

URBAN 1
FRONT ELEV.-2
UNIT 83-4
LOWES HILL CIRCLE
403462

URBAN 1
FRONT ELEV.-2
UNIT 83-2
LOWES HILL CIRCLE
403462

URBAN 1
FRONT ELEV.-2
UNIT 83-1
LOWES HILL CIRCLE
403462

ASPHALT SHINGLES
2X6 BRG/BRK.
12" FIN. OH.
R.T.M.C
WW/ 2X6 FASCIA
PARTY WALL
2-2X4 STUD
WW/ 1-2X4 STUD ABOVE
MILTON
(23.3, 8.0, 7.4)

BY JESSIE

Job Track: **50033**
Plan Log: **201706**
Layout ID: **405493**

Project: **LAMBERT LANE PH.2**

Date: 9/10/2019 Sales: Mario DiCano Designer: AC

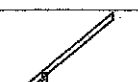
Model / Elevation:
BLOCK 83 / UNITS 83-1 - 83-4

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Mitek ver 8.3.0.234

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST				
	Lumber Yard:	TAMARACK LUMBER		Job Track:	50033
	Builder:	GREEN PARK HOMES		PlanLog:	201706
	Project:	LAMBERT LANE PH.2		Layout ID:	405494
	Location:	CALEDON		Ref #	
	Model:	BLOCK 83		Page:	1 of 1
Lot #:			Date:	09/10/2019	
Elevation:	83-1, 83-2, 83-3, 83-4		Designer:	Andrew Conway	
			Sales Rep:	Mario DiCano	

Roof Trusses

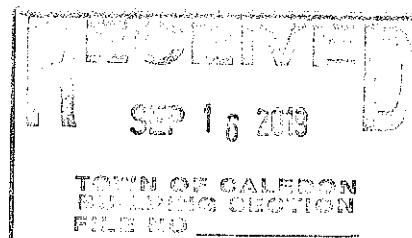
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	H6 Monopitch	5/12	22-02-10	10-03-10	2 x 4	1-03-08	1-00-09 10-03-11	860.46 535.50		
	1 2-ply	H31 Flat Girder	0/12	22-02-10	4-10-07	2 x 4 2 x 6	1-03-08	4-10-07 4-10-07	228.57 143.33		
	2	H32 Flat	0/12	22-02-10	6-06-07	2 x 4	1-03-08	6-06-07 6-06-07	196.02 120.67		
	2	H33 Flat	0/12	22-02-10	8-02-07	2 x 4	1-03-08	8-02-07 8-02-07	235 144.00		
	2	H34 Flat	0/12	22-02-10	9-10-07	2 x 4	1-03-08	9-10-07 9-10-07	256.77 157.33		
	3	H35A Roof Special Structural Gable	10/12	11-10-00	6-06-14	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	174.39 119.50		
	6	J8 Jack-Open	10/12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	82.48 52.00		
	6	J13 Jack-Open	4/12	6-00-00	2-08-14	2 x 4	1-03-08	7-15 2-04-15	98.04 64.00		
	6	J14 Jack-Closed	4/12	6-03-00	3-00-14	2 x 4	1-03-08	7-15 2-08-15	185.77 108.00		

TOTAL # TRUSS= 38 TOTAL BFT OF ALL TRUSSES= 1444.33 BFT. TOTAL WEIGHT OF ALL TRSSES 2295.5 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LUS24	
1	Hardware	HGUS26-2	

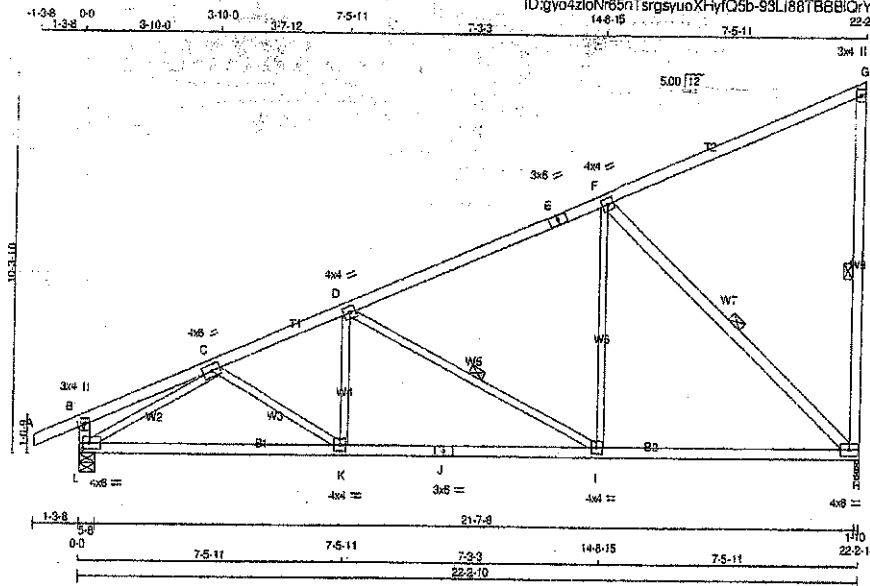
TOTAL NUMBER OF
ITEMS= 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405486	H6	36	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 36 X 99 = 3558 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - S	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	6.0		
D	TMVW-1	MT20	4.0	4.0		
E	TS-1	MT20	3.0	5.0		
F	TMVW-1	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	6.0		
I	BMVW-1	MT20	4.0	4.0		
J	SS-1	MT20	3.0	6.0		
K	BMVW-1	MT20	4.0	4.0		
L	BMVW-1	MT20	4.0	6.0	2.00	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER							
<u>BEARINGS</u>							
	FACTORED		MAXIMUM FACTORED		INPUT	RECORD	
	GROSS REACTION		GROSS REACTION		BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1339	0	1339	0	0	1-10	1-10
L	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	942	644 : 0	0 : 0	0 : 0	0 : 0	288 : 0	0 : 0
L	1037	723 : 0	0 : 0	0 : 0	0 : 0	314 : 0	0 : 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-J, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED (PLF)
FR-TO				FR-TO			
A-B	0/26	-102.0	-102.0 0.13 (1)	K-D	0/139	0.05 (4)	
B-C	3/8	-102.0	-102.0 0.20 (1)	D-I	-841/0	0.38 (1)	
C-D	-1943/0	-102.0	-102.0 0.34 (1)	I-F	0/547	0.12 (1)	
D-E	-1140/0	-102.0	-102.0 0.88 (1)	F-H	-1454/0	0.78 (1)	
E-F	-1140/0	-102.0	-102.0 0.89 (1)	C-K	0/35	0.02 (4)	
F-G	-32/0	-102.0	-102.0 0.70 (1)	L-C	-2119/0	0.68 (1)	
G-H	298/0	0.0	0.0 0.18 (1)				
L-G	-309/0	0.0	0.0 0.03 (1)				
L-K	0/1804	-18.5	-18.5 0.42 (1)				
K-J	0/1812	-18.5	-18.5 0.43 (1)				
J-I	0/1812	-18.5	-18.5 0.43 (1)				
I-H	0/1089	-18.5	-18.5 0.32 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.70/1.00 (F-G:1), BC=0.43/1.00 (I-K:1)
WB=0.76/1.00 (F-H:1), SS=0.33/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

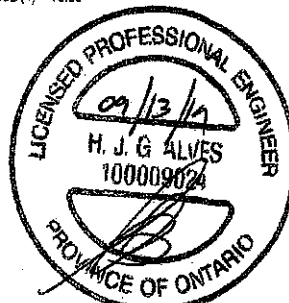
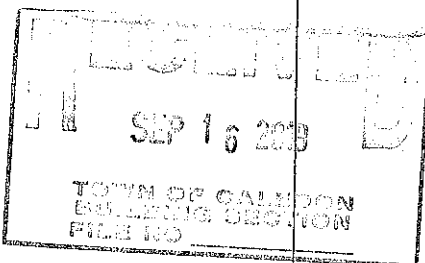
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (L) (INPUT = 0.80)
JSI METAL = 0.59 (J) (INPUT = 1.00)

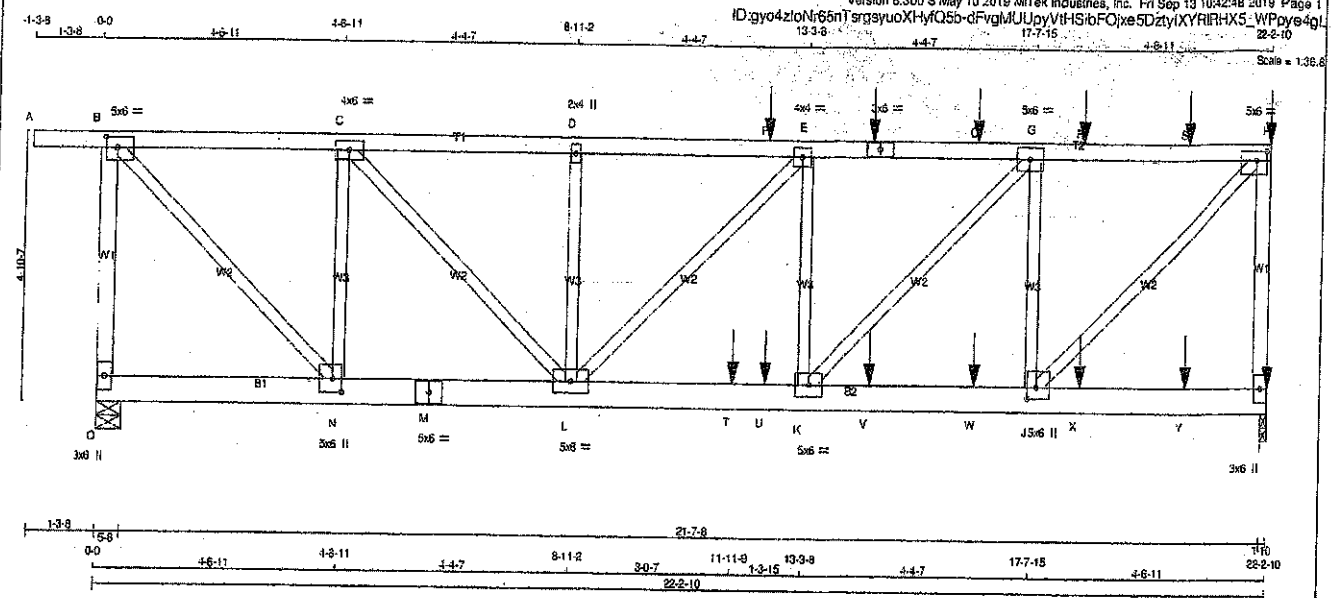


Structural component only
DWG# T-1923426

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
405486	H31	4	2	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:gyo4zloN66n1rgayuoXHYIQ5b-dFvgMUUyVH5bFOxe5DztyIXYRHX5-WPpye4gI
13-3-8 13-3-8 17-7-15 22-2-10
Scale = 1:36.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
O - B	2x4 DRY	No.2	SPF		
A - F	2x4 DRY	No.2	SPF		
F - H	2x4 DRY	No.2	SPF		
I - H	2x4 DRY	No.2	SPF		
O - M	2x6 DRY	No.2	SPF		
M - I	2x6 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
O - B	12	TOP SIDE(0.0)
A - F	12	SIDE(61.0)
F - H	12	TOP
H - I	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
O - M	12	TOP SIDE(0.0)
M - I	12	TOP
WEBS : (0.122'X3') SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-1	MT20	5.0	5.0	2.25	2.50
C	TMWV-1	MT20	4.0	3.0		
D	TMWV-1	MT20	2.0	4.0		
E	TMWV-1	MT20	4.0	4.0		
F	TS-1	MT20	3.0	5.0		
G	TMWV-1	MT20	5.0	5.0		
H	TMWV-1	MT20	5.0	5.0	2.00	2.50
I	BMV1-p	MT20	3.0	6.0		
J	BMVW-1	MT20	5.0	6.0	2.50	2.00
K	BMVW-1	MT20	5.0	6.0		
L	BMVWV-1	MT20	5.0	6.0		
M	BS-1	MT20	5.0	6.0		
N	BMVW-1	MT20	5.0	6.0	3.00	2.00
O	BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	1916	0	1916	0
I	2277	0	2277	0

UNFACTORED REACTIONS	1ST LOOSE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
O	1348	944/0	0/0
I	1801	1107/0	0/0

BEARING MATERIAL TO BE SPF#2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LO1	MAX	UNBRACED	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX
FR-TO												
O-B	-1871/0	0.0	0.0	0.34(1)	7.81	J-H	0/2578	0.32(1)				
A-B	0/0	-102.0	-102.0	0.06(1)	10.00	B-N	0/2210	0.27(1)				
B-C	-1573/0	-102.0	-102.0	0.19(1)	5.25	J-G	-1748/0	0.30(1)				
C-D	-2533/0	-102.0	-102.0	0.19(1)	5.49	N-C	-1504/0	0.25(1)				
D-P	-2533/0	-102.0	-102.0	0.18(1)	5.50	K-G	0/1378	0.17(1)				
P-E	-2533/0	-102.0	-102.0	0.18(1)	5.50	C-L	0/1377	0.17(1)				
E-F	-2788/0	-102.0	-102.0	0.28(1)	5.16	K-E	-339/0	0.06(1)				
F-Q	-2788/0	-102.0	-102.0	0.28(1)	5.16	L-D	-432/0	0.07(1)				
Q-G	-2788/0	-102.0	-102.0	0.28(1)	5.16	L-E	-385/0	0.13(1)				
G-R	-1834/0	-102.0	-102.0	0.26(1)	6.09							
R-S	-1834/0	-102.0	-102.0	0.26(1)	6.09							
S-H	-1834/0	-102.0	-102.0	0.26(1)	6.09							
H-I	-2189/0	0.0	0.0	0.40(1)	7.52							

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LO1	MAX	UNBRACED	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX
FR-TO												
O-N	0/0	-18.5	-18.5	0.02(4)	10.00							
N-M	0/1573	-18.5	-18.5	0.14(1)	10.00							
M-L	0/1573	-18.5	-18.5	0.14(1)	10.00							
L-T	0/2788	-18.5	-18.5	0.36(1)	10.00							
T-U	0/2788	-18.5	-18.5	0.36(1)	10.00							
U-K	0/2788	-18.5	-18.5	0.36(1)	10.00							
K-V	0/1834	-18.5	-18.5	0.19(1)	10.00							
V-W	0/1834	-18.5	-18.5	0.19(1)	10.00							
W-J	0/1834	-18.5	-18.5	0.19(1)	10.00							
J-X	0/0	-18.5	-18.5	0.04(4)	10.00							
X-Y	0/0	-18.5	-18.5	0.04(4)	10.00							
Y-I	0/0	-18.5	-18.5	0.04(4)	10.00							

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LO1	MAX-	MAX+	FACE	DIR.	TYPE	WIND	CONN.
F	14-8-14	-95	-95			FRONT	VERT	TOTAL		C1
H	22-2-10	-137	-137			FRONT	VERT	TOTAL		C1
I	22-2-10	-25	-25			FRONT	VERT	TOTAL		C1
J	12-8-14	-98	-98			FRONT	VERT	TOTAL		C1
Q	16-8-14	-98	-98			FRONT	VERT	TOTAL		C1
R	18-8-14	-98	-98			FRONT	VERT	TOTAL		C1
S	20-8-14	-98	-98			FRONT	VERT	TOTAL		C1
T	11-11-9	-680	-680			FRONT	VERT	TOTAL		C1
U	12-8-14	-17	-17			FRONT	VERT	TOTAL		C1
V	14-8-14	-17	-17			FRONT	VERT	TOTAL		C1
W	18-8-14	-17	-17			FRONT	VERT	TOTAL		C1
X	18-8-14	-17	-17			FRONT	VERT	TOTAL		C1
Y	20-8-14	-17	-17			FRONT	VERT	TOTAL		C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.40/1.00 (H-I), BC=0.36/1.00 (K-L),
WB=0.32/1.00 (H-J), SS=0.18/1.00 (K-L)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 318 354 1867 788 1987 1856

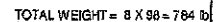
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

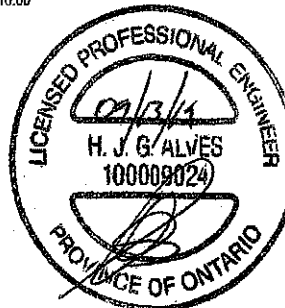
JSI GRIP= 0.48 (J) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)

Structural component only
DWG# T-1923427

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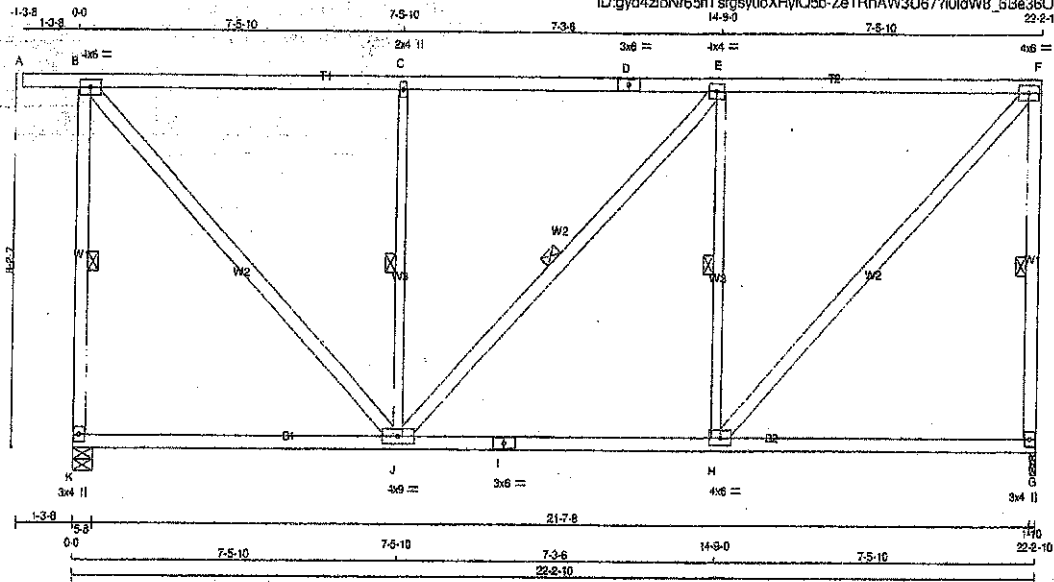
SEP 16 2013
TOWN OF CALEDON
BUILDING DEPARTMENT
FILE NO. _____



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405486	H39	8	1	TRUSS DESC.		

Tamareck Roof Truss, Burlington

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ID: gyo4z0N65nTsgsyoXhYQ5b-Ze1RnAW3U6770ldW8_5B3e360EvePa7PTdUiy4gJ



Scale = 1/4" = 1'-0"

TOTAL WEIGHT = 8 X 118 = 940 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
K - B	2x4	DRY	No.2	SPF		
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
G - I	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
I - G	2x4	DRY	No.2	SPF		
ALL WEBS	2x4	DRY	No.2	SPF		
EXCEPT						
H - E	2x3	DRY	No.2	SPF		
J - C	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW-1	MT20	4.0	6.0		
C TMW-w	MT20	2.0	4.0		
D TS-1	MT20	3.0	6.0		
E TMWV-1	MT20	4.0	4.0		
F TMWV-1	MT20	4.0	6.0		
G BMV1-p	MT20	3.0	4.0		
H BMWV-1	MT20	4.0	6.0		
I BS-1	MT20	3.0	6.0		
J BMWVW-1	MT20	4.0	6.0		
K BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT
K 1471	0	1471	0	0
G 1339	0	1339	0	0

UNFACTORED REACTIONS

1ST L.CABE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1033	719 / 0	0 / 0	0 / 0	0 / 0	313 / 0	0 / 0
G	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, F-G, E-H, C-J, E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM TO		FR-TO	
K-B	-1414 / 0	0.0	0.0 0.43 (1)	H-F	0 / 1342
A-B	0 / 0	-102.0	-102.0 0.12 (1)	B-F	0 / 1340
B-C	-920 / 0	-102.0	-102.0 0.85 (1)	H-E	-826 / 0
C-D	-820 / 0	-102.0	-102.0 0.85 (1)	J-C	-825 / 0
D-E	-820 / 0	-102.0	-102.0 0.85 (1)	J-E	-825 / 0
E-F	-921 / 0	-102.0	-102.0 0.85 (1)		0.00 (1)
G-F	-1263 / 0	0.0	0.0 0.39 (1)		
K-J	0 / 0	-18.5	-18.5 0.23 (4)		
J-I	0 / 821	-18.5	-18.5 0.30 (4)		
I-H	0 / 921	-18.5	-18.5 0.30 (4)		
H-G	0 / 0	-18.5	-18.5 0.23 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.3 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CS: TC=0.85/1.00 (E-F-1), BC=0.30/1.00 (H-J-4), WB=0.35/1.00 (E-H-1), SG=0.36/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

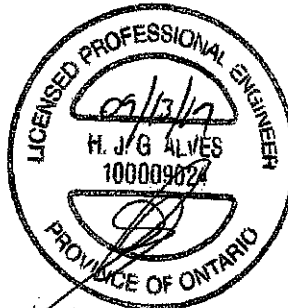
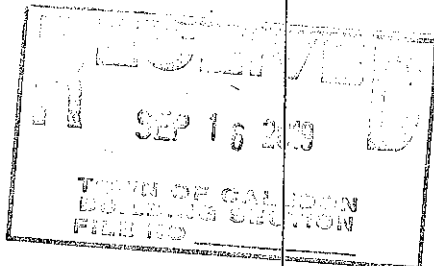
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSD) (PLD) (PLD)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 783 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

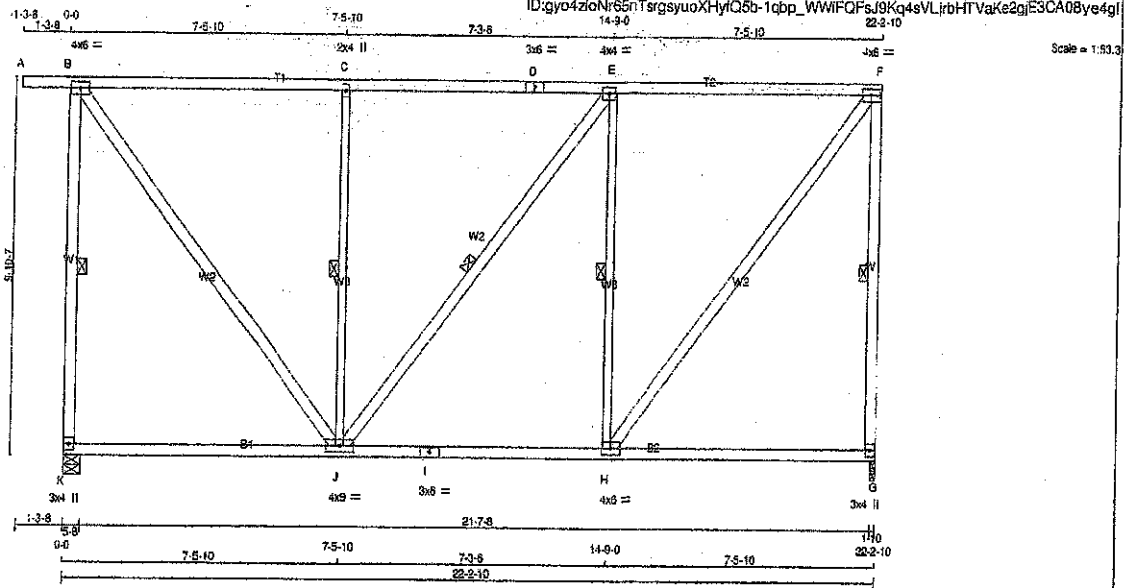
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (H) (INPUT = 0.90)
JSI METAL = 0.31 (F) (INPUT = 1.00)



Structural component only
DWG# T-1923429

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	DRWG NO.
405486	H34	8	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



N. L. C. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
K - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF
J - C	2x3	DRY	No.2	SPF

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	6.0		
C	TMVW-1	MT20	2.0	4.0		
D	TS-1	MT20	3.0	6.0		
E	TMVW-1	MT20	4.0	4.0		
F	TMVW-1	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	5.0		
I	BS-1	MT20	3.0	6.0		
J	BMVWW-1	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
K	VERT	DOWN	UPLIFT	IN-SX
K	1471	0	0	5-8
G	1339	0	0	1-10

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
K	1033	718/0
G	942	644/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, F-G, E-H, C-J, E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSH (LC)	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSH (LC)
FR-TO			FROM TO			FR-TO			
K-B	-1414/0	0.0	0.0	0.86 (1)	5.48	H-F	0/1238	0.20 (1)	
A-B	0/0	-102.0	-102.0	0.12 (1)	10.00	B-J	0/1238	0.20 (1)	
B-C	-780/0	-102.0	-102.0	0.82 (1)	5.18	H-E	-826/0	0.54 (1)	
C-D	-780/0	-102.0	-102.0	0.83 (1)	5.13	J-C	-826/0	0.54 (1)	
D-E	-780/0	-102.0	-102.0	0.83 (1)	5.13	J-E	-2/0	0.00 (1)	
E-F	-781/0	-102.0	-102.0	0.83 (1)	5.13				
G-F	-1263/0	0.0	0.0	0.60 (1)	5.68				
K-J	0/0	-18.5	-18.5	0.23 (4)	10.00				
J-I	0/781	-18.5	-18.5	0.29 (4)	10.00				
I-H	0/781	-18.5	-18.5	0.29 (4)	10.00				
H-G	0/0	-18.5	-18.5	0.23 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/960 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSH: TO=0.83/1.00 (E-F:1), BC=0.29/1.00 (H-J:4), WB=0.54/1.00 (E-H:1), SS=0.39/1.00 (E-F:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

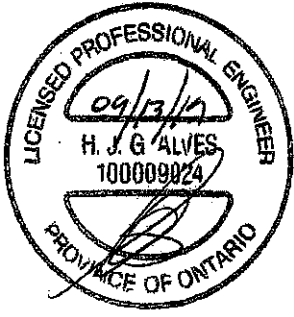
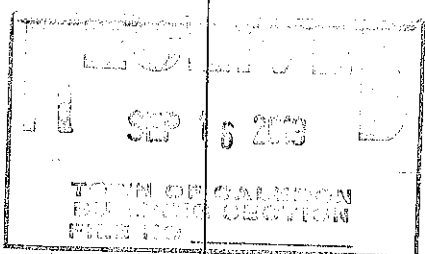
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1857 788 1987 1856

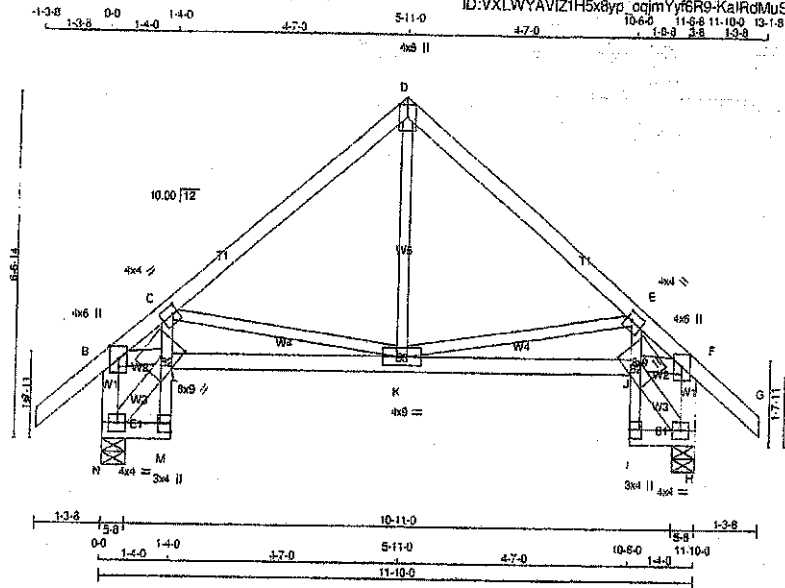
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (F) (INPUT = 0.80)
JSI METAL = 0.29 (F) (INPUT = 1.00)



Structural component only
DWG# T-1923430

JOB NAME 405493	TRUSS NAME H35A-Cond1	QUANTITY 12	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS NO. DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 5 May 10 2019 Mitek Industries, Inc. Fri Sep 15 11:12:17 2019 Page 1 ID:VXLWYAVIZIH5x8yp_cqjmyf6R9-KaIRdMuSM1H-BZKEXIBUIGIdOKiPrUp7d?nye4E	
Scale = 1/32					



LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x3	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x3	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS			
EXCEPT	2x3	DRY	No.2
K - D	2x3	DRY	No.2
K - E	2x3	DRY	No.2
C - K	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	V	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW-i	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-i	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVW-i	MT20	4.0	4.0		
I	BMVW+p	MT20	4.0	4.0		
J	BMVW+w	MT20	8.0	9.0	4.50	3.75
K	BMVW-w	MT20	4.0	4.0		
L	BMVW+w	MT20	8.0	9.0	4.50	3.75
M	BMVW-p	MT20	3.0	4.0	2.00	Edge
N	BMVW-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
SPF	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	DOWN	UPLIFT
N	854	0	5-8
H	854	0	5-8

UNFACTORED REACTIONS			
1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
N	899	424 / 0	0 / 0
H	899	424 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED	FACTORED	WESS
	FORCE	VERT. LOAD LC1	MAX. FACTORED
	(LBS)	(PLF)	FORCE
FR-TO	FROM	TO	CS (LC)
A-B	0 / 46	-102.0	-102.0
B-C	-1082 / 0	-102.0	-102.0
C-D	-572 / 0	-102.0	-102.0
D-E	-372 / 0	-102.0	-102.0
E-F	-1082 / 0	-102.0	-102.0
F-G	0 / 46	-102.0	-102.0
G-H	-832 / 0	0.0	0.0
H-I	-832 / 0	0.0	0.0
I-J	0 / 10	-18.5	-18.5
J-K	0 / 10	-18.5	-18.5
K-L	0 / 48	0.0	0.0
L-M	0 / 865	-18.5	-18.5
M-N	0 / 865	-18.5	-18.5
N-O	0 / 10	0.0	0.0
O-P	0 / 10	0.0	0.0
P-Q	0 / 10	0.0	0.0
Q-R	0 / 10	0.0	0.0
R-S	0 / 10	0.0	0.0
S-T	0 / 10	0.0	0.0
T-U	0 / 10	0.0	0.0
U-V	0 / 10	0.0	0.0
V-W	0 / 10	0.0	0.0
W-X	0 / 10	0.0	0.0
X-Y	0 / 10	0.0	0.0
Y-Z	0 / 10	0.0	0.0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/990 (0.39")
CALCULATED VERT. DEF. (LL) = L/999 (0.01")
ALLOWABLE DEF. (TL) = L/360 (0.39")
CALCULATED VERT. DEF. (TL) = L/999 (0.03")

CS: TO=0.27/1.00 (D-E:1), BC=0.20/1.00 (K-L:1), WB=0.18/1.00 (C-K:1), SS=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

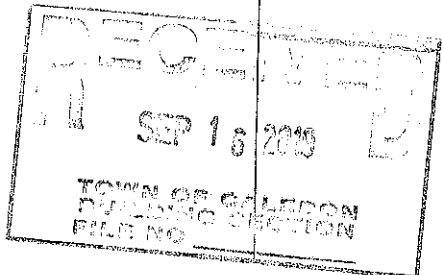
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

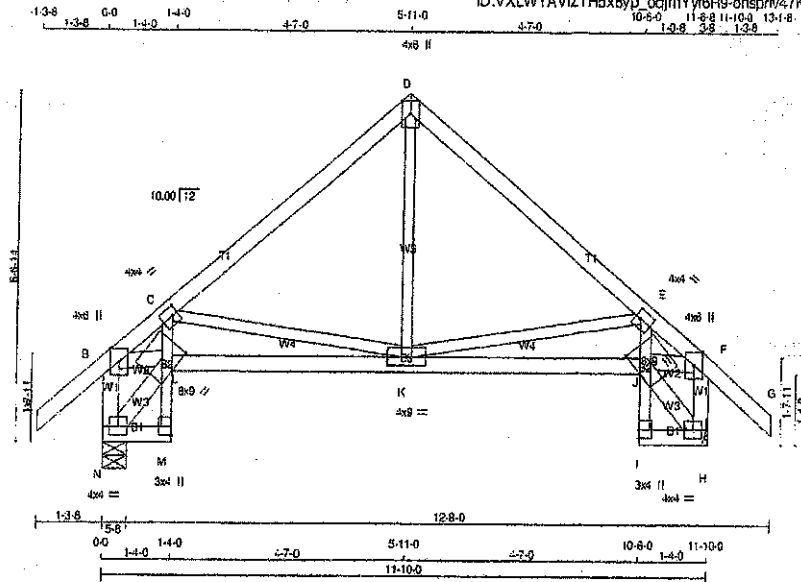
JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.50 (F) (INPUT = 1.00)



Structural component only
DWG# T-1923456

JOB NAME 405493	TRUSS NAME H35A-Cond2	QUANTITY 12	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.300 5 May 10 2019 Mitek Industries, Inc. Fri Sep 13 11:12:16 2019 Page 1
ID: VXLWYAVIZ1H5x8yp_oqjmYf6R9-onsprv47K18pivQ4SijrTHOMog68lpYtkAXDye4Eh



TOTAL WEIGHT = 12 X 56 = 696 lb
[M/F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
K - D	2x3	DRY	No.2	SPF
K - E	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
N	854	0	5-8	5-8
H	854	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

	1ST LOSE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
N	599	424 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0	
H	599	424 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.39")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.39")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.27/1.00 (D-E-1), BC=0.20/1.00 (K-L-1), WB=0.18/1.00 (C-K-1), SSI=0.17/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVEHEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION	
(PSI)	(PL)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667	788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (L) (INPUT = 0.90)
JSI METAL = 0.50 (F) (INPUT = 1.00)

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW+L	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+L	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMVW+L	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVWVW+L	MT20	8.0	9.0	4.50	3.75
K	BMVWVW+L	MT20	4.0	8.0		
L	BMVWVW+L	MT20	8.0	9.0	4.50	3.75
M	BMV+p	MT20	3.0	4.0	2.00	Edge
N	BMVW+L	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACINGS

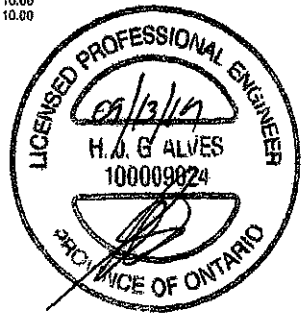
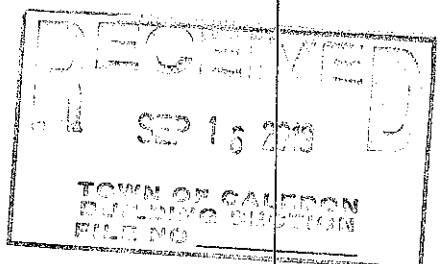
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX. FACTORED	MEMB.	MAX. FACTORED	MAX. FACTORED
	FORCE (LBS)	(PLF)	CS (LC)	UNBRAC		FORCE (LBS)	CS (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 45	-102.0 -102.0	0.14 (1)	10.00	K-D	0 / 302	0.07 (1)
B-C	-1082 / 0	-102.0 -102.0	0.21 (1)	5.87	K-E	-455 / 0	0.18 (1)
C-D	-572 / 0	-102.0 -102.0	0.27 (1)	6.25	C-K	-455 / 0	0.18 (1)
D-E	-572 / 0	-102.0 -102.0	0.27 (1)	6.25	N-L	-15 / 0	0.00 (1)
E-F	-1082 / 0	-102.0 -102.0	0.21 (1)	5.87	B-L	0 / 851	0.14 (1)
F-G	0 / 45	-102.0 -102.0	0.14 (1)	10.00	J-H	-15 / 0	0.00 (1)
N-B	-832 / 0	0.0	0.0	0.09 (1)	J-F	0 / 851	0.14 (1)
H-F	-832 / 0	0.0	0.0	0.09 (1)			
N-M	0 / 10	-18.5	-18.5	0.01 (4)	10.00		
M-L	0 / 13	0.0	0.0	0.04 (1)	10.00		
L-C	0 / 48	0.0	0.0	0.05 (1)	10.00		
L-K	0 / 885	-18.5	-18.5	0.20 (1)	10.00		
K-J	0 / 385	-18.5	-18.5	0.20 (1)	10.00		
I-J	0 / 13	0.0	0.0	0.04 (1)	10.00		
J-E	0 / 48	0.0	0.0	0.05 (1)	10.00		
I-H	0 / 10	-18.5	-18.5	0.01 (4)	10.00		

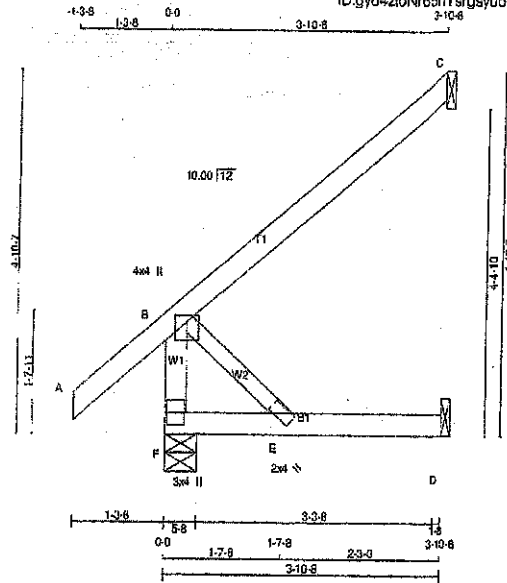


Structural component only
DWG# T-1923457

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405486	J8	24	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 10:43:00 2016 Page 1
ID:gyo4zloNr65nTgrgsyuoXHyIQ5o-HZdDtdL7BNauYWy5F9SbtY8hKfEN2Iyu9r7ye4g9



Scale = 1:27.0

TOTAL WEIGHT = 24 X 15 = 367 lb

LUMBER

N.L. & A. RULES

CHORDS SIZE

F - B 2x4 DRY No.2

A - C 2x4 DRY No.2

F - D 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

F 375 0 375 0 0 5-8 5-8

C 198 0 198 0 0 1-3 1-3

D 96 0 40 0 0 1-8 1-8

SEE MITEK STANDARD DETAIL B9779 IH FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE MAX/MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL

F 261 193/0 0/0 0/0 0/0 69/0 0/0

C 136 112/0 0/0 0/0 0/0 29/0 0/0

D 29 0/0 0/0 0/0 0/0 29/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. MAX. (PLF) CSI (LC) UNBRACED LENGTH FR-TO FR-TO

F-B -339/0 0.0 0.0 0.04 (1) 7.81 9-E 0/0 0.00 (1)

A-B 0/45 -102.0 -102.0 0.15 (5) 10.00

B-C 0/0 -102.0 -102.0 0.28 (1) 10.00

F-E 0/0 -18.5 -18.5 0.08 (4) 10.00

E-D 0/0 -18.5 -18.5 0.08 (4) 10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOY CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

85 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19')

CALCULATED VERT. DEFL.(LL) = L/369 (0.00')

ALLOWABLE DEFL.(TL) = L/360 (0.19')

CALCULATED VERT. DEFL.(TL) = L/369 (0.01')

CSI: TC=0.28/1.00 (B-C-1); BC=0.08/1.00 (D-E-4);

WB=0.00/1.00 (B-E-1); SS=0.12/1.00 (B-C-1)

DOL LUMBER=1.06 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (FS) (PL) (PL)

MAX MIN MAX MIN MAX MIN

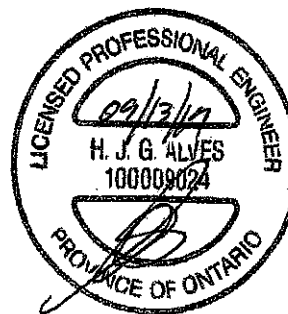
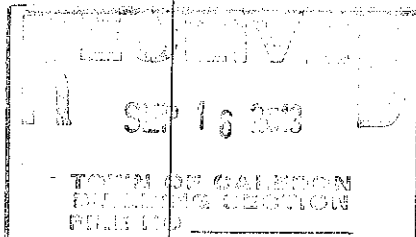
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

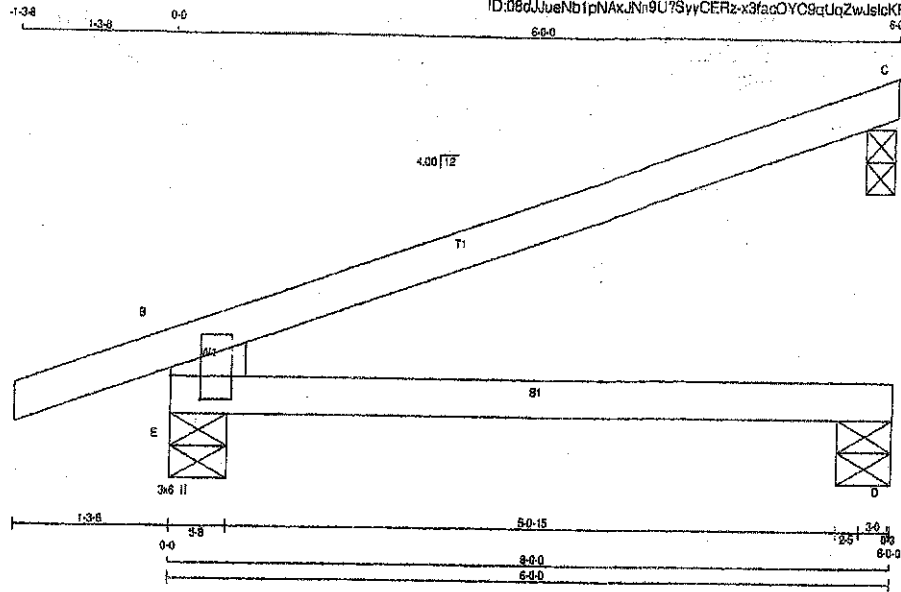
JSI GRIP = 0.26 (B) (INPUT = 0.80)

JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-1923438

JOB NAME 405490	TRUSS NAME J13	QUANTITY 24	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 5 May 10 2019 Mitel Industries, Inc. Fri Sep 13 10:56:25 2019 Page 1			
		ID:08dUueNb1pNAx.Nn9U?SyyCERz-x3facOYC9cUqZwJstKRTmCsBsXID1oi_nvXye4Qm			
		Scale = 1:15.6			



TOTAL WEIGHT = 24 X 16 = 392 lb

LUMBER

N.L.G.A. RULES
CHORDS SIZE
E - B 2x8
A - C 2x4
E - D 2x4

LUMBER
No.2
No.2
No.2

DESCR.
SPF
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
E	MT20	3.0	6.0	2.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	DOWN	BRG	BRG
	HORZ	HORZ	IN-SX	IN-SX
E	588	0	5-8	5-8
C	230	0	3-0	3-0
D	42	0	5-8	5-8

BEVELED PLATE OR SHM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
E	411	295 / 0	0 / 0	0 / 0	118 / 0	0 / 0
C	138	131 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
E-B	-619 / 0	0.0	0.0	7.81
A-B	0 / 22	-102.0	-102.0	10.00
B-C	-24 / 0	-102.0	-102.0	6.25
E-D	0 / 0	-18.5	-18.5	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF NBC 2010, NBC 2012
-CSA 086-09, CSA 086-14
-TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 3.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.63/1.00 (B-C-1), BC=0.18/1.00 (D-E-4),
WB=0.00/1.00 (n/a/0), SS=0.28/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

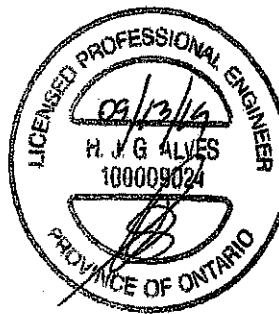
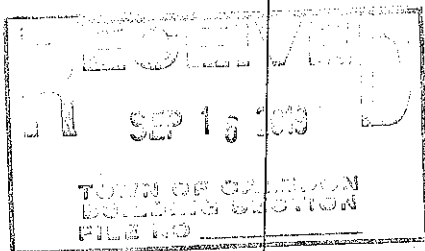
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (E) (INPUT = 0.30)
JSI METAL = 0.10 (E) (INPUT = 1.00)

Structural component only
DWG# T-1923453



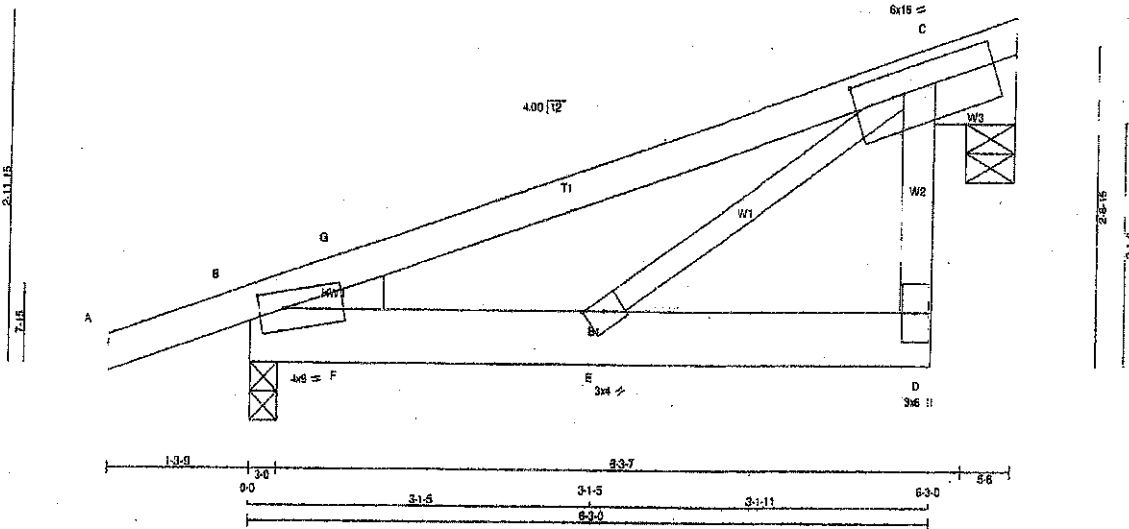
JOB NAME 405493	TRUSS NAME J14	QUANTITY 24	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.300 5 May 10 2019 Mitek Industries, Inc. Fri Sep 13 11:12:19 2019 Page 1

ID:08dJueNb1pNAxJNn9U?SyyCERz-GzQ822wue??QsJde9DyNhgYCC_CimEyn7UJ3gye4Eg

1-3-8 1-3-8 0-0 2-1-5 3-1-9 3-1-11 6-3-0
Scale: 1:17.5



TOTAL WEIGHT = 24 X 28 = 699 LB

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMBH1-m	MT20	4.0	9.0	1.75	2.75
C TMVWV1-1	MT20	6.0	18.0	2.50	5.50
D BMV+p	MT20	3.0	6.0		
E BMV+w	MT20	3.0	4.0	2.00	1.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
C	377	0	377	0	5-8	5-8	
B	513	0	513	0	3-0	3-0	2x4 L

SEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
C	265	181/0	0/0	0/0	0/0	84/0	0/0
B	359	259/0	0/0	0/0	0/0	100/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S): C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADINGS

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LG1 (FLP)	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO				
A-B	0 / 0	-102.0 -102.0	0.13 (1)	10.00	E-C	0 / 455	0.10 (1)	
B-G	-283 / 0	-102.0 -102.0	0.28 (1)	8.25	F-G	-560 / 0	0.00 (1)	
G-C	-394 / 0	-102.0 -102.0	0.33 (1)	8.25				
D-C	-6 / 24	0.0 0.0	0.01 (4)	7.81				
B-F	0 / 359	-16.5 -16.5	0.28 (1)	10.00				
F-E	0 / 359	-16.5 -16.5	0.28 (1)	10.00				
E-D	0 / 0	-16.5 -16.5	0.07 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/899 (0.02")

CSI: TO=0.33/1.00 (C-G:1), BC=0.28/1.00 (B-F:1), WB=0.10/1.00 (C-E:1), SS=0.24/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

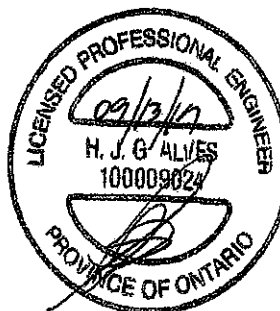
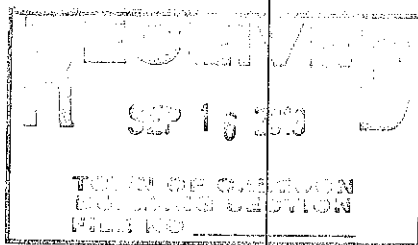
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	615 354	1857 785

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.33 (E) (INPUT = 0.90)
JSI METAL = 0.11 (E) (INPUT = 1.00)



Structural component only
DWG# T-1923458



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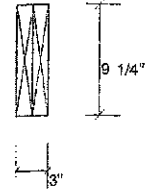
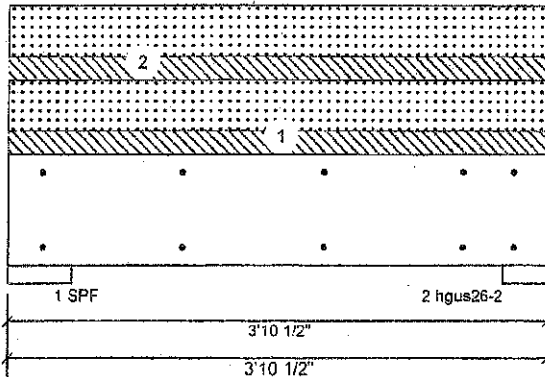
Client:
Project:
Address:

Date: 2/20/19
Designer:
Job Name: 405493
Project #:

Page 1 of 2

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal
Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015 / OBC 2012
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	185	401	0
2	0	174	376	0

Bearings and Factored Reactions

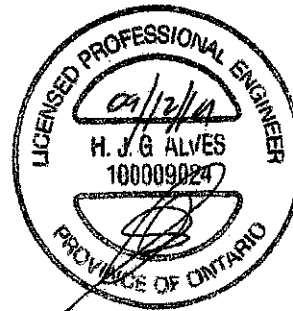
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	8%	232 / 602	833 L 1.25D+1.5S
2 - hgs2...	4.000"	11%	217 / 564	781 L 1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	536 ft-lb	2'	6039 ft-lb	0.089 (9%)	1.25D+1.5S	L
Unbraced	536 ft-lb	2'	5734 ft-lb	0.094 (9%)	1.25D+1.5S	L
Shear	669 lb	12"	3984 lb	0.168 (17%)	1.25D+1.5S	L
LL Defl inch	0.002 (L/21944)	2'	0.107 (L/360)	0.020 (2%)	S	L
TL Defl inch	0.003 (L/15009)	2'	0.107 (L/360)	0.020 (2%)	D+S	L

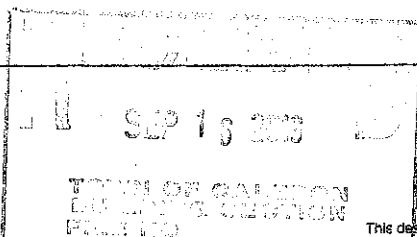
Design Notes

- 1 Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17923 459
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-11-0	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	



This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115





isDesign™

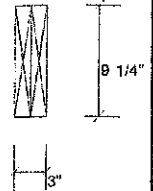
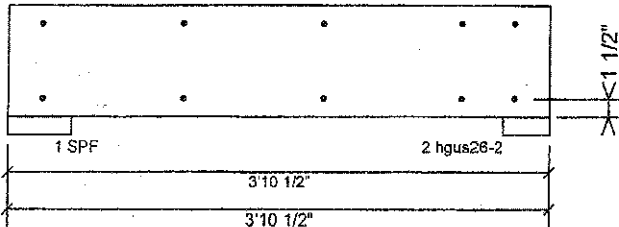
Client:
Project:
Address:

Date: 2/2019
Designer:
Job Name: 405493
Project #:

Page 2 of 2

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

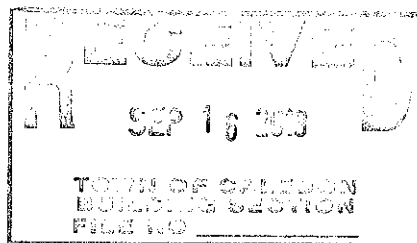
Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	78.6 %
Load	178.2 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

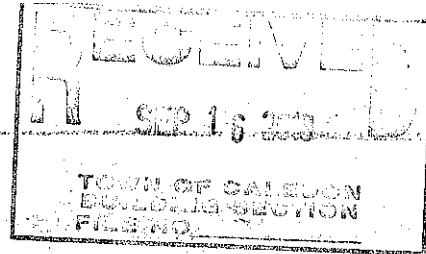


DWG NO. TAM 71923 469
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115
	TAMARACK LUMBER INC ALFA LUMBER GROUP

This design is valid until 12/11/2021





Simpson Strong-Tie® Wood Construction Connectors — Canadian Limit States Design

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



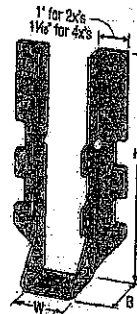
Double-Shear
Nailing
Top View



Double-Shear
Nailing
Side View;
Do not
bend tab



Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



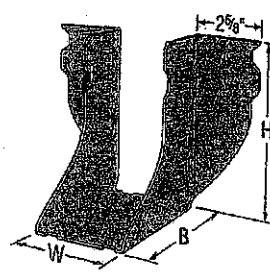
✓ LUS28



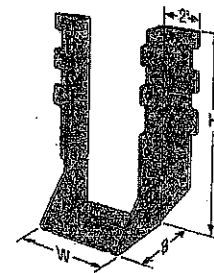
LU26L



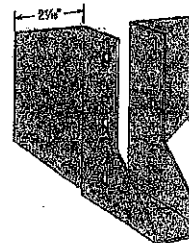
✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2

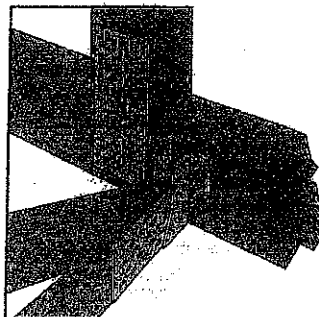


HHUS210-2



LJS26DS

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

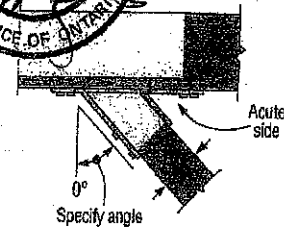
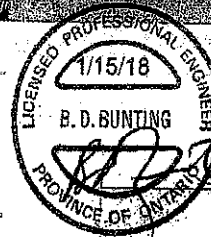
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

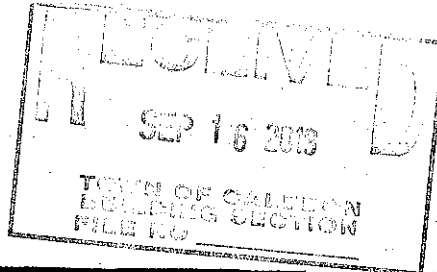
These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Plated Truss Connectors

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _a	Header	Joist		D-S-P		S-P-F	
									Uplift (K _u = 1.5)	Normal (K _n = 1.0)	Uplift (K _u = 1.5)	Normal (K _n = 1.0)
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Single 2x Sizes												
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d		710	1625	645	1155
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"		360	1020	320	725
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"		720	1605	645	1140
LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d		1420	2170	1290	1630
HUS26	16	1 1/8	5 1/4	3	3 1/8	(14) 16d	(8) 16d		2705	4940	2065	3875
LJS26DS	18	1 1/8	5	3 1/4	4 1/4	(18) 16d	(6) 16d		2055	4265	1480	4115
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d		2685	6625	2685	5700
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"		1140	2185	1020	1650
LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d		1420	2520	1290	1790
HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d		3605	5365	2675	4345
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(38) 16d	(12) 16d		3310	7675	3310	6900
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"		1140	2455	1020	1770
LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d		1420	2785	1290	2210

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. c_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.



Simpson Strong-Tie Wood Construction Connectors - Canadian Limit States Design

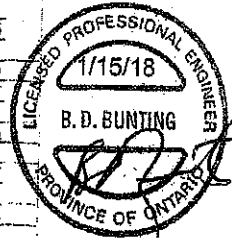
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _g	Header	Joist	U-P-F		S-P-F		
								Uplift	Normal	Uplift	Normal	
								(K ₁ = 2.15) lb.	(K ₂ = 2.00) lb.	(K ₁ = 1.15) lb.	(K ₂ = 1.00) lb.	
Double 2x Sizes												
LUS24-2	18	3 1/8	3 1/8	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435	
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920	
HHUS26-2	14	3 1/8	6 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205	
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355	
LUS28-2	18	3 1/8	7 1/8	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575	
HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(6) 16d	3765	8940	2675	6345	
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(28) 16d	(12) 16d	6070	12980	4310	9215	
LUS210-2	18	3 1/8	9 1/8	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195	
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000	
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270	
Triple 2x Sizes												
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355	
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215	
HHUS210-3	14	4 1/8	9 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	8965	
HGUS216-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400	
Quadruple 2x Sizes												
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355	
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215	
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210	
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400	
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(66) 16d	(20) 16d	7640	14995	5425	10645	
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645	
4x Sizes												
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920	
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2065	5205	
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355	
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575	
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(6) 16d	3765	8940	2675	6345	
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215	
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195	
HGUS410	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270	
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(66) 16d	(20) 16d	7640	14995	5425	10645	
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645	



Plated Truss Connectors

See footnotes on p. 258.

SEP 16 2013

TOWN OF GALELON
BUILDING SECTION

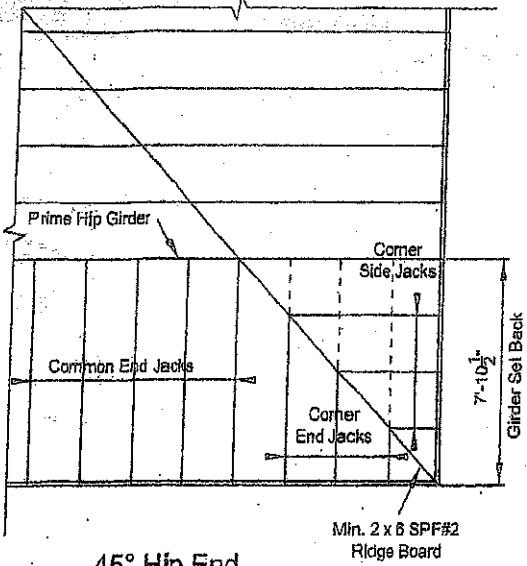
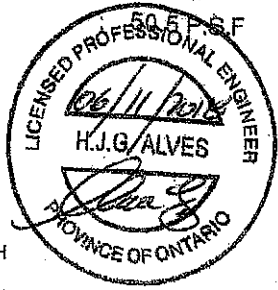
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

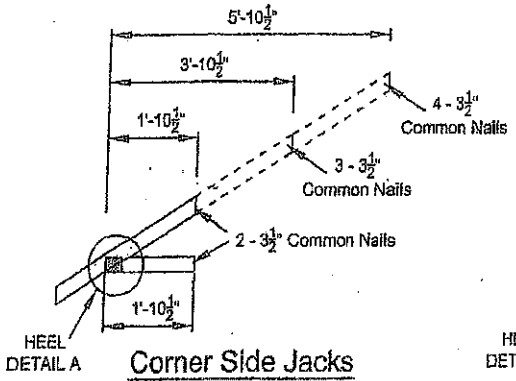
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

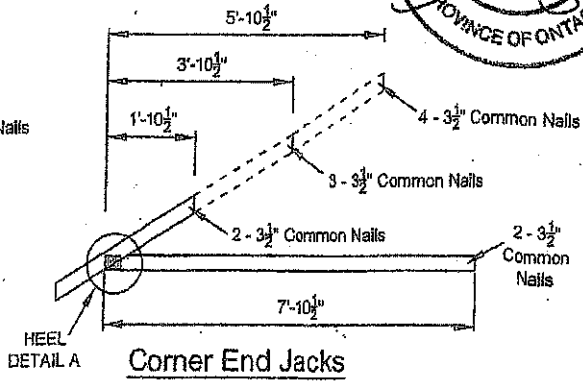
TOTAL LOAD



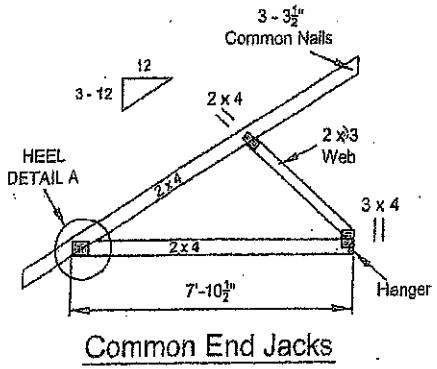
45° Hip End



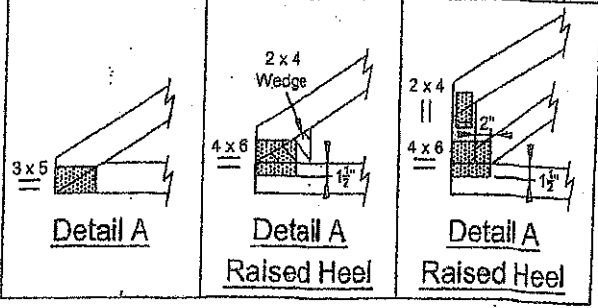
Corner Side Jacks



Corner End Jacks

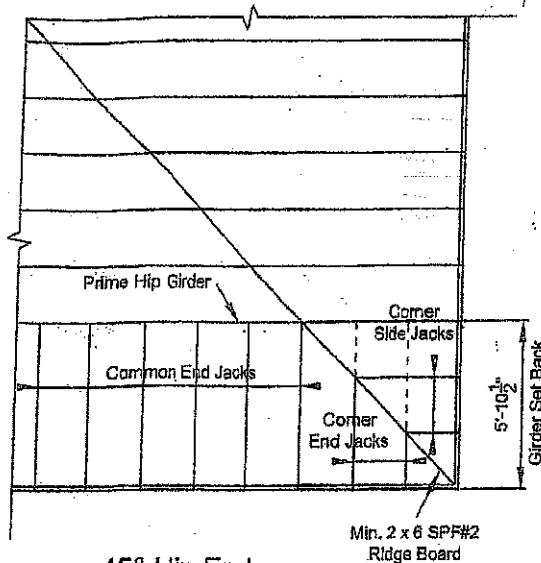
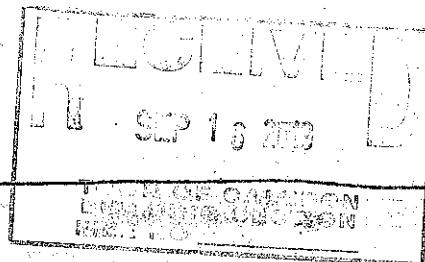


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



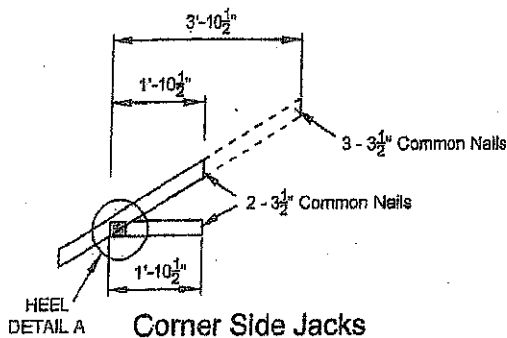
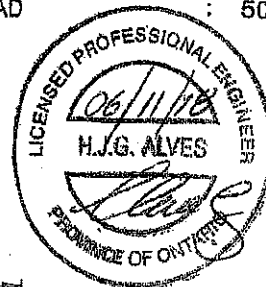
45° Hip End

LUMBER SPECIFICATION

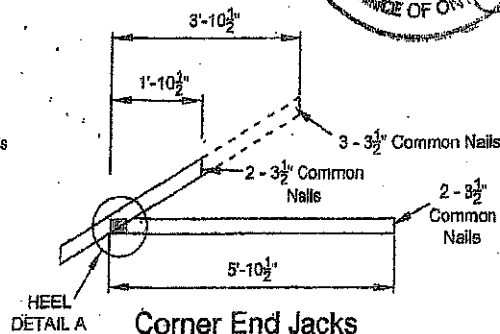
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

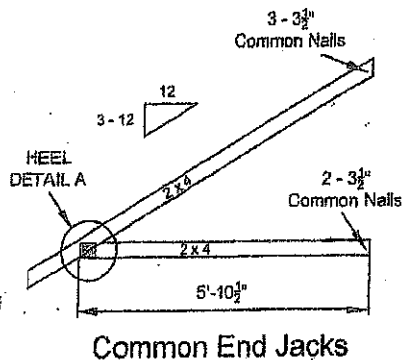
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 50.5 P.S.F.



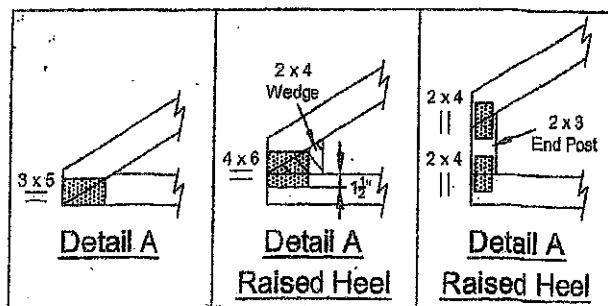
Corner Side Jacks



Corner End Jacks



Common End Jacks



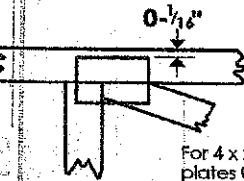
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



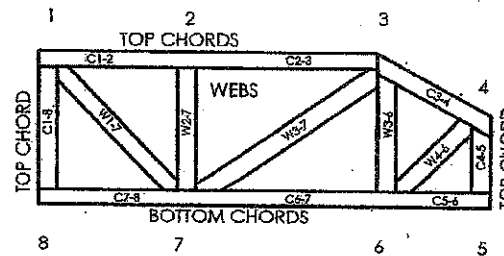
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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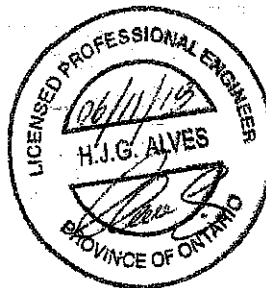
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

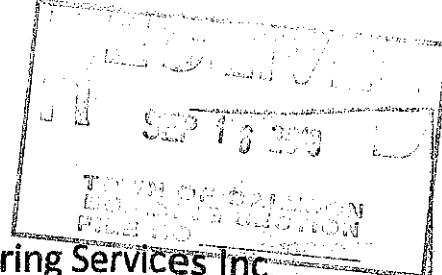
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455



RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

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