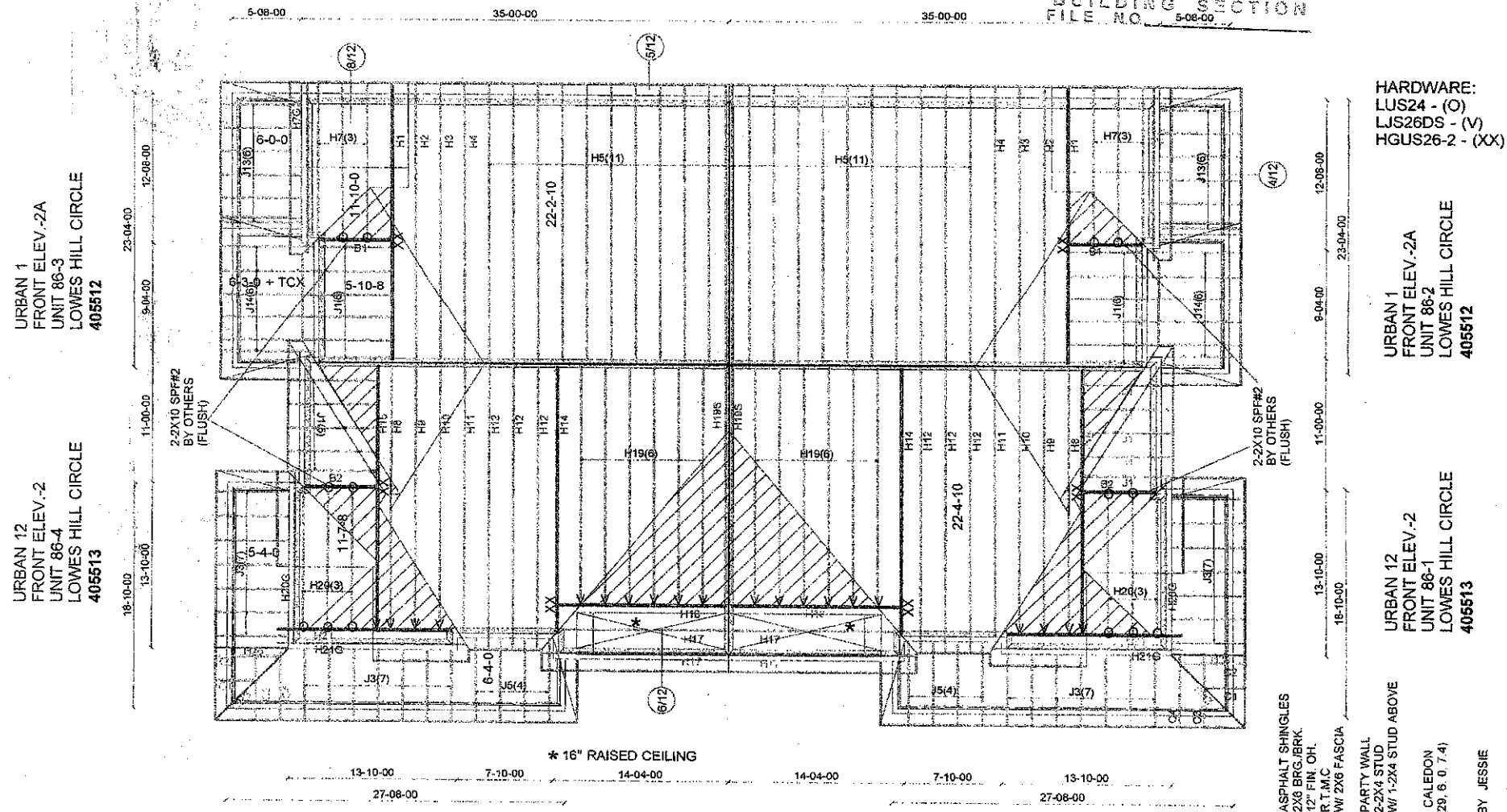


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TOWN OF CALEDON
BUILDING SECTION
FILE NO. 5-08-00



M 12630



Layout ID: 405511

Designer: AC

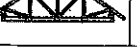
BLOCK 86 / UNITS 86-1 - 86-4

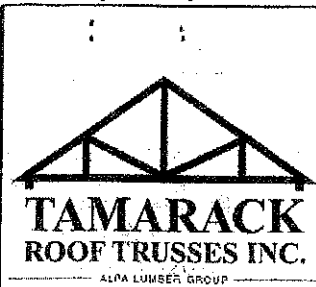
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 DELIVERY SHIPLIST		
TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: LAMBERT LANE PH.2 Location: CALEDON Model: BLOCK 86 Lot #: Elevation: UNITS 86-1, 86-4	Job Track: 50033 Plan Log: 201713 Layout ID: 405513 Ref # Page: 1 of 2 Date: 09/10/2019 Designer: Andrew Conway Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	H8 Half Hip	5/12	22-00-02	5-03-13	2 x 4		1-02-07 5-03-13	87.8 55.33		
	1	H9 Half Hip	5/12	22-00-02	6-07-13	2 x 4		1-02-07 6-07-13	94.89 61.17		
	1	H10 Half Hip	5/12	22-00-02	7-11-13	2 x 4		1-02-07 7-11-13	96.28 60.83		
	1	H11 Half Hip	5/12	22-04-10	9-03-13	2 x 4		1-00-09 9-03-13	103.89 65.17		
	3	H12 Monopitch	5/12	22-04-10	10-04-08	2 x 4	1-03-08	1-00-09 10-04-08	298.43 183.50		
	1 2-ply	H14 Monopitch Girder	5/12	22-04-10	10-04-08	2 x 4 2 x 6		1-00-09 10-04-08	244.78 150.33		
	1 2-ply	H15 Hip Girder	5/12	22-00-02	5-03-13	2 x 4 2 x 6		1-02-07 1-03-10	201.49 129.33		
	2	H17 Monopitch Structural Gable	6/12 12/12	13-08-10	8-00-05	2 x 4		1-02-00 6-08-05	115.24 77.33		
	1 2-ply	H18 Jack-Closed Girder	6/12	13-08-10	8-00-05	2 x 4 2 x 6		1-02-00 8-00-05	141.36 88.00		
	6	H19 Common	5/12	19-09-08	10-04-08	2 x 4		2-01-09 10-04-08	531.48 330.00		
	1	H19S GABLE	5/12	18-09-08	10-04-08	2 x 4		2-06-09 10-04-08	101.77 64.33		
	3	H20 Common	8/12	11-07-08	5-08-13	2 x 4		1-04-13 2-03-13	144.13 95.00		
	1	H20G GABLE	8/12	11-07-08	5-08-13	2 x 4	1-03-08	1-04-13 2-03-13	52.6 34.33		
	1 2-ply	H21 Common Girder	8/12	13-00-00	5-08-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	137.09 85.67		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BLOCK 86
 Lot #:
 Elevation: UNITS 86-1, 86-4

Job Track: 50033
 Plan Log: 201713
 Layout ID: 405513
 Ref # SEP 16 2019
 Page: 2 of 2
 Date: 09/10/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	H21G GABLE	8/12	13-00-00	5-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	57.01 36.33		
	1	H26 Monopitch Girder	4/12	4-07-00	2-03-03	2 x 4	1-03-08	7-15 2-02-04	17.84 12.67		
	5	J1 Jack-Open	8/12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	89.15 53.33		
	14	J3 Jack-Open	4/12	5-04-00	2-06-03	2 x 4	1-03-08	7-15 2-02-04	206.9 149.33		
	4	J5 Jack-Open	4/12	6-04-00	2-10-03	2 x 4	1-03-08	7-15 2-06-04	68.48 48.00		

TOTAL # TRUSS= 53 TOTAL BFT OF ALL TRUSSES= 1779.98 BFT. TOTAL WEIGHT OF ALL TRSSES 2790.61 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LUS24	
10	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF
ITEMS= 17

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST			
	Lumber Yard: TAMARACK LUMBER	Job Track: 50033	Plan Log: 201713	
Builder: GREEN PARK HOMES	Layout ID: 405512	Ref #		
Project: LAMBERT LANE PH.2	Page: 1 of 1	Date: 09/13/2019		
Location: CALEDON	Designer: Andrew Conway	Sales Rep: Mario DiCano		
Model: BLOCK 86				
Lot #:				
Elevation: UNITS 86-2, 86-3				

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	H1 Half Hip Girder	5 / 12	22-02-10	5-03-13	2 x 4 2 x 6	1-03-08	1-00-09 5-03-13	219.48 135.00		
	1	H2 Half Hip	5 / 12	22-02-10	6-07-13	2 x 4	1-03-08	1-00-09 6-07-13	98.92 62.50		
	1	H3 Half Hip	5 / 12	22-02-10	7-11-13	2 x 4	1-03-08	1-00-09 7-11-13	98.18 61.67		
	1	H4 Half Hip	5 / 12	22-02-10	9-03-13	2 x 4	1-03-08	1-00-09 9-03-13	105.08 66.50		
	11	H5 Monopitch	5 / 12	22-02-10	10-03-10	2 x 4	1-03-08	1-00-09 10-03-10	1086.42 672.63		
	3	H7 Common	8 / 12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	157.12 95.50		
	1	H7G GABLE	8 / 12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	48.73 30.83		
	6	J1 Jack-Open	8 / 12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	106.98 64.00		
	6	J13 Jack-Open	4 / 12	6-00-00	2-08-14	2 x 4	1-03-08	7-15 2-04-15	98.04 64.00		
	6	J14 Jack-Closed	4 / 12	6-03-00	3-00-14	2 x 4	1-03-08	7-15 2-08-15	186.79 119.00		

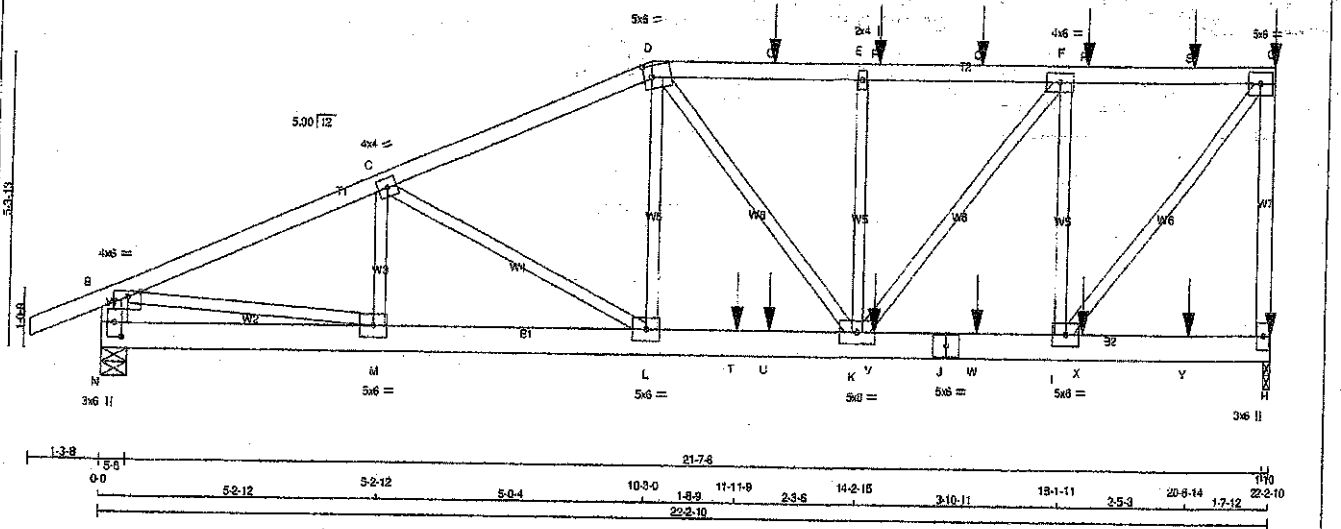
TOTAL # TRUSS= 38 TOTAL BFT OF ALL TRUSSES= 1371.83 BFT. TOTAL WEIGHT OF ALL TRSSES 2183.74 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF
ITEMS= 3

JOB NAME 405481	TRUSS NAME H1	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						
Version 8.300 9 May 10 2019 Mitek Industries, Inc. Fri Sep 13 12:21:51 2019 Page 1						
ID:CGPEtWIRwz3f2dD6UHFYfR8a-6bVNIcPINU2xQQR2M3fTtCsVqPOXf7C5Uvrye3DU						
Scale = 1/32"						



LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	SIDE(61.0)
G-H 1	12	TOP
N-B 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N-J 2	12	SIDE(183.1)
J-H 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	4.0	6.0	Edge	
C	TMWV-w	MT20	4.0	4.0		
D	TMWV-m	MT20	5.0	6.0	2.50	1.75
E	TMWV-w	MT20	2.0	4.0		
F	TMWV-i	MT20	4.0	6.0		
G	TMWV-i	MT20	5.0	6.0		
H	BMV1-p	MT20	3.0	6.0		
I, L, M						
J	BS-2	MT20	5.0	6.0		
K	BMWVW-i	MT20	5.0	8.0		
N	BMV1-p	MT20	3.0	6.0	3.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
H	2748	0	0	2748	0	0	0	1-10	1-10
N	2223	0	0	2223	0	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOOSE	MAX. MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL
H	1931	1327.0	0.0	0.0	0.0	604.0
N	1590	1094.0	0.0	0.0	0.0	465.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		VERT. LOAD LC1		MAX. UNBRACED LENGTH FR-TO		WEBS		FACTORED	
MEMB.	FORCE (LBS)	FROM	TO	PLF	CSI (LC)	MEMB.	FORCE (LBS)	FROM	TO	MEMB.	FORCE (LBS)
FR-TO											
A-B	0.726	-102.0	-102.0	0.07 (1)	10.00	M-C	-466.0	0.05 (1)			
B-C	-3551.0	-102.0	-102.0	0.30 (1)	4.88	C-L	-206.0	0.08 (1)			
C-D	-3401.0	-102.0	-102.0	0.29 (1)	4.78	L-D	0.1040	0.13 (1)			
D-O	-3024.0	-102.0	-102.0	0.20 (1)	5.11	I-G	0.2913	0.36 (1)			
O-E	-3024.0	-102.0	-102.0	0.20 (1)	5.07	D-K	-184.0	0.08 (1)			
E-P	-3024.0	-102.0	-102.0	0.23 (1)	5.07	F-F	-2208.0	0.45 (1)			
P-Q	-3024.0	-102.0	-102.0	0.23 (1)	5.07	K-E	-672.0	0.14 (1)			
Q-F	-3024.0	-102.0	-102.0	0.23 (1)	5.07	K-F	0.1890	0.23 (1)			
F-R	-1854.0	-102.0	-102.0	0.21 (1)	6.13	B-M	0.3324	0.41 (1)			
R-S	-1854.0	-102.0	-102.0	0.21 (1)	6.13						
S-G	-1854.0	-102.0	-102.0	0.21 (1)	6.13						
H-G	-2843.0	0.0	0.0	0.61 (1)	7.00						
N-S	-2138.0	0.0	0.0	0.07 (1)	7.81						
N-M	0.0	-18.5	-18.5	0.07 (1)	10.00						
M-L	0.3298	-18.5	-18.5	0.35 (1)	10.00						
L-T	0.3139	-18.5	-18.5	0.52 (1)	10.00						
T-U	0.3139	-18.5	-18.5	0.52 (1)	10.00						
U-K	0.3139	-18.5	-18.5	0.52 (1)	10.00						
K-V	0.1854	-18.5	-18.5	0.21 (1)	10.00						
V-J	0.1854	-18.5	-18.5	0.21 (1)	10.00						
J-W	0.1854	-18.5	-18.5	0.21 (1)	10.00						
W-I	0.1854	-18.5	-18.5	0.21 (1)	10.00						
I-X	0.0	-18.5	-18.5	0.04 (4)	10.00						
X-Y	0.0	-18.5	-18.5	0.04 (4)	10.00						
Y-H	0.0	-18.5	-18.5	0.04 (4)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
G	22-2-10	-164	-164		FRONT	VERT	-164
H	22-2-10	-35	-35		FRONT	VERT	-35
O	12-4-14	-123	-123		FRONT	VERT	-123
P	14-6-14	-123	-123		FRONT	VERT	-123
Q	16-6-14	-123	-123		FRONT	VERT	-123
R	18-6-14	-123	-123		FRONT	VERT	-123
S	20-6-14	-123	-123		FRONT	VERT	-123
T	11-11-9	-1206	-1206		FRONT	VERT	-1206
U	12-6-14	-27	-27		FRONT	VERT	-27
V	14-6-14	-27	-27		FRONT	VERT	-27
W	16-6-14	-27	-27		FRONT	VERT	-27
X	18-6-14	-27	-27		FRONT	VERT	-27
Y	20-6-14	-27	-27		FRONT	VERT	-27

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")

CALCULATED VERT. DEFL.(LL) = L/999 (0.07")

ALLOWABLE DEFL.(TL) = L/360 (0.74")

CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TO=0.61/1.00 (G-H-1), BC=0.52/1.00 (K-L-1), WB=0.45/1.00 (F-I-1), SSI=0.26/1.00 (K-L-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEBL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	PLATE GRIP(WET)	SECTION
(PS)	(PS)	(PS)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818	354	1667
			1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

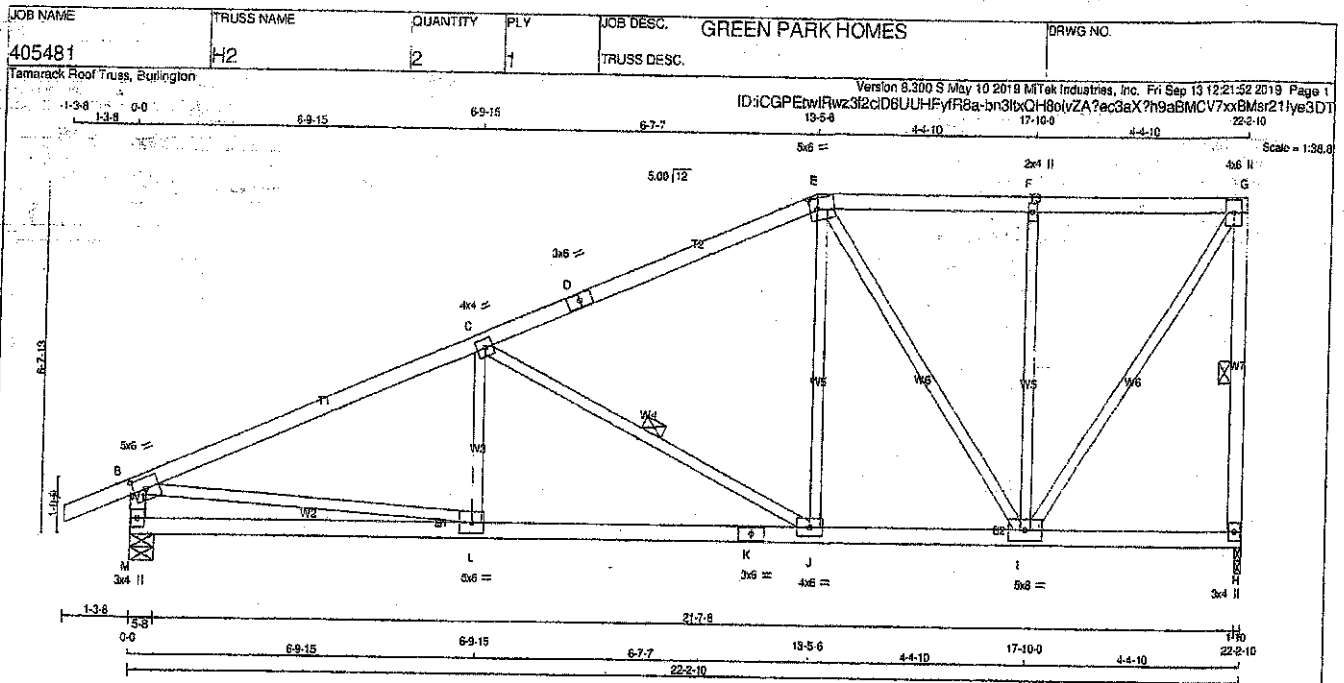
JSI GRIP= 0.88 (B) (INPUT = 0.90)

JSI METAL= 0.41 (M) (INPUT = 1.00)

Structural component only

DWG# T-1923489 CONTINUED ON PAGE 2

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE CANADIAN BUILDING CODE



TOTAL WEIGHT = 2 X 97 = 194 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		
M - B	2x4 DRY	No.2	SPF		
M - K	2x4 DRY	No.2	SPF		
K - H	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVV-t	MT20	5.0	6.0	2.50	2.75	
C TMWW-t	MT20	4.0	4.0			
D TS-t	MT20	3.0	6.0			
E TTWW-m	MT20	5.0	6.0	2.50	1.75	
F TMWW-w	MT20	2.0	4.0			
G TMVV-w	MT20	4.0	6.0			
H BMV1-p	MT20	3.0	4.0			
I BMWW-t	MT20	5.0	6.0			
J BMWW-t	MT20	4.0	6.0			
K BS-t	MT20	3.0	6.0			
L BMWW-t	MT20	5.0	6.0			
M BMV1-p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

SECTIONS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORIZ	DOWN	HORIZ
	UPLIFT	IN-SX	IN-SX	IN-SX
JT	1339	0	1338	0
H	1478	0	1476	0
M	1478	0	1476	0

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	942	644 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0
H	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.85 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING											
TOTAL LOAD CASES: (4)											
CHORDS						WEBS					
MAX. FACTORED			FACTORED			MAX.			MAX.		
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)				(LBS)		(LBS)	CSI (LC)	
FR-TO		FROM TO			LENGTH	FR-TO					
A-B	0.26	-102.0	-102.0	0.13 (1)	10.00	L-C	-72 / 98			0.03 (4)	
B-C	-2088 / 0	-102.0	-102.0	0.70 (1)	3.85	C-J	-929 / 0			0.37 (1)	
C-D	-1244 / 0	-102.0	-102.0	0.62 (1)	4.87	J-E	0 / 542			0.12 (1)	
D-E	-1244 / 0	-102.0	-102.0	0.62 (1)	4.87	E-I	-604 / 0			0.70 (1)	
E-F	-786 / 0	-102.0	-102.0	0.25 (1)	6.25	I-F	-552 / 0			0.40 (1)	
F-G	-786 / 0	-102.0	-102.0	0.25 (1)	6.25	I-G	0 / 1376			0.31 (1)	
G-H	-1303 / 0	0.0	0.0	0.26 (1)	5.68	B-L	0 / 1949			0.44 (1)	
H-M	-1423 / 0	0.0	0.0	0.14 (1)	6.84						
M-L	0 / 0	-18.5	-18.5	0.21 (4)	10.00						
L-K	0 / 1938	-18.5	-18.5	0.40 (1)	10.00						
K-J	0 / 1938	-18.5	-18.5	0.40 (1)	10.00						
J-I	0 / 1127	-18.5	-18.5	0.25 (1)	10.00						
I-H	0 / 0	-18.5	-18.5	0.07 (4)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA D88-09, CSA D88-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.74")

CALCULATED VERT. DEFL. (LL) = L/999 (0.08")

ALLOWABLE DEFL. (TL) = L/360 (0.74")

CALCULATED VERT. DEFL. (TL) = L/999 (0.15")

CS: TC=0.70/1.00 (B-C-1), BC=0.40/1.00 (J-L-1), WB=0.70/1.00 (E-I-1), SS=0.31/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(FS) (PL) (PL)

MAX MIN MAX MIN MAX MIN

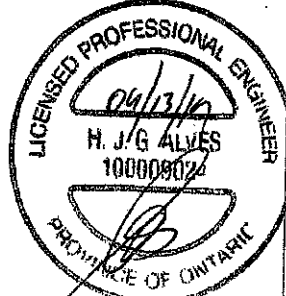
MT20 618 354 1667 788 1987 1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (I) (INPUT = 0.90)

JSI METAL = 0.99 (K) (INPUT = 1.00)



THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

Structural component only
DWG# T-1923490

JOB NAME 405481	TRUSS NAME H-3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:CGPEtWIRwz3f2cd6UUFyR8a-3zd74HRvV5UBKaq9n6mYujpmZQsSKibWabakyE3DS

1-3-8 0-0 5-8-4 5-8-4 5-5-12 11-2-1 5-5-12 16-7-13 5-5-13 4x6 II 4x6 II

1-3-8 1-3-8 5-8-4 5-8-4 5-5-12 11-2-1 5-5-12 16-7-13 5-5-13 4x6 II 4x6 II

5.00 / 12

Scale = 1/4" = 1'-0"

LUMBER

N.L.G.A. RULES

CHORDS SIZE LUMBER DESCR.

A - D 2x4 DRY No.2 SPF

D - F 2x4 DRY No.2 SPF

F - G 2x4 DRY No.2 SPF

H - B 2x4 DRY No.2 SPF

M - K 2x4 DRY No.2 SPF

K - H 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED MAXIMUM FACTORED INPUT REQD

GROSS REACTION GROSS REACTION BRG BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

H 1339 0 1339 0 0 1-10 1-10

M 1476 0 1476 0 0 5-8 5-8

UNFACTORED REACTIONS

1ST LCASE MAX/MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

H 942 644 / 0 0 / 0 0 / 0 0 / 0 298 / 0 0 / 0

M 1037 723 / 0 0 / 0 0 / 0 0 / 0 314 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS. WEBS

MEMB. FORCE FACTORED MAX. FACTORED

(LBS) (PLF) CSI (LC) UNBRAC (LBS) MAX (LBS) CSI (LC)

FR-TO FROM TO LENGTH FR-TO

A-B 0 / 26 -102.0 -102.0 0.13 (1) 10.00 L-C -143 / 51 0.08 (1)

B-C -2067 / 0 -102.0 -102.0 0.45 (1) 4.26 C-J -559 / 0 0.40 (1)

C-D -1581 / 0 -102.0 -102.0 0.36 (1) 4.90 J-E 0 / 378 0.08 (1)

D-E -1581 / 0 -102.0 -102.0 0.36 (1) 4.90 E-I 997 / 0 0.40 (1)

E-F -805 / 0 -102.0 -102.0 0.35 (1) 6.25 I-G -207 / 13 0.25 (1)

F-G -721 / 0 -102.0 -102.0 0.41 (1) 6.25 I-G 0 / 1245 0.28 (1)

H-G -1298 / 0 0.0 0.0 0.37 (1) 5.68 B-L 0 / 1843 0.44 (1)

M-B -1430 / 0 0.0 0.0 0.14 (1) 6.82

M-L 0 / 0 -18.5 -18.5 0.13 (4) 10.00

L-K 0 / 1927 -18.5 -18.5 0.36 (1) 10.00

K-J 0 / 1927 -18.5 -18.5 0.36 (1) 10.00

J-I 0 / 1441 -18.5 -18.5 0.30 (1) 10.00

I-H 0 / 0 -18.5 -18.5 0.14 (4) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018 , CBC 2012

- CSA 089-09, CSA 086-14

- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.I. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/980 (0.74")

CALCULATED VERT. DEFL.(LL) = L/999 (0.07")

ALLOWABLE DEFL.(TL) = L/980 (0.74")

CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.45/1.00 (B-C:1), BC=0.38/1.00 (J-L:1), WB=0.44/1.00 (B-L:1), SS=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1897 798 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 3.0 Deg.

JSI GRIP = 0.84 (L) (INPUT = 0.90)

JSI METAL = 0.57 (K) (INPUT = 1.00)

Structural component only

DWG# T-1923491

LICENSED PROFESSIONAL ENGINEER

09/13/17

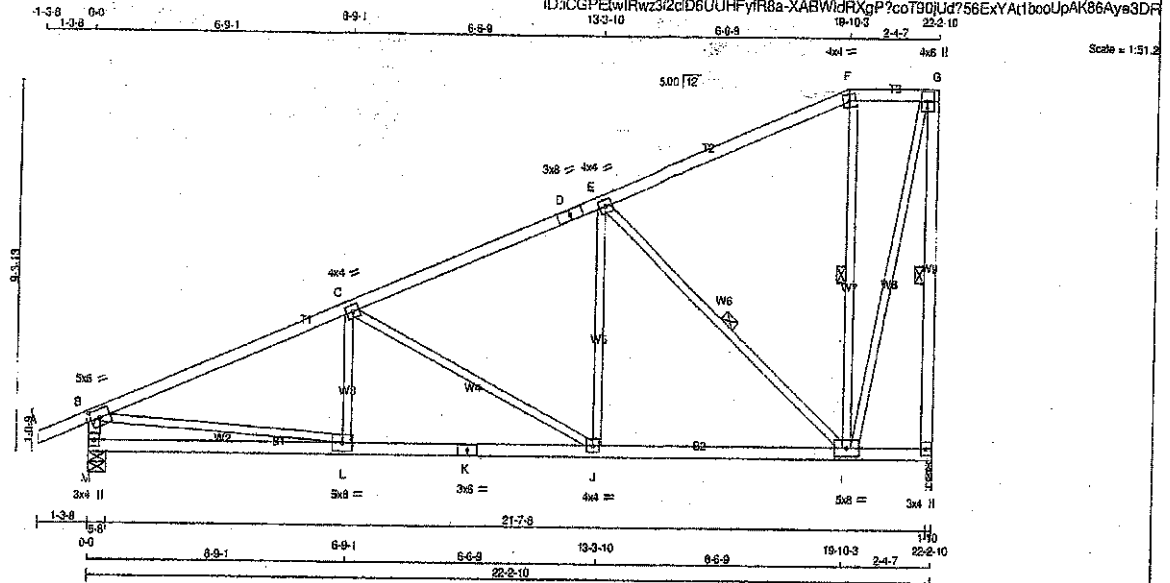
H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

JOB NAME 405481	TRUSS NAME H4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 5 May 10 2019 MTOK Industries, Inc. Fr Sep 13 12:21:54 2019 Page 1
ID:JGPEWIRwz32cD6UHFYfR8a-XABWdRXqP?coT90Jd756EXYAtIbcoUpAK86Ays3DR



TOTAL WEIGHT = 2 X 105 = 210 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMVW-t	MT20	4.0	4.0		
D	TS-l	MT20	3.0	8.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-p	MT20	4.0	6.0		
H	BMV1-p	MT20	3.0	4.0		
I	BMVWV-t	MT20	5.0	8.0		
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	1339	0	1339	0	1-10	1-10
M	1476	0	1476	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, E-I, F-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	L-C	-79 / 58	0.03 (4)	
B-C	-2042 / 0	-102.0	-102.0	0.65 (1)	3.99	C-J	-787 / 0	0.87 (1)	
C-D	-1318 / 0	-102.0	-102.0	0.50 (1)	5.00	E-J	0 / 505	0.11 (1)	
D-E	-1318 / 0	-102.0	-102.0	0.50 (1)	5.00	E-I	-1252 / 0	0.73 (1)	
E-F	-358 / 0	-102.0	-102.0	0.51 (1)	6.25	F-F	-261 / 0	0.15 (1)	
F-G	-304 / 0	-102.0	-102.0	0.07 (1)	6.25	I-G	0 / 1249	0.28 (1)	
H-G	-1331 / 0	0.0	0.0	0.54 (1)	5.82	G-L	0 / 1920	0.43 (1)	
M-B	-1428 / 0	0.0	0.0	0.14 (1)	6.84				
M-L	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
L-K	0 / 1908	-18.5	-18.5	0.40 (1)	10.00				
K-J	0 / 1908	-18.5	-18.5	0.40 (1)	10.00				
J-I	0 / 1217	-18.5	-18.5	0.29 (1)	10.00				
I-H	0 / 0	-18.5	-18.5	0.11 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, CSC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(85% OF 97.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.85/1.00 (B-C:1), BC=0.40/1.00 (J-L:1), WB=0.87/1.00 (C-J:1), SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (G) (INPUT = 0.90)
JSI METAL=0.83 (K) (INPUT = 1.00)

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**

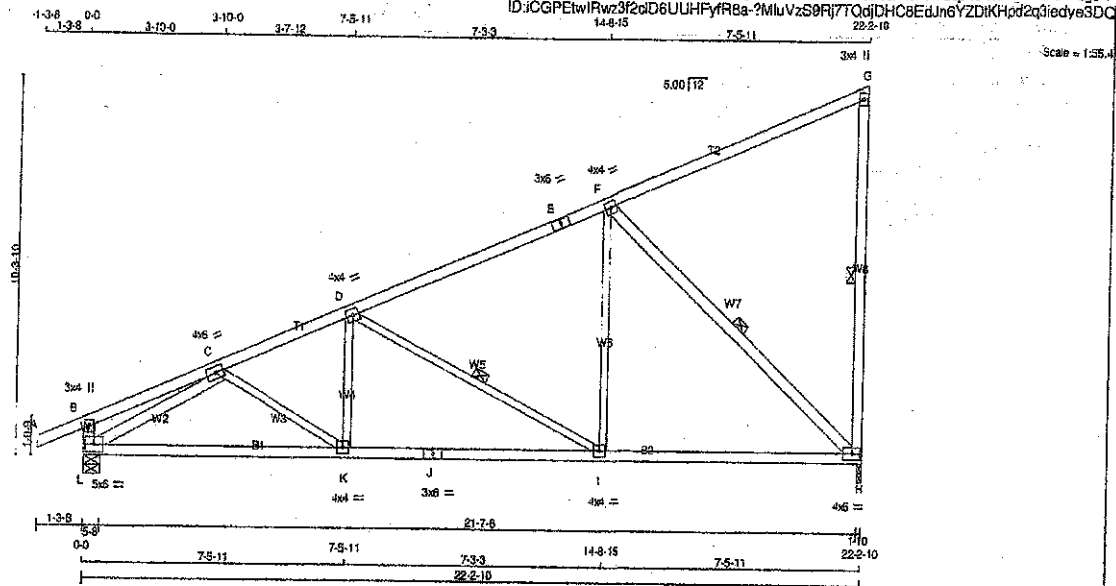


Structural component only
DWG# T-1923492

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H5	22	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 12:21:55 2019 Page 1
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22-2-10



TOTAL WEIGHT = 22 X 100 = 2203 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4 DRY	No.2			SPF
S - G	2x4 DRY	No.2			SPF
H - G	2x4 DRY	No.2			SPF
L - S	2x4 DRY	No.2			SPF
J - J	2x4 DRY	No.2			SPF
L - H	2x4 DRY	No.2			SPF
ALL WEBS	2x3 DRY	No.2			SPF
EXCEPT					
F - H	2x4 DRY	No.2			SPF
L - C	2x4 DRY	No.2			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	3.0		
D	TMVW-1	MT20	4.0	4.0		
E	TS-1	MT20	3.0	6.0		
F	TMVW-1	MT20	4.0	4.0		
G	TMV-p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	6.0		
I	BMVW-1	MT20	4.0	4.0		
J	BS-1	MT20	3.0	6.0		
K	BMVW-1	MT20	4.0	4.0		
L	BMVW-1	MT20	6.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	1339	0	1339	0
H	1478	0	1478	0

UNFACTORED REACTIONS	1ST CASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERALIVE	WIND	DEAD	SOIL
JT	COMBINED							
H	942	844 / 0	0 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	1037	723 / 0	0 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING	CHORDS	WEBS
TOTAL LOAD CASES: (4)	MAX. FACTORED	MAX. FACTORED
	FORCE (LBS)	FORCE (LBS)
	VERT. LOAD (LBS)	VERT. LOAD (LBS)
	FROM TO	FROM TO
FR-TO		
A-B	0 / 26	-102.0 -102.0 0.13 (1)
B-C	0 / 8	-102.0 -102.0 0.20 (1)
C-D	-1944 / 0	-102.0 -102.0 0.33 (1)
D-E	-1140 / 0	-102.0 -102.0 0.69 (1)
E-F	-1140 / 0	-102.0 -102.0 0.69 (1)
F-G	-32 / 0	-102.0 -102.0 0.70 (1)
G-H	-298 / 0	0.0 0.0 0.16 (1)
H-I	-308 / 0	0.0 0.0 0.03 (1)
L-K	0 / 1606	-18.5 -18.5 0.42 (1)
K-J	0 / 1812	-18.5 -18.5 0.43 (1)
J-I	0 / 1812	-18.5 -18.5 0.43 (1)
I-H	0 / 1069	-18.5 -18.5 0.32 (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 088-09, CSA 086-14
- TFC 2011, TFC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/980 (0.74")
CALCULATED VERT. DEFL.(LL) = L/989 (0.07")
ALLOWABLE DEFL.(TL) = L/880 (0.74")
CALCULATED VERT. DEFL.(TL) = L/889 (0.15")

CSI: TC=0.70/1.00 (F-G:1), BC=0.49/1.00 (I-K:1), WB=0.78/1.00 (F-H:1), SS=0.39/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

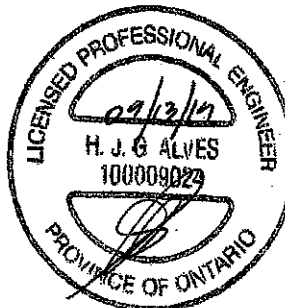
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (F) (INPUT = 0.90)
JSI METAL = 0.57 (J) (INPUT = 1.00)

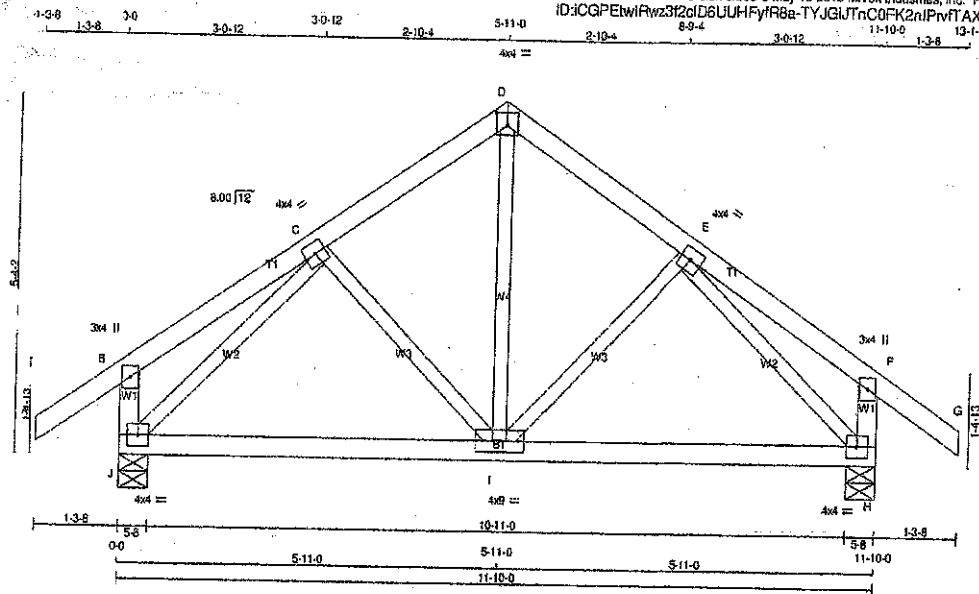


RECEIVED
SEP 16 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

Structural component only
DWG# T-1923493

JOB NAME 405481	TRUSS NAME H7	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 5 May 10 2016 MITek Industries, Inc. Fri Sep 13 12:21:58 2019 Page 1 ID:CGPEWIRwz312dID6UHFYIR8a-TYJGIJThC0FK2niPvTtAXKP2z3sQnHUpFA3ye3DF		



Scale = 1/32.0

TOTAL WEIGHT = 6 X 52 = 312 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2			
D - G	2x4 DRY	No.2			
J - B	2x4 DRY	No.2			
H - F	2x4 DRY	No.2			
J - H	2x4 DRY	No.2			
ALL WEBS	2x3 DRY	No.2			
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0			
C TMWW-1	MT20	4.0	4.0			
D TTW-p	MT20	4.0	4.0	2.25	2.00	
E TMWW-1	MT20	4.0	4.0			
F TMV+p	MT20	3.0	4.0			
H BMVW-1	MT20	4.0	4.0			
I BMVWW-1	MT20	4.0	9.0			
J BMVW-1	MT20	4.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
JT 853	0	0	0
H 853	0	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED							
JT 596	423 / 0	0 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
H 596	423 / 0	0 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE	FACTORED	VERT. LOAD	MAX	MAX.	MEMB.	FORCE	FACTORED	MAX
FR-TO		(LBS)	(PLF)	CS (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CS (LC)	
A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00	I-D	0 / 342	0.08 (1)		
B-C	0 / 18	-102.0	-102.0	0.14 (1)	10.00	I-E	-145 / 0	0.04 (1)		
C-D	-540 / 0	-102.0	-102.0	0.11 (1)	6.25	C-I	-145 / 0	0.04 (1)		
D-E	-540 / 0	-102.0	-102.0	0.11 (1)	6.25	J-C	-770 / 0	0.22 (1)		
E-F	0 / 18	-102.0	-102.0	0.14 (1)	10.00	E-H	-770 / 0	0.22 (1)		
F-G	0 / 39	-102.0	-102.0	0.14 (1)	10.00					
J-B	-257 / 0	0.0	0.0	0.03 (1)	7.81					
H-F	-257 / 0	0.0	0.0	0.03 (1)	7.81					
J-I	0 / 539	-18.5	-18.5	0.22 (4)	10.00					
I-H	0 / 539	-18.5	-18.5	0.22 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CS: TC=0.14/1.00 (A-B-1), BC=0.22/1.00 (I-J-4), WB=0.22/1.00 (C-J-1), SS=0.12/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

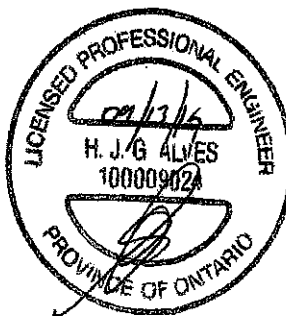
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PSI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1657 786 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)



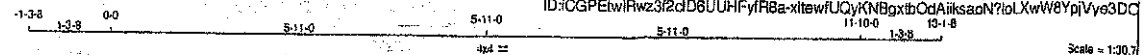
Structural component only
DWG# T-1923494

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H7G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:CGPEtwRwz3f2cdD6UHFyR8a-xlwWfUKyKNGxbOdAmsaon7ioLXwW8YpVye3DC



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - S	2x4 DRY	No.2	SPF		
A - E	2x4 DRY	No.2	SPF		
E - I	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
P - J	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
ALL GABLE WEBS	2x3 DRY	No.2	SPF		
DRY, SEASONED LUMBER.					

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C, D, F, G					
C TMW+w	MT20	2.0	4.0		
E TMV+p	MT20	4.0	4.0	2.25	2.00
H TMV+p	MT20	3.0	4.0		
J BMV1+p	MT20	3.0	4.0		
K, L, M, N, O					
K BMW1+w	MT20	2.0	4.0		
P BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (L/C)	
P-B	-241 / 0	0.0	0.04 (1)	H-E	-250 / 0	0.10 (1)	
A-B	0 / 39	-102.0	-102.0 0.14 (1)	N-D	-217 / 0	0.05 (1)	
B-C	0 / 15	-102.0	-102.0 0.09 (1)	O-C	-181 / 0	0.03 (1)	
C-D	0 / 39	-102.0	-102.0 0.08 (1)	L-F	-217 / 0	0.05 (1)	
D-E	0 / 38	-102.0	-102.0 0.08 (1)	K-G	-161 / 0	0.03 (1)	
E-F	0 / 38	-102.0	-102.0 0.08 (1)				
F-G	0 / 39	-102.0	-102.0 0.08 (1)				
G-H	0 / 15	-102.0	-102.0 0.09 (1)				
H-I	0 / 39	-102.0	-102.0 0.14 (1)				
J-H	-241 / 0	0.0	0.04 (1)				
P-O	-23 / 0	-18.5	-18.5 0.02 (4)				
O-N	-29 / 0	-18.5	-18.5 0.02 (4)				
N-M	-33 / 0	-18.5	-18.5 0.01 (4)				
M-L	-33 / 0	-18.5	-18.5 0.01 (4)				
L-K	-29 / 0	-18.5	-18.5 0.02 (4)				
K-J	-23 / 0	-18.5	-18.5 0.02 (4)				

TOTAL WEIGHT = 2 X 48 = 97 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NSCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (K-L:4),
WB=0.10/1.00 (E-M:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

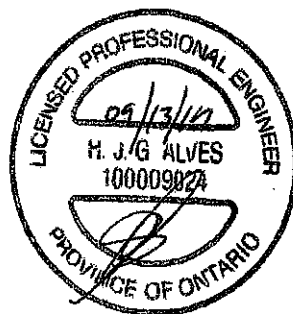
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 798 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

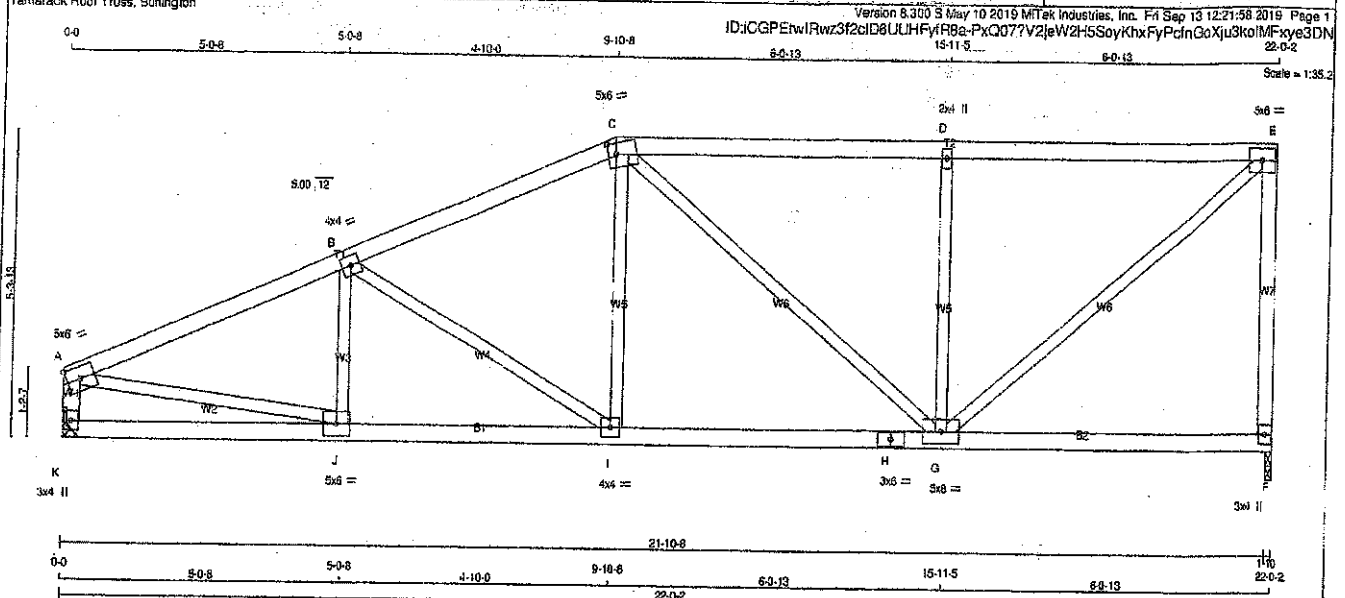
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (E) (INPUT = 0.90)
JSI METAL = 0.11 (D) (INPUT = 1.00)



Structural component only
DWG# T-1923495

JOB NAME 405481	TRUSS NAME H8	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER				DESIGN CRITERIA			
N.L.G.A. RULES				TOTAL WEIGHT = 2 X 88 = 176 lb			
CHORDS				BUILDING DESIGNER			
A - C	2x4	DRY	No.2	BEARINGS			
C - E	2x4	DRY	No.2	FACTORED MAXIMUM FACTORED INPUT RECORD			
F - E	2x4	DRY	No.2	GROSS REACTION GROSS REACTION BRG BRG			
K - A	2x4	DRY	No.2	JT VERT SNOW DOWN HORZ UPLIFT IN-SX IN-SX			
K - F	2x4	DRY	No.2	SPF 1328 0 1328 0 0 1-10 1-10			
H - F	2x4	DRY	No.2	SPF 1328 0 1328 0 0 MECHANICAL			
ALL WEBS 2x3 DRY No.2				A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, MINIMUM BEARING LENGTH AT JOINT K = 3'-6"			
EXCEPT				SPF			

DRY: SEASONED LUMBER				UNFACTORED REACTIONS			
PLATES (table is in inches)				1ST CASE MAX/MIN COMPONENT REACTIONS			
JT TYPE	PLATES	W	LEN	Y	X	Edge	
A	TMVW-1	MT20	5.0	8.0			
B	TMVW-1	MT20	4.0	4.0			
C	TMVW-m	MT20	5.0	6.0	2.25	1.75	
D	TMVW-w	MT20	2.0	4.0			
E	TMVW-t	MT20	5.0	6.0			
F	BMV1-p	MT20	3.0	4.0			
G	BMVWW-1	MT20	5.0	8.0			
H	BS-t	MT20	3.0	6.0			
I	BMVWW-t	MT20	4.0	4.0			
J	BMVWW-1	MT20	5.0	6.0			
K	BMV1-p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING				CHORDS				WEBS			
TOTAL LOAD CASES: (4)				MAX. FACTORED				MAX. FACTORED			
				MEMB. FORCE (LBS)				MEMB. FORCE (LBS)			
				VERT. LOAD (LBS)				VERT. LOAD (LBS)			
				MAX. (PLF)				MAX. (PLF)			
				CS1 (LC)				CS1 (LC)			
				MAX. (LC)				MAX. (LC)			
				UNBRACED				UNBRACED			
				LENGTH FR-TO				LENGTH FR-TO			
				A-B				A-B			
				B-C				B-C			
				C-D				C-D			
				D-E				D-E			
				E-F				E-F			
				F-G				F-G			
				G-H				G-H			
				H-I				H-I			
				I-J				I-J			
				J-K				J-K			
				K-L				K-L			

A-B				-1850 / 0				-102.0				-102.0				0.49 (1)				4.27				J-B				-232 / 12				0.05 (1)			
B-C				-1854 / 0				-102.0				-102.0				0.46 (1)				4.58				B-1				-444 / 0				0.25 (1)			
C-D				-1287 / 3				-102.0				-102.0				0.70 (1)				4.50				C-C				0 / 332				0.07 (1)			
D-E				-1287 / 0				-102.0				-102.0				0.71 (1)				4.50				C-G				-235 / 0				0.29 (1)			
E-F				-1280 / 0				0.0				0.0				0.63 (1)				7.13				G-D				-767 / 0				0.32 (1)			
F-G				-1284 / 0				0.0				0.0				0.13 (1)				7.11				G-E				0 / 1841				0.37 (1)			
G-H				0 / 0				-18.5				-18.5				0.09 (4)				10.00				A-J				0 / 1849				0.42 (1)			
H-I				0 / 1820				-18.5				-18.5				0.35 (1)				10.00															
I-J				0 / 1448				-18.5				-18.5				0.31 (1)				10.00															
J-K				0 / 1448				-18.5				-18.5				0.31 (1)				10.00															
K-L				0 / 0				-18.5				-18.5				0.17 (4)				10.00															

ALLOWABLE DEFL. (LL) = L/860 (0.73")

CALCULATED VERT. DEFL. (LL) = L/999 (0.06")

ALLOWABLE DEFL. (TL) = L/860 (0.73")

CALCULATED VERT. DEFL. (TL) = L/999 (0.12")

CS1: TC=0.71/1.00 (D-E:1), 2C=0.35/1.00 (J-K:1), WB=0.42/1.00 (A-J:1), SS1=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

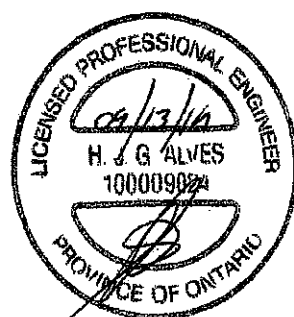
MT20 618 354 1687 786 1687 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (G) (INPUT = 0.90)

JSI METAL = 0.50 (A) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRAWG NO.
405481	H9	2	1	TRUSS DESC.	

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0-0 5-7-11 6-7-11 6-5-3 13-0-14 4-5-10 17-6-8 4-5-10 22-0-2

Scale = 1:38.3

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
K - A	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY
EXCEPT

DRY; SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-1	MT20	5.0	8.0		Edges
B	TMWV-1	MT20	4.0	4.0		
C	TTWV-m	MT2C	5.0	8.0	2.50	1.75
D	TMW-v	MT20	2.0	4.0		
E	TMWV+p	MT20	4.0	8.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWVW-1	MT20	5.0	8.0		
H	BS-1	MT20	3.0	8.0		
I	BMWV-1	MT20	4.0	4.0		
J	BMWV-1	MT26	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1326	0	1326	0	0	1-10	1-10
K	1326	0	1326	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-B.

UNFACTORED REACTIONS							
1ST CASE		MAX MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	933	638/0	0/0	0/0	0/0	295/0	0/0
K	933	638/0	0/0	0/0	0/0	295/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX. UNBRAC	MAX. FACTORED		MAX. CSI (LC)		
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)		MEMB.	FORCE (LBS)			
FR-TO		FROM-TO			FR-TO				
A-B	-1961 / 0	-102.0	-102.0	0.85 (1)	4.03	J-B	-114 / 80	0.03 (1)	
B-C	-1232 / 0	-102.0	-102.0	0.58 (1)	4.98	B-I	-837 / 0	0.94 (1)	
C-D	-786 / 0	-102.0	-102.0	0.29 (1)	6.25	I-C	0 / 512	0.12 (1)	
D-E	-780 / 0	-102.0	-102.0	0.26 (1)	6.25	C-G	-671 / 0	0.68 (1)	
E-F	-1290 / 0	0.0	0.0	0.28 (1)	5.88	G-D	-563 / 0	0.41 (1)	
K-A	-1275 / 0	0.0	0.0	0.13 (1)	7.13	G-E	0 / 1385	0.31 (1)	
						A-J	0 / 1855	0.42 (1)	
K-J	0 / 0	-18.5	-18.5	0.19 (4)	10.00				
J-I	0 / 1838	-18.5	-18.5	0.38 (1)	10.00				
I-H	0 / 1116	-18.5	-18.5	0.24 (1)	10.00				
H-G	0 / 1116	-18.5	-18.5	0.24 (1)	10.00				
G-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BC8C 2018, OBC 2
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

155 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN
LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF
LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.73")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.07")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.73")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.65/1.00 (A-B:1), BC=0.38/1.00 (I-J:1),
WB=0.94/1.00 (B-I:1), SSI=0.30/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

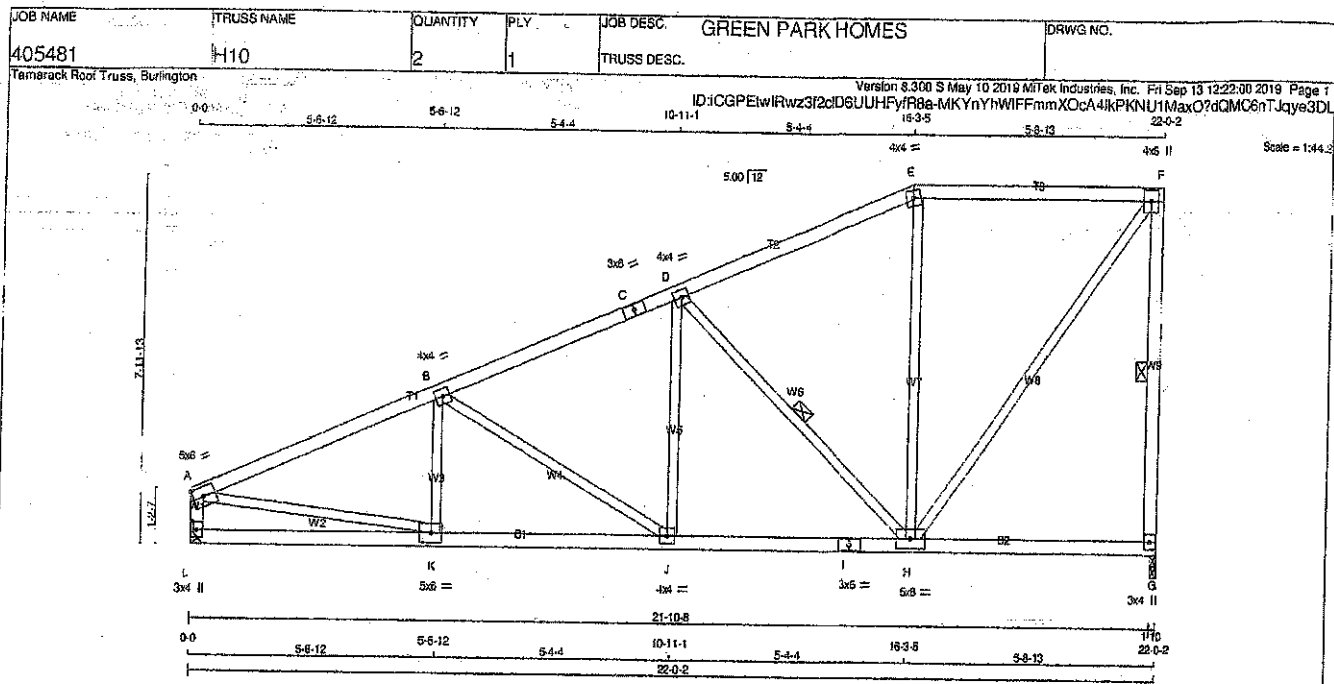
NAIL VALUES	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
WT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.83 (G) (INPUT = 0.80)
SI METAL=0.51 (A) (INPUT = 1.00)

Structural component only
DWG# T-1923497



LUMBER	N.L.G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
L - A	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0		Edge
B	TMVW-1	MT20	4.0	4.0		
C	TS-1	MT20	3.0	6.0		
D	TMVW-1	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-p	MT20	4.0	6.0		
G	BMV1-p	MT20	3.0	4.0		
H	BMVWW-1	MT20	5.0	8.0		
I	BS-1	MT20	3.0	6.0		
J	BMVW-1	MT20	4.0	4.0		
K	BMVWW-1	MT20	5.0	6.0		
L	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
G	1328	0	1-10	1-10
L	1328	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3.8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0	0 / 0
L	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		PROM TO	LENGTH FR-TO	FR-TO			
A-B	-1945 / 0	-102.0	-102.0 0.42 (1)	4.40	K-B	-185 / 36	0.04 (1)
B-C	-1515 / 0	-102.0	-102.0 0.34 (1)	4.97	B-J	-485 / 0	0.34 (1)
C-D	-1515 / 0	-102.0	-102.0 0.34 (1)	4.97	J-D	0 / 346	0.08 (1)
D-E	-811 / 0	-102.0	-102.0 0.34 (1)	6.25	D-H	-945 / 0	0.38 (1)
E-F	-726 / 0	-102.0	-102.0 0.44 (1)	6.25	H-E	-208 / 13	0.25 (1)
G-F	-1284 / 0	0.0	0.0 0.37 (1)	5.69	H-F	0 / 1230	0.28 (1)
L-A	-1281 / 0	0.0	0.0 0.13 (1)	7.13	A-K	0 / 1837	0.41 (1)
L-K	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
K-J	0 / 1814	-18.5	-18.5 0.34 (1)	10.00			
J-I	0 / 1396	-18.5	-18.5 0.30 (1)	10.00			
I-H	0 / 1396	-18.5	-18.5 0.30 (1)	10.00			
H-G	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

TOTAL WEIGHT = 2 X 96 = 192 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPIC 2014

(65% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.73")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.73")
CALCULATED VERT. DEFL. (TL) = L/999 (0.13")

CSI: TC=0.44/1.00 (E-F:1), BC=0.34/1.00 (J-K:1), WB=0.41/1.00 (A-K:1), SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

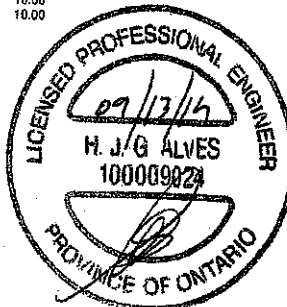
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (K) (INPUT = 0.80)
JSI METAL = 0.50 (A) (INPUT = 1.00)



RECEIVED
SEP 16 2019

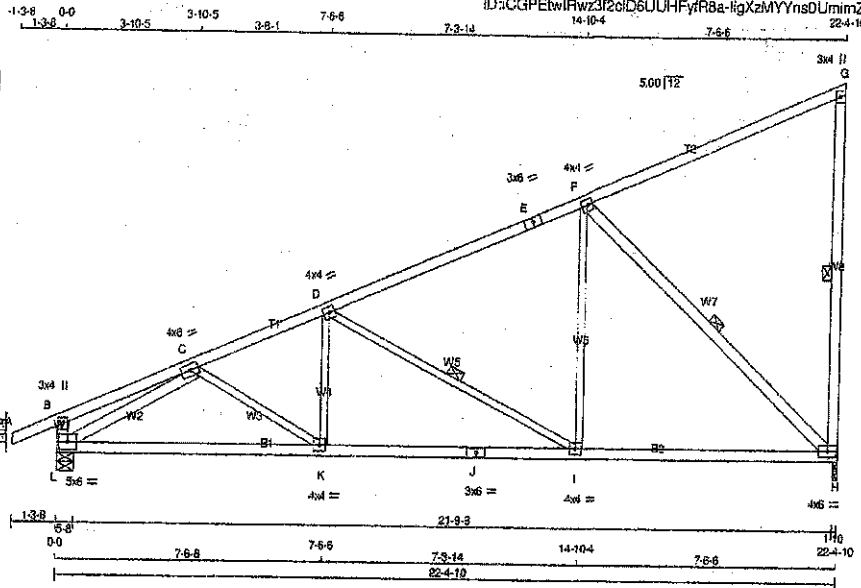
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

Structural component only
DWG# T-1923498

JOB NAME 405481	TRUSS NAME H12	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1/32" = 1'-0"

TOTAL WEIGHT = 6 X 101 = 606 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT	F - H	2x4	DRY
	L - C	2x4	DRY

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMV+p	MT20	3.0	4.0	X
C	TMVW-1	MT20	4.0	6.0	
D	TMVW-1	MT20	4.0	4.0	
E	TS-1	MT20	3.0	6.0	
F	TMVW-1	MT20	4.0	4.0	
G	TMV+p	MT20	3.0	4.0	
H	BMVW-1	MT20	4.0	6.0	
I	BMVW-1	MT20	4.0	4.0	
J	BS-1	MT20	3.0	6.0	
K	BMVW-1	MT20	4.0	4.0	
L	BMVW-1	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	UPLIFT	IN-SX	IN-SX	IN-SX
H	1349	0	1349	0	0	1-10	1-10	1-10	1-10
L	1488	0	1488	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW
H	949	649	0	0	0	0	0	0	0	0	0	0	0	0	0
L	1044	728	0	0	0	0	0	0	0	0	0	0	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)		CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	CS (LC)	MAX. FACTORED	CS (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED	CS (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED	CS (LC)
FR-TO						FR-TO				FR-TO			
A-E	0.25	-102.0	-102.0	0.13 (1)	10.00	K-D	0.144	0.05 (4)		K-D	0.144	0.05 (4)	
B-C	0.75	-102.0	-102.0	0.20 (1)	10.00	D-I	-850	0.40 (1)		D-I	-850	0.40 (1)	
C-D	-1982	-102.0	-102.0	0.34 (1)	4.48	F-H	0.531	0.12 (1)		F-H	0.531	0.12 (1)	
D-E	-1149	-102.0	-102.0	0.71 (1)	4.81	F-H	-1405	0.78 (1)		F-H	-1405	0.78 (1)	
E-F	-1149	-102.0	-102.0	0.71 (1)	4.81	C-K	0.754	0.02 (4)		C-K	0.754	0.02 (4)	
F-G	-32	-102.0	-102.0	0.71 (1)	6.25	L-C	-2144	0.51 (1)		L-C	-2144	0.51 (1)	
H-G	-300	0.0	0.0	0.18 (1)	5.25								
L-B	-309	0.0	0.0	0.03 (1)	7.81								
L-K	0/1826	-18.5	-18.5	0.42 (1)	10.00								
K-J	0/1829	-18.5	-18.5	0.43 (1)	10.00								
J-I	0/1829	-18.5	-18.5	0.43 (1)	10.00								
I-H	0/1078	-18.5	-18.5	0.32 (4)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.75")
CALCULATED VERT. DEFL. (LL) = 1/989 (0.07")
ALLOWABLE DEFL. (TL) = 1/360 (0.75")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.15")

CSI: TC=0.71/1.00 (F-G:1), BC=0.43/1.00 (I-K:1), WB=0.78/1.00 (F-H:1), SSI=0.34/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

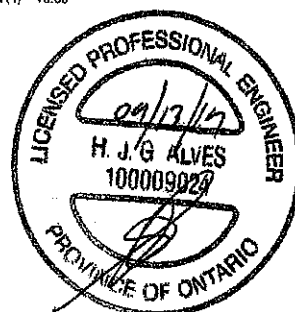
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

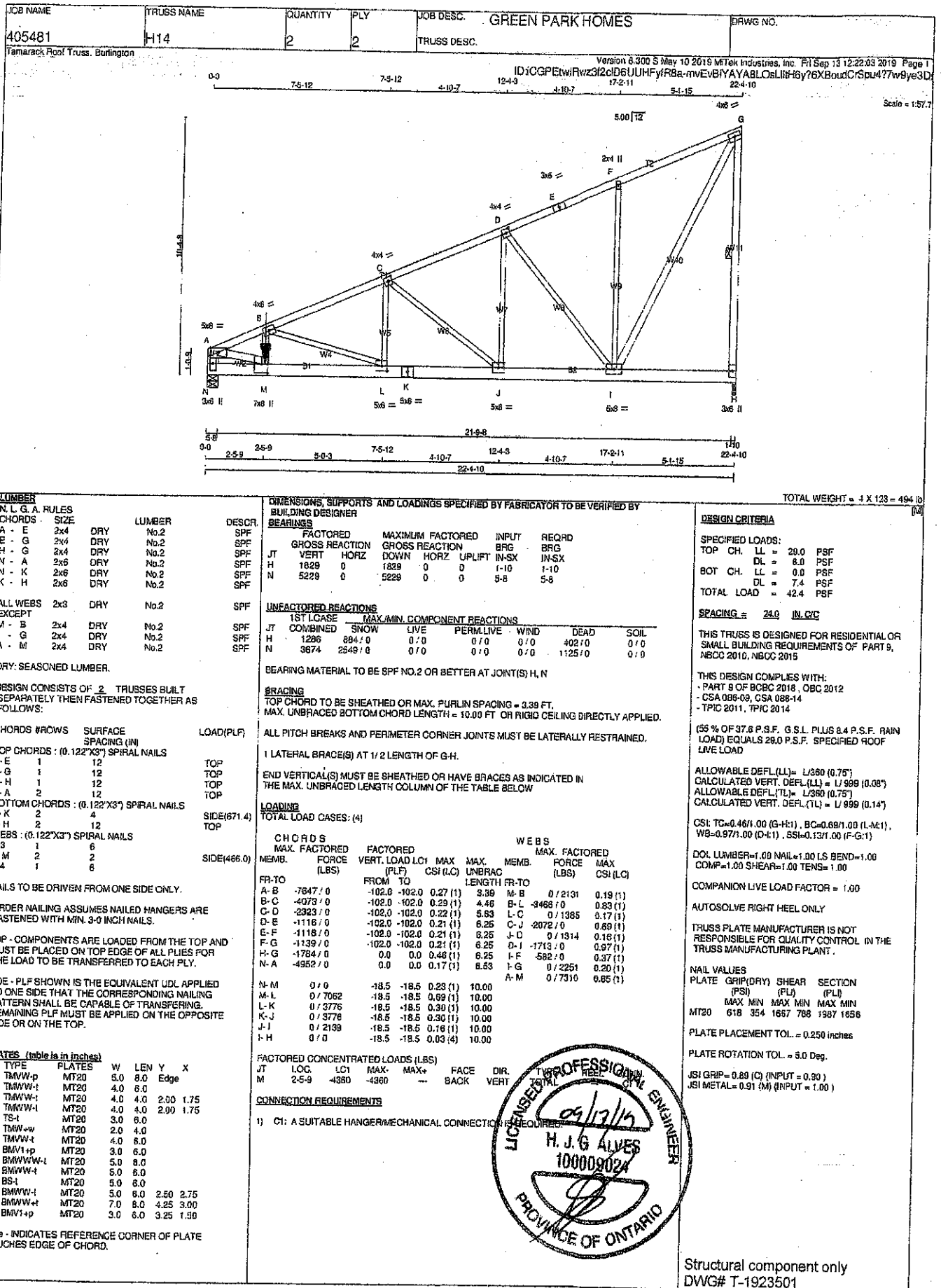
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.58 (J) (INPUT = 1.00)



Structural component only
DWG# T-1923500

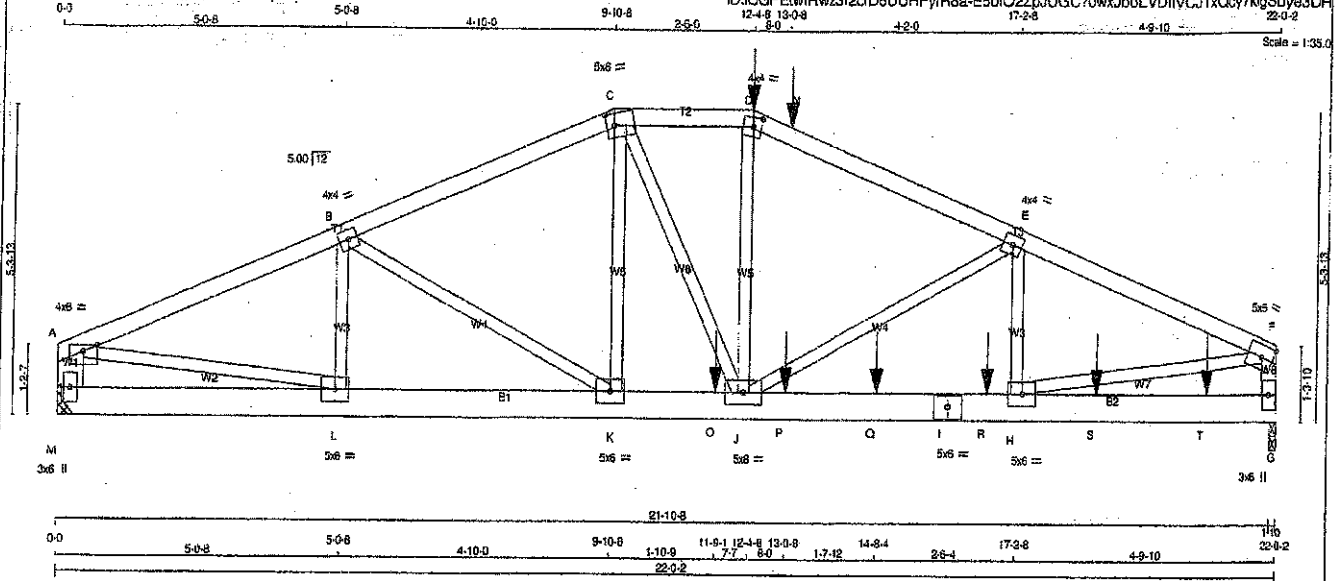


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H15	2	2	TRUSS DESC.		

Framed Roof Truss, Burlington

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ID:ICGPEtw/Rwz312cdD8UHFyR8a-E5oiO2ZpJUGC70wxJboLVDIVCITxOcy7KqSbye3DH
12-4-8 13-0-8 17-2-8 22-0-2
Scale = 1:35.0



CHORDS	SIZE	DRY	NO.2	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	SIDE(81.0)
D-F 1	12	SIDE(81.0)
G-F 1	12	TOP
M-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
M-I 2	12	SIDE(0.0)
I-G 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
J-D 1	8	SIDE(98.5)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-9 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (tablets in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW-p	MT20	4.0	8.0	Edge	
B TMVW-l	MT20	4.0	4.0		
C TTWW-m	MT20	5.0	6.0	2.50	1.75
D TTW-m	MT20	4.0	4.0	2.00	1.75
E TMVW-l	MT20	4.0	4.0		
F TMVW-l	MT20	5.0	6.0	Edge	
G BMV1-p	MT20	3.0	6.0		
H, K, L					
H BMVW-l	MT20	5.0	3.0		
I BS-l	MT20	5.0	5.0		
J BMVW-l	MT20	5.0	3.0		
M BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
FACTORED				
GROSS REACTION				
VERT				
HORIZ				
UP/LIFT				
IN-SX				
MECHANICAL				
1-10				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M MINIMUM BEARING LENGTH AT JOINT M = 3-8.

UNFACTORED REACTIONS

1ST L.CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED		
M	1582	1115 / 0
G	1777	1224 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	-3688 / 0	-102.0	-102.0	0.29 (1)	L-B	-534 / 0	0.05 (1)	
B-C	-3543 / 0	-102.0	-102.0	0.28 (1)	I-K	-96 / 0	0.03 (1)	
C-D	-3584 / 0	-102.0	-102.0	0.10 (1)	K-C	0 / 389	0.05 (1)	
D-N	-3989 / 0	-102.0	-102.0	0.30 (1)	C-J	0 / 787	0.09 (1)	
N-E	-3989 / 0	-102.0	-102.0	0.30 (1)	J-D	0 / 262	0.04 (1)	
E-F	-3884 / 0	-102.0	-102.0	0.30 (1)	H-E	-80 / 0	0.03 (1)	
G-F	-2433 / 0	0.0	0.0	0.19 (1)	H-E	-610 / 0	0.08 (1)	
M-A	-2184 / 0	0.0	0.0	0.08 (1)	H-F	0 / 3785	0.47 (1)	
M-L	0 / 0	-18.5	-18.5	0.05 (1)	A-L	0 / 3468	0.43 (1)	
L-K	0 / 3423	-18.5	-18.5	0.27 (1)				
K-O	0 / 3349	-18.5	-18.5	0.33 (1)				
O-J	0 / 3349	-18.5	-18.5	0.33 (1)				
J-P	0 / 3599	-18.5	-18.5	0.29 (1)				
P-Q	0 / 3599	-18.5	-18.5	0.29 (1)				
Q-I	0 / 3599	-18.5	-18.5	0.29 (1)				
I-H	0 / 3599	-18.5	-18.5	0.29 (1)				
R-H	0 / 3599	-18.5	-18.5	0.29 (1)				
H-S	0 / 0	-18.5	-18.5	0.05 (1)				
S-T	0 / 0	-18.5	-18.5	0.06 (1)				
T-G	0 / 0	-18.5	-18.5	0.06 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR.	TYPE	MIN.
D	12-4-8	-125	-140	FRONT	VERT	DEAD	
D	12-4-8	-713	-713	FRONT	VERT	SNOW	
N	13-0-8	-140	-140	FRONT	VERT	TOTAL	
O	11-8-1	-1018	-1018	FRONT	VERT	TOTAL	
P	13-0-8	-27	-27	FRONT	VERT	TOTAL	
Q	14-8-4	-27	-27	FRONT	VERT	TOTAL	
R	16-8-4	-27	-27	FRONT	VERT	TOTAL	
S	18-8-4	-27	-27	FRONT	VERT	TOTAL	
T	20-8-4	-27	-27	FRONT	VERT	TOTAL	

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	29.0	PSF
	DL	6.0	PSF
BOT CH.	LL	0.0	PSF
	DL	7.4	PSF
TOTAL LOAD		42.4	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.12")

CSI: TC=0.30/1.00 (D-E-1), BC=0.33/1.00 (J-K-1), WB=0.47/1.00 (F-H-1), SSI=0.27/1.00 (J-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

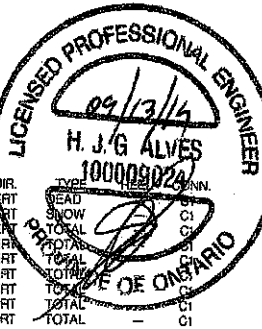
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1957

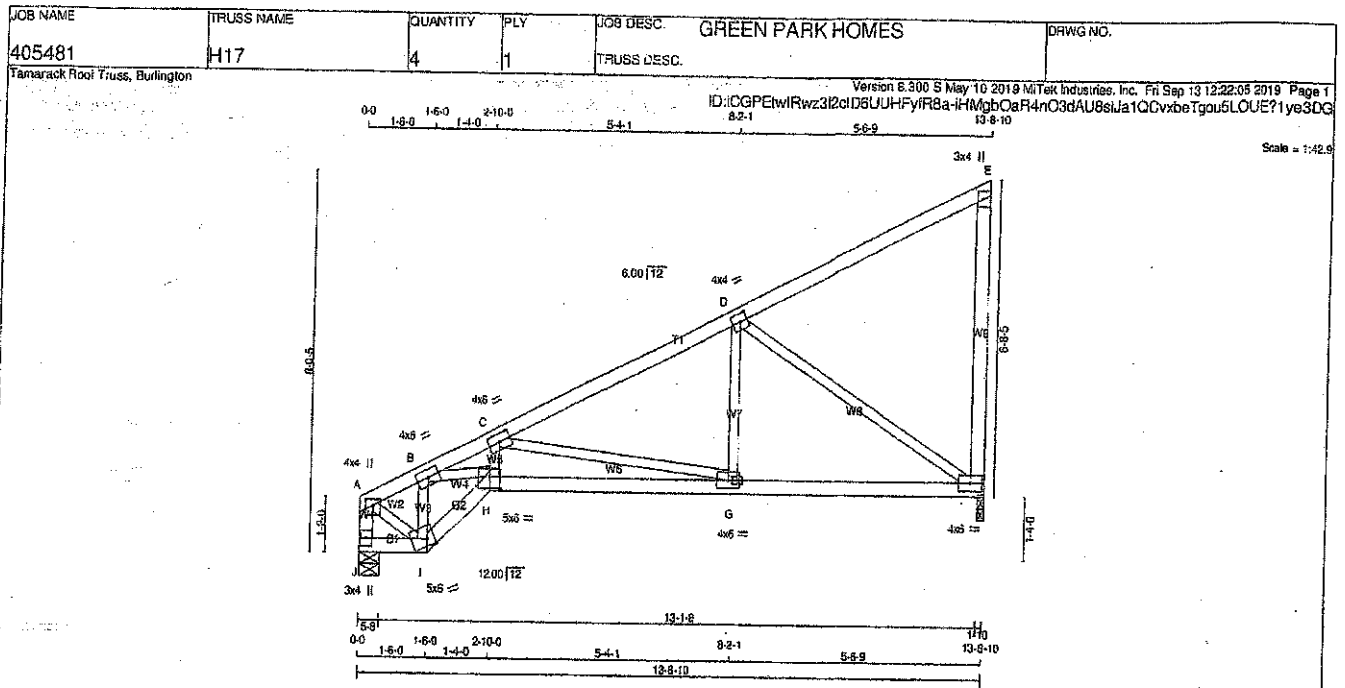
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (A) (INPUT = 0.90)
JSI METAL = 0.50 (F) (INPUT = 1.00)

Structural component only
DWG# T-1923502





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - E	2x4	DRY No.2
F - E	2x4	DRY No.2
J - A	2x4	DRY No.2
J - I	2x4	DRY No.2
I - H	2x4	DRY No.2
H - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50 2.00
B	TMVW-i	MT20	4.0	6.0	
C	TMVW-i	MT20	4.0	6.0	
D	TMVW-i	MT20	4.0	4.0	2.00 1.75
E	TMV+p	MT20	3.0	4.0	
F	BMVW-i	MT20	4.0	6.0	
G	BMVW-i	MT20	4.0	6.0	
H	SBVW+p	MT20	5.0	6.0	3.00 2.75
I	SBVW-m	MT20	5.0	6.0	1.75 2.00
J	BMV-i+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	DOWN	UP/LIFT	IN-SX
F 827	0	0	1-10
J 827	0	0	5-6

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED							
F 582	398 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0	0 / 0
J 582	398 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC1)	MAX. FACTORED (LC1)	MEMB.	FORCE (LBS)	MAX. FACTORED (LC1)
FR-TO					FR-TO		
A-B	-775 / 0	-102.0	-102.0 0.05 (1)	5.25	I-B	-975 / 0	0.14 (1)
B-C	-2318 / 0	-102.0	-102.0 0.12 (1)	4.41	B-H	0 / 1480	0.34 (1)
C-D	-949 / 0	-102.0	-102.0 0.38 (1)	5.88	H-C	0 / 488	0.11 (1)
D-E	-28 / 0	-102.0	-102.0 0.37 (1)	8.25	C-G	-1198 / 0	0.58 (1)
E-F	-220 / 0	0.0	0.0 0.20 (1)	7.81	G-D	0 / 334	0.08 (1)
F-A	-813 / 0	0.0	0.0 0.08 (1)	7.81	D-F	-1036 / 0	0.78 (1)
					A-I	0 / 784	0.15 (1)
J-I	0 / 0	-18.5	-18.5 0.01 (4)	10.00			
I-H	0 / 877	-18.5	-18.5 0.15 (1)	10.00			
H-G	0 / 2045	-18.5	-18.5 0.41 (1)	10.00			
G-F	0 / 870	-18.5	-18.5 0.22 (4)	10.00			

TOTAL WEIGHT = 4 X 58 = 230 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 089-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46')
CALCULATED VERT. DEFL.(LL) = L/999 (0.08')
ALLOWABLE DEFL.(TL) = L/360 (0.45')
CALCULATED VERT. DEFL.(TL) = L/999 (0.11')

CSI: TC=0.38/1.00 (C-D:1), BC=0.41/1.00 (G-H:1),
WB=0.78/1.00 (D-F:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

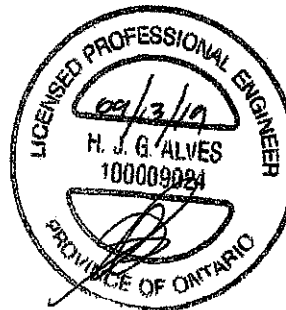
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (B) (INPUT = 0.90)
JSI METAL = 0.40 (B) (INPUT = 1.00)

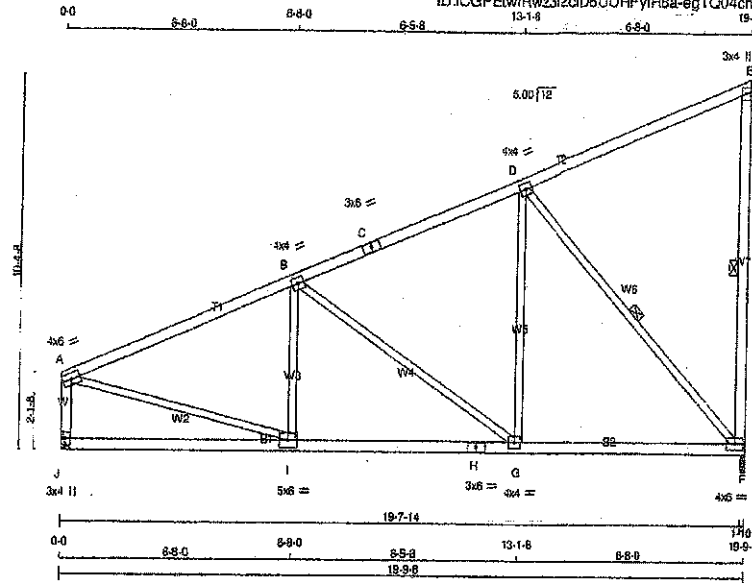


Structural component only
DWG# T-1923503

JOB NAME 405481	TRUSS NAME H19	QUANTITY 12	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.					

Truss/Roof Truss, Burlington

Version 8.300 S May 10 2019 MITek Industries, Inc. Fri Sep 13 12:22:07 2019 Page 1
ID:1CGPEw/Rwz312cd6UHFyIR8a-egTQ04chcPamsTeW_jM26HCXPLb8h6OpizL3wye3DE



TOTAL WEIGHT = 12 X 89 = 1068 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2		
C - E	2x4 DRY	No.2		
F - E	2x4 DRY	No.2		
J - A	2x4 DRY	No.2		
J - H	2x4 DRY	No.2		
H - F	2x4 DRY	No.2		
ALL WEBS	2x3 DRY	No.2		
EXCEPT				

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	6.0		
B	TMVW-1	MT20	4.0	4.0		
C	TS-1	MT20	3.0	6.0		
D	TMVW-1	MT20	4.0	4.0		
E	TMVW-1	MT20	3.0	4.0		
F	BMVW-1	MT20	4.0	6.0		
G	BMVW-1	MT20	4.0	4.0		
H	BS-1	MT20	3.0	6.0		
I	BMVW-1	MT20	5.0	6.0		
J	BMVW-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	FACTORED	MAXIMUM FACTORED	INPUT	REQD
SPF	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	DOWN	UP
F	1193	0	1193	0
J	1193	0	1193	0
SPF				
SPF				

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3'-8".

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	839	574 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	839	574 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS	MAX. FACTORED	FORCE	MAX
		(LBS)	(PLF)			(LBS)	CS1 (LC)			(LBS)		CS1 (LC)
FR-TO			FROM TO				LENGTH	FR-TO				
A-B	-1333 / 0	-102.0	-102.0	0.57 (1)	4.85	J-B	-213 / 42	0.08 (1)				
B-C	-859 / 0	-102.0	-102.0	0.48 (1)	5.90	B-G	-584 / 0	0.74 (1)				
C-D	-859 / 0	-102.0	-102.0	0.48 (1)	5.90	G-D	0 / 461	0.10 (1)				
D-E	-25 / 0	-102.0	-102.0	0.52 (1)	6.25	D-F	-1176 / 0	0.80 (1)				
F-E	-274 / 0	0.0	0.0	0.15 (1)	6.25	A-I	0 / 1288	0.29 (1)				
J-A	-1141 / 0	0.0	0.0	0.13 (1)	7.44							
J-I	0 / 0	-18.5	-18.5	0.18 (4)	10.00							
I-H	0 / 1253	-18.5	-18.5	0.30 (1)	10.00							
H-G	0 / 1253	-18.5	-18.5	0.30 (1)	10.00							
G-F	0 / 794	-18.5	-18.5	0.25 (4)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA C88-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/899 (0.66")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.04")
ALLOWABLE DEFL. (TL) = 1/380 (0.66")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.06")

CS1: TC=0.57/1.00 (A-B:1), BC=0.30/1.00 (G-H:1), WB=0.80/1.00 (D-F:1), SS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

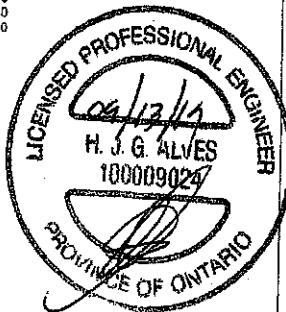
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.74 (D) (INPUT = 0.90)
JSI METAL = 0.39 (H) (INPUT = 1.00)



Structural component only
DWG# T-1923505

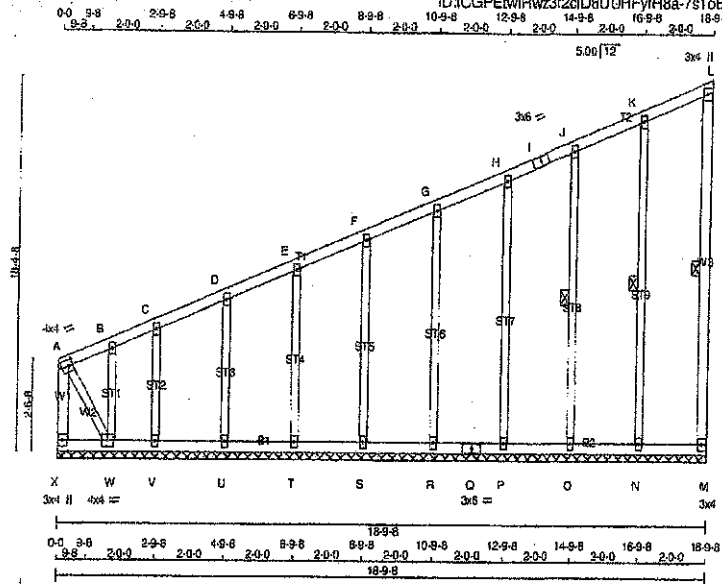
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H19S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:56.4



TOTAL WEIGHT = 2 X 102 = 204 lb

LUMBER	CHORDS	SIZE	LUMBER
N. L. G. A. RULES			
A - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
M - L	2x4	DRY	No.2
X - A	2x4	DRY	No.2
X - Q	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

GABLE STUDS SPACED AT 24-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	4.0	4.0	2.00 1.50
B	C, D, E, F, G, H, J, K				
L	TMVW-p	MT20	3.0	4.0	
M	BMV1-p	MT20	3.0	4.0	
N, O, P, R, S, T, U, V					
N	BMV1-w	MT20	2.0	4.0	
Q	BS-I	MT20	3.0	6.0	
W	BMVW1-I	MT20	4.0	4.0	
X	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, K-N, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	
FR-TO				FR-TO			
A-B	-15.0	-102.0	-102.0 0.02 (1)	6.25	N-K	-227.0	0.14 (1)
B-C	-15.0	-102.0	-102.0 0.03 (1)	6.25	O-J	-201.0	0.10 (1)
C-D	-12.0	-102.0	-102.0 0.05 (1)	6.25	P-H	-203.0	0.24 (1)
D-E	-11.0	-102.0	-102.0 0.05 (1)	6.25	R-G	-203.0	0.17 (1)
E-F	-8.0	-102.0	-102.0 0.05 (1)	10.00	S-F	-203.0	0.13 (1)
F-G	-6.0	-102.0	-102.0 0.05 (1)	10.00	T-E	-202.0	0.09 (1)
G-H	-5.0	-102.0	-102.0 0.05 (1)	10.00	U-D	-208.0	0.06 (1)
H-I	-3.0	-102.0	-102.0 0.05 (1)	10.00	V-C	-189.0	0.04 (1)
I-J	-3.0	-102.0	-102.0 0.05 (1)	10.00	W-B	-144.0	0.03 (1)
J-K	0.0	-102.0	-102.0 0.05 (1)	10.00	A-W	0.34	0.01 (1)
K-L	-7.0	-102.0	-102.0 0.05 (1)	6.25			
M-L	-86.0	0.0	0.0 0.05 (1)	6.25			
X-A	-97.0	0.0	0.0 0.01 (1)	7.81			
X-W	0.0	-18.5	-18.5 0.01 (4)	10.00			
W-V	0.18	-18.5	-18.5 0.01 (4)	10.00			
V-U	0.13	-18.5	-18.5 0.02 (4)	10.00			
U-T	0.10	-18.5	-18.5 0.02 (4)	10.00			
T-S	0.8	-18.5	-18.5 0.01 (4)	10.00			
S-R	0.6	-18.5	-18.5 0.01 (4)	10.00			
R-Q	0.4	-18.5	-18.5 0.01 (4)	10.00			
Q-P	0.4	-18.5	-18.5 0.01 (4)	10.00			
P-O	0.3	-18.5	-18.5 0.01 (4)	10.00			
O-N	0.1	-18.5	-18.5 0.02 (4)	10.00			
N-M	0.0	-18.5	-18.5 0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010, CBC 2012

- CSA 086-08, CSA 086-14

- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (K-L:1), BC=0.02/1.00 (N-O:4), WB=0.24/1.00 (H-P:1), SS=0.09/1.00 (K-L:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

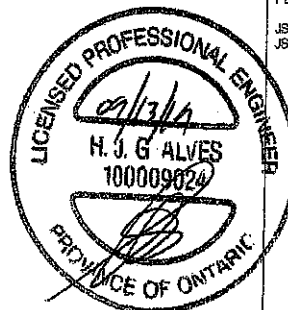
MT20 618 354 1667 798 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.53 (A) (INPUT = 0.80)

JSI METAL=0.08 (K) (INPUT = 1.00)



Structural component only
DWG# T-1923506

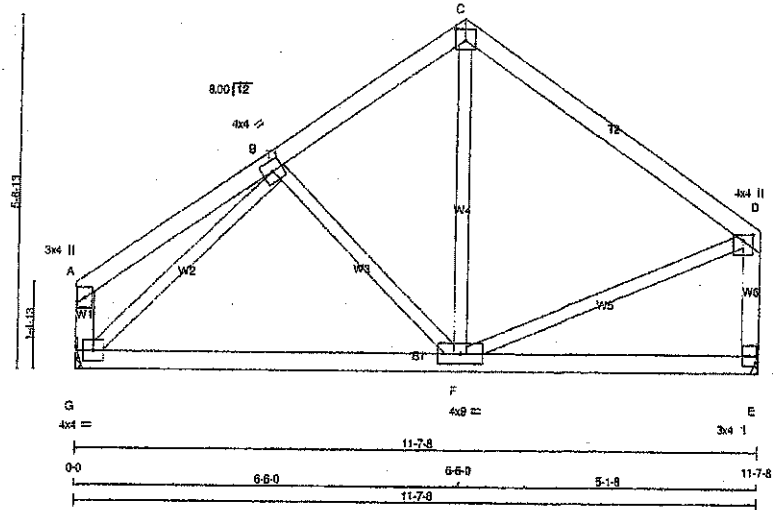
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H20	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington.

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0-0 3-4-4 3-4-4 3-1-12 6-0-0 5-1-8 11-7-8

Scale = 1:33.1



TOTAL WEIGHT = 8 X 48 = 288 lb

LUMBER

N.L.G.A. RULES	SIZE	LUMBER
A - C	2x4 DRY	No.2
C - D	2x4 DRY	No.2
G - A	2x4 DRY	No.2
E - D	2x4 DRY	No.2
G - E	2x4 DRY	No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY; SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
E 701 0	701 0 0	MECHANICAL	
G 701 0	701 0 0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, G. MINIMUM BEARING LENGTH AT JOINT E = 1-8, JOINT G = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL	
E 493 337/0 0/0 0/0 0/0 156/0 0/0	
G 493 337/0 0/0 0/0 0/0 156/0 0/0	

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						
A-B	0/21	-102.0	-102.0	0.17 (1)	10.00	B-F -197/0 0.07 (1)
B-C	-483/0	-102.0	-102.0	0.13 (1)	6.25	F-C 0/123 0.04 (4)
C-D	-462/0	-102.0	-102.0	0.35 (1)	6.25	G-B -742/0 0.25 (1)
G-A	-127/0	0.0	0.0	0.01 (1)	7.81	F-D 0/419 0.03 (1)
E-D	-688/0	0.0	0.0	0.08 (1)	7.81	
G-F	0/525	-18.5	-18.5	0.22 (4)	10.00	
F-E	0/0	-18.5	-18.5	0.19 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.39")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.39")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TO=0.35/1.00 (C-D:1), BC=0.22/1.00 (F-G:4), WB=0.25/1.00 (B-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (B) (INPUT = 0.90)
JSI METAL = 0.23 (B) (INPUT = 1.00)

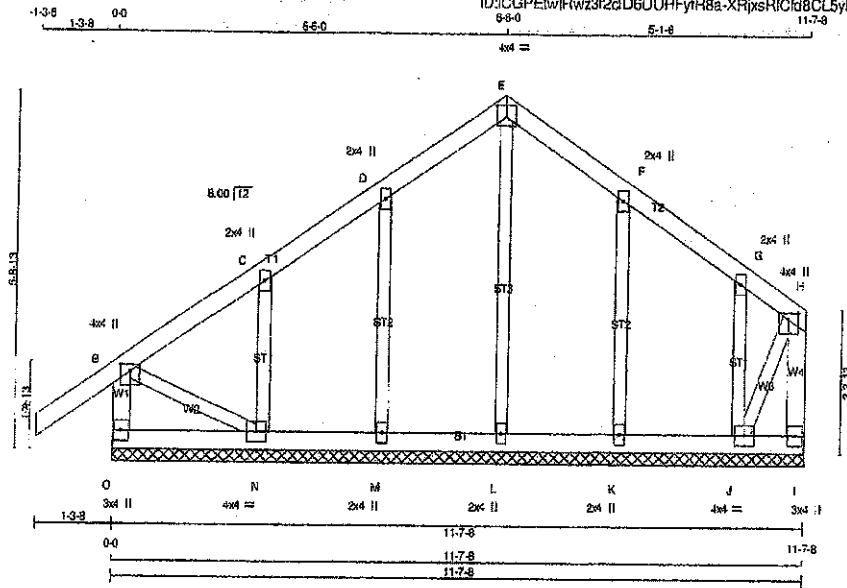


Structural component only
DWG# T-1923507

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H20G	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:32.7

TOTAL WEIGHT = 2 X 53 = 105 lb

LUMBER

N.L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

GABLE STUDS SPACED AT 2-0-0 OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8-25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10-00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G					
C TMVW+w	MT20	2.0	4.0		
E TMVW+p	MT20	4.0	4.0	2.25	2.00
H TMVW+p	MT20	4.0	4.0	1.25	2.00
I BMV1+p	MT20	3.0	4.0		
J BMVW1+1	MT20	4.0	4.0		
K, L, M					
K BMV1+w	MT20	2.0	4.0		
N BMVW1+1	MT20	4.0	4.0		
O BMV1+p	MT20	3.0	4.0		

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)
FR-TO						FR-TO			
O-B	-257 / 0	0.0	0.0	0.03 (1)	7.81	L-E	-156 / 0	0.08 (1)	
A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00	M-D	-216 / 0	0.08 (1)	
B-C	-16 / 0	-102.0	-102.0	0.07 (1)	6.25	H-C	-251 / 0	0.05 (1)	
C-D	-24 / 0	-102.0	-102.0	0.07 (1)	6.25	K-F	-238 / 0	0.07 (1)	
D-E	-25 / 0	-102.0	-102.0	0.05 (1)	6.25	J-G	-183 / 0	0.03 (1)	
E-F	-26 / 0	-102.0	-102.0	0.06 (1)	6.25	B-N	0 / 26	0.01 (1)	
F-G	-14 / 0	-102.0	-102.0	0.06 (1)	6.25	J-H	0 / 44	0.01 (1)	
G-H	-14 / 0	-102.0	-102.0	0.03 (1)	6.25				
H-I	-84 / 0	0.0	0.0	0.01 (1)	7.81				
O-N	0 / 0	-18.5	-18.5	0.03 (4)	10.00				
N-M	0 / 16	-18.5	-18.5	0.03 (4)	10.00				
M-L	0 / 12	-18.5	-18.5	0.02 (4)	10.00				
L-K	0 / 12	-18.5	-18.5	0.02 (4)	10.00				
K-J	0 / 17	-18.5	-18.5	0.02 (4)	10.00				
J-I	0 / 0	-18.5	-18.5	0.01 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSK TC=0.14/1.00 (A-B:1), BD=0.03/1.00 (M-N:4), WB=0.08/1.00 (E-L:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (OR)	Y	SHEAR	SECTION
(PSI)	(PLI)	(PLI)		
MAX MIN	MAX MIN	MAX MIN		
MT20	618	354	1867	788
			1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

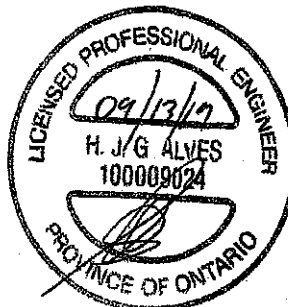
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.80)

JSI METAL= 0.13 (C) (INPUT = 1.00)

RECEIVED
SEP 16 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

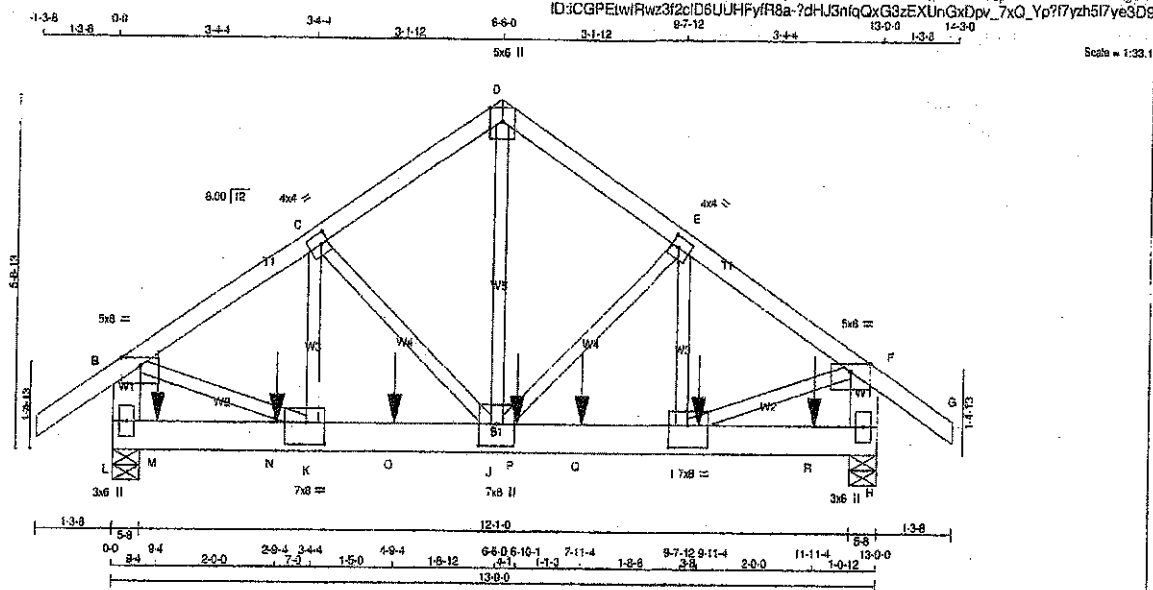


Structural component only
DWG# T-1923508

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H21	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/32

TOTAL WEIGHT = 4 X 69 = 274 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
L - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3") SPIRAL NAILS			
A-D	1	12	TOP
D-G	1	12	TOP
L-B	2	12	TOP
H-F	2	12	TOP
BOTTOM CHORDS : (0.122'X3") SPIRAL NAILS			
L-H	2	12	SIDE(183.1)
WEBS : (0.122'X3") SPIRAL NAILS			
E-I	1	6	SIDE(158.1)
J-D	1	6	SIDE(400.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	L	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-p	MT20	5.0	6.0	Edge	
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1-p	MT20	3.0	6.0		
I	BMVW-t	MT20	7.0	8.0	4.25	3.75
J	BMVW-t	MT20	7.0	8.0	4.25	3.50
K	BMVW-t	MT20	7.0	8.0	4.25	3.75
L	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	4550	0	0	4550	0	0	5-8	5-8	5-8
H	5507	0	0	5507	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS								
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
L	3180	2247.0	0.0	0.0	0.0	948.0	0.0		
H	3694	2709.0	0.0	0.0	0.0	1155.0	0.0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	PLF	CS (LBS)	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS (LBS)
FR-TO						FR-TO			
A-B	0.739	-102.0	-102.0	0.08 (1)	10.00	J-D	0.74498	0.56 (1)	
B-C	-4507.0	-102.0	-102.0	0.18 (1)	4.39	J-E	-1406.0	0.24 (1)	
C-D	-4323.0	-102.0	-102.0	0.14 (1)	4.49	I-E	0.71300	0.18 (1)	
D-E	-4320.0	-102.0	-102.0	0.15 (1)	4.49	C-J	-245.0	0.04 (1)	
E-F	-5483.0	-102.0	-102.0	0.21 (1)	4.00	K-C	-81.27	0.01 (1)	
F-G	0.739	-102.0	-102.0	0.08 (1)	10.00	B-K	0.73921	0.49 (1)	
L-B	-3915.0	0.0	0.0	0.14 (1)	7.16	I-F	0.74766	0.59 (1)	
H-F	-4700.0	0.0	0.0	0.17 (1)	6.67				
L-M	0.0	-18.5	-18.5	0.18 (1)	10.00				
M-N	0.0	-18.5	-18.5	0.18 (1)	10.00				
N-K	0.0	-18.5	-18.5	0.18 (1)	10.00				
K-O	0.73759	-18.5	-18.5	0.48 (1)	10.00				
O-J	0.73759	-18.5	-18.5	0.48 (1)	10.00				
J-P	0.74569	-18.5	-18.5	0.84 (1)	10.00				
P-Q	0.74569	-18.5	-18.5	0.84 (1)	10.00				
Q-I	0.74569	-18.5	-18.5	0.21 (1)	10.00				
I-R	0.0	-18.5	-18.5	0.21 (1)	10.00				
R-H	0.0	-18.5	-18.5	0.31 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TOTAL
I	9-11-4	-1308	-1308	---	BACK	VERT	00009027
M	9-4	-684	-684	---	BACK	VERT	
N	2-9-4	-682	-682	---	BACK	VERT	
O	4-8-4	-682	-682	---	BACK	VERT	
P	8-10-1	-2236	-2236	---	BACK	VERT	
Q	7-11-4	-1308	-1308	---	BACK	VERT	
R	11-11-4	-1308	-1308	---	BACK	VERT	

CONNECTION REQUIREMENTS

1) G1: A SUITABLE HANGER MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN./OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CS: TC=0.21/1.00 (E-F:1), BC=0.64/1.00 (I-J:1), WB=0.59/1.00 (F-I:1), SS=0.93/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

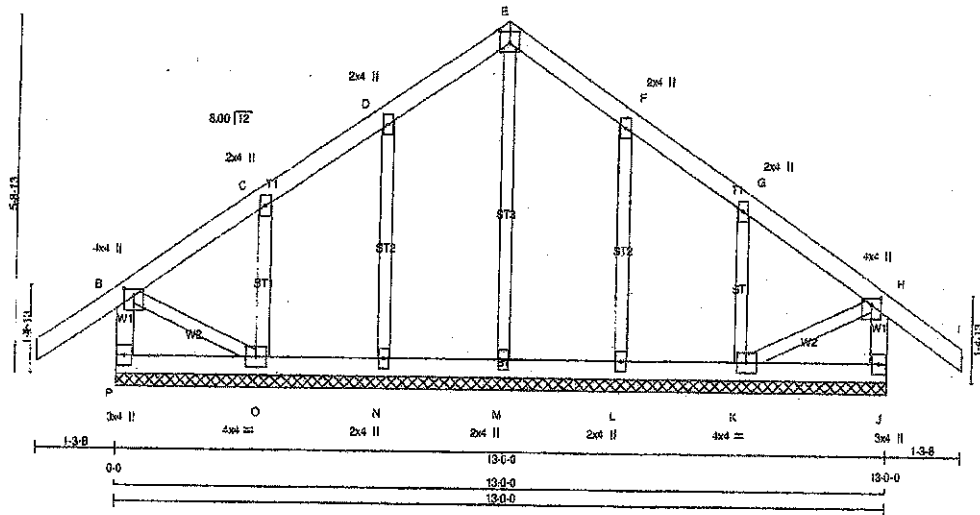
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.85 (D) (INPUT = 0.90)
SI METAL=0.44 (F) (INPUT = 1.00)

Structural component only
DWG# T-1923556

JOB NAME 406481	TRUSS NAME H21G	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.300 S May 10 2019 MTak Industries, Inc. Fri Sep 13 12:22:13 2019 Page 1					
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12-6-8 13-0-0 14-3-8					
Scale = 1/32.7					



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G					
C TMVW+w	MT20	2.0	4.0		
E TMVW+p	MT20	4.0	4.0	2.25	2.00
H TMVW+p	MT20	4.0	4.0	1.25	2.00
J BMV1+p	MT20	3.0	4.0		
K BMVW1+t	MT20	4.0	4.0		
L, M, N					
L BMV1+w	MT20	2.0	4.0		
O BMVW1+t	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. UNBRACED LENGTH FR-TO
P-B	-272 / 0	0.0	0.0	M-E	-153 / 0	0.07 (1)	0.07 (1)
A-B	0 / 39	-102.0	-102.0	N-D	-216 / 0	0.08 (1)	0.08 (1)
B-C	-21 / 0	-102.0	-102.0	O-C	-251 / 0	0.05 (1)	0.05 (1)
C-D	-29 / 0	-102.0	-102.0	L-F	-216 / 0	0.08 (1)	0.08 (1)
D-E	-30 / 0	-102.0	-102.0	K-G	-251 / 0	0.05 (1)	0.05 (1)
E-F	-30 / 0	-102.0	-102.0	B-O	0 / 31	0.01 (1)	0.01 (1)
F-G	-29 / 0	-102.0	-102.0	K-H	0 / 31	0.01 (1)	0.01 (1)
G-H	-21 / 0	-102.0	-102.0				
H-I	0 / 39	-102.0	-102.0				
J-H	-272 / 0	0.0	0.0				
P-O	0 / 0	-18.5	-18.5				
O-N	0 / 21	-18.5	-18.5				
N-M	0 / 16	-18.5	-18.5				
M-L	0 / 16	-18.5	-18.5				
L-K	0 / 21	-18.5	-18.5				
K-J	0 / 0	-18.5	-18.5				

TOTAL WEIGHT = 2 X 57 = 114 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/0.00 (A-B:1), BC=0.09/1.00 (N-O:4), WS=0.07/1.00 (E-M:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

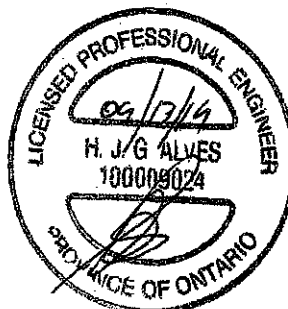
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN MAX MIN MAX MIN		
MT20 518 354 1687 788 1987 1656		

PLATE PLACEMENT TOL. = 0.250 inches

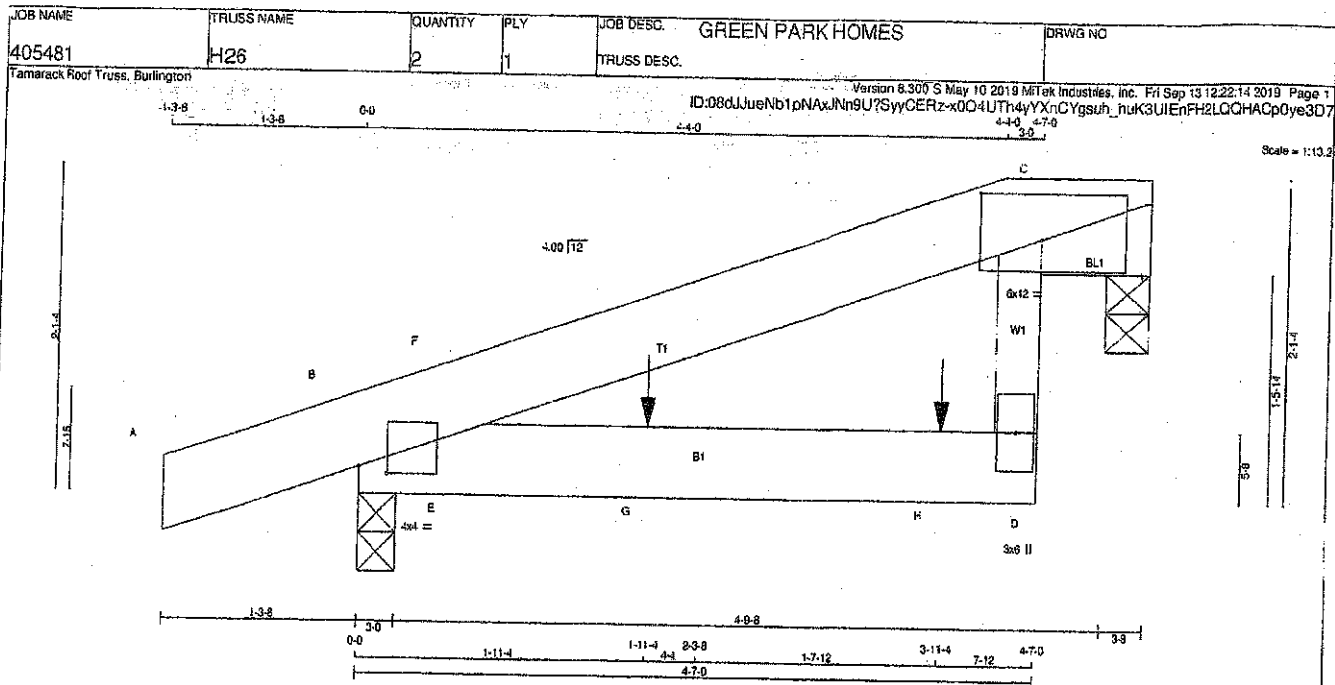
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (H) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE



Structural component only
DWG# T-1923557



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x6	DRY	No.2

BEARING BLOCKS

BL1	SIZE	LUMBER	DESCR.
BL1	2x6	DRY	No.2

DRY, SEASONED LUMBER.

<u>PLATES (table is in inches)</u>						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	0.50	2.25
C	TMVK1-p	MT20	6.0	12.0	3.50	5.00
D	BMV-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX	REQD BRG
C	296	0	296	0	0	3-8	3-8
B	425	0	425	0	0	3-0	3-0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS
C	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
C	211	132/0 0/0 0/0 0/0 78/0 0/0
B	296	211/0 0/0 0/0 0/0 87/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S): C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LBS)	MAX. FACTORED UNBRACED LENGTH (LBS)	MAX. FACTORED CS (LBS)
A-B	0/13	-102.0	-102.0	0.07 (1)	10.00
B-F	-317.0	-102.0	-102.0	0.04 (4)	6.25
F-C	0/3	-102.0	-102.0	0.15 (1)	10.00
D-C	0/92	0.0	0.0	0.02 (4)	10.00
B-E	0/0	-18.5	-18.5	0.11 (1)	10.00
E-G	0/0	-18.5	-18.5	0.11 (1)	10.00
G-H	0/0	-18.5	-18.5	0.11 (1)	10.00
H-D	0/0	-18.5	-18.5	0.11 (1)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-13	-14		FRONT	VERT	TOTAL		CI
H	3-11-4	-17	-17		FRONT	VERT	TOTAL		CI

CONNECTION REQUIREMENTS

1) CT: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

TOTAL WEIGHT = 2 X 24 = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 5.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19')
CALCULATED VERT. DEFL. (LL) = $L/989$ (0.01')
ALLOWABLE DEFL. (TL) = $L/360$ (0.19')
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02')

CSI: TC=0.15/1.00 (C-F:1), BC=0.11/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SS=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

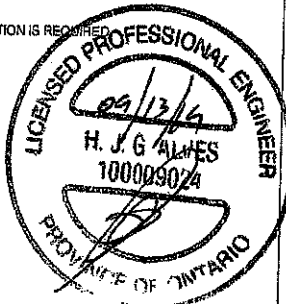
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1367 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)

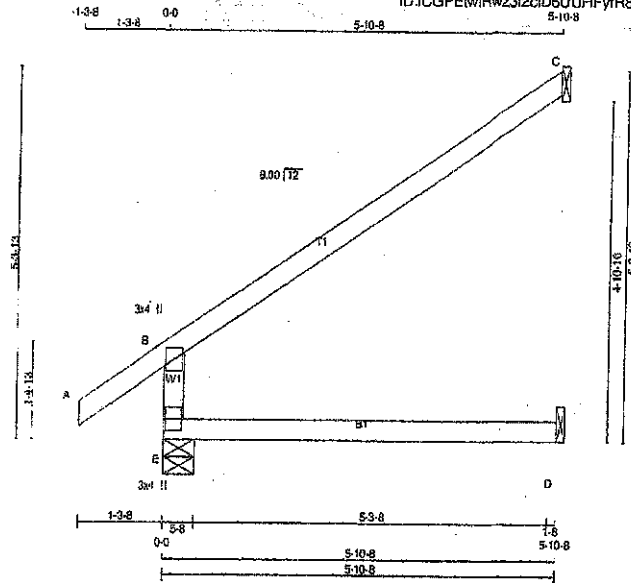


Structural component only
DWG# T-1923558

JOB NAME 405481	TRUSS NAME J1	QUANTITY 22	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.300 5 May 10 2019 Mitek Industries, Inc. Fri Sep 13 12:22:15 2019 Page 1
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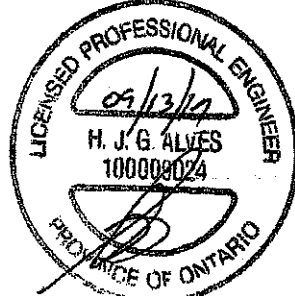


Scale = 1:20.2

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER E - B 2x4 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY: SEASONED LUMBER.		DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 29.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 42.4 PSF SPACING = 24.0 IN/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, OBC 2012 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/999 (0.00") ALLOWABLE DEFL.(TL) = L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/999 (0.00") CSI: TC=0.80/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (W:A:0), SS=0.24/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1887 1856 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP = 0.22 (B) (INPUT = 0.99) JSI METAL = 0.17 (B) (INPUT = 1.00)																																																																																																																	
PLATES (table in inches) JT TYPE PLATES W LEN Y X B TMV+p MT20 3.0 4.0 E DMV1+p MT20 3.0 4.0		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table border="1"> <thead> <tr> <th>JT</th> <th>FACTORED GROSS REACTION VERT</th> <th>FACTORED GROSS REACTION HORZ</th> <th>INPUT BRG IN-SX</th> <th>RECORD BRG IN-SX</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>578</td> <td>0</td> <td>5-8</td> <td>5-8</td> </tr> <tr> <td>C</td> <td>225</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> <tr> <td>D</td> <td>46</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> </tbody> </table> SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D UNFACTORED REACTIONS <table border="1"> <thead> <tr> <th>JT</th> <th>1ST LCASE</th> <th>MAX/MIN COMPONENT REACTIONS</th> <th>SNOW</th> <th>LIVE</th> <th>PERM.LIVE</th> <th>WIND</th> <th>DEAD</th> <th>SOIL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>404</td> <td>293 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>111 / 0</td> <td>0 / 0</td> <td>0 / 0</td> </tr> <tr> <td>C</td> <td>154</td> <td>128 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>26 / 0</td> <td>0 / 0</td> <td>0 / 0</td> </tr> <tr> <td>D</td> <td>38</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>35 / 0</td> <td>0 / 0</td> <td>0 / 0</td> </tr> </tbody> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) <table border="1"> <thead> <tr> <th rowspan="2">MEMB.</th> <th colspan="2">CHORDS</th> <th colspan="2">FACTORED</th> <th colspan="2">WEBS</th> <th colspan="2">FACTORED</th> </tr> <tr> <th>MAX. FORCE (LBS)</th> <th>VERT. LOAD (PLF)</th> <th>VERT. LOAD (PLF)</th> <th>CS (LC)</th> <th>MAX. UNBRACED LENGTH</th> <th>MAX. FORCE (LBS)</th> <th>CS (LC)</th> </tr> </thead> <tbody> <tr> <td>FR-TO</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>E-B</td> <td>-515 / 0</td> <td>0.0</td> <td>0.0</td> <td>0.12 (4)</td> <td>7.81</td> <td></td> <td></td> </tr> <tr> <td>A-B</td> <td>0 / 39</td> <td>-102.0</td> <td>-102.0</td> <td>0.14 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> <tr> <td>B-C</td> <td>-42 / 0</td> <td>-102.0</td> <td>-102.0</td> <td>0.60 (1)</td> <td>6.25</td> <td></td> <td></td> </tr> <tr> <td>E-D</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5</td> <td>0.13 (4)</td> <td>10.00</td> <td></td> <td></td> </tr> </tbody> </table>		JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	RECORD BRG IN-SX	E	578	0	5-8	5-8	C	225	0	1-8	1-8	D	46	0	1-8	1-8	JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	E	404	293 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	0 / 0	C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0	0 / 0	D	38	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0	0 / 0	MEMB.	CHORDS		FACTORED		WEBS		FACTORED		MAX. FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	CS (LC)	MAX. UNBRACED LENGTH	MAX. FORCE (LBS)	CS (LC)	FR-TO								E-B	-515 / 0	0.0	0.0	0.12 (4)	7.81			A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00			B-C	-42 / 0	-102.0	-102.0	0.60 (1)	6.25			E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00		
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TOWN OF CALEDON
BUILDING SECTION
FILE NO.

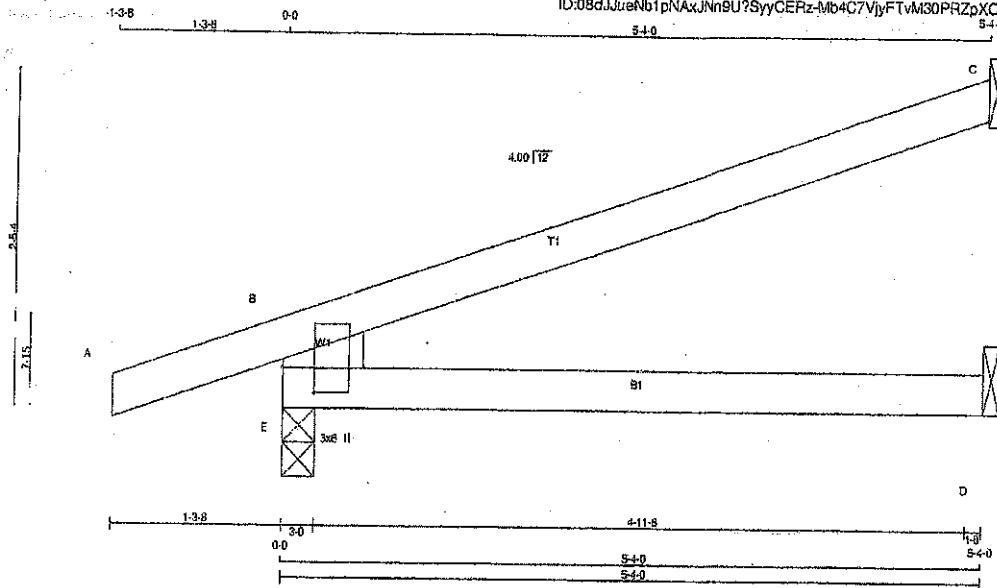


Structural component only
DWG# T-1923559

JOB NAME 405481	TRUSS NAME J3	QUANTITY 42	PLY 1	JOB DESC. GREEN PARK HOMES	BRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:14.8



TOTAL WEIGHT = 42 X 15 = 621 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x8	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B					
E	TMBMV1-p	MT20	3.0	6.0	2.25 2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
JT	GROSS REACTION		GROSS REACTION		BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
E	538	0	538	0	0	3-0
C	204	0	204	0	0	1-8
D	37	0	42	0	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) E, C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	SNOW	LIVE				
E	376	271.0	0.0	0.0	0.0	0.0	105.0	0.0
C	140	116.0	0.0	0.0	0.0	0.0	24.0	0.0
D	30	0.0	0.0	0.0	0.0	0.0	30.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS1 (L/C)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (L/C)
FR-TO		FROM TO			FR-TO		
E-B	-477.0	0.0 0.0	0.05 (4)	7.81			
A-B	0.22	-102.0 -102.0	0.13 (1)	10.00			
B-C	-22.0	-102.0 -102.0	0.48 (1)	6.25			
E-D	0.0	-18.5 -18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 42.4 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(5% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (TL) = L/360 (0.19")

CALCULATED VERT. DEFL (TL) = L/999 (0.02")

CS1: TC=0.49/1.00 (B-C:1), BC=0.15/1.00 (D-E:4), WB=0.00/1.00 (V=0), SS=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

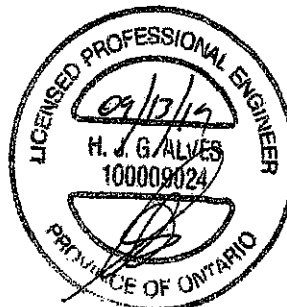
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1987 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (E) (INPUT = 0.90)

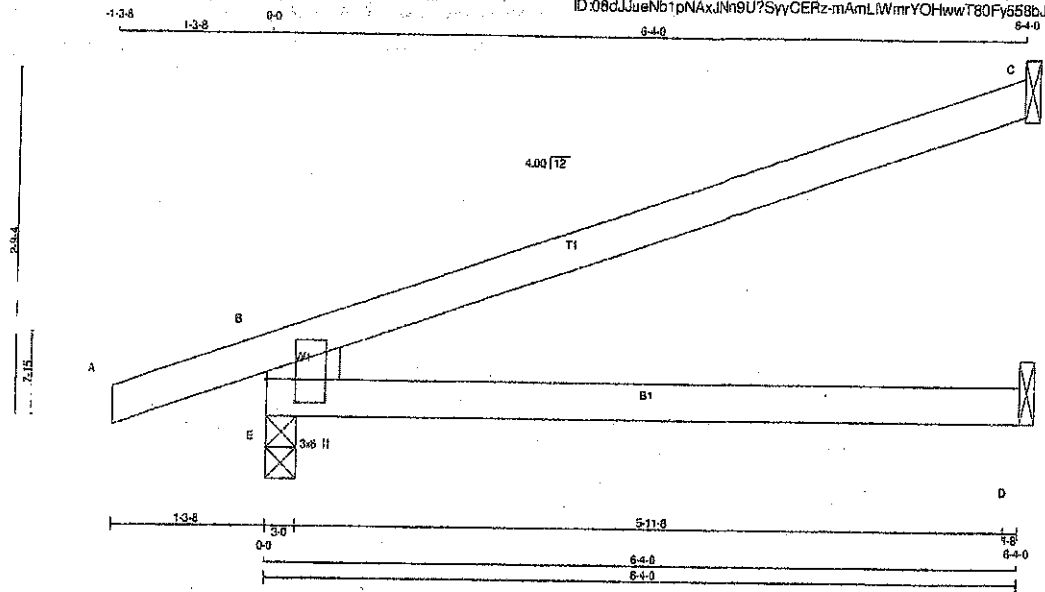
JSI METAL = 0.09 (E) (INPUT = 1.00)



Structural component only
DWG# T-1923560

JOB NAME 405481	TRUSS NAME J5	QUANTITY 10	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/16"

TOTAL WEIGHT = 10 X 17 = 171 LB (M/P)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x8	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX					
E	613	0	0	0	0	0	0	0	0
C	242	0	0	0	0	0	0	0	0
D	44	0	0	0	0	0	0	0	0

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL = 29.0	PSF	
DL = 6.0	PSF		
BOT CH.	LL = 0.0	PSF	
DL = 7.4	PSF		
TOTAL LOAD	= 42.4	PSF	

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
E	TMBMV1+p MT20	3.0	6.0	2.25 2.75

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS				
E	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
C	429	307 / 0	0 / 0	0 / 0	0 / 0	122 / 0
D	166	138 / 0	0 / 0	0 / 0	0 / 0	29 / 0
	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
E-B	-540 / 0	E-B	0.0 0.0 0.05 (4) 7.81
A-B	0 / 22	A-B	-102.0 -102.0 0.13 (1) 10.00
B-C	-26 / 0	B-C	-102.0 -102.0 0.54 (1) 6.25
E-D	0 / 0	E-D	-18.5 -18.5 0.21 (4) 10.00

ALLOWABLE DEFL.(LL) = $L/360$ (0.21")

CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")

ALLOWABLE DEFL.(TL) = $L/360$ (0.21")

CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.54/1.00 (B-C:1), BC=0.21/1.00 (D-E:4), WB=0.00/1.00 (W=0), SSI=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1967 788	1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

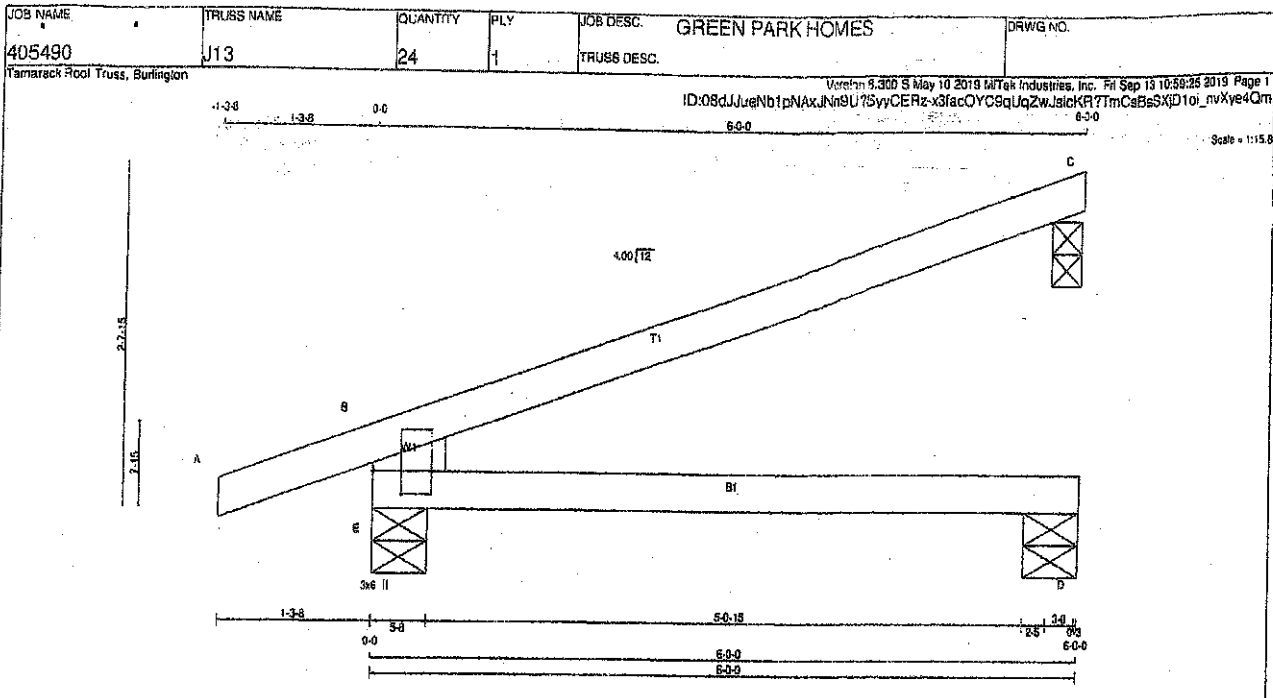
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.27 (E) (INPUT = 0.90)

JSI METAL = 0.11 (E) (INPUT = 1.00)



Structural component only
DWG# T-1923562



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x8	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY, SEASONED LUMBER.

<u>PLATES (Table 1a in inches)</u>						
JT	TYPE	PLATES	W	LEN	Y	X
B						
E	TMBMV1+p	MT20	3.0	6.0	2.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECD BRG
	VERT	HORZ	DOWN	HORZ		
E	588	0	588	0	5-8	5-8
C	230	0	230	0	3-0	3-0
D	42	0	47	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	411	295 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
C	158	131 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS				WEBS			
		FACTORED		MAX. FACTORED		FACTORED		MAX. FACTORED	
		VERT. LOAD (PL)	CS1 (LC)	VERT. LOAD (PL)	CS1 (LC)	FORCE (PL)	CS1 (LC)	FORCE (PL)	CS1 (LC)
FR-TO		FROM	TO	FROM	TO	FROM	TO	FROM	TO
E-B	-519 / 0	0.0	0.0	0.08 (4)	7.81				
A-B	0 / 22	-102.0	-102.0	0.13 (1)	10.00				
B-C	-24 / 0	-102.0	-102.0	0.83 (1)	8.25				
E-D	0 / 0	-18.5	-18.5	0.19 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 42.4 PSF

SPACING = 24" IN DG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA C08-08, CSA C08-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
- 85 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.23")

CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: YC=0.63/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (A-B:0), SS=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

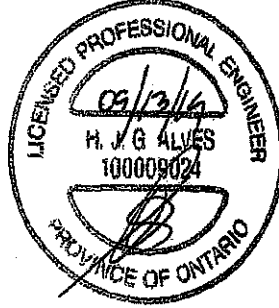
NAIL VALUES						
PLATE	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PL)		(PL)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1887	785	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP = 0.26 (E) (INPUT = 0.30)

JSI METAL = 0.10 (E) (INPUT = 1.00)

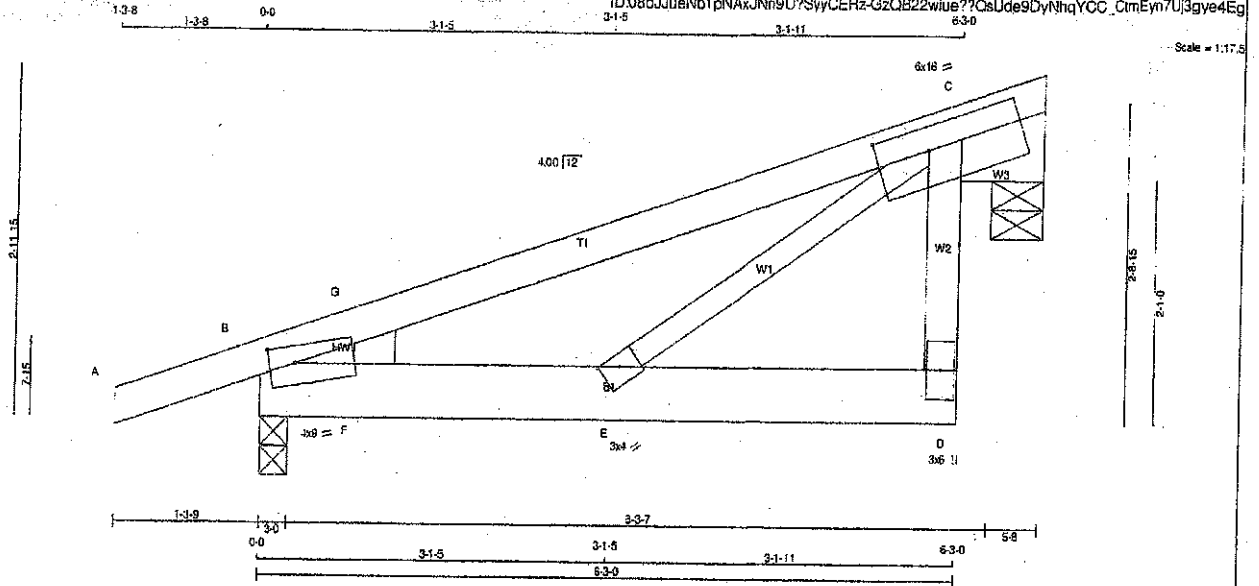


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SEP 16 2019

TOWN OF GALLON
BUILDING SECTION
FILE NO.

Structural component only
DWG# T-1923453

JOB NAME 405493	TRUSS NAME J14	QUANTITY 24	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 Mtek Industries, Inc. Fri Sep 13 11:12:19 2019 Page 1 ID:08dJueNb1pNaxJNn9U?SyyCERz-GzQB22wIue??CsUde9DyNhqYCC_CmEyn7Uj3gye4Eg		



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-m	MT20	4.0	9.0	1.75 2.75
C	TMVWW1-1	MT20	6.0	18.0	2.50 5.50
D	BMV+p	MT20	3.0	6.0	
E	BMV+ww	MT20	3.0	4.0	2.00 1.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD	HEEL
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG	WEDGE
		DOWN	DOWN	IN-SX	IN-SX	
C	377	0	377	0	5-8	
B	513	0	513	0	3-0	2x4 L

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	265	181 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
B	359	259 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CS1 (LC)	FR-TO	
A-B	0 / 8	-102.0 -102.0	0.13 (1)	E-C	0 / 455
B-G	-263 / 0	-102.0 -102.0	0.28 (1)	F-G	-900 / 0
G-C	-394 / 0	-102.0 -102.0	0.33 (1)		
D-C	-6 / 24	0.0	0.0		
B-F	0 / 359	-18.5	-18.5		
F-E	0 / 359	-18.5	-18.5		
E-D	0 / 0	-18.5	-18.5		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.21")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.21")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.02")

CS1: TC=0.33/1.00 (C-G-1), EC=0.28/1.00 (B-F-1), WB=0.10/1.00 (C-E-1), SS=0.24/1.00 (B-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

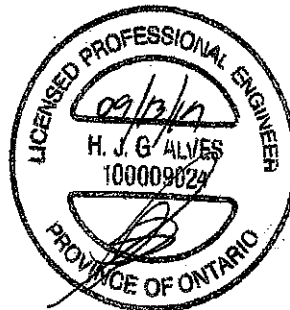
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1687 788	1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.33 (E) (INPUT = 0.90)
JSI METAL = 0.11 (E) (INPUT = 1.00)



Structural component only
DWG# T-1923458



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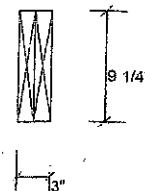
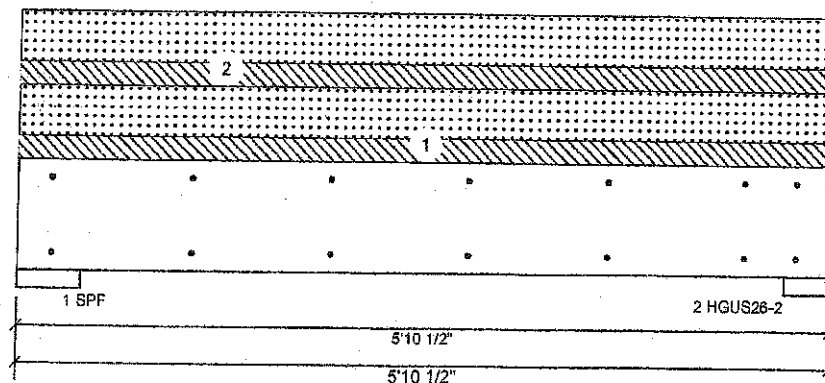
Client:
Project:
Address:

Date: 2/2019
Designer:
Job Name: 405481
Project #:

Page 1 of 4

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015 / CBC 2012
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	278	602	0
2	0	266	577	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	348 / 903	1250 L
2 - HGUS...	4.000"	16%	333 / 865	1198 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1413 ft-lb	3'	8039 ft-lb	0.234 (23%)	1.25D+1.5S	L
Unbraced	1413 ft-lb	3'	5236 ft-lb	0.270 (27%)	1.25D+1.5S	L
Shear	1085 lb	1'2"	3984 lb	0.272 (27%)	1.25D+1.5S	L
LL Defl inch	0.012 (L/5129)	3'	0.174 (L/360)	0.070 (7%)	S	L
TL Defl inch	0.018 (L/3608)	3'	0.174 (L/360)	0.100 (10%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of Pneumatic Gun Nail (120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



OWG NO. TAM 1923564
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-11-0	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE CANADIAN BUILDING CODE**

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115



This design is valid until 12/11/2021





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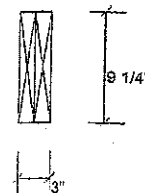
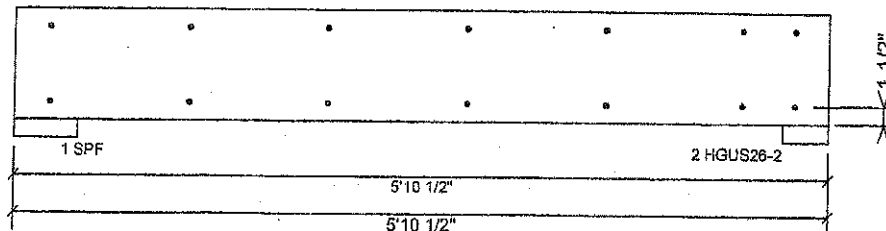
Client:
Project:
Address:

Date: 2/20/19
Designer:
Job Name: 405481
Project #:

Page 2 of 4

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



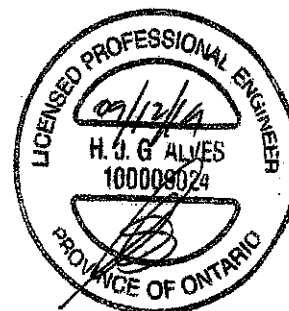
Multi-Ply Analysis

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	78.6 %
Load	178.2 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

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TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DWG NO. TAM 17923564
STRUCTURAL
COMPONENT ONLY

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115



This design is valid until 12/11/2021



isDesign™

Client:
Project:
Address:

Date: 2/20/19

Page 3 of 4

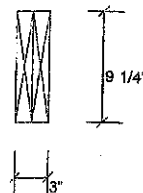
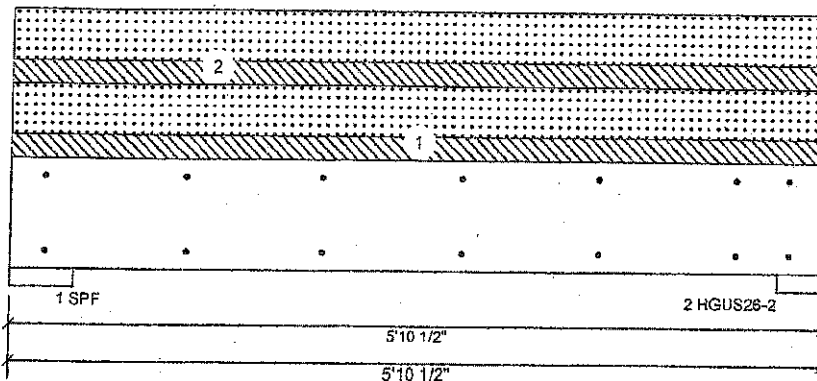
Designer:

Job Name: 405481

Project #:

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015 / OBC 2012
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	274	593	0
2	0	262	568	0

Bearings and Factored Reactions

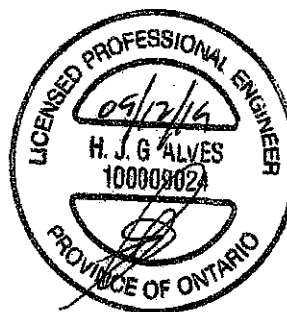
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	342 / 889	1231 L
2 - HGUS...	4.000"	16%	328 / 852	1180 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1392 ft-lb	3'	6039 ft-lb	0.230 (23%)	1.25D+1.5S	L
Unbraced	1392 ft-lb	3'	5236 ft-lb	0.266 (27%)	1.25D+1.5S	L
Shear	1069 lb	4'10"	3984 lb	0.268 (27%)	1.25D+1.5S	L
LL Defl inch	0.012 (L/5208)	3'	0.174 (L/360)	0.070 (7%)	S	L
TL Defl inch	0.018 (L/3562)	3'	0.174 (L/360)	0.100 (10%)	D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1723563
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-9-12	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115



This design is valid until 12/11/2021





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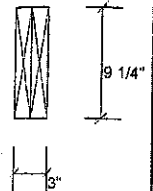
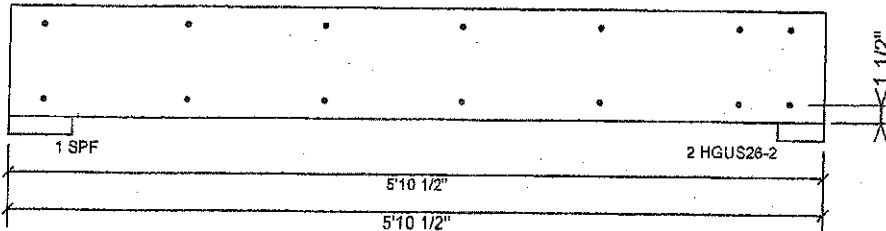
Client:
Project:
Address:

Date: 2/20/19
Designer:
Job Name: 405481
Project #:

Page 4 of 4

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	77.2 %
Load	175.1 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00



DWG NO. TAM T1923563
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115
	TAMARACK LUMBER INC ALFA LUMBER GROUP

This design is valid until 12/11/2021



RECEIVED
SEP 16 2019

Simpson Strong-Tie® Wood Construction Connectors — Canadian Limit States Design

LUL/LUS/LJS/HUS/HHUS/HGUS

BUILDING
FILE NO.
SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

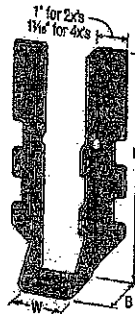
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



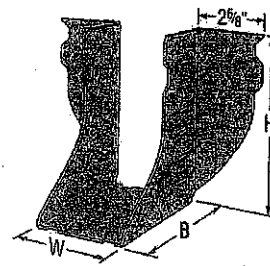
LUS26



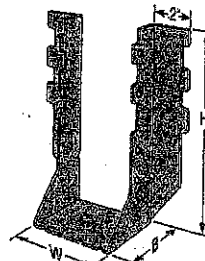
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



HHUS210-2



Double-Shear
Nailing
Top View

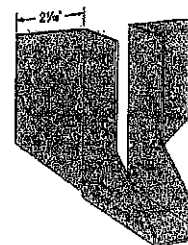
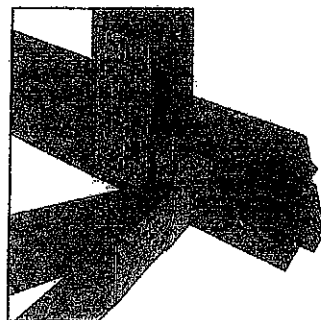


Double-Shear
Nailing
Side View;
Do not
bend tab



Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

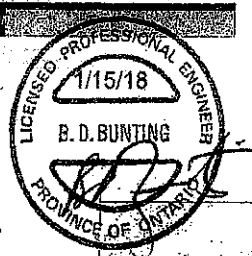
Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS



SIMPSON
Strong-Tie

HHUS/HGUS

See Hanger Options Information on pp. 125–127.

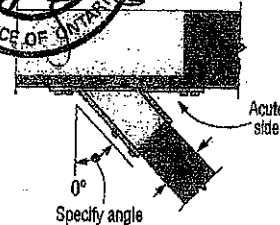
HHUS – Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.82 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGIS - Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Jolst	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

- These products are available with additional corrosion protection. For more information, see p. 24.

- These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	E	d _a ^a	Header	Joist	F-E-F		S-P-F	
								Ultimate (K _u = 1.75)	Normal (K _n = 3.0/2.2)	Ultimate (K _u = 1.75)	Normal (K _n = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1½	3½	1¾	2¼	(4) 10d	(2) 10d	710	1625	645	1155
								2047	4650	2387	5174
								360	1020	320	728
LU24L	22	1½	3	1¾	2½	(4) 10d	(2) 10d x 1½"	720	1605	645	1140
								2040	4605	2387	5087
								360	1020	320	728
LUS26	18	1½	4¾	1¾	3¼	(4) 10d	(4) 10d	1420	2170	1290	1630
								3960	5865	3574	4726
								2705	4940	2085	3875
HUS26	18	1¾	5¾	3	3½	(14) 16d	(6) 16d	1720	2190	1320	1724
								4760	5940	3920	4724
								2055	4265	1480	4115
LUS26DS	18	1½	5	3½	4¾	(18) 16d	(6) 16d	1720	2190	1320	1724
								4760	5940	3920	4724
								2055	4265	1480	4115
HGUS26	12	1¾	5¾	5	4¾	(26) 16d	(8) 16d	2685	6625	2685	5700
								7460	18360	7460	15500
								1140	2185	1020	1559
LU28L	20	1½	6¾	1¾	5¾	(8) 10d	(8) 10d x 1½"	1420	2520	1290	1790
								3960	6060	3574	4796
								2705	5385	2085	3875
LUS28	18	1½	6¾	1¾	3¾	(8) 10d	(4) 10d	1420	2520	1290	1790
								3960	6060	3574	4796
								2705	5385	2085	3875
HUS28	16	1¾	7¾	3	6¾	(22) 16d	(8) 16d	3605	5385	2675	4945
								9910	14565	7180	10333
								3310	7675	3310	6900
HGUS28	12	1¾	7¾	5	6¾	(38) 16d	(12) 16d	3720	6170	3310	5073
								10260	16860	10260	14773
								1140	2495	1020	1770
LU210L	20	1½	8	1¾	7¾	(10) 10d	(6) 10d x 1½"	1420	2785	1290	2210
								3960	6170	3574	4983
								2705	5385	2085	3875
LUS210	18	1½	7¾	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
								3960	6170	3574	4983
								2705	5385	2085	3875

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. c_{de} is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.127" dia., x 3 1/2" long. See pp. 27-28 for other nail sizes and information.

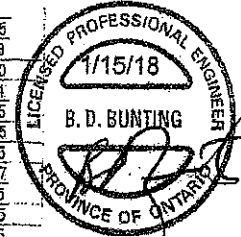
C-C-CAN2018 © 2017 SIMPSON STRONG-TIE COMPANY INC.

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _a ^a	Header	Joist	D-F-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K ₀ = 1.6)	(K ₀ = 1.0)	(K ₀ = 1.15)	(K ₀ = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 3/4	3 3/4	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
SS LUS26-2	18	3 3/4	4 3/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS26-2	14	3 3/4	5 3/4	3	3 3/4	(14) 16d	(6) 16d	2850	7335	2085	5205
HGUS26-2	12	3 3/4	5 3/4	4	4 3/4	(20) 16d	(8) 16d	4385	8950	3110	6355
SS LUS28-2	18	3 3/4	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2675
HHUS28-2	14	3 3/4	7 3/4	3	6 3/4	(22) 16d	(8) 16d	3785	8940	2675	6345
HGUS28-2	12	3 3/4	7 3/4	4	8 3/4	(36) 16d	(12) 16d	6070	12980	4310	9215
SS LUS210-2	18	3 3/4	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
HHUS210-2	14	3 3/4	9 3/4	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HGUS210-2	12	3 3/4	9 3/4	4	8 3/4	(48) 16d	(16) 16d	6840	14015	4855	10270
Triple 2x Sizes											
HGUS28-3	12	4 3/4	5 1/2	4	4 3/4	(24) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-3	12	4 3/4	7 1/4	4	6 3/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-3	14	4 3/4	9	3	7 3/4	(30) 16d	(10) 16d	4670	9570	4235	6865
HGUS210-3	12	4 3/4	9 3/4	4	8 3/4	(48) 16d	(16) 16d	6840	14645	4855	10400
Quadruple 2x Sizes											
HGUS26-4	12	6 3/4	5 3/4	4	4 3/4	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-4	12	6 3/4	7 3/4	4	6 3/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-4	14	6 3/4	8 3/4	3	7 3/4	(30) 16d	(10) 16d	4670	10155	4235	7210
HGUS210-4	12	6 3/4	9 3/4	4	8 3/4	(48) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 3/4	10 3/4	4	10 3/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 3/4	12 3/4	4	11 3/4	(68) 16d	(22) 16d	10130	16400	7195	11645
4x Sizes											
LUS46	18	3 3/4	4 3/4	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS46	14	3 3/4	5 3/4	3	3 3/4	(14) 16d	(6) 16d	2540	7335	2085	5205
HGUS46	12	3 3/4	5 3/4	4	4 3/4	(20) 16d	(8) 16d	4385	8950	3110	6355
LUS48	18	3 3/4	6 3/4	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2675
HHUS48	14	3 3/4	7 3/4	3	6 3/4	(22) 16d	(8) 16d	3785	8940	2675	6345
HGUS48	12	3 3/4	7 3/4	4	6 3/4	(36) 16d	(12) 16d	6070	12980	4310	9215
LUS410	18	3 3/4	8 3/4	2	5 3/4	(8) 16d	(6) 16d	2580	4500	2320	3195
HGUS410	12	3 3/4	9	4	8 3/4	(48) 16d	(16) 16d	6840	14015	4855	10270
HGUS412	12	3 3/4	10 3/4	4	10 3/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS414	12	3 3/4	12 3/4	4	11 3/4	(68) 16d	(22) 16d	10130	16400	7195	11645



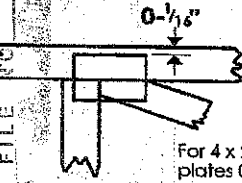
Plated Truss Connectors

See footnotes on p. 258.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



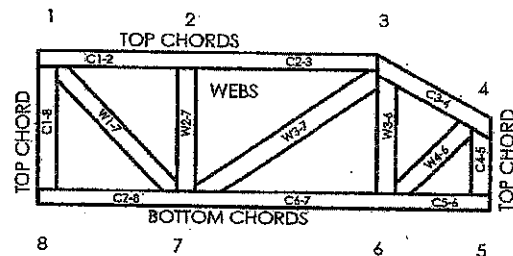
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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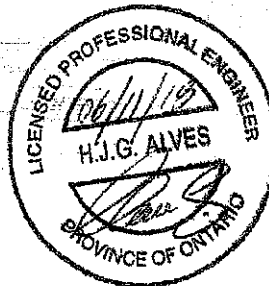
MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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BUILDING SECTION
FILE NO.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900219

Feb 09, 2018