

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

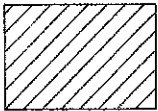
HUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

DENOTE:

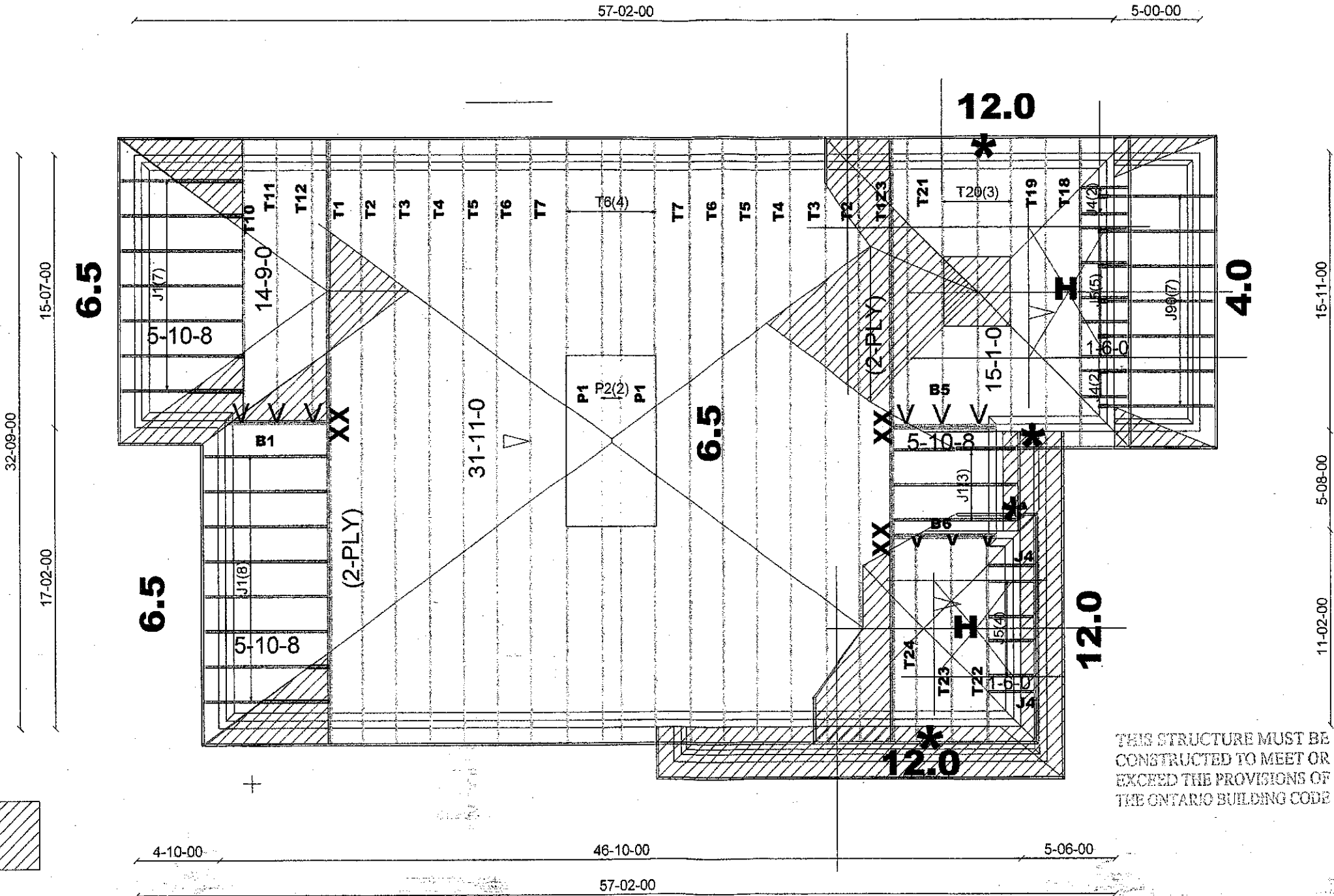
ALL B- 2-2X10 (FLUSH)

* -24" RAISED
FASCIA ONLY
H -24" RAISED CEILING

DENOTES:
CONVENTIONAL
FRAMING



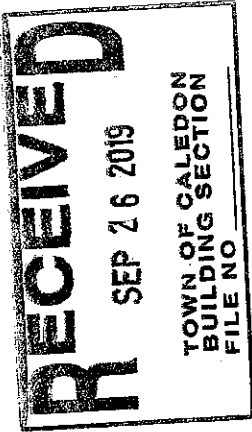
9/12 PITCHES (TYP.)
UNLESS NOTED



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

GROUND SNOW LOAD Ss= 1.8kPa
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



M6256



Job Track: 50120
Plan Log: 200598
Layout ID: 403060

Builder / Location:
GREEN PARK HOMES / Caledon
Project: LAMBERTS LANE HOME CORP.
Date: 5/9/2019 Sales: Mario DiCano Designer: Sunny Jane

Model / Elevation:
KINMOUNT 6A/3 Lot 39

Mikek ver 8.2.3.228

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DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #: 39
 Elevation: 3

Job Track: 50120
 PlanLog: 200598
 Layout ID: 403060
 Ref #
 Page: 1 of 2
 Date: 05/09/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	31-11-00	4-05-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	339.54 204.00		
	1 2-ply	T1Z3 Hip Girder	9/12	31-11-00	4-05-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	339.54 204.00		
	2	T2 Hip	9/12	31-11-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	254.66 160.33		
	2	T3 Hip	9/12	31-11-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.12 177.00		
	2	T4 Hip	9/12	31-11-00	7-07-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	294.24 187.33		
	2	T5 Hip	9/12	31-11-00	8-08-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	307.46 193.00		
	6	T6 Hip	9/12	31-11-00	9-09-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	966.5 610.00		
	2	T7 Hip	9/12	31-11-00	10-10-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	334.92 208.00		
	1	T10 Hip Girder	9/12	14-09-00	4-04-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	73.14 47.67		
	1	T11 Hip	9/12	14-09-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	64.66 42.50		
	1	T12 Hip	9/12	14-09-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	69.72 46.67		
	1	T18 Hip Girder	12/12	15-01-00	5-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	94.69 61.50		
	1	T19 Hip	12/12	15-01-00	7-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	94.02 60.83		
	3	T20 Hip	12/12	15-01-00	9-04-08	2 x 4	1-03-08	3-10-08 3-10-08	247.52 159.00		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #: 39
 Elevation: 3

Job Track: 50120
 PlanLog: 200598
 Layout ID: 403060
 Ref #
 Page: 2 of 2
 Date: 05/09/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T21 Hip	12 / 12	15-01-00	7-04-08	2 x 4	1-03-08	3-10-08 3-10-08	85.26 54.83		
	1	T22 Hip Girder	12 / 12	10-04-00	5-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	69.54 46.33		
	1	T23 Hip	12 / 12	10-04-00	7-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	81.01 53.87		
	1	T24 Hip	12 / 12	10-04-00	8-07-00	2 x 6 2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	68.34 43.17		
	2	P1 Piggyback	9 / 12	9-09-05	2-02-00	2 x 4			55.84 36.00		
	2	P2 Piggyback	9 / 12	9-09-05	3-03-00	2 x 4			54.88 36.67		
	18	J1 Jack-Open	6.5 / 12	5-10-08	4-04-14	2 x 4	1-03-08	1-02-11 4-04-14	389.3 248.00		
	6	J4 Jack-Open	12 / 12	1-06-00	5-04-08	2 x 4	1-03-08	3-10-08 5-04-08	81.06 56.00		
	9	J5 Jack-Open	12 / 12	1-06-00	3-04-08	2 x 4	1-03-08	1-10-08 3-04-08	86.6 63.00		
	7	J90 Jack-Open	4 / 12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	101.14 65.33		

TOTAL # TRUSS= 76 TOTAL BFT OF ALL TRUSSES= 3062.83 BFT. TOTAL WEIGHT OF ALL TRSSES 4836.7 LBS

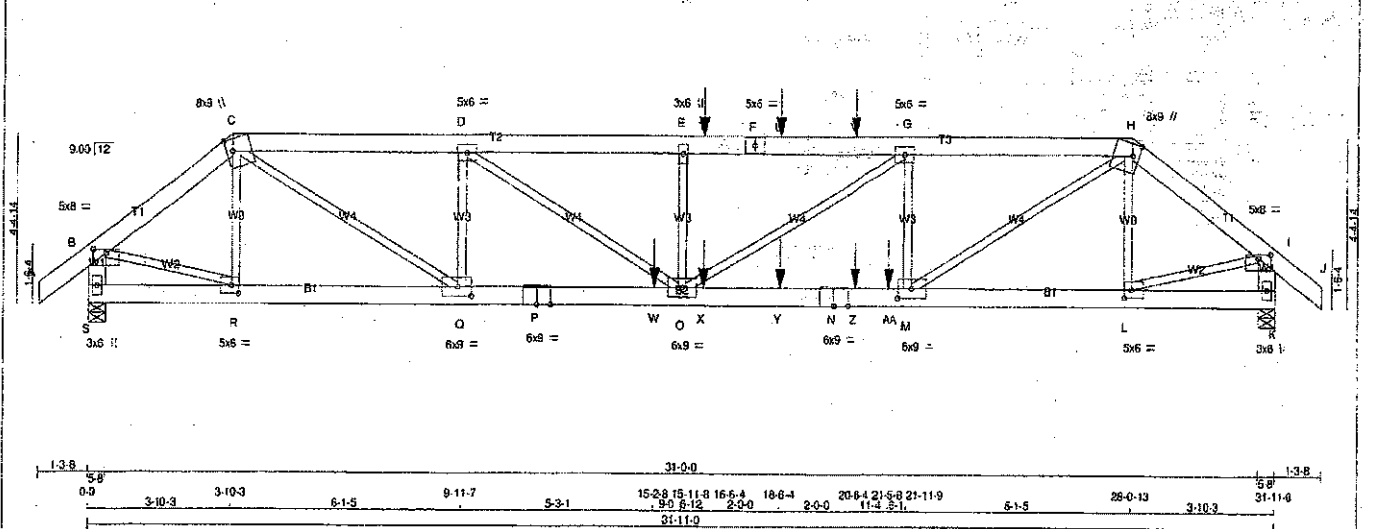
HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
9	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 12

DWG# T-1911482 CONTINUED ON PAGE 2

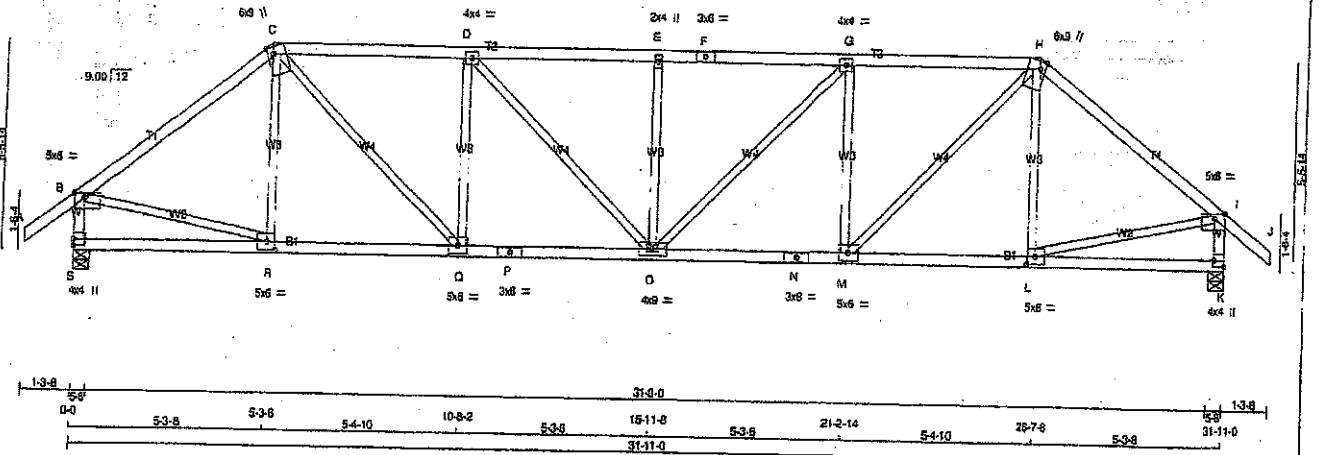
JOB NAME 403060	TRUSS NAME T1Z3	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 5 Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:25:32 2019 Page 1	
ID: D7uIPnW9?D2Gkx1HTFaFreyNCCCH-Hn_xBn2dghmFwWCJKx8dG7n_TILVqFPB?r2GcaH				31-11-0 232.8	
Scale = 1:30.0					



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x6 DRY No.2 SPF C - F 2x6 DRY No.2 SPF F - H 2x6 DRY No.2 SPF H - J 2x6 DRY No.2 SPF S - B 2x6 DRY No.2 SPF K - I 2x6 DRY No.2 SPF S - P 2x6 DRY 1650F 1.5E SPF P - N 2x6 DRY 1650F 1.5E SPF N - K 2x6 DRY 1650F 1.5E SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REORD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX S 4185 0 4185 0 0 5-8 5-8 K 4678 0 4678 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL S 3103 1791/0 565/0 0/0 0/0 747/0 0/0 K 3465 2010/0 524/0 0/0 0/0 631/0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FACTORED WEBS MEMB. FORCE VERT. LOAD LC1 MAX. MAX. MEMB. FORCE MAX. (LBS) (PLF) (LC) UNBRAC (LBS) (LC) FR-TO FROM TO LENGTH FR-TO A-B 0/44 -102.1 -102.1 0.04 (1) 10.00 R-C -747/0 0.10 (1) B-C -4707/0 -102.1 -102.1 0.10 (1) 5.25 C-D 0/5705 0.71 (1) C-D -8534/0 -102.1 -102.1 0.28 (1) 3.99 D-E -2659/0 0.35 (1) D-E -11632/0 -102.1 -102.1 0.35 (1) 3.42 E-F 0/3708 0.48 (1) E-F -11632/0 -102.1 -102.1 0.50 (1) 3.25 F-G -922/0 0.12 (1) F-G -11632/0 -102.1 -102.1 0.50 (1) 3.25 G-H 0/2184 0.27 (1) G-H -11632/0 -102.1 -102.1 0.50 (1) 3.25 H-I -2127/0 0.28 (1) H-I -9799/0 -102.1 -102.1 0.34 (1) 3.69 I-J 0/6622 0.82 (1) I-J -11632/0 -102.1 -102.1 0.50 (1) 3.25 J-K -846/0 0.11 (1) J-K -9799/0 -102.1 -102.1 0.34 (1) 3.69 K-L 0/3872 0.48 (1) K-L -5329/0 -102.1 -102.1 0.11 (1) 4.99 L-M 0/3872 0.48 (1) L-M 0/44 -102.1 -102.1 0.04 (1) 10.00 M-N 0/15 (1) 7.01 M-N -4139/0 0.0 0.0 0.15 (1) 7.01 N-O 0/17 (1) 6.70 N-O -4641/0 0.0 0.0 0.17 (1) 6.70 O-P 0/0 0.0 0.0 0.04 (2) 10.00 O-P 0/3732 -38.5 -38.5 0.20 (1) 10.00 P-Q 0/8534 -38.5 -38.5 0.55 (1) 10.00 P-Q 0/8534 -38.5 -38.5 0.55 (1) 10.00 Q-R 0/8534 -38.5 -38.5 0.55 (1) 10.00 Q-R 0/9799 -38.5 -38.5 0.55 (1) 10.00 R-S 0/9799 -38.5 -38.5 0.55 (1) 10.00 R-S 0/9799 -38.5 -38.5 0.55 (1) 10.00 S-T 0/9799 -38.5 -38.5 0.55 (1) 10.00 S-T 0/9799 -38.5 -38.5 0.55 (1) 10.00 T-U 0/9799 -38.5 -38.5 0.55 (1) 10.00 T-U 0/9799 -38.5 -38.5 0.55 (1) 10.00 U-V 0/9799 -38.5 -38.5 0.55 (1) 10.00 U-V 0/9799 -38.5 -38.5 0.55 (1) 10.00 V-W 0/9799 -38.5 -38.5 0.55 (1) 10.00 V-W 0/9799 -38.5 -38.5 0.55 (1) 10.00 W-X 0/9799 -38.5 -38.5 0.55 (1) 10.00 W-X 0/9799 -38.5 -38.5 0.55 (1) 10.00 X-Y 0/9799 -38.5 -38.5 0.55 (1) 10.00 X-Y 0/9799 -38.5 -38.5 0.55 (1) 10.00 Y-Z 0/9799 -38.5 -38.5 0.55 (1) 10.00 Y-Z 0/9799 -38.5 -38.5 0.55 (1) 10.00 Z-AA 0/9799 -38.5 -38.5 0.55 (1) 10.00 Z-AA 0/9799 -38.5 -38.5 0.55 (1) 10.00 AA-BB 0/4226 -38.5 -38.5 0.21 (1) 10.00 AA-BB 0/0 -38.5 -38.5 0.04 (2) 10.00 BB-CC 0/0 -38.5 -38.5 0.04 (2) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR. TYPE HEEL CONN. T 18-6-4 -198 -198 --- BACK VERT TOTAL --- U 18-6-4 -198 -198 --- BACK VERT TOTAL --- V 20-6-4 -198 -198 --- BACK VERT TOTAL --- W 15-2-8 -198 -198 --- TOP VERT TOTAL --- X 18-6-4 -67 -65 --- BACK VERT TOTAL --- Y 18-6-4 -67 -65 --- BACK VERT TOTAL --- Z 20-6-4 -67 -65 --- BACK VERT TOTAL --- AA 21-5-8 -1320 -1320 --- TOP VERT TOTAL ---				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 29.0 PSF DL = 6.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 52.5 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015 THIS DESIGN COMPLES WITH: PART 9 OF CBC 2010, OBC 2012 CSA 086-09, CSA 086-14 TPIC 2011, TPIC 2014 (65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL. (LL) = L/360 (1.06") CALCULATED VERT. DEFL. (LL) = L/999 (0.25") ALLOWABLE DEFL. (TL) = L/360 (1.06") CALCULATED VERT. DEFL. (TL) = L/919 (0.42") CSI: TC=0.50/1.00 (E-G-1), BC=0.55/1.00 (O-Q-1), WS=0.32/1.00 (H-M-1), SS=0.63/1.00 (O-Q-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1657 788 1987 1658 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP = 0.89 (G) (INPUT = 0.90) JSI METAL = 0.85 (P) (INPUT = 1.00) Structural component only DWG# T-1911508 CONTINUED ON PAGE 2			
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JOB NAME: 403045 TRUSS NAME: T2 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington

Version 8.300 S Apr 23 2019 MT&T Industries, Inc. Tue May 14 10:41:21 2019 Page 1
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 Scale = 1:50.3



CHORDS

CHORDS	SIZE	DRY	NO.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVV-p	MT20	5.0	8.0	1.50	3.50
C TWWV-m	MT20	6.0	9.0	Edge	2.00
D TMWW-t	MT20	4.0	4.0		
E TMW-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
G TMWW-t	MT20	4.0	4.0		
H TWWV-m	MT20	5.0	8.0	Edge	2.00
I TMVV-p	MT20	5.0	8.0	1.50	3.50
K BMV1-p	MT20	4.0	4.0	2.00	Edge
L BMWW-t	MT20	5.0	6.0	2.50	2.75
M SS-t	MT20	3.0	6.0		
O BMWWV-t	MT20	4.0	9.0		
P BS-t	MT20	3.0	8.0		
Q BMWW-t	MT20	5.0	6.0		
R BMWW-t	MT20	5.0	6.0	2.50	2.75
S BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	DOWN	GROSS REACTION	UPLIFT		
JT	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
S	2385	0	2385	0	5-8	5-8
K	2385	0	2385	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN COMPONENT REACTIONS				WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
JT	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		VERT. LOAD	LC1				
FR-TO		FROM	TO		FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	R-C	-195 / 98	0.09 (1)
B-C	-2428 / 0	-102.1	-102.1	0.73 (1)	C-Q	0 / 1667	0.38 (1)
C-D	-3138 / 0	-102.1	-102.1	0.85 (1)	Q-D	-961 / 0	0.43 (1)
D-E	-3494 / 0	-102.1	-102.1	0.89 (1)	14-D	0 / 500	0.11 (1)
E-F	-3494 / 0	-102.1	-102.1	0.89 (1)	14-E	-497 / 0	0.22 (1)
F-G	-3494 / 0	-102.1	-102.1	0.89 (1)	14-G	0 / 500	0.11 (1)
G-H	-3138 / 0	-102.1	-102.1	0.85 (1)	M-H	-961 / 0	0.43 (1)
H-I	-2428 / 0	-102.1	-102.1	0.73 (1)	M-H	0 / 1667	0.38 (1)
I-J	0 / 42	-102.1	-102.1	0.14 (1)	L-H	-195 / 98	0.09 (1)
S-B	-2301 / 0	0.0	0.0	0.24 (1)	B-R	0 / 1987	0.45 (1)
K-I	-2301 / 0	0.0	0.0	0.24 (1)	L-I	0 / 1987	0.45 (1)
S-R	0 / 0	-38.5	-38.5	0.21 (3)			
R-O	0 / 1986	-38.5	-38.5	0.47 (2)			
Q-P	0 / 3138	-38.5	-38.5	0.60 (1)			
P-O	0 / 3138	-38.5	-38.5	0.60 (1)			
O-N	0 / 3138	-38.5	-38.5	0.60 (1)			
N-M	0 / 3138	-38.5	-38.5	0.60 (1)			
M-L	0 / 1936	-38.5	-38.5	0.47 (2)			
L-K	0 / 0	-38.5	-38.5	0.21 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 25.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012
 - CSA 086-08, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/980 (1.06")
 CALCULATED VERT. DEFL. (LL) = L/989 (0.18")
 ALLOWABLE DEFL. (TL) = L/980 (1.08")
 CALCULATED VERT. DEFL. (TL) = L/989 (0.29")

CSI: TC=0.73/1.00 (H-I), SC=0.60/1.00 (O-Q),
 WB=0.45/1.00 (I-L), SB=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENG=1.10

COMPANION LIVE LOAD FACTOR = 1.00

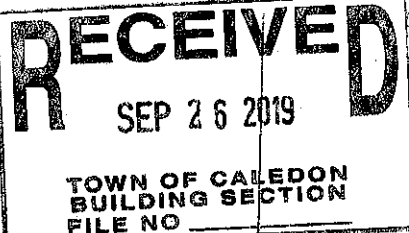
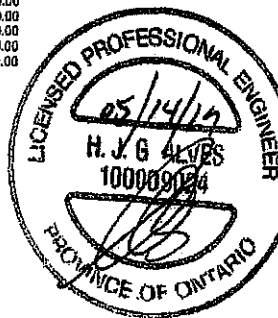
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PS) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (L) (INPUT = 0.90)
 JSI METAL = 0.98 (F) (INPUT = 1.00)



Structural component only
 DWG# T-1911484

Structural drawing of a roof truss system. The drawing shows a series of truss members (top and bottom chords, verticals, and diagonals) supporting a roof. Key dimensions and labels include:

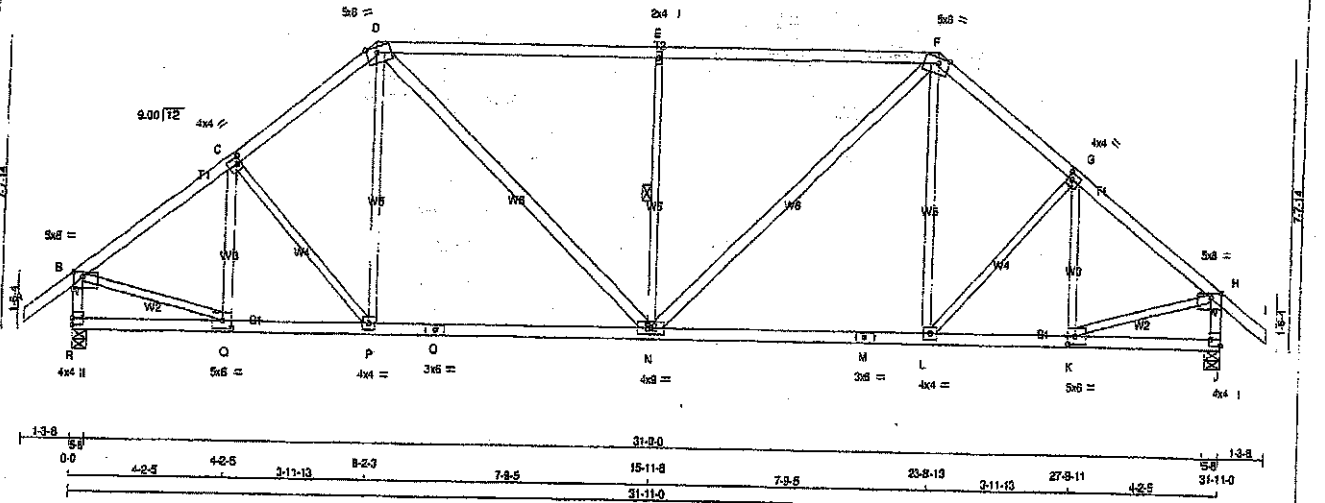
- Top Chord:** Labeled with points D, E, F, G, H. Horizontal segments are marked with dimensions: 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //.
- Bottom Chord:** Labeled with points R, Q, P, O, N, M, L. Horizontal segments are marked with dimensions: 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //.
- Verticals:** Labeled with points B, C, D, E, F, G, H, I, J, K. Vertical segments are marked with dimensions: 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //.
- Diagonals:** Labeled with points A, B, C, D, E, F, G, H, I, J, K. Diagonal segments are marked with dimensions: 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //, 3x6 //.
- Dimensions:**
 - Overall length: 31-0-0
 - Overall height: 13-8
 - Horizontal dimensions: 1-3-8, 5-8, 5-8-13, 5-8-13, 6-2-3, 12-11-0, 6-0-15, 19-0-0, 6-2-3, 25-2-3, 5-8-13, 5-8, 1-3-8.
 - Vertical dimensions: 1-3-8, 5-8, 5-8-13, 5-8-13, 6-2-3, 12-11-0, 6-0-15, 19-0-0, 6-2-3, 25-2-3, 5-8-13, 5-8, 1-3-8.
- Labels:** A, B, C, D, E, F, G, H, I, J, K, L, M, N, O, P, Q, R, S, T, U, V, W, X, Y, Z.

A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The seal contains the following text: "LICENSED PROFESSIONAL ENGINEER" at the top, "H. J. G. ALVES" in the center, "100099024" below the name, and "PROVINCE OF ONTARIO" at the bottom. There are handwritten marks: "05/14/17" in the upper half and a signature in the lower half.

Structural component only
DWG# T-1911485

JOB NAME 403045	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: D7uIPnW9?DzGkx1HTPaFreyNCCH-L9QVD?_?WpyDyoV4dUymeVIT9Ms8MRw8nZSWzGdDg
1-3-8 0-0 4-2-5 4-2-5 3-11-13 8-2-3 7-8-5 15-11-8 7-8-5 23-8-13 3-11-13 27-8-11 4-2-5 31-11-13 30-8-4 1-3-8
Scale = 1/32"



LUMBER
N.L.G.A. RULES
CHORDS

SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2
D - F	2x4 DRY	No.2
F - I	2x4 DRY	No.2
R - B	2x4 DRY	No.2
J - H	2x4 DRY	No.2
R - O	2x4 DRY	No.2
O - M	2x4 DRY	No.2
M - J	2x4 DRY	No.2
ALL WEBS EXCEPT	2x3 DRY	No.2
D - N	2x4 DRY	No.2
N - F	2x4 DRY	No.2

DRY-SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50 3.50
C	TMVW-1	MT20	4.0	4.0	2.00 1.50
D	TTWW-m	MT20	5.0	8.0	Edge 3.50
E	TMVW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	8.0	Edge 3.50
G	TMVW-1	MT20	4.0	4.0	2.00 1.50
H	TMVW-p	MT20	5.0	8.0	1.50 3.50
J	BMV1-p	MT20	4.0	4.0	2.00 Edge
K	BMVW-1	MT20	5.0	8.0	2.50 2.50
L	BMVW-1	MT20	4.0	4.0	
M	BS-1	MT20	3.0	8.0	
N	BMVW-1	MT20	4.0	9.0	
O	BS-1	MT20	3.0	8.0	
P	BMVW-1	MT20	4.0	4.0	
Q	BMVW-1	MT20	5.0	8.0	2.50 2.50
R	BMV1-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	0	0
R	HORIZ	HORIZ	0	0
J	UPLIFT	UPLIFT	0	0

UNFACTORED REACTIONS

1ST CASE	MAXIMUM COMPONENT REACTIONS	PERALIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	0/0	0/0
R	1773	1007/0	335/0	0/0	0/0
J	1773	1007/0	335/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0/42	-102.1 -102.1 0.14 (1)	10.00	Q-C	-410/0
B-C	-2391/0	-102.1 -102.1 0.29 (1)	4.21	P-D	-89/0
C-D	-2377/0	-102.1 -102.1 0.28 (1)	4.22	N-E	0/363
D-E	-2561/0	-102.1 -102.1 1.00 (1)	2.73	M-F	0/939
E-F	-2561/0	-102.1 -102.1 1.00 (1)	2.73	L-F	0/363
F-G	-2377/0	-102.1 -102.1 0.28 (1)	4.22	N-F	0/939
G-H	-2391/0	-102.1 -102.1 0.29 (1)	4.21	L-F	0/363
H-I	0/42	-102.1 -102.1 0.14 (1)	10.00	L-G	-89/0
R-B	-2311/0	0.0 0.0 0.24 (1)	5.82	K-G	-410/0
J-H	-2311/0	0.0 0.0 0.24 (1)	5.82	B-Q	0/2012
R-Q	0/0	-38.5 -38.5 0.12 (3)	10.00	K-H	0/2012
Q-P	0/1935	-38.5 -38.5 0.50 (2)	10.00		
P-O	0/1879	-38.5 -38.5 0.82 (2)	10.00		
O-N	0/1879	-38.5 -38.5 0.82 (2)	10.00		
N-M	0/1879	-38.5 -38.5 0.82 (2)	10.00		
M-L	0/1879	-38.5 -38.5 0.82 (2)	10.00		
L-K	0/1935	-38.5 -38.5 0.50 (2)	10.00		
K-J	0/0	-38.5 -38.5 0.12 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/998 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=1.00/1.00 (E-F-1), BC=0.82/1.00 (N-P-2), WB=0.45/1.00 (B-C-1), SS=0.39/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

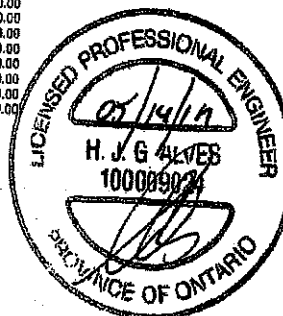
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.80)
JSI METAL= 0.95 (C) (INPUT = 1.00)

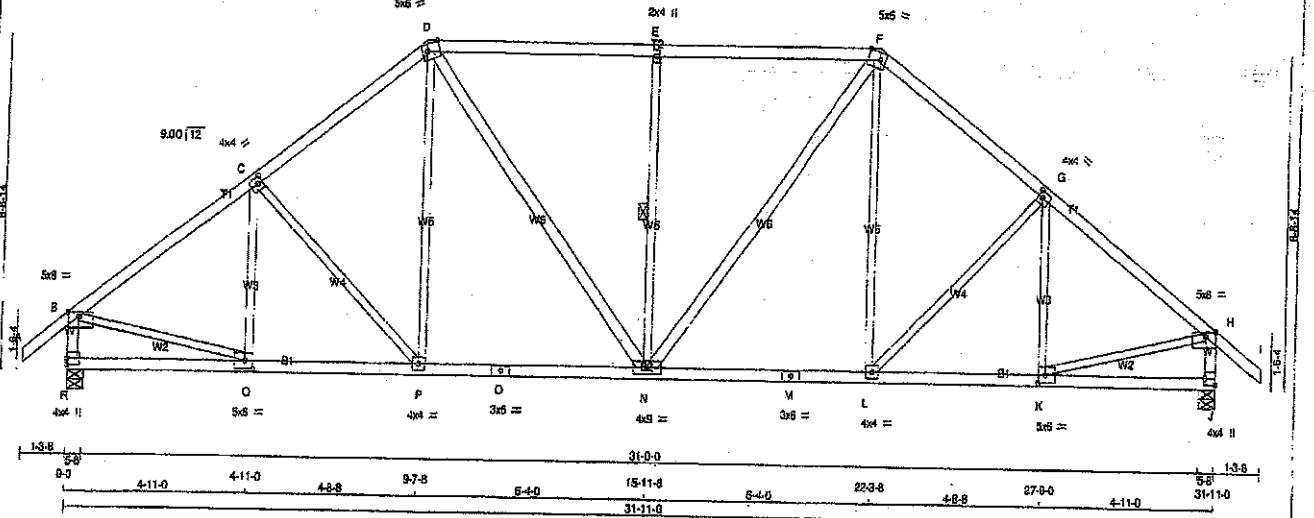


Structural component only
DWG# T-1911486

JOB NAME: 403045 TRUSS NAME: T5 QUANTITY: 2 PLV: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington

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ID: D7uIPnW9?DzGkx1HTFaFreyNCOCH-pL_IQL?dH744ax4HBCQBj1ZFYj3pa3LRJ0ZJzGdDf

Scale = 1/32



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - I	2x4	DRY No.2
R - B	2x4	DRY No.2
J - H	2x4	DRY No.2
R - O	2x4	DRY No.2
O - M	2x4	DRY No.2
M - J	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
D - N 2x4 DRY No.2
N - F 2x4 DRY No.2

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-l	MT20	4.0	4.0	2.00	1.50
D	TIWW-m	MT20	5.0	6.0	Edge	4.25
E	TMWW-w	MT20	2.0	4.0		
F	TIWW-m	MT20	5.0	6.0	Edge	4.25
G	TMWW-l	MT20	4.0	4.0	2.00	1.50
H	TMWW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1-p	MT20	4.0	4.0	2.00	Edge
K	BMV1-l	MT20	5.0	6.0	2.50	2.50
L	BMV1-w	MT20	4.0	4.0		
M	BS-l	MT20	3.0	8.0		
N	BS-l	MT20	4.0	8.0		
O	BS-l	MT20	3.0	8.0		
P	BS-l	MT20	4.0	4.0		
Q	BS-l	MT20	5.0	6.0	2.50	2.50
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	BEARINGS					
SPF	FACTORED		MAXIMUM FACTORED		INPUT	REQD
SPF	GROSS REACTION		GROSS REACTION		BRG	BRG
SPF	JT	VERT	DOWN	HORIZ	IN-SX	IN-SX
SPF	R	2385 0	2385 0	0	5-8	5-8
SPF	J	2385 0	2385 0	0	5-8	5-8
SPF	UNFACTORED REACTIONS					

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD LC1	PLF	CSI (LC)	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO					FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	Q-C	-265 / 38	0.11 (1)
B-C	-2445 / 0	-102.1	-102.1	0.40 (1)	4.05	C-P	-265 / 0	0.23 (1)
C-D	-2268 / 0	-102.1	-102.1	0.38 (1)	4.18	P-D	0 / 425	0.10 (2)
D-E	-2193 / 0	-102.1	-102.1	0.81 (1)	3.89	D-N	0 / 652	0.10 (1)
E-F	-2183 / 0	-102.1	-102.1	0.81 (1)	4.18	N-E	-781 / 0	0.39 (1)
F-G	-2288 / 0	-102.1	-102.1	0.38 (1)	4.05	E-F	0 / 652	0.10 (1)
G-H	-2445 / 0	-102.1	-102.1	0.40 (1)	10.00	L-F	0 / 425	0.10 (2)
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	L-G	-265 / 0	0.23 (1)
I-B	-2304 / 0	0.0	0.0	0.24 (1)	5.62	K-G	-265 / 38	0.11 (1)
J-H	-2304 / 0	0.0	0.0	0.24 (1)	5.62	B-Q	0 / 2041	0.46 (1)
R-Q	0 / 0	-38.5	-38.5	0.15 (3)	10.00	K-H	0 / 2041	0.46 (1)
Q-P	0 / 1983	-38.5	-38.5	0.45 (1)	10.00			
P-O	0 / 1802	-38.5	-38.5	0.49 (2)	10.00			
O-N	0 / 1802	-38.5	-38.5	0.49 (2)	10.00			
N-M	0 / 1802	-38.5	-38.5	0.49 (2)	10.00			
M-L	0 / 1802	-38.5	-38.5	0.49 (2)	10.00			
L-K	0 / 1983	-38.5	-38.5	0.45 (1)	10.00			
K-J	0 / 0	-38.5	-38.5	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/899 (0.10")
ALLOWABLE DEFL.(TL) = L/850 (1.06")
CALCULATED VERT. DEFL.(TL) = L/899 (0.17")

CSI: TC=0.91/1.00 (D-E1), SC=0.49/1.00 (N-P2), WS=0.48/1.00 (H-K1), SS=0.31/1.00 (E-F1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.16 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

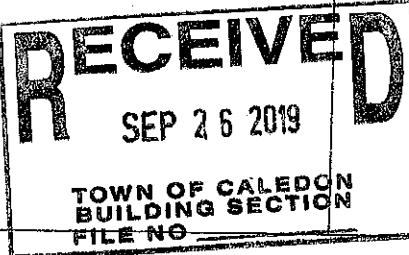
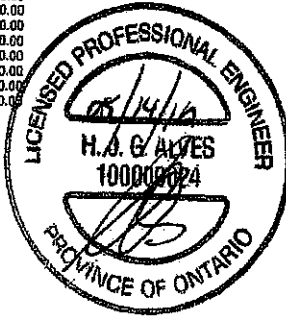
NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 918 354 1867 798 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

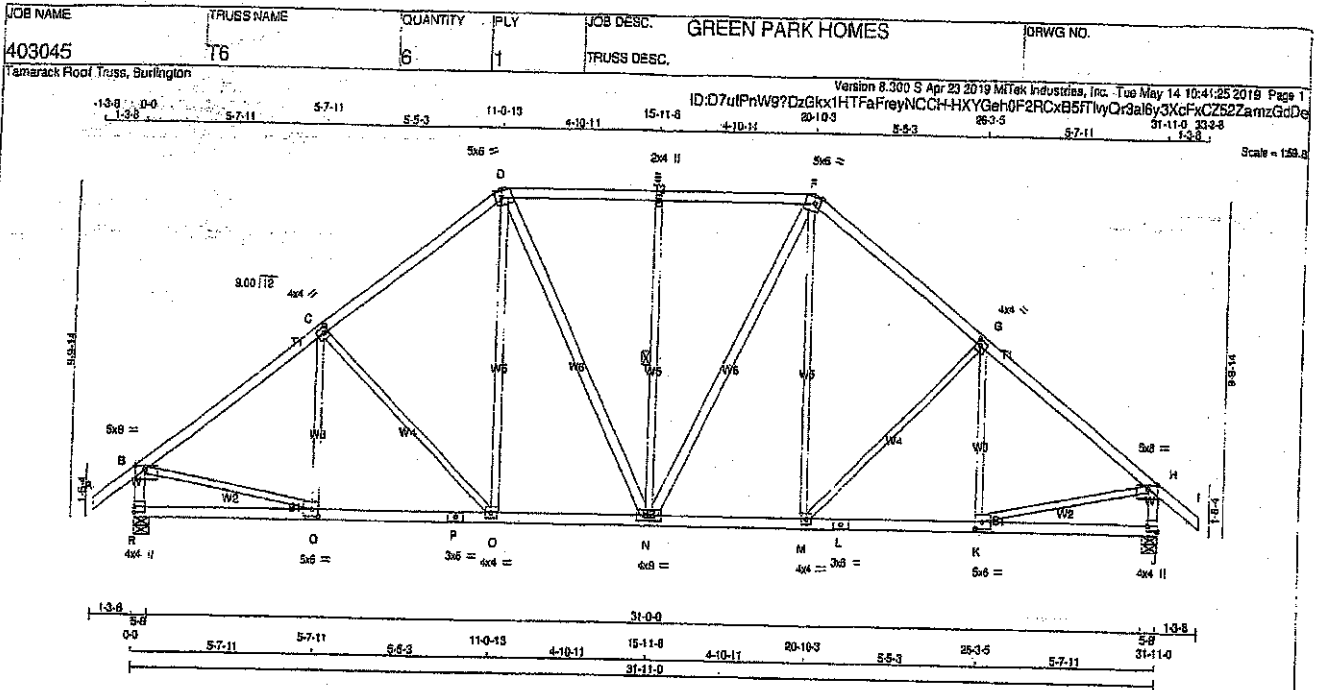
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (C) (INPUT = 0.90)

JSI METAL = 0.88 (C) (INPUT = 1.00)



Structural component only
DWG# T-1911487



LUMBER

N.L.G. & A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS

EXCEPT

CHORDS	SIZE	DRY	LUMBER	DESCR.
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50 3.50
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TMVW-m	MT20	5.0	6.0	Edge 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TMVW-m	MT20	5.0	6.0	Edge 2.00
G	TMVW-t	MT20	4.0	4.0	2.00 1.50
H	TMVW-p	MT20	5.0	8.0	1.50 3.50
J	BMVt-p	MT20	4.0	4.0	2.00 Edge
K	BMVW-t	MT20	5.0	8.0	2.50 2.50
L	BS-t	MT20	3.0	6.0	
M	BMVW-t	MT20	4.0	4.0	
N	BMVW-t	MT20	4.0	9.0	
O	BMVW-t	MT20	4.0	4.0	
P	BS-t	MT20	3.0	6.0	
Q	BMVW-t	MT20	5.0	8.0	2.50 2.50
R	BMVt-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORIZ	UP-LIFT
R	2385	0	0
J	2385	0	0

UNFACTORED REACTIONS

1ST CASE

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S), R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)
FR-TO				FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	C-C	-185 / 119	0.09 (1)	
B-C	-2474 / 0	-102.1 -102.1	0.54 (1)	D-D	-422 / 0	0.52 (1)	
C-D	-2181 / 0	-102.1 -102.1	0.49 (1)	E-E	0 / 498	0.11 (2)	
D-E	-1916 / 0	-102.1 -102.1	0.35 (1)	F-F	0 / 440	0.07 (1)	
E-F	-1916 / 0	-102.1 -102.1	0.35 (1)	G-G	-403 / 0	0.39 (1)	
F-G	-2181 / 0	-102.1 -102.1	0.49 (1)	H-H	0 / 440	0.07 (1)	
G-H	-2474 / 0	-102.1 -102.1	0.54 (1)	I-I	0 / 498	0.11 (2)	
H-I	0 / 42	-102.1 -102.1	0.14 (1)	J-J	-422 / 0	0.52 (1)	
I-J	-2286 / 0	0.0 0.0	0.24 (1)	K-K	-185 / 119	0.09 (1)	
J-K	-2286 / 0	0.0 0.0	0.24 (1)	L-L	0 / 2055	0.46 (1)	
K-L				M-M	0 / 2055	0.46 (1)	
L-M				N-N			
M-N				O-O			
N-O				P-P			
O-P				Q-Q			
P-Q				R-R			
Q-R				S-S			
R-S				T-T			
S-T				U-U			
T-U				V-V			
U-V				W-W			
V-W				X-X			
W-X				Y-Y			
X-Y				Z-Z			
Y-Z							
Z-A							

TOTAL WEIGHT = 6 X 101 = 966 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-08, CSA 089-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/998 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/998 (0.15")

CSI: TC=0.54/1.00 (B-C-1), BC=0.48/1.00 (C-D-2), WB=0.52/1.00 (C-O-1), ES=0.24/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

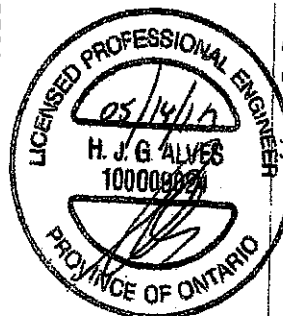
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1856

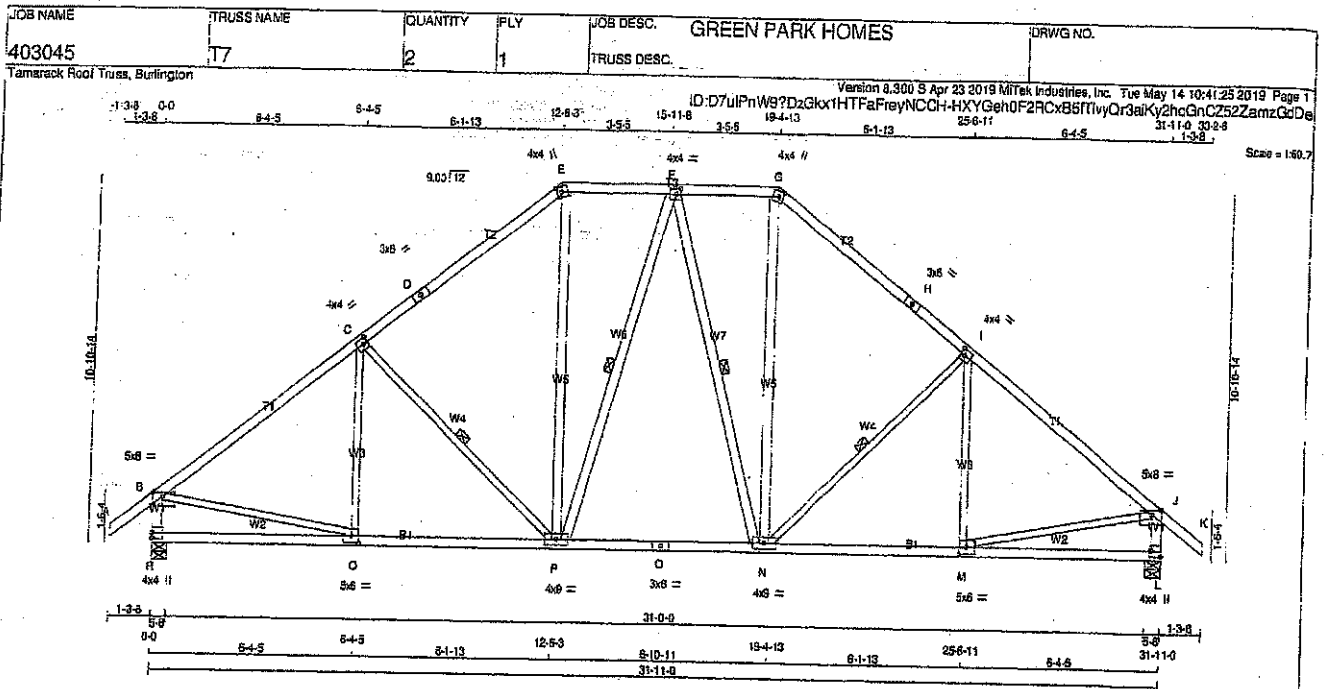
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRP = 0.85 (B) (INPUT = 0.90)
METAL = 0.80 (P) (INPUT = 1.00)



Structural component only
DWG# T-1911488



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
P - E	2x4	DRY	No.2
P - F	2x4	DRY	No.2
F - N	2x4	DRY	No.2
N - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	1.50	3.50
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	8.0		
E TTW-m	MT20	4.0	4.0	2.00	1.75
F TMVW-t	MT20	4.0	4.0	2.00	1.75
G TTW-m	MT20	4.0	4.0	2.00	1.75
H TS-t	MT20	3.0	8.0		
I TMVW-t	MT20	4.0	4.0	2.00	1.50
J TMVW-p	MT20	5.0	8.0	1.50	3.50
L BMV1-p	MT20	4.0	4.0	2.00	Edge
M BMVW-t	MT20	5.0	6.0	2.50	2.75
N BMVW-t	MT20	4.0	9.0		
O BS-t	MT20	3.0	8.0		
P BMVW-t	MT20	4.0	9.0		
Q BMVW-t	MT20	5.0	6.0	2.50	2.75
R BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORIZ	UPLIFT
R 2385	0	2385	0
L 2385	0	2385	0

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, N-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO	0 / 42	FROM	TO	FR-TO	0 / 174	0.07 (1)	
A-B	-102.1	-102.1	0.14 (1)	C-C	-569 / 0	0.29 (1)	
B-C	-2481 / 0	-102.1	-102.1	0.72 (1)	C-P	0 / 932	0.13 (1)
C-D	-2070 / 0	-102.1	-102.1	0.84 (1)	P-E	-212 / 0	0.15 (1)
D-E	-2070 / 0	-102.1	-102.1	0.84 (1)	F-N	-216 / 0	0.15 (1)
E-F	-1627 / 0	-102.1	-102.1	0.16 (1)	N-G	0 / 837	0.13 (1)
F-G	-1627 / 0	-102.1	-102.1	0.84 (1)	N-I	-569 / 0	0.29 (1)
G-H	-2070 / 0	-102.1	-102.1	0.84 (1)	M-I	-111 / 174	0.07 (1)
H-I	-2070 / 0	-102.1	-102.1	0.84 (1)	M-J	0 / 2057	0.46 (1)
I-J	-2481 / 0	-102.1	-102.1	0.72 (1)	B-Q	0 / 2057	0.46 (1)
J-K	0 / 42	-102.1	-102.1	0.14 (1)			
R-B	-2284 / 0	0.0	0.0	0.24 (1)			
L-J	-2284 / 0	0.0	0.0	0.24 (1)			
R-Q	0 / 0	-38.5	-38.5	0.28 (3)			
C-P	0 / 2022	-38.5	-38.5	0.54 (2)			
P-O	0 / 1691	-38.5	-38.5	0.48 (2)			
C-N	0 / 1691	-38.5	-38.5	0.48 (2)			
N-M	0 / 2022	-38.5	-38.5	0.54 (2)			
M-L	0 / 0	-38.5	-38.5	0.28 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 92.5 PSF

SPACING = 24.0 IN O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/999 (0.16")

CSI: TC=0.72/1.00 (B-C:1), BC=0.54/1.00 (P-Q:2), WB=0.48/1.00 (B-Q:1), SB=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

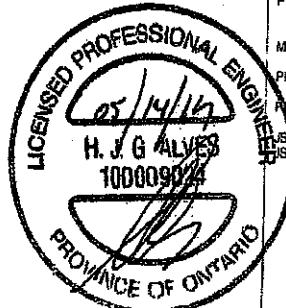
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PL) (FL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

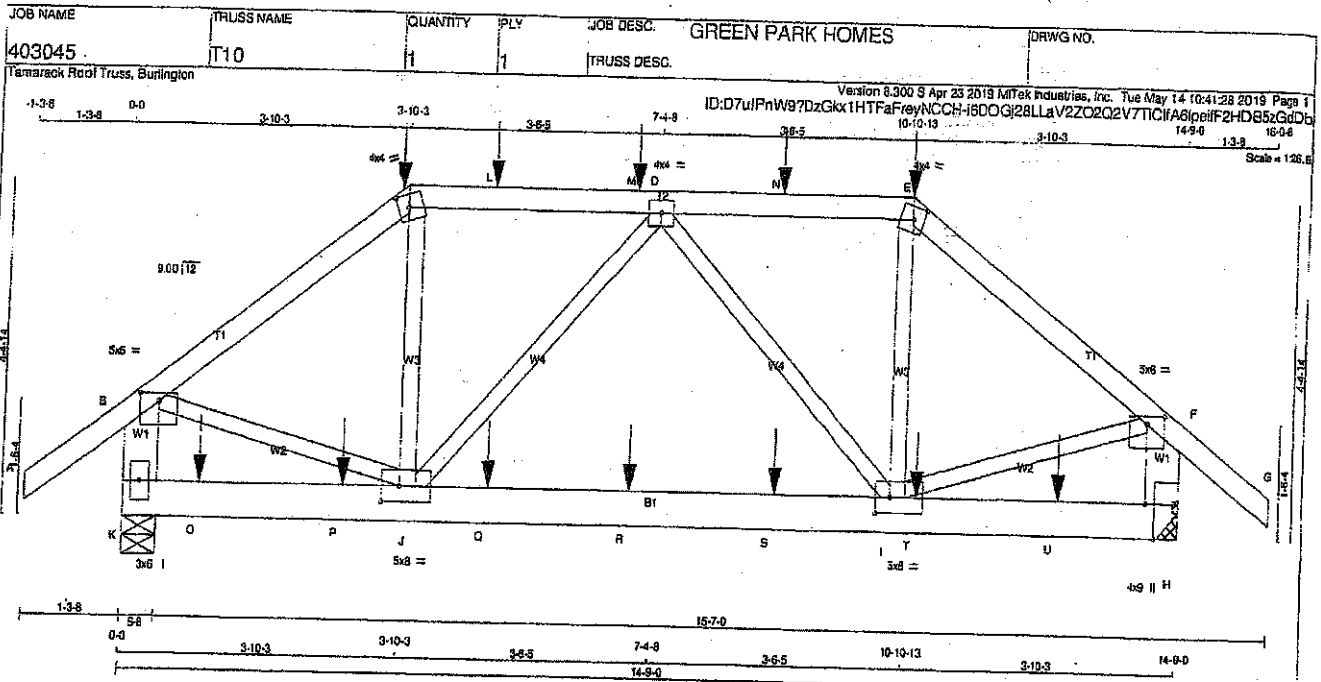
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP = 0.88 (Q) (INPUT = 0.90)
METAL = 0.89 (Q) (INPUT = 1.00)



Structural component only
DWG# T-1911489



LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DES. CR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
K - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
K - H	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.00
C	TTW-m	MT20	4.0	4.0	2.00	1.75
D	TMVW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	2.00	1.75
F	TMVW-p	MT20	5.0	8.0	1.25	3.00
H	BMV1-t	MT20	4.0	9.0	Edge	1.50
I	BMVWW-t	MT20	5.0	8.0	2.50	2.50
J	BMVWW-t	MT20	5.0	8.0	2.50	3.00
K	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	BRG	BRG
K	1936	0	5-8
H	1988	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3.8.

UNFACTORED REACTIONS

1ST LOSE	MAX. MIN. COMPONENT REACTIONS	PERMANENT	WIND	DEAD	SOIL
JT COMBINED	LIVE	0/0	0/0	348/0	0/0
K	1437	824/0	285/0	0/0	350/0
H	1454	848/0	258/0	0/0	350/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (L)	MAX. (L)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CSI (LC)	FR-TO	MAX. (L)
A-B	0/42	-102.1	-102.1 0.15 (1)	J-C	0/427
B-C	-1837/0	-102.1	-102.1 0.36 (1)	E	0/378
C-L	-1482/0	-102.1	-102.1 0.39 (1)	B-J	0/1521
L-M	-1482/0	-102.1	-102.1 0.39 (1)	F	0/1585
M-D	-1482/0	-102.1	-102.1 0.39 (1)	J-D	-483/0
D-N	-1522/0	-102.1	-102.1 0.39 (1)	D-I	-420/0
N-E	-1522/0	-102.1	-102.1 0.39 (1)		
E-F	-1891/0	-102.1	-102.1 0.37 (1)		
F-G	0/42	-102.1	-102.1 0.15 (1)		
K-B	-1849/0	0.0	0.0 0.13 (1)		
H-F	-1893/0	0.0	0.0 0.14 (1)		
K-O	0/0	-38.5	-38.5 0.20 (3)		
O-P	0/0	-38.5	-38.5 0.20 (3)		
P-J	0/0	-38.5	-38.5 0.20 (3)		
J-Q	0/1791	-38.5	-38.5 0.39 (1)		
Q-R	0/1791	-38.5	-38.5 0.39 (1)		
R-S	0/1791	-38.5	-38.5 0.39 (1)		
S-I	0/1791	-38.5	-38.5 0.39 (1)		
I-T	0/0	-38.5	-38.5 0.19 (3)		
T-U	0/0	-38.5	-38.5 0.19 (3)		
U-H	0/0	-38.5	-38.5 0.19 (3)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-3	-49	-55	FRONT	VERT	DEAD		
E	3-10-3	-282	-282	FRONT	VERT	SNOW		
E	10-10-13	-49	-55	FRONT	VERT	DEAD		
E	10-10-13	-129	-129	FRONT	VERT	TOTAL		
E	10-10-13	-282	-282	FRONT	VERT	SNOW		
L	5-0-12	-123	-123	FRONT	VERT	TOTAL		
M	7-0-12	-123	-123	FRONT	VERT	TOTAL		
N	9-0-12	-123	-123	FRONT	VERT	TOTAL		
O	1-0-12	-55	-70	FRONT	VERT	TOTAL		
P	3-0-12	-55	-70	FRONT	VERT	TOTAL		
Q	5-0-12	-55	-70	FRONT	VERT	TOTAL		
R	7-0-12	-55	-70	FRONT	VERT	TOTAL		
S	9-0-12	-55	-70	FRONT	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.6 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCS 2010, NBCS 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = U/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = U/999 (0.08")

CR: TC=0.39/1.00 (D-E), SC=0.39/1.00 (I-J),
WB=0.39/1.00 (F-I), SS=0.34/1.00 (C-D-I)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 1.00)

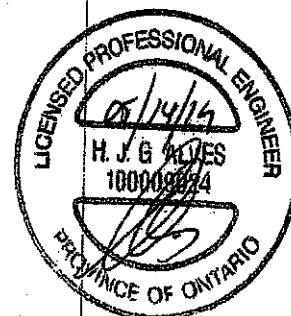
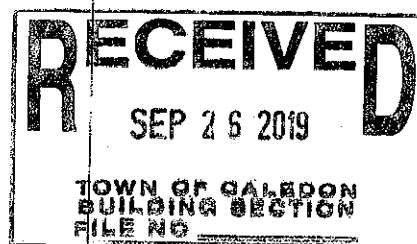
Structural component only

DWG# T-1911492 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T10	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

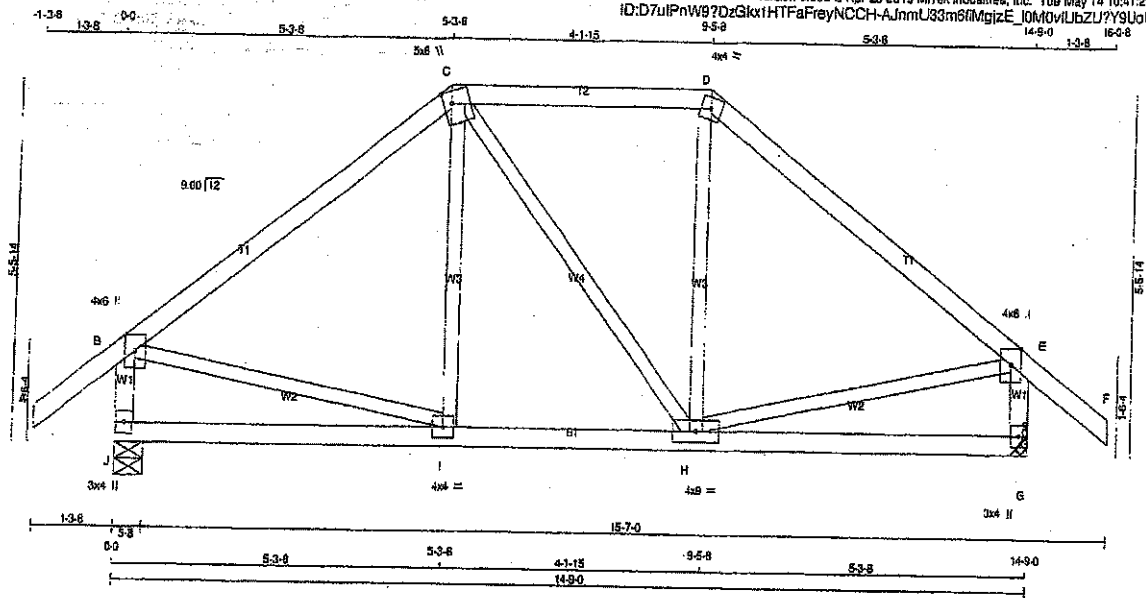
Version 8.300 S Apr 23 2019 Mittek Industries, Inc. Tue May 14 10:41:28 2019 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
T	11-0-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
U	13-0-12	-55	-70	--	FRONT	VERT	TOTAL	--	--



Structural component only
DWG# T-191 1492

JOB NAME 403045	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER	N.L.G.A. RULES	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	1.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTWW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
F	BMVW+p	MT20	3.0	4.0		
G	BMVW+p	MT20	4.0	4.0		
H	BMVW+p	MT20	4.0	4.0		
I	BMVW+p	MT20	4.0	4.0		
J	BMVW+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
J	1178	0	0	5-8
G	1178	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	872	508 / 0	155 / 0	0 / 0	0 / 0	208 / 0	0 / 0	0 / 0
G	872	508 / 0	155 / 0	0 / 0	0 / 0	208 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	WEBS
FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
A-B	0 / 42	0 / 174
B-C	-872 / 0	0 / 4
C-D	-889 / 0	0 / 177
D-E	-873 / 0	0 / 713
E-F	0 / 42	0 / 714
J-B	-1080 / 0	
G-E	-1089 / 0	
J-I	0 / 0	
I-H	0 / 897	
H-G	0 / 0	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 32.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 086-08, CSA 086-14
- TRC 2011, TRC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.49")
CALCULATED VERT. DEFL. (LL) = 1/889 (0.04")
ALLOWABLE DEFL. (TL) = 1/360 (0.49")
CALCULATED VERT. DEFL. (TL) = 1/889 (0.07")

CSI: TC=0.38/1.00 (D-E-I), BC=0.26/1.00 (H-I-2), WB=0.18/1.00 (E-H-I), SB=0.17/1.00 (D-E-I)

DOL LUMBER=1.03 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

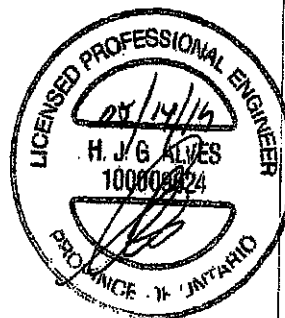
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

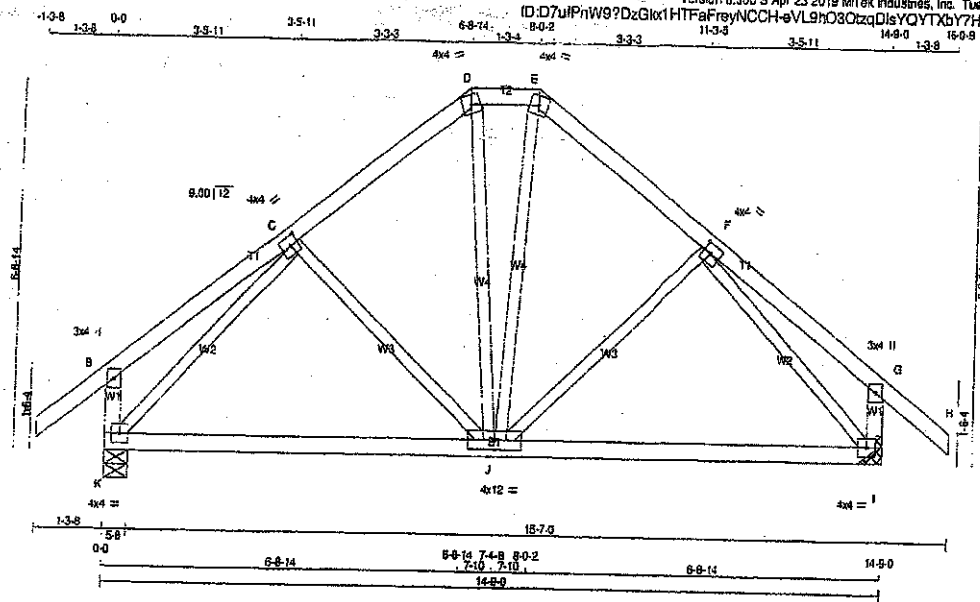
PLATE ROTATION TOL = 5.0 Deg.

SI GRIP = 0.82 (0) (INPUT = 0.80)
SI METAL = 0.45 (E) (INPUT = 1.00)



Structural component only
DWG# T-1911493

JOB NAME 403045	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						
Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 10:41:30 2019 Page 1						
ID: D7uIPmW9?DzGlx1HTFaFreyNCCH-eVL9h030tqDlsYOYTxbY7HTzmoHXZxjMmKGzGdDZ						
Scale = 1/32"						



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
K - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMVWW1-t	MT20	4.0	12.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
K	VERT	DOWN	0	0	5-8
I	HORIZ	UP	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 5-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	872	508 / 0	155 / 0	0 / 0	0 / 0	208 / 0	0 / 0
I	872	508 / 0	155 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TD				FR-TD			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	10.00	C-J	-137 / 33	0.07 (1)
B-C	0 / 23	-102.1	-102.1 0.18 (1)	10.00	J-F	-137 / 33	0.07 (1)
C-D	-800 / 0	-102.1	-102.1 0.14 (1)	8.25	K-C	-1074 / 0	0.43 (1)
D-E	-847 / 0	-102.1	-102.1 0.83 (1)	8.25	F-I	-1074 / 0	0.43 (1)
E-F	-800 / 0	-102.1	-102.1 0.14 (1)	8.25	D-J	0 / 275	0.06 (2)
F-G	0 / 23	-102.1	-102.1 0.18 (1)	10.00	J-E	0 / 275	0.06 (2)
G-H	0 / 42	-102.1	-102.1 0.14 (1)	10.00			
K-B	-273 / 0	0.0	0.0 0.03 (1)	7.91			
I-G	-273 / 0	0.0	0.0 0.03 (1)	7.91			
K-J	0 / 721	-38.5	-38.5 0.54 (2)	10.00			
J-I	0 / 721	-38.5	-38.5 0.54 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 888-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.49")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.49")
CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CS: TC=0.18/1.00 (B-C:1), BC=0.54/1.00 (J-K:2), WB=0.43/1.00 (C-K:1), SS=0.19/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

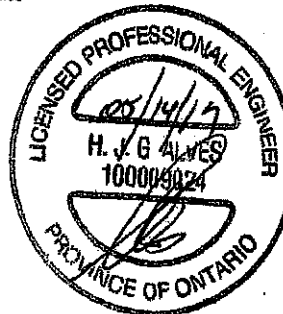
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1087 788 1987 1858

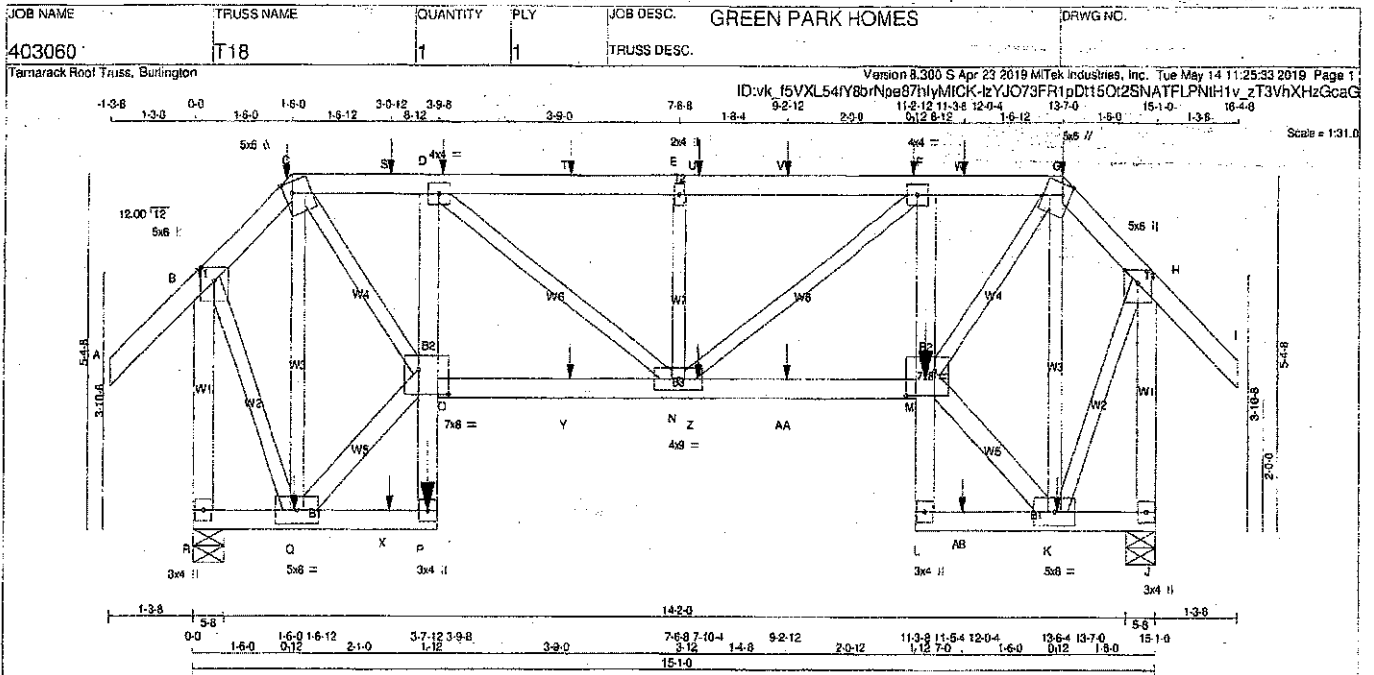
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

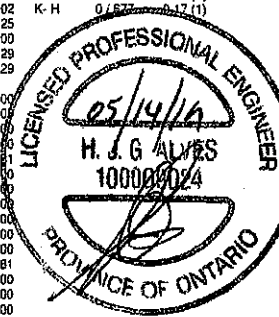
JSI GRIP = 0.87 (G) (INPUT = 0.90)
JSI METAL = 0.39 (F) (INPUT = 1.00)



Structural component only
DWG# T-1911494



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - G 2x4 DRY No.2 SPF G - I 2x4 DRY No.2 SPF I - H 2x4 DRY No.2 SPF H - B 2x4 DRY No.2 SPF B - P 2x4 DRY No.2 SPF P - D 2x4 DRY No.2 SPF D - M 2x4 DRY No.2 SPF M - F 2x4 DRY No.2 SPF F - J 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT O - O 2x4 DRY No.2 SPF M - K 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table> <tr> <th></th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG</th><th>REQ'D BRG</th></tr> <tr> <th>JT</th><th>VERT</th><th>DOWN</th><th>UP</th><th>IN-SX</th></tr> <tr> <td>H</td><td>1208</td><td>0</td><td>0</td><td>5-8</td></tr> <tr> <td>J</td><td>1208</td><td>0</td><td>0</td><td>5-8</td></tr> </table> UNFACTORED REACTIONS <table> <tr> <th>JT</th><th>1ST LOASE</th><th>MAX/MIN. COMPONENT REACTIONS</th><th>DEAD</th><th>SOIL</th></tr> <tr> <th></th><th>COMBINED</th><th>SNOW</th><th>LIVE</th><th>PERM. LIVE</th></tr> <tr> <td>R</td><td>895</td><td>519/0</td><td>181/0</td><td>0/0</td></tr> <tr> <td>J</td><td>895</td><td>519/0</td><td>181/0</td><td>0/0</td></tr> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (7) <table> <tr> <th>MEMB.</th><th>CHORDS</th><th>FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (PL)</th><th>MAX. CS (LC)</th><th>UNBRACED LENGTH</th><th>MEMB.</th><th>WEBS</th><th>FACTORED FORCE (LBS)</th><th>MAX. CS (LC)</th></tr> <tr> <th>FR-TO</th><th></th><th></th><th></th><th></th><th></th><th>FR-TO</th><th></th><th></th><th></th></tr> <tr> <td>A-B</td><td>0/50</td><td>-102.1</td><td>-102.1</td><td>0.16 (1)</td><td>10.00</td><td>C-C</td><td>-800/0</td><td>0.36 (1)</td><td></td></tr> <tr> <td>B-C</td><td>-448/0</td><td>-102.1</td><td>-102.1</td><td>0.15 (1)</td><td>6.25</td><td>O-O</td><td>0/369</td><td>0.07 (1)</td><td></td></tr> <tr> <td>C-S</td><td>-1016/0</td><td>-102.1</td><td>-102.1</td><td>0.14 (1)</td><td>6.02</td><td>C-O</td><td>0/1245</td><td>0.31 (1)</td><td></td></tr> <tr> <td>S-D</td><td>-1016/0</td><td>-102.1</td><td>-102.1</td><td>0.14 (1)</td><td>6.02</td><td>D-N</td><td>0/368</td><td>0.10 (1)</td><td></td></tr> <tr> <td>D-T</td><td>-1332/0</td><td>-102.1</td><td>-102.1</td><td>0.23 (1)</td><td>5.32</td><td>N-E</td><td>-422/0</td><td>0.09 (1)</td><td></td></tr> <tr> <td>T-E</td><td>-1332/0</td><td>-102.1</td><td>-102.1</td><td>0.23 (1)</td><td>5.32</td><td>E-F</td><td>0/368</td><td>0.10 (1)</td><td></td></tr> <tr> <td>E-U</td><td>-1332/0</td><td>-102.1</td><td>-102.1</td><td>0.23 (1)</td><td>5.32</td><td>M-K</td><td>0/369</td><td>0.07 (1)</td><td></td></tr> <tr> <td>U-V</td><td>-1332/0</td><td>-102.1</td><td>-102.1</td><td>0.23 (1)</td><td>5.32</td><td>M-G</td><td>0/1245</td><td>0.31 (1)</td><td></td></tr> <tr> <td>V-F</td><td>-1332/0</td><td>-102.1</td><td>-102.1</td><td>0.23 (1)</td><td>5.32</td><td>K-G</td><td>-800/0</td><td>0.36 (1)</td><td></td></tr> <tr> <td>F-W</td><td>-1016/0</td><td>-102.1</td><td>-102.1</td><td>0.14 (1)</td><td>6.02</td><td>B-Q</td><td>0/677</td><td>0.17 (1)</td><td></td></tr> <tr> <td>W-G</td><td>-1016/0</td><td>-102.1</td><td>-102.1</td><td>0.14 (1)</td><td>6.02</td><td>K-H</td><td>0/677</td><td>0.17 (1)</td><td></td></tr> <tr> <td>G-H</td><td>-448/0</td><td>-102.1</td><td>-102.1</td><td>0.15 (1)</td><td>6.25</td><td></td><td></td><td></td><td></td></tr> <tr> <td>H-I</td><td>0/50</td><td>-102.1</td><td>-102.1</td><td>0.16 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>I-B</td><td>-1187/0</td><td>0.0</td><td>0.0</td><td>0.31 (1)</td><td>7.29</td><td></td><td></td><td></td><td></td></tr> <tr> <td>J-H</td><td>-1187/0</td><td>0.0</td><td>0.0</td><td>0.31 (1)</td><td>7.29</td><td></td><td></td><td></td><td></td></tr> <tr> <td>R-Q</td><td>0/0</td><td>-38.5</td><td>-38.5</td><td>0.03 (3)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>Q-X</td><td>0/15</td><td>-38.5</td><td>-38.5</td><td>0.03 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>X-P</td><td>0/15</td><td>-38.5</td><td>-38.5</td><td>0.03 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>P-O</td><td>0/47</td><td>0.0</td><td>0.0</td><td>0.06 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>O-D</td><td>-563/0</td><td>0.0</td><td>0.0</td><td>0.06 (1)</td><td>7.81</td><td></td><td></td><td></td><td></td></tr> <tr> <td>Q-Y</td><td>0/1029</td><td>-38.5</td><td>-38.5</td><td>0.28 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>Y-N</td><td>0/1029</td><td>-38.5</td><td>-38.5</td><td>0.28 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>N-Z</td><td>0/1029</td><td>-38.5</td><td>-38.5</td><td>0.28 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>Z-AA</td><td>0/1029</td><td>-38.5</td><td>-38.5</td><td>0.28 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>AA-M</td><td>0/1029</td><td>-38.5</td><td>-38.5</td><td>0.28 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>L-M</td><td>0/41</td><td>0.0</td><td>0.0</td><td>0.05 (1)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>M-F</td><td>-563/0</td><td>0.0</td><td>0.0</td><td>0.06 (1)</td><td>7.81</td><td></td><td></td><td></td><td></td></tr> <tr> <td>L-AB</td><td>0/15</td><td>-38.5</td><td>-38.5</td><td>0.03 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>AB-K</td><td>0/15</td><td>-38.5</td><td>-38.5</td><td>0.03 (2)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> <tr> <td>K-J</td><td>0/0</td><td>-38.5</td><td>-38.5</td><td>0.03 (3)</td><td>10.00</td><td></td><td></td><td></td><td></td></tr> </table> FACTORED CONCENTRATED LOADS (LBS) <table> <tr> <th>JT</th><th>LOC.</th><th>LC1</th><th>MAX.</th><th>FACE</th><th>DIR.</th><th>TYPE</th><th>HEEL</th><th>CONN.</th></tr> <tr> <td>C</td><td>1-6-0</td><td>1</td><td>1</td><td>144</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td></tr> <tr> <td>D</td><td>3-10-4</td><td>1</td><td>1</td><td>144</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td></tr> <tr> <td>F</td><td>11-2-12</td><td>1</td><td>1</td><td>144</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td></tr> <tr> <td>G</td><td>13-7-0</td><td>1</td><td>1</td><td>144</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td></tr> <tr> <td>K</td><td>13-6-4</td><td>1</td><td>1</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td></tr> <tr> <td>M</td><td>11-5-4</td><td>-6</td><td>-7</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td></tr> <tr> <td>P</td><td>3-7-12</td><td>-6</td><td>-7</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td></tr> <tr> <td>Q</td><td>1-6-12</td><td>1</td><td>1</td><td>---</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>---</td></tr> </table>					FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG	JT	VERT	DOWN	UP	IN-SX	H	1208	0	0	5-8	J	1208	0	0	5-8	JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	DEAD	SOIL		COMBINED	SNOW	LIVE	PERM. LIVE	R	895	519/0	181/0	0/0	J	895	519/0	181/0	0/0	MEMB.	CHORDS	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CS (LC)	UNBRACED LENGTH	MEMB.	WEBS	FACTORED FORCE (LBS)	MAX. CS (LC)	FR-TO						FR-TO				A-B	0/50	-102.1	-102.1	0.16 (1)	10.00	C-C	-800/0	0.36 (1)		B-C	-448/0	-102.1	-102.1	0.15 (1)	6.25	O-O	0/369	0.07 (1)		C-S	-1016/0	-102.1	-102.1	0.14 (1)	6.02	C-O	0/1245	0.31 (1)		S-D	-1016/0	-102.1	-102.1	0.14 (1)	6.02	D-N	0/368	0.10 (1)		D-T	-1332/0	-102.1	-102.1	0.23 (1)	5.32	N-E	-422/0	0.09 (1)		T-E	-1332/0	-102.1	-102.1	0.23 (1)	5.32	E-F	0/368	0.10 (1)		E-U	-1332/0	-102.1	-102.1	0.23 (1)	5.32	M-K	0/369	0.07 (1)		U-V	-1332/0	-102.1	-102.1	0.23 (1)	5.32	M-G	0/1245	0.31 (1)		V-F	-1332/0	-102.1	-102.1	0.23 (1)	5.32	K-G	-800/0	0.36 (1)		F-W	-1016/0	-102.1	-102.1	0.14 (1)	6.02	B-Q	0/677	0.17 (1)		W-G	-1016/0	-102.1	-102.1	0.14 (1)	6.02	K-H	0/677	0.17 (1)		G-H	-448/0	-102.1	-102.1	0.15 (1)	6.25					H-I	0/50	-102.1	-102.1	0.16 (1)	10.00					I-B	-1187/0	0.0	0.0	0.31 (1)	7.29					J-H	-1187/0	0.0	0.0	0.31 (1)	7.29					R-Q	0/0	-38.5	-38.5	0.03 (3)	10.00					Q-X	0/15	-38.5	-38.5	0.03 (2)	10.00					X-P	0/15	-38.5	-38.5	0.03 (2)	10.00					P-O	0/47	0.0	0.0	0.06 (1)	10.00					O-D	-563/0	0.0	0.0	0.06 (1)	7.81					Q-Y	0/1029	-38.5	-38.5	0.28 (2)	10.00					Y-N	0/1029	-38.5	-38.5	0.28 (2)	10.00					N-Z	0/1029	-38.5	-38.5	0.28 (2)	10.00					Z-AA	0/1029	-38.5	-38.5	0.28 (2)	10.00					AA-M	0/1029	-38.5	-38.5	0.28 (2)	10.00					L-M	0/41	0.0	0.0	0.05 (1)	10.00					M-F	-563/0	0.0	0.0	0.06 (1)	7.81					L-AB	0/15	-38.5	-38.5	0.03 (2)	10.00					AB-K	0/15	-38.5	-38.5	0.03 (2)	10.00					K-J	0/0	-38.5	-38.5	0.03 (3)	10.00					JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.	C	1-6-0	1	1	144	BACK	VERT	TOTAL	---	D	3-10-4	1	1	144	BACK	VERT	TOTAL	---	F	11-2-12	1	1	144	BACK	VERT	TOTAL	---	G	13-7-0	1	1	144	BACK	VERT	TOTAL	---	K	13-6-4	1	1	---	BACK	VERT	TOTAL	---	M	11-5-4	-6	-7	---	BACK	VERT	TOTAL	---	P	3-7-12	-6	-7	---	BACK	VERT	TOTAL	---	Q	1-6-12	1	1	---	BACK	VERT	TOTAL	---	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 29.0 PSF DL = 6.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 52.5 PSF SPACING = 24.0 IN/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, CBC 2012 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (65 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.50") CALCULATED VERT. DEFL.(LL) = L/999 (0.04") ALLOWABLE DEFL.(TL) = L/360 (0.50") CALCULATED VERT. DEFL.(TL) = L/999 (0.07") CSI: TC=0.31/1.00 (B-R:1), BC=0.28/1.00 (N-O:2), WB=0.36/1.00 (G-K:1), SI=0.18/1.00 (D-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL) MAX MIN MAX MIN MAX MIN MT20 618 354 1697 768 1987 1658 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP = 0.83 (G) (INPUT = 0.80) JSI METAL = 0.32 (C) (INPUT = 1.00)			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG																																																																																																																																																																																																																																																																																																																																																																																																																																																																
JT	VERT	DOWN	UP	IN-SX																																																																																																																																																																																																																																																																																																																																																																																																																																																																
H	1208	0	0	5-8																																																																																																																																																																																																																																																																																																																																																																																																																																																																
J	1208	0	0	5-8																																																																																																																																																																																																																																																																																																																																																																																																																																																																
JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	DEAD	SOIL																																																																																																																																																																																																																																																																																																																																																																																																																																																																
	COMBINED	SNOW	LIVE	PERM. LIVE																																																																																																																																																																																																																																																																																																																																																																																																																																																																
R	895	519/0	181/0	0/0																																																																																																																																																																																																																																																																																																																																																																																																																																																																
J	895	519/0	181/0	0/0																																																																																																																																																																																																																																																																																																																																																																																																																																																																
MEMB.	CHORDS	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CS (LC)	UNBRACED LENGTH	MEMB.	WEBS	FACTORED FORCE (LBS)	MAX. CS (LC)																																																																																																																																																																																																																																																																																																																																																																																																																																																											
FR-TO						FR-TO																																																																																																																																																																																																																																																																																																																																																																																																																																																														
A-B	0/50	-102.1	-102.1	0.16 (1)	10.00	C-C	-800/0	0.36 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
B-C	-448/0	-102.1	-102.1	0.15 (1)	6.25	O-O	0/369	0.07 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
C-S	-1016/0	-102.1	-102.1	0.14 (1)	6.02	C-O	0/1245	0.31 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
S-D	-1016/0	-102.1	-102.1	0.14 (1)	6.02	D-N	0/368	0.10 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
D-T	-1332/0	-102.1	-102.1	0.23 (1)	5.32	N-E	-422/0	0.09 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
T-E	-1332/0	-102.1	-102.1	0.23 (1)	5.32	E-F	0/368	0.10 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
E-U	-1332/0	-102.1	-102.1	0.23 (1)	5.32	M-K	0/369	0.07 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
U-V	-1332/0	-102.1	-102.1	0.23 (1)	5.32	M-G	0/1245	0.31 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
V-F	-1332/0	-102.1	-102.1	0.23 (1)	5.32	K-G	-800/0	0.36 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
F-W	-1016/0	-102.1	-102.1	0.14 (1)	6.02	B-Q	0/677	0.17 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
W-G	-1016/0	-102.1	-102.1	0.14 (1)	6.02	K-H	0/677	0.17 (1)																																																																																																																																																																																																																																																																																																																																																																																																																																																												
G-H	-448/0	-102.1	-102.1	0.15 (1)	6.25																																																																																																																																																																																																																																																																																																																																																																																																																																																															
H-I	0/50	-102.1	-102.1	0.16 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
I-B	-1187/0	0.0	0.0	0.31 (1)	7.29																																																																																																																																																																																																																																																																																																																																																																																																																																																															
J-H	-1187/0	0.0	0.0	0.31 (1)	7.29																																																																																																																																																																																																																																																																																																																																																																																																																																																															
R-Q	0/0	-38.5	-38.5	0.03 (3)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
Q-X	0/15	-38.5	-38.5	0.03 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
X-P	0/15	-38.5	-38.5	0.03 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
P-O	0/47	0.0	0.0	0.06 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
O-D	-563/0	0.0	0.0	0.06 (1)	7.81																																																																																																																																																																																																																																																																																																																																																																																																																																																															
Q-Y	0/1029	-38.5	-38.5	0.28 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
Y-N	0/1029	-38.5	-38.5	0.28 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
N-Z	0/1029	-38.5	-38.5	0.28 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
Z-AA	0/1029	-38.5	-38.5	0.28 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
AA-M	0/1029	-38.5	-38.5	0.28 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
L-M	0/41	0.0	0.0	0.05 (1)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
M-F	-563/0	0.0	0.0	0.06 (1)	7.81																																																																																																																																																																																																																																																																																																																																																																																																																																																															
L-AB	0/15	-38.5	-38.5	0.03 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
AB-K	0/15	-38.5	-38.5	0.03 (2)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
K-J	0/0	-38.5	-38.5	0.03 (3)	10.00																																																																																																																																																																																																																																																																																																																																																																																																																																																															
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.																																																																																																																																																																																																																																																																																																																																																																																																																																																												
C	1-6-0	1	1	144	BACK	VERT	TOTAL	---																																																																																																																																																																																																																																																																																																																																																																																																																																																												
D	3-10-4	1	1	144	BACK	VERT	TOTAL	---																																																																																																																																																																																																																																																																																																																																																																																																																																																												
F	11-2-12	1	1	144	BACK	VERT	TOTAL	---																																																																																																																																																																																																																																																																																																																																																																																																																																																												
G	13-7-0	1	1	144	BACK	VERT	TOTAL	---																																																																																																																																																																																																																																																																																																																																																																																																																																																												
K	13-6-4	1	1	---	BACK	VERT	TOTAL	---																																																																																																																																																																																																																																																																																																																																																																																																																																																												
M	11-5-4	-6	-7	---	BACK	VERT	TOTAL	---																																																																																																																																																																																																																																																																																																																																																																																																																																																												
P	3-7-12	-6	-7	---	BACK	VERT	TOTAL	---																																																																																																																																																																																																																																																																																																																																																																																																																																																												
Q	1-6-12	1	1	---	BACK	VERT	TOTAL	---																																																																																																																																																																																																																																																																																																																																																																																																																																																												



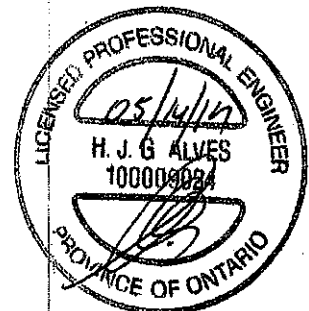
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403060	T18	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 S Apr 23 2019 Mtek Industries, Inc. Tue May 14 11:25:33 2019 Page 2
ID:vk I5VXL54FY8pNpe87hlyMICK-izYJO73FR1pDh15QI2SNATFLPNH1v zT3VhXHzGcaG

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	3-0-12	1	1	144	BACK	VERT	TOTAL	---	---
T	5-10-4	1	1	144	BACK	VERT	TOTAL	---	---
U	7-10-4	1	1	144	BACK	VERT	TOTAL	---	---
V	9-2-12	1	1	144	BACK	VERT	TOTAL	---	---
W	12-0-4	1	1	144	BACK	VERT	TOTAL	---	---
X	3-0-12	1	1	---	BACK	VERT	TOTAL	---	---
Y	5-10-4	1	1	---	BACK	VERT	TOTAL	---	---
Z	7-10-4	1	1	---	BACK	VERT	TOTAL	---	---
AA	9-2-12	1	1	---	BACK	VERT	TOTAL	---	---
AB	12-0-4	1	1	---	BACK	VERT	TOTAL	---	---

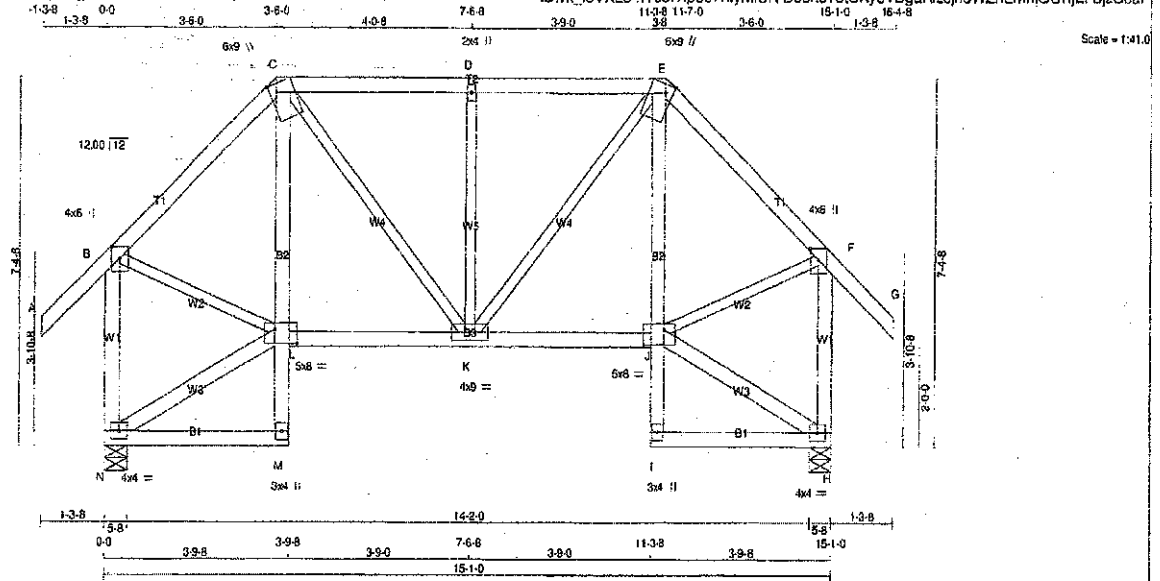


Structural component only
DWG# T-1911509-2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403060	T19	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 5 Apr 23 2019 MTEK Industries, Inc. Tue May 14 11:25:34 2019 Page 1
ID:vk_15VXL54Y8orNpe87hlyMICK-D99hcT3tCKy3VBgaRlzcjhoWZnEmmOS7jEF3zGcaF



TOTAL WEIGHT = 94 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
N - M	2x4	DRY	No.2	SPF	
M - C	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
I - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - L	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	6.0	2.75	2.00
C TTVW+m	MT20	6.0	9.0	Edge	1.75
D TMVW+w	MT20	2.0	4.0		
E TTVW+m	MT20	6.0	9.0	Edge	1.75
F TMVW+p	MT20	4.0	6.0	2.75	2.00
H BMVW1-i	MT20	4.0	4.0		
J BMVW+p	MT20	3.0	4.0		
K BMVWVW-i	MT20	5.0	8.0	3.50	5.50
L BMVWVW-i	MT20	4.0	9.0		
M BMVW+p	MT20	5.0	8.0	3.50	5.50
N BMVW1-i	MT20	3.0	4.0		
H BMVW1-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
N	1203	0	1203	0
H	1203	0	1203	0

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	890	519 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0
H	890	519 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	LC2 MAX (LBS)
FR-TO		FROM	TO	LENGTH
A-B	0 / 50	-102.1	-102.1	0.14 (1)
B-C	-823 / 0	-102.1	-102.1	0.23 (1)
C-D	-837 / 0	-102.1	-102.1	0.28 (1)
D-E	-837 / 0	-102.1	-102.1	0.23 (1)
E-F	-823 / 0	-102.1	-102.1	0.23 (1)
F-G	0 / 50	-102.1	-102.1	0.14 (1)
N-B	-1130 / 0	0.0	0.0	0.29 (1)
H-F	-1130 / 0	0.0	0.0	0.29 (1)
N-M	0 / 4	-38.5	-38.5	0.12 (3)
M-L	0 / 89	0.0	0.0	0.02 (2)
L-C	-82 / 57	0.0	0.0	0.02 (3)
L-K	0 / 578	-38.5	-38.5	0.20 (2)
K-J	0 / 578	-38.5	-38.5	0.20 (2)
J-I	0 / 89	0.0	0.0	0.02 (2)
J-E	-82 / 67	0.0	0.0	0.02 (3)
I-H	0 / 4	-38.5	-38.5	0.12 (3)



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OSC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.29/1.00 (B-N:1), BC=0.20/1.00 (J-K:2), WB=0.21/1.00 (D-K:1), SS=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MT20	618	354 1667 788 1987 1656

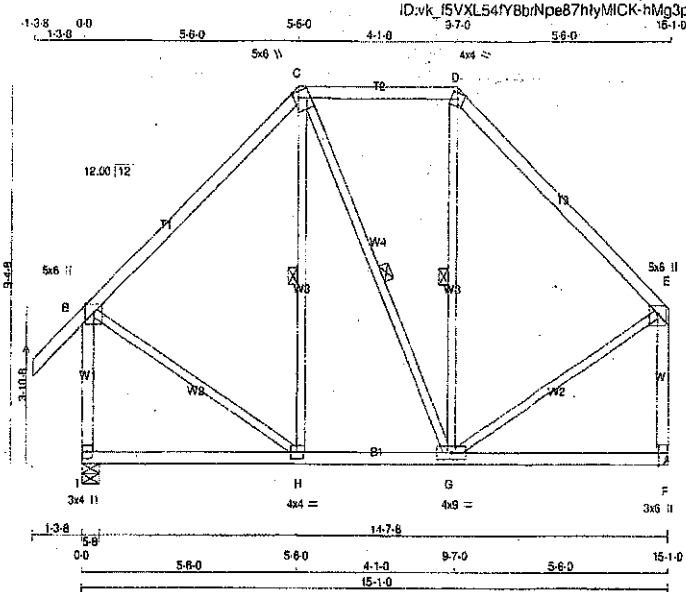
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP = 0.78 (F) (INPUT = 0.90)

SI METAL = 0.39 (F) (INPUT = 1.00)

Structural component only
DWG# T-1911510



Scale = 1/50.0

LUMBER TOTAL WEIGHT = 3 X 83 = 249 lb (M/F)

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - E	2x4 DRY	No.2	SPF
E - F	2x4 DRY	No.2	SPF
F - G	2x4 DRY	No.2	SPF
G - H	2x4 DRY	No.2	SPF
H - I	2x4 DRY	No.2	SPF
I - F	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+1	MT20	3.0	6.0	Edge	0.50
G	BMVWV-1	MT20	4.0	9.0		
H	BMVWV-1	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	BRG	BRG	IN-SX	IN-SX
I	1203	0	1203	0	0	5-8	5-8		
F	1080	0	1080	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS	DEAD	SOIL
I	890	519 / 0	158 / 0	0 / 0	0 / 0	213 / 0
F	792	438 / 0	158 / 0	0 / 0	0 / 0	196 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-H, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	CS (LC)	MEMB.	FORCE (LBS)	MAX	CS (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 50	-102.1	-102.1 0.14 (1)	10.00	H-C	-65 / 128	0.04 (1)
B-C	-598 / 0	-102.1	-102.1 0.40 (1)	6.25	C-G	0 / 0	0.00 (1)
C-D	-421 / 0	-102.1	-102.1 0.22 (1)	6.25	G-D	-66 / 129	0.04 (1)
D-E	-598 / 0	-102.1	-102.1 0.40 (1)	6.25	B-H	0 / 498	0.11 (1)
E-F	-1112 / 0	0.0	0.0 0.25 (1)	7.51	G-E	0 / 498	0.11 (1)
F-E	-989 / 0	0.0	0.0 0.25 (1)	7.81			
I-H	0 / 0	-38.5	-38.5 0.20 (3)	10.00			
H-G	0 / 421	-38.5	-38.5 0.23 (2)	10.00			
G-F	0 / 0	-38.5	-38.5 0.20 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CS: TC=0.40/1.00 (B-C:1), BC=0.23/1.00 (G-H:2), WB=0.11/1.00 (B-H:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

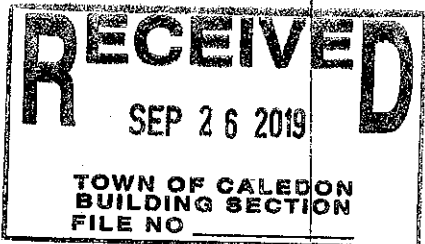
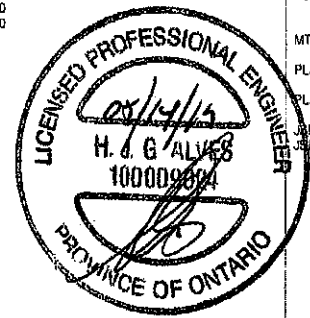
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	
MAX	MIN	MAX	MIN
MT20	618	354	1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.57 (B) (INPUT = 0.90)
METAL = 0.27 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911511



TOTAL WEIGHT = 85 lb

DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BOILING BEARINGS

A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/50	-102.1 -102.1	0.14 (1)	10.00	J-C	-240/117	0.23 (1)
B-C	-593/0	-102.1 -102.1	0.22 (1)	5.25	C-I	0/368	0.08 (1)
C-D	-596/0	-102.1 -102.1	0.27 (1)	8.25	I-D	-500/0	0.48 (1)
D-E	-596/0	-102.1 -102.1	0.27 (1)	6.25	E-E	0/368	0.08 (1)
E-F	-593/0	-102.1 -102.1	0.22 (1)	6.25	H-E	-240/17	0.23 (1)
K-B	-1150/9	0.0	0.0	7.42	B-J	0/583	0.13 (1)
G-F	-1007/0	0.0	0.0	2.66 (1)	H-F	0/583	0.13 (1)
K-J	0/0	-38.5	-38.5	0.10 (3)	10.00		
J-I	0/414	-38.5	-38.5	0.16 (2)	10.00		
I-H	0/414	-38.5	-38.5	0.16 (2)	10.00		
H-G	0/0	-38.5	-38.5	0.10 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.50")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.50")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.29/1.00 (B-K:1), BC=0.16/1.00 (H-I:2),
WB=0.48/1.00 (D-I:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

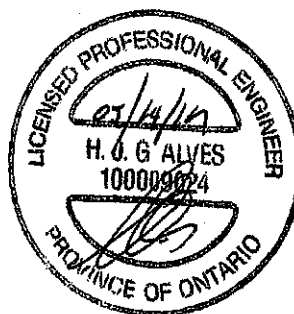
NAIL VALUES						
PLATE	GRIP (DRY) (PSI)		SHEAR (PL)		SECTION (PL)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1887	788	1987	1656

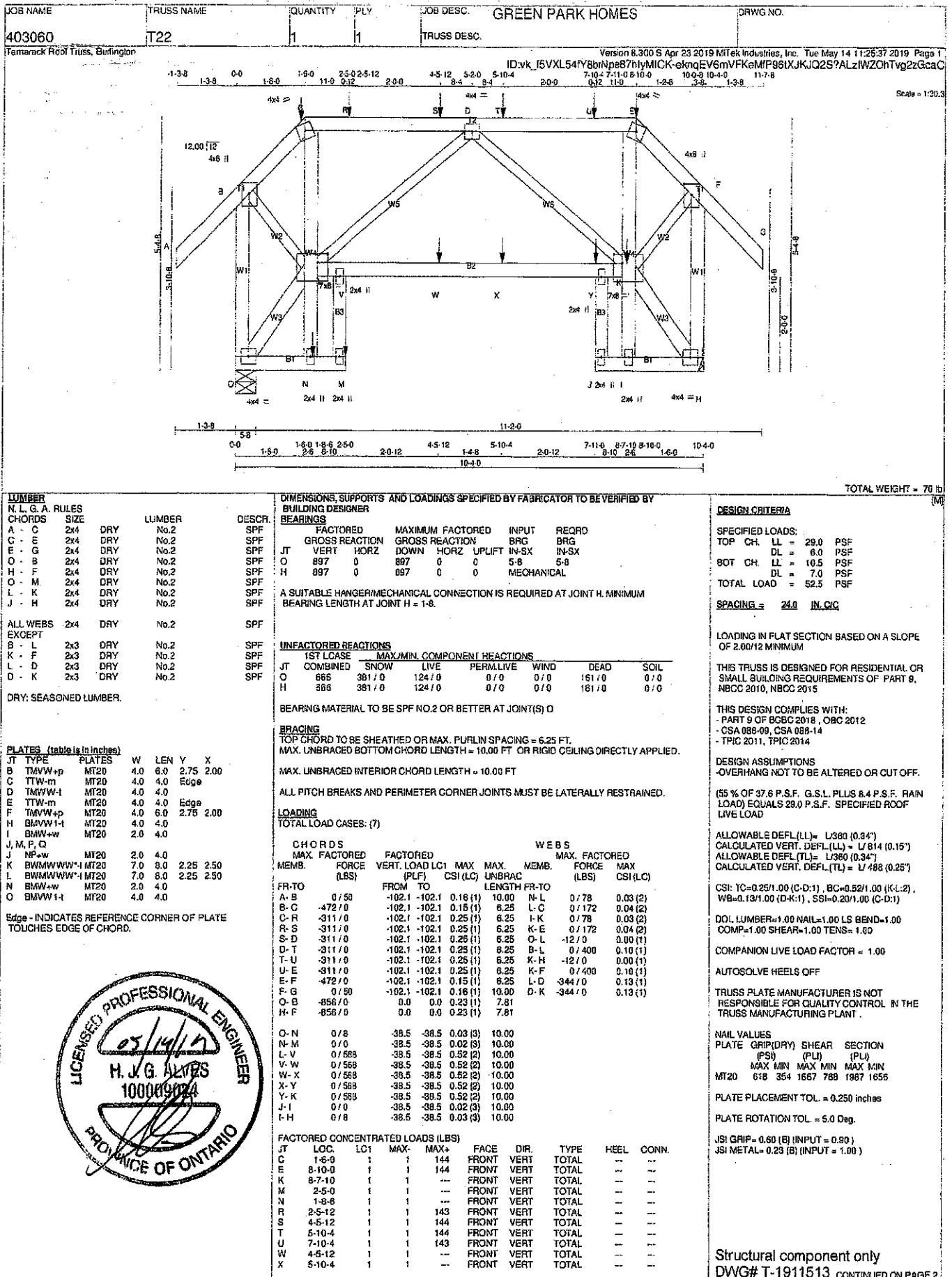
PLATE PLACEMENT TOL = 0.250 inches

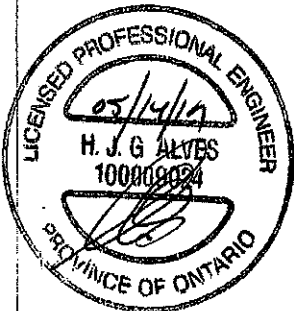
PLATE ROTATION TOL. = 5.0 Deg.

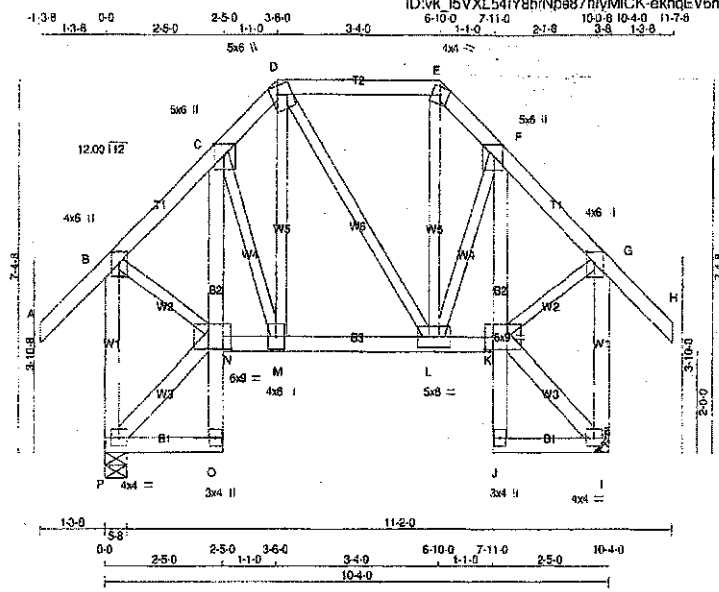
JSI GRIP= 0.81 (F) (INPUT = 0.80)
JSI METAL= 0.30 (B) (INPUT = 1.00)

Structural component only
DWG# T-1911512





JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.																																	
403060	T22	1	1	TRUSS DESC.																																			
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:25:37 2019 Page 2 ID:vk_15VXL54fY8brNpe87hlyMICK-ekngEV6mVFKemIP98IXJKJO2S?ALzIWZOhtvg2zGcaC																																			
<table border="1"><thead><tr><th colspan="6">FACTORED CONCENTRATED LOADS (LBS)</th><th>FACE</th><th>DIR.</th><th>TYPE</th><th>HEEL</th><th>CONN.</th></tr><tr><th>JT</th><th>LOC.</th><th>LC1</th><th>MAX</th><th>MAX+</th><th></th><th>FRONT</th><th>VERT</th><th>TOTAL</th><th>--</th><th>--</th></tr></thead><tbody><tr><td>Y</td><td>7-11-0</td><td>1</td><td>1</td><td>--</td><td></td><td></td><td></td><td></td><td></td><td></td></tr></tbody></table>							FACTORED CONCENTRATED LOADS (LBS)						FACE	DIR.	TYPE	HEEL	CONN.	JT	LOC.	LC1	MAX	MAX+		FRONT	VERT	TOTAL	--	--	Y	7-11-0	1	1	--						
FACTORED CONCENTRATED LOADS (LBS)						FACE	DIR.	TYPE	HEEL	CONN.																													
JT	LOC.	LC1	MAX	MAX+		FRONT	VERT	TOTAL	--	--																													
Y	7-11-0	1	1	--																																			
 Structural component only DWG# T-1911513-2																																							



Scale = 1/4\"/>

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - C	2x4	DRY	No.2
N - K	2x4	DRY	No.2
J - F	2x4	DRY	No.2
J - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
P - N	2x4	DRY	No.2
K - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

PLATE	W	LEN	Y	X
B - TMVW+p	MT20	4.0	6.0	2.75
C - TMVW+p	MT20	5.0	6.0	Edge 2.25
D - TTW+m	MT20	5.0	6.0	2.00
E - TTW-m	MT20	4.0	4.0	Edge
F - TMVW+p	MT20	5.0	6.0	Edge 2.25
G - TMVW+p	MT20	4.0	5.0	2.75
I - BMVW-1	MT20	4.0	4.0	
J - BMV+p	MT20	3.0	4.0	
K - BVMWV-1	MT20	6.0	9.0	4.50
L - BVMWV-1	MT20	5.0	8.0	
M - BVMWV-1	MT20	4.0	6.0	
N - BVMWV-1	MT20	6.0	9.0	4.50
O - BMV+p	MT20	3.0	4.0	
P - BVMWV-1	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	DOWN	IN-SX	IN-SX
HORIZ	UPLIFT	5-B	5-B
MECHANICAL	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-B.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
COMBINED	SNOW	LIVE
PERM.LIVE	WIND	DEAD
SOIL		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LC)
FR-TO					FR-TO		
A-B	0/50	-102.1	-102.1	0.14 (1)	10.00	C-M	-106/0
B-C	-509/0	-102.1	-102.1	0.06 (1)	8.25	M-D	0/163
C-D	-502/0	-102.1	-102.1	0.04 (1)	6.25	D-L	0/2
D-E	-359/0	-102.1	-102.1	0.15 (1)	3.25	L-E	0/165
E-F	-504/0	-102.1	-102.1	0.04 (1)	3.25	L-F	-103/0
F-G	-509/0	-102.1	-102.1	0.06 (1)	8.25	P-N	-5/0
G-H	0/50	-102.1	-102.1	0.14 (1)	10.00	B-N	0/425
P-B	-821/0	0.0	0.0	0.21 (1)	7.81	K-I	-5/0
I-G	-821/0	0.0	0.0	0.21 (1)	7.81	K-G	0/425
P-O	0/4	-38.5	-38.5	0.05 (3)	10.00		
O-N	0/58	0.0	0.0	0.02 (2)	10.00		
N-C	-102/0	0.0	0.0	0.01 (1)	7.81		
N-M	0/373	-38.5	-38.5	0.09 (2)	10.00		
M-L	0/338	-38.5	-38.5	0.09 (2)	10.00		
L-K	0/372	-38.5	-38.5	0.10 (2)	10.00		
J-K	0/58	0.0	0.0	0.02 (2)	10.00		
K-F	-105/0	0.0	0.0	0.01 (1)	7.81		
J-I	0/4	-38.5	-38.5	0.05 (3)	10.00		

TOTAL WEIGHT = 81 LB

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 9.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.21/1.00 (G-I-1), BC=0.10/1.00 (K-L-2), WB=0.10/1.00 (G-K-1), SSI=0.13/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

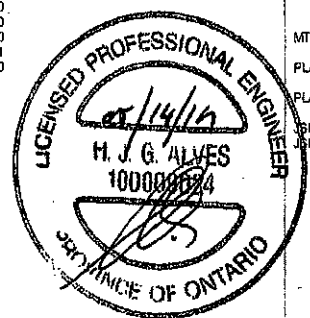
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP=0.58 (G) (INPUT = 0.80)
SI METAL=0.26 (G) (INPUT = 1.00)



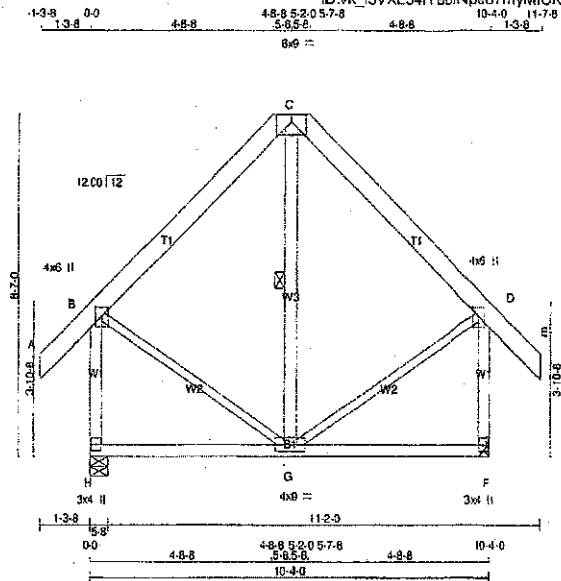
Structural component only
DWG# T-1911514

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403060	T24	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:vk_15VXL54YBbrNpe87hlyMICK-6xLCRr6OGZSV_pzMgb2YUyDDOb4IEpidLCSUzGcaB



Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 71 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT G - C	2x3	DRY	No.2	SPF
	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVWm	MT20	6.0	9.0	2.25	4.50
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-1	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORIZ	DOWN	HORIZ		
H	875	0	875	0	5-8	5-8
F	875	0	875	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	645	384/0	109/0	0/0	0/0	152/0	0/0	0/0
F	645	384/0	109/0	0/0	0/0	152/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0/52	-102.1	-102.1	0.08 (1)	10.00	B-G	0/280	0.06 (1)	
B-C	-330/0	-102.1	-102.1	0.17 (1)	6.25	G-D	0/280	0.06 (1)	
C-D	-330/0	-102.1	-102.1	0.17 (1)	6.25	G-C	-51/159	0.03 (3)	
D-E	0/52	-102.1	-102.1	0.08 (1)	10.00				
H-B	-799/0	0.0	0.0	0.18 (1)	7.81				
F-D	-799/0	0.0	0.0	0.18 (1)	7.81				
H-G	0/0	-38.5	-38.5	0.24 (3)	10.00				
G-F	0/0	-38.5	-38.5	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $L/998$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $L/998$ (0.04")

CSI: TC=0.18/1.00 (B-H:1), BC=0.24/1.00 (G-H:3),
WB=0.05/1.00 (B-G:1), SSI=0.13/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

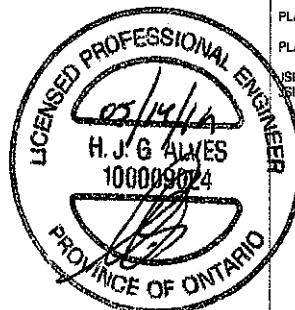
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

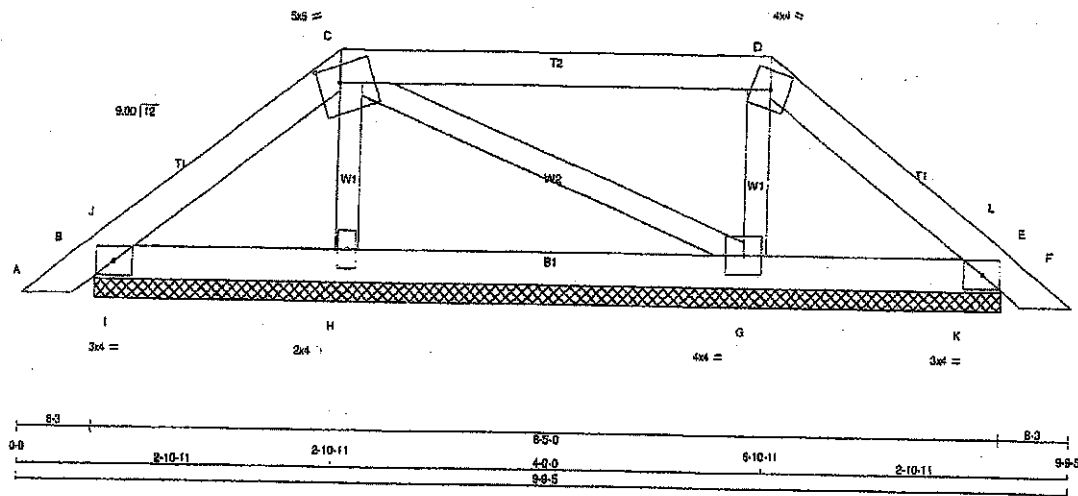
PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.56 (B) (INPUT = 0.90)
SI METAL= 0.23 (D) (INPUT = 1.00)



Structural component only
DWG# T-1911515

JOB NAME 403045	TRUSS NAME P1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.300 S Apr 23 2019 MTEK Industries, Inc. Tue May 14 10:41:17 2019 Page 1 ID:D7uPnW9?DzGkx1HTFaFreyNCCH-W73EyyvEx2B3E12xHeQYXNFSKRWkR1kr68IDzGdDm					
Scale = 1:17.9					



LUMBER
N.L.G.A. RULES
CHORDS SIZE LUMBER

A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	257	0	257	0	8-5-0	8-5-0
E	243	0	243	0	8-5-0	8-5-0
H	366	0	366	0	8-5-0	8-5-0
G	417	0	417	0	8-5-0	8-5-0

TOTAL WEIGHT = 2 X 26 = 52 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 20.0 PSF
DL = 6.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	IMB14	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	8.0	1.75	2.25
D	TTWW-m	MT20	4.0	4.0		
E	IMB14	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMWW1-w	MT20	2.0	4.0		

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	183	130/0	16/0	0/0	0/0	37/0	0/0
E	173	122/0	16/0	0/0	0/0	35/0	0/0
H	295	144/0	73/0	0/0	0/0	78/0	0/0
G	318	161/0	73/0	0/0	0/0	82/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO	
A-B	0/17	-102.1 -102.1	0.03 (1)	10.00	H-C	-232/0 0.03 (1)
B-J	-75/0	-102.1 -102.1	0.02 (1)	6.25	C-G	-21/0 0.01 (1)
J-C	-98/0	-102.1 -102.1	0.08 (1)	6.25	G-D	-255/0 0.04 (1)
C-D	-42/0	-102.1 -102.1	0.28 (1)	6.25	I-J	-146/0 0.00 (1)
D-L	-75/0	-102.1 -102.1	0.06 (1)	6.25	K-L	-146/0 0.00 (1)
L-E	-51/0	-102.1 -102.1	0.02 (1)	6.25		
E-F	0/17	-102.1 -102.1	0.03 (1)	10.00		
B-I	0/75	-38.5 -38.5	0.07 (1)	10.00		
I-H	0/75	-38.5 -38.5	0.08 (2)	10.00		
H-G	0/91	-38.5 -38.5	0.08 (2)	10.00		
G-K	0/57	-38.5 -38.5	0.08 (2)	10.00		
K-E	0/57	-38.5 -38.5	0.08 (1)	10.00		

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(85% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (C-D:1), RC=0.08/1.00 (H:2), WB=0.04/1.00 (D-G:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

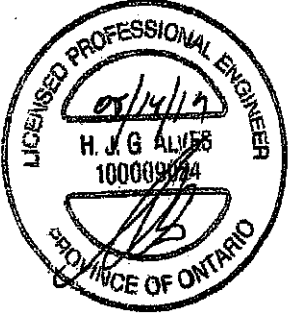
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1697

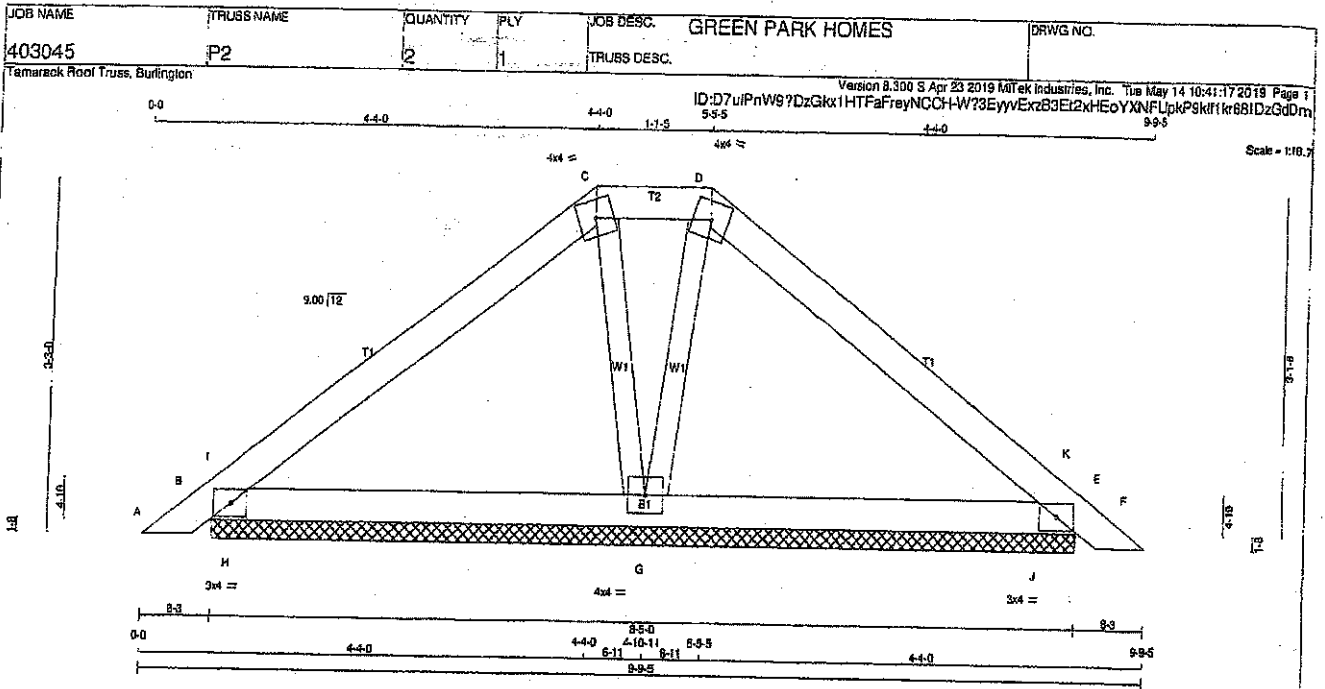
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.80)
JSI METAL = 0.05 (H) (INPUT = 1.80)



Structural component only
DWG# T-1911479



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1-I	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
B	401	0	401	0	8-5-0	8-5-0
E	380	0	380	0	8-5-0	8-5-0
G	514	0	514	0	8-5-0	8-5-0

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM LIVE			
B	293	182/0	43/0	0/0	0/0	57/0	0/0
E	285	179/0	42/0	0/0	0/0	55/0	0/0
G	390	197/0	52/0	0/0	0/0	102/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED MOMENT (LBS-FT)
FR-TO		FROM TO			FR-TO		
A-B	8 / 17	-102.1 -102.1	0.03 (1)	10.00	C-G	-127 / 0	0.02 (1)
B-I	-117 / 0	-102.1 -102.1	0.05 (1)	6.25	G-D	-129 / 0	0.02 (1)
I-C	-165 / 0	-102.1 -102.1	0.18 (1)	6.25	H-I	-304 / 76	0.00 (1)
C-D	-98 / 0	-102.1 -102.1	0.02 (1)	6.25	J-K	-327 / 56	0.00 (1)
D-K	-157 / 0	-102.1 -102.1	0.17 (1)	6.25			
K-E	-103 / 0	-102.1 -102.1	0.06 (1)	6.25			
E-F	0 / 17	-102.1 -102.1	0.03 (1)	10.00			
B-H	0 / 126	-38.5 -38.5	0.14 (1)	10.00			
H-G	0 / 126	-38.5 -38.5	0.20 (2)	10.00			
G-J	0 / 119	-38.5 -38.5	0.19 (2)	10.00			
J-E	0 / 119	-38.5 -38.5	0.15 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2015
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

65% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-I), SC=0.20/1.00 (B-H-2), WB=0.02/1.00 (D-G-1), SS=0.20/1.00 (E-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

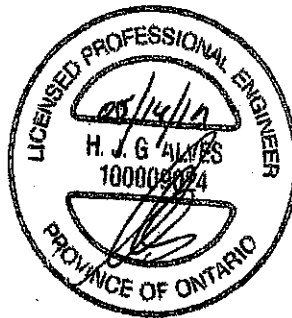
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

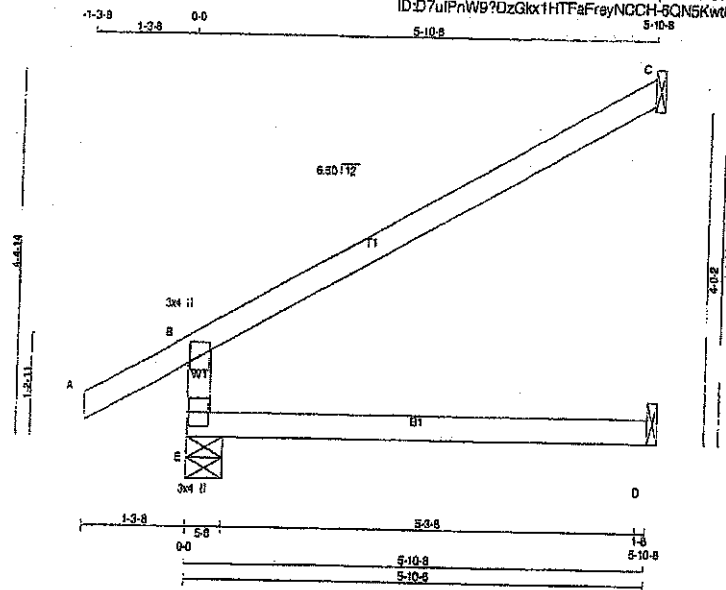
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.30 (B) (INPUT = 0.80)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911480

JOB NAME 403045	TRUSS NAME U1	QUANTITY 17	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:14 2019 Page 1 ID:D7uIPnW9?DzGkx1HTFaFreyNCOH-5QNSKwtMe2pVNPJMo6FrvdsxWNBXOhb1tuUhzGdDp		



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	3MV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	846	0	846	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	94	0	119	0	1-8	1-8

SEE MITER STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	473	282 / 0	72 / 0	0 / 0	0 / 0	109 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0
D	65	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	WEBS MAX. FACTORED FORCE (LBS)	
			FROM TO	FROM TO
FR-TO				
E-B	-514 / 0	0.0	0.0	0.21 (3)
A-B	0 / 23	-102.1	-102.1	0.13 (1)
B-C	-36 / 0	-102.1	-102.1	0.80 (1)
E-D	0 / 0	-38.5	-38.5	0.21 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 37.5 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/699 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/930 (0.08")

CS: TD=0.00/1.00 (B-C:1), BC=0.21/1.00 (D-E:3), WB=0.00/1.00 (B-C:1), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

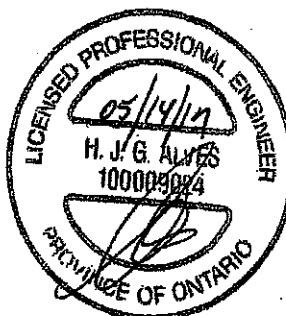
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 798 1987 1856

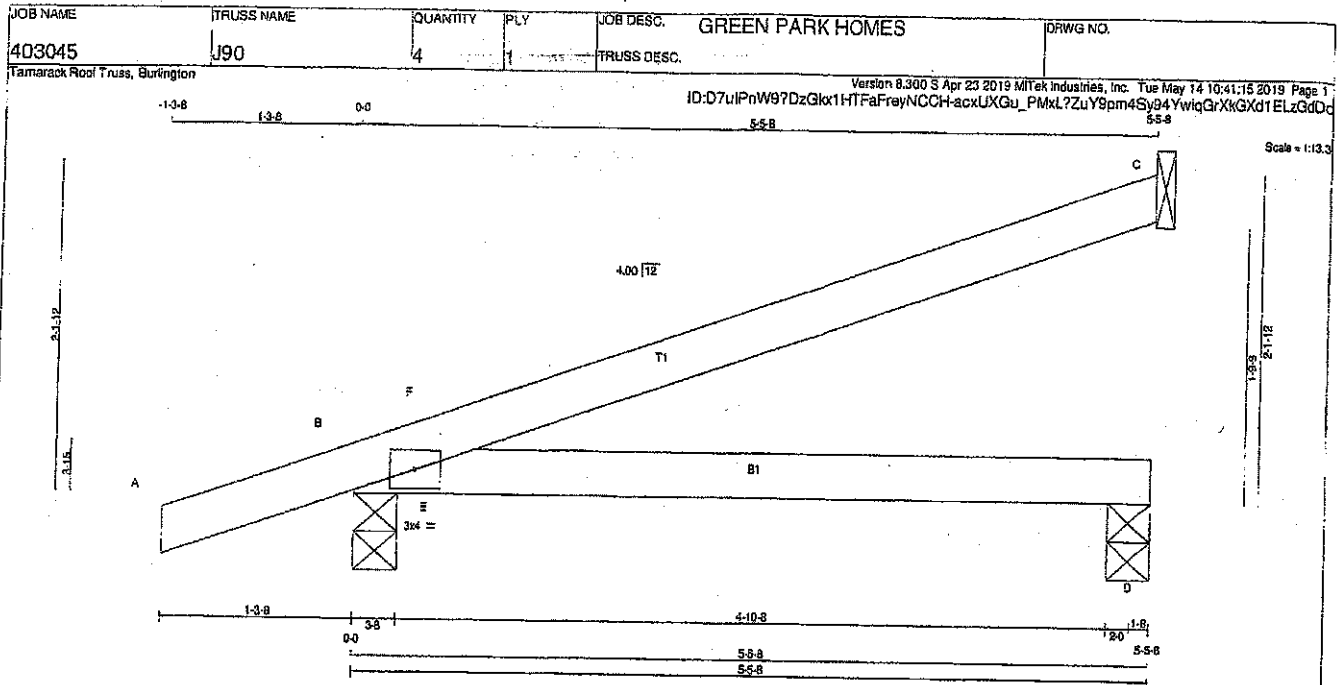
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.22 (B) (INPUT = 0.90)
JSI METAL = 0.15 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911476



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
A - C 2x4 DRY No.2
B - D 2x4 DRY No.2
DRY, SEASONED LUMBER.

PLATES (tablets in inches)
JT TYPE PLATES W LEN Y X
B TMB1-1 MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	RECORD
C	250	0	250	0	0	1-8	1-8	1-8
B	520	0	520	0	0	3-8	3-8	3-8
D	133	0	145	0	0	3-8	3-8	3-8

SEE MITEK STANDARD DETAIL B9779 IH FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	32 / 0	0 / 0
B	380	235 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO					FR-TO		
A-B	0 / 20	-102.1	-102.1	0.13 (1)	E-F	-254 / 122	0.00 (1)
B-F	-51 / 0	-102.1	-102.1	0.13 (3)			
F-C	0 / 8	-102.1	-102.1	0.42 (1)			
B-E	0 / 0	-38.5	-38.5	0.28 (1)			
E-D	0 / 0	-38.5	-38.5	0.31 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF CBC 2018, CBC 2012
CSA 086-09, CSA 086-14
TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/842$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/469$ (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.28/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

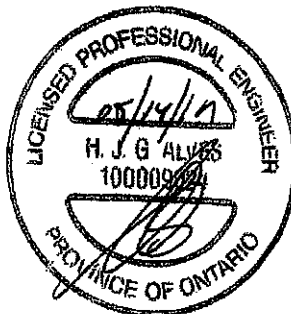
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
	MAX	MIN	MAX
MT20	618	354	1687

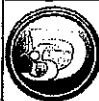
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.31 (B) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911477



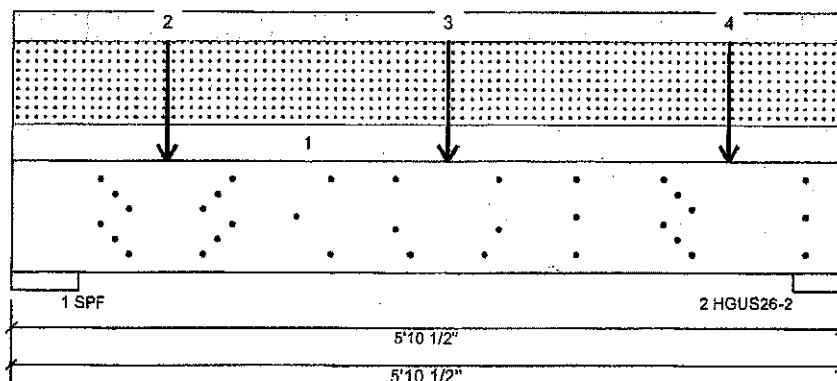
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Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 1 of 13

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	376	503	1210	0
2	314	415	1001	0

Bearings and Factored Reactions

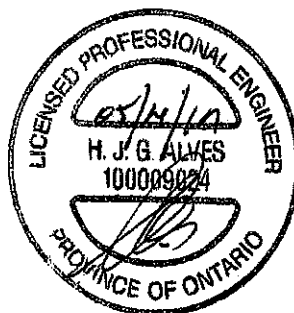
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	28%	629 / 2190	2819 L
2 - HGUS...	4.000"	32%	519 / 1815	2334 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2982 ft-lb	3'1"	6039 ft-lb	0.494 (49%)	1.25D+1.5S	L
Unbraced	2982 ft-lb	3'1"	5236 ft-lb	0.570 (57%)	1.25D+1.5S	L
Shear	2763 lb	1'2"	3984 lb	0.694 (69%)	1.25D+1.5S	L
LL Defl inch	0.025 (L/2459)	2'11 9/16"	0.174 (L/360)	0.150 (15%)	S+0.5L	L
TL Defl inch	0.035 (L/1808)	2'11 9/16"	0.174 (L/360)	0.200 (20%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

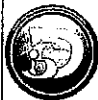
DWG NO. TAM1911517
STRUCTURAL
COMPONENT ONLY 1/3

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	1-1-0		Far Face	350 lb	256 lb	848 lb	0 lb	
3	Point	3-1-0		Far Face	208 lb	155 lb	508 lb	0 lb	
4	Point	5-1-0		Far Face	208 lb	155 lb	508 lb	0 lb	

Manufacturer InfoTamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

This design is valid until 12/11/2021





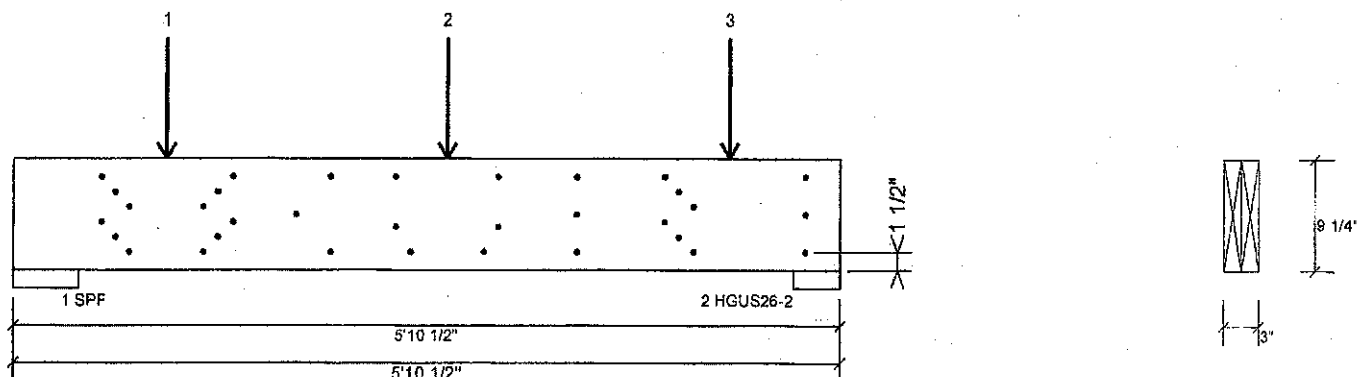
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 2 of 13

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Multi-Ply Analysis**

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

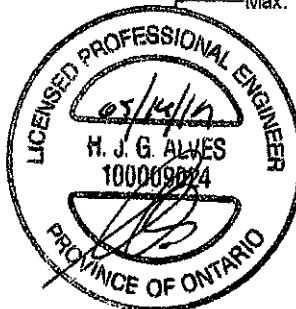
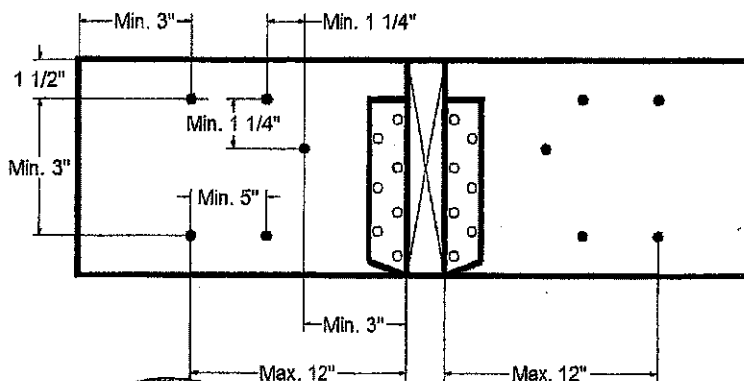
Fasten at concentrated side load at 1-1-0 with a minimum of (12) – Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	72.3 %
Load	982.8lb.
Total Yield Limit	1360.2 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 3-1-0 with a minimum of (6) – Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	86.5 %
Load	588.5lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads

DWG NO. TAM 1911517
STRUCTURAL
COMPONENT ONLY 2/3

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
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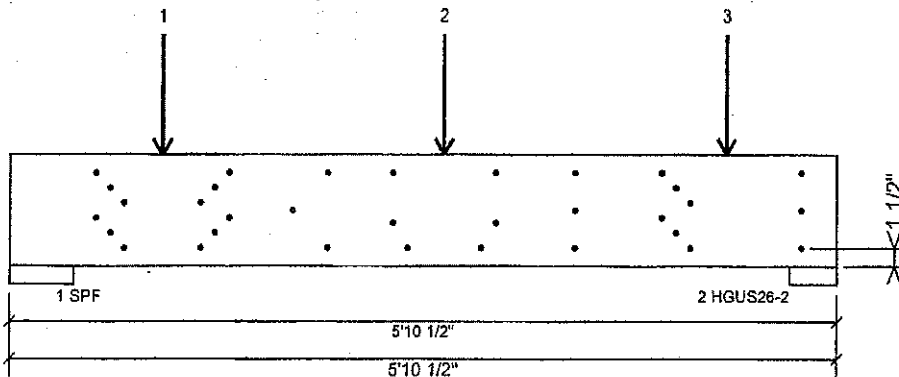
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

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B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c., except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

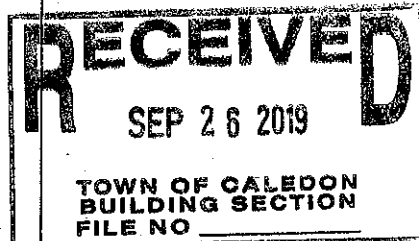
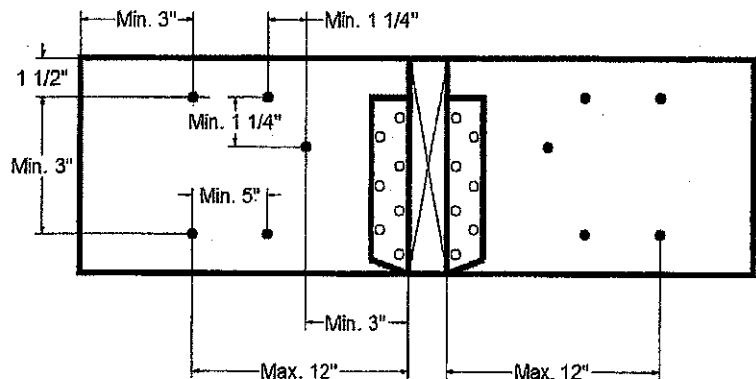
Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 5-1-0 with a minimum of (6) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	88.5 %
Load	588.5lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



DWG NO. TAM 1911517
STRUCTURAL
COMPONENT ONLY 3/3

Manufacturer Info

Tamarack Roof Trusses
3260 North Service Rd., ON
Canada
L7N3G2
(805) 335-1116



This design is valid until 12/11/2021



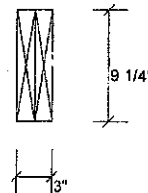
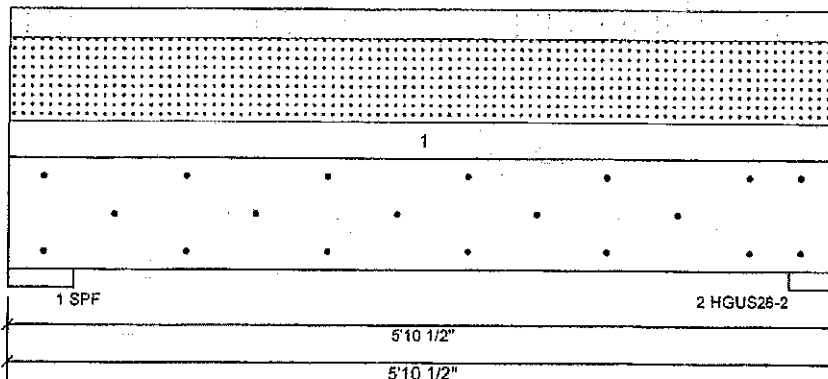
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

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B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	269	333	756	0
2	258	319	724	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	18% 416 / 1403	1819 L	1.25D+1.5S +L
2 - HGUS...	4.000"	24% 399 / 1345	1744 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2056 ft-lb	3'	6039 ft-lb	0.341 (34%)	1.25D+1.5S +L	L
Unbraced	2056 ft-lb	3'	5236 ft-lb	0.393 (39%)	1.25D+1.5S +L	L
Shear	1579 lb	1'2"	3884 lb	0.396 (40%)	1.25D+1.5S +L	L
LL Defl inch	0.018 (L/3466)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.025 (L/2523)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1194521
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-6-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info		Tamarack Roof Trusses 3269 North Service Rd., ON Canada L7H3G2 (905) 335-1115
		TAMARACK LUMBER INC ALMA LUMBER GROUP

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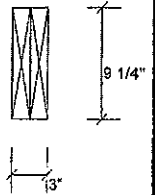
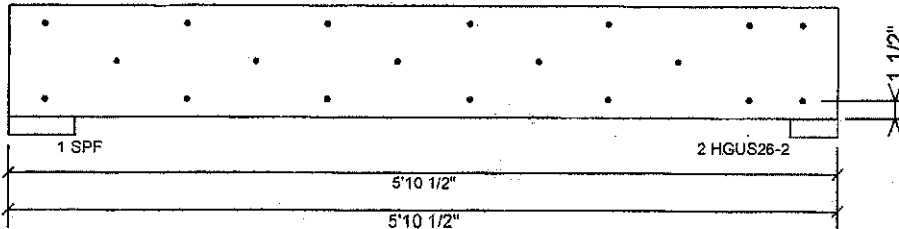
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

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B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	89.2 %
Load	303.2 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. 1AM 11911521
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3200 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





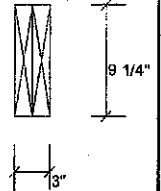
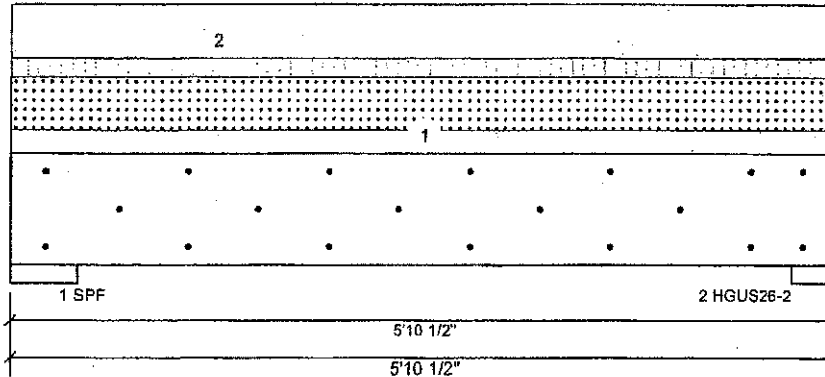
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Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 9 of 13

B6 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	194	331	546	0
2	186	317	523	0

Bearings and Factored Reactions

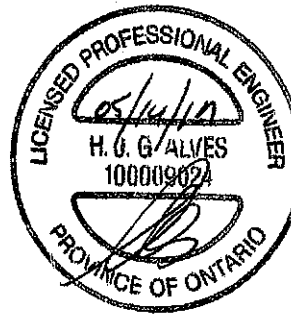
Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5,600"	14%	413 / 1013	1426	L	1.25D+1.5S +L
2 - HGUS...	4,000"	19%	396 / 971	1367	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1612 ft-lb	3'	6039 ft-lb	0.267 (27%)	1.25D+1.5S +L	L
Unbraced	1612 ft-lb	3'	5236 ft-lb	0.308 (31%)	1.25D+1.5S +L	L
Shear	1209 lb	12"	3984 lb	0.303 (30%)	1.25D+1.5S +L	L
LL Defl inch	0.013 (L4801)	3'	0.174 (L/360)	0.070 (7%)	S+0.5L	L
TL Defl inch	0.020 (L/3171)	3'	0.174 (L/360)	0.110 (11%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 11911520
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-2-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Uniform			Top	30 PLF	0 PLF	0 PLF	0 PLF	

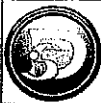
Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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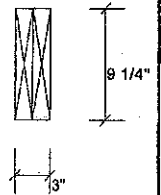
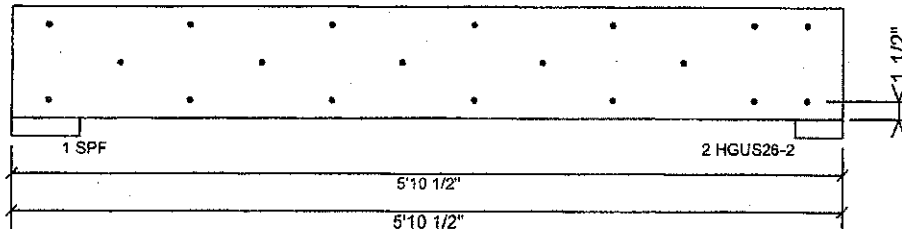
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 9 of 13

B6 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	64.4 %
Load	218.9 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17711520
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N6G2
(805) 335-1115



This design is valid until 12/11/2021

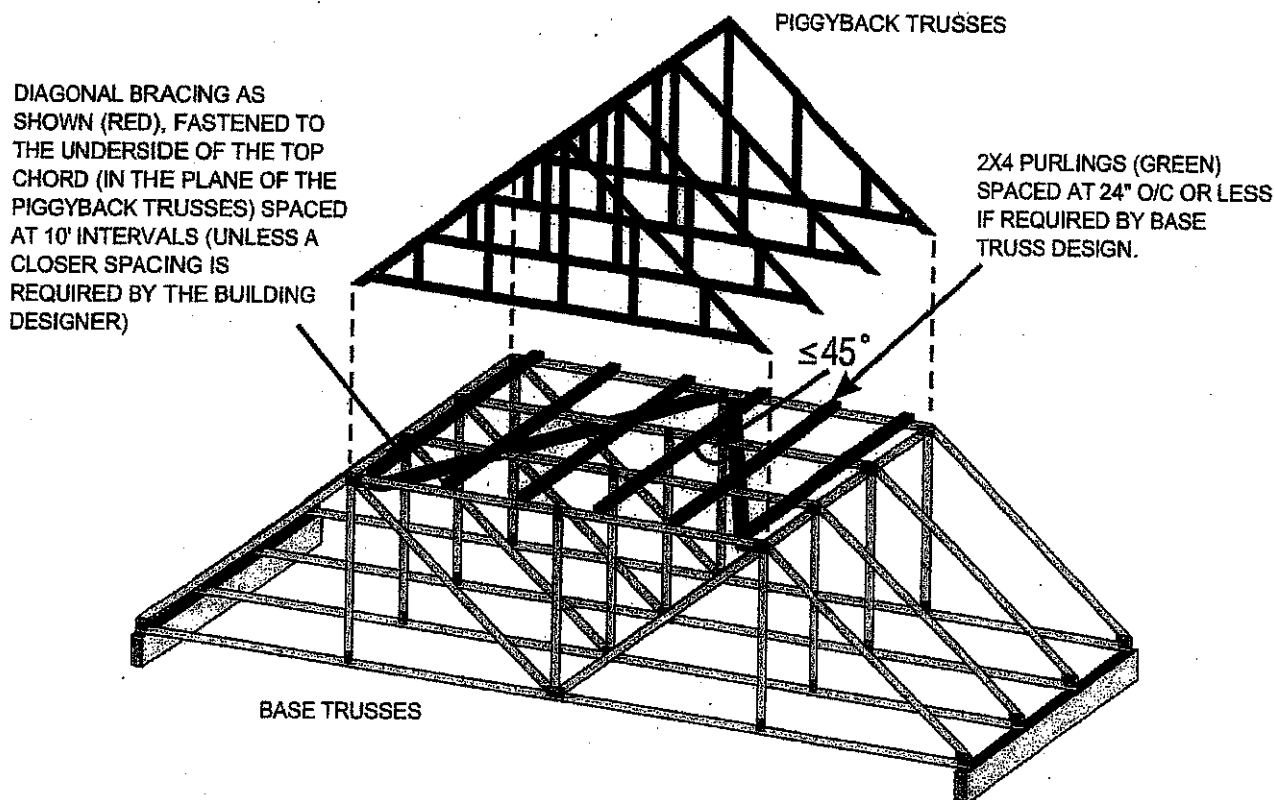


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

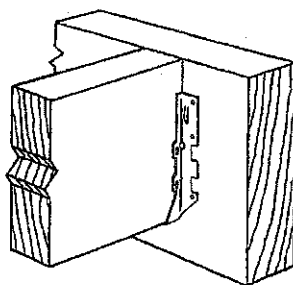
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

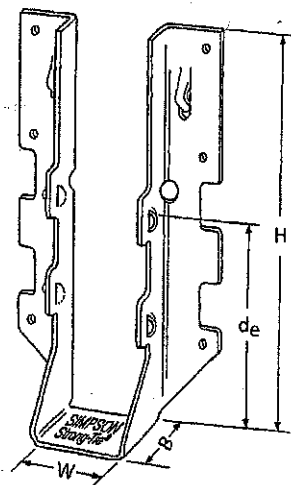
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



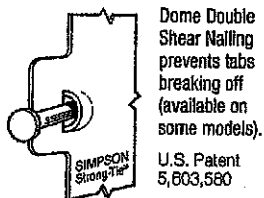
Typical LUS Installation



LUS28

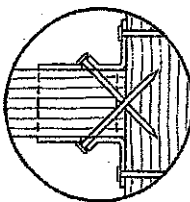
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1½	3½	1½	1½	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4½	1½	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4½	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4½	4½	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1½	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4½	6½	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7½	1½	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4½	8½	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

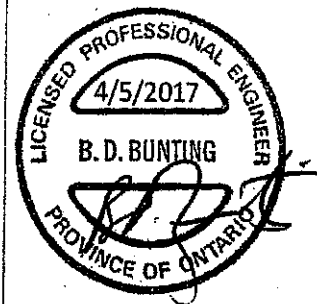


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



STATE DESIGN

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ESPEC-0517 9/17 edn. 8/18

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HUS/LJS -- Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

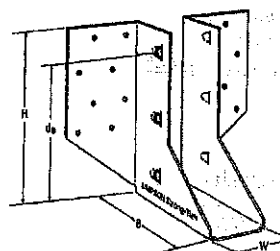
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

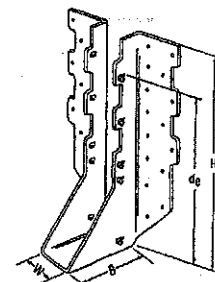
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

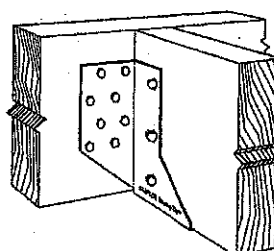
- See current catalogue for options



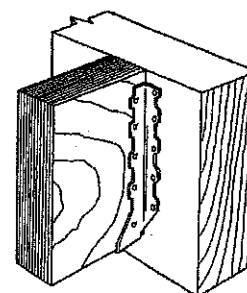
LJS26DS



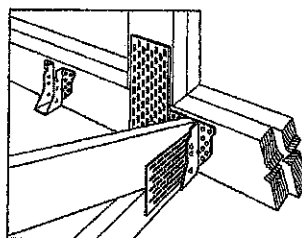
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



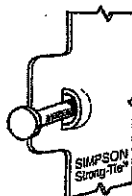
Typical HUS
Installation



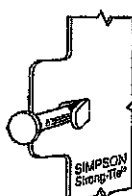
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 1/4	(14) 16d	(8) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/4	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

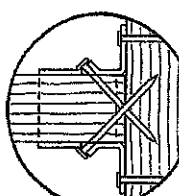
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



DESIGN

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SIMPSON STRONG-TIE COMPANY
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(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

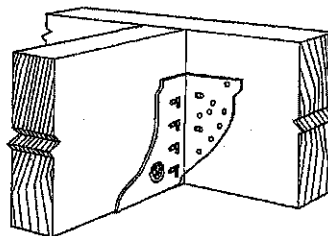
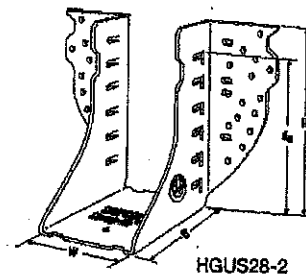
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

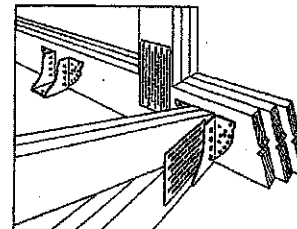
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



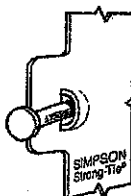
Typical HGUS
Installation



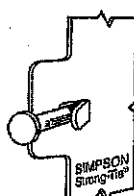
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 7/8	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/2	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/2	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/2	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/2	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/2	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/2	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/2	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/2	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/2	(66) 16d	(22) 16d	10130	16400	7195	11645

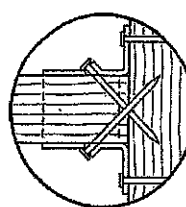
1. d₀ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



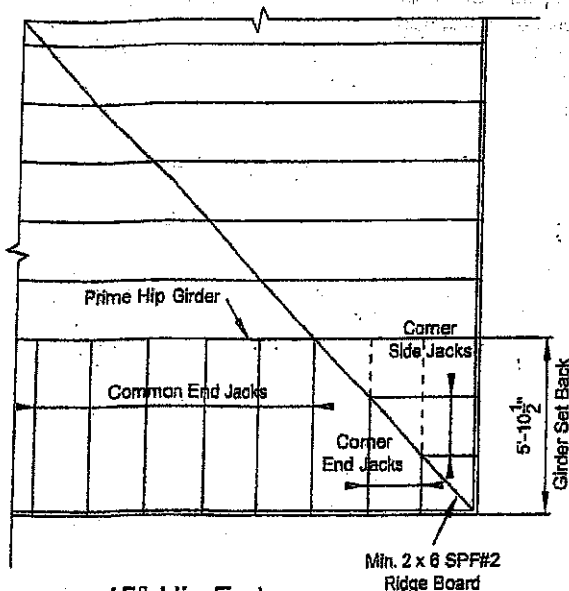
Double
Shear
Nailing
Top View.



DESIGN

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45° Hip End

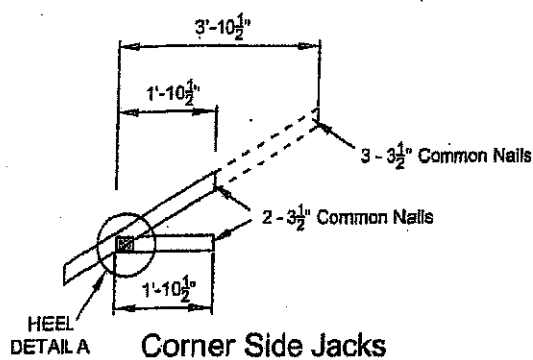
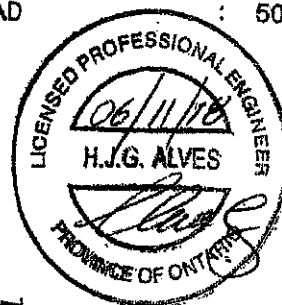
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

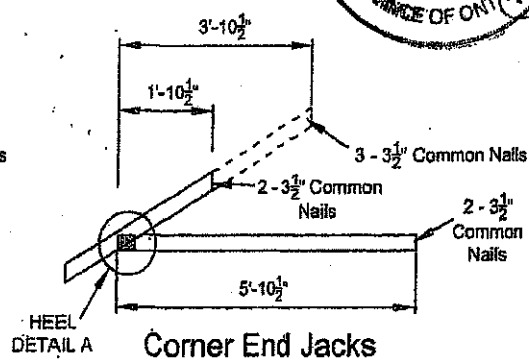
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

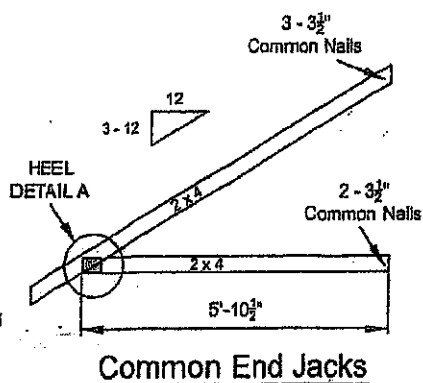
TOTAL LOAD : 50.5 P.S.F.



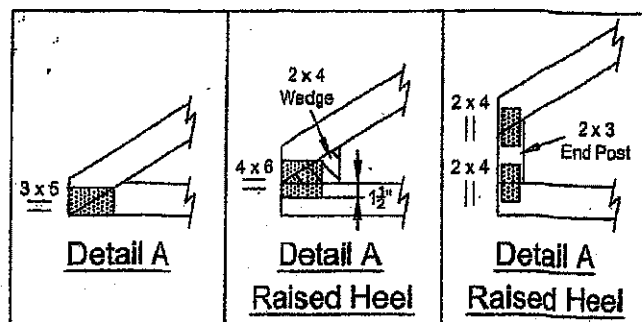
Corner Side Jacks



Corner End Jacks

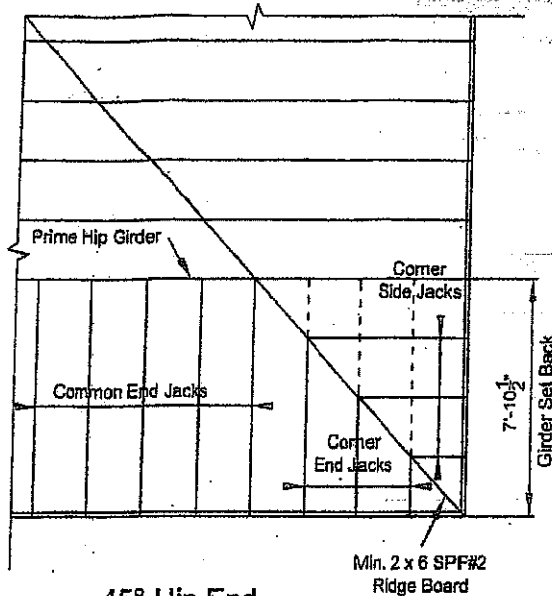


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

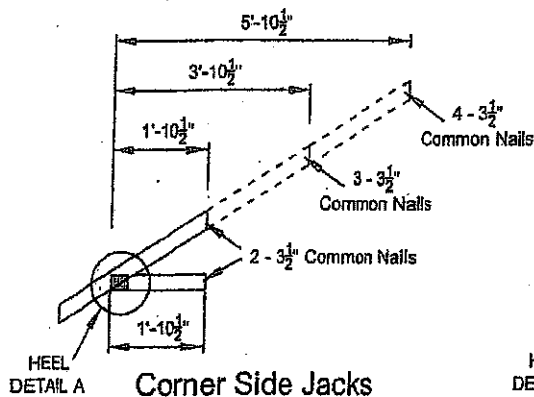
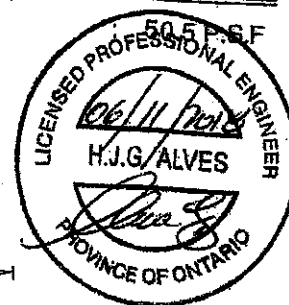
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

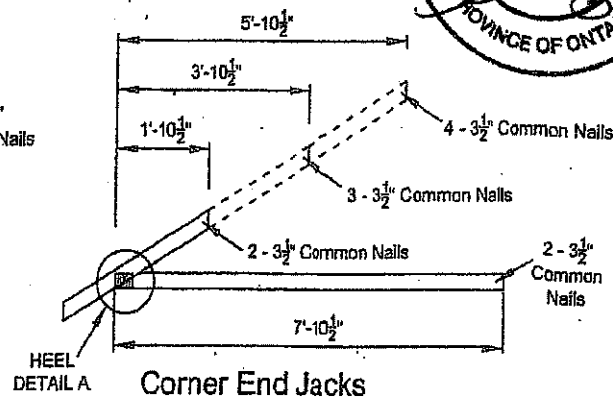
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

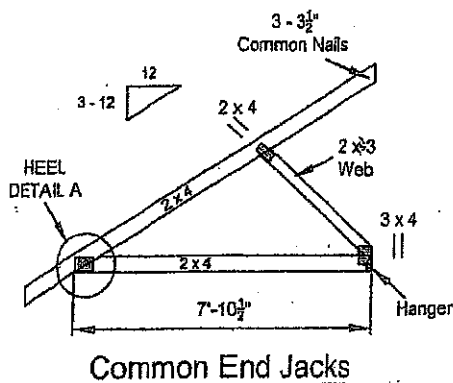
TOTAL LOAD



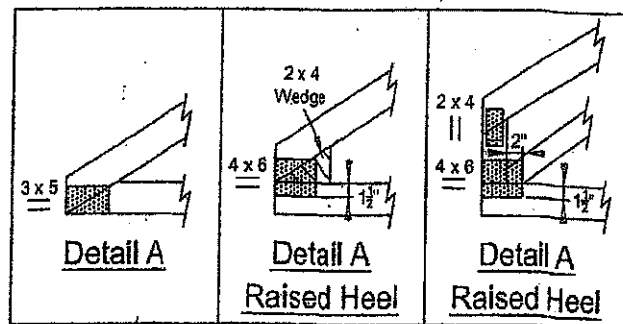
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

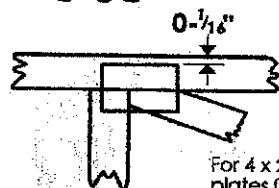
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



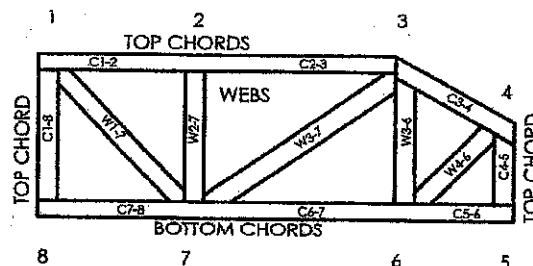
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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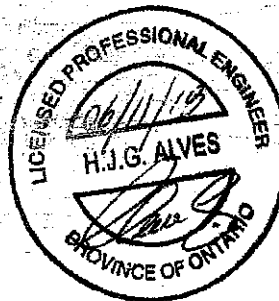
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218

Feb 09, 2018