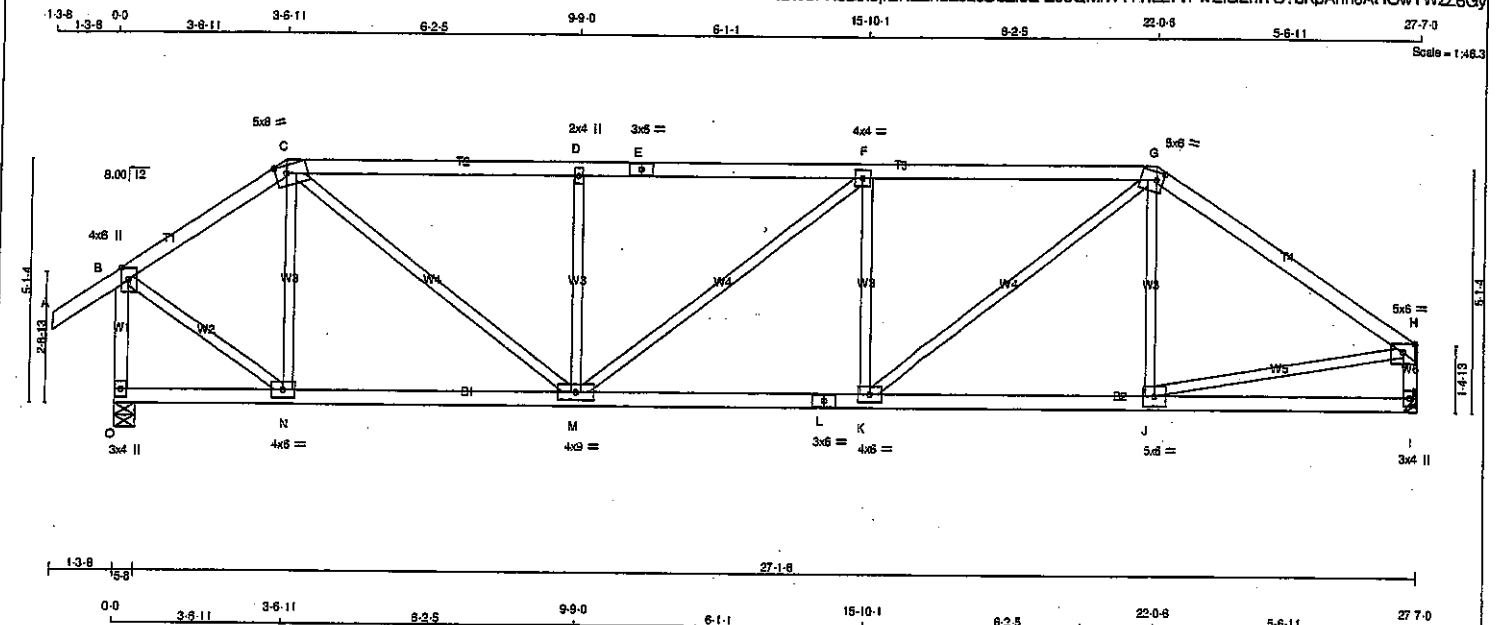
SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	23-6-6	-175	-175	—	FRONT	VERT	SNOW	—	C1
K	23-5-9	-21	-21	—	FRONT	VERT	TOTAL	—	C1
L	19-0-8	-1067	-1067	—	FRONT	VERT	TOTAL	—	C1
R	20-5-9	-76	-76	—	FRONT	VERT	TOTAL	—	C1
S	21-5-9	-76	-76	—	FRONT	VERT	TOTAL	—	C1
T	20-5-9	-21	-21	—	FRONT	VERT	TOTAL	—	C1
U	21-5-9	-21	-21	—	FRONT	VERT	TOTAL	—	C1
V	25-5-9	-21	-21	—	FRONT	VERT	TOTAL	—	C1
W	26-5-9	-21	-21	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



JOB NAME 417441	TRUSS NAME T23	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Sat Mar 20 08:38:25 2021 Page 1 ID:v5FX5BJoJrZNLenu26zoDzZi9Z-Z9oQMw41TIL27vPwEIG2m1O7bKpAhhoAHOWTWzZ6Gy			
		Scale = 1:48.3			



TOTAL WEIGHT = 111 lb

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF
G - H	2x4 DRY	No.2	SPF
O - B	2x4 DRY	No.2	SPF
I - H	2x4 DRY	No.2	SPF
O - L	2x4 DRY	No.2	SPF
L - J	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	8.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMW-w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	8.0		
K	BMVW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMVWW-t	MT20	4.0	9.0		
N	BMVW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
O	1647	0	1847	0	5-8	5-8
I	1521	0	1521	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1162	776 / 0	0 / 0	0 / 0	0 / 0	386 / 0	0 / 0
I	1075	708 / 0	0 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)
FR-TO					FR-TO		
A-B	0 / 35	-91.8	-91.8	0.12 (1)	N-C	-558 / 0	0.21 (1)
B-C	-1211 / 0	-91.8	-91.8	0.22 (1)	C-M	0 / 1362	0.31 (1)
C-D	-2070 / 0	-91.8	-91.8	0.61 (1)	M-D	-612 / 0	0.24 (1)
D-E	-2070 / 0	-91.8	-91.8	0.61 (1)	M-F	-178 / 0	0.22 (1)
E-F	-2070 / 0	-91.8	-91.8	0.61 (1)	K-F	-502 / 0	0.19 (1)
F-G	-2210 / 0	-91.8	-91.8	0.63 (1)	K-G	0 / 999	0.22 (1)
G-H	-1713 / 0	-91.8	-91.8	0.63 (1)	J-G	-149 / 43	0.08 (1)
O-B	-1626 / 0	0.0	0.0	0.23 (1)	B-N	0 / 1204	0.27 (1)
I-H	-1479 / 0	0.0	0.0	0.15 (1)	J-H	0 / 1449	0.33 (1)
O-N	0 / 0	-18.5	-18.5	0.11 (4)			
N-M	0 / 993	-18.5	-18.5	0.23 (1)			
M-L	0 / 2210	-18.5	-18.5	0.42 (1)			
L-K	0 / 2210	-18.5	-18.5	0.42 (1)			
K-J	0 / 1420	-18.5	-18.5	0.31 (1)			
J-I	0 / 0	-18.5	-18.5	0.15 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015.

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA C86-14
- TFC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TO=0.63/1.00 (F-G:1), BC=0.42/1.00 (K-M:1), WB=0.33/1.00 (H-J:1), SS=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

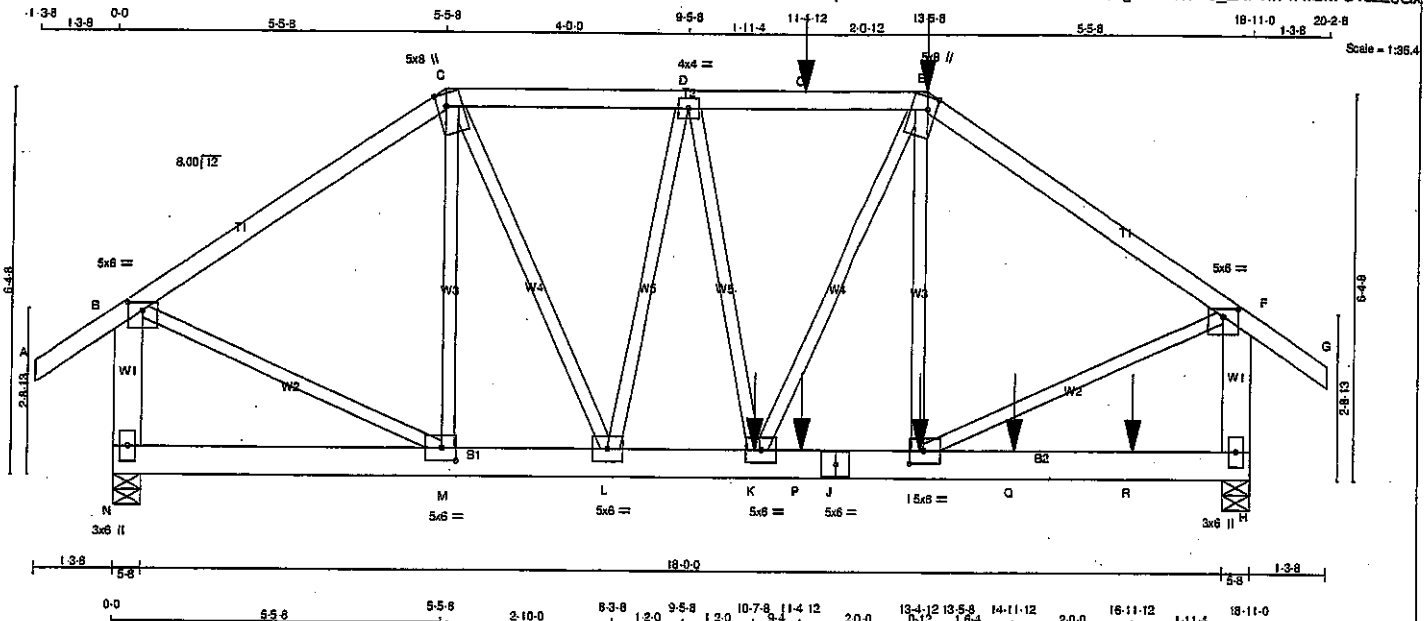
JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.63 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108119

JOB NAME 417441	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:v5FX5BJlqjZNLenu26zoDzZl9Z-1LMoZ2XiomQCgHUcTxDVb_aXF?iv4FxOx7U?zzZ6Gx



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF
N - B	2x6 DRY	No.2	SPF
H - F	2x6 DRY	No.2	SPF
N - J	2x6 DRY	No.2	SPF
J - H	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 1.50 3.00
C	TTWW-m	MT20	5.0	8.0 Edge 1.75
D	TMVW-t	MT20	4.0	4.0
E	TTWW-m	MT20	5.0	8.0 Edge 1.75
F	TMVW-p	MT20	5.0	6.0 1.50 3.00
H	BMV1-p	MT20	3.0	6.0
I	BMVW-t	MT20	5.0	6.0 2.50 2.75
J	BS-t	MT20	5.0	6.0
K	BMVW-t	MT20	5.0	6.0
L	BMVW-t	MT20	5.0	6.0
M	BMVW-t	MT20	5.0	6.0 2.50 2.75
N	BMV1-p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	VERT	HORZ	DOWN	UP
N	1946	0	1946	0	0
H	2420	0	2420	0	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1371	927.0	0.0	0.0	0.0	445.0	0.0
H	1707	1144.0	0.0	0.0	0.0	563.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0/35	M-C	-480/0
B-C	-1691/0	I-E	-399/0
C-D	-1877/0	B-M	0/1523
D-O	-2148/0	I-F	0/1988
O-E	-2148/0	C-L	0/1138
E-F	-2184/0	L-D	-948/0
F-G	0/35	D-K	0/419
N-B	-1905/0	K-E	0/804
H-F	-2352/0		
N-M	0/0		
M-L	0/1397		
L-K	0/2065		
K-P	0/1808		
P-J	0/1808		
J-I	0/1808		
I-Q	0/0		
Q-R	0/0		
R-H	0/0		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	FACE	DIR.	TYPE	HEEL	CONN.
E	13-5-8	-304	-304	FRONT	VERT	TOTAL	---	C1
I	13-4-12	-49	-49	FRONT	VERT	TOTAL	---	C1
K	10-7-8	-869	-869	FRONT	VERT	TOTAL	---	C1
O	11-4-12	-86	-86	FRONT	VERT	TOTAL	---	C1
P	11-4-12	-49	-49	FRONT	VERT	TOTAL	---	C1
Q	14-11-12	-49	-49	FRONT	VERT	TOTAL	---	C1
R	16-11-12	-26	-26	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 39.0 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (0.63")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.63")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.73/1.00 (E-F:1), BC=0.40/1.00 (K-L:1), WB=0.63/1.00 (D-L:1), SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	850 371	1747 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

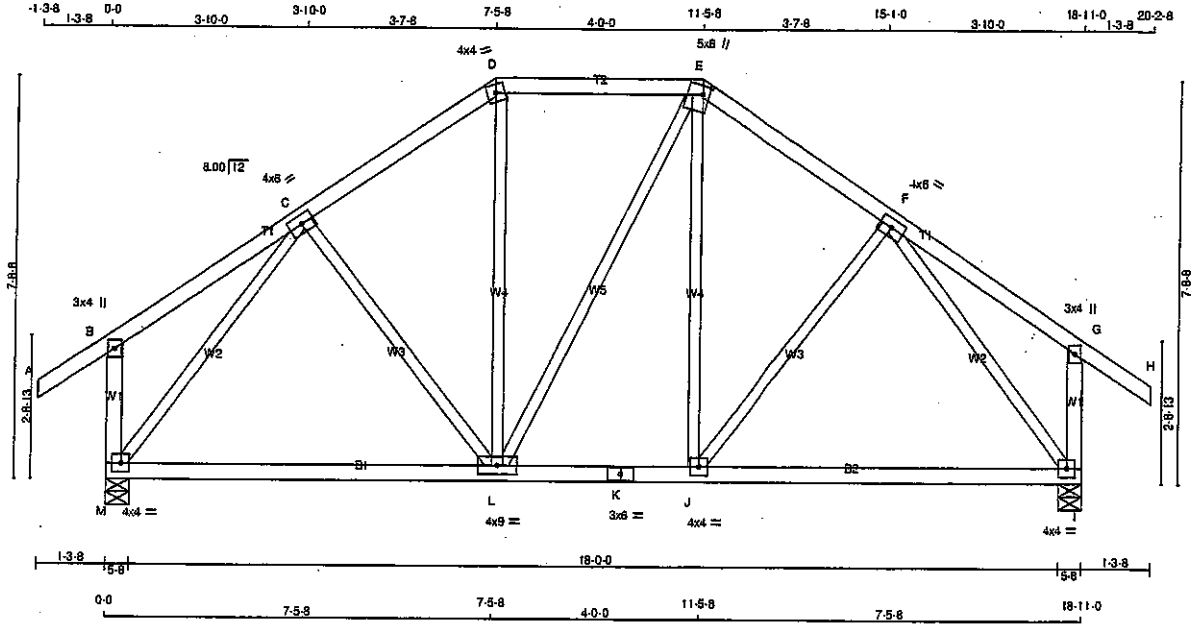
JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108120

JOB NAME 417441	TRUSS NAME T25	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 3 X 93 = 279 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW-t	MT20	4.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	RECORD BRG IN-SX
M	1169	0	1169	0	5-8	5-8
I	1169	0	1169	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	MAX/MIN. COMPONENT REACTIONS LIVE	MAX/MIN. COMPONENT REACTIONS PERM. LIVE	MAX/MIN. COMPONENT REACTIONS WIND	MAX/MIN. COMPONENT REACTIONS DEAD	MAX/MIN. COMPONENT REACTIONS SOIL
M	824	554 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
I	824	554 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	C-L	-5 / 44	0.02 (4)
B-C	0 / 22	-91.8 -91.8	0.20 (1)	10.00	L-D	0 / 119	0.03 (4)
C-D	-600 / 0	-91.8 -91.8	0.16 (1)	6.25	L-E	0 / 0	0.00 (1)
D-E	-648 / 0	-91.8 -91.8	0.19 (1)	6.25	J-E	0 / 119	0.03 (4)
E-F	-800 / 0	-91.8 -91.8	0.18 (1)	6.25	J-F	-5 / 44	0.02 (4)
F-G	0 / 22	-91.8 -91.8	0.21 (1)	10.00	M-C	-1071 / 0	0.69 (1)
G-H	0 / 35	-91.8 -91.8	0.12 (1)	10.00	F-I	-1070 / 0	0.69 (1)
M-B	-258 / 0	0.0 0.0	0.04 (1)	7.81			
I-G	-258 / 0	0.0 0.0	0.04 (1)	7.81			
M-L	0 / 649	-18.5 -18.5	0.27 (4)	10.00			
L-K	0 / 648	-18.5 -18.5	0.25 (4)	10.00			
K-J	0 / 648	-18.5 -18.5	0.25 (4)	10.00			
J-I	0 / 649	-18.5 -18.5	0.27 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.11")

CSI: TO=0.21/1.00 (F-G:1), BC=0.27/1.00 (I-J:4), WB=0.69/1.00 (C-M:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	650	371	1747 788 1987 1873

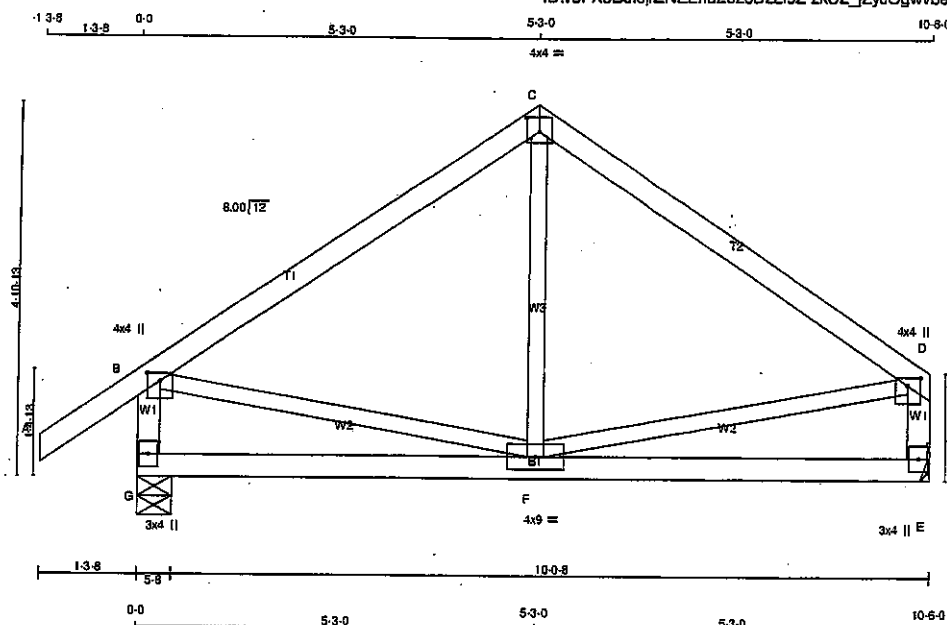
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (M) (INPUT = 0.90)
JSI METAL = 0.34 (K) (INPUT = 1.00)



JOB NAME 417441	TRUSS NAME T26	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 08:38:28 2021 Page 1			
		ID: v5FX5BJ0jrZNL Enu26zoDzI9Z-zkUZ_jZyJ0gwvba_bMGzPzf3pPIN6HEsFcb4rzZ6Gv			
		Scale = 1:29.0			



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW+p	MT20	4.0	4.0	1.25 2.00
C TTW+p	MT20	4.0	4.0	2.25 2.00
D TMVW+p	MT20	4.0	4.0	1.25 2.00
E BMV1+p	MT20	3.0	4.0	
F BMWWV-1	MT20	4.0	9.0	
G BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
G	705	0	705	0	0	5-8	5-8		
E	579	0	579	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS							
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
G	495	339 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0		
E	409	269 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	10.00	F-C	-22 / 82	0.03 (4)
B-C	-414 / 0	-91.8 -91.8	0.33 (1)	6.25	B-F	0 / 352	0.08 (1)
C-D	-414 / 0	-91.8 -91.8	0.33 (1)	6.25	F-D	0 / 352	0.08 (1)
G-B	-657 / 0	0.0 0.0	0.07 (1)	7.81			
E-D	-541 / 0	0.0 0.0	0.06 (1)	7.81			
G-F	0 / 0	-18.5 -18.5	0.14 (4)	10.00			
F-E	0 / 0	-18.5 -18.5	0.14 (4)	10.00			

TOTAL WEIGHT = 42 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.33/1.00 (B-C:1), BC=0.14/1.00 (F-G:4),
WB=0.08/1.00 (B-F:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.47 (D) (INPUT = 0.90)
JSI METAL = 0.14 (B) (INPUT = 1.00)

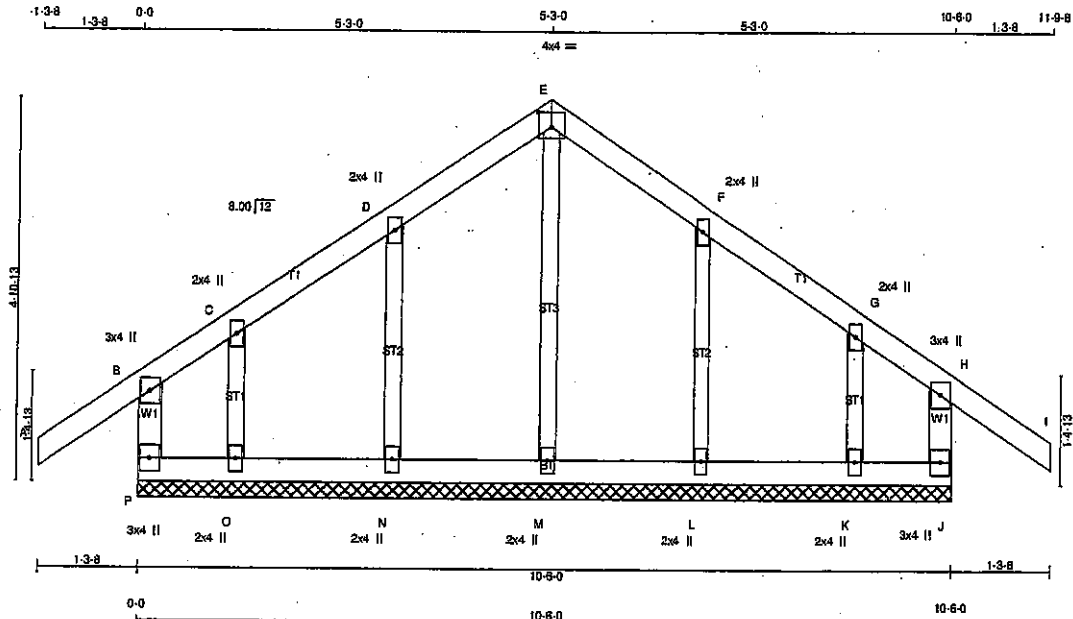


Structural component only
DWG# T-2108122

JOB NAME 417441	TRUSS NAME T26G	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 44 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C, D, F, G					
C	TMW+w	MT20	2.0	4.0	
E	TTW+p	MT20	4.0	4.0	2.25 2.00
H	TMV+p	MT20	3.0	4.0	
J	BMV1+p	MT20	3.0	4.0	
K, L, M, N, O					
K	BMW1+w	MT20	2.0	4.0	
P	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
P-B	-195 / 0	0.0	0.0 0.04 (1)	M-E	-230 / 0	0.08 (1)	
A-B	0 / 35	-91.8	-91.8 0.12 (1)	N-D	-199 / 0	0.04 (1)	
B-C	0 / 13	-91.8	-91.8 0.08 (1)	O-C	-98 / 0	0.01 (1)	
C-D	0 / 43	-91.8	-91.8 0.08 (1)	L-F	-199 / 0	0.04 (1)	
D-E	0 / 40	-91.8	-91.8 0.08 (1)	K-G	-98 / 0	0.01 (1)	
E-F	0 / 40	-91.8	-91.8 0.08 (1)				
F-G	0 / 43	-91.8	-91.8 0.08 (1)				
G-H	0 / 13	-91.8	-91.8 0.08 (1)				
H-I	0 / 35	-91.8	-91.8 0.12 (1)				
J-H	-195 / 0	0.0	0.0 0.04 (1)				
P-O	-26 / 0	-18.5	-18.5 0.01 (4)				
O-N	-30 / 0	-18.5	-18.5 0.02 (4)				
N-M	-35 / 0	-18.5	-18.5 0.02 (4)				
M-L	-35 / 0	-18.5	-18.5 0.02 (4)				
L-K	-30 / 0	-18.5	-18.5 0.02 (4)				
K-J	-26 / 0	-18.5	-18.5 0.01 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.12/1.00 (H-I), BC=0.02/1.00 (L-M), WB=0.08/1.00 (E-M), SSI=0.08/1.00 (H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

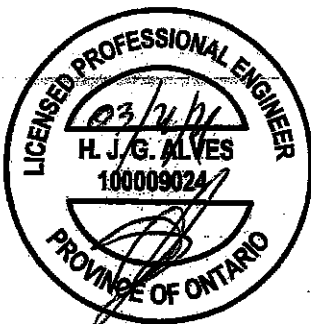
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

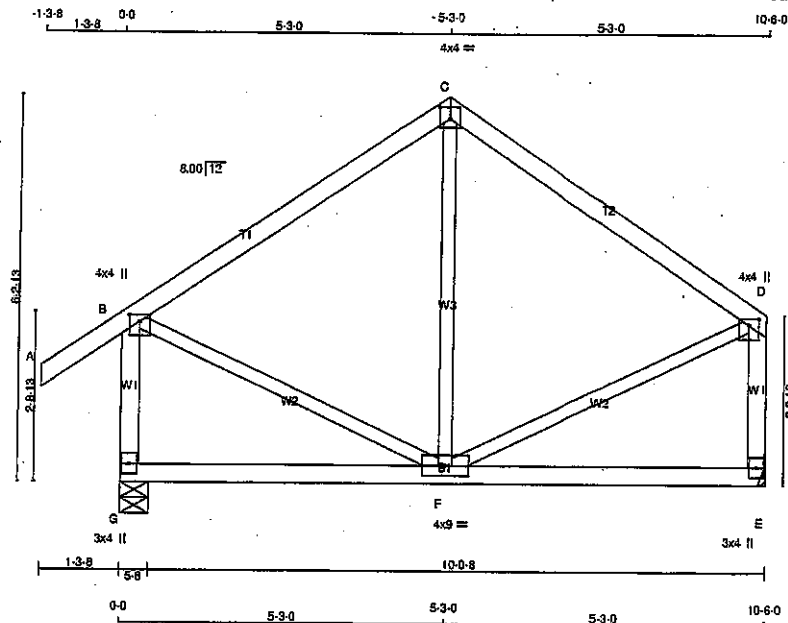
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.18 (E) (INPUT = 0.90)
JSI METAL=0.10 (D) (INPUT = 1.00)



Structural component only
DWG# T-2108123

JOB NAME 417441	TRUSS NAME T27	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:38:29 2021 Page 1	



TOTAL WEIGHT = 2 X 47 = 94 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C TTW+p	MT20	4.0	4.0	2.25	2.00
D TMVW+p	MT20	4.0	4.0	1.25	2.00
E BMV1+p	MT20	3.0	4.0		
F BMVWW+t	MT20	4.0	9.0		
G BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
G	705	0	705	0	0
E	579	0	579	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	495	339 / 0	0 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
E	409	269 / 0	0 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 35	-91.8	-91.8 0.12 (1)	F-C	-126 / 44	0.08 (1)	
B-C	-321 / 0	-91.8	-91.8 0.32 (1)	B-F	0 / 294	0.07 (1)	
C-D	-321 / 0	-91.8	-91.8 0.32 (1)	F-D	0 / 294	0.07 (1)	
G-B	-668 / 0	0.0	0.0 0.10 (1)				
E-D	-542 / 0	0.0	0.0 0.08 (1)				
G-F	0 / 0	-18.5	-18.5 0.15 (4)				
F-E	0 / 0	-18.5	-18.5 0.15 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.35")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.32/1.00 (B-C:1), BC=0.15/1.00 (F-G:4), WB=0.08/1.00 (C-F:1), SI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

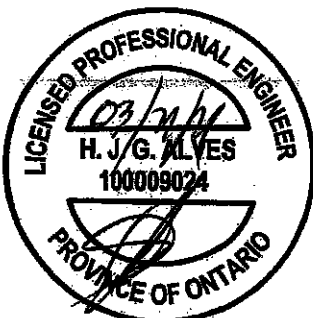
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.44 (D) (INPUT = 0.90)
JSI METAL = 0.13 (D) (INPUT = 1.00)

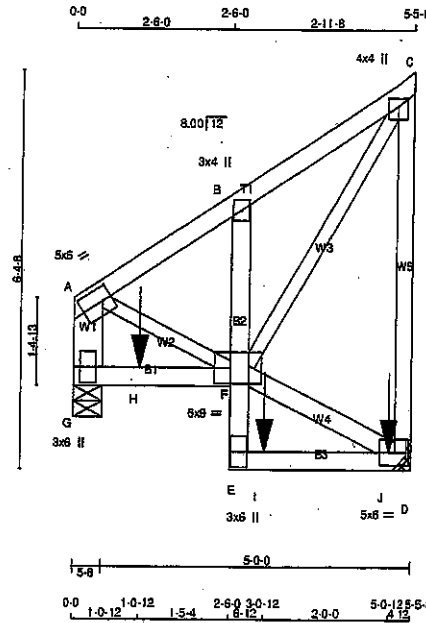


Structural component only
DWG# T-2108124

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	T28	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/32

TOTAL WEIGHT = 2 X 36 = 72 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
G - A	2x6	DRY	No.2
G - F	2x4	DRY	No.2
E - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
F - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	TOP
C-D	12	SIDE(86.6)
G-A	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G-F	12	SIDE(0.0)
B-E	12	TOP
E-D	12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	8	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	1252	0	1252	0	0
G	1041	0	1041	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	884	592 / 0	0 / 0	0 / 0	0 / 0	292 / 0	0 / 0
G	734	493 / 0	0 / 0	0 / 0	0 / 0	241 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-644 / 0	-91.8 -91.8	0.05 (1)	6.25	A-F	0 / 590	0.07 (1)
B-C	-641 / 0	-91.8 -91.8	0.05 (1)	6.25	F-D	-33 / 0	0.00 (1)
D-C	-642 / 0	0.0 0.0	0.26 (1)	7.81	F-C	0 / 1023	0.13 (1)
G-A	-682 / 0	0.0 0.0	0.02 (1)	7.81			
G-H	0 / 0	-18.5 -18.5	0.28 (1)	10.00			
H-F	0 / 0	-18.5 -18.5	0.28 (1)	10.00			
E-F	0 / 587	0.0 0.0	0.08 (1)	10.00			
F-B	-279 / 0	0.0 0.0	0.03 (1)	7.81			
E-I	0 / 30	-18.5 -18.5	0.19 (1)	10.00			
I-J	0 / 30	-18.5 -18.5	0.19 (1)	10.00			
J-D	0 / 30	-18.5 -18.5	0.19 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	1-0-12	-395	-395	---	BACK	VERT	TOTAL	---	C1
I	3-0-12	-399	-399	---	BACK	VERT	TOTAL	---	C1
J	5-0-12	-399	-399	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.26/1.00 (C-D:1), BC=0.28/1.00 (F-G:1), WB=0.13/1.00 (C-F:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

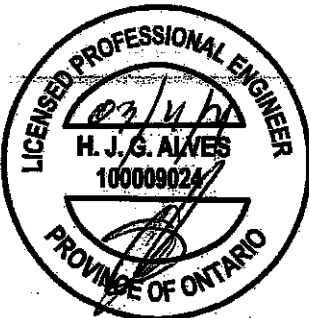
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (C) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 1.00)



Structural component only
DWG# T-2108125 1/12

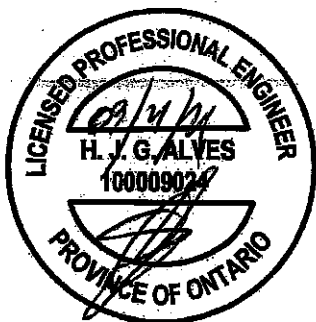
JOB NAME 417441	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MilTek Industries, Inc. Sat Mar 20 08:38:30 2021 Page 2 ID:v5FX5Bj0irZNLEnu26zoDzZi9Z-v6wJPpAr7we8uoNiniRlqkKcc37r02XJZ5h8kzZ6Gt	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TMV+p	MT20	3.0	4.0		
C	TMVW+p	MT20	4.0	4.0	1.50	2.00
D	BMVW1-t	MT20	5.0	6.0		
E	BMV+p	MT20	3.0	6.0		
F	BVMWWW-t	MT20	6.0	9.0	3.00	3.00
G	BMV1+p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T31	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	8.0		
D	TTWW-m	MT20	5.0	8.0	2.00	3.00
E	TMWW-t	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TMWW-t	MT20	4.0	6.0		
K	TTWW-m	MT20	5.0	8.0	2.00	3.00
L	TMWW-t	MT20	5.0	6.0		
M	TMV+p	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0	2.50	2.75
P, Q, S, U, W, X						
P	BMWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	5.0	8.0		
V	BS-t	MT20	5.0	6.0		
Y	BMVW1-t	MT20	5.0	6.0	2.50	2.75

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

LOADING

TOTAL LOAD CASES: (4)

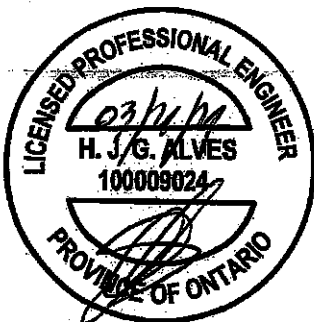
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO		FROM	TO		FR-TO		
Q-AR	0 / 4702	-18.5	-18.5 0.33 (1)	10.00			
AR-P	0 / 4702	-18.5	-18.5 0.33 (1)	10.00			
P-AS	0 / 4100	-18.5	-18.5 0.31 (1)	10.00			
AS-AT	0 / 4100	-18.5	-18.5 0.31 (1)	10.00			
AT-O	0 / 4100	-18.5	-18.5 0.31 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-231	-231		FRONT	VERT	TOTAL		C1
F	13-11-4	-76	-76		FRONT	VERT	TOTAL		C1
H	21-10-12	-76	-76		FRONT	VERT	TOTAL		C1
I	23-10-12	-76	-76		FRONT	VERT	TOTAL		C1
K	31-11-8	-231	-231		FRONT	VERT	TOTAL		C1
P	31-10-12	-21	-21		FRONT	VERT	TOTAL		C1
R	25-10-12	-21	-21		FRONT	VERT	TOTAL		C1
S	23-10-12	-21	-21		FRONT	VERT	TOTAL		C1
U	13-11-4	-21	-21		FRONT	VERT	TOTAL		C1
V	11-11-4	-21	-21		FRONT	VERT	TOTAL		C1
X	5-11-4	-21	-21		FRONT	VERT	TOTAL		C1
Z	7-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AA	9-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AB	11-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AC	15-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AD	17-11-4	-76	-76		FRONT	VERT	TOTAL		C1
AE	19-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AF	25-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AG	27-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AH	29-10-12	-76	-76		FRONT	VERT	TOTAL		C1
AI	1-11-4	-20	-20		FRONT	VERT	TOTAL		C1
AJ	3-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AK	7-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AL	9-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AM	15-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AN	17-11-4	-21	-21		FRONT	VERT	TOTAL		C1
AO	19-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AP	21-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AQ	27-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AR	29-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AS	33-10-12	-21	-21		FRONT	VERT	TOTAL		C1
AT	35-10-12	-20	-20		FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-2108138

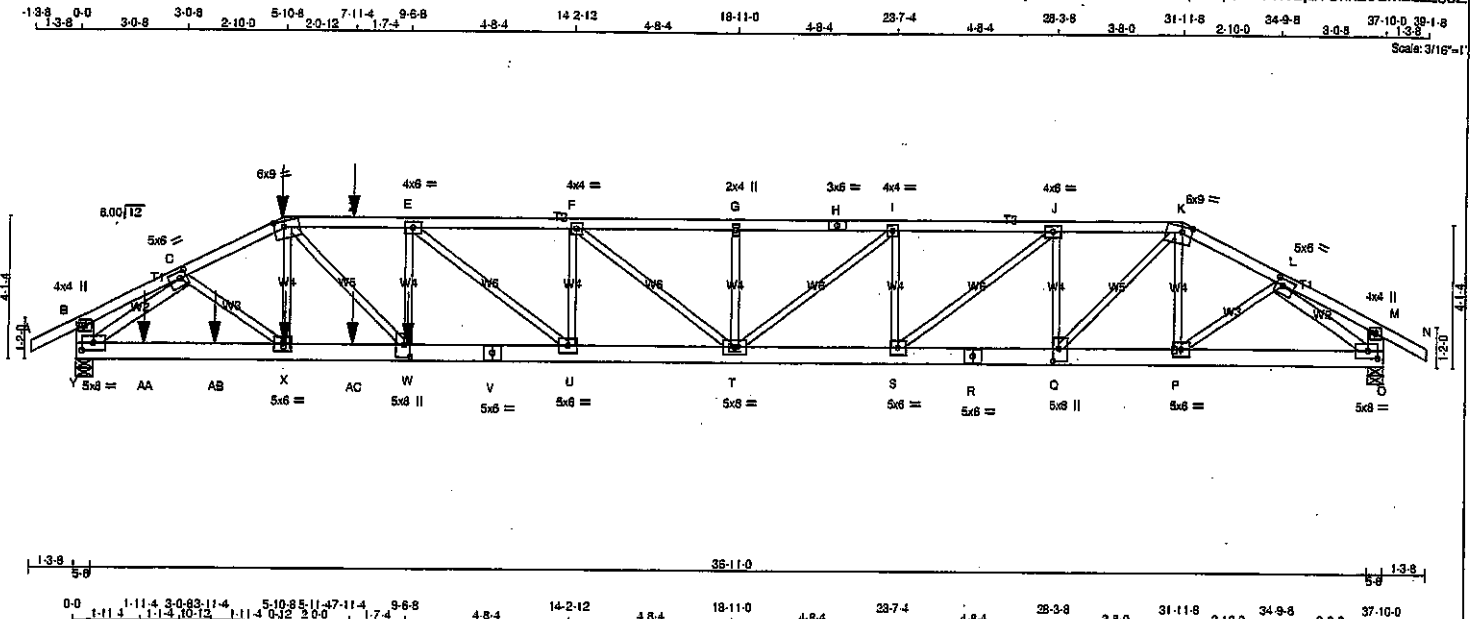
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T31Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:wa2i6usWauA5BCJTGMC4FzZjxO-z14PDkueCnGIMQw7djaEHV1n3dITChxE6Um2LzZ63Z

Scale: 3/16"=1'



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF
D - H	2x4 DRY	No.2	SPF
H - K	2x4 DRY	No.2	SPF
K - N	2x4 DRY	No.2	SPF
Y - B	2x6 DRY	No.2	SPF
O - M	2x6 DRY	No.2	SPF
Y - V	2x6 DRY	No.2	SPF
V - R	2x6 DRY	No.2	SPF
R - O	2x6 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF
Y - C	2x4 DRY	No.2	SPF
L - O	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-D 1	12	SIDE(81.0)
D-H 1	12	SIDE(81.0)
H-K 1	12	TOP
K-N 1	12	TOP
Y-B 2	12	TOP
O-M 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
Y-V 2	12	SIDE(183.1)
V-R 2	12	TOP
R-O 2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	SIDE(811.3)
E-W 1	3	
J-Q 1	3	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT		
O	3067	0	3067	0	5-8	5-8
Y	4948	0	4948	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	2165	1441 / 0	0 / 0	0 / 0	0 / 0	725 / 0	0 / 0
Y	3486	2311 / 0	0 / 0	0 / 0	0 / 0	1185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, Y

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO							
A-B	0 / 28	-91.8	-91.8	0.07 (1)	10.00	X-D	-513 / 0
B-C	-1 / 3	-91.8	-91.8	0.07 (1)	10.00	P-K	-273 / 0
C-D	-7960 / 0	-91.8	-91.8	0.28 (1)	3.30	W-E	-418 / 0
D-Z	-10450 / 0	-91.8	-91.8	0.60 (1)	2.62	Q-J	-2031 / 0
Z-E	-10450 / 0	-91.8	-91.8	0.60 (1)	2.62	Q-K	0 / 2944
E-F	-10410 / 0	-91.8	-91.8	0.62 (1)	2.69	D-W	0 / 4793
F-G	-9625 / 0	-91.8	-91.8	0.55 (1)	2.86	S-J	0 / 2812
G-H	-9625 / 0	-91.8	-91.8	0.55 (1)	2.86	E-U	-51 / 2
H-I	-9625 / 0	-91.8	-91.8	0.55 (1)	2.86	S-I	-1528 / 0
I-J	-8252 / 0	-91.8	-91.8	0.44 (1)	3.17	U-F	0 / 186
J-K	-8208 / 0	-91.8	-91.8	0.27 (1)	3.75	T-I	0 / 1755
K-L	-4652 / 0	-91.8	-91.8	0.13 (1)	4.38	F-T	-1003 / 0
L-M	0 / 7	-91.8	-91.8	0.08 (1)	10.00	T-G	-422 / 0
M-N	0 / 28	-91.8	-91.8	0.07 (1)	10.00	C-X	0 / 1286
Y-B	-254 / 0	0.0	0.0	0.01 (1)	7.81	Y-C	-7629 / 0
O-M	-244 / 0	0.0	0.0	0.01 (1)	7.81	P-L	0 / 837
						L-O	-4574 / 0
Y-AA	0 / 6069	-18.5	-18.5	0.43 (1)	10.00		
AA-AB	0 / 6069	-18.5	-18.5	0.43 (1)	10.00		
AB-X	0 / 6069	-18.5	-18.5	0.43 (1)	10.00		
X-AC	0 / 7086	-18.5	-18.5	0.56 (1)	10.00		
AC-W	0 / 7086	-18.5	-18.5	0.56 (1)	10.00		
W-V	0 / 10450	-18.5	-18.5	0.78 (1)	10.00		
V-U	0 / 10450	-18.5	-18.5	0.78 (1)	10.00		
U-T	0 / 10410	-18.5	-18.5	0.74 (1)	10.00		
T-S	0 / 8252	-18.5	-18.5	0.60 (1)	10.00		
S-R	0 / 8208	-18.5	-18.5	0.44 (1)	10.00		
R-Q	0 / 8208	-18.5	-18.5	0.44 (1)	10.00		
Q-P	0 / 4142	-18.5	-18.5	0.29 (1)	10.00		
P-O	0 / 3639	-18.5	-18.5	0.26 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-231	-231		BACK	VERT	TOTAL		C1
W	9-6-8	-2149	-2149		BACK	VERT	TOTAL		C1
X	5-11-4	-21	-21		BACK	VERT	TOTAL		C1
Z	7-11-4	-76	-76		BACK	VERT	TOTAL		C1
AA	1-11-4	-20	-20		BACK	VERT	TOTAL		C1
AB	3-11-4	-21	-21		BACK	VERT	TOTAL		C1
AC	7-11-4	-21	-21		BACK	VERT	TOTAL		C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.33")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/755 (0.60")

CSI: TC=0.82/1.00 (E-F:1), BC=0.78/1.00 (U-W:1), WB=0.66/1.00 (C-Y:1), SS=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (D) (INPUT = 0.90)
JSI METAL = 0.98 (V) (INPUT = 1.00)



Structural component only
DWG# T-2108139

CONTINUED ON PAGE 2

JOB NAME 417443	TRUSS NAME T31Z	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:52:42 2021 Page 2 ID:wa2t6usWauAI5BCJTGMC4FzZixO-z14PDkueCnGIMqW7diaEHV1rv3idTChxE6Um2LzZ63Z		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	4.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW-m	MT20	6.0	9.0	2.00	3.50
E	TMWW-t	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0		
J	TMWW-t	MT20	4.0	6.0		
K	TTWW-m	MT20	6.0	9.0	2.00	3.50
L	TMWW-t	MT20	5.0	6.0	2.25	2.25
M	TMV+p	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	8.0	2.50	3.50
P, S, U, X						
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	8.0	4.25	2.00
R	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	5.0	8.0		
V	BS-t	MT20	5.0	6.0		
W	BMWW-t	MT20	5.0	8.0	4.25	2.00
Y	BMVW1-t	MT20	5.0	8.0	2.50	3.50

CONNECTION REQUIREMENTS

- 1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

NOTES- (1)
1) Lateral braces to be a minimum of 2X+ SPF #2.

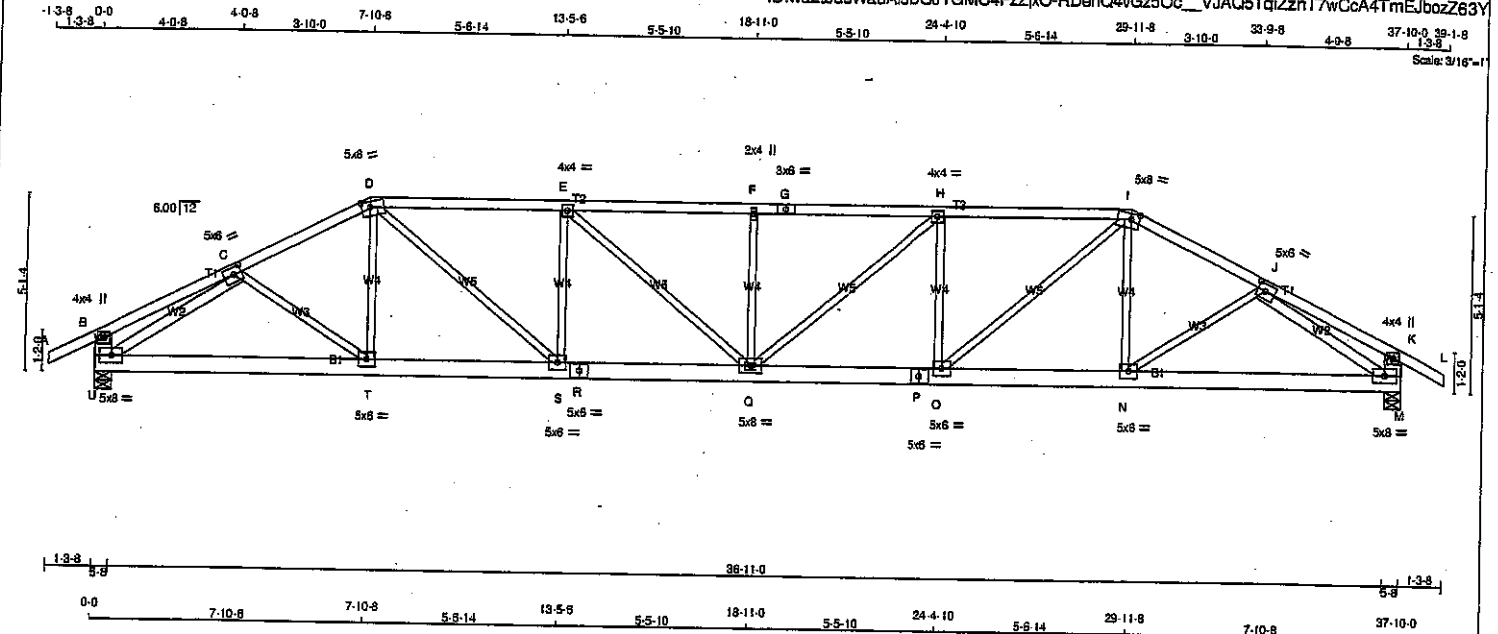


Structural component only
DWG# T-2108139 *m*

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T32	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Sat Mar 20 08:52:43 2021 Page 1
ID:wa218usWauA15BCJTGM4FzZkO-RDnQ4vGz5Oc_VJAQ5TqIZznT7wCcA4TmEJbozZ63Y



N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	4.0	4.0		
C TMWW-t	MT20	5.0	6.0	2.50	2.75
D TTWW-m	MT20	5.0	8.0	2.00	3.00
E TMWW-t	MT20	4.0	4.0		
F TMW-w	MT20	2.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMWW-t	MT20	4.0	4.0		
I TTWW-m	MT20	5.0	8.0	2.00	3.00
J TMWW-t	MT20	5.0	6.0	2.50	2.75
K TMV+p	MT20	4.0	4.0		
M BMWV-t	MT20	5.0	8.0		
N, O, S, T					
N BMWV-t	MT20	5.0	8.0		
P BS-t	MT20	5.0	8.0		
Q BMWVW-t	MT20	5.0	8.0		
R BS-t	MT20	5.0	8.0		
U BMWV-t	MT20	5.0	8.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
U	2210	0	2210	0	5-8	5-8
M	2210	0	2210	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1561	1037.0	0.0	0.0	0.0	523.0	0.0
M	1561	1037.0	0.0	0.0	0.0	523.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0/28	-91.8	-91.8	0.12 (1)	10.00	C-T	0/95	0.03 (4)
B-C	0/16	-91.8	-91.8	0.20 (1)	10.00	T-D	0/116	0.04 (4)
C-D	-3116/0	-91.8	-91.8	0.35 (1)	3.67	N-I	0/116	0.04 (4)
D-E	-3881/0	-91.8	-91.8	0.76 (1)	2.96	N-J	0/95	0.03 (4)
E-F	-4201/0	-91.8	-91.8	0.81 (1)	2.80	U-C	-3286/0	0.83 (1)
F-G	-4201/0	-91.8	-91.8	0.81 (1)	2.80	J-M	-3286/0	0.83 (1)
G-H	-4201/0	-91.8	-91.8	0.81 (1)	2.80	O-I	0/1453	0.33 (1)
H-I	-3881/0	-91.8	-91.8	0.76 (1)	2.96	D-S	0/1453	0.33 (1)
I-J	-3116/0	-91.8	-91.8	0.35 (1)	3.67	O-H	-849/0	0.30 (1)
J-K	0/16	-91.8	-91.8	0.20 (1)	10.00	S-E	-849/0	0.30 (1)
K-L	0/28	-91.8	-91.8	0.12 (1)	10.00	Q-H	0/424	0.10 (1)
U-B	-270/0	0.0	0.0	0.02 (1)	7.81	E-Q	0/424	0.10 (1)
M-K	-270/0	0.0	0.0	0.02 (1)	7.81	Q-F	-461/0	0.17 (1)
U-T	0/2698	-18.5	-18.5	0.39 (1)	10.00			
T-S	0/2772	-18.5	-18.5	0.38 (1)	10.00			
S-R	0/3881	-18.5	-18.5	0.52 (1)	10.00			
R-Q	0/3881	-18.5	-18.5	0.52 (1)	10.00			
Q-P	0/3881	-18.5	-18.5	0.52 (1)	10.00			
P-O	0/3881	-18.5	-18.5	0.52 (1)	10.00			
O-N	0/2772	-18.5	-18.5	0.38 (1)	10.00			
N-M	0/2698	-18.5	-18.5	0.39 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.45")

CSI: TC=0.81/1.00 (F-H:1), BC=0.52/1.00 (Q-S:1),
WB=0.83/1.00 (C-U:1), SS=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.75 (C) (INPUT = 1.00)



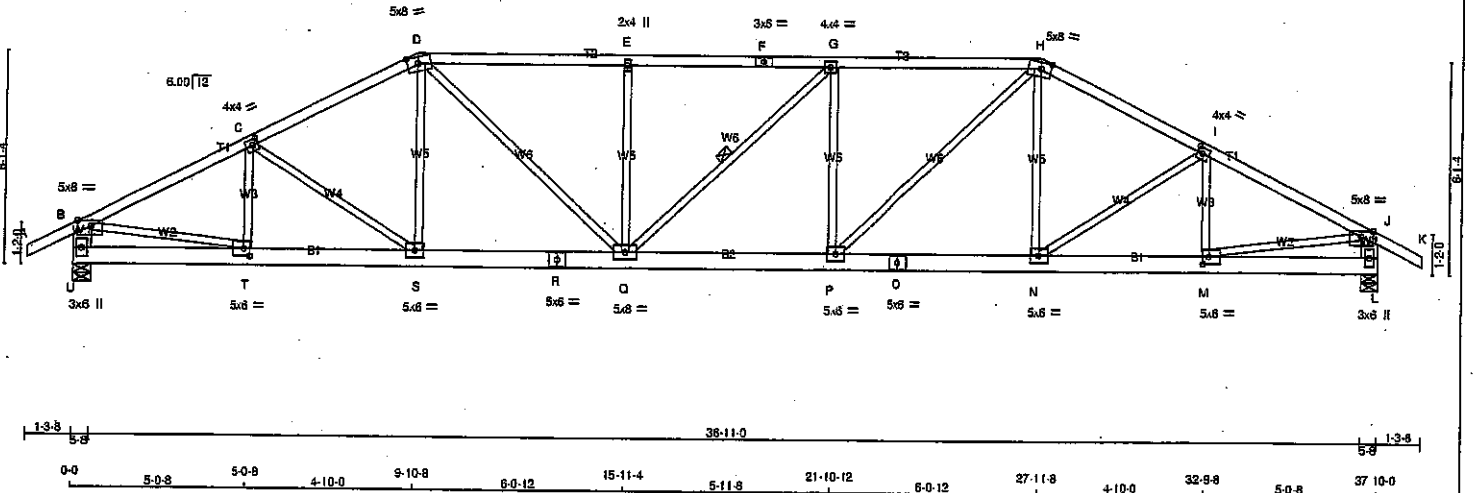
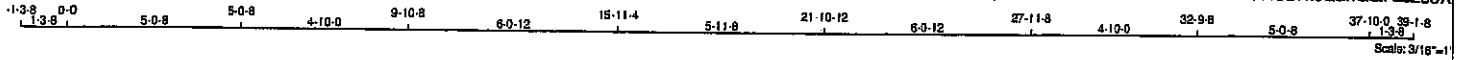
Structural component only
DWG# T-2108140

JOB NAME 417443	TRUSS NAME T33	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:wa2t6susWauAI5BCJTGMCF4ZzXO-VPC9sQwukOWTb84VkdIMw6BrsU7x6QEhQzI7EzZ63X



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
U - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	5.0	8.0	Edge
C TMVW-t	MT20	4.0	4.0	2.00 1.75
D TTWW-m	MT20	5.0	8.0	2.25 3.50
E TMVW-w	MT20	2.0	4.0	
F TS-t	MT20	3.0	6.0	
G TMVW-t	MT20	4.0	4.0	
H TTWW-m	MT20	5.0	8.0	2.25 3.50
I TMVW-t	MT20	4.0	4.0	2.00 1.75
J TMVW-p	MT20	5.0	8.0	Edge
L BMV1-p	MT20	3.0	6.0	
M BMVW-t	MT20	5.0	6.0	2.50 2.00
N, P, S				
N BMVW-t	MT20	5.0	8.0	
O BS-t	MT20	5.0	8.0	
Q BMVW-t	MT20	5.0	8.0	
R BS-t	MT20	5.0	8.0	
T BMVW-t	MT20	5.0	8.0	2.50 2.00
U BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
U	2210	0	2210	0	0	5-8	5-8	
L	2210	0	2210	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
U	1561	1037 : 0	0 : 0	0 : 0	0 : 0	523 : 0	0 : 0	
L	1561	1037 : 0	0 : 0	0 : 0	0 : 0	523 : 0	0 : 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	T-C	-389 / 0	0.08 (1)
B-C	-3126 / 0	-91.8 -91.8	0.42 (1)	3.62	C-S	-175 / 0	0.10 (1)
C-D	-3011 / 0	-91.8 -91.8	0.40 (1)	3.70	S-D	0 / 244	0.05 (1)
D-E	-3410 / 0	-91.8 -91.8	0.59 (1)	3.30	D-Q	0 / 1012	0.23 (1)
E-F	-3410 / 0	-91.8 -91.8	0.55 (1)	3.36	Q-E	-597 / 0	0.33 (1)
F-G	-3410 / 0	-91.8 -91.8	0.55 (1)	3.36	Q-G	-8 / 0	0.00 (1)
G-H	-3416 / 0	-91.8 -91.8	0.59 (1)	3.30	P-G	-595 / 0	0.32 (1)
H-I	-3009 / 0	-91.8 -91.8	0.40 (1)	3.70	P-H	0 / 1022	0.23 (1)
I-J	-3127 / 0	-91.8 -91.8	0.42 (1)	3.62	N-H	0 / 238	0.05 (1)
J-K	0 / 28	-91.8 -91.8	0.12 (1)	10.00	N-I	-177 / 0	0.11 (1)
U-B	-2147 / 0	0.0 0.0	0.14 (1)	6.94	M-I	-387 / 0	0.08 (1)
L-J	-2147 / 0	0.0 0.0	0.14 (1)	6.94	B-T	0 / 2849	0.64 (1)
					M-J	0 / 2849	0.64 (1)
U-T	0 / 0	-18.5 -18.5	0.07 (1)	10.00			
T-S	0 / 2816	-18.5 -18.5	0.39 (1)	10.00			
S-R	0 / 2675	-18.5 -18.5	0.36 (1)	10.00			
R-Q	0 / 2675	-18.5 -18.5	0.38 (1)	10.00			
Q-P	0 / 3417	-18.5 -18.5	0.46 (1)	10.00			
P-O	0 / 2673	-18.5 -18.5	0.36 (1)	10.00			
O-N	0 / 2673	-18.5 -18.5	0.36 (1)	10.00			
N-M	0 / 2816	-18.5 -18.5	0.39 (1)	10.00			
M-L	0 / 0	-18.5 -18.5	0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 38.0 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 089-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.37")

CSI: TC=0.59/1.00 (G-H:1), BC=0.46/1.00 (P-Q:1), WB=0.64/1.00 (J-M:1), SS=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

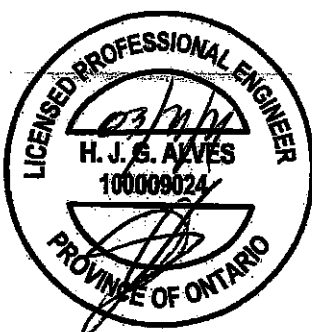
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (H) (INPUT=0.90)
JSI METAL=0.64 (M) (INPUT=1.00)

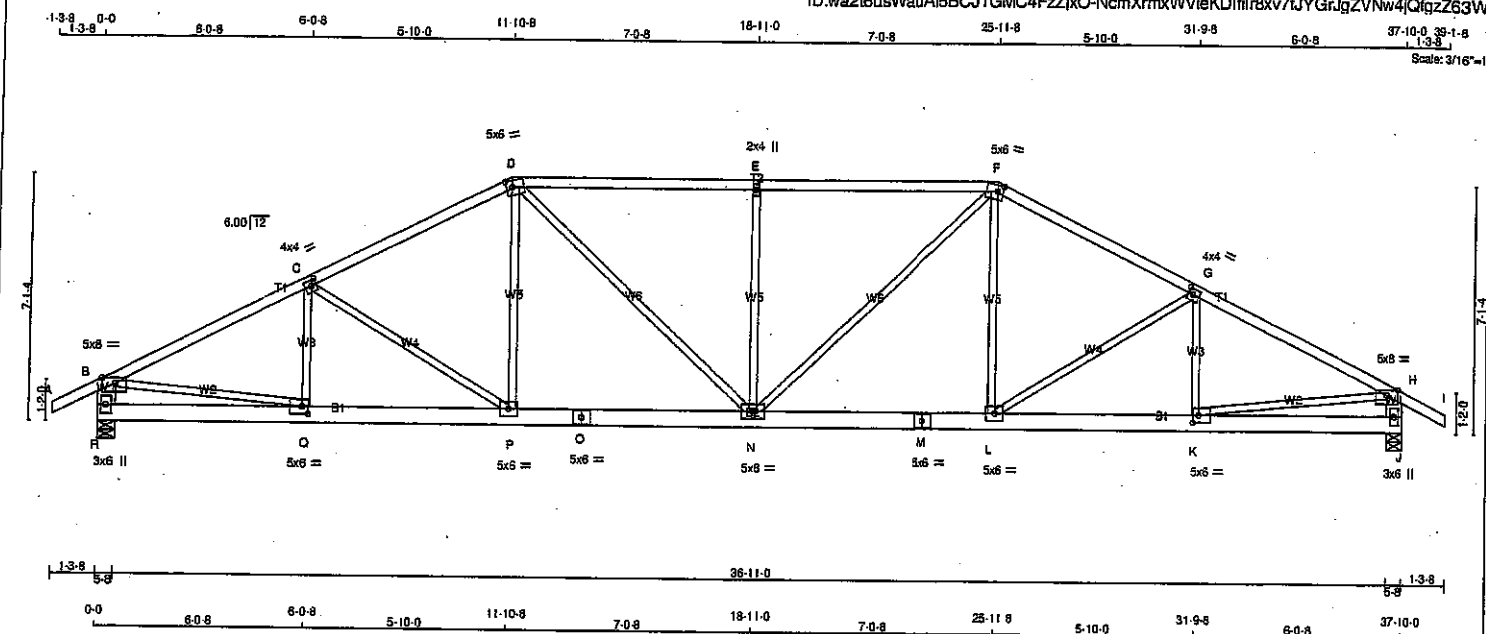


Structural component only
DWG# T-2108141

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T34	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:wa2t6usWauA15BCJTGMC4FZjxO-NcmXrmxWVieKDifilr8xv7fJYGrJgZVNw4jQ1qzZ63W



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - H	2x6	DRY	No.2	SPF
J - N	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.00
L	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.00
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
R	2210	2210	0	5-8
J	2210	2210	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	2ND CASE	MAX/MIN. COMPONENT REACTIONS
R	1581	1037 / 0	0 / 0	0 / 0
J	1581	1037 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MAX. UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO							FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00		Q-C	-290 / 0	0.07 (1)	
B-C	-3187 / 0	-91.8	-91.8	0.82 (1)	3.39		C-P	-402 / 0	0.37 (1)	
C-D	-2887 / 0	-91.8	-91.8	0.56 (1)	3.61		P-D	0 / 389	0.08 (1)	
D-E	-3018 / 0	-91.8	-91.8	0.79 (1)	3.17		D-N	0 / 854	0.15 (1)	
E-F	-3018 / 0	-91.8	-91.8	0.79 (1)	3.17		N-E	-795 / 0	0.68 (1)	
F-G	-2867 / 0	-91.8	-91.8	0.56 (1)	3.61		N-F	0 / 854	0.15 (1)	
G-H	-3187 / 0	-91.8	-91.8	0.82 (1)	3.39		L-F	0 / 389	0.08 (1)	
H-I	0 / 28	-91.8	-91.8	0.12 (1)	10.00		L-G	-402 / 0	0.37 (1)	
R-B	-2145 / 0	0.0	0.0	0.14 (1)	6.95		K-G	-290 / 0	0.07 (1)	
J-H	-2145 / 0	0.0	0.0	0.14 (1)	6.95		B-Q	0 / 2899	0.65 (1)	
R-Q	0 / 0	-18.5	-18.5	0.08 (4)	10.00		K-H	0 / 2899	0.65 (1)	
Q-P	0 / 2875	-18.5	-18.5	0.40 (1)	10.00					
P-O	0 / 2542	-18.5	-18.5	0.36 (1)	10.00					
O-N	0 / 2542	-18.5	-18.5	0.36 (1)	10.00					
N-M	0 / 2542	-18.5	-18.5	0.36 (1)	10.00					
M-L	0 / 2542	-18.5	-18.5	0.36 (1)	10.00					
L-K	0 / 2875	-18.5	-18.5	0.40 (1)	10.00					
K-J	0 / 0	-18.5	-18.5	0.08 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.32")

CSI: TC=0.79/1.00 (E-F:1), BC=0.40/1.00 (P-Q:1),
WB=0.65/1.00 (B-Q:1), SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

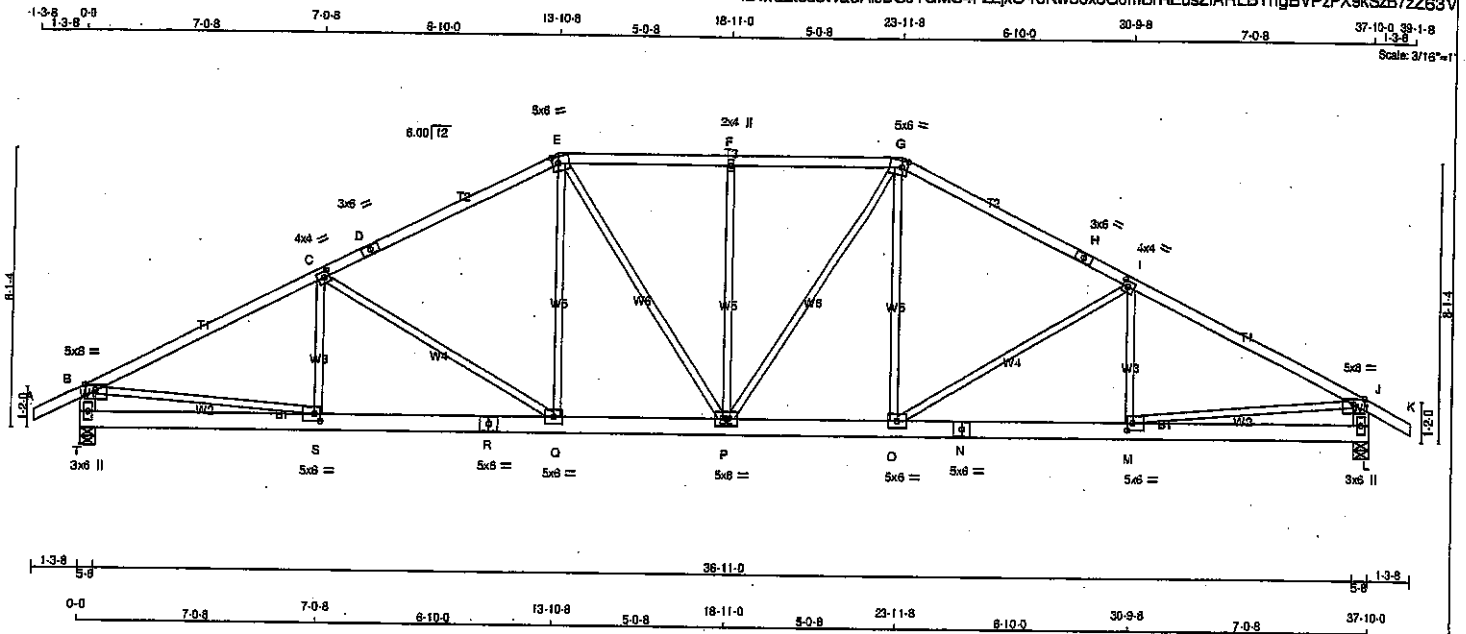
JSI GRIP = 0.88 (B) (INPUT = 0.90)
JSI METAL = 0.65 (Q) (INPUT = 1.00)



Structural component only
DWG# T-2108142

JOB NAME 417443	TRUSS NAME T35	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID:wa2f6usWauAI5BCJTGMC4FZjxO-roKw36x8G0mBrEusZfARLTngBVPzPX9kSzB7zZ63V



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x6	DRY	No.2
T - J	2x6	DRY	No.2
T - R	2x6	DRY	No.2
R - N	2x6	DRY	No.2
N - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	
C TMVW-t	MT20	4.0	4.0	2.00	1.75
D TS-t	MT20	3.0	6.0		
E TTWV-m	MT20	5.0	6.0	2.25	2.00
F TMVW-w	MT20	2.0	4.0		
G TTWV-m	MT20	5.0	6.0	2.25	2.00
H TS-t	MT20	3.0	6.0		
I TMVW-t	MT20	4.0	4.0	2.00	1.75
J TMVW-p	MT20	5.0	8.0	Edge	
L BMV1-p	MT20	3.0	6.0		
M BMVW-t	MT20	5.0	6.0	2.50	2.00
N BS-t	MT20	5.0	6.0		
O BMVW-t	MT20	5.0	6.0		
P BMVW-t	MT20	5.0	6.0		
Q BMVW-t	MT20	5.0	6.0		
R BS-t	MT20	5.0	6.0		
S BMVW-t	MT20	5.0	6.0	2.50	2.00
T BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	IN-SX	BRG	IN-SX
T	2210	0	0	2210	0	0	5-8	5-8	5-8
L	2210	0	0	2210	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1561	1037.0	0.0	0.0	0.0	523.0	0.0
L	1561	1037.0	0.0	0.0	0.0	523.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	CSF (LC)	UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. (LBS)	CSF (LC)
FR-TO						FR-TO			
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	S-C	-204 / 48	0.06 (1)	
B-C	-3212 / 0	-91.8	-91.8	0.89 (1)	3.05	C-Q	-618 / 0	0.87 (1)	
C-D	-2894 / 0	-91.8	-91.8	0.76 (1)	3.42	Q-E	0 / 464	0.10 (1)	
D-E	-2894 / 0	-91.8	-91.8	0.76 (1)	3.42	E-P	0 / 371	0.08 (1)	
E-F	-2586 / 0	-91.8	-91.8	0.36 (1)	3.96	P-F	-563 / 0	0.87 (1)	
F-G	-2586 / 0	-91.8	-91.8	0.36 (1)	3.96	P-G	0 / 371	0.08 (1)	
G-H	-2894 / 0	-91.8	-91.8	0.76 (1)	3.42	O-G	0 / 464	0.10 (1)	
H-I	-2894 / 0	-91.8	-91.8	0.76 (1)	3.42	O-I	-618 / 0	0.87 (1)	
I-J	-3212 / 0	-91.8	-91.8	0.89 (1)	3.05	M-I	-204 / 48	0.06 (1)	
J-K	0 / 28	-91.8	-91.8	0.12 (1)	10.00	B-S	0 / 2921	0.66 (1)	
T-B	-2143 / 0	0.0	0.0	0.14 (1)	6.95	M-J	0 / 2921	0.66 (1)	
L-J	-2143 / 0	0.0	0.0	0.14 (1)	6.95				
T-S	0 / 0	-18.5	-18.5	0.10 (4)	10.00				
S-R	0 / 2903	-18.5	-18.5	0.40 (1)	10.00				
R-Q	0 / 2903	-18.5	-18.5	0.40 (1)	10.00				
Q-P	0 / 2384	-18.5	-18.5	0.33 (1)	10.00				
P-O	0 / 2384	-18.5	-18.5	0.33 (1)	10.00				
O-N	0 / 2903	-18.5	-18.5	0.40 (1)	10.00				
N-M	0 / 2903	-18.5	-18.5	0.40 (1)	10.00				
M-L	0 / 0	-18.5	-18.5	0.10 (4)	10.00				

TOTAL WEIGHT = 2 X 182 = 365 lb (M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF NBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.30")

CSI: TC=0.89/1.00 (I-J:1), BC=0.40/1.00 (M-O:1),
WB=0.87/1.00 (I-O:1), SS=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

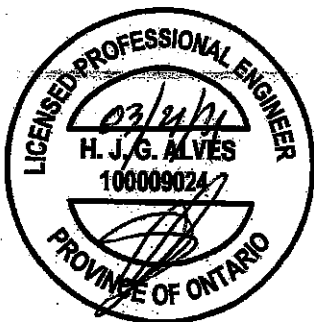
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.66 (M) (INPUT = 1.00)



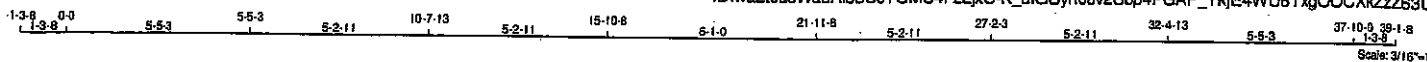
Structural component only
DWG# T-2108143

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T36	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:wa2t6usWauA15BCJTGMCFzZkO-K_uIGSyn0Jv2Sbp4PGAP_YkE4WU8TxgOOCXkZz63U



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
T - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
T - R	2x6	DRY	No.2
R - O	2x6	DRY	No.2
O - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
E - Q	2x4	DRY	No.2
Q - G	2x4	DRY	No.2
P - H	2x4	DRY	No.2
T - C	2x4	DRY	No.2
J - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+P	MT20	4.0	4.0		
C TMWW+T	MT20	5.0	6.0	2.50	2.00
D TS+T	MT20	3.0	6.0		
E TMWW+T	MT20	4.0	4.0	2.00	1.50
F TTW+M	MT20	4.0	6.0		
G TTWW+M	MT20	5.0	6.0	2.25	2.00
H TMWW+T	MT20	4.0	4.0	2.00	1.50
I TS+T	MT20	3.0	6.0		
J TMWW+T	MT20	5.0	6.0	2.50	2.00
K TMV+P	MT20	4.0	4.0		
M BMVW+T	MT20	5.0	8.0		
N, P, S					
N BMWW+T	MT20	5.0	8.0		
O BS+T	MT20	5.0	6.0		
Q BMWW+T	MT20	5.0	8.0		
R BS+T	MT20	5.0	6.0		
T BMVW+T	MT20	5.0	8.0		

NOTES: (1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
		FACTORED		MAXIMUM FACTORED			INPUT	REQRD	
JT	GROSS REACTION		GROSS REACTION		BRG		BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	
T	2210	0	2210	0	0	5-8	5-8		
M	2210	0	2210	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1581	1037	0	0	0	523	0
M	1581	1037	0	0	0	523	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 28	-91.8	-91.8	0.12 (1)	C-S	-110 / 37	0.04 (1)
B-C	0 / 20	-91.8	-91.8	0.32 (1)	S-E	0 / 258	0.06 (4)
C-D	-3071 / 0	-91.8	-91.8	0.43 (1)	E-Q	-670 / 0	0.65 (1)
D-E	-3071 / 0	-91.8	-91.8	0.43 (1)	Q-F	0 / 635	0.14 (1)
E-F	-2488 / 0	-91.8	-91.8	0.39 (1)	G-Q	0 / 3	0.00 (1)
F-G	-2213 / 0	-91.8	-91.8	0.52 (1)	P-G	0 / 631	0.14 (1)
G-H	-2488 / 0	-91.8	-91.8	0.39 (1)	P-H	-672 / 0	0.65 (1)
H-I	-3072 / 0	-91.8	-91.8	0.43 (1)	H-N	0 / 262	0.06 (4)
I-J	-3072 / 0	-91.8	-91.8	0.43 (1)	N-J	-110 / 37	0.04 (1)
J-K	0 / 20	-91.8	-91.8	0.32 (1)	T-C	-3351 / 0	0.64 (1)
K-L	0 / 28	-91.8	-91.8	0.12 (1)	J-M	-3352 / 0	0.64 (1)
T-B	-324 / 0	0.0	0.0	0.02 (1)			
M-K	-324 / 0	0.0	0.0	0.02 (1)			
T-S	0 / 2816	-18.5	-18.5	0.42 (1)			
S-R	0 / 2646	-18.5	-18.5	0.37 (1)			
R-Q	0 / 2646	-18.5	-18.5	0.37 (1)			
Q-P	0 / 2211	-18.5	-18.5	0.31 (1)			
P-O	0 / 2645	-18.5	-18.5	0.37 (1)			
O-N	0 / 2645	-18.5	-18.5	0.37 (1)			
N-M	0 / 2816	-18.5	-18.5	0.41 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.521/0 (F-G:1), BC=0.42/1.00 (S-T:1), WB=0.66/1.00 (H-P:1), SS=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

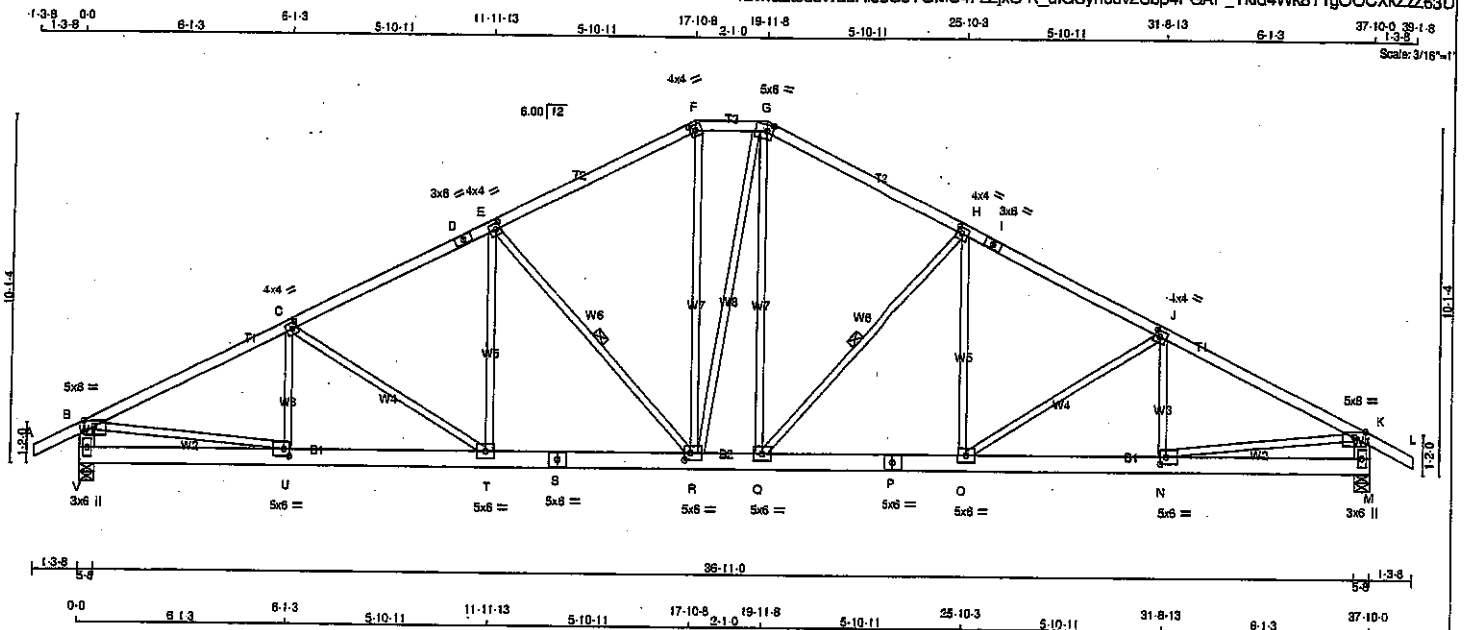
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.75 (J) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
417443	T37	2	1	GREENPARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:wa2t6usWauA5BCJTGMCFZzXO-K_uIGSyn0Jv2Sbp4PGAP_Ykld4Wk8T1gOOCXkZz83U



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
V - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	8.0 Edge
C, E, H, J				
C	TMVW-t	MT20	4.0	4.0 2.00 1.75
D	TS-t	MT20	3.0	6.0
F	TTW-h	MT20	4.0	4.0 2.00 1.75
G	TTWV-m	MT20	5.0	6.0 2.25 2.00
I	TS-t	MT20	3.0	6.0
K	TMVW-p	MT20	5.0	8.0 Edge
M	BMV1-p	MT20	3.0	6.0
N	BMVW-t	MT20	5.0	6.0 2.50 2.00
O, Q, T				
O	BMVW-t	MT20	5.0	6.0
P	BS-t	MT20	5.0	6.0
R	BMVW-t	MT20	5.0	6.0 2.50 2.00
S	BS-t	MT20	5.0	6.0
U	BMVW-t	MT20	5.0	6.0 2.50 2.00
V	BMV1-p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
V	2210	0	2210	0	5-8	5-8
M	2210	0	2210	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	IN-SX			
V	1561	1037 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0
M	1561	1037 / 0	0 / 0	0 / 0	0 / 0	523 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-R, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	U-C	-267 / 7	0.07 (1)
B-C	-3177 / 0	-91.8 -91.8	0.62 (1)	3.42	C-T	-347 / 0	0.32 (1)
C-D	-2875 / 0	-91.8 -91.8	0.50 (1)	3.70	T-E	0 / 293	0.07 (1)
D-E	-2875 / 0	-91.8 -91.8	0.50 (1)	3.70	E-R	-823 / 0	0.46 (1)
E-F	-2290 / 0	-91.8 -91.8	0.47 (1)	4.08	R-F	0 / 703	0.16 (1)
F-G	-2034 / 0	-91.8 -91.8	0.09 (1)	4.69	R-G	0 / 59	0.01 (1)
G-H	-2278 / 0	-91.8 -91.8	0.47 (1)	4.09	Q-G	0 / 839	0.14 (1)
H-I	-2880 / 0	-91.8 -91.8	0.50 (1)	3.70	Q-H	-846 / 0	0.47 (1)
I-J	-2880 / 0	-91.8 -91.8	0.50 (1)	3.70	O-H	0 / 318	0.07 (1)
J-K	-3175 / 0	-91.8 -91.8	0.62 (1)	3.42	O-J	-340 / 0	0.31 (1)
K-L	0 / 28	-91.8 -91.8	0.12 (1)	10.00	N-J	-273 / 4	0.07 (1)
V-B	-2148 / 0	0.0 0.0	0.14 (1)	6.94	B-U	0 / 2885	0.65 (1)
M-K	-2147 / 0	0.0 0.0	0.14 (1)	6.94	N-K	0 / 2883	0.65 (1)
V-U	0 / 0	-18.5 -18.5	0.08 (4)	10.00			
U-T	0 / 2862	-18.5 -18.5	0.40 (1)	10.00			
T-S	0 / 2571	-18.5 -18.5	0.37 (1)	10.00			
S-R	0 / 2571	-18.5 -18.5	0.37 (1)	10.00			
R-Q	0 / 2022	-18.5 -18.5	0.30 (1)	10.00			
Q-P	0 / 2576	-18.5 -18.5	0.35 (1)	10.00			
P-O	0 / 2576	-18.5 -18.5	0.35 (1)	10.00			
O-N	0 / 2860	-18.5 -18.5	0.39 (1)	10.00			
N-M	0 / 0	-18.5 -18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.62/1.00 (B-C:1), BC=0.40/1.00 (T-U:1), WB=0.65/1.00 (B-U:1), SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	850	371	1747 769 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

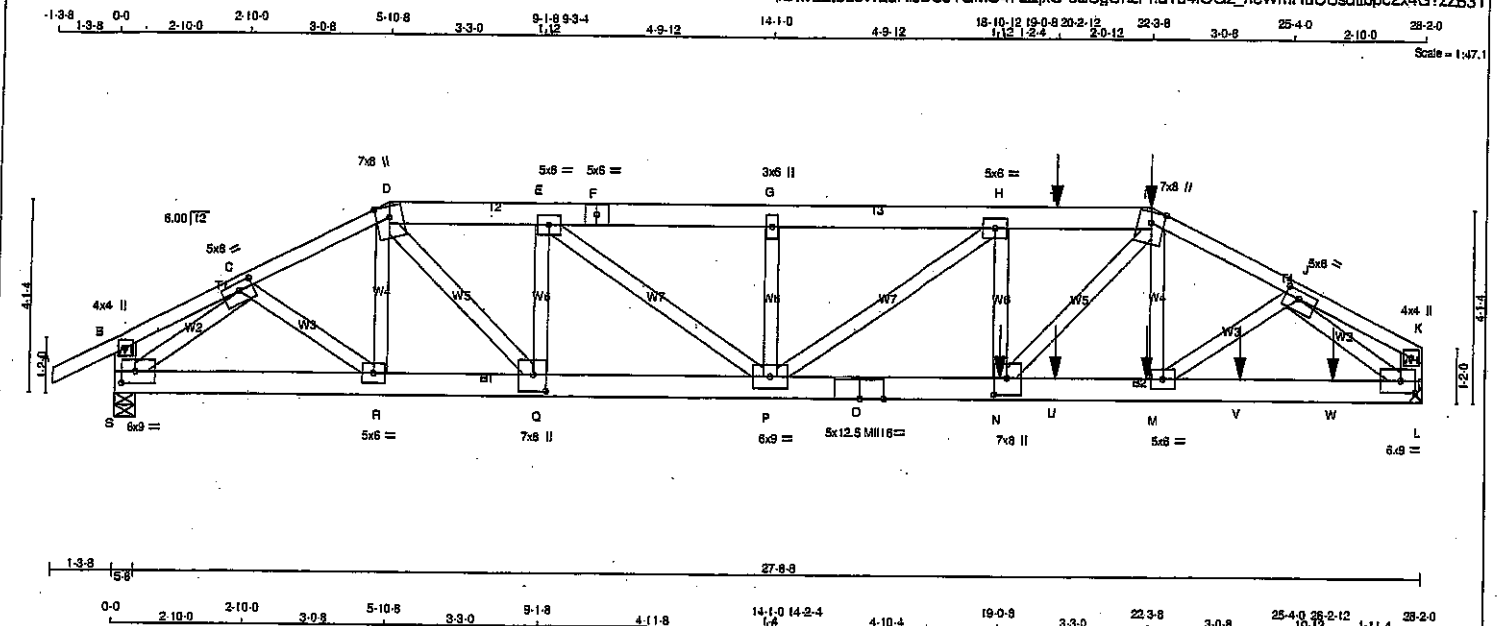
JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.65 (U) (INPUT = 1.00)



JOB NAME 417443	TRUSS NAME T38	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER						
N. L. G. A. RULES						
CHORDS		SIZE	LUMBER		DESCR	
A - D	2x4	DRY	No.2			SPF
D - F	2x6	DRY	No.2			SPF
F - I	2x6	DRY	No.2			SPF
I - K	2x4	DRY	No.2			SPF
S - B	2x6	DRY	No.2			SPF
L - K	2x6	DRY	No.2			SPF
S - O	2x6	DRY	2100F 1.8E			SPF
O - L	2x6	DRY	2100F 1.8E			SPF

ALL WEBS 2x4 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	4.0	4.0		
C	TMVW+T	MT20	5.0	8.0	2.00	3.50
D	TTVW+M	MT20	7.0	8.0	Edge	
E	TMVW+T	MT20	5.0	8.0		
F	TS+T	MT20	5.0	8.0		
G	TMVW+M	MT20	3.0	8.0		
H	TMVW+T	MT20	5.0	8.0		
I	TTVW+M	MT20	7.0	8.0	Edge	
J	TMVW+T	MT20	5.0	8.0	2.00	3.50
K	TMV+P	MT20	4.0	4.0		
L	BMVW+T	MT20	6.0	9.0	3.00	3.75
M	BMVW+T	MT20	5.0	6.0		
N	BMVW+T	MT20	7.0	8.0	4.25	3.25
O	BS+T	MT16	5.0	12.5		
P	BMVW+T	MT20	6.0	9.0		
Q	BMVW+T	MT20	7.0	8.0	4.25	3.25
R	BMVW+T	MT20	5.0	6.0		
S	BMVW+T	MT20	6.0	9.0	3.00	3.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	UPLIFT	IN-SX
L 3406 0	3406 0	0	MECHANICAL
S 2478 0	2478 0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM. LIVE
L 2408 1582 0	0 0	0 0	826 0
S 1749 1189 0	0 0	0 0	580 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	W E B S	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO		FROM TO		FR-TO		
A-B	0 / 28	-91.8 -91.8 0.13 (1)	10.00	R-D	-225 / 0	0.04 (1)
B-C	0 / 10	-91.8 -91.8 0.10 (1)	10.00	M-I	-348 / 0	0.06 (1)
C-D	-3803 / 0	-91.8 -91.8 0.31 (1)	3.42	C-R	0 / 522	0.09 (1)
D-E	-4659 / 0	-91.8 -91.8 0.20 (1)	3.88	S-C	-3561 / 0	0.56 (1)
E-F	-6115 / 0	-91.8 -91.8 0.35 (1)	3.28	M-J	0 / 826	0.16 (1)
F-G	-6115 / 0	-91.8 -91.8 0.36 (1)	3.28	J-L	-3222 / 0	0.82 (1)
G-H	-6115 / 0	-91.8 -91.8 0.38 (1)	3.28	Q-E	-1519 / 0	0.26 (1)
H-T	-6788 / 0	-91.8 -91.8 0.40 (1)	3.07	D-Q	0 / 2147	0.38 (1)
T-I	-6788 / 0	-91.8 -91.8 0.40 (1)	3.07	N-H	-20 / 57	0.02 (4)
I-J	-5422 / 0	-91.8 -91.8 0.55 (1)	2.54	N-I	0 / 2881	0.51 (1)
J-K	0 / 8	-91.8 -91.8 0.10 (1)	10.00	P-H	-832 / 0	0.38 (1)
S-B	-227 / 0	0.0 0.0 0.02 (1)	7.81	E-P	0 / 1808	0.32 (1)
L-K	-109 / 0	0.0 0.0 0.01 (1)	7.81	P-G	-479 / 0	0.08 (1)

S-R	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
R-Q	0 / 3237	-18.5	-18.5 0.18 (1)	10.00				
Q-P	0 / 4659	-18.5	-18.5 0.28 (1)	10.00				
P-O	0 / 6786	-18.5	-18.5 0.42 (1)	10.00				
O-N	0 / 6786	-18.5	-18.5 0.42 (1)	10.00				
N-U	0 / 4878	-18.5	-18.5 0.31 (1)	10.00				
U-M	0 / 4878	-18.5	-18.5 0.31 (1)	10.00				
M-V	0 / 4111	-18.5	-18.5 0.28 (1)	10.00				
V-W	0 / 4111	-18.5	-18.5 0.28 (1)	10.00				
W-L	0 / 4111	-18.5	-18.5 0.28 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
I	22-3-8	-231	-231	FRONT	VERT	TOTAL		C1
M	22-2-12	-21	-21	FRONT	VERT	TOTAL		C1
N	19-0-8	-1484	-1484	FRONT	VERT	TOTAL		C1
T	20-2-12	-76	-76	FRONT	VERT	TOTAL		C1
U	20-2-12	-21	-21	FRONT	VERT	TOTAL		C1
V	24-2-12	-21	-21	FRONT	VERT	TOTAL		C1
W	26-2-12	-20	-20	FRONT	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/880 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/380 (0.94")
CALCULATED VERT. DEFL.(TL) = L/956 (0.35")

CSI: TC=0.55/1.00 (F-J:1), BC=0.42/1.00 (N-P:1),
WB=0.82/1.00 (J-L:1), SS=0.16/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR	SECTION (PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
MM16	438	302	2547

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (S) (INPUT = 0.90)
JSI METAL= 0.99 (C) (INPUT = 1.00)

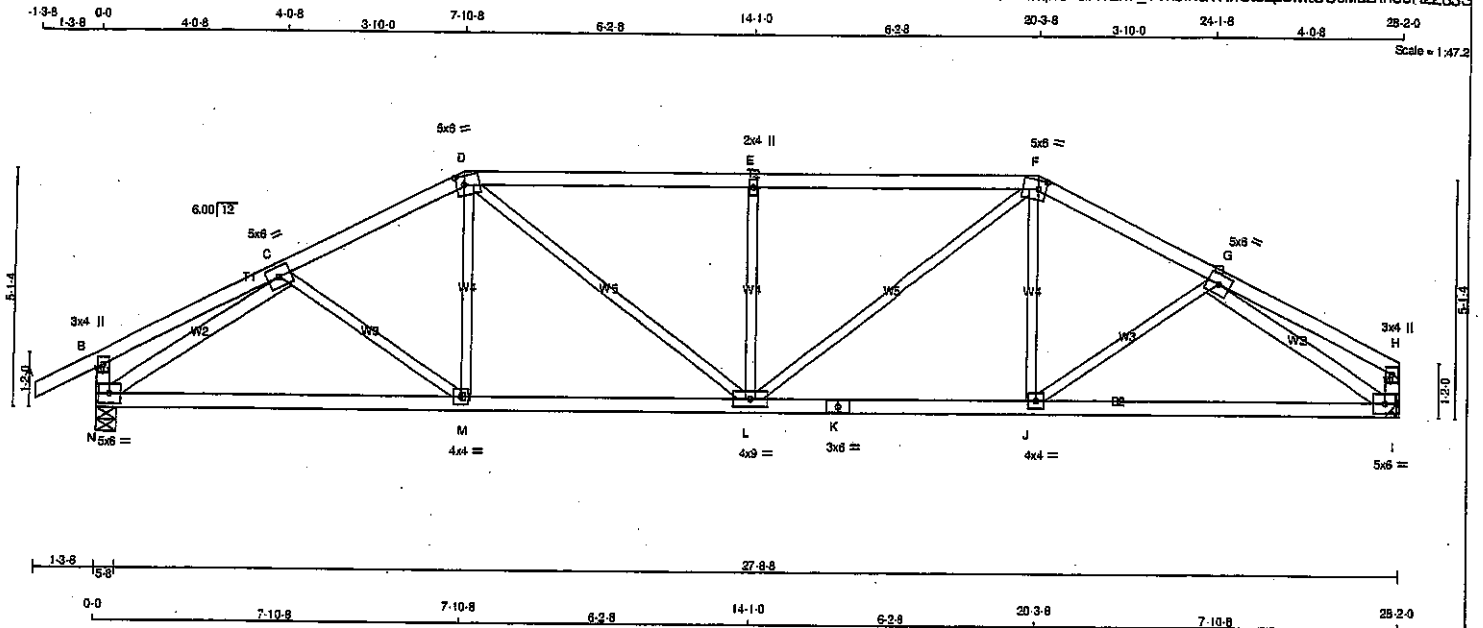


Structural component only
DWG# T-2108146

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T39	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 111 lb (M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - C	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TTWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	9.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
N	1677	0	1677	0	0	5-8	5-8		
I	1553	0	1553	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1184	790.0	0.0	0.0	0.0	394.0	0.0
I	1098	721.0	0.0	0.0	0.0	377.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 28	C-M	-35 / 52
B-C	0 / 17	D-L	0 / 178
C-D	-2091 / 0	E-F	0 / 640
D-E	-2363 / 0	F-G	-899 / 0
E-F	-2363 / 0	G-H	0 / 640
F-G	-2091 / 0	H-I	0 / 178
G-H	0 / 17	I-J	-35 / 52
H-I	-288 / 0	J-K	-2315 / 0
I-J	-144 / 0	K-L	-2315 / 0
N-M	0 / 1882		
M-L	0 / 1857		
L-K	0 / 1857		
K-J	0 / 1857		
J-I	0 / 1882		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.54/1.00 (D-E:1), BC=0.45/1.00 (M-N:1),
WB=0.84/1.00 (C-N:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

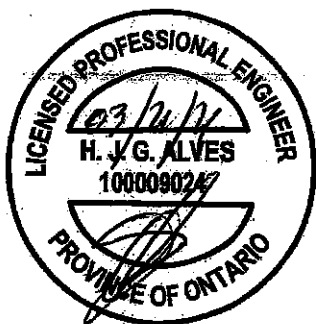
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.83 (M) (INPUT = 0.90)
JSI METAL = 0.60 (K) (INPUT = 1.00)



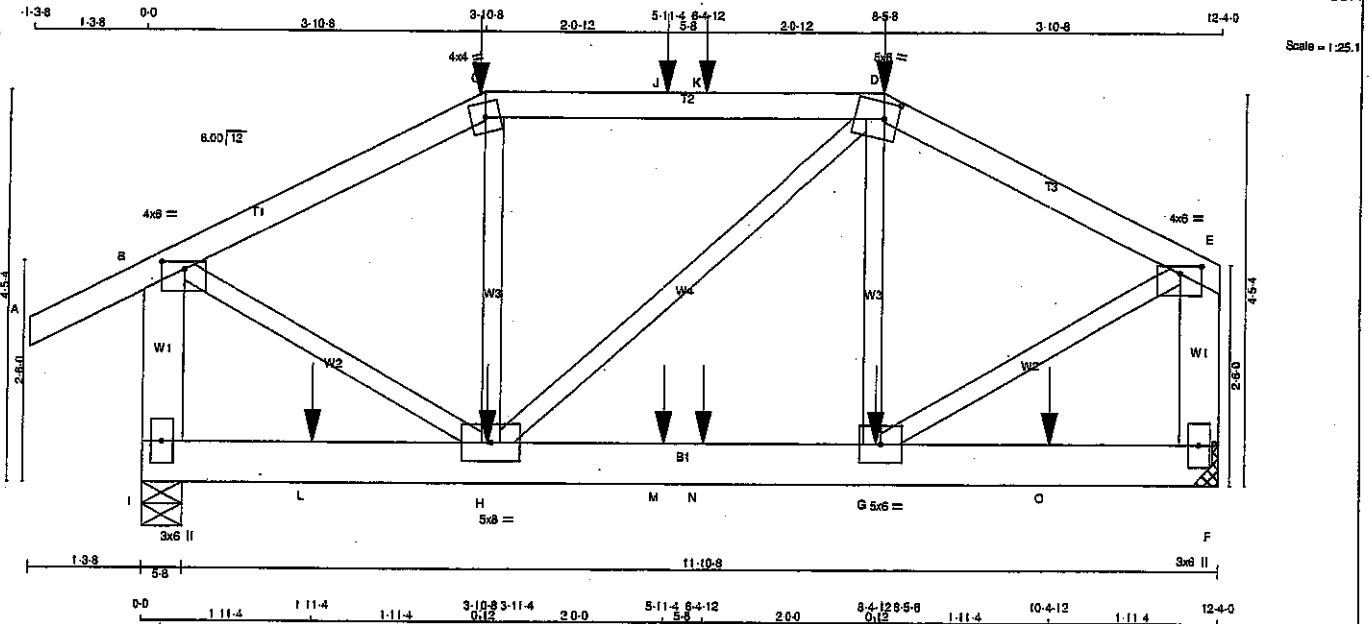
Structural component only
DWG# T-2108147

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T40	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 63 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x6	DRY	No.2	SPF	
F - G	2x6	DRY	No.2	SPF	
G - H	2x6	DRY	No.2	SPF	
H - I	2x6	DRY	No.2	SPF	
I - J	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TTW-m	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-p	MT20	4.0	6.0	1.00	3.00
F	BMV1-p	MT20	3.0	6.0		
G	BMVW-t	MT20	5.0	6.0		
H	BMVWW-t	MT20	5.0	8.0		
I	BMV1-p	MT20	3.0	6.0		

NOTES- (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
I	1132	0	1132	0	5-8	5-8
F	1008	0	1008	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	797	542.0	0.0	0.0	0.0	255.0	0.0
F	712	473.0	0.0	0.0	0.0	239.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
		LC1	LC2				
FR-TO					FR-TO		
A-B	0.28	-91.8	-91.8	0.13 (1)	H-C	-171.0	0.05 (1)
B-C	-884.0	-91.8	-91.8	0.28 (1)	H-D	-3.0	0.00 (1)
C-J	-788.0	-91.8	-91.8	0.41 (1)	G-D	-167.0	0.05 (1)
J-K	-788.0	-91.8	-91.8	0.41 (1)	B-H	0.874	0.22 (1)
K-D	-788.0	-91.8	-91.8	0.41 (1)	G-E	0.877	0.22 (1)
D-E	-867.0	-91.8	-91.8	0.28 (1)			
I-B	-1099.0	0.0	0.0	0.10 (1)			
F-E	-877.0	0.0	0.0	0.09 (1)			
I-L	0.0	-18.5	-18.5	0.05 (4)			
L-H	0.0	-18.5	-18.5	0.05 (4)			
H-M	0.771	-18.5	-18.5	0.19 (1)			
M-N	0.771	-18.5	-18.5	0.19 (1)			
N-G	0.771	-18.5	-18.5	0.19 (1)			
G-O	0.0	-18.5	-18.5	0.05 (4)			
O-F	0.0	-18.5	-18.5	0.05 (4)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-113	-113		BACK	VERT	TOTAL		C1
D	8-5-8	-113	-113		BACK	VERT	TOTAL		C1
G	8-4-12	-48	-48		BACK	VERT	TOTAL		C1
H	3-11-4	-48	-48		BACK	VERT	TOTAL		C1
J	5-11-4	-8	-8		BACK	VERT	TOTAL		C1
K	6-4-12	-8	-8		BACK	VERT	TOTAL		C1
L	1-11-4	-14	-14		BACK	VERT	TOTAL		C1
M	5-11-4	-48	-48		BACK	VERT	TOTAL		C1
N	6-4-12	-48	-48		BACK	VERT	TOTAL		C1
O	10-4-12	-14	-14		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN.CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.41/1.00 (C-D:1), BC=0.19/1.00 (G-H:1), WB=0.22/1.00 (E-G:1), SS=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

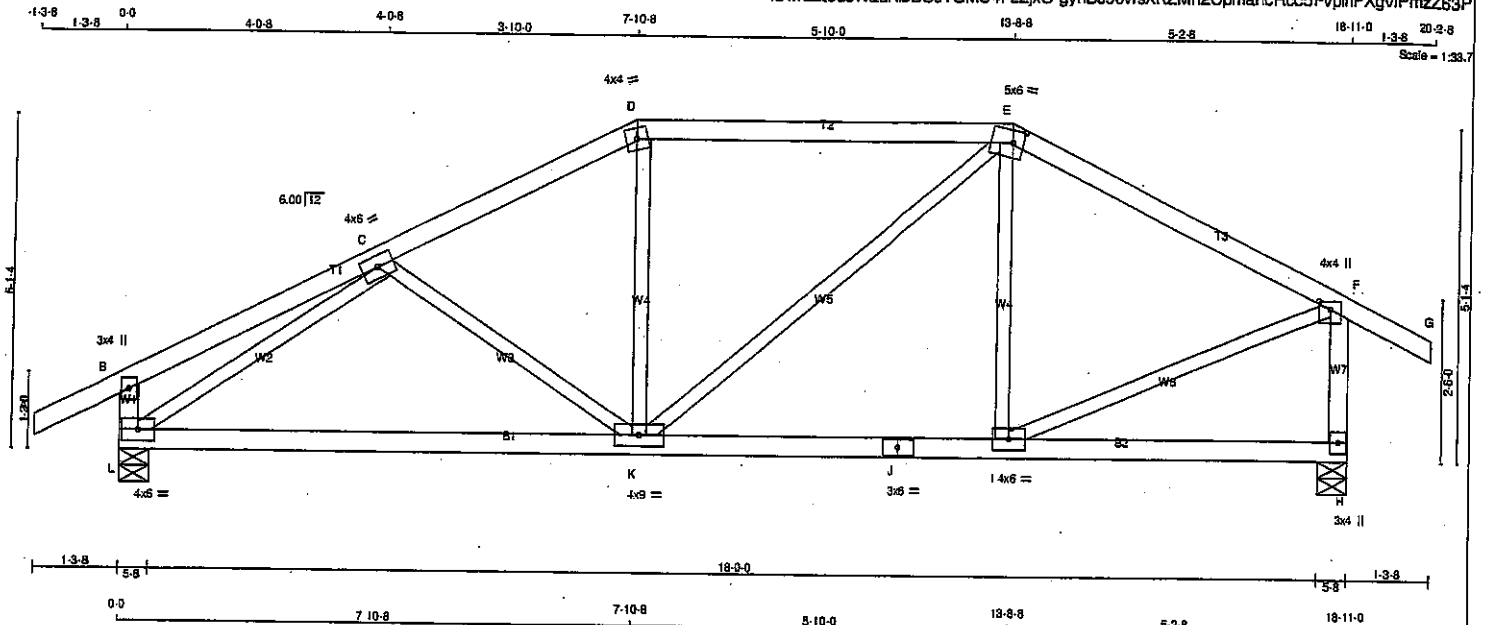
JSI GRIP = 0.55 (B) (INPUT = 0.90)
JSI METAL = 0.21 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108148

JOB NAME 417443	TRUSS NAME T42	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 77 b

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF
L - B	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
L - J	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-t	MT20	4.0	6.0
D	TRW-m	MT20	4.0	4.0
E	TTWW-m	MT20	5.0	6.0 2.25 2.00
F	TMVW+p	MT20	4.0	4.0 1.50 2.00
H	BMV1+p	MT20	3.0	4.0
I	BMWW-t	MT20	4.0	6.0
J	BS-t	MT20	3.0	6.0
K	BMWWW-t	MT20	4.0	9.0
L	BMVW1-t	MT20	4.0	6.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	1167	0	1167	0	5-8
H	1167	0	1167	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
L	823	553.0	0.0
H	823	553.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
	FORCE (LBS)		FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0.28	C-K	-192.0
B-C	0.18	K-D	0.106
C-D	-1152.0	E-F	0.245
D-E	-1016.0	F-G	-248.0
E-F	-933.0	G-H	-1441.0
F-G	0.28	H-I	0.902
G-H	-266.0	I-J	0.03
H-I	-1125.0	J-K	0.03
		K-L	0.03
		L-M	0.03
		M-N	0.03
		N-O	0.03
		O-P	0.03
		P-Q	0.03
		Q-R	0.03
		R-S	0.03
		S-T	0.03
		T-U	0.03
		U-V	0.03
		V-W	0.03
		W-X	0.03
		X-Y	0.03
		Y-Z	0.03
		Z-A	0.03

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.42/1.00 (D-E:1), BC=0.34/1.00 (K-L:4), WB=0.55/1.00 (C-L:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

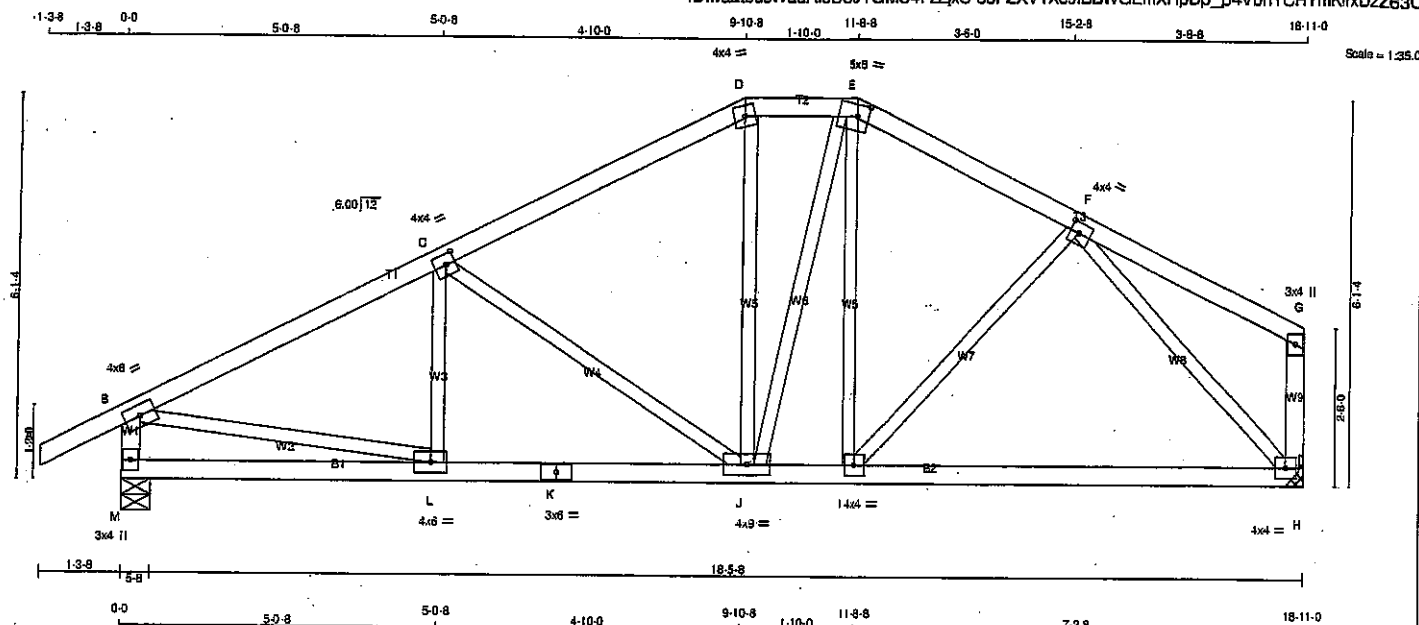
JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T43	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:wa216usWauA5BGJTGMC4FzZjxO-88FZXV1Xc9fBBWGEHXHpDp_p4VbhYCHYmKfxDzZ63C



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	4.0	6.0
O	TMVW-t	MT20	4.0	4.0 2.00 1.75
D	TTW-m	MT20	4.0	4.0
E	TTWV-m	MT20	5.0	6.0 2.25 2.00
F	TMVW-t	MT20	4.0	4.0 2.00 1.75
G	TMV+p	MT20	3.0	4.0
H	BMVW-t	MT20	4.0	4.0
I	BMVW-t	MT20	4.0	4.0
J	BMVW-t	MT20	4.0	9.0
K	BS-t	MT20	3.0	6.0
L	BMVW-t	MT20	4.0	6.0
M	BMV+p	MT20	3.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1167	0	1167	0	0	5-8	5-8
H	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LOOSE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	823	553 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
H	737	484 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-91.8 -91.8	0.12 (1)	10.00	L-C	-106 / 51	0.02 (1)
B-C	-1352 / 0	-91.8 -91.8	0.31 (1)	5.24	C-J	-466 / 0	0.29 (1)
C-D	-970 / 0	-91.8 -91.8	0.29 (1)	5.96	J-D	0 / 173	0.04 (1)
D-E	-949 / 0	-91.8 -91.8	0.05 (1)	6.25	J-E	-7 / 104	0.02 (1)
E-F	-931 / 0	-91.8 -91.8	0.15 (1)	6.24	I-E	0 / 113	0.04 (4)
F-G	0 / 17	-91.8 -91.8	0.19 (1)	10.00	I-F	0 / 69	0.02 (4)
M-B	-1128 / 0	0.0 0.0	0.11 (1)	7.48	B-L	0 / 1248	0.28 (1)
H-G	-128 / 0	0.0 0.0	0.02 (1)	7.81	F-H	-1159 / 0	0.54 (1)
M-L	0 / 0	-18.5 -18.5	0.11 (4)	10.00			
L-K	0 / 1230	-18.5 -18.5	0.25 (1)	10.00			
K-J	0 / 1230	-18.5 -18.5	0.25 (1)	10.00			
J-I	0 / 819	-18.5 -18.5	0.30 (4)	10.00			
I-H	0 / 776	-18.5 -18.5	0.29 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 085-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.63")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.63")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.10")

CSI: TO=0.31/1.00 (B-C:1), BC=0.30/1.00 (I-J:4), WB=0.54/1.00 (F-H:1), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

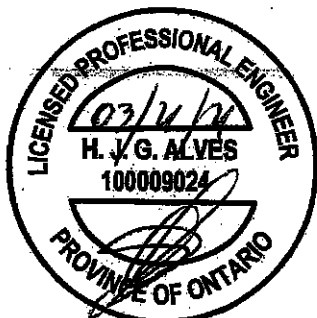
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT-TOL = 0.250 inches

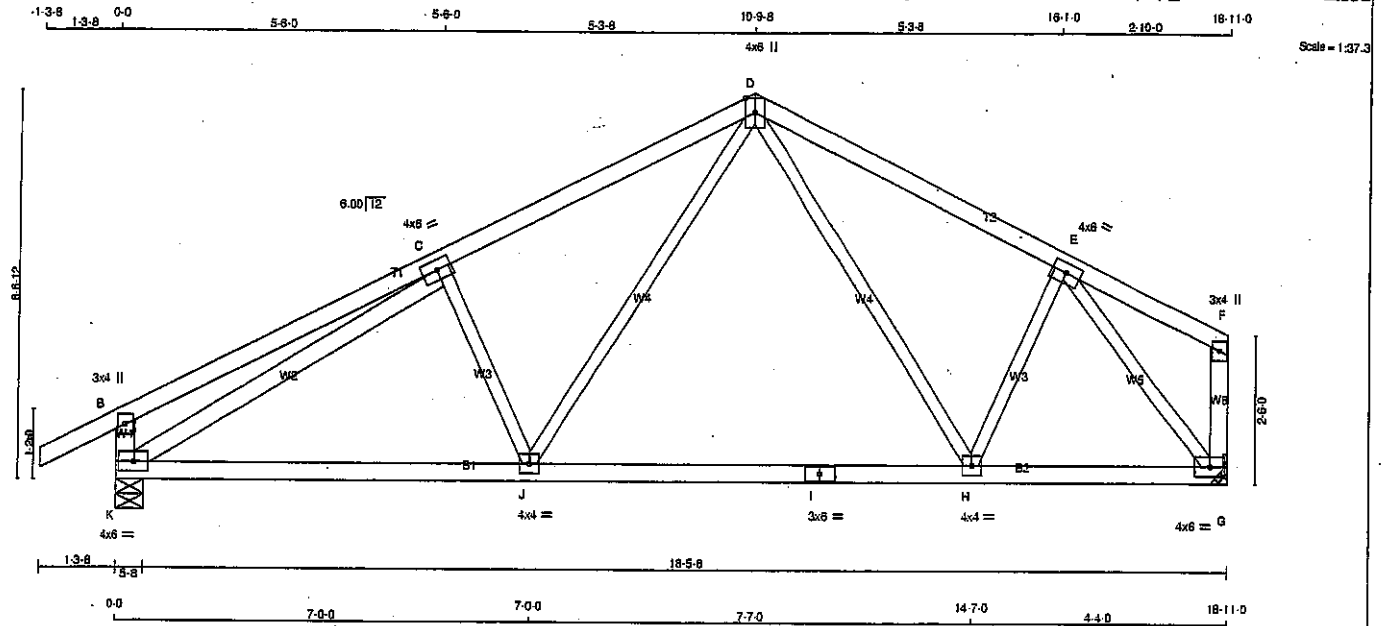
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (H) (INPUT = 0.90)
JSI METAL = 0.41 (K) (INPUT = 1.00)



Structural component only
DWG# T-2108151

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	T44	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						
Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:52:53 2021 Page 1						
ID:wa2t6usWauAI5BCJTGMC4FzZjXO-88FZXV1Xc9fBBWGEEmXHpDp_n3Va4Y9nYmKfnDz263C						



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT	K - C	2x4	DRY
K - C			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTWW+p	MT20	4.0	6.0	Edge
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
G	BMVW1-t	MT20	4.0	6.0	
H	BMWW-t	MT20	4.0	4.0	
I	BS-t	MT20	3.0	6.0	
J	BMWW-t	MT20	4.0	4.0	
K	BMVW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
K	1167	0	1167	0	0
G	1043	0	1043	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	823	553 / 0	0 / 0	0 / 0	0 / 0	270 / 0	0 / 0
G	737	484 / 0	0 / 0	0 / 0	0 / 0	253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 28	-91.8	-91.8 0.12 (1)	10.00	C-J	-374 / 0	0.10 (1)
B-C	0 / 26	-91.8	-91.8 0.43 (1)	10.00	J-D	0 / 577	0.13 (1)
C-D	-1215 / 0	-91.8	-91.8 0.36 (1)	5.38	D-H	0 / 57	0.02 (4)
D-E	-902 / 0	-91.8	-91.8 0.31 (1)	6.08	H-E	0 / 94	0.03 (4)
E-F	0 / 38	-91.8	-91.8 0.32 (1)	10.00	K-C	-1460 / 0	0.76 (1)
K-B	-313 / 0	0.0	0.0 0.03 (1)	7.81	E-G	-1232 / 0	0.39 (1)
G-F	-43 / 0	0.0	0.0 0.01 (1)	7.81			
K-J	0 / 1219	-18.5	-18.5 0.34 (4)	10.00			
J-I	0 / 764	-18.5	-18.5 0.30 (4)	10.00			
I-H	0 / 784	-18.5	-18.5 0.30 (4)	10.00			
H-G	0 / 755	-18.5	-18.5 0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.43/1.00 (B-C:1), BC=0.34/1.00 (J-K:4), WB=0.78/1.00 (C-K:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

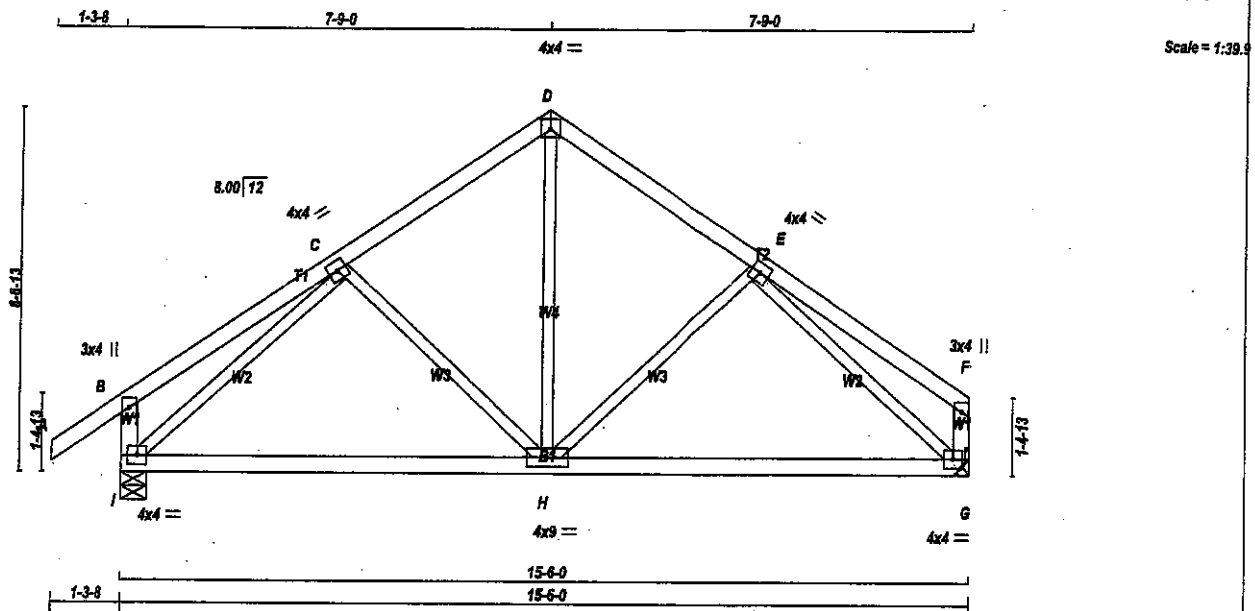
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)



Structural component only
DWG# T-2108152



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV-p	MT20	3.0	4.0	
C	TMVW-1	MT20	4.0	4.0	
D	TTW-p	MT20	4.0	4.0	2.25 2.00
E	TMVW-1	MT20	4.0	4.0	
F	TMV-p	MT20	3.0	4.0	
G	BMVW-1	MT20	4.0	4.0	
H	BMVW-1	MT20	4.0	9.0	
I	BMVW-1	MT20	4.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	GROSS REACTION	BRG	BRG
I	980	0	980	0	5-8
G	854	0	854	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	691	467 / 0	0 / 0	0 / 0	0 / 0	224 / 0	0 / 0
G	604	397 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED	MEMB.	MAX. FACTORED
	FORCE (LBS)		FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 35	H-D	0 / 460
B-C	0 / 23	H-E	-208 / 0
C-D	-885 / 0	C-H	-208 / 0
D-E	-885 / 0	I-C	-965 / 0
E-F	0 / 23	E-G	-965 / 0
I-B	-262 / 0		
G-F	-135 / 0		
I-H	0 / 701		
H-G	0 / 701		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPI 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.22/1.00 (B-C-1), BC=0.37/1.00 (H-I-4), WB=0.44/1.00 (C-I-1), SS=0.14/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

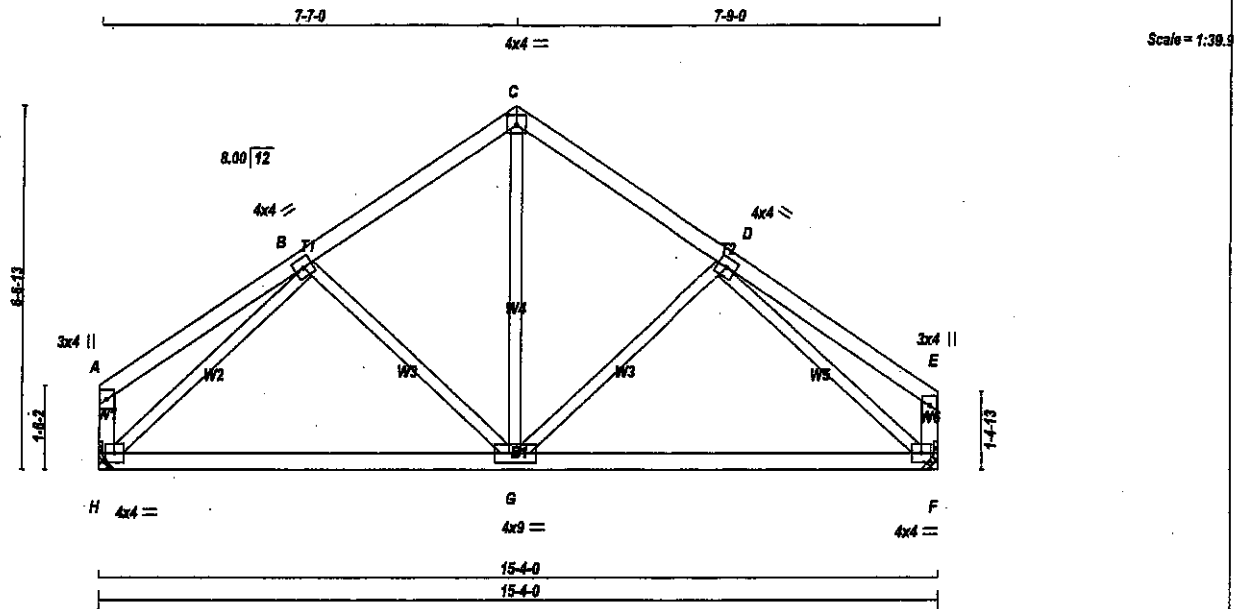
JSI GRIP = 0.77 (G) (INPUT = 0.90)
JSI METAL = 0.31 (C) (INPUT = 1.00)



JOB NAME 421219	TRUSS NAME T351A	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 5 Jan 21 2021 MiTek Industries, Inc. Tue Aug 24 17:17:32 2021 Page 1
ID:GV6EnpZB3IFFw?AU1oPMDZzmp-wyfrV11jSKZuPsRLcQIAMRSxyJxStg58gLYkamit



TOTAL WEIGHT = 82 lb

<u>LUMBER</u>			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMVW1-t	MT20	4.0	9.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	845	0	845	0
F	845	0	845	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	598	392 / 0	0 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
F	598	392 / 0	0 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	MAX. UNBRAC	MEMB.	WEBS	MAX. FACTORED	FORCE	MAX
FR-TO			(LBS)	(PLF)	CS1 (LC)	LENGTH	FR-TO			(LBS)	CS1 (LC)	
A-B	0 / 23	-91.8	-91.8	0.21 (1)	10.00	B-G	-181 / 0	0.09 (1)				
B-C	-670 / 0	-91.8	-91.8	0.16 (1)	6.25	G-C	0 / 442	0.10 (1)				
C-D	-671 / 0	-91.8	-91.8	0.17 (1)	6.25	G-D	-209 / 0	0.10 (1)				
D-E	0 / 23	-91.8	-91.8	0.22 (1)	10.00	H-B	-943 / 0	0.41 (1)				
H-A	-129 / 0	0.0	0.0	0.01 (1)	7.81	D-F	-951 / 0	0.44 (1)				
F-E	-136 / 0	0.0	0.0	0.01 (1)	7.81							
H-G	0 / 671	-18.5	-18.5	0.36 (4)	10.00							
G-F	0 / 691	-18.5	-18.5	0.37 (4)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CS1: TC=0.22/1.00 (D-E:1), BC=0.37/1.00 (F-G:4),
WB=0.44/1.00 (D-F:1), SS=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

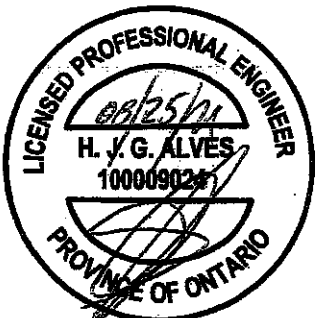
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.76 (F) (INPUT = 0.90)
JSI METAL = 0.30 (D) (INPUT = 1.00)

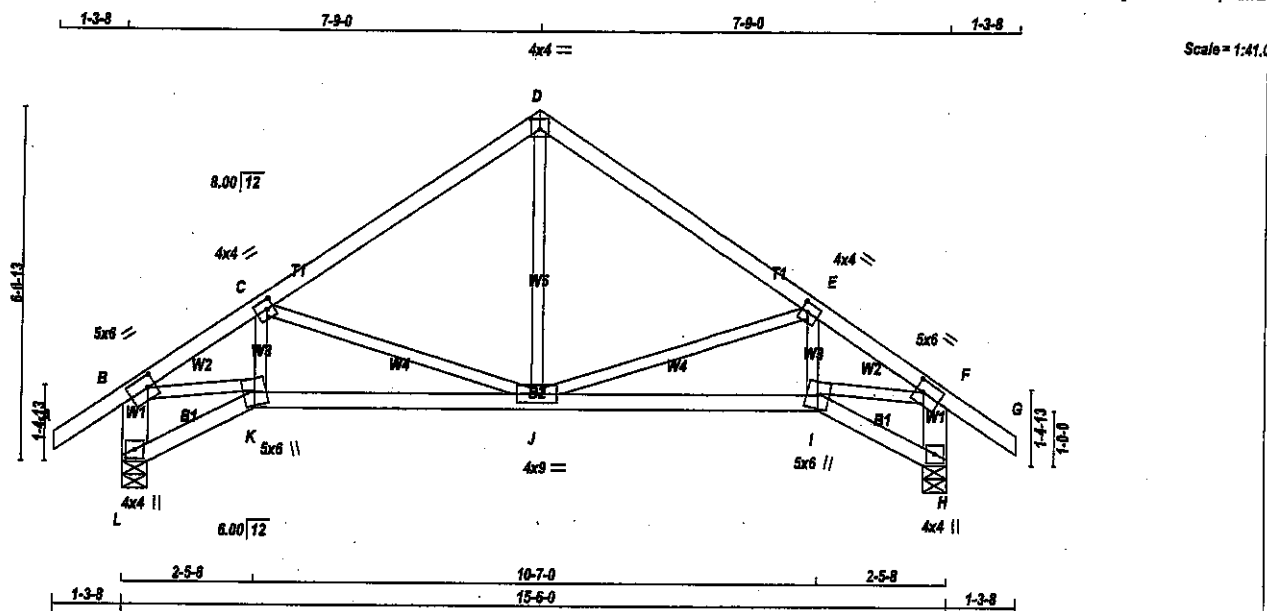


Structural component only
DWG# T-2129742

JOB NAME 421219	TRUSS NAME T351S	QUANTITY 3	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Scale = 1/4" = 1'-0"

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - G	2x4 DRY	No.2	SPF
L - B	2x6 DRY	No.2	SPF
H - F	2x6 DRY	No.2	SPF
L - K	2x4 DRY	No.2	SPF
K - I	2x4 DRY	No.2	SPF
I - H	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-t	MT20	5.0	8.0 2.50 1.75
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TTW-p	MT20	4.0	4.0 2.25 2.00
E	TMVW-t	MT20	4.0	4.0 2.00 1.50
F	TMVW-t	MT20	5.0	8.0 2.50 1.75
H	BMV1+p	MT20	4.0	4.0
I	BMVW+m	MT20	5.0	8.0
J	BMVW+m	MT20	4.0	9.0
K	BMVW+m	MT20	5.0	8.0
L	BMV1+p	MT20	4.0	4.0

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT				
L	980	0	5-8	5-8
H	980	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LOSE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	691	467 / 0	0 / 0	0 / 0	0 / 0	224 / 0	0 / 0
H	691	467 / 0	0 / 0	0 / 0	0 / 0	224 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-91.8 -91.8	0.12 (1)	J-D	0 / 481	0.10 (1)	
B-C	-1328 / 0	-91.8 -91.8	0.23 (1)	J-E	-527 / 0	0.27 (1)	
C-D	-795 / 0	-91.8 -91.8	0.30 (1)	I-E	0 / 57	0.02 (4)	
D-E	-795 / 0	-91.8 -91.8	0.30 (1)	C-J	-527 / 0	0.27 (1)	
E-F	-1328 / 0	-91.8 -91.8	0.23 (1)	K-C	0 / 57	0.02 (4)	
F-G	0 / 35	-91.8 -91.8	0.12 (1)	B-K	0 / 1143	0.26 (1)	
L-B	-958 / 0	0.0 0.0	0.07 (1)	I-F	0 / 1143	0.26 (1)	
H-F	-958 / 0	0.0 0.0	0.07 (1)				
L-K	0 / 0	-18.5 -18.5	0.03 (4)				
K-J	0 / 1141	-18.5 -18.5	0.27 (1)				
J-I	0 / 1141	-18.5 -18.5	0.27 (1)				
I-H	0 / 0	-18.5 -18.5	0.03 (4)				

TOTAL WEIGHT = 3 X 66 = 198 lb

(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(65% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.30/1.00 (D-E-1), BC=0.27/1.00 (I-J-1), WB=0.27/1.00 (E-J-1), SS=0.18/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

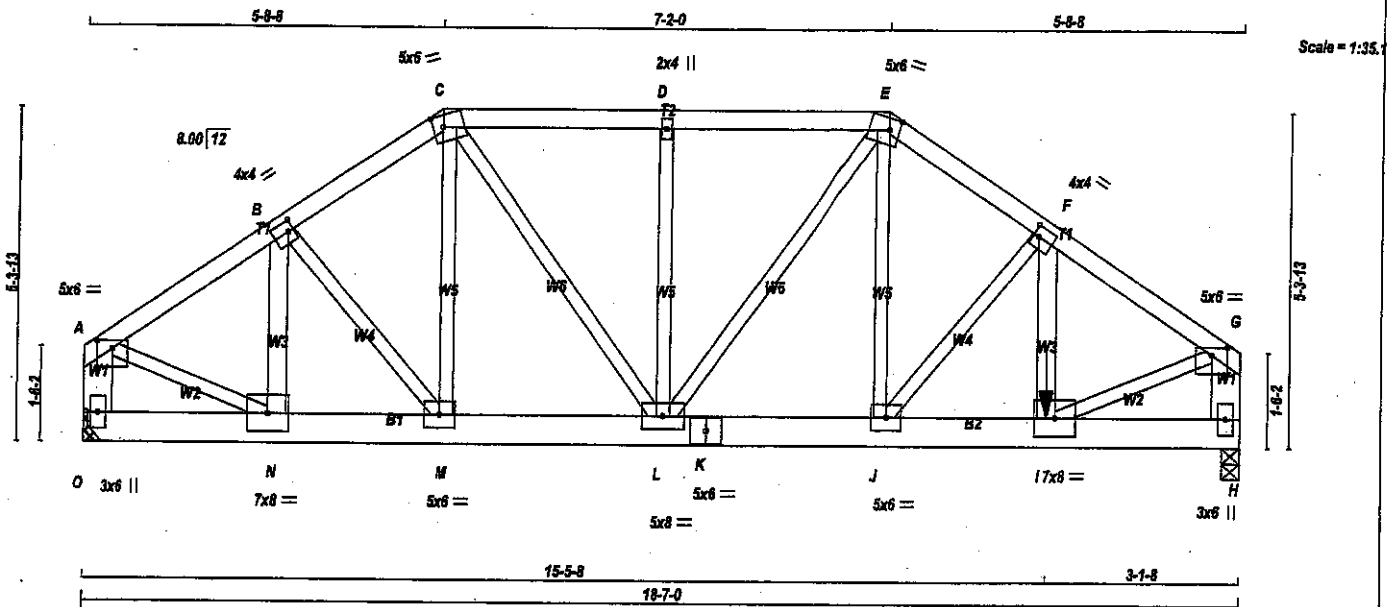
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.74 (B) (INPUT = 0.80)
JSI METAL = 0.54 (I) (INPUT = 1.00)



Structural component only
DWG# T-2129743

JOB NAME 421219	TRUSS NAME T352	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 9 Jan 21 2021 MITek Industries, Inc. Tue Aug 24 17:17:34 2021 Page 1 ID:GV6EnpZB3IFFw?AU1oPMDZzmpn-sLnbwJ2zG4aG70qS1huqbSnlzcnq1A8PenPRYkamF			



TOTAL WEIGHT = 99 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2
	C - E	2x4	DRY	No.2
	E - G	2x4	DRY	No.2
	O - A	2x6	DRY	No.2
	H - G	2x6	DRY	No.2
	O - K	2x6	DRY	No.2
	K - H	2x6	DRY	No.2
	ALL WEBS	2x3	DRY	No.2
	EXCEPT			
	N - B	2x4	DRY	No.2
	I - F	2x4	DRY	No.2
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.00
D	TMVW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TMVW-t	MT20	4.0	4.0	2.00	1.00
G	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMV1-p	MT20	3.0	6.0		
I	BMVW-t	MT20	7.0	8.0		
J	BMVW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	5.0	6.0		
N	BMVW-t	MT20	7.0	8.0		
O	BMV1-p	MT20	3.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
O	1248	0	1248	0	0	MECHANICAL
H	2120	0	2120	0	0	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O. MINIMUM BEARING LENGTH AT JOINT O = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	880	581 / 0	0 / 0	0 / 0	0 / 0	299 / 0	0 / 0
H	1496	999 / 0	0 / 0	0 / 0	0 / 0	497 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	-1266 / 0	-91.8	-91.8	0.15 (1)	5.54	N-B	-360 / 0	0.06 (1)	
B-C	-1324 / 0	-91.8	-91.8	0.15 (1)	5.44	B-M	0 / 28	0.01 (1)	
C-D	-1404 / 0	-91.8	-91.8	0.23 (1)	5.21	M-C	0 / 71	0.02 (4)	
D-E	-1404 / 0	-91.8	-91.8	0.23 (1)	5.21	C-L	0 / 542	0.13 (1)	
E-F	-1692 / 0	-91.8	-91.8	0.13 (1)	4.98	L-D	-391 / 0	0.16 (1)	
F-G	-2251 / 0	-91.8	-91.8	0.20 (1)	4.35	L-E	-7 / 0	0.00 (1)	
G-A	-1205 / 0	0.0	0.0	0.09 (1)	7.81	J-E	0 / 715	0.18 (1)	
H-G	-2045 / 0	0.0	0.0	0.15 (1)	7.04	J-F	-759 / 0	0.22 (1)	
						I-F	0 / 593	0.10 (1)	
O-N	0 / 0	-18.5	-18.5	0.03 (1)	10.00	A-N	0 / 1130	0.28 (1)	
N-M	0 / 1085	-18.5	-18.5	0.17 (1)	10.00	I-G	0 / 1997	0.49 (1)	
M-L	0 / 1085	-18.5	-18.5	0.16 (1)	10.00				
L-K	0 / 1408	-18.5	-18.5	0.21 (1)	10.00				
K-J	0 / 1408	-18.5	-18.5	0.21 (1)	10.00				
J-I	0 / 1882	-18.5	-18.5	0.35 (1)	10.00				
I-H	0 / 0	-18.5	-18.5	0.11 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	15-5-8	-928	-	-	BACK	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.23/1.00 (D-E:1), BC=0.35/1.00 (H:1), WB=0.49/1.00 (G-I:1), SS=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.82 (A) (INPUT = 0.90)
JSI METAL = 0.41 (G) (INPUT = 1.00)

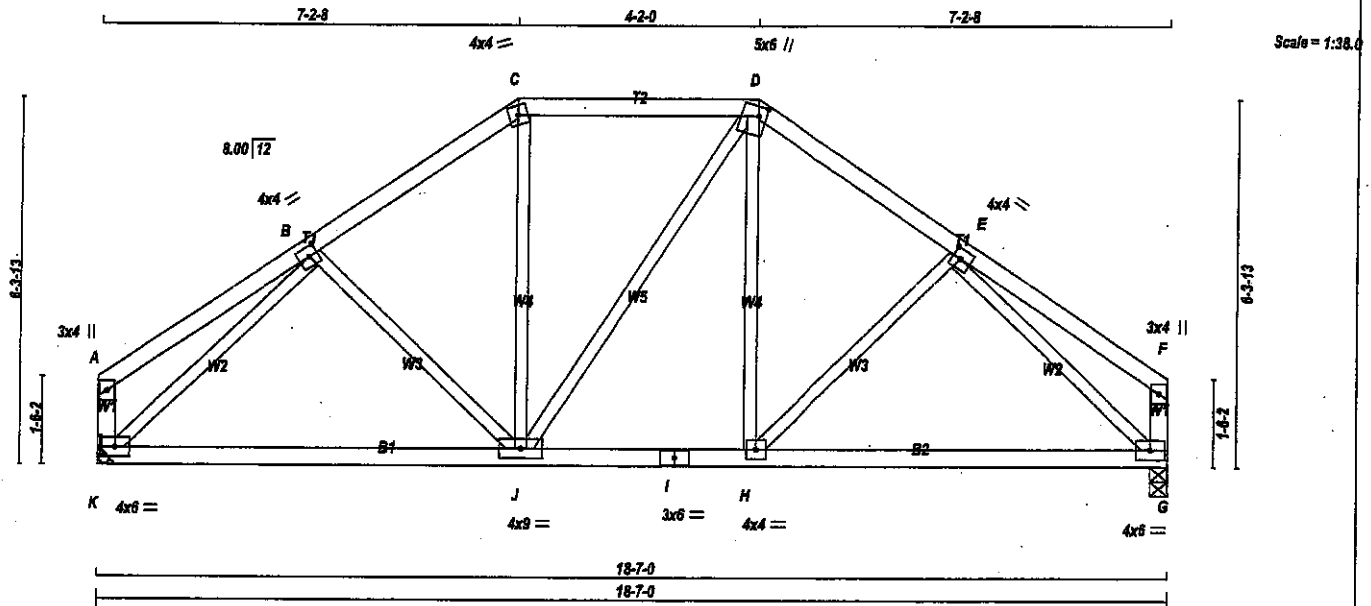


JOB NAME 421219	TRUSS NAME T353	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 79 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
K - A	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTW-m	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+p	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	6.0		
H	BMVW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	9.0		
K	BMVW-t	MT20	4.0	6.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1024	0	1024	0	0	MECHANICAL	
G	1024	0	1024	0	0	3-8	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	724	475 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0
G	724	475 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LOAD LC1 MAX CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 21	-91.8	-91.8 0.18 (1)	10.00	B-J	-110 / 11	0.05 (1)
B-C	-938 / 0	-91.8	-91.8 0.15 (1)	6.23	J-C	0 / 191	0.04 (1)
C-D	-765 / 0	-91.8	-91.8 0.21 (1)	6.25	J-D	0 / 0	0.00 (1)
D-E	-937 / 0	-91.8	-91.8 0.15 (1)	6.23	H-D	0 / 190	0.04 (1)
E-F	0 / 21	-91.8	-91.8 0.18 (1)	10.00	H-E	-110 / 11	0.05 (1)
K-A	-128 / 0	0.0	0.0 0.01 (1)	7.81	K-B	-1188 / 0	0.49 (1)
G-F	-128 / 0	0.0	0.0 0.01 (1)	7.81	E-G	-1187 / 0	0.49 (1)
K-J	0 / 841	-18.5	-18.5 0.27 (4)	10.00			
J-I	0 / 785	-18.5	-18.5 0.25 (4)	10.00			
I-H	0 / 785	-18.5	-18.5 0.25 (4)	10.00			
H-G	0 / 840	-18.5	-18.5 0.27 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 8.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.21/1.00 (C-D:1), BC=0.27/1.00 (G-H:4), WB=0.49/1.00 (B-K:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

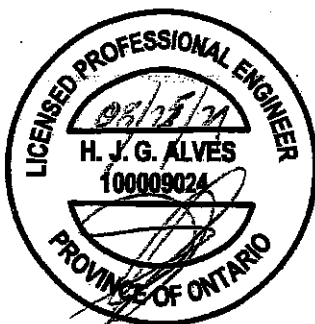
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 550 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

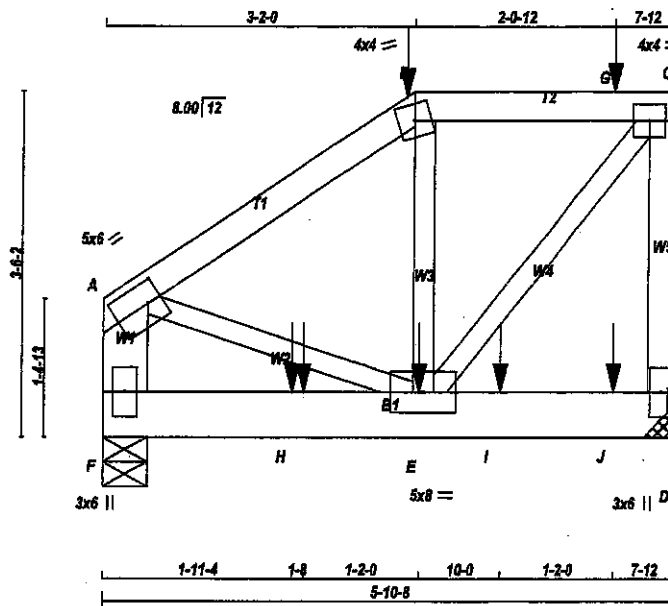
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
JSI METAL = 0.39 (B) (INPUT = 1.00)



Structural component only
DWG# T-2129745

JOB NAME 421219	TRUSS NAME T354	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Tue Aug 24 17:17:35 2021 Page 1 ID:GV6EnpZB3IFFw?AU1oPMODzZmnp-KOLz733b1N171b00IC7Np__NN?cWMSJM3OKytkamE			



Scale = 1:22.5

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1 12		SIDE(61.0)
B-C 1 12		SIDE(61.0)
C-D 1 12		TOP
F-A 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2 12		SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 8		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-t	MT20	5.0	6.0	2.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	1336	0	1336	0	MECHANICAL
F	1229	0	1229	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	943	830 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
F	867	578 / 0	0 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-1011 / 0	-91.8 -91.8	0.09 (1)	8.25	E-B	0 / 124	0.02 (4)
B-G	-843 / 0	-91.8 -91.8	0.08 (1)	6.25	E-C	0 / 1320	0.16 (1)
G-C	-843 / 0	-91.8 -91.8	0.08 (1)	6.25	A-E	0 / 877	0.11 (1)
D-C	-1189 / 0	0.0 0.0	0.11 (1)	7.81			
F-A	-964 / 0	0.0 0.0	0.03 (1)	7.81			
F-H	0 / 0	-18.5 -18.5	0.18 (1)	10.00			
H-E	0 / 0	-18.5 -18.5	0.18 (1)	10.00			
E-I	0 / 0	-18.5 -18.5	0.13 (1)	10.00			
I-J	0 / 0	-18.5 -18.5	0.13 (1)	10.00			
J-D	0 / 0	-18.5 -18.5	0.13 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-2-0	-103	-103		BACK	VERT	TOTAL		C1
E	3-2-12	-9	-9		BACK	VERT	TOTAL		C1
G	5-2-12	-48	-48		BACK	VERT	TOTAL		C1
H	1-11-4	-9	-9		BACK	VERT	TOTAL		C1
I	2-0-12	-589	-589		FRONT	VERT	TOTAL		C1
J	4-0-12	-583	-583		FRONT	VERT	TOTAL		C1
J	5-2-12	-11	-11		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.11/1.00 (C-D:1), BC=0.18/1.00 (E-F:1), WB=0.16/1.00 (C-E:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

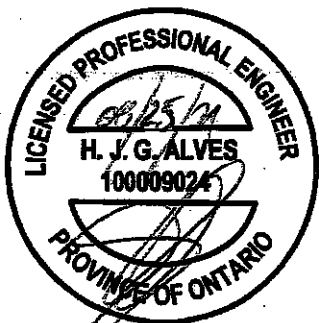
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.66 (C) (INPUT = 0.90)
JSI METAL = 0.18 (C) (INPUT = 1.00)



Structural component only
DWG# T-2129746

CONTINUED ON PAGE 2

JOB NAME 421219	TRUSS NAME T354	QUANTITY 1	PLY 2	JOB DESC. GREENPARK HOMES TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.480 S Jan 21 2021 MiTek Industries, Inc. Tue Aug 24 17:17:35 2021 Page 2 ID:GV6EnpZB3lFFw?AU1oPMODzZmnp-KXLz733b1NI7ib00IC7Np NN?cWMSJM3OKytkamE	

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TTW-m	MT20	4.0	4.0	
C	TMVW-t	MT20	4.0	4.0	
D	BMV1+p	MT20	3.0	6.0	
E	BMVWW-t	MT20	5.0	8.0	
F	BMV1+p	MT20	3.0	6.0	

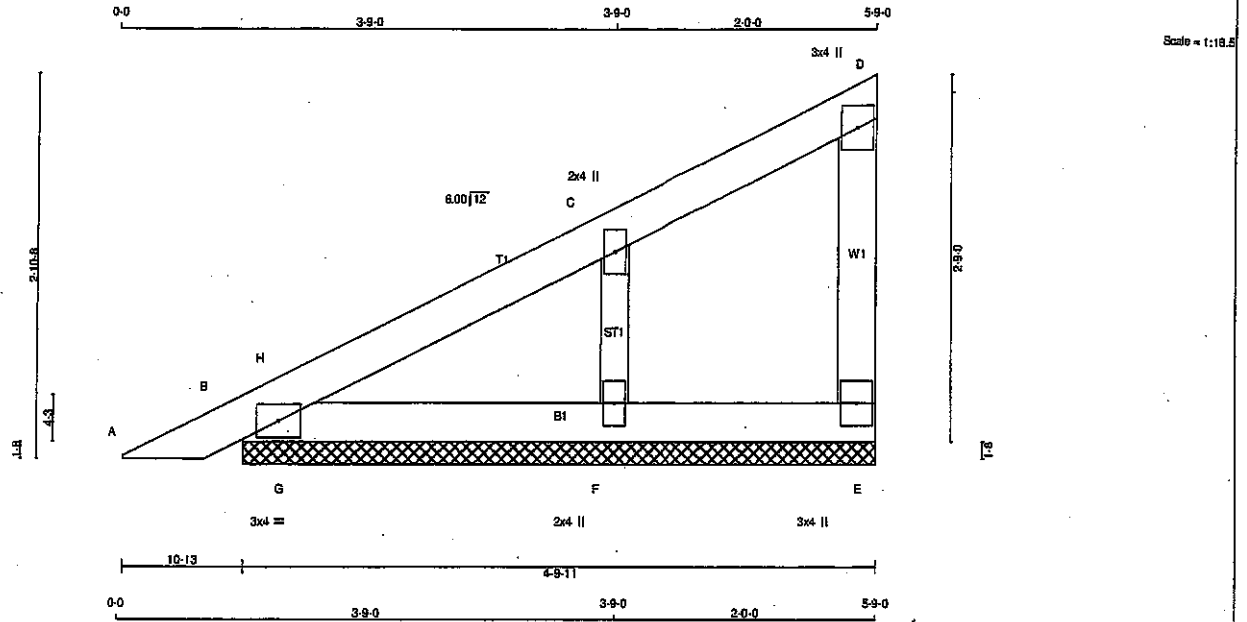
NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.



Structural component only
DWG# T-2129746 *3/2*

JOB NAME 417422	TRUSS NAME PB1G	QUANTITY 4	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.420 5 Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:17:05 2021 Page 1
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TOTAL WEIGHT = 4 X 16 = 65 lb

LUMBER					DESCR.
N. L. & A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2		SPF
E - D	2x4	DRY	No.2		SPF
B - E	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
ALL GABLE WEBS	2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2'-0" OC.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TMBV+w	MT20	2.0	4.0	
D	TMBV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	
F	BMV1+w	MT20	2.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.									
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.									
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	0 / 16	-91.8	-91.8	0.05 (1)	10.00	F-C	-252 / 0	0.04 (1)	
B-H	-19 / 0	-91.8	-91.8	0.01 (4)	6.25	G-H	-87 / 3	0.00 (1)	
H-C	0 / 1	-91.8	-91.8	0.08 (1)	10.00				
C-D	-13 / 0	-91.8	-91.8	0.08 (1)	6.25				
E-D	-66 / 0	0.0	0.0	0.01 (1)	7.81				
B-G	0 / 12	-18.5	-18.5	0.05 (1)	10.00				
G-F	0 / 12	-18.5	-18.5	0.05 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1), BC=0.05/1.00 (B-G:1), WB=0.04/1.00 (C-F:1), SS=0.09/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

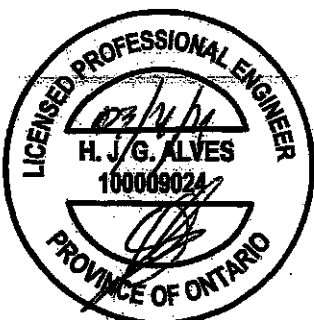
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.10 (C) (INPUT = 1.00)



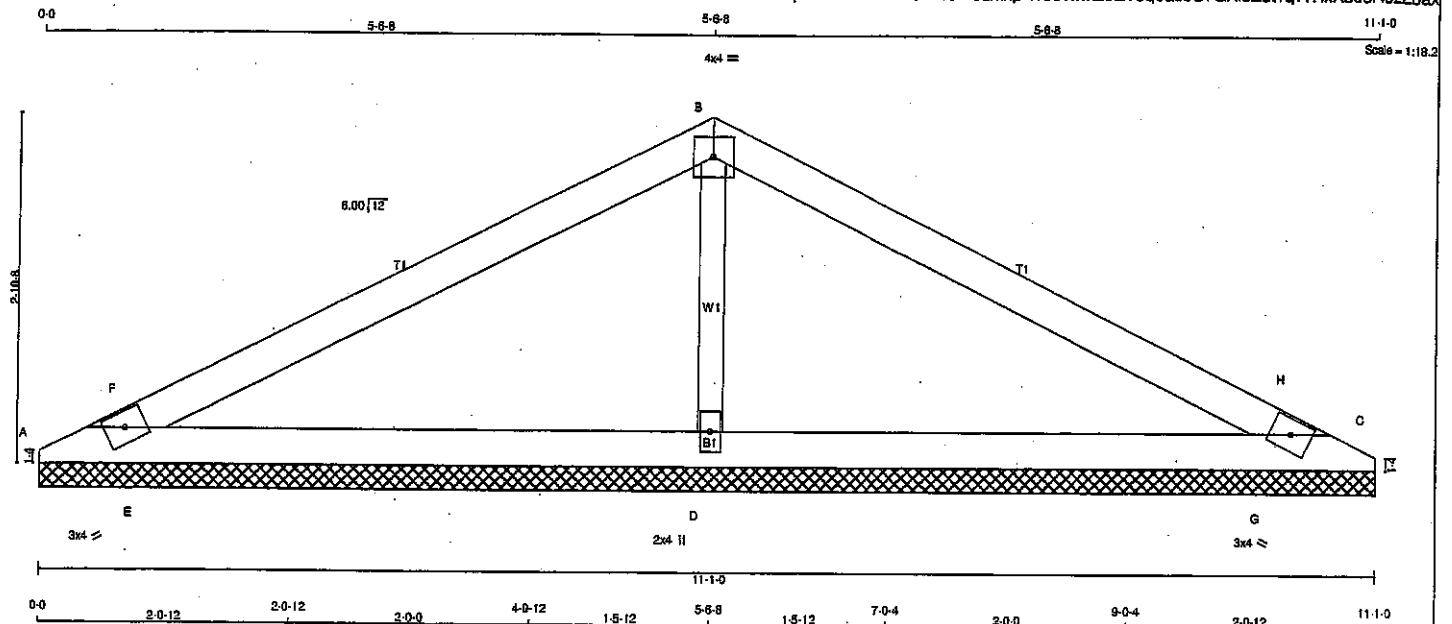
Structural component only
DWG# T-2108084

JOB NAME 417422	TRUSS NAME PB2	QUANTITY 19	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 19 X 28 = 525 lb

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - B	2x4	DRY	No.2	SPF	SPF
B - C	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
A	73	0	73	0	0	11-1-0	11-1-0	11-1-0
C	73	0	73	0	0	11-1-0	11-1-0	11-1-0
D	1076	0	1076	0	0	11-1-0	11-1-0	11-1-0

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
A	52	33 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
C	52	33 / 0	0 / 0	0 / 0	0 / 0	19 / 0	0 / 0
D	761	501 / 0	0 / 0	0 / 0	0 / 0	260 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED				MEMB.	MAX. FORCE (LBS)	MAX	FACTORED
		VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH				MAX CSI (LC)
FR-TO		FROM TO				FR-TO			
A-F	0 / 505	-91.8	-91.8	0.18 (1)	10.00	D-B	-832 / 0		0.14 (1)
F-B	0 / 478	-91.8	-91.8	0.37 (1)	10.00	E-F	-322 / 0		0.00 (1)
B-H	0 / 478	-91.8	-91.8	0.37 (1)	10.00	G-H	-322 / 0		0.00 (1)
H-C	0 / 505	-91.8	-91.8	0.18 (1)	10.00				
A-E	-476 / 0	-18.5	-18.5	0.26 (1)	6.25				
E-D	-433 / 0	-18.5	-18.5	0.27 (1)	6.25				
D-G	-433 / 0	-18.5	-18.5	0.27 (1)	6.25				
G-C	-476 / 0	-18.5	-18.5	0.26 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2015
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.37/1.00 (B-F:1), BC=0.27/1.00 (D-E:1), WB=0.14/1.00 (B-D:1), SSI=0.18/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.44 (B) (INPUT = 0.90)
JSI METAL = 0.24 (B) (INPUT = 1.00)

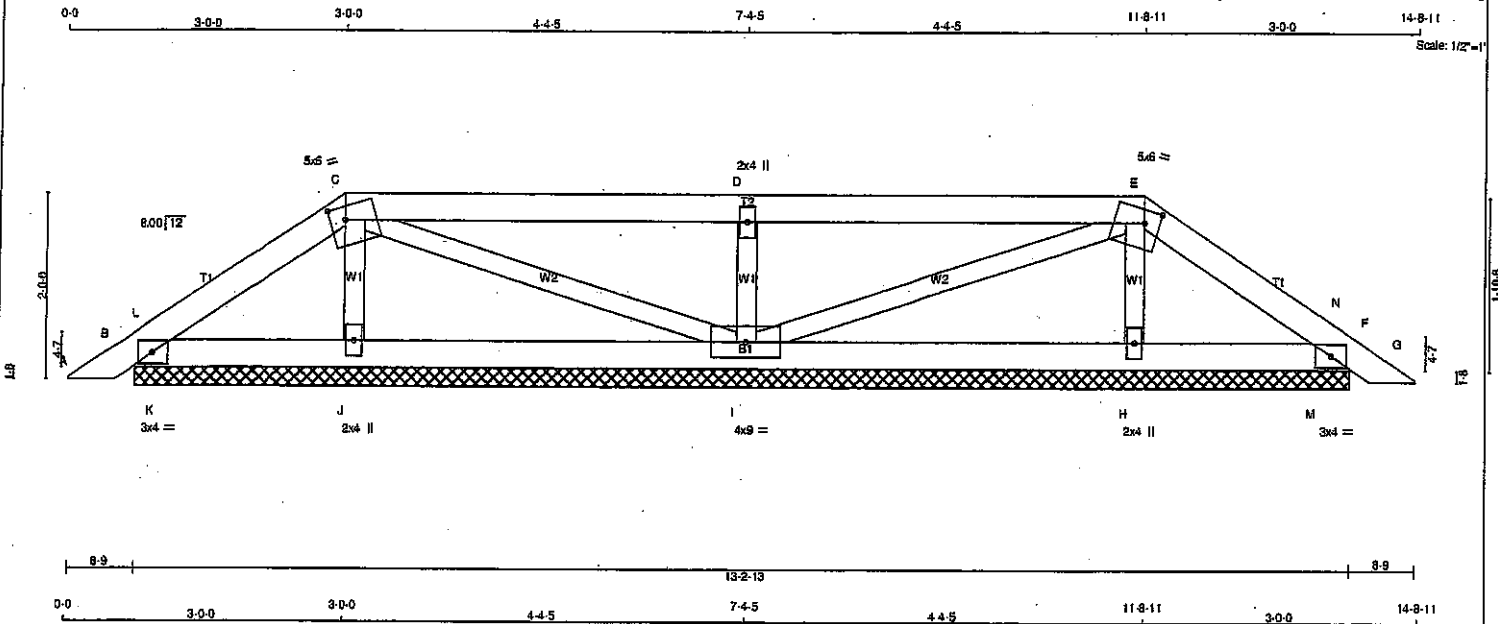


Structural component only
DWG# T-2108085

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	PB11	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 M/Tek Industries, Inc. Sat Mar 20 08:38:13 2021 Page 1
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LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	6.0	1.75	2.00
D	TMW-w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	1.75	2.00
F	TM81-I	MT20	3.0	4.0		
H	BMW1-w	MT20	2.0	4.0		
I	BMW1-w	MT20	4.0	9.0		
J	BMW1-w	MT20	2.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	203	0	203	0	13-2-13	13-2-13
F	203	0	203	0	13-2-13	13-2-13
J	288	0	288	0	13-2-13	13-2-13
I	595	0	595	0	13-2-13	13-2-13
H	288	0	288	0	13-2-13	13-2-13

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	141	106 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
F	141	106 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
J	206	123 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
I	419	285 / 0	0 / 0	0 / 0	0 / 0	134 / 0	0 / 0
H	206	123 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. (LC) (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. (LC) (LBS)
FR-TO				FR-TO		
A-B	0 / 15	-91.8	-91.8 0.03 (1)	10.00	J-C	-197 / 0
B-L	-37 / 0	-91.8	-91.8 0.01 (1)	8.25	C-I	-26 / 0
L-C	-56 / 0	-91.8	-91.8 0.05 (1)	6.25	I-D	-500 / 0
C-D	-7 / 0	-91.8	-91.8 0.30 (1)	10.00	I-E	-28 / 0
D-E	-7 / 0	-91.8	-91.8 0.30 (1)	10.00	H-E	-197 / 0
E-N	-56 / 0	-91.8	-91.8 0.05 (1)	6.25	K-L	-134 / 0
N-F	-37 / 0	-91.8	-91.8 0.01 (1)	8.25	M-N	-134 / 0
F-G	0 / 15	-91.8	-91.8 0.03 (1)	10.00		
B-K	0 / 44	-18.5	-18.5 0.06 (1)	10.00		
K-J	0 / 44	-18.5	-18.5 0.06 (1)	10.00		
J-I	0 / 31	-18.5	-18.5 0.07 (4)	10.00		
I-H	0 / 31	-18.5	-18.5 0.07 (4)	10.00		
H-M	0 / 44	-18.5	-18.5 0.06 (1)	10.00		
M-F	0 / 44	-18.5	-18.5 0.06 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIG 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (C-D:1), BC=0.07/1.00 (I-J:4), WB=0.07/1.00 (D-I:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.25 (D) (INPUT = 0.90)
JSI METAL = 0.10 (D) (INPUT = 1.00)

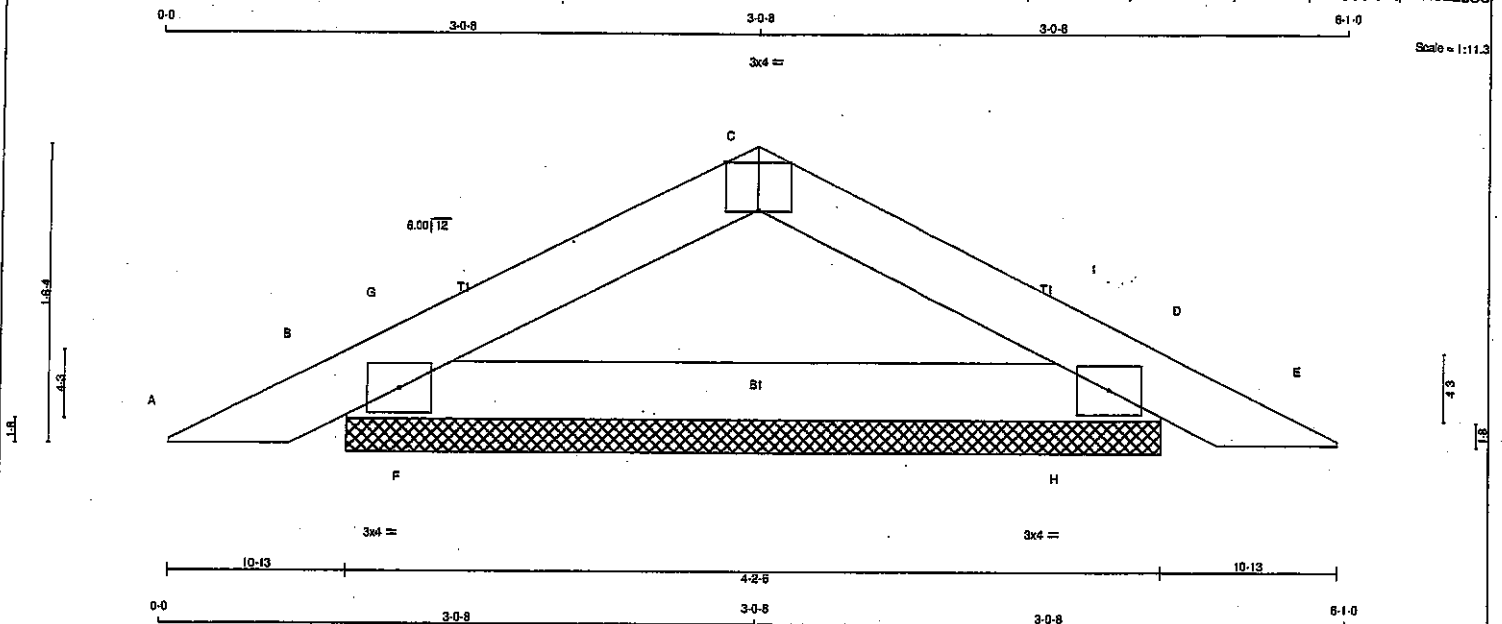


Structural component only
DWG# T-2108107

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	PB31	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MITek Industries, Inc. Sat Mar 20 08:52:37 2021 Page 1
ID:wa2t6usWauA5BCJTGMC4FzJxO-c3HWA1qVNFsG329qA73aRK7p2BS05IC4qn?N8zZ63e



TOTAL WEIGHT = 3 X 12 = 37 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	309	0	309	0	0	4-2-6	4-2-6
D	309	0	309	0	0	4-2-6	4-2-6

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	217	151.0	0.0	0.0	0.0	66.0	0.0
D	217	151.0	0.0	0.0	0.0	66.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 16	-91.8	-91.8	0.05 (1)	10.00	F-G	0 / 58	0.00 (1)
B-G	-264 / 0	-91.8	-91.8	0.05 (4)	6.25	H-I	0 / 58	0.00 (1)
G-C	-207 / 0	-91.8	-91.8	0.07 (1)	6.25			
C-I	-207 / 0	-91.8	-91.8	0.07 (1)	6.25			
I-D	-264 / 0	-91.8	-91.8	0.05 (4)	6.25			
D-E	0 / 16	-91.8	-91.8	0.05 (1)	10.00			
B-F	0 / 193	-18.5	-18.5	0.04 (1)	10.00			
F-H	0 / 193	-18.5	-18.5	0.08 (1)	10.00			
H-D	0 / 193	-18.5	-18.5	0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-I:1), BC=0.08/1.00 (F-H:1), WB=0.00/1.00 (F-G:1), SS=0.07/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

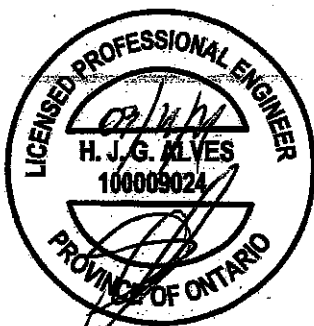
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

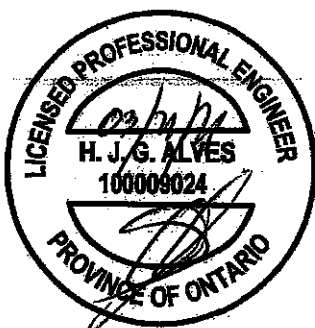
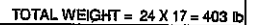
JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108134

Tamarack Roof Truss, Burlington

Scale = 1:23.3

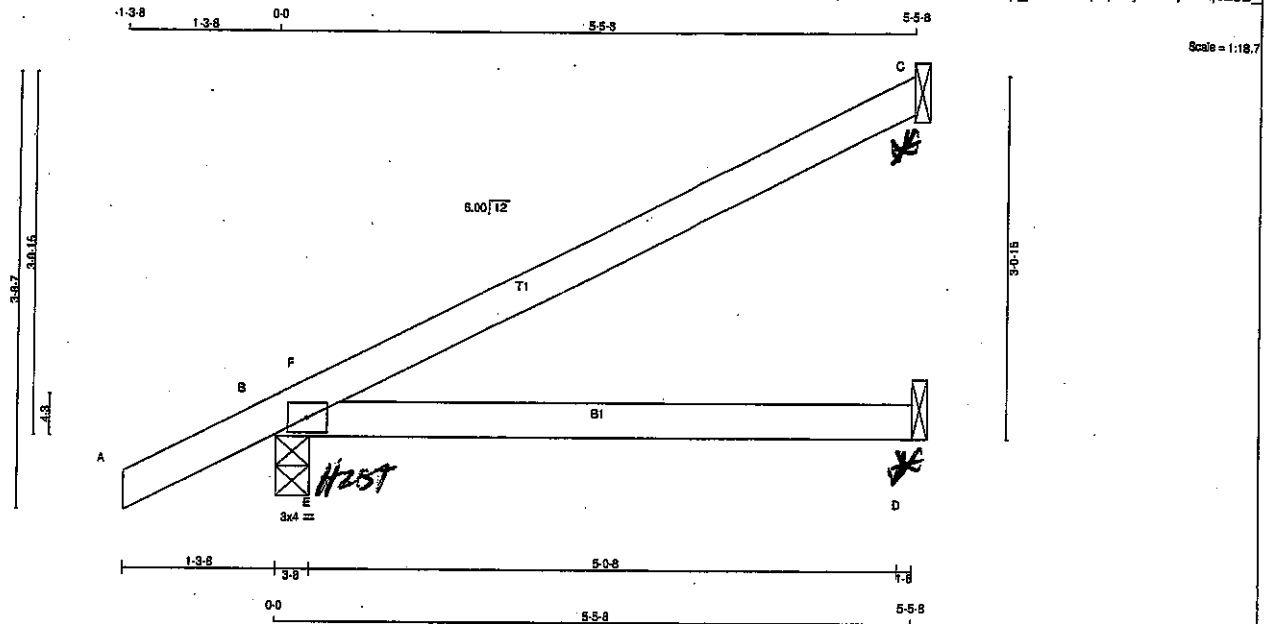


Structural component only
DWG# T-2108080

JOB NAME 417422	TRUSS NAME J3	QUANTITY 11	PLY 1	JOB DESC. GREENPARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 08:17:03 2021 Page 1
ID:GV6EnpZB3lFFw7AU1oPMODzZmnp-SWLmL?wxanFF6v0p_id5QxNspqYoyP5Nqr4UqzZ6b



TOTAL WEIGHT = 11 X 15 = 165 lb

LUMBER	N. L. G. A. RULES
CHORDS	SIZE
A - C	2x4 DRY No.2
B - D	2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX		
C	219	0	225	0	-172	1-8	1-8	
B	425	0	425	207	-247	3-8	3-8	
D	82	0	82	0	-112	1-8	1-8	

SEE MITK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 172 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT B FOR 247 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

PROVIDE FOR 207 LBS FACTORED HORIZONTAL REACTION AT JOINT B

UNFACTORED REACTIONS								
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
C	152	118 / 0	0 / 0	0 / 0	147 / -144	33 / 0	0 / 0	
B	299	209 / 0	0 / 0	0 / 0	0 / -234	89 / 0	0 / 0	
D	61	21 / 0	0 / 0	0 / 0	0 / -106	40 / 0	0 / 0	

HORIZONTAL REACTIONS		MAX. MIN. COMPONENT REACTIONS	
1ST CASE	2ND CASE	SNOW	LIVE
B	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (12)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 27	-91.8	-91.8	0.12 (1)	10.00	E-F	-514 / 213
B-F	-200 / 130	-91.8	-91.8	0.13 (3)	6.25		0.00 (1)
F-C	-89 / 4	-91.8	-91.8	0.36 (1)	6.25		
B-E	0 / 0	-18.5	-18.5	0.24 (1)	10.00		
E-D	0 / 0	-18.5	-18.5	0.25 (1)	10.00		

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF (9.6) PSF AT (20-0-0) FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE (MAIN WIND FORCE RESISTING SYSTEM) INTERNAL WIND PRESSURE IS BASED ON DESIGN (CATEGORY 2) BUILDING MAY BE LOCATED ON (OPEN TERRAIN), AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST (0-0) FT-IN-SX AWAY FROM EAVE. TRUSS UPLIFT IS BASED ON TOP AND BOTTOM CHORD DEAD LOADS OF 8.0 PSF AND 7.4 PSF RESPECTIVELY.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/727 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/615 (0.11")

CSI: TC=0.36/1.00 (C-F:1), BC=0.25/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.34/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

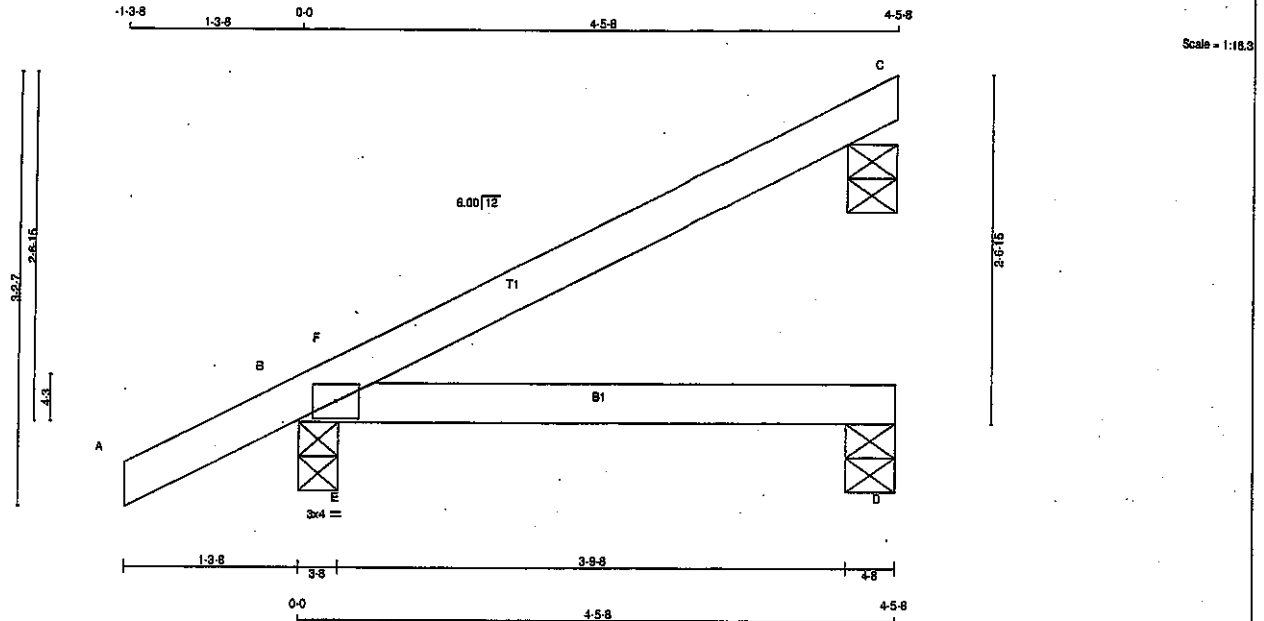
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.34 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



JOB NAME 417422	TRUSS NAME J4	QUANTITY 6	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.420 S Jan 21 2021 MtTek Industries, Inc. Sat Mar 20 08:17:04 2021 Page 1
ID:GV6EnpZB3IFFw?AU1oPMODzZmnp-Zju9Vg0Yitv6sGUDNIDseUaVDB?XPIEcUae?GzZ6az



TOTAL WEIGHT = 6 X 13 = 75 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-4	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
C	178	0	0	178	0	4-8	4-8	4-8	4-8
B	370	0	0	370	0	3-8	3-8	3-8	3-8
D	67	0	0	67	0	4-8	4-8	4-8	4-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
C	123	96'0	0'0	0'0	0'0
B	260	183'0	0'0	0'0	0'0
D	50	18'0	0'0	0'0	0'0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 27	-91.8 -91.8	0.12 (1)	10.00	E-F	-288'6	0.00 (1)
B-F	-20 / 39	-91.8 -91.8	0.05 (1)	6.25			
F-C	-3 / 2	-91.8 -91.8	0.24 (1)	10.00			
B-E	0 / 0	-18.5 -18.5	0.17 (1)	10.00			
E-D	0 / 0	-18.5 -18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.24/1.00 (C-F:1), BC=0.17/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.24/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

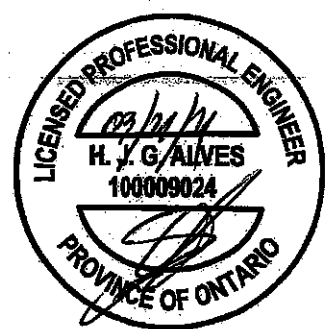
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
				MAX	MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.30 (B) (INPUT=0.90)
JSI METAL= 0.06 (B) (INPUT=1.00)

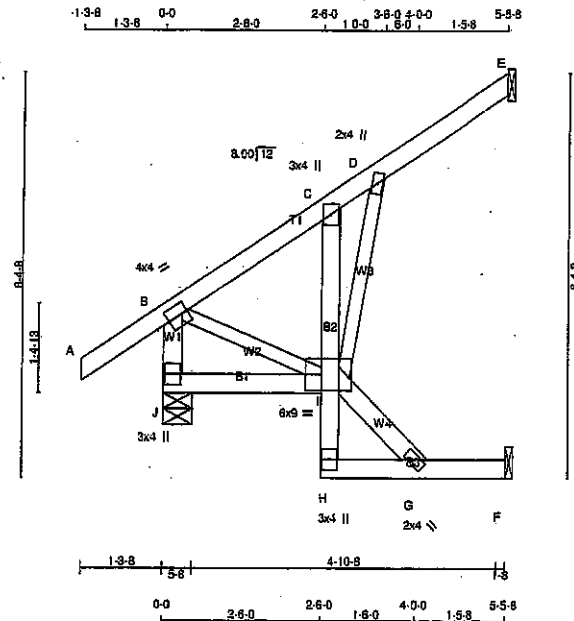


Structural component only
DWG# T-2108083

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	J11	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.420 S Jan 21 2021 MiTek Industries, Inc. Sat Mar 20 08:38:12 2021 Page 1
ID:v5FX5BJlojZNLEnu26zoDzZ19Z-RfWVdFMw5TfB_7Qv1?zD01?EkMxQdtytAlkkWnzZ6H9



Scale = 1/32

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
J - B	2x4	DRY	No.2	SPF	
A - E	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
H - C	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
I - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
C	TMV+p	MT20	3.0	4.0		
D	TMV+w	MT20	2.0	4.0		
G	BMV+w	MT20	2.0	4.0		
H	BMV+p	MT20	3.0	4.0		
I	BMVWW-I	MT20	6.0	9.0	3.00	3.00
J	BMV1+p	MT20	3.0	4.0		

NOTES: (1)

1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
J	427	0	427	0	5-8	5-8
E	214	0	214	0	1-8	1-8
F	87	0	87	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) E, F

UNFACTORED REACTIONS

JT	COMBINED	MAX. MIN. COMPONENT REACTIONS					
		1ST LOSE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
J	299	210	0	0	0	80	0
E	149	109	0	0	0	40	0
F	63	30	0	0	0	33	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		W E B S	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
J-B	-402 / 0	0.0	0.0	0.04 (1)
A-B	0 / 35	-91.8	-91.8	0.12 (1)
B-C	-189 / 0	-91.8	-91.8	0.07 (1)
C-D	-335 / 0	-91.8	-91.8	0.34 (1)
D-E	0 / 84	-91.8	-91.8	0.35 (1)
J-I	0 / 0	-18.5	-18.5	0.04 (4)
H-I	0 / 91	0.0	0.0	0.25 (1)
I-C	-531 / 0	0.0	0.0	0.24 (1)
H-G	0 / 128	-18.5	-18.5	0.17 (1)
G-F	0 / 0	-18.5	-18.5	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/804 (0.08")

CSI: TC=0.35/1.00 (D-E:1), BC=0.25/1.00 (H-I:1),
WB=0.10/1.00 (D-I:1), SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1997 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.46 (D) (INPUT = 0.90)
JSI METAL = 0.17 (I) (INPUT = 1.00)

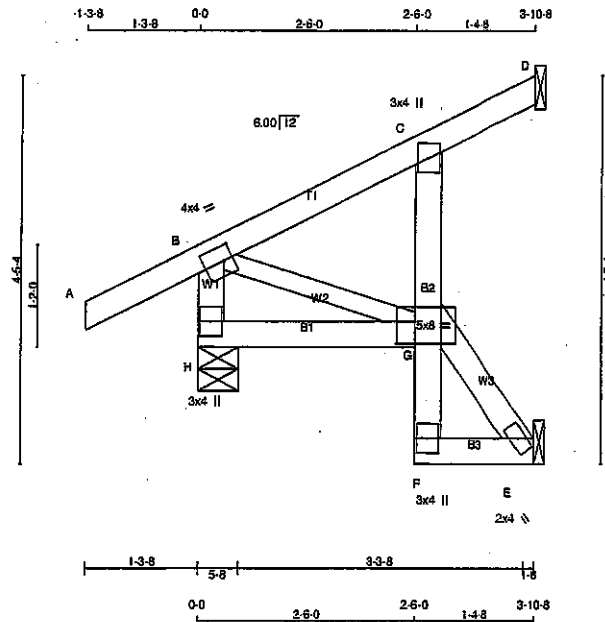


Structural component only
DWG# T-2108105

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417443	J31	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:wa216usWauA15BCJTGMG4FzZjxO-8t18yhqtcxWbevTzGSTq1Enx2era3eB2sA1SrizZ63f



Scale = 1/25.3

TOTAL WEIGHT = 4 X 20 = 79 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
E	BMV1+w	MT20	2.0	4.0		
F	BMV+p	MT20	3.0	4.0		
G	BVMWW-t	MT20	5.0	8.0	3.00	2.50
H	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
H	386	0	386	0
D	94	0	94	0
E	88	0	88	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	256	183 / 0	0 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
D	66	46 / 0	0 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
E	63	38 / 0	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		FR-TO			
H-B	-326 / 0	0.0	0.0 0.05 (1)	7.81	B-G	0 / 80	0.02 (1)
A-B	0 / 28	-91.8	-91.8 0.13 (5)	10.00	G-E	-126 / 0	0.01 (1)
B-C	-77 / 0	-91.8	-91.8 0.12 (1)	6.25			
C-D	0 / 16	-91.8	-91.8 0.07 (1)	10.00			
H-G	-19 / 2	-18.5	-18.5 0.09 (1)	6.25			
F-G	0 / 30	0.0	0.0 0.12 (1)	10.00			
G-C	-87 / 3	0.0	0.0 0.08 (1)	7.81			
F-E	0 / 78	-18.5	-18.5 0.04 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSK: TC=0.13/1.00 (A-B-5), BC=0.12/1.00 (F-G-1), WB=0.02/1.00 (B-G-1), SS=0.10/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

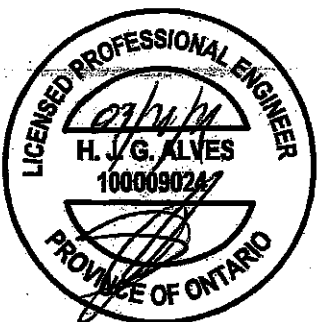
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

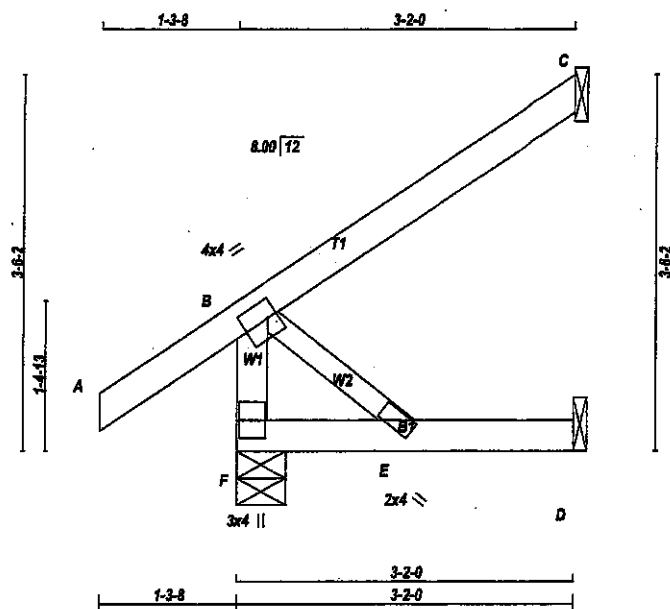
JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-24004103

JOB NAME 421219	TRUSS NAME J351	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:GV6EnpZB3lFFw7AU1oPMODzZmnp-Z?sySKZawwiGne_UY33F27bh8leem08XpSvLykamM



TOTAL WEIGHT = 2 X 12 = 25 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.00
E	BMV-w	MT20	2.0	4.0	
F	BMV1-p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
F	301	0	301	0	0	5-8	5-8	
C	145	0	145	0	0	1-8	1-8	
D	29	0	33	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	210	151/0	0/0	0/0	0/0	58/0	0/0
C	100	81/0	0/0	0/0	0/0	19/0	0/0
D	23	0/0	0/0	0/0	0/0	23/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-271/0	0.0 0.0 0.03 (1)	7.81	B-E	0/0	0.00 (1)	
A-B	0/35	-91.8 -91.8 0.13 (5)	10.00				
B-C	0/0	-91.8 -91.8 0.15 (1)	10.00				
F-E	0/0	-18.5 -18.5 0.05 (4)	10.00				
E-D	0/0	-18.5 -18.5 0.05 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (B-C:1), BC=0.05/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

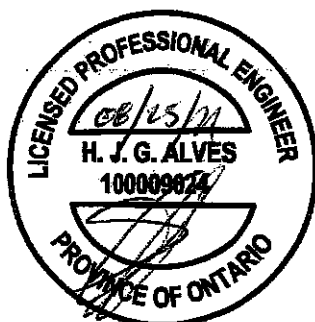
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.17 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)

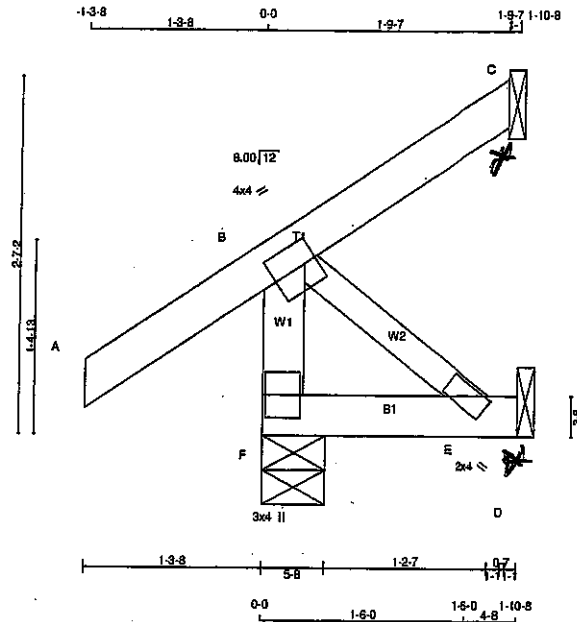


Structural component only
DWG# T-2129736

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417422	C1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:GV6EnpZB3IFFw7AU1oPMODzZmnp4ZXtEdyn618y8L130R7SONEamoAv7hAMDY7KpdzZ8b3



Scale: 3/4"=1'

TOTAL WEIGHT = 9 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMVW-1	MT20	4.0	4.0	2.00
E	BMVW-w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
F	274	0	274	0	5-8	5-8	5-8	5-8
C	34	0	34	0	-39	1-8	1-8	1-8
D	17	0	19	0	0	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	23	19 / -27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS			FACTORED				WEBS		
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
F-B	-256 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 35	-91.8	-91.8	0.12 (1)	10.00				
B-C	-27 / 0	-91.8	-91.8	0.12 (1)	8.25				
F-E	0 / 0	-18.5	-18.5	0.02 (4)	10.00				
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6	PSF
DL = 6.0	PSF	
BOT CH.	LL = 0.0	PSF
DL = 7.4	PSF	
TOTAL LOAD = 39.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.16 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)

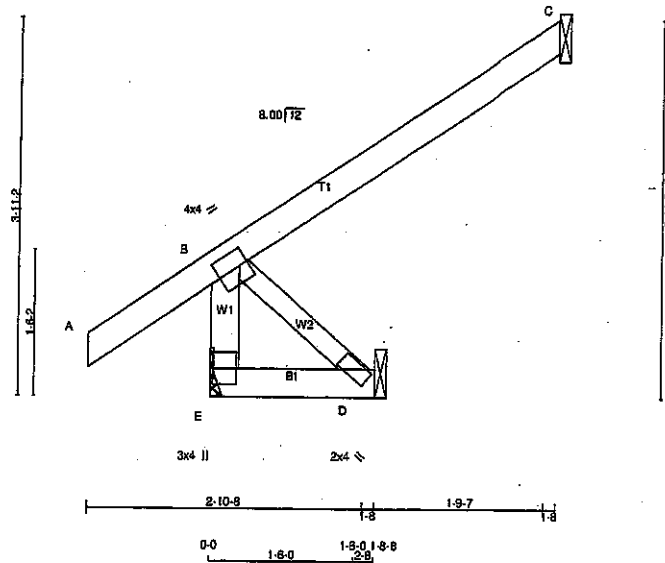


Structural component only
DWG# T-2108076

JOB NAME 417422	TRUSS NAME C2A	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:GV6EnpZB3IFFw7AU1oPMODzZmnp-Di5GSzyPtLgpmVbFa9ehwamjFCVFs8QVSCtL3zZ6b2

Scale = 1:25.0



TOTAL WEIGHT = 12 lb

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	DRY	LUMBER	
CHORDS			No.2	SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	4.0	4.0	2.00 1.00
D	BMV1-w	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	306	0	306	0	0	0
C	166	0	166	0	1-8	1-8
D	14	0	16	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
E	212	163 / 0	0 / 0	0 / 0	49 / 0	0 / 0	0 / 0
C	114	93 / 0	0 / 0	0 / 0	22 / 0	0 / 0	0 / 0
D	11	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (PLF)	MAX. FACTORED (LC)		MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
FR-TO			FROM	TO	FR-TO		
E-B	-292 / 0	0.0	0.0	0.03 (1)	B-D	0 / 0	0.00 (1)
A-B	0 / 35	-91.8	-91.8	0.13 (5)			
B-C	0 / 0	-91.8	-91.8	0.20 (1)			
E-D	0 / 0	-18.5	-18.5	0.01 (4)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.20/1.00 (B-C:1), BC=0.01/1.00 (D-E:4),
WB=0.00/1.00 (B-D:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

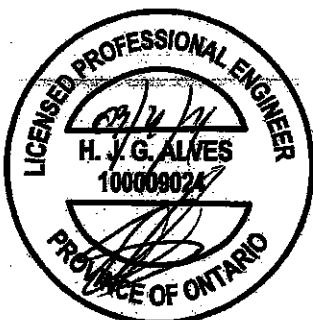
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.30)
JSI METAL = 0.06 (B) (INPUT = 1.00)

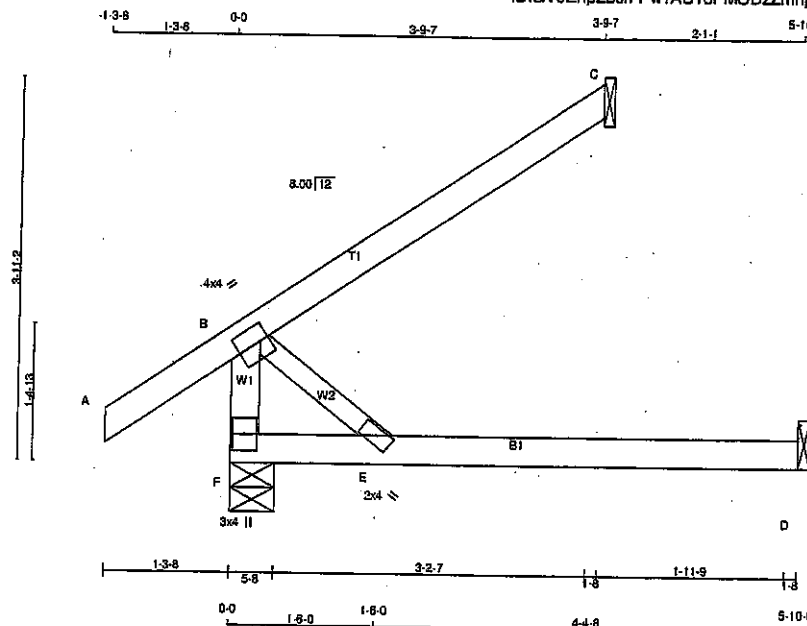


Structural component only
DWG# T-2108077

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417422	C4	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 16 lb

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMVW-1	MT20	4.0	4.0	2.00
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
F	354	0	354	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	54	0	61	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	COMBINED	167 / 0	0 / 0	0 / 0	0 / 0	83 / 0
C		97 / 0	0 / 0	0 / 0	0 / 0	23 / 0
D		0 / 0	0 / 0	0 / 0	0 / 0	43 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	WEBS		MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
				FROM	TO			
FR-TO								
F-B		-300 / 0	0.0	0.0	0.03 (1)	7.81	0 / 0	0.00 (1)
A-B		0 / 35	-91.8	-91.8	0.12 (1)	10.00		
B-C		0 / 0	-91.8	-91.8	0.22 (1)	10.00		
F-E		0 / 0	-18.5	-18.5	0.14 (4)	10.00		
E-D		0 / 0	-18.5	-18.5	0.19 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.8 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIQ 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.22/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

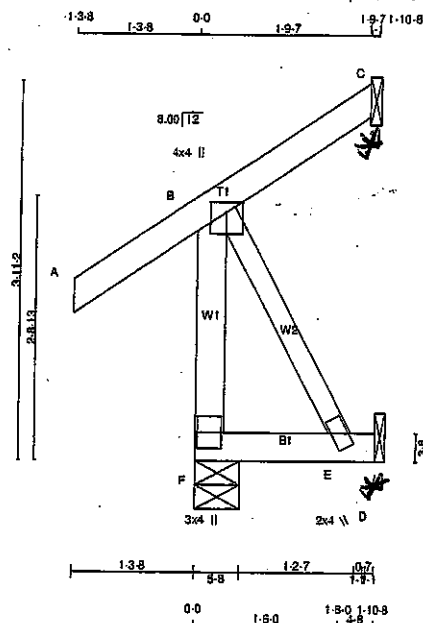
JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108079

JOB NAME 417441	TRUSS NAME C11	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/22.5

TOTAL WEIGHT = 11 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.25 2.00
E	BMV+w	MT20	2.0	4.0
F	BMV1+p	MT20	3.0	4.0

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	274	0	274	0	5-8	5-8
F	34	0	34	0	1-8	1-8
C	34	0	34	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	190	143 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
F	23	19 / 27	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
C	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	UNBRACED LENGTH	MEMB. MAX. FORCE (LBS)	LC1 MAX. (LC)	UNBRACED LENGTH	MEMB. MAX. FORCE (LBS)
FR-TO								
F-B	-256 / 0	0.0	0.0 0.04 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 35	-91.8	-91.8 0.12 (1)	10.00				
B-C	-27 / 0	-91.8	-91.8 0.12 (1)	6.25				
F-E	0 / 0	-18.5	-18.5 0.02 (4)	10.00				
E-D	0 / 0	-18.5	-18.5 0.01 (4)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 25.6 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

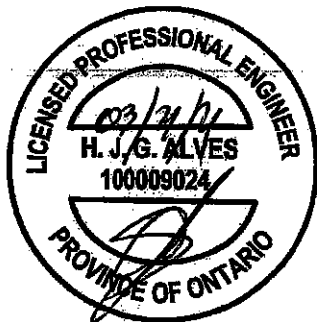
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	850 371 1747 788 1987 1873	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

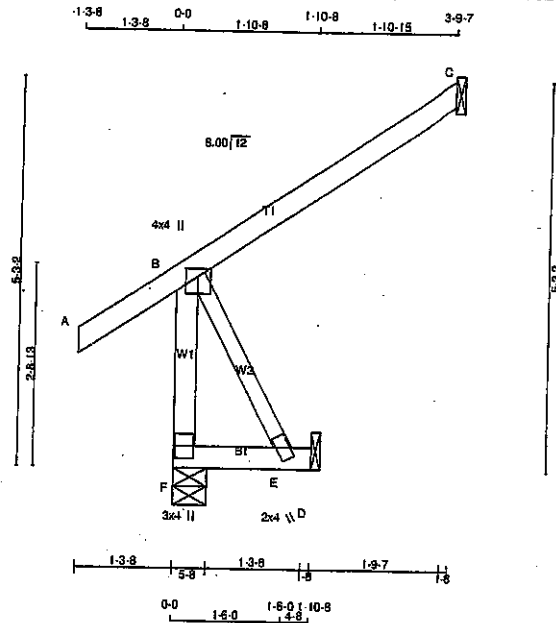


Structural component only
DWG# T-2108101

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENPARK HOMES	DRWG NO.
417441	C12	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
F - B	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
F - D	2x4	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25
E	BMV+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	317	0	317	0	5-8	5-8
C	174	0	174	0	1-8	1-8
D	17	0	19	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	220	167.0	0.0	0.0	0.0	53.0	0.0
C	120	97.0	0.0	0.0	0.0	23.0	0.0
D	14	0.0	0.0	0.0	0.0	14.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

FR-TO	CHORDS			WEBS		
	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED CSI (LC)
F-B	-300.0	0.0	0.04 (1)	7.81	0.0	0.00 (1)
A-B	0.35	-91.8	-91.8 0.13 (5)	10.00		
B-C	0.0	-91.8	-91.8 0.22 (1)	10.00		
F-E	0.0	-18.5	-18.5 0.02 (4)	10.00		
E-D	0.0	-18.5	-18.5 0.01 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 14 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.22/1.00 (B-C:1), SC=0.02/1.00 (E-F:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

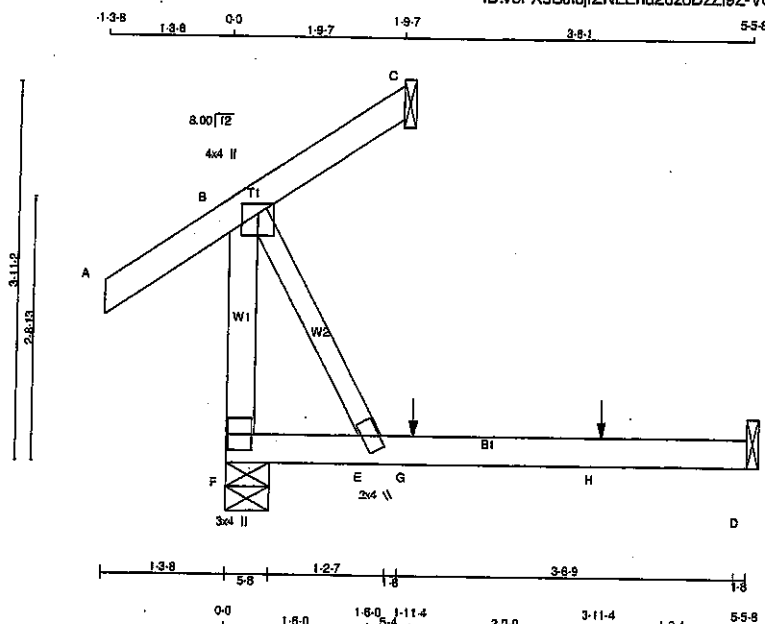
JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108102

JOB NAME 417441	TRUSS NAME C13	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 15 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	SPF
F - B	2x4	DRY	No.2	SPF	SPF
A - C	2x4	DRY	No.2	SPF	SPF
F - D	2x4	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
E BMW+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	307	0	307	0	5-8	5-8
C	34	0	34	0	1-8	1-8
D	50	0	57	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
F	217	143.0	0.0	0.0	0.0	74.0	0.0
C	23	19.0	0.0	0.0	0.0	4.0	0.0
D	40	0.0	0.0	0.0	0.0	40.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC LENGTH	MAX. FORCE (LBS)	MAX. LC1 (LC)	MAX. FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO			FR-TO			
F-B	-258.0	0.0	0.0	0.04 (1)	7.81	B-E	0.0	0.00 (1)
A-B	0.35	-91.8	-91.8	0.13 (1)	10.00			
B-C	-27.0	-91.8	-91.8	0.12 (1)	6.25			
F-E	0.0	-18.5	-18.5	0.13 (4)	10.00			
E-G	0.0	-18.5	-18.5	0.16 (4)	10.00			
G-H	0.0	-18.5	-18.5	0.16 (4)	10.00			
H-D	0.0	-18.5	-18.5	0.16 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	--	FRONT	VERT	TOTAL	--	C1
H	3-11-4	1	1	--	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.8	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD = 39.0 PSF				

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/699 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.13/1.00 (A-B:1), BC=0.16/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=0.98 NAIL=0.98 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

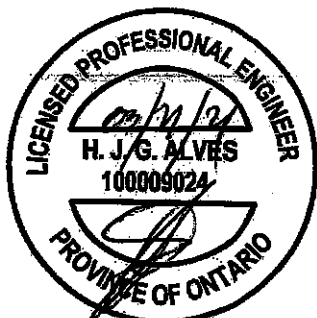
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	PLI
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)

JSI METAL= 0.05 (B) (INPUT = 1.00)



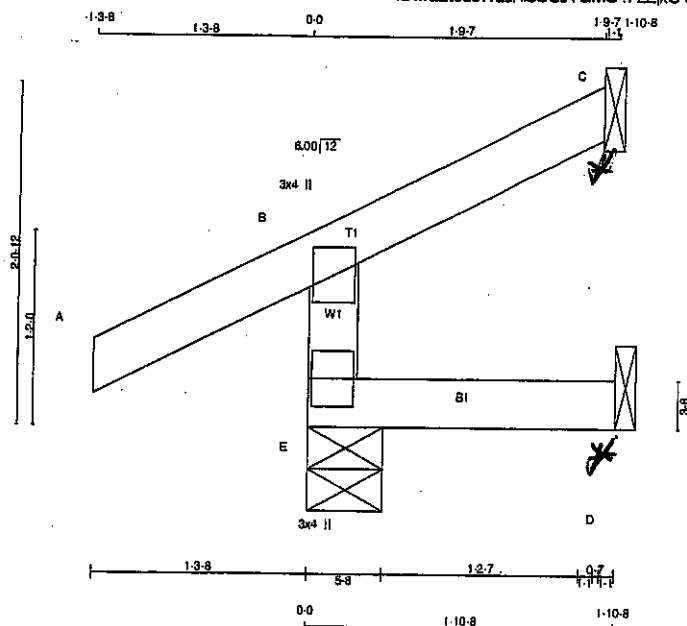
Structural component only
DWG# T-2108103

JOB NAME 417443	TRUSS NAME C31	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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Scale = 1:13.3



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	271	0	271	0	5-8	5-8
C	45	0	45	0	1-8	1-8
D	8	0	17	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	188	141 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	31	24 / -18	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	7	0 / -8	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO						
E-B	-244 / 0	0.0	0.0	0.04 (5)	7.81	
A-B	0 / 28	-91.8	-91.8	0.12 (1)	10.00	
B-C	-17 / 0	-91.8	-91.8	0.09 (1)	8.25	
E-D	0 / 0	-18.5	-18.5	0.04 (5)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 7 = 35 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 25.6 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TO=0.12/1.00 (A-B:1), BC=0.04/1.00 (D-E:5), WB=0.00/1.00 (n/a:0), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

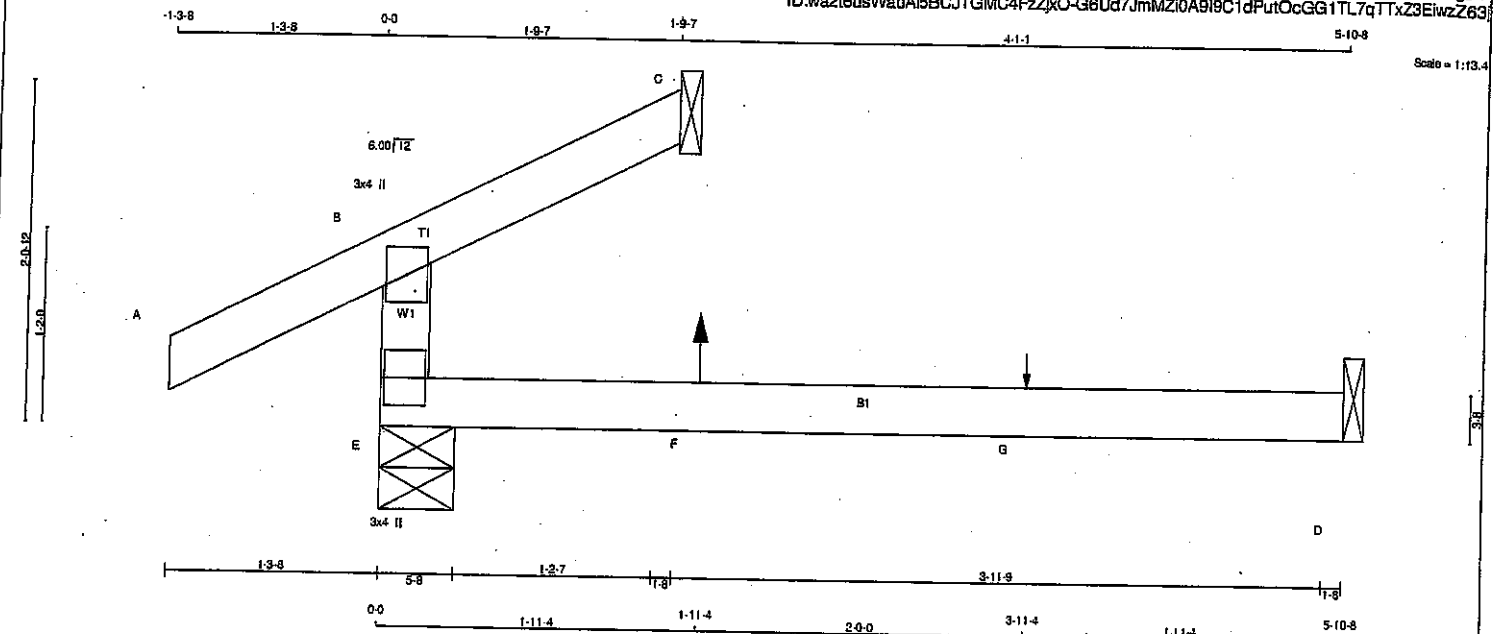
JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108126

JOB NAME 417443	TRUSS NAME C33	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

NOTES: (1)
1) Lateral braces to be a minimum of 2x4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	284	0	284	0	5-8	5-8
C	63	0	63	0	1-8	1-8
D	44	0	52	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	200	137.0	0.0	0.0	0.0	62.0	0.0
C	48	21.0	0.0	0.0	0.0	25.0	0.0
D	35	0.0	0.0	0.0	0.0	37.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	CHORDS		WEBS	
			MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO						
E-B	-227.0	0.0	0.0	0.11 (4)	7.81	
A-B	0.0	-91.8	-91.8	0.12 (1)	10.00	
B-C	-9.9	-91.8	-91.8	0.08 (4)	10.00	
E-F	0.0	-18.5	-18.5	0.14 (4)	10.00	
F-G	0.0	-18.5	-18.5	0.14 (4)	10.00	
G-D	0.0	-18.5	-18.5	0.14 (4)	10.00	

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	5	1	8	FRONT	VERT	TOTAL	-	C1
G	3-11.4	1	1	-	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 5 X 12 = 60 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSI: TC=0.12/1.00 (A-B:1), BC=0.14/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.09/1.00 (A-B:1)

DOL LUMBER=0.99 NAIL=0.99 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

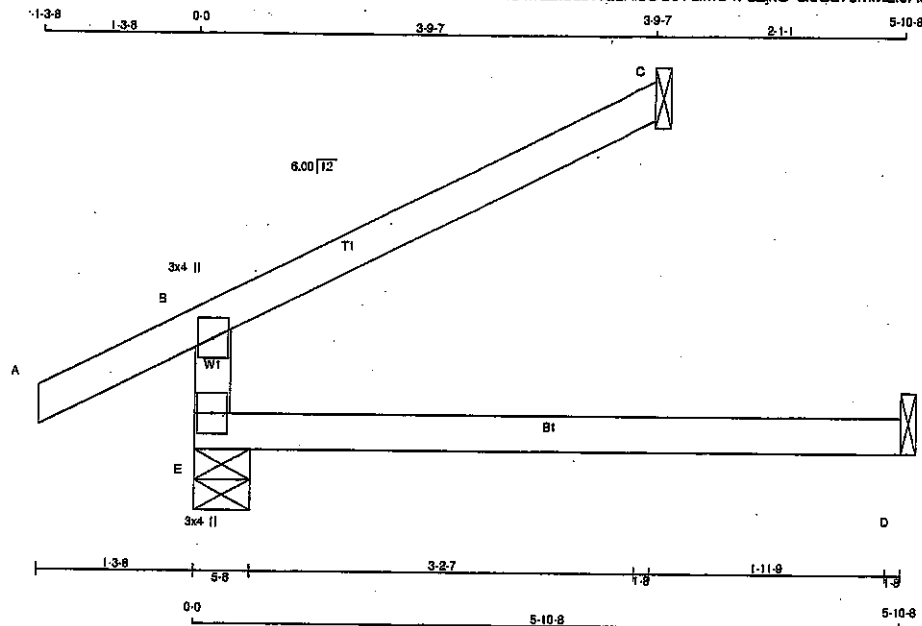
JSI GRIP = 0.09 (E) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108128

JOB NAME 417443	TRUSS NAME C34	QUANTITY 5	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:18.2

TOTAL WEIGHT = 5 X 14 = 71 lb

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				SPF
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
E	405	0	405	0	0	5-8	5-8	
C	130	0	130	0	0	1-8	1-8	
D	45	0	50	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
E	286	190	0	0	0	96	0
C	90	73	0	0	0	17	0
D	35	0	0	0	0	36	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT. LOAD	FACTORED		MAX. UNBRACED LENGTH	FR-TO	WESS	
	MAX. FACTORED FORCE (LBS)	PLF		LC1	MAX. (LBS)			MAX. FACTORED FORCE (LBS)	MAX. (LBS)
FR-TO		FROM	TO						
E-B	-342	0	0.0	0.0	0.13 (4)	7.81			
A-B	0	28	-91.8	-91.8	0.12 (1)	10.00			
B-C	-19	0	-91.8	-91.8	0.22 (1)	6.25			
E-D	0	0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.22/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

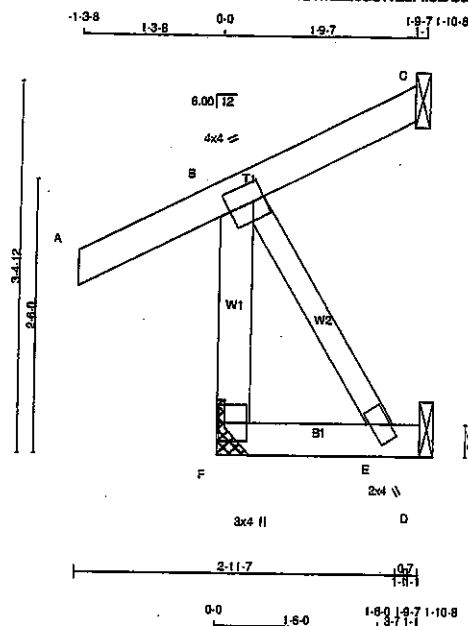
JSI GRIP= 0.14 (E) (INPUT = 0.80)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108129

JOB NAME 417443	TRUSS NAME C35	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/20.2

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	No.2
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00 1.25
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
F	271	0	271	0	0	MECHANICAL	IN-SX
C	35	0	35	0	-38	1-8	1-8
D	17	0	19	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	189	142 / 0	0 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	24	19 / -25	0 / 0	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO
F-B	-254 / 0	0.0 0.0 0.03 (1)	7.81
A-B	0 / 28	-91.8 -91.8 0.12 (1)	10.00
B-C	-21 / 0	-91.8 -91.8 0.11 (1)	6.25
F-E	0 / 0	-18.5 -18.5 0.02 (4)	10.00
E-D	0 / 0	-18.5 -18.5 0.01 (4)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 11 = 21 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT. CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.12/1.00 (A-B:1), BC=0.02/1.00 (E-F:4),
WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLJ) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1673

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.15 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)

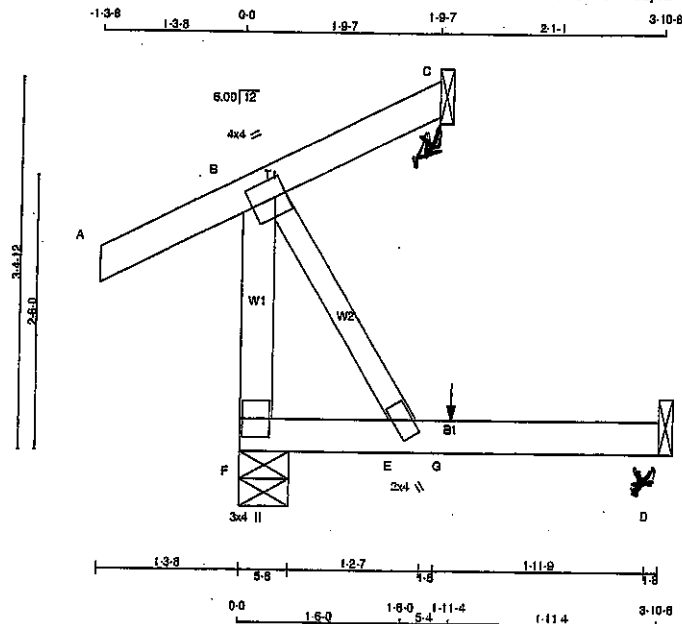


Structural component only
DWG# T-2108130

JOB NAME 417443	TRUSS NAME C36	QUANTITY 2	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/20

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESCR.	SPF
SPF	
SPF	
SPF	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
VERT	HORZ	DOWN	HORZ	UPLIFT
F	290	0	290	0
C	35	0	35	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
COMBINED	MAX. MIN.	COMPONENT REACTIONS					
F	203	142 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	24	19 / -26	0 / 0	0 / 0	0 / 0	5 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED	FACTORED	MAX. FACTORED	FACTORED
FR-TO	FORCE (LBS)	VERT. LOAD (PLF)	FORCE (LBS)	FORCE (LBS)
F-B	-254 / 0	0.0	0.0	0.0
A-B	0 / 28	-91.8	-91.8	0.14 (5)
B-C	-21 / 0	-91.8	-91.8	0.13 (5)
F-E	0 / 0	-18.5	-18.5	0.08 (4)
E-G	0 / 0	-18.5	-18.5	0.08 (4)
G-D	0 / 0	-18.5	-18.5	0.08 (4)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	1	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 13 = 26 lb

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 25.6 PSF
	DL = 8.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 39.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/380 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSK: TC=0.14/1.00 (A-B-5), BC=0.08/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SSI=0.10/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

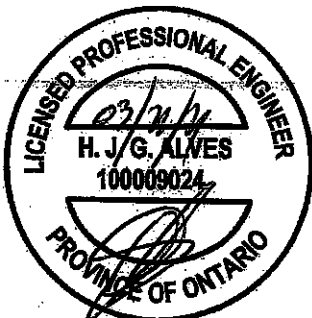
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

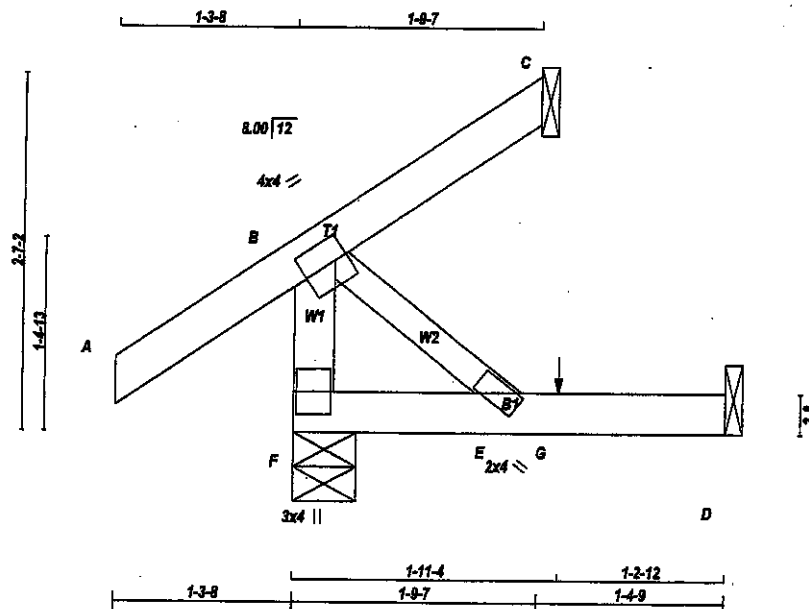
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.15 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)



Structural component only
DWG# T-2108131

JOB NAME 421219	TRUSS NAME C351	QUANTITY 1	PLY 1	JOB DESC. GREENPARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
Version 8.420 S Jan 21 2021 Mitek Industries, Inc. Tue Aug 24 17:17:28 2021 Page 1 ID:GV6EnpZB3IFFw?AU1oPMODzZmnp-5oIzE_yy9caPAUPH_MY0Ww6QFzPvKm?ISIL7vykamN					



Scale: 3/4"=1'

TOTAL WEIGHT = 10 b

LUMBER	DESCR.
N. L. G. A. RULES	
CHORDS	
F - B	2x4 DRY No.2 SPF
A - C	2x4 DRY No.2 SPF
F - D	2x4 DRY No.2 SPF
ALL WEBS	2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.	

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B - TMVW-t	MT20	4.0	4.0	2.00 1.00
E - BMV-w	MT20	2.0	4.0	
F - BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) Lateral braces to be a minimum of 2X4 SPF #2.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	286	0	286	0
C	34	0	34	0
D	29	0	33	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
F	200	143 / 0 0 / 0 0 / 0 0 / 0 57 / 0 0 / 0
C	23	19 / -27 0 / 0 0 / 0 0 / 0 4 / 0 0 / 0
D	23	0 / 0 0 / 0 0 / 0 0 / 0 23 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
F-B	-256 / 0	0.0 0.0 0.03 (1)	B-E	0 / 0 0.00 (1)
A-B	0 / 35	-91.8 -91.8 0.14 (5)		
B-C	-27 / 0	-91.8 -91.8 0.13 (5)		
F-E	0 / 0	-18.5 -18.5 0.05 (4)		
E-G	0 / 0	-18.5 -18.5 0.05 (4)		
G-D	0 / 0	-18.5 -18.5 0.05 (4)		

SPECIFIED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	MAX+	FRONT	VERT	TOTAL	-	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.9 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B-5), BC=0.05/1.00 (D-E-4), WB=0.00/1.00 (B-E-1), SS=0.09/1.00 (A-B-5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.16 (B) (INPUT = 0.90)
JSI METAL = 0.05 (B) (INPUT = 1.00)



Structural component only
DWG# T-2129735



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300218

Feb 09, 2018

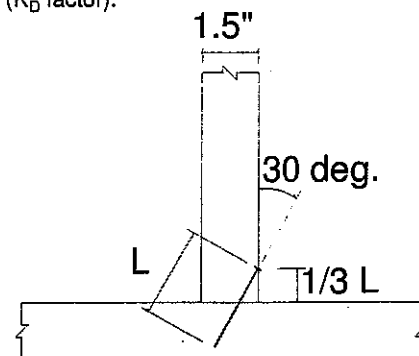
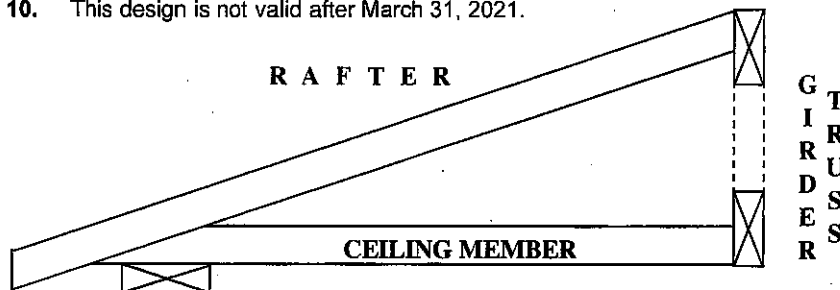
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B97791H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177
COMMON SPIRAL	3.00	0.122	97	108
	3.25	0.122	97	108
	3.50	0.152	145	162

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-14, section 12.9.4
10. This design is not valid after March 31, 2021.



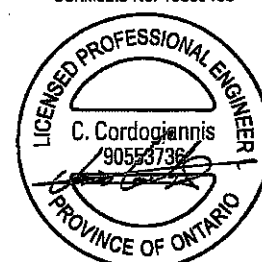
TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire	Common spiral
Nail dia. (in)	0.160	0.152	0.144	0.122
	(3.5" nail)		(3" and 3.25" nail)	
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS			
2X4 SPF	2	2	3	3
2X4 D. Fir	2	2	2	2
2X6 SPF	4	4	4	5
2X6 D. Fir	3	3	3	4

MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

PEO
Certificate No. 10889485



BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

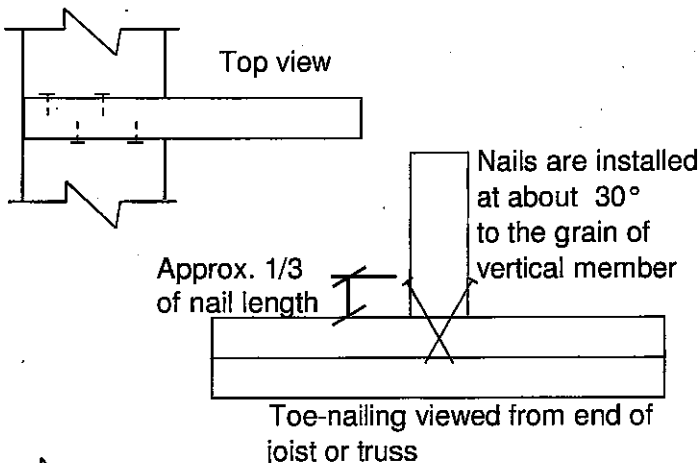
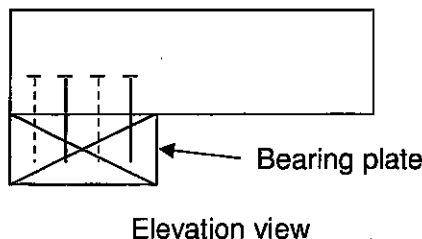
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

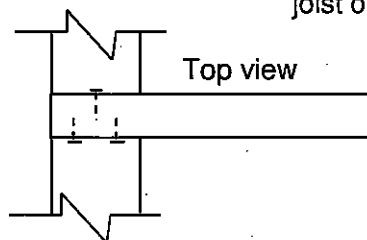
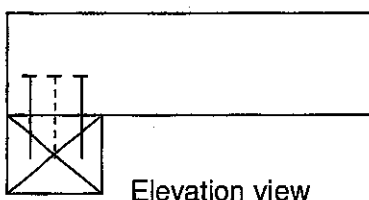
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



PEO
Certificate No. 10889485



MiTek® MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

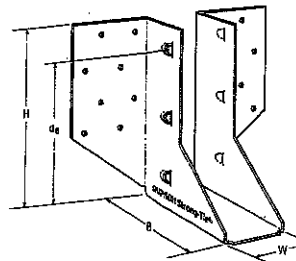
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

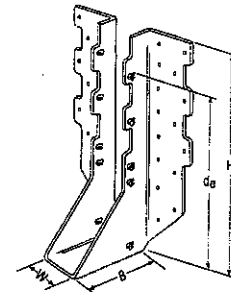
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

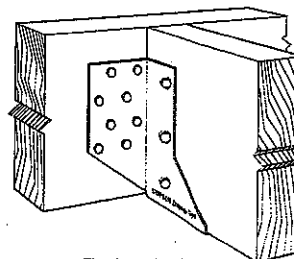
- See current catalogue for options



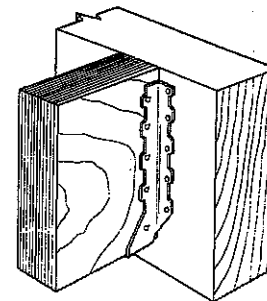
LJS26DS



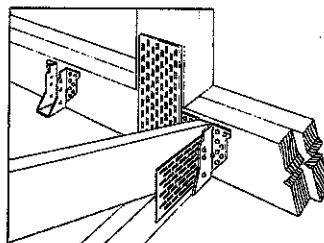
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



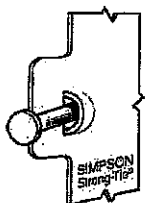
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

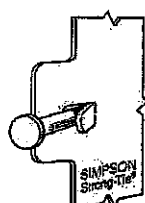
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1½	5	3½	4½	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5½	3	3½	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6½	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9½	3	7½	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

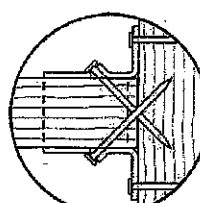


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

© 2020 Simpson Strong-Tie Company Inc.

T-SPECHUS20_3/20_exp_6/22

(800) 999-5099
strongtie.com

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

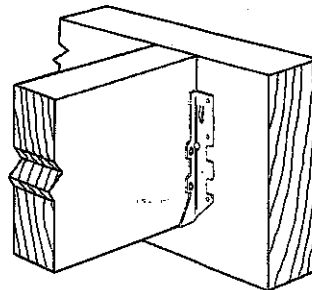
Finish: G90 galvanized

Design:

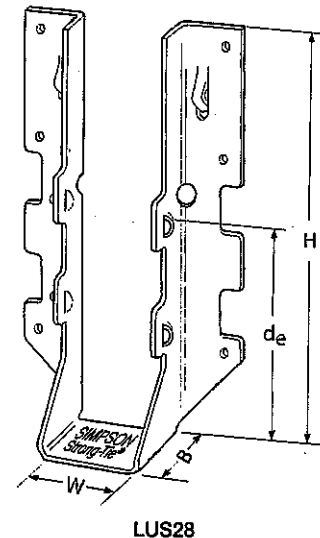
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

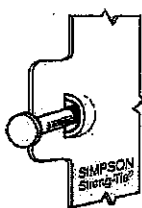


Typical LUS
Installation



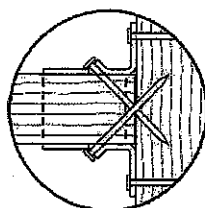
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/16	4 3/8	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 13/16	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



UNIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020.
This information is updated periodically and should not be relied upon after June 30, 2022.
Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

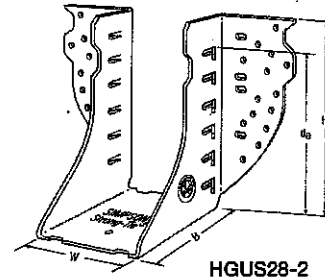
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

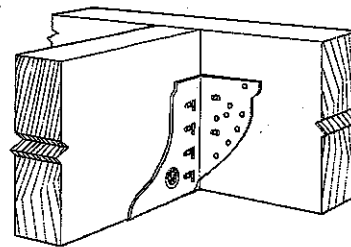
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

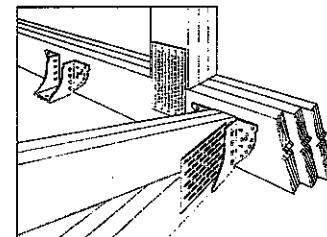
- See current catalogue for options



HGUS28-2



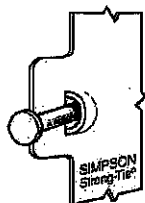
Typical HGUS Installation



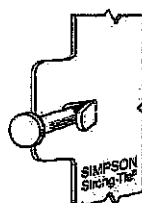
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _a ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 3/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 3/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 3/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 3/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/16	7 1/4	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 3/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 1/8	9 1/8	5	7 7/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 3/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 1/16	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 3/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 3/16	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 3/16	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

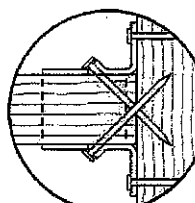
1. d_a is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

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T-SPECHGUS20-3/20 exp. 6/22

(800) 999-5099
strongtie.com

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

Material: 18 gauge **Finish:** G90 galvanized

Design: • Factored resistances are in accordance with CSA O86-14

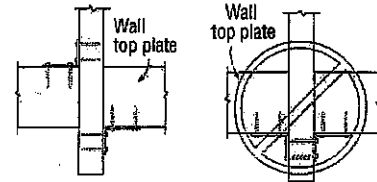
- Factored resistances have been increased 15%. No further increase is permitted.

Installation: • Use all specified fasteners

- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

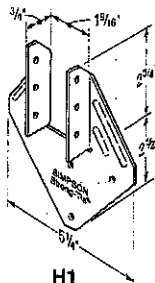
Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

Hurricane Tie Installations to Achieve Twice the Load (Top View)

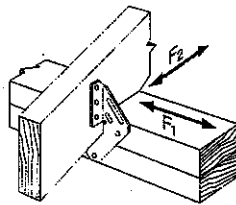


Install diagonally across from each other for minimum 2x truss.

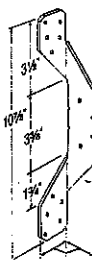
Nailing into both sides of a single ply 2x truss may cause the wood to split.



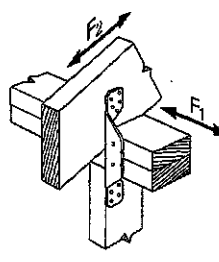
H1



H1 Installation



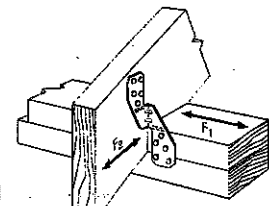
H2A



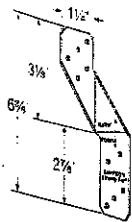
H2A Installation



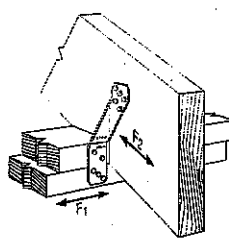
H2.5A



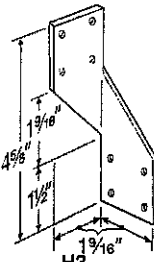
H2.5A Installation



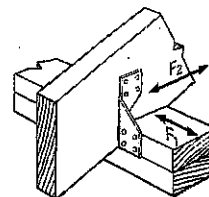
H2.5T



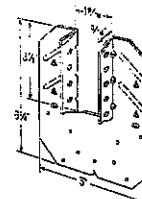
H2.5T Installation
(Nails into both top plates)



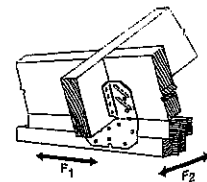
H3



H3 Installation



H10A



H10A Installation

Model No.	Ga.	Fasteners			Factored Resistance (lb.)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Normal		Uplift	Normal	
						F ₁	F ₂		F ₁	F ₂
						(K _p =1.15)			(K _p =1.15)	
H1	18	(6) 8d x 1½"	(4) 8d	—	740	685	300	680	485	215
H2A	18	(5) 8d x 1½"	(2) 8d x 1½"	(5) 8d x 1½"	830	220	75	590	155	55
H2.5A	18	(5) 8d	(5) 8d	—	805	160	160	755	160	160
H2.5T	18	(5) 8d	(5) 8d	—	835	175	240	740	160	210
H3	18	(4) 8d	(4) 8d	—	740	180	265	615	125	190
H10A	18	(9) 10d x 1½"	(9) 10d x 1½"	—	1735	795	410	1505	565	290

1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.

3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



LIMIT
STATES
DESIGN

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T-SPECH20 3/20 exp. 6/22

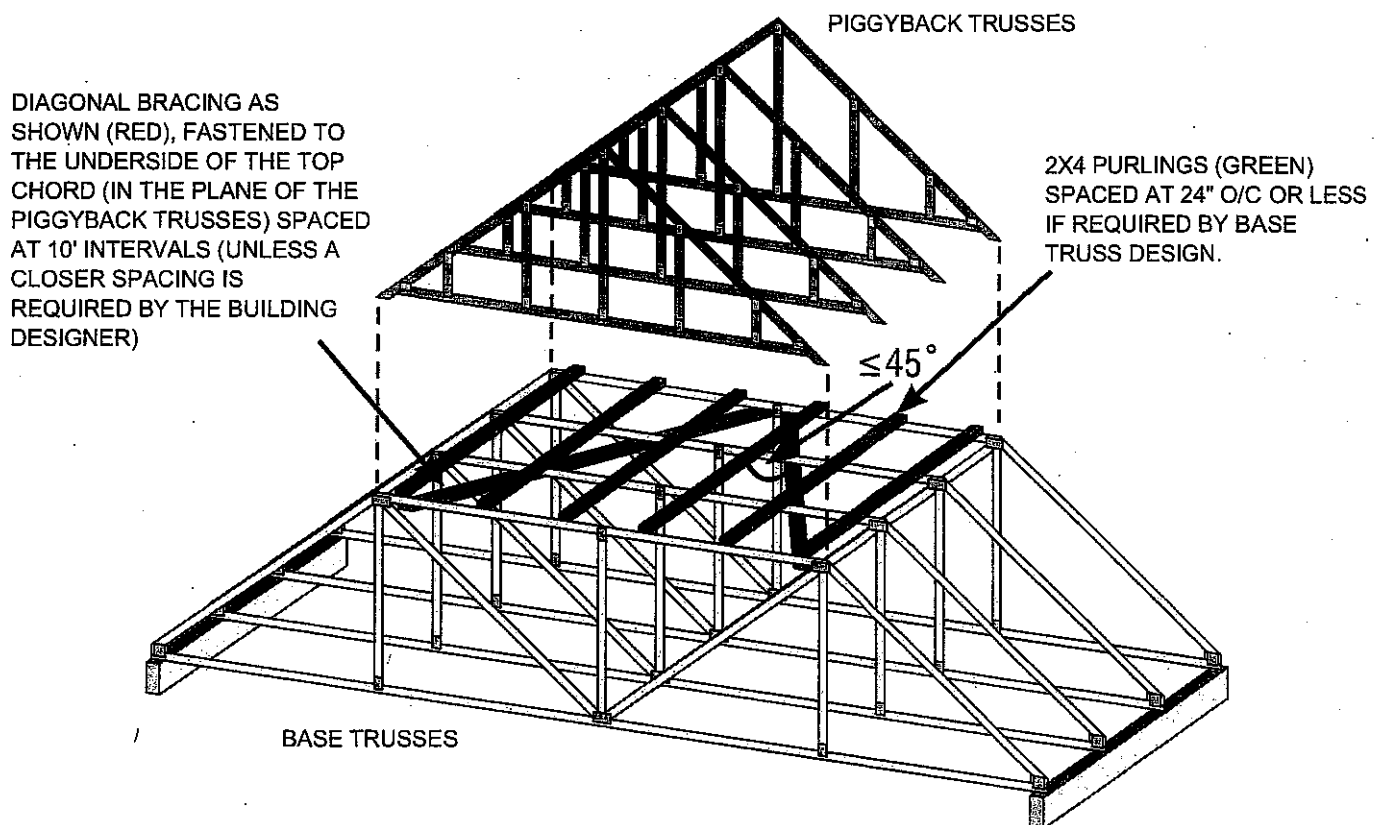
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected otop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

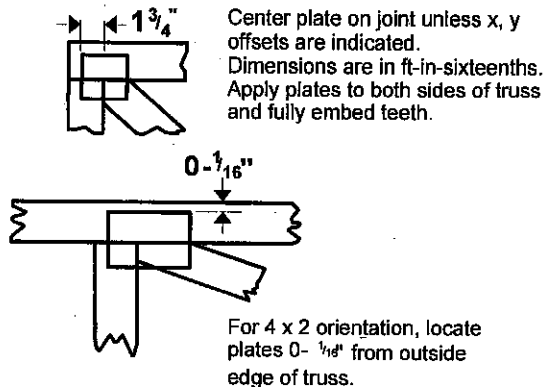
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

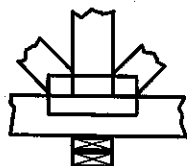
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

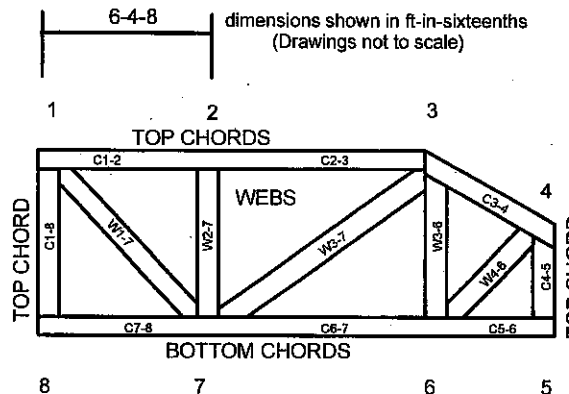


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MIL-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.