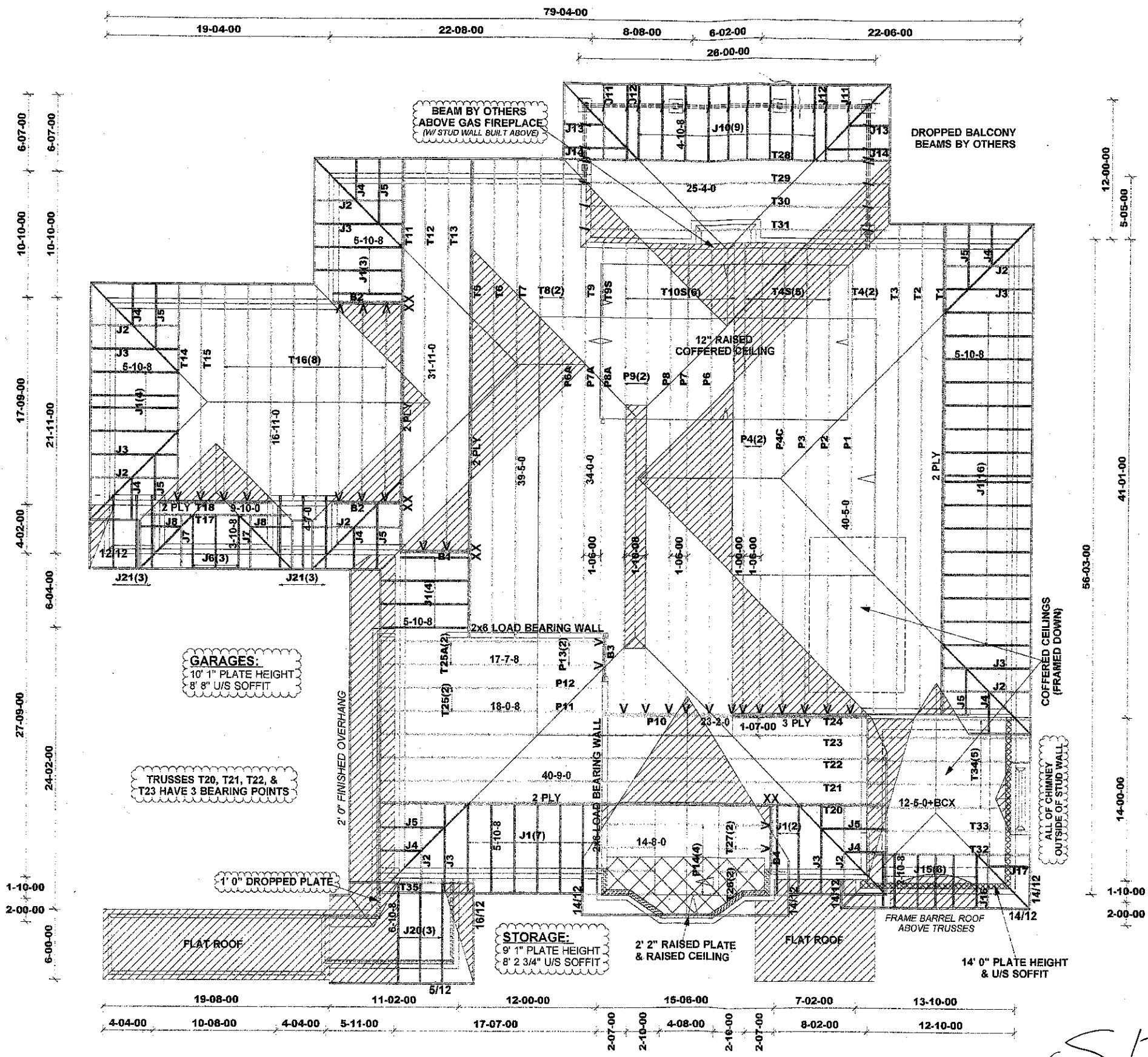




ASPHALT SHINGLES
 FINISHED OVERHANG: 16"
 CLADDING ALLOWANCE: 5"
 2x6 EXTERIOR WALLS
 2x6 FASCIA BOARD
 HEEL: R.T.M.C.

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DENOTES:
 CONVENTIONAL
 FRAMING

DESIGN CONFORMS WITH OBC 2012
 OCCUPANCY: RESIDENTIAL | PART: 9
 Ss = 31.3 psf | Sr = 8.4 psf

DESIGN LOADS:
 TCSL = 25.6 psf
 TCDL = 6.0 psf
 BCCL = 0.0 psf
 BCDL = 7.4 psf

HARDWARE:
 LJS26DS - (V)
 HGUS26-2 - (XX)
 H2.5T - (I)

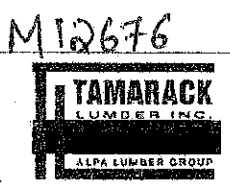
BEAMS:
 B1 - B4: 2 - 2 x 10 SPF #2

ALL ROOF SLOPES ARE 9/12
 UNLESS OTHERWISE NOTED

CITY OF RICHMOND HILL
BUILDING SERVICES DIVISION

 MAY 25 2020
RECEIVED
 Per: _____

SITE



Job Track: 50916
 Plan Log: 201766
 Layout ID: 405684

Builder / Location: **GREENYORK HOMES / RICHMOND HILL**
 Project: **BALTRIO**
 Date: 9/27/2019 Sales: Mario DiCano

Model / Elevation: **AVIGNON 3**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milek ver 8.3.0.234



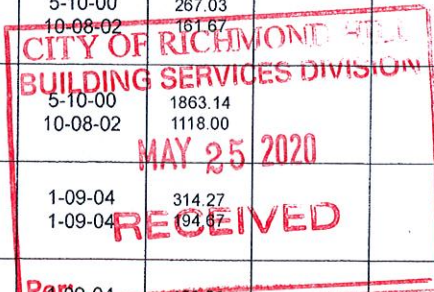
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENYORK HOMES
 Project: BALTRIO
 Location: RICHMOND HILL
 Model: AVIGNON 3
 Lot #:
 Elevation: CUSTOM

Job Track: 50916
 PlanLog: 201766
 Layout ID: 405684
 Ref #: 11515
 Page: 1 of 6
 Date: 09/27/2019
 Designer:
 Sales Rep: Mario DiCano

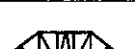
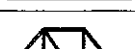
Roof Trusses

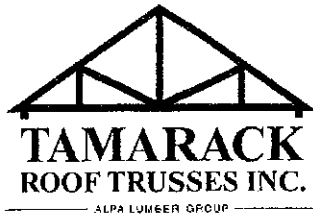
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	40-05-00	6-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	397.26 245.33		
	1	T2 Hip	9/12	40-05-00	7-08-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	211.62 131.50		
	1	T3 Hip	9/12	40-05-00	9-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	226.4 139.50		
	2	T4 Hip	9/12	40-05-00	10-08-02	2 x 4 2 x 6		1-09-04 1-09-04	464.87 285.33		
	5	T4S Piggyback Base	9/12	40-05-00	10-08-02	2 x 4 2 x 6		1-09-04 1-09-04	1231.54 760.83		
	1 2-ply	T5 Half Hip Girder	9/12	39-05-00	6-02-02	2 x 4 2 x 6	1-07-08	1-09-04 6-02-02	392.08 240.67		
	1	T6 Half Hip	9/12	39-05-00	7-08-02	2 x 4 2 x 6	1-07-08	1-09-04 7-08-02	211.17 128.50		
	1	T7 Half Hip	9/12	39-05-00	9-02-02	2 x 4 2 x 6	1-07-08	1-09-04 9-02-02	257.08 157.17		
	2	T8 Half Hip	9/12	39-05-00	10-08-02	2 x 4 2 x 6	1-07-08	1-09-04 10-08-02	513.19 302.67		
	1	T9 Piggyback Base	9/12	34-00-00	10-08-02	2 x 4 2 x 6		5-10-00 10-08-02	240.21 143.33		
	1	T9S Piggyback Base	9/12	34-00-00	10-08-02	2 x 4 2 x 6		5-10-00 10-08-02	267.03 161.67		
	6	T10S Half Hip	9/12	40-05-00	10-08-02	2 x 4 2 x 6		5-10-00 10-08-02	1863.14 1118.00		
	1 2-ply	T11 Hip Girder	9/12	31-11-00	6-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	314.27 194.67		
	1	T12 Hip	9/12	31-11-00	7-08-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	153.59 97.50		



 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST							
	Lumber Yard: TAMARACK LUMBER						Job Track: 50916	
	Builder: GREENYORK HOMES						PlanLog: 201766	
	Project: BALTRIO						Layout ID: 405684	
	Location: RICHMOND HILL						Ref #: 11515	
	Model: AVIGNON 3						Page: 2 of 6	
Lot #:						Date: 09/27/2019		
Elevation: CUSTOM						Designer:		
						Sales Rep: Mario DiCano		

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T13 Hip	9/12	31-11-00	9-02-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	160.34 99.17		
	1	T14 Hip Girder	9/12	16-11-00	6-02-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	78.46 50.33		
	1	T15 Hip	9/12	16-11-00	7-08-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	81.41 52.67		
	8	T16 Common	9/12	16-11-00	8-01-06	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	614.84 378.67		
	1	T17 Hip Girder	9/12	9-10-00	3-11-02	2 x 4	1-07-08 1-07-08	1-09-04 1-09-04	46.46 30.17		
	1 2-ply	T18 Hip Girder	9/12	9-10-00	4-08-08	2 x 4 2 x 6		1-09-04 1-09-04	100.07 66.67		
	1 2-ply	T20 Hip Girder	9/12	40-09-00	6-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	407.1 250.67		
	1	T21 Hip	9/12	40-09-00	7-08-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	227.84 141.67		
	1	T22 Hip	9/12	40-09-00	9-02-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	228.96 140.67		
	1	T23 Hip	9/12	40-09-00	10-08-02	2 x 4 2 x 6	1-07-08 1-07-08	1-09-04 1-09-04	248.92 152.67		
	1 3-ply	T24 Piggyback Base Girder	9/12	23-02-00	10-08-02	2 x 6		1-09-04 10-08-02	531.53 320.00		
	2	T25 Piggyback Base	9/12	18-00-08	10-08-02	2 x 4	1-07-08	1-09-04 10-08-02	201.55 127.67		
	2	T25A Piggyback Base	9/12	17-07-08	10-08-02	2 x 4	1-07-08	1-09-04 10-08-02	199.2 125.00		
	2	T26 Piggyback Base	14/12	14-08-00	7-10-00	2 x 4	1-07-08 1-07-08	2-06-00 2-06-00	154.21 97.67		



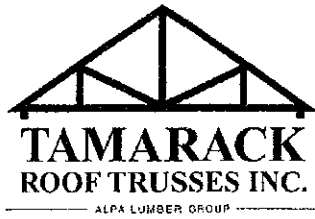
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENYORK HOMES
 Project: BALTRIO
 Location: RICHMOND HILL
 Model: AVIGNON 3
 Lot #:
 Elevation: CUSTOM

Job Track: 50916
 PlanLog: 201766
 Layout ID: 405684
 Ref #: 11515
 Page: 3 of 6
 Date: 09/27/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T27 Piggyback Base	14 /12	14-08-00	10-00-00	2 x 4		4-08-00 4-08-00	173.93 111.33		
	1	T28 Hip Girder	9 /12	25-04-00	5-04-06	2 x 4	1-06-08 1-06-08	1-08-08 1-08-08	113.5 72.00		
	1	T29 Hip	9 /12	25-04-00	6-10-06	2 x 4	1-06-08 1-06-08	1-08-08 1-08-08	116.34 73.50		
	1	T30 Hip	9 /12	25-04-00	8-04-06	2 x 4	1-06-08 1-06-08	1-08-08 1-08-08	123.06 77.33		
	1	T31 Hip	9 /12	25-04-00	9-10-06	2 x 4	1-06-08 1-06-08	1-08-08 1-08-08	126.12 80.33		
	1	T32 Hip Girder	14 /12	12-05-00	5-10-04	2 x 4	1-07-08	2-06-00 3-02-03	70.44 46.67		
	1	T33 Hip	14 /12	12-05-00	8-02-04	2 x 4	1-07-08	2-06-00 3-02-03	71.42 46.00		
	5	T34 Common	14 /12	12-05-00	10-01-00	2 x 4	1-07-08	2-06-00 3-02-03	373.08 238.33		
	1	T35 Roof Special Girder	9 /12	6-08-00	3-10-15	2 x 4	2-03-08 1-07-08	1-10-04 1-11-05	37.8 25.33		
	1	P1 Piggyback	9 /12	16-08-00	1-06-00	2 x 4			40.82 25.83		
	1	P2 Piggyback	9 /12	16-08-00	3-00-00	2 x 4			51.88 32.83		
	1	P3 Piggyback	9 /12	16-08-00	4-06-00	2 x 4			50.69 31.67		
	2	P4 Piggyback	9 /12	16-08-00	6-03-00	2 x 4			106.1 66.67		
	1	P4C Piggyback	9 /12	16-08-00	6-00-00	2 x 4			52.71 33.33		



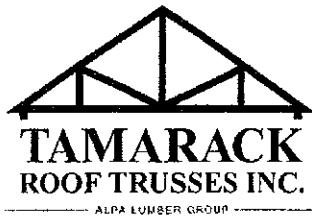
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENYORK HOMES
 Project: BALTRIO
 Location: RICHMOND HILL
 Model: AVIGNON 3
 Lot #:
 Elevation: CUSTOM

Job Track: 50916
 PlanLog: 201766
 Layout ID: 405684
 Ref #: 11515
 Page: 4 of 6
 Date: 09/27/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	P6 Piggyback	9/12	33-11-08	1-06-00	2 x 4		1-06-00	86.5 53.33		
	1	P6A Piggyback	9/12	27-06-08	1-06-00	2 x 4		1-06-00	70.95 44.17		
	1	P7 Piggyback	9/12	33-11-08	3-00-00	2 x 4		1-05-10	113.88 72.50		
	1	P7A Piggyback	9/12	27-06-08	3-00-00	2 x 4		3-00-00	93.12 59.33		
	1	P8 Piggyback	9/12	33-11-08	4-01-08	2 x 4		1-05-10	119.74 74.67		
	1	P8A Piggyback	9/12	27-06-08	4-01-08	2 x 4		4-01-08	99.12 62.67		
	2	P9 Piggyback	9/12	33-11-08	5-07-08	2 x 4		1-05-10	264.58 164.00		
	1 3-ply	P10 Piggyback	9/12	11-03-08	1-02-04	2 x 4		1-02-04	89.74 58.00		
	1	P11 Piggyback	9/12	6-02-00	1-06-00	2 x 4		1-06-00	18.6 13.17		
	1	P12 Piggyback	9/12	6-02-00	3-00-00	2 x 4		3-00-00	21.36 14.33		
	2	P13 Piggyback	9/12	5-09-00	4-03-12	2 x 4		4-03-12	40.87 25.67		
	4	P14 Piggyback	14/12	5-06-05	3-02-11	2 x 4			66.95 46.00		
	36	J1 Jack-Open	9/12	5-10-08	6-02-02	2 x 4	1-07-08	1-09-04 6-02-02	745.27 468.00		
	8	J2 Jack-Open Girder	9/12	1-09-07	3-01-05	2 x 4	1-07-08 4-01-01	1-09-04 3-01-05	106.66 69.33		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREENYORK HOMES
 Project: BALTRIO
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Job Track: 50916
 PlanLog: 201766
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 Page: 5 of 6
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	7	J3 Jack-Open	9/12	3-09-07	4-07-05	2 x 4	1-07-08 2-01-01	1-09-04 4-07-05	113.28 70.00		
	8	J4 Jack-Open	9/12	1-09-07	3-01-05	2 x 4	1-07-08 1-01	1-09-04 3-01-05	70.17 48.00		
	8	J5 Jack-Open	9/12	1-10-08	4-07-05	2 x 4	1-07-08 1-10-15	1-09-04 3-02-02	92.97 58.67		
	3	J6 Jack-Open	9/12	2-10-08	3-11-02	2 x 4	1-07-08	1-09-04 3-11-02	39.01 25.00		
	2	J7 Jack-Open Girder	9/12	1-09-07	3-01-05	2 x 4	1-07-08 1-01-01	1-09-04 3-01-05	19.82 13.33		
	2	J8 Jack-Open	9/12	1-09-07	3-01-05	2 x 4	1-07-08 1-01	1-09-04 3-01-05	17.54 12.00		
	9	J10 Jack-Open	9/12	4-10-08	5-04-06	2 x 4	1-06-08	1-08-08 5-04-06	170.44 111.00		
	2	J11 Jack-Open Girder	9/12	1-09-07	3-00-09	2 x 4	1-06-08 3-01-01	1-08-08 3-00-09	26.11 17.33		
	2	J12 Jack-Open	9/12	3-09-07	4-06-09	2 x 4	1-06-08 1-01-01	1-08-08 4-06-09	31.81 20.00		
	2	J13 Jack-Open	9/12	1-09-07	3-00-09	2 x 4	1-06-08 1-01	1-08-08 3-00-09	19.27 13.33		
	2	J14 Jack-Open	9/12	1-10-08	4-06-09	2 x 4	1-06-08 1-10-15	1-08-08 3-01-06	24.97 16.00		
	6	J15 Jack-Open	14/12	2-10-08	5-10-04	2 x 4	1-07-08	2-06-00 5-10-04	95.99 65.00		
	1	J16 Jack-Open Girder	14/12	1-09-07	4-07-00	2 x 4	1-07-08 1-01-01	2-06-00 4-07-00	12.04 8.00		
	1	J17 Jack-Open	14/12	1-09-07	4-07-00	2 x 4	1-07-08 1-01	2-06-00 4-07-00	10.9 7.33		

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP		DELIVERY SHIPLIST	
		Lumber Yard: TAMARACK LUMBER Builder: GREENYORK HOMES Project: BALTRIO Location: RICHMOND HILL Model: AVIGNON 3 Lot #: Elevation: CUSTOM	Job Track: 50916 PlanLog: 201766 Layout ID: 405684 Ref #: 11515 Page: 6 of 6 Date: 09/27/2019 Designer: Sales Rep: Mario DiCano

Roof Trusses

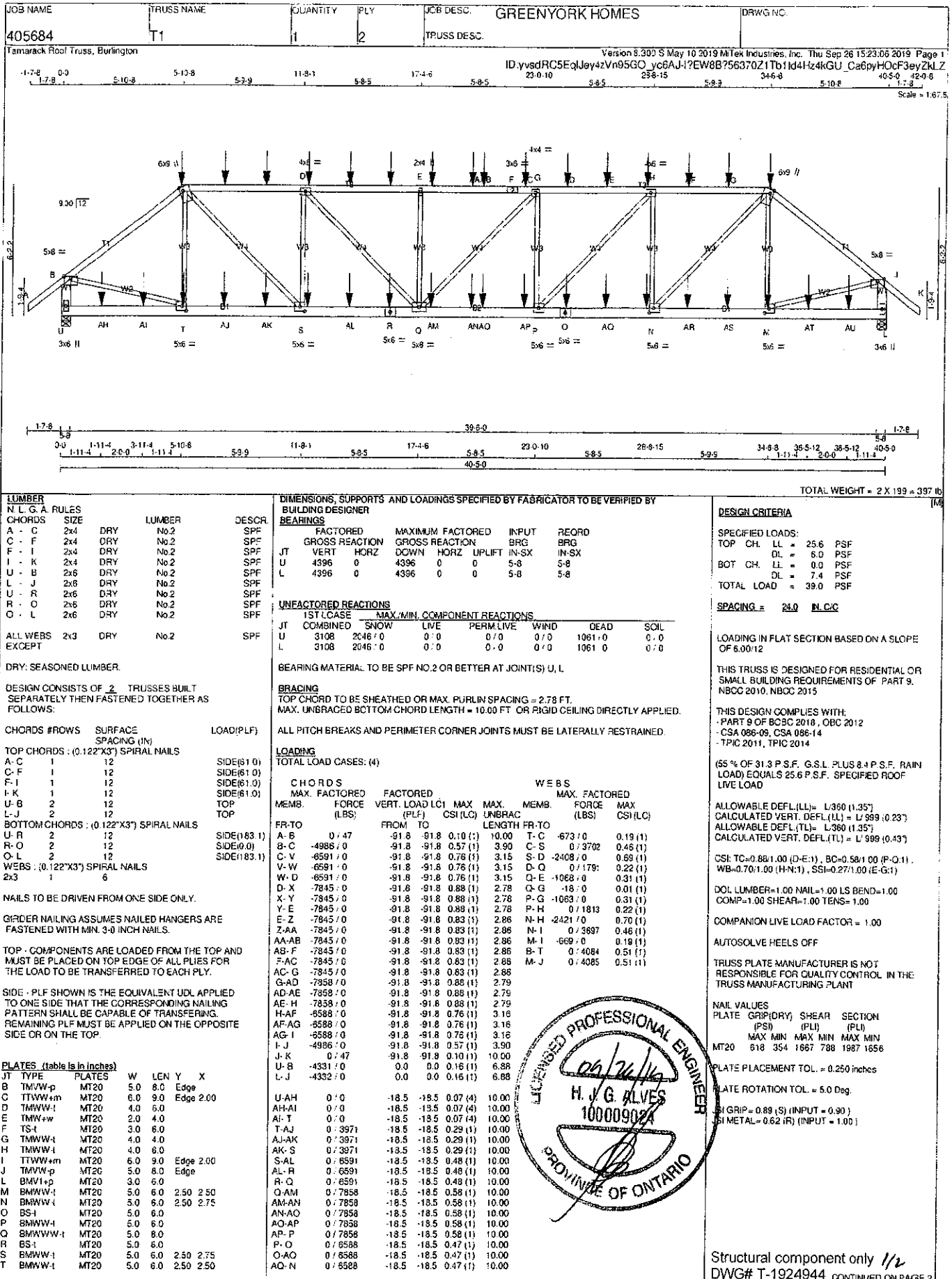
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	J20 Jack-Open	5 /12	6-10-08	3-10-05	2 x 4	1-07-08	4-01 3-02-07	62.88 40.00		
	6	J21 Jack-Open	12 /12	4-07-00	6-07-07	2 x 4	1-07-08	9-08 5-04-08	96.02 64.00		

TOTAL # TRUSS= 203 TOTAL BFT OF ALL TRUSSES= 8915.68 BFT. TOTAL WEIGHT OF ALL TRSSES ^{14372.7}₆ LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
28	Hardware	LJS26DS	
4	Hardware	HGUS26-2	
27	Hardware	H2.5T	

TOTAL NUMBER OF
 ITEMS= 59



JOB NAME 405684	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC GREENYORK HOMES	DRAWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 Mitek Industries, Inc. Thu Sep 26 15:23:05 2019 Page 2	
				ID:yvsdRC5EqJey4zVn95GO yc6AJ-I?EW8B75637021Tb1Id4Hz4kGU_Ca6oyH0cF3eyZkLZ	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMV1+ip	MT20	30	60		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

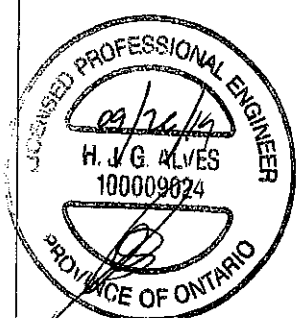
CHORDS				WEBS			
MEMB	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MAX. UNBRAC	MEMB. FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO		
N-AR	0 / 3971	-18.5	-18.5	0.29 (1)	10.00		
AR-AS	0 / 3971	-18.5	-18.5	0.29 (1)	10.00		
AS-M	0 / 3971	-18.5	-18.5	0.29 (1)	10.00		
M-AT	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
AT-AU	0 / 0	-18.5	-18.5	0.07 (4)	10.00		
AU-L	0 / 0	-18.5	-18.5	0.07 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

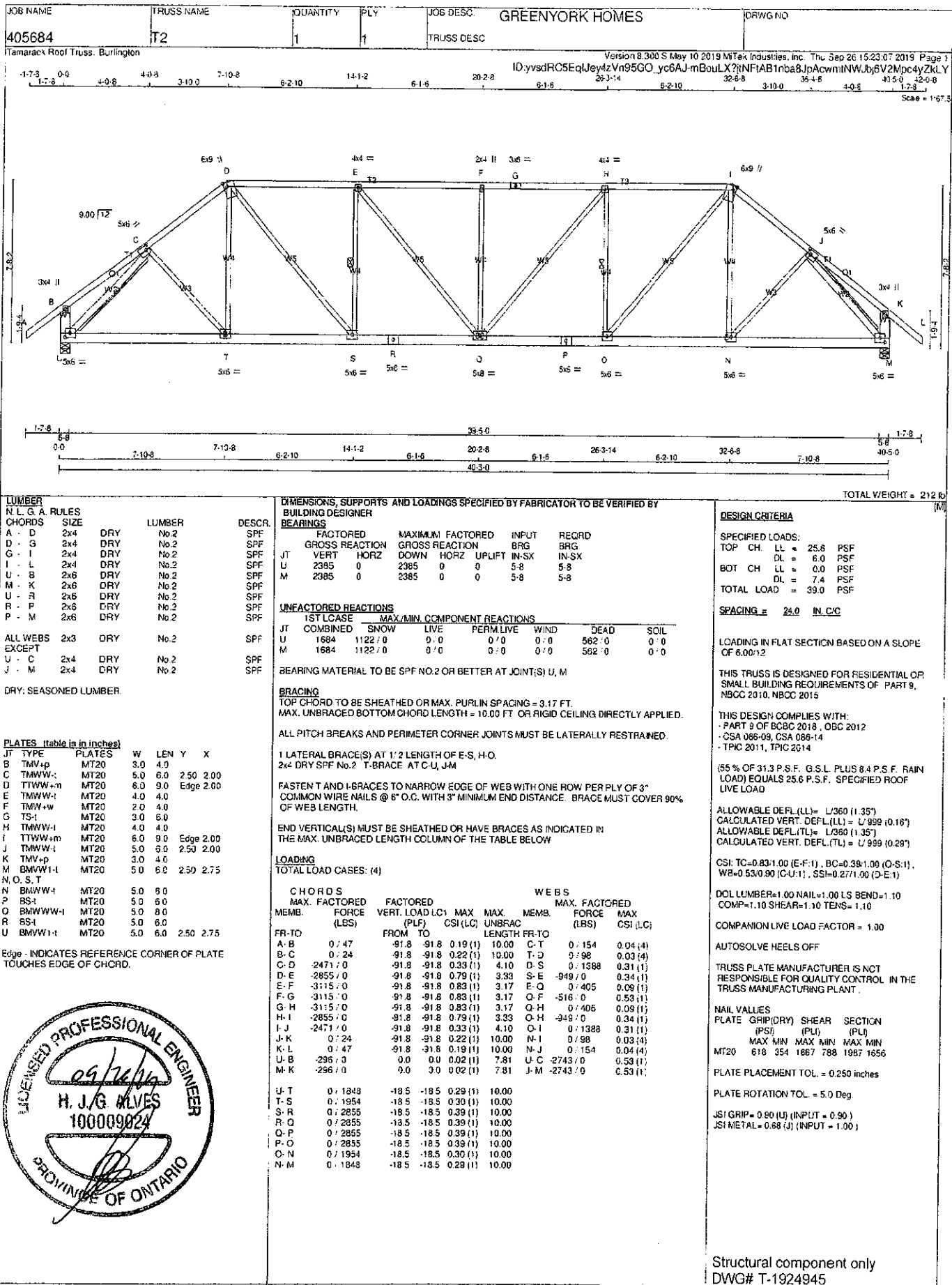
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-428	-428	---	BACK	VERT	TOTAL	---	C1
D	11-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
H	28-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
I	34-6-8	-428	-428	---	BACK	VERT	TOTAL	---	C1
M	34-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
N	28-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
O	24-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
R	15-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
S	11-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
T	5-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
V	7-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
W	9-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
X	13-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
Y	15-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
Z	17-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
AA	19-11-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
AB	20-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AC	22-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AD	24-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AE	26-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AF	30-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AG	32-5-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AH	1-11-4	-23	-23	---	BACK	VERT	TOTAL	---	C1
AI	3-11-4	-28	-28	---	BACK	VERT	TOTAL	---	C1
AJ	7-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AK	9-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AL	13-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AM	17-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AN	19-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AO	20-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AP	22-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AQ	26-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AR	30-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AS	32-5-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AT	36-5-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AU	38-5-12	-23	-23	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

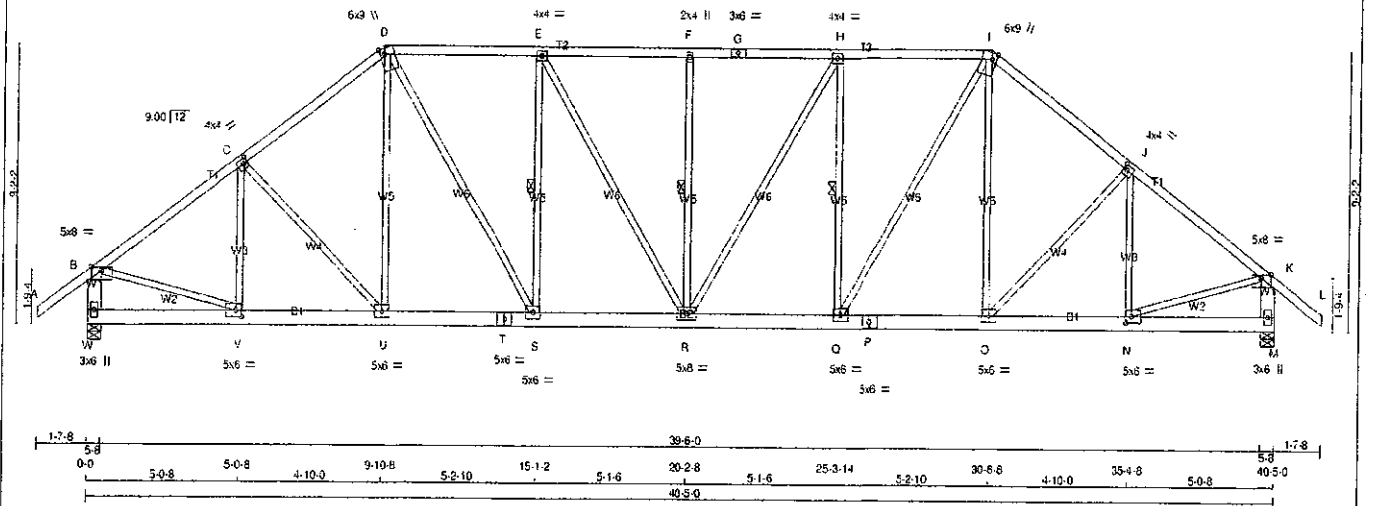
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



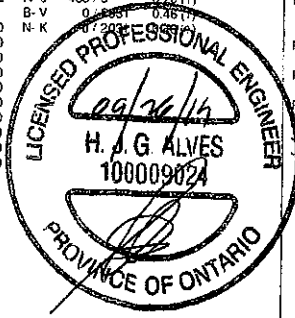
Structural component only
DWG# T-1924944



JOB NAME 405684	TRUSS NAME T3	QUANTITY 1	FLY 1	JOB DESC. GREENYORK HOMES	DRWG NO.
Temarack Roof Truss Burlington				Version 8.300 S May 10 2019 Mitok Industries, Inc. Thu Sep 26 15:23:09 2019 Page 1	
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Scale = 1:37.5					

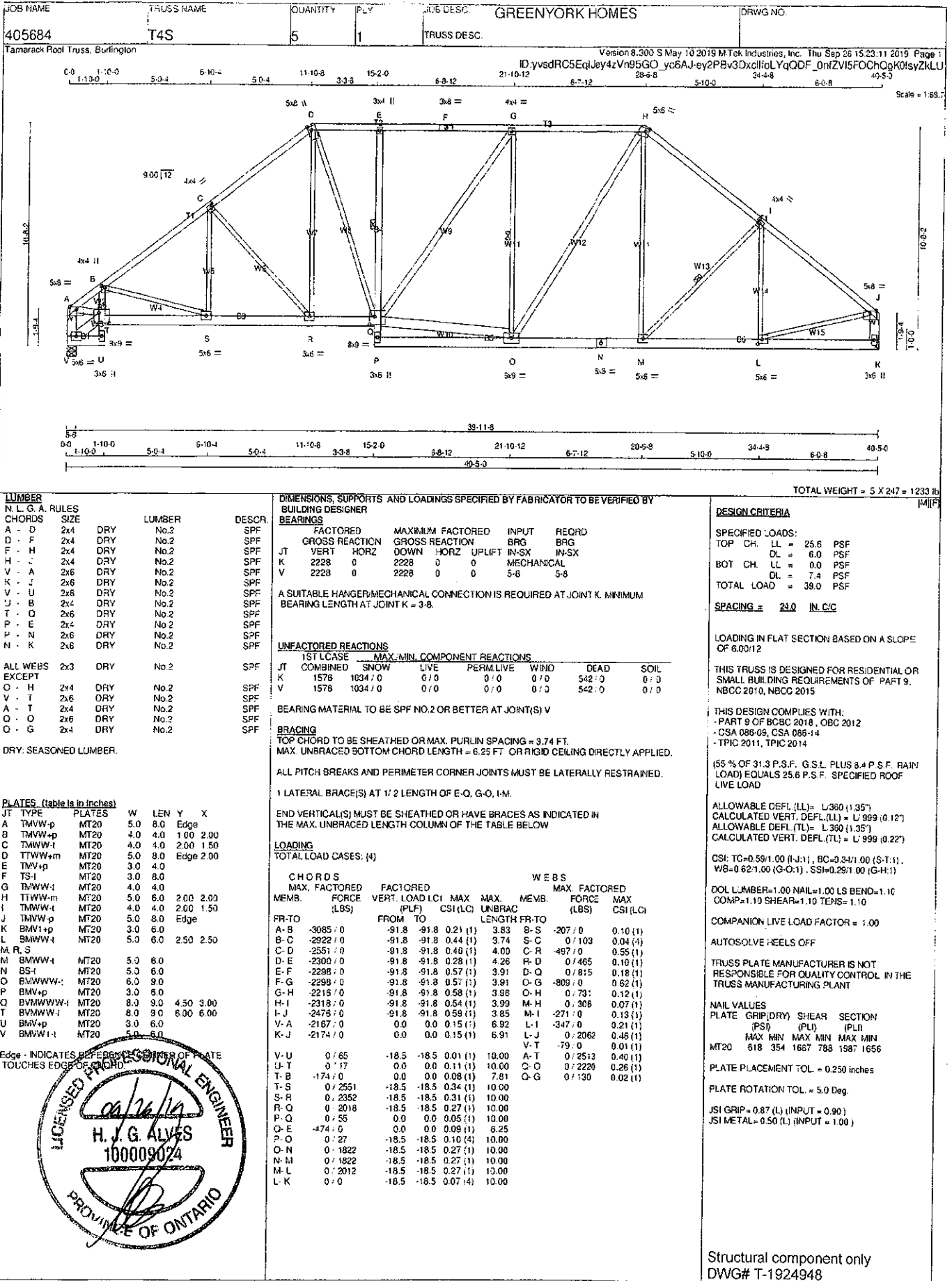


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - D 2x4 DRY No.2 SPF D - G 2x4 DRY No.2 SPF G - I 2x4 DRY No.2 SPF I - L 2x4 DRY No.2 SPF W - B 2x6 DRY No.2 SPF M - K 2x6 DRY No.2 SPF W - T 2x6 DRY No.2 SPF T - P 2x5 DRY No.2 SPF P - M 2x5 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY, SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table border="1"> <thead> <tr> <th></th> <th>FACTORED GROSS REACTION</th> <th>MAXIMUM FACTORED GROSS REACTION</th> <th>INPUT BRG</th> <th>RECORD BRG</th> </tr> </thead> <tbody> <tr> <td>JT VERT</td> <td>2385</td> <td>0</td> <td>5-8</td> <td>5-8</td> </tr> <tr> <td>W</td> <td>2385</td> <td>0</td> <td>5-8</td> <td>5-8</td> </tr> <tr> <td>M</td> <td>2385</td> <td>0</td> <td>5-8</td> <td>5-8</td> </tr> </tbody> </table> UNFACTORED REACTIONS <table border="1"> <thead> <tr> <th>1ST CASE</th> <th>MAX. MIN. COMPONENT REACTIONS</th> </tr> </thead> <tbody> <tr> <td>JT COMBINED</td> <td>SNOW LIVE PERM LIVE WIND DEAD SOIL</td> </tr> <tr> <td>W</td> <td>1684 1122.0 0.0 0.0 0.0 562.0 0.0</td> </tr> <tr> <td>M</td> <td>1684 1122.0 0.0 0.0 0.0 562.0 0.0</td> </tr> </tbody> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.51 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E, S, F, R, H, Q. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) <table border="1"> <thead> <tr> <th colspan="4">CHORDS</th> <th colspan="4">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>MAX. CSI (LC)</th> <th>MEMB.</th> <th>MAX. 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CSI (LC)</th> </tr> </thead> <tbody> <tr> <td>FR-TO</td> <td></td> <td>FROM TO</td> <td></td> <td>FR-TO</td> <td></td> <td></td> </tr> <tr> <td>A-B</td> <td>0 / 47</td> <td>-91.8 -91.8 0.19 (1)</td> <td>10.00</td> <td>V-C</td> <td>-453.0</td> <td>0.20 (1)</td> </tr> <tr> <td>B-C</td> <td>-2420 / 0</td> <td>-91.8 -91.8 0.55 (1)</td> <td>3.91</td> <td>C-U</td> <td>-99.0</td> <td>0.09 (1)</td> </tr> <tr> <td>C-D</td> <td>-2408 / 0</td> <td>-91.8 -91.8 0.53 (1)</td> <td>3.92</td> <td>U-D</td> <td>0 / 176</td> <td>0.04 (4)</td> </tr> <tr> <td>D-E</td> <td>-2427 / 0</td> <td>-91.8 -91.8 0.49 (1)</td> <td>3.92</td> <td>D-S</td> <td>0 / 1034</td> <td>0.23 (1)</td> </tr> <tr> <td>E-F</td> <td>-2572 / 0</td> <td>-91.8 -91.8 0.50 (1)</td> <td>3.81</td> <td>S-E</td> <td>-784 / 0</td> <td>0.41 (1)</td> </tr> <tr> <td>F-G</td> <td>-2572 / 0</td> <td>-91.8 -91.8 0.50 (1)</td> <td>3.81</td> <td>E-R</td> <td>0 / 290</td> <td>0.07 (1)</td> </tr> <tr> <td>G-H</td> <td>-2572 / 0</td> <td>-91.8 -91.8 0.50 (1)</td> <td>3.81</td> <td>R-F</td> <td>-431 / 0</td> <td>0.23 (1)</td> </tr> <tr> <td>H-I</td> <td>-2427 / 0</td> <td>-91.8 -91.8 0.49 (1)</td> <td>3.92</td> <td>F-H</td> <td>0 / 290</td> <td>0.07 (1)</td> </tr> <tr> <td>I-J</td> <td>-2408 / 0</td> <td>-91.8 -91.8 0.53 (1)</td> <td>3.92</td> <td>Q-H</td> <td>-784 / 0</td> <td>0.41 (1)</td> </tr> <tr> <td>J-K</td> <td>-2420 / 0</td> <td>-91.8 -91.8 0.55 (1)</td> <td>3.91</td> <td>H-I</td> <td>0 / 1034</td> <td>0.23 (1)</td> </tr> <tr> <td>K-L</td> <td>0 / 47</td> <td>-91.8 -91.8 0.19 (1)</td> <td>10.00</td> <td>I-L</td> <td>0 / 176</td> <td>0.04 (4)</td> </tr> <tr> <td>W-B</td> <td>-2335 / 0</td> <td>0.0 0.0 0.18 (1)</td> <td>6.72</td> <td>O-J</td> <td>-93 / 0</td> <td>0.09 (1)</td> </tr> <tr> <td>M-K</td> <td>-2335 / 0</td> <td>0.0 0.0 0.18 (1)</td> <td>6.72</td> <td>N-J</td> <td>-453.0</td> <td>0.20 (1)</td> </tr> <tr> <td>W-V</td> <td>0 / 0</td> <td>-18.5 -18.5 0.05 (4)</td> <td>10.00</td> <td>B-V</td> <td>0 / 290</td> <td>0.07 (1)</td> </tr> <tr> <td>V-U</td> <td>0 / 1961</td> <td>-18.5 -18.5 0.26 (1)</td> <td>10.00</td> <td>N-K</td> <td>0 / 290</td> <td>0.07 (1)</td> </tr> <tr> <td>U-T</td> <td>0 / 1899</td> <td>-18.5 -18.5 0.26 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>T-S</td> <td>0 / 1899</td> <td>-18.5 -18.5 0.26 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>S-R</td> <td>0 / 2427</td> <td>-18.5 -18.5 0.33 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>R-Q</td> <td>0 / 2427</td> <td>-18.5 -18.5 0.33 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>Q-P</td> <td>0 / 1899</td> <td>-18.5 -18.5 0.26 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>P-O</td> <td>0 / 1899</td> <td>-18.5 -18.5 0.25 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>O-N</td> <td>0 / 1961</td> <td>-18.5 -18.5 0.28 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>N-M</td> <td>0 / 0</td> <td>-18.5 -18.5 0.05 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>					FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	JT VERT	2385	0	5-8	5-8	W	2385	0	5-8	5-8	M	2385	0	5-8	5-8	1ST CASE	MAX. MIN. COMPONENT REACTIONS	JT COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL	W	1684 1122.0 0.0 0.0 0.0 562.0 0.0	M	1684 1122.0 0.0 0.0 0.0 562.0 0.0	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	FR-TO		FROM TO		FR-TO			A-B	0 / 47	-91.8 -91.8 0.19 (1)	10.00	V-C	-453.0	0.20 (1)	B-C	-2420 / 0	-91.8 -91.8 0.55 (1)	3.91	C-U	-99.0	0.09 (1)	C-D	-2408 / 0	-91.8 -91.8 0.53 (1)	3.92	U-D	0 / 176	0.04 (4)	D-E	-2427 / 0	-91.8 -91.8 0.49 (1)	3.92	D-S	0 / 1034	0.23 (1)	E-F	-2572 / 0	-91.8 -91.8 0.50 (1)	3.81	S-E	-784 / 0	0.41 (1)	F-G	-2572 / 0	-91.8 -91.8 0.50 (1)	3.81	E-R	0 / 290	0.07 (1)	G-H	-2572 / 0	-91.8 -91.8 0.50 (1)	3.81	R-F	-431 / 0	0.23 (1)	H-I	-2427 / 0	-91.8 -91.8 0.49 (1)	3.92	F-H	0 / 290	0.07 (1)	I-J	-2408 / 0	-91.8 -91.8 0.53 (1)	3.92	Q-H	-784 / 0	0.41 (1)	J-K	-2420 / 0	-91.8 -91.8 0.55 (1)	3.91	H-I	0 / 1034	0.23 (1)	K-L	0 / 47	-91.8 -91.8 0.19 (1)	10.00	I-L	0 / 176	0.04 (4)	W-B	-2335 / 0	0.0 0.0 0.18 (1)	6.72	O-J	-93 / 0	0.09 (1)	M-K	-2335 / 0	0.0 0.0 0.18 (1)	6.72	N-J	-453.0	0.20 (1)	W-V	0 / 0	-18.5 -18.5 0.05 (4)	10.00	B-V	0 / 290	0.07 (1)	V-U	0 / 1961	-18.5 -18.5 0.26 (1)	10.00	N-K	0 / 290	0.07 (1)	U-T	0 / 1899	-18.5 -18.5 0.26 (1)	10.00				T-S	0 / 1899	-18.5 -18.5 0.26 (1)	10.00				S-R	0 / 2427	-18.5 -18.5 0.33 (1)	10.00				R-Q	0 / 2427	-18.5 -18.5 0.33 (1)	10.00				Q-P	0 / 1899	-18.5 -18.5 0.26 (1)	10.00				P-O	0 / 1899	-18.5 -18.5 0.25 (1)	10.00				O-N	0 / 1961	-18.5 -18.5 0.28 (1)	10.00				N-M	0 / 0	-18.5 -18.5 0.05 (4)	10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 8.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. O.C. LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, OBC 2012 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 (55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL. (LL) = L/360 (1.35") CALCULATED VERT. DEFL. (LL) = U/899 (0.13") ALLOWABLE DEFL. (TL) = L/360 (1.35") CALCULATED VERT. DEFL. (TL) = U/959 (0.24") CSI: TO = 0.55/1.00 (B-C:1), BC = 0.33/1.00 (R-S:1), WB = 0.46/1.00 (B-V:1), SSI = 0.23/1.00 (D-E:1) DOL LUMBER: 1.00 NAIL = 1.00 LS BEND = 1.10 COMP = 1.10 SHEAR = 1.10 TENS = 1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 351 1667 788 1987 1656 PLATE PLACEMENT TOL. = 3.250 inches PLATE ROTATION TOL. = 5.0 Deg. SI GRIP = 0.85 (N) (INPUT = 0.90) SI METAL = 0.49 (N) (INPUT = 1.00)			
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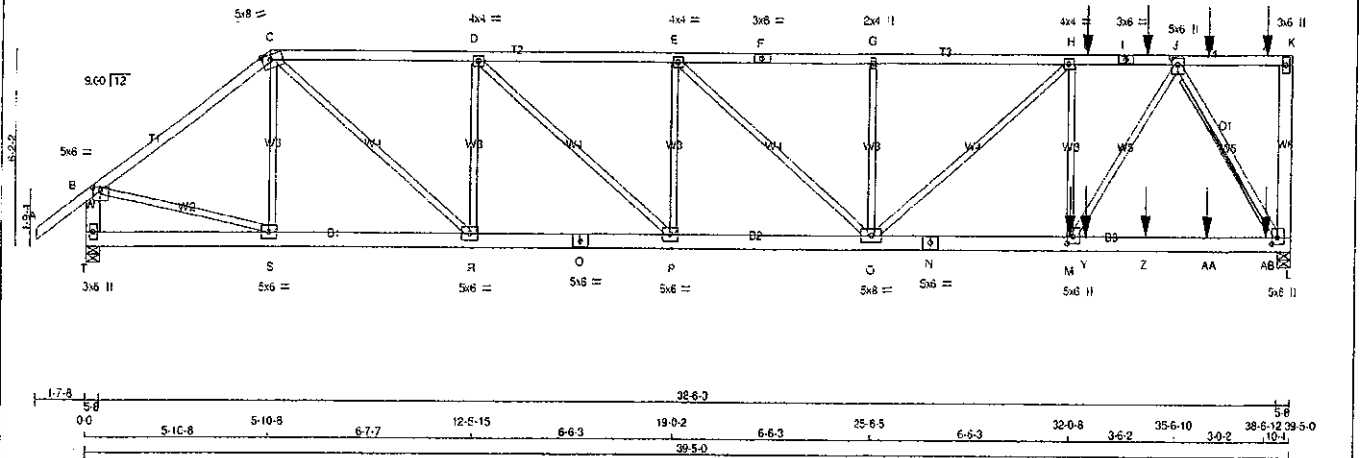
Structural component only
DWG# T-1924946

[illegible]



JOB NAME: 405684 TRUSS NAME: T5 QUANTITY: 1 PLY: 2 JOB DESC: GREENYORK HOMES TRUSS DESC: DRWG NO. 1

Version 8.300 S May 10 2019 Mitek Industries, Inc. Thu Sep 26 15:23:13 2019 Page 1
ID: yvsdRC5EqJey4zVn85GO_yc6AJbL99co4UTD70u6VxyrFj3Rs78IQqF_u_p7pkyZkLS
1:7-8 9-0 5-10-8 6-7-7 12-5-15 6-6-3 19-0-2 25-6-5 6-6-3 32-0-8 3-6-2 35-6-10 3-0-2 38-6-12 39-5-0
Scale = 1/8" = 1'-0"



LUMBER	N L G A RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - K	2x4	DRY	No.2	SPF	
L - K	2x6	DRY	No.2	SPF	
T - B	2x8	DRY	No.2	SPF	
T - Q	2x6	DRY	No.2	SPF	
O - N	2x6	DRY	No.2	SPF	
N - L	2x6	DRY	No.2	SPF	

ALL WEBS EXCEPT DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	TOP
F - I	12	SIDE (0.0)
I - K	12	SIDE (0.0)
K - L	12	TOP
L - B	12	TOP
BOTTOM CHORDS: (0.122'X3") SPIRAL NAILS		
T - Q	12	TOP
Q - N	12	TOP
N - L	12	SIDE (183.1)
WEBS: (0.122'X3") SPIRAL NAILS		
H - M	6	SIDE (405.9)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-p	MT20	5.0	6.0	1.25	3.00
C TTWW-m	MT20	5.0	8.0	1.75	3.25
D E, H					
D TMWW-i	MT20	4.0	4.0		
F TS-i	MT20	3.0	6.0		
G TMWW-w	MT20	2.0	4.0		
I TS-i	MT20	3.0	6.0		
J TMWW-i	MT20	5.0	6.0	2.50	2.50
K TMV-p	MT20	3.0	6.0		
L BMWW-i-p	MT20	5.0	6.0	2.50	2.25
M BMWW-i	MT20	5.0	6.0	2.75	2.25
N BS-i	MT20	5.0	6.0		
O BMWWW-i	MT20	5.0	8.0		
P, R, S					
P BMWW-i	MT20	5.0	6.0		
O BS-i	MT20	5.0	6.0		
T SMV-i-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	4787	0	0	0
L	4787	0	0	0
T	2836	0	0	0

UNFACTORED REACTIONS

1ST LOOSE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	3380	2248	0	0	0
L	3380	2248	0	0	0
T	2001	1337	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.73 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

2x6 DRY SPF No.2 T-BRACE AT J-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED TORSION (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED TORSION (LBS)
FR-TO					FR-TO		
A-B	0 / 47	-91.8	-91.8	0.10 (1)	10.00	S-C	-412 / 0
B-C	-3021 / 0	-91.8	-91.8	0.41 (1)	4.90	C-R	0 / 2592
C-D	-4358 / 0	-91.8	-91.8	0.50 (1)	4.16	R-D	-1595 / 0
D-E	-5395 / 0	-91.8	-91.8	0.58 (1)	3.74	D-P	0 / 1389
E-F	-5846 / 0	-91.8	-91.8	0.54 (1)	3.73	P-E	-794 / 0
F-G	-5846 / 0	-91.8	-91.8	0.54 (1)	3.73	E-O	0 / 335
G-H	-5846 / 0	-91.8	-91.8	0.54 (1)	3.73	O-G	-623 / 0
H-U	-5091 / 0	-91.8	-91.8	0.32 (1)	4.00	G-H	0 / 743
U-I	-5091 / 0	-91.8	-91.8	0.32 (1)	4.00	H-I	-1124 / 0
I-V	-5091 / 0	-91.8	-91.8	0.32 (1)	4.00	B-S	0 / 2475
V-J	-5091 / 0	-91.8	-91.8	0.32 (1)	4.00	M-J	0 / 2475
J-W	0 / 0	-91.8	-91.8	0.20 (1)	10.00	J-L	0 / 2475
W-X	0 / 0	-91.8	-91.8	0.20 (1)	10.00		
X-K	0 / 0	-91.8	-91.8	0.20 (1)	10.00		
L-K	-297 / 0	0.0	0.0	0.07 (1)	7.81		
T-B	-2783 / 0	0.0	0.0	0.10 (1)	7.81		
T-S	0 / 0	-18.5	-18.5	0.05 (4)	10.00		
S-R	0 / 2406	-18.5	-18.5	0.18 (1)	10.00		
R-Q	0 / 4358	-18.5	-18.5	0.31 (1)	10.00		
Q-P	0 / 4358	-18.5	-18.5	0.31 (1)	10.00		
P-O	0 / 5395	-18.5	-18.5	0.39 (1)	10.00		
O-N	0 / 5091	-18.5	-18.5	0.36 (1)	10.00		
N-M	0 / 5091	-18.5	-18.5	0.36 (1)	10.00		
M-Y	0 / 2905	-18.5	-18.5	0.27 (1)	10.00		
Y-Z	0 / 2905	-18.5	-18.5	0.27 (1)	10.00		
Z-AA	0 / 2905	-18.5	-18.5	0.27 (1)	10.00		
AA-AB	0 / 2905	-18.5	-18.5	0.27 (1)	10.00		
AB-L	0 / 2905	-18.5	-18.5	0.27 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
CL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPC 2011, TPC 2014

155% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD
EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.31")
CALCULATED VERT. DEFL. (LL) = L/999 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (1.31")
CALCULATED VERT. DEFL. (TL) = L/999 (0.30")

CSI: TC=0.58/1.00 (D-E-I), BC=0.39/1.00 (O-P-I),
WB=0.80/1.00 (J-L-I), SS=0.18/1.00 (J-K-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

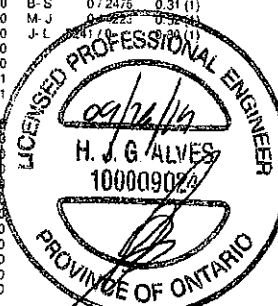
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1967 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (C) (INPUT = 0.90)

JSI METAL = 0.90 (J) (INPUT = 1.00)



Structural component only 1/2
DWG# T-1924949 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREENYORK HOMES	DRWG NO.
405684	T5	1	2	TRUSS DESC.		

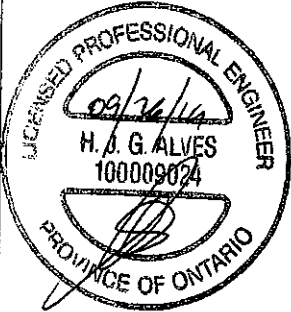
Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 MTek Industries, Inc. Thu Sep 26 15:23:13 2019 Page 2
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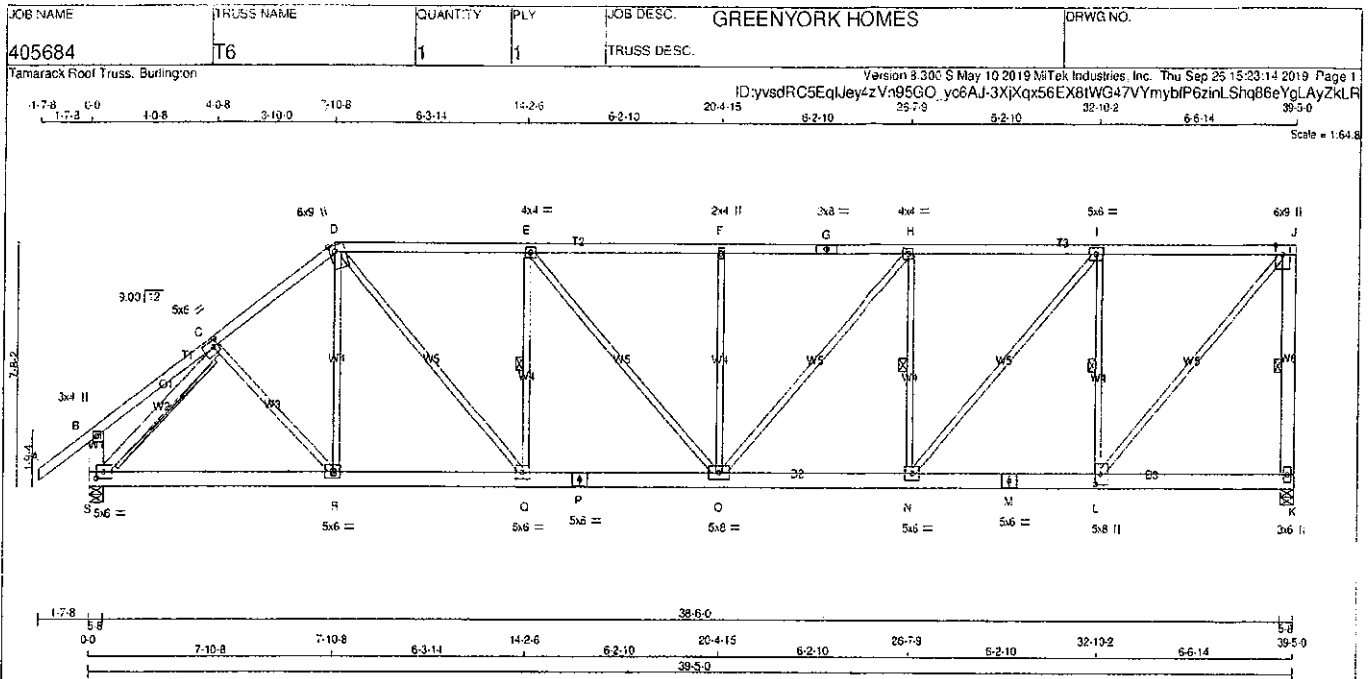
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
M	32-0-8	-2257	-2257	---	FRONT	VERT	TOTAL	---	C1
U	32-6-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
V	34-6-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
W	36-6-12	-178	-178	---	FRONT	VERT	TOTAL	---	C1
X	38-6-12	-184	-184	---	FRONT	VERT	TOTAL	---	C1
Y	32-6-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
Z	34-6-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
AA	36-6-12	-36	-36	---	FRONT	VERT	TOTAL	---	C1
AB	28-6-12	-37	-37	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
 DWG# T-1924949 72



TOTAL WEIGHT = 211 lb

LUMBER

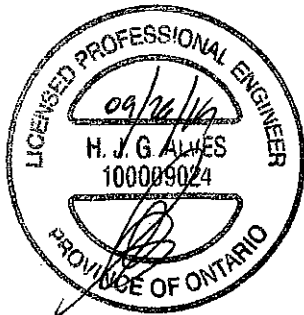
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	5.0	6.0	2.50	2.00
C	TMWW-m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-m	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	5.0	6.0		
J	TMWW+p	MT20	6.0	9.0	Edge	
K	BMV+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	4.00	2.00
M	BS-t	MT20	5.0	6.0		
N	Q, R	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	8.0		
P	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
K	2173	0	2173	0
S	2330	0	2330	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1537	1008 / 0	0 / 0	0 / 0	0 / 0	528	0
S	1645	1098 / 0	0 / 0	0 / 0	0 / 0	549	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, L-L, E-Q, H-N.

2x3 DRY SPF No.2 T-BRACE AT C-S

FASTEN T AND L-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 50% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
A-B	0 / 47	-91.8	-91.8 0.19 (1)	C-R	0 / 140	0.03 (4)	
B-C	0 / 24	-91.8	-91.8 0.22 (1)	D-O	0 / 102	0.04 (4)	
C-D	-2396 / 0	-91.8	-91.8 0.32 (1)	S-C	-2688 / 0	0.89 (1)	
D-E	-2758 / 0	-91.8	-91.8 0.80 (1)	L-J	0 / 2529	0.57 (1)	
E-F	-2966 / 0	-91.8	-91.8 0.83 (1)	D-Q	0 / 1318	0.30 (1)	
F-G	-2966 / 0	-91.8	-91.8 0.85 (1)	L-I	-1746 / 0	0.62 (1)	
G-H	-2966 / 0	-91.8	-91.8 0.85 (1)	Q-E	-887 / 0	0.31 (1)	
H-I	-2610 / 0	-91.8	-91.8 0.81 (1)	R-I	0 / 1417	0.32 (1)	
I-J	-1692 / 0	-91.8	-91.8 0.69 (1)	E-O	0 / 321	0.07 (1)	
K-J	-2122 / 0	0.0	0.0 0.34 (1)	N-H	-967 / 0	0.34 (1)	
S-B	-296 / 0	0.0	0.0 0.02 (1)	O-F	-554 / 0	0.56 (1)	
				O-H	0 / 549	0.12 (1)	
S-R	0 / 1797	-18.5	-18.5 0.29 (1)				
R-Q	0 / 1894	-18.5	-18.5 0.29 (1)				
Q-P	0 / 2758	-18.5	-18.5 0.37 (1)				
P-O	0 / 2758	-18.5	-18.5 0.37 (1)				
O-N	0 / 2610	-18.5	-18.5 0.36 (1)				
N-M	0 / 1692	-18.5	-18.5 0.24 (1)				
M-L	0 / 1692	-18.5	-18.5 0.24 (1)				
L-K	0 / 0	-18.5	-18.5 0.09 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. G.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.83/1.00 (E-F:1), BC=0.37/1.00 (O-Q:1),
WB=0.89/0.90 (C-S:1), SSI=0.28/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

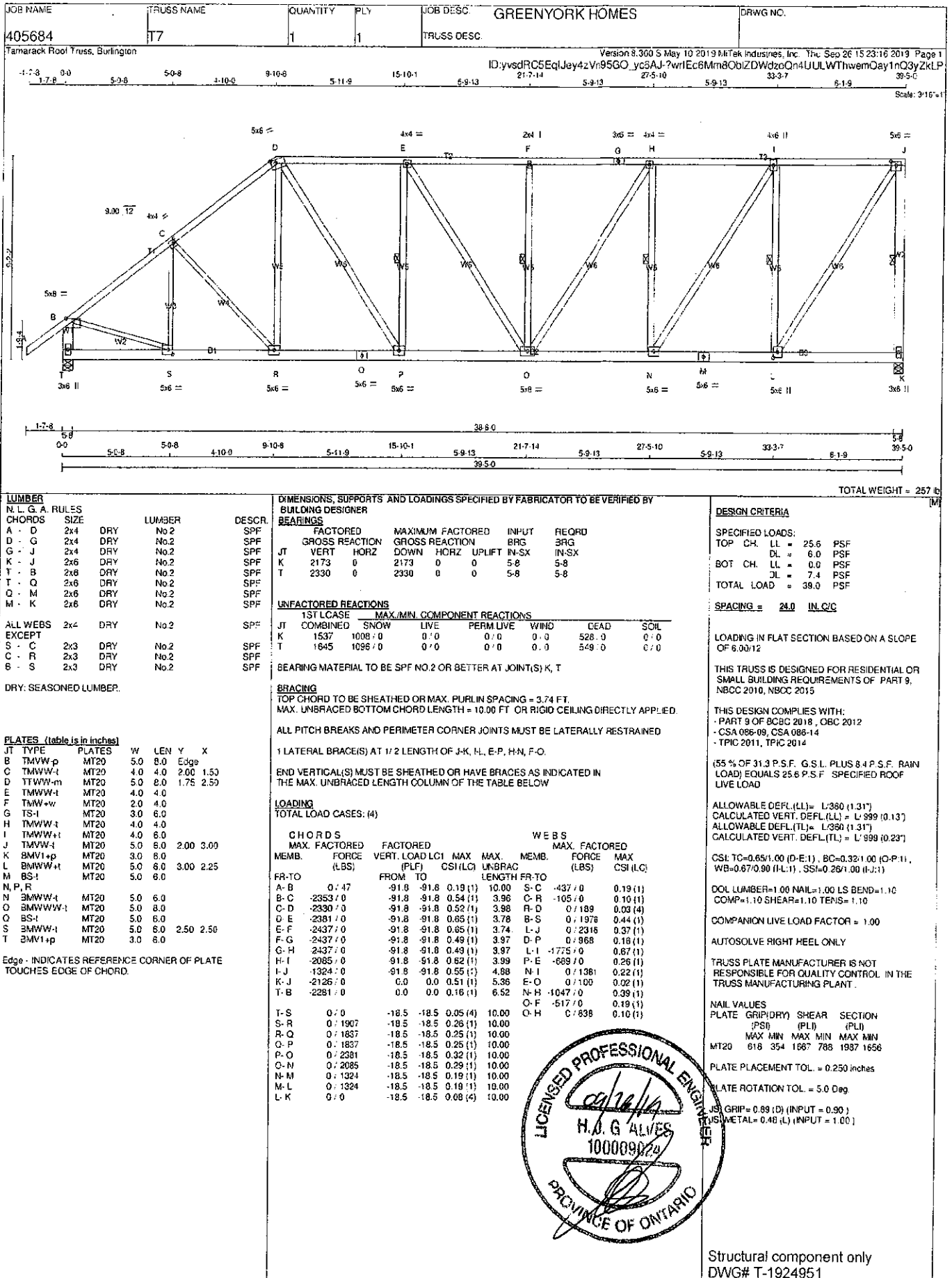
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

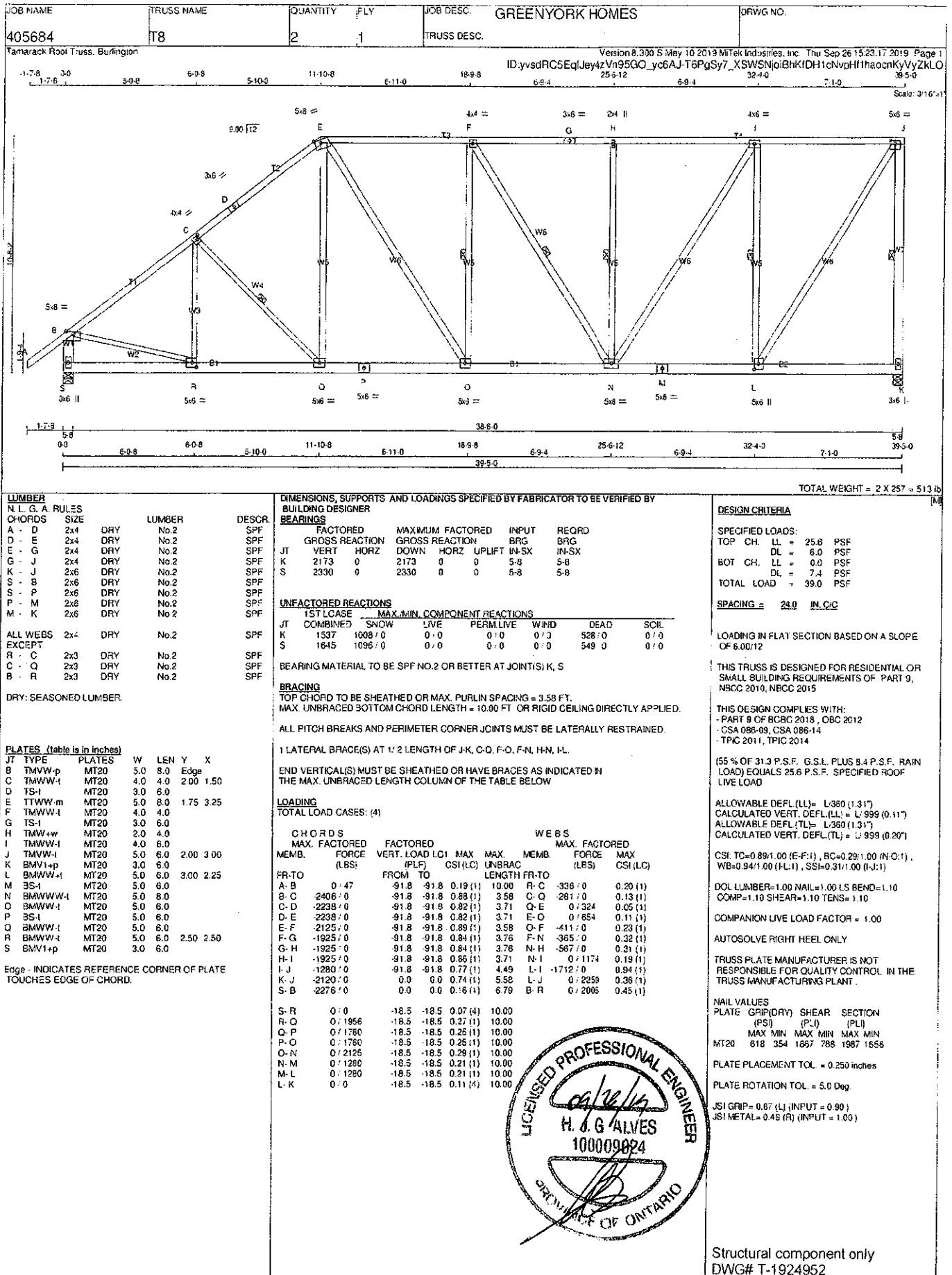
PLATE PLACEMENT TOL. = 0.250 inches

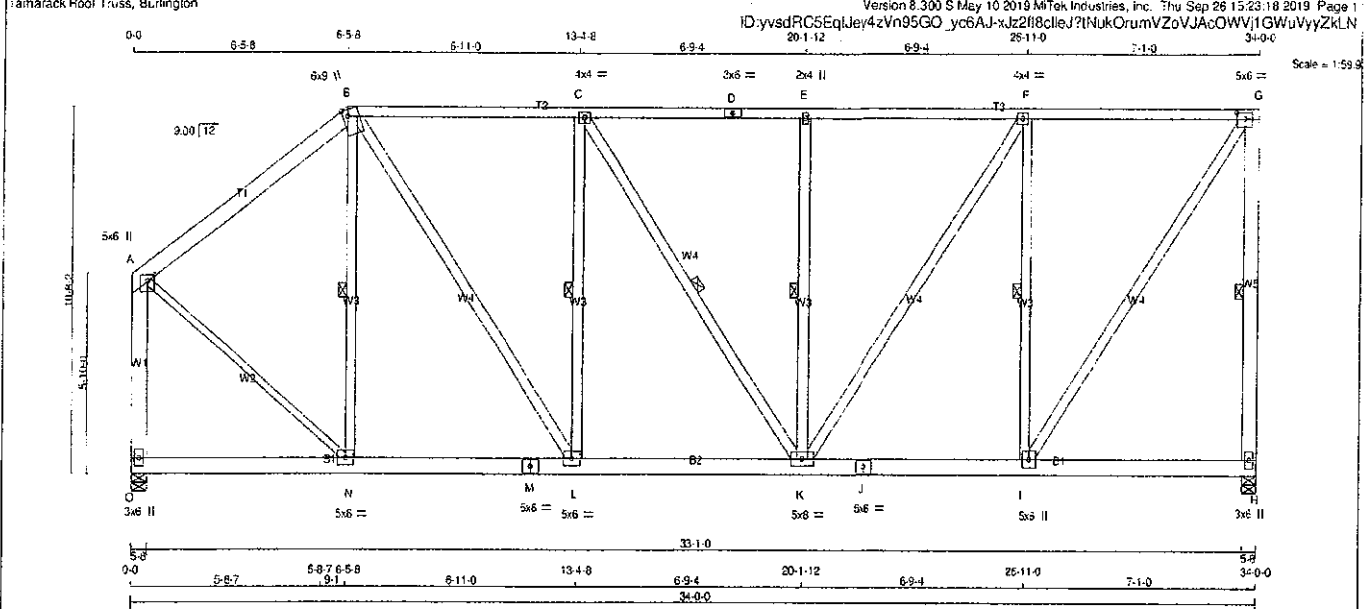
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (S) (INPUT = 0.90)
JSI METAL = 0.66 (C) (INPUT = 1.00)

Structural component only
DWG# T-1924950







LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
A - N	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	5.0	6.0	2.25
B	TTWW+m	MT20	6.0	9.0	Edge 2.00
C	TMWV-i	MT20	4.0	4.0	
D	TS-i	MT20	3.0	6.0	
E	TMWV+w	MT20	2.0	4.0	
F	TMWV-i	MT20	4.0	4.0	
G	TMWV-i	MT20	5.0	6.0	2.25
H	BMV1+p	MT20	3.0	6.0	
I	BMWV+i	MT20	5.0	6.0	
J	BS-i	MT20	5.0	6.0	
K	BMWVW-i	MT20	5.0	8.0	
L	BMWVW-i	MT20	5.0	6.0	
M	BS-i	MT20	5.0	6.0	
N	BMWVW-i	MT20	5.0	6.0	
O	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	IN-SX	IN-SX
H 1874	0	5-8	5-8
O 1874	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOSE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM.LIVE	
H 1325	870.0	0.0	0.0	456.0
O 1325	870.0	0.0	0.0	456.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, H-N, C-L, C-K, E-K, F-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	-1228.0	-91.8	-91.8 0.34 (1)	N-B	-645.0	0.36 (1)	
B-C	-1526.0	-91.8	-91.8 0.78 (1)	B-L	0.997	0.16 (1)	
C-D	-1524.0	-91.8	-91.8 0.78 (1)	L-C	-702.0	0.39 (1)	
D-E	-1524.0	-91.8	-91.8 0.78 (1)	C-K	-473	0.00 (1)	
E-F	-1524.0	-91.8	-91.8 0.80 (1)	K-E	-567.0	0.31 (1)	
F-G	-1075.0	-91.8	-91.8 0.75 (1)	K-F	0.818	0.13 (1)	
H-G	-1821.0	0.0	0.0 0.84 (1)	F-F	-1414.0	0.78 (1)	
O-A	-1827.0	0.0	0.0 0.70 (1)	F-G	0.1897	0.30 (1)	
O-N	0.0	-18.5	-18.5 0.10 (4)	A-N	0.1254	0.28 (1)	
N-M	0.968	-18.5	-18.5 0.17 (1)				
M-L	0.968	-18.5	-18.5 0.17 (1)				
L-K	0.1526	-18.5	-18.5 0.22 (1)				
K-J	0.1075	-18.5	-18.5 0.18 (1)				
J-I	0.1075	-18.5	-18.5 0.18 (1)				
I-H	0.0	-18.5	-18.5 0.11 (4)				

TOTAL WEIGHT = 240 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	25.6	PSF
ROT CH.	LL	PSF
LL	0.0	PSF
LL	7.4	PSF
TOTAL LOAD	39.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF BCBC 2018, OBC 2012
CSA 086-09, CSA 086-14
TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.80/1.00 (E-F:1), BC=0.22/1.00 (K-L:1), WB=0.78/1.00 (F-L:1), SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

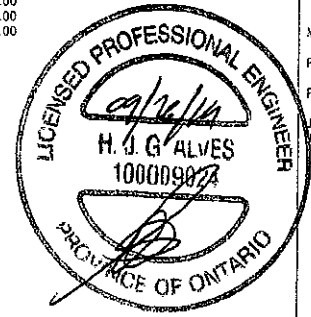
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

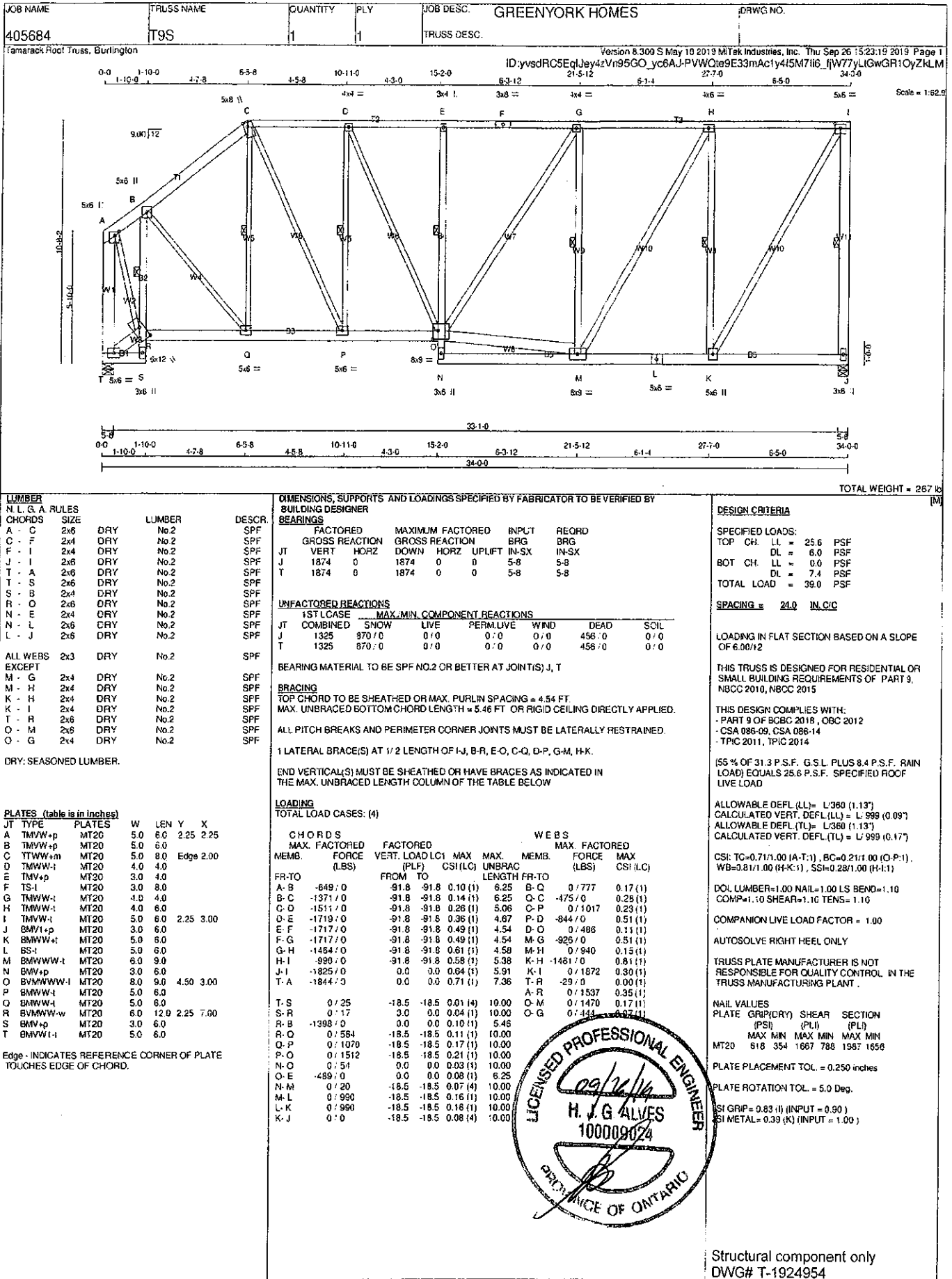
PLATE PLACEMENT TOL = 0.250 inches

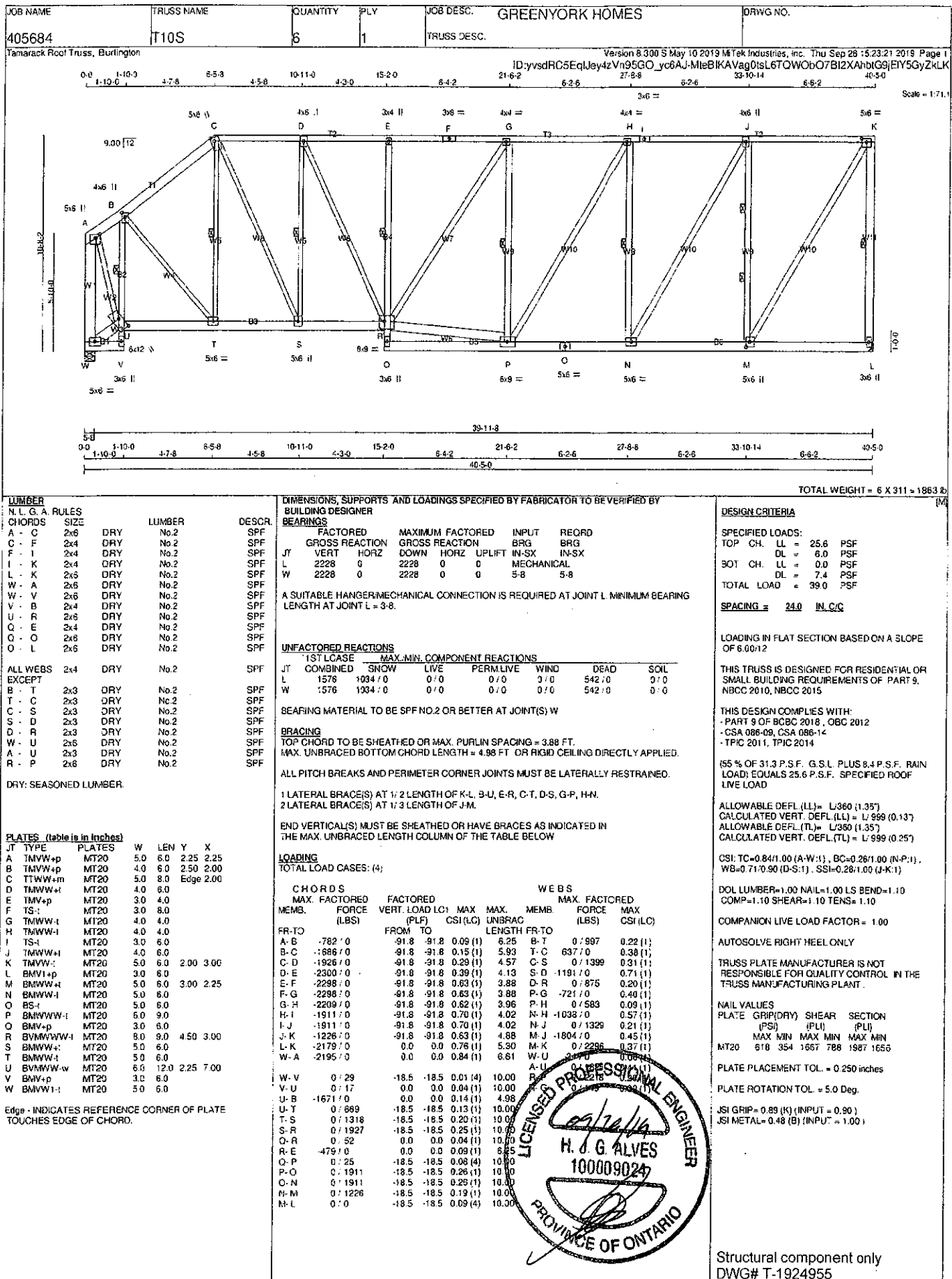
PLATE ROTATION TOL = 5.0 Deg.

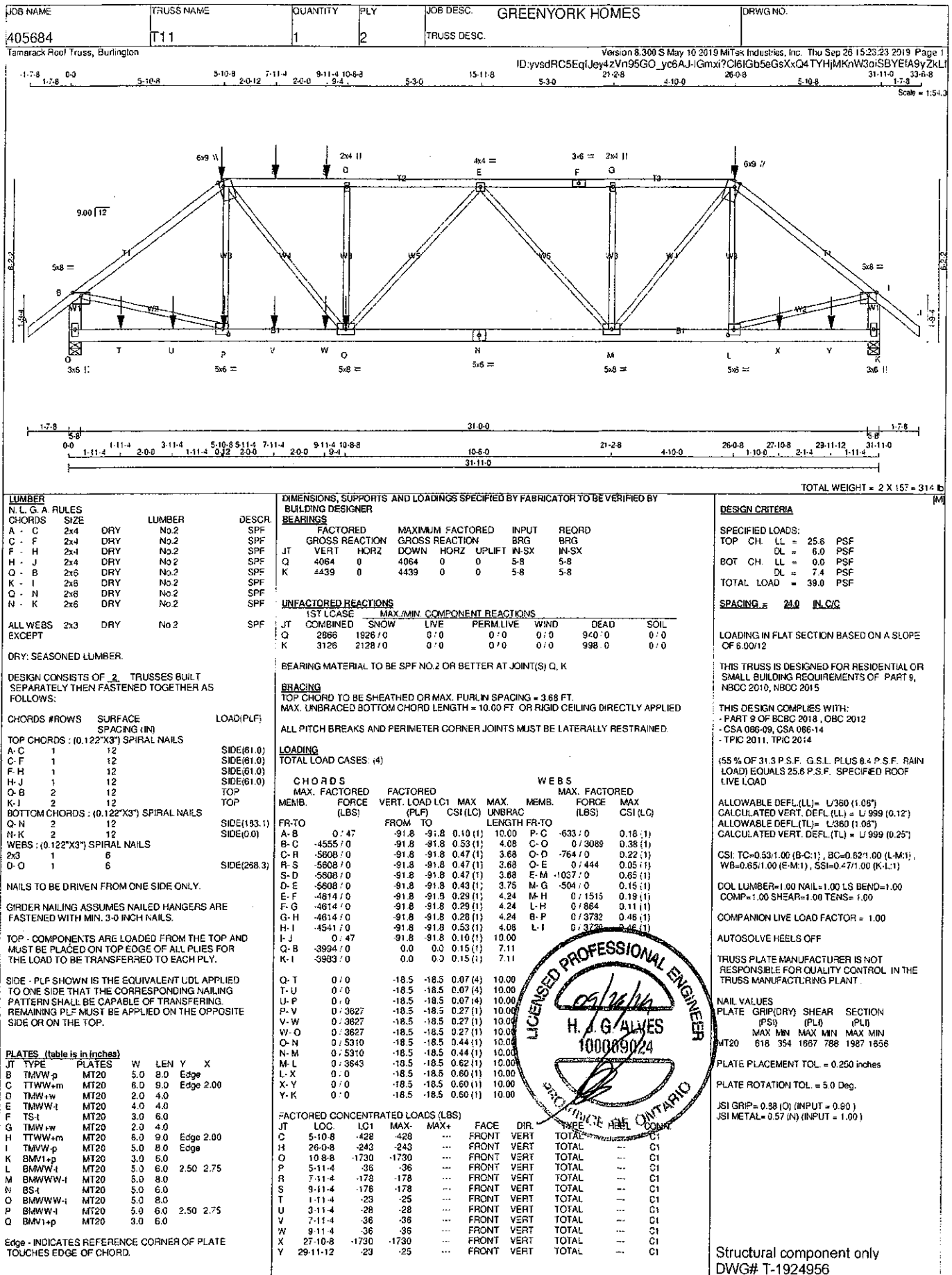
ISI GRIP= 0.85 (F) (INPUT = 0.90)
ISI METAL= 0.50 (A) (INPUT = 1.00)



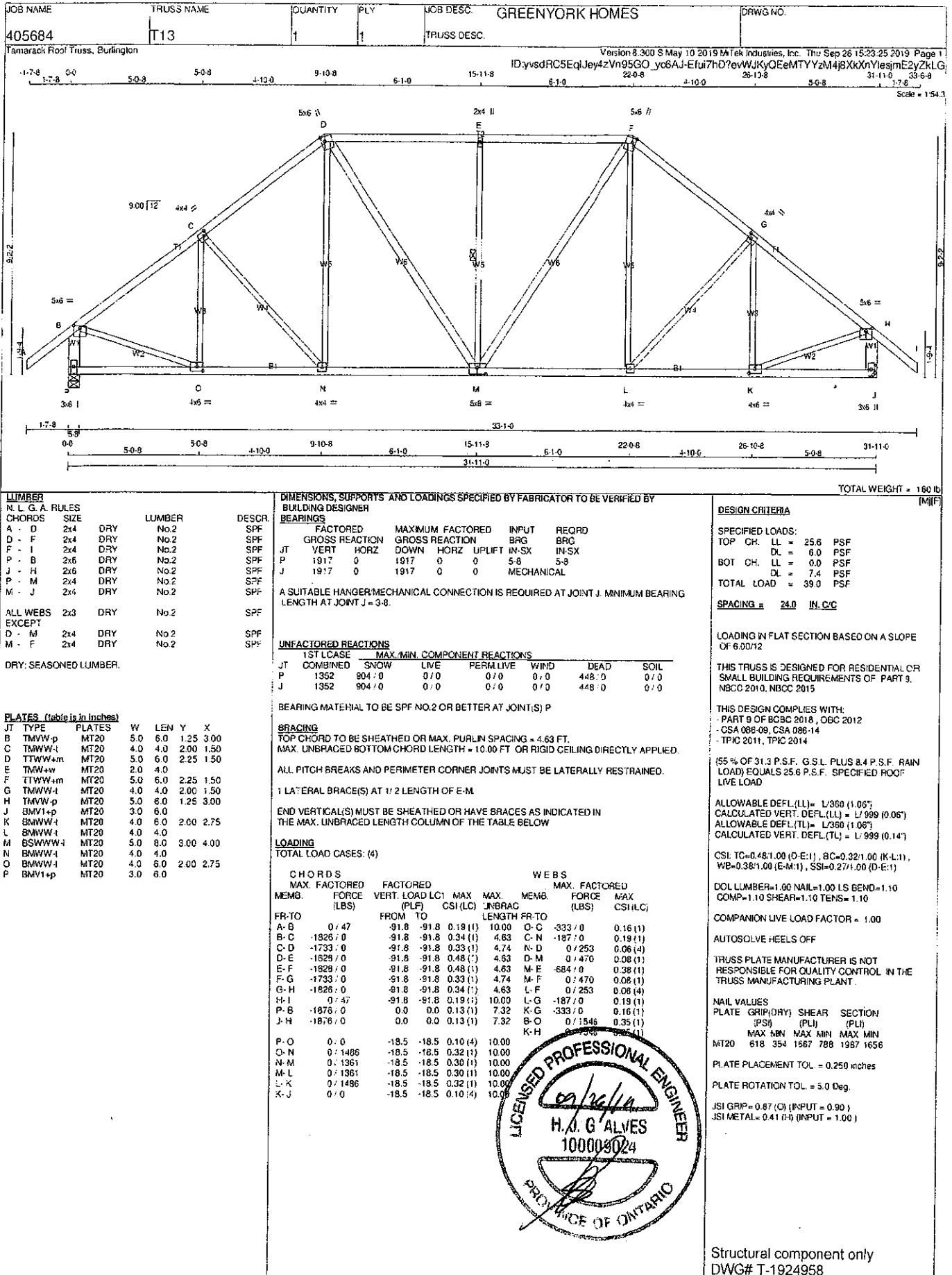
Structural component only
DWG# T-1924953

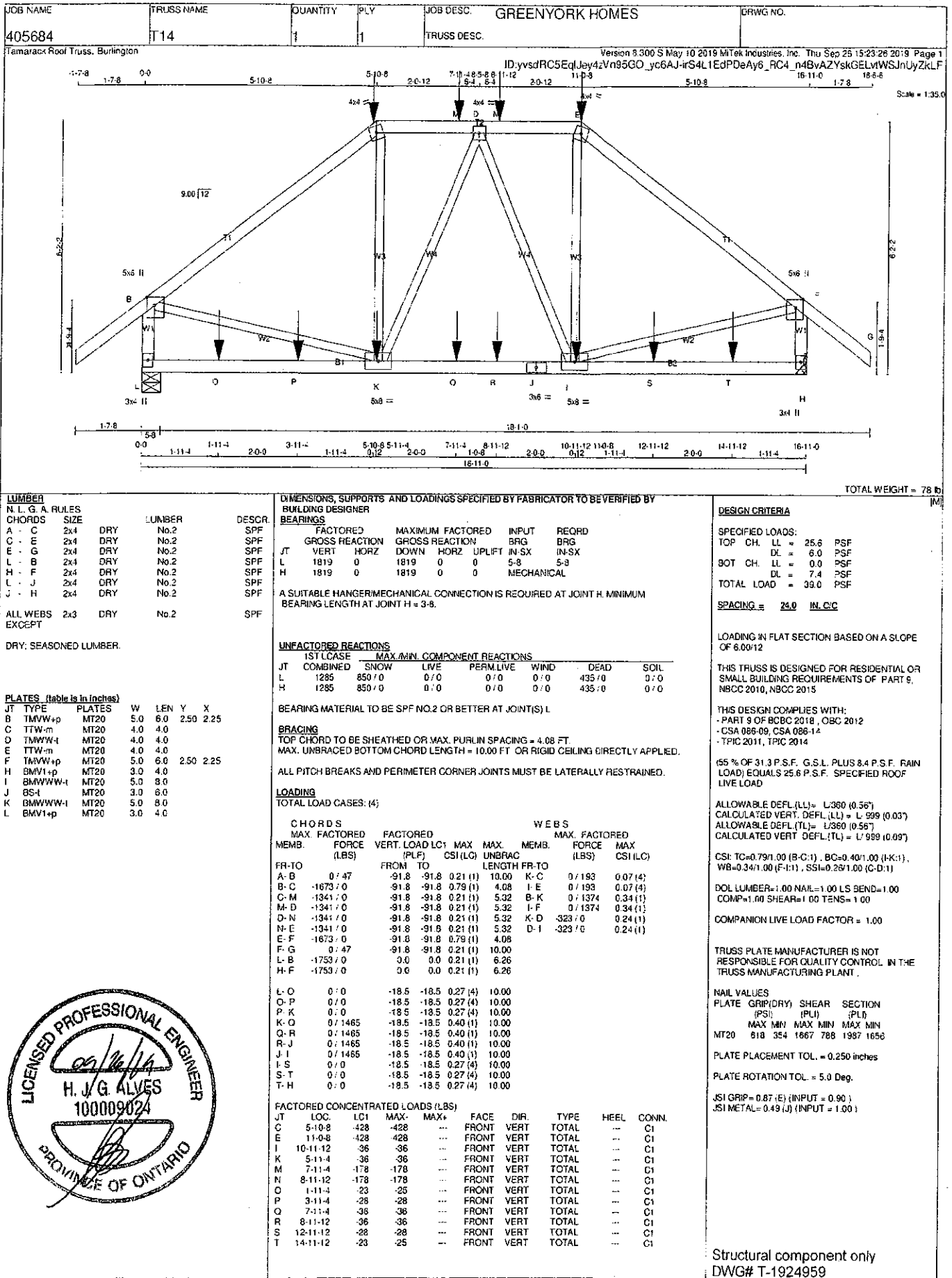


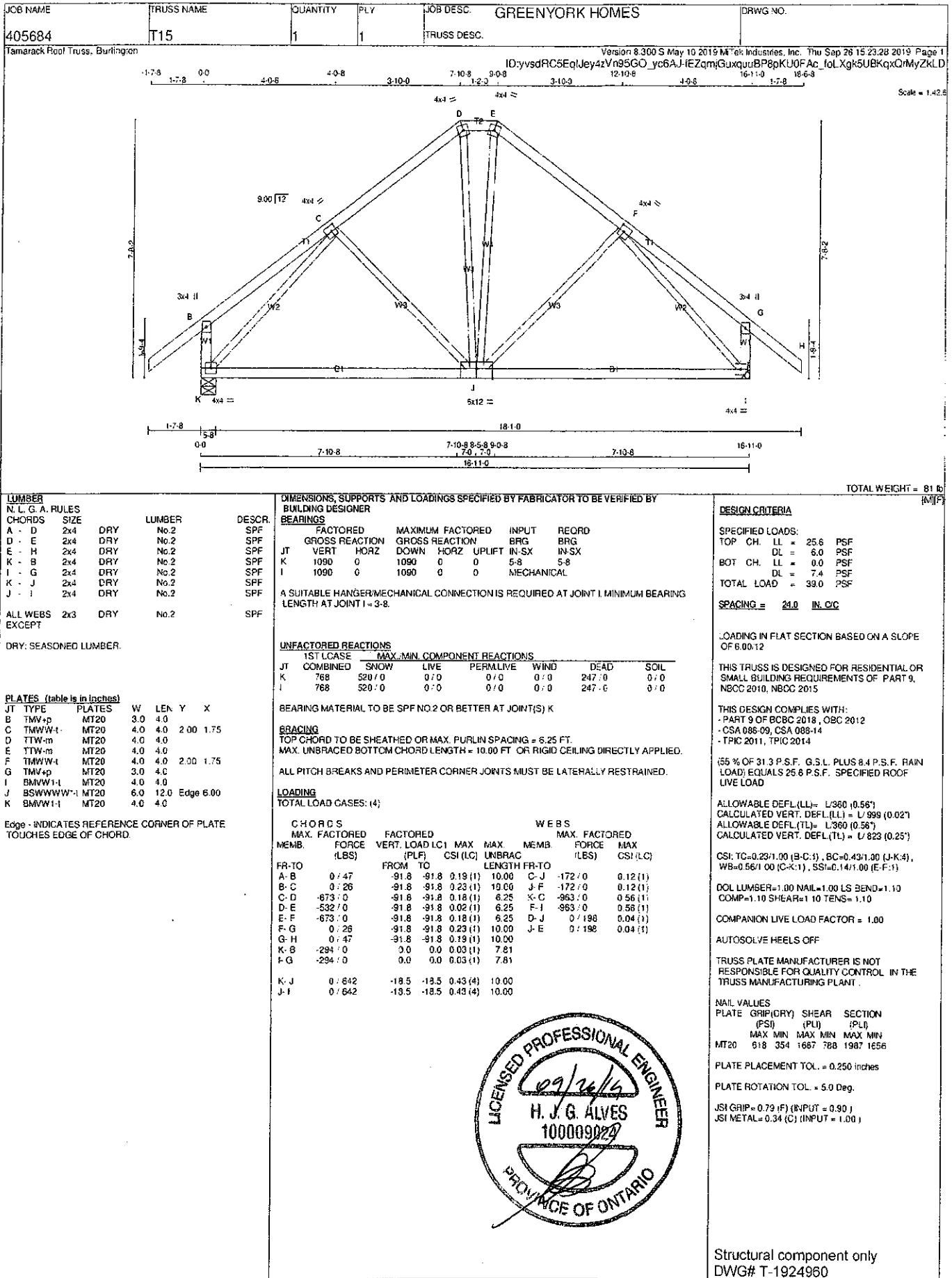




[illegible]

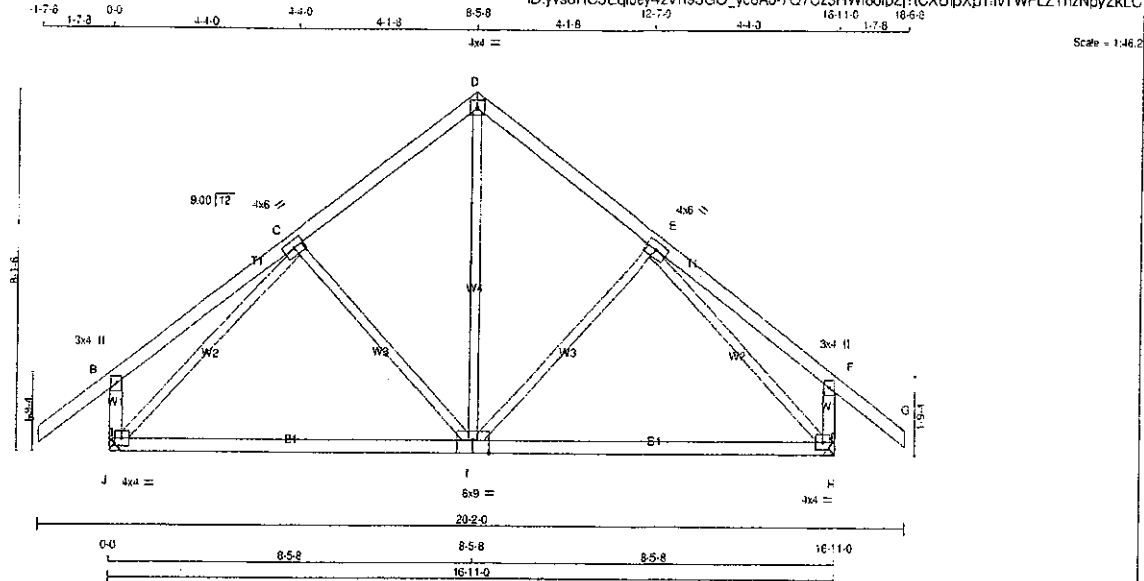






JOB NAME 405684	TRUSS NAME T16	QUANTITY 8	FLY 1	JOB DESC GREENYORK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 8 X 77 = 615 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-I	MT20	4.0	6.0	
D	TTW-p	MT20	4.0	4.0	2.25 2.00
E	TMVW-I	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW-I	MT20	4.0	4.0	
I	BSVW-I	MT20	6.0	9.0	Edge 4.50
J	BMVW-I	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1090	0	1090	0	0
H	1090	0	1090	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. H. MINIMUM BEARING LENGTH AT JOINT J = 3'-8", JOINT H = 3'-8".

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	768	520 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0
H	768	520 / 0	0 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 47	-91.8 -91.8	0.19 (1)	I-D	0 / 461	0.10 (1)	
B-C	0 / 28	-91.8 -91.8	0.27 (1)	I-E	-207 / 0	0.14 (1)	
C-D	-660 / 0	-91.8 -91.8	0.21 (1)	C-I	-207 / 0	0.14 (1)	
D-E	-660 / 0	-91.8 -91.8	0.21 (1)	J-C	-958 / 0	0.84 (1)	
E-F	0 / 28	-91.8 -91.8	0.27 (1)	E-H	-958 / 0	0.84 (1)	
F-G	0 / 47	-91.8 -91.8	0.19 (1)				
J-B	-304 / 0	0.0 0.0	0.03 (1)				
H-F	-304 / 0	0.0 0.0	0.03 (1)				
J-I	0 / 647	-18.5 -18.5	0.43 (4)				
I-H	0 / 647	-18.5 -18.5	0.43 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF BCBC 2018, CBC 2012
CSA 086-09, CSA 086-14
TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.56")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = 1/360 (0.56")
CALCULATED VERT. DEFL.(TL) = 1/816 (0.25")

CSI: TC=0.27/1.00 (B-C-1), BC=0.43/1.00 (I-J-4), WB=0.64/1.00 (E-H-1), SSI=0.15/1.00 (C-D-1)

OCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1987	1656

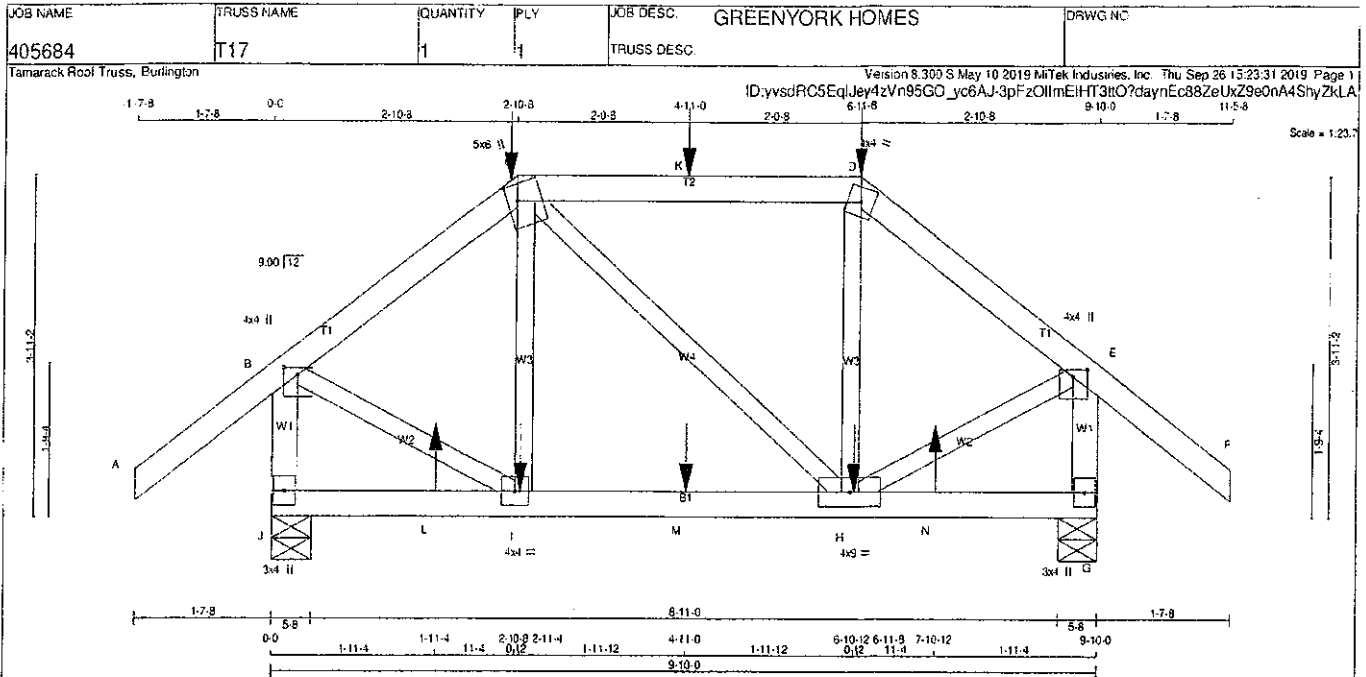
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.83 (D) (INPUT = 0.90)
JSI METAL = 0.24 (J) (INPUT = 1.00)



Structural component only
DWG# T-1924961



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTW-m	MT20	4.0	4.0	
E	TMVW+p	MT20	4.0	4.0	1.00 2.00
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	4.0	9.0	
I	BMVWW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX	IN-SX
J	814	0	814	0	5-8
G	814	0	814	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
J	574	369.0	0.0
G	574	369.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
MEMB.	FR-TO						
A-B	0 / 47	-91.8	-91.8	0.21 (1)	10.00	I-C	-118 / 26
B-C	-512 / 0	-91.8	-91.8	0.15 (1)	6.25	C-H	0 / 0
C-K	-404 / 0	-91.8	-91.8	0.35 (1)	6.25	H-D	-119 / 26
K-D	-404 / 0	-91.8	-91.8	0.35 (1)	6.25	D-E	0 / 453
D-E	-511 / 0	-91.8	-91.8	0.15 (1)	6.25	E-F	0 / 453
E-F	0 / 47	-91.8	-91.8	0.21 (1)	10.00	F-G	0 / 453
F-G	-795 / 0	0.0	0.0	0.09 (1)	7.81	G-H	0 / 453
G-H	-795 / 0	0.0	0.0	0.09 (1)	7.81	H-I	0 / 453
H-I	0 / 0	-18.5	-18.5	0.06 (4)	10.00	I-J	0 / 0
I-J	0 / 0	-18.5	-18.5	0.06 (4)	10.00	J-K	0 / 0
J-K	0 / 405	-18.5	-18.5	0.11 (1)	10.00	K-L	0 / 405
K-L	0 / 405	-18.5	-18.5	0.11 (1)	10.00	L-M	0 / 0
L-M	0 / 0	-18.5	-18.5	0.06 (4)	10.00	M-N	0 / 0
M-N	0 / 0	-18.5	-18.5	0.06 (4)	10.00	N-O	0 / 0

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT										
C	2-10-8	-88	-88	155	FRONT	VERT	TOTAL	---	C1	
D	6-11-8	-88	-88	155	FRONT	VERT	TOTAL	---	C1	
H	6-10-12	-8	-8	---	FRONT	VERT	TOTAL	---	C1	
I	2-11-4	-8	-8	---	FRONT	VERT	TOTAL	---	C1	
K	4-11-0	-40	-40	---	FRONT	VERT	TOTAL	---	C1	
L	1-11-4	5	1	5	FRONT	VERT	TOTAL	---	C1	
M	4-11-0	-8	-8	---	FRONT	VERT	TOTAL	---	C1	
N	7-10-12	5	1	5	FRONT	VERT	TOTAL	---	C1	

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.33")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.33")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.35/1.00 (C-D:1), BC=0.11/1.00 (H-I:1), WB=0.11/1.00 (B-I:1), SS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 783 1987 1658

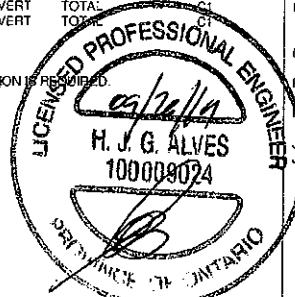
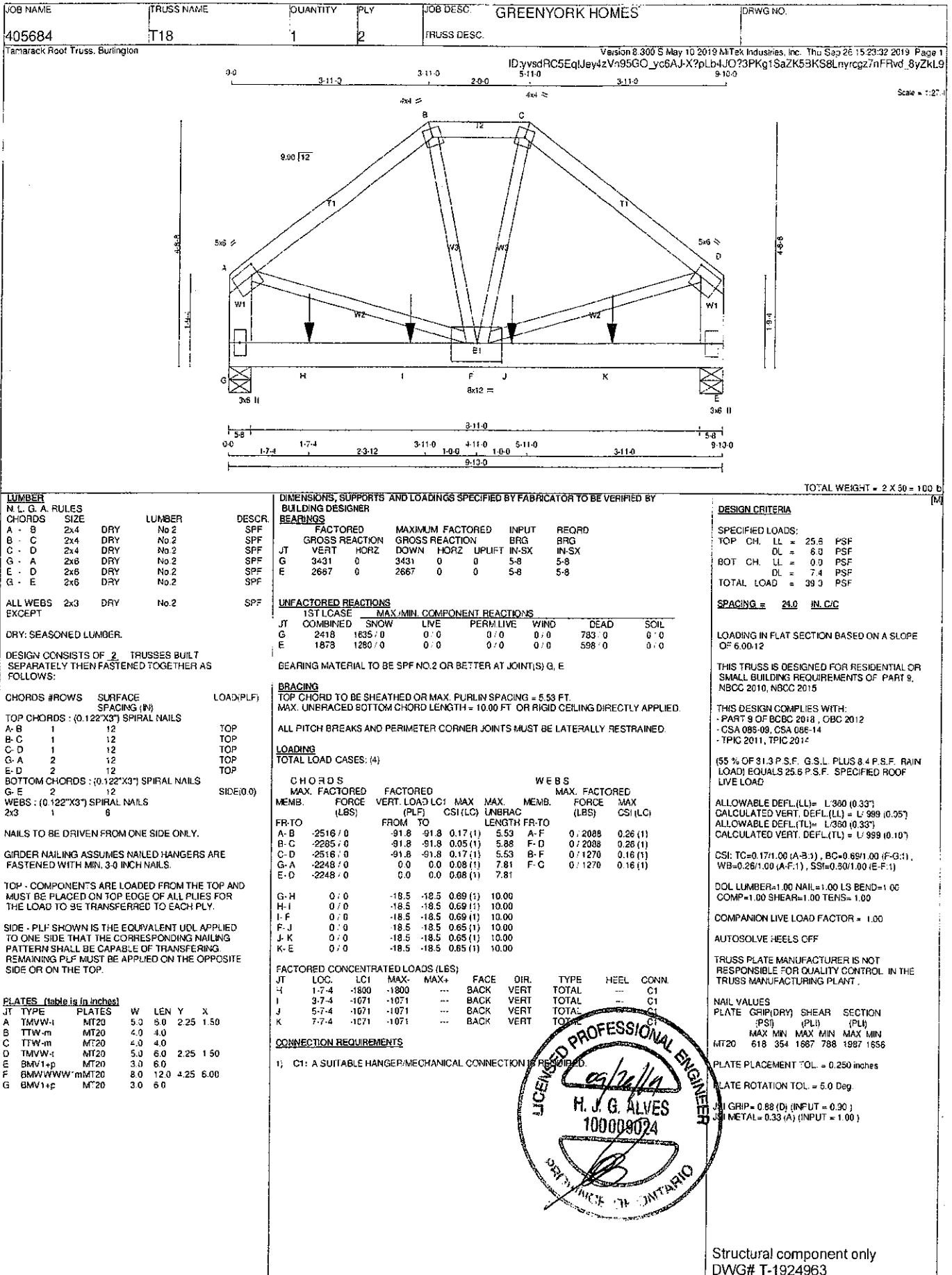
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.69 (B) (INPUT = 0.90)
JSI METAL = 0.18 (B) (INPUT = 1.00)

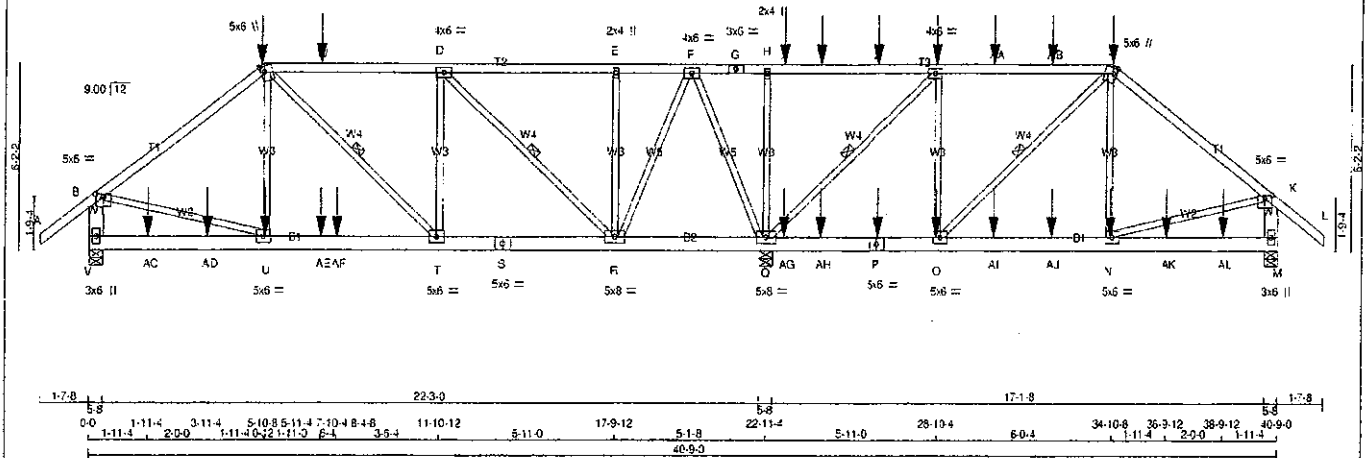
Structural component only
DWG# T-1924862



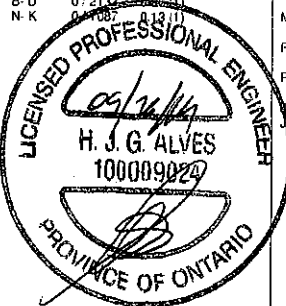


Structural component only
DWG# T-1924963

JOB NAME 405684	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. GREENYORK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 Mitek Industries, Inc. Thu Sep 26 15:23:34 2019 Page 1	
1-7-8 0-0 5-10-8 5-10-8 7-10-4 4-0-8 11-10-12 5-11-0 17-9-12 20-4-8 22-11-4 22-6-12 2-6-12 5-3-8 28-10-4 34-10-8 5-10-8 40-9-0 32-4-8 1-7-8 1-7-8				ID: yvvdRCSEqL4y4zVn95GO_yc6AJ-TOx50mKWgf1wLczg7iPtEapmX68nA4j1Ok30yZkL7	
				Scale = 1/64.0	



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF A - C 2x4 DRY No.2 SPF C - G 2x4 DRY No.2 SPF G - J 2x4 DRY No.2 SPF J - L 2x4 DRY No.2 SPF V - B 2x6 DRY No.2 SPF M - K 2x6 DRY No.2 SPF V - S 2x6 DRY No.2 SPF S - P 2x6 DRY No.2 SPF P - M 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS: (0.122"x3") SPIRAL NAILS A - C 1 12 SIDE(61.0) C - G 1 12 SIDE(61.0) G - J 1 12 SIDE(61.0) J - L 1 12 TOP V - B 2 12 TOP M - K 2 12 TOP BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS V - S 2 12 SIDE(183.1) S - P 2 12 SIDE(60.0) P - M 2 12 SIDE(183.1) WEBS: (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMVW-p MT20 5.0 6.0 1.25 3.00 C TTWW-m MT20 5.0 6.0 2.25 1.50 D, F, I D TMVW-l MT20 4.0 6.0 E TMVW-w MT20 2.0 4.0 G TS-l MT20 3.0 5.0 H TMVW-w MT20 2.0 4.0 J TTWW-m MT20 5.0 5.0 2.25 1.50 K TMVW-p MT20 5.0 6.0 1.25 3.00 M BMV-l-p MT20 3.0 5.0 N, O, T, U N BMVW-l MT20 5.0 6.0 P BS-l MT20 5.0 6.0 Q BMVWW-l MT20 5.0 8.0 R BMVWW-l MT20 5.0 8.0 S BS-l MT20 5.0 6.0 V BMV-l-p MT20 3.0 5.0				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQD BRG BRG JT VERT 2446 0 2446 0 0 5-8 5-8 Q 4531 0 4531 0 0 5-8 5-8 M 1530 0 1530 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST CASE MAX. MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM LIVE WIND DEAD SOIL V 1728 1147 0 0 0 0 0 581 0 0 0 Q 3200 2123 0 0 0 0 0 1077 0 0 0 M 1082 709 0 0 0 0 0 374 0 0 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, Q, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T, D-R, I-Q, J-O. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) CHORDS FACTORED WEBS FACTORED MEMB. MAX. FORCE VERT. LOAD LC1 MAX. LENGTH FR-TO MEMB. MAX. FORCE VERT. LOAD LC1 MAX. LENGTH FR-TO (LBS) (PLF) (LC) (LC) (LBS) (LC) FR-TO A-B 0 47 -91.8 -91.8 0.10 (1) 10.00 U-G 0 537 0.07 (1) B-C -2603 0 -91.8 -91.8 0.39 (1) 5.21 C-T -13 3 0.00 (4) C-W -2091 0 -91.8 -91.8 0.40 (1) 5.60 T-D 0 887 0.09 (1) W-D -2091 0 -91.8 -91.8 0.40 (1) 5.60 D-R -2012 0 0.47 (1) D-E -854 0 -91.8 -91.8 0.34 (1) 6.25 R-E -415 0 0.12 (1) E-F -854 0 -91.8 -91.8 0.11 (1) 6.25 F-F 0 1902 0.24 (1) F-G 0 874 -91.8 -91.8 0.39 (1) 10.00 F-Q -1680 0 0.67 (1) G-H 0 874 -91.8 -91.8 0.39 (1) 10.00 Q-H -865 0 0.25 (1) H-X 0 874 -91.8 -91.8 0.58 (1) 10.00 Q-I -2361 0 0.55 (1) X-Y 0 874 -91.8 -91.8 0.58 (1) 10.00 O-I 0 401 0.06 (4) Y-Z 0 874 -91.8 -91.8 0.58 (1) 10.00 O-J -344 0 0.08 (1) Z-I 0 874 -91.8 -91.8 0.58 (1) 10.00 N-J 2 175 0.03 (4) I-AA -612 0 -91.8 -91.8 0.52 (1) 6.25 S-U 0 2132 0.86 (4) AA-AB -612 0 -91.8 -91.8 0.52 (1) 6.25 N-K 0 1087 0.13 (1) AB-J -612 0 -91.8 -91.8 0.52 (1) 6.25 J-K -1327 0 -91.8 -91.8 0.33 (1) 6.25 K-L 0 47 -91.8 -91.8 0.10 (1) 10.00 V-B -2465 0 0.0 0.0 0.09 (1) 7.81 M-K -1466 0 0.0 0.0 0.05 (1) 7.91 V-AC 0 0 -18.5 -18.5 0.22 (1) 10.00 AC-AD 0 0 -18.5 -18.5 0.22 (1) 10.00 AD-U 0 0 -18.5 -18.5 0.22 (1) 10.00 U-AE 0 2089 -18.5 -18.5 0.59 (1) 10.00 AE-AF 0 2089 -18.5 -18.5 0.59 (1) 10.00 AF-T 0 2089 -18.5 -18.5 0.59 (1) 10.00 T-S 0 2091 -18.5 -18.5 0.32 (1) 10.00 S-R 0 2091 -18.5 -18.5 0.32 (1) 10.00 R-Q -114 0 -18.5 -18.5 0.10 (1) 6.25 Q-AG 0 812 -18.5 -18.5 0.15 (1) 10.00 AG-AH 0 812 -18.5 -18.5 0.15 (1) 10.00 AH-P 0 812 -18.5 -18.5 0.15 (1) 10.00 P-O 0 812 -18.5 -18.5 0.15 (1) 10.00 O-AI 0 1060 -18.5 -18.5 0.11 (4) 10.00 AI-AJ 0 1060 -18.5 -18.5 0.11 (4) 10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 25.6 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 39.0 PSF SPACING = 24.0 IN. OC LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, OBC 2012 - CSA 086-09, CSA 085-14 - TPIC 2011, TPIC 2014 (55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL. (L) = L/360 (0.76") CALCULATED VERT. DEFL. (LL) = L/989 (0.07") ALLOWABLE DEFL. (TL) = L/360 (0.76") CALCULATED VERT. DEFL. (TL) = L/999 (0.13") CSI: TC=0.58/1.00 (H-I), RC=0.59/1.00 (T-U), WB=0.67/1.00 (F-Q), SS=0.26/1.00 (T-U, I) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1987 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg SI GRIP= 0.78 (F) (INPUT = 0.90) SI METAL= 0.32 (G) (INPUT = 1.00)	
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Structural component only 1/2
DWG# T-1924964 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREENYORK HOMES	DRWG NO
405684	T20	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 MITek Industries, Inc. Thu Sep 26 15:23:34 2019 Page 2
ID:yvscRC5EqJey4zVn95GO_vc6AJ-TOx50mKfWg1wLczq7fPIEapmXe8nA-jlOk30vZKL7

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRAC	MEMB. CS1 (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
AJ-N	0.0	1060	-18.5	-18.5	0.11 (4)	10.00			
N-AK	0.0		-18.5	-18.5	0.07 (4)	10.00			
AK-AL	0.0		-18.5	-18.5	0.07 (4)	10.00			
AL-M	0.0		-18.5	-18.5	0.07 (4)	10.00			

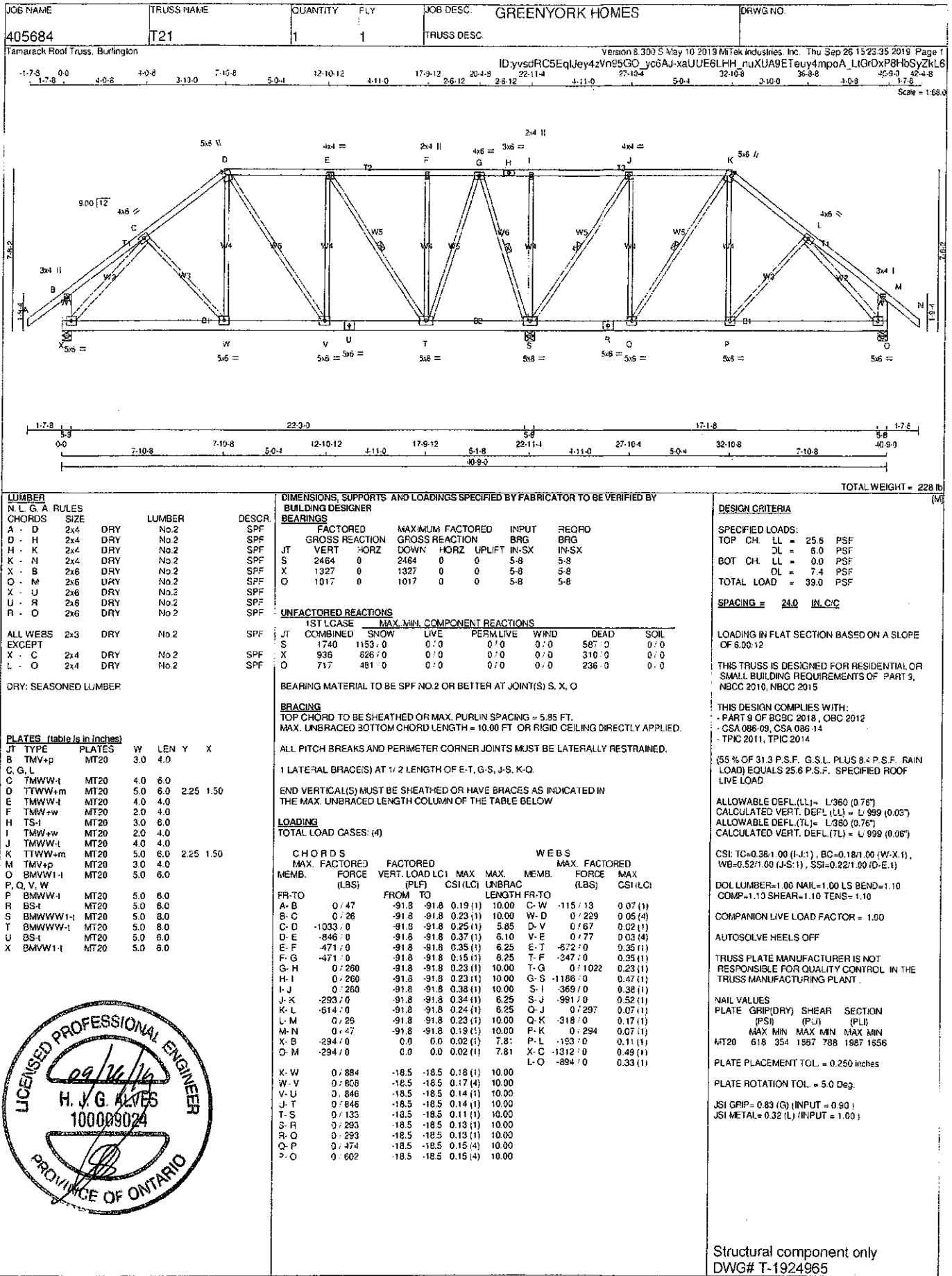
FACTORED CONCENTRATED LOADS (LBS)

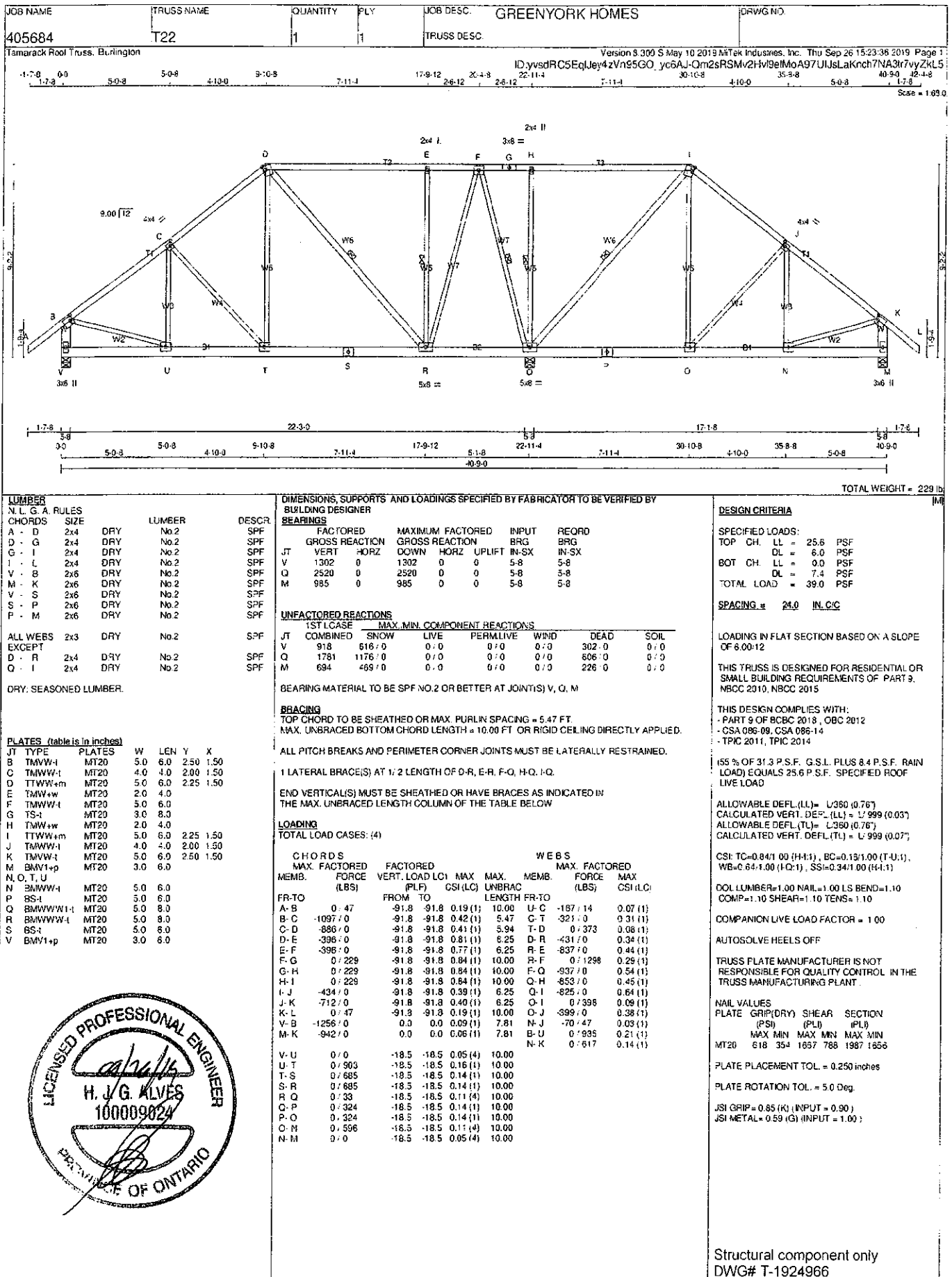
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-428	-428	---	BACK	VERT	TOTAL	---	C1
I	28-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
J	34-10-8	-428	-428	---	BACK	VERT	TOTAL	---	C1
N	34-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
O	29-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
P	26-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
U	3-11-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
W	7-10-4	-178	-178	---	BACK	VERT	TOTAL	---	C1
X	23-6-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
Y	24-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
Z	28-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AA	30-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AB	32-9-12	-178	-178	---	BACK	VERT	TOTAL	---	C1
AC	1-11-4	-23	-25	---	BACK	VERT	TOTAL	---	C1
AD	3-11-4	-28	-29	---	BACK	VERT	TOTAL	---	C1
AE	7-10-4	-36	-36	---	BACK	VERT	TOTAL	---	C1
AF	8-4-8	-1174	-1174	---	BACK	VERT	TOTAL	---	C1
AG	23-6-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AH	24-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AI	30-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AJ	32-9-12	-36	-36	---	BACK	VERT	TOTAL	---	C1
AK	36-9-12	-28	-28	---	BACK	VERT	TOTAL	---	C1
AL	38-9-12	-23	-25	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER-MECHANICAL CONNECTION IS REQUIRED.

Structural component only
DWG# T-1924964 4/2

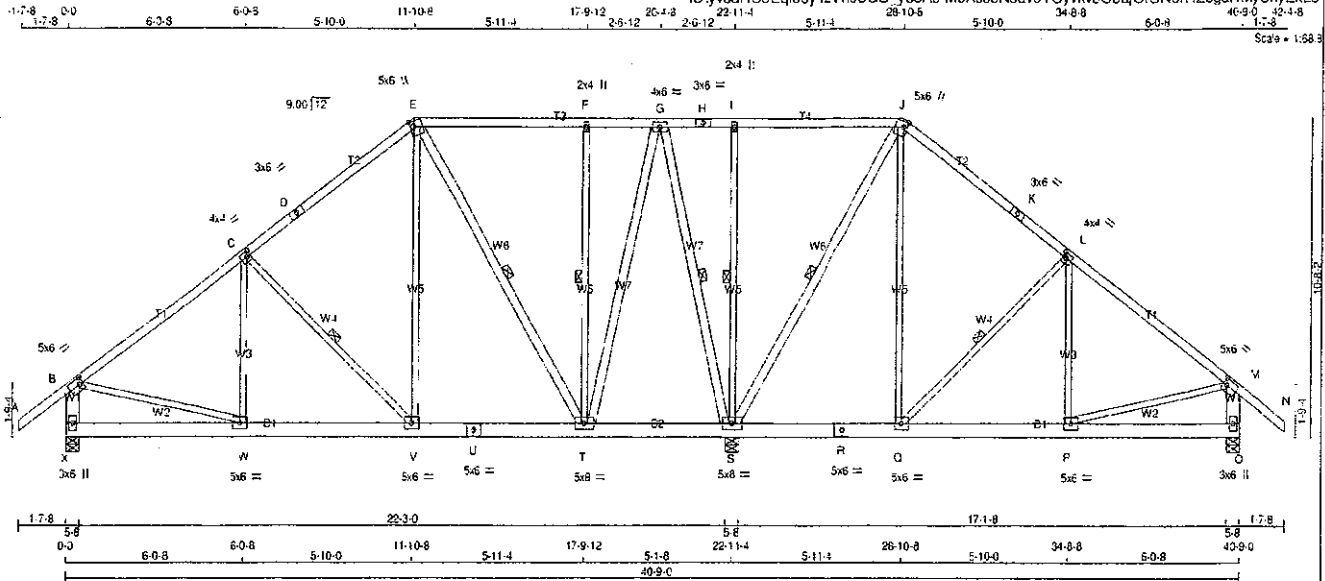




JOB NAME	TRUSS NAME	QUANTITY	PLY	JCE DESC.	GREENYORK HOMES	DRWG NO.
405684	T23	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: yvsdRC5EqJey4zVn95GQ_yo6AJ-M9AcS8NS9vT0ykvvtCbajOION0h4ZogdNMMyCnyZkL3



TOTAL WEIGHT = 249 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - H	2x4	DRY	No.2	SPF	
H - J	2x4	DRY	No.2	SPF	
J - K	2x4	DRY	No.2	SPF	
K - N	2x4	DRY	No.2	SPF	
X - B	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
X - U	2x6	DRY	No.2	SPF	
U - R	2x6	DRY	No.2	SPF	
R - O	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
E - T	2x4	DRY	No.2	SPF	
T - G	2x4	DRY	No.2	SPF	
G - S	2x4	DRY	No.2	SPF	
S - J	2x4	DRY	No.2	SPF	

DRY, SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
B TMWW-1	MT20	5.0	6.0	2.50	1.50
C TMWW-1	MT20	4.0	4.0	2.00	1.50
D, H, K					
D TS-1	MT20	3.0	6.0		
E TTWW+m	MT20	5.0	6.0	2.25	1.50
F TMWW+m	MT20	2.0	4.0		
G TMWW-1	MT20	4.0	6.0		
I TMWW+m	MT20	2.0	4.0		
J TTWW+m	MT20	5.0	6.0	2.25	1.50
L TMWW-1	MT20	4.0	4.0	2.00	1.50
M TMWW-1	MT20	5.0	6.0	2.50	1.50
O BMV1+p	MT20	3.0	6.0		
P, G, V, W					
P BMWW-1	MT20	5.0	6.0		
R BS-1	MT20	5.0	6.0		
S BMWWVW-1	MT20	5.0	6.0		
T BMWWVW-1	MT20	5.0	6.0		
U BS-1	MT20	5.0	6.0		
X BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 1268	DOWN 1268	0	5-8
X	2598	0	0	5-8
O	941	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
X	394	600.0	0.0	0.0	0.0	284.0	0.0
S	1836	1212.0	0.0	0.0	0.0	624.0	0.0
O	863	448.0	0.0	0.0	0.0	215.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, S, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1:2 LENGTH OF C-V. E-T, F-T, G-S, I-S, J-S, L-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO		
A-B	0.47	-91.8	-91.8	0.19 (1)	W-C	-31.70	0.05 (1)
B-C	-1050.0	-91.8	-91.8	0.45 (1)	C-V	-485.0	0.23 (1)
C-D	-700.0	-91.8	-91.8	0.43 (1)	V-E	-0.459	0.13 (1)
D-E	-700.0	-91.8	-91.8	0.43 (1)	E-T	-560.0	0.44 (1)
E-F	-251.0	-91.8	-91.8	0.36 (1)	T-F	-571.0	0.44 (1)
F-G	-251.0	-91.8	-91.8	0.31 (1)	G-S	-1120.0	0.86 (1)
G-H	0.299	-91.8	-91.8	0.48 (1)	S-I	-588.0	0.45 (1)
H-I	0.299	-91.8	-91.8	0.48 (1)	I-J	-900.0	0.71 (1)
I-J	0.299	-91.8	-91.8	0.48 (1)	J-K	-223.0	0.11 (1)
J-K	-223.0	-91.8	-91.8	0.42 (1)	K-L	-223.0	0.11 (1)
K-L	-223.0	-91.8	-91.8	0.42 (1)	L-M	-634.0	0.26 (1)
L-M	-634.0	-91.8	-91.8	0.44 (1)	M-N	0.47	0.03 (1)
M-N	0.47	-91.8	-91.8	0.19 (1)	N-O	-1218.0	0.20 (1)
N-O	-1218.0	0.0	0.0	0.08 (1)	O-P	-894.0	0.12 (1)
O-P	-894.0	0.0	0.0	0.06 (1)			
X-W	0.0	-18.5	-18.5	0.07 (4)			
W-U	0.871	-18.5	-18.5	0.13 (1)			
U-V	0.532	-18.5	-18.5	0.10 (1)			
V-T	0.532	-18.5	-18.5	0.10 (1)			
T-S	-29.0	-18.5	-18.5	0.07 (1)			
S-R	0.151	-18.5	-18.5	0.10 (1)			
R-O	0.151	-18.5	-18.5	0.10 (1)			
O-P	0.539	-18.5	-18.5	0.11 (4)			
P-O	0.0	-18.5	-18.5	0.07 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

BOT CH. LL = 6.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 080-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.48(1.00 (J-I)), BC=0.13(1.00 (V-W)), WB=0.71(1.00 (J-S)), SSI=0.25(1.00 (J-I))

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

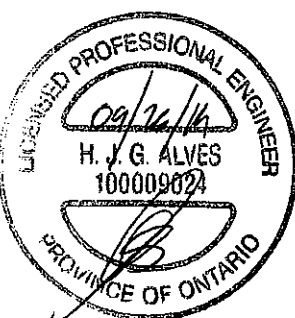
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	788	1987	1656

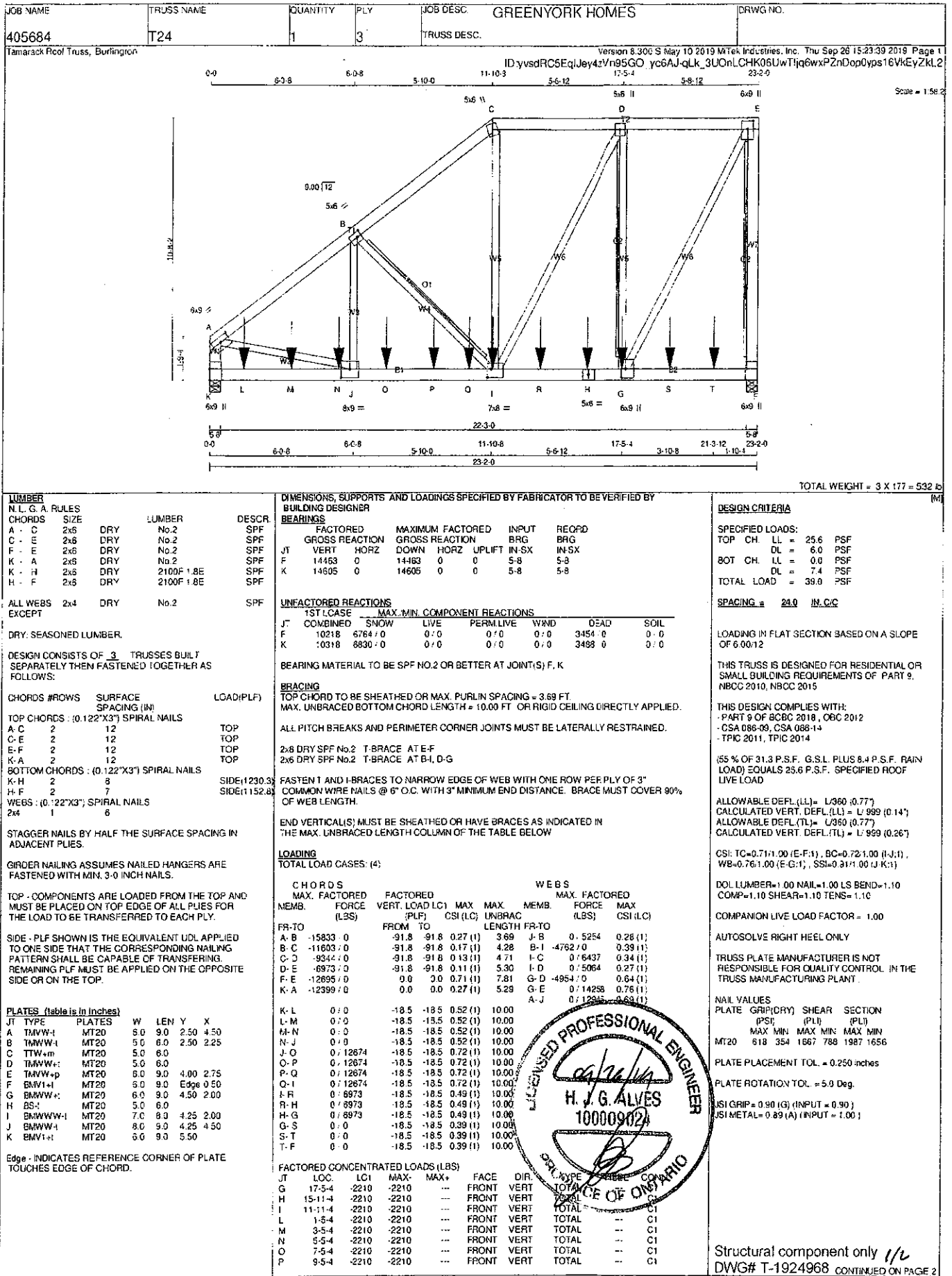
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (G) (INPUT = 0.90)
JSI METAL = 0.33 (G) (INPUT = 1.00)

Structural component only
DWG# T-1924967





JOB NAME 405684	TRUSS NAME T24	QUANTITY 1	PLY 3	JOB DESC. GREENYORK HOMES	TRUSS DESC.	DRWG NO.
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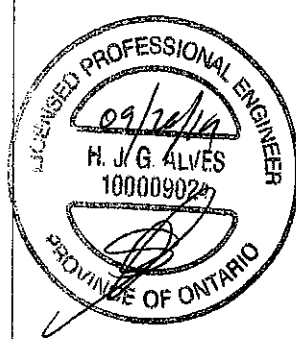
Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 MiTek Industries, Inc. Thu Sep 26 15:23:39 2019 Page 2
ID:yvscRC5EqJev4zVn95GO_vo6AJ-qLk_3UOnLCHK06UwTtig5wxPZnDop0yps16VkeYzkl2

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	10-11-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
R	13-11-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
S	19-5-4	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1
T	21-3-12	-2210	-2210	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
 DWG# T-1924968

JOB NAME 405684	TRUSS NAME T25	QUANTITY 2	PLY 1	JOB DESC GREENYORK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington
Version 8.390 S May 10 2019 MIT Industries, Inc. Thu Sep 26 15:23:41 2019 Page 1

ID:yvsdRC5EqJey4zv95GO_yc6AJ.mksIU9Q2tqX2FPaJqIIBL0oeb06H2a6KLbcp6yZkL0

Scale = 1/8" = 1'-0"

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - E	2x4	DRY No.2
E - F	2x4	DRY No.2
G - F	2x4	DRY No.2
K - B	2x4	DRY No.2
K - I	2x4	DRY No.2
I - G	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
EXCEPT		
H - E	2x4	DRY No.2
H - F	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-i	MT20	4.0	4.0	2.00	1.50
D	TS-i	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-i	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-i	MT20	4.0	9.0		
I	BS-i	MT20	3.0	6.0		
J	BMWW-i	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

SPF	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	995	0	995	0	5-8	5-8
G	1152	0	1152	0	5-8	5-8
K						

UNFACTORED REACTIONS

SPF	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
JT	703	482 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0
G	812	549 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
K							

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, C-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX. CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)	UNBRACED LENGTH (FT)
A-B	0 / 47	-91.8	-91.8 0.19 (1)	10.00	J-C	-58 / 78	0.04 (1)
B-C	-893 / 0	-91.8	-91.8 0.45 (1)	5.88	C-H	-504 / 0	0.24 (1)
C-D	-529 / 0	-91.8	-91.8 0.43 (1)	6.25	H-E	-190 / 14	0.11 (1)
D-E	-529 / 0	-91.8	-91.8 0.43 (1)	6.25	H-F	0 / 772	0.12 (1)
E-F	-387 / 0	-91.8	-91.8 0.45 (1)	6.25	B-J	0 / 767	0.17 (1)
G-F	-949 / 0	0.0	0.0 0.55 (1)	6.25			
K-B	-1106 / 0	0.0	0.0 0.12 (1)	7.53			
K-J	0 / 0	-18.5	-18.5 0.15 (4)	10.00			
J-I	0 / 746	-18.5	-18.5 0.22 (4)	10.00			
I-H	0 / 746	-18.5	-18.5 0.22 (4)	10.00			
H-G	0 / 0	-18.5	-18.5 0.15 (4)	10.00			

TOTAL WEIGHT = 2 X 101 = 202 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.60")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.60")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.55/1.00 (F-G-1), BC=0.22/1.00 (H-J-4),
WB=0.24/1.00 (C-H-1), SSI=0.22/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

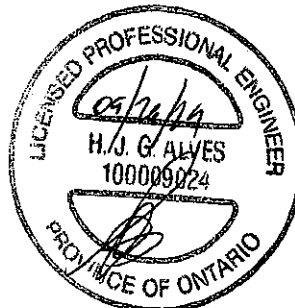
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 738 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

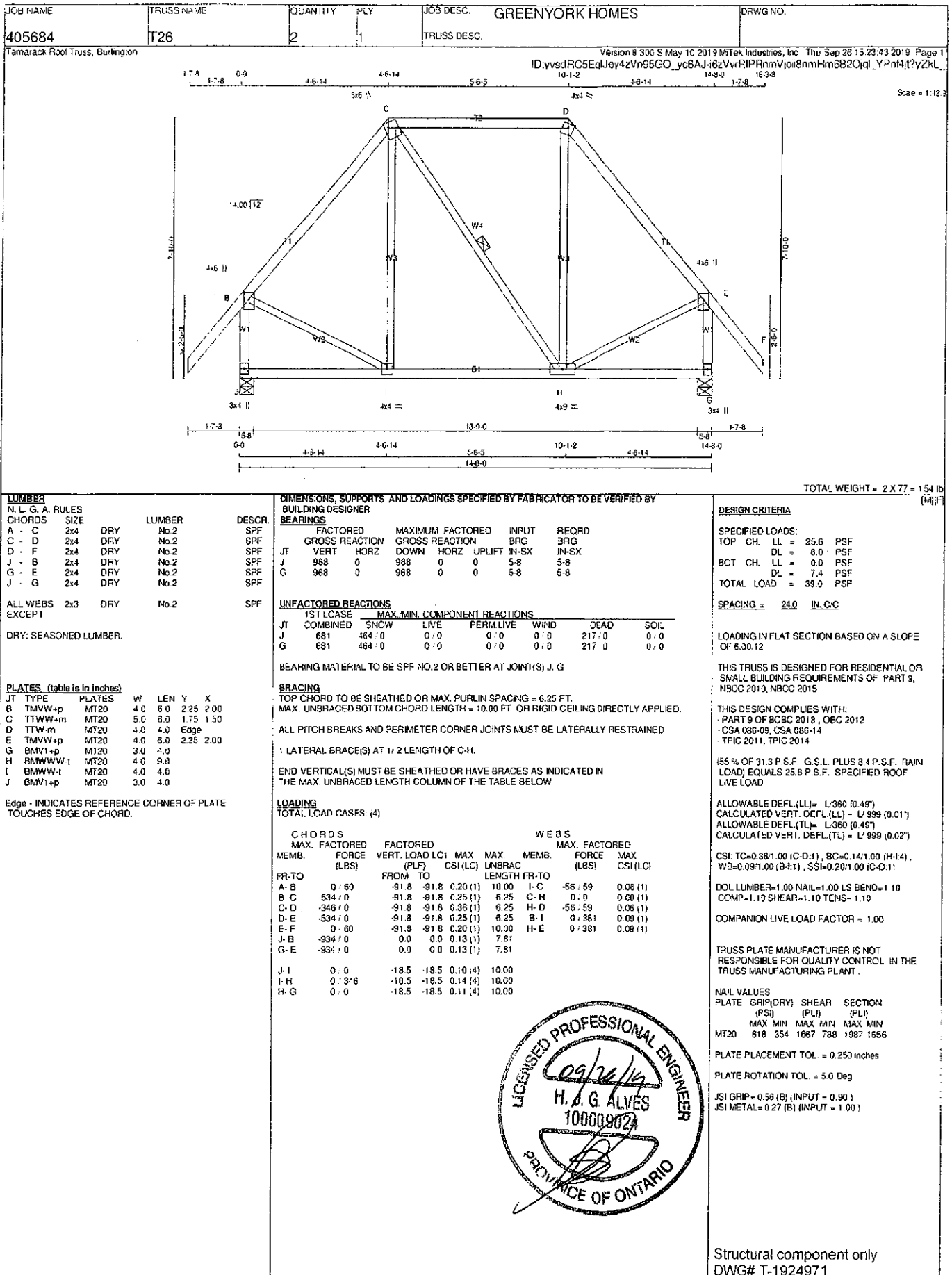
PLATE ROTATION TOL = 5.0 Deg.

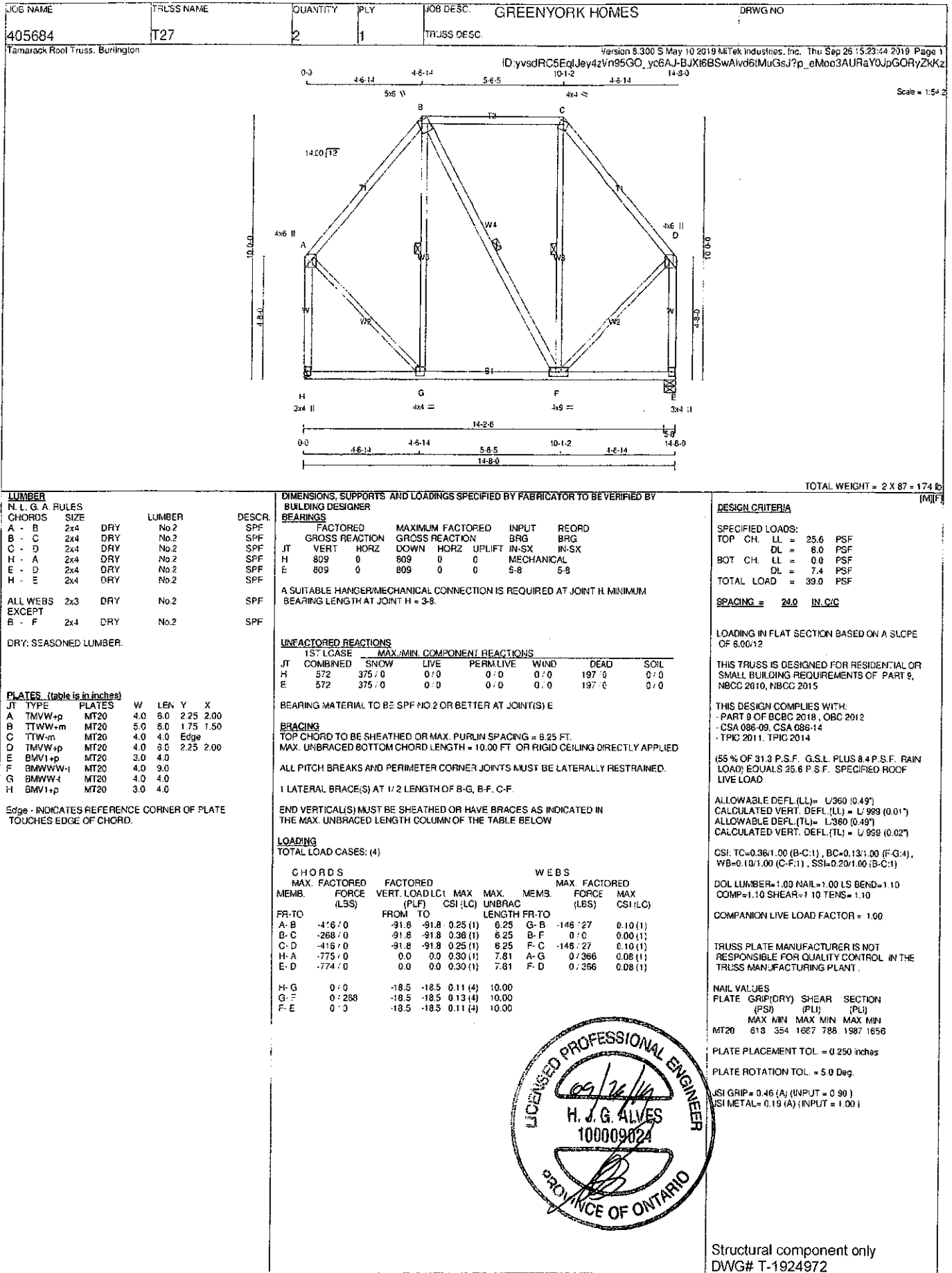
JSI GRIP = 0.90 (J) (INPUT = 0.90)
JSI METAL = 0.51 (B) (INPUT = 1.00)

Structural component only
DWG# T-1924969



[illegible]





JOB NAME T28 **TRUSS NAME** T28 **QUANTITY** 1 **P-LY** 1 **JOB DESC.** GREENYORK HOMES **DRWG NO.**

405684 **T28** **1** **1** **TRUSS DESC.**

Tanarack Roof Truss, Burlington

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ID:yvsdRC5EqJey4zVn95GO_yc8AJ-7hleXUaiMAKMBWHLNLTUPkaicccYErtIdINTKYZkKx 25-10-8 25-10-8
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1-6-8 0-0 1-13-8 4-10-8 5-2-12 10-1-4 2-5-12 12-8-0 2-5-12 5-2-12 20-5-8 4-10-8 25-4-0 1-6-8 1-13-8

9'0" / 12'

5'x8" =

3'x6" I

5'x8" =

3'x6" I

1-7-12 0-0 1-11-4 2-0-0 3-11-4 4-10-8 5-2-12 10-1-4 5-1-8 15-2-12 5-2-12 20-5-8 21-4-12 23-4-12 1-11-4 2-0-0 1-11-4 25-4-0

LUMBER

N L G A RULES	CHORDS	SIZE	DRY	No.2	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	5.0	8.0	Edge
C	TTWW-m	MT20	6.0	9.0	Edge 2.00
D	TMWW-w	MT20	2.0	4.0	
E	TMWW-l	MT20	4.0	4.0	
F	TMWW-w	MT20	2.0	4.0	
G	TTWW-m	MT20	6.0	9.0	Edge 2.00
H	TMWW-p	MT20	5.0	8.0	Edge
J	BMVl+p	MT20	3.0	6.0	
K	BMWW-l	MT20	5.0	6.0	2.50 2.00
L	BMWW-W-l	MT20	5.0	8.0	
M	BS-I	MT20	3.0	8.0	
N	BMWW-W-l	MT20	5.0	8.0	
O	BMWW-l	MT20	5.0	5.0	2.50 2.00
P	BMVl+p	MT20	3.0	5.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
VERT	HORZ	DOWN	HORZ UPLIFT IN-SX
JT 2472 0	2472 0	0	3-8
P 2472 0	2472 0	0	3-8
J 2472 0	2472 0	0	3-8

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERMILIVE	WIND	DEAD	SOIL
JT	COMBINED	1158 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0
P	1747	1158 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0
J	1747	1158 / 0	0 / 0	0 / 0	0 / 0	590 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. COI (LC)	UNBRAC LENGTH FR-TO	MEMB.	WE B'S MAX. FORCE (LBS)	FACTORED MAX. COI (LC)
FR-TO					FR-TO		
A-B	0 / 45	-91.8	-91.8 0.19 (1)	10.00	C-C	-375 / 11	0.17 (1)
B-C	-2496 / 0	-91.8	-91.8 0.62 (1)	3.71	C-N	0 / 1514	0.37 (1)
C-Q	3074 / 0	-91.8	-91.8 0.84 (1)	2.93	N-D	-836 / 0	0.37 (1)
Q-R	-3074 / 0	-91.8	-91.8 0.84 (1)	2.93	L-F	-836 / 0	0.37 (1)
R-D	-3074 / 0	-91.8	-91.8 0.84 (1)	2.93	L-G	0 / 1514	0.37 (1)
D-S	-3074 / 0	-91.8	-91.8 0.72 (1)	2.93	K-G	-375 / 11	0.17 (1)
S-E	-3074 / 0	-91.8	-91.8 0.72 (1)	2.93	B-O	0 / 2069	0.51 (1)
E-T	-3074 / 0	-91.8	-91.8 0.72 (1)	2.93	K-H	0 / 2069	0.51 (1)
T-F	-3074 / 0	-91.8	-91.8 0.72 (1)	2.93	N-E	-80 / 0	0.03 (1)
F-U	-3074 / 0	-91.8	-91.8 0.84 (1)	2.93	E-L	60 / 0	0.03 (1)
U-V	-3074 / 0	-91.8	-91.8 0.84 (1)	2.93			
V-G	-3074 / 0	-91.8	-91.8 0.84 (1)	2.93			
G-H	-2496 / 0	-91.8	-91.8 0.62 (1)	3.71			
H-I	0 / 45	-91.8	-91.8 0.19 (1)	10.00			
P-B	-2428 / 0	0.0	0.0 0.19 (1)	6.58			
J-H	-2428 / 0	0.0	0.0 0.19 (1)	6.58			

WE B'S

MEMB.	FORCE (LBS)	FACTORED MAX. COI (LC)
P-W	0 / 0	-18.5 -18.5 0.19 (4)
W-X	0 / 0	-18.5 -18.5 0.19 (4)
X-O	0 / 0	-18.5 -18.5 0.19 (4)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREENYORK HOMES	DRWG NO.
405684	T28	1	1	TRUSS DESC.		

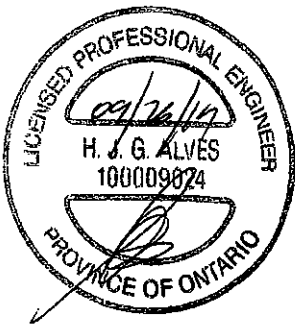
Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 MITek Industries, Inc. Thu Sep 26 15:23:46 2019 Page 2
ID:yvsdRC5EqJey4zVn95GO_yc6AJ-7hleXIUAIIMAKMBWHNGLTupKalcotyEcrTdINTKyZkKx

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	8-11-4	-132	-132	---	FRONT	VERT	TOTAL	---	C1
R	8-11-4	-132	-132	---	FRONT	VERT	TOTAL	---	C1
S	10-11-4	-132	-132	---	FRONT	VERT	TOTAL	---	C1
T	14-4-12	-132	-132	---	FRONT	VERT	TOTAL	---	C1
U	16-4-12	-132	-132	---	FRONT	VERT	TOTAL	---	C1
V	18-4-12	-132	-132	---	FRONT	VERT	TOTAL	---	C1
W	1-11-4	-13	-15	---	FRONT	VERT	TOTAL	---	C1
X	3-11-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1
Y	5-11-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
Z	8-11-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AA	10-11-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AB	12-8-0	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AC	14-4-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AD	16-4-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AE	18-4-12	-27	-27	---	FRONT	VERT	TOTAL	---	C1
AF	21-4-12	-17	-17	---	FRONT	VERT	TOTAL	---	C1
AG	23-4-12	-13	-15	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



Structural component only
DWG# T-1924973 *yl*

JOB NAME 045684	TRUSS NAME IT29	QUANTITY 1	PLY 1	JOB DESC. GREENYORK HOMES	DWG NO. T-1924974
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Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 Mitek Industries, Inc. Thu Sep 26 15:23:47 2019 Page 1
ID:yvvdRC5EqJey4zVn95GO_yc6AJ-buD0iDupSgIBzk5Tx_sIRoGs3?2GgIJ_iH2w?myZkKw

Scale = 1/4"=1'-0"

LUMBER

N.L.G.A.RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
M - N	2x4	DRY	No.2	SPF
O - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-i	MT20	5.0	6.0	
D	TTWW+m	MT20	5.0	6.0	2.25 1.50
E	TMW+w	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	6.0	2.25 1.50
G	TMWW-i	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
I	BMVW-i	MT20	5.0	6.0	
J	BMVW-i	MT20	4.0	4.0	
K	BS-i	MT20	3.0	6.0	
L	BMVW+i	MT20	4.0	9.0	
M	BMVW-i	MT20	4.0	4.0	
N	BMVW-i	MT20	4.0	4.0	
O	BMVW-i	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT VERT	VERT	HORZ	UPLIFT	
J	1546	0	1548	0
O	1546	0	1548	0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
J	1091	732 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0
O	1091	731 / 0	0 / 0	0 / 0	0 / 0	359 / 0	0 / 0

BEARING MATERIAL TO BE SPF No.2 OR BETTER AT JOINT(S) J, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					LENGTH FR-TO		
A-B	0 / 45	-91.8	-91.8	0.17 (1)	10.00	C-N	0 / 54
B-C	0 / 20	-91.8	-91.8	0.16 (1)	10.00	N-D	0 / 117
C-D	-1373 / 0	-91.8	-91.8	0.15 (1)	5.41	D-M	0 / 491
D-E	-1408 / 0	-91.8	-91.8	0.42 (1)	4.99	M-E	-651 / 0
E-F	-1406 / 0	-91.8	-91.8	0.42 (1)	4.99	M-F	0 / 491
F-G	-1373 / 0	-91.8	-91.8	0.15 (1)	5.41	K-F	0 / 117
G-H	0 / 20	-91.8	-91.8	0.16 (1)	10.00	K-G	0 / 54
H-I	0 / 45	-91.8	-91.8	0.17 (1)	10.00	O-C	-1616 / 0
O-B	-272 / 0	0.0	0.0	0.02 (1)	7.81	G-J	-1616 / 0
J-H	-272 / 0	0.0	0.0	0.02 (1)	7.81		
O-N	0 / 1058	-18.5	-18.5	0.29 (4)	10.00		
N-M	0 / 1081	-18.5	-18.5	0.30 (4)	10.00		
M-L	0 / 1081	-18.5	-18.5	0.30 (4)	10.00		
L-K	0 / 1081	-18.5	-18.5	0.30 (4)	10.00		
K-J	0 / 1058	-18.5	-18.5	0.29 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	25.6 PSF
DL	=	6.0 PSF
BOT CH. LL	=	0.0 PSF
DL	=	7.4 PSF
TOTAL LOAD	=	39.0 PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55' OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.84")

CALCULATED VERT. DEFL.(LL) = L/999 (0.05")

ALLOWABLE DEFL.(TL)= L/360 (0.84")

CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.42/1.00 (D-E); SC=0.30/1.00 (M-N); WB=0.68/1.00 (C-O); SS=0.26/1.00 (D-E);

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1587	788
	1587	1356

PLATE PLACEMENT TOL. = 0.250 inches

[illegible]

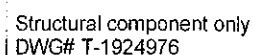
Version 8.300 S May 10 2019 MiTek Industries, Inc. Thu Sep 26 15:23:50 2019 Page 1

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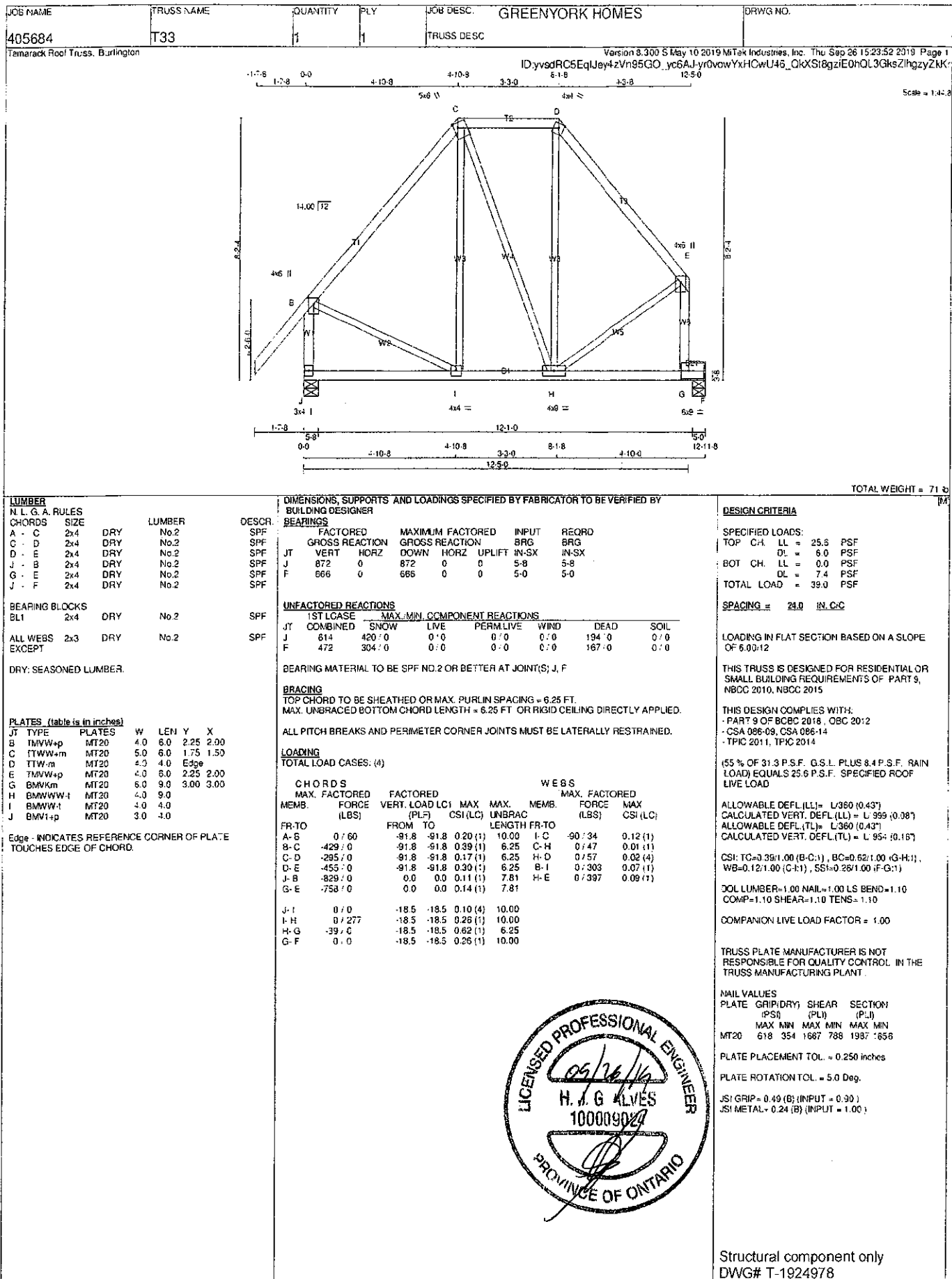


TOTAL WEIGHT = 126 lb

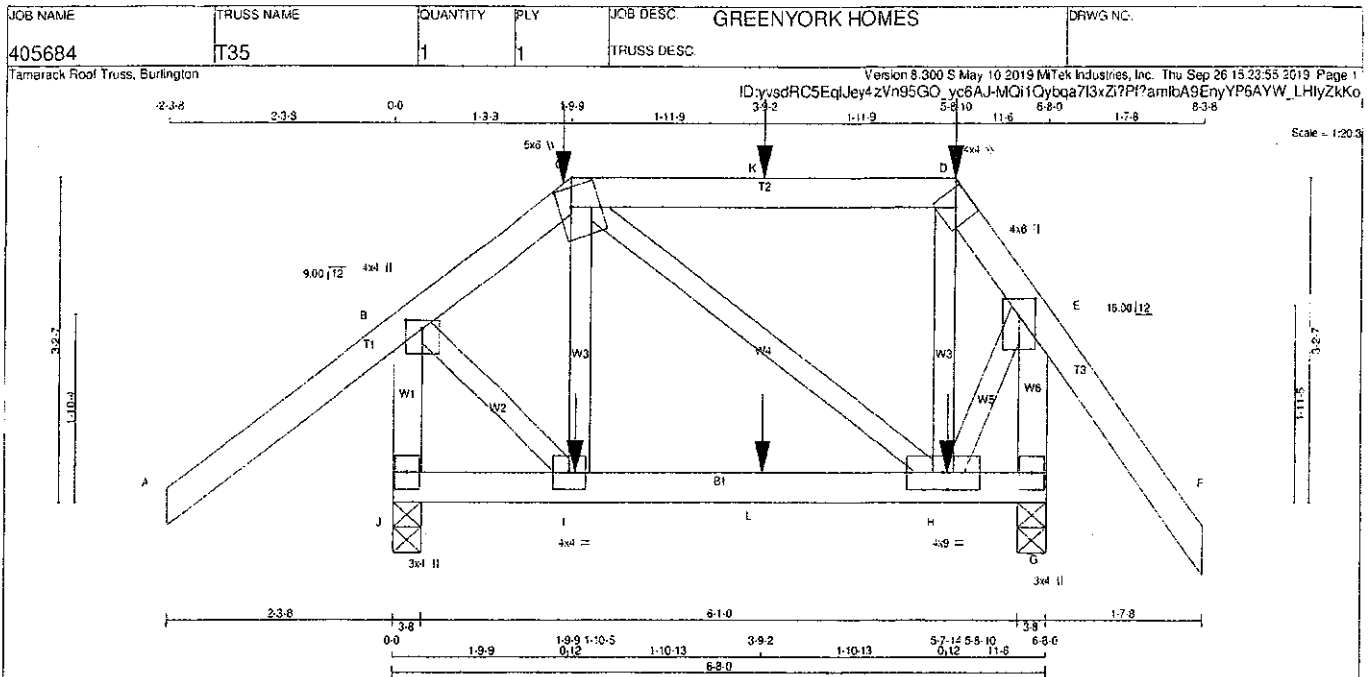
JSI GRIP= 0.66 (G) (INPUT = 0.90)
JSI METAL= 0.33 (G) (INPUT = 1.00)



[illegible]



[illegible]



TOTAL WEIGHT = 38 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTW-h	MT20	4.0	4.0	
E	TMVW+p	MT20	4.0	6.0	2.00 2.00
G	BMV1+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	4.0	9.0	
I	BMVW-t	MT20	4.0	4.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	962	0	962	0
G	972	0	972	0

UNFACTORED REACTIONS

	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
J	676	466 0	0 0	0 0	0 0	211 0	0 0
G	685	461 -22	0 0	0 0	0 0	225 0	0 0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0.65	-91.8	-91.8 0.61 (5)	I-C	0.189	0.05 (1)	
B-C	-528.70	-91.8	-91.8 0.59 (5)	C-H	-121.727	0.05 (1)	
C-K	-260.70	-91.8	-91.8 0.29 (1)	H-D	0.236	0.06 (1)	
D-E	-260.70	-91.8	-91.8 0.29 (1)	B-I	0.448	0.11 (1)	
E-F	-549.70	-91.8	-91.8 0.21 (1)	H-E	0.449	0.11 (1)	
F-G	0.64	-91.8	-91.8 0.22 (1)				
J-B	-991.70	0.0	0.0 0.12 (1)				
G-E	-1070.70	0.0	0.0 0.13 (1)				
J-I	0.70	-18.5	-18.5 0.13 (1)				
I-L	0.356	-18.5	-18.5 0.31 (1)				
L-H	0.356	-18.5	-18.5 0.31 (1)				
H-G	0.70	-18.5	-18.5 0.17 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-9-9	-25	-25	---	FRONT	VERT.	TOTAL	---	C1
D	5-8-10	-25	-25	---	FRONT	VERT.	TOTAL	---	C1
H	5-7-14	-251	-251	---	FRONT	VERT.	TOTAL	---	C1
I	1-10-5	-251	-251	---	FRONT	VERT.	TOTAL	---	C1
K	3-9-2	-18	-18	---	FRONT	VERT.	TOTAL	---	C1
L	3-9-2	-251	-251	---	FRONT	VERT.	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	25.6	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	39.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00:12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.22")

CALCULATED VERT. DEFL. (LL) = L/989 (0.02")

ALLOWABLE DEFL. (LL) = L/360 (0.22")

CALCULATED VERT. DEFL. (LL) = L/999 (0.03")

CSI: TC=0.61:1.00 (A-B.5), BC=0.31:1.00 (H-I.1)

WB=0.11:1.00 (E-H.1), SS=0.23:1.00 (A-B.5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	618	354	1667

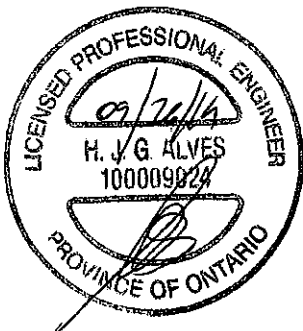
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.73 (B) (INPUT = 0.90)

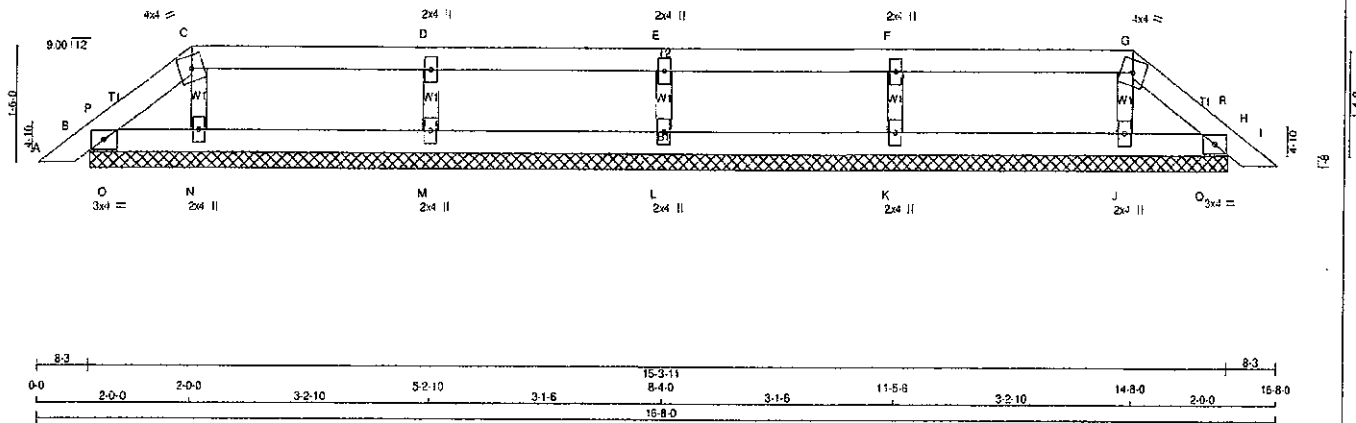
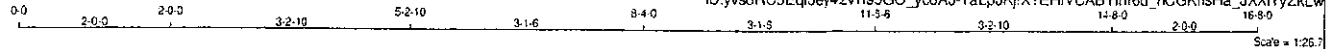
JSI METAL = 0.22 (E) (INPUT = 1.00)

Structural component only
DWG# T-1924980



JOB NAME 405684	TRUSS NAME P1	QUANTITY 1	PLY 1	JOB DESC. GREENYORK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: yvscRC5EqJey4zVn95GO_yo6AJ-TaLpJkX?EHIVCABYihrd_hCGKnsHa_JXXiTyZkLw



TOTAL WEIGHT = 41 k

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - G	2x4 DRY	No.2	SPF
G - I	2x4 DRY	No.2	SPF
I - H	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1+1	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D, E, F					
D	TMW+w	MT20	2.0	4.0	
G	TTW-m	MT20	4.0	4.0	
H	TMB1-1	MT20	3.0	4.0	
J, K, L, M, N					
J	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
B	137	0	137	0	0	15-3-11	15-3-11		
H	137	0	137	0	0	15-3-11	15-3-11		
N	210	0	210	0	0	15-3-11	15-3-11		
M	391	0	391	0	0	15-3-11	15-3-11		
L	319	0	319	0	0	15-3-11	15-3-11		
K	391	0	391	0	0	15-3-11	15-3-11		
J	210	0	210	0	0	15-3-11	15-3-11		

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
B	95	74/0	0/0	0/0	0/0
H	95	74/0	0/0	0/0	0/0
N	150	90/0	0/0	0/0	0/0
M	276	185/0	0/0	0/0	0/0
L	226	146/0	0/0	0/0	0/0
K	276	185/0	0/0	0/0	0/0
J	150	90/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H, N, M, L, K, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (LBS)
FR-TO		FROM	TO	FR-TO			
A-B	0/15	-91.8	-91.8 0.02 (1)	N-C	-146/0	0.02 (1)	
B-P	-26/0	-91.8	-91.8 0.00 (1)	M-D	-334/0	0.05 (1)	
P-C	-30/0	-91.8	-91.8 0.02 (1)	L-E	-282/0	0.04 (1)	
C-D	-8/0	-91.8	-91.8 0.14 (1)	K-F	-334/0	0.05 (1)	
D-E	-8/0	-91.8	-91.8 0.14 (1)	J-G	-146/0	0.02 (1)	
E-F	-8/0	-91.8	-91.8 0.14 (1)	O-P	-65/0	0.00 (1)	
F-G	-8/0	-91.8	-91.8 0.14 (1)	Q-R	-65/0	0.00 (1)	
G-R	-30/0	-91.8	-91.8 0.02 (1)				
R-H	-26/0	-91.8	-91.8 0.00 (1)				
H-I	0/15	-91.8	-91.8 0.02 (1)				
B-O	0/23	-18.5	-18.5 0.02 (1)				
O-N	0/23	-18.5	-18.5 0.03 (4)				
N-M	0/8	-18.5	-18.5 0.04 (4)				
M-L	0/8	-18.5	-18.5 0.04 (4)				
L-K	0/8	-18.5	-18.5 0.04 (4)				
K-J	0/8	-18.5	-18.5 0.04 (4)				
J-Q	0/23	-18.5	-18.5 0.03 (4)				
Q-H	0/23	-18.5	-18.5 0.02 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF NBC 2010, NBC 2012
CSA 086-09, CSA 086-14
TPIC 2011, TPIC 2014

(5% OF 31.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F/G-1), BC=0.04/1.00 (M/N-4), WB=0.05/1.00 (O-M-1), SS=0.14/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

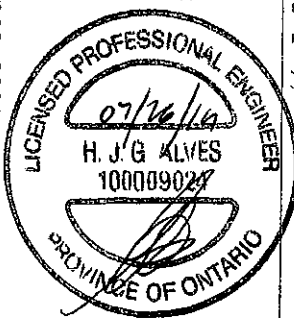
NAIL VALUES		PLATE GRIP (DPRY)		SHEAR SECTION	
(PSI)	(PLU)	(PSI)	(PLU)	(PSI)	(PLU)
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.17 (M) (INPUT = 0.90)

JSI METAL = 0.07 (F) (INPUT = 1.00)



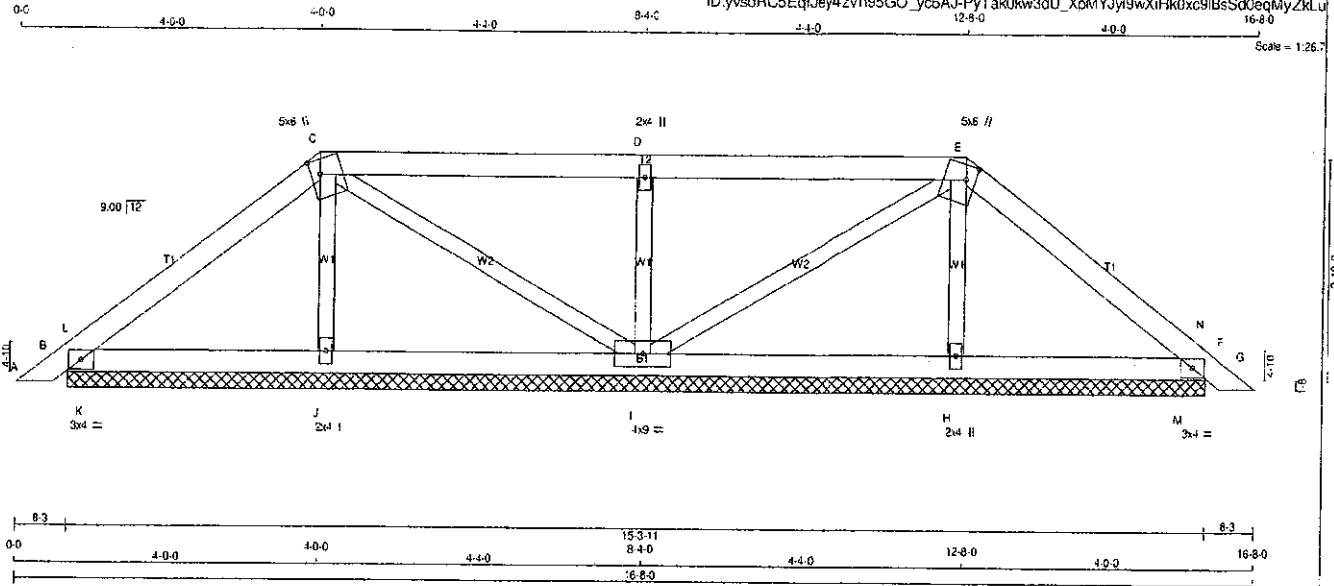
Structural component only
DWG# T-1924927

JOB NAME 405684	TRUSS NAME P2	QUANTITY 1	PLY 1	JOB DESC. GREENYORK HOMES	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:yvsdRC5EqJey4zVn85GO_yc6AJ-PyTak0kw3dU_XoMYJyI9wXihk0xc9IBsSd0eqMyZkLu



TOTAL WEIGHT = 52 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
G - I	2x4 DRY	No.2	SPF		
I - K	2x4 DRY	No.2	SPF		
K - L	2x4 DRY	No.2	SPF		
L - M	2x4 DRY	No.2	SPF		
M - N	2x4 DRY	No.2	SPF		
N - O	2x4 DRY	No.2	SPF		
O - P	2x4 DRY	No.2	SPF		
P - Q	2x4 DRY	No.2	SPF		
Q - R	2x4 DRY	No.2	SPF		
R - S	2x4 DRY	No.2	SPF		
S - T	2x4 DRY	No.2	SPF		
T - U	2x4 DRY	No.2	SPF		
U - V	2x4 DRY	No.2	SPF		
V - W	2x4 DRY	No.2	SPF		
W - X	2x4 DRY	No.2	SPF		
X - Y	2x4 DRY	No.2	SPF		
Y - Z	2x4 DRY	No.2	SPF		
Z - A	2x4 DRY	No.2	SPF		

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	IM31-I	MT20	3.0	4.0			
C	TTWW+m	MT20	5.0	6.0	2.25	1.50	
D	TTWW+m	MT20	2.0	4.0			
E	TTWW+m	MT20	5.0	6.0	2.25	1.50	
F	TTWW+m	MT20	2.0	4.0			
G	TTWW+m	MT20	5.0	6.0	2.25	1.50	
H	TTWW+m	MT20	2.0	4.0			
I	TTWW+m	MT20	5.0	6.0	2.25	1.50	
J	TTWW+m	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	284	0	284	0	0	15-3-11	15-3-11	15-3-11	15-3-11
F	284	0	284	0	0	15-3-11	15-3-11	15-3-11	15-3-11
J	304	0	304	0	0	15-3-11	15-3-11	15-3-11	15-3-11
I	620	0	620	0	0	15-3-11	15-3-11	15-3-11	15-3-11
H	304	0	304	0	0	15-3-11	15-3-11	15-3-11	15-3-11

UNFACTORED REACTIONS															
1ST CASE		MAX./MIN. COMPONENT REACTIONS													
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	199	144 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0	B	199	144 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
F	199	144 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0	F	199	144 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
J	217	128 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0	J	217	128 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0
I	436	300 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0	I	436	300 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	217	128 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0	H	217	128 / 0	0 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO					FR-TO			FR-TO			
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00	J-C	-190 / 0	0.03 (1)			
B-L	-23 / 0	-91.8	-91.8	0.05 (1)	6.25	C-I	-58 / 0	0.02 (1)			
L-C	-103 / 0	-91.8	-91.8	0.12 (1)	6.25	I-D	-496 / 0	0.09 (1)			
C-D	-19 / 0	-91.8	-91.8	0.29 (1)	6.25	D-E	-58 / 0	0.02 (1)			
D-E	-19 / 0	-91.8	-91.8	0.29 (1)	6.25	E-H	-190 / 0	0.03 (1)			
E-N	-103 / 0	-91.8	-91.8	0.12 (1)	6.25	H-L	-281 / 0	0.00 (1)			
N-F	-23 / 0	-91.8	-91.8	0.05 (1)	6.25	M-N	-281 / 0	0.00 (1)			
F-G	0 / 15	-91.8	-91.8	0.02 (1)	10.00						
B-K	0 / 77	-18.5	-18.5	0.11 (1)	10.00						
K-J	0 / 77	-18.5	-18.5	0.11 (1)	10.00						
J-I	0 / 69	-18.5	-18.5	0.07 (1)	10.00						
I-H	0 / 69	-18.5	-18.5	0.07 (1)	10.00						
H-M	0 / 77	-18.5	-18.5	0.11 (1)	10.00						
M-F	0 / 77	-18.5	-18.5	0.11 (1)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D), BC=0.11/1.00 (J-K), WB=0.09/1.00 (D-E), SS=0.22/1.00 (B-K, I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

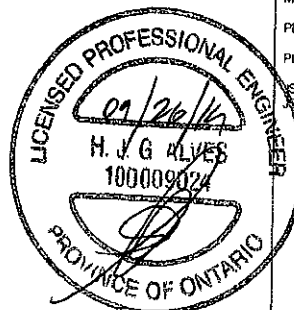
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

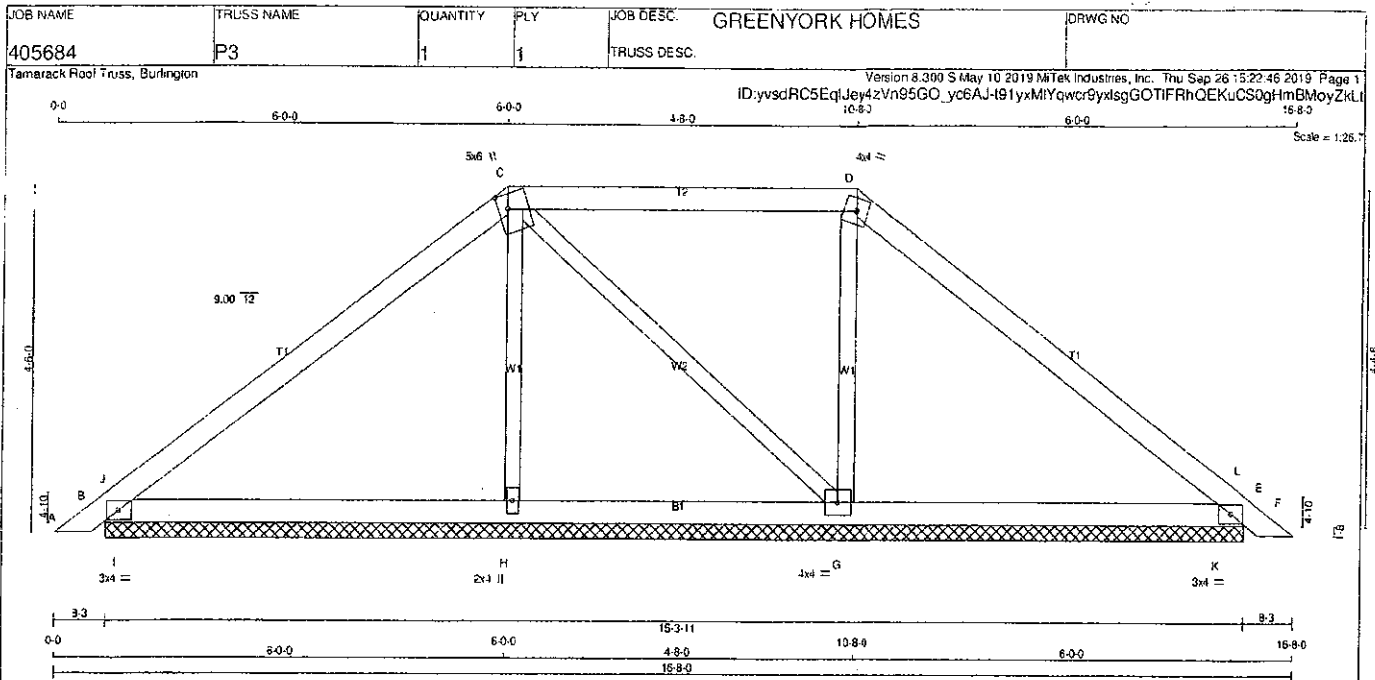
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

SI GRIP= 0.26 (D) (INPUT = 0.90)
SI METAL= 0.10 (D) (INPUT = 1.00)



Structural component only
DWG# T-1924928



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0		
C	TTWW-I	MT20	5.0	6.0	2.25	1.50
D	TTWW-I	MT20	4.0	4.0		
E	TM81-I	MT20	3.0	4.0		
G	BMWV1-I	MT20	4.0	4.0		
H	BMWV1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
B	492	0	0	15-3-11
E	462	0	0	15-3-11
H	370	0	0	15-3-11
G	473	0	0	15-3-11

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
B	345 244 / 0 0 / 0 0 / 0 100 0 0 / 0
E	324 227 / 0 0 / 0 0 / 0 97 0 0 / 0
H	264 157 / 0 0 / 0 0 / 0 108 0 0 / 0
G	335 216 / 0 0 / 0 0 / 0 119 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 15	-91.8 -91.8 0.02 (1)	10.00	H-C	-230 / 0	0.07 (1)	
S-J	-69 / 10	-91.8 -91.8 0.16 (1)	6.25	C-G	-55 / 0	0.03 (1)	
J-C	-274 / 0	-91.8 -91.8 0.33 (1)	6.25	G-D	-298 / 0	0.08 (1)	
C-D	-161 / 0	-91.8 -91.8 0.34 (1)	6.25	F-J	-655 / 0	0.00 (1)	
D-L	-225 / 0	-91.8 -91.8 0.33 (1)	6.25	K-L	-660 / 0	0.00 (1)	
L-E	-61 / 63	-91.8 -91.8 0.17 (1)	6.25				
E-F	0 / 15	-91.8 -91.8 0.02 (1)	10.00				
B-I	0 / 208	-18.5 -18.5 0.27 (1)	10.00				
I-H	0 / 208	-18.5 -18.5 0.27 (1)	10.00				
H-G	0 / 202	-18.5 -18.5 0.15 (1)	10.00				
G-K	0 / 169	-18.5 -18.5 0.27 (1)	10.00				
K-E	0 / 169	-18.5 -18.5 0.27 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.34/1.00 (C-D:1), BC=0.27/1.00 (H-I:1), WB=0.08/1.00 (D-G:1), SSI=0.52/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

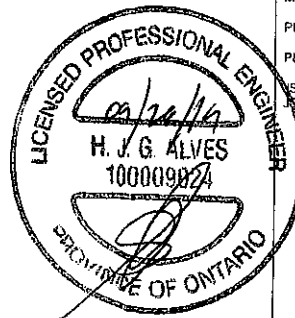
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

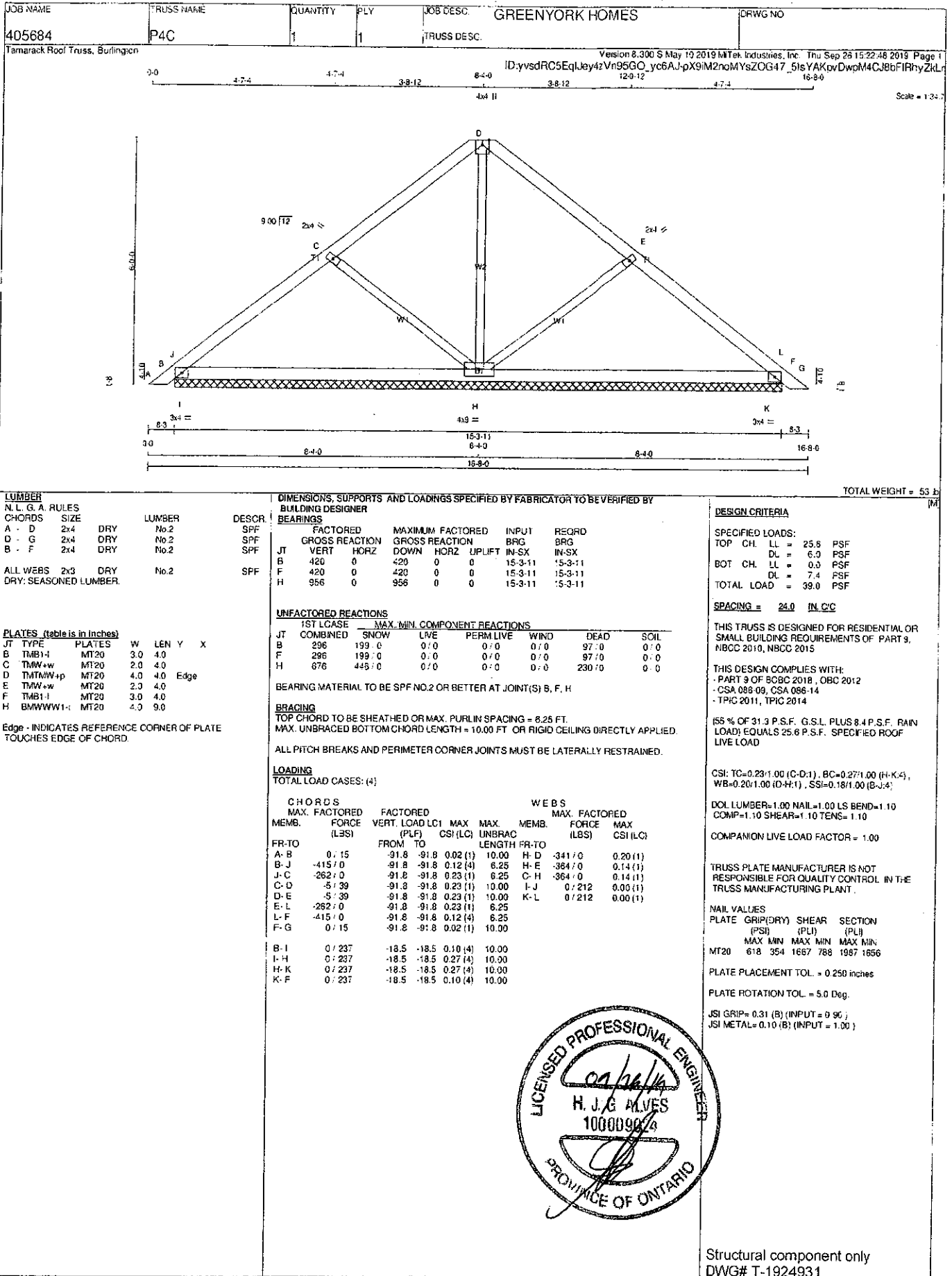
PLATE ROTATION TOL = 5.0 Deg.

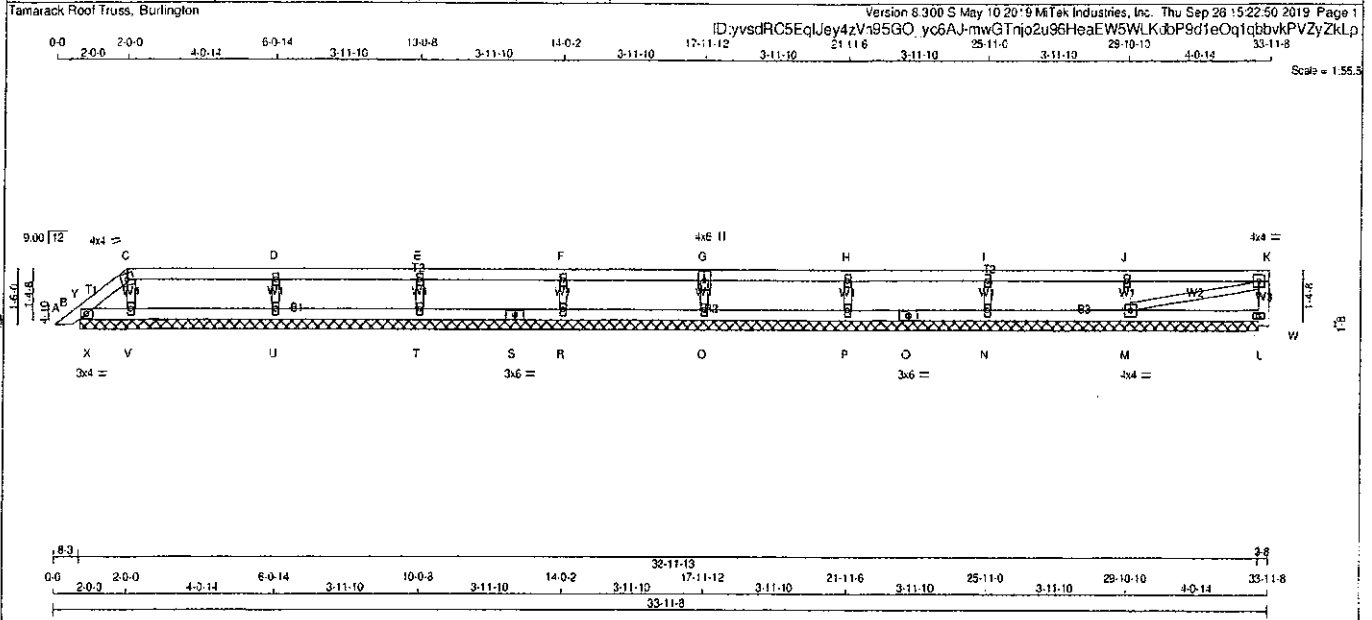
SSI GRIP = 0.41 (B) (INPUT = 0.90)
SSI METAL = 0.11 (B) (INPUT = 1.00)



Structural component only
DWG# T-1924929

[illegible]





LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - G	2x4	DRY No.2	SPF
G - K	2x4	DRY No.2	SPF
W - K	2x4	DRY No.2	SPF
B - S	2x4	DRY No.2	SPF
S - O	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TTW-m	MT20	4.0	4.0		
D, E, F, H, I, J					
D TMW-w	MT20	2.0	4.0		
G TSW-w	MT20	4.0	6.0		
K TMW-v	MT20	4.0	4.0		
L BVM1-I	MT20	2.0	4.0		
M BMW1-I	MT20	4.0	4.0		
N, P, Q, R, T, U, V					
N BMW1-w	MT20	2.0	4.0		
O BS-I	MT20	3.0	6.0		
S BS-I	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP
B	129	0	129	0
V	256	0	256	0
U	497	0	497	0
T	409	0	409	0
R	490	0	490	0
O	359	0	359	0
P	491	0	491	0
N	407	0	407	0
M	507	0	507	0
L	178	0	178	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	89	72 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
V	183	108 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
U	351	235 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
T	260	188 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0
R	348	232 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0
O	255	160 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
P	346	232 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
N	288	188 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
M	358	236 / 0	0 / 0	0 / 0	0 / 0	123 / 0	0 / 0
L	126	83 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, V, U, T, R, Q, P, N, M, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 15	-91.8	-91.8	0.02 (1)	10.00	V-C	-178 / 0	0.03 (1)	
B-Y	-16 / 4	-91.8	-91.8	0.01 (4)	6.25	U-D	-423 / 0	0.06 (1)	
Y-C	-26 / 0	-91.8	-91.8	0.01 (1)	6.25	T-E	-336 / 0	0.05 (1)	
C-D	-1 / 0	-91.8	-91.8	0.22 (1)	10.00	R-F	-416 / 0	0.06 (1)	
D-E	-2 / 0	-91.8	-91.8	0.22 (1)	10.00	Q-G	-288 / 0	0.04 (1)	
E-F	-2 / 0	-91.8	-91.8	0.21 (1)	10.00	P-H	-416 / 0	0.06 (1)	
F-G	-2 / 0	-91.8	-91.8	0.21 (1)	10.00	N-I	-336 / 0	0.05 (1)	
G-H	-2 / 0	-91.8	-91.8	0.21 (1)	10.00	M-J	-423 / 0	0.06 (1)	
H-I	-2 / 0	-91.8	-91.8	0.21 (1)	10.00	M-K	0 / 2	0.00 (1)	
I-J	-2 / 0	-91.8	-91.8	0.22 (1)	10.00	X-Y	-75 / 0	0.00 (1)	
J-K	-2 / 0	-91.8	-91.8	0.22 (1)	10.00				
W-L	0 / 0	0.0	0.0	0.00 (1)	10.00				
L-K	-148 / 0	0.0	0.0	0.02 (1)	7.81				
B-X	0 / 19	-18.5	-18.5	0.02 (1)	10.00				
X-V	0 / 19	-18.5	-18.5	0.05 (4)	10.00				
V-U	0 / 2	-18.5	-18.5	0.06 (4)	10.00				
U-T	0 / 2	-18.5	-18.5	0.06 (4)	10.00				
T-S	0 / 2	-18.5	-18.5	0.06 (4)	10.00				
S-R	0 / 2	-18.5	-18.5	0.06 (4)	10.00				
R-O	0 / 2	-18.5	-18.5	0.06 (4)	10.00				
O-P	0 / 2	-18.5	-18.5	0.06 (4)	10.00				
P-O	0 / 2	-18.5	-18.5	0.06 (4)	10.00				
O-N	0 / 2	-18.5	-18.5	0.06 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, DBC 2012
- CSA 089-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.221/00 (D-E-I), BC=0.071/00 (M-N-I), WB=0.061/00 (D-U-I), SSI=0.181/00 (C-D-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRP (DRY) SHEAR SECTION (PSI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1587 1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.64 (M) (INPUT = 0.90)

JSI METAL = 0.13 (L) (INPUT = 1.00)

Structural component only

DWG# T-1924932 CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREENYORK HOMES	DWG NO.
405684	P6	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 MTx Industries, Inc. Thu Sep 26 15:22:50 2019 Page 2
ID:yvsdRC5EqJey4zVn95GO_yc6AJ-mwGTrjo2u96HeaEW5WlKdcP9d1eQq1cbbvkPVZyZkLo

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	FR-TO
N-M	0.2	-18.5	-18.5	0.07 (4)	10.00
M-L	0.0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
-------	---------------------------	--------------

09/26/19

H. J/G ALVES

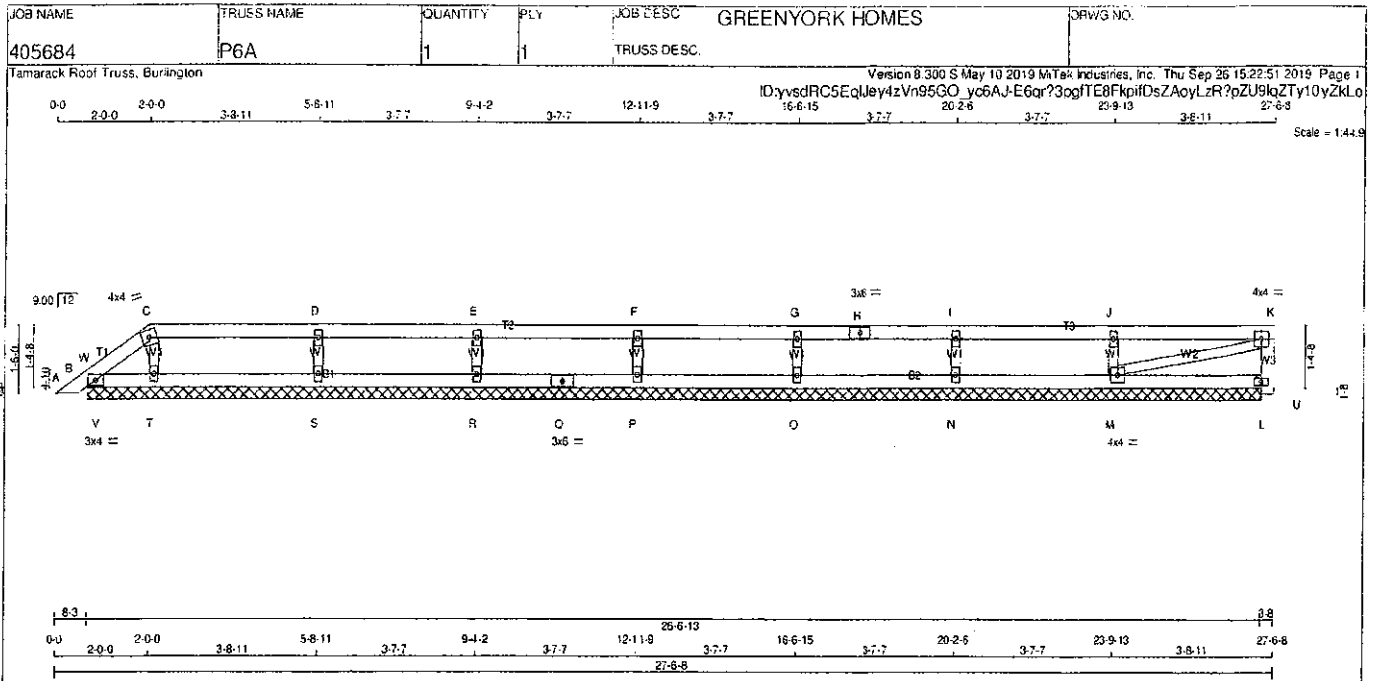
100009024

LICENSED PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

Structural component only

DWG# T-1924932



TOTAL WEIGHT = 71 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
U - K	2x4	DRY No.2	SPF
B - Q	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB-I	MT20	3.0	4.0		
C TTW-m	MT20	4.0	4.0		
D, E, F, G, I, J					
D TMW-w	MT20	2.0	4.0		
H TS-I	MT20	3.0	6.0		
K TMW-I	MT20	4.0	4.0		
L BVM-I	MT20	2.0	4.0		
M BMW-I	MT20	4.0	4.0		
N, O, P, R, S, T					
N BMW-I	MT20	2.0	4.0		
O BS-I	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	131	0	131	0	26-6-13 (15-1208-13)	
T	239	0	239	0	26-6-13 (15-1208-13)	
S	451	0	451	0	26-6-13 (15-1208-13)	
R	385	0	385	0	26-6-13 (15-1208-13)	
P	402	0	402	0	26-6-13 (15-1208-13)	
O	402	0	402	0	26-6-13 (15-1208-13)	
N	383	0	383	0	26-6-13 (15-1208-13)	
M	450	0	450	0	26-6-13 (15-1208-13)	
L	163	0	163	0	26-6-13 (15-1208-13)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	90	72.0	0.0	0.0	0.0	19.0	0.0
T	171	102.0	0.0	0.0	0.0	69.0	0.0
S	318	213.0	0.0	0.0	0.0	105.0	0.0
R	273	178.0	0.0	0.0	0.0	95.0	0.0
P	284	187.0	0.0	0.0	0.0	97.0	0.0
O	285	187.0	0.0	0.0	0.0	98.0	0.0
N	270	178.0	0.0	0.0	0.0	93.0	0.0
M	325	213.0	0.0	0.0	0.0	112.0	0.0
L	115	76.0	0.0	0.0	0.0	40.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, T, S, R, P, O, N, M, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LBS)
FR-TO					
A-B	0.15	-11.8	-91.8 0.02 (1)	10.00	T-C -167.0 0.02 (1)
B-W	-17.1	-91.8	-91.8 0.00 (4)	6.25	S-D -383.0 0.05 (1)
W-C	-25.0	-91.8	-91.8 0.02 (1)	6.25	R-E -318.0 0.04 (1)
C-D	-2.0	-91.8	-91.8 0.18 (1)	10.00	P-F -335.0 0.05 (1)
D-E	-2.0	-91.8	-91.8 0.18 (1)	10.00	O-G -335.0 0.05 (1)
E-F	-2.0	-91.8	-91.8 0.14 (1)	10.00	N-I -318.0 0.04 (1)
F-G	-2.0	-91.8	-91.8 0.14 (1)	10.00	M-J -383.0 0.05 (1)
G-H	-2.0	-91.8	-91.8 0.14 (1)	10.00	M-K 0.2 0.00 (1)
H-I	-2.0	-91.8	-91.8 0.14 (1)	10.00	V-W -71.0 0.00 (1)
I-J	-2.0	-91.8	-91.8 0.18 (1)	10.00	
J-K	-2.0	-91.8	-91.8 0.18 (1)	10.00	
U-L	0.0	0.0	0.0 0.00 (1)	10.00	
L-K	-136.0	0.0	0.0 0.01 (1)	7.81	
S-V	3.18	-18.5	-18.5 0.02 (1)	10.00	
V-T	0.18	-18.5	-18.5 0.04 (4)	10.00	
T-S	0.2	-18.5	-18.5 0.05 (4)	10.00	
S-R	0.2	-18.5	-18.5 0.05 (4)	10.00	
R-Q	0.2	-18.5	-18.5 0.05 (4)	10.00	
Q-P	0.2	-18.5	-18.5 0.05 (4)	10.00	
P-O	0.2	-18.5	-18.5 0.05 (4)	10.00	
O-N	0.2	-18.5	-18.5 0.05 (4)	10.00	
N-M	0.2	-18.5	-18.5 0.06 (4)	10.00	
M-L	0.0	-18.5	-18.5 0.08 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TO=0.18/1.00 (D-E-I), BC=0.06/1.00 (L-M-A),

WB=0.05/1.00 (D-S-I), SS=0.15/1.00 (C-D-I)

COL LUMBER=1.00 NAIL=1.00 L5 BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1556

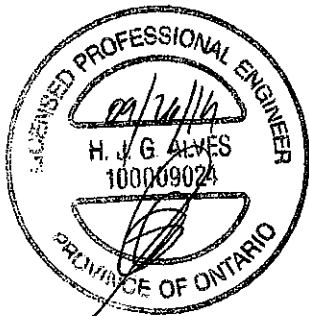
PLATE PLACEMENT TOL. = 0.250 inches

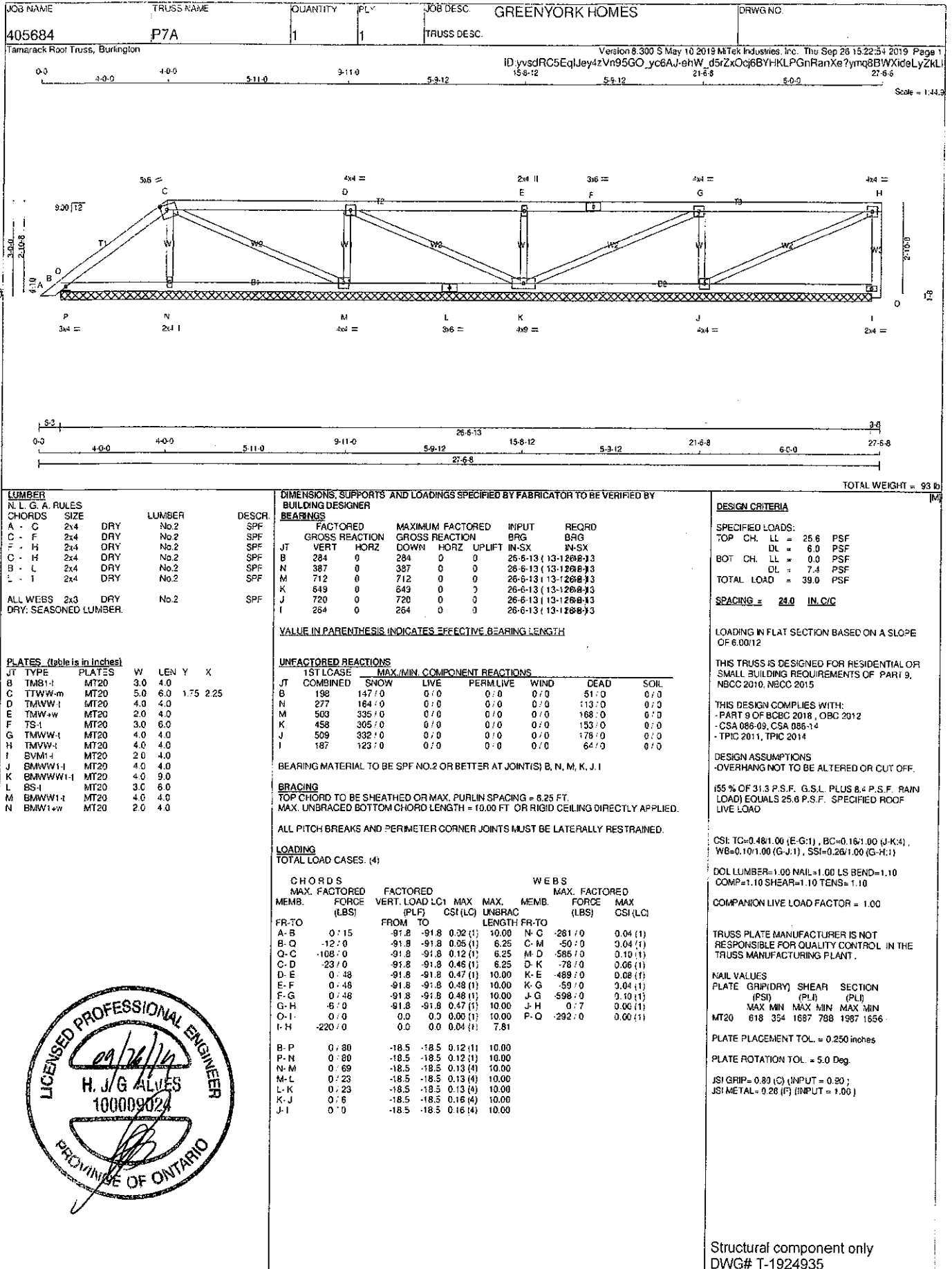
PLATE ROTATION TOL. = 5.0 Deg.

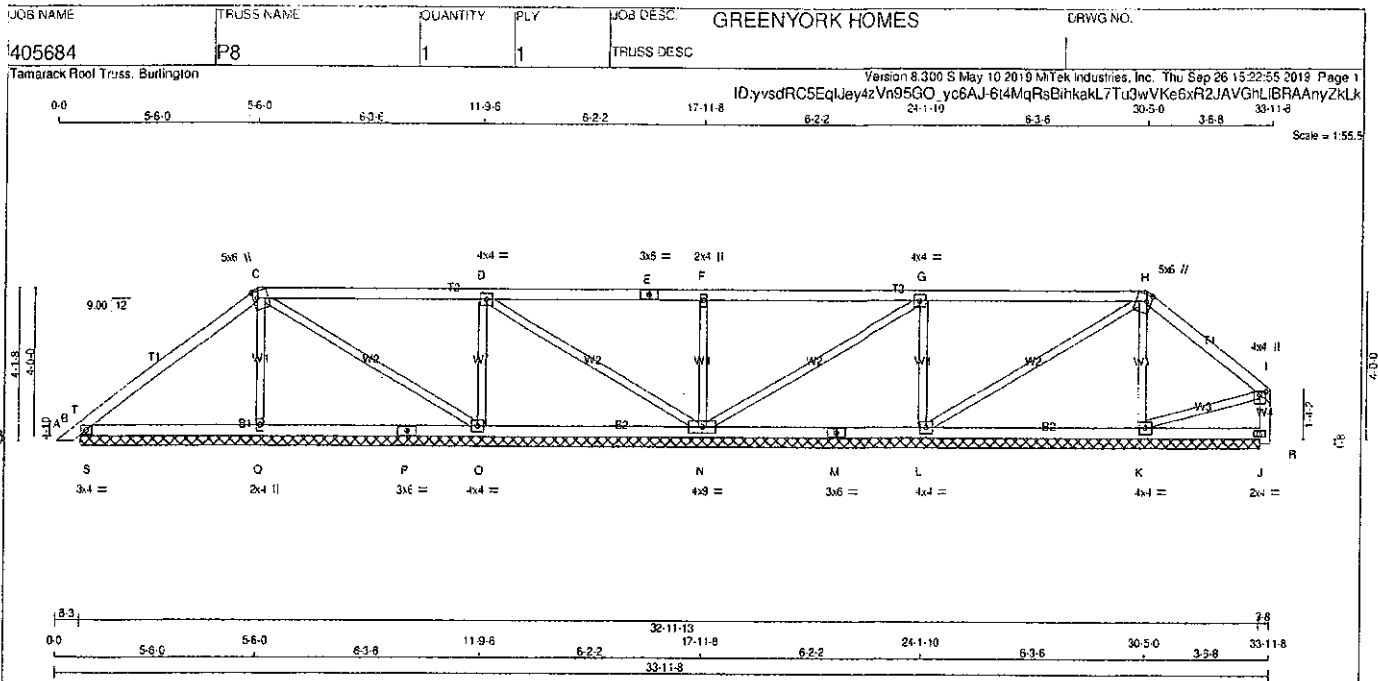
JSI GRIP = 0.65 (N) (INPUT = 0.99)

JSI METAL = 0.12 (L) (INPUT = 1.00)

Structural component only
DWG# T-1924933







TOTAL WEIGHT = 120 LB

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
R - I	2x4	DRY	No.2
B - P	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-1	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTWW-1	MT20	4.0	4.0	
E	TS-1	MT20	3.0	6.0	
F	TMW+w	MT20	2.0	4.0	
G	TMWW-1	MT20	4.0	4.0	
H	TTWW+m	MT20	5.0	6.0	2.25 1.50
I	TMW+w	MT20	4.0	4.0	1.00 2.00
J	BVM1-1	MT20	2.0	4.0	
K, L, O					
K	BMWW1-1	MT20	4.0	4.0	
M	BS-1	MT20	3.0	6.0	
N	BMWW1-1	MT20	4.0	9.0	
P	BS-1	MT20	3.0	6.0	
Q	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					
BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	404	0	404	0	0
Q	429	0	429	0	0
O	738	0	738	0	0
N	762	0	762	0	0
L	742	0	742	0	0
K	391	0	391	0	0
J	258	0	258	0	0

UNFACTORED REACTIONS						
1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
B	282	203 / 0	0 / 0	0 / 0	0 / 0	79 / 0
Q	307	190 / 0	0 / 0	0 / 0	0 / 0	127 / 0
O	521	346 / 0	0 / 0	0 / 0	0 / 0	175 / 0
N	537	382 / 0	0 / 0	0 / 0	0 / 0	176 / 0
L	524	348 / 0	0 / 0	0 / 0	0 / 0	178 / 0
K	281	162 / 0	0 / 0	0 / 0	0 / 0	119 / 0
J	180	132 / 0	0 / 0	0 / 0	0 / 0	48 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, Q, O, N, L, K, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)
FR-TO		FROM TO	
A-B	0 / 15	-91.8 -91.8 0.02 (1)	10.00
B-T	-29 / 61	-91.8 -91.8 0.14 (1)	6.25
T-C	-178 / 0	-91.8 -91.8 0.26 (1)	6.25
C-D	-44 / 0	-91.8 -91.8 0.52 (1)	6.25
D-E	0 / 70	-91.8 -91.8 0.53 (1)	10.00
E-F	0 / 70	-91.8 -91.8 0.53 (1)	10.00
F-G	0 / 70	-91.8 -91.8 0.53 (1)	10.00
G-H	-21 / 0	-91.8 -91.8 0.52 (1)	6.25
H-I	-91 / 0	-91.8 -91.8 0.20 (1)	6.25
I-J	0 / 0	0.0 0.0 0.00 (1)	10.00
J-I	-238 / 0	0.0 0.0 0.02 (1)	7.81
B-S	0 / 132	-18.5 -18.5 0.23 (1)	10.00
S-Q	0 / 132	-18.5 -18.5 0.23 (1)	10.00
Q-P	0 / 124	-18.5 -18.5 0.15 (4)	10.00
P-O	0 / 124	-18.5 -18.5 0.15 (4)	10.00
O-N	0 / 45	-18.5 -18.5 0.14 (4)	10.00
N-M	0 / 21	-18.5 -18.5 0.15 (4)	10.00
M-L	0 / 21	-18.5 -18.5 0.15 (4)	10.00
L-K	0 / 84	-18.5 -18.5 0.15 (4)	10.00
K-J	0 / 0	-18.5 -18.5 0.11 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.53/1.00 (F-G:1), BC=0.23/1.00 (B-S:1), WB=0.15/1.00 (G-L:1), SS=0.44/1.00 (B-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

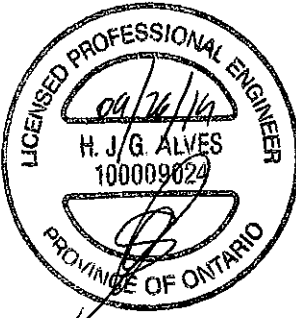
PLATE	GRIP(DRY)	SHEAR	SECTION (PSI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	615	354	1687	788	1987

PLATE PLACEMENT TOL = 0.250 inches

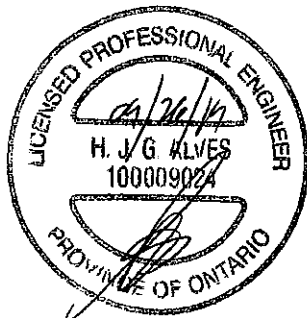
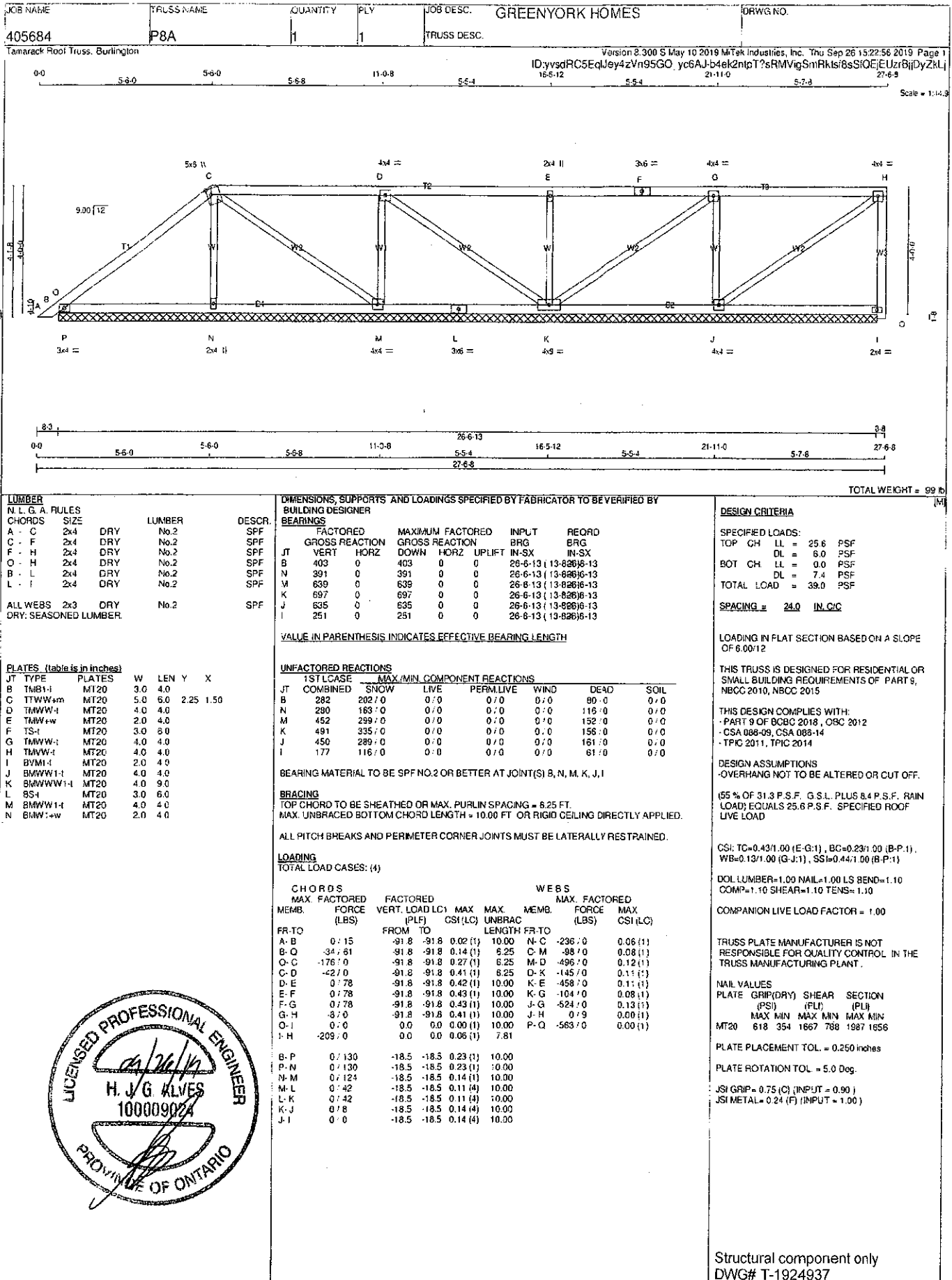
PLATE ROTATION TOL = 5.0 Deg.

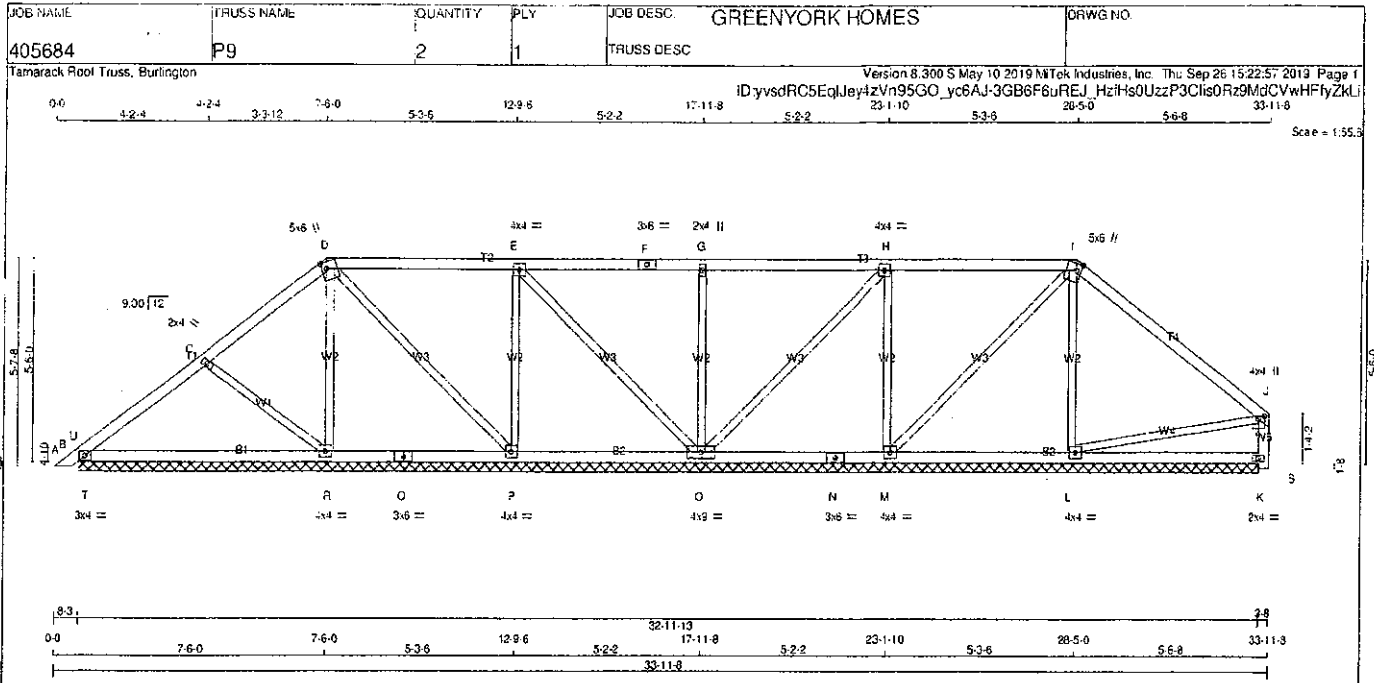
JSI GRIP = 0.87 (C) (INPUT = 0.90)
JSI METAL = 0.21 (J) (INPUT = 1.00)

Structural component only
DWG# T-1924936



Structural component only
DWG# T-1924936





LUMBER

N.L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - I	2x4	DRY No.2
I - J	2x4	DRY No.2
S - J	2x4	DRY No.2
B - Q	2x4	DRY No.2
Q - N	2x4	DRY No.2
N - K	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB14	MT20	3.0	4.0		
C TMW+w	MT20	2.0	4.0		
D TTWW+m	MT20	5.0	6.0	2.25	1.50
E TMWW+1	MT20	4.0	4.0		
F TS-1	MT20	3.0	6.0		
G TMW+w	MT20	2.0	4.0		
H TMW-1	MT20	4.0	4.0		
I TTWW+m	MT20	5.0	6.0	2.25	1.50
J TMW+p	MT20	4.0	4.0	1.00	2.00
K BVM-1	MT20	2.0	4.0		
L M, P, R					
N BMWW14	MT20	4.0	4.0		
O BS-1	MT20	3.0	6.0		
P BMWW14	MT20	4.0	9.0		
Q BS-1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SK	REQD BRG IN-SK
	VERT	HORZ	DOWN	HORZ		
B	428	0	428	0	32-11-13 (11-32-12-13)	
R	597	0	597	0	32-11-13 (11-32-12-13)	
P	557	0	557	0	32-11-13 (11-32-12-13)	
O	775	0	775	0	32-11-13 (11-32-12-13)	
M	590	0	590	0	32-11-13 (11-32-12-13)	
L	407	0	407	0	32-11-13 (11-32-12-13)	
K	369	0	369	0	32-11-13 (11-32-12-13)	

UNFACTORED REACTIONS

1ST CASE MAX/MIN COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	301	207/0	0/0	0/0	0/0	94/0	0/0
R	425	264/0	0/0	0/0	0/0	161/0	0/0
P	393	282/0	0/0	0/0	0/0	131/0	0/0
O	545	378/0	0/0	0/0	0/0	168/0	0/0
M	416	279/0	0/0	0/0	0/0	137/0	0/0
L	293	182/0	0/0	0/0	0/0	130/0	0/0
K	259	182/0	0/0	0/0	0/0	77/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, R, P, O, M, L, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC) UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
A-B	0/15	-91.8	-91.8 0.02 (1)	10.00	C-R	-328/0	0.11 (1)
B-U	-487/0	-91.8	-91.8 0.11 (4)	6.25	R-D	-273/0	0.12 (1)
U-C	-303/0	-91.8	-91.8 0.18 (1)	6.25	D-P	-44/0	0.05 (1)
C-D	-31/0	-91.8	-91.8 0.18 (1)	6.25	P-E	-442/0	0.20 (1)
D-E	0/31	-91.8	-91.8 0.37 (1)	10.00	E-O	-153/0	0.15 (1)
E-F	0/138	-91.8	-91.8 0.39 (1)	10.00	O-G	-437/0	0.20 (1)
F-G	0/138	-91.8	-91.8 0.39 (1)	10.00	G-H	-183/0	0.19 (1)
G-H	0/138	-91.8	-91.8 0.39 (1)	10.00	H-M	-420/0	0.19 (1)
H-I	0/9	-91.8	-91.8 0.37 (1)	10.00	M-L	-115/0	0.12 (1)
I-J	-98/0	-91.8	-91.8 0.48 (1)	6.25	L-I	-306/0	0.14 (1)
J-K	0/0	0.0	0.0 0.00 (1)	10.00	L-J	0/80	0.02 (1)
K-J	-328/0	0.0	0.0 0.03 (1)	7.81	J-U	0/210	0.00 (1)
B-T	0/269	-18.5	-18.5 0.10 (4)	10.00			
T-R	0/269	-18.5	-18.5 0.18 (4)	10.00			
R-O	0/4	-18.5	-18.5 0.16 (4)	10.00			
O-P	0/4	-18.5	-18.5 0.16 (4)	10.00			
P-Q	-31/0	-18.5	-18.5 0.10 (4)	6.25			
Q-N	-9/0	-18.5	-18.5 0.10 (4)	10.00			
N-M	-9/0	-18.5	-18.5 0.10 (4)	10.00			
M-L	0/73	-18.5	-18.5 0.14 (4)	10.00			
L-K	0/0	-18.5	-18.5 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF
 DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (I-J:1), BC=0.18/1.00 (R-T:4),
 WB=0.20/1.00 (E-P:1), SSI=0.23/1.00 (I-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

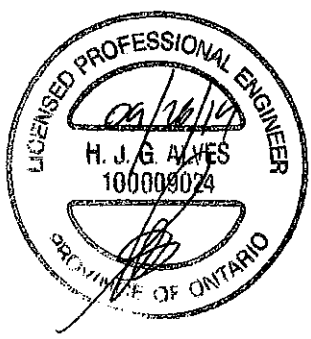
MT20 618 354 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

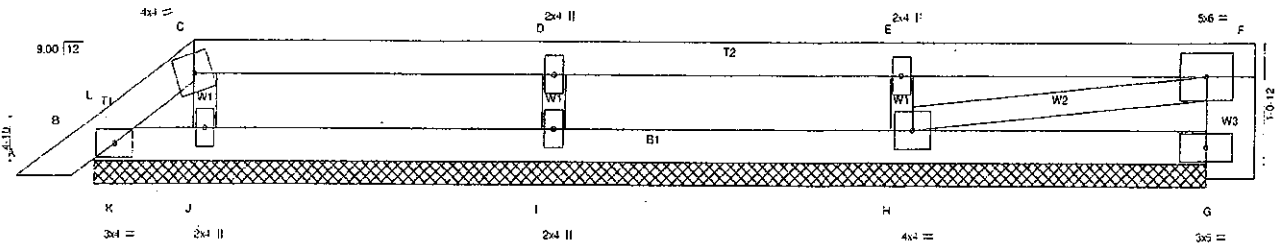
JSI GRIP = 0.64 (L) (INPUT = 0.90)
 JSI METAL = 0.29 (K) (INPUT = 1.00)

Structural component only
 DWG# T-1924938



JOB NAME 405684	TRUSS NAME P10	QUANTITY 1	PLY 3	JOB DESC. GREENYORK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 6 300.5 May 10 2019 M/Tek Industries, Inc. Thu Sep 26 15:22:59 2019 Page 1 ID:yvsdRC5EqUey4zVn95GO_yc6AJ-?fJsgovimwF?DyQE7v?RUUHI4fkQR6hwgpPOKYyZkLg		

0.0 1.7-0 1.7-0 3.3-4 4.10-4 3.2-0 3.3-4 11-3-8 Scale = 1:16.1



0.0 1.7-0 1.7-0 3.3-4 4.10-4 10.1-13 3.0-12 7.1-10 3.4-5 11-3-8
11-3-6

TOTAL WEIGHT = 3 X 30 = 90 lb

LUMBER			
N.L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
G - F	2x6	DRY	No.2
B - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

DESIGN CONSISTS OF 1 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-F 1	12	TOP
F-G 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B-G 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMW-w	MT20	2.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TMW-I	MT20	5.0	6.0		
G	BVM-I	MT20	3.0	6.0		
H	BMW1-I	MT20	4.0	4.0		
I	BMW1-w	MT20	2.0	4.0		
J	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	110	0	110	0	0	10-1-13	10-1-13	10-1-13	10-1-13
G	145	0	145	0	0	10-1-13	10-1-13	10-1-13	10-1-13
J	196	0	196	0	0	10-1-13	10-1-13	10-1-13	10-1-13
H	392	0	392	0	0	10-1-13	10-1-13	10-1-13	10-1-13
I	381	0	381	0	0	10-1-13	10-1-13	10-1-13	10-1-13

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	75	63.0	0.0	0.0	0.0	12.0	0.0
G	103	68.0	0.0	0.0	0.0	35.0	0.0
J	141	81.0	0.0	0.0	0.0	60.0	0.0
H	277	181.0	0.0	0.0	0.0	96.0	0.0
I	269	181.0	0.0	0.0	0.0	88.0	0.0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) B, G, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED	CS1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0.15	-91.8	-91.8 0.01 (1)	J-C	-133.0	0.01 (1)	
B-L	-25.0	-91.8	-91.8 0.00 (4)	H-E	-324.0	0.02 (1)	
L-C	-27.0	-91.8	-91.8 0.00 (1)	H-F	0.2	0.00 (1)	
C-D	-2.0	-91.8	-91.8 0.04 (1)	I-D	-324.0	0.02 (1)	
D-E	-2.0	-91.8	-91.8 0.04 (1)	K-L	-45.0	0.00 (1)	
E-F	-2.0	-91.8	-91.8 0.04 (1)				
G-F	-121.0	0.0	0.0 0.00 (1)				
B-K	0.20	-18.5	-18.5 0.00 (1)				
K-J	3.20	-18.5	-18.5 0.01 (4)				
J-I	0.2	-18.5	-18.5 0.01 (4)				
I-H	0.2	-18.5	-18.5 0.02 (4)				
H-G	0.0	-18.5	-18.5 0.02 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.30/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.04/1.00 (C-D:1), BC=0.02/1.00 (G-H:4), WB=0.02/1.00 (D-E:1), SS1=0.05/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

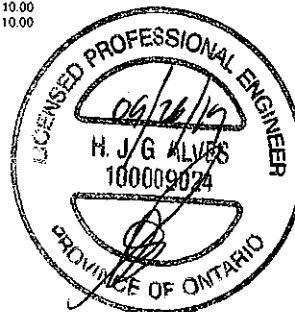
NAIL VALUES

PLATE	GPI(DRY)	SHEAR (PS)	SECTION (PLI)
MT20	618	354	1667 788 1987 1556

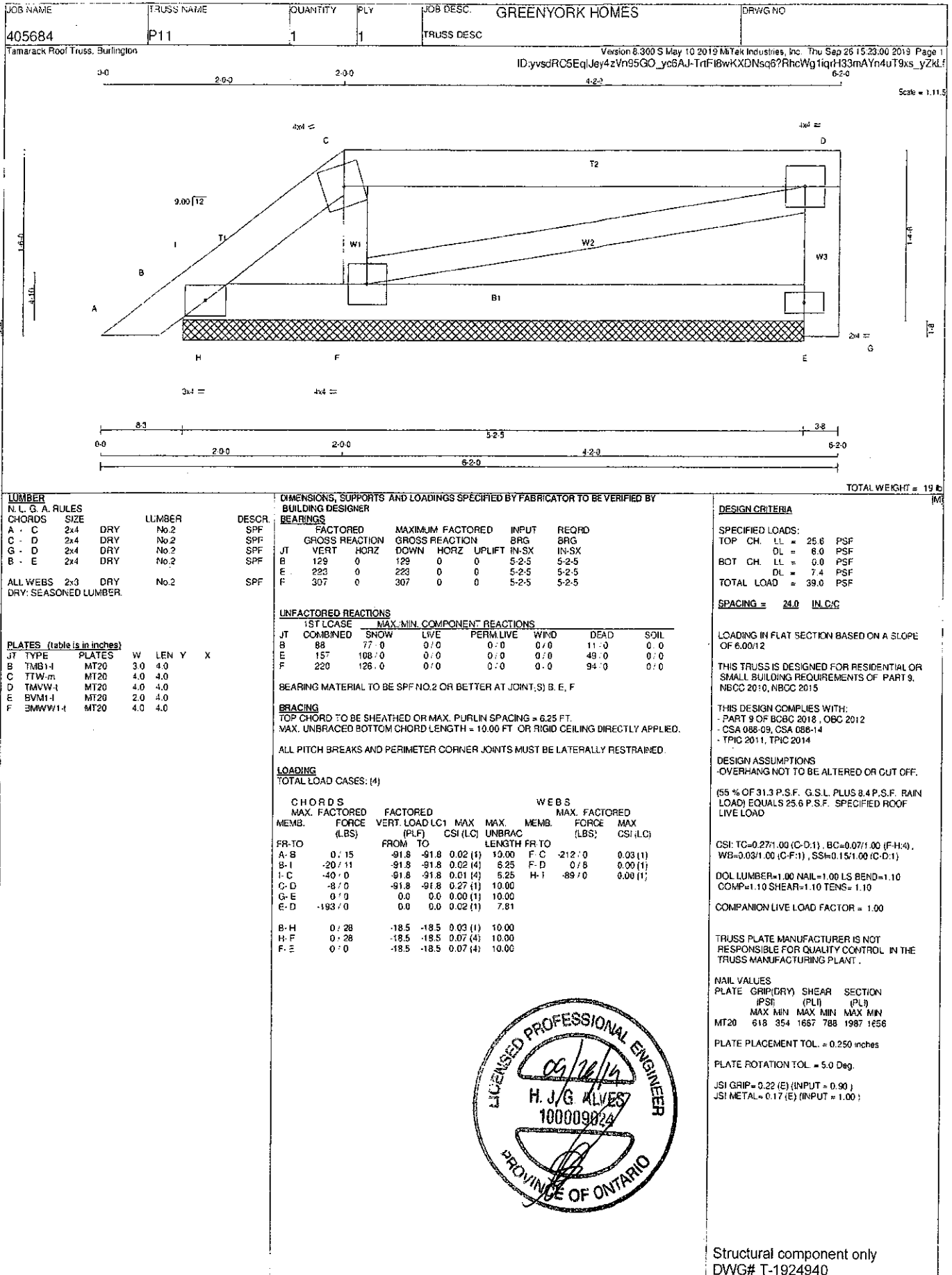
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

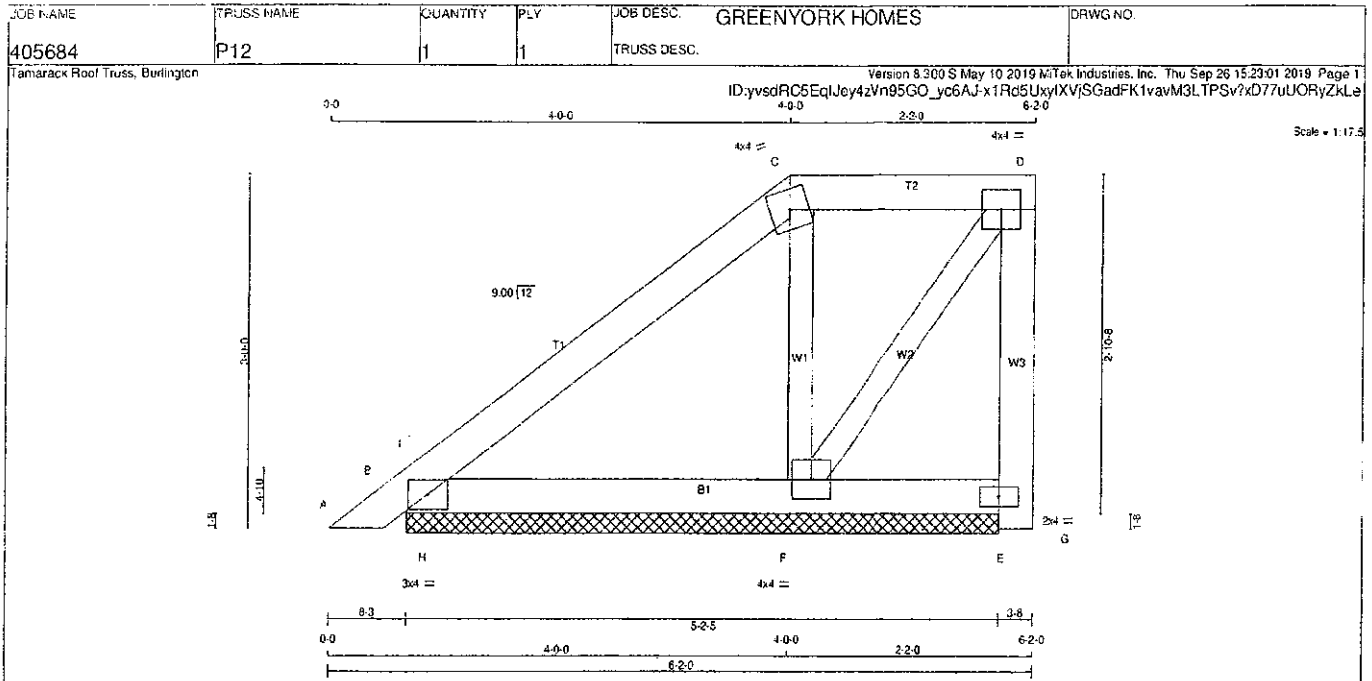
JSI GRIP = 0.06 (H) (INPUT = 0.90)
JSI METAL = 0.02 (G) (INPUT = 1.00)



Structural component only
DWG# T-1924939



100009824



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - D	2x4	DRY No.2
G - D	2x4	DRY No.2
B - E	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-1	MT20	3.0	4.0		
C ITW-m	MT20	4.0	4.0		
D TMVW-1	MT20	4.0	4.0		
E BVM1-1	MT20	2.0	4.0		
F BMVW1-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
B	253	0	253	0	5-2-5	5-2-5
E	131	0	131	0	5-2-5	5-2-5
F	275	0	275	0	5-2-5	5-2-5

UNFACTORED REACTIONS

1ST CASE MAX. MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	178	125 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
E	92	65 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
F	196	121 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS (B), E, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 15	-91.8 -91.8	0.02 (1)	F-C	-202 / 0	0.03 (1)	
B-I	-17 / 39	-91.8 -91.8	0.05 (1)	F-D	0 / 44	0.01 (1)	
I-C	-51 / 0	-91.8 -91.8	0.12 (1)	H-I	-283 / 0	0.00 (1)	
C-D	-27 / 0	-91.8 -91.8	0.07 (1)				
G-E	0 / 0	0.0 0.0	0.00 (1)				
E-D	-133 / 0	0.0 0.0	0.02 (1)				
			7.81				
B-H	0 / 35	-18.5 -18.5	0.11 (1)				
H-F	0 / 35	-18.5 -18.5	0.11 (1)				
F-E	0 / 0	-18.5 -18.5	0.06 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 25.6 PSF
DL = 5.0 PSF
BOT CH. LL = 3.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (C-I), BC=0.11/1.00 (F-H), WB=0.03/1.00 (C-F), SSI=0.22/1.00 (B-H)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

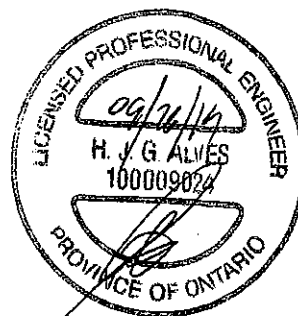
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FS) (FL) (FL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.12 (E) (INPUT = 1.00)



Structural component only
DWG# T-1924941

JOB NAME 405684	TRUSS NAME P13	QUANTITY 2	PLY 1	JOB DESC. GREENYORK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington
Version 8.300 S May 10 2019 MiTek Industries, Inc. Thu Sep 26 15:23:02 2019 Page 1

ID:yvsdRC5EqUey4zVn95GO_yc8AJ-PE??lqya3rda4Q9po1Y867vEQikheSyNMne2wtyZkLd

Scale = 1:23.5

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No 2
F - D	2x4	DRY	No 2
B - E	2x4	DRY	No 2

ALL WEBS 2x3 DRY No 2

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB11	MT20	3.0	4.0		
C	TVMW	MT20	2.0	4.0		
D	TVMW	MT20	3.0	4.0		
E	BVMW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
B	334	0	334	0	0	4-9-5	4-9-5	
E	279	0	279	0	0	4-9-5	4-9-5	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
B	235	160 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0	
E	198	130 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	WEBS		FACTORED		MAX. CS1 (LC)
	MAX	FACTORED	VERT. LOAD	(PLF)				FORCE (LBS)	MAX	FORCE (LBS)	MAX	
FR-TO							FR-TO					
A-B	0 / 15		-91.8	-91.8	0.02 (1)	10.00	C-E	-251 / 0		0.05 (1)		
B-H	-373 / 0		-91.8	-91.8	0.09 (4)	6.25	G-H	0 / 175		0.00 (1)		
H-C	-224 / 0		-91.8	-91.8	0.10 (1)	6.25						
C-D	-17 / 0		-91.8	-91.8	0.10 (1)	6.25						
F-E	0 / 0		0.0	0.0	0.00 (1)	10.00						
E-D	-92 / 0		0.0	0.0	0.03 (1)	7.81						
B-G	0 / 204		-18.5	-18.5	0.08 (4)	10.00						
G-E	0 / 204		-18.5	-18.5	0.11 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 25.6 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 39.0 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 31.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.6 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.10/1.00 (C-H:1), BC=0.11/1.00 (E-G:4), WB=0.05/1.00 (C-E:1), SSI=0.14/1.00 (B-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

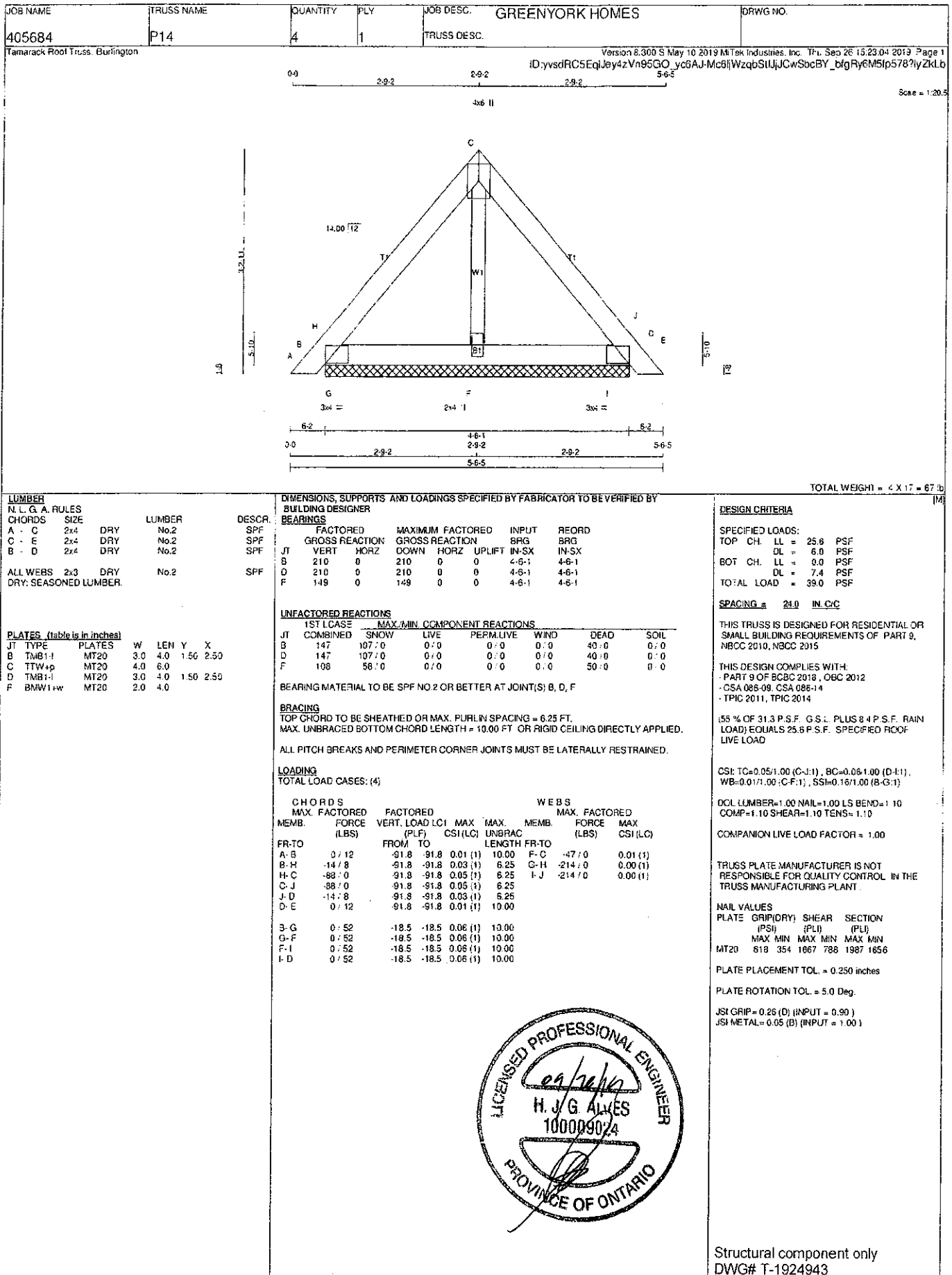
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)

JSI METAL = 0.28 (B) (INPUT = 1.00)

Structural component only

DWG# T-1924942





Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800213

Feb 09, 2018

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

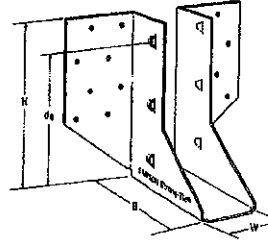
- Factored resistances are in accordance with CSA Q86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

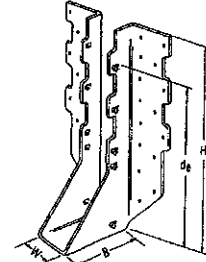
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

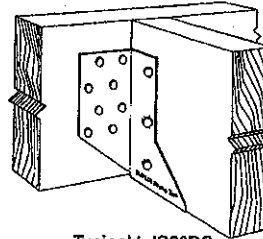
- See current catalogue for options



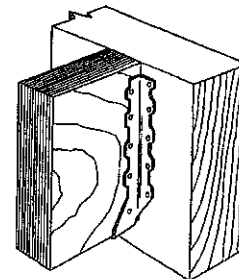
LJS26DS



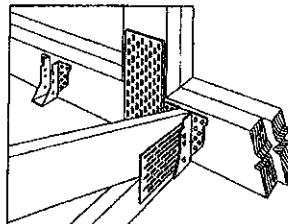
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



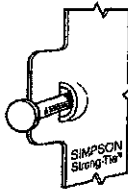
Typical HUS
Installation



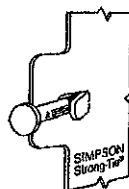
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LJS26DS	18	1 1/4	5	3 1/2	4 1/4	(16) 16d	(6) 16d		2055	4265	1460	4115
HUS26	16	1 1/4	5 1/2	3	3 1/4	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1 1/4	7 1/2	3	6 1/2	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1 1/4	9 1/2	3	7 1/2	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1 1/4	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

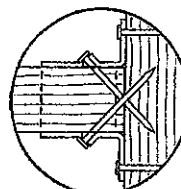
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



BYT
STATES
DESIGN

This technical bulletin is effective until June 30, 2017. For more information, visit www.simpsonstrongtie.com or call 1-800-999-5099.

This information is intended for informational purposes only. It is not a contract. The user is responsible for verifying the applicability of this information to their specific project.

Contact Simpson Strong-Tie for more information or to request a copy of this bulletin.

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TSPEC-HUS17-01/17 01p 0/10

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

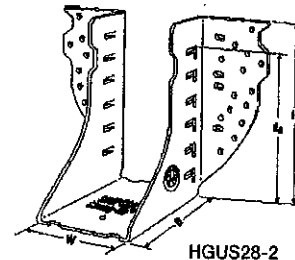
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

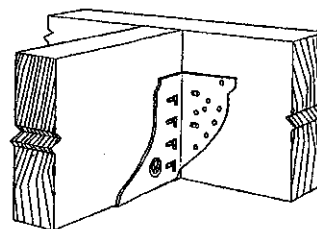
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

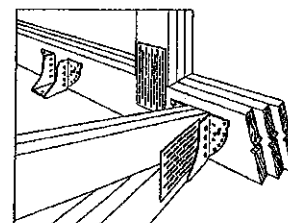
- See current catalogue for options



HGUS28-2



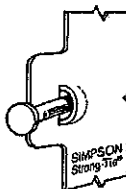
Typical HGUS
Installation



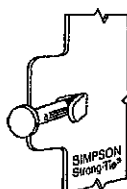
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 3/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

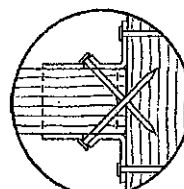
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



DESIGN
SIMPSON
STRONG-TIE

For technical information, contact Simpson Strong-Tie at 1-800-999-5099 or visit our website at strongtie.com.
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SPECH0317-2/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

H – Seismic and Hurricane Ties

SIMPSON
Strong-Tie

The H connector series provides wind and seismic ties for trusses and rafters.

MATERIAL: 18 gauge **FINISH:** G90 galvanized

DESIGN: • Factored resistances are in accordance with CSA O86-14

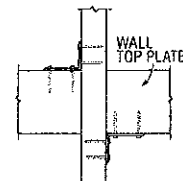
- Factored resistances have been increased 15%. No further increase is permitted.

INSTALLATION: • Use all specified fasteners

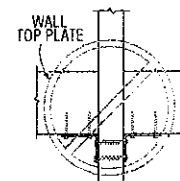
- Nails: 8d = 0.131" dia. x 2½" long common wire, 8d x 1½" = 0.131" x 1½" long, 10d x 1½" = 0.146" x 1½" long
- H1 can be installed with flanges facing outwards
- Hurricane ties do not replace solid blocking

Factored resistances for more than one direction for a single connection cannot be added together. A factored load which can be divided into components in the directions given must be evaluated as follows: Factored Shear/Resisting Shear + Factored Tension/Resisting Tension ≤ 1.0.

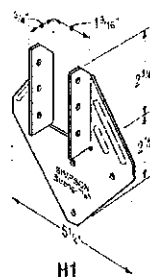
Hurricane Tie Installations to achieve Twice the Load (Top View)



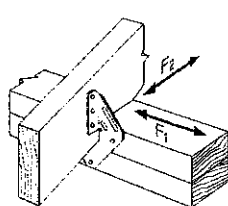
Install diagonally across from each other for minimum 2x truss.



Nailing into both sides of a single ply 2x truss may cause the wood to split.



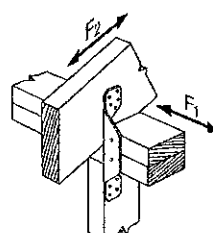
H1



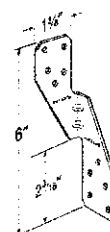
H1 Installation



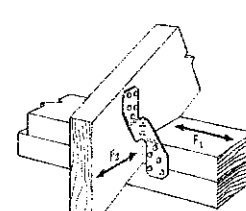
H2A



H2A Installation



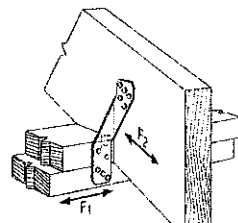
H2.5A



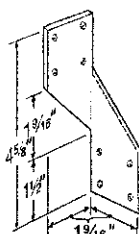
H2.5A Installation



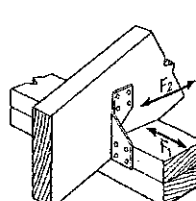
H2.5T



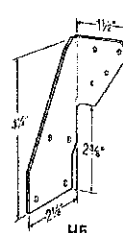
H2.5T Installation
(Nails into both top plates)



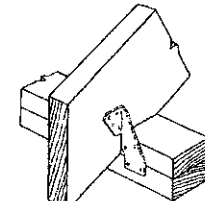
H3



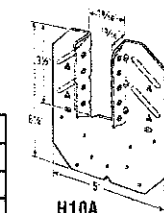
H3 Installation



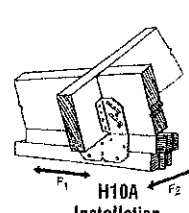
H5



H5 Installation
(Nails into both top plates)



H10A



H10A Installation

Model No.	Ga	Fasteners			Factored Resistance (lbs)					
					D.Fir-L			S-P-F		
		To Rafter	To Plates	To Studs	Uplift	Lateral		Uplift	Lateral	
						F ₁	F ₂		F ₁	F ₂
						(K _D = 1.15)			(K _D = 1.15)	
H1	18	6-8dx1½	4-8d	—	740	685	300	680	485	215
H2A	18	5-8dx1½	2-8dx1½	5-8dx1½	830	220	75	590	155	55
H2.5A	18	5-8d	5-8d	—	805	160	160	755	160	160
H2.5T	18	5-8d	5-8d	—	835	175	240	740	160	210
H3	18	4-8d	4-8d	—	740	180	265	615	125	190
H5	18	4-8d	4-8d	—	685	180	305	500	130	215
H10A	18	9-10dx1½	9-10dx1½	—	1735	795	410	1505	565	290

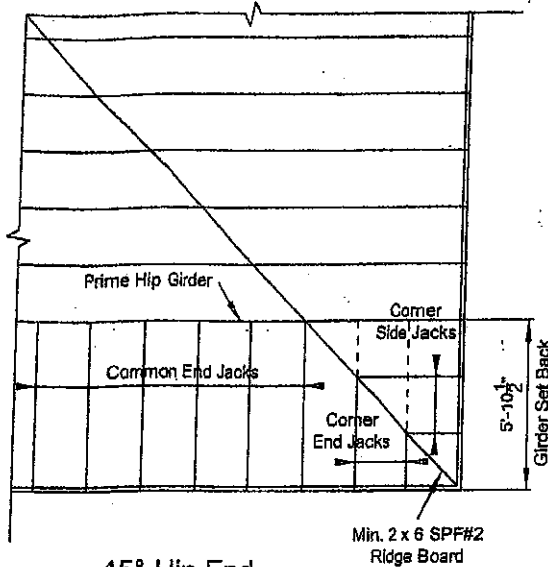
1. Factored resistances have been increased 15% for earthquake or wind loading with no further increase allowed.
2. Factored resistances are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. When cross-grain bending or cross-grain tension cannot

- be avoided, mechanical reinforcement to resist such forces should be considered.
4. Hurricane ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.



This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.
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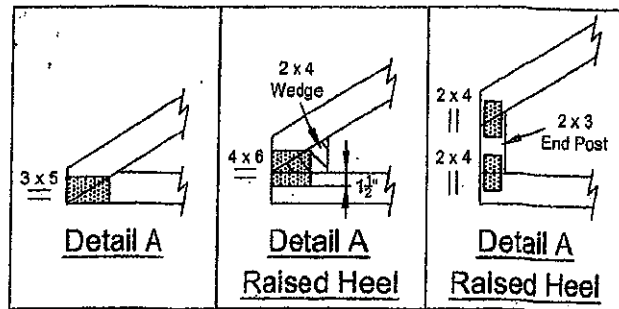
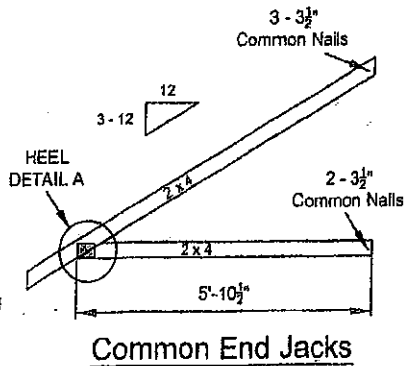
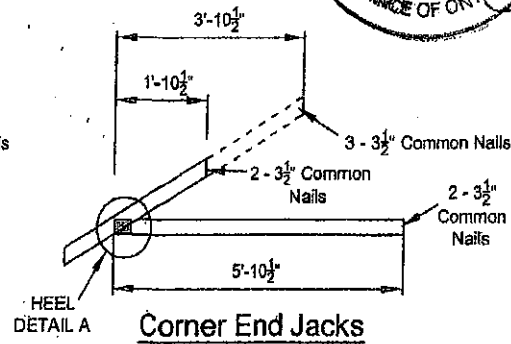
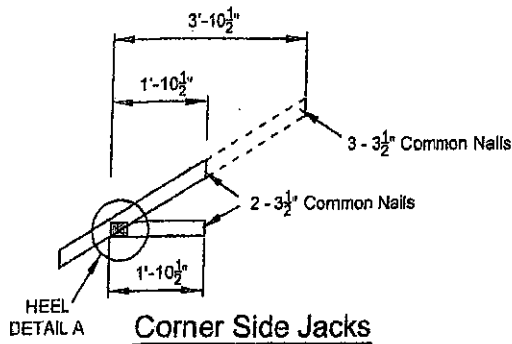
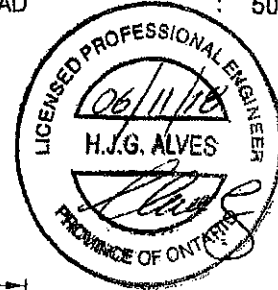
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

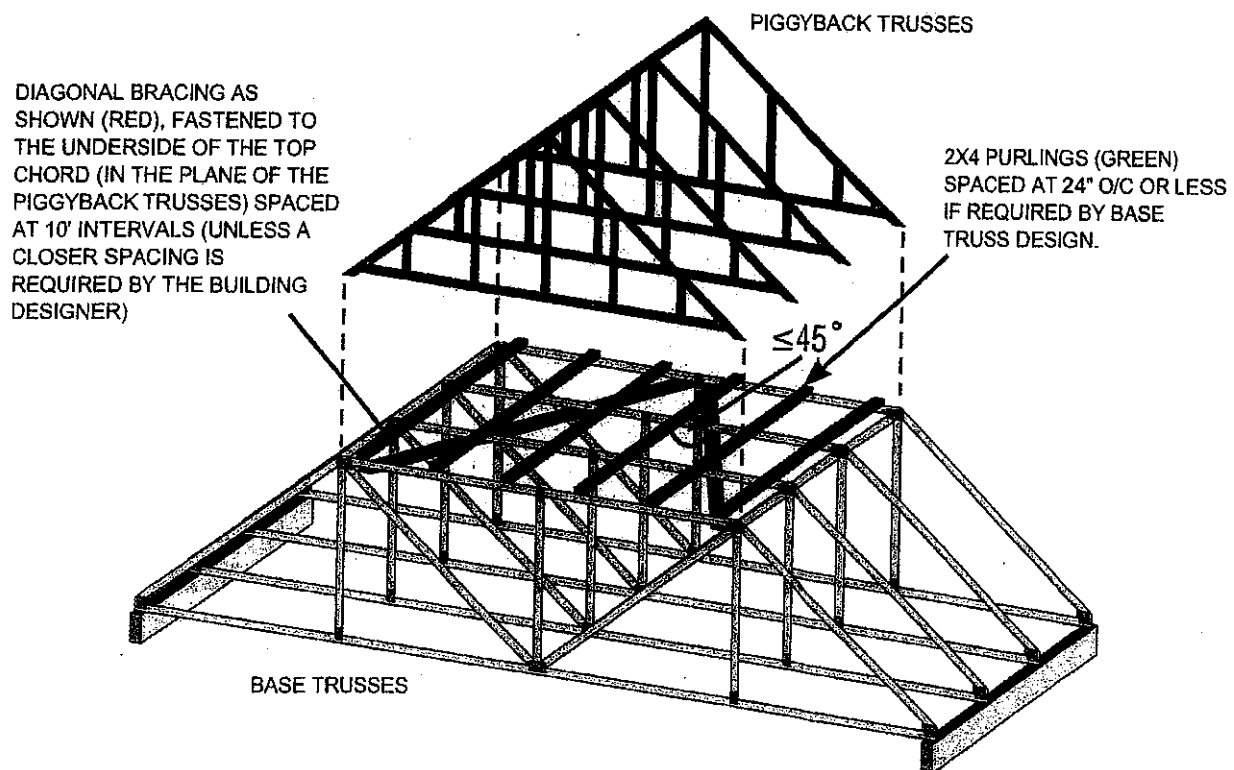
T-1800216

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

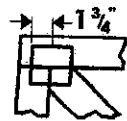
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

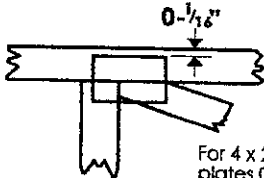
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



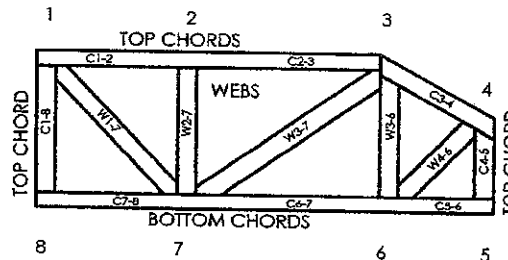
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
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MiTek Engineering Reference Sheet: Mil-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSL.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.