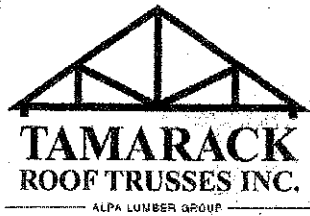


Mitek ver 8.3.0.234



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BLOCK 88
 Lot #:
 Elevation: UNITS 88-1 - 88-4

Job Track: 50033
 PlanLog: 201718
 Layout ID: 405533
 Ref #
 Page: 2 of 2
 Date: 09/10/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	J26S Jack-Open	10/12	3-06-00	4-10-07	2 x 4		1-11-07 4-10-07	70.46 46.67		
	2	J27 Jack-Open	8/12	1-10-08	3-01-10	2 x 4	1-03-08 8-11	1-04-13 2-07-13	17.56 10.67		
	4	J28 Jack-Open	10/12	1-08-00	2-08-03	2 x 4	10-08	1-03-08 2-08-03	27.74 18.67		

TOTAL # TRUSS= 48

TOTAL BFT OF ALL TRUSSES= 1393.17

BFT. TOTAL WEIGHT OF ALL TRSSES 2184.88 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	
1	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF
ITEMS= 8

 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: LAMBERT LANE PH.2 Location: CALEDON Model: BLOCK 88 Lot #: Elevation: UNITS 88-1 - 88-4		Job Track: 50033 PlanLog: 201718 Layout ID: 405533 Ref #: Page: 1 of 2 Date: 09/10/2019 Designer: Andrew Conway Sales Rep: Mario DiCano	
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	H41 Half Hip Girder	8 /12	22-02-10	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	210.24 132.00		
	1	H42 Half Hip	8 /12	22-02-10	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	92.51 57.67		
	1	H43 Half Hip	5 /12	22-02-10	6-01-04	2 x 4	1-03-08	1-00-09 6-01-04	93.88 59.83		
	1	H44 Half Hip	5 /12	22-02-10	7-01-04	2 x 4	1-03-08	1-00-09 7-01-04	94.33 59.00		
	1	H45 Half Hip	5 /12	22-02-10	8-01-04	2 x 4	1-03-08	1-00-09 8-01-04	98.76 62.17		
	1	H46 Half Hip	5 /12	22-02-10	9-01-04	2 x 4	1-03-08	1-00-09 9-01-04	103.9 64.83		
	1	H47 Half Hip	5 /12	22-02-10	10-01-04	2 x 4	1-03-08	1-00-09 10-01-04	101.28 62.83		
	8	H48 Monopitch	5 /12	22-02-10	10-03-10	2 x 4	1-03-08	1-00-09 10-03-11	764.85 476.00		
	1	H49 Common	8 /12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	52.37 31.83		
	1	H63 Hip Girder	8 /12	11-10-00	4-10-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	54.26 33.83		
	1	H64 Hip Girder	8 /12	9-00-00	2-08-03	2 x 4 2 x 6		1-01-08 1-01-08	39.2 26.17		
	6	J3 Jack-Open	4 /12	5-04-00	2-06-03	2 x 4	1-03-08	7-15 2-02-04	88.67 64.00		
	6	J4 Jack-Closed	4 /12	6-07-00	3-02-03	2 x 4	1-03-08	7-15 2-10-04	174.1 123.00		
	6	J18 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		

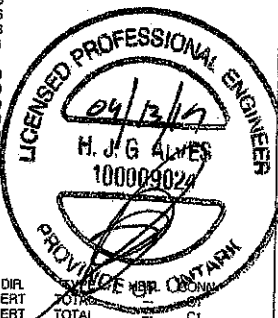
Version 8.300 S May 10 2019 MITek Industries, Inc. Fri Sep 13 10:59:14 2019 Page 1
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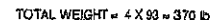
Circumstance	All respondents (%)	Men (%)	Women (%)
If someone is attacking you	~85	~85	~85
If someone is threatening you	~75	~75	~75
If someone is harassing you	~65	~65	~65
If someone is insulting you	~55	~55	~55
If someone is annoying you	~45	~45	~45

18-6-14	-26	-26	---	FRONT	VERT	TOTAL	---	C1
20-6-14	-26	-26	---	FRONT	VERT	TOTAL	---	C1

Structural component only
DW/G# T-1923440



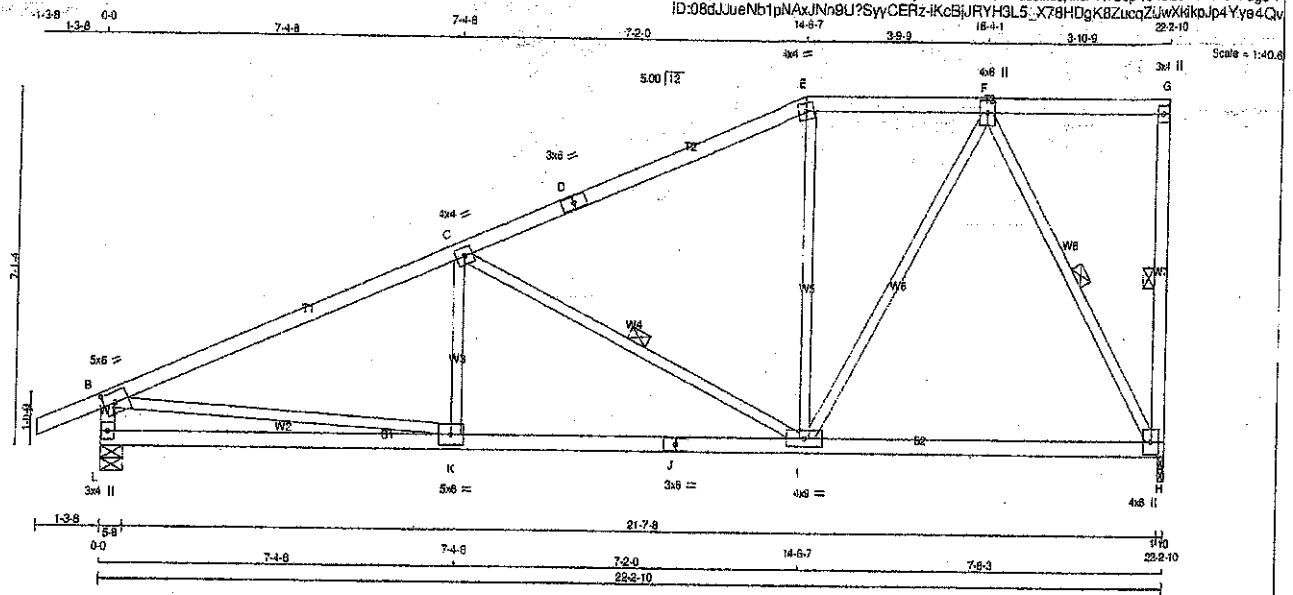
Tamarack Roof Truss, Burlington



Structural component only
DWG# T-1923441

[illegible]

JOB NAME 405490	TRUSS NAME H44	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 S May 10 2019 MTK Industries, Inc. Fri Sep 13 10:52:16 2019 Page 1			
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CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE	MT20	5.0	6.0	2.50 2.75
B TMWV-t	MT20	4.0	4.0	
C TMWV-t	MT20	3.0	6.0	
D TS-t	MT20	4.0	4.0	
E TTW-m	MT20	4.0	6.0	
F TMWV-t	MT20	3.0	4.0	
G TMWV-t	MT20	4.0	6.0	
H BMWV-t	MT20	4.0	6.0	
I SS-t	MT20	3.0	6.0	
J SS-t	MT20	5.0	6.0	
K BMWV-t	MT20	3.0	4.0	
L BMV1-p	MT20	3.0	4.0	

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	IN-SX	IN-SX
H 1339 0	1339 0	1-10	1-10
L 1476 0	1476 0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS	DEAD	SOIL
JT COMBINED	JT COMBINED	JT COMBINED	JT COMBINED
H 942 644 / 0	0 / 0	0 / 0	298 / 0
L 1037 723 / 0	0 / 0	0 / 0	314 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. CS1 (LC)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 26	-102.0 -102.0 0.13 (1)	10.00		K-C	-52 / 104	0.04 (4)
B-C	-2045 / 0	-102.0 -102.0 0.84 (1)	3.58		C-I	-1043 / 0	0.48 (1)
C-D	-1114 / 0	-102.0 -102.0 0.73 (1)	4.79		I-E	-5 / 84	0.02 (4)
D-E	-1114 / 0	-102.0 -102.0 0.73 (1)	4.79		E-K	0 / 1929	0.48 (1)
E-F	-995 / 0	-102.0 -102.0 0.20 (1)	6.02		F-H	0 / 735	0.17 (1)
F-G	0 / 0	-102.0 -102.0 0.25 (1)	10.00				
G-H	-151 / 0	0.0 0.0 0.03 (1)	6.25				
H-I	-1418 / 0	0.0 0.0 0.14 (1)	6.85				
L-K	0 / 0	-18.5 -18.5 0.21 (4)	10.00				
K-J	0 / 1919	-18.5 -18.5 0.45 (1)	10.00				
J-I	0 / 1919	-18.5 -18.5 0.45 (1)	10.00				
I-H	0 / 645	-18.5 -18.5 0.29 (4)	10.00				

TOTAL WEIGHT = 4 X 94 = 377 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 28.0 PSF
LL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
LL	= 7.4 PSF
TOTAL LOAD	= 42.4 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(85 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/960$ (0.74")
CALCULATED VERT. DEFL.(LL) = $L/920$ (0.08")
ALLOWABLE DEFL.(TL) = $L/960$ (0.74")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.16")

CS1: TO=0.84/1.00 (B-C:1), BC=0.45/1.00 (I-K:1)

WB=0.53/1.00 (F-H:1), SS=0.33/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

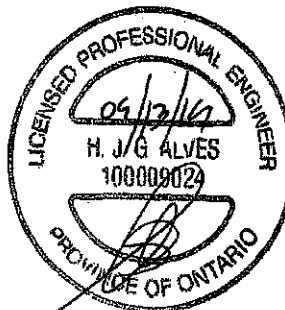
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818 354 1687 788 1987 1858	

PLATE PLACEMENT TOL. = 0.250 inches

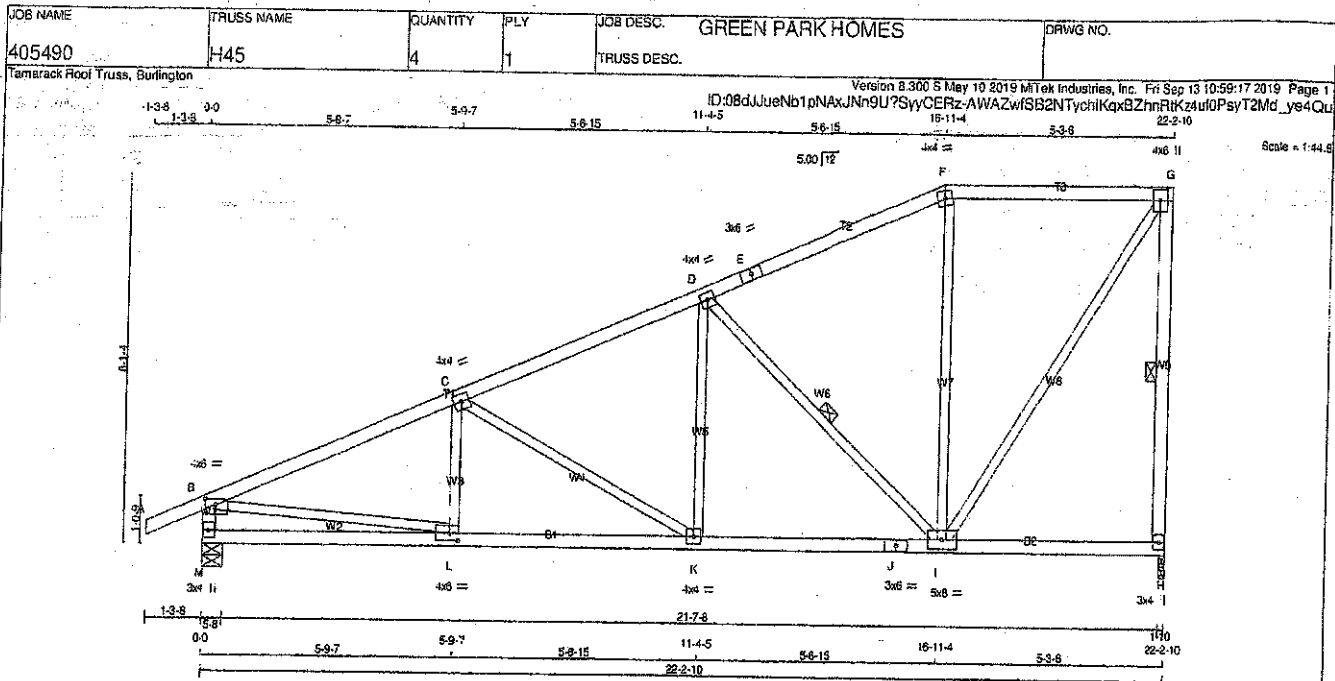
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (B) (INPUT = 0.90)

JSI METAL = 0.60 (J) (INPUT = 1.00)



Structural component only
DWG# T-1923443



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - E	2x4	DRY No.2	SPF
E - F	2x4	DRY No.2	SPF
F - G	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
M - B	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	4.0	6.0	1.50 2.75
C TMVW-t	MT20	4.0	4.0	
D TMVW-t	MT20	4.0	4.0	
E TS-t	MT20	3.0	6.0	
F TTW-m	MT20	4.0	4.0	
G TMVW-p	MT20	4.0	6.0	
H BMV1-p	MT20	3.0	4.0	
I BMVWW-t	MT20	5.0	8.0	
J BS-t	MT20	3.0	6.0	
K BMVW-t	MT20	4.0	4.0	
L BMVW-t	MT20	4.0	6.0	2.00 2.25
M BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	1338	0	1338	0	1-10
M	1476	0	1476	0	5-8

UNFACTORED REACTIONS

1ST LOSE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
H	842	844 / 0	0 / 0	0 / 0	0 / 0	298 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 26	L-C	-137 / 54
B-C	-2086 / 0	C-K	-580 / 0
C-D	-1540 / 0	K-D	0 / 388
D-E	-766 / 0	D-I	-1021 / 0
E-F	-766 / 0	I-G	-212 / 9
F-G	-684 / 0	G-L	0 / 1238
H-G	-1301 / 0	B-L	0 / 1942
M-B	-1429 / 0		
M-L	0 / 0		
L-K	0 / 1926		
K-J	0 / 1422		
J-I	0 / 1422		
I-H	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	DL = 29.0	PSF
	LL = 8.0	PSF
BOT CH.	DL = 0.0	PSF
	LL = 7.4	PSF
TOTAL LOAD	= 42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.47/1.00 (B-C:1), BC=0.38/1.00 (K-L:1),
WB=0.44/1.00 (B-L:1), SS=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

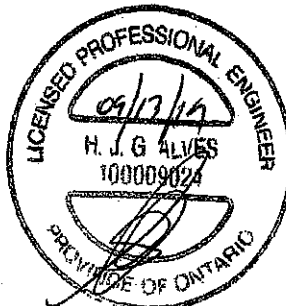
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1987 1656

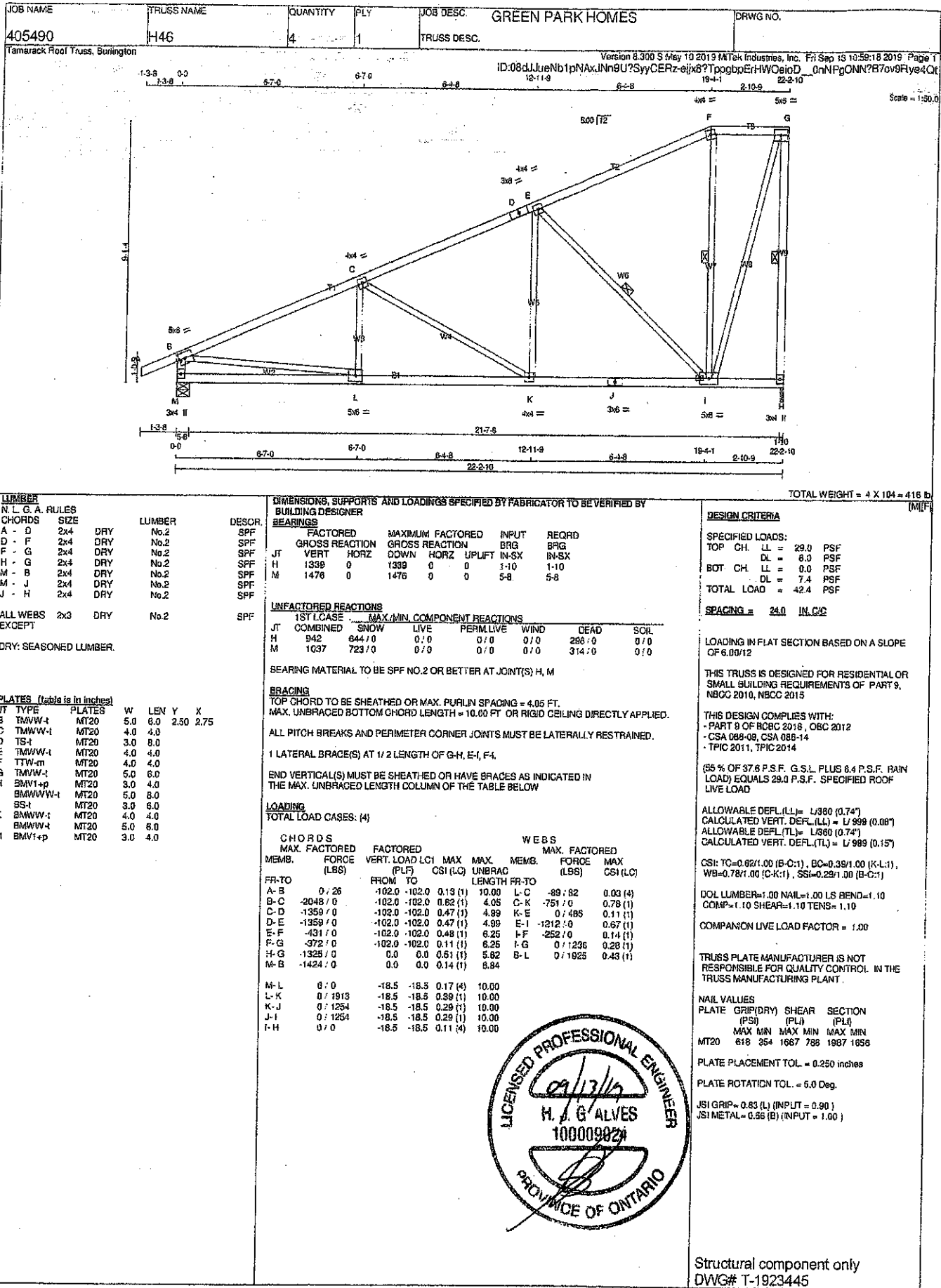
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.48 (L) (INPUT = 1.00)



Structural component only
DWG# T-1923444



[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRAWING NO.
405490	H48	32	1	TRUSS DESC.		

Tamarack Roof Truss - Burlington

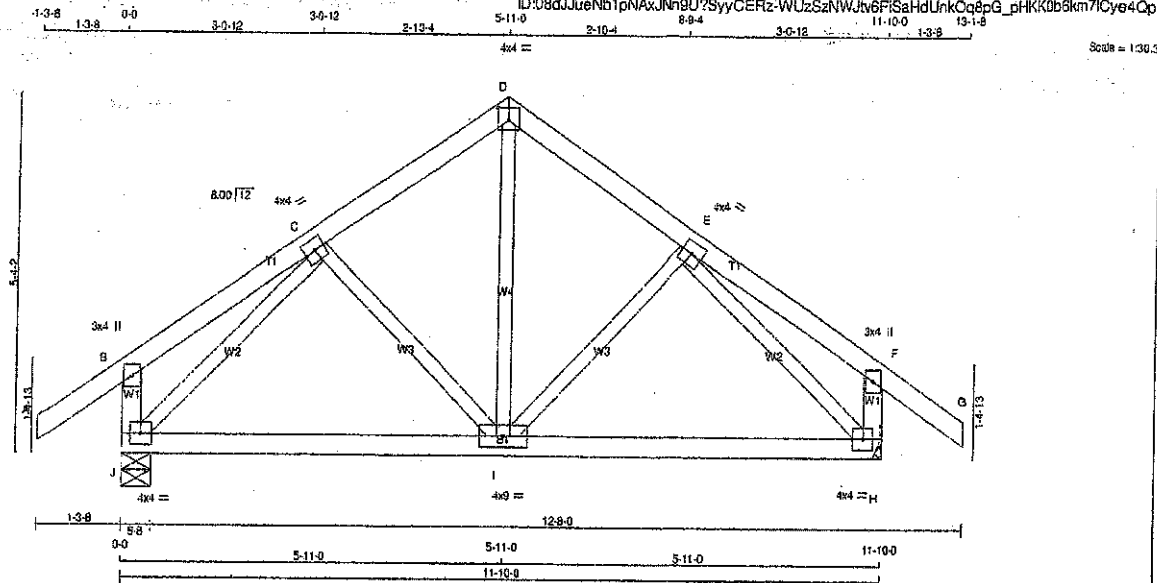
Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 10:59:21 2019 Page 1
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14-8-15 22-2-10 Scale = 1:55

Diagram Dimensions:
Top Chord: 1-3-8, 0-0, 3-10-0, 3-10-0, 3-7-12, 7-5-11, 7-3-3, 14-8-15, 7-5-11, 3-6-11, 0-0
Bottom Chord: 0-0, 7-5-11, 7-5-11, 7-3-3, 14-8-15, 7-5-11, 22-2-10
Vertical Height: 10-3-10
Member Labels: W1, W2, W3, W4, W5, W6, W7, W8, W9, W10, W11, W12, W13, W14, W15, W16, W17, W18, W19, W20, W21, W22, W23, W24, W25, W26, W27, W28, W29, W30, W31, W32, W33, W34, W35, W36, W37, W38, W39, W40, W41, W42, W43, W44, W45, W46, W47, W48, W49, W50, W51, W52, W53, W54, W55, W56, W57, W58, W59, W60, W61, W62, W63, W64, W65, W66, W67, W68, W69, W70, W71, W72, W73, W74, W75, W76, W77, W78, W79, W80, W81, W82, W83, W84, W85, W86, W87, W88, W89, W90, W91, W92, W93, W94, W95, W96, W97, W98, W99, W100, W101, W102, W103, W104, W105, W106, W107, W108, W109, W110, W111, W112, W113, W114, W115, W116, W117, W118, W119, W120, W121, W122, W123, W124, W125, W126, W127, W128, W129, W130, W131, W132, W133, W134, W135, W136, W137, W138, W139, W140, W141, W142, W143, W144, W145, W146, W147, W148, W149, W150, W151, W152, W153, W154, W155, W156, W157, W158, W159, W160, W161, W162, W163, W164, W165, W166, W167, W168, W169, W170, W171, W172, W173, W174, W175, W176, W177, W178, W179, W180, W181, W182, W183, W184, W185, W186, W187, W188, W189, W190, W191, W192, W193, W194, W195, W196, W197, W198, W199, W200, W201, W202, W203, W204, W205, W206, W207, W208, W209, W210, W211, W212, W213, W214, W215, W216, W217, W218, W219, W220, W221, W222, W223, W224, W225, W226, W227, W228, W229, W230, W231, W232, W233, W234, W235, W236, W237, W238, W239, W240, W241, W242, W243, W244, W245, W246, W247, W248, W249, W250, W251, W252, W253, W254, W255, W256, W257, W258, W259, W260, W261, W262, W263, W264, W265, W266, W267, W268, W269, W270, W271, W272, W273, W274, W275, W276, W277, W278, W279, W280, W281, W282, W283, W284, W285, W286, W287, W288, W289, W290, W291, W292, W293, W294, W295, W296, W297, W298, W299, W300, W301, W302, W303, W304, W305, W306, W307, W308, W309, W310, W311, W312, W313, W314, W315, W316, W317, W318, W319, W320, W321, W322, W323, W324, W325, W326, W327, W328, W329, W330, W331, W332, W333, W334, W335, W336, W337, W338, W339, W340, W341, W342, W343, W344, W345, W346, W347, W348, W349, W350, W351, W352, W353, W354, W355, W356, W357, W358, W359, W360, W361, W362, W363, W364, W365, W366, W367, W368, W369, W370, W371, W372, W373, W374, W375, W376, W377, W378, W379, W380, W381, W382, W383, W384, W385, W386, W387, W388, W389, W390, W391, W392, W393, W394, W395, W396, W397, W398, W399, W400, W401, W402, W403, W404, W405, W406, W407, W408, W409, W410, W411, W412, W413, W414, W415, W416, W417, W418, W419, W420, W421, W422, W423, W424, W425, W426, W427, W428, W429, W430, W431, W432, W433, W434, W435, W436, W437, W438, W439, W440, W441, W442, W443, W444, W445, W446, W447, W448, W449, W450, W451, W452, W453, W454, W455, W456, W457, W458, W459, W460, W461, W462, W463, W464, W465, W466, W467, W468, W469, W470, W471, W472, W473, W474, W475, W476, W477, W478, W479, W480, W481, W482, W483, W484, W485, W486, W487, W488, W489, W490, W491, W492, W493, W494, W495, W496, W497, W498, W499, W500, W501, W502, W503, W504, W505, W506, W507, W508, W509, W510, W511, W512, W513, W514, W515, W516, W517, W518, W519, W520, W521, W522, W523, W524, W525, W526, W527, W528, W529, W530, W531, W532, W533, W534, W535, W536, W537, W538, W539, W540, W541, W542, W543, W544, W545, W546, W547, W548, W549, W550, W551, W552, W553, W554, W555, W556, W557, W558, W559, W560, W561, W562, W563, W564, W565, W566, W567, W568, W569, W570, W571, W572, W573, W574, W575, W576, W577, W578, W579, W580, W581, W582, W583, W584, W585, W586, W587, W588, W589, W590, W591, W592, W593, W594, W595, W596, W597, W598, W599, W600, W601, W602, W603, W604, W605, W606, W607, W608, W609, W610, W611, W612, W613, W614, W615, W616, W617, W618, W619, W620, W621, W622, W623, W624, W625, W626, W627, W628, W629, W630, W631, W632, W633, W634, W635, W636, W637, W638, W639, W640, W641, W642, W643, W644, W645, W646, W647, W648, W649, W650, W651, W652, W653, W654, W655, W656, W657, W658, W659, W660, W661, W662, W663, W664, W665, W666, W667, W668, W669, W670, W671, W672, W673, W674, W675, W676, W677, W678, W679, W680, W681, W682, W683, W684, W685, W686, W687, W688, W689, W690, W691, W692, W693, W694, W695, W696, W697, W698, W699, W700, W701, W702, W703, W704, W705, W706, W707, W708, W709, W710, W711, W712, W713, W714, W715, W716, W717, W718, W719, W720, W721, W722, W723, W724, W725, W726, W727, W728, W729, W730, W731, W732, W733, W734, W735, W736, W737, W738, W739, W740, W741, W742, W743, W744, W745, W746, W747, W748

JOB NAME 405490	TRUSS NAME H49-Cond1	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 6 X 52 = 314 lb (MIP)

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW+1	MT20	4.0	4.0		
D TTW+p	MT20	4.0	4.0	2.25	2.00
E TMVW+1	MT20	4.0	4.0		
F TMV+p	MT20	3.0	4.0		
H BMVW+1	MT20	4.0	4.0		
I BMVW+1	MT20	4.0	9.0		
J BMVW+1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	853	0	853	0
H	853	0	853	0
			MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
J	598	423 / 0	0 / 0
H	598	423 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)
FR-TO				
A-B	0 / 39	-102.0 -102.0	0.14 (1)	10.00
B-C	0 / 18	-102.0 -102.0	0.14 (1)	10.00
C-D	-540 / 0	-102.0 -102.0	0.11 (1)	6.25
D-E	-540 / 0	-102.0 -102.0	0.11 (1)	6.25
E-F	0 / 18	-102.0 -102.0	0.14 (1)	10.00
F-G	0 / 39	-102.0 -102.0	0.14 (1)	10.00
J-B	-257 / 0	0.0	0.03 (1)	7.81
H-F	-257 / 0	0.0	0.03 (1)	7.81
J-I	0 / 536	-18.5	-18.5	0.22 (4)
I-H	0 / 536	-18.5	-18.5	0.22 (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 8.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")

CALCULATED VERT. DEFL.(LL) = L/999 (0.01")

ALLOWABLE DEFL.(TL) = L/600 (0.39")

CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.14/1.00 (F-G:1), BC=0.22/1.00 (I-J:4),

WB=0.22/1.00 (C-J:1), SI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

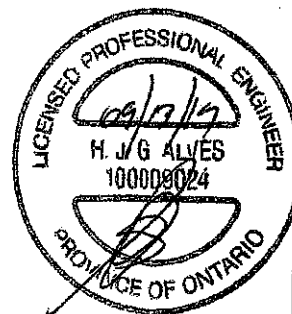
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	818	354	1667
	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

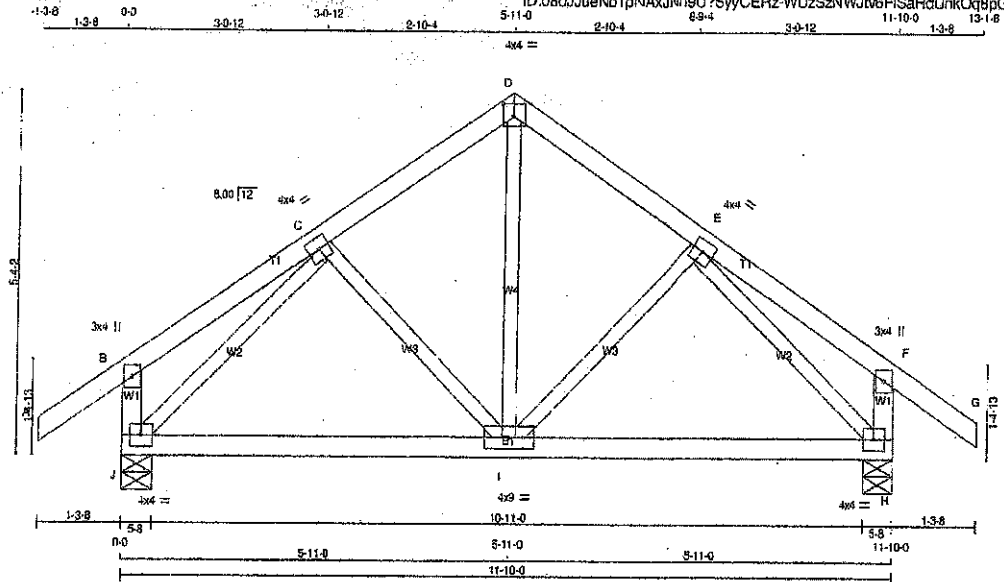
JSI GRIP=0.70 (E) (INPUT = 0.90)

JSI METAL=0.27 (E) (INPUT = 1.00)



Structural component only
DWG# T-1923448

JOB NAME 405490	TRUSS NAME H49-Cond2	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					
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ID:08dJueNb1pNAXJn8U?SyyCERz-WUzS2NWJt6FISaHdUnkOq8pG_pHKk0b6km7ICy64Qp					



TOTAL WEIGHT = 6 X 52 = 314 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY, SEASONED LUMBER.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT	TMV+p	MT20	3.0	4.0	
B	TMV+p	MT20	4.0	4.0	
C	TMV+p	MT20	4.0	4.0	
D	TMV+p	MT20	4.0	4.0	2.25
E	TMV+p	MT20	4.0	4.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-i	MT20	4.0	4.0	
I	BMVW1-i	MT20	4.0	4.0	
J	BMVW1-i	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	VERT	DOWN	0	0
H	VERT	DOWN	0	0
	HORIZ	HORIZ	5-8	5-8
	UPLIFT	UPLIFT	5-8	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	SNOW	LIVE	PERM. LIVE
J	583	423.0	0.0
H	583	423.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING	TOTAL LOAD CASES: (4)
CHORDS	MAX. FACTORED
MEMB.	FORCE (LBS)
FR-TO	FROM TO
A-B	0.39
B-C	0.18
C-D	-540.0
D-E	-540.0
E-F	0.18
F-G	0.39
G-H	-257.0
H-I	-257.0
I-J	0.536
J-K	0.536
WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)
FR-TO	FROM TO
I-D	0.342
D-E	-145.0
E-F	-145.0
F-G	-770.0
G-H	-770.0
H-I	7.81
I-J	7.81
J-K	18.5
K-L	18.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TRC 2011, TRC 2014

(65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.39")
CALCULATED VERT. DEFL.(LL) = 1/888 (0.01")
ALLOWABLE DEFL.(TL) = 1/360 (0.39")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")
CSI: TC=0.14/1.00 (F-G); BC=0.22/1.00 (I-J);
WB=0.22/1.00 (G-H); SI=0.12/1.00 (C-D);
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00

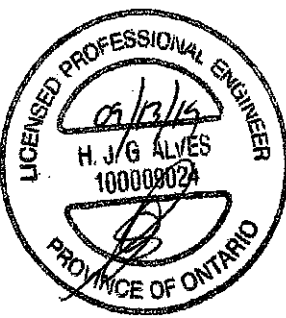
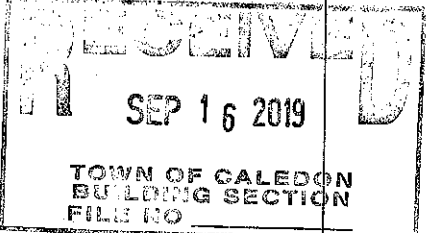
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.70 (E) (INPUT = 0.90)
JSI METAL = 0.27 (E) (INPUT = 1.00)



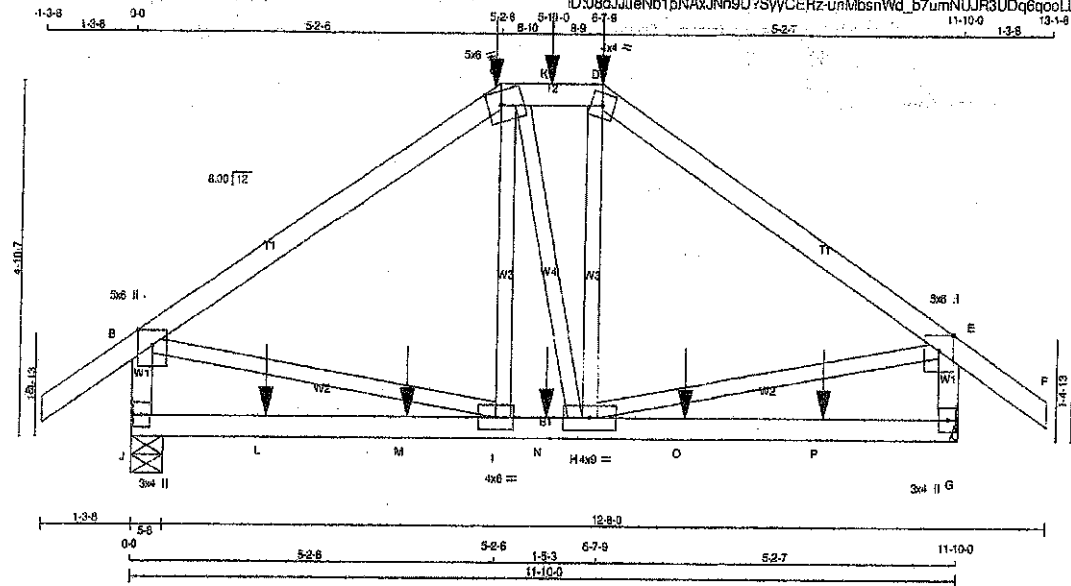
Structural component only
DWG# T-1923449

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405532	H63	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2" = 1'-0"

LUMBER	N. L. & A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2			SPF
C - D	2x4	DRY	No.2			SPF
D - F	2x4	DRY	No.2			SPF
J - B	2x4	DRY	No.2			SPF
G - E	2x4	DRY	No.2			SPF
J - G	2x4	DRY	No.2			SPF
ALL WEBS	2x3	DRY	No.2			SPF
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	5.0	6.0	Edge		
C	TTWV-m	MT20	5.0	6.0	2.25	2.25	
D	TTW-m	MT20	4.0	4.0			
E	TMWV+p	MT20	5.0	6.0	Edge		
G	BMV1+p	MT20	3.0	4.0			
H	BMWVW-t	MT20	4.0	9.0			
J	BMWVW-t	MT20	4.0	6.0			
J	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	VERT	DOWN	IN-SX	IN-SX
J	1175	0	5-8	5-8
G	1175	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	824	582 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0
G	824	582 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(LBS)	(PLF)	(LBS)	(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 39	-102.0 -102.0 0.15 (1)	I-C	-78 / 47
B-C	-993 / 0	-102.0 -102.0 0.58 (1)	C-H	0 / 7
C-K	-824 / 0	-102.0 -102.0 0.10 (1)	H-D	-75 / 83
K-D	-824 / 0	-102.0 -102.0 0.10 (1)	D-I	0 / 842
D-E	-998 / 0	-102.0 -102.0 0.58 (1)	I-E	0 / 844
E-F	0 / 39	-102.0 -102.0 0.15 (1)		
F-B	-1126 / 0	0.0 0.0 0.13 (1)		
G-E	-1124 / 0	0.0 0.0 0.13 (1)		
J-L	0 / 0	-18.5 -18.5 0.16 (4)		
L-M	0 / 0	-18.5 -18.5 0.16 (4)		
M-I	0 / 0	-18.5 -18.5 0.16 (4)		
I-N	0 / 823	-18.5 -18.5 0.24 (4)		
N-H	0 / 823	-18.5 -18.5 0.24 (4)		
H-O	0 / 0	-18.5 -18.5 0.18 (4)		
O-P	0 / 0	-18.5 -18.5 0.18 (4)		
P-G	0 / 0	-18.5 -18.5 0.18 (4)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	5-2-6	238	238	---	BACK	VERT	---	C1
D	6-7-8	238	238	---	BACK	VERT	---	C1
K	5-11-0	-105	-105	---	BACK	VERT	---	C1
L	1-11-4	-13	-13	---	BACK	VERT	---	C1
M	3-11-4	-13	-13	---	BACK	VERT	---	C1
N	5-11-0	-13	-13	---	BACK	VERT	---	C1
O	7-10-12	-13	-13	---	BACK	VERT	---	C1
P	9-10-12	-13	-13	---	BACK	VERT	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT. CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/989 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.58/1.00 (D-E:1), BC=0.24/1.00 (H-I:4),
WB=0.21/1.00 (E-H:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

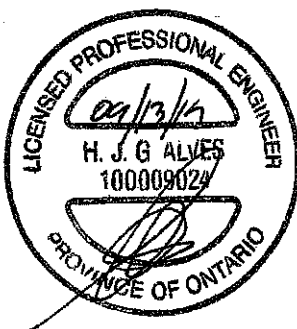
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX (MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1056

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.45 (E) (INPUT = 1.00)

Structural component only
DWG# T-1923460

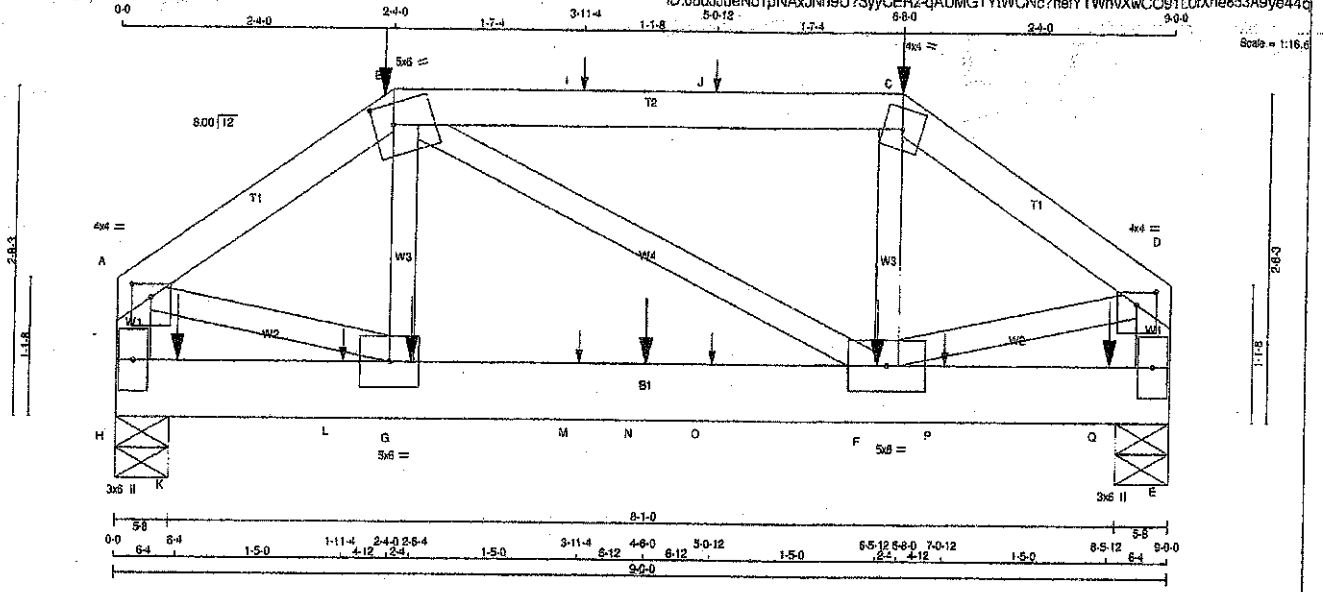


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405532	H64	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:08dJueN01pNAXJNn9U?SyyCERz-qAUMGTyWCNC?heYTWhwXwCC091LUXhe853A9ye44c



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2		
B - C	2x4	DRY	No.2		
C - D	2x4	DRY	No.2		
H - A	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		
H - E	2x6	DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
A - TMVW-m	MT20	4.0	4.0	1.25 2.00
B - TTWW-m	MT20	5.0	6.0	2.00 2.00
C - TTWW-m	MT20	4.0	4.0	
D - TMVW-p	MT20	4.0	4.0	1.25 2.00
E - BMV1+p	MT20	3.0	6.0	
F - BMVWW-1	MT20	5.0	6.0	
G - BMVW-1	MT20	5.0	6.0	
H - BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
SPF	GROSS REACTION	GROSS REACTION	BAG	BAG
JT	VERT	DOWN	IN-SX	IN-SX
H	1055	0	0	5-8
E	1050	0	0	5-8

UNFACTORED REACTIONS	1ST LOASE	MAX/MIN COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	
H	737	532 / 0	0 / 0	0 / 0	206 / 0
E	734	529 / 0	0 / 0	0 / 0	205 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	-977 / 0	-102.0	-102.0	0.12 (1)	8.14	G-B	0 / 173	0.04 (1)	
B-I	-828 / 0	-102.0	-102.0	0.38 (1)	6.09	B-F	0 / 12	0.00 (1)	
I-J	-828 / 0	-102.0	-102.0	0.38 (1)	6.09	F-C	0 / 179	0.04 (1)	
J-C	-828 / 0	-102.0	-102.0	0.38 (1)	6.09	A-G	0 / 845	0.21 (1)	
C-D	-878 / 0	-102.0	-102.0	0.12 (1)	8.14	F-D	0 / 843	0.21 (1)	
H-A	-904 / 0	0.0	0.0	0.10 (1)	7.81				
E-D	-893 / 0	0.0	0.0	0.10 (1)	7.81				
H-K	0 / 0	-18.5	-18.5	0.06 (1)	10.00				
K-L	0 / 0	-18.5	-18.5	0.06 (1)	10.00				
L-G	0 / 0	-18.5	-18.5	0.08 (1)	10.00				
G-M	0 / 818	-18.5	-18.5	0.23 (1)	10.00				
M-N	0 / 818	-18.5	-18.5	0.23 (1)	10.00				
N-O	0 / 818	-18.5	-18.5	0.23 (1)	10.00				
O-F	0 / 818	-18.5	-18.5	0.23 (1)	10.00				
F-P	0 / 0	-18.5	-18.5	0.06 (1)	10.00				
P-Q	0 / 0	-18.5	-18.5	0.06 (1)	10.00				
Q-E	0 / 0	-18.5	-18.5	0.06 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	2'-4"	-27	-27	---	FRONT	VERT	TOTAL	---	C1
B	6'-0"	-27	-27	---	FRONT	VERT	TOTAL	---	C1
C	6'-5"	-191	-191	---	BACK	VERT	TOTAL	---	C1
G	2'-6"	-191	-191	---	BACK	VERT	TOTAL	---	C1
I	3'-11"	1	1	---	FRONT	VERT	TOTAL	---	C1
J	5'-0"	1	1	---	FRONT	VERT	TOTAL	---	C1
K	6'-4"	-196	-196	---	BACK	VERT	TOTAL	---	C1
L	1'-11"	1	1	---	FRONT	VERT	TOTAL	---	C1
M	3'-11"	1	1	---	FRONT	VERT	TOTAL	---	C1
N	4'-8"	-191	-191	---	BACK	VERT	TOTAL	---	C1
O	5'-0"	1	1	---	FRONT	VERT	TOTAL	---	C1
P	7'-0"	1	1	---	FRONT	VERT	TOTAL	---	C1
Q	8'-5"	-199	-199	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.38/1.00 (B-C:1), BC=0.23/1.00 (F-G:1),
WB=0.21/1.00 (A-G:1), SS=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

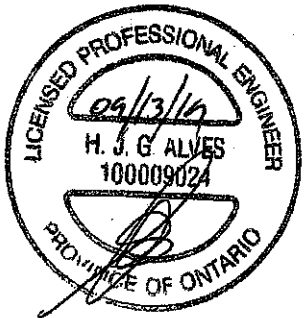
NAIL VALUES	PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.79 (A) (INPUT = 0.80)

JSI METAL = 0.26 (D) (INPUT = 1.00)



Structural component only
DWG# T-1923461

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	J4	12	1	TRUSS DESC.		

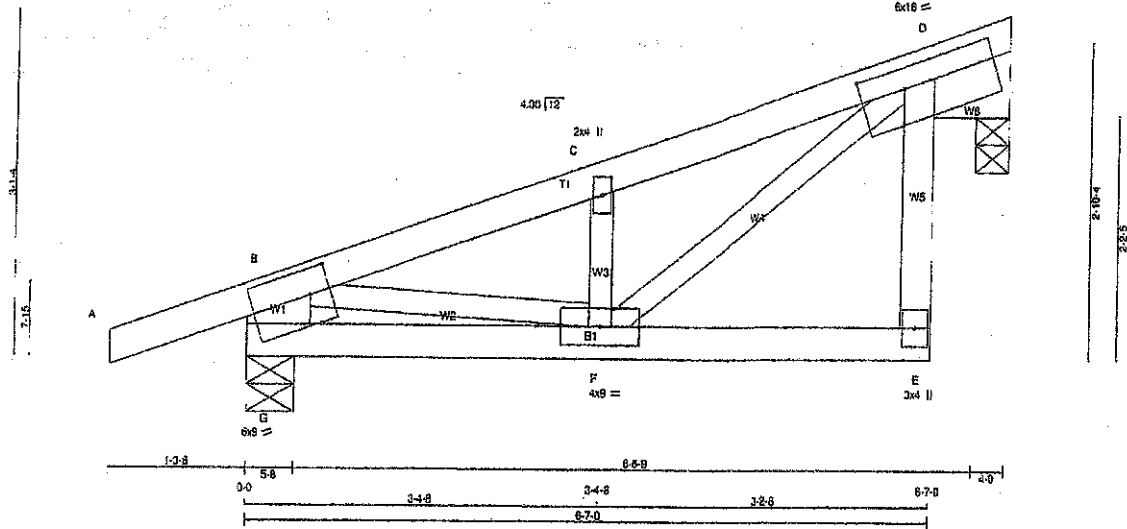
Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 AITex Industries, Inc. Fri Sep 13 12:22:19 2019 Page 1

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1-3-8 1-3-8 0-0 3-4-8 3-4-8 3-2-8 6-7-0

Scale = 1:10.4



TOTAL WEIGHT = 12 X 29 = 348 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
E - H	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
G - B	2x8	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMW4w	MT20	2.0	4.0	
B	TMW4w	MT20	6.0	16.0	2.25 5.00
C	BMV4p	MT20	3.0	4.0	
D	BMV4p	MT20	4.0	9.0	
E	BMV4p	MT20	4.0	9.0	
F	BMV4p	MT20	4.0	9.0	
G	TMW4w	MT20	6.0	9.0	2.75 2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
D	378	378	4-0 (3-15)	4-0
G	548	548	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

REVELED PLATE OR SHM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
D	286	182/0	0/0	0/0	84/0	0/0
G	381	277/0	0/0	0/0	104/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
A-B		0:26	-102.0	-102.0	0.19 (1)	10.00	0:26	-102.0	-102.0	0.19 (1)	10.00	0:26	-102.0
B-C		-494/0	-102.0	-102.0	0.15 (1)	8.25	0:26	-102.0	-102.0	0.15 (1)	8.25	0:26	-102.0
C-D		-479/0	-102.0	-102.0	0.15 (1)	8.25	0:26	-102.0	-102.0	0.15 (1)	8.25	0:26	-102.0
D-E		0:29	0.0	0.0	0.01 (4)	10.00	0:29	0.0	0.0	0.01 (4)	10.00	0:29	0.0
E-F		0:0	-18.5	-18.5	0.04 (4)	10.00	0:0	-18.5	-18.5	0.04 (4)	10.00	0:0	-18.5
F-G		0:0	-18.5	-18.5	0.04 (4)	10.00	0:0	-18.5	-18.5	0.04 (4)	10.00	0:0	-18.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. CIG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.19/1.00 (A-B:1), BC=0.04/1.00 (E-F:4), WB=0.13/1.00 (D-F:1), SE=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

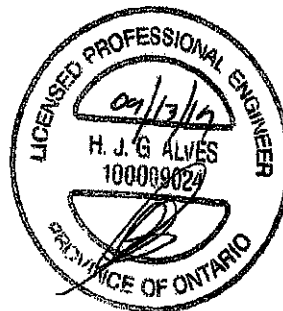
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 364	1887 788	1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.42 (G) (INPUT = 0.80)
JSI METAL = 0.13 (F) (INPUT = 1.00)



Structural component only
DWG# T-1923561

JOB NAME 405490	TRUSS NAME J18	QUANTITY 24	PLY 1	JOB DESC. TAMRACK ROOF TRUSS, BURLINGTON	GREEN PARK HOMES.	DWG# NO.
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ID:08dJJueNb1pNAXjNN9U?SyvCEHrZtSmL14aSbPkXpETfQ1NV5urYI7Y7djKF0TuzPye4Qk

Scale = 1/23.1

The truss diagram shows a side elevation of a roof truss system. The main chord is labeled T1. A horizontal member at the bottom is labeled B1. Vertical members are shown connecting the top and bottom chords. Dimensions include a total height of 6.00 ft, a base width of 13'-9", and various intermediate heights and widths like 5'-8" and 5'-10-8". Members are identified as E-B, A-C, D-E, and others. Connections are detailed at joints C and D.

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	No.2
E - B	2x4	DRY	No.2			
A - C	2x4	DRY	No.2			
E - D	2x4	DRY	No.2			

SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
E	BMV-p	MT20	3.0	4.0		

DESIGNER'S DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX		REQD BRG IN-SX	
	VERT	HORZ	DOWN	HORZ	UP/LIFT	IN-SX	IN-SX	
E	577	0	577	0	0	5-8	5-8	
C	225	0	225	0	0	1-8	1-8	
D	45	0	50	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	FEM/LIVE	WIND	DEAD	SOIL
E	403	292 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	154	125 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

	CHORDS				WEBS			
	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX CS (LG)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX CS (LG)
FR-TO								
E-B	-513 / 0	0.0	0.0	0.13 (4)	7.81			
A-B	0 / 31	-102.0	-102.0	0.13 (1)	10.00			
B-C	-33 / 0	-102.0	-102.0	0.60 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH LL = 29.0 PSF
DL = 6.0 PSF
BOT CH LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF BCBC 2015, OBC 2012
CSA 089-09, CSA 086-14
TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.60/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SS=0.26/1.00 (B-C:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (E) (INPUT = 0.90)
JSI METAL = 0.14 (B) (INPUT = 1.00)

Structural component only
DWG# T-1923454

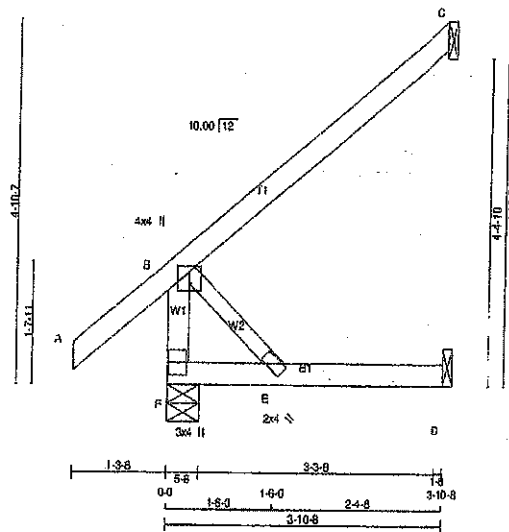
9/13/19
H. J. G ALVES
100009029
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405554	J26	10	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

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ID:08dJJu6N51pNAXJNn9U7SyyCERz-v4igw4eyZEK8wML3RXIY51pC9q6zHYYVKAz3ye3NNW

1-3-8 1-3-8 0-0 3-10-8 3-10-8

Scale = 1:27.3



TOTAL WEIGHT = 10 X 15 = 152 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

DESCR.

SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	375	0	375	0
C	198	0	198	0
D	36	0	40	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	261	193/0	0/0	0/0	0/0	69/0
C	136	112/0	0/0	0/0	0/0	23/0
D	29	0/0	0/0	0/0	0/0	29/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD L _{C1}	MEMB.	FORCE
FR-TO	(LBS)	(PLF)	MAX. LENGTH FR-TO	(LBS)
F-B	-339/0	0.0	0.0	0.04 (1)
A-B	0/45	-102.0	-102.0	0.15 (5)
B-C	0/0	-102.0	-102.0	0.26 (1)
F-E	0/0	-18.5	-18.5	0.08 (4)
E-D	0/0	-18.5	-18.5	0.08 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH. LL	=	29.0 PSF
DL	=	6.0 PSF
BOT CH. LL	=	0.0 PSF
DL	=	7.4 PSF
TOTAL LOAD	=	42.4 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.26/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

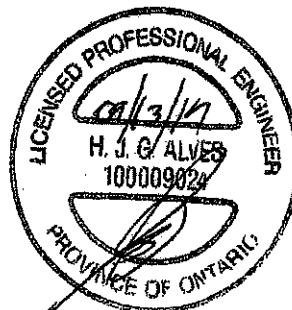
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

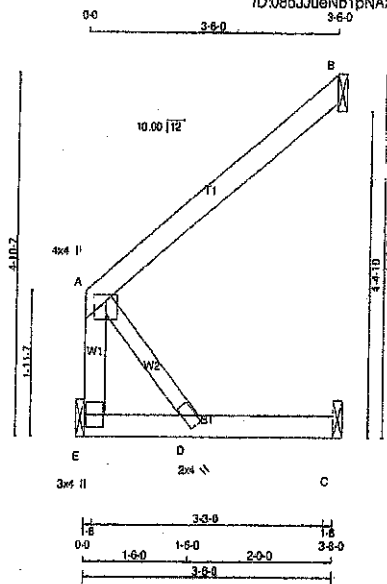
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-1923462

JOB NAME 405554	TRUSS NAME J26S	QUANTITY 10	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 Mitek Industries, Inc. File Sep 13 12:04:47 2019 Page 1 ID:08dJJueNb1pNAXJNn9U?SyyCERz5bdG10KXscK5e8e3lhAzXCYYAS_TW9Juf2JCye3TU		



Scale = 1:27.3

LUMBER			
N.L.G.A. RULES	SIZE	DRY	LUMBER
E - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
E - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT
				IN-SX	IN-SX
E	211	0	211	0	1-8
B	179	0	179	0	1-8
C	32	0	36	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) E, B, C

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED						
E	148	102/0	0/0	0/0	0/0	47/0	0/0
B	123	102/0	0/0	0/0	0/0	21/0	0/0
C	26	0/0	0/0	0/0	0/0	28/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
E-A	-179/0	A-D	0/0
A-B	0/0		
E-D	0/0		
D-C	0/0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 42.4 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.21/1.00 (A-B:1), BC=0.07/1.00 (C-D:4), WB=0.00/1.00 (A-D:1), SS=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

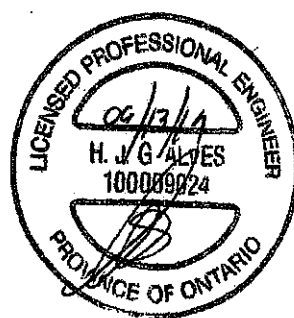
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.14 (A) (INPUT = 0.90)
JSI METAL = 0.04 (A) (INPUT = 1.00)



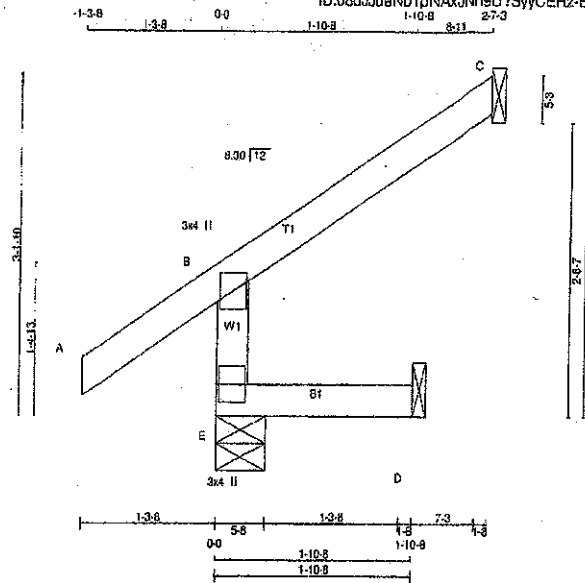
Structural component only
DWG# T-1923482

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405532	J27	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 11:22:52 2019 Page 1

ID:08dJuaNb1pNAxJn9LU?SyycERz-EIAUvVamp7IBs8NHDc4OXAYmQM5Nh3X7L&JnTy44n



Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 8 X 9 = 70 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
	E - B	2x4	DRY	No.2	SPF
	A - C	2x4	DRY	No.2	SPF
	E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
E	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
JT				
E	324	0	324	0
C	100	0	100	0
D	16	0	16	0

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERALIVE	WIND	DEAD	SOIL
E	225	174/0	0/0	0/0	0/0	51/0	0/0
C	69	57/0	0/0	0/0	0/0	12/0	0/0
D	13	0/0	0/0	0/0	0/0	13/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO			FROM TO	
E-B	-305/0	0.0	0.0	0.01 (4)
A-B	0/39	-102.0	-102.0	0.14 (1)
B-C	-18/0	-102.0	-102.0	0.12 (1)
E-D	0/0	-18.5	-18.5	0.02 (4)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/998$ (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (D-E:1), WB=0.00/1.00 (n/a:0), SB=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

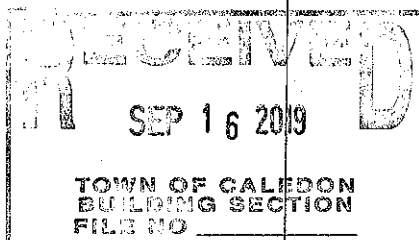
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788	1987 1555

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.80)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-1923463

JOB NAME 405532	TRUSS NAME J28	QUANTITY 16	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tansack Roof Truss, Burlington		Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 11:22:53 2019 Page 1 ID:08dJJueN61pNAXJNn9U7SyyCERz-ixjs8rbOaRt2UlyTnJbd4N4yCmRlQWnH2m3Hjwye44m			

Scale = 1:16

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER E - B 2x4 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY: SEASONED LUMBER. PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 3.0 4.0 E BMV1+p MT20 3.0 4.0	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th rowspan="2">DESCR</th> <th colspan="2">FACTORED</th> <th colspan="2">MAXIMUM FACTORED</th> <th rowspan="2">INPUT</th> <th rowspan="2">RECORD</th> </tr> <tr> <th>GROSS REACTION</th> <th>GROSS REACTION</th> <th>DOWN</th> <th>UP</th> </tr> <tr> <td>JT</td> <td>VERT</td> <td>HORZ</td> <td>DOWN</td> <td>HORZ</td> <td>UPLIFT</td> <td>IN-SK</td> </tr> <tr> <td>E</td> <td>221</td> <td>0</td> <td>221</td> <td>0</td> <td>5-8</td> <td>5-8</td> </tr> <tr> <td>C</td> <td>65</td> <td>0</td> <td>65</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> <tr> <td>D</td> <td>14</td> <td>0</td> <td>16</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> </table> SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D UNFACTORED REACTIONS <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th rowspan="2">JT</th> <th rowspan="2">1ST LCASE</th> <th colspan="6">MAX./MIN. COMPONENT REACTIONS</th> </tr> <tr> <th>SNOW</th> <th>LIVE</th> <th>PERM.LIVE</th> <th>WIND</th> <th>DEAD</th> <th>SOIL</th> </tr> <tr> <td>E</td> <td>COMBINED</td> <td>118.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>37.0</td> <td>0.0</td> </tr> <tr> <td>C</td> <td></td> <td>37.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>8.0</td> <td>0.0</td> </tr> <tr> <td>D</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>11.0</td> <td>0.0</td> </tr> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (5) <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th colspan="2">CHORDS</th> <th colspan="2">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (LBS)</th> <th>FACTORED MAX. MEMB. FORCE (LBS)</th> </tr> <tr> <td>FR-TO</td> <td></td> <td>FROM TO</td> <td></td> </tr> <tr> <td>E-B</td> <td>-204.0</td> <td>0.0 0.0 0.01 (4)</td> <td>7.81</td> </tr> <tr> <td>A-B</td> <td>0.32</td> <td>-102.0 -102.0 0.07 (1)</td> <td>10.00</td> </tr> <tr> <td>B-C</td> <td>-13.0</td> <td>-102.0 -102.0 0.05 (1)</td> <td>6.25</td> </tr> <tr> <td>E-D</td> <td>0.0</td> <td>-18.5 -18.5 0.01 (4)</td> <td>10.00</td> </tr> </table> CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN	DESCR	FACTORED		MAXIMUM FACTORED		INPUT	RECORD	GROSS REACTION	GROSS REACTION	DOWN	UP	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SK	E	221	0	221	0	5-8	5-8	C	65	0	65	0	1-8	1-8	D	14	0	16	0	1-8	1-8	JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	COMBINED	118.0	0.0	0.0	0.0	37.0	0.0	C		37.0	0.0	0.0	0.0	8.0	0.0	D		0.0	0.0	0.0	0.0	11.0	0.0	CHORDS		WEBS		MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED MAX. MEMB. FORCE (LBS)	FR-TO		FROM TO		E-B	-204.0	0.0 0.0 0.01 (4)	7.81	A-B	0.32	-102.0 -102.0 0.07 (1)	10.00	B-C	-13.0	-102.0 -102.0 0.05 (1)	6.25	E-D	0.0	-18.5 -18.5 0.01 (4)	10.00	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 29.0 PSF DL = 8.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 42.4 PSF SPACING = 24.0 IN./C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, OGC 2012 - CSA 086-09, CSA 086-14 - TRC 2011, TRC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.19") CALCULATED VERT. DEFL.(LL) = 1/999 (0.00") ALLOWABLE DEFL.(TL) = L/360 (0.19") CALCULATED VERT. DEFL.(TL) = 1/999 (0.00") CSI: TC=0.07/1.00 (A-B:1), BC=0.01/1.00 (D-E:4), WB=0.00/1.00 (W:0), SSI=0.06/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1867 788 1967 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP = 0.10 (B) (INPUT = 0.90) JSI METAL = 0.07 (B) (INPUT = 1.00)
DESCR	FACTORED		MAXIMUM FACTORED		INPUT	RECORD																																																																																																					
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Structural component only
 DWG# T-1923464



isDesign™

Client:
Project:
Address:

Date: 2/2019

Page 1 of 2

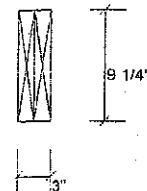
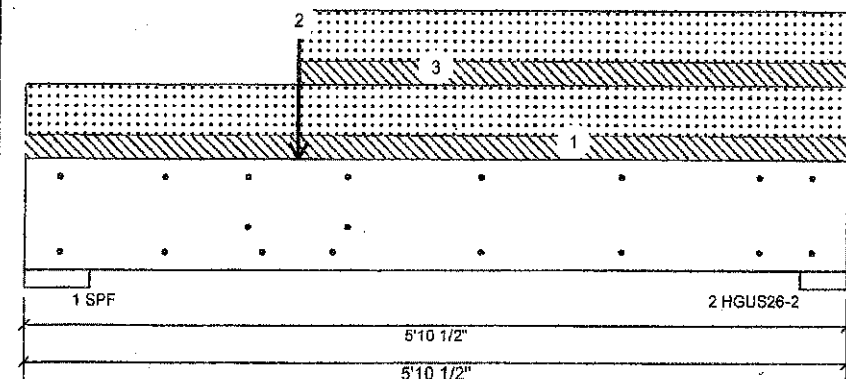
Designer:

Job Name: 405532

Project #:

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015 / OBC 2012
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	313	670	0
2	0	320	689	0

Bearings and Factored Reactions

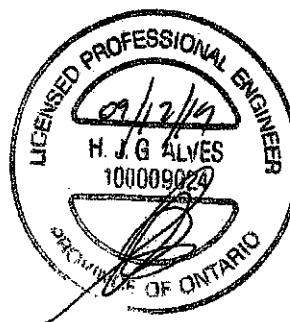
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	391 / 1004	1396 L	1.25D+1.5S
2 - HGUS...	4.000"	19%	400 / 1034	1434 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2094 ft-lb	2'5 3/16"	8039 ft-lb	0.347 (35%)	1.25D+1.5S	L
Unbraced	2094 ft-lb	2'5 3/16"	5236 ft-lb	0.400 (40%)	1.25D+1.5S	L
Shear	1846 lb	1'2"	3984 lb	0.413 (41%)	1.25D+1.5S	L
LL Defl inch	0.018 (L/3542)	2'11 1/8"	0.174 (L/360)	0.100 (10%)	S	L
TL Defl inch	0.026 (L/2416)	2'11 1/8"	0.174 (L/360)	0.150 (15%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



DWG NO. TAM-T1923465
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Point	1-11-4		Near Face	242 lb	0 lb	513 lb	0 lb	
3	Part. Uniform	1-11-4 to 5-10-8	5-11-0	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115



This design is valid until 12/11/2021





isDesign™

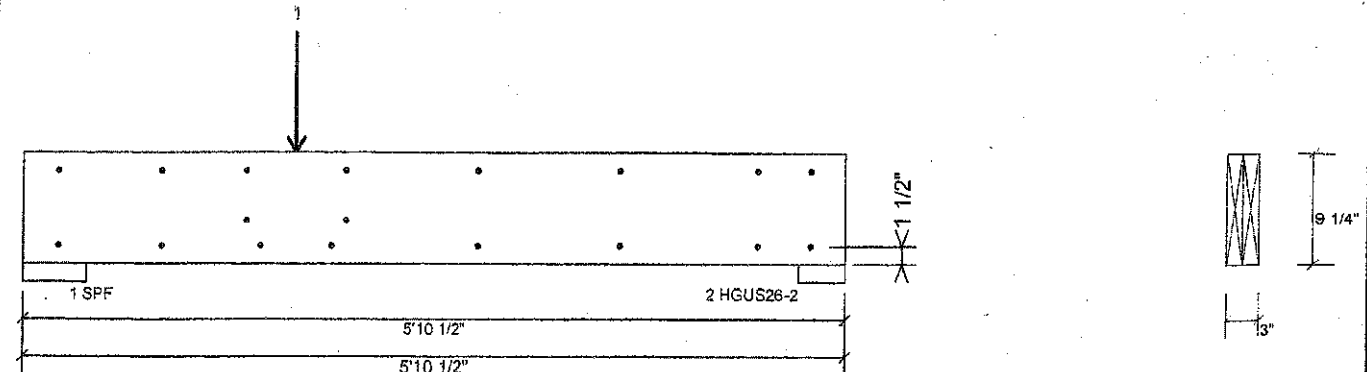
Client:
Project:
Address:

Date: 2/2019
Designer:
Job Name: 405532
Project #:

Page 2 of 2

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c., except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

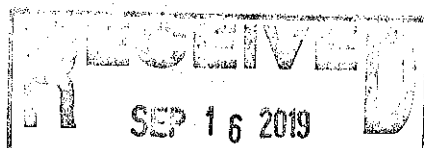
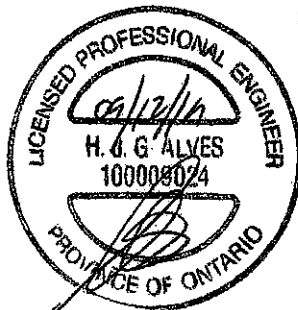
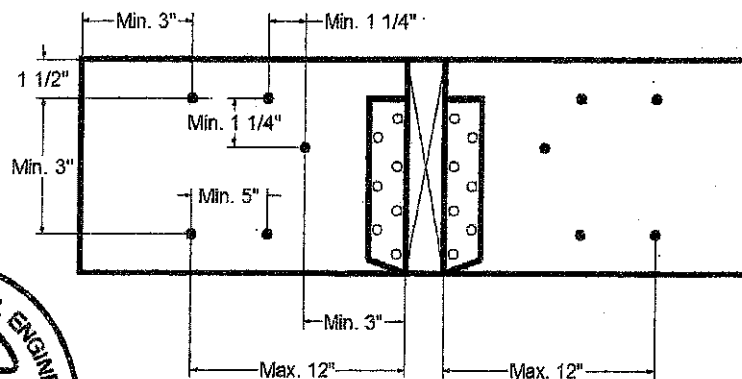
Capacity	78.8 %
Load	178.2 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 1-11-4 with a minimum of (6) – Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	78.8 %
Load	536.0 lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Load Combination	1.25D+1.5S
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



DWG NO. TAM 11923465
STRUCTURAL
COMPONENT ONLY

This design is valid until 12/11/2021

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115



Version 18.80.245 Powered by iStruct™
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LUL/LUS/LJS/HUS/HHUS/HGUS



Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



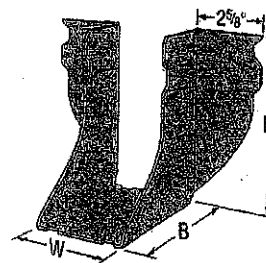
LUS26



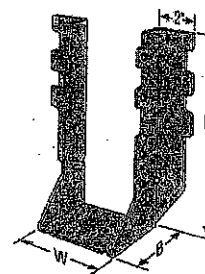
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



HHUS210-2



Double-Shear
Nailing
Top View

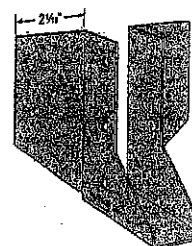
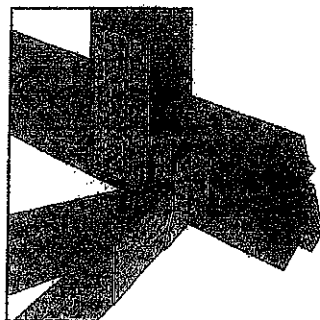


Double-Shear
Nailing
Side View;
Do not
bend tab



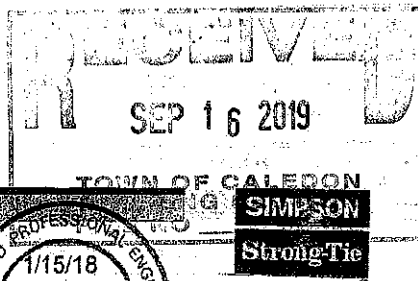
Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors



Simpson Strong-Tie® Wood Construction Connectors — Canadian Limit States Design

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

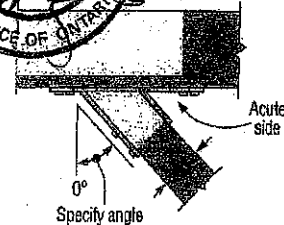
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance					
		W	H	B	d _e ^a	Header	Joist	D.F.C.		S-B-F			
								Uplift (K _u = 1.15)	Normal (K _u = 1.00)	Uplift (K _u = 1.15)	Normal (K _u = 1.00)		
								lb.	lb.	lb.	lb.		
Single 2x Sizes										lb.	lb.	kN	kN
LUS24	18	1 ¹ / ₈	3 ¹ / ₈	1 ¹ / ₈	2 ¹ / ₄	(4) 10d	(2) 10d	710	1625	645	1155		
								360	1020	320	725		
LU24L	22	1 ¹ / ₈	3	1 ¹ / ₈	2 ¹ / ₄	(4) 10d	(2) 10d x 1 1/2"	720	1805	645	1140		
								720	1805	645	1140		
LU26L	22	1 ¹ / ₈	5	1 ¹ / ₈	4 ¹ / ₈	(6) 10d	(4) 10d x 1 1/2"	1420	2170	1290	1630		
								1420	2170	1290	1630		
LUS26	18	1 ¹ / ₈	4 ¹ / ₈	1 ¹ / ₈	3 ¹ / ₈	(4) 10d	(4) 10d	2705	4940	2065	3875		
								2705	4940	2065	3875		
HUS26	16	1 ¹ / ₈	5 ¹ / ₈	3	3 ¹ / ₈	(14) 16d	(8) 16d	2055	4265	1460	4115		
								2055	4265	1460	4115		
LJS26DS	18	1 ¹ / ₈	5	3 ¹ / ₈	4 ¹ / ₈	(16) 16d	(6) 16d	2685	6625	2685	5700		
								2685	6625	2685	5700		
HGUS26	12	1 ¹ / ₈	5 ¹ / ₈	5	4 ¹ / ₈	(20) 16d	(8) 16d	1140	2185	1020	1550		
								1140	2185	1020	1550		
LU28L	20	1 ¹ / ₈	6 ¹ / ₈	1 ¹ / ₈	5 ¹ / ₈	(8) 10d	(6) 10d x 1 1/2"	1420	2620	1290	1790		
								1420	2620	1290	1790		
LUS28	18	1 ¹ / ₈	6 ¹ / ₈	1 ¹ / ₈	3 ¹ / ₈	(6) 10d	(4) 10d	3805	5395	2675	4345		
								3805	5395	2675	4345		
HUS28	16	1 ¹ / ₈	7 ¹ / ₈	3	6 ¹ / ₈	(22) 16d	(8) 16d	3310	7675	3310	6900		
								3310	7675	3310	6900		
HGUS28	12	1 ¹ / ₈	7 ¹ / ₈	5	6 ¹ / ₈	(36) 16d	(12) 16d	1140	2495	1020	1770		
								1140	2495	1020	1770		
LU210L	20	1 ¹ / ₈	8	1 ¹ / ₈	7 ¹ / ₈	(10) 10d	(6) 10d x 1 1/2"	1420	2785	1290	2210		
								1420	2785	1290	2210		
LUS210	18	1 ¹ / ₈	7 ¹ / ₈	1 ¹ / ₈	3 ¹ / ₈	(8) 10d	(4) 10d	645	1230	574	983		
								645	1230	574	983		

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply glider truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

C-C-CAN2018 © 2017 SIMPSON STRONG-TIE COMPANY INC.

SEP 16 2019

Simpson Strong-Tie® Wood Construction Connectors -- Canadian Limit States Design

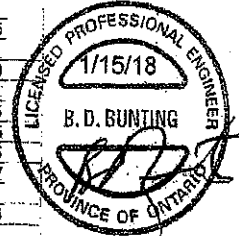
Face-Mount Hangers

BUILDING S
FILE NO
SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

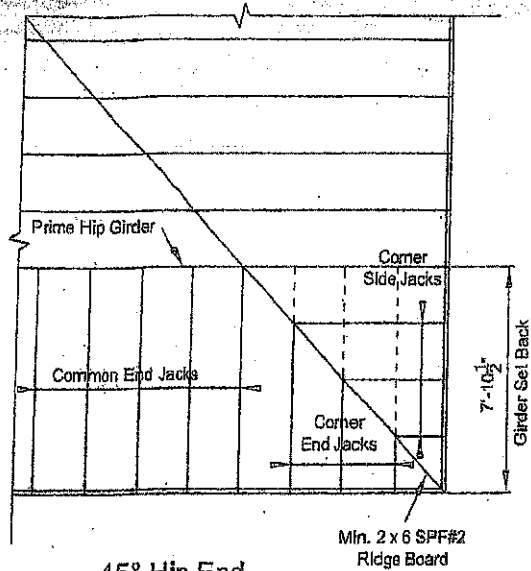
These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _h	Header	Joist		D-F-L		S-P-F	
									Uplift (K _u = 1.15)	Normal (K _n = 2.00)	Uplift (K _u = 1.15)	Normal (K _n = 1.00)
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Double 2x Sizes												
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d		835	2020	590	1435
SS LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d		2850	7335	2065	5205
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3110	6355
SS LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2575
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d		3765	8940	2675	6345
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
SS LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d		2580	4500	2320	3195
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d		4670	9660	4235	7000
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d		6840	14015	4855	10270
Triple 2x Sizes												
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3110	6355
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d		4670	9670	4235	6865
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d		6840	14645	4855	10400
Quadruple 2x Sizes												
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3110	6355
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HHUS210-4	14	6 1/8	9	3	7 1/8	(30) 16d	(10) 16d		4670	10155	4235	7210
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d		6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d		7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d		10130	16400	7195	11645
4x Sizes												
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d		1720	2595	1545	1920
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d		2540	7335	2065	5205
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3110	6355
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d		1720	3325	1545	2575
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d		3765	8940	2675	6345
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d		2580	4500	2320	3195
HGUS410	12	3 1/8	9	4	8 1/8	(48) 16d	(16) 16d		6840	14015	4855	10270
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d		7640	14995	5425	10645
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d		10130	16400	7195	11645



Plated Truss Connectors

RECEIVED
SEP 16 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO



45° Hip End

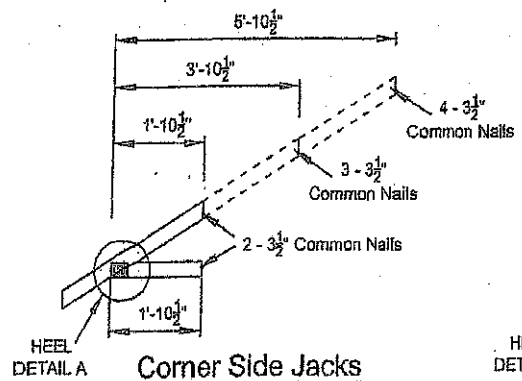
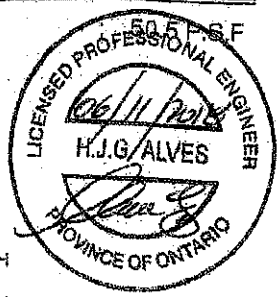
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

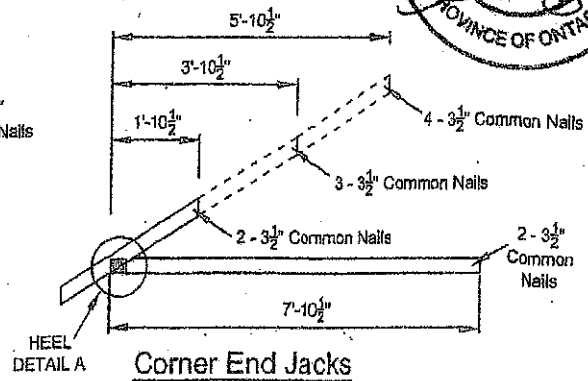
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

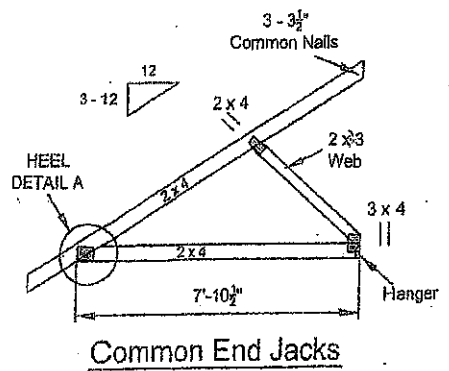
TOTAL LOAD



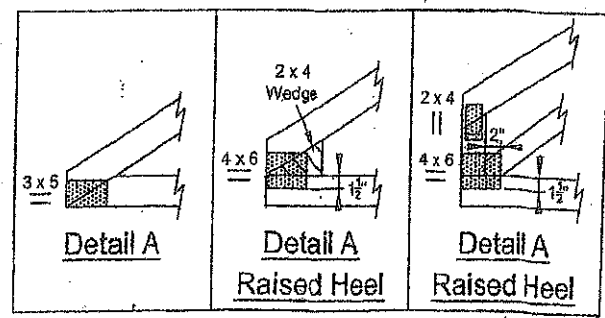
Corner Side Jacks



Corner End Jacks



Common End Jacks



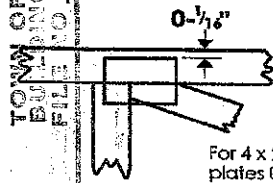
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/2" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



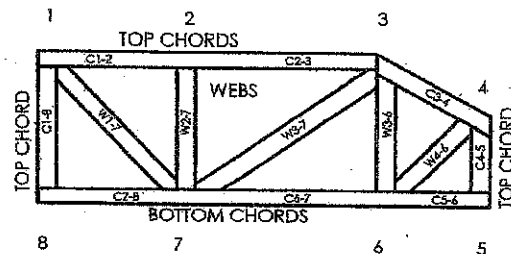
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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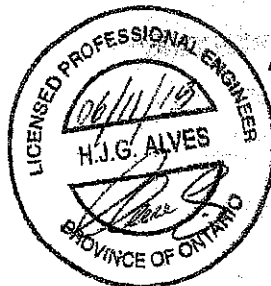
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MJT-7473C rev. 10-08

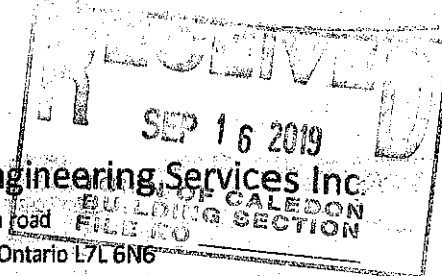
General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455



RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

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