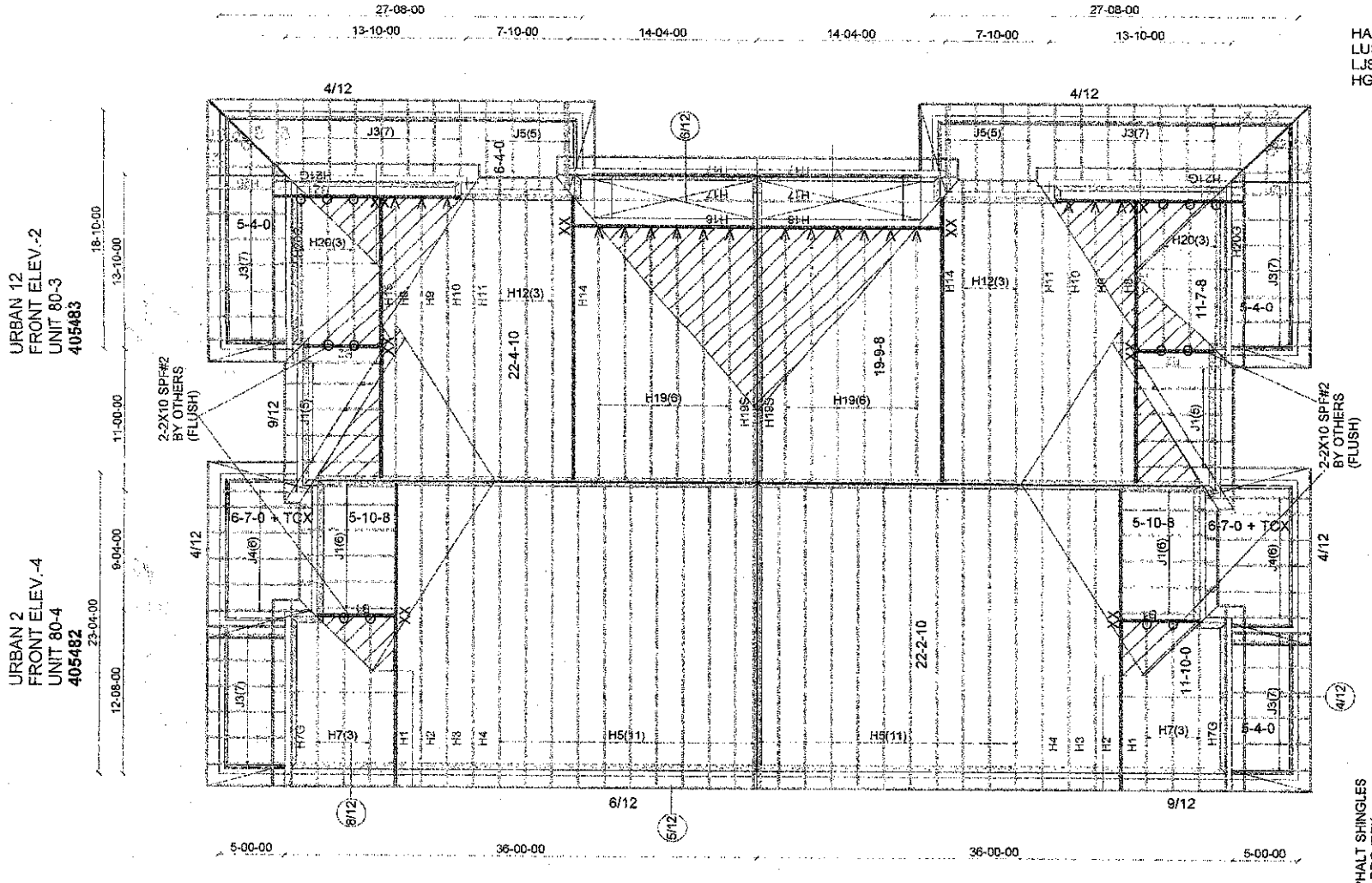
THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE



M12626

Job Track: 50033	Builder / Location: GREENPARK HOMES / CALEDON		Model / Elevation: BLOCK 80 / UNITS 80-1 - 80-4
Plan Log: 201703	Project: LAMBERTS LANE PH.2		
Layout ID: 405481	Date: 9/9/2019	Sales: Mario DiCano	Designer: AC

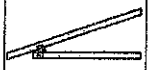
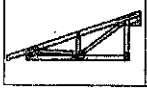
Mitek ver 8.3.0.234

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	201703
Project:	LAMBERT LANE PH.2	Layout ID:	405482
Location:	CALEDON	Ref #	
Model:	BLOCK 80	Page:	1 of 1
Lot #:		Date:	09/09/2019
Elevation:	UNITS 80-1, 80-4	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	H1 Half Hip Girder	5 /12	**22-02-10**	5-03-13	2 x 6	1-03-08	1-00-09 5-03-13	258.6 156.67		
	1	H2 Half Hip	5 /12	**22-02-10**	6-07-13	2 x 4	1-03-08	1-00-09 6-07-13	96.92 62.50		
	1	H3 Half Hip	5 /12	**22-02-10**	7-11-13	2 x 4	1-03-08	1-00-09 7-11-13	98.18 61.67		
	1	H4 Half Hip	5 /12	**22-02-10**	9-03-13	2 x 4	1-03-08	1-00-09 9-03-13	105.08 66.50		
	11	H5 Monopitch	5 /12	**22-02-10**	10-03-10	2 x 4	1-03-08	1-00-09 10-03-10	1051.67 654.50		
	3	H7 Common	8 /12	**11-10-00**	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	157.12 95.50		
	1	H7G GABLE	8 /12	**11-10-00**	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	48.73 30.83		
	6	J1 Jack-Open	8 /12	**5-10-08**	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	106.98 64.00		
	7	J3 Jack-Open	4 /12	**5-04-00**	2-06-03	2 x 4	1-03-08	7-15 2-02-04	103.45 74.67		
	6	J4 Jack-Open	4 /12	**6-07-00**	3-02-03	2 x 4	1-03-08	7-15 2-10-04	174.1 123.00		

TOTAL # TRUSS= **39** TOTAL BFT OF ALL TRUSSES= **1391.84** BFT. TOTAL WEIGHT OF ALL TRSSES **2200.84** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
1	Hardware	HGUS26-2	

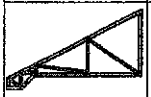
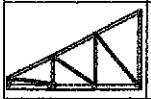
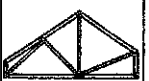
TOTAL NUMBER OF ITEMS= **3**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	201703
Project:	LAMBERT LANE PH.2	Layout ID:	405483
Location:	CALEDON	Ref #	
Model:	BLOCK 80	Page:	1 of 2
Lot #:		Date:	09/09/2019
Elevation:	UNITS 80-2, 80-3	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

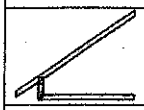
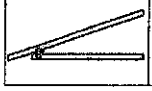
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	H8 Half Hip	5 /12	22-00-02	5-03-13	2 x 4		1-02-07 5-03-13	87.8 55.33		
	1	H9 Half Hip	5 /12	22-00-02	6-07-13	2 x 4		1-02-07 6-07-13	94.89 61.17		
	1	H10 Half Hip	5 /12	22-00-02	7-11-13	2 x 4		1-02-07 7-11-13	96.28 60.83		
	1	H11 Half Hip	5 /12	22-04-10	9-03-13	2 x 4		1-00-09 9-03-13	103.89 65.17		
	3	H12 Monopitch	5 /12	22-04-10	10-04-08	2 x 4	1-03-08	1-00-09 10-04-08	288.88 178.50		
	1 2-ply	H14 Monopitch Girder	5 /12	22-04-10	10-04-08	2 x 6		1-00-09 10-04-08	278.01 173.00		
	1 2-ply	H15 Hip Girder	5 /12	22-00-02	5-03-13	2 x 4 2 x 6		1-02-07 1-03-10	214.38 133.00		
	2	H17 Monopitch Structural Gable	6 /12 12 /12	13-08-10	8-00-05	2 x 4		1-02-00 6-08-05	115.24 77.33		
	1 2-ply	H18 Jack-Closed Girder	6 /12	13-08-10	8-00-05	2 x 4 2 x 6		1-02-00 8-00-05	141.36 88.00		
	6	H19 Common	5 /12	19-09-08	10-04-08	2 x 4		2-01-09 10-04-08	531.48 330.00		
	1	H19S GABLE	5 /12	18-09-08	10-04-08	2 x 4		2-06-09 10-04-08	101.77 64.33		
	3	H20 Common	8 /12	11-07-08	5-08-13	2 x 4		1-04-13 2-03-13	144.13 95.00		
	1	H20G GABLE	8 /12	11-07-08	5-08-13	2 x 4	1-03-08	1-04-13 2-03-13	52.6 34.33		
	1 2-ply	H21 Common Girder	8 /12	13-00-00	5-08-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	137.09 85.67		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	201703
Project:	LAMBERT LANE PH.2	Layout ID:	405483
Location:	CALEDON	Ref #	
Model:	BLOCK 80	Page:	2 of 2
Lot #:		Date:	09/09/2019
Elevation:	UNITS 80-2, 80-3	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	H21G GABLE	8 /12	13-00-00	5-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	57.01 36.33		
	1	H26 Monopitch Girder	4 /12	4-07-00	2-03-03	2 x 4	1-03-08	7-15 2-02-04	17.85 12.67		
	5	J1 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	89.15 53.33		
	14	J3 Jack-Open	4 /12	5-04-00	2-06-03	2 x 4	1-03-08	7-15 2-02-04	206.9 149.33		
	5	J5 Jack-Open	4 /12	6-04-00	2-10-03	2 x 4	1-03-08	7-15 2-06-04	85.6 60.00		

TOTAL # TRUSS= **54** TOTAL BFT OF ALL TRUSSES= **1813.32** BFT. TOTAL WEIGHT OF ALL TRSSES **2844.31** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LUS24	
9	Hardware	LJS26DS	
3	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **17**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H1	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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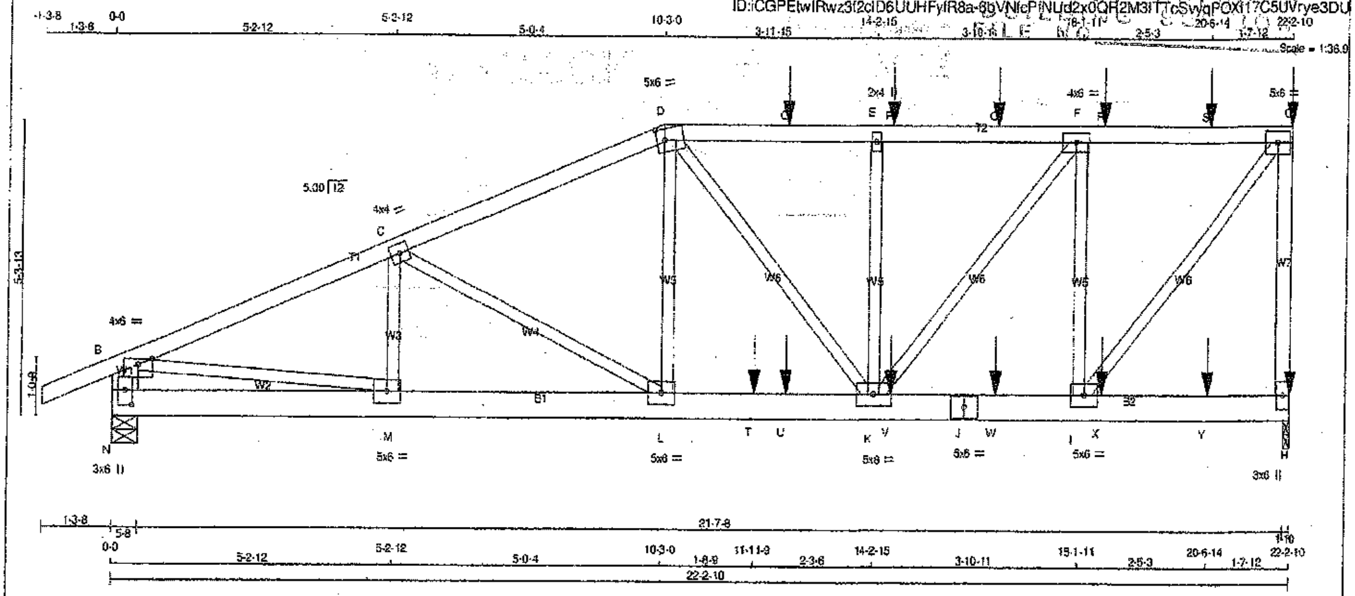

TOTAL WEIGHT = 4 X 110 = 439 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	SIDE(61.0)
G- H	1	12	TOP
N- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- J	2	12	SIDE(183.1)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.50	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I, L, M		-				
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	6.0	3.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2745	0	2745	0	0	1-10	1-10
N	2223	0	2223	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1931	1327 / 0	0 / 0	0 / 0	0 / 0	604 / 0	0 / 0
N	1560	1094 / 0	0 / 0	0 / 0	0 / 0	466 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-102.0	-102.0	0.07 (1)	10.00
B- C	-3551 / 0	-102.0	-102.0	0.30 (1)	4.69
C- D	-3401 / 0	-102.0	-102.0	0.29 (1)	4.78
D- O	-3024 / 0	-102.0	-102.0	0.20 (1)	5.11
O- E	-3024 / 0	-102.0	-102.0	0.20 (1)	5.11
E- P	-3024 / 0	-102.0	-102.0	0.23 (1)	5.07
P- Q	-3024 / 0	-102.0	-102.0	0.23 (1)	5.07
Q- F	-3024 / 0	-102.0	-102.0	0.23 (1)	5.07
F- R	-1854 / 0	-102.0	-102.0	0.21 (1)	6.13
R- S	-1854 / 0	-102.0	-102.0	0.21 (1)	6.13
S- G	-1854 / 0	-102.0	-102.0	0.21 (1)	6.13
H- G	-2643 / 0	0.0	0.0	0.61 (1)	7.00
N- B	-2138 / 0	0.0	0.0	0.07 (1)	7.81
N- M	0 / 0	-18.5	-18.5	0.07 (1)	10.00
M- L	0 / 3298	-18.5	-18.5	0.35 (1)	10.00
L- T	0 / 3139	-18.5	-18.5	0.52 (1)	10.00
T- U	0 / 3139	-18.5	-18.5	0.52 (1)	10.00
U- K	0 / 3139	-18.5	-18.5	0.52 (1)	10.00
K- V	0 / 1854	-18.5	-18.5	0.21 (1)	10.00
V- J	0 / 1854	-18.5	-18.5	0.21 (1)	10.00
J- W	0 / 1854	-18.5	-18.5	0.21 (1)	10.00
W- I	0 / 1854	-18.5	-18.6	0.21 (1)	10.00
I- X	0 / 0	-18.5	-18.5	0.04 (4)	10.00
X- Y	0 / 0	-18.5	-18.5	0.04 (4)	10.00
Y- H	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-466 / 0	0.05 (1)
C- L	-206 / 0	0.06 (1)
L- D	0 / 1040	0.13 (1)
I- G	0 / 2913	0.36 (1)
D- K	-184 / 0	0.06 (1)
I- F	-2208 / 0	0.45 (1)
K- E	-672 / 0	0.14 (1)
K- F	0 / 1890	0.23 (1)
B- M	0 / 3324	0.41 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	22-2-10	-164	-164	---	FRONT	VERT	TOTAL	---	C1
H	22-2-10	-35	-35	---	FRONT	VERT	TOTAL	---	C1
O	12-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
P	14-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
Q	16-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
R	18-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
S	20-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
T	11-11-9	-1206	-1206	---	FRONT	VERT	TOTAL	---	C1
U	12-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1
V	14-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1
W	16-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1
X	18-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1
Y	20-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.61/1.00 (G-H:1) , BC=0.52/1.00 (K-L:1) , WB=0.45/1.00 (F-I:1) , SSI=0.26/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1367	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.41 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J/G ALVES
100009824
PROVINCE OF ONTARIO

Structural component only
DWG# T1923489

CONTINUED ON PAGE 2

THIS STRUCTURE MUST BE CONSTRUCTED ... EXCEED THE PROVISIONS OF ... ONTARIO BUILDING CODE

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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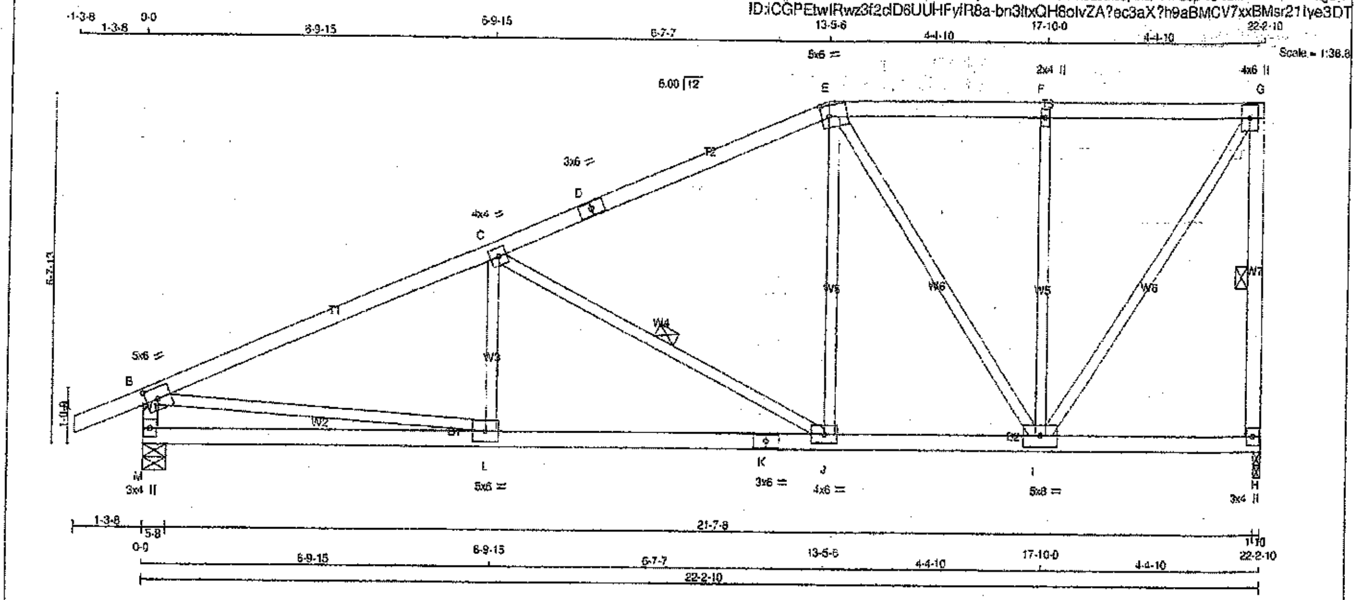

TOTAL WEIGHT = 2 X 97 = 194 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY. SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.50	1.75
F	TMW+w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1339	0	1339	0	0	1-10	1-10
M	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00
B-C	-2068 / 0	-102.0	-102.0	0.70 (1)	3.85
C-D	-1244 / 0	-102.0	-102.0	0.62 (1)	4.87
D-E	-1244 / 0	-102.0	-102.0	0.62 (1)	4.87
E-F	-786 / 0	-102.0	-102.0	0.25 (1)	6.25
F-G	-786 / 0	-102.0	-102.0	0.25 (1)	6.25
H-G	-1303 / 0	0.0	0.0	0.26 (1)	5.66
M-B	-1423 / 0	0.0	0.0	0.14 (1)	6.84
M-L	0 / 0	-18.5	-18.5	0.21 (4)	10.00
L-K	0 / 1938	-18.5	-18.5	0.40 (1)	10.00
K-J	0 / 1938	-18.5	-18.5	0.40 (1)	10.00
J-I	0 / 1127	-18.5	-18.5	0.25 (1)	10.00
I-H	0 / 0	-18.5	-18.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-72 / 96	0.03 (4)
C-J	-929 / 0	0.37 (1)
J-E	0 / 542	0.12 (1)
E-I	-604 / 0	0.70 (1)
I-F	-552 / 0	0.40 (1)
I-G	0 / 1376	0.31 (1)
B-L	0 / 1949	0.44 (1)

LICENSED PROFESSIONAL ENGINEER
04/13/10
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.70/1.00 (B-C:1) , BC=0.40/1.00 (J-L:1) , WB=0.70/1.00 (E-I:1) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.58 (K) (INPUT = 1.00)

Structural component only
DWG# T-1923490

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H3	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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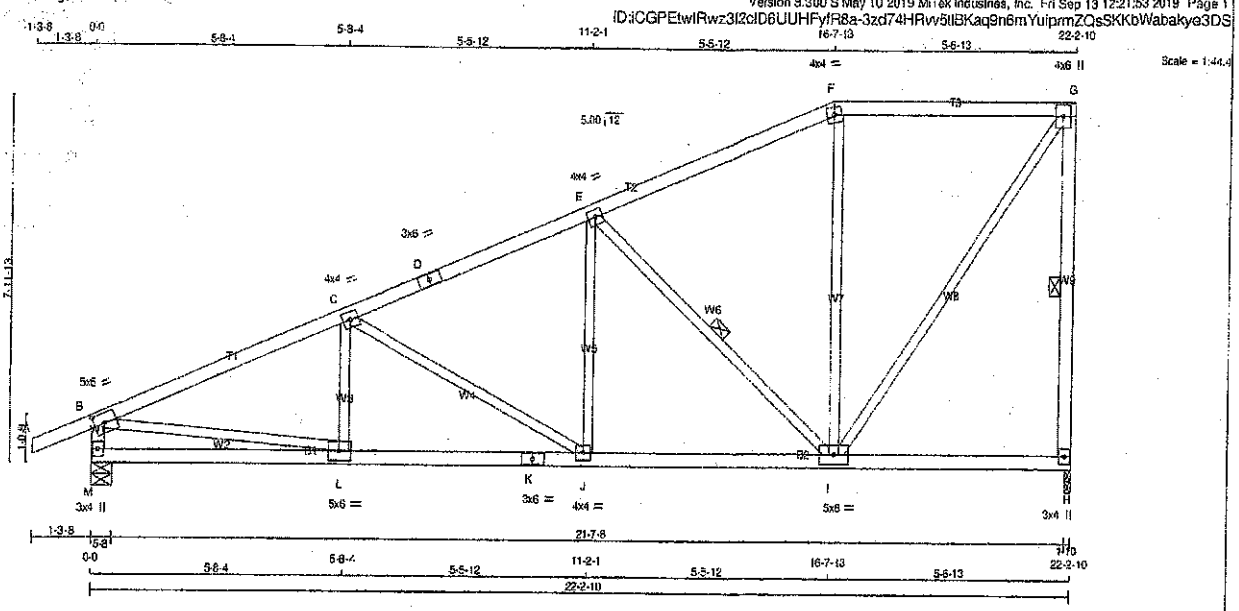

TOTAL WEIGHT = 2 X 98 = 196 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1339	0	1339	0	0	1-10	1-10
M	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 28	-102.0	-102.0	0.13 (1)	10.00
B-C	-2067 / 0	-102.0	-102.0	0.45 (1)	4.26
C-D	-1561 / 0	-102.0	-102.0	0.36 (1)	4.90
D-E	-1561 / 0	-102.0	-102.0	0.36 (1)	4.90
E-F	-805 / 0	-102.0	-102.0	0.35 (1)	6.25
F-G	-721 / 0	-102.0	-102.0	0.41 (1)	6.25
H-G	-1298 / 0	0.0	0.0	0.37 (1)	5.66
M-B	-1430 / 0	0.0	0.0	0.14 (1)	6.82
M-L	0 / 0	-18.5	-18.5	0.13 (4)	10.00
L-K	0 / 1927	-18.5	-18.5	0.38 (1)	10.00
K-J	0 / 1927	-18.5	-18.5	0.36 (1)	10.00
J-I	0 / 1441	-18.5	-18.5	0.30 (1)	10.00
I-H	0 / 0	-18.5	-18.5	0.14 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-143 / 51	0.03 (1)
C-J	-559 / 0	0.40 (1)
J-E	0 / 376	0.08 (1)
E-I	-997 / 0	0.40 (1)
I-F	-207 / 13	0.25 (1)
I-G	0 / 1245	0.28 (1)
B-L	0 / 1943	0.44 (1)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	5.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.36/1.00 (J-L:1) , WB=0.44/1.00 (B-L:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.57 (K) (INPUT = 1.00)

Structural component only
DWG# T-1923491

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H4	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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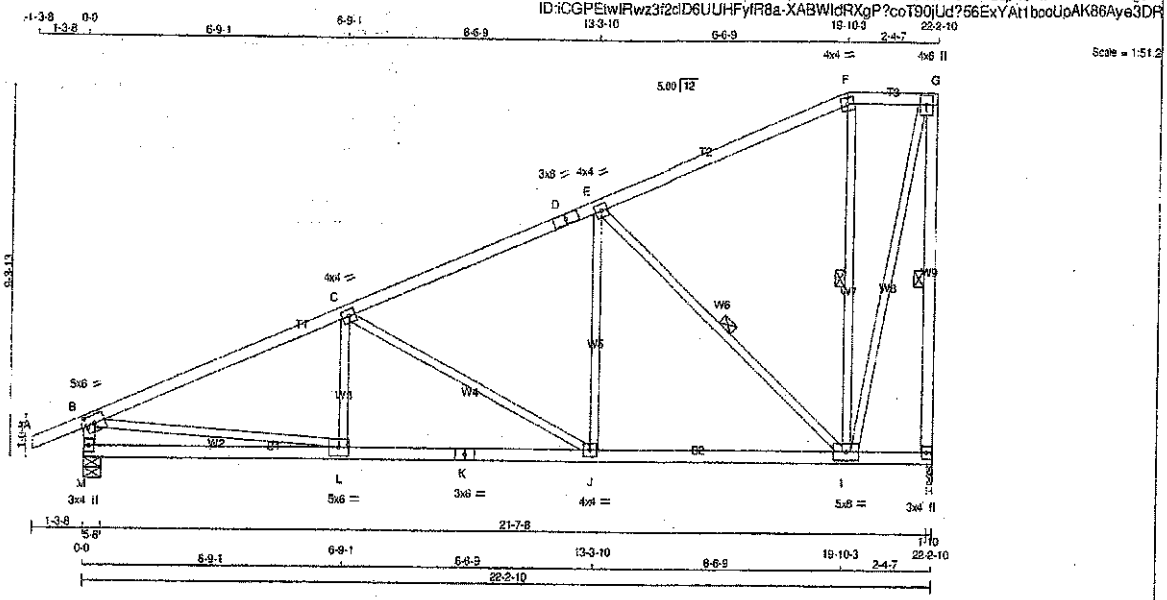

TOTAL WEIGHT = 2 X 105 = 210 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	5.0		
L	BMWW-t	MT20	5.0	8.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1339	0	1339	0	0	1-10	1-10
M	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-I, F-I,

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	L-C	-79 / 88	0.03 (4)
B-C	-2042 / 0	-102.0	-102.0	0.65 (1)	3.99	C-J	-787 / 0	0.87 (1)
C-D	-1318 / 0	-102.0	-102.0	0.50 (1)	5.00	J-E	0 / 505	0.11 (1)
D-E	-1318 / 0	-102.0	-102.0	0.50 (1)	5.00	E-I	-1252 / 0	0.73 (1)
E-F	-356 / 0	-102.0	-102.0	0.51 (1)	6.25	I-F	-251 / 0	0.15 (1)
F-G	-304 / 0	-102.0	-102.0	0.07 (1)	6.25	I-G	0 / 1249	0.28 (1)
H-G	-1331 / 0	0.0	0.0	0.54 (1)	5.82	B-L	0 / 1920	0.43 (1)
M-B	-1423 / 0	0.0	0.0	0.14 (1)	6.84			
M-L	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
L-K	0 / 1908	-18.5	-18.5	0.40 (1)	10.00			
K-J	0 / 1908	-18.5	-18.5	0.40 (1)	10.00			
J-I	0 / 1217	-18.5	-18.5	0.29 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.65/1.00 (B-C:1) , BC=0.40/1.00 (J-L:1) , WB=0.87/1.00 (C-J:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1867	788	1987	1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.83 (K) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. KLOES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923492

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H5	22	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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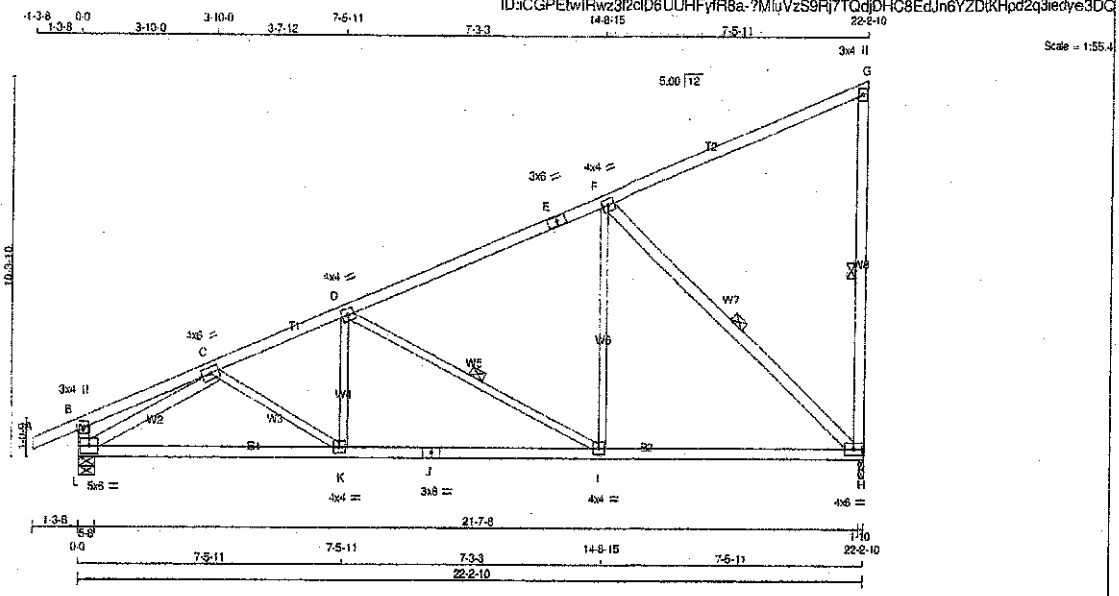

TOTAL WEIGHT = 22 X 100 = 2203 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TMWW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BMWW-t	MT20	4.0	4.0	
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	4.0	4.0	
L	BMVW1-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1339	0	1339	0	0	1-10	1-10
L	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	K- D	0 / 142	0.05 (4)
B- C	0 / 8	-102.0	-102.0	0.20 (1)	10.00	D- I	-841 / 0	0.39 (1)
C- D	-1944 / 0	-102.0	-102.0	0.39 (1)	4.50	I- F	0 / 546	0.12 (1)
D- E	-1140 / 0	-102.0	-102.0	0.69 (1)	4.86	F- H	-1454 / 0	0.76 (1)
E- F	-1140 / 0	-102.0	-102.0	0.69 (1)	4.86	C- K	0 / 54	0.02 (4)
F- G	-32 / 0	-102.0	-102.0	0.70 (1)	6.25	L- C	-2122 / 0	0.50 (1)
H- G	-298 / 0	0.0	0.0	0.16 (1)	6.25			
L- B	-308 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 1808	-18.5	-18.5	0.42 (1)	10.00			
K- J	0 / 1812	-18.5	-18.5	0.43 (1)	10.00			
J- I	0 / 1812	-18.5	-18.5	0.43 (1)	10.00			
I- H	0 / 1069	-18.5	-18.5	0.32 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.70/1.00 (F-G:1) , BC=0.43/1.00 (I-K:1) , WB=0.76/1.00 (F-H:1) , SSI=0.33/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (F) (INPUT = 0.90)
JSI METAL = 0.57 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009027
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923493

RECEIVED
SEP 16 2019
TOWN OF OAKVILLE
BUILDING SERVICES
FILE NO.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H7	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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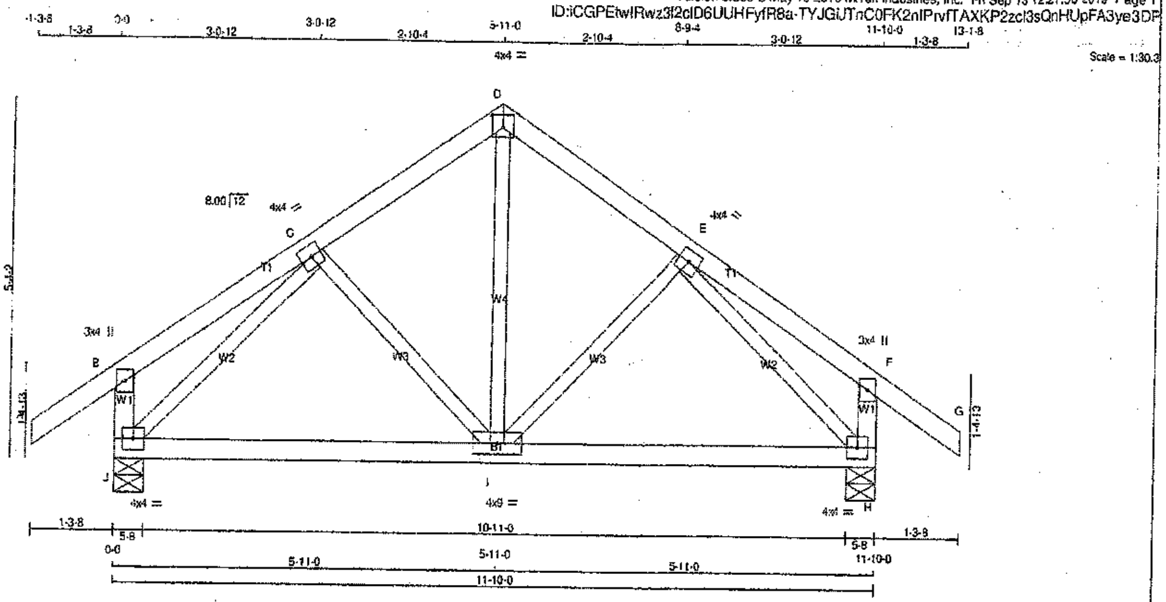

TOTAL WEIGHT = 6 X 52 = 314 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	853	0	853	0	0	5-8	5-8
H	853	0	853	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	598	423 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
H	598	423 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.0	-102.0	0.14 (1)	10.00
B- C	0 / 18	-102.0	-102.0	0.14 (1)	10.00
C- D	-540 / 0	-102.0	-102.0	0.11 (1)	6.25
D- E	-540 / 0	-102.0	-102.0	0.11 (1)	6.25
E- F	0 / 18	-102.0	-102.0	0.14 (1)	10.00
F- G	0 / 39	-102.0	-102.0	0.14 (1)	10.00
J- B	-257 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-257 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 536	-18.5	-18.5	0.22 (4)	10.00
I- H	0 / 536	-18.5	-18.5	0.22 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 342	0.08 (1)
I- E	-145 / 0	0.04 (1)
C- I	-145 / 0	0.04 (1)
J- C	-770 / 0	0.22 (1)
E- H	-770 / 0	0.22 (1)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.22/1.00 (I-J:4) , WB=0.22/1.00 (C-J:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)

Structural component only
DWG# T-1923494

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H7G	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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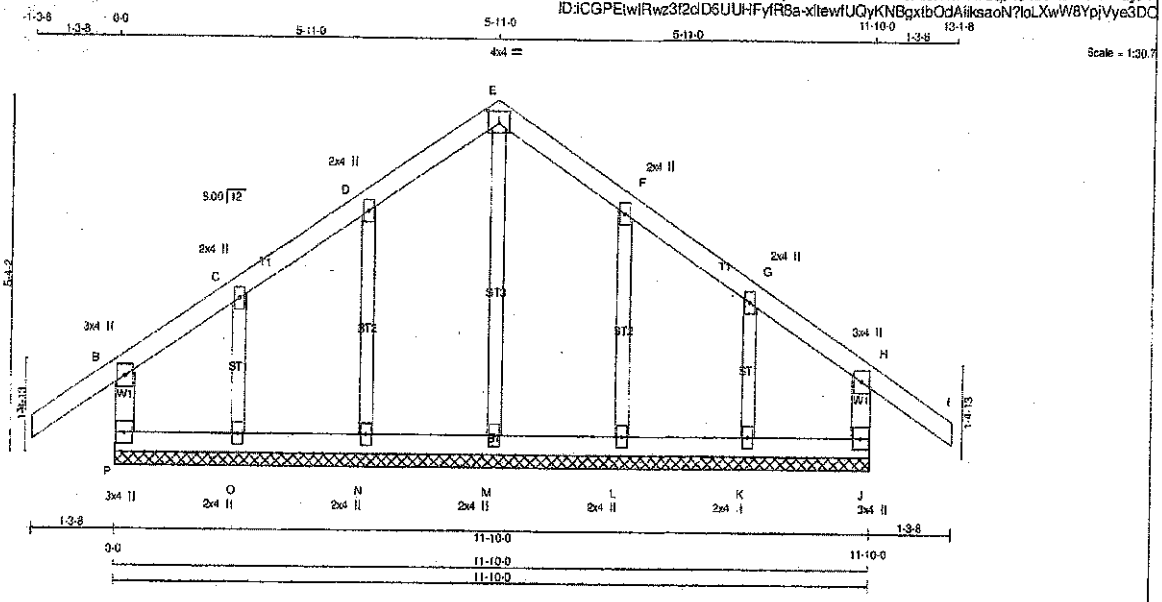

TOTAL WEIGHT = 2 X 49 = 97 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N, O						
K	BMW1+w	MT20	2.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
P-B	-241 / 0	0.0	0.0	0.04 (1)	7.81	M-E	-250 / 0	0.10 (1)
A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00	N-D	-217 / 0	0.05 (1)
B-C	0 / 15	-102.0	-102.0	0.09 (1)	10.00	O-C	-161 / 0	0.03 (1)
C-D	0 / 39	-102.0	-102.0	0.06 (1)	10.00	L-F	-217 / 0	0.05 (1)
D-E	0 / 38	-102.0	-102.0	0.06 (1)	10.00	K-G	-161 / 0	0.03 (1)
E-F	0 / 38	-102.0	-102.0	0.06 (1)	10.00			
F-G	0 / 39	-102.0	-102.0	0.06 (1)	10.00			
G-H	0 / 15	-102.0	-102.0	0.09 (1)	10.00			
H-I	0 / 39	-102.0	-102.0	0.14 (1)	10.00			
J-H	-241 / 0	0.0	0.0	0.04 (1)	7.81			
P-O	-23 / 0	-18.5	-18.5	0.02 (4)	6.25			
O-N	-29 / 0	-18.5	-18.5	0.02 (4)	6.25			
N-M	-33 / 0	-18.5	-18.5	0.01 (4)	6.25			
M-L	-33 / 0	-18.5	-18.5	0.01 (4)	6.25			
L-K	-29 / 0	-18.5	-18.5	0.02 (4)	6.25			
K-J	-23 / 0	-18.5	-18.5	0.02 (4)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	5.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (K-L:4) , WB=0.10/1.00 (E-M:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (E) (INPUT = 0.90)
JSI METAL = 0.11 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923495

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H8	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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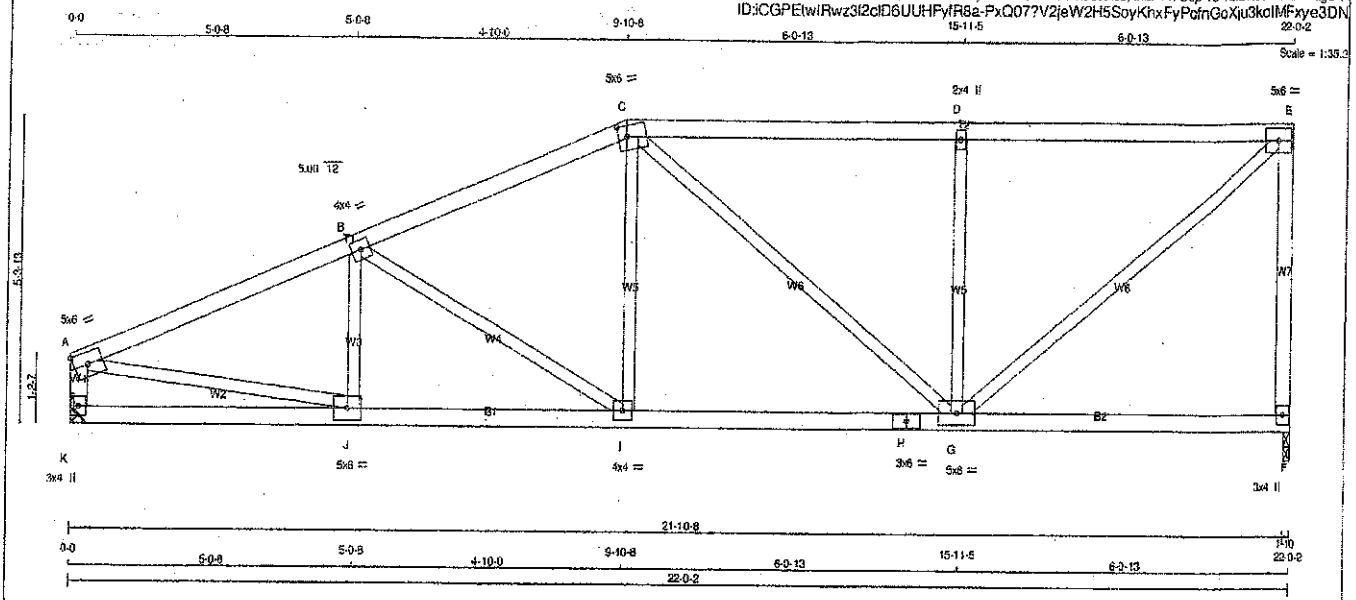


TOTAL WEIGHT = 2 X 88 = 176 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1326	0	1326	0	0	1-10	1-10
K	1326	0	1326	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0
K	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

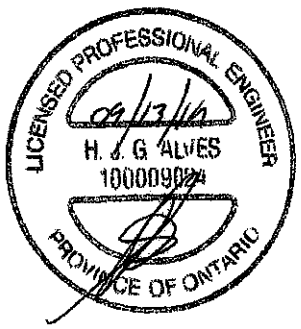
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1950 / 0	-102.0	-102.0	0.49 (1)	4.27
B-C	-1584 / 0	-102.0	-102.0	0.46 (1)	4.66
C-D	-1267 / 0	-102.0	-102.0	0.70 (1)	4.60
D-E	-1267 / 0	-102.0	-102.0	0.71 (1)	4.60
F-E	-1280 / 0	0.0	0.0	0.63 (1)	7.13
K-A	-1284 / 0	0.0	0.0	0.13 (1)	7.11
K-J	0 / 0	-18.5	-18.5	0.09 (4)	10.00
J-I	0 / 1820	-18.5	-18.5	0.35 (1)	10.00
I-H	0 / 1448	-18.5	-18.5	0.31 (1)	10.00
H-G	0 / 1448	-18.5	-18.5	0.31 (1)	10.00
G-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-B	-232 / 12	0.05 (1)
B-I	-444 / 0	0.25 (1)
I-C	0 / 332	0.07 (1)
C-G	-295 / 0	0.29 (1)
G-D	-767 / 0	0.32 (1)
G-E	0 / 1641	0.37 (1)
A-J	0 / 1849	0.42 (1)



THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF [illegible] BUILDING CODE

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	9.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.71/1.00 (D-E:1) , BC=0.35/1.00 (A-J:1) , WB=0.42/1.00 (A-J:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (G) (INPUT = 0.90)
JSI METAL = 0.50 (A) (INPUT = 1.00)

Structural component only
DWG# T-1923496

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H9	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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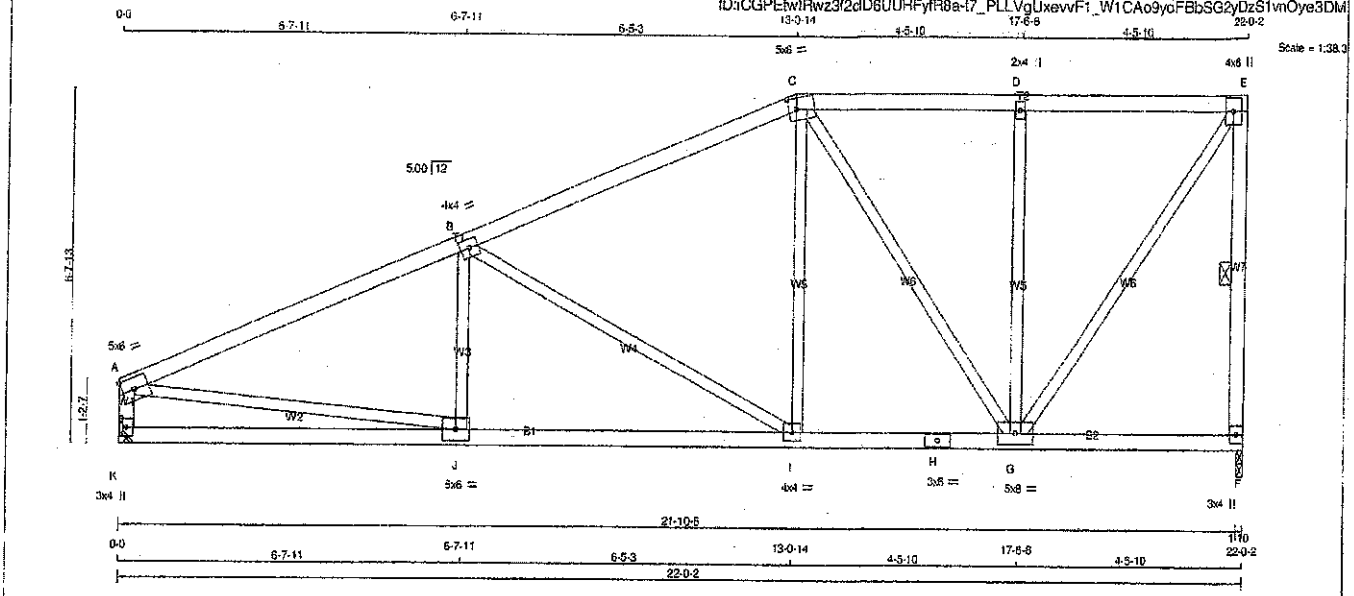

TOTAL WEIGHT = 2 X 95 = 190 lb (M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.50	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1326	0	1326	0	0	1-10	1-10
K	1326	0	1326	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0
K	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1961 / 0	-102.0	-102.0	0.65 (1)		4.03
B-C	-1232 / 0	-102.0	-102.0	0.58 (1)		4.96
C-D	-789 / 0	-102.0	-102.0	0.26 (1)		6.25
D-E	-790 / 0	-102.0	-102.0	0.26 (1)		6.25
F-E	-1290 / 0	0.0	0.0	0.26 (1)		5.68
K-A	-1275 / 0	0.0	0.0	0.13 (1)		7.13
K-J	0 / 0	-18.5	-18.5	0.19 (4)		10.00
J-I	0 / 1838	-18.5	-18.5	0.38 (1)		10.00
I-H	0 / 1116	-18.5	-18.5	0.24 (1)		10.00
H-G	0 / 1116	-18.5	-18.5	0.24 (1)		10.00
G-F	0 / 0	-18.5	-18.5	0.08 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-B	-114 / 80	0.03 (1)
B-I	-837 / 0	0.94 (1)
I-C	0 / 512	0.12 (1)
C-G	-571 / 0	0.68 (1)
G-D	-563 / 0	0.41 (1)
G-E	0 / 1365	0.31 (1)
A-J	0 / 1855	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT. CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.65/1.00 (A-B:1) , BC=0.38/1.00 (I-J:1) , WB=0.94/1.00 (B-I:1) , SSI=0.30/1.00 (A-B:t)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (G) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/13/15
H. J. G ALVES
100008024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923497

RECEIVED
SEP 16 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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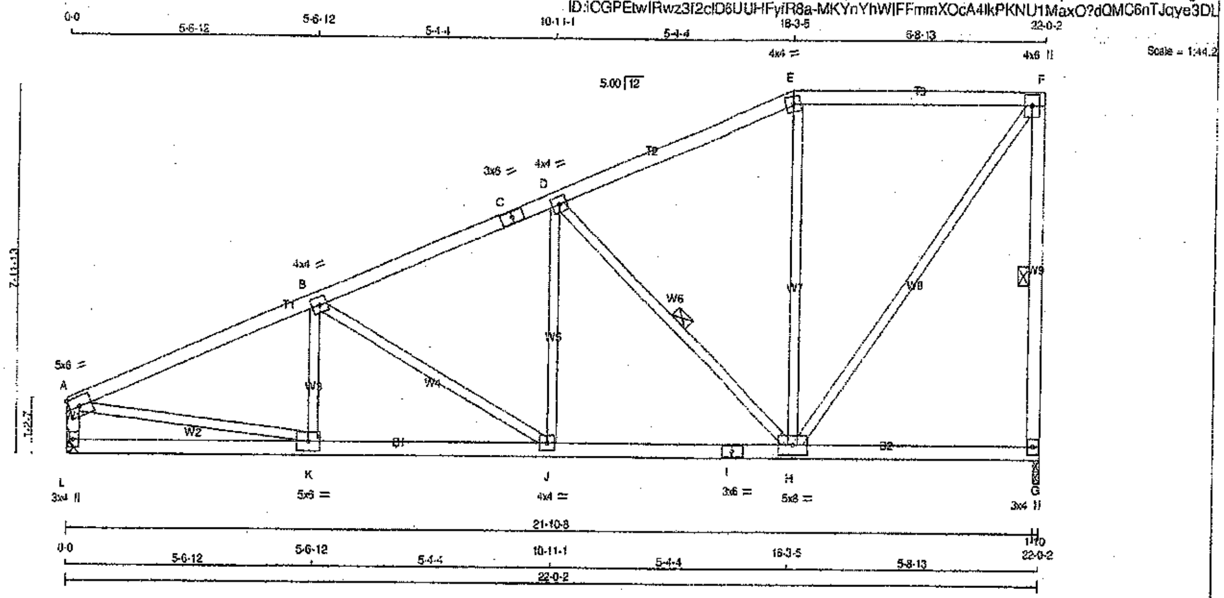

TOTAL WEIGHT = 2 X 96 = 193 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1326	0	1326	0	0	1-10	1-10
L	1326	0	1326	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0
L	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1945 / 0	-102.0	-102.0	0.42 (1)	4.40	K- B	-185 / 36	0.04 (1)
B- C	-1515 / 0	-102.0	-102.0	0.34 (1)	4.97	B- J	-485 / 0	0.34 (1)
C- D	-1515 / 0	-102.0	-102.0	0.34 (1)	4.97	J- D	0 / 345	0.08 (1)
D- E	-811 / 0	-102.0	-102.0	0.34 (1)	6.25	D- H	-945 / 0	0.38 (1)
E- F	-726 / 0	-102.0	-102.0	0.44 (1)	6.25	H- E	-208 / 13	0.25 (1)
G- F	-1284 / 0	0.0	0.0	0.37 (1)	5.69	H- F	0 / 1230	0.28 (1)
L- A	-1281 / 0	0.0	0.0	0.13 (1)	7.13	A- K	0 / 1837	0.41 (1)
L- K	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
K- J	0 / 1814	-18.5	-18.5	0.34 (1)	10.00			
J- I	0 / 1398	-18.5	-18.5	0.30 (1)	10.00			
I- H	0 / 1398	-18.5	-18.5	0.30 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	42.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.44/1.00 (E-F:1) , BC=0.34/1.00 (J-K:1) , WB=0.41/1.00 (A-K:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (K) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923498

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H11	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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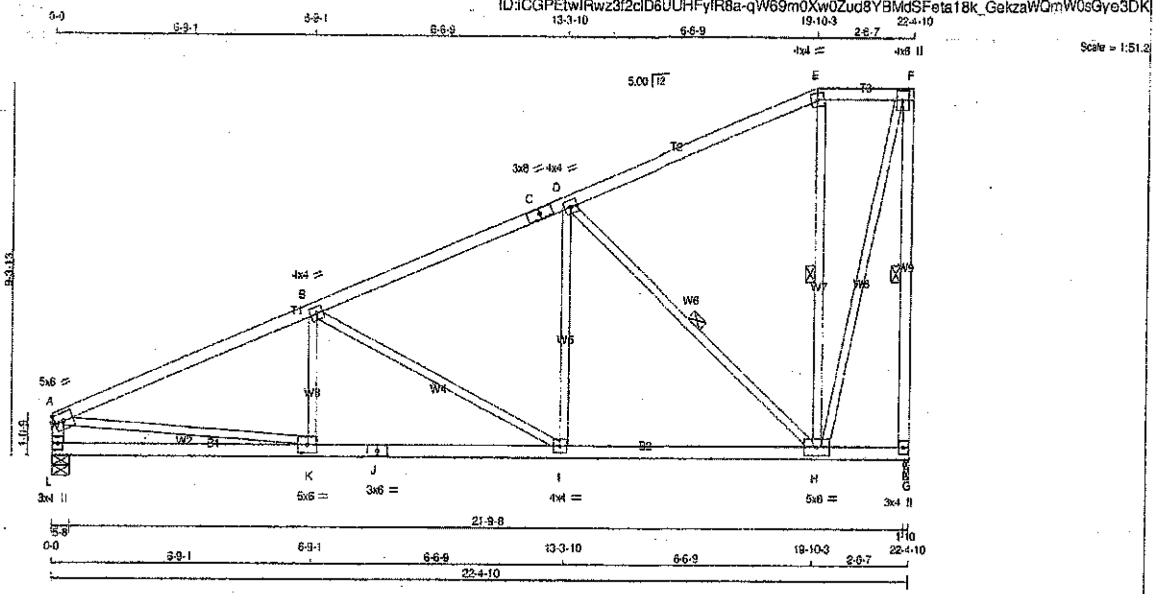

TOTAL WEIGHT = 2 X 104 = 208 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	8.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1349	0	1349	0	0	1-10	1-10
L	1349	0	1349	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	949	649 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
L	949	649 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2063 / 0	-102.0	-102.0	0.65 (1)	3.98	K- B	-81 / 87	0.03 (4)
B- C	-1341 / 0	-102.0	-102.0	0.50 (1)	4.97	B- I	-784 / 0	0.87 (1)
C- D	-1341 / 0	-102.0	-102.0	0.50 (1)	4.97	I- D	0 / 503	0.11 (1)
D- E	-382 / 0	-102.0	-102.0	0.51 (1)	6.25	D- H	-1250 / 0	0.73 (1)
E- F	-326 / 0	-102.0	-102.0	0.08 (1)	6.25	H- E	-261 / 0	0.15 (1)
G- F	-1340 / 0	0.0	0.0	0.55 (1)	5.60	H- F	0 / 1254	0.28 (1)
L- A	-1295 / 0	0.0	0.0	0.13 (1)	7.10	A- K	0 / 1939	0.44 (1)
L- K	0 / 0	-18.5	-18.5	0.18 (4)	10.00			
K- J	0 / 1927	-18.5	-18.5	0.40 (1)	10.00			
J- I	0 / 1927	-18.5	-18.5	0.40 (1)	10.00			
I- H	0 / 1238	-18.5	-18.5	0.29 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.65/1.00 (A-B:1) , BC=0.40/1.00 (I-K:1) , WB=0.87/1.00 (B-I:1) , SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.58 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J/G ALVES
100009024
PROVINCE OF ONTARIO

SEP 16 2019

Structural component only
DWG# T-1923499

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H12	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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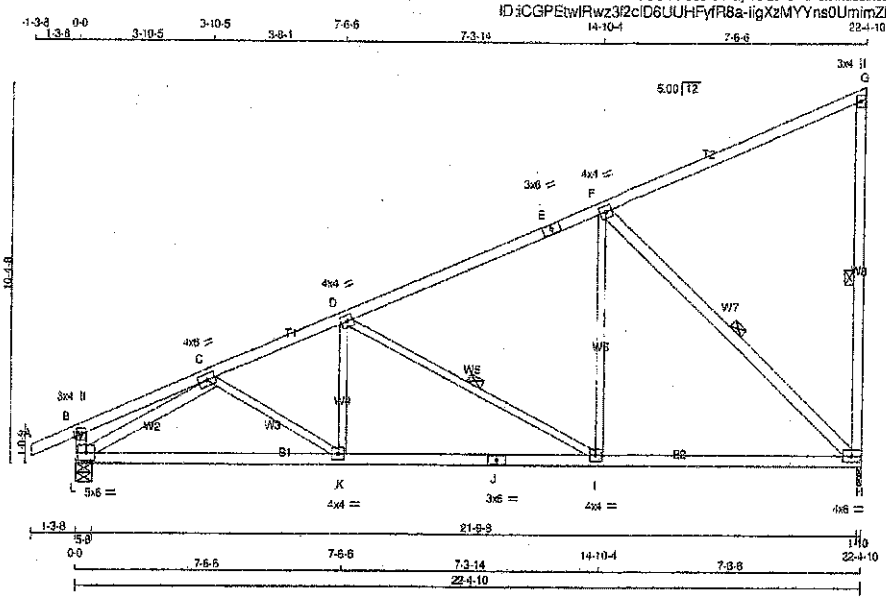


TOTAL WEIGHT = 6 X 101 = 605 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TMWW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMV+p	MT20	3.0	4.0	
H	BMWW1-t	MT20	4.0	6.0	
I	BMWW-t	MT20	4.0	4.0	
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	4.0	4.0	
L	BMVW1-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1349	0	1349	0	0	1-10	1-10
L	1486	0	1486	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	MAX./MIN. COMPONENT REACTIONS PERM.LIVE	WIND	DEAD	SOIL
H	949	649 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
L	1044	728 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

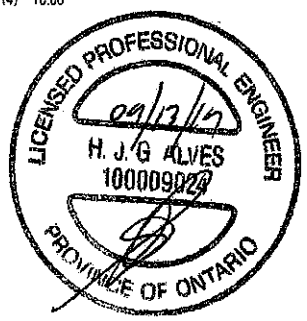
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	K-D	0 / 144	0.05 (4)
B-C	0 / 8	-102.0	-102.0	0.20 (1)	10.00	D-I	-850 / 0	0.40 (1)
C-D	-1982 / 0	-102.0	-102.0	0.34 (1)	4.48	I-F	0 / 551	0.12 (1)
D-E	-1149 / 0	-102.0	-102.0	0.71 (1)	4.81	F-H	-1465 / 0	0.78 (1)
E-F	-1149 / 0	-102.0	-102.0	0.71 (1)	4.81	C-K	0 / 54	0.02 (4)
F-G	-32 / 0	-102.0	-102.0	0.71 (1)	6.25	L-C	-2141 / 0	0.51 (1)
H-G	-300 / 0	0.0	0.0	0.16 (1)	6.25			
L-B	-309 / 0	0.0	0.0	0.03 (1)	7.81			
L-K	0 / 1826	-18.5	-18.5	0.42 (1)	10.00			
K-J	0 / 1829	-18.5	-18.5	0.43 (1)	10.00			
J-I	0 / 1829	-18.5	-18.5	0.43 (1)	10.00			
I-H	0 / 1078	-18.5	-18.5	0.32 (4)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.71/1.00 (F-G:1) , BC=0.43/1.00 (I-K:1) , WB=0.78/1.00 (F-H:1) , SSI=0.34/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1387	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.58 (J) (INPUT = 1.00)

Structural component only
DWG# T-1923500

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H14	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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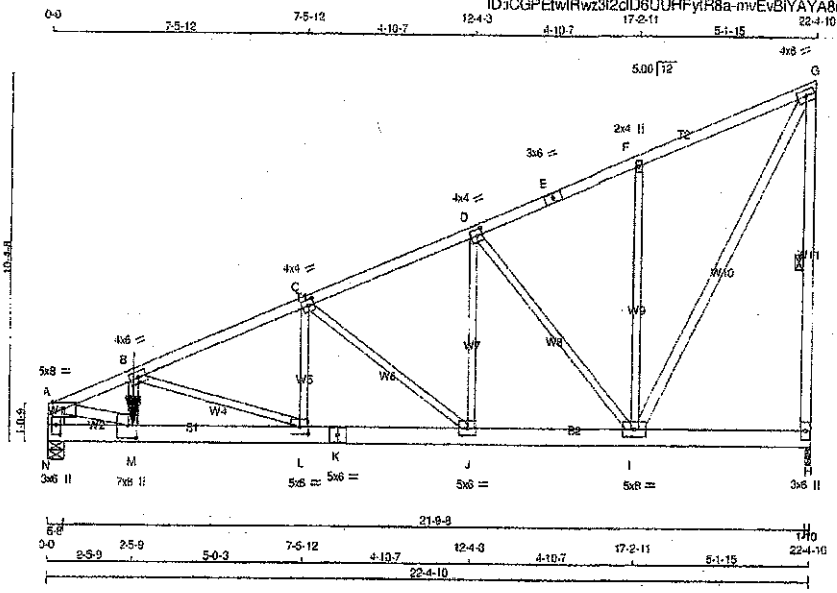

TOTAL WEIGHT = 4 X 123 = 494 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A-E 1	12	TOP
E-G 1	12	TOP
G-H 1	12	TOP
N-A 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
N-K 2	4	SIDE(671.4)
K-H 2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	
B-M 2	2	SIDE(486.0)
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW+t	MT20	7.0	8.0	4.25	3.00
N	BMV1+p	MT20	3.0	6.0	3.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1829	0	1829	0	0	1-10	1-10
N	5229	0	5229	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	1286	884 / 0	0 / 0	0 / 0	0 / 0	402 / 0	0 / 0
N	3674	2548 / 0	0 / 0	0 / 0	0 / 0	1125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LCI	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-7647 / 0	-102.0	-102.0	0.27 (1)	3.39	
B-C	-4073 / 0	-102.0	-102.0	0.29 (1)	4.46	
C-D	-2323 / 0	-102.0	-102.0	0.22 (1)	5.63	
D-E	-1116 / 0	-102.0	-102.0	0.21 (1)	6.25	
E-F	-1116 / 0	-102.0	-102.0	0.21 (1)	6.25	
F-G	-1139 / 0	-102.0	-102.0	0.21 (1)	6.25	
H-G	-1784 / 0	0.0	0.0	0.46 (1)	6.25	
N-A	-4952 / 0	0.0	0.0	0.17 (1)	6.53	
N-M	0 / 0	-18.5	-18.5	0.23 (1)	10.00	
M-L	0 / 7062	-18.5	-18.5	0.69 (1)	10.00	
L-K	0 / 3776	-18.5	-18.5	0.30 (1)	10.00	
K-J	0 / 3776	-18.5	-18.5	0.30 (1)	10.00	
J-I	0 / 2139	-18.5	-18.5	0.16 (1)	10.00	
I-H	0 / 0	-18.5	-18.5	0.03 (4)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
M-B	0 / 2131	0.19 (1)
B-L	-3466 / 0	0.83 (1)
L-C	0 / 1385	0.17 (1)
C-J	-2072 / 0	0.69 (1)
J-D	0 / 1314	0.16 (1)
D-I	-1713 / 0	0.97 (1)
I-F	-582 / 0	0.37 (1)
I-G	0 / 2251	0.20 (1)
A-M	0 / 7310	0.65 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LCI	MAX-	MAX+	FACE	DIR.	TYP
M	2-5-9	-4360	-4360	---	BACK	VERT	TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.46/1.00 (G-H:1) , BC=0.69/1.00 (L-M:1) , WB=0.97/1.00 (D-I:1) , SSI=0.13/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN
MT20	618	354	1667	788
			1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.91 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100089024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923501

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H15	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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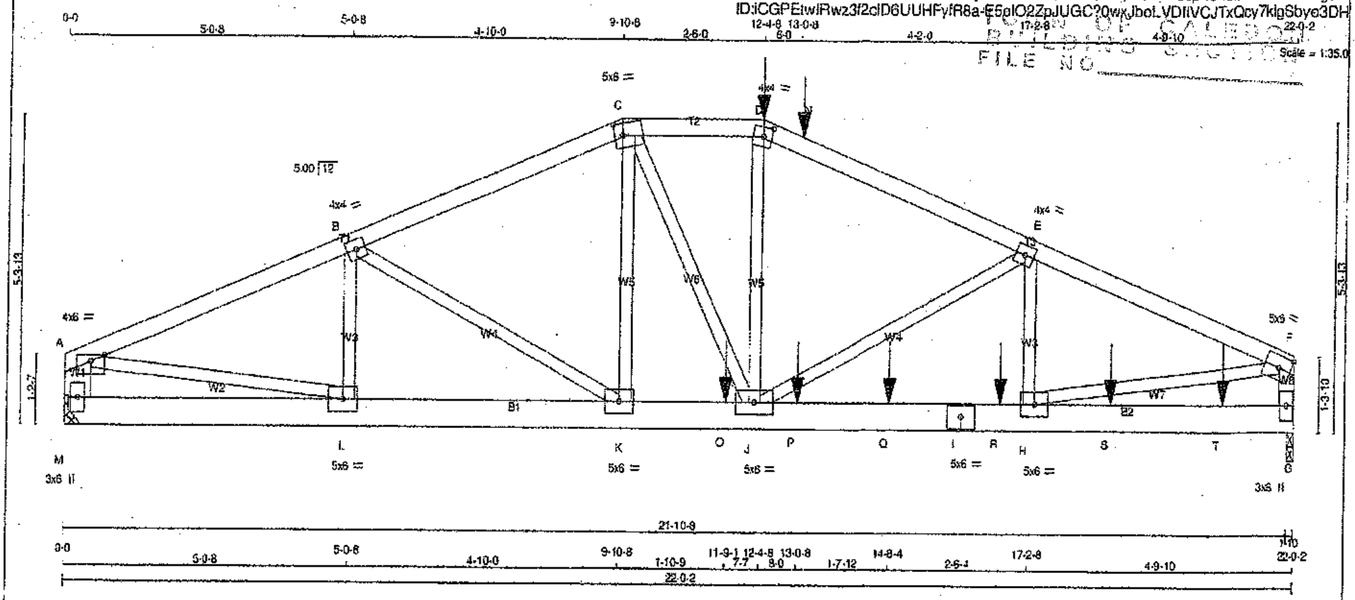

TOTAL WEIGHT = 4 X 100 = 401 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(61.0)
G- F	1	12	TOP
M- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- I	2	12	SIDE(0.0)
I- G	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
J- D	1	6	SIDE(96.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.50	1.75
D	TTW-m	MT20	4.0	4.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		Edge
G	BMV1+p	MT20	3.0	6.0		
H, K, L						
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2256	0	2256	0	0	MECHANICAL	
G	2527	0	2527	0	0	1-10	1-10

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1582	1115 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0
G	1777	1224 / 0	0 / 0	0 / 0	0 / 0	553 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3680 / 0	-102.0	-102.0	0.29 (1)	4.64
B- C	-3643 / 0	-102.0	-102.0	0.26 (1)	4.66
C- D	-3684 / 0	-102.0	-102.0	0.10 (1)	4.83
D- N	-3989 / 0	-102.0	-102.0	0.30 (1)	4.47
N- E	-3989 / 0	-102.0	-102.0	0.30 (1)	4.47
E- F	-3984 / 0	-102.0	-102.0	0.30 (1)	4.48
G- F	-2433 / 0	0.0	0.0	0.13 (1)	7.23
M- A	-2184 / 0	0.0	0.0	0.08 (1)	7.81
M- L	0 / 0	-18.5	-18.5	0.05 (1)	10.00
L- K	0 / 3423	-18.5	-18.5	0.27 (1)	10.00
K- O	0 / 3349	-18.5	-18.5	0.33 (1)	10.00
O- J	0 / 3349	-18.5	-18.5	0.33 (1)	10.00
J- P	0 / 3698	-18.5	-18.5	0.29 (1)	10.00
P- Q	0 / 3698	-18.5	-18.5	0.29 (1)	10.00
Q- I	0 / 3698	-18.5	-18.5	0.29 (1)	10.00
I- R	0 / 3698	-18.5	-18.5	0.29 (1)	10.00
R- H	0 / 3698	-18.5	-18.5	0.29 (1)	10.00
H- S	0 / 0	-18.5	-18.5	0.06 (1)	10.00
S- T	0 / 0	-18.5	-18.5	0.06 (1)	10.00
T- G	0 / 0	-18.5	-18.5	0.06 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-534 / 0	0.05 (1)
B- K	-96 / 0	0.03 (1)
K- C	0 / 389	0.05 (1)
C- J	0 / 767	0.09 (1)
J- D	0 / 262	0.04 (4)
J- E	-80 / 0	0.03 (4)
H- E	-610 / 0	0.06 (1)
H- F	0 / 3765	0.47 (1)
A- L	0 / 3468	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	12-4-8	-125	-140	---	FRONT	VERT	DEAD	---	C1
D	12-4-8	-713	-713	---	FRONT	VERT	SNOW	---	C1
N	13-0-8	-140	-140	---	FRONT	VERT	TOTAL	---	C1
O	11-9-1	-1018	-1018	---	FRONT	VERT	TOTAL	---	C1
P	13-0-8	-27	-27	---	FRONT	VERT	TOTAL	---	C1
Q	14-8-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
R	16-8-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
S	18-8-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1
T	20-8-4	-27	-27	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO
[illegible]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.33/1.00 (J-K:1) , WB=0.47/1.00 (F-H:1) , SSI=0.27/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.50 (F) (INPUT = 1.00)

Structural component only
DWG# T-1923502

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H17	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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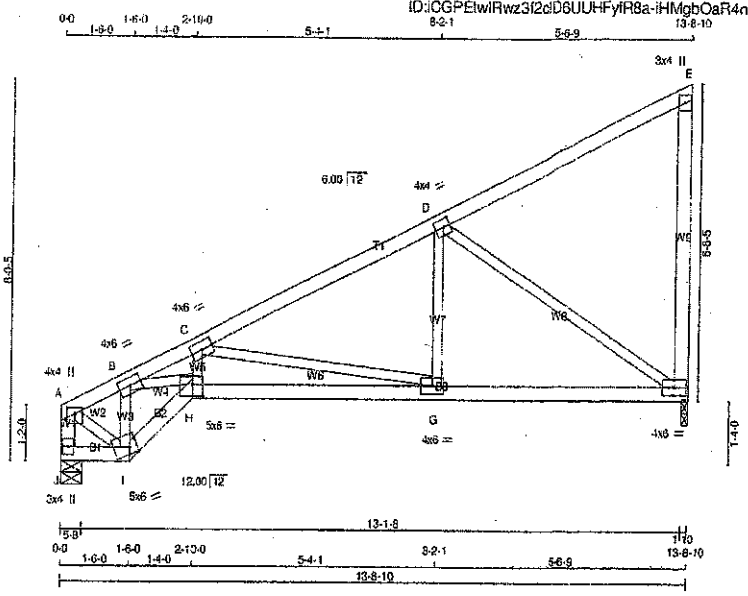


TOTAL WEIGHT = 4 X 58 = 230 lb (M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	6.0		
H	BBWW-p	MT20	5.0	6.0	3.00	2.75
I	BBWW-m	MT20	5.0	6.0	1.75	2.00
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	827	0	827	0	0	1-10	1-10
J	827	0	827	0	0	5-9	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	582	398 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0
J	582	398 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	-776 / 0	-102.0	-102.0	0.05 (1)		6.25
B-C	-2318 / 0	-102.0	-102.0	0.12 (1)		4.41
C-D	-949 / 0	-102.0	-102.0	0.38 (1)		5.86
D-E	-28 / 0	-102.0	-102.0	0.57 (1)		6.25
F-E	-220 / 0	0.0	0.0	0.20 (1)		7.81
J-A	-813 / 0	0.0	0.0	0.08 (1)		7.81
J-I	0 / 0	-18.5	-18.5	0.01 (4)		10.00
I-H	0 / 877	-18.5	-18.5	0.15 (1)		10.00
H-G	0 / 2045	-18.5	-18.5	0.41 (1)		10.00
G-F	0 / 870	-18.5	-18.5	0.22 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-B	-975 / 0	0.14 (1)
B-H	0 / 1490	0.34 (1)
H-C	0 / 488	0.11 (1)
C-G	-1196 / 0	0.58 (1)
G-D	0 / 334	0.08 (1)
D-F	-1038 / 0	0.76 (1)
A-I	0 / 784	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF 6CBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.41/1.00 (G-H:1) , WB=0.78/1.00 (D-F:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

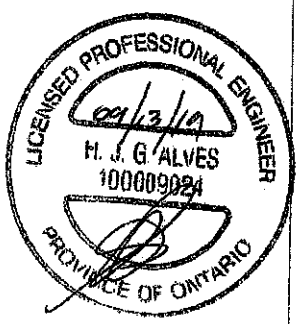
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (B) (INPUT = 0.90)
JSI METAL = 0.40 (B) (INPUT = 1.00)



Structural component only
DWG# T-1923503

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H18	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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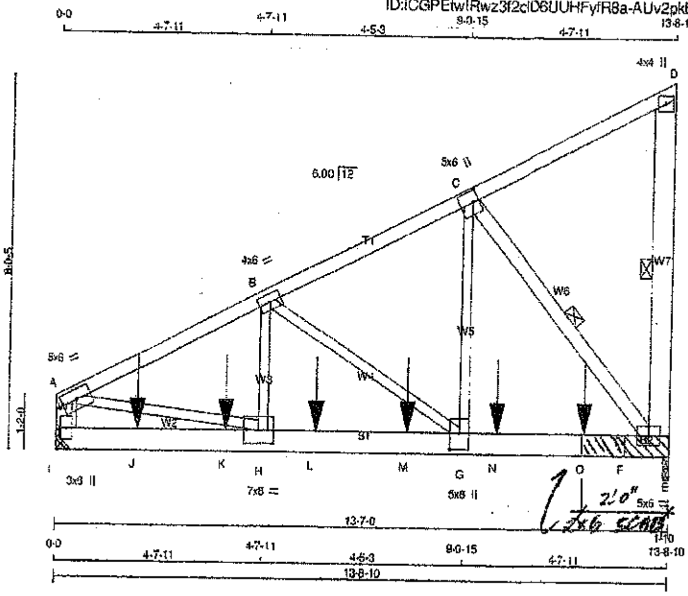

TOTAL WEIGHT = 4 X 79 = 317 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
I-A	2	12	TOP
D-E	2	12	TOP
A-D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I-E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0		
B	TMWW-t	MT20	4.0	6.0		
C	TMWW+t	MT20	5.0	6.0	3.00	1.50
D	TMV+p	MT20	4.0	4.0		
E	BMVW1-t	MT20	5.0	6.0		
G	BMWW+t	MT20	5.0	8.0	4.25	2.50
H	BMWW-t	MT20	7.0	8.0		
I	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	4379	0	4379	0	0	MECHANICAL	
E	4320	0	4320	0	0	1-10	1-10 & BLOCK

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	3076	2135 / 0	0 / 0	0 / 0	0 / 0	941 / 0	0 / 0
E	3035	2106 / 0	0 / 0	0 / 0	0 / 0	929 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

2x6 DRY SPF No.2 BEARING BLOCK 12" LONG AT JT. E ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-A	-3621 / 0	0.0	0.0	0.13 (1)	7.39	A-H	0 / 5187	0.64 (1)
A-B	-5708 / 0	-102.0	-102.0	0.33 (1)	3.83	H-B	0 / 2024	0.25 (1)
B-C	-3385 / 0	-102.0	-102.0	0.23 (1)	4.86	B-G	-2536 / 0	0.63 (1)
C-D	-22 / 0	-102.0	-102.0	0.17 (1)	6.25	G-C	0 / 4460	0.55 (1)
E-D	-188 / 0	0.0	0.0	0.02 (1)	6.25	C-E	-4615 / 0	0.54 (1)
I-J	0 / 0	-18.5	-18.5	0.49 (1)	10.00			
J-K	0 / 0	-18.5	-18.5	0.49 (1)	10.00			
K-H	0 / 0	-18.5	-18.5	0.49 (1)	10.00			
H-L	0 / 5116	-18.5	-18.5	0.78 (1)	10.00			
L-M	0 / 5116	-18.5	-18.5	0.78 (1)	10.00			
M-G	0 / 5116	-18.5	-18.5	0.78 (1)	10.00			
G-N	0 / 3037	-18.5	-18.5	0.65 (1)	10.00			
N-O	0 / 3037	-18.5	-18.5	0.65 (1)	10.00			
O-F	0 / 3037	-18.5	-18.5	0.65 (1)	10.00			
F-E	0 / 3037	-18.5	-18.5	0.65 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	1-9-10	-1174	-1174	---	FRONT	VERT	TOTAL	--	C1
K	3-9-10	-1174	-1174	---	FRONT	VERT	TOTAL	--	C1
L	5-9-10	-1174	-1174	---	FRONT	VERT	TOTAL	--	C1
M	7-9-10	-1174	-1174	---	FRONT	VERT	TOTAL	--	C1
N	9-9-10	-1174	-1174	---	FRONT	VERT	TOTAL	--	C1
O	11-9-10	-1174	-1174	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.78/1.00 (G-H:1) , WB=0.64/1.00 (A-H:1) , SSI=0.59/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)

Structural component only
DWG# T-1923504

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H19	12	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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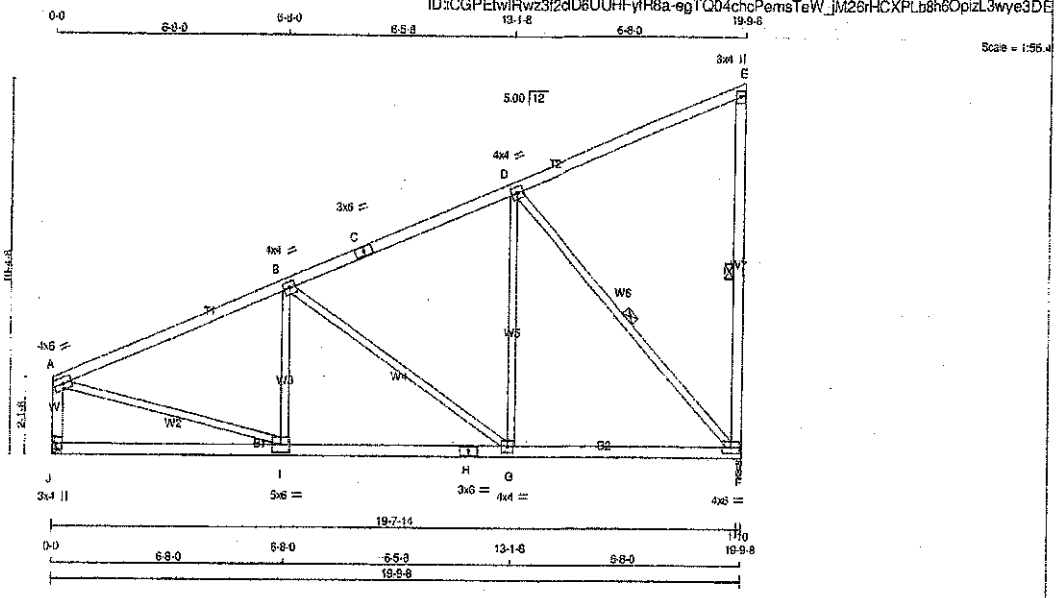


TOTAL WEIGHT = 12 X 89 = 1069 lb [MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	8.0		
G	BMWW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1193	0	1193	0	0	1-10	1-10
J	1193	0	1193	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	839	574 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	839	574 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

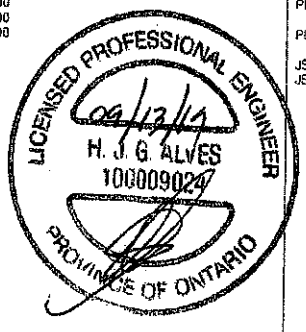
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1333 / 0	-102.0	-102.0	0.57 (1)		4.85
B- C	-859 / 0	-102.0	-102.0	0.48 (1)		5.90
C- D	-858 / 0	-102.0	-102.0	0.48 (1)		5.90
D- E	-25 / 0	-102.0	-102.0	0.52 (1)		6.25
F- E	-274 / 0	0.0	0.0	0.15 (1)		6.25
J- A	-1141 / 0	0.0	0.0	0.13 (1)		7.44
J- I	0 / 0	-18.5	-18.5	0.18 (4)		10.00
I- H	0 / 1253	-18.5	-18.5	0.30 (1)		10.00
H- G	0 / 1253	-18.5	-18.5	0.30 (1)		10.00
G- F	0 / 794	-18.5	-18.5	0.25 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-213 / 42	0.08 (1)
B- G	-564 / 0	0.74 (1)
G- D	0 / 461	0.10 (1)
D- F	-1176 / 0	0.80 (1)
A- I	0 / 1299	0.29 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.57/1.00 (A-B:1) , BC=0.30/1.00 (G-I:1) , WB=0.80/1.00 (D-F:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.39 (H) (INPUT = 1.00)

Structural component only
DWG# T-1923505

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H19S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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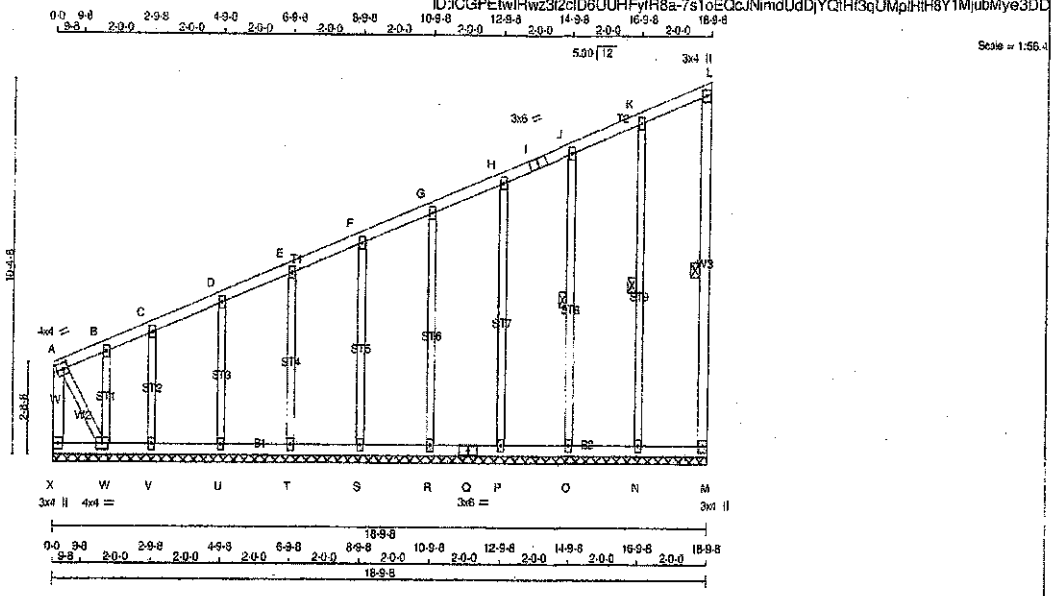

TOTAL WEIGHT = 2 X 102 = 204 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - A	2x4	DRY	No.2	SPF
X - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-t	MT20	4.0	4.0	2.00	1.50
B, C, D, E, F, G, H, J, K						
B	TMW+w	MT20	2.0	4.0		
I	TS-t	MT20	3.0	6.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, R, S, T, U, V						
N	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	6.0		
W	BMWW1-t	MT20	4.0	4.0		
X	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, K-N, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	-15 / 0	-102.0	-102.0	0.02 (1)		6.25
B-C	-15 / 0	-102.0	-102.0	0.03 (1)		6.25
C-D	-12 / 0	-102.0	-102.0	0.05 (1)		6.25
D-E	-11 / 0	-102.0	-102.0	0.05 (1)		6.25
E-F	-8 / 0	-102.0	-102.0	0.05 (1)		10.00
F-G	-6 / 0	-102.0	-102.0	0.05 (1)		10.00
G-H	-5 / 0	-102.0	-102.0	0.05 (1)		10.00
H-I	-3 / 0	-102.0	-102.0	0.05 (1)		10.00
I-J	-3 / 0	-102.0	-102.0	0.05 (1)		10.00
J-K	0 / 0	-102.0	-102.0	0.05 (1)		10.00
K-L	-7 / 0	-102.0	-102.0	0.05 (1)		10.00
M-L	-86 / 0	0.0	0.0	0.05 (1)		6.25
X-A	-97 / 0	0.0	0.0	0.01 (1)		7.81
X-W	0 / 0	-18.5	-18.5	0.01 (4)		10.00
W-V	0 / 16	-18.5	-18.5	0.01 (4)		10.00
V-U	0 / 13	-18.5	-18.5	0.02 (4)		10.00
U-T	0 / 10	-18.5	-18.5	0.02 (4)		10.00
T-S	0 / 8	-18.5	-18.5	0.01 (4)		10.00
S-R	0 / 6	-18.5	-18.5	0.01 (4)		10.00
R-Q	0 / 4	-18.5	-18.5	0.01 (4)		10.00
Q-P	0 / 4	-18.5	-18.5	0.01 (4)		10.00
P-O	0 / 3	-18.5	-18.5	0.01 (4)		10.00
O-N	0 / 1	-18.5	-18.5	0.02 (4)		10.00
N-M	0 / 0	-18.5	-18.5	0.02 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-K	-227 / 0	0.14 (1)
O-J	-201 / 0	0.10 (1)
P-H	-203 / 0	0.24 (1)
R-G	-203 / 0	0.17 (1)
S-F	-203 / 0	0.13 (1)
T-E	-202 / 0	0.09 (1)
U-D	-209 / 0	0.06 (1)
V-C	-168 / 0	0.04 (1)
W-B	-144 / 0	0.03 (1)
A-W	0 / 34	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (K-L:1) , BC=0.02/1.00 (N-O:4) , WB=0.24/1.00 (H-P:1) , SSI=0.09/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (A) (INPUT = 0.90)
JSI METAL= 0.08 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

RECEIVED
SEP 16 2019
TOWN OF CALEDON
BUILDING DIVISION
FILE NO

Structural component only
DWG# T-1923506

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H20	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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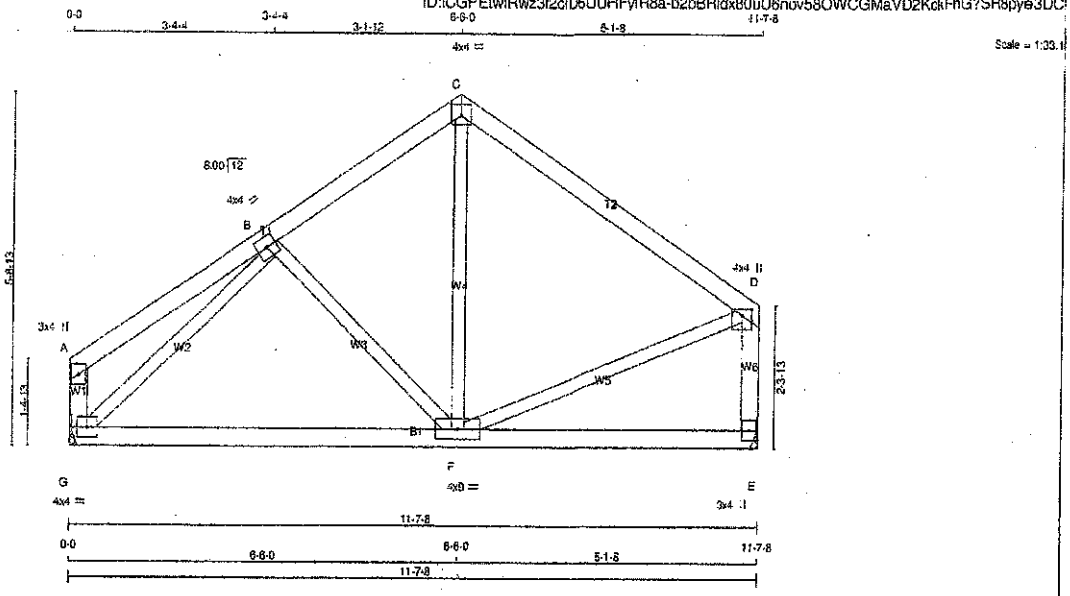

TOTAL WEIGHT = 6 X 48 = 288 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW-+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	701	0	701	0	0	MECHANICAL	
G	701	0	701	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, G. MINIMUM BEARING LENGTH AT JOINT E = 1-8, JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	493	337 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0
G	493	337 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-102.0	-102.0	0.17 (1)	10.00
B- C	-483 / 0	-102.0	-102.0	0.13 (1)	6.25
C- D	-462 / 0	-102.0	-102.0	0.35 (1)	6.25
G- A	-127 / 0	0.0	0.0	0.01 (1)	7.81
E- D	-668 / 0	0.0	0.0	0.08 (1)	7.81
G- F	0 / 525	-18.5	-18.5	0.22 (4)	10.00
F- E	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- F	-197 / 0	0.07 (1)
F- C	0 / 123	0.04 (4)
G- B	-742 / 0	0.25 (1)
F- D	0 / 413	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.22/1.00 (F-G:4) , WB=0.25/1.00 (B-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.64 (B) (INPUT = 0.90)
JSI METAL = 0.26 (B) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923507

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H20G	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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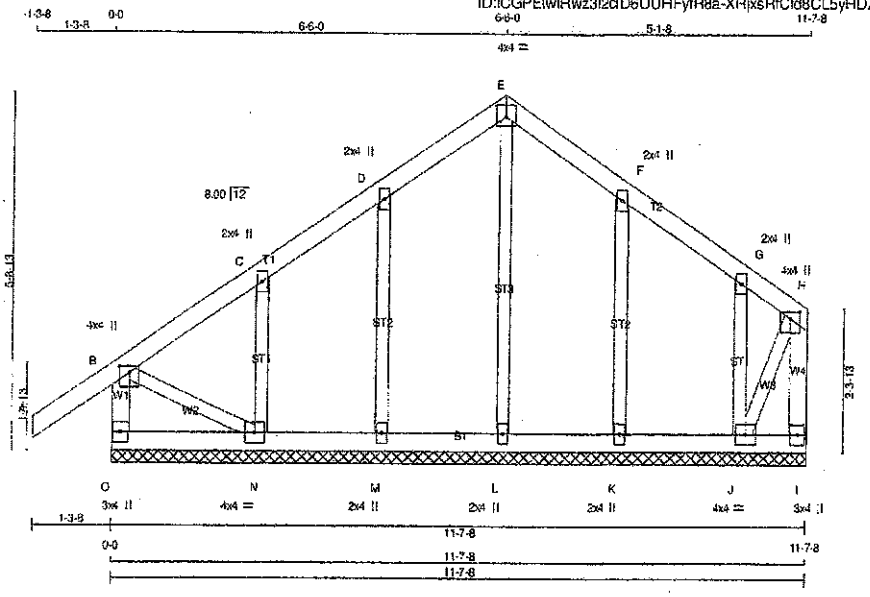

TOTAL WEIGHT = 2 X 53 = 105 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		
K, L, M						
K	BMW1+w	MT20	2.0	4.0		
N	BMWW1-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
O-B	-267 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00
B-C	-16 / 0	-102.0	-102.0	0.07 (1)	6.25
C-D	-24 / 0	-102.0	-102.0	0.07 (1)	6.25
D-E	-25 / 0	-102.0	-102.0	0.06 (1)	6.25
E-F	-26 / 0	-102.0	-102.0	0.06 (1)	6.25
F-G	-14 / 0	-102.0	-102.0	0.06 (1)	6.25
G-H	-14 / 0	-102.0	-102.0	0.03 (1)	6.25
I-H	-84 / 0	0.0	0.0	0.01 (1)	7.81
O-N	0 / 0	-18.5	-18.5	0.03 (4)	10.00
N-M	0 / 16	-18.5	-18.5	0.03 (4)	10.00
M-L	0 / 12	-18.5	-18.5	0.02 (4)	10.00
L-K	0 / 12	-18.5	-18.5	0.02 (4)	10.00
K-J	0 / 17	-18.5	-18.5	0.02 (4)	10.00
J-I	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-E	-156 / 0	0.06 (1)
M-D	-216 / 0	0.06 (1)
N-C	-251 / 0	0.05 (1)
K-F	-238 / 0	0.07 (1)
J-G	-163 / 0	0.03 (1)
B-N	0 / 26	0.01 (1)
J-H	0 / 44	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (M-N:4) , WB=0.08/1.00 (E-L:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923508

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H21	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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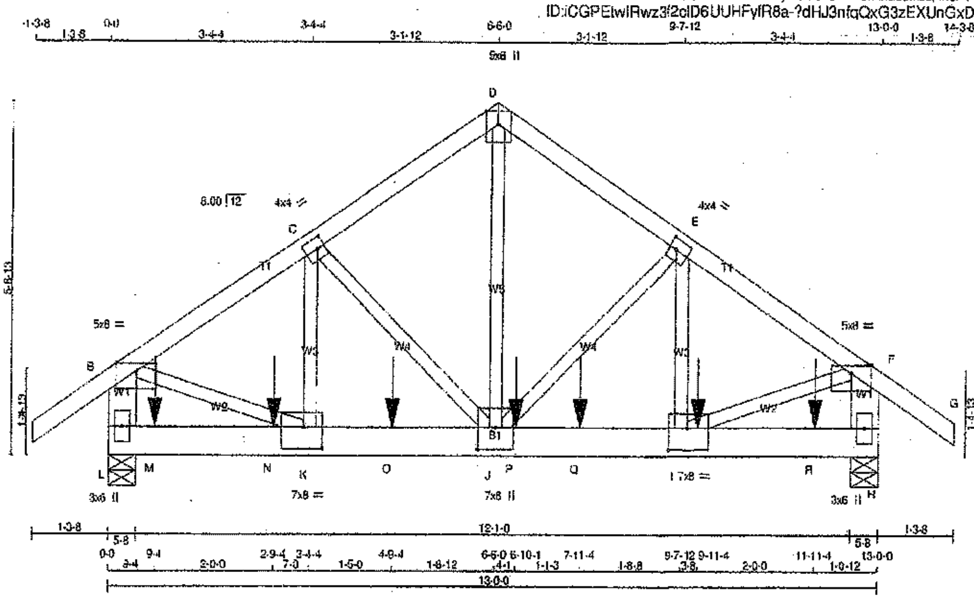

TOTAL WEIGHT = 4 X 69 = 274 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
L- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- H	2	12	SIDE(163.1)
WEBS : (0.122"X3") SPIRAL NAILS			
E- I	1	6	SIDE(158.1)
J- D	1	6	SIDE(400.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	5.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	7.0	8.0	4.25	3.75
J	BMWWW+t	MT20	7.0	8.0	4.25	3.50
K	BMWW-t	MT20	7.0	8.0	4.25	3.75
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	4550	0	4550	0	0	5-8	5-8
H	5507	0	5507	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
L	3190	2247 / 0	0 / 0	0 / 0	0 / 0	943 / 0	0 / 0
H	3864	2709 / 0	0 / 0	0 / 0	0 / 0	1155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED;

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-102.0	-102.0	0.08 (1)	10.00	J- D	0 / 4498	0.56 (1)
B- C	-4507 / 0	-102.0	-102.0	0.16 (1)	4.39	J- E	-1406 / 0	0.24 (1)
C- D	-4323 / 0	-102.0	-102.0	0.14 (1)	4.49	I- E	0 / 1300	0.16 (1)
D- E	-4320 / 0	-102.0	-102.0	0.15 (1)	4.49	C- J	-245 / 0	0.04 (1)
E- F	-5483 / 0	-102.0	-102.0	0.21 (1)	4.00	K- C	-81 / 27	0.01 (1)
F- G	0 / 39	-102.0	-102.0	0.08 (1)	10.00	B- K	0 / 3921	0.49 (1)
L- B	-3915 / 0	0.0	0.0	0.14 (1)	7.16	I- F	0 / 4766	0.59 (1)
H- F	-4700 / 0	0.0	0.0	0.17 (1)	6.57			
L- M	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
M- N	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
N- K	0 / 0	-18.5	-18.5	0.18 (1)	10.00			
K- O	0 / 3759	-18.5	-18.5	0.48 (1)	10.00			
O- J	0 / 3759	-18.5	-18.5	0.48 (1)	10.00			
J- P	0 / 4569	-18.5	-18.5	0.64 (1)	10.00			
P- Q	0 / 4569	-18.5	-18.5	0.64 (1)	10.00			
Q- I	0 / 4569	-18.5	-18.5	0.64 (1)	10.00			
I- R	0 / 0	-18.5	-18.5	0.31 (1)	10.00			
R- H	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	CONN.
I	9-11-4	-1308	-1308	---	BACK	VERT	TOTAL	C1
M	9-4	-684	-684	---	BACK	VERT	TOTAL	C1
N	2-9-4	-682	-682	---	BACK	VERT	TOTAL	C1
O	4-9-4	-682	-682	---	BACK	VERT	TOTAL	C1
P	6-10-1	-2238	-2238	---	BACK	VERT	TOTAL	C1
Q	7-11-4	-1308	-1308	---	BACK	VERT	TOTAL	C1
R	11-11-4	-1308	-1308	---	BACK	VERT	TOTAL	C1

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. GEE-ALVES
100009022
PROVINCE OF ONTARIO

CONNECTION REQUIREMENTS

() C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	42.4	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.21/1.00 (E-F:1) , BC=0.64/1.00 (I-J:1) , WB=0.59/1.00 (F-I:1) , SSI=0.93/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.85 (D) (INPUT = 0.90)
SI METAL= 0.44 (F) (INPUT = 1.00)

Structural component only
DWG# T-1923556

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H21G	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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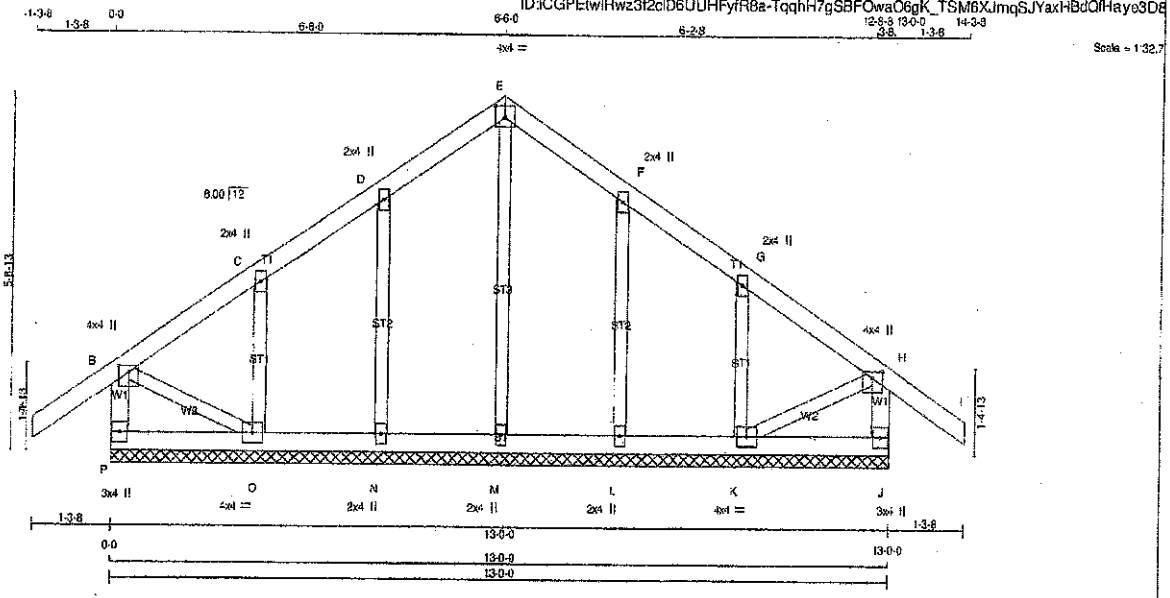

TOTAL WEIGHT = 2 X 57 = 114 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
G, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S).

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH
FR-TO					
P-B	-272 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00
B-C	-21 / 0	-102.0	-102.0	0.07 (1)	6.25
C-D	-29 / 0	-102.0	-102.0	0.07 (1)	6.25
D-E	-30 / 0	-102.0	-102.0	0.05 (1)	6.25
E-F	-30 / 0	-102.0	-102.0	0.05 (1)	6.25
F-G	-29 / 0	-102.0	-102.0	0.07 (1)	6.25
G-H	-21 / 0	-102.0	-102.0	0.07 (1)	6.25
H-I	0 / 39	-102.0	-102.0	0.14 (1)	10.00
J-H	-272 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 0	-18.5	-18.5	0.03 (4)	10.00
O-N	0 / 21	-18.5	-18.5	0.03 (4)	10.00
N-M	0 / 16	-18.5	-18.5	0.02 (4)	10.00
M-L	0 / 16	-18.5	-18.5	0.02 (4)	10.00
L-K	0 / 21	-18.5	-18.5	0.03 (4)	10.00
K-J	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
M-E	-153 / 0	0.07 (1)
N-D	-216 / 0	0.08 (1)
O-C	-251 / 0	0.05 (1)
L-F	-216 / 0	0.08 (1)
K-G	-251 / 0	0.05 (1)
B-O	0 / 31	0.01 (1)
K-H	0 / 31	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (N-O:4) , WB=0.07/1.00 (E-M:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (H) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923557

SEP 16 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H26	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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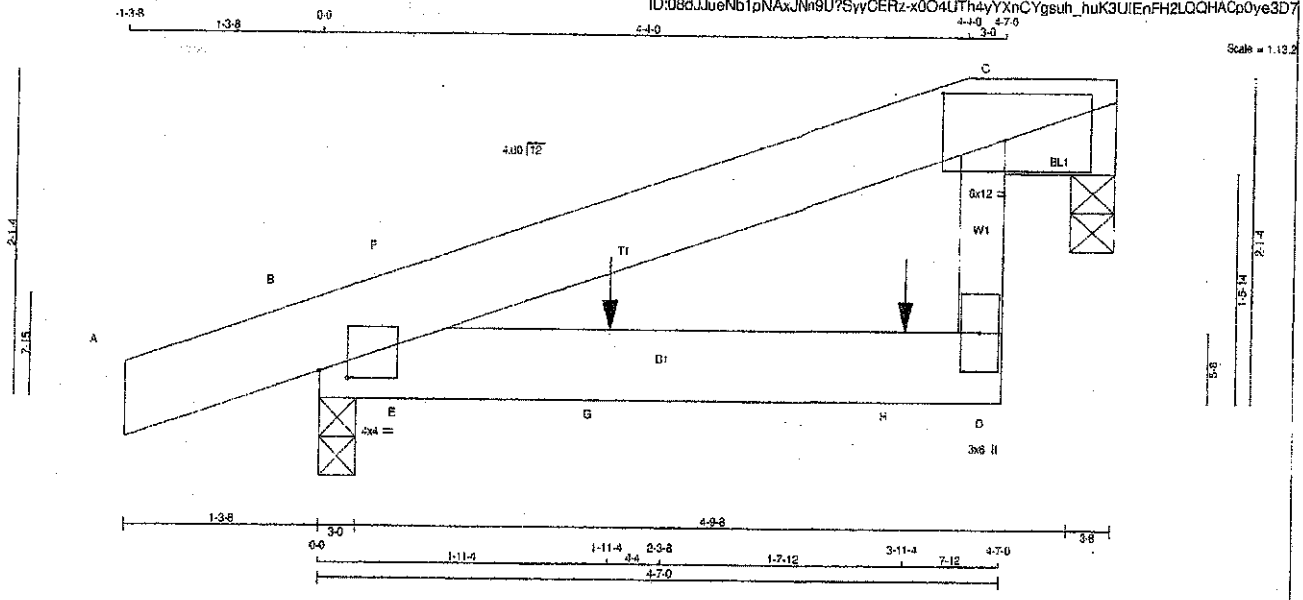


TOTAL WEIGHT = 2 X 24 = 47 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF

BEARING BLOCKS

BL1	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	0.50	2.25
C	TMVK1-p	MT20	6.0	12.0	3.50	5.00
D	BMV+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	296	0	296	0	0	3-8	3-8
B	425	0	425	0	0	3-0	3-0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	211	132 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
B	298	211 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

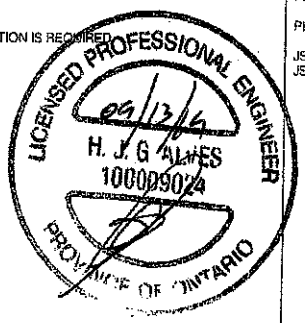
CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 13	-102.0	-102.0	0.07 (1)	10.00	E-F	-214 / 29	0.00 (1)
B-F	-31 / 0	-102.0	-102.0	0.04 (4)	6.25			
F-C	0 / 3	-102.0	-102.0	0.15 (1)	10.00			
D-C	0 / 92	0.0	0.0	0.02 (4)	10.00			
B-E	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
E-G	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
G-H	0 / 0	-18.5	-18.5	0.11 (1)	10.00			
H-D	0 / 0	-18.5	-18.5	0.11 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-13	-14	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (C-F:1) , BC=0.11/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	798	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

Structural component only
DWG# T-1923558

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	J1	22	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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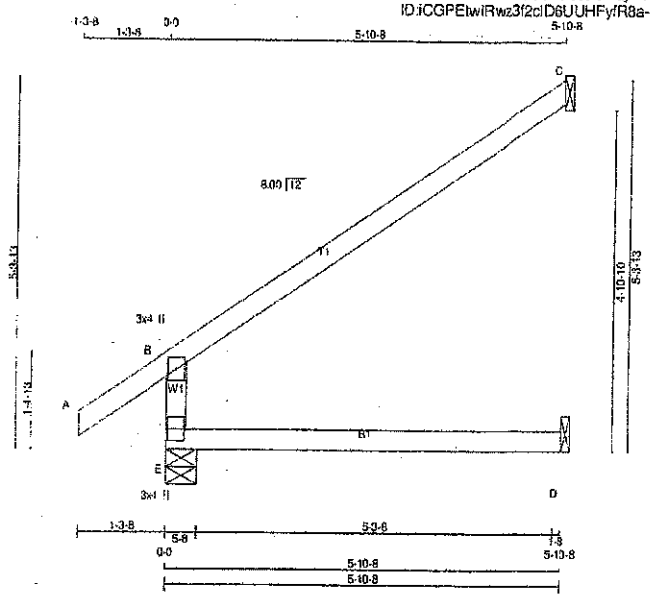

TOTAL WEIGHT = 22 X 18 = 392 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	578	0	578	0	0	5-8	5-8
C	225	0	225	0	0	1-8	1-8
D	46	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 89779TH FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	404	293 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH FR-TO
E-B	-515 / 0	0.0	0.0	0.12 (4)	7.81
A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00
B-C	-42 / 0	-102.0	-102.0	0.60 (1)	6.25
E-D	0 / 0	-16.5	-16.5	0.13 (4)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923559

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	J3	42	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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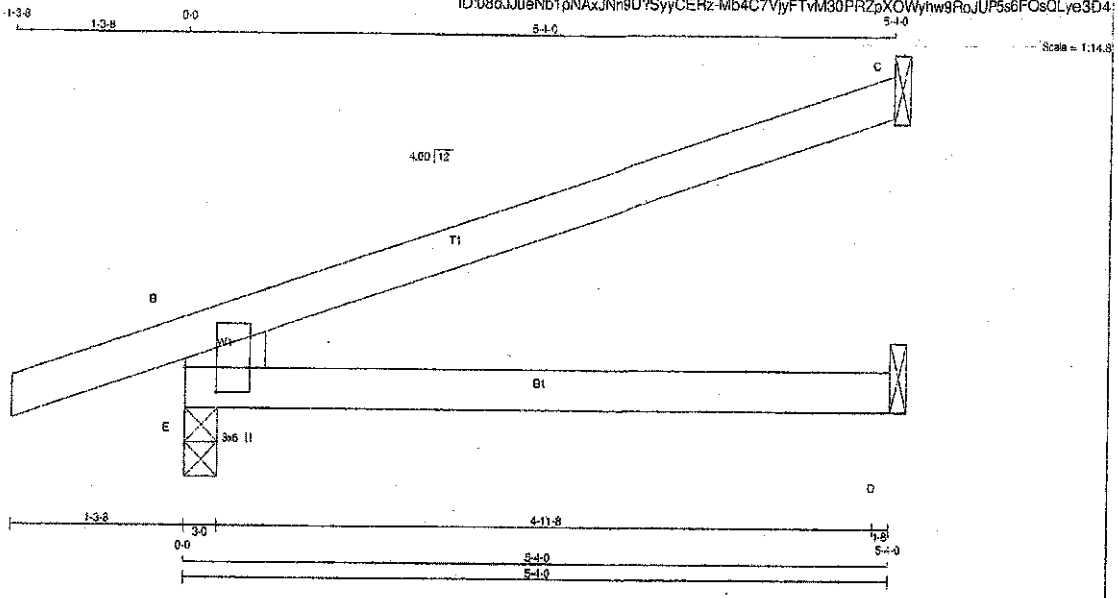

TOTAL WEIGHT = 42 X 15 = 621 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
E						
E	TMBMV1+p	MT20	3.0	6.0	2.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	538	0	538	0	0	3-0	3-0
C	204	0	204	0	0	1-8	1-8
D	37	0	42	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	376	271 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
C	140	118 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	30	0 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-477 / 0	0.0 0.0	0.05 (4)	7.81			
A-B	0 / 22	-102.0 -102.0	0.13 (1)	10.00			
B-C	-22 / 0	-102.0 -102.0	0.49 (1)	5.25			
E-D	0 / 0	-18.5 -18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.15/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.25/1.00 (D-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.24 (E) (INPUT = 0.90)
JSI METAL = 0.09 (E) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
09/12/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

RECEIVED
SEP 16 2019
TOWN OF OAKVILLE
BUILDING SECTION
FILE NO.

Structural component only
DWG# T-1923560

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	J4	12	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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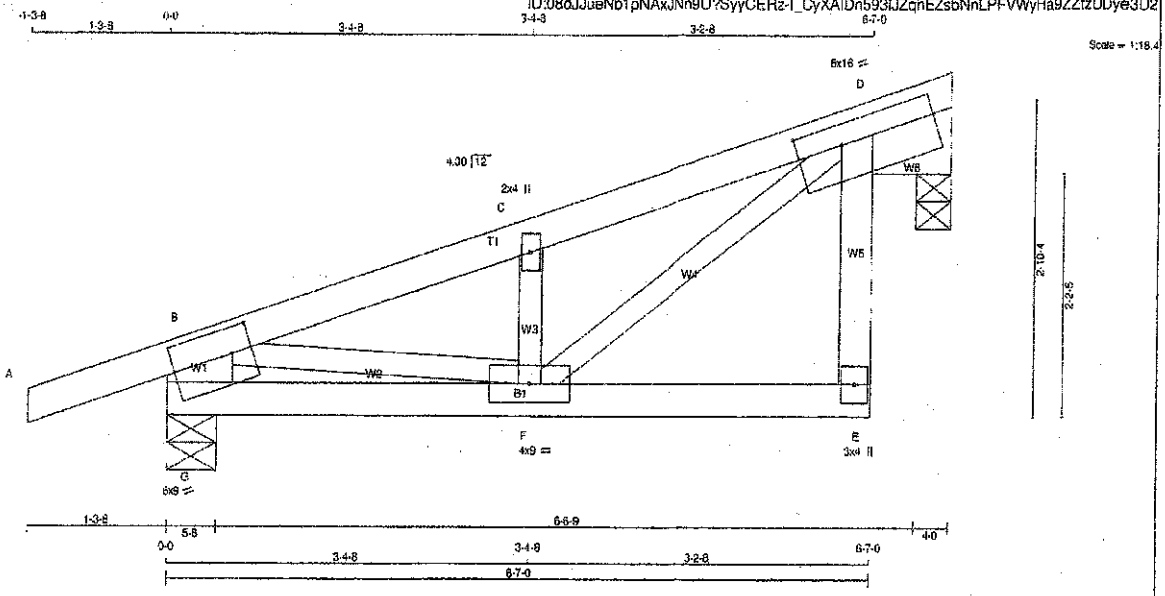

TOTAL WEIGHT = 12 X 29 = 348 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - B	2x8	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-t	MT20	6.0	16.0	2.25	5.00
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G						
G	TMBMWW1-t	MT20	6.0	9.0	2.75	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	379	0	379	0	0	4-0 (3-15)	4-0
G	546	0	546	0	0	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	266	182 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
G	381	277 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 26	-102.0	-102.0	0.19 (1)	10.00	G- B	-517 / 0	0.02 (1)
B- C	-494 / 0	-102.0	-102.0	0.15 (1)	6.25	B- F	0 / 484	0.11 (1)
C- D	-479 / 0	-102.0	-102.0	0.15 (1)	6.25	F- C	-377 / 0	0.05 (1)
E- D	0 / 29	0.0	0.0	0.01 (4)	10.00	F- D	0 / 575	0.13 (1)
G- F	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.04 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.19/1.00 (A-B:1) , BC=0.04/1.00 (E-F:4) , WB=0.13/1.00 (D-F:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.42 (G) (INPUT = 0.90)
JSI METAL = 0.13 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923561

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	J5	10	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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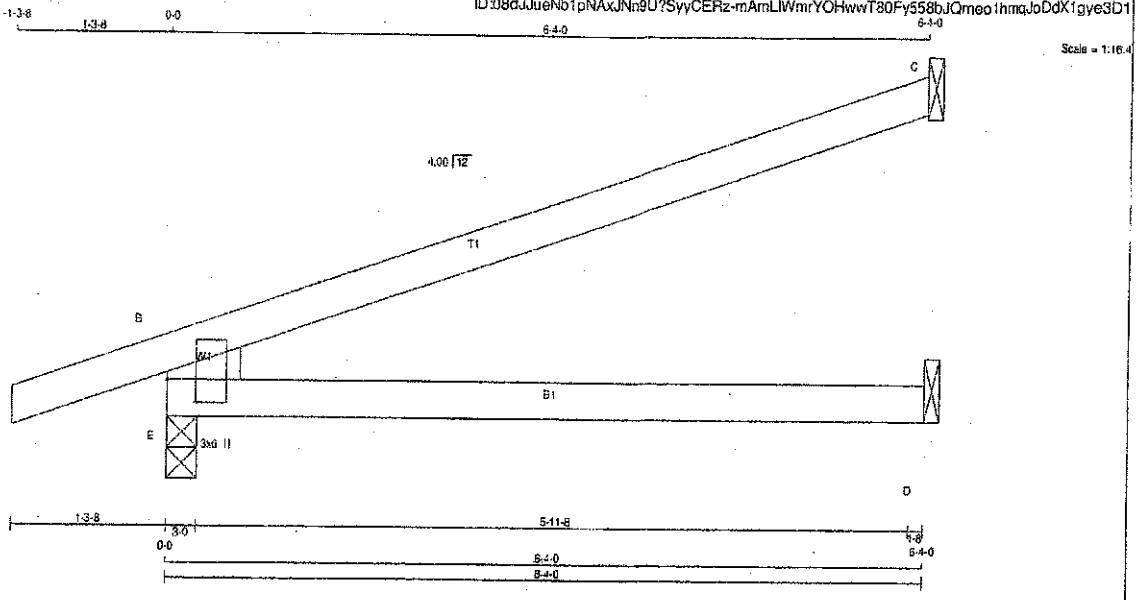


TOTAL WEIGHT = 10 X 17 = 171 lb [MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
E						
E	TMSMV1+p	MT20	3.0	6.0	2.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	613	0	613	0	0	3-0	3-0
C	242	0	242	0	0	1-8	1-8
D	44	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	429	307 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0
C	166	138 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LCI (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-540 / 0	0.0	0.0	0.05 (4)	7.81			
A-B	0 / 22	-102.0	-102.0	0.13 (1)	10.00			
B-C	-26 / 0	-102.0	-102.0	0.54 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.21/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923562

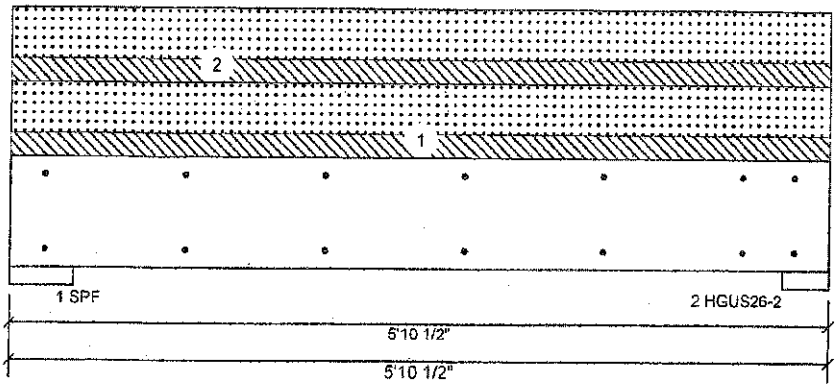
isDesign™

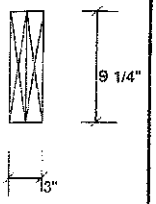
Client:
Project:
Address:

Date: /3/2019
Designer:
Job Name: 405481
Project #:

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015 / OBC 2012
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	278	602	0
2	0	266	577	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	348 / 903	1250	L	1.25D+1.5S
2 - HGUS...	4.000"	16%	333 / 865	1198	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1413 ft-lb	3'	6039 ft-lb	0.234 (23%)	1.25D+1.5S	L
Unbraced	1413 ft-lb	3'	5236 ft-lb	0.270 (27%)	1.25D+1.5S	L
Shear	1085 lb	1'2"	3984 lb	0.272 (27%)	1.25D+1.5S	L
LL Defl inch	0.012 (L/5129)	3'	0.174 (L/360)	0.070 (7%)	S	L
TL Defl inch	0.018 (L/3508)	3'	0.174 (L/360)	0.100 (10%)	D+S	L

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO
9/12/19

DWG NO. TAM 19235 64
STRUCTURAL COMPONENT ONLY

Design Notes

1 Fasten all plies using 2 rows of Pneumatic Gun Nail (120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-11-0	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF [illegible] BUILDING CODE

SEP 16 2019
TOWN OF SALUBRIA
BUILDING SECTION
FILE NO

This design is valid until 12/11/2021

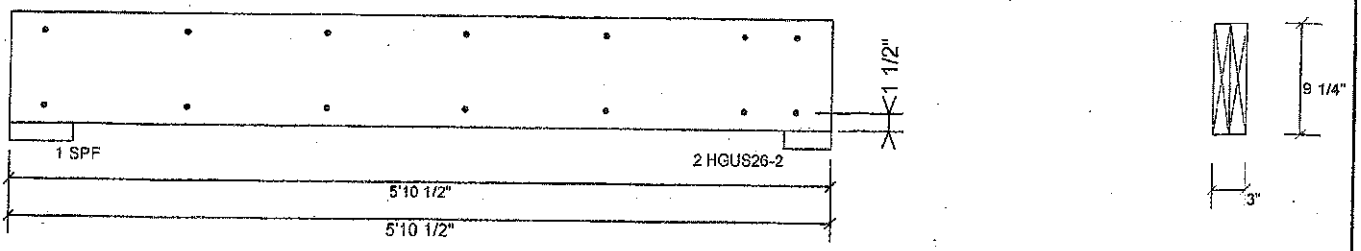
Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	78.6 %
Load	178.2 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

LICENSED PROFESSIONAL ENGINEER
09/12/19
H. J. G. ALVES
100000024
PROVINCE OF ONTARIO

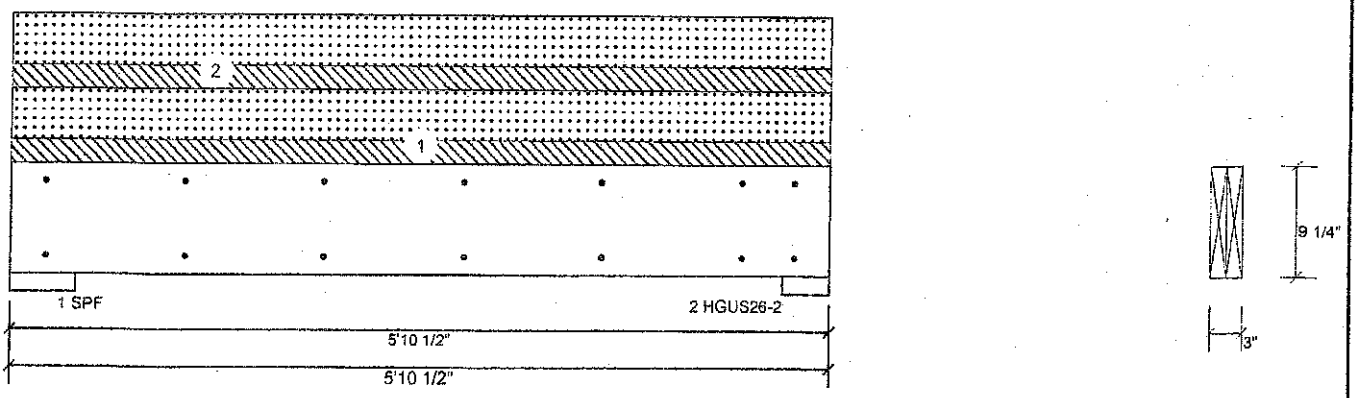
DWG NO. TAM 1923564
STRUCTURAL
COMPONENT ONLY

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115

This design is valid until 12/11/2021

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015 / OBC 2012
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	274	593	0
2	0	262	568	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	342 / 889	1231	L	1.25D+1.5S
2 - HGUS...	4.000"	16%	328 / 852	1180	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1392 ft-lb	3'	6039 ft-lb	0.230 (23%)	1.25D+1.5S	L
Unbraced	1392 ft-lb	3'	5236 ft-lb	0.266 (27%)	1.25D+1.5S	L
Shear	1069 lb	4'10"	3984 lb	0.268 (27%)	1.25D+1.5S	L
LL Defl inch	0.012 (L/5208)	3'	0.174 (L/360)	0.070 (7%)	S	L
TL Defl inch	0.018 (L/3562)	3'	0.174 (L/360)	0.100 (10%)	D+S	L

Design Notes

1 Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
09/12/16
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 7723563
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-9-12	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	

This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

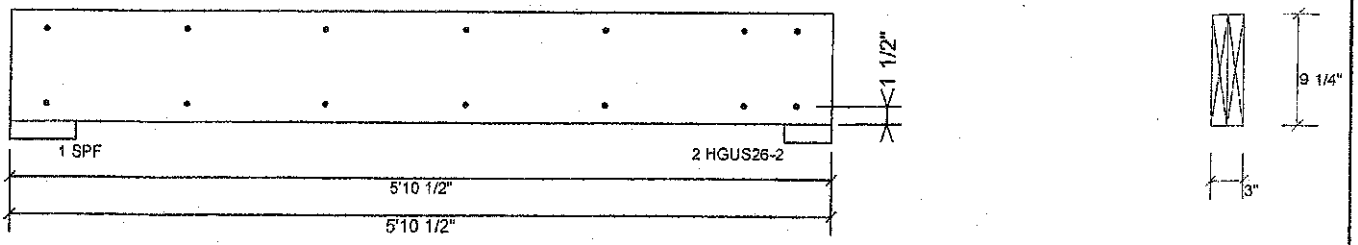
isDesign™

Client:
Project:
Address:

Date: 2/2019
Designer:
Job Name: 405481
Project #:

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	77.2 %
Load	175.1 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
09/12/19
H. J. G ALVES
100099024
PROVINCE OF ONTARIO

DWG NO. TAM T1923563
STRUCTURAL COMPONENT ONLY 3/2

RECEIVED
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CITY OF CALEDON
BUILDING DIVISION
FILE NO.

This design is valid until 12/11/2021

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

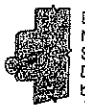
Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

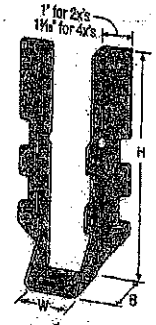
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.


Double-Shear Nailing Top View

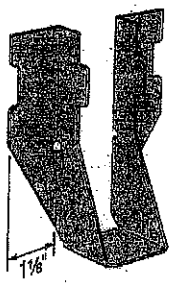

Double-Shear Nailing Side View; Do not bend tab


Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

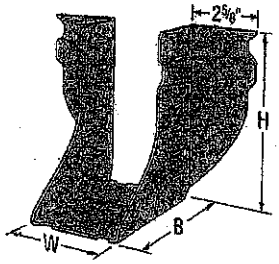


LUS28

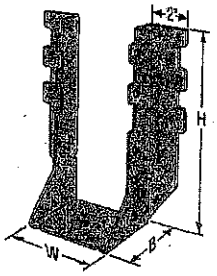


LU26L

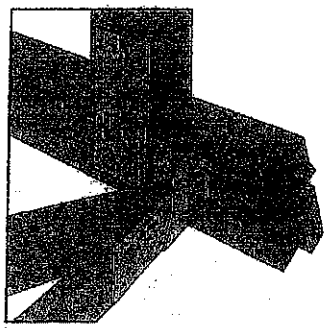
HUS210
(HUS26, HUS28, and HHUS similar)

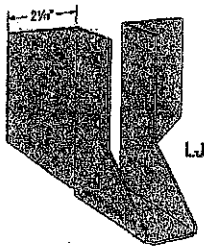


HGUS28-2



HHUS210-2


Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

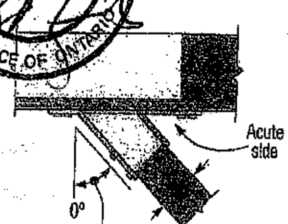
HHUS – Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS – Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[2]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1 9/16	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								[illegible]	[illegible]	2.87	5.14
LU24L	22	1 9/16	3	1 5/8	2 11/16	(4) 10d	(2) 10d x 1 1/2"	380	1020	320	725
								[illegible]	[illegible]	1.42	3.22
LU26L	22	1 9/16	5	1 5/8	4 5/8	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								[illegible]	[illegible]	2.87	5.07
LUS26	18	1 9/16	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								[illegible]	[illegible]	5.74	7.25
HUS26	16	1 5/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
								[illegible]	[illegible]	9.20	17.24
LJS26DS	18	1 9/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
								[illegible]	[illegible]	6.49	18.31
HGUS26	12	1 5/8	5 3/8	5	4 1/8	(20) 16d	(8) 16d	2685	6625	2685	5700
								11.96	[illegible]	11.96	25.35
LU28L	20	1 9/16	6 3/4	1 5/8	5 7/8	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								[illegible]	[illegible]	4.54	6.89
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								[illegible]	[illegible]	5.74	7.96
HUS28	16	1 5/8	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3605	5365	2675	4345
								16.04	[illegible]	11.90	19.33
HGUS28	12	1 5/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3310	6900
								14.74	[illegible]	14.74	30.73
LU210L	20	1 9/16	8	1 5/8	7 5/8	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1 9/16	7 13/16	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

SIMPSON Strong-Tie®

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	Dimensions (in.) H	Dimensions (in.) B	Dimensions (in.) d_e[3]	Fasteners Header	Fasteners Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	Factored Resistance D.Fir-L Normal (K_D = 1.00) lb. / kN	Factored Resistance S-P-F Uplift (K_D = 1.15) lb. / kN	Factored Resistance S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835	2020	590	1436
								3.71	8.99	2.62	6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 5/16	5⅜	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2066	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 5/16	7 5/16	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 5/16	9 5/16	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6⅛	8⅞	3	7 15/16	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 9/16	4¾	2	3 7/16	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3⅝	5¼	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 9/16	6¾	2	3 7/16	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3⅝	7 1/16	4	6 1/16	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 9/16	8¾	2	5 9/16	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3⅝	9	4	8 1/16	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3⅝	10 7/16	4	10 1/16	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3⅝	12 7/16	4	11 1/16	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80

See footnotes on p. 258.

LICENSED PROFESSIONAL ENGINEER
1/15/18
B. D. BUNTING
PROVINCE OF ONTARIO

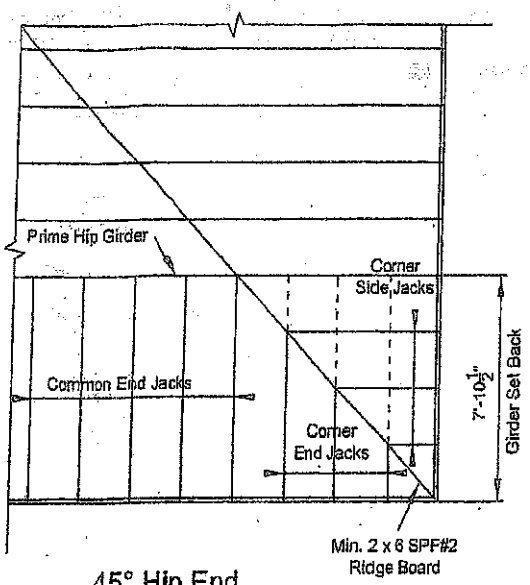
Plated Truss Connectors

LUMBER SPECIFICATION

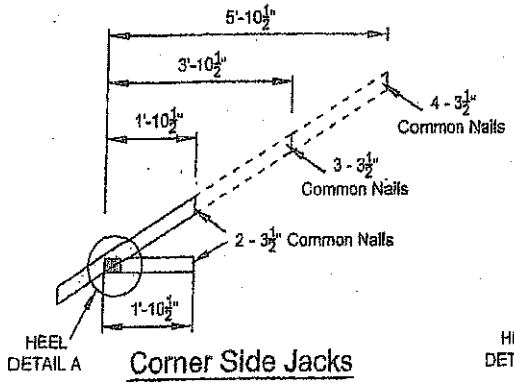
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

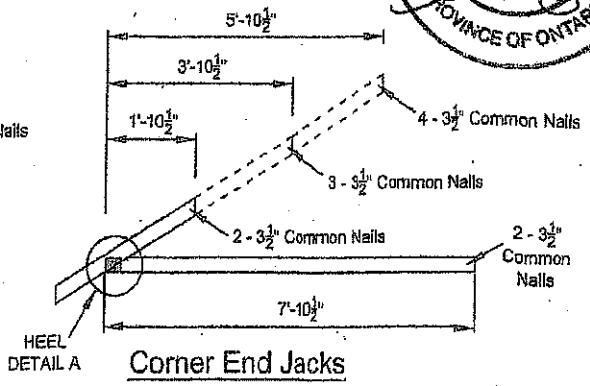
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F.



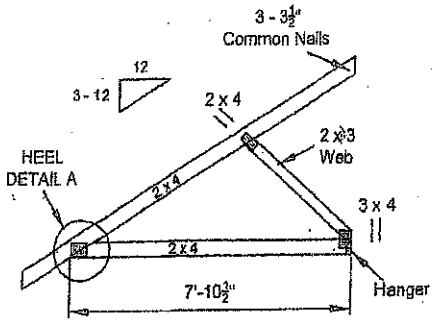
45° Hip End



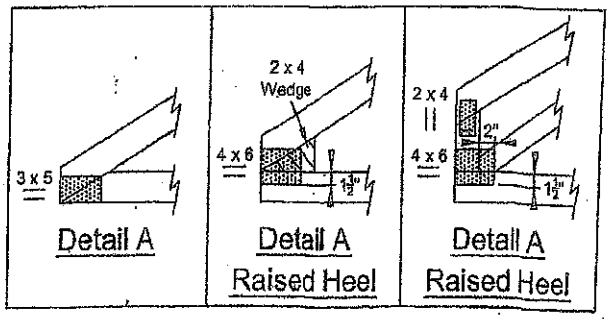
Corner Side Jacks



Corner End Jacks



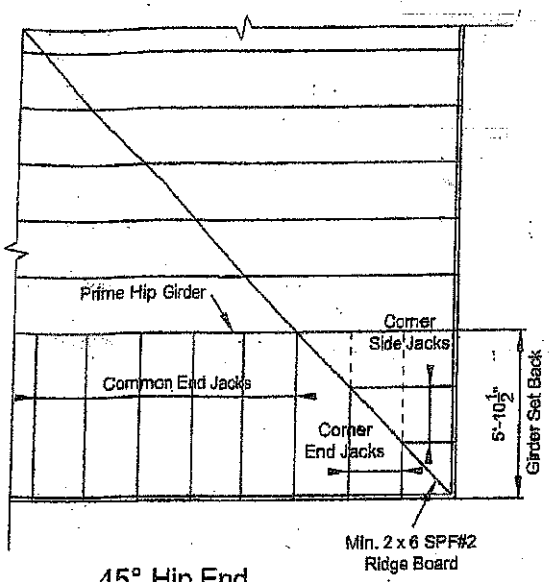
Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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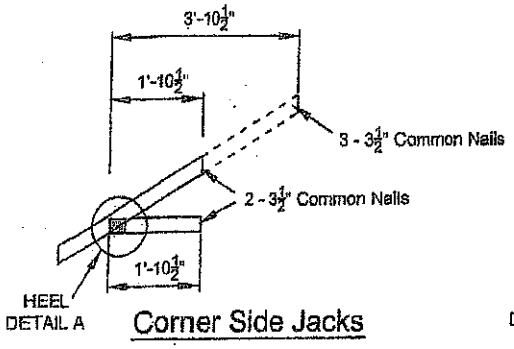
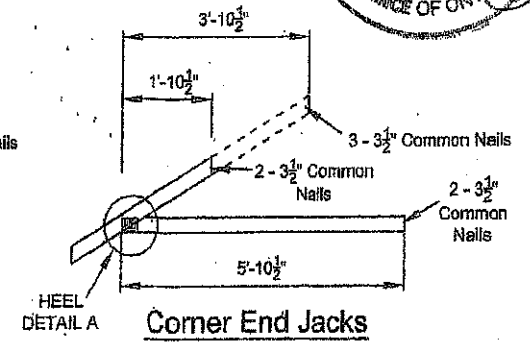
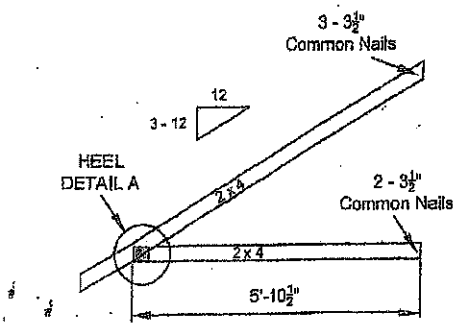


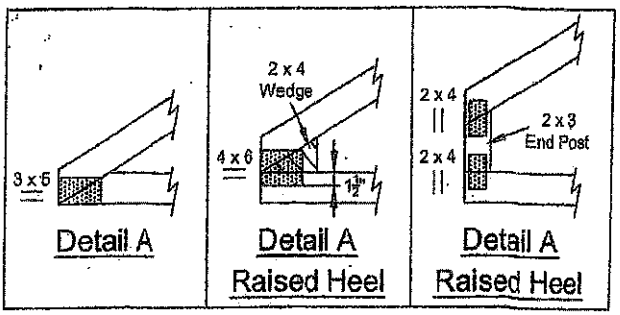
Corner Side Jacks



Corner End Jacks



Common End Jacks

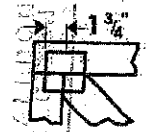


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

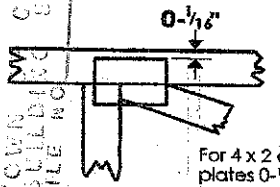
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

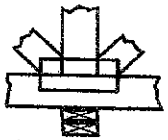
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

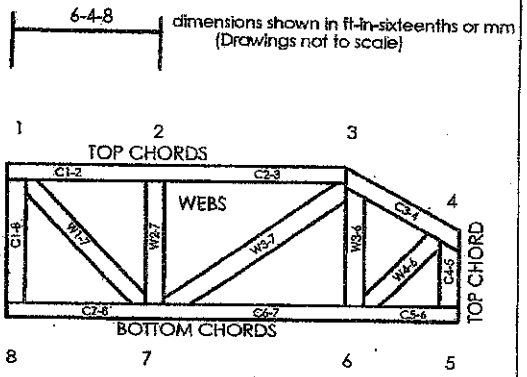


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications For Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

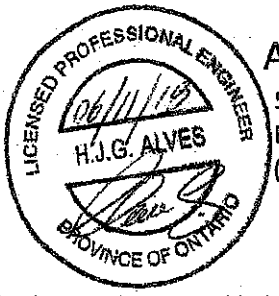
MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018