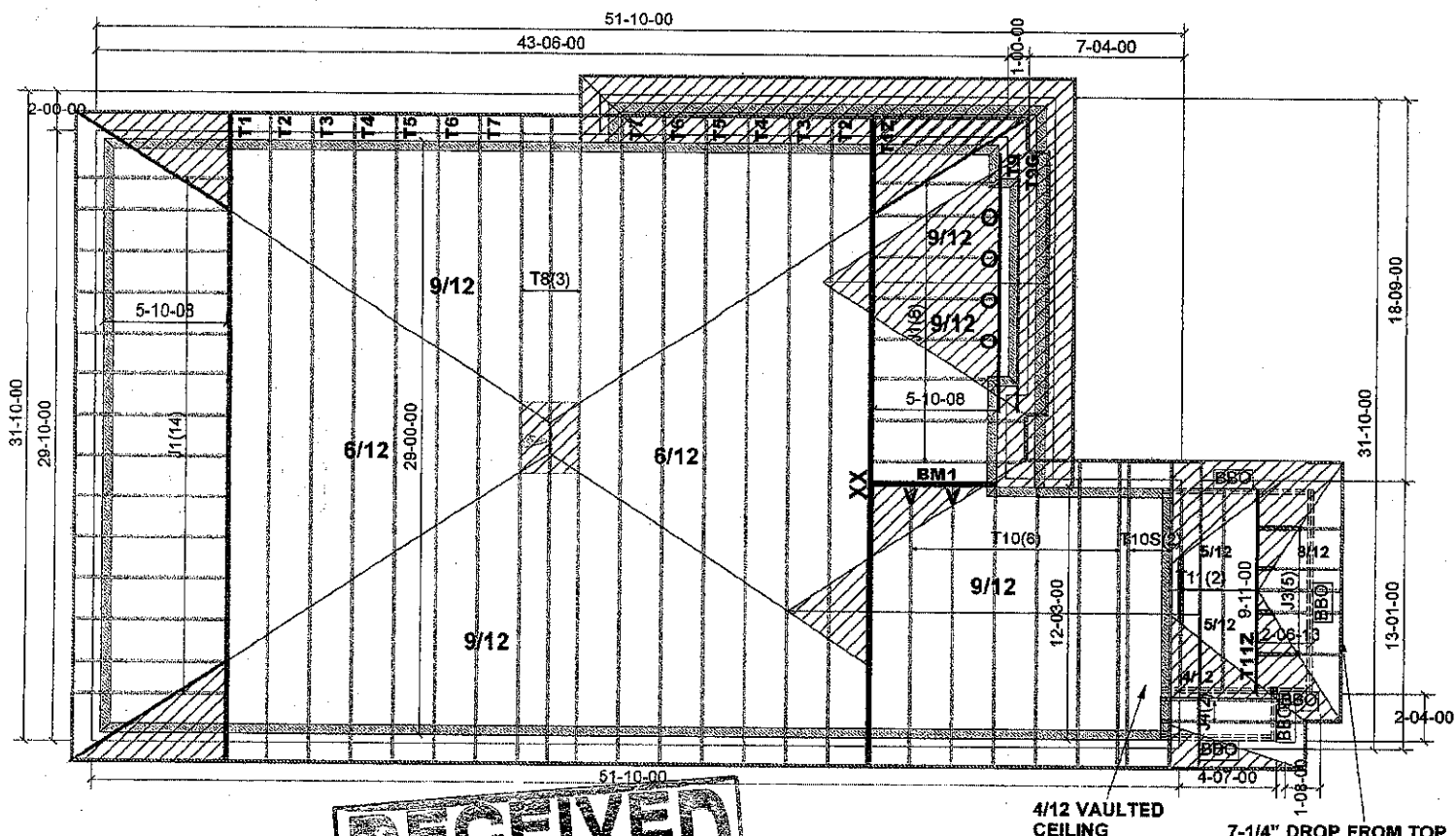




**TRUSS PROFILES TO BE VERIFIED BY
BUILDING DESIGNER**

ALL CONVENTIONAL ROOF FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"O.C. WITH 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED CONFORM WITH:
ONTARIO BUILDING CODE (2012)
OCCUPANCY: RESIDENTIAL | PART: 9

DESIGN LOADS:

CITY: CALEDON
G.S.L. = 37.6 psf
TC DL = 6 psf
BC LL = 10.50 psf
BC DL = 7.00 psf

NOTES:

FIN. OH.: 12"
HEEL TYPE: R.T.M. CANT.
EXT. WALLS: 2X6
CLAD. TYPE 1: BRICK/5"
CLAD. TYPE 2: SIDING/0"
FSC SIZE: 2X6
SHEATHING: ASPHALT SHINGLE

IF DESIGNED COMMERCIAL, REFER TO SEALED TRUSS DOCS FOR UPLIFT DESIGN

HARDWARE:

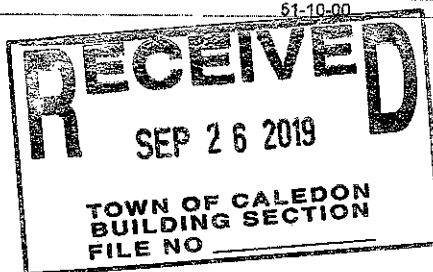
LJS26DS (V) 2pcs
HGUS26-2 (XX) 1pcs
LUS24 (O) 4 pcs

CONV FRM BY OTHERS

T- 180737

COMMENTS:

BM1: 2-2"x10" SPF #2



	Job Track: 50120 Plan Log: 200168 Layout ID: 400364	Builder / Location: Greenpark / Caledon Project: Lamberts Lane Home Corp. Date: 2019-01-30 Designer: Brian	Model / Elevation: Preston 1 / Elv. 1 Lot 21 THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Milek ver 8.2.3.229
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DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 1
 Lot #: 21
 Elevation: 1

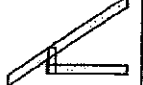
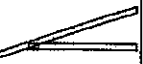
Job Track: 50120
 PlanLog: 200168
 Layout ID: 400364
 Ref #
 Page: 1 of 2
 Date: 01-30-2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	277.77 176.00		
	1 2-ply	T1Z Hip Girder	9/12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	277.77 176.00		
	2	T2 Hip	9/12	29-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	257.01 162.67		
	2	T3 Hip	9/12	29-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	254.02 162.67		
	2	T4 Hip	9/12	29-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	249.89 166.33		
	2	T5 Hip	9/12	29-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	284.41 183.33		
	2	T6 Hip	9/12	29-00-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.37 181.00		
	2	T7 Hip	9/12	29-00-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	276.24 180.00		
	3	T8 Hip	9/12	29-00-00	11-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	449.62 288.00		
	1	T9 Common Girder	9/12	10-02-00	5-04-00	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	50.43 32.83		
	1	T9G GABLE	9/12	10-02-00	5-04-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	46.57 31.50		
	6	T10 Common	9/12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	338.76 226.00		
	2	T10S Scissor	9/12 4/12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	108.28 72.67		
	2	T11 Common	5/12	9-11-00	2-06-02	2 x 4 2 x 6		5-06 5-06	63.43 38.00		

		DELIVERY SHIPLIST	
TAMARACK LUMBER INC. ALFA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: Greenpark Project: Lamberts Lane Home Corp. Location: Caledon Model: Preston 1 Lot #: 21 Elevation: 1	Job Track: 50120 PlanLog: 200168 Layout ID: 400364 Ref # Page: 2 of 2 Date: 01-30-2019 Designer: Brian Faneca Sales Rep: Mario DiCano	

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T11Z Common Girder	5 /12	9-11-00	2-06-02	2 x 4 2 x 6		5-06 5-06	31.71 19.00		
	22	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	369.48 234.67		
	5	J3 Jack-Open	8 /12	2-06-13	2-11-01	2 x 4	1-03-08	9-10 2-06-02	44.21 30.00		
	2	J4 Jack-Open	4 /12	5-05-08	2-07-03	2 x 4	1-05-00	3-15 2-01-12	29.2 18.67		

TOTAL # TRUSS= 61

TOTAL BFT OF ALL TRUSSES= 2379.34

BFT.

TOTAL WEIGHT OF ALL TRSSES 3691.17 LBS

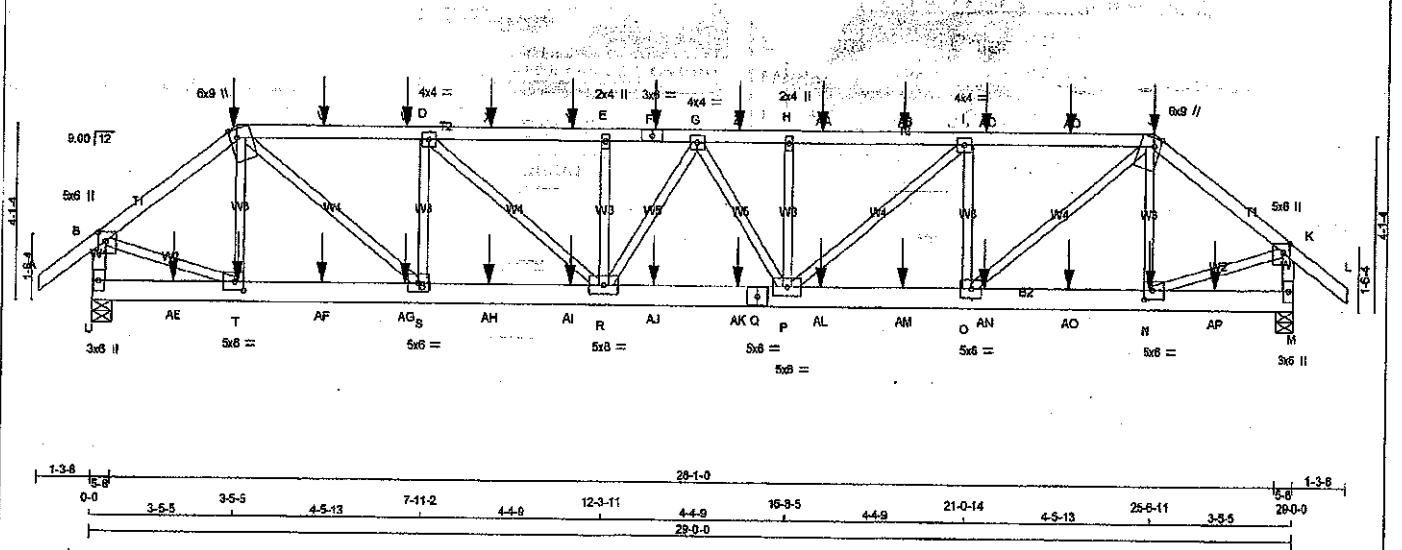
HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
4	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 7

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T1	1	2	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2019 MiTek Industries, Inc. Tue Feb 5 20:42:02 2019 Page 1
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 1-3-8 0-0 3-5-5 3-5-5 4-5-13 7-11-2 4-4-9 12-3-11 14-6-0 2-2-5 16-8-5 4-4-9 21-0-14 4-5-13 25-6-11 3-5-5 28-0-0 30-3-8
 Scale = 1:50.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - J	2x4	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
U - Q	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	

EXCEPT
 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A-C	12	SIDE (61.0)
C-F	12	SIDE (61.0)
F-J	12	SIDE (61.0)
J-L	12	SIDE (61.0)
U-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
U-Q	12	SIDE (183.1)
Q-M	12	SIDE (183.1)
WEBS : (0.122'X3') SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table ts in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMWV+p	MT20	5.0	6.0	2.50	2.00	
C TTWW+m	MT20	6.0	9.0	Edge	2.00	
D, G, I						
D TMWV-I	MT20	4.0	4.0			
E TMWV-w	MT20	2.0	4.0			
F TS-I	MT20	3.0	6.0			
H TMWV-w	MT20	2.0	4.0			
J TTWW+m	MT20	6.0	9.0	Edge	2.00	
K TMWV+p	MT20	5.0	6.0	2.50	2.00	
M BMV1+p	MT20	3.0	6.0			
N BMWV-I	MT20	5.0	6.0	2.50	2.50	
O BMWV-I	MT20	5.0	6.0			
P BMWVW-I	MT20	5.0	8.0			
Q BS-I	MT20	5.0	6.0			
R BMWVW-I	MT20	5.0	8.0			
S BMWV-I	MT20	5.0	6.0			
T BMWV-I	MT20	5.0	6.0	2.50	2.50	
U BMV1+p	MT20	3.0	6.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
U	3644	0	3644	0
M	3643	0	3643	0

UNFACTORED REACTIONS	1ST LOASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
U	2712	1532 / 0
M	2711	1531 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO					FR-TO			
A-B	0 / 42		-102.1	-102.1	0.08 (1)	T-C	-833 / 0	0.08 (1)
B-C	-3827 / 0		-102.1	-102.1	0.18 (1)	C-S	0 / 3202	0.40 (1)
C-V	-5503 / 0		-102.1	-102.1	0.40 (1)	S-D	-1769 / 0	0.22 (1)
V-W	-5503 / 0		-102.1	-102.1	0.40 (1)	D-R	0 / 1473	0.18 (1)
W-D	-5503 / 0		-102.1	-102.1	0.40 (1)	R-E	-549 / 0	0.07 (1)
D-X	-6624 / 0		-102.1	-102.1	0.46 (1)	E-H	-543 / 0	0.07 (1)
X-Y	-6624 / 0		-102.1	-102.1	0.46 (1)	H-I	0 / 1472	0.18 (1)
Y-E	-6624 / 0		-102.1	-102.1	0.46 (1)	I-O	0 / 3201	0.40 (1)
E-F	-6624 / 0		-102.1	-102.1	0.26 (1)	O-J	-833 / 0	0.08 (1)
F-G	-6624 / 0		-102.1	-102.1	0.26 (1)	J-K	0 / 3200	0.40 (1)
G-Z	-6622 / 0		-102.1	-102.1	0.26 (1)	K-N	0 / 3199	0.40 (1)
Z-H	-6622 / 0		-102.1	-102.1	0.26 (1)	N-K	-145 / 0	0.02 (1)
H-AA	-6622 / 0		-102.1	-102.1	0.46 (1)	G-P	-150 / 0	0.02 (1)
AA-AB	-6622 / 0		-102.1	-102.1	0.46 (1)			
AB-I	-6622 / 0		-102.1	-102.1	0.46 (1)			
I-AC	-5501 / 0		-102.1	-102.1	0.40 (1)			
AC-AD	-5501 / 0		-102.1	-102.1	0.40 (1)			
AD-J	-5501 / 0		-102.1	-102.1	0.40 (1)			
J-K	-3826 / 0		-102.1	-102.1	0.18 (1)			
K-L	0 / 42		-102.1	-102.1	0.08 (1)			
U-B	-3578 / 0		0.0	0.0	0.20 (1)			
M-K	-3577 / 0		0.0	0.0	0.20 (1)			
U-AE	0 / 0		-38.5	-38.5	0.05 (3)			
AE-T	0 / 0		-38.5	-38.5	0.05 (3)			
T-AF	0 / 3036		-38.5	-38.5	0.24 (1)			
AF-AG	0 / 3036		-38.5	-38.5	0.24 (1)			
AG-S	0 / 3036		-38.5	-38.5	0.24 (1)			
S-AH	0 / 5503		-38.5	-38.5	0.42 (1)			
AH-AI	0 / 5503		-38.5	-38.5	0.42 (1)			
AI-R	0 / 5503		-38.5	-38.5	0.42 (1)			
R-AJ	0 / 6698		-38.5	-38.5	0.50 (1)			
AJ-AK	0 / 6698		-38.5	-38.5	0.50 (1)			
AK-Q	0 / 6698		-38.5	-38.5	0.50 (1)			
Q-P	0 / 6698		-38.5	-38.5	0.50 (1)			
P-AL	0 / 5501		-38.5	-38.5	0.42 (1)			
AL-AM	0 / 5501		-38.5	-38.5	0.42 (1)			
AM-O	0 / 5501		-38.5	-38.5	0.42 (1)			
O-AN	0 / 3035		-38.5	-38.5	0.24 (1)			
AN-AO	0 / 3035		-38.5	-38.5	0.24 (1)			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012
 - CSA 088-08, CSA 088-14
 - TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CS1: TC=0.48/1.00 (D-E:1), BC=0.50/1.00 (P-R:1),
 WB=0.40/1.00 (C-S:1), SS=0.20/1.00 (I-J:1)

DCL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

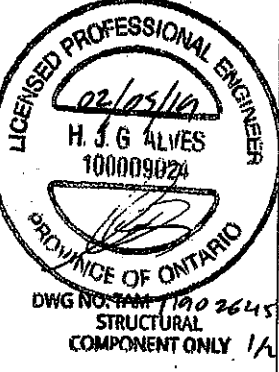
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PS) (PLF)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1997 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (U) (INPUT = 0.90)
 JSI METAL= 0.02 (O) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T1	1	2	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:02 2019 Page 2
ID:IFxPHXfe38FX8N7VhzQQIYyQBwWV-y3L3s65Ho6YZ77iQ5CsLDbz7qDH5NqYNWu5zo56j

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

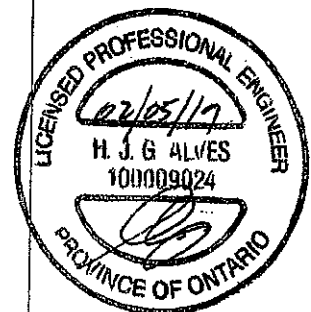
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED UNBRAC. LENGTH FR-TO	MAX. CSI (LC)
FR-TO		FROM TO					
AO-N	0/3035	-38.5	-38.5 0.24 (1)			10.00	
N-AP	0/0	-38.5	-38.5 0.05 (3)			10.00	
AP-M	0/0	-38.5	-38.5 0.05 (3)			10.00	

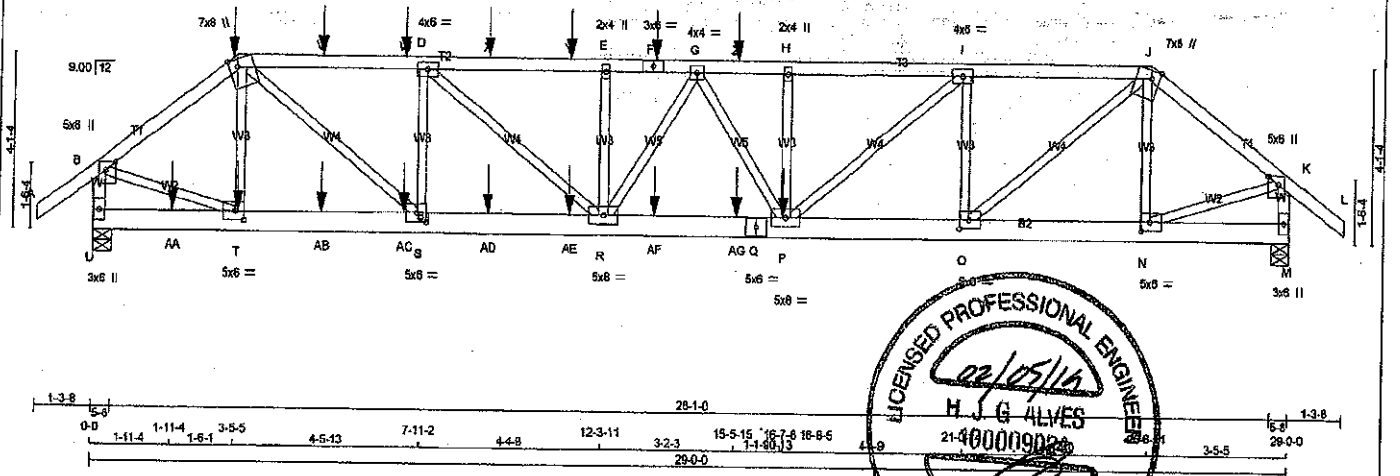
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-50	-56	---	FRONT	VERT	DEAD	---	---
C	3-5-5	-130	-130	---	FRONT	VERT	TOTAL	---	---
C	3-5-5	-285	-285	---	FRONT	VERT	SNOW	---	---
F	13-8-1	-123	-123	---	FRONT	VERT	TOTAL	---	---
J	25-6-11	-50	-56	---	FRONT	VERT	DEAD	---	---
J	25-6-11	-130	-130	---	FRONT	VERT	TOTAL	---	---
J	25-6-11	-285	-285	---	FRONT	VERT	SNOW	---	---
N	25-5-15	-55	-70	---	FRONT	VERT	TOTAL	---	---
T	3-6-1	-55	-70	---	FRONT	VERT	TOTAL	---	---
V	5-8-1	-123	-123	---	FRONT	VERT	TOTAL	---	---
W	7-6-1	-123	-123	---	FRONT	VERT	TOTAL	---	---
X	9-8-1	-123	-123	---	FRONT	VERT	TOTAL	---	---
Y	11-6-1	-123	-123	---	FRONT	VERT	TOTAL	---	---
Z	15-5-15	-123	-123	---	FRONT	VERT	TOTAL	---	---
AA	17-5-15	-123	-123	---	FRONT	VERT	TOTAL	---	---
AB	19-5-15	-123	-123	---	FRONT	VERT	TOTAL	---	---
AC	21-5-15	-123	-123	---	FRONT	VERT	TOTAL	---	---
AD	23-5-15	-123	-123	---	FRONT	VERT	TOTAL	---	---
AE	1-11-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
AF	5-6-1	-55	-70	---	FRONT	VERT	TOTAL	---	---
AG	7-6-1	-55	-70	---	FRONT	VERT	TOTAL	---	---
AH	9-6-1	-55	-70	---	FRONT	VERT	TOTAL	---	---
AI	11-6-1	-55	-70	---	FRONT	VERT	TOTAL	---	---
AJ	13-6-1	-55	-70	---	FRONT	VERT	TOTAL	---	---
AK	15-5-15	-55	-70	---	FRONT	VERT	TOTAL	---	---
AL	17-5-15	-55	-70	---	FRONT	VERT	TOTAL	---	---
AM	19-5-15	-55	-70	---	FRONT	VERT	TOTAL	---	---
AN	21-5-15	-55	-70	---	FRONT	VERT	TOTAL	---	---
AO	23-5-15	-55	-70	---	FRONT	VERT	TOTAL	---	---
AP	27-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM T1902645
STRUCTURAL
COMPONENT ONLY 7/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	DRWG NO.
200168-400364	T1Z	1	2	Preston_1	
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:03 2018 Page 1	
ID: IFxHXf38FX8N?VhzQQIYyQBwVW-QFv4GB7j25EPB8W8cRPyII2RUyJ0X3C63QYz0561				21-0-14 4-5-13 25-0-11 3-5-5 28-0-0 30-3-8	
1-3-8 0-0 3-5-5 3-5-5 4-5-13 7-11-2 4-4-9 12-3-11 14-5-0 15-5-15 16-8-5 21-0-14 4-5-13 25-0-11 3-5-5 28-0-0 30-3-8				Scale = 1:50.0	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
U - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-F	1	12	SIDE(61.0)
F-J	1	12	SIDE(61.0)
J-L	1	12	TOP
U-B	1	12	TOP
M-K	1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
U-Q	2	12	SIDE(183.1)
Q-M	2	12	SIDE(183.1)

WEBS : (0.122"x3") SPIRAL NAILS
2x3 1 6
H-P 1 6
SIDE(176.7)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW+p	MT20	5.0	6.0	Edge 3.00
C D TMVW+m	MT20	7.0	8.0	Edge 2.50
D TMVW+l	MT20	4.0	8.0	
E TMVW+w	MT20	2.0	4.0	
F TS-1	MT20	3.0	6.0	
G TMVW+l	MT20	4.0	4.0	
H TMVW+w	MT20	2.0	4.0	
I TMVW+l	MT20	4.0	6.0	
J TMVW+m	MT20	7.0	8.0	Edge 2.50
K TMVW+p	MT20	5.0	6.0	Edge 3.00
M BMVW+l	MT20	3.0	6.0	
N BMVW+l	MT20	5.0	6.0	2.50 2.50
O BMVW+l	MT20	5.0	8.0	2.50 2.75
P BMVWVW+t	MT20	5.0	8.0	
Q BS-1	MT20	5.0	6.0	
R BMVWVW+t	MT20	5.0	8.0	
S BMVW+l	MT20	5.0	6.0	2.50 2.75
T BMVW+l	MT20	5.0	6.0	2.50 2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REGRD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	3955	0	3955	0	0	5-8	5-8
M	3425	0	3425	0	0	5-8	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2935	1684 / 0	538 / 0	0 / 0	0 / 0	712 / 0	0 / 0
M	2540	1464 / 0	463 / 0	0 / 0	0 / 0	612 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 42	-102.1 -102.1	0.08 (1)	T-C	-719 / 0	0.09 (1)	
B-C	-4194 / 0	-102.1 -102.1	0.19 (1)	C-S	0 / 3879	0.48 (1)	
C-V	-6160 / 0	-102.1 -102.1	0.43 (1)	S-D	-2079 / 0	0.25 (1)	
V-W	-6160 / 0	-102.1 -102.1	0.43 (1)	D-R	0 / 1955	0.24 (1)	
W-D	-6160 / 0	-102.1 -102.1	0.43 (1)	R-E	-535 / 0	0.07 (1)	
D-X	-7649 / 0	-102.1 -102.1	0.52 (1)	P-H	-380 / 0	0.05 (1)	
X-Y	-7649 / 0	-102.1 -102.1	0.52 (1)	H-I	0 / 2779	0.34 (1)	
Y-E	-7649 / 0	-102.1 -102.1	0.52 (1)	E-I	-2308 / 0	0.29 (1)	
E-F	-7649 / 0	-102.1 -102.1	0.30 (1)	I-J	0 / 3890	0.48 (1)	
F-G	-7649 / 0	-102.1 -102.1	0.30 (1)	J-N	-897 / 0	0.09 (1)	
G-Z	-7962 / 0	-102.1 -102.1	0.28 (1)	N-T	0 / 3507	0.43 (1)	
Z-H	-7962 / 0	-102.1 -102.1	0.28 (1)	H-K	0 / 3008	0.37 (1)	
H-I	-7962 / 0	-102.1 -102.1	0.44 (1)	I-K	-490 / 0	0.06 (1)	
I-J	-5848 / 0	-102.1 -102.1	0.31 (1)	K-G	0 / 128	0.02 (1)	
J-K	-3595 / 0	-102.1 -102.1	0.17 (1)				
K-L	0 / 42	-102.1 -102.1	0.08 (1)				
U-B	-3891 / 0	0.0	0.02 (1)				
M-K	-3381 / 0	0.0	0.01 (1)				

U-AA	0 / 0	-38.5	-38.5	0.05 (3)	10.00
AA-T	0 / 0	-38.5	-38.5	0.05 (3)	10.00
T-AB	0 / 3327	-38.5	-38.5	0.28 (1)	10.00
AB-AC	0 / 3327	-38.5	-38.5	0.28 (1)	10.00
AC-S	0 / 3327	-38.5	-38.5	0.28 (1)	10.00
S-AD	0 / 6161	-38.5	-38.5	0.46 (1)	10.00
AD-AE	0 / 6161	-38.5	-38.5	0.46 (1)	10.00
AE-R	0 / 6161	-38.5	-38.5	0.46 (1)	10.00
R-AF	0 / 7897	-38.5	-38.5	0.60 (1)	10.00
AF-AG	0 / 7897	-38.5	-38.5	0.60 (1)	10.00
AG-Q	0 / 7897	-38.5	-38.5	0.60 (1)	10.00
Q-P	0 / 7897	-38.5	-38.5	0.60 (1)	10.00
P-O	0 / 5946	-38.5	-38.5	0.44 (1)	10.00
O-N	0 / 2849	-38.5	-38.5	0.22 (1)	10.00
N-M	0 / 0	-38.5	-38.5	0.04 (2)	10.00

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL
C	3-5-5	-50	-56	FRONT	VERT	DEAD	
C	3-5-5	-130	-130	BACK	VERT	TOTAL	
C	3-5-5	-285	-285	FRONT	VERT	SNOW	

DESIGN CRITERIA
SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.03/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 068-09, CSA 068-14
- TPC 2011, TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.19")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.52/1.00 (D-E:1), BC=0.60/1.00 (P-R:1), WB=0.48/1.00 (J-O:1), SS=0.20/1.00 (C-D:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES							
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN		
MT20	618	354	1667	768	1997	1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.76 (Q) (INPUT = 1.00)

DWG NO. TAM T1902546
STRUCTURAL
COMPONENT ONLY 1/2

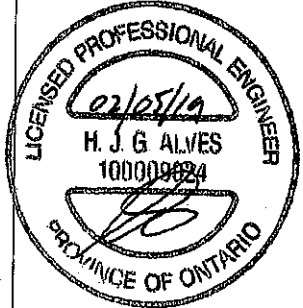
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Preston 1	DRWG NO.
200168-400364	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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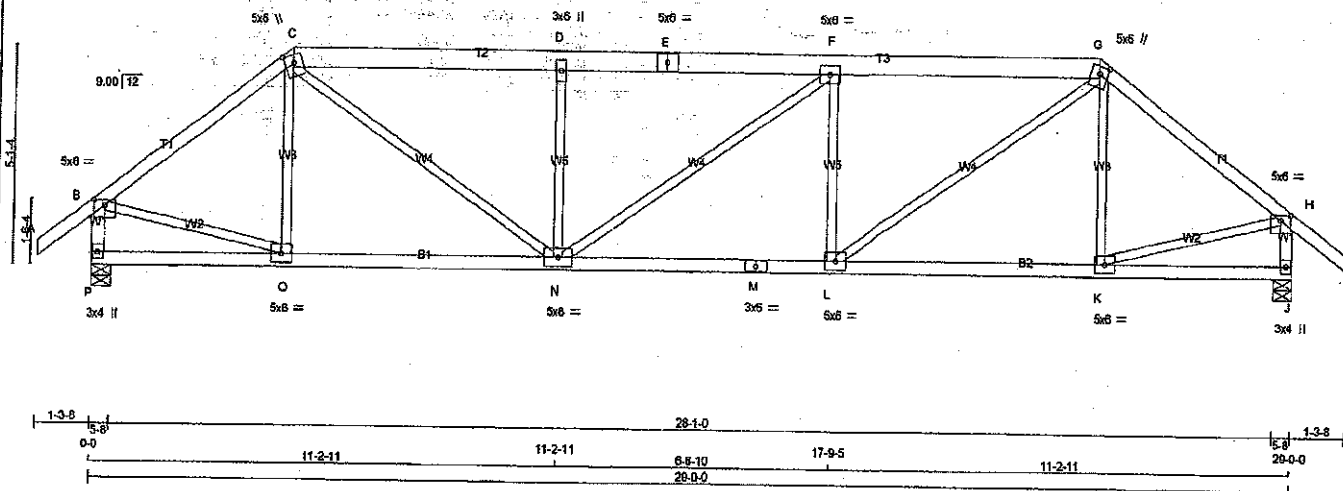
PLATES (table is in inches)				FACTORED CONCENTRATED LOADS (LBS)											
JT	TYPE	PLATES	W	LEN	Y	X	LOC	LC1	MAX	MAX	FACE	DIR.	TYPE	HEEL	CONN.
F							13-6-1	-123	-123		BACK	VERT	TOTAL		
P							18-7-8	-1379	-1379		BACK	VERT	TOTAL		
T							3-8-1	-55	-70		BACK	VERT	TOTAL		
V							5-8-1	-123	-123		BACK	VERT	TOTAL		
W							7-8-1	-123	-123		BACK	VERT	TOTAL		
X							9-8-1	-123	-123		BACK	VERT	TOTAL		
Y							11-8-1	-123	-123		BACK	VERT	TOTAL		
Z							15-5-15	-123	-123		BACK	VERT	TOTAL		
AA							1-11-4	-55	-70		BACK	VERT	TOTAL		
AB							5-8-1	-55	-70		BACK	VERT	TOTAL		
AC							7-8-1	-55	-70		BACK	VERT	TOTAL		
AD							9-8-1	-55	-70		BACK	VERT	TOTAL		
AE							11-8-1	-55	-70		BACK	VERT	TOTAL		
AF							13-8-1	-55	-70		BACK	VERT	TOTAL		
AG							15-5-15	-55	-70		BACK	VERT	TOTAL		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



02/05/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T1902646
STRUCTURAL
COMPONENT ONLY 4/2



TOTAL WEIGHT = 2 X 128 = 257 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
E - G	2x6	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	1.50	3.00
C	TTWV+m	MT20	5.0	8.0	2.25	2.50
D	TMW-w	MT20	3.0	8.0		
E	TS-t	MT20	5.0	8.0		
F	TMWV-t	MT20	5.0	8.0		
G	TTWV+m	MT20	5.0	8.0	2.25	2.50
H	TMWV-p	MT20	5.0	8.0	1.50	3.00
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	8.0		
M	BS-t	MT20	3.0	8.0		
N	BMWVW-t	MT20	5.0	8.0		
P	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
P	2180	0	2180	0
J	2180	0	2180	0

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
P	1620	922/0	305/0	0/0	0/0	394/0
J	1620	922/0	305/0	0/0	0/0	394/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM	FR-TO	
A-B	0/42	-102.1	O-C	-201/98
B-C	-2157/0	-102.1	K-G	-201/98
C-D	-3071/0	-102.1	B-O	0/1788
D-E	-3071/0	-102.1	K-H	0/1788
E-F	-3071/0	-102.1	L-G	0/1681
F-G	-3071/0	-102.1	C-N	0/1681
G-H	-2157/0	-102.1	L-F	-732/0
H-I	0/42	-102.1	N-D	-732/0
P-B	-2115/0	0.0	N-F	0/1
J-H	-2115/0	0.0		
P-O	0/0	-38.5		
O-N	0/1734	-38.5		
N-M	0/3071	-38.5		
M-L	0/3071	-38.5		
L-K	0/1734	-38.5		
K-J	0/0	-38.5		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/969 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/969 (0.23")

CSI: TC=0.54/1.00 (B-C:1), BC=0.67/1.00 (L-N:1),
WB=0.40/1.00 (H-K:1), SS=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

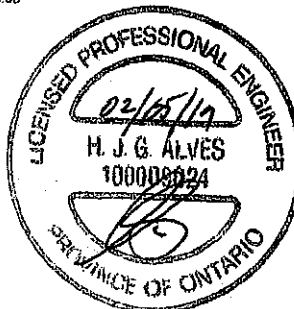
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

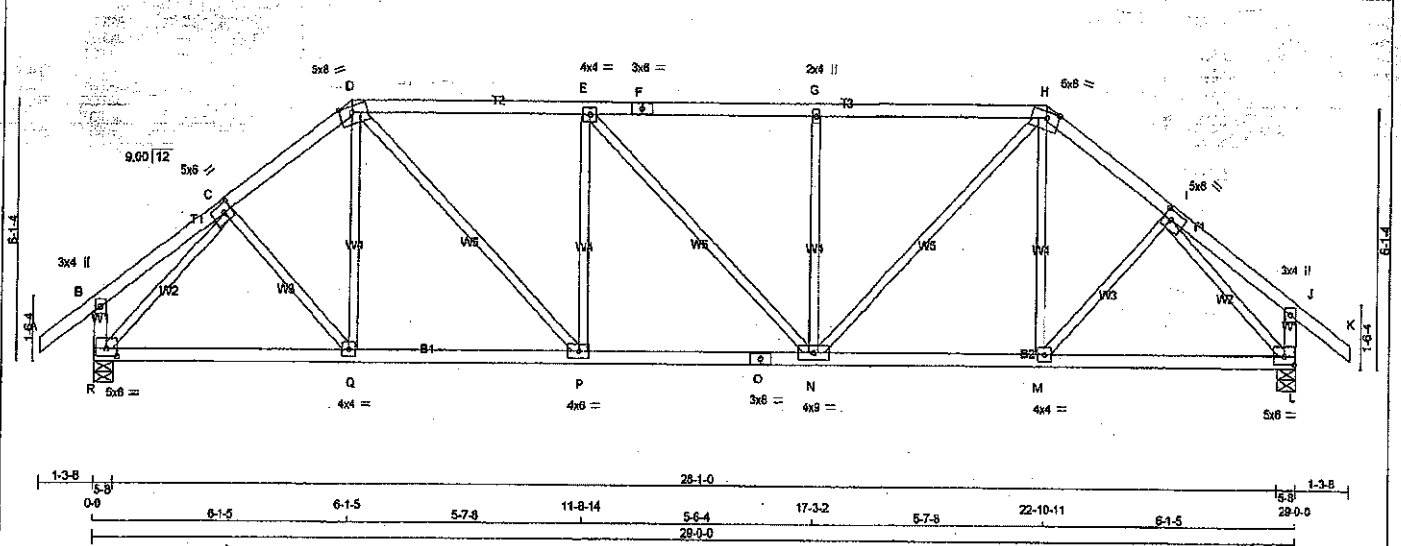
JSI METAL= 0.96 (M) (INPUT = 1.00)



DWG NO. TAM 1190 2647
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T3	2	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:05 2019 Page 1
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 1-3-8 0.0 3-1-15 3-1-15 2-1-17 8-1-5 5-7-8 11-8-14 5-6-4 17-3-2 5-7-8 22-10-11 2-11-7 25-10-1 3-1-15 1-3-8
 Scale = 1:50.0



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-I	MT20	5.0	8.0	2.50 2.25
D	TTWW-m	MT20	5.0	8.0	Edge 3.50
E	TMWW-I	MT20	4.0	4.0	
F	TS-I	MT20	3.0	6.0	
G	TMV+w	MT20	2.0	4.0	
H	TTWW-m	MT20	5.0	8.0	Edge 3.50
I	TMWW-I	MT20	5.0	8.0	2.50 2.25
J	TMV+p	MT20	3.0	4.0	
L	BMVW-I	MT20	5.0	8.0	2.25 3.00
M	BMWW-I	MT20	4.0	4.0	
N	BMWW-I	MT20	4.0	9.0	
O	BS-I	MT20	3.0	8.0	
P	BMWW-I	MT20	4.0	6.0	
Q	BMWW-I	MT20	4.0	4.0	
R	BMVW-I	MT20	5.0	8.0	2.25 3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	DOWN	UPLIFT	IN-SX	IN-SX
R	2180	0	2180	0	0	5-8	5-8	5-8	5-8
L	2180	0	2180	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS		LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM	LIVE
R	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0	0 / 0	1620	922 / 0	305 / 0	0 / 0	0 / 0
L	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0	0 / 0	1620	922 / 0	305 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE	VERT. LOAD	LC1 MAX
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	C-Q	0 / 212	0.05 (2)	0.05 (2)
B-C	0 / 18	-102.1	-102.1 0.12 (1)	Q-D	0 / 202	0.05 (3)	0.05 (3)
C-D	-2175 / 0	-102.1	-102.1 0.18 (1)	D-P	0 / 1140	0.28 (1)	0.28 (1)
D-E	-2517 / 0	-102.1	-102.1 0.58 (1)	P-E	-615 / 0	0.36 (1)	0.36 (1)
E-F	-2518 / 0	-102.1	-102.1 0.59 (1)	E-N	-2 / 0	0.00 (1)	0.00 (1)
F-G	-2516 / 0	-102.1	-102.1 0.59 (1)	N-G	-614 / 0	0.36 (1)	0.36 (1)
G-H	-2515 / 0	-102.1	-102.1 0.59 (1)	N-H	0 / 1137	0.28 (1)	0.28 (1)
H-I	-2175 / 0	-102.1	-102.1 0.18 (1)	M-H	0 / 202	0.05 (3)	0.05 (3)
I-J	0 / 18	-102.1	-102.1 0.12 (1)	M-I	0 / 212	0.05 (2)	0.05 (2)
J-K	0 / 42	-102.1	-102.1 0.14 (1)	R-C	-2413 / 0	0.82 (1)	0.82 (1)
R-B	-268 / 0	0.0	0.0 0.03 (1)	I-L	-2413 / 0	0.82 (1)	0.82 (1)
L-J	-266 / 0	0.0	0.0 0.03 (1)				
R-Q	0 / 1589	-38.5	-38.5 0.46 (2)				
Q-P	0 / 1723	-38.5	-38.5 0.46 (2)				
P-O	0 / 2517	-38.5	-38.5 0.51 (1)				
O-N	0 / 2517	-38.5	-38.5 0.51 (1)				
N-M	0 / 1723	-38.5	-38.5 0.46 (2)				
M-L	0 / 1589	-38.5	-38.5 0.46 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
 CALCULATED VERT. DEFL.(LL) = 1 / 999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (0.87")
 CALCULATED VERT. DEFL.(TL) = 1 / 899 (0.20")

CSI: TC=0.59/1.00 (D-E), BC=0.51/1.00 (N-P), WB=0.82/1.00 (I-L), SS=0.27/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

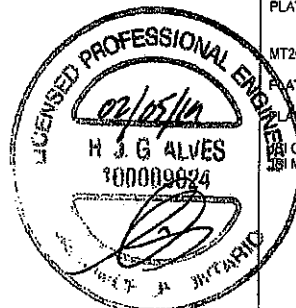
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP=0.87 (L) (INPUT = 0.90)
 METAL=0.78 (O) (INPUT = 1.00)



DWG NO. TAM T1902648
 STRUCTURAL
 COMPONENT ONLY

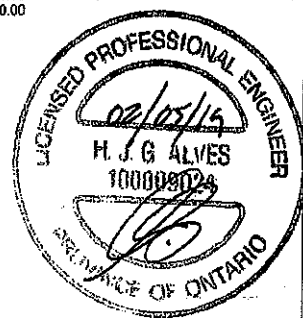
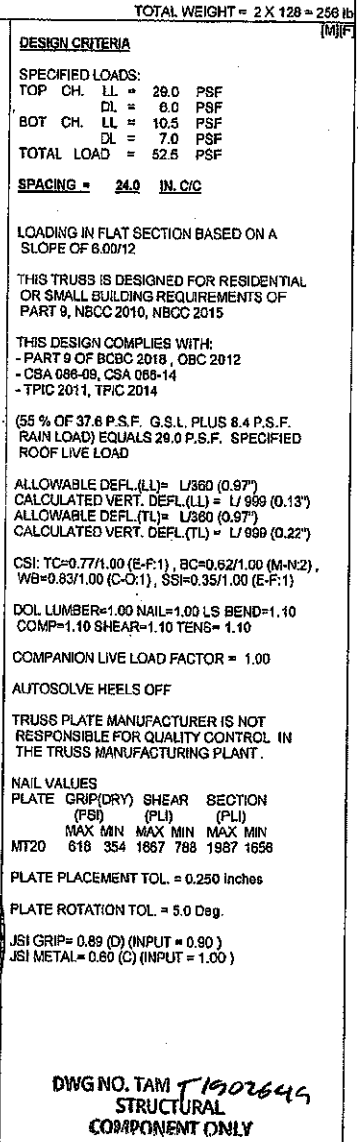
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:05 2019 Page 1

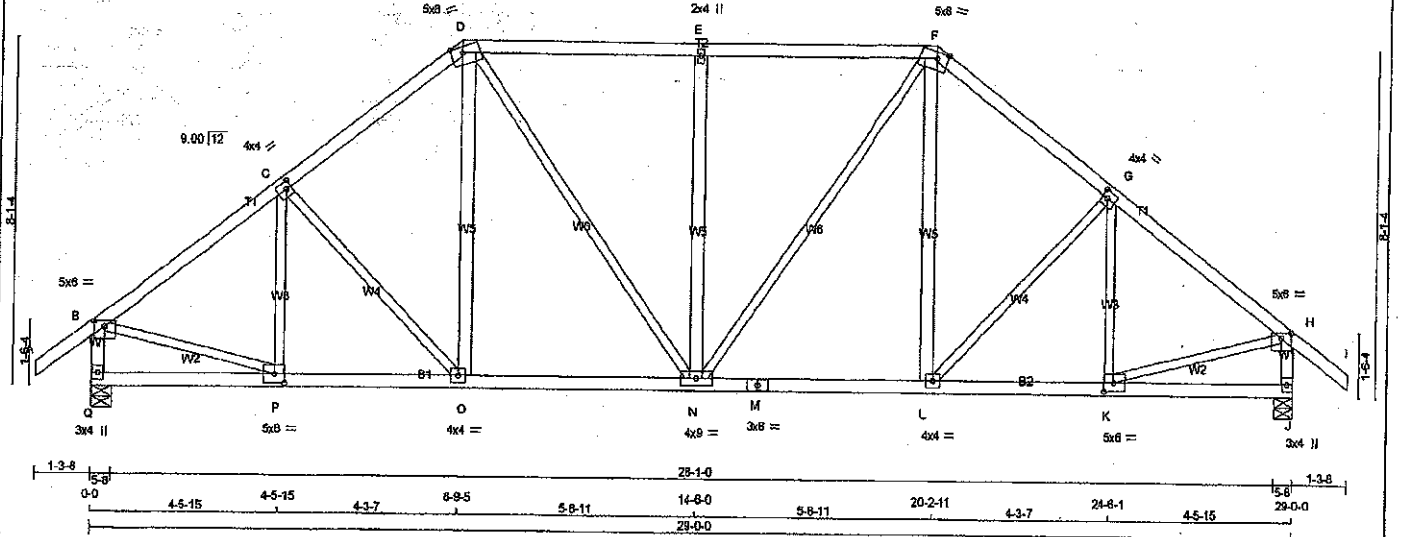
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1-3-8 0-0 3-9-15 3-9-15 3-7-7 7-5-5 7-0-11 14-8-0 21-8-11 25-2-1 3-9-15 29-0-0 30-3-8

Scale: 1/2"=1'-0"



DWG NO. TAM T1902645
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 142 = 284 LB (MIF)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
Q - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
Q - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
O - D	2x4	DRY No.2	SPF
N - E	2x4	DRY No.2	SPF
L - F	2x4	DRY No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.50	3.00
C	TMWV-l	MT20	4.0	4.0	2.00	1.50
D	TMWV-m	MT20	5.0	8.0	1.75	3.25
E	TMWV-w	MT20	2.0	4.0		
F	TMWV-n	MT20	5.0	8.0	1.75	3.25
G	TMWV-l	MT20	4.0	4.0	2.00	1.50
H	TMWV-p	MT20	5.0	6.0	1.50	3.00
J	BMV1-p	MT20	3.0	4.0		
K	BMWV-l	MT20	5.0	6.0	2.50	2.75
L	BMWV-l	MT20	4.0	4.0		
M	BS-l	MT20	3.0	6.0		
N	BMWVW-l	MT20	4.0	8.0		
O	BMWV-l	MT20	4.0	4.0		
P	BMWV-l	MT20	5.0	6.0	2.50	2.75
Q	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
Q	2180	0	2180	0	5-8	5-8
J	2180	0	2180	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM/LIVE			
Q	1620	922/0	305/0	0/0	0/0	394/0	0/0
J	1620	922/0	305/0	0/0	0/0	394/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0/42	-102.1 -102.1	0.14 (1)	10.00	P-C	-288/24	0.10 (1)		
B-C	-2168/0	-102.1 -102.1	0.31 (1)	4.35	C-O	-219/0	0.16 (1)		
C-D	-2045/0	-102.1 -102.1	0.30 (1)	4.46	O-D	0/376	0.06 (2)		
D-E	-1956/0	-102.1 -102.1	0.46 (1)	4.29	D-N	0/579	0.13 (1)		
E-F	-1956/0	-102.1 -102.1	0.46 (1)	4.29	N-E	-713/0	0.64 (1)		
F-G	-2045/0	-102.1 -102.1	0.30 (1)	4.46	N-F	0/579	0.13 (1)		
G-H	-2168/0	-102.1 -102.1	0.31 (1)	4.35	L-F	0/376	0.06 (2)		
H-I	0/42	-102.1 -102.1	0.14 (1)	10.00	L-G	-219/0	0.16 (1)		
Q-B	-2106/0	0.0 0.0	0.22 (1)	5.84	K-G	-288/24	0.10 (1)		
J-H	-2106/0	0.0 0.0	0.22 (1)	5.84	B-P	0/1820	0.41 (1)		
					K-H	0/1820	0.41 (1)		
Q-P	0/0	-38.5 -38.5	0.13 (3)	10.00					
P-O	0/1759	-38.5 -38.5	0.39 (1)	10.00					
O-N	0/1613	-38.5 -38.5	0.40 (2)	10.00					
N-M	0/1613	-38.5 -38.5	0.40 (2)	10.00					
M-L	0/1513	-38.5 -38.5	0.40 (2)	10.00					
L-K	0/1759	-38.5 -38.5	0.39 (1)	10.00					
K-J	0/0	-38.5 -38.5	0.13 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00H/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(95 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = 1/898 (0.14")

CSI: TC=0.48/1.00 (D-E:1), BC=0.40/1.00 (N-O:2), WB=0.64/1.00 (E-N:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

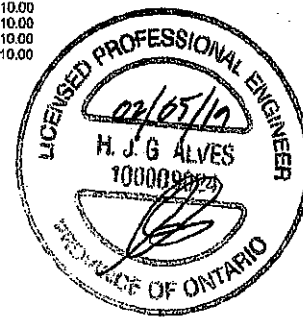
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

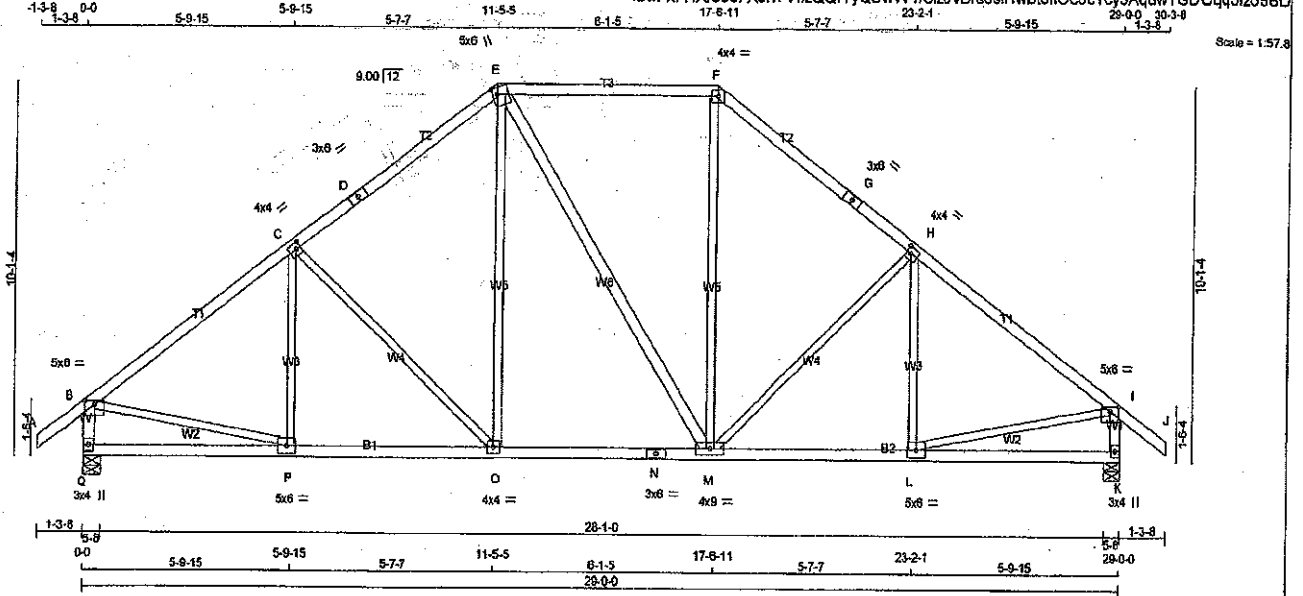
JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL = 0.51 (M) (INPUT = 1.00)



DWG NO. TAM T102650
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T7	2	1	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:IFxHXfe38FX8N?VhzQQIYyQBwW-nCizJvBrseHwb3hC5cTcy3AqdwYGDUqq5Izo56D



TOTAL WEIGHT = 2 X 138 = 275 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
L - M	2x4	DRY	No.2	SPF	
M - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - M	2x4	DRY	No.2	SPF	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TS-i	MT20	3.0	6.0		
E	TYWV+m	MT20	5.0	6.0	2.25	1.50
F	TTW-i	MT20	4.0	4.0		
G	TS-i	MT20	3.0	6.0		
H	TMVW-i	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	4.0		
L	BMVWV-i	MT20	5.0	6.0		
M	BMVWV-i	MT20	4.0	4.0		
N	BS-i	MT20	3.0	6.0		
O	BMVWV-i	MT20	4.0	4.0		
P	BMVWV-i	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
Q	2180	0	2180	0
K	2180	0	2180	0

UNFACTORED REACTIONS

	1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
Q	1620	922 / 0	305 / 0	0 / 0
K	1620	922 / 0	305 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	WEBS	MEMB.	MAX. FACTORED
		FORCE (LBS)	VERT. LOAD LC1 (PLF)			FORCE (LBS)
FR-TO			FROM TO	FR-TO		
A-B	0 / 42		-102.1 -102.1 0.14 (1)	10.00	P-C	-123 / 150
B-C	-2208 / 0		-102.1 -102.1 0.55 (1)	4.04	C-O	-492 / 0
C-D	-1855 / 0		-102.1 -102.1 0.50 (1)	4.39	O-E	0 / 571
D-E	-1855 / 0		-102.1 -102.1 0.50 (1)	4.39	E-M	0 / 1
E-F	-1454 / 0		-102.1 -102.1 0.52 (1)	4.75	M-F	0 / 573
F-G	-1855 / 0		-102.1 -102.1 0.50 (1)	4.39	M-H	-491 / 0
G-H	-1855 / 0		-102.1 -102.1 0.50 (1)	4.39	H-I	-125 / 149
H-I	-2209 / 0		-102.1 -102.1 0.55 (1)	4.04	I-J	0 / 1838
I-J	0 / 42		-102.1 -102.1 0.14 (1)	10.00	J-K	0 / 1838
J-K	-2088 / 0		0.0 0.0 0.22 (1)	5.86		
K-L	-2087 / 0		0.0 0.0 0.22 (1)	5.86		
Q-P	0 / 0		-38.5 -38.5 0.24 (3)	10.00		
P-O	0 / 1801		-38.5 -38.5 0.46 (2)	10.00		
O-N	0 / 1453		-38.5 -38.5 0.39 (2)	10.00		
N-M	0 / 1453		-38.5 -38.5 0.39 (2)	10.00		
M-L	0 / 1800		-38.5 -38.5 0.46 (2)	10.00		
L-K	0 / 0		-38.5 -38.5 0.23 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.97")
CALCULATED VERT. DEFL. (LL) = 1/989 (0.08")
ALLOWABLE DEFL. (TL) = 1/360 (0.97")
CALCULATED VERT. DEFL. (TL) = 1/989 (0.14")

CSI: TC=0.55/1.00 (B-C-1), BC=0.46/1.00 (O-P-2),

WB=0.65/1.00 (C-O-1), SS=0.24/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

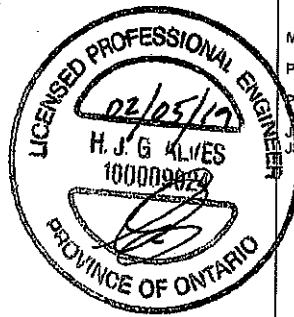
PLATE	GRIP (DRY)	SHEAR	SECTION
(P8)	(FLI)	(FLI)	(FLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JN GRIP=0.88 (F) (INPUT = 0.90)

JN METAL=0.46 (N) (INPUT = 1.00)

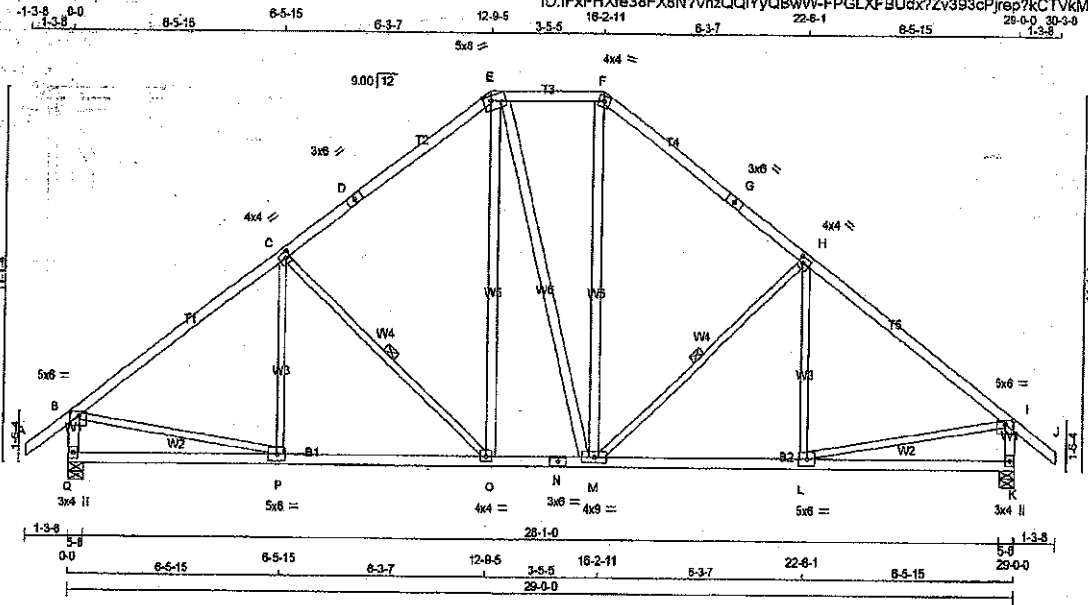


DWG NO. TAM 7902652
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T8	3	1	Preston 1	

Tamarack Roof Truss, Burlington

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ID:IFxHFXe38FX8N7VhzQQIYyQBwW-FPGLXFBUdx?Zv393cPjrep?KCTVkmRZPR8ZOdBzo56C



Scale: 3/16"=1'

TOTAL WEIGHT = 3 X 150 = 450 LB

LUMBER	N.L.O.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - F	2x4	DRY	No.2	SPF		
F - G	2x4	DRY	No.2	SPF		
G - J	2x4	DRY	No.2	SPF		
Q - B	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
Q - N	2x4	DRY	No.2	SPF		
N - K	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
O - E	2x4	DRY	No.2	SPF		
E - M	2x4	DRY	No.2	SPF		
M - F	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVV-p	MT20	5.0	6.0	1.50	3.00
C TMWW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	6.0		
E TTVW-m	MT20	5.0	8.0	1.75	2.75
F TTVW-m	MT20	4.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMWW-t	MT20	4.0	4.0	2.00	1.50
I TMVV-p	MT20	5.0	6.0	1.50	3.00
K BMV1-p	MT20	3.0	4.0		
L BMWW-t	MT20	5.0	6.0		
M BMWW-t	MT20	4.0	9.0		
N BS-t	MT20	3.0	6.0		
O BMWW-t	MT20	4.0	4.0		
P BMWW-t	MT20	5.0	8.0		
Q BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
Q	2180	0	2180	0	5-8	5-8
K	2180	0	2180	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
K	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00	P-C	-43 / 221	0.05 (3)	
B-C	-2211 / 0	-102.1 -102.1 0.71 (1)	3.81	C-O	-628 / 0	0.34 (1)	
C-D	-1738 / 0	-102.1 -102.1 0.63 (1)	4.30	O-E	0 / 597	0.10 (1)	
D-E	-1738 / 0	-102.1 -102.1 0.63 (1)	4.30	E-M	0 / 4	0.00 (1)	
E-F	-1359 / 0	-102.1 -102.1 0.18 (1)	5.40	M-F	0 / 601	0.10 (1)	
F-G	-1739 / 0	-102.1 -102.1 0.63 (1)	4.30	M-H	-628 / 0	0.33 (1)	
G-H	-1739 / 0	-102.1 -102.1 0.63 (1)	4.30	L-H	-46 / 210	0.05 (3)	
H-I	-2210 / 0	-102.1 -102.1 0.71 (1)	3.81	B-P	0 / 1836	0.41 (1)	
I-J	0 / 42	-102.1 -102.1 0.14 (1)	10.00	L-I	0 / 1836	0.41 (1)	
Q-B	-2080 / 0	0.0 0.0 0.22 (1)	5.87				
K-I	-2080 / 0	0.0 0.0 0.22 (1)	5.87				
Q-P	0 / 0	-38.5 -38.5 0.32 (3)	10.00				
P-O	0 / 1806	-38.5 -38.5 0.55 (2)	10.00				
O-N	0 / 1357	-38.5 -38.5 0.33 (1)	10.00				
N-M	0 / 1357	-38.5 -38.5 0.33 (1)	10.00				
M-L	0 / 1808	-38.5 -38.5 0.54 (2)	10.00				
L-K	0 / 0	-38.5 -38.5 0.32 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.71/1.00 (B-C-1), BC=0.55/1.00 (O-P-2), WE=0.41/1.00 (B-P-1), SI=0.28/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

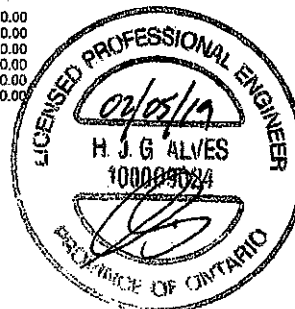
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 769 1887 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (Q) (INPUT= 0.90)
JSI METAL= 0.49 (N) (INPUT= 1.00)



DWG NO. TAM T1902653
STRUCTURAL
COMPONENT ONLY

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 ID:IFxHXf838FX8N?VzhQQiYyQBwwjbbqkBC6OF7QWDkGA6E4B1YxM(tN5vzJgoJxAezo56B
 1-3-8 1.3.8 0-0
 5-1-0



TOTAL WEIGHT = 50 lb

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H. F.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

FACTORED CONCENTRATED LOADS (LBS)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.62/1.00 (C-D:1), BC=0.71/1.00 (G-H:1),
WB=0.36/1.00 (C-G:1), SS=0.72/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1667	786	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

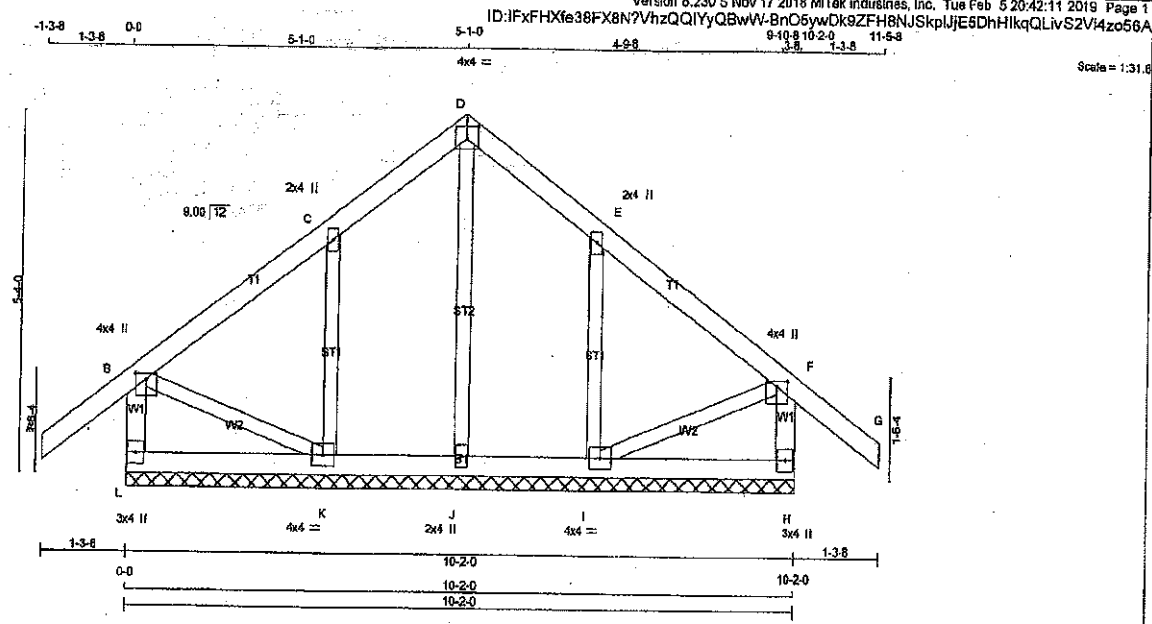
PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.84 (D) (INPUT = 0.90)

CONFIDENCE= 0.99 (C) (INFO= 1.00)

DWG NO. TAM 71902654
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T9G	QUANTITY 1	PLY 1	JOB DESC. Preston 1	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY No.2	SPF
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

ALL GABLE WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TMVW+w	MT20	2.0	4.0		
D TTW+p	MT20	4.0	4.0	2.25	2.00
E TMVW+w	MT20	2.0	4.0		
F TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMVW1+H	MT20	4.0	4.0		
J BMV1+w	MT20	2.0	4.0		
K BMVW1+H	MT20	4.0	4.0		
L BMV1+p	MT20	3.0	4.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED LC1 (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM	TO	FR-TO		FROM	TO
L-B	-283 / 0	0.0	0.0	J-D	-117 / 0	0.05 (1)	7.81
A-B	0 / 42	-102.1	-102.1	K-C	-325 / 0	0.08 (1)	10.00
B-C	-3 / 0	-102.1	-102.1	I-E	-325 / 0	0.08 (1)	10.00
C-D	-35 / 0	-102.1	-102.1	B-K	0 / 17	0.00 (1)	6.25
D-E	-35 / 0	-102.1	-102.1	I-F	0 / 17	0.00 (1)	6.25
E-F	-3 / 0	-102.1	-102.1				10.00
F-G	0 / 42	-102.1	-102.1				10.00
H-F	-283 / 0	0.0	0.0				7.81
L-K	0 / 0	-38.5	-38.5				10.00
K-J	0 / 0	-38.5	-38.5				10.00
J-I	0 / 0	-38.5	-38.5				10.00
I-H	0 / 0	-38.5	-38.5				10.00

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G-1), BC=0.07/1.00 (I-J-3), WB=0.08/1.00 (E-F-1), SS=0.11/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

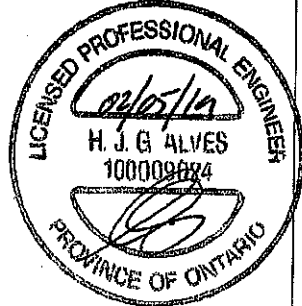
PLATE GRIP (DRY) SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1867	788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

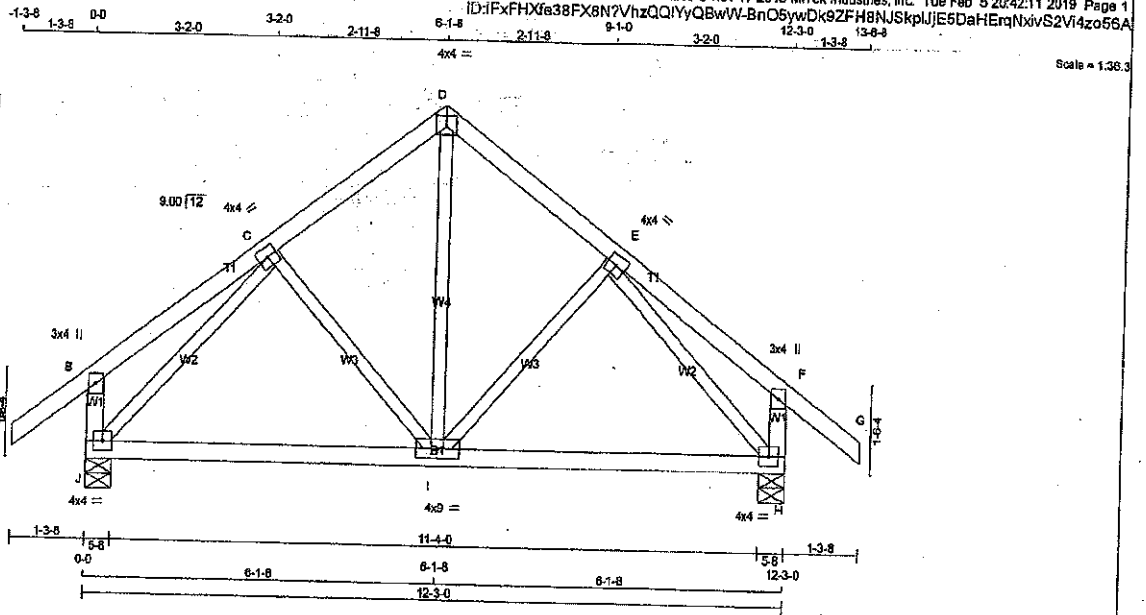
JSI GRIP= 0.23 (C) (INPUT = 0.80)

JSI METAL= 0.17 (C) (INPUT = 1.00)



DWG NO. TAM 7902655
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T10-Cond1	6	1	Preston 1	
Tamarack Roof Truss, Burlington					



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - D	2x4	DRY	No.2	SPF
	D - G	2x4	DRY	No.2	SPF
	J - B	2x4	DRY	No.2	SPF
	H - F	2x4	DRY	No.2	SPF
	J - H	2x4	DRY	No.2	SPF
	ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-H	MT20	4.0	4.0		
D	TMV-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-H	MT20	4.0	4.0		
F	TMV-p	MT20	3.0	4.0		
H	BMVW-H	MT20	4.0	4.0		
I	BMVW-H	MT20	4.0	9.0		
J	BMVW-H	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRO BRG
JT	VERT	DOWN	UPLIFT	IN-SX
J	1002	0	0	5-8
H	1002	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	740	436 / 0	129 / 0	0 / 0	0 / 0	176 / 0	0 / 0
H	740	436 / 0	129 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00								
B-C	0 / 21	-102.1	-102.1	0.15 (1)	10.00								
C-D	-827 / 0	-102.1	-102.1	0.11 (1)	6.25								
D-E	-827 / 0	-102.1	-102.1	0.11 (1)	6.25								
E-F	0 / 21	-102.1	-102.1	0.15 (1)	10.00								
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00								
J-B	-262 / 0	0.0	0.0	0.03 (1)	7.81								
H-F	-262 / 0	0.0	0.0	0.03 (1)	7.81								
J-I	0 / 589	-38.5	-38.5	0.38 (2)	10.00								
I-H	0 / 589	-38.5	-38.5	0.38 (2)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.41")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.15/1.00 (B-C:1), BC=0.38/1.00 (J:2), WB=0.29/1.00 (C-J:1), SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

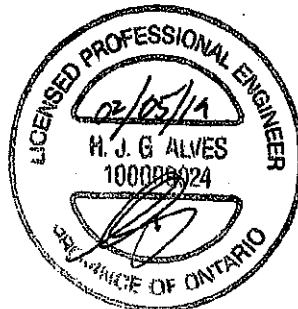
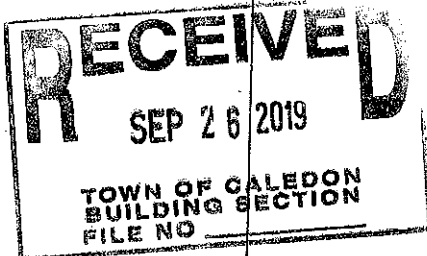
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

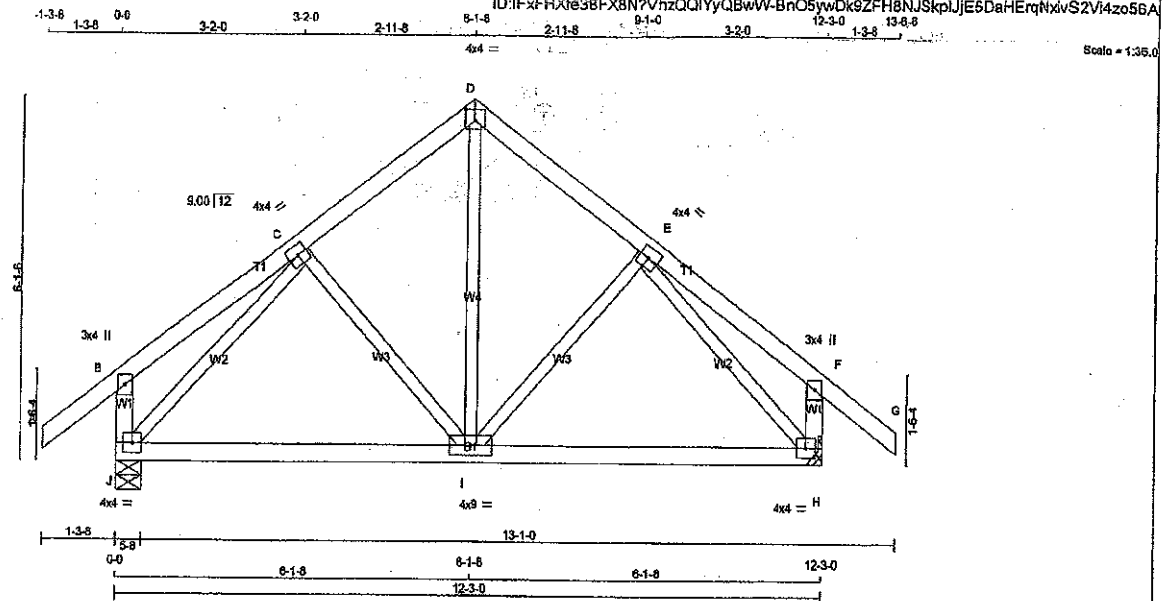
JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)



DRWG NO. TAM 19402656
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T10-Cond2	6	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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TOTAL WEIGHT = 6 X 56 = 336 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table 6 in inches)	W	LEN	Y	X
JT TYPE	MT20	3.0	4.0	
B TMV+p	MT20	4.0	4.0	
C TMVW-t	MT20	4.0	4.0	
D TTW-p	MT20	4.0	4.0	2.25 2.00
E TMVW-t	MT20	4.0	4.0	
F TMV+p	MT20	3.0	4.0	
H BMVW-t	MT20	4.0	4.0	
I BMVW-t	MT20	4.0	9.0	
J BMVW-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	1002	0	1002	0
H	1002	0	1002	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	740	436 / 0	128 / 0	0 / 0	176 / 0	0 / 0
H	740	436 / 0	128 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	I-D	0 / 481
B-C	0 / 21	-102.1	-102.1	0.15 (1)	10.00	I-E	-127 / 29
C-D	-827 / 0	-102.1	-102.1	0.11 (1)	6.25	C-I	-127 / 29
D-E	-827 / 0	-102.1	-102.1	0.11 (1)	6.25	J-C	-854 / 0
E-F	0 / 21	-102.1	-102.1	0.15 (1)	10.00	E-H	-854 / 0
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
J-B	-282 / 0	0.0	0.0	0.03 (1)	7.81		
H-F	-282 / 0	0.0	0.0	0.03 (1)	7.81		
J-I	0 / 569	-38.5	-38.5	0.36 (2)	10.00		
I-H	0 / 569	-38.5	-38.5	0.36 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 28.0 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.41")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.41")
CALCULATED VERT. DEFL. (TL) = $L/899$ (0.07")

CSI: TC=0.15/1.00 (B-C:1), BC=0.38/1.00 (I-J:2), WB=0.29/1.00 (C-J:1), SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

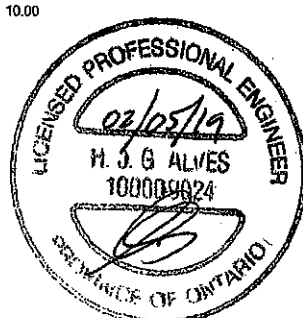
NAIL VALUES
PLATE GRIP (DRY) (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 788 1587 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)

JSI METAL= 0.31 (C) (INPUT = 1.00)

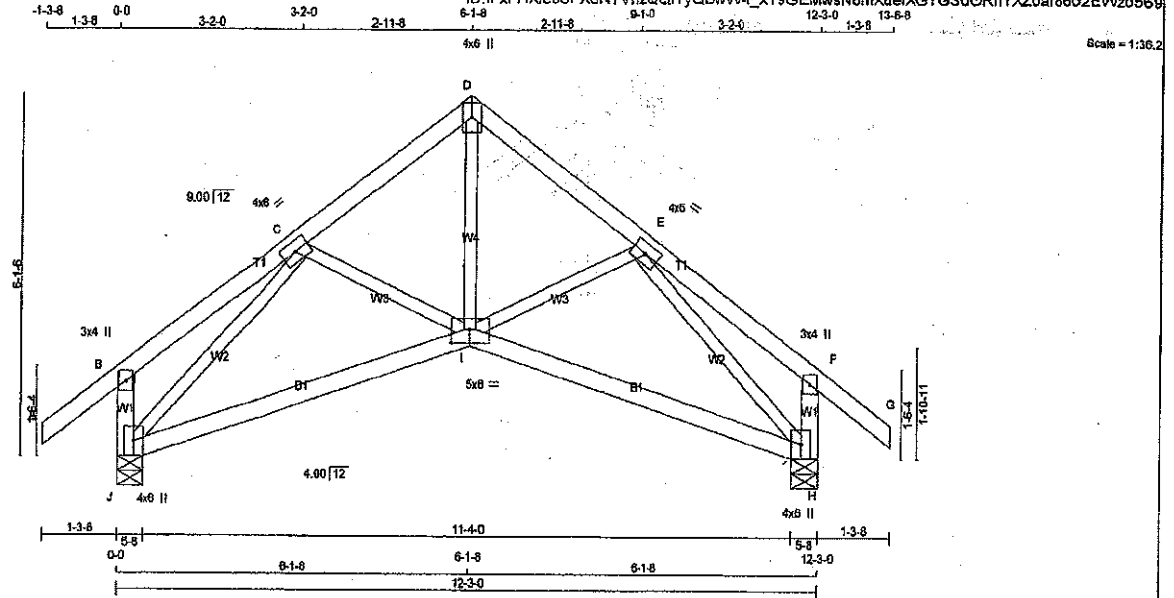


DWG NO. TAM 17902657
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T10S	2	1	Preston 1	

Tamersack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 54 = 108 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
A - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TMVW-p	MT20	4.0	6.0		Edge
E	TMVW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BVMW1+p	MT20	4.0	6.0		
I	BBVWV-p	MT20	5.0	8.0	2.75	4.00
J	BVMW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
J	1002	0	1002	0	0	5-8	5-8		
H	1002	0	1002	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	740	438 / 0	128 / 0	0 / 0	0 / 0	176 / 0	0 / 0
H	740	438 / 0	128 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	I-D	0 / 787	0.17 (1)	
B-C	0 / 19	-102.1 -102.1	0.13 (1)	I-E	-52 / 55	0.01 (3)	
C-D	-866 / 0	-102.1 -102.1	0.11 (1)	C-I	-52 / 55	0.01 (3)	
D-E	-866 / 0	-102.1 -102.1	0.11 (1)	J-C	-1124 / 0	0.40 (1)	
E-F	0 / 19	-102.1 -102.1	0.13 (1)	E-H	-1124 / 0	0.40 (1)	
F-G	0 / 42	-102.1 -102.1	0.14 (1)				
J-B	-286 / 0	0.0	0.03 (1)				
H-F	-286 / 0	0.0	0.03 (1)				
J-I	0 / 764	-38.5	-38.5 0.41 (2)				
I-H	0 / 764	-38.5	-38.5 0.41 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")
CALCULATED VERT. DEFL. (LL) = L/989 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (0.41")
CALCULATED VERT. DEFL. (TL) = L/846 (0.17")

CSI: TC=0.14/1.00 (A-B:1), BC=0.41/1.00 (I-J:2), WB=0.40/1.00 (C-J:1), SS=0.12/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 L3 BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

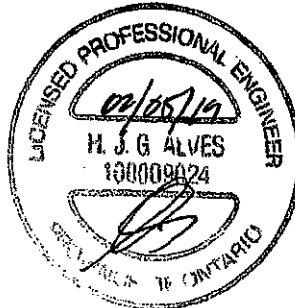
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 610 354 1087 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (J) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

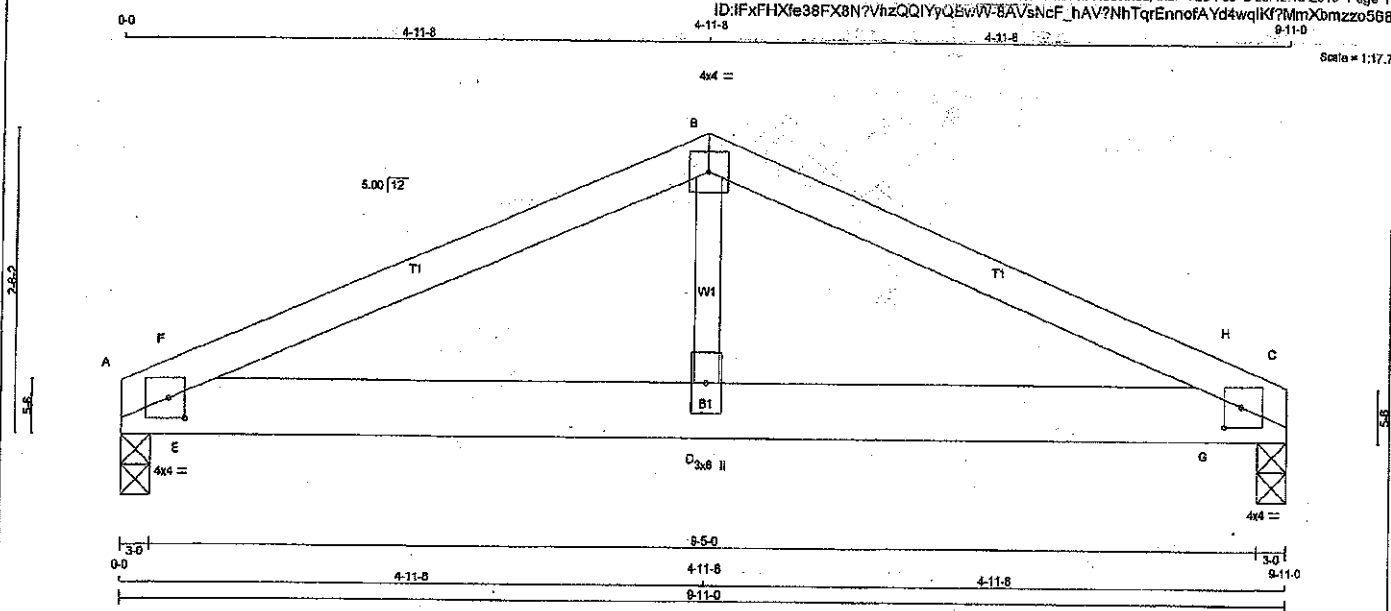


DRWG NO. TAM 71902658
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T11	2	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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TOTAL WEIGHT = 2 X 32 = 63 lb (MIF)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - B	2x4 DRY	No.2	SPF
B - C	2x4 DRY	No.2	SPF
A - C	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMB1-I	MT20	4.0	4.0	2.00 1.75
B	ITW-p	MT20	4.0	4.0	
C	TMB1-I	MT20	4.0	4.0	2.00 1.75
D	BMW+w	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
A		697	0	697	0	0	3-0	3-0	
C		697	0	697	0	0	3-0	3-0	

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
A	521	288 / 0	104 / 0	0 / 0	0 / 0	129 / 0	0 / 0	
C	521	288 / 0	104 / 0	0 / 0	0 / 0	129 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO
A-F	-897 / 0	-102.1 -102.1 0.17 (1)	6.25 D-B 0 / 385
F-B	-899 / 0	-102.1 -102.1 0.24 (1)	5.99 E-F -452 / 0
B-H	-899 / 0	-102.1 -102.1 0.24 (1)	5.99 G-H -452 / 0
H-C	-897 / 0	-102.1 -102.1 0.17 (1)	6.25
A-E	0 / 901	-38.5 -38.5 0.28 (1)	10.00
E-D	0 / 901	-38.5 -38.5 0.28 (1)	10.00
D-G	0 / 901	-38.5 -38.5 0.28 (1)	10.00
G-C	0 / 901	-38.5 -38.5 0.28 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/998 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/998 (0.03")

CSI: TC=0.24/1.00 (B-F:1) , BC=0.28/1.00 (A-E:1) , WB=0.09/1.00 (B-D:2) , SS=0.27/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

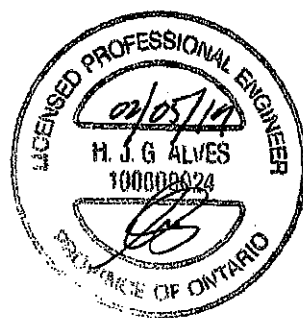
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 766 1967 1656

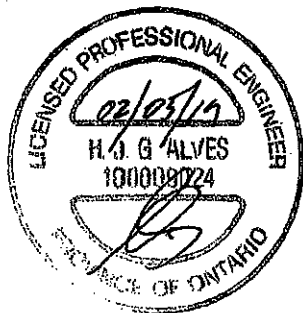
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (A) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

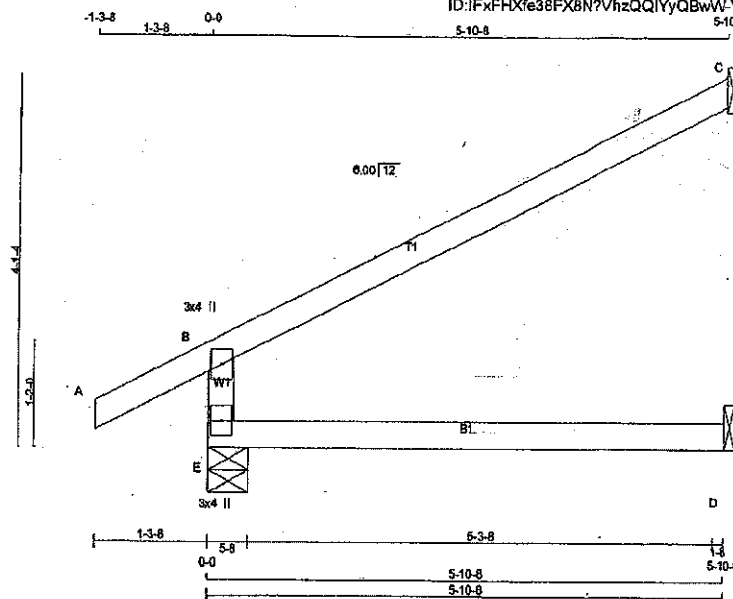


DWG NO. TAM 17902659
STRUCTURAL
COMPONENT ONLY

Scale = 1:17.7

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J1-Cond1	22	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:23.6

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMV+p	MT20	3.0	4.0
E BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	646	0	646	0	0	5-8	5-8
C	225	0	225	0	0	1-8	1-8
D	93	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	292 / 0	72 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
E-B	-513 / 0	0.0	0.0 0.22 (3)	7.81	
A-B	0 / 31	-102.1	-102.1 0.13 (1)	10.00	
B-C	-34 / 0	-102.1	-102.1 0.60 (1)	6.25	
E-D	0 / 0	-38.5	-38.5 0.22 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/990 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/942 (0.07")

CSI: TC=0.80/1.00 (B-C-1), BC=0.22/1.00 (D-E-3),
WB=0.00/1.00 (H/A-0), SSI=0.26/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

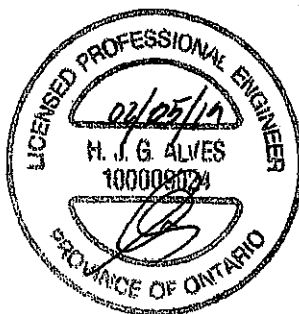
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

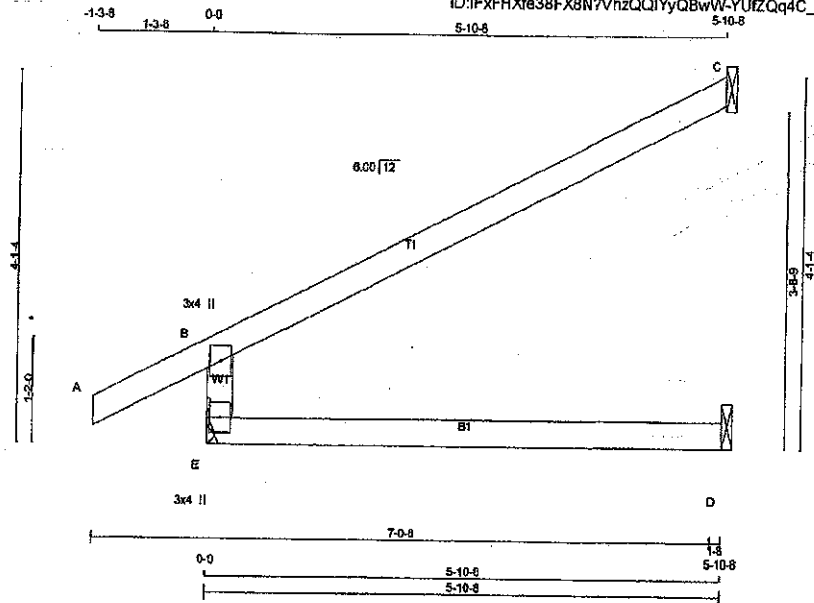
JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)



DWG NO. TAM 77902661
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J1-Cond2	22	1	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/2" = 1'-0"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
E - D	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	GROSS REACTION	DOWN	UP		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
E	648	0	648	0	0	MECHANICAL
C	225	0	225	0	1-8	1-8
D	93	0	119	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	473	292 / 0	72 / 0	0 / 0	0 / 0	109 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	28 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
E-B	-513 / 0	0.0	0.0	0.18 (3)
A-B	0 / 31	-102.1	-102.1	0.13 (1)
B-C	-34 / 0	-102.1	-102.1	0.46 (1)
E-D	0 / 0	-38.5	-38.5	0.22 (3)

TOTAL WEIGHT = 22 X 17 = 369 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0 PSF
	DL = 8.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = 1/399 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = 1/342 (0.07")

CSI: TC=0.46/1.00 (B-C:1), BC=0.22/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	788
	1867	1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

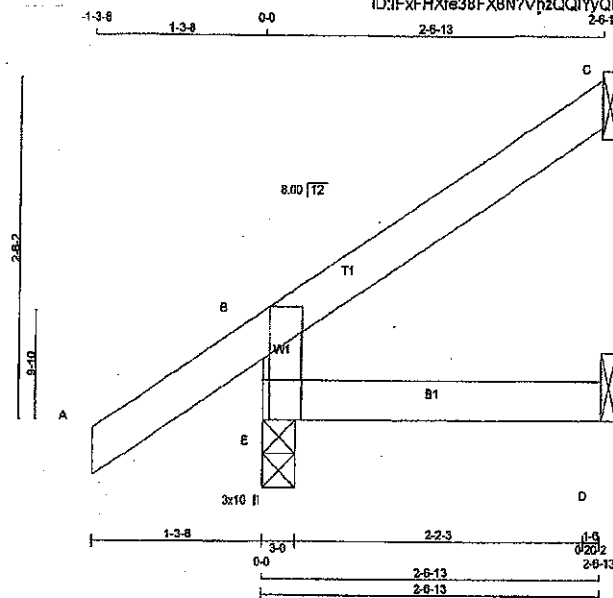


DWG NO. TAM 77902662
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Presion 1	DRWG NO.
200168-400364	J3	5	1	TRUSS DESC.		

Tamrack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:41:59 2019 Page 1
ID:IFxHFXfe38FX8N7VhzQQIYyQBwW-YUfZQq4C_tk_XP81DXVEibAkRCI00Yx8aBsHnzo58M



Scale = 1:15.8

TOTAL WEIGHT = 5 X 9 = 44 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B					
E	TMBMV1+p	MT20	3.0	10.0	Edge 0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT HORZ	JT VERT HORZ	UPLIFT	IN-SX	IN-SX
E 351 0	361 0	3-0	3-0	
C 88 0	88 0	1-8	1-8	
D 42 0	53 0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	VERT	SNOW	LIVE	PERM	LIVE	WIND
E 281	173/0	31/0	0/0	0/0	57/0	0/0
C 68	56/0	0/0	0/0	0/0	12/0	0/0
D 38	0/0	23/0	0/0	0/0	15/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-304/0	0.0	0.04 (3)				
A-B	0/39	-102.1	-102.1 0.14 (1)				
B-C	-18/0	-102.1	-102.1 0.11 (1)				
E-D	0/0	-38.5	-38.5 0.04 (3)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.14/1.00 (A-B:1), BC=0.04/1.00 (D-E:3), WB=0.00/1.00 (A-E:0), SS1=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

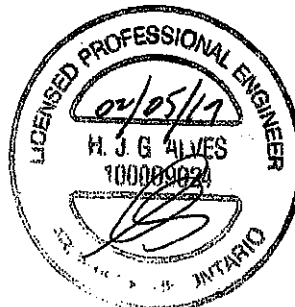
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1887 788	1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (E) (INPUT = 0.80)
JSI METAL= 0.10 (E) (INPUT = 1.00)



DWG NO. TAM 17902663
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 M&T Industries, Inc. Tue Feb 5 20:42:00 2019 Page 1
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TOTAL WEIGHT = $2 \times 15 = 29$ lb

DWG NO. TAM 77902664
STRUCTURAL
COMPONENT ONLY



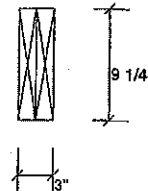
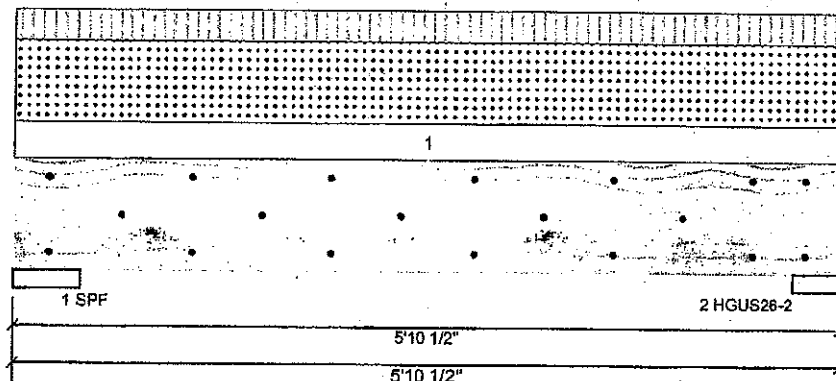
isDesign™

Client: Greenpark Homes
 Project: Preston 1
 Address: Caledon

Date: 2019-02-05
 Designer: Brian
 Job Name: Lamberts Lane Homes Corp.
 Project #: 200168

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	231	273	609	0
2	221	262	584	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	15%	341 / 1145	1486 L	1.25D+1.5S +L
2 - HGUS...	4.000"	19%	327 / 1097	1424 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1679 ft-lb	3'	6039 ft-lb	0.278 (28%)	1.25D+1.5S +L	L
Unbraced	1679 ft-lb	3'	5236 ft-lb	0.321 (32%)	1.25D+1.5S +L	L
Shear	1290 lb	12"	3984 lb	0.324 (32%)	1.25D+1.5S +L	L
Perm Defl in. (L/11306)	0.006	3'	0.174 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.015 (L/4260)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.020 (L/3094)	3'	0.174 (L/360)	0.120 (12%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 71902665
 STRUCTURAL
 COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		7'-0-0	Near Face	13 PSF	11 PSF	20 PSF	0 PSF	

Manufacturer Info

TAMARACK LUMBER
 3255 NORTH SERVICE RD, ON
 CANADA
 (905) 335-1115



This design is valid until 2021-12-11





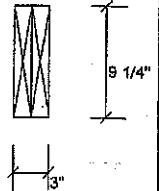
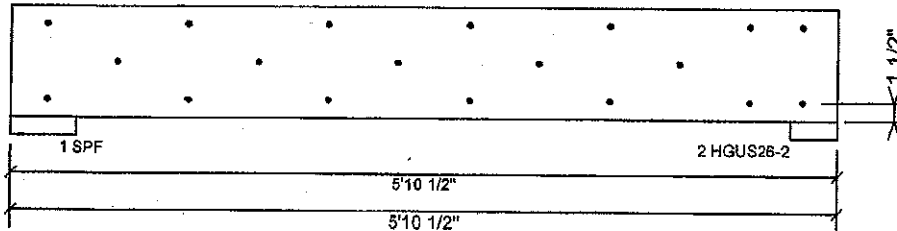
isDesign™

Client: Greenpark Homes
Project: Preston 1
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200168

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

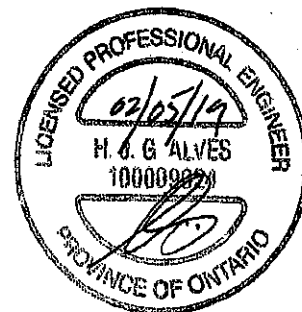
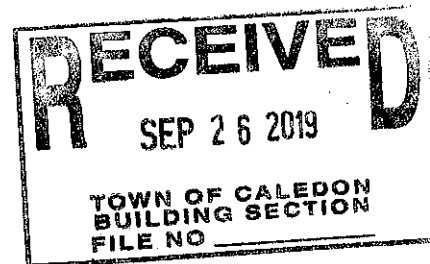
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	72.8 %
Load	247.6 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO: TAM 77902665
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info

TAMARACK LUMBER
3255 NORTH SERVICE RD, ON
CANADA
(905) 335-1115



This design is valid until 2021-12-11



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

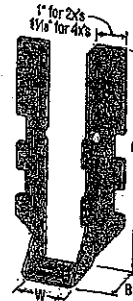
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



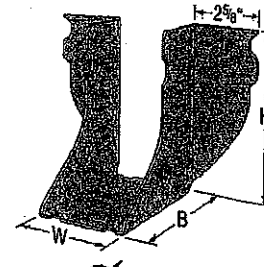
✓ LUS28



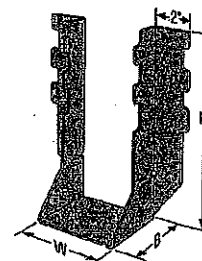
LU26L



✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



Double-Shear
Nailing
Top View

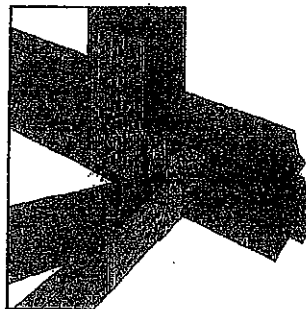


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

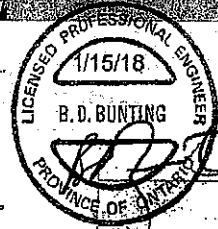
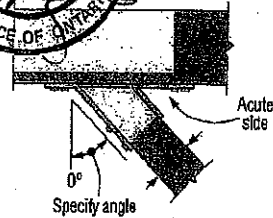
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value

**SIMPSON**
Strong-Tie

Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	da	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	da ³	Header	Joist	S-P-F			
								Uplift		Normal	
								lb.	lb.	(K _o = 1.15)	(K _o = 1.00)
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								360	1020	320	725
								720	1605	645	1140
LUS24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	1420	2170	1290	1630
								2705	4040	2065	3876
								2055	4285	1480	4115
LUS26	22	1 1/8	5	1 1/4	4 1/8	(6) 10d	(4) 10d x 1 1/2"	2885	6625	2685	5700
								1140	2185	1020	1550
								1420	2520	1290	1790
LUS26S	18	1 1/8	4 1/4	1 3/4	3 1/4	(4) 10d	(4) 10d	3605	5305	2675	4345
								3310	7675	3310	8900
								3110	7475	3110	8700
LUS26DS	18	1 1/8	5	3 1/2	4 1/4	(10) 10d	(6) 16d	1140	2495	1020	1770
								1420	2785	1290	2210
								1620	3075	1480	2430
HUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	1140	2495	1020	1770
								1420	2785	1290	2210
								1620	3075	1480	2430
LUS28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								1420	2785	1290	2210
								1620	3075	1480	2430
LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	1140	2495	1020	1770
								1420	2785	1290	2210
								1620	3075	1480	2430
HUS28	16	1 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	1140	2495	1020	1770
								1420	2785	1290	2210
								1620	3075	1480	2430
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	1140	2495	1020	1770
								1420	2785	1290	2210
								1620	3075	1480	2430
LUS210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								1420	2785	1290	2210
								1620	3075	1480	2430
LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1140	2495	1020	1770
								1420	2785	1290	2210
								1620	3075	1480	2430

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.

2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.

3. da is the distance from the bearing seat to the top joist nail.

4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSON and/or see installation notes.

5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

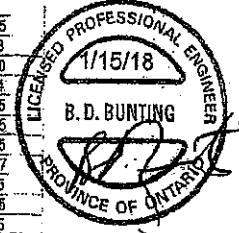
Face-Mount Hangers

SIMPSON
Strong-Tie

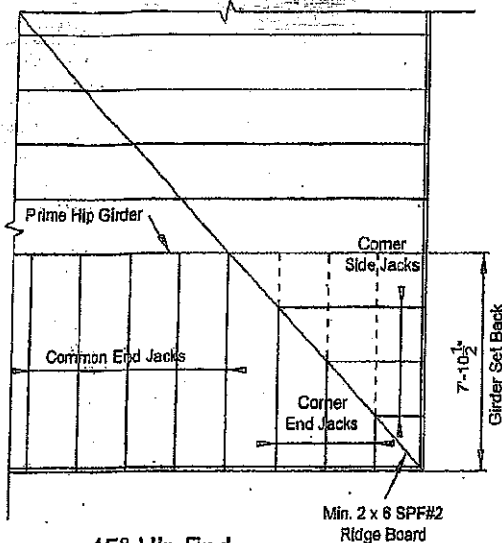
These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	dg	Header	Joist	D.F. Ed.		S-P-F	
								Uplift (K _o = 1.15)	Normal (K _o = 1.00)	Uplift (K _o = 1.15)	Normal (K _o = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/4	3 1/4	2	1 1/4	(4) 16d	(2) 16d	835	2020	580	1435
SS LUS26-2	18	3 1/4	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS26-2	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2650	7335	2085	5205
HGUS26-2	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
SS LUS28-2	18	3 1/4	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
HHUS28-2	14	3 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3765	8940	2675	6345
HGUS28-2	12	3 1/4	7 1/4	4	8 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
SS LUS210-2	18	3 1/4	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
HHUS210-2	14	3 1/4	9 1/4	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HGUS210-2	12	3 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	6840	14015	4855	10270
Triple 2x Sizes											
HGUS26-3	12	4 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-3	12	4 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-3	14	4 1/4	9	3	7 1/4	(30) 16d	(10) 16d	4670	9670	4235	6865
HGUS210-3	12	4 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	6840	14645	4855	10400
Quadruple 2x Sizes											
HGUS26-4	12	6 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-4	12	6 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-4	14	6 1/4	9	3	7 1/4	(30) 16d	(10) 16d	4670	10155	4235	7210
HGUS210-4	12	6 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d	10130	16400	7195	11645
4x Sizes											
LUS46	18	3 1/4	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS46	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2640	7335	2085	5205
HGUS46	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
LUS48	18	3 1/4	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2575
HHUS48	14	3 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3765	8940	2675	6345
HGUS48	12	3 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
LUS410	18	3 1/4	8 1/4	2	5 1/4	(8) 16d	(6) 16d	2580	4500	2320	3195
HGUS410	12	3 1/4	9	4	8 1/4	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS412	12	3 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS414	12	3 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d	10130	16400	7195	11645



Plated Truss Connectors



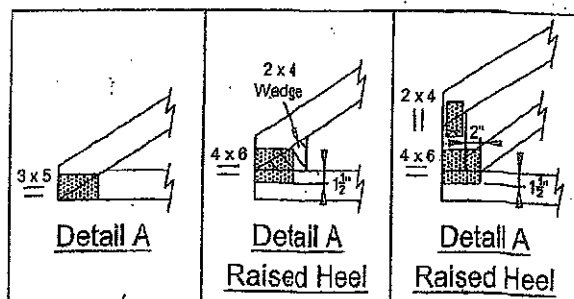
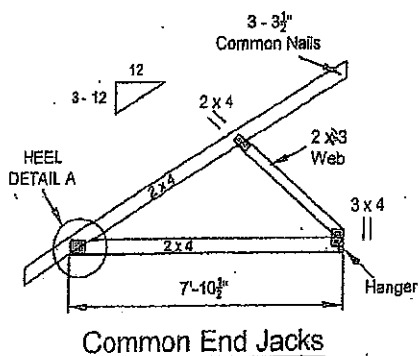
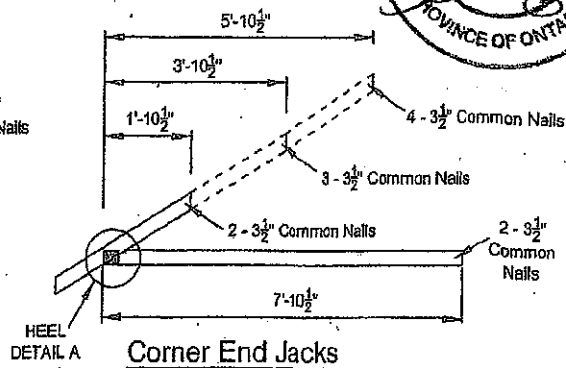
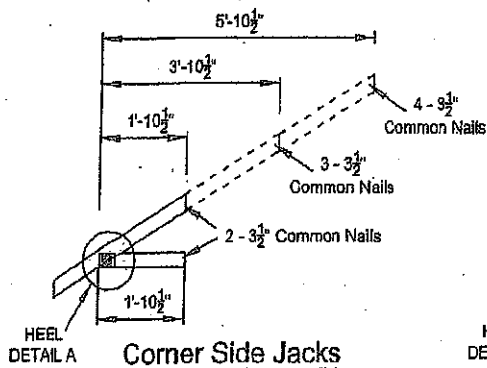
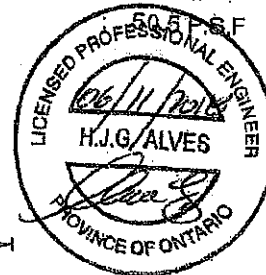
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

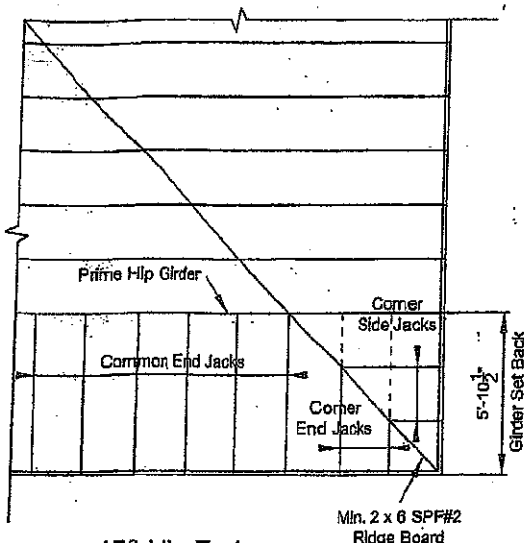
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

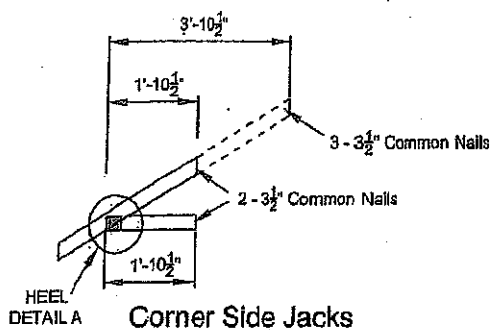
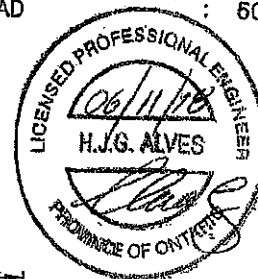
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

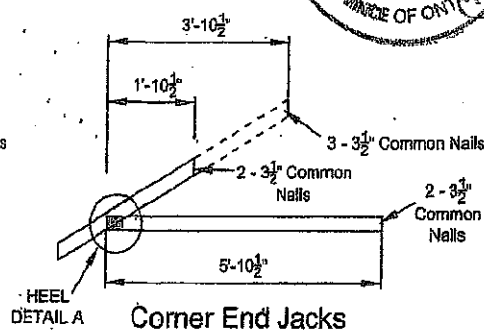
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

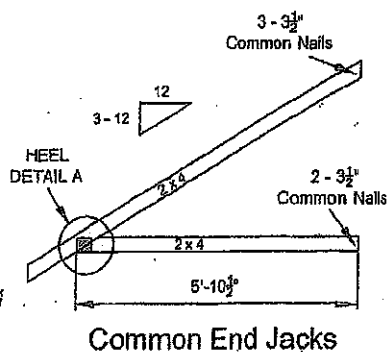
TOTAL LOAD : 50.5 P.S.F



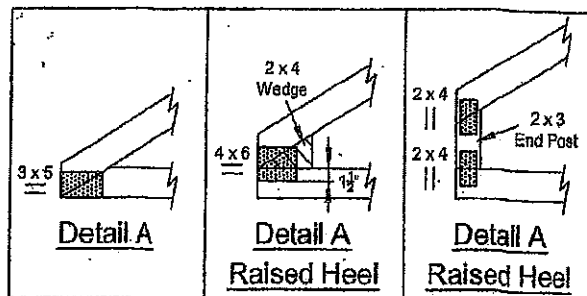
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

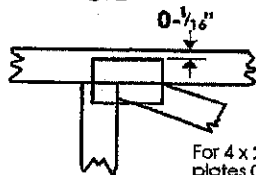
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

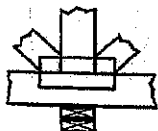
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

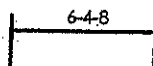


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

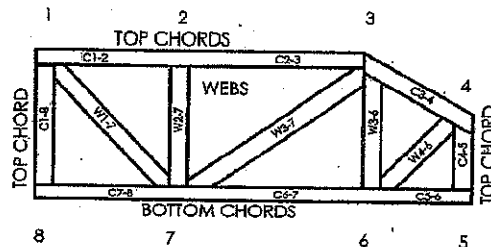
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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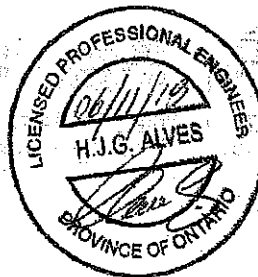
MiTek Engineering Reference Sheet: MII-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900219

Feb 09, 2018