CERTIFIED MODEL
PRE-APPROVED
FOR PERMIT APPLICATION AS PER THE ONTARIO BUILDING CODE
TOWN OF CALEDON BUILDING DIVISION
REVIEWED BY JS J.SNELL
DATE JULY 25/19
FILE # HORIZON 3ES - ELEV. 1

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

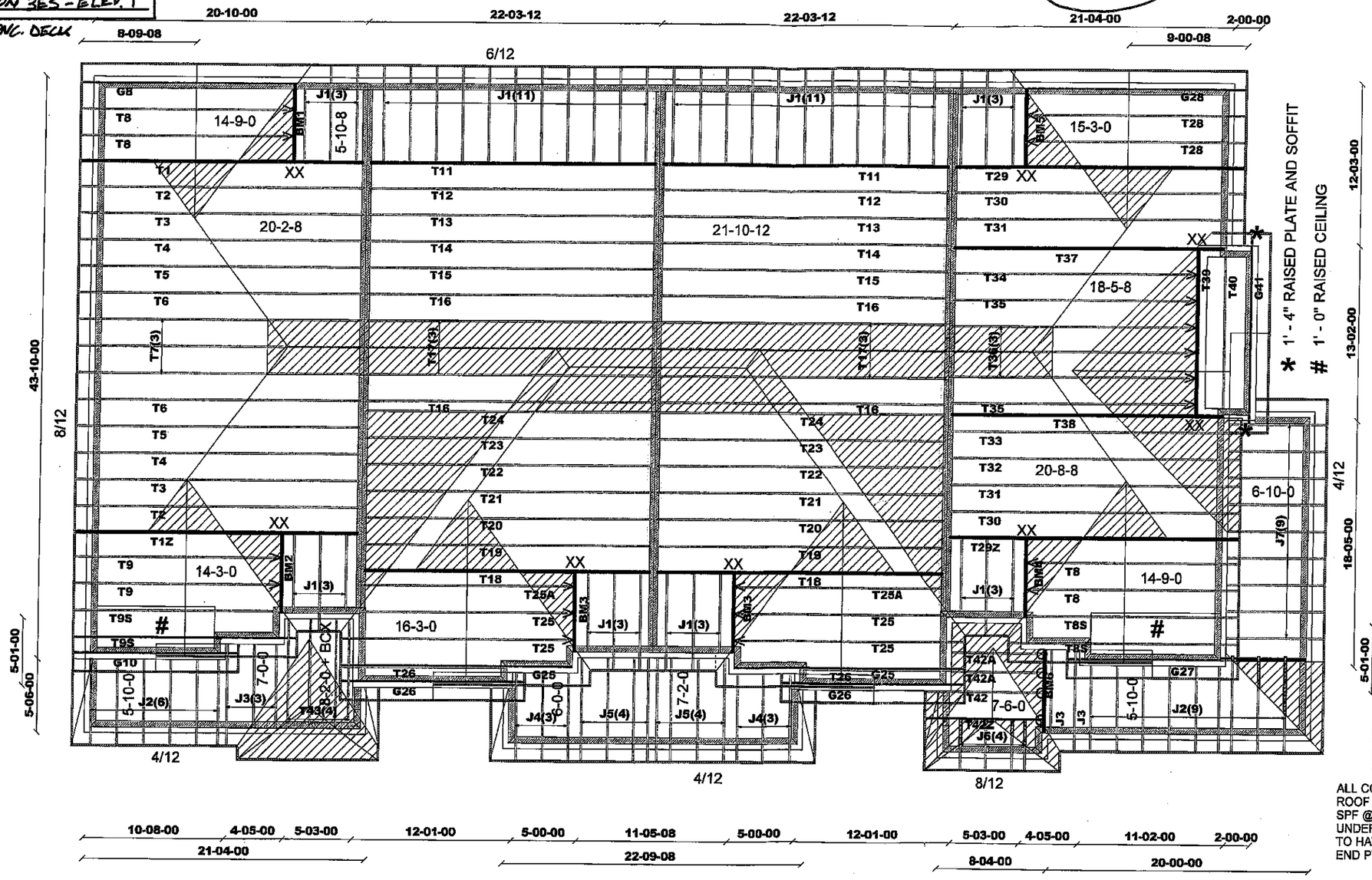
OPT. CONC. DECK

HORIZON 3E
TH 95-1 / 2
401779

HORIZON 2
TH 95-2 / 2
401780

HORIZON 2
TH 95-3 / 2
401781

HORIZON 3ES
TH 95-4 / 2
401782



ASPHALT SHINGLES
12"FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C.2012 PART 9
DESIGN LOADS:
SNOW LOAD 29.02 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

BM1 - BM6 = 2-2X10

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4
SPF @ 24"O.C. WITH A 2X4 VERT. POST TO THE TRUSS
UNDER NEATH AT EACH CROSS PT. VERT. POST LONGER THAN 8
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6"

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TOWN OF CALEDON
BUILDING SECTION
FILE NO

TOWN COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track: 50033	Builder / Location: GREEN PARK HOMES / CALEDON	Model / Elevation: BLOCK 95 / TH95-1 - TH95-4	Mitek ver 8.2.3.229
Plan Log: 200637	Project: LAMBERT LANE PH.2	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
Layout ID: 401769	Date: 3/8/2019 Sales: Mario DiCano Designer: AC		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401779
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	1 of 2
Lot #:		Date:	03/08/2019
Elevation:	TH95-1 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Half Hip Girder	8 /12	20-02-08	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	187.4 118.33		
	1 2-ply	T1Z Half Hip Girder	8 /12	20-02-08	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	187.4 118.33		
	2	T2 Half Hip	8 /12	20-02-08	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	173.45 110.00		
	2	T3 Half Hip	8 /12	20-02-08	6-01-04	2 x 4	1-03-08	1-04-13 6-01-04	174.28 110.67		
	2	T4 Half Hip	8 /12	20-02-08	7-01-04	2 x 4	1-03-08	1-04-13 7-01-04	187.92 119.00		
	2	T5 Half Hip	8 /12	20-02-08	8-01-04	2 x 4	1-03-08	1-04-13 8-01-04	198.8 124.33		
	2	T6 Half Hip	8 /12	20-02-08	9-01-04	2 x 4	1-03-08	1-04-13 9-01-04	210.2 131.67		
	3	T7 Half Hip	8 /12	20-02-08	10-01-04	2 x 4	1-03-08	1-04-13 10-01-04	329.66 208.00		
	2	T8 Common	8 /12	14-09-00	6-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	126.65 78.00		
	1	G8 GABLE	8 /12	14-09-00	6-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	65.08 42.00		
	2	T9 Common	8 /12	14-03-00	6-01-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	122.89 78.00		
	2	T9S Roof Special Structural Gable	8 /12	14-03-00	6-01-13	2 x 4	1-03-08 1-03-08	1-04-13 4-13	128.67 83.33		
	1	G10 GABLE	8 /12	9-10-00	4-05-14	2 x 4	1-03-08 1-03-08	4-13 4-13	[illegible] [illegible]		
	4	T43 Monopitch	4 /12	8-02-00	3-00-10	2 x 4		3-15 3-00-10	110.7 70.67		

THIS SHIPLIST IS ACCEPTED FOR ONTARIO BUILDING CODE

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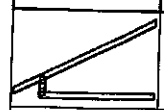
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401779
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	2 of 2
Lot #:		Date:	03/08/2019
Elevation:	TH95-1 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	6	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		
	6	J2 Jack-Open	4 /12	**5-10-00**	2-08-03	2 x 4	1-03-08	3-15 2-00-04	91.96 56.00		
	3	J3 Jack-Open	4 /12	**7-00-00**	3-00-14	2 x 4	1-03-08	3-15 2-04-15	66.73 40.00		

TOTAL # TRUSS= **44** TOTAL BFT OF ALL TRUSSES= **1573.66** BFT. TOTAL WEIGHT OF ALL TRSSES **2496.6** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **6**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401780
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	1 of 2
Lot #:		Date:	03/08/2019
Elevation:	TH95-2 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T11 Flat Girder	0 /12	21-10-12	4-01-04	2 x 4		4-01-04 4-01-04	173.9 109.33		
	1	T12 Flat	0 /12	21-10-12	5-01-04	2 x 4		5-01-04 5-01-04	87.9 54.50		
	1	T13 Flat	0 /12	21-10-12	6-01-04	2 x 4		6-01-04 6-01-04	93.24 56.83		
	1	T14 Flat	0 /12	21-10-12	7-01-04	2 x 4		7-01-04 7-01-04	98.74 60.67		
	1	T15 Flat	0 /12	21-10-12	8-01-04	2 x 4		8-01-04 8-01-04	114.41 70.00		
	2	T16 Flat	0 /12	21-10-12	9-01-04	2 x 4		9-01-04 9-01-04	241.79 148.67		
	3	T17 Flat	0 /12	21-10-12	10-01-04	2 x 4		10-01-04 10-01-04	382.54 230.00		
	1 2-ply	T18 Half Hip Girder	8 /12	21-10-12	4-01-04	2 x 4 2 x 6		1-07-02 4-01-04	203.49 126.67		
	1	T19 Half Hip	8 /12	21-10-12	5-01-04	2 x 4		1-07-02 5-01-04	89.94 56.33		
	1	T20 Half Hip	8 /12	21-10-12	6-01-04	2 x 4		1-07-02 6-01-04	98.02 62.67		
	1	T21 Half Hip	8 /12	21-10-12	7-01-04	2 x 4		1-07-02 7-01-04	97.25 61.83		
	1	T22 Half Hip	8 /12	21-10-12	8-01-04	2 x 4		1-07-02 8-01-04	102.52 64.50		
	1	T23 Half Hip	8 /12	21-10-12	9-01-04	2 x 4		1-07-02 9-01-04	108.04 67.[illegible]		
	1	T24 Half Hip	8 /12	21-10-12	10-01-04	2 x 4		1-07-02 10-01-04	120.57 75.87		

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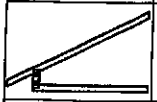
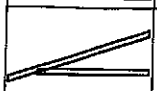
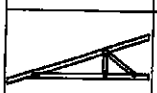
DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401780
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	2 of 2
Lot #:		Date:	03/08/2019
Elevation:	TH95-2 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T25 Common	8 /12	**16-03-00**	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	144.92 91.33		
	1	T25A Common	8 /12	**15-11-08**	6-09-13	2 x 4	1-03-08	1-04-13 1-04-13	70.01 44.50		
	1	G25 GABLE	8 /12	**16-03-00**	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	72.69 46.17		
	1	T26 Common	8 /12	**11-03-00**	5-01-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	46.73 29.83		
	1	G26 GABLE	8 /12	**11-03-00**	5-01-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49.01 31.83		
	14	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	3	J4 Jack-Open	4 /12	**6-00-00**	2-08-14	2 x 4	1-03-08	3-15 2-00-15	47.15 28.00		
	4	J5 Jack-Open	4 /12	**6-05-00**	3-01-08	2 x 4	1-03-08	3-15 2-05-10	85.14 58.00		

TOTAL # TRUSS= **46** TOTAL BFT OF ALL TRUSSES= **1723.83** BFT. TOTAL WEIGHT OF ALL TRSSES **2763.12** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **4**

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TOWN OF CALEDON
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FILE NO

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401781
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	1 of 2
Lot #:		Date:	03/08/2019
Elevation:	TH95-3 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T11 Flat Girder	0 /12	21-10-12	4-01-04	2 x 4		4-01-04 4-01-04	173.9 109.33		
	1	T12 Flat	0 /12	21-10-12	5-01-04	2 x 4		5-01-04 5-01-04	87.9 54.50		
	1	T13 Flat	0 /12	21-10-12	6-01-04	2 x 4		6-01-04 6-01-04	93.24 56.83		
	1	T14 Flat	0 /12	21-10-12	7-01-04	2 x 4		7-01-04 7-01-04	98.74 60.67		
	1	T15 Flat	0 /12	21-10-12	8-01-04	2 x 4		8-01-04 8-01-04	114.41 70.00		
	2	T16 Flat	0 /12	21-10-12	9-01-04	2 x 4		9-01-04 9-01-04	241.79 148.67		
	3	T17 Flat	0 /12	21-10-12	10-01-04	2 x 4		10-01-04 10-01-04	382.54 230.00		
	1 2-ply	T18 Half Hip Girder	8 /12	21-10-12	4-01-04	2 x 4 2 x 6		1-07-02 4-01-04	203.49 126.67		
	1	T19 Half Hip	8 /12	21-10-12	5-01-04	2 x 4		1-07-02 5-01-04	89.94 56.33		
	1	T20 Half Hip	8 /12	21-10-12	6-01-04	2 x 4		1-07-02 6-01-04	98.02 62.67		
	1	T21 Half Hip	8 /12	21-10-12	7-01-04	2 x 4		1-07-02 7-01-04	97.25 61.83		
	1	T22 Half Hip	8 /12	21-10-12	8-01-04	2 x 4		1-07-02 8-01-04	102.52 64.50		
	1	T23 Half Hip	8 /12	21-10-12	9-01-04	2 x 4		1-07-02 9-01-04	108.0[illegible] [illegible]		
	1	T24 Half Hip	8 /12	21-10-12	10-01-04	2 x 4		1-07-02 10-01-04	120.57 75.67		

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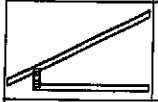
DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401781
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	2 of 2
Lot #:		Date:	03/08/2019
Elevation:	TH95-3 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T25 Common	8 /12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	144.92 91.33		
	1	T25A Common	8 /12	15-11-08	6-09-13	2 x 4	1-03-08	1-04-13 1-04-13	70.01 44.50		
	1	G25 GABLE	8 /12	16-03-00	6-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	72.69 46.17		
	1	T26 Common	8 /12	11-03-00	5-01-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	46.73 29.83		
	1	G26 GABLE	8 /12	11-03-00	5-01-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	49.01 31.83		
	14	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	235.12 149.33		
	3	J4 Jack-Open	4 /12	6-00-00	2-08-14	2 x 4	1-03-08	3-15 2-00-15	47.15 28.00		
	4	J5 Jack-Open	4 /12	6-05-00	3-01-08	2 x 4	1-03-08	3-15 2-05-10	85.14 58.00		

TOTAL # TRUSS= **46** TOTAL BFT OF ALL TRUSSES= **1723.83** BFT. TOTAL WEIGHT OF ALL TRSSES **2763.12** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **4**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401782
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	1 of 3
Lot #:		Date:	03/08/2019
Elevation:	TH95-4 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

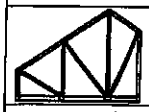
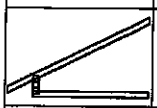
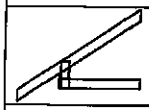
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T8 Common	8 /12	14-09-00	6-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	126.65 78.00		
	2	T8S Roof Special	8 /12	14-09-00	6-03-13	2 x 4	1-03-08 1-03-08	1-04-13 4-13	131.94 84.33		
	1	G27 GABLE	8 /12	10-04-00	4-07-14	2 x 4	1-03-08 1-03-08	4-13 4-13	35.7 22.00		
	2	T28 Common	8 /12	15-03-00	6-05-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	130.4 79.33		
	1	G28 GABLE	8 /12	15-03-00	6-05-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	67.6 42.67		
	1 2-ply	T29 Half Hip Girder	8 /12	20-08-08	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	199.48 127.33		
	1 2-ply	T29Z Half Hip Girder	8 /12	20-08-08	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	199.48 127.33		
	2	T30 Half Hip	8 /12	20-08-08	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	176.32 111.33		
	2	T31 Half Hip	8 /12	20-08-08	6-01-04	2 x 4	1-03-08	1-04-13 6-01-04	192.48 119.67		
	1	T32 Half Hip	8 /12	20-08-08	7-01-04	2 x 4	1-03-08	1-04-13 7-01-04	95.36 60.17		
	1	T33 Half Hip	8 /12	20-08-08	8-01-04	2 x 4	1-03-08	1-04-13 8-01-04	100.76 63.17		
	1	T34 Half Hip	8 /12	18-05-08	8-02-00	2 x 4		2-10-13 8-02-00	92.78 59.00		
	2	T35 Half Hip	8 /12	18-05-08	9-02-00	2 x 4		2-10-13 9-02-00	202.04 128.67		
	3	T36 Half Hip	8 /12	18-05-08	10-02-00	2 x 4		2-10-13 10-02-00	317.17 198.50		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401782
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	2 of 3
Lot #:		Date:	03/08/2019
Elevation:	TH95-4 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T37 Half Hip Girder	8 /12	**20-08-08**	7-02-00	2 x 4 2 x 6		1-04-13 7-02-00	239.33 150.33		
	1 2-ply	T38 Half Hip Girder	8 /12	**20-08-08**	8-08-08	2 x 4 2 x 6		1-04-13 8-08-08	237.1 145.67		
	1 2-ply	T39 Common Girder	8 /12	**12-04-00**	9-01-12	2 x 4 2 x 6		3-00-13 7-00-00	152.14 98.00		
	1	T40 Common	8 /12	**12-04-00**	7-09-12	2 x 4	1-03-08	1-08-13 5-08-00	61.81 40.00		
	1	G41 GABLE	8 /12	**12-04-00**	5-10-02	2 x 4	1-03-08 1-03-08	1-08-13 1-08-13	56.35 37.67		
	1	T42 Common	6 /12	**7-06-00**	2-10-03	2 x 4	1-03-08 1-03-08	6-12 6-12	24.55 15.67		
	2	T42A Common	6 /12	**6-11-00**	2-10-03	2 x 4	1-03-08	6-12 10-04	50.52 33.00		
	1	T42Z Common Girder	6 /12	**7-06-00**	2-10-03	2 x 4	1-03-08 1-03-08	6-12 6-12	24.55 15.67		
	6	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		
	9	J2 Jack-Open	4 /12	**5-10-00**	2-08-03	2 x 4	1-03-08	3-15 2-00-04	137.94 84.00		
	2	J3 Jack-Open	4 /12	**7-00-00**	3-00-14	2 x 4	1-03-08	3-15 2-04-15	44.49 26.67		
	4	J6 Jack-Open	8 /12	**2-05-08**	2-10-03	2 x 4	1-03-08	9-10 2-05-04	34.27 24.00		
	9	J7 Jack-Open	4 /12	**6-01-00**	3-00-03	2 x 4	1-03-08	3-15 2-04-04	183.01 118.50		

TOTAL # TRUSS= **66** TOTAL BFT OF ALL TRUSSES= **2154.68** BFT. TOTAL WEIGHT OF ALL TRSSES **3414.98** LBS

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200637
Project:	LAMBERT LANE PH.2	Layout ID:	401782
Location:	CALEDON	Ref #	
Model:	BLOCK 95	Page:	3 of 3
Lot #:		Date:	03/08/2019
Elevation:	TH95-4 / 2	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	
10	Hardware	LJS26DS	
4	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **18**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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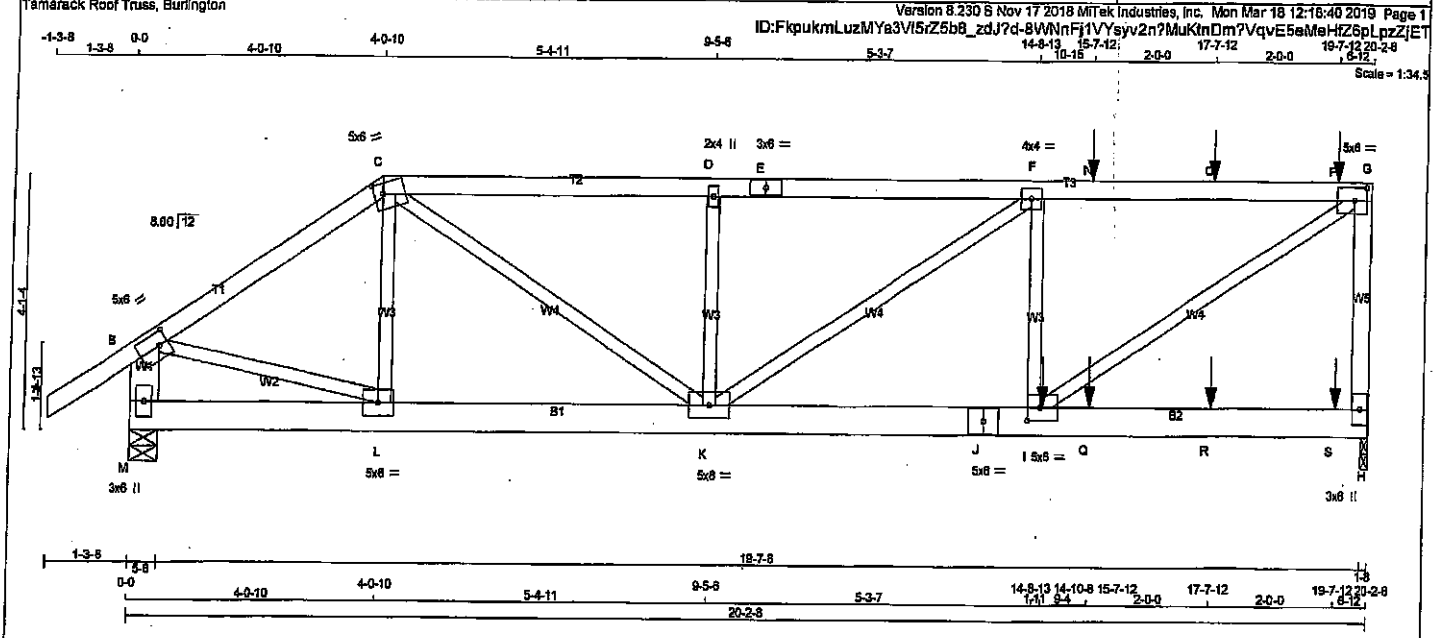

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	SIDE(0.0)
G- H	1	12	TOP
M- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(0.1)
J- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
I- F	1	6	SIDE(177.3)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2628	0	2628	0	0	1-8	1-8
M	1780	0	1780	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1844	1293 / 0	0 / 0	0 / 0	0 / 0	552 / 0	0 / 0
M	1247	883 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.08 (1)	10.00	
B- C	-1860 / 0	-102.1	-102.1	0.18 (1)	6.18	
C- D	-2779 / 0	-102.1	-102.1	0.25 (1)	5.23	
D- E	-2779 / 0	-102.1	-102.1	0.36 (1)	5.07	
E- F	-2779 / 0	-102.1	-102.1	0.36 (1)	5.07	
F- N	-3087 / 0	-102.1	-102.1	0.42 (1)	4.78	
N- O	-3087 / 0	-102.1	-102.1	0.42 (1)	4.78	
O- P	-3087 / 0	-102.1	-102.1	0.42 (1)	4.78	
P- G	-3087 / 0	-102.1	-102.1	0.42 (1)	4.78	
H- G	-2526 / 0	0.0	0.0	0.31 (1)	7.13	
M- B	-1753 / 0	0.0	0.0	0.06 (1)	7.81	
M- L	0 / 0	-17.5	-17.5	0.03 (4)	10.00	
L- K	0 / 1536	-17.5	-17.5	0.12 (1)	10.00	
K- J	0 / 3087	-17.5	-17.5	0.24 (1)	10.00	
J- I	0 / 3087	-17.5	-17.5	0.24 (1)	10.00	
I- Q	0 / 0	-17.5	-17.5	0.06 (4)	10.00	
Q- R	0 / 0	-17.5	-17.5	0.06 (4)	10.00	
R- S	0 / 0	-17.5	-17.5	0.06 (4)	10.00	
S- H	0 / 0	-17.5	-17.5	0.06 (4)	10.00	

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-285 / 0	0.03 (1)
B- L	0 / 1589	0.20 (1)
I- G	0 / 3735	0.46 (1)
C- K	0 / 1507	0.19 (1)
I- F	-610 / 0	0.08 (1)
K- D	-552 / 0	0.07 (1)
K- F	-377 / 0	0.05 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE		
I	14-10-8	-1361	-1361	—	BACK	VERT	TOTAL	—	—
N	15-7-12	-123	-123	—	BACK	VERT	TOTAL	—	—
O	17-7-12	-123	-123	—	BACK	VERT	TOTAL	—	—
P	19-7-12	-145	-145	—	BACK	VERT	TOTAL	—	—
Q	15-7-12	-25	-25	—	BACK	VERT	TOTAL	—	—
R	17-7-12	-25	-25	—	BACK	VERT	TOTAL	—	—
S	19-7-12	-29	-29	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.42/1.00 (F-G:1) , BC=0.24/1.00 (I-K:1) , WB=0.46/1.00 (G-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT 0.60)
JSI METAL= 0.99 (I) (INPUT 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
DWG NO. TAM 7129-5
CALEDON
STRUCTURAL SECTION
COMPONENTS ONLY
BUILDING
FILE NOW

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T1Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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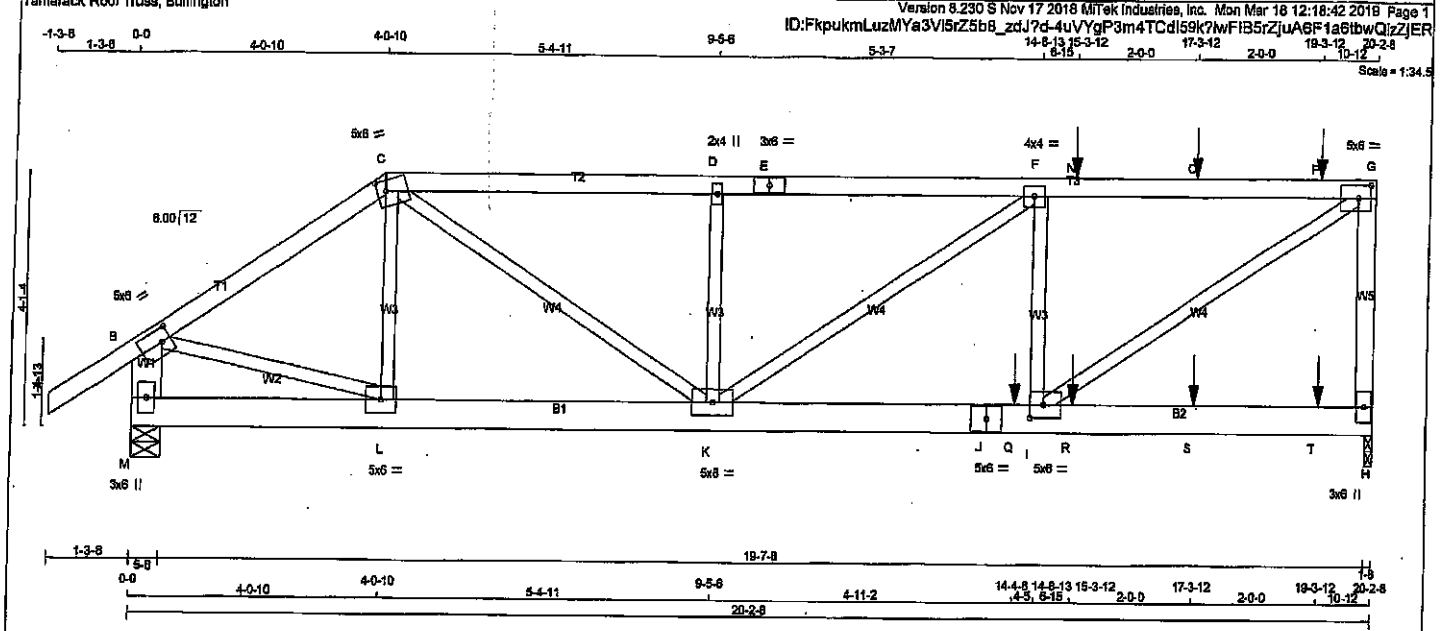

TOTAL WEIGHT = 2 X 94 = 187 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C 1	12		TOP
C-E 1	12		TOP
E-G 1	12		SIDE(0.0)
G-H 1	12		TOP
M-B 2	12		TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-J 2	12		TOP
J-H 2	12		SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3 1	6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.50
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2575	0	2575	0	0	1-8	1-8
M	1810	0	1810	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOADE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1606	1266 / 0	0 / 0	0 / 0	0 / 0	540 / 0	0 / 0
M	1268	898 / 0	0 / 0	0 / 0	0 / 0	370 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 39	-102.1	-102.1	0.08 (1)	10.00	
B-C	-1695 / 0	-102.1	-102.1	0.18 (1)	6.13	
C-D	-2875 / 0	-102.1	-102.1	0.25 (1)	5.16	
D-E	-2875 / 0	-102.1	-102.1	0.36 (1)	5.00	
E-F	-2875 / 0	-102.1	-102.1	0.36 (1)	5.00	
F-N	-3128 / 0	-102.1	-102.1	0.40 (1)	4.77	
N-O	-3128 / 0	-102.1	-102.1	0.40 (1)	4.77	
O-P	-3128 / 0	-102.1	-102.1	0.40 (1)	4.77	
P-G	-3128 / 0	-102.1	-102.1	0.40 (1)	4.77	
H-G	-2516 / 0	0.0	0.0	0.31 (1)	7.14	
M-B	-1779 / 0	0.0	0.0	0.06 (1)	7.81	
M-L	0 / 0	-17.5	-17.5	0.02 (4)	10.00	
L-K	0 / 1566	-17.5	-17.5	0.12 (1)	10.00	
K-J	0 / 3128	-17.5	-17.5	0.33 (1)	10.00	
J-Q	0 / 3128	-17.5	-17.5	0.33 (1)	10.00	
Q-I	0 / 3128	-17.5	-17.5	0.33 (1)	10.00	
I-R	0 / 0	-17.5	-17.5	0.06 (4)	10.00	
R-S	0 / 0	-17.5	-17.5	0.06 (4)	10.00	
S-T	0 / 0	-17.5	-17.5	0.06 (4)	10.00	
T-H	0 / 0	-17.5	-17.5	0.06 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-293 / 0	0.04 (1)
B-L	0 / 1618	0.20 (1)
I-G	0 / 3784	0.47 (1)
C-K	0 / 1590	0.20 (1)
I-F	-689 / 0	0.08 (1)
K-D	-552 / 0	0.07 (1)
K-E	-409 / 0	0.12 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	15-3-12	-123	-123	—	FRONT	VERT	TOTAL	—	—
O	17-3-12	-123	-123	—	FRONT	VERT	TOTAL	—	—
P	19-3-12	-128	-128	—	FRONT	VERT	TOTAL	—	—
Q	14-4-8	-1378	-1378	—	FRONT	VERT	TOTAL	—	—
R	15-3-12	-25	-25	—	FRONT	VERT	TOTAL	—	—
S	17-3-12	-25	-25	—	FRONT	VERT	TOTAL	—	—
T	19-3-12	-26	-26	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.40/1.00 (F-G:1) , BC=0.33/1.00 (I-K:1) , WB=0.47/1.00 (G-I:1) , SSI=0.45/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 30 Deg.

JSI GRIP= 0.77 (G) (INPUT = 0.90)
JSI SHEAR= 0.42 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009684
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
TOWN OF CALEDON
PLANNING SECTION

DWG NO. 401769 R10 T1Z
TRUSS STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T2	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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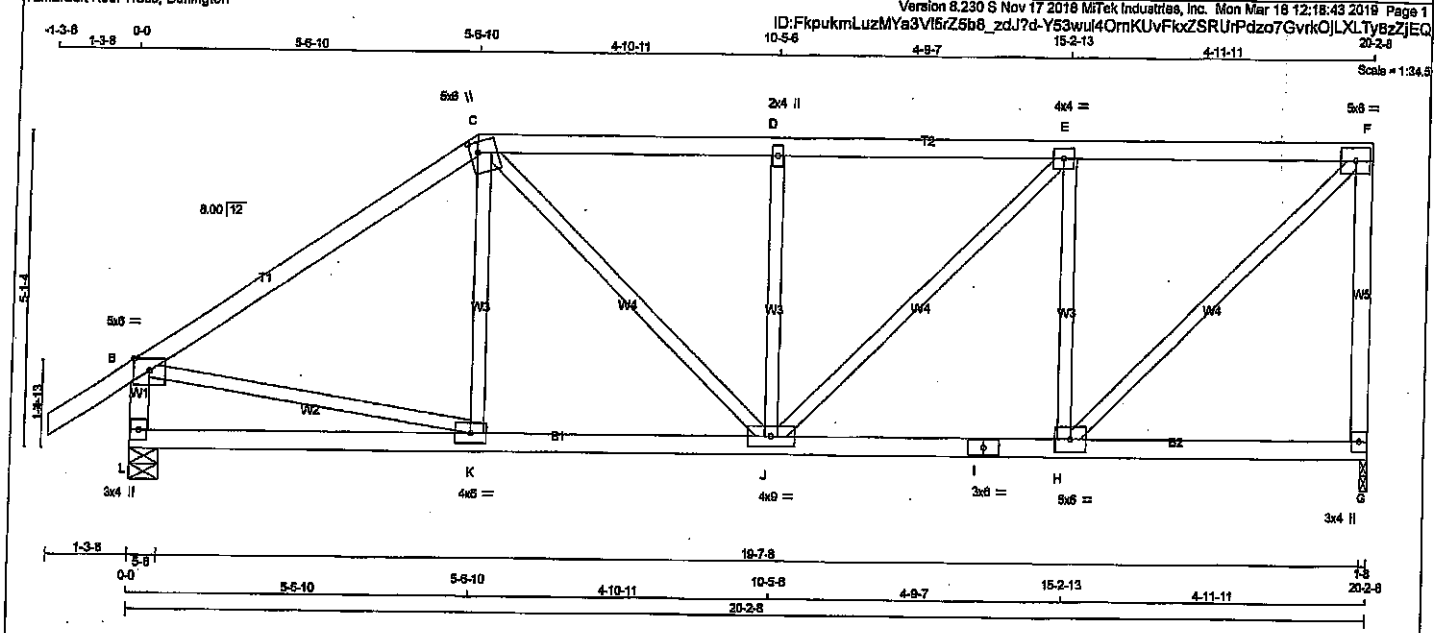

TOTAL WEIGHT = 2 X 87 = 173 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1208	0	1208	0	0	1-8	1-8
L	1349	0	1349	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	849	587 / 0	0 / 0	0 / 0	0 / 0	263 / 0	0 / 0
L	946	666 / 0	0 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	-1237 / 0	-102.1	-102.1	0.63 (1)	4.88	
C- D	-1317 / 0	-102.1	-102.1	0.35 (1)	5.20	
D- E	-1317 / 0	-102.1	-102.1	0.37 (1)	5.17	
E- F	-999 / 0	-102.1	-102.1	0.35 (1)	5.76	
G- F	-1173 / 0	0.0	0.0	0.52 (1)	7.36	
L- B	-1308 / 0	0.0	0.0	0.13 (1)	7.05	
L- K	0 / 0	-17.5	-17.5	0.12 (4)	10.00	
K- J	0 / 1026	-17.5	-17.5	0.23 (1)	10.00	
J- I	0 / 999	-17.5	-17.5	0.21 (1)	10.00	
I- H	0 / 999	-17.5	-17.5	0.21 (1)	10.00	
H- G	0 / 0	-17.5	-17.5	0.10 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-88 / 59	0.03 (1)
B- K	0 / 1047	0.24 (1)
H- F	0 / 1360	0.31 (1)
C- J	0 / 407	0.09 (1)
H- E	-868 / 0	0.33 (1)
J- D	-534 / 0	0.21 (1)
J- E	0 / 451	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.63/1.00 (B-C:1) , BC=0.23/1.00 (J-K:1) , WB=0.33/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.30 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T190 5354
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T3	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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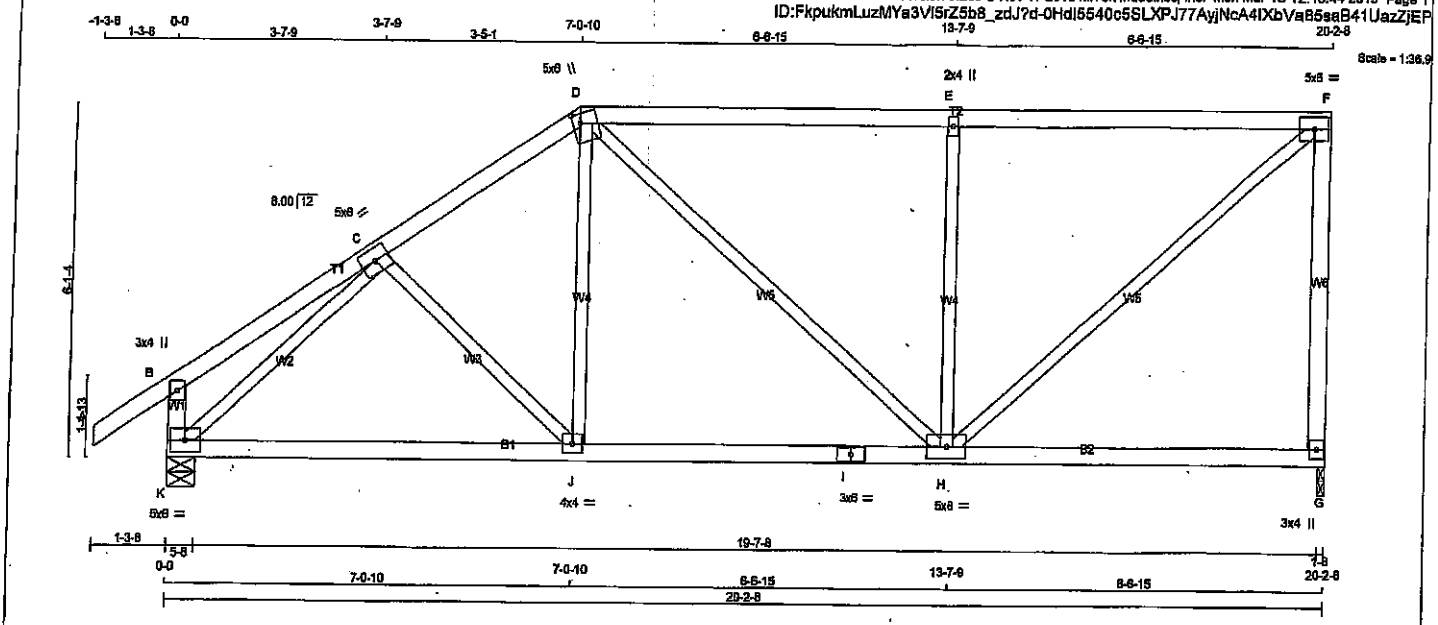

TOTAL WEIGHT = 2 X 87 = 174 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1208	0	1206	0	0	1-8	1-8
K	1349	0	1349	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	849	567 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	946	666 / 0	0 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	0 / 22	-102.1	-102.1	0.19 (1)	10.00	
C- D	-1188 / 0	-102.1	-102.1	0.22 (1)	5.80	
D- E	-1029 / 0	-102.1	-102.1	0.82 (1)	4.61	
E- F	-1029 / 0	-102.1	-102.1	0.82 (1)	4.61	
G- F	-1160 / 0	0.0	0.0	0.84 (1)	7.39	
K- B	-280 / 0	0.0	0.0	0.03 (1)	7.61	
K- J	0 / 1044	-17.5	-17.5	0.27 (1)	10.00	
J- I	0 / 972	-17.5	-17.5	0.27 (4)	10.00	
I- H	0 / 972	-17.5	-17.5	0.27 (4)	10.00	
H- G	0 / 0	-17.5	-17.5	0.16 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-105 / 16	0.04 (1)
J- D	0 / 204	0.05 (4)
D- H	0 / 76	0.02 (1)
H- E	-833 / 0	0.49 (1)
H- F	0 / 1369	0.31 (1)
K- C	-1458 / 0	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.84/1.00 (F-G:1) , BC=0.27/1.00 (J-K:1) , WB=0.56/1.00 (C-K:1) , SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.73 (H) (INPUT = 0.90)
MSI METAL= 0.35 (C) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. G. ALVES
100090024
PROVINCE OF ONTARIO

DWG NO. TAM T1905355
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T4	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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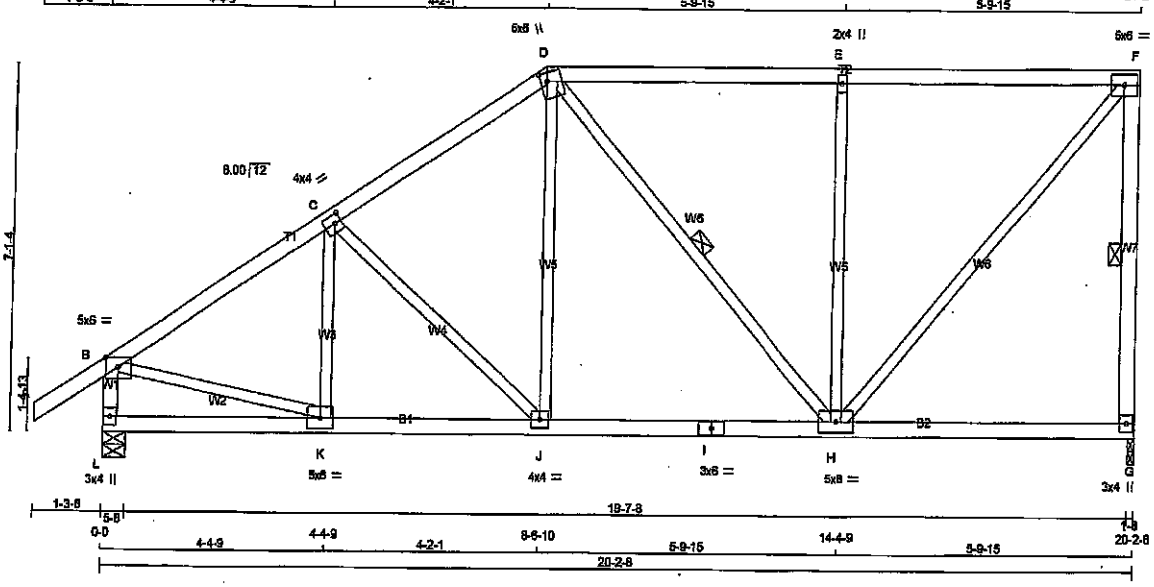

TOTAL WEIGHT = 2 X 94 = 188 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1208	0	1208	0	0	1-8	1-8
L	1349	0	1349	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	849	587 / 0	0 / 0	0 / 0	0 / 0	263 / 0	0 / 0
L	946	666 / 0	0 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B- C	-1290 / 0	-102.1	-102.1	0.35 (1)	5.26
C- D	-1086 / 0	-102.1	-102.1	0.33 (1)	5.62
D- E	-808 / 0	-102.1	-102.1	0.61 (1)	5.66
E- F	-809 / 0	-102.1	-102.1	0.61 (1)	5.66
G- F	-1167 / 0	0.0	0.0	0.27 (1)	5.90
L- B	-1315 / 0	0.0	0.0	0.14 (1)	7.05
L- K	0 / 0	-17.5	-17.5	0.07 (4)	10.00
K- J	0 / 1096	-17.5	-17.5	0.22 (1)	10.00
J- I	0 / 882	-17.5	-17.5	0.22 (1)	10.00
I- H	0 / 882	-17.5	-17.5	0.22 (1)	10.00
H- G	0 / 0	-17.5	-17.5	0.15 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-193 / 10	0.05 (1)
C- J	-299 / 0	0.18 (1)
J- D	0 / 290	0.07 (1)
D- H	-114 / 0	0.08 (1)
H- E	-737 / 0	0.64 (1)
H- F	0 / 1239	0.28 (1)
B- K	0 / 1129	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.61/1.00 (E-F:1) , BC=0.22/1.00 (J-K:1) , WB=0.64/1.00 (E-H:1) , SSI=0.29/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.31 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009022
PROVINCE OF ONTARIO

RECEIVED
MAR 22 2019
DWG NO. TAMC-7825356
STRUCTURAL COMPONENT ONLY
TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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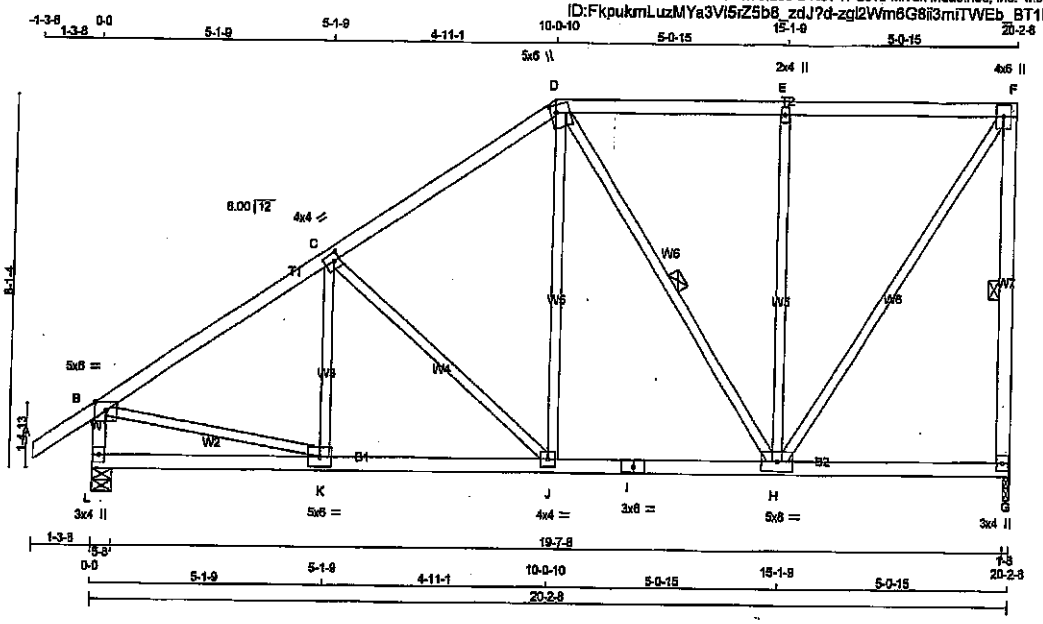

TOTAL WEIGHT = 2 X 99 = 199 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	8.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1208	0	1208	0	0	1-8	1-8
L	1349	0	1349	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	849	587 / 0	0 / 0	0 / 0	0 / 0	263 / 0	0 / 0
L	946	666 / 0	0 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	-1284 / 0	-102.1	-102.1	0.50 (1)	5.02	
C- D	-977 / 0	-102.1	-102.1	0.47 (1)	5.62	
D- E	-636 / 0	-102.1	-102.1	0.45 (1)	6.25	
E- F	-637 / 0	-102.1	-102.1	0.45 (1)	6.25	
G- F	-1171 / 0	0.0	0.0	0.35 (1)	5.90	
L- B	-1311 / 0	0.0	0.0	0.14 (1)	7.06	
L- K	0 / 0	-17.5	-17.5	0.11 (4)	10.00	
K- J	0 / 1105	-17.5	-17.5	0.23 (1)	10.00	
J- I	0 / 787	-17.5	-17.5	0.18 (1)	10.00	
I- H	0 / 787	-17.5	-17.5	0.18 (1)	10.00	
H- G	0 / 0	-17.5	-17.5	0.11 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-135 / 42	0.05 (1)
C- J	-433 / 0	0.36 (1)
J- D	0 / 373	0.08 (1)
D- H	-276 / 0	0.18 (1)
H- E	-641 / 0	0.81 (1)
H- F	0 / 1162	0.26 (1)
B- K	0 / 1129	0.25 (1)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.50/1.00 (B-C:1) , BC=0.23/1.00 (J-K:1) , WB=0.81/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.29 (F) (INPUT = 1.00)

DWG NO. TAM T905357
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T6	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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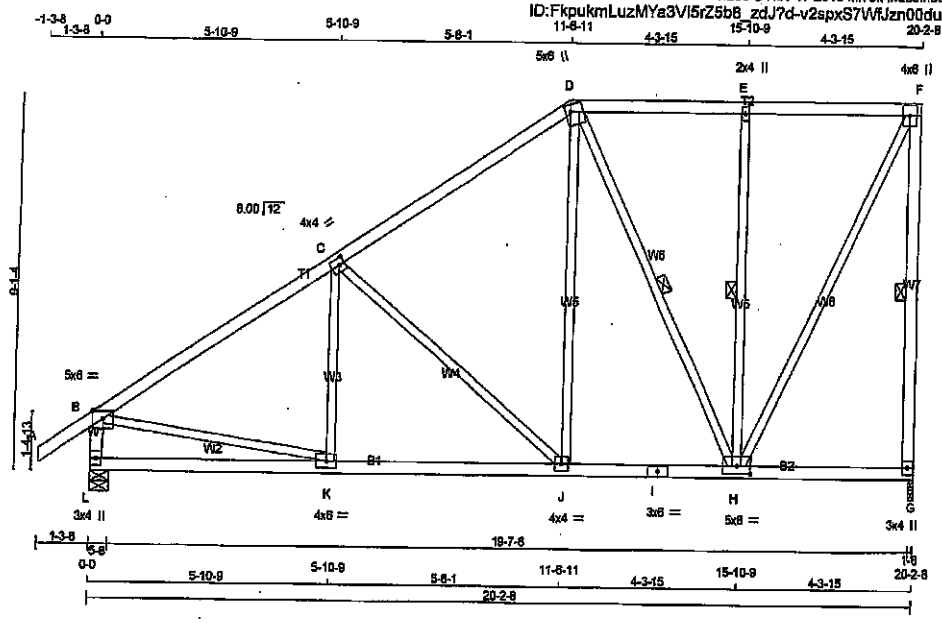

TOTAL WEIGHT = 2 X 105 = 210 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	4.00
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1208	0	1208	0	0	1-8	1-8
L	1349	0	1349	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	849	567 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	946	666 / 0	0 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B- C	-1285 / 0	-102.1	-102.1	0.48 (1)	5.09
C- D	-857 / 0	-102.1	-102.1	0.45 (1)	5.99
D- E	-497 / 0	-102.1	-102.1	0.24 (1)	6.25
E- F	-498 / 0	-102.1	-102.1	0.24 (1)	6.25
G- F	-1176 / 0	0.0	0.0	0.45 (1)	5.88
L- B	-1306 / 0	0.0	0.0	0.13 (1)	7.08
L- K	0 / 0	-17.5	-17.5	0.15 (4)	10.00
K- J	0 / 1101	-17.5	-17.5	0.25 (1)	10.00
J- I	0 / 684	-17.5	-17.5	0.15 (1)	10.00
I- H	0 / 684	-17.5	-17.5	0.15 (1)	10.00
H- G	0 / 0	-17.5	-17.5	0.07 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-87 / 71	0.04 (1)
C- J	-560 / 0	0.66 (1)
J- D	0 / 451	0.10 (1)
D- H	-425 / 0	0.29 (1)
H- E	-544 / 0	0.29 (1)
H- F	0 / 1120	0.25 (1)
B- K	0 / 1120	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.25/1.00 (J-K:1) , WB=0.66/1.00 (C-J:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1667	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.77 (B) (INPUT = 0.90)
USI METAL= 0.28 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
M. J. G. ALVES
100009076
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 19-405358
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T7	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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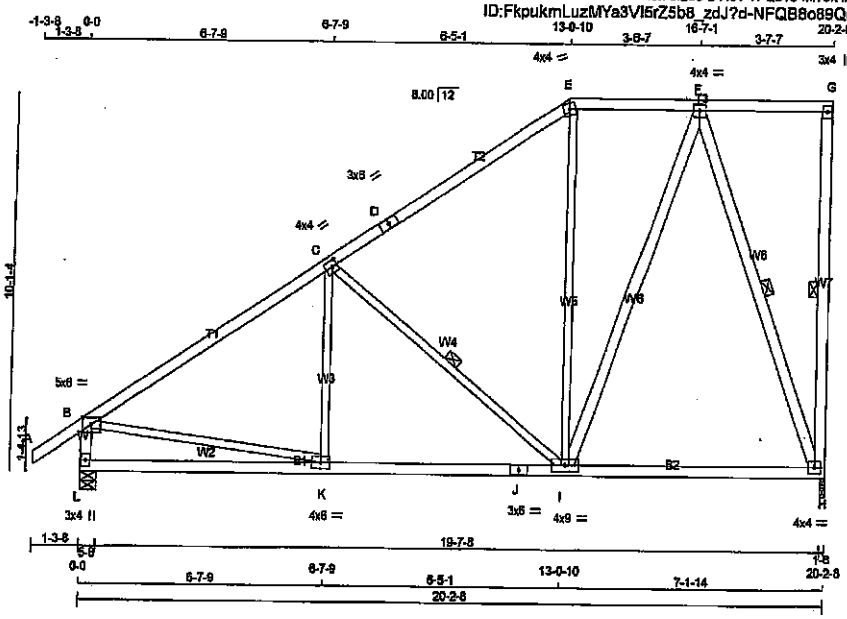

TOTAL WEIGHT = 3 X 110 = 330 lb
[MIF]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TB-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1208	0	1208	0	0	1-8	1-8
L	1349	0	1349	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	849	587 / 0	0 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	946	666 / 0	0 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, C-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B- C	-1262 / 0	-102.1	-102.1	0.63 (1)	4.85
C- D	-736 / 0	-102.1	-102.1	0.58 (1)	6.02
D- E	-736 / 0	-102.1	-102.1	0.58 (1)	6.02
E- F	-573 / 0	-102.1	-102.1	0.17 (1)	6.25
F- G	0 / 0	-102.1	-102.1	0.22 (1)	10.00
H- G	-141 / 0	0.0	0.0	0.07 (1)	6.25
L- B	-1300 / 0	0.0	0.0	0.13 (1)	7.08
L- K	0 / 0	-17.5	-17.5	0.17 (4)	10.00
K- J	0 / 1088	-17.5	-17.5	0.29 (1)	10.00
J- I	0 / 1088	-17.5	-17.5	0.29 (1)	10.00
I- H	0 / 375	-17.5	-17.5	0.22 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-56 / 65	0.03 (4)
C- I	-673 / 0	0.34 (1)
I- E	0 / 57	0.02 (4)
B- K	0 / 1102	0.25 (1)
I- F	0 / 589	0.10 (1)
F- H	-1085 / 0	0.62 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.63/1.00 (B-C:1) , BC=0.29/1.00 (I-K:1) , WB=0.62/1.00 (F-H:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.36 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
W. J. B. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1905359
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T8	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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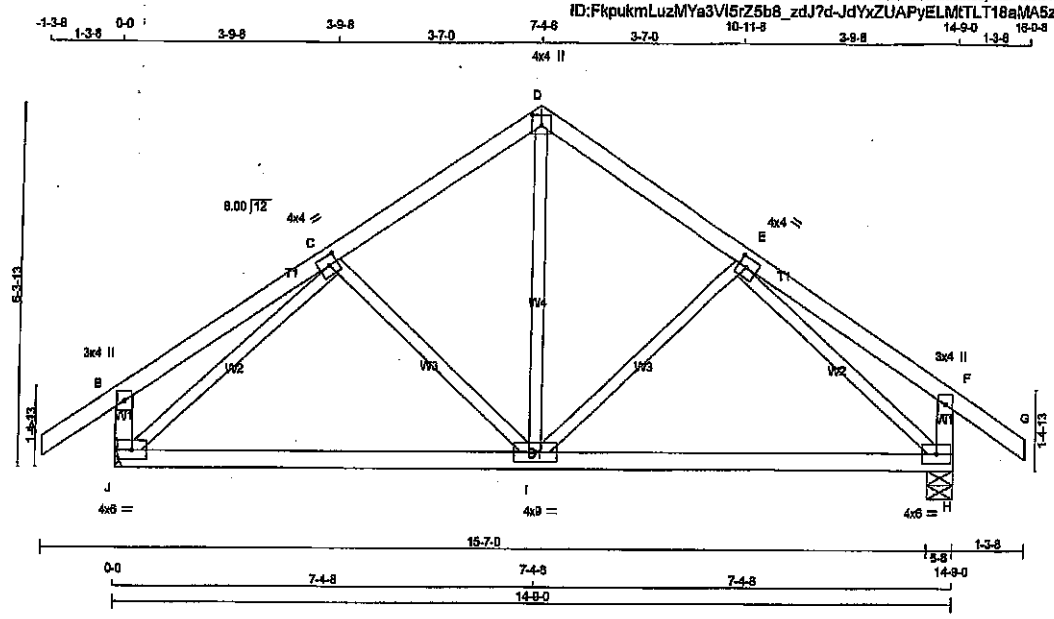

TOTAL WEIGHT = 4 X 63 = 253 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1022	0	1022	0	0	MECHANICAL	
H	1022	0	1022	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	716	508 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0
H	716	508 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	0 / 24	-102.1	-102.1	0.22 (1)	10.00	
C- D	-698 / 0	-102.1	-102.1	0.17 (1)	6.25	
D- E	-698 / 0	-102.1	-102.1	0.17 (1)	6.25	
E- F	0 / 24	-102.1	-102.1	0.22 (1)	10.00	
F- G	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
J- B	-285 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-285 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 716	-17.5	-17.5	0.32 (4)	10.00	
I- H	0 / 716	-17.5	-17.5	0.32 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 456	0.10 (1)
I- E	-215 / 0	0.09 (1)
C- I	-215 / 0	0.09 (1)
J- C	-993 / 0	0.41 (1)
E- H	-993 / 0	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.32/1.00 (I-J:4) , WB=0.41/1.00 (E-H:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (C) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H.J. G. ALVES
100009829
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
DWG NO. TAM 79053
STRUCTURAL OF COMPONENTS
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T8S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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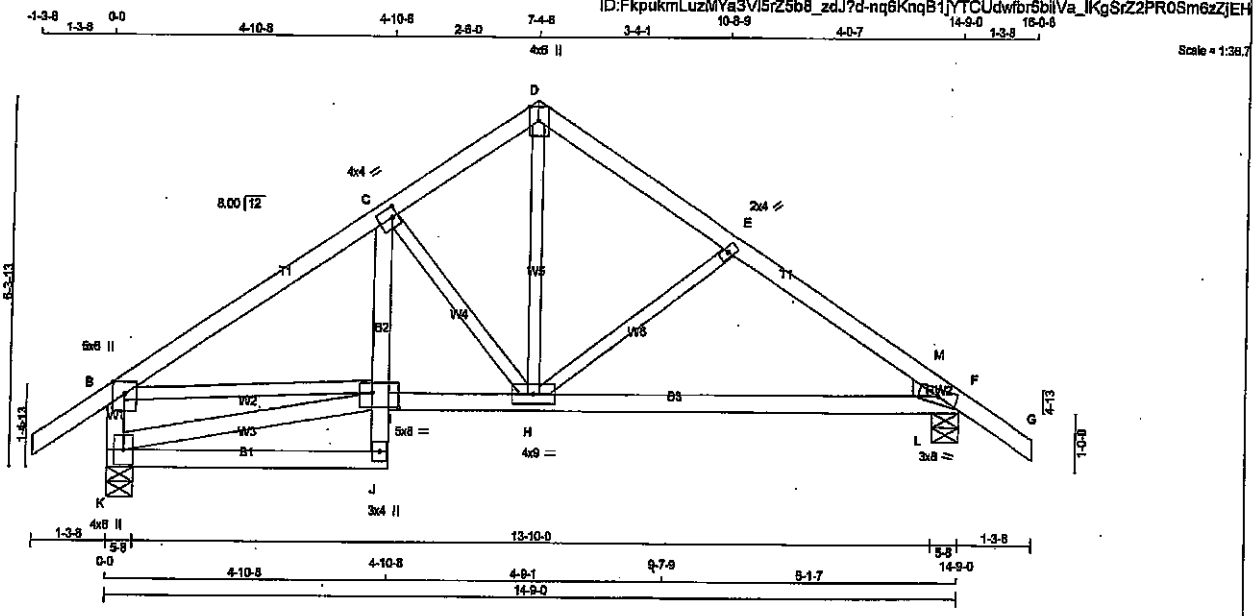

TOTAL WEIGHT = 2 X 66 = 132 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-m	MT20	3.0	8.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BVMWW-l	MT20	5.0	8.0	3.00	5.50
J	BMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
K	1022	0	1022	0	0	5-8	5-8	
F	1022	0	1022	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	716	508 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0
F	716	508 / 0	0 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	-1096 / 0	-102.1	-102.1	0.28 (1)	5.71	
C- D	-841 / 0	-102.1	-102.1	0.21 (1)	6.25	
D- E	-824 / 0	-102.1	-102.1	0.16 (1)	6.25	
E- M	-1134 / 0	-102.1	-102.1	0.17 (1)	5.79	
M- F	-1277 / 0	-102.1	-102.1	0.09 (4)	5.66	
F- G	0 / 36	-102.1	-102.1	0.14 (1)	10.00	
K- B	-976 / 0	0.0	0.0	0.10 (1)	7.81	
K- J	0 / 21	-17.5	-17.5	0.12 (4)	10.00	
J- I	0 / 46	0.0	0.0	0.04 (1)	10.00	
I- C	0 / 39	0.0	0.0	0.03 (1)	10.00	
I- H	0 / 937	-17.5	-17.5	0.25 (1)	10.00	
H- L	0 / 969	-17.5	-17.5	0.26 (1)	10.00	
L- F	0 / 969	-17.5	-17.5	0.17 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- I	-22 / 0	0.01 (1)
B- I	0 / 931	0.21 (1)
H- D	0 / 695	0.16 (1)
C- H	-429 / 0	0.12 (1)
H- E	-387 / 0	0.12 (1)
L- M	0 / 120	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.26/1.00 (H-L:1) , WB=0.21/1.00 (B-I:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
DWG. NO. AM 77105
STRUCTURAL COMPONENT ONLY
TOWN OF ORANGEVILLE
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	G8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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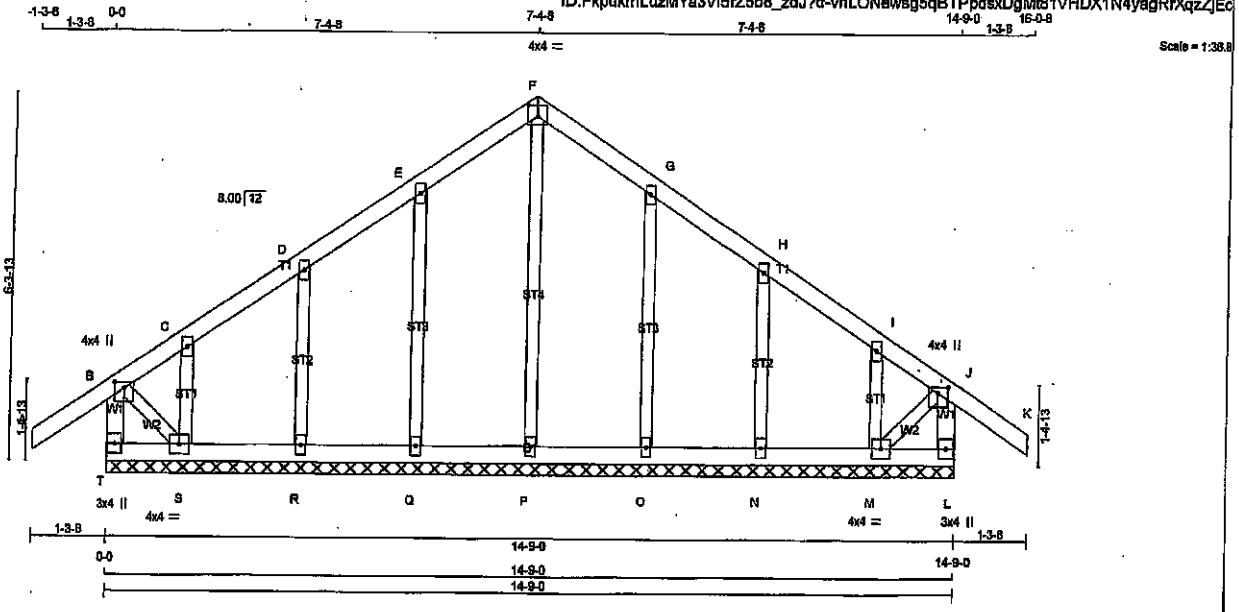

TOTAL WEIGHT = 65 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW/+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-303 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	-62 / 0	-102.1	-102.1	0.13 (1)	6.25	
C- D	-14 / 0	-102.1	-102.1	0.05 (1)	6.25	
D- E	-13 / 0	-102.1	-102.1	0.06 (1)	6.25	
E- F	-22 / 0	-102.1	-102.1	0.06 (1)	6.25	
F- G	-22 / 0	-102.1	-102.1	0.06 (1)	6.25	
G- H	-13 / 0	-102.1	-102.1	0.06 (1)	6.25	
H- I	-14 / 0	-102.1	-102.1	0.05 (1)	6.25	
I- J	-62 / 0	-102.1	-102.1	0.13 (1)	6.25	
J- K	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
L- J	-303 / 0	0.0	0.0	0.03 (1)	7.81	
T- S	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
S- R	0 / 18	-17.5	-17.5	0.01 (4)	10.00	
R- Q	0 / 12	-17.5	-17.5	0.01 (4)	10.00	
Q- P	0 / 8	-17.5	-17.5	0.01 (4)	10.00	
P- O	0 / 8	-17.5	-17.5	0.01 (4)	10.00	
O- N	0 / 12	-17.5	-17.5	0.01 (4)	10.00	
N- M	0 / 18	-17.5	-17.5	0.01 (4)	10.00	
M- L	0 / 0	-17.5	-17.5	0.01 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
P- F	-160 / 0	0.10 (1)
Q- E	-229 / 0	0.08 (1)
R- D	-211 / 0	0.05 (1)
S- C	-86 / 0	0.01 (1)
O- G	-229 / 0	0.08 (1)
N- H	-211 / 0	0.05 (1)
M- I	-86 / 0	0.01 (1)
B- S	0 / 27	0.01 (1)
M- J	0 / 27	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.01/1.00 (M-N:4) , WB=0.10/1.00 (F-P:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (J) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
K. J. G. ALVES
100008927
PROVINCE OF ONTARIO

DWG NO. TAM 71905361
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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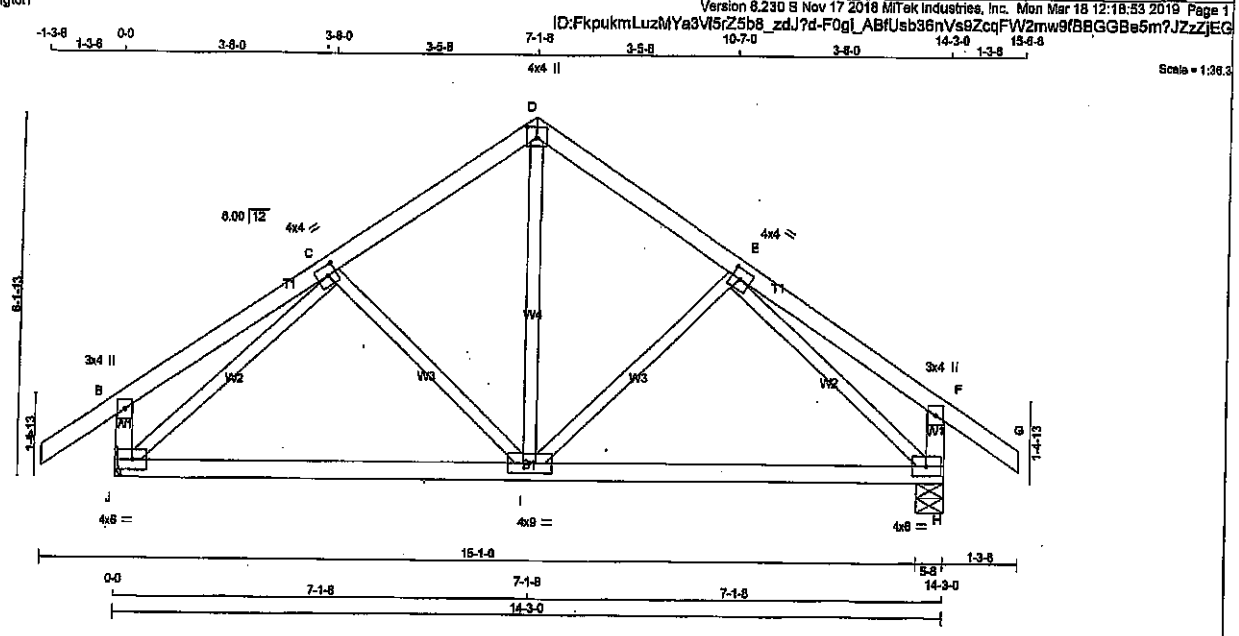

TOTAL WEIGHT = 2 X 61 = 123 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	992	0	992	0	0	MECHANICAL	
H	992	0	992	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	695	493 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0
H	695	493 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 23	-102.1	-102.1	0.20 (1)	10.00
C- D	-670 / 0	-102.1	-102.1	0.16 (1)	6.25
D- E	-670 / 0	-102.1	-102.1	0.16 (1)	6.25
E- F	0 / 23	-102.1	-102.1	0.20 (1)	10.00
F- G	0 / 39	-102.1	-102.1	0.14 (1)	10.00
J- B	-280 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-280 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 684	-17.5	-17.5	0.30 (4)	10.00
I- H	0 / 684	-17.5	-17.5	0.30 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 435	0.10 (1)
I- E	-203 / 0	0.08 (1)
G- I	-203 / 0	0.08 (1)
J- C	-954 / 0	0.37 (1)
E- H	-954 / 0	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.30/1.00 (I-J:4) , WB=0.37/1.00 (C-J:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	766	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (E) (INPUT = 0.90)
JSI METAL= 0.34 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1905363
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T9S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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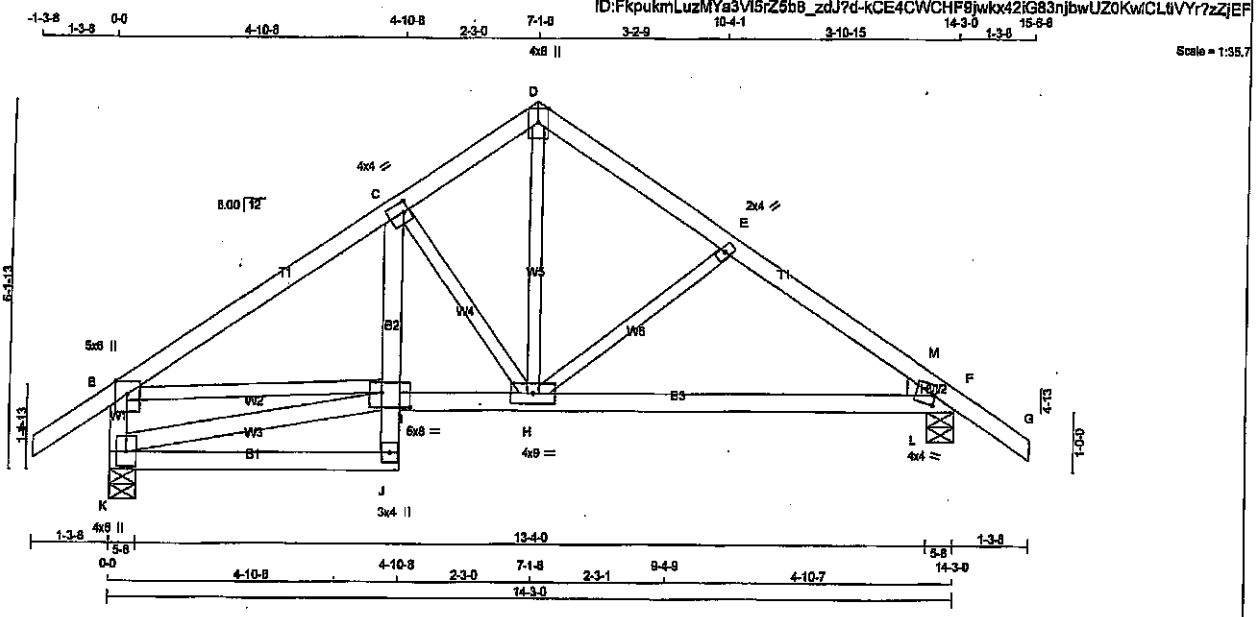

TOTAL WEIGHT = 2 X 64 = 129 lb [M]F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMBH1-m	MT20	4.0	4.0	2.00	0.50
H	BMWWW-t	MT20	4.0	9.0		
I	BVMWW-I	MT20	5.0	8.0	3.00	5.50
J	BMV+p	MT20	3.0	4.0		
K	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
K	992	0	992	0	0	5-8	5-8	
F	992	0	992	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	685	493 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0
F	685	493 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	-1043 / 0	-102.1	-102.1	0.28 (1)	5.82	
C- D	-818 / 0	-102.1	-102.1	0.21 (1)	6.25	
D- E	-795 / 0	-102.1	-102.1	0.15 (1)	6.25	
E- M	-1093 / 0	-102.1	-102.1	0.15 (1)	5.88	
M- F	-1230 / 0	-102.1	-102.1	0.08 (4)	5.75	
F- G	0 / 36	-102.1	-102.1	0.14 (1)	10.00	
K- B	-947 / 0	0.0	0.0	0.10 (1)	7.81	
K- J	0 / 20	-17.5	-17.5	0.12 (4)	10.00	
J- I	0 / 46	0.0	0.0	0.03 (1)	10.00	
I- C	0 / 35	0.0	0.0	0.03 (1)	10.00	
I- H	0 / 893	-17.5	-17.5	0.24 (1)	10.00	
H- L	0 / 933	-17.5	-17.5	0.24 (1)	10.00	
L- F	0 / 933	-17.5	-17.5	0.16 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- I	-21 / 0	0.01 (1)
B- I	0 / 887	0.20 (1)
H- D	0 / 689	0.15 (1)
C- H	-421 / 0	0.11 (1)
H- E	-372 / 0	0.11 (1)
L- M	0 / 113	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.24/1.00 (H-L:1) , WB=0.20/1.00 (B-I:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (F) (INPUT = 0.80)
JSI METAL= 0.47 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009029
PROVINCE OF ONTARIO

DWG NO. TAM 71905364
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	G10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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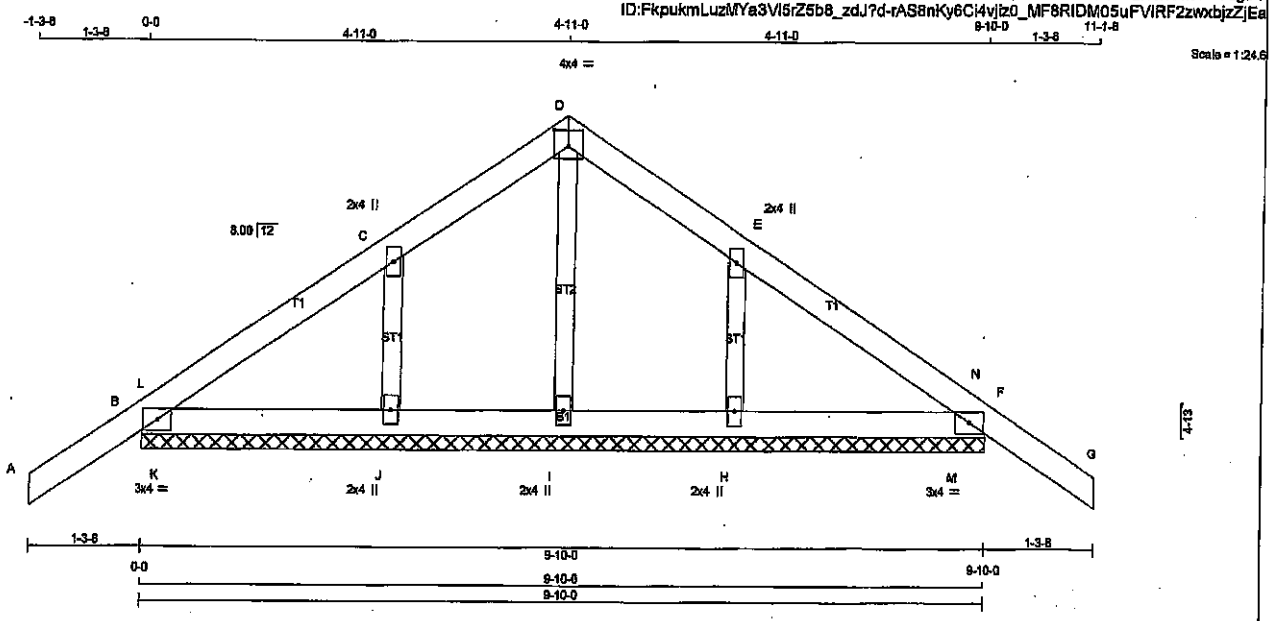

TOTAL WEIGHT = 34 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 36	-102.1	-102.1	0.14 (1)	10.00
B-L	-50 / 0	-102.1	-102.1	0.01 (1)	6.25
L-C	-55 / 0	-102.1	-102.1	0.09 (1)	6.25
C-D	-71 / 0	-102.1	-102.1	0.09 (1)	6.25
D-E	-71 / 0	-102.1	-102.1	0.09 (1)	6.25
E-N	-55 / 0	-102.1	-102.1	0.09 (1)	6.25
N-F	-50 / 0	-102.1	-102.1	0.01 (1)	6.25
F-G	0 / 36	-102.1	-102.1	0.14 (1)	10.00
B-K	0 / 56	-17.5	-17.5	0.06 (1)	10.00
K-J	0 / 56	-17.5	-17.5	0.06 (1)	10.00
J-I	0 / 44	-17.5	-17.5	0.04 (1)	10.00
I-H	0 / 44	-17.5	-17.5	0.04 (1)	10.00
H-M	0 / 56	-17.5	-17.5	0.06 (1)	10.00
M-F	0 / 56	-17.5	-17.5	0.06 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	-88 / 0	0.02 (1)
J-C	-285 / 0	0.04 (1)
H-E	-285 / 0	0.04 (1)
K-L	-145 / 3	0.00 (1)
M-N	-145 / 3	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1) , BC=0.06/1.00 (B-K:1) , WB=0.04/1.00 (E-H:1) , GSI=0.12/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (F) (INPUT = 0.80)
JSI METAL= 0.15 (C) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
02/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
DWG NO. TAM 7190 G10
STRUCTURAL COMPONENTS SECTION
BUILDING ONLY
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T11	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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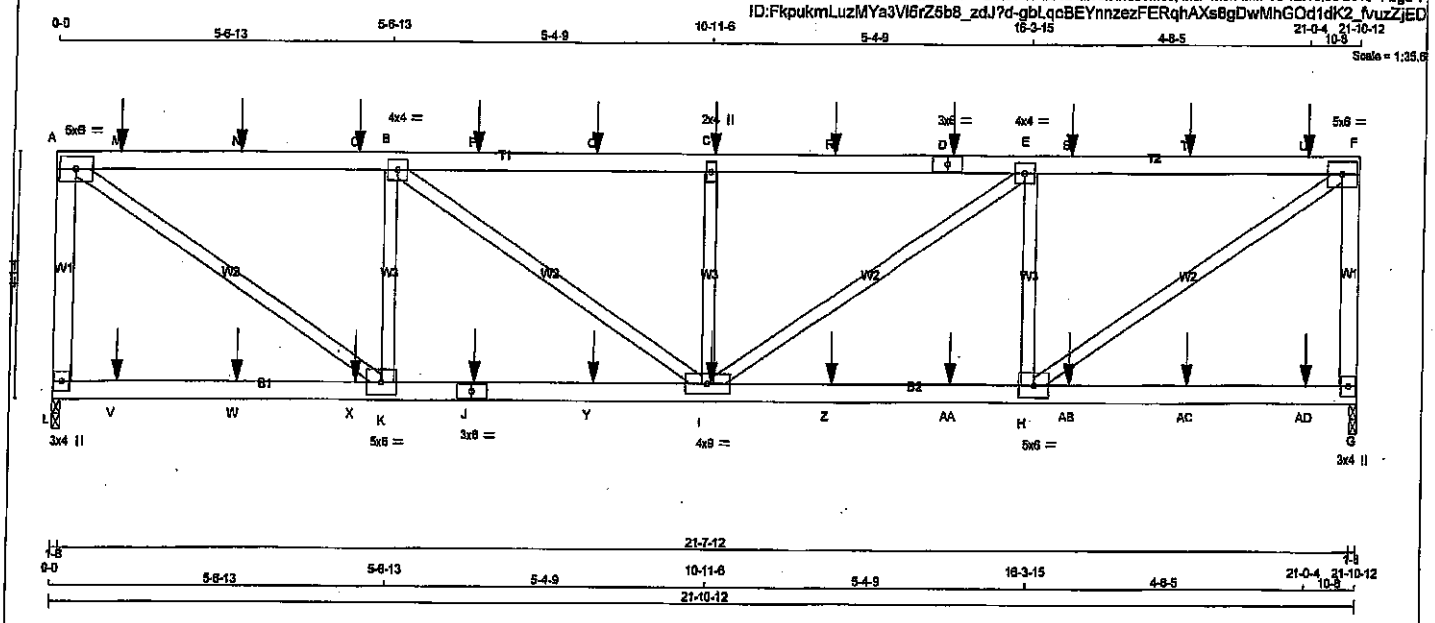

TOTAL WEIGHT = 4 X 87 = 348 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
L- A	1	12	TOP
A- D	1	12	SIDE(61.0)
D- F	1	12	SIDE(0.0)
F- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- J	1	12	SIDE(0.0)
J- G	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMWW-t	MT20	5.0	6.0	
B	TMWW-t	MT20	4.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	4.0	4.0	
F	TMWW-t	MT20	5.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMWW-t	MT20	5.0	6.0	
I	BMWWW-t	MT20	4.0	9.0	
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	5.0	6.0	
L	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2119	0	2119	0	0	1-8	1-8
G	2134	0	2134	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1491	1018 / 0	0 / 0	0 / 0	0 / 0	473 / 0	0 / 0
G	1502	1026 / 0	0 / 0	0 / 0	0 / 0	477 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- A	-2050 / 0	0.0	0.0		0.28 (1)	7.72
A- M	-2468 / 0	-102.1	-102.1		0.45 (1)	5.16
M- N	-2468 / 0	-102.1	-102.1		0.45 (1)	5.16
N- O	-2468 / 0	-102.1	-102.1		0.45 (1)	5.16
O- B	-2468 / 0	-102.1	-102.1		0.45 (1)	5.16
B- P	-3138 / 0	-102.1	-102.1		0.47 (1)	4.66
P- Q	-3138 / 0	-102.1	-102.1		0.47 (1)	4.66
Q- C	-3138 / 0	-102.1	-102.1		0.47 (1)	4.66
C- R	-3138 / 0	-102.1	-102.1		0.47 (1)	4.66
R- D	-3138 / 0	-102.1	-102.1		0.47 (1)	4.66
D- E	-3138 / 0	-102.1	-102.1		0.47 (1)	4.66
E- S	-2468 / 0	-102.1	-102.1		0.45 (1)	5.15
S- T	-2468 / 0	-102.1	-102.1		0.45 (1)	5.15
T- U	-2468 / 0	-102.1	-102.1		0.45 (1)	5.15
U- F	-2468 / 0	-102.1	-102.1		0.45 (1)	5.15
G- F	-2052 / 0	0.0	0.0		0.28 (1)	7.70
L- V	0 / 0	-17.5	-17.5		0.12 (4)	10.00
V- W	0 / 0	-17.5	-17.5		0.12 (4)	10.00
W- X	0 / 0	-17.5	-17.5		0.12 (4)	10.00
X- K	0 / 0	-17.5	-17.5		0.12 (4)	10.00
K- J	0 / 2468	-17.5	-17.5		0.28 (1)	10.00
J- Y	0 / 2468	-17.5	-17.5		0.28 (1)	10.00
Y- I	0 / 2468	-17.5	-17.5		0.28 (1)	10.00
I- Z	0 / 2468	-17.5	-17.5		0.28 (1)	10.00
Z- AA	0 / 2468	-17.5	-17.5		0.28 (1)	10.00
AA- H	0 / 2468	-17.5	-17.5		0.28 (1)	10.00
H- AB	0 / 0	-17.5	-17.5		0.12 (4)	10.00
AB- AC	0 / 0	-17.5	-17.5		0.12 (4)	10.00
AC- AD	0 / 0	-17.5	-17.5		0.12 (4)	10.00
AD- G	0 / 0	-17.5	-17.5		0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- F	0 / 2988	0.37 (1)
A- K	0 / 2991	0.37 (1)
H- E	-1502 / 0	0.20 (1)
K- B	-1503 / 0	0.20 (1)
I- E	0 / 824	0.10 (1)
B- I	0 / 821	0.10 (1)
I- C	-805 / 0	0.11 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	11-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
D	15-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
I	11-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
J	7-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
M	1-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
N	3-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.47/1.00 (C-E:1) , BC=0.28/1.00 (H-I:1) , WB=0.37/1.00 (A-K:1) , SSI=0.25/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (K) (INPUT = 0.60)
JSI METAL= 0.38 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM 7905366
STRUCTURAL
COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T11	2	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	5-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
P	7-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
Q	9-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
R	13-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
S	17-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
T	19-0-4	-123	-123	—	BACK	VERT	TOTAL	—	—
U	21-0-4	-129	-129	—	BACK	VERT	TOTAL	—	—
V	1-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
W	3-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
X	5-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
Y	9-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
Z	13-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
AA	15-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
AB	17-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
AC	19-0-4	-25	-25	—	BACK	VERT	TOTAL	—	—
AD	21-0-4	-26	-26	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. G. ALVES
100008921
PROVINCE OF ONTARIO

RECEIVED
MAR 22 2019
DWG NO. T11 - 72905366
STRUCTURAL
COMPONENT ONLY - EDON
TOWN OF
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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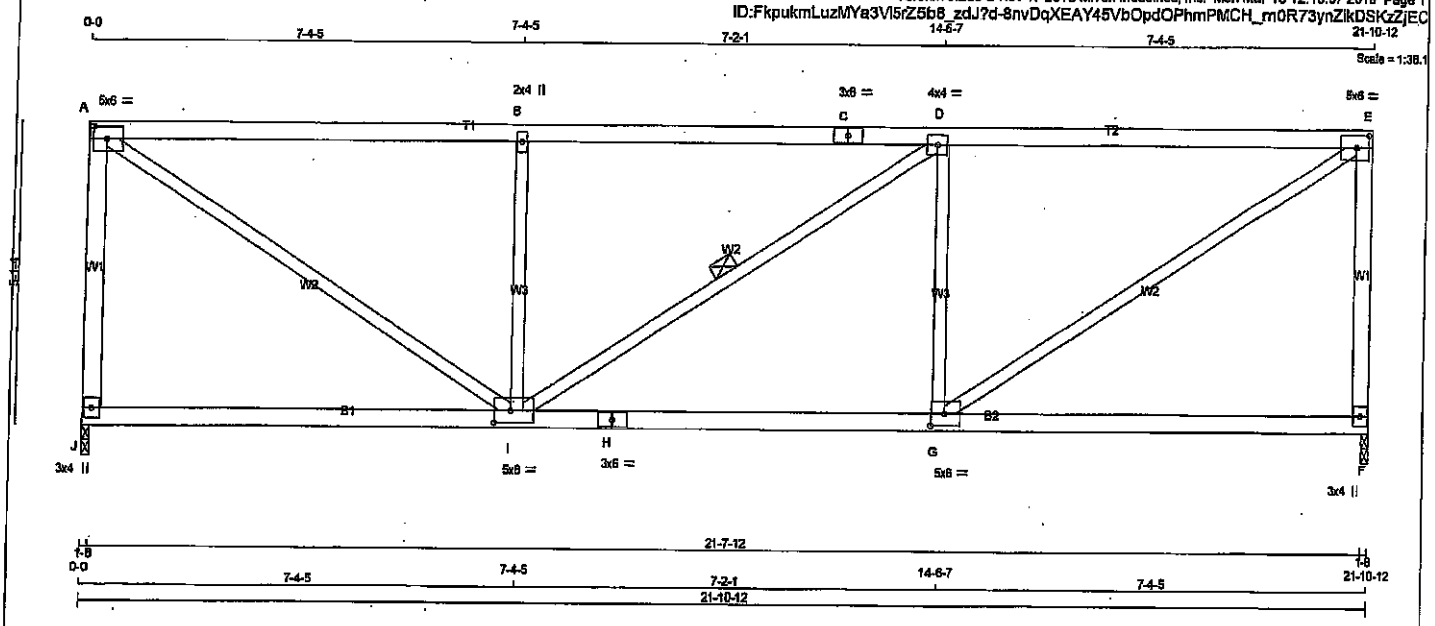

TOTAL WEIGHT = 2 X 88 = 176 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.75
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	5.0	6.0	2.50	2.75
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	5.0	6.0	2.50	2.75
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0	2.50	3.25
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1309	0	1309	0	0	1-8	1-8
F	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	920	635 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
F	920	635 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-1256 / 0	0.0	0.0	0.55 (1)	7.17
A- B	-1455 / 0	-102.1	-102.1	0.90 (1)	3.99
B- C	-1455 / 0	-102.1	-102.1	0.89 (1)	3.98
C- D	-1455 / 0	-102.1	-102.1	0.89 (1)	3.98
D- E	-1457 / 0	-102.1	-102.1	0.90 (1)	3.98
F- E	-1256 / 0	0.0	0.0	0.55 (1)	7.17
J- I	0 / 0	-17.5	-17.5	0.21 (4)	10.00
I- H	0 / 1457	-17.5	-17.5	0.35 (1)	10.00
H- G	0 / 1457	-17.5	-17.5	0.35 (1)	10.00
G- F	0 / 0	-17.5	-17.5	0.21 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 1740	0.39 (1)
A- I	0 / 1739	0.39 (1)
G- D	-814 / 0	0.31 (1)
I- B	-813 / 0	0.31 (1)
I- D	-1 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.90/1.00 (D-E:1) , BC=0.35/1.00 (G-I:1) , WB=0.39/1.00 (E-G:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.76 (E) (INPUT = 0.90)
SI METAL= 0.41 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 77905367
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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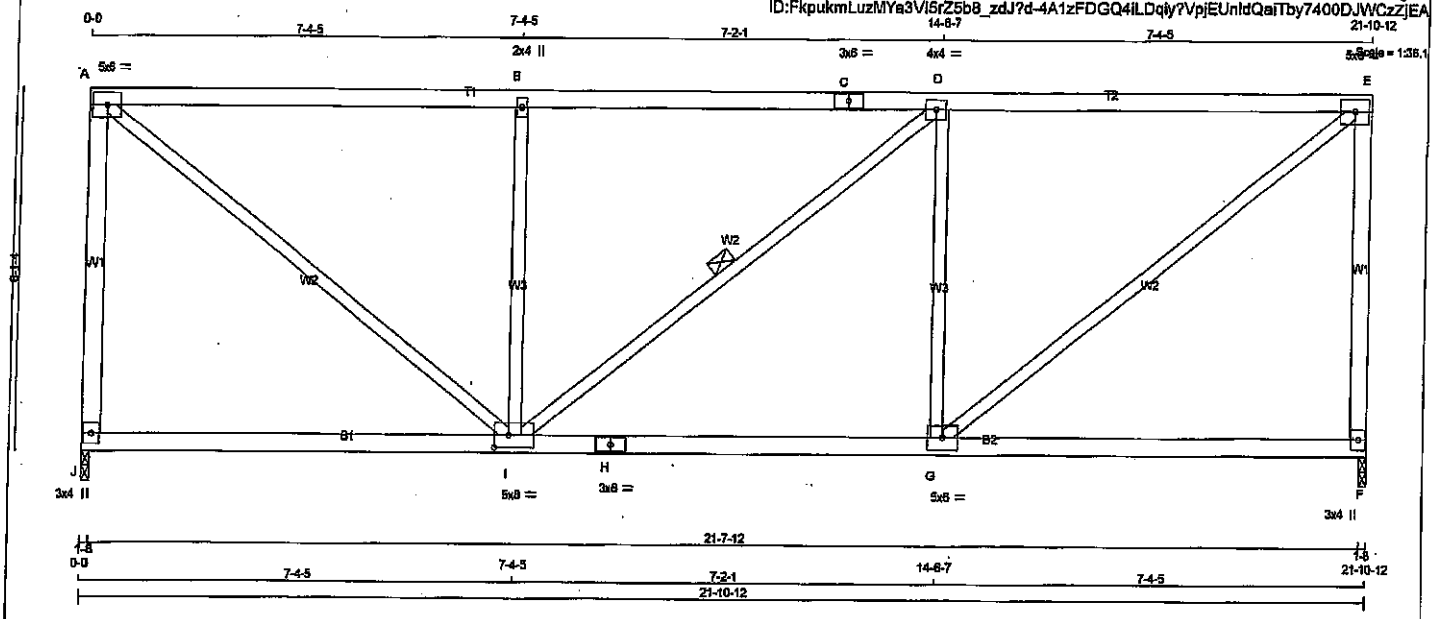

TOTAL WEIGHT = 2 X 93 = 186 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	6.0	2.50	3.00
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1309	0	1309	0	0.	1-8	1-8
F	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	920	636 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0
F	920	636 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-1259 / 0	0.0	0.0	0.91 (1)	7.17	
A- B	-1206 / 0	-102.1	-102.1	0.86 (1)	4.35	
B- C	-1206 / 0	-102.1	-102.1	0.86 (1)	4.32	
C- D	-1206 / 0	-102.1	-102.1	0.86 (1)	4.32	
D- E	-1207 / 0	-102.1	-102.1	0.86 (1)	4.32	
F- E	-1256 / 0	0.0	0.0	0.91 (1)	7.17	
J- I	0 / 0	-17.5	-17.5	0.21 (4)	10.00	
I- H	0 / 1207	-17.5	-17.5	0.31 (1)	10.00	
H- G	0 / 1207	-17.5	-17.5	0.31 (1)	10.00	
G- F	0 / 0	-17.5	-17.5	0.21 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 1538	0.35 (1)
A- I	0 / 1536	0.35 (1)
G- D	-814 / 0	0.48 (1)
I- B	-813 / 0	0.48 (1)
I- D	-2 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.91/1.00 (E-F:1) , BC=0.31/1.00 (G-I:1) , WB=0.48/1.00 (D-G:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.74 (E) (INPUT = 0.90)
SI METAL= 0.35 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H.J. G ALVES
100009827
PROVINCE OF ONTARIO

DWG NO. TAM T1405368
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T14	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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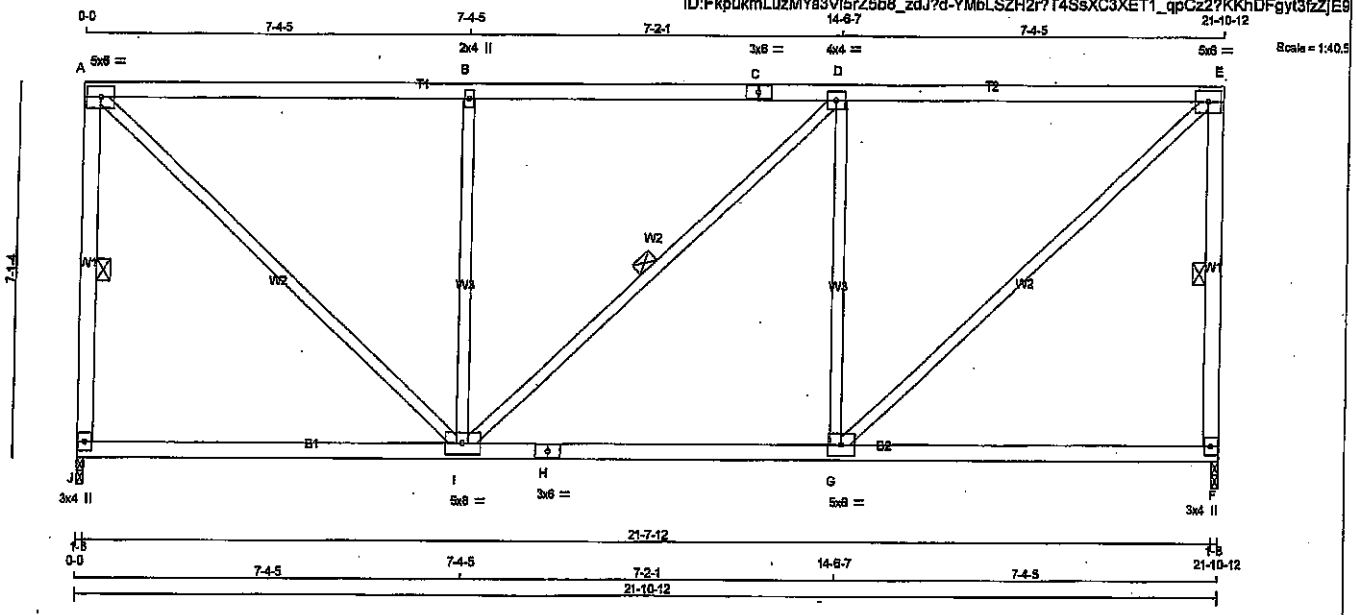

TOTAL WEIGHT = 2 X 99 = 197 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMW-t	MT20	5.0	6.0	
B	TMW+w	MT20	2.0	4.0	
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMW-t	MT20	5.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWW-t	MT20	5.0	6.0	
H	BS-t	MT20	3.0	6.0	
I	BMWWW-t	MT20	5.0	6.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1309	0	1309	0	0	1-8	1-8
F	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
F	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-J, E-F, D-I,

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-1256 / 0	0.0	0.0		0.29 (1)	5.74
A- B	-1029 / 0	-102.1	-102.1		0.83 (1)	4.63
B- C	-1029 / 0	-102.1	-102.1		0.83 (1)	4.63
C- D	-1029 / 0	-102.1	-102.1		0.83 (1)	4.63
D- E	-1030 / 0	-102.1	-102.1		0.84 (1)	4.63
F- E	-1257 / 0	0.0	0.0		0.29 (1)	5.74
J- I	0 / 0	-17.5	-17.5		0.21 (4)	10.00
I- H	0 / 1030	-17.5	-17.5		0.29 (4)	10.00
H- G	0 / 1030	-17.5	-17.5		0.29 (4)	10.00
G- F	0 / 0	-17.5	-17.5		0.21 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 1404	0.32 (1)
A- I	0 / 1402	0.32 (1)
G- D	-814 / 0	0.71 (1)
I- B	-813 / 0	0.71 (1)
I- D	-2 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.84/1.00 (D-E:1) , BC=0.29/1.00 (G-I:4) , WB=0.71/1.00 (D-G:1) , SS=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

PSI GRIP= 0.75 (I) (INPUT = 0.90)
PSI METAL= 0.31 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G ALVES
100000025
PROVINCE OF ONTARIO

DWG NO. TAM 7405369
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T15	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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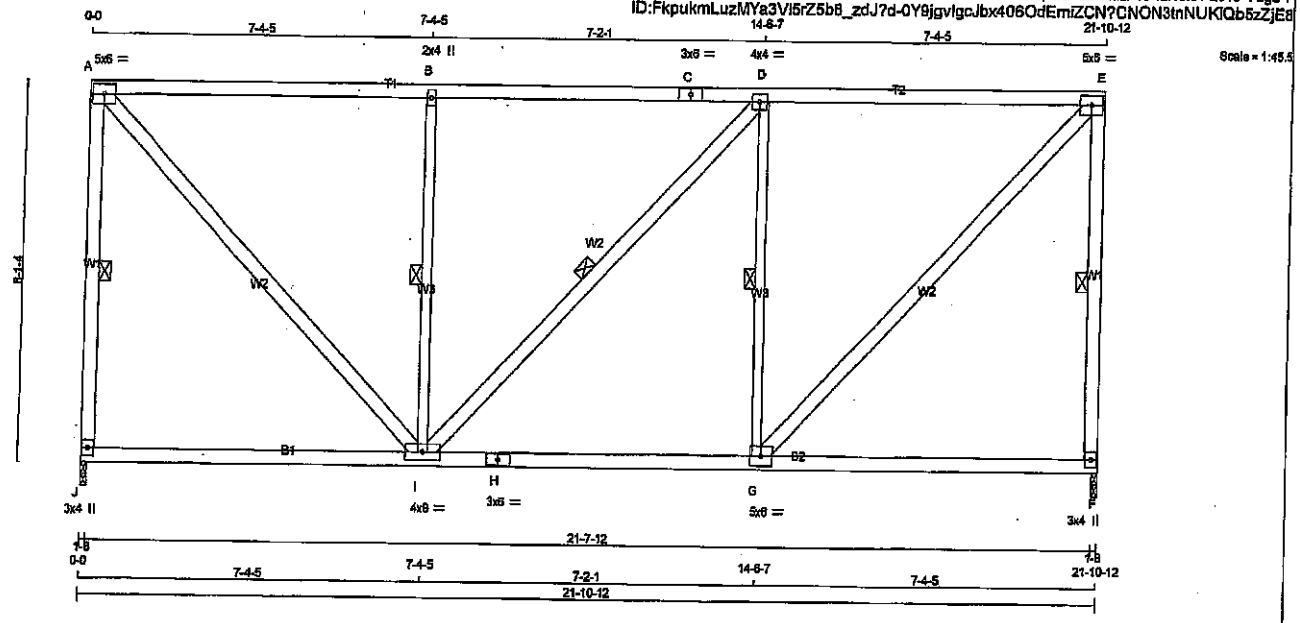

TOTAL WEIGHT = 2 X 114 = 229 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
G - D	2x3	DRY	No.2	SPF
I - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMWW-t	MT20	5.0	6.0	
B	TMW+w	MT20	2.0	4.0	
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMWW-t	MT20	5.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWW-t	MT20	5.0	6.0	
H	BS-t	MT20	3.0	6.0	
I	BMWWW-t	MT20	4.0	9.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1309	0	1309	0	0	1-8	1-8
F	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
F	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-G, B-I, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-1256 / 0	0.0	0.0	0.37 (1)	5.74
A- B	-898 / 0	-102.1	-102.1	0.82 (1)	4.92
B- C	-898 / 0	-102.1	-102.1	0.82 (1)	4.90
C- D	-898 / 0	-102.1	-102.1	0.82 (1)	4.90
D- E	-899 / 0	-102.1	-102.1	0.82 (1)	4.88
F- E	-1257 / 0	0.0	0.0	0.37 (1)	5.74
J- I	0 / 0	-17.5	-17.5	0.21 (4)	10.00
I- H	0 / 899	-17.5	-17.5	0.28 (4)	10.00
H- G	0 / 899	-17.5	-17.5	0.28 (4)	10.00
G- F	0 / 0	-17.5	-17.5	0.21 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 1311	0.21 (1)
A- I	0 / 1309	0.21 (1)
G- D	-814 / 0	0.34 (1)
I- B	-814 / 0	0.34 (1)
I- D	-2 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.82/1.00 (D-E:1) , BC=0.28/1.00 (G-I:4) , WB=0.34/1.00 (D-G:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (I) (INPUT = 0.90)
JSI METAL= 0.29 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 77905370
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T16	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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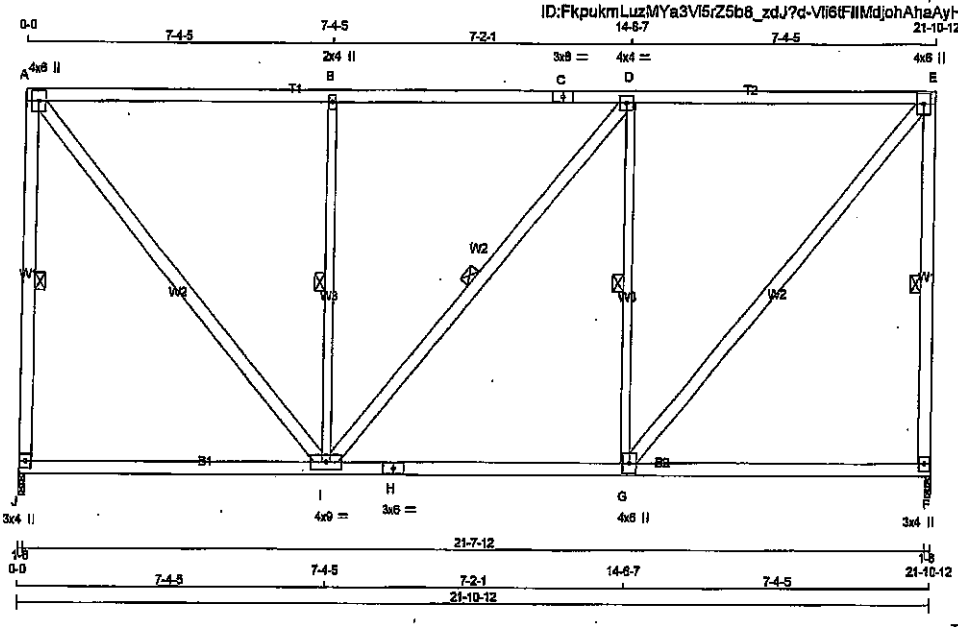

TOTAL WEIGHT = 4 X 121 = 484 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
G - D	2x3	DRY	No.2	SPF
I - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0		
B	TMW+w	MT20	2.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW+t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1309	0	1309	0	0	1-8	1-8
F	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
F	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-G, B-I, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-1256 / 0	0.0	0.0	0.48 (1)	5.74	
A- B	-796 / 0	-102.1	-102.1	0.80 (1)	5.18	
B- C	-796 / 0	-102.1	-102.1	0.81 (1)	5.14	
C- D	-796 / 0	-102.1	-102.1	0.81 (1)	5.14	
D- E	-797 / 0	-102.1	-102.1	0.81 (1)	5.13	
F- E	-1257 / 0	0.0	0.0	0.48 (1)	5.74	
J- I	0 / 0	-17.5	-17.5	0.21 (4)	10.00	
I- H	0 / 797	-17.5	-17.5	0.27 (4)	10.00	
H- G	0 / 797	-17.5	-17.5	0.27 (4)	10.00	
G- F	0 / 0	-17.5	-17.5	0.21 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 1243	0.20 (1)
A- I	0 / 1242	0.20 (1)
G- D	-814 / 0	0.44 (1)
I- B	-814 / 0	0.44 (1)
I- D	-2 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.81/1.00 (D-E:1) , BC=0.27/1.00 (G-I:4) , WB=0.44/1.00 (D-G:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.76 (I) (INPUT = 0.90)
SI METAL= 0.36 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 7905371
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T17	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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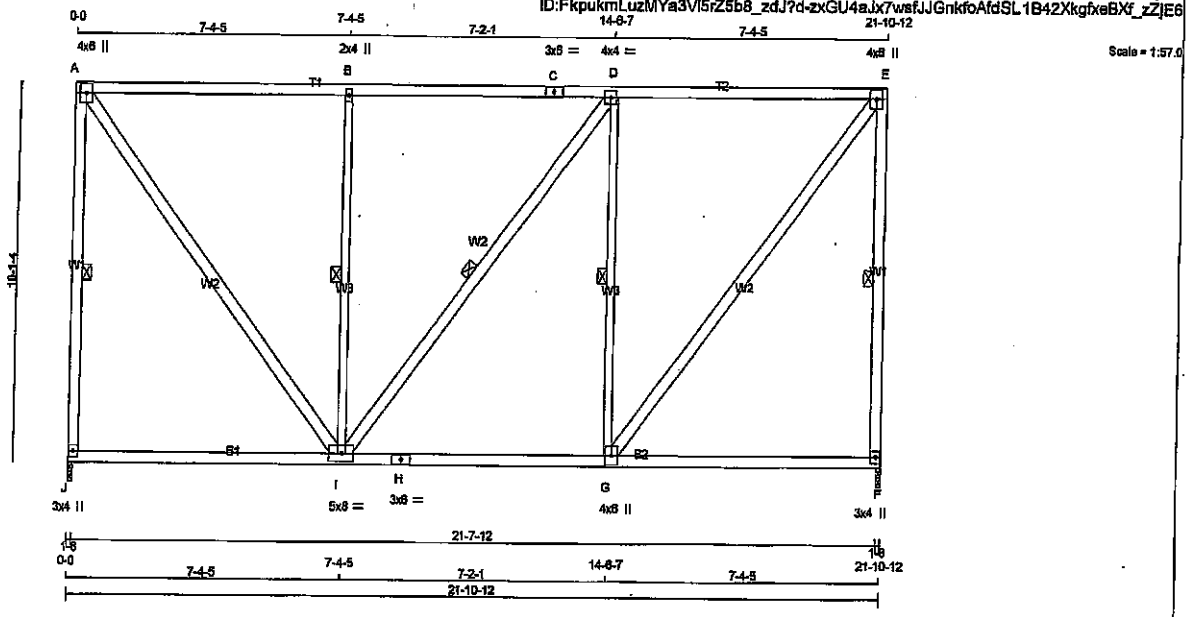

TOTAL WEIGHT = 6 X 128 = 765 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF
I - B	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW+p	MT20	4.0	6.0	
B	TMW+w	MT20	2.0	4.0	
C	TS-t	MT20	3.0	6.0	
D	TMWW-t	MT20	4.0	4.0	
E	TMVW+p	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	4.0	
G	BMWW+t	MT20	4.0	6.0	
H	BS-t	MT20	3.0	6.0	
I	BMWWW-t	MT20	5.0	8.0	
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1309	0	1309	0	0	1-8	1-8
F	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
F	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-J, E-F, D-G, B-I, D-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J-A	-1258 / 0	0.0	0.0	0.63 (1)	5.74
A-B	-715 / 0	-102.1	-102.1	0.79 (1)	5.40
B-C	-715 / 0	-102.1	-102.1	0.80 (1)	5.36
C-D	-715 / 0	-102.1	-102.1	0.80 (1)	5.36
D-E	-716 / 0	-102.1	-102.1	0.80 (1)	5.36
F-E	-1257 / 0	0.0	0.0	0.63 (1)	5.74
J-I	0 / 0	-17.5	-17.5	0.21 (4)	10.00
I-H	0 / 716	-17.5	-17.5	0.27 (4)	10.00
H-G	0 / 716	-17.5	-17.5	0.27 (4)	10.00
G-F	0 / 0	-17.5	-17.5	0.21 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-E	0 / 1193	0.19 (1)
A-I	0 / 1191	0.19 (1)
G-D	-814 / 0	0.57 (1)
I-B	-813 / 0	0.57 (1)
I-D	-2 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.80/1.00 (D-E:1) , BC=0.27/1.00 (G-I:4) , WB=0.57/1.00 (D-G:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (E) (INPUT = 0.60)
JSI METAL= 0.32 (E) (INPUT = 0.30)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE.

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009036
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 7705
STRUCTURAL
COMPONENT ENGINEERING

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T18	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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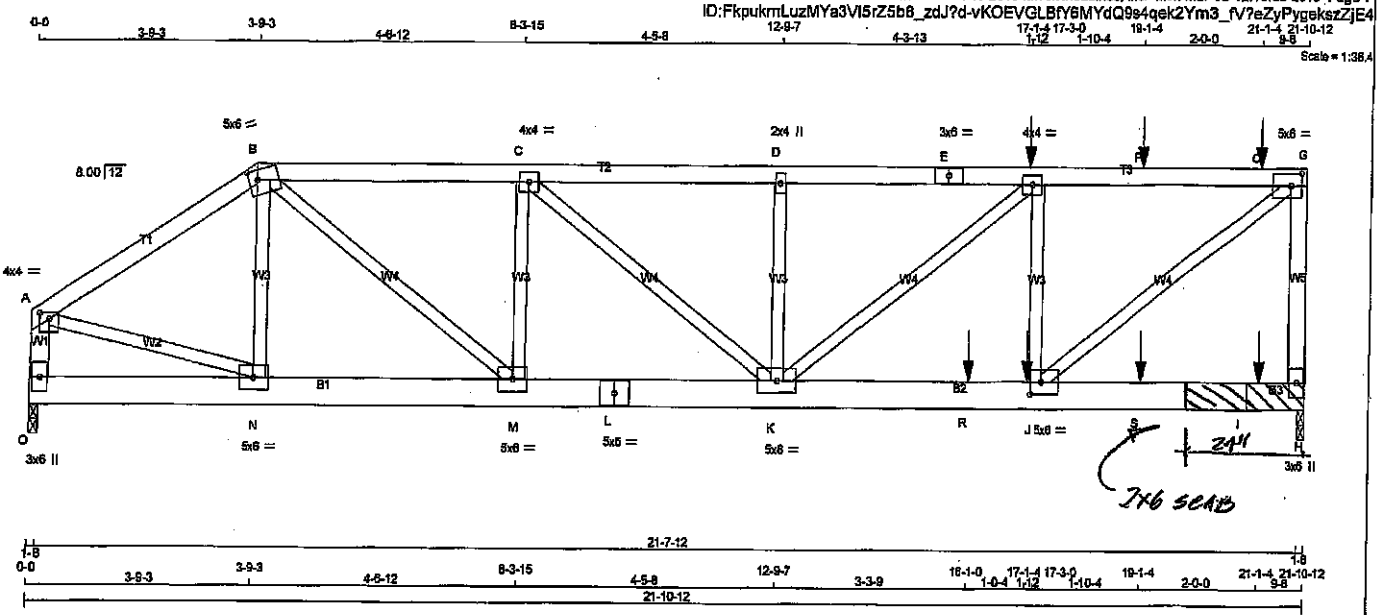

TOTAL WEIGHT = 4 X 104 = 414 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- E	1	12	TOP
E- G	1	12	SIDE(61.0)
G- H	1	12	TOP
O- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	TOP
L- H	2	12	SIDE(163.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	5.0	6.0	2.00	1.75
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.25
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	3053	0	3053	0	0	1-8	1-8 & BLOCK
O	1651	0	1651	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2142	1498 / 0	0 / 0	0 / 0	0 / 0	644 / 0	0 / 0
O	1300	906 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, O

2x6 DRY SPF No.2 BEARING BLOCK 24" LONG AT JT. H ATTACHED TO FRONT SIDE. WITH 3 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 3" C.C. 27 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2017 / 0	-102.1	-102.1	0.16 (1)	6.02	
B- C	-3049 / 0	-102.1	-102.1	0.21 (1)	5.08	
C- D	-3831 / 0	-102.1	-102.1	0.23 (1)	4.62	
D- E	-3831 / 0	-102.1	-102.1	0.28 (1)	4.55	
E- F	-3831 / 0	-102.1	-102.1	0.28 (1)	4.55	
F- P	-3426 / 0	-102.1	-102.1	0.30 (1)	4.74	
P- Q	-3426 / 0	-102.1	-102.1	0.30 (1)	4.74	
Q- G	-3426 / 0	-102.1	-102.1	0.30 (1)	4.74	
H- G	-3066 / 0	0.0	0.0	0.36 (1)	6.56	
O- A	-1831 / 0	0.0	0.0	0.10 (1)	7.81	
O- N	0 / 0	-17.5	-17.5	0.02 (4)	10.00	
N- M	0 / 1664	-17.5	-17.5	0.15 (1)	10.00	
M- L	0 / 3049	-17.5	-17.5	0.31 (1)	10.00	
L- K	0 / 3049	-17.5	-17.5	0.31 (1)	10.00	
K- R	0 / 3426	-17.5	-17.5	0.65 (1)	10.00	
R- J	0 / 3426	-17.5	-17.5	0.65 (1)	10.00	
J- S	0 / 0	-17.5	-17.5	0.18 (1)	10.00	
S- I	0 / 0	-17.5	-17.5	0.18 (1)	10.00	
I- H	0 / 0	-17.5	-17.5	0.18 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-392 / 0	0.05 (1)
A- N	0 / 1753	0.22 (1)
J- G	0 / 4392	0.54 (1)
B- M	0 / 1766	0.22 (1)
J- F	-1124 / 0	0.14 (1)
M- C	-1178 / 0	0.15 (1)
K- F	0 / 527	0.07 (1)
C- K	0 / 1066	0.13 (1)
K- D	0 / 76	0.05 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL
F	17-1-4	-123	-123	—	FRONT	VERT	TOTAL	—
I	21-1-4	-27	-27	—	FRONT	VERT	TOTAL	—
J	17-1-4	-25	-25	—	FRONT	VERT	TOTAL	—
P	19-1-4	-123	-123	—	FRONT	VERT	TOTAL	—
Q	21-1-4	-134	-134	—	FRONT	VERT	TOTAL	—
R	16-1-0	-1629	-1629	—	FRONT	VERT	TOTAL	—
S	19-1-4	-25	-25	—	FRONT	VERT	TOTAL	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.38/1.00 (G-H:1) , BC=0.85/1.00 (J-K:1) , WB=0.54/1.00 (G-J:1) , SSI=0.48/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.43 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H J G ALVES
100008724
PROVINCE OF ONTARIO

DWG NO. TAM T1905373
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T19	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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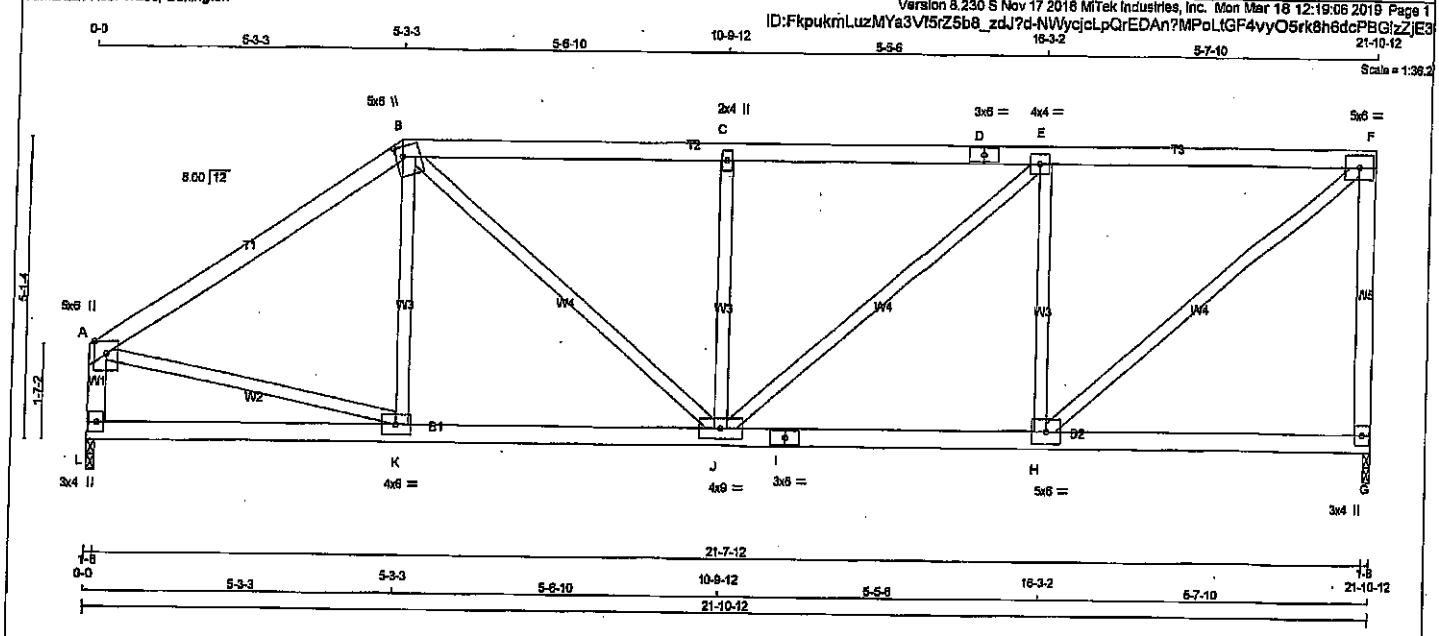

TOTAL WEIGHT = 2 X 90 = 180 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	5.0	6.0	Edge	
B	TTWV+m	MT20	5.0	6.0	2.00	1.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1309	0	1309	0	0	1-8	1-8
L	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
L	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1331 / 0	-102.1	-102.1	0.57 (1)	4.85
B- C	-1555 / 0	-102.1	-102.1	0.48 (1)	4.69
C- D	-1555 / 0	-102.1	-102.1	0.50 (1)	4.66
D- E	-1555 / 0	-102.1	-102.1	0.50 (1)	4.66
E- F	-1213 / 0	-102.1	-102.1	0.47 (1)	5.16
G- F	-1289 / 0	0.0	0.0	0.56 (1)	7.15
L- A	-1272 / 0	0.0	0.0	0.13 (1)	7.14
L- K	0 / 0	-17.5	-17.5	0.12 (4)	10.00
K- J	0 / 1103	-17.5	-17.5	0.25 (1)	10.00
J- I	0 / 1213	-17.5	-17.5	0.26 (1)	10.00
I- H	0 / 1213	-17.5	-17.5	0.26 (1)	10.00
H- G	0 / 0	-17.5	-17.5	0.13 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-154 / 39	0.06 (1)
A- K	0 / 1137	0.26 (1)
H- F	0 / 1595	0.36 (1)
B- J	0 / 598	0.13 (1)
H- E	-925 / 0	0.36 (1)
J- C	-608 / 0	0.23 (1)
J- E	0 / 457	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.57/1.00 (A-B:1) , BC=0.26/1.00 (H-J:1) , WB=0.36/1.00 (F-H:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS:GRIP= 0.82 (A) (INPUT = 0.90)
JS:METAL= 0.54 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H J G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 7905374
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T20	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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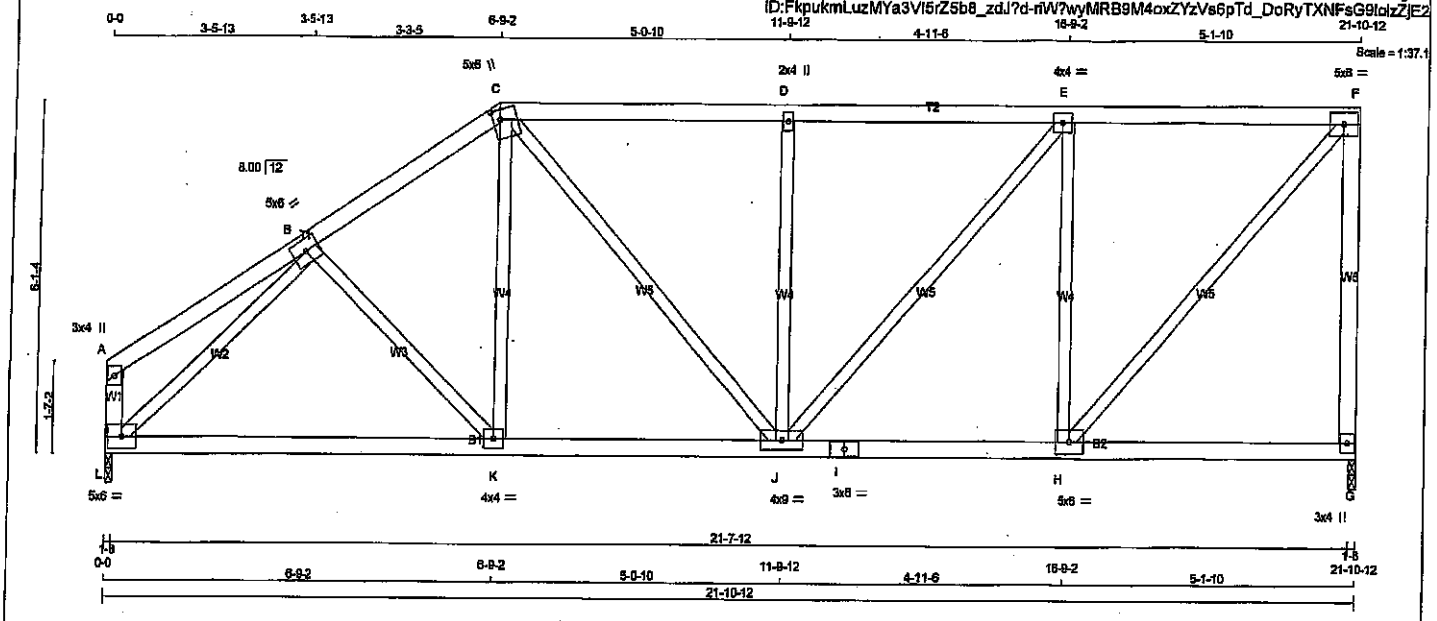

TOTAL WEIGHT = 2 X 98 = 196 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1309	0	1309	0	0	1-8	1-8
L	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	920	636 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0
L	920	636 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-102.1	-102.1	0.17 (1)	10.00
B- C	-1305 / 0	-102.1	-102.1	0.20 (1)	5.43
C- D	-1269 / 0	-102.1	-102.1	0.37 (1)	5.24
D- E	-1269 / 0	-102.1	-102.1	0.39 (1)	5.21
E- F	-937 / 0	-102.1	-102.1	0.38 (1)	5.86
G- F	-1273 / 0	0.0	0.0	0.92 (1)	7.14
L- A	-135 / 0	0.0	0.0	0.01 (1)	7.81
L- K	0 / 1077	-17.5	-17.5	0.27 (1)	10.00
K- J	0 / 1069	-17.5	-17.5	0.27 (1)	10.00
J- I	0 / 937	-17.5	-17.5	0.21 (1)	10.00
I- H	0 / 937	-17.5	-17.5	0.21 (1)	10.00
H- G	0 / 0	-17.5	-17.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- K	-15 / 40	0.01 (4)
K- C	0 / 136	0.04 (4)
L- B	-1557 / 0	0.59 (1)
H- F	0 / 1415	0.32 (1)
C- J	0 / 305	0.07 (1)
H- E	-957 / 0	0.56 (1)
J- D	-552 / 0	0.32 (1)
J- E	0 / 513	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.60/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.92/1.00 (F-G:1) , BC=0.27/1.00 (K-L:1) , WB=0.59/1.00 (B-L:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (H) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009827
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
DWG NO. 1190537
STRUCTURAL COMPONENTS ONLY
TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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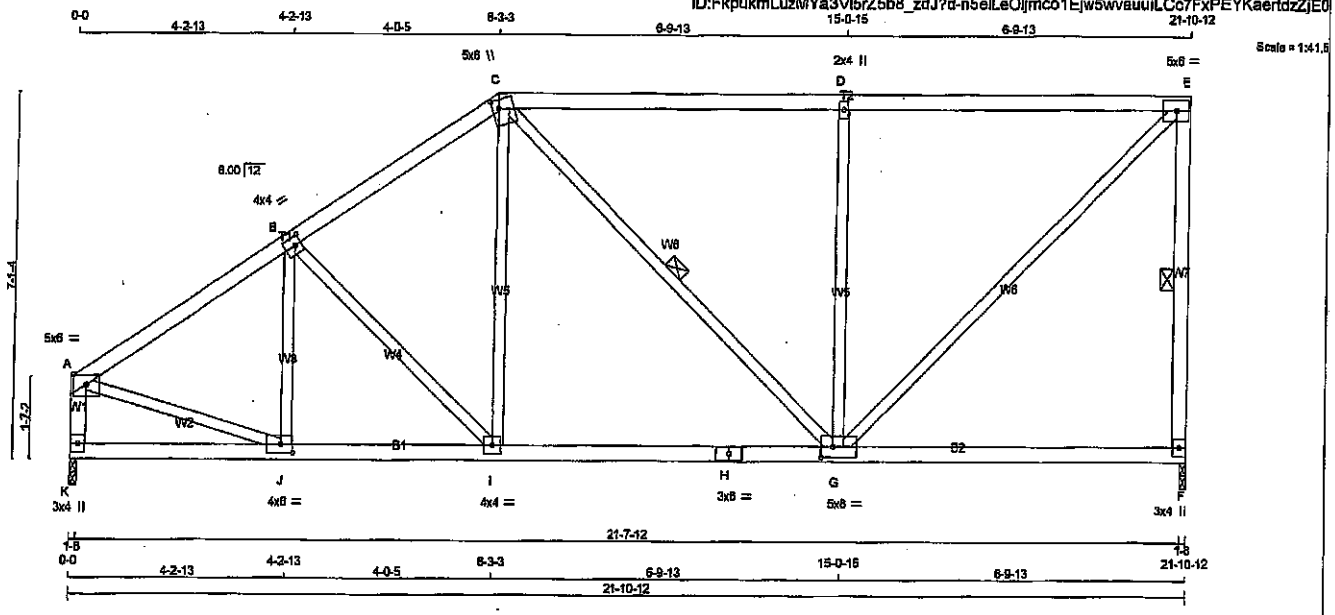

TOTAL WEIGHT = 2 X 97 = 195 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0	2.50	2.50
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1309	0	1309	0	0	1-8	1-8
K	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1349 / 0	-102.1	-102.1	0.32 (1)	5.19	
B- C	-1225 / 0	-102.1	-102.1	0.31 (1)	6.40	
C- D	-1002 / 0	-102.1	-102.1	0.88 (1)	4.39	
D- E	-1002 / 0	-102.1	-102.1	0.69 (1)	4.39	
F- E	-1261 / 0	0.0	0.0	0.29 (1)	5.73	
K- A	-1276 / 0	0.0	0.0	0.13 (1)	7.13	
K- J	0 / 0	-17.5	-17.5	0.07 (4)	10.00	
J- I	0 / 1145	-17.5	-17.5	0.24 (1)	10.00	
I- H	0 / 998	-17.5	-17.5	0.28 (4)	10.00	
H- G	0 / 998	-17.5	-17.5	0.28 (4)	10.00	
G- F	0 / 0	-17.5	-17.5	0.20 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
J- B	-275 / 0	0.08 (1)
B- I	-212 / 0	0.12 (1)
I- C	0 / 245	0.08 (1)
C- G	-8 / 6	0.01 (4)
G- D	-864 / 0	0.76 (1)
G- E	0 / 1412	0.32 (1)
A- J	0 / 1196	0.27 (1)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.89/1.00 (D-E:1) , BC=0.28/1.00 (G-I:4) , WB=0.76/1.00 (D-G:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.75 (A) (INPUT = 0.90)
USI METAL= 0.33 (H) (INPUT = 1.00)

DWG NO. TAM T1905376
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T22	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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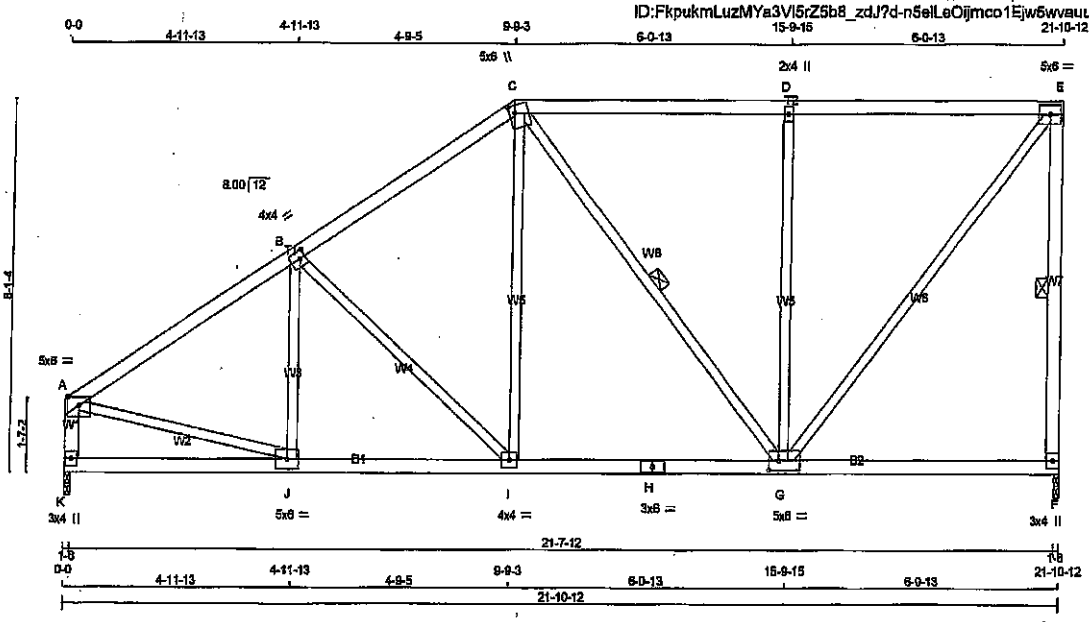

TOTAL WEIGHT = 2 X 103 = 205 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	2.50
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1309	0	1309	0	0	1-8	1-8
K	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1370 / 0	-102.1	-102.1	0.47 (1)	4.96	
B- C	-1128 / 0	-102.1	-102.1	0.45 (1)	5.36	
C- D	-804 / 0	-102.1	-102.1	0.67 (1)	5.53	
D- E	-804 / 0	-102.1	-102.1	0.67 (1)	5.52	
F- E	-1266 / 0	0.0	0.0	0.36 (1)	5.73	
K- A	-1272 / 0	0.0	0.0	0.13 (1)	7.14	
K- J	0 / 0	-17.5	-17.5	0.09 (4)	10.00	
J- I	0 / 1167	-17.5	-17.5	0.24 (1)	10.00	
I- H	0 / 914	-17.5	-17.5	0.23 (1)	10.00	
H- G	0 / 914	-17.5	-17.5	0.23 (1)	10.00	
G- F	0 / 0	-17.5	-17.5	0.16 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-207 / 16	0.07 (1)
B- I	-354 / 0	0.29 (1)
I- C	0 / 334	0.08 (1)
C- G	-179 / 0	0.12 (1)
G- D	-768 / 0	0.97 (1)
G- E	0 / 1306	0.29 (1)
A- J	0 / 1204	0.27 (1)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H J G ALVES
100090954
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.24/1.00 (I-J:1) , WB=0.97/1.00 (D-G:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (A) (INPUT = 0.90)
JSI METAL= 0.33 (H) (INPUT = 1.00)

DWG NO. TAM T1405377
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T23	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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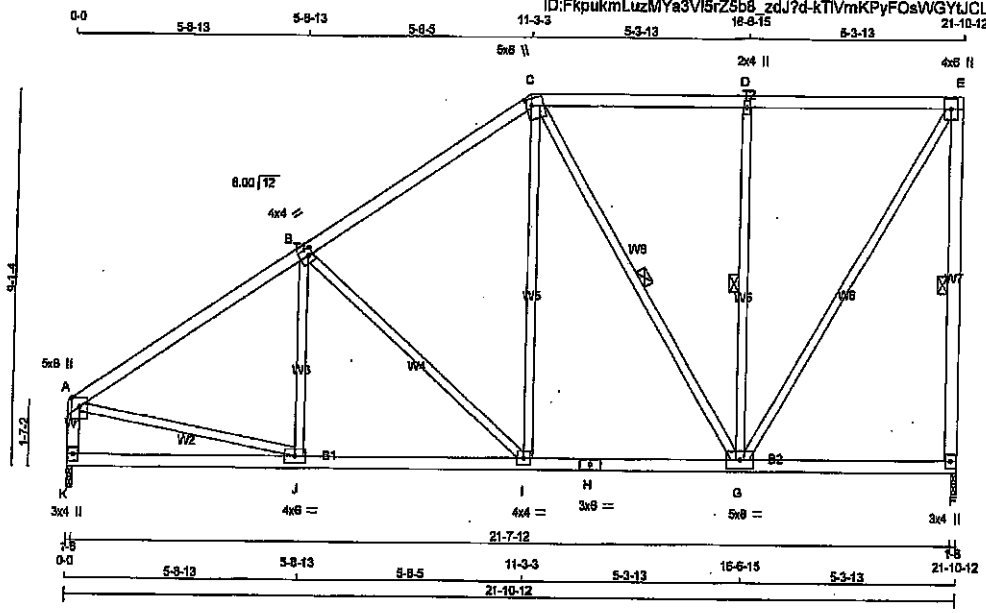

TOTAL WEIGHT = 2 X 108 = 216 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1309	0	1309	0	0	1-8	1-8
K	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1373 / 0	-102.1	-102.1	0.46 (1)	4.99
B- C	-1019 / 0	-102.1	-102.1	0.43 (1)	5.83
C- D	-645 / 0	-102.1	-102.1	0.37 (1)	6.25
D- E	-646 / 0	-102.1	-102.1	0.37 (1)	6.25
F- E	-1270 / 0	0.0	0.0	0.49 (1)	5.71
K- A	-1266 / 0	0.0	0.0	0.13 (1)	7.15
K- J	0 / 0	-17.5	-17.5	0.14 (4)	10.00
J- I	0 / 1174	-17.5	-17.5	0.26 (1)	10.00
I- H	0 / 818	-17.5	-17.5	0.19 (1)	10.00
H- G	0 / 818	-17.5	-17.5	0.19 (1)	10.00
G- F	0 / 0	-17.5	-17.5	0.12 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-150 / 48	0.07 (1)
I- B	-468 / 0	0.56 (1)
I- C	0 / 417	0.09 (1)
C- G	-336 / 0	0.26 (1)
G- D	-672 / 0	0.36 (1)
G- E	0 / 1243	0.28 (1)
A- J	0 / 1203	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.49/1.00 (E-F:1) , BC=0.26/1.00 (I-J:1) , WB=0.56/1.00 (B-I:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	789	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.00°

JSI GRIP= 0.98 (JSI INPUT = 0.80)
JSI METAL = 0.66 (JSI INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009[illegible]
PROVINCE OF ONTARIO

RECEIVED
AUG 0 2 2019
TOWN OF CALEDON
BUILDING SECTION
DWG NO. 1119-01-1379
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T24	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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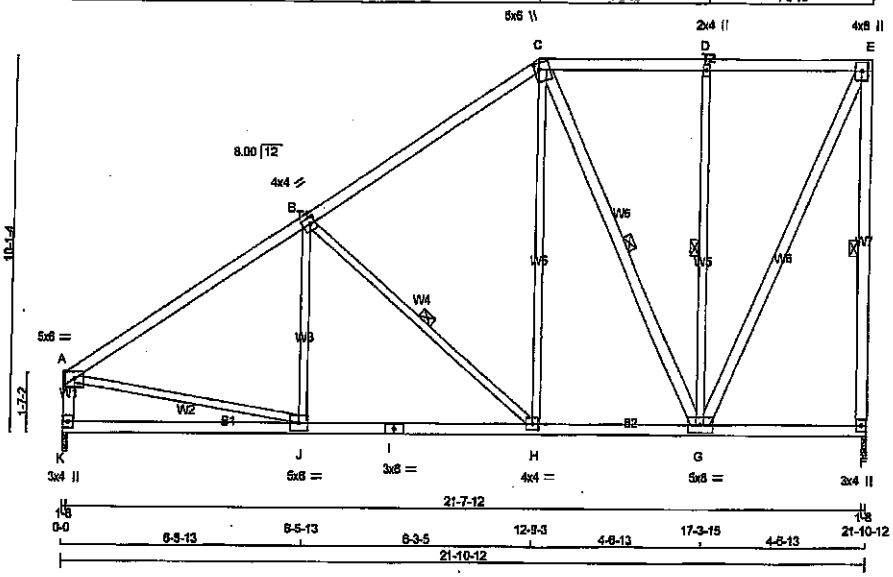

TOTAL WEIGHT = 2 X 121 = 241 lb

(MI(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV-p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1309	0	1309	0	0	1-8	1-8
K	1309	0	1309	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	920	636 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, B-H, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1364 / 0	-102.1	-102.1	0.60 (1)	4.75
B- C	-900 / 0	-102.1	-102.1	0.56 (1)	5.64
C- D	-514 / 0	-102.1	-102.1	0.27 (1)	6.25
D- E	-514 / 0	-102.1	-102.1	0.27 (1)	6.25
F- E	-1275 / 0	0.0	0.0	0.64 (1)	5.71
K- A	-1263 / 0	0.0	0.0	0.13 (1)	7.16
K- J	0 / 0	-17.5	-17.5	0.18 (4)	10.00
J- I	0 / 1171	-17.5	-17.5	0.28 (1)	10.00
I- H	0 / 1171	-17.5	-17.5	0.28 (1)	10.00
H- G	0 / 716	-17.5	-17.5	0.17 (1)	10.00
G- F	0 / 0	-17.5	-17.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-102 / 77	0.08 (1)
B- H	-613 / 0	0.30 (1)
H- C	0 / 498	0.11 (1)
C- G	-461 / 0	0.30 (1)
G- D	-575 / 0	0.40 (1)
G- E	0 / 1209	0.19 (1)
A- J	0 / 1193	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.64/1.00 (E-F:1) , BC=0.28/1.00 (H-J:1) , WB=0.40/1.00 (D-G:1) , SSI=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.75 (A) (INPUT = 0.90)
JSI METAL= 0.40 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G ALVES
100009929
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
TOWN OF CALEDON
STRUCTURAL BUILDING SECTION
FILE NO ONLY

DWG NO. TAM 0279825-79

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T25	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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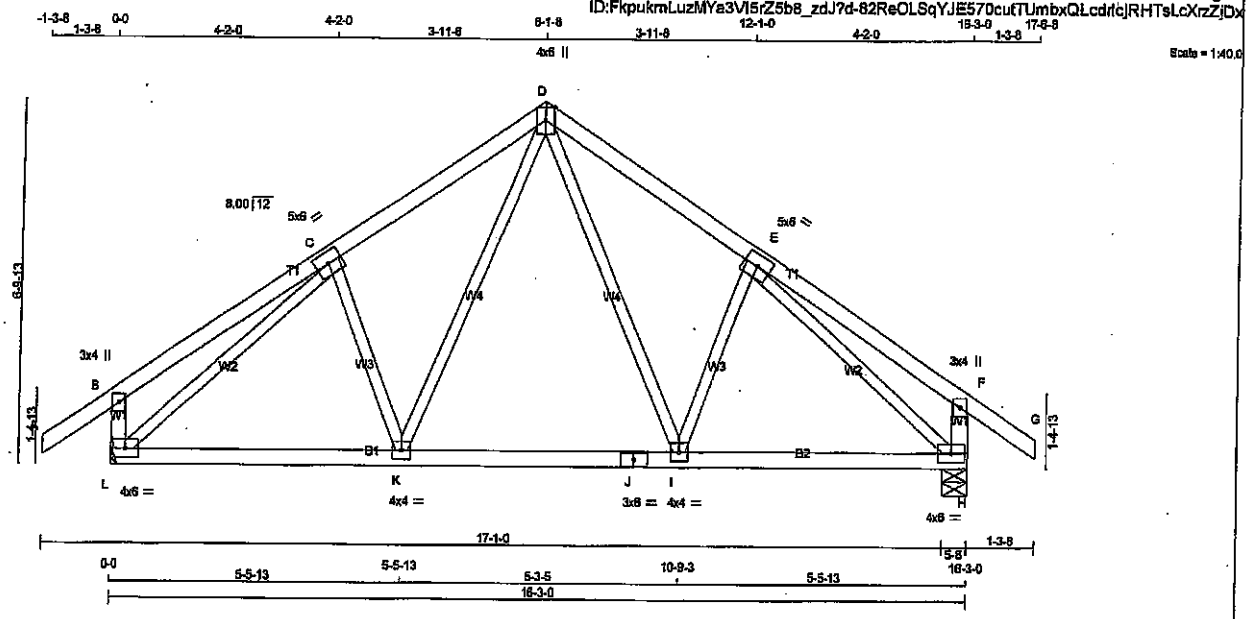

TOTAL WEIGHT = 4 X 72 = 290 lb

[MIF]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1112	0	1112	0	0	MECHANICAL	
H	1112	0	1112	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	779	552 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0
H	779	552 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	0 / 27	-102.1	-102.1	0.27 (1)	10.00	
C- D	-915 / 0	-102.1	-102.1	0.22 (1)	6.20	
D- E	-915 / 0	-102.1	-102.1	0.22 (1)	6.20	
E- F	0 / 27	-102.1	-102.1	0.27 (1)	10.00	
F- G	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
L- B	-298 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-298 / 0	0.0	0.0	0.03 (1)	7.81	
L- K	0 / 828	-17.5	-17.5	0.20 (1)	10.00	
K- J	0 / 605	-17.5	-17.5	0.16 (4)	10.00	
J- I	0 / 605	-17.5	-17.5	0.16 (4)	10.00	
I- H	0 / 828	-17.5	-17.5	0.20 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 358	0.08 (1)
I- E	-246 / 0	0.07 (1)
K- D	0 / 358	0.08 (1)
C- K	-246 / 0	0.07 (1)
L- C	-1133 / 0	0.57 (1)
E- H	-1133 / 0	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.27/1.00 (E-F:1) , BC=0.20/1.00 (K-L:1) , WB=0.57/1.00 (E-H:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (L) (INPUT = 0.90)
JSI METAL= 0.29 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009
PROVINCE OF ONTARIO

DWG NO. TAM 7905380
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T25A	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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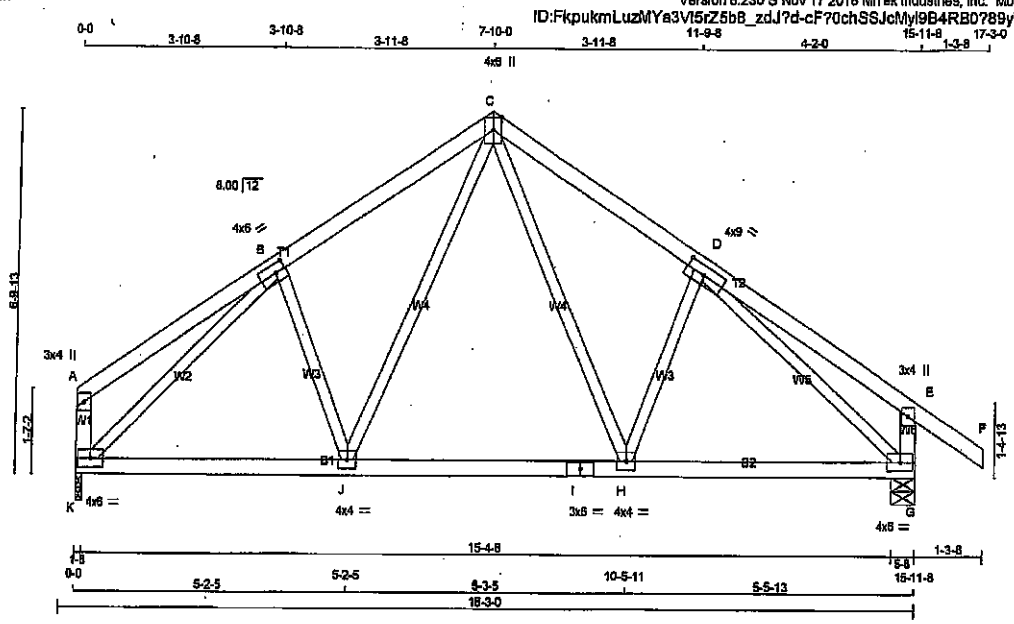

TOTAL WEIGHT = 2 X 70 = 140 lb

MiTek

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0	2.00	2.25
C	TTWW+p	MT20	4.0	8.0	Edge	
D	TMWW-t	MT20	4.0	9.0	2.00	4.25
E	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	954	0	954	0	0	1-8	1-8
G	1094	0	1094	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	871	463 / 0	0 / 0	0 / 0	0 / 0	207 / 0	0 / 0
G	787	543 / 0	0 / 0	0 / 0	0 / 0	224 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-102.1	-102.1	0.25 (1)	10.00	
B- C	-667 / 0	-102.1	-102.1	0.20 (1)	6.25	
C- D	-895 / 0	-102.1	-102.1	0.21 (1)	6.25	
D- E	0 / 26	-102.1	-102.1	0.27 (1)	10.00	
E- F	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
K- A	-148 / 0	0.0	0.0	0.02 (1)	7.81	
G- E	-303 / 0	0.0	0.0	0.03 (1)	7.81	
K- J	0 / 769	-17.5	-17.5	0.18 (1)	10.00	
J- I	0 / 583	-17.5	-17.5	0.16 (4)	10.00	
I- H	0 / 583	-17.5	-17.5	0.16 (4)	10.00	
H- G	0 / 809	-17.5	-17.5	0.19 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	0 / 371	0.08 (1)
H- D	-256 / 0	0.07 (1)
J- C	0 / 312	0.07 (1)
B- J	-202 / 0	0.06 (1)
K- B	-1087 / 0	0.50 (1)
D- G	-1104 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.27/1.00 (D-E:1) , BC=0.19/1.00 (G-H:1) , WB=0.55/1.00 (D-G:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	766	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (K) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009021
PROVINCE OF ONTARIO

DWG NO. TAM 17705382
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	G25	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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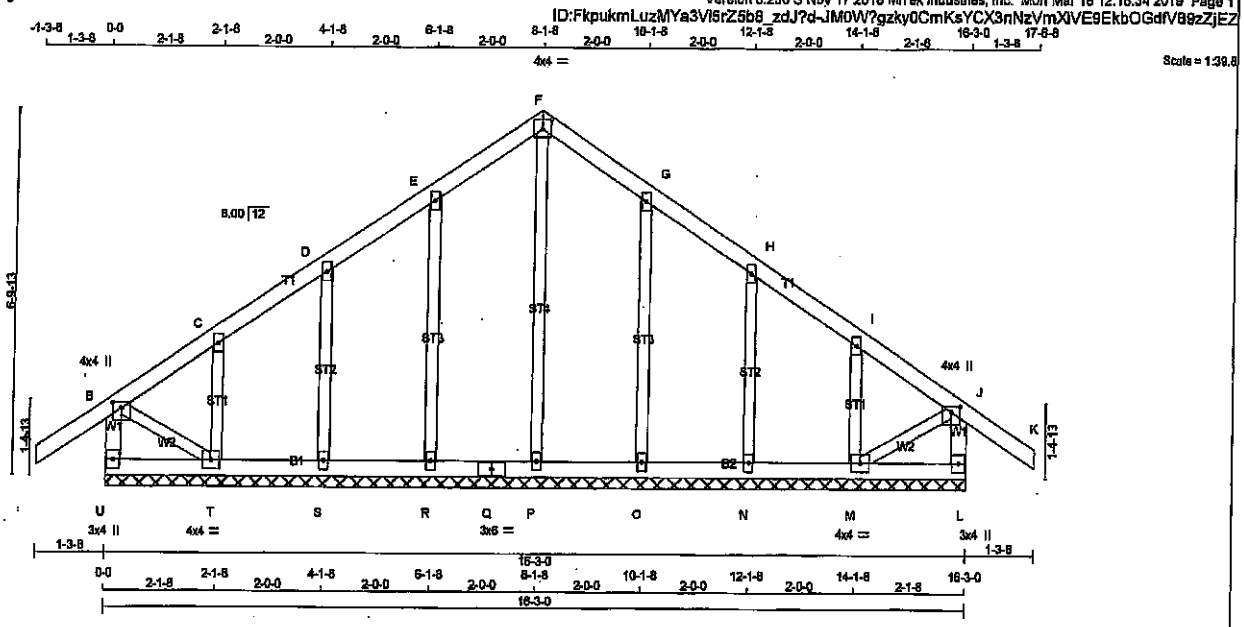

TOTAL WEIGHT = 2 X 73 = 145 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - Q	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, R, S						
N	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	6.0		
T	BMWW1-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B-C	-26 / 0	-102.1	-102.1	0.06 (1)	6.25
C-D	-33 / 0	-102.1	-102.1	0.06 (1)	6.25
D-E	-20 / 0	-102.1	-102.1	0.06 (1)	6.25
E-F	-31 / 0	-102.1	-102.1	0.06 (1)	6.25
F-G	-31 / 0	-102.1	-102.1	0.06 (1)	6.25
G-H	-20 / 0	-102.1	-102.1	0.06 (1)	6.25
H-I	-33 / 0	-102.1	-102.1	0.06 (1)	6.25
I-J	-26 / 0	-102.1	-102.1	0.06 (1)	6.25
J-K	0 / 39	-102.1	-102.1	0.14 (1)	10.00
U-B	-280 / 0	0.0	0.0	0.03 (1)	7.81
L-J	-280 / 0	0.0	0.0	0.03 (1)	7.81
U-T	0 / 0	-17.5	-17.5	0.02 (4)	10.00
T-S	0 / 24	-17.5	-17.5	0.02 (4)	10.00
S-R	0 / 20	-17.5	-17.5	0.01 (4)	10.00
R-Q	0 / 16	-17.5	-17.5	0.01 (4)	10.00
Q-P	0 / 16	-17.5	-17.5	0.01 (4)	10.00
P-O	0 / 16	-17.5	-17.5	0.01 (4)	10.00
O-N	0 / 20	-17.5	-17.5	0.01 (4)	10.00
N-M	0 / 24	-17.5	-17.5	0.02 (4)	10.00
M-L	0 / 0	-17.5	-17.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-F	-149 / 0	0.11 (1)
R-E	-234 / 0	0.11 (1)
S-D	-188 / 0	0.05 (1)
T-C	-230 / 0	0.04 (1)
O-G	-234 / 0	0.11 (1)
N-H	-188 / 0	0.05 (1)
M-I	-230 / 0	0.04 (1)
B-T	0 / 35	0.01 (1)
M-J	0 / 35	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (J-K:1) , BC=0.02/1.00 (M-N:4) , WB=0.11/1.00 (F-P:1) , SSI=0.09/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (T) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H.J. C ALVES
100010874
PROVINCE OF ONTARIO

DWG NO. TAM 11705381
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T26	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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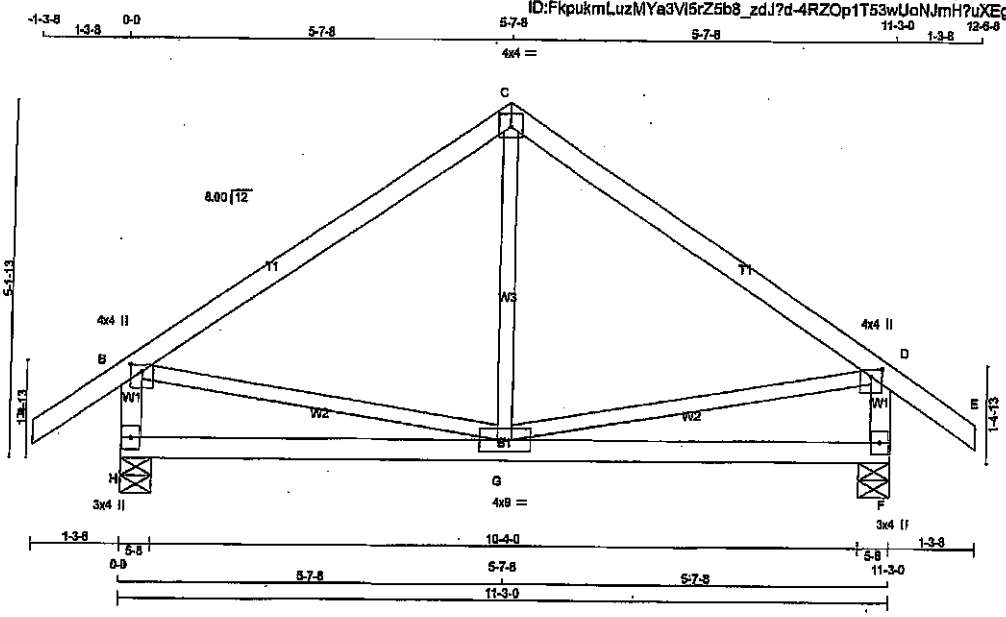

TOTAL WEIGHT = 2 X 47 = 93 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	813	0	813	0	0	5-8	5-8
F	813	0	813	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	569	406 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0
F	569	406 / 0	0 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B-C	-487 / 0	-102.1	-102.1	0.42 (1)	6.25	
C-D	-487 / 0	-102.1	-102.1	0.42 (1)	6.25	
D-E	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
H-B	-775 / 0	0.0	0.0	0.08 (1)	7.81	
F-D	-775 / 0	0.0	0.0	0.08 (1)	7.81	
H-G	0 / 0	-17.5	-17.5	0.16 (4)	10.00	
G-F	0 / 0	-17.5	-17.5	0.16 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	-35 / 85	0.03 (4)
B-G	0 / 412	0.09 (1)
G-D	0 / 412	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.16/1.00 (F-G:4) , WB=0.09/1.00 (D-G:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	765	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (D) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G ALVES
100009122
PROVINCE OF ONTARIO

DWG NO. TAM T905383
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	G26	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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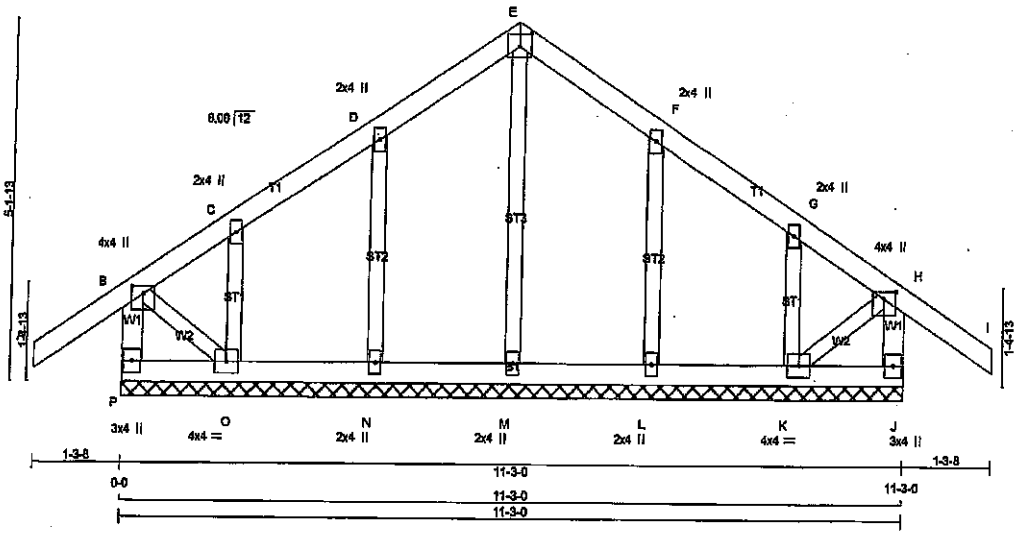

TOTAL WEIGHT = 2 X 49 = 98 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-B	-296 / 0	0.0	0.0	0.03 (1)	7.81	
A-B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B-C	-50 / 0	-102.1	-102.1	0.13 (1)	6.25	
C-D	-4 / 0	-102.1	-102.1	0.07 (1)	10.00	
D-E	-20 / 0	-102.1	-102.1	0.07 (1)	6.25	
E-F	-20 / 0	-102.1	-102.1	0.07 (1)	6.25	
F-G	-4 / 0	-102.1	-102.1	0.07 (1)	10.00	
G-H	-50 / 0	-102.1	-102.1	0.13 (1)	6.25	
H-I	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
J-H	-296 / 0	0.0	0.0	0.03 (1)	7.81	
P-O	0 / 0	-17.5	-17.5	0.01 (4)	10.00	
O-N	0 / 12	-17.5	-17.5	0.01 (4)	10.00	
N-M	0 / 6	-17.5	-17.5	0.01 (4)	10.00	
M-L	0 / 6	-17.5	-17.5	0.01 (4)	10.00	
L-K	0 / 12	-17.5	-17.5	0.01 (4)	10.00	
K-J	0 / 0	-17.5	-17.5	0.01 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-157 / 0	0.08 (1)
N-D	-247 / 0	0.08 (1)
O-C	-103 / 0	0.02 (1)
L-F	-247 / 0	0.06 (1)
K-G	-103 / 0	0.02 (1)
B-O	0 / 19	0.00 (1)
K-H	0 / 19	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.01/1.00 (N-O:4) , WB=0.08/1.00 (E-M:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (H) (INPUT = 0.90)
JSI METAL= 0.13 (F) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
07/8/19
H. J. GALVES
100009
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
DWG NO. TAM
STRUCTURAL
TOWN OF ORANGEVILLE
BUILDING DEPARTMENT ONLY
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	G27	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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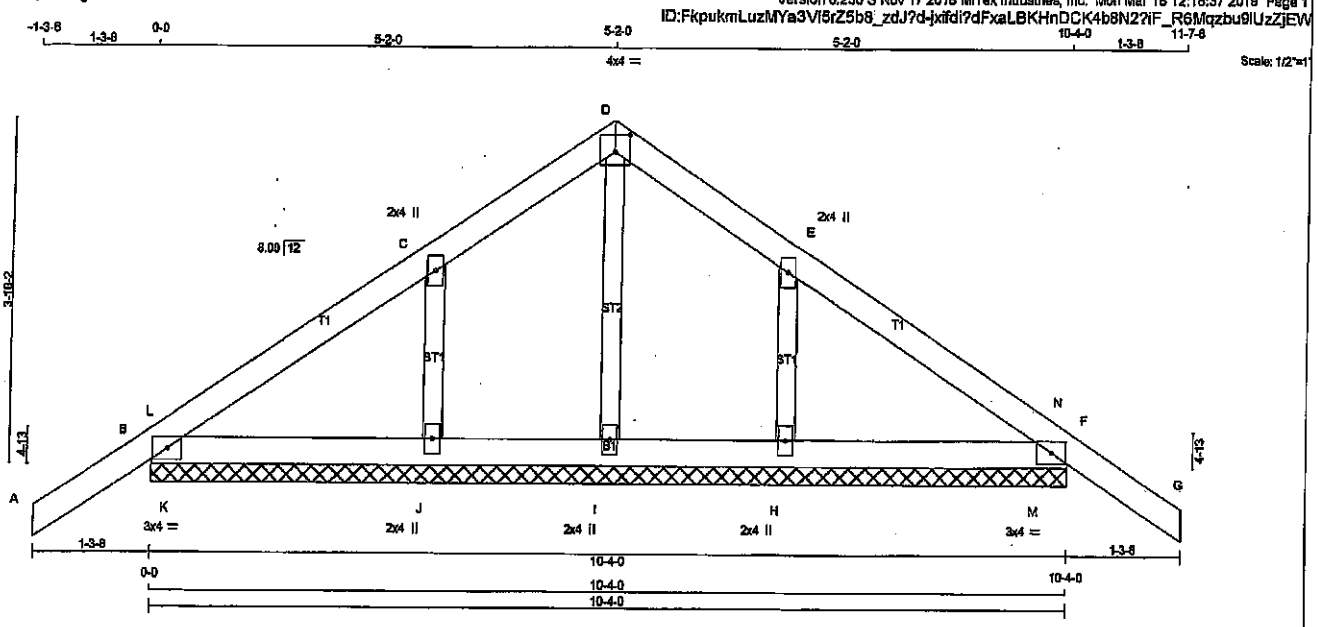

TOTAL WEIGHT = 36 lb

JM

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 36	-102.1	-102.1	0.14 (1)	10.00	
B-L	-44 / 0	-102.1	-102.1	0.02 (1)	6.25	
L-C	-60 / 0	-102.1	-102.1	0.11 (1)	6.25	
C-D	-79 / 0	-102.1	-102.1	0.11 (1)	6.25	
D-E	-79 / 0	-102.1	-102.1	0.11 (1)	6.25	
E-N	-60 / 0	-102.1	-102.1	0.11 (1)	6.25	
N-F	-44 / 0	-102.1	-102.1	0.02 (1)	6.25	
F-G	0 / 36	-102.1	-102.1	0.14 (1)	10.00	
B-K	0 / 60	-17.5	-17.5	0.07 (1)	10.00	
K-J	0 / 60	-17.5	-17.5	0.07 (1)	10.00	
J-I	0 / 49	-17.5	-17.5	0.05 (1)	10.00	
I-H	0 / 49	-17.5	-17.5	0.05 (1)	10.00	
H-M	0 / 60	-17.5	-17.5	0.07 (1)	10.00	
M-F	0 / 60	-17.5	-17.5	0.07 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	-73 / 0	0.02 (1)
J-C	-302 / 0	0.05 (1)
H-E	-302 / 0	0.05 (1)
K-L	-175 / 2	0.00 (1)
M-N	-175 / 2	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1) , BC=0.07/1.00 (B-K:1) , WB=0.05/1.00 (C-J:1) , SSI=0.14/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (F) (INPUT = 0.90)
JSI METAL= 0.15 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 17905385
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T28	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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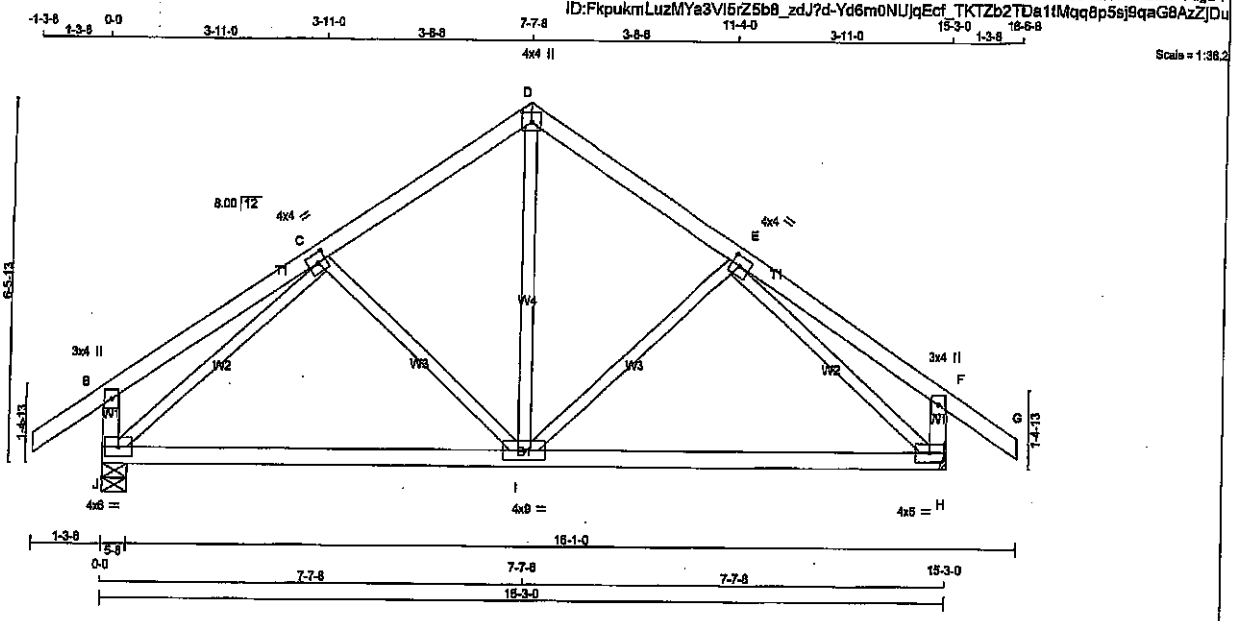

TOTAL WEIGHT = 2 X 65 = 130 lb

MiTek

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1052	0	1052	0	0	5-8	5-8
H	1052	0	1052	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	737	522 / 0	0 / 0	0 / 0	0 / 0	215 / 0	0 / 0
H	737	522 / 0	0 / 0	0 / 0	0 / 0	215 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 25	-102.1	-102.1	0.24 (1)	10.00
C- D	-726 / 0	-102.1	-102.1	0.18 (1)	6.25
D- E	-726 / 0	-102.1	-102.1	0.18 (1)	6.25
E- F	0 / 25	-102.1	-102.1	0.24 (1)	10.00
F- G	0 / 39	-102.1	-102.1	0.14 (1)	10.00
J- B	-290 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-290 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 748	-17.5	-17.5	0.34 (4)	10.00
I- H	0 / 748	-17.5	-17.5	0.34 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 477	0.11 (1)
I- E	-227 / 0	0.11 (1)
C- I	-227 / 0	0.11 (1)
J- C	-1032 / 0	0.46 (1)
E- H	-1032 / 0	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55.% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.24/1.00 (E-F:1) , BC=0.34/1.00 (H-I:4) , WB=0.46/1.00 (E-H:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (E) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 7905386
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	G28	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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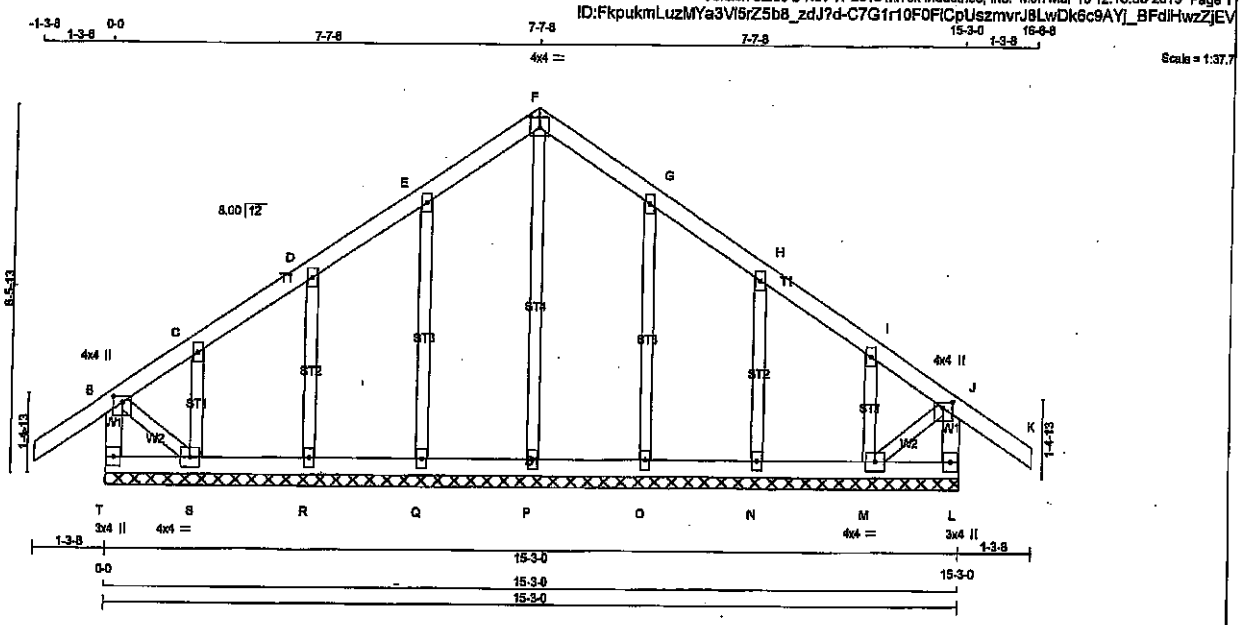

TOTAL WEIGHT = 68 lb
[M]

LUMBER
N. L G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-304 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B- C	-58 / 0	-102.1	-102.1	0.13 (1)	6.25
C- D	-16 / 0	-102.1	-102.1	0.05 (1)	6.25
D- E	-15 / 0	-102.1	-102.1	0.06 (1)	6.25
E- F	-24 / 0	-102.1	-102.1	0.06 (1)	6.25
F- G	-24 / 0	-102.1	-102.1	0.06 (1)	6.25
G- H	-15 / 0	-102.1	-102.1	0.06 (1)	6.25
H- I	-16 / 0	-102.1	-102.1	0.05 (1)	6.25
I- J	-58 / 0	-102.1	-102.1	0.13 (1)	6.25
J- K	0 / 39	-102.1	-102.1	0.14 (1)	10.00
L- J	-304 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-17.5	-17.5	0.01 (4)	10.00
S- R	0 / 19	-17.5	-17.5	0.01 (4)	10.00
R- Q	0 / 14	-17.5	-17.5	0.01 (4)	10.00
Q- P	0 / 10	-17.5	-17.5	0.01 (4)	10.00
P- O	0 / 10	-17.5	-17.5	0.01 (4)	10.00
O- N	0 / 14	-17.5	-17.5	0.01 (4)	10.00
N- M	0 / 19	-17.5	-17.5	0.01 (4)	10.00
M- L	0 / 0	-17.5	-17.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-158 / 0	0.11 (1)
Q- E	-229 / 0	0.09 (1)
R- D	-210 / 0	0.05 (1)
S- C	-112 / 0	0.02 (1)
O- G	-229 / 0	0.09 (1)
N- H	-210 / 0	0.05 (1)
M- I	-112 / 0	0.02 (1)
B- S	0 / 28	0.01 (1)
M- J	0 / 28	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.01/1.00 (R-S:4) , WB=0.11/1.00 (F-P:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1887	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (J) (INPUT = 0.90)
JSI METAL= 0.12 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H J G ALVES
100009854
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
TOWN OF CALEDON
BUILDING SECTION
DWG NO. 401769 G28
FILE COPY ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T29	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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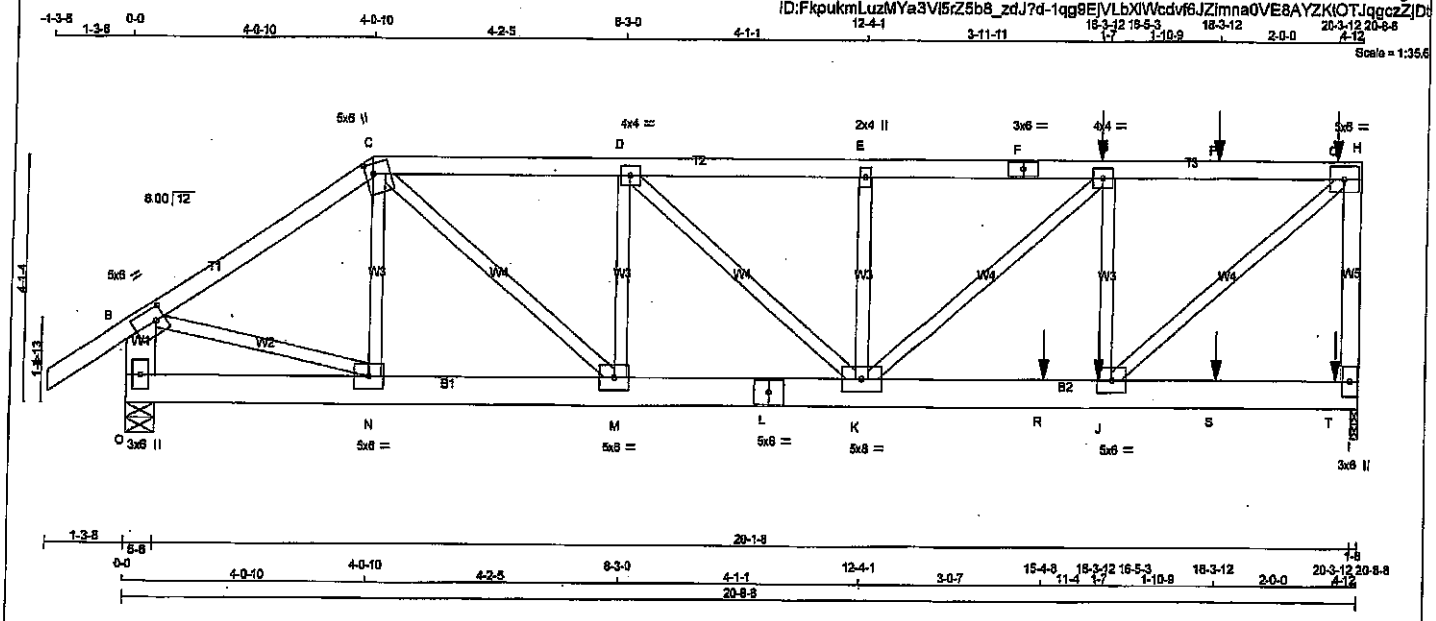

TOTAL WEIGHT = 2 X 100 = 199 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	SIDE(61.0)
H- I	1	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	TOP
L- I	2	12	SIDE(193.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DDWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2716	0	2716	0	0	1-8	1-8
O	1793	0	1793	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1905	1335 / 0	0 / 0	0 / 0	0 / 0	570 / 0	0 / 0
O	1257	889 / 0	0 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.08 (1)	10.00
B- C	-1876 / 0	-102.1	-102.1	0.18 (1)	6.16
C- D	-2593 / 0	-102.1	-102.1	0.17 (1)	5.47
D- E	-3102 / 0	-102.1	-102.1	0.18 (1)	5.08
E- F	-3102 / 0	-102.1	-102.1	0.22 (1)	5.03
F- G	-3102 / 0	-102.1	-102.1	0.22 (1)	5.03
G- P	-2707 / 0	-102.1	-102.1	0.24 (1)	5.29
P- Q	-2707 / 0	-102.1	-102.1	0.24 (1)	5.29
Q- H	-2707 / 0	-102.1	-102.1	0.24 (1)	5.29
I- H	-2717 / 0	0.0	0.0	0.34 (1)	6.93
O- B	-1765 / 0	0.0	0.0	0.09 (1)	7.81
O- N	0 / 0	-17.5	-17.5	0.02 (4)	10.00
N- M	0 / 1551	-17.5	-17.5	0.13 (1)	10.00
M- L	0 / 2593	-17.5	-17.5	0.24 (1)	10.00
L- K	0 / 2593	-17.5	-17.5	0.24 (1)	10.00
K- R	0 / 2707	-17.5	-17.5	0.48 (1)	10.00
R- J	0 / 2707	-17.5	-17.5	0.48 (1)	10.00
J- S	0 / 0	-17.5	-17.5	0.12 (1)	10.00
S- T	0 / 0	-17.5	-17.5	0.12 (1)	10.00
T- I	0 / 0	-17.5	-17.5	0.12 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-266 / 0	0.03 (1)
B- N	0 / 1803	0.20 (1)
J- H	0 / 3592	0.44 (1)
C- M	0 / 1392	0.17 (1)
J- G	-1085 / 0	0.14 (1)
M- D	-945 / 0	0.12 (1)
K- G	0 / 534	0.07 (1)
D- K	0 / 669	0.09 (1)
K- E	-356 / 0	0.06 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	16-3-12	-123	-123	—	FRONT	VERT	TOTAL	—	—
J	16-3-12	-25	-25	—	FRONT	VERT	TOTAL	—	—
P	18-3-12	-123	-123	—	FRONT	VERT	TOTAL	—	—
Q	20-3-12	-154	-154	—	FRONT	VERT	TOTAL	—	—
R	15-4-8	-1412	-1412	—	FRONT	VERT	TOTAL	—	—
S	18-3-12	-25	-25	—	FRONT	VERT	TOTAL	—	—
T	20-3-12	-30	-30	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.34/1.00 (H-I:1) , BC=0.48/1.00 (J-K:1) , WB=0.44/1.00 (H-J:1) , SSI=0.37/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.38 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100009[illegible]
PROVINCE OF ONTARIO

DWG NO. TAM 77905388
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T29Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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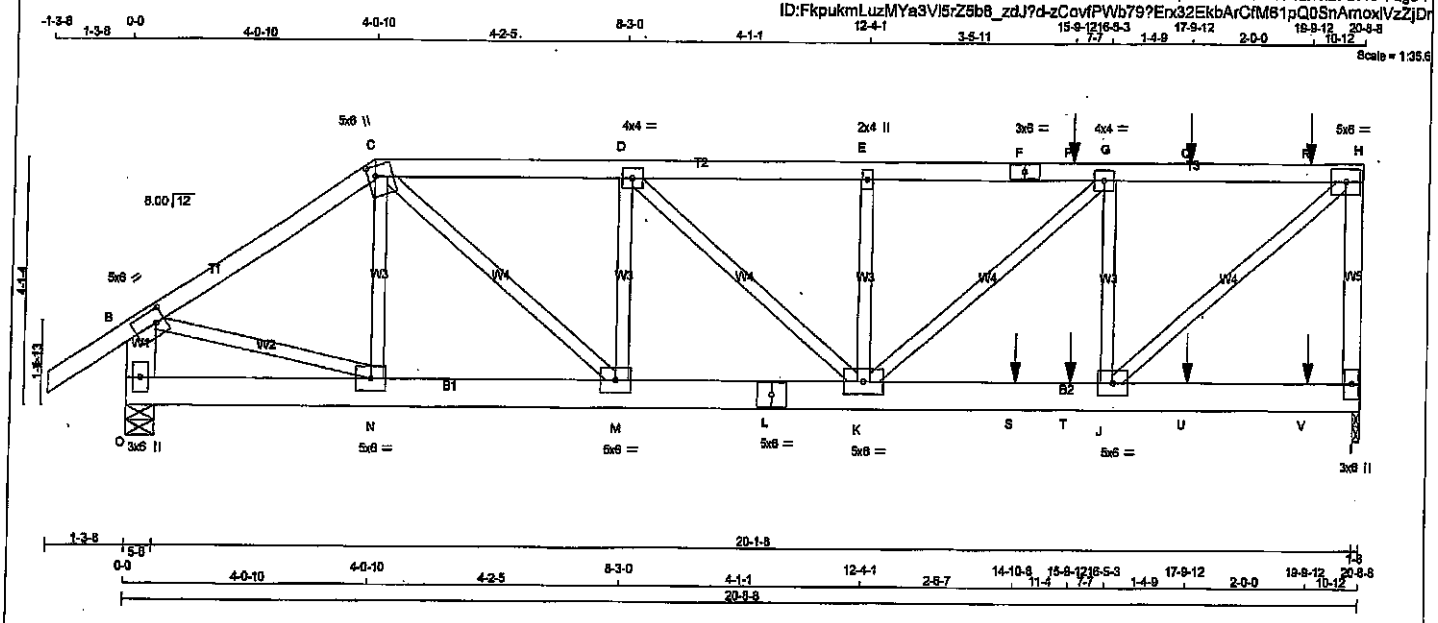

TOTAL WEIGHT = 2 X 100 = 199 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-F	1	12	TOP
F-H	1	12	SIDE(0.0)
H-I	1	12	TOP
O-B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-L	2	12	TOP
L-I	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	1.75
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2639	0	2639	0	0	1-8	1-8
O	1838	0	1838	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1851	1298 / 0	0 / 0	0 / 0	0 / 0	554 / 0	0 / 0
O	1288	912 / 0	0 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 39	-102.1	-102.1	0.08 (1)	10.00	
B-C	-1937 / 0	-102.1	-102.1	0.18 (1)	6.09	
C-D	-2684 / 0	-102.1	-102.1	0.17 (1)	5.40	
D-E	-3278 / 0	-102.1	-102.1	0.18 (1)	4.97	
E-F	-3278 / 0	-102.1	-102.1	0.26 (1)	4.88	
F-P	-3278 / 0	-102.1	-102.1	0.26 (1)	4.88	
P-G	-3278 / 0	-102.1	-102.1	0.26 (1)	4.88	
G-Q	-2717 / 0	-102.1	-102.1	0.24 (1)	5.27	
Q-R	-2717 / 0	-102.1	-102.1	0.24 (1)	5.27	
R-H	-2717 / 0	-102.1	-102.1	0.24 (1)	5.27	
I-H	-2684 / 0	0.0	0.0	0.33 (1)	6.98	
O-B	-1811 / 0	0.0	0.0	0.06 (1)	7.81	
O-N	0 / 0	-17.5	-17.5	0.02 (4)	10.00	
N-M	0 / 1601	-17.5	-17.5	0.14 (1)	10.00	
M-L	0 / 2684	-17.5	-17.5	0.28 (1)	10.00	
L-K	0 / 2684	-17.5	-17.5	0.28 (1)	10.00	
K-S	0 / 2717	-17.5	-17.5	0.56 (1)	10.00	
S-T	0 / 2717	-17.5	-17.5	0.56 (1)	10.00	
T-J	0 / 2717	-17.5	-17.5	0.56 (1)	10.00	
J-U	0 / 0	-17.5	-17.5	0.14 (1)	10.00	
U-V	0 / 0	-17.5	-17.5	0.14 (1)	10.00	
V-I	0 / 0	-17.5	-17.5	0.14 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-269 / 0	0.03 (1)
B-N	0 / 1654	0.20 (1)
J-H	0 / 3604	0.45 (1)
C-M	0 / 1446	0.18 (1)
J-G	-1262 / 0	0.16 (1)
M-D	-1020 / 0	0.13 (1)
K-G	0 / 759	0.09 (1)
D-K	-0 / 803	0.10 (1)
K-E	-368 / 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	15-9-12	-123	-123	—	BACK	VERT	TOTAL	—	—
Q	17-9-12	-123	-123	—	BACK	VERT	TOTAL	—	—
R	19-9-12	-128	-128	—	BACK	VERT	TOTAL	—	—
S	14-10-8	-1410	-1410	—	BACK	VERT	TOTAL	—	—
T	15-9-12	-25	-25	—	BACK	VERT	TOTAL	—	—
U	17-9-12	-25	-25	—	BACK	VERT	TOTAL	—	—
V	19-9-12	-26	-26	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.33/1.00 (H-I:1) , BC=0.56/1.00 (J-K:1) , WB=0.45/1.00 (H-J:1) , SSI=0.32/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.38 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100008924
PROVINCE OF ONTARIO

RECEIVED
DRWG NO. TAM
STRUCTURAL COMPONENT ONLY
TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T30	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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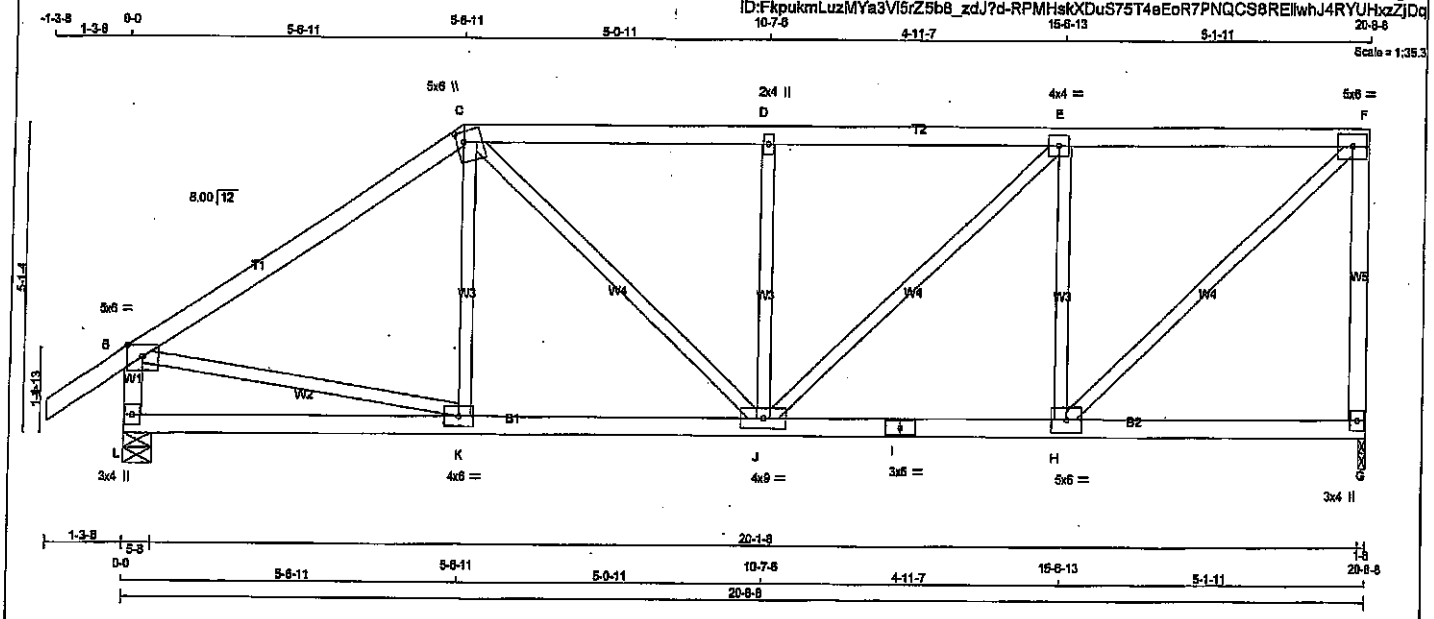

TOTAL WEIGHT = 2 X 88 = 176 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1379	0	1379	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	870	601 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
L	967	681 / 0	0 / 0	0 / 0	0 / 0	286 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B-C	-1278 / 0	-102.1	-102.1	0.63 (1)	4.60
C-D	-1385 / 0	-102.1	-102.1	0.38 (1)	5.05
D-E	-1385 / 0	-102.1	-102.1	0.40 (1)	5.03
E-F	-1058 / 0	-102.1	-102.1	0.36 (1)	5.60
G-F	-1201 / 0	0.0	0.0	0.53 (1)	7.29
L-B	-1338 / 0	0.0	0.0	0.14 (1)	7.00
L-K	0 / 0	-17.5	-17.5	0.13 (4)	10.00
K-J	0 / 1061	-17.5	-17.5	0.24 (1)	10.00
J-I	0 / 1056	-17.5	-17.5	0.23 (1)	10.00
I-H	0 / 1056	-17.5	-17.5	0.23 (1)	10.00
H-G	0 / 0	-17.5	-17.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-94 / 58	0.04 (1)
B-K	0 / 1082	0.24 (1)
H-F	0 / 1447	0.33 (1)
C-J	0 / 447	0.10 (1)
H-E	-667 / 0	0.34 (1)
J-D	-552 / 0	0.21 (1)
J-E	0 / 459	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.63/1.00 (B-C:1) , BC=0.24/1.00 (J-K:1) , WB=0.34/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.36 (I) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009
PROVINCE OF ONTARIO

DWG NO. TAM 7905390
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T31	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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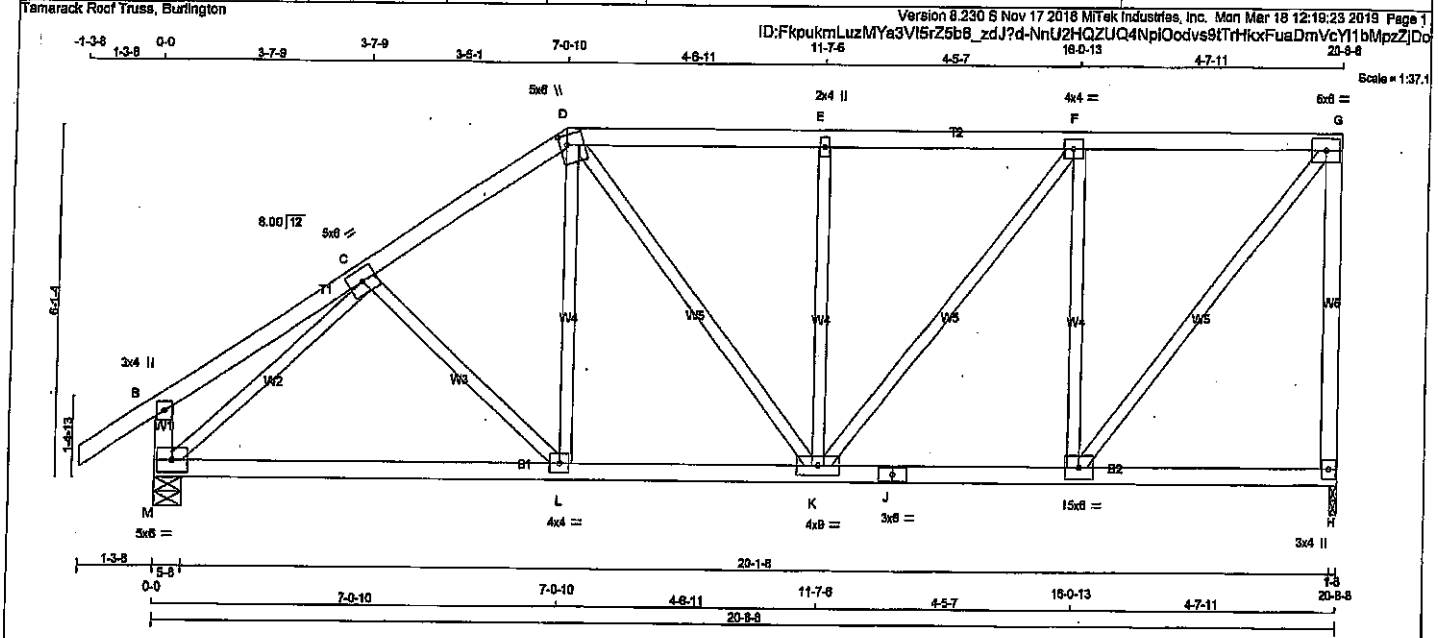

TOTAL WEIGHT = 2 X 96 = 192 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1238	0	1238	0	0	1-8	1-8
M	1379	0	1379	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	570	601 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
M	967	881 / 0	0 / 0	0 / 0	0 / 0	286 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	0 / 22	-102.1	-102.1	0.19 (1)	10.00	
C- D	-1231 / 0	-102.1	-102.1	0.22 (1)	5.53	
D- E	-1121 / 0	-102.1	-102.1	0.29 (1)	5.61	
E- F	-1121 / 0	-102.1	-102.1	0.31 (1)	5.58	
F- G	-809 / 0	-102.1	-102.1	0.30 (1)	6.25	
H- G	-1206 / 0	0.0	0.0	0.87 (1)	7.29	
M- B	-280 / 0	0.0	0.0	0.03 (1)	7.81	
M- L	0 / 1075	-17.5	-17.5	0.28 (1)	10.00	
L- K	0 / 1008	-17.5	-17.5	0.27 (4)	10.00	
K- J	0 / 809	-17.5	-17.5	0.18 (1)	10.00	
J- I	0 / 809	-17.5	-17.5	0.18 (1)	10.00	
I- H	0 / 0	-17.5	-17.5	0.09 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-97 / 18	0.04 (1)
L- D	0 / 195	0.05 (4)
M- C	-1500 / 0	0.58 (1)
I- G	0 / 1297	0.29 (1)
D- K	0 / 183	0.04 (1)
I- F	-918 / 0	0.54 (1)
K- E	-497 / 0	0.29 (1)
K- F	0 / 512	0.12 (1)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009029
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.87/1.00 (G-H:1) , BC=0.28/1.00 (L-M:1) , WB=0.58/1.00 (C-M:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (G) (INPUT = 0.80)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM 7905391
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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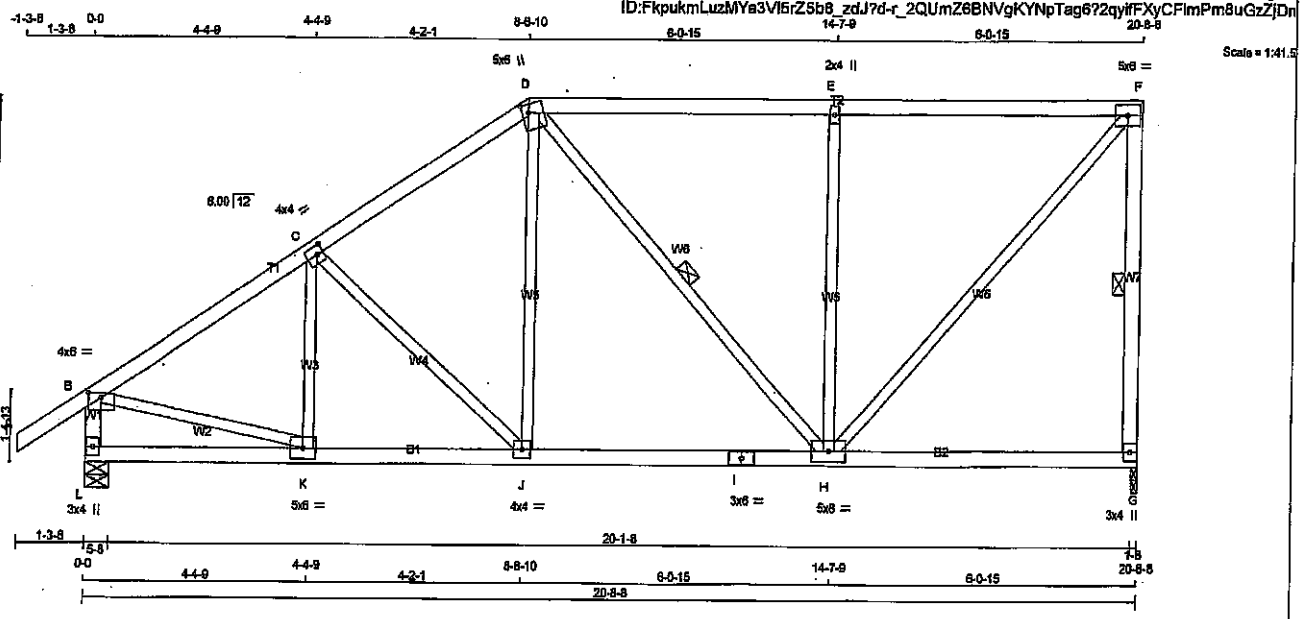

TOTAL WEIGHT = 95 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	B6-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1379	0	1379	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	870	601 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
L	967	681 / 0	0 / 0	0 / 0	0 / 0	286 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B-C	-1329 / 0	-102.1	-102.1	0.35 (1)	5.19	
C-D	-1132 / 0	-102.1	-102.1	0.33 (1)	5.53	
D-E	-860 / 0	-102.1	-102.1	0.68 (1)	5.37	
E-F	-860 / 0	-102.1	-102.1	0.68 (1)	5.37	
G-F	-1195 / 0	0.0	0.0	0.27 (1)	5.85	
L-B	-1344 / 0	0.0	0.0	0.14 (1)	6.99	
L-K	0 / 0	-17.5	-17.5	0.07 (4)	10.00	
K-J	0 / 1129	-17.5	-17.5	0.23 (1)	10.00	
J-I	0 / 920	-17.5	-17.5	0.23 (4)	10.00	
I-H	0 / 920	-17.5	-17.5	0.23 (4)	10.00	
H-G	0 / 0	-17.5	-17.5	0.16 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-C	-202 / 6	0.06 (1)
C-J	-291 / 0	0.17 (1)
J-D	0 / 268	0.08 (1)
D-H	-91 / 0	0.05 (1)
H-E	-769 / 0	0.67 (1)
H-F	0 / 1267	0.29 (1)
B-K	0 / 1163	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.23/1.00 (H-J:4) , WB=0.67/1.00 (E-H:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G ALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM T1905392
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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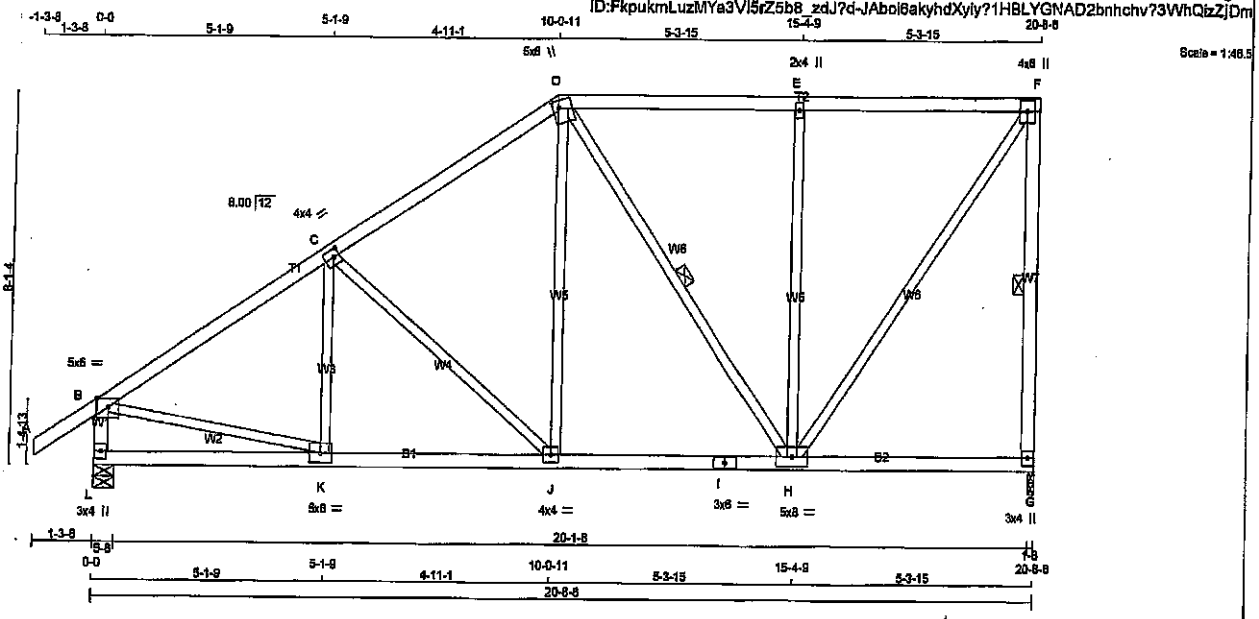

TOTAL WEIGHT = 101 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1379	0	1379	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	870	601 / 0	0 / 0	0 / 0	0 / 0	269 / 0	0 / 0
L	967	681 / 0	0 / 0	0 / 0	0 / 0	286 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00	
B- C	-1335 / 0	-102.1	-102.1	0.50 (1)	4.98	
C- D	-1023 / 0	-102.1	-102.1	0.47 (1)	5.52	
D- E	-660 / 0	-102.1	-102.1	0.50 (1)	6.25	
E- F	-681 / 0	-102.1	-102.1	0.50 (1)	6.25	
G- F	-1200 / 0	0.0	0.0	0.38 (1)	5.84	
L- B	-1340 / 0	0.0	0.0	0.14 (1)	7.00	
L- K	0 / 0	-17.5	-17.5	0.10 (4)	10.00	
K- J	0 / 1139	-17.5	-17.5	0.23 (1)	10.00	
J- I	0 / 826	-17.5	-17.5	0.19 (1)	10.00	
I- H	0 / 826	-17.5	-17.5	0.19 (1)	10.00	
H- G	0 / 0	-17.5	-17.5	0.12 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-143 / 40	0.05 (1)
C- J	-426 / 0	0.36 (1)
J- D	0 / 371	0.08 (1)
D- H	-259 / 0	0.16 (1)
H- E	-673 / 0	0.65 (1)
H- F	0 / 1202	0.27 (1)
B- K	0 / 1164	0.26 (1)

LICENSED PROFESSIONAL ENGINEER
02/18/19
H. J. G. ALVES
100009020
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.50/1.00 (E-F:1) , BC=0.23/1.00 (J-K:1) , WB=0.65/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.31 (F) (INPUT = 1.00)

DWG NO. TAM T190539.3
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T34	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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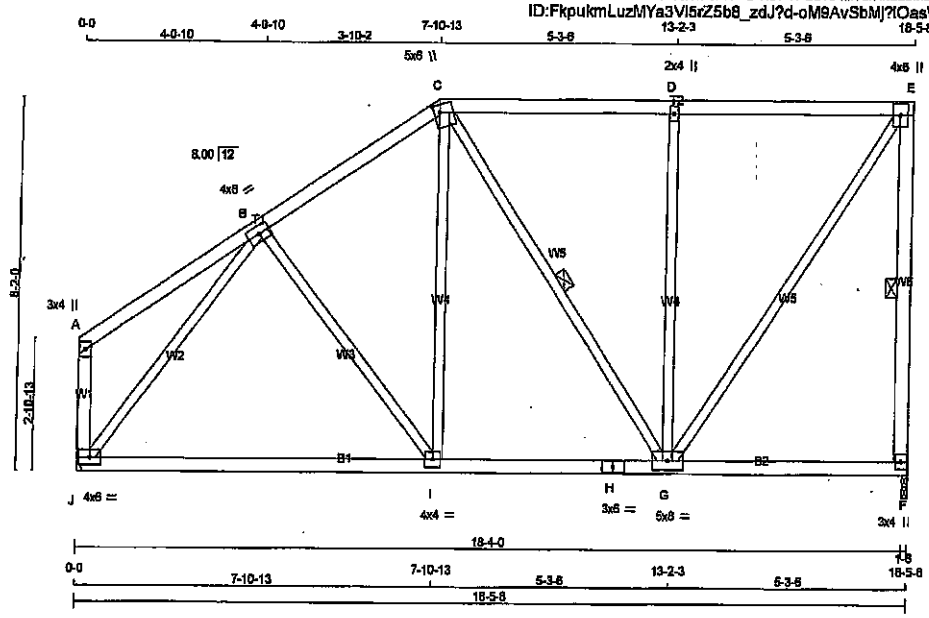

TOTAL WEIGHT = 93 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1104	0	1104	0	0	1-8	1-8
J	1104	0	1104	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	776	536 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0
J	776	536 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-102.1	-102.1	0.26 (1)	10.00	
B- C	-803 / 0	-102.1	-102.1	0.27 (1)	6.25	
C- D	-576 / 0	-102.1	-102.1	0.49 (1)	6.25	
D- E	-576 / 0	-102.1	-102.1	0.49 (1)	6.25	
F- E	-1062 / 0	0.0	0.0	0.32 (1)	6.12	
J- A	-155 / 0	0.0	0.0	0.02 (1)	7.81	
J- I	0 / 676	-17.5	-17.5	0.28 (4)	10.00	
I- H	0 / 648	-17.5	-17.5	0.28 (4)	10.00	
H- G	0 / 648	-17.5	-17.5	0.28 (4)	10.00	
G- F	0 / 0	-17.5	-17.5	0.11 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- I	-51 / 33	0.04 (1)
I- C	0 / 181	0.05 (4)
C- G	-126 / 0	0.08 (1)
G- D	-567 / 0	0.86 (1)
G- E	0 / 1032	0.23 (1)
J- B	-1118 / 0	0.84 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.28/1.00 (G-I:4) , WB=0.86/1.00 (D-G:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (J) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 11905394
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T35	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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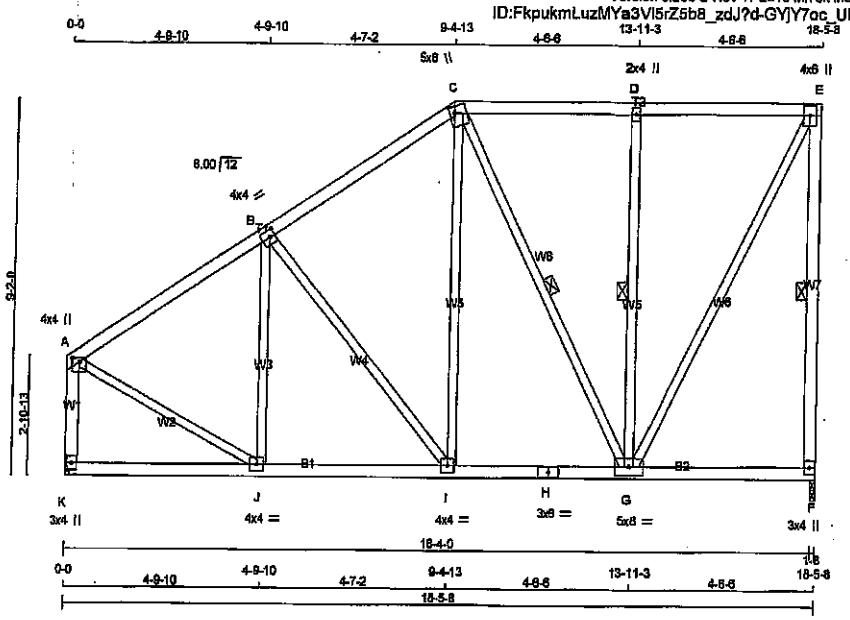

TOTAL WEIGHT = 2 X 101 = 202 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1104	0	1104	0	0	1-8	1-8
K	1104	0	1104	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	776	536 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0
K	776	536 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-849 / 0	-102.1	-102.1	0.30 (1)	6.24
B- C	-725 / 0	-102.1	-102.1	0.30 (1)	6.25
C- D	-459 / 0	-102.1	-102.1	0.26 (1)	6.25
D- E	-459 / 0	-102.1	-102.1	0.26 (1)	6.25
F- E	-1070 / 0	0.0	0.0	0.42 (1)	6.10
K- A	-1070 / 0	0.0	0.0	0.16 (1)	7.62
K- J	0 / 0	-17.5	-17.5	0.10 (4)	10.00
J- I	0 / 732	-17.5	-17.5	0.17 (1)	10.00
I- H	0 / 578	-17.5	-17.5	0.13 (1)	10.00
H- G	0 / 578	-17.5	-17.5	0.13 (1)	10.00
G- F	0 / 0	-17.5	-17.5	0.08 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-299 / 0	0.18 (1)
B- I	-250 / 0	0.27 (1)
I- C	0 / 269	0.06 (1)
C- G	-263 / 0	0.18 (1)
G- D	-570 / 0	0.31 (1)
G- E	0 / 1002	0.23 (1)
A- J	0 / 831	0.19 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF.
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.42/1.00 (E-F:1) , BC=0.17/1.00 (I-J:1) , WB=0.31/1.00 (D-G:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	788	1697	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 2.00 Deg.

USI GRIP=0.66 (J) (INPUT = 0.90)
USI METAL= 0.27 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
08/16/19
H. J. G. ALVES
100009
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO. 05395

REVIEWED FOR STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T36	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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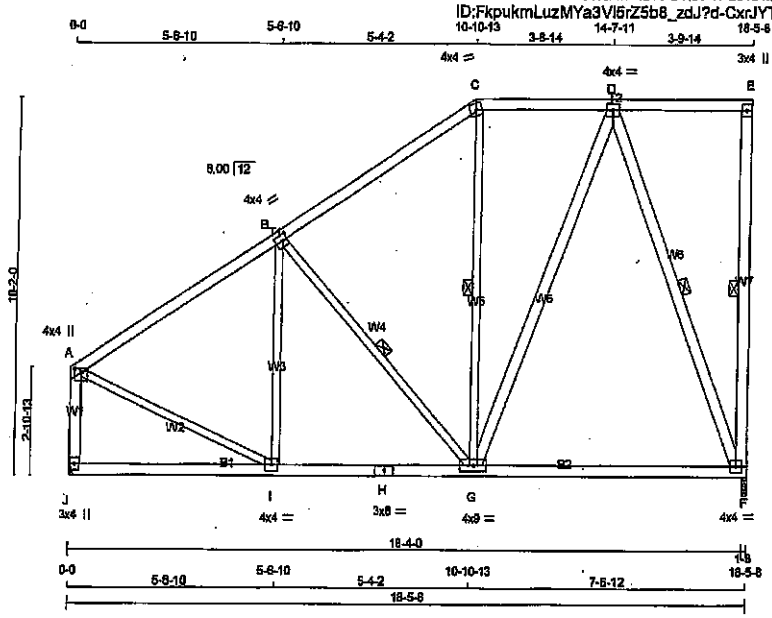

TOTAL WEIGHT = 3 X 106 = 317 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF.
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y.	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMW/1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1104	0	1104	0	0	1-8	1-8
J	1104	0	1104	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	776	536 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0
J	776	536 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, B-G, C-G, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-862 / 0	-102.1	-102.1	0.41 (1)	6.03
B-C	-646 / 0	-102.1	-102.1	0.40 (1)	6.25
C-D	-505 / 0	-102.1	-102.1	0.19 (1)	6.25
D-E	0 / 0	-102.1	-102.1	0.24 (1)	10.00
F-E	-148 / 0	0.0	0.0	0.08 (1)	6.25
J-A	-1062 / 0	0.0	0.0	0.16 (1)	7.64
J-I	0 / 0	-17.5	-17.5	0.11 (4)	10.00
I-H	0 / 748	-17.5	-17.5	0.26 (4)	10.00
H-G	0 / 748	-17.5	-17.5	0.26 (4)	10.00
G-F	0 / 349	-17.5	-17.5	0.23 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-B	-253 / 3	0.16 (1)
B-G	-371 / 0	0.17 (1)
G-C	-10 / 50	0.02 (4)
G-D	0 / 451	0.07 (1)
D-F	-967 / 0	0.57 (1)
A-I	0 / 625	0.19 (1)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009026
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.41/1.00 (A-B:1) , BC=0.26/1.00 (G-I:4) , WB=0.57/1.00 (D-F:1) , SSI=0.23/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.28 (I) (INPUT = 1.00)

DWG NO. TAM 17905396
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T37	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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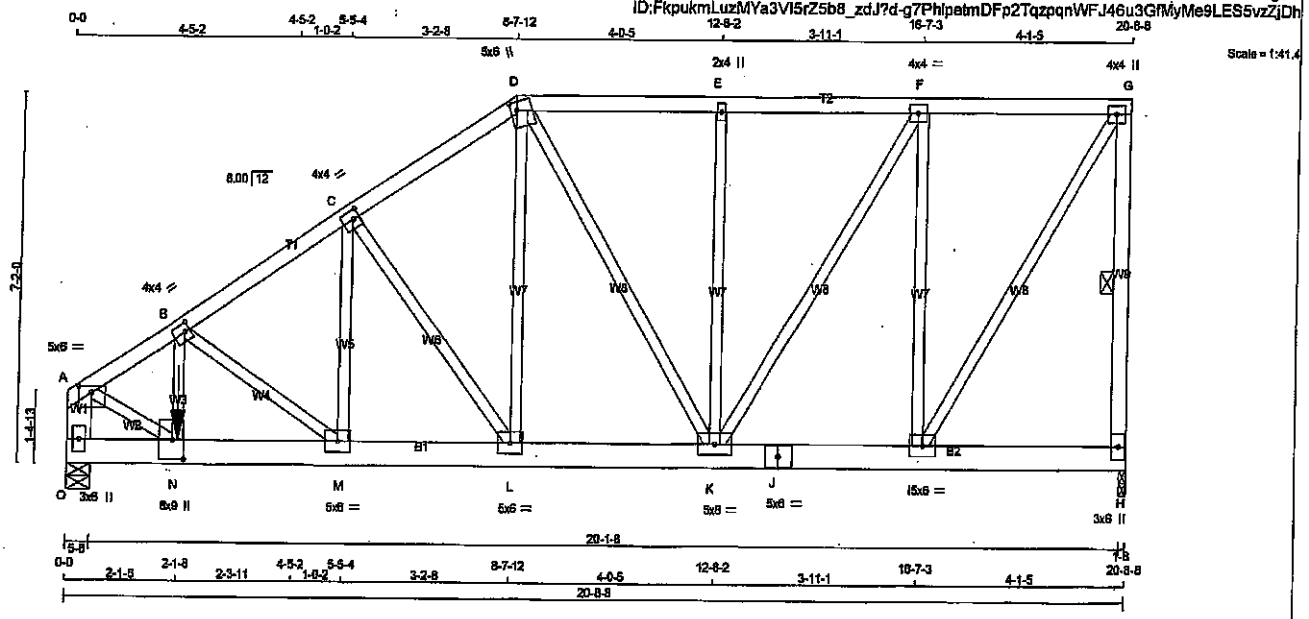

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
O - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
G- H	1	12	TOP
O- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- J	2	5	SIDE(549.3)
J- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
B- N	1	2	SIDE(411.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	1.50	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	6.0		
I, L, M						
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
N	BMWW+t	MT20	6.0	9.0	4.50	2.75
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1616	0	1616	0	0	1-8	1-8
O	4544	0	4544	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1136	788 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0
O	3169	2235 / 0	0 / 0	0 / 0	0 / 0	954 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4494 / 0	-102.1	-102.1	0.11 (1)	4.45	
B- C	-2611 / 0	-102.1	-102.1	0.12 (1)	5.36	
C- D	-1918 / 0	-102.1	-102.1	0.10 (1)	6.21	
D- E	-1364 / 0	-102.1	-102.1	0.12 (1)	6.25	
E- F	-1364 / 0	-102.1	-102.1	0.13 (1)	6.25	
F- G	-855 / 0	-102.1	-102.1	0.12 (1)	6.25	
H- G	-1584 / 0	0.0	0.0	0.18 (1)	6.25	
O- A	-4353 / 0	0.0	0.0	0.16 (1)	6.87	
O- N	0 / 0	-17.5	-17.5	0.13 (1)	10.00	
N- M	0 / 3740	-17.5	-17.5	0.38 (1)	10.00	
M- L	0 / 2354	-17.5	-17.5	0.16 (1)	10.00	
L- K	0 / 1594	-17.5	-17.5	0.12 (1)	10.00	
K- J	0 / 856	-17.5	-17.5	0.07 (1)	10.00	
J- I	0 / 855	-17.5	-17.5	0.07 (1)	10.00	
I- H	0 / 0	-17.5	-17.5	0.02 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	0 / 1666	0.21 (1)
L- D	0 / 1143	0.14 (1)
A- N	0 / 4131	0.51 (1)
I- G	0 / 1851	0.20 (1)
D- K	-450 / 0	0.28 (1)
I- F	-1332 / 0	0.57 (1)
K- E	-435 / 0	0.19 (1)
K- F	0 / 1017	0.13 (1)
B- M	-1715 / 0	0.25 (1)
M- C	0 / 1195	0.15 (1)
C- L	-1335 / 0	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL CONN.
N	2-1-8	-3684	-3684	—	BACK	VERT.	TOTAL	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.18/1.00 (G-H:1) , BC=0.38/1.00 (M-N:1) , WB=0.57/1.00 (F-I:1) , SSI=0.11/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (N) (INPUT = 0.90)
SI METAL= 0.56 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T905377
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T38	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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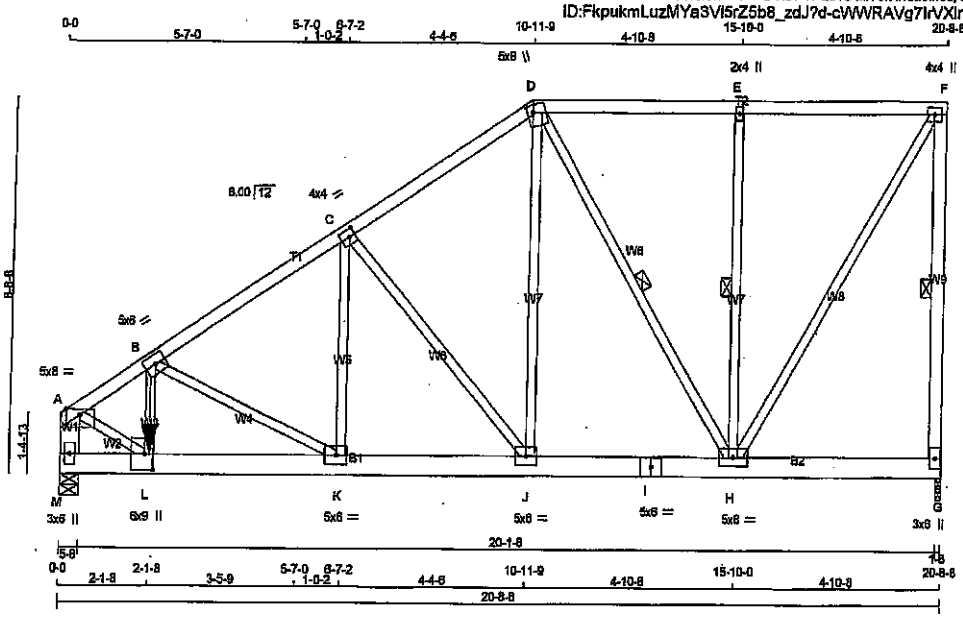

TOTAL WEIGHT = 2 X 119 = 237 lb
(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS.			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
M- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- I	2	4	SIDE(671.4)
I- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
B- L	1	2	SIDE(444.4)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	5.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	5.0	6.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	6.0	9.0	4.50	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1677	0	1677	0	0	1-8	1-8
M	5077	0	5077	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	18T LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1176	818 / 0	0 / 0	0 / 0	0 / 0	360 / 0	0 / 0
M	3562	2496 / 0	0 / 0	0 / 0	0 / 0	1066 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-5060 / 0	-102.1	-102.1	0.12 (1)	4.22	L- B
B- C	-2666 / 0	-102.1	-102.1	0.21 (1)	5.36	J- D
C- D	-1552 / 0	-102.1	-102.1	0.18 (1)	6.25	D- H
D- E	-853 / 0	-102.1	-102.1	0.22 (1)	6.25	H- E
E- F	-854 / 0	-102.1	-102.1	0.22 (1)	6.25	H- F
G- F	-1839 / 0	0.0	0.0	0.28 (1)	6.25	A- L
M- A	-4879 / 0	0.0	0.0	0.17 (1)	6.57	B- K
						K- C
M- L	0 / 0	-17.5	-17.5	0.14 (1)	10.00	C- J
L- K	0 / 4221	-17.5	-17.5	0.41 (1)	10.00	
K- J	0 / 2236	-17.5	-17.5	0.17 (1)	10.00	
J- I	0 / 1281	-17.5	-17.5	0.10 (1)	10.00	
I- H	0 / 1281	-17.5	-17.5	0.10 (1)	10.00	
H- G	0 / 0	-17.5	-17.5	0.03 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	0 / 2074	0.26 (1)
J- D	0 / 1224	0.15 (1)
D- H	-852 / 0	0.28 (1)
H- E	-614 / 0	0.15 (1)
H- F	0 / 1680	0.21 (1)
A- L	0 / 4662	0.58 (1)
B- K	-2256 / 0	0.49 (1)
K- C	0 / 1271	0.16 (1)
C- J	-1520 / 0	0.70 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR. VERT	TYPE	HEEL	CONN. TOP
L	2-1-8	-4277	-4277	—	FRONT				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.69")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.69")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.28/1.00 (F-G:1) , BC=0.41/1.00 (K-L:1) , WB=0.70/1.00 (C-J:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.63 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100008024
PROVINCE OF ONTARIO

DWG NO. TAM T905398
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T39	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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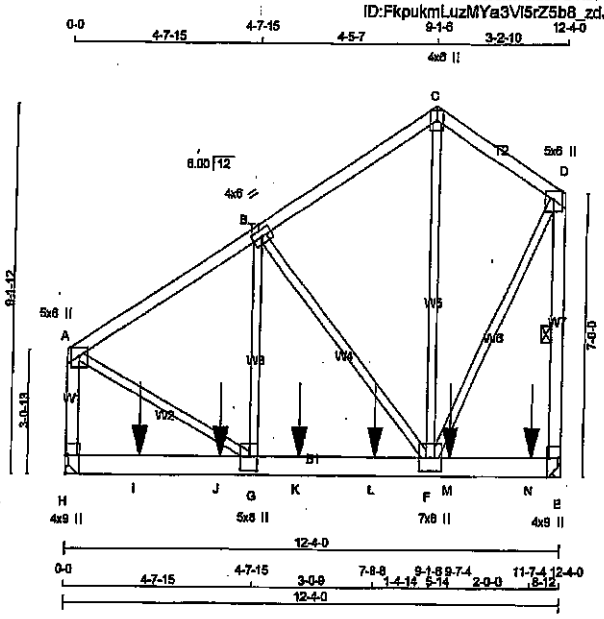

TOTAL WEIGHT = 2 X 78 = 152 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
H- A	1	12	TOP
E- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW+p	MT20	5.0	6.0	Edge	
E	BMV1+t	MT20	4.0	9.0	Edge 0.50	
F	BMWWW+t	MT20	7.0	8.0		
G	BMWW+t	MT20	5.0	8.0	4.25	2.25
H	BMV1+t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	3702	0	3702	0	0	IN-SX	IN-SX
E	4295	0	4295	0	0	MECHANICAL	
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2597	1820 / 0	0 / 0	0 / 0	0 / 0	777 / 0	0 / 0
E	3013	2112 / 0	0 / 0	0 / 0	0 / 0	902 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2757 / 0	-102.1	-102.1	0.23 (1)	5.27	
B- C	-1500 / 0	-102.1	-102.1	0.19 (1)	5.25	
C- D	-1470 / 0	-102.1	-102.1	0.11 (1)	6.25	
H- A	-3056 / 0	0.0	0.0	0.25 (1)	6.81	
E- D	-3466 / 0	0.0	0.0	0.40 (1)	5.01	
H- I	0 / 0	-17.5	-17.5	0.44 (1)	10.00	
I- J	0 / 0	-17.5	-17.5	0.44 (1)	10.00	
J- G	0 / 0	-17.5	-17.5	0.44 (1)	10.00	
G- K	0 / 2317	-17.5	-17.5	0.59 (1)	10.00	
K- L	0 / 2317	-17.5	-17.5	0.59 (1)	10.00	
L- F	0 / 2317	-17.5	-17.5	0.59 (1)	10.00	
F- M	0 / 0	-17.5	-17.5	0.27 (1)	10.00	
M- N	0 / 0	-17.5	-17.5	0.27 (1)	10.00	
N- E	0 / 0	-17.5	-17.5	0.27 (1)	.10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	0 / 1578	0.20 (1)
B- F	-1790 / 0	0.92 (1)
F- C	0 / 1293	0.16 (1)
A- G	0 / 2869	0.33 (1)
F- D	0 / 2799	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONT
I	1-9-12	-1086	-1086	—	FRONT	VERT	TOTAL	—	—
J	3-9-12	-1086	-1086	—	FRONT	VERT	TOTAL	—	—
K	5-9-12	-1086	-1086	—	FRONT	VERT	TOTAL	—	—
L	7-8-8	-1086	-1086	—	FRONT	VERT	TOTAL	—	—
M	9-7-4	-1088	-1088	—	FRONT	VERT	TOTAL	—	—
N	11-7-4	-1089	-1089	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.59/1.00 (F-G:1) , WB=0.92/1.00 (B-F:1) , SSI=0.53/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (G) (INPUT = 0.90)
JSI METAL= 0.53 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009827
PROVINCE OF ONTARIO

DWG NO. TAM 7905399
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T40	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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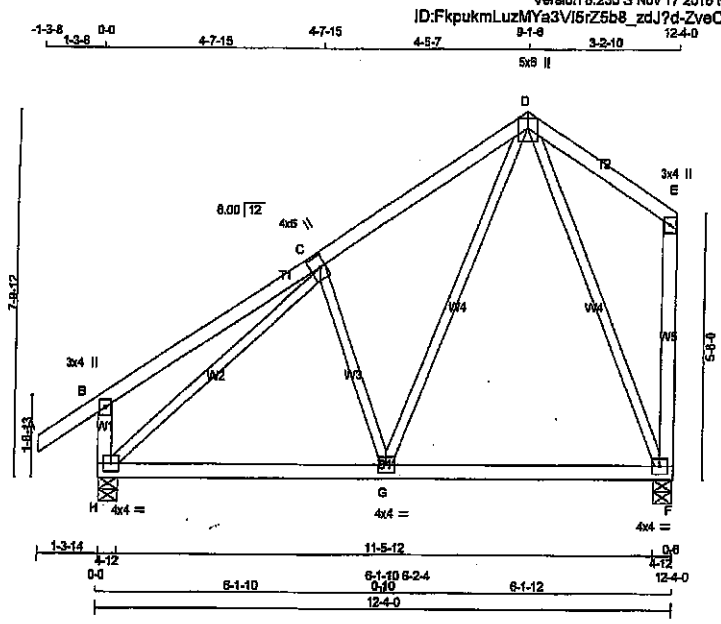

TOTAL WEIGHT = 62 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (tab/g is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+t	MT20	4.0	6.0	3.00	1.25
D	TTWW+p	MT20	5.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT.	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	738	0	738	0	0	4-12	4-12
H	878	0	878	0	0	4-12	4-12

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	518	358 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0
H	615	436 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 31	-102.1	-102.1	0.35 (1)	10.00
C- D	-523 / 0	-102.1	-102.1	0.27 (1)	6.25
D- E	0 / 0	-102.1	-102.1	0.18 (1)	10.00
H- B	-317 / 0	0.0	0.0	0.03 (1)	7.81
F- E	-164 / 0	0.0	0.0	0.10 (1)	7.81
H- G	0 / 531	-17.5	-17.5	0.23 (4)	10.00
G- F	0 / 228	-17.5	-17.5	0.20 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- D	0 / 497	0.11 (1)
D- F	-576 / 0	0.79 (1)
C- G	-353 / 0	0.13 (1)
H- C	-742 / 0	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.23/1.00 (G-H:4) , WB=0.79/1.00 (D-F:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches.

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 7905400
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	G41	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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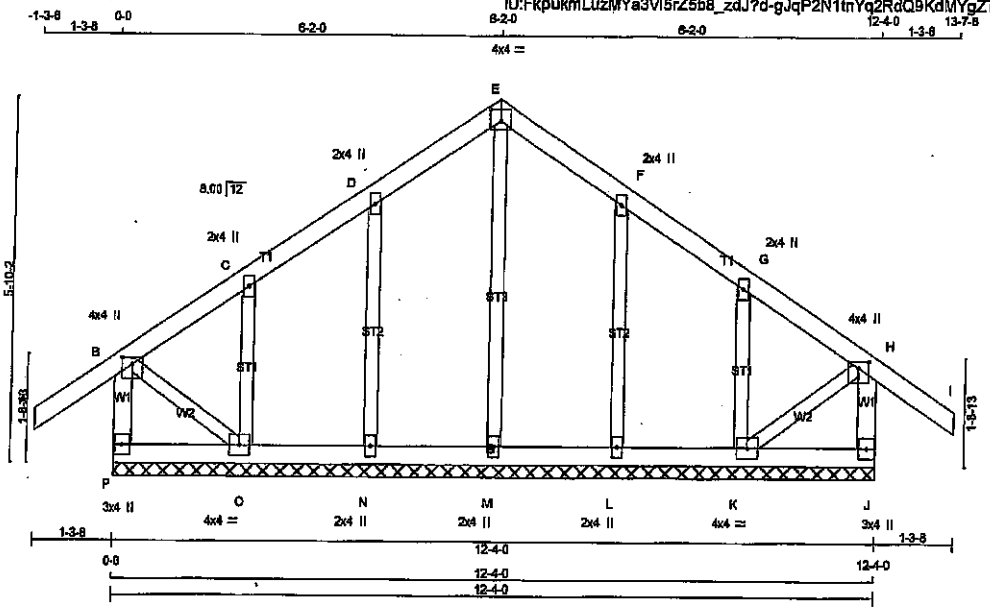

TOTAL WEIGHT = 56 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	2.25	2.00
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-B	-255 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B-C	-15 / 0	-102.1	-102.1	0.06 (1)	6.25
C-D	-19 / 0	-102.1	-102.1	0.06 (1)	6.25
D-E	-24 / 0	-102.1	-102.1	0.06 (1)	6.25
E-F	-24 / 0	-102.1	-102.1	0.06 (1)	6.25
F-G	-19 / 0	-102.1	-102.1	0.06 (1)	6.25
G-H	-15 / 0	-102.1	-102.1	0.06 (1)	6.25
H-I	0 / 39	-102.1	-102.1	0.14 (1)	10.00
J-H	-255 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 0	-17.5	-17.5	0.02 (4)	10.00
O-N	0 / 15	-17.5	-17.5	0.02 (4)	10.00
N-M	0 / 11	-17.5	-17.5	0.01 (4)	10.00
M-L	0 / 11	-17.5	-17.5	0.01 (4)	10.00
L-K	0 / 15	-17.5	-17.5	0.02 (4)	10.00
K-J	0 / 0	-17.5	-17.5	0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-156 / 0	0.08 (1)
N-D	-225 / 0	0.07 (1)
O-C	-226 / 0	0.04 (1)
L-F	-225 / 0	0.07 (1)
K-G	-226 / 0	0.04 (1)
B-O	0 / 26	0.01 (1)
K-H	0 / 26	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (K-L:4) , WB=0.08/1.00 (E-M:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (H) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100090073
PROVINCE OF ONTARIO

DWG NO. TAM 17905401
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T42	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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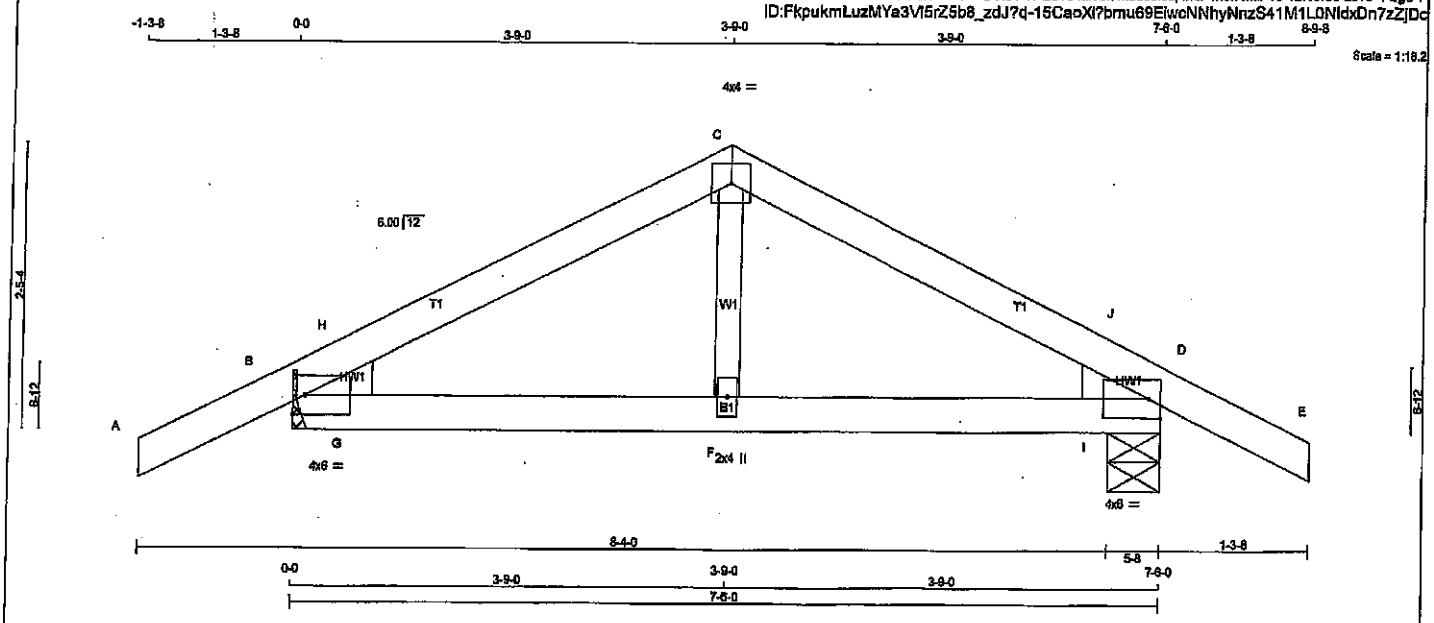

TOTAL WEIGHT = 25 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		Edge
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	4.0	6.0		Edge
F	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	587	0	587	0	0	MECHANICAL		2x4 L
D	587	0	587	0	0	5-8	5-8	2x4 R

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT B. MINIMUM BEARING LENGTH AT JOINT B = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	410	297 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0
D	410	297 / 0	0 / 0	0 / 0	0 / 0	114 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-102.1	-102.1	0.13 (1)	10.00
B- H	-509 / 0	-102.1	-102.1	0.07 (1)	6.25
H- C	-480 / 0	-102.1	-102.1	0.13 (1)	6.25
C- J	-480 / 0	-102.1	-102.1	0.13 (1)	6.25
J- D	-509 / 0	-102.1	-102.1	0.07 (1)	6.25
D- E	0 / 21	-102.1	-102.1	0.13 (1)	10.00
B- G	0 / 421	-17.5	-17.5	0.15 (1)	10.00
G- F	0 / 421	-17.5	-17.5	0.15 (1)	10.00
F- I	0 / 421	-17.5	-17.5	0.15 (1)	10.00
I- D	0 / 421	-17.5	-17.5	0.15 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 130	0.03 (1)
G- H	-123 / 20	0.00 (1)
I- J	-123 / 20	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.13/1.00 (C-H:1) , BC=0.15/1.00 (B-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.13/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (D) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H.J. GALVES
100009424
PROVINCE OF ONTARIO

DWG NO. TAM T1905402
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T42A	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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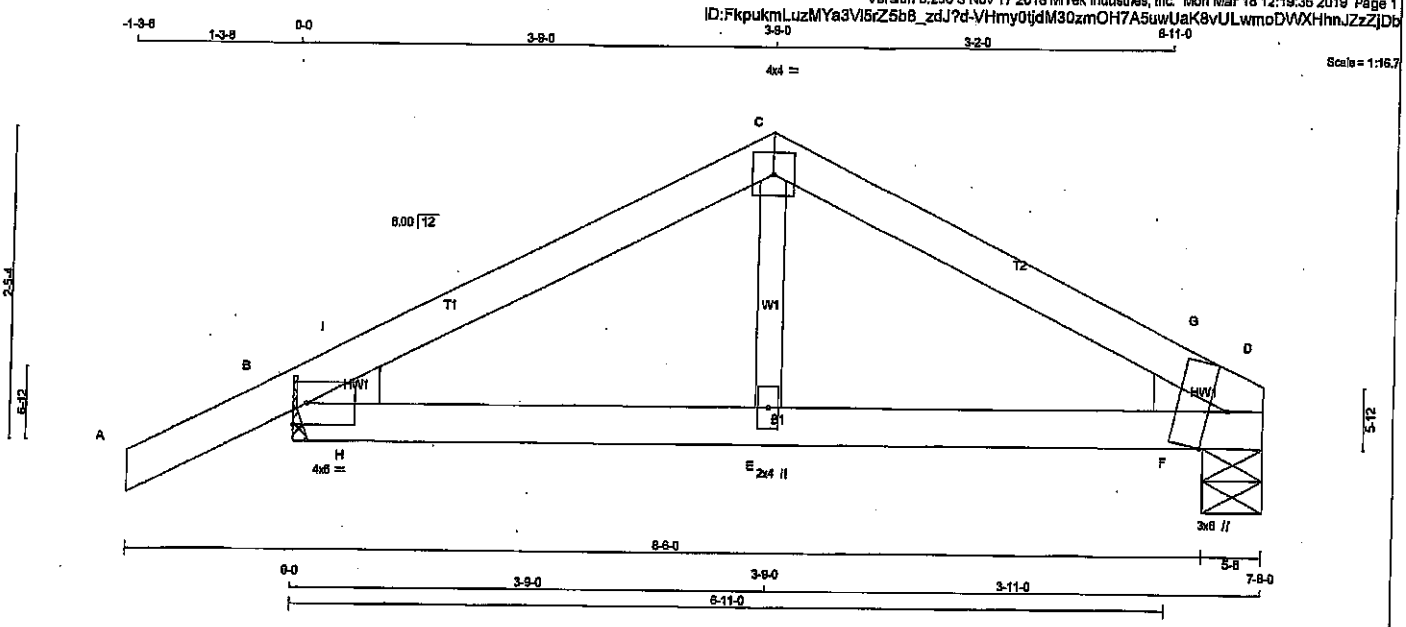

TOTAL WEIGHT = 2 X 23 = 46 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		Edge
C	TTW-p	MT20	4.0	4.0		
D	TBMH1+m	MT20	3.0	6.0	Edge	1.75
E	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
D	458	0	458	0	0	5-8	5-8	2x4 R
B	597	0	597	0	0	MECHANICAL		2x4 L

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT B. MINIMUM BEARING LENGTH AT JOINT B = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	322	223 / 0	0 / 0	0 / 0	0 / 0	100 / 0	0 / 0
B	417	301 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-102.1	-102.1	0.13 (1)	10.00
B- I	-534 / 0	-102.1	-102.1	0.07 (1)	6.25
I- C	-505 / 0	-102.1	-102.1	0.13 (1)	6.25
C- G	-501 / 0	-102.1	-102.1	0.15 (1)	6.25
G- D	-521 / 0	-102.1	-102.1	0.04 (1)	6.25
B- H	0 / 443	-17.5	-17.5	0.15 (1)	10.00
H- E	0 / 443	-17.5	-17.5	0.16 (1)	10.00
E- F	0 / 443	-17.5	-17.5	0.19 (1)	10.00
F- D	0 / 443	-17.5	-17.5	0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- C	0 / 144	0.03 (1)
F- G	-155 / 6	0.00 (1)
H- I	-123 / 20	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (C-G:1) , BC=0.19/1.00 (D-F:1) , WB=0.03/1.00 (C-E:1) , SSI=0.13/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (B) (INPUT = 0.60)
JSI METAL= 0.34 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H.J. G ALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM 77905403
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T42Z	1.	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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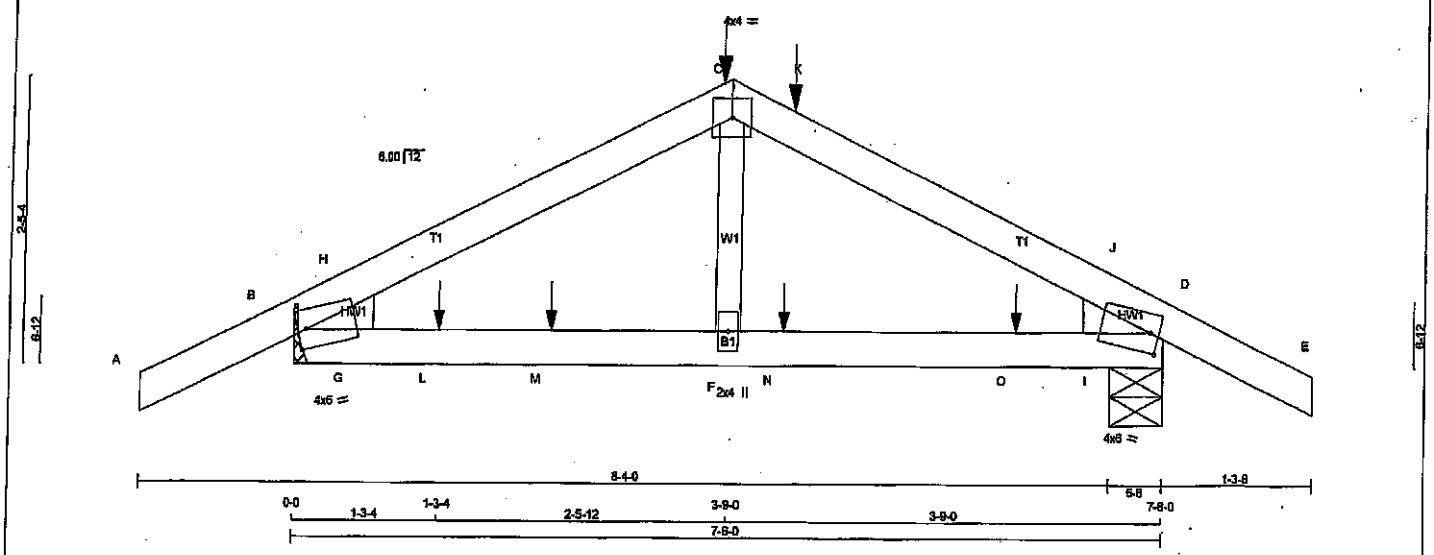

TOTAL WEIGHT = 25 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	6.0	2.00	0.75
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	4.0	6.0	2.00	0.75
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	687	0	687	0	0	MECHANICAL		2x4 L
D	689	0	689	0	0	5-8	5-8	2x4 R

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT B. MINIMUM BEARING LENGTH AT JOINT B = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	480	351 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
D	481	- 353 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-102.1	-102.1	0.14 (1)	10.00
B- H	-727 / 0	-102.1	-102.1	0.12 (1)	6.25
H- C	-668 / 0	-102.1	-102.1	0.16 (1)	6.25
C- K	-665 / 0	-102.1	-102.1	0.16 (1)	6.25
K- J	-665 / 0	-102.1	-102.1	0.16 (1)	6.25
J- D	-725 / 0	-102.1	-102.1	0.12 (1)	6.25
D- E	0 / 21	-102.1	-102.1	0.14 (1)	10.00
B- G	0 / 588	-17.5	-17.5	0.16 (1)	10.00
G- L	0 / 588	-17.5	-17.5	0.17 (1)	10.00
L- M	0 / 588	-17.5	-17.5	0.17 (1)	10.00
M- F	0 / 588	-17.5	-17.5	0.17 (1)	10.00
F- N	0 / 588	-17.5	-17.5	0.17 (1)	10.00
N- O	0 / 588	-17.5	-17.5	0.17 (1)	10.00
O- I	0 / 588	-17.5	-17.5	0.17 (1)	10.00
I- D	0 / 588	-17.5	-17.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 107	0.03 (4)
G- H	-69 / 31	0.00 (1)
I- J	-35 / 30	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-9-0	-31	-35	—	FRONT	VERT	DEAD		
C	3-9-0	-150	-150	—	FRONT	VERT	SNOW	—	—
K	4-2-12	-19	-19	—	BACK	VERT	TOTAL	—	—
L	1-3-4	-1	-1	—	BACK	VERT	TOTAL	—	—
M	2-2-12	-1	-1	—	BACK	VERT	TOTAL	—	—
N	4-2-12	-1	-1	—	BACK	VERT	TOTAL	—	—
O	6-2-12	-1	-1	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.25")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.25")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (C-J:1) , BC=0.17/1.00 (F-I:1) , WB=0.03/1.00 (C-F:4) , SSI=0.15/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (D) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H.J. GALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 77054 04
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	T43	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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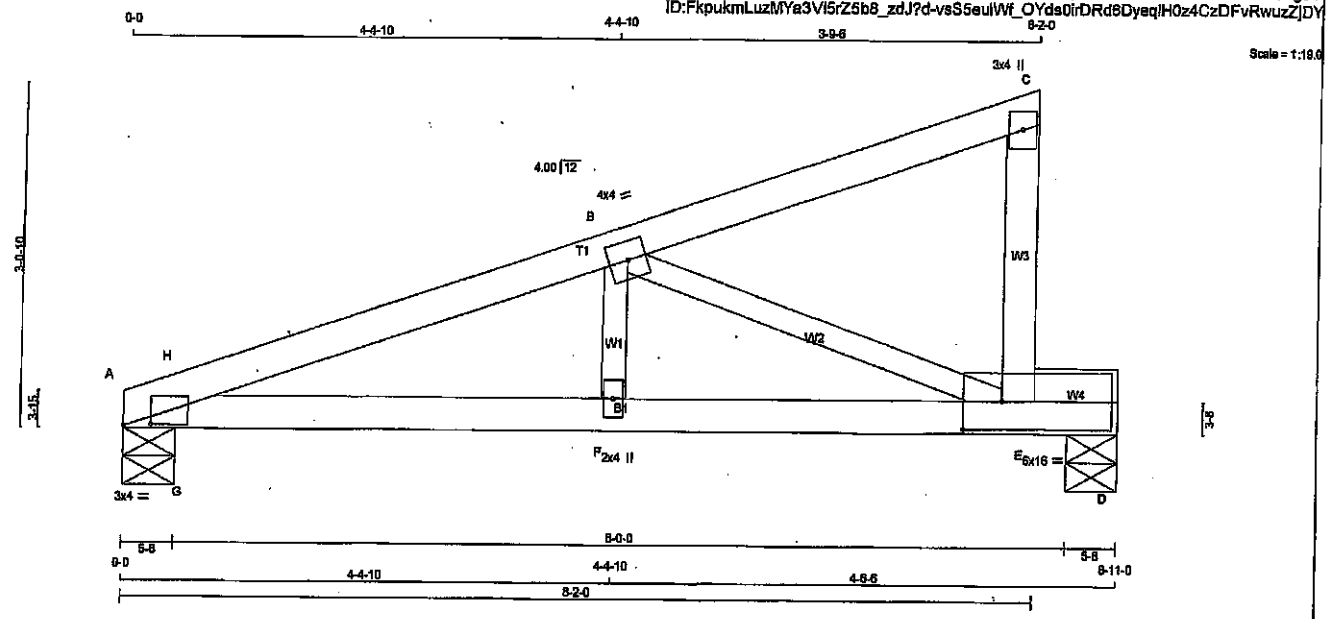

TOTAL WEIGHT = 4 X 28 = 111 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	3.0	4.0	0.25	3.00
B	TMWW-t	MT20	4.0	4.0		
C	TMV+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	6.0	16.0	3.00	4.25
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	530	0	530	0	0	5-8	5-8
D	460	0	460	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	373	257 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
D	324	217 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- H	-1019 / 0	-102.1	-102.1		0.03 (1)	6.18
H- B	-1004 / 0	-102.1	-102.1		0.17 (1)	6.05
B- C	-13 / 0	-102.1	-102.1		0.16 (1)	6.25
E- C	-153 / 0	0.0	0.0		0.02 (1)	7.81
A- G	0 / 963	-17.5	-17.5		0.29 (1)	10.00
G- F	0 / 963	-17.5	-17.5		0.31 (1)	10.00
F- E	0 / 963	-17.5	-17.5		0.62 (1)	10.00
E- D	0 / 0	-17.5	-17.5		0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	0 / 240	0.05 (1)
B- E	-1035 / 0	0.27 (1)
G- H	-175 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 885 (0.12")

CSI: TC=0.17/1.00 (B-H:1) , BC=0.62/1.00 (E-F:1) , WB=0.27/1.00 (B-E:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Degrees

JSI GRIP= 0.68 (B) (INPUT= 0.60)
JSI METAL = 0.90 (A) (INPUT= 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. O. GALVES
100009021
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO. 19-05405
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	J1	40	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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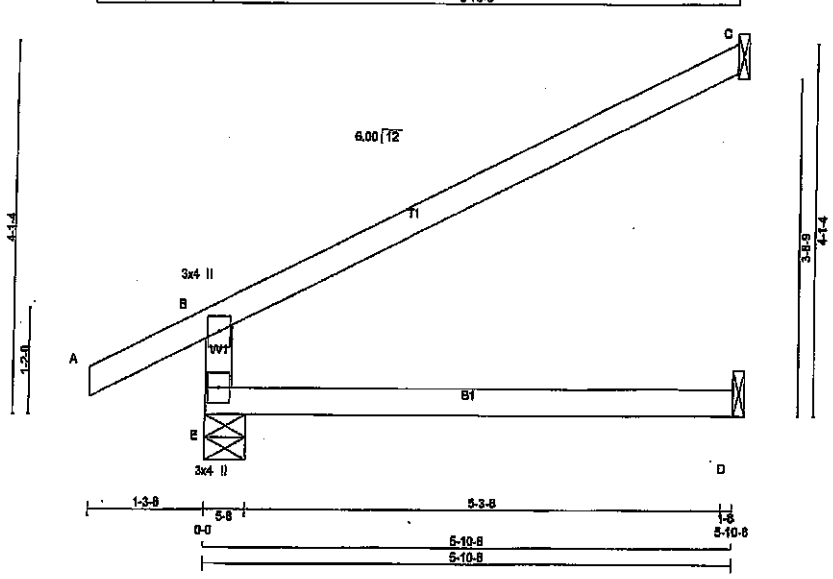

TOTAL WEIGHT = 40 X 17 = 672 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	574	0	574	0	0	5-8	5-8
C	225	0	225	0	0	1-8	1-8
D	42	0	48	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	401	292 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-513 / 0	0.0	0.0	0.12 (4)	7.81			
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.00			
B- C	-34 / 0	-102.1	-102.1	0.60 (1)	6.25			
E- D	0 / 0	-17.5	-17.5	0.12 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.12/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SS=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19
H. J. G. ALVES
100089227
PROVINCE OF ONTARIO

DWG NO. TAM 7905406
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	J2	15	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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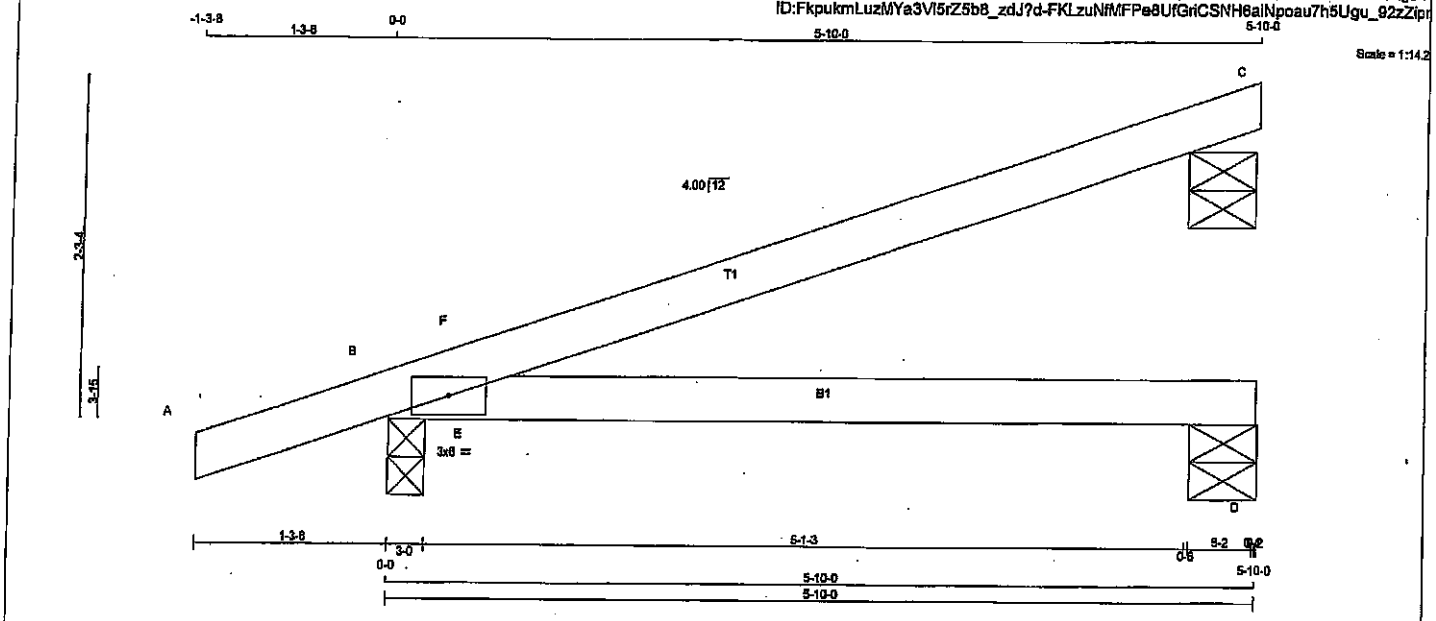

TOTAL WEIGHT = 15 X 15 = 230 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	259	0	259	0	0	5-8	5-8
B	485	0	485	0	0	3-0	3-0
D	90	0	90	0	0	5-8 (5-8)	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	178	143 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0
B	339	247 / 0	0 / 0	0 / 0	0 / 0	92 / 0	0 / 0
D	67	26 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	
B- F	-18 / 24	-102.1	-102.1	0.08 (4)	6.25	
F- C	-2 / 2	-102.1	-102.1	0.45 (1)	10.00	
B- E	0 / 0	-17.5	-17.5	0.32 (1)	10.00	
E- D	0 / 0	-17.5	-17.5	0.32 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-373 / 6	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 902 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 456 (0.15")

CSI: TC=0.45/1.00 (C-F:1) , BC=0.32/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.30/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00 .

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100069020
PROVINCE OF ONTARIO

DWG NO. TAM T1905407
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	J3	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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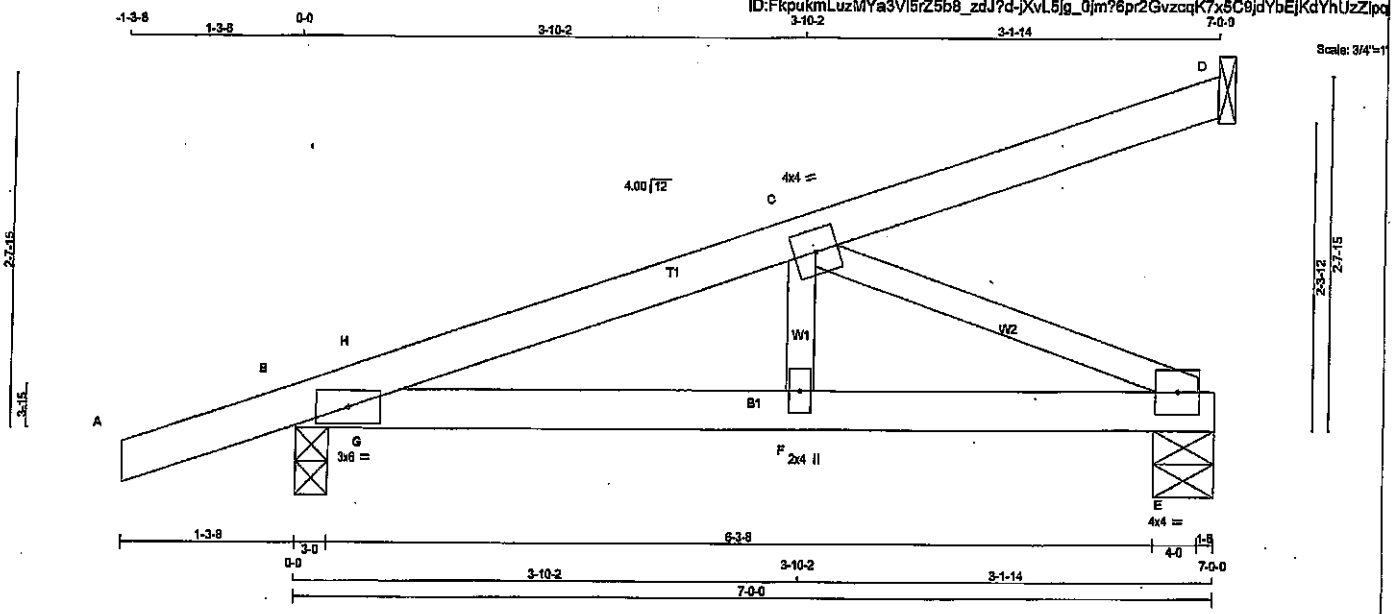

TOTAL WEIGHT = 5 X 21 = 107 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	6.0		
C	TMWW-I	MT20	4.0	4.0		
E	BMW1-I	MT20	4.0	4.0		
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	124	0	124	0	0	1-8	1-8
B	543	0	543	0	0	3-0	3-0
E	302	0	302	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	85	70 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
B	379	275 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0
E	214	138 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00
B-H	-675 / 0	-102.1	-102.1	0.02 (4)	6.25
H-C	-645 / 0	-102.1	-102.1	0.13 (1)	6.25
C-D	-12 / 0	-102.1	-102.1	0.12 (1)	6.25
B-G	0 / 624	-17.5	-17.5	0.19 (1)	10.00
G-F	0 / 624	-17.5	-17.5	0.19 (1)	10.00
F-E	0 / 624	-17.5	-17.5	0.13 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 88	0.03 (4)
C-E	-685 / 0	0.15 (1)
G-H	-106 / 9	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.13/1.00 (C-H:1) , BC=0.19/1.00 (F-G:1) , WB=0.15/1.00 (C-E:1) , SSI=0.15/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (LB) MAX	MIN	WITHDRAWAL (PSI) MAX	MIN	SIDE (LB) MAX	MIN
MT20	618	354	867	788	1987	1656		

PLATE PLACEMENT TOL. = 0.25 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.42 (C) (INPUT = 0.90)
CSI METAL= 0.24 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100000924
PROVINCE OF ONTARIO

RECEIVED
AUG 02 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 71905408
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	J4	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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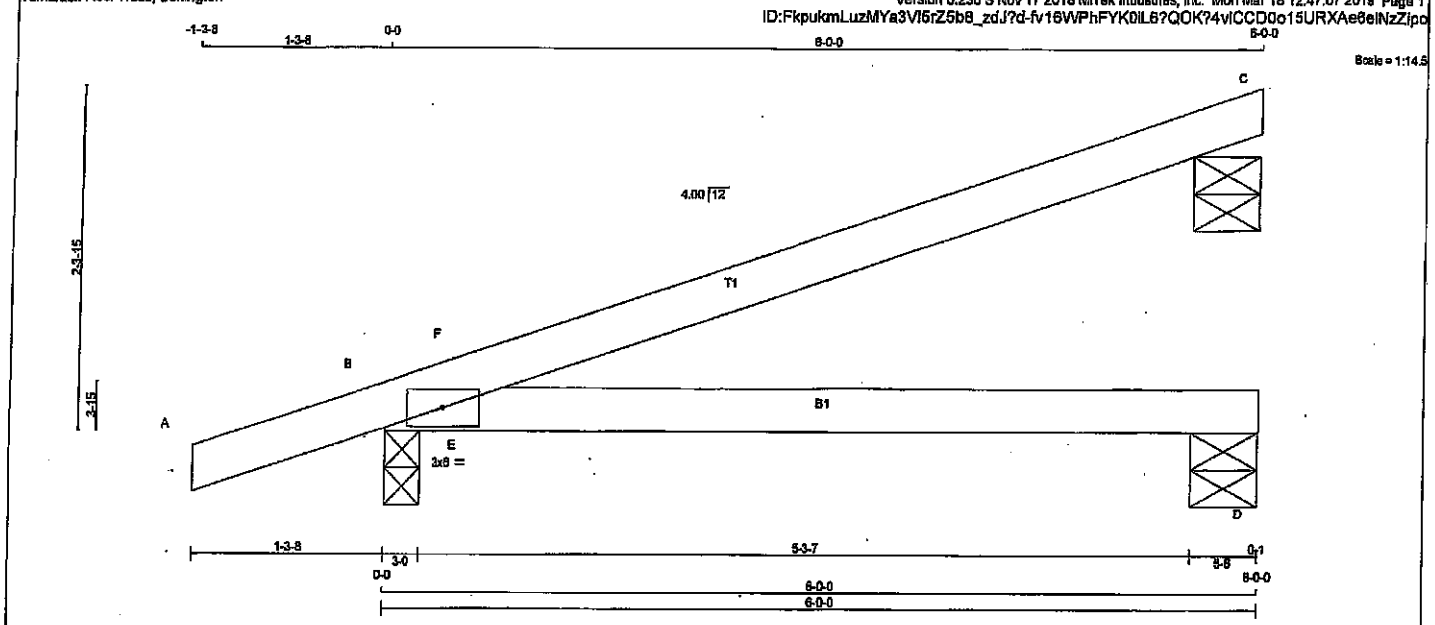

TOTAL WEIGHT = 6 X 16 = 94 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	266	0	266	0	0	5-8	5-8
B	495	0	495	0	0	3-0	3-0
D	92	0	92	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	184	147 / 0	0 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	346	252 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
D	69	27 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	
B- F	-19 / 28	-102.1	-102.1	0.07 (1)	6.25	
F- C	-3 / 2	-102.1	-102.1	0.47 (1)	10.00	
B- E	0 / 0	-17.5	-17.5	0.33 (1)	10.00	
E- D	0 / 0	-17.5	-17.5	0.33 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-393 / 8	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 832 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 429 (0.17")

CSI: TC=0.47/1.00 (C-F:1) , BC=0.33/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.32/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/18/19
H. G. ALVES
100100024
PROVINCE OF ONTARIO

DWG NO. TAM 1905409
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	J5	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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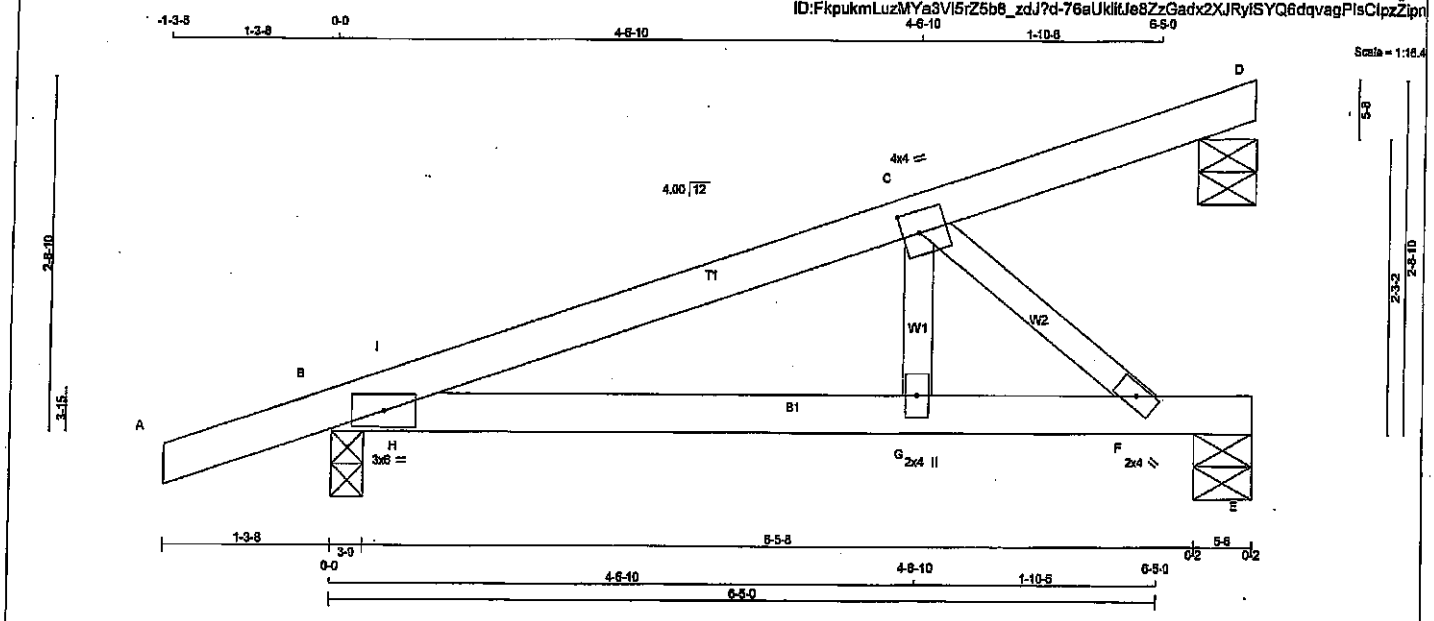


TOTAL WEIGHT = 8 X 21 = 170 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-J	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	BMW+w	MT20	2.0	4.0		
G	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	565	0	565	0	0	3-0	3-0
E	333	0	333	0	0	5-8	5-8
D	96	0	96	0	0	5-8 (5-6)	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	395	288 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
E	235	154 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
D	66	54 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

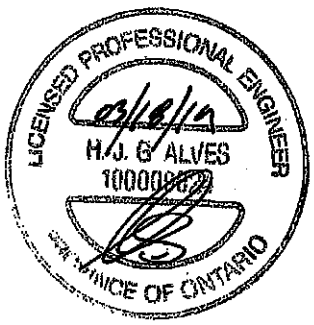
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	
B- I	-622 / 0	-102.1	-102.1	0.03 (1)	6.25	
I- C	-619 / 0	-102.1	-102.1	0.18 (1)	6.25	
C- D	-12 / 0	-102.1	-102.1	0.11 (1)	6.25	
B- H	0 / 593	-17.5	-17.5	0.26 (1)	10.00	
H- G	0 / 593	-17.5	-17.5	0.26 (1)	10.00	
G- F	0 / 597	-17.5	-17.5	0.50 (1)	10.00	
F- E	0 / 0	-17.5	-17.5	0.40 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- F	-801 / 0	0.14 (1)
G- C	0 / 339	0.08 (1)
H- I	-218 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (C-I:1) , BC=0.50/1.00 (F-G:1) , WB=0.14/1.00 (C-F:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (F) (INPUT = 0.90)
JSI METAL= 0.42 (F) (INPUT = 1.00)

DWG NO. TAM 7/905410
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401769	J6	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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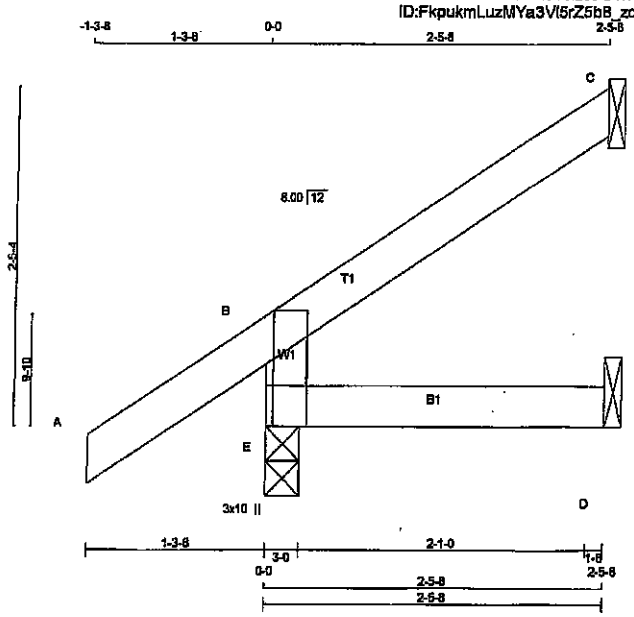

TOTAL WEIGHT = 4 X 9 = 34 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
E						
E	TMBMV1+p	MT20	3.0	10.0	Edge	0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	322	0	322	0	0	3-0	3-0
C	94	0	94	0	0	1-8	1-8
D	18	0	20	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	224	169 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	85	54 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	14	0 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-297 / 0	0.0	0.0		0.02 (4)	7.81
A- B	0 / 39	-102.1	-102.1		0.14 (1)	10.00
B- C	-17 / 0	-102.1	-102.1		0.10 (1)	6.25
E- D	0 / 0	-17.5	-17.5		0.02 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/18/19
H. J. G ALVES
100009084
PROVINCE OF ONTARIO

DWG NO. TAM 7905411
STRUCTURAL
CONSIGNMENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401769	J7	9	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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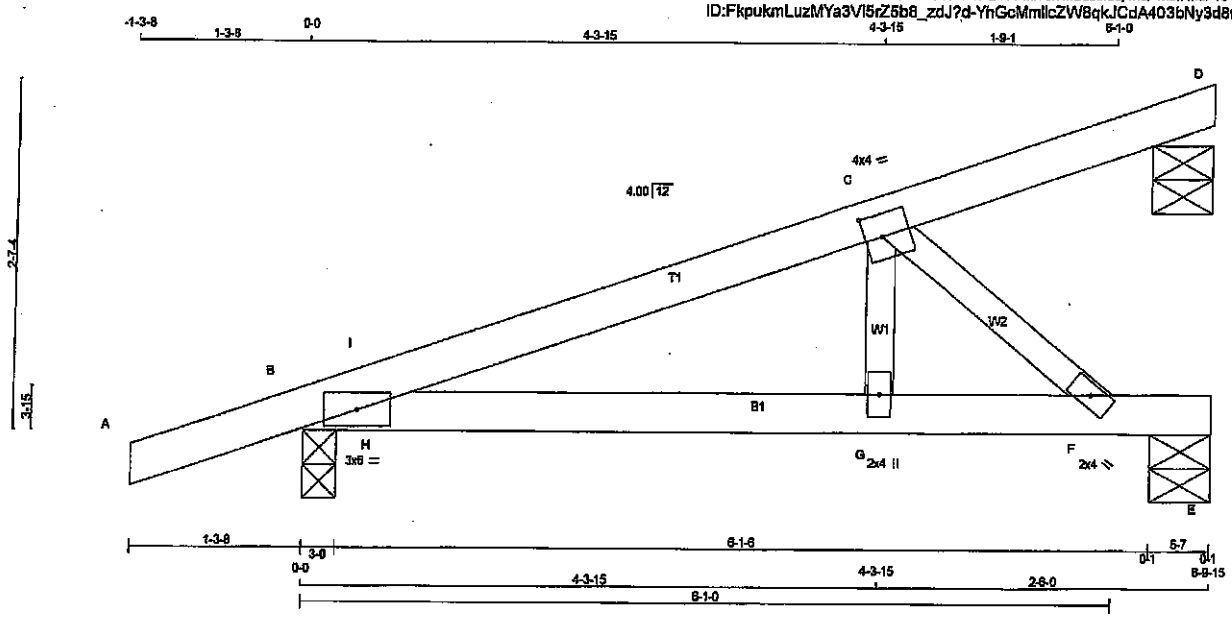

TOTAL WEIGHT = 9 X 20 = 183 lb
(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	BMW+w	MT20	2.0	4.0		
G	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	545	0	545	0	0	3-0	3-0
E	314	0	314	0	0	5-8	5-8
D	94	0	94	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	381	276 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
E	222	145 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
D	65	53 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	
B-I	-592 / 0	-102.1	-102.1	0.02 (1)	6.25	
I-C	-587 / 0	-102.1	-102.1	0.16 (1)	6.25	
C-D	-11 / 0	-102.1	-102.1	0.09 (1)	6.25	
B-H	0 / 563	-17.5	-17.5	0.24 (1)	10.00	
H-G	0 / 563	-17.5	-17.5	0.24 (1)	10.00	
G-F	0 / 566	-17.5	-17.5	0.47 (1)	10.00	
F-E	0 / 0	-17.5	-17.5	0.38 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-F	-767 / 0	0.13 (1)
G-C	0 / 333	0.07 (1)
H-I	-202 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.16/1.00 (C-I:1) , BC=0.47/1.00 (F-G:1) , WB=0.13/1.00 (C-F:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 1.00)

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100008024
PROVINCE OF ONTARIO

DWG NO. TAM T905412
STRUCTURAL
COMPONENT ONLY

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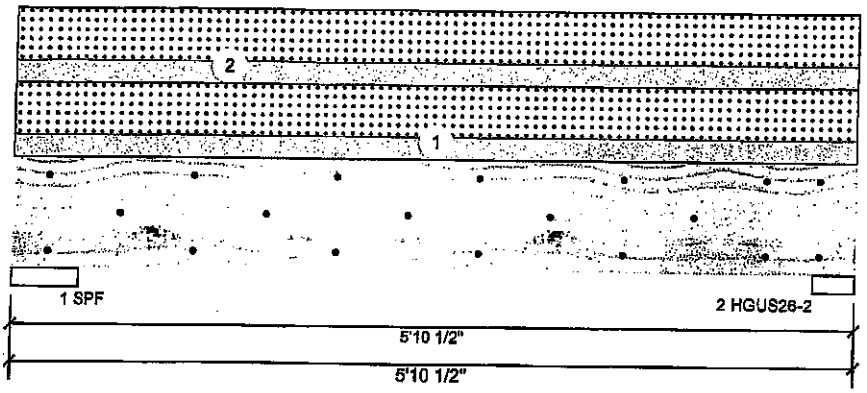
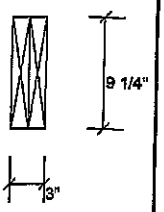
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	329	759	0
2	0	311	717	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	15%	411 / 1138	1550	L	1.25D+1.5S
2 - HGUS...	3.500"	23%	388 / 1076	1464	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1767 ft-lb	3' 1/4"	6039 ft-lb	0.293 (29%)	1.25D+1.5S	L
Unbraced	1767 ft-lb	3' 1/4"	5223 ft-lb	0.338 (34%)	1.25D+1.5S	L
Shear	1347 lb	1'2"	3984 lb	0.338 (34%)	1.25D+1.5S	L
Perm Defl in.	0.007 (L/9227)	3' 1/4"	0.175 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.016 (L/3998)	3' 1/4"	0.175 (L/360)	0.090 (9%)	S	L
TL Defl inch	0.023 (L/2789)	3' 1/4"	0.175 (L/360)	0.130 (13%)	D+S	L

Design Notes

1. Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2. Refer to last page of calculations for fasteners required for specified loads.
3. Girders are designed to be supported on the bottom edge only.
4. Top braced at bearings.
5. Bottom braced at bearings.
6. Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009021
PROVINCE OF ONTARIO

DWG NO. TAM 7190541 3
STRUCTURAL
COMPONENT ONLY
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13 PSF	0 PSF	30 PSF	0 PSF	
2	Uniform		7-4-8	Near Face	13 PSF	0 PSF	30 PSF	0 PSF	

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BUILDING SECTION
FILE NO

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

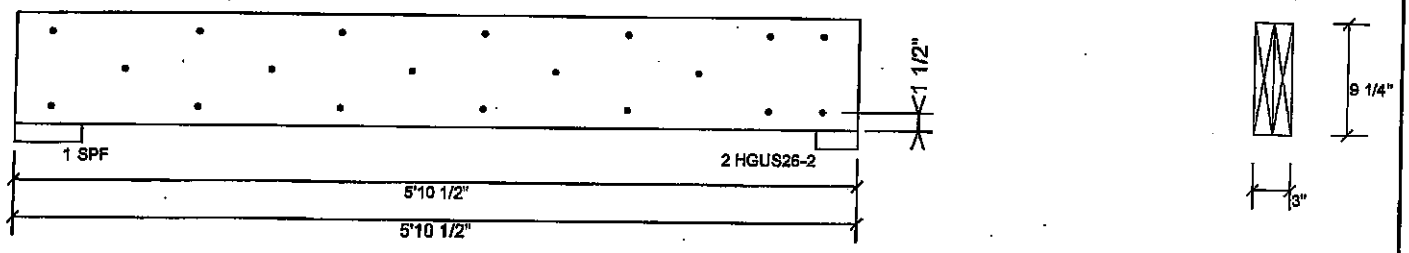
isDesign™

Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	66.4 %
Load	225.9 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

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H. J. G. ALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM 7905413
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

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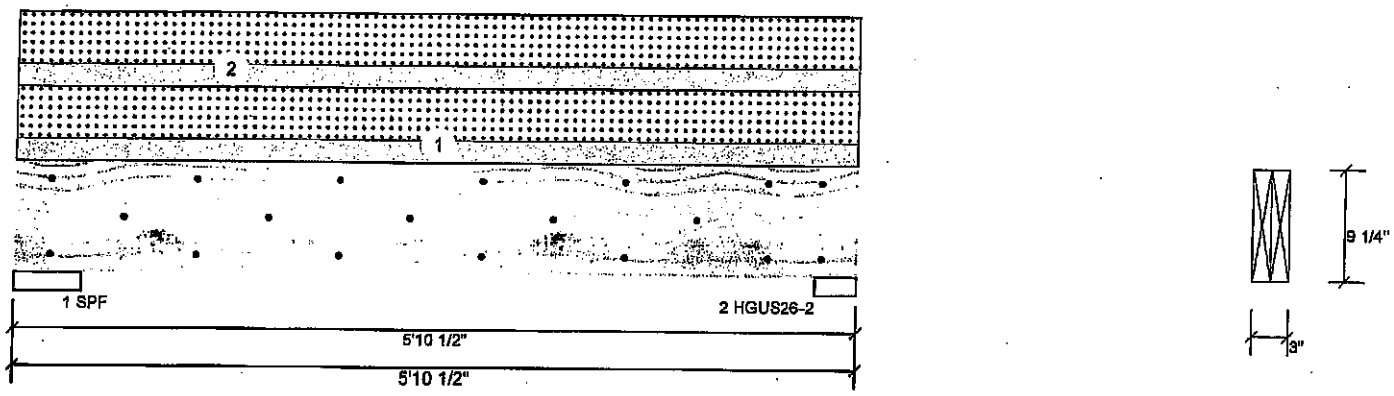
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	319	736	0
2	0	301	696	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	15%	399 / 1104	1503	L	1.25D+1.5S
2 - HGUS...	3.500"	22%	377 / 1044	1420	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1715 ft-lb	3' 1/4"	6039 ft-lb	0.284 (28%)	1.25D+1.5S	L
Unbraced	1715 ft-lb	3' 1/4"	5223 ft-lb	0.328 (33%)	1.25D+1.5S	L
Shear	1306 lb	1'2"	3984 lb	0.328 (33%)	1.25D+1.5S	L
Perm Defl in.	0.007 (L/9511)	3' 1/4"	0.175 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.015 (L/4121)	3' 1/4"	0.175 (L/360)	0.090 (9%)	S	L
TL Defl inch	0.022 (L/2875)	3' 1/4"	0.175 (L/360)	0.130 (13%)	D+S	L

Design Notes

1. Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2. Refer to last page of calculations for fasteners required for specified loads.
3. Girders are designed to be supported on the bottom edge only.
4. Top braced at bearings.
5. Bottom braced at bearings.
6. Lateral slenderness ratio based on single ply width.

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H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T190 5414
STRUCTURAL
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1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13 PSF	0 PSF	30 PSF	0 PSF	
2	Uniform		7-1-8	Near Face	13 PSF	0 PSF	30 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

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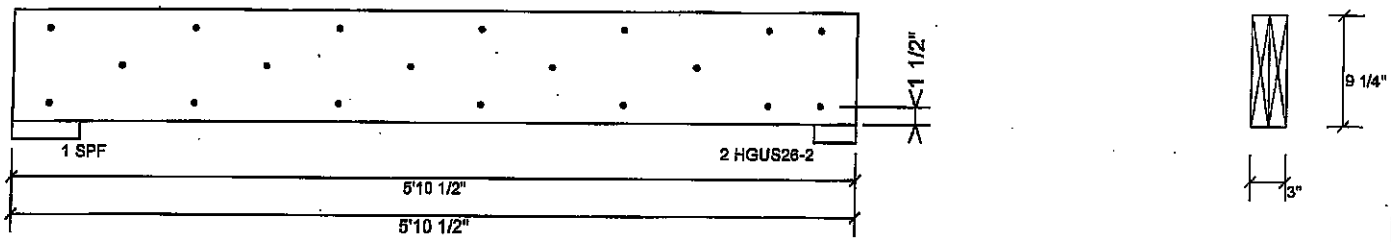
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	64.2 %
Load	218.2 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

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03/18/19
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

DWG NO. TAM T19054 14
STRUCTURAL
CONSTRUCT COMV 3/2

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

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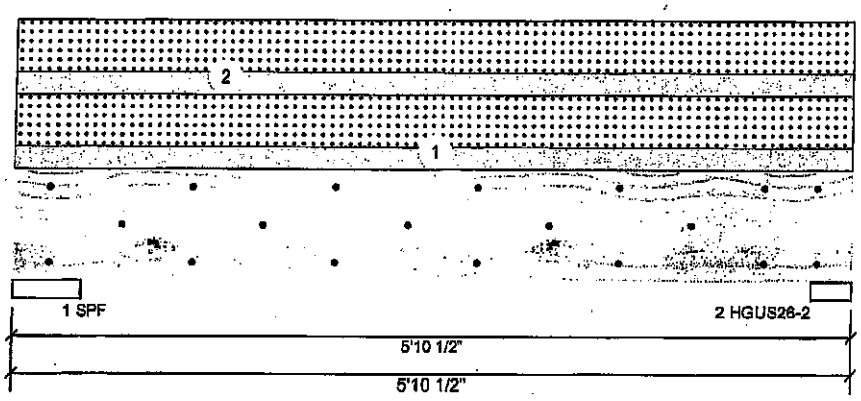
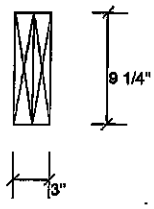
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	358	827	0
2	0	339	781	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	17%	448 / 1240	1688	L	1.25D+1.5S
2 - HGUS...	3.500"	25%	423 / 1172	1595	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1926 ft-lb	3' 1/4"	6039 ft-lb	0.319 (32%)	1.25D+1.5S	L
Unbraced	1926 ft-lb	3' 1/4"	5223 ft-lb	0.369 (37%)	1.25D+1.5S	L
Shear	1467 lb	1'2"	3984 lb	0.368 (37%)	1.25D+1.5S	L
Perm Defl in.	0.007 (L/8468)	3' 1/4"	0.175 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.017 (L/3670)	3' 1/4"	0.175 (L/360)	0.100 (10%)	S	L
TL Defl inch	0.025 (L/2560)	3' 1/4"	0.175 (L/360)	0.140 (14%)	D+S	L

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009
PROVINCE OF ONTARIO

DWG NO. TAM T190 54 15
STRUCTURAL
COMPONENT ONLY
1/2

Design Notes

1. Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2. Refer to last page of calculations for fasteners required for specified loads.
3. Girders are designed to be supported on the bottom edge only.
4. Top braced at bearings.
5. Bottom braced at bearings.
6. Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13 PSF	0 PSF	30 PSF	0 PSF	
2	Uniform		8-1-8	Near Face	13 PSF	0 PSF	30 PSF	0 PSF	

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Manufacturer Info

Tamarack Roof Trusses
13255 North Service Road, ON
L7C 0J2
905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

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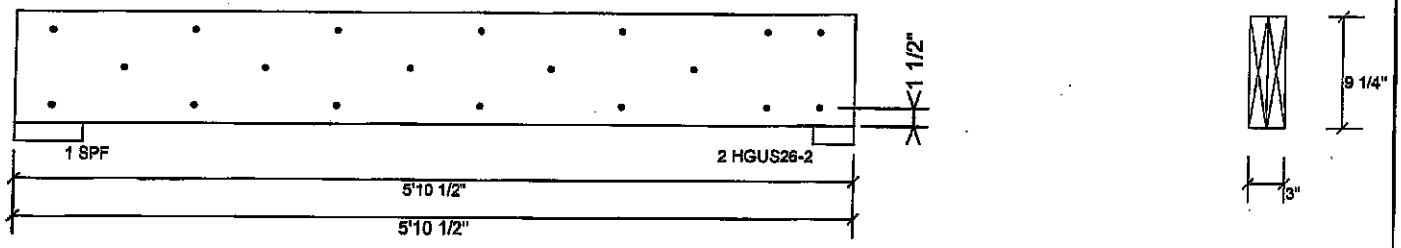
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	73.2 %
Load	248.8 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

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03/19/19
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100008024
PROVINCE OF ONTARIO

DWG NO. TAM T905415
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-336-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

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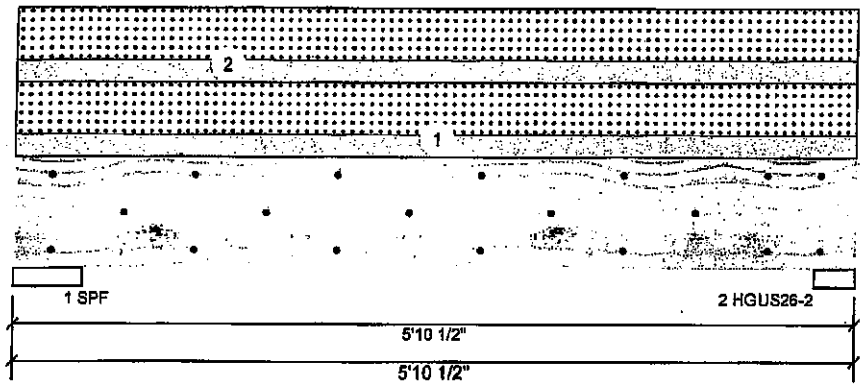
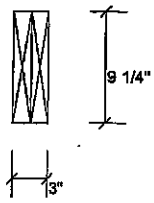
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	329	759	0
2	0	311	717	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	15%	411 / 1138	1550	L	1.25D+1.5S
2 - HGUS...	3.500"	23%	388 / 1076	1464	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1767 ft-lb	3' 1/4"	6039 ft-lb	0.293 (29%)	1.25D+1.5S	L
Unbraced	1767 ft-lb	3' 1/4"	5223 ft-lb	0.338 (34%)	1.25D+1.5S	L
Shear	1347 lb	1'2"	3984 lb	0.338 (34%)	1.25D+1.5S	L
Perm Defl in.	0.007 (L/9227)	3' 1/4"	0.175 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.016 (L/3998)	3' 1/4"	0.175 (L/360)	0.090 (9%)	S	L
TL Defl inch	0.023 (L/2789)	3' 1/4"	0.175 (L/360)	0.130 (13%)	D+S	L

Design Notes

1. Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2. Refer to last page of calculations for fasteners required for specified loads.
3. Girders are designed to be supported on the bottom edge only.
4. Top braced at bearings.
5. Bottom braced at bearings.
6. Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T9405416
STRUCTURAL
COMPONENT ONLY
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13 PSF	0 PSF	30 PSF	0 PSF	
2	Uniform		7-4-8	Near Face	13 PSF	0 PSF	30 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

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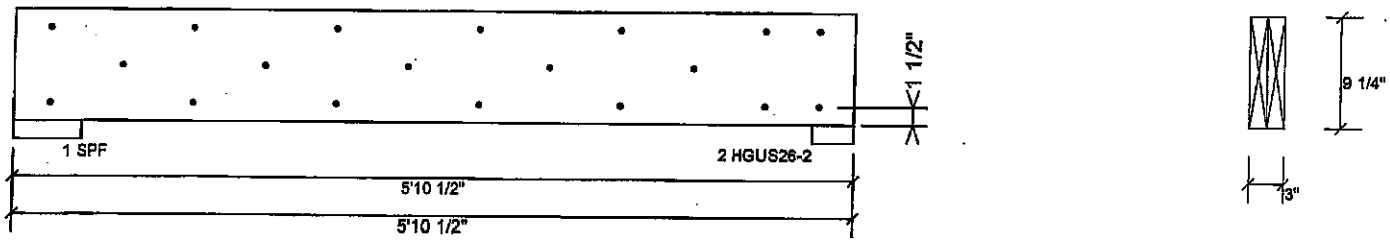
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401789
Project #:

BM4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	68.4 %
Load	225.9 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
03/18/19
H.J. G. ALVES
100008020
PROVINCE OF ONTARIO

RECEIVED
BMS NO. TAM 17-005-016
STRUCTURAL
AUGMENT ONLY
TOWN OF CALEDON
BUILDING SECTION

Manufacturer Info	
	Tamarack Roof Trusses 13335 North Service Road, ON L7N 3G2 905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

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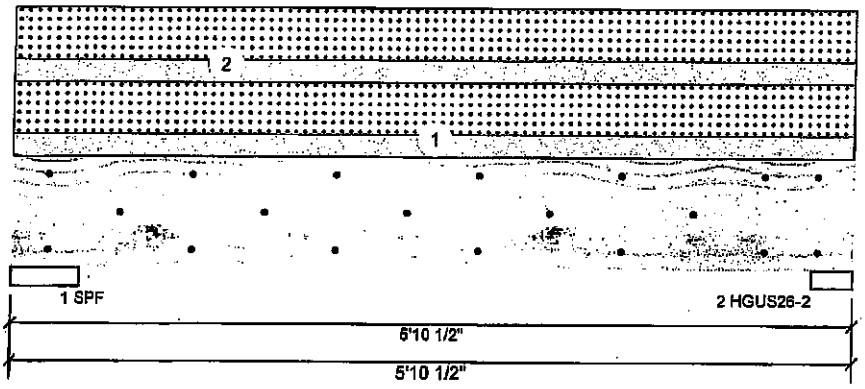
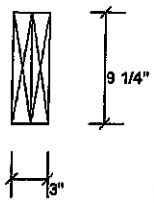
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	339	782	0
2	0	320	739	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	16%	423 / 1172	1696	L	1.25D+1.5S
2 - HGUS...	3.500"	23%	400 / 1108	1508	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1820 ft-lb	3' 1/4"	6039 ft-lb	0.301 (30%)	1.25D+1.5S	L
Unbraced	1820 ft-lb	3' 1/4"	5223 ft-lb	0.348 (35%)	1.25D+1.5S	L
Shear	1387 lb	1'2"	3984 lb	0.348 (35%)	1.25D+1.5S	L
Perm Defl in.	0.007 (L/8959)	3' 1/4"	0.175 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.016 (L/3882)	3' 1/4"	0.175 (L/360)	0.090 (9%)	S	L
TL Defl inch	0.023 (L/2709)	3' 1/4"	0.175 (L/360)	0.130 (13%)	D+S	L

Design Notes

1. Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2. Refer to last page of calculations for fasteners required for specified loads.
3. Girders are designed to be supported on the bottom edge only.
4. Top braced at bearings.
5. Bottom braced at bearings.
6. Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM P905417
STRUCTURAL
COMPONENT ONLY
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13 PSF	0 PSF	30 PSF	0 PSF	
2	Uniform		7-7-8	Near Face	13 PSF	0 PSF	30 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G[illegible]
905-335-1115

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MAR 22 2019
TAMARACK LUMBER INC
ALPA LUMBER INC
CALEDON
BUILDING SECTION
FILE NO

This design is valid until 12/11/2021

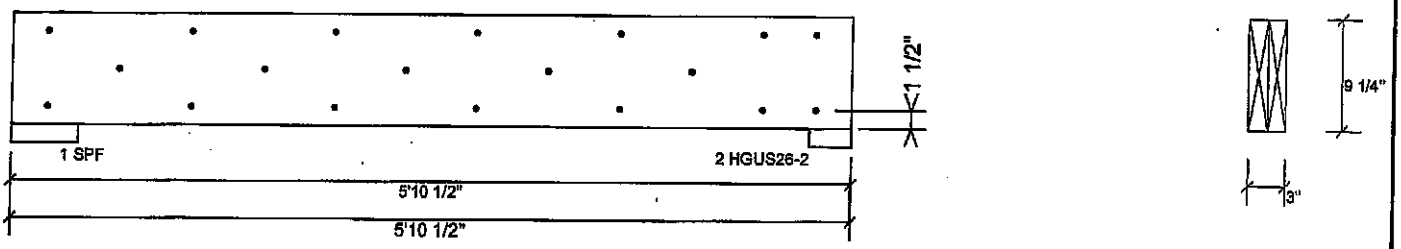
isDesign™

Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	68.7 %
Load	233.5 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

LICENSED PROFESSIONAL ENGINEER
03/6/17
H. A. G. ALVES
100086074
PROVINCE OF ONTARIO

DWG NO. TAM 79054/7
STRUCTURAL
COMMENTARY ONLY
2/2

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

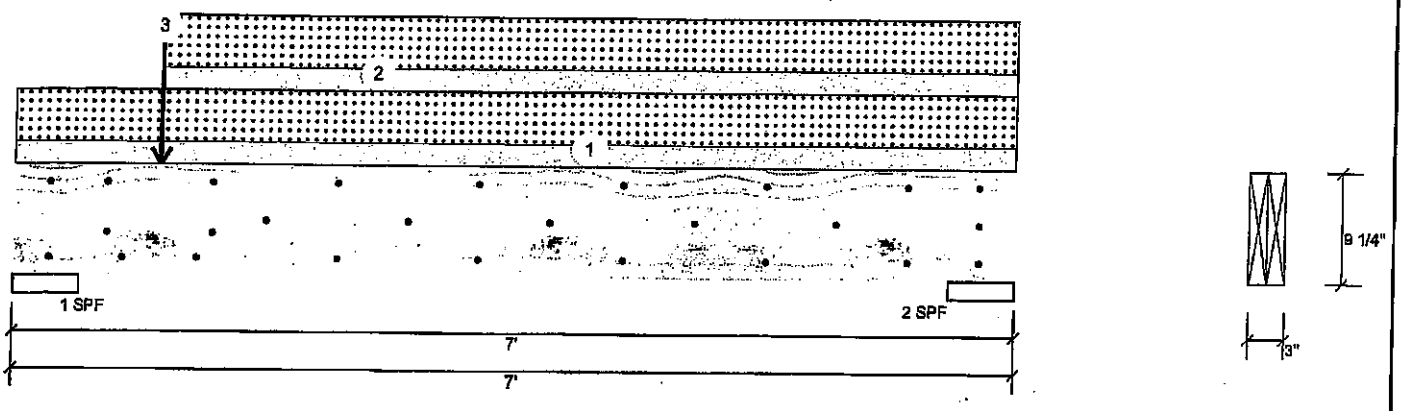
isDesign™

Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM6 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	287	1083	0
2	0	228	573	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	20%	358 / 1624	1982	L	1.25D+1.5S
2 - SPF	5.500"	11%	285 / 860	1144	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1821 ft-lb	3' 13/16"	6039 ft-lb	0.301 (30%)	1.25D+1.5S	L
Unbraced	1821 ft-lb	3' 13/16"	4897 ft-lb	0.372 (37%)	1.25D+1.5S	L
Shear	2102 lb	1'2"	3984 lb	0.528 (53%)	1.25D+1.5S	L
Perm Defl in.	0.009 (L/8588)	3'5 3/6"	0.207 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.024 (L/3045)	3'4 1/2"	0.207 (L/360)	0.120 (12%)	S	L
TL Defl inch	0.033 (L/2248)	3'4 3/4"	0.207 (L/360)	0.160 (16%)	D+S	L

Design Notes

1. Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2. Refer to last page of calculations for fasteners required for specified loads.
3. Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
4. Girders are designed to be supported on the bottom edge only.
5. Top braced at bearings.
6. Bottom braced at bearings.
7. Lateral slenderness ratio based on single ply width.

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03/18/19
H. J. G. ALVES
100088024
PROVINCE OF ONTARIO

DWG NO. TAM T190541B
STRUCTURAL
COMPONENT ONLY 42

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13 PSF	0 PSF	30 PSF	0 PSF	
2	Part. Uniform	1-0-4 to 7-0-0	3-9-0	Near Face	13 PSF	0 PSF	30 PSF	0 PSF	
3	Point	1-0-4		Near Face	132 lb	0 lb	773 lb	0 lb	

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

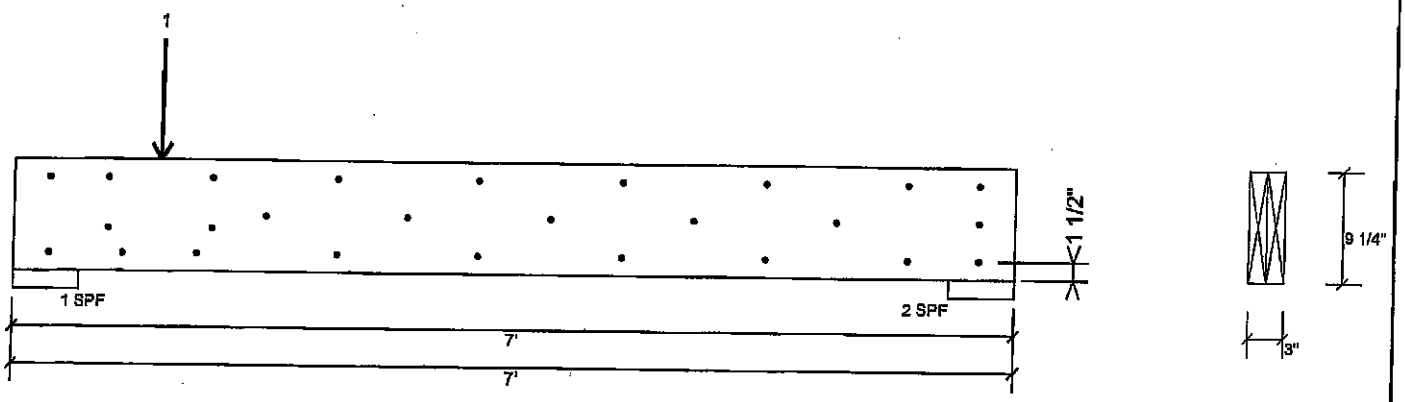
isDesign™

Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 401769
Project #:

BM6 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

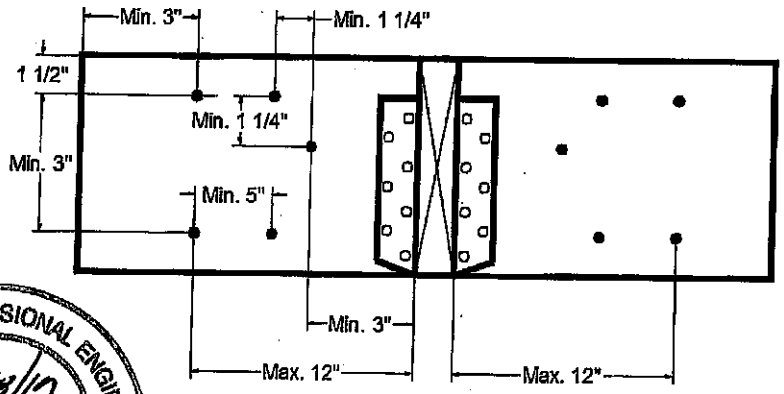
Capacity	33.8 %
Load	114.8 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 1-0-4 with a minimum of (6) – Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	97.4 %
Load	662.3lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. GALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM TM054182/2
STRUCTURAL

This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115

TAMARACK LUMBER INC
ALTA LUMBER GROUP

Version 18.80.245 Powered by iStruct™

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie®

Standard and Double-Shear Joist Hangers

VALUE ENGINEERED

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

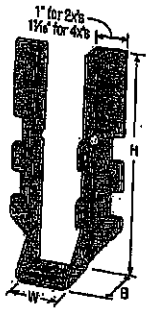
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

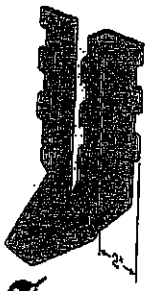
Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.

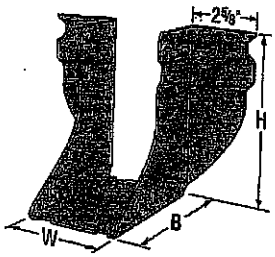


LUS28

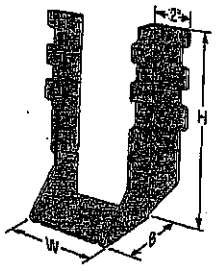
LU26L



HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2

Double-Shear Nailing Top View

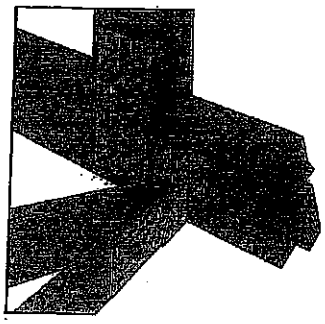
Double-Shear Nailing Side View; Do not bend tab

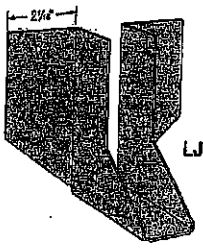
Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

Plated Truss Connectors

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LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie

LICENSED PROFESSIONAL ENGINEER 1/15/18 B. D. BUNTING PROVINCE OF ONTARIO

HHUS/HGUS

See Hanger Options information on pp. 125–127.

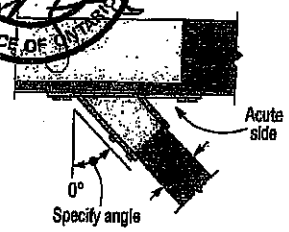
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p.24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[2]	Fasteners Header	Joist	Factored Resistance Uplift (Kd = 1.15) lb. / kN	Normal (Kd = 1.00) lb. / kN	SJ-F Uplift (Kd = 1.15) lb. / kN	Normal (Kd = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1 9/16	3 1/8	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								3.16	7.23	2.87	5.14
LUJ24L	22	1 9/16	3	1 5/8	2 11/16	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								1.60	4.54	1.42	3.22
LUJ26L	22	1 9/16	5	1 5/8	4 5/8	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1 9/16	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.65	5.74	7.25
HUS26	16	1 5/8	5 5/8	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
								12.03	21.97	9.20	17.24
LUS26DS	18	1 9/16	5	3 1/2	4 5/8	(18) 16d	(6) 16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1 5/8	5 5/8	5	4 1/8	(26) 16d	(8) 16d	2685	6625	2685	5700
								11.95	29.47	11.95	25.35
LU28L	20	1 9/16	6 3/4	1 5/8	5 7/8	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1 5/8	7 1/16	3	6 1/16	(22) 16d	(8) 16d	3605	5365	2675	4345
								16.03	23.86	11.90	19.33
HGUS28	12	1 5/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3310	6900
								14.72	34.14	14.74	30.73
LU210L	20	1 9/16	8	1 5/8	7 5/8	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1 9/16	7 13/16	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

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SIMPSON Strong-Tie®

Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_B[2]	Fasteners Header	Joist	Factored Resistance C-P-F Uplift (K_D = 1.15) lb. kN	C-P-F Normal (K_D = 1.00) lb. kN	S-P-F Uplift (K_D = 1.15) lb. kN	S-P-F Normal (K_D = 1.00) lb. kN
Double 2x Sizes											
LUS24-2	18	3⅛	6⅛	2	1½	(4) 16d	(2) 16d	835	2020	590	1495
								3.71	8.99	2.62	6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3⅝	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3⅝	7⅝	3	6⅛	(22) 16d	(8) 16d	3785	8940	2675	6345
								16.84	39.77	11.90	28.22
HGUS28-2	12	3⅝	7 7/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3⅝	9 9/16	3	8	(30) 16d	(10) 16d	4670	9860	4235	7000
								20.77	43.87	18.84	31.14
HGUS210-2	12	3⅝	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 15/16	5⅛	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 15/16	7¼	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 15/16	9	3	7 15/16	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.01	18.84	30.54
HGUS210-3	12	4 15/16	9¼	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.13	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 9/16	7 7/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6⅞	9⅜	3	7 15/16	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6 9/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.13	21.60	46.26
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3 9/16	4¼	2	3⅞	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3⅝	5¼	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3 9/16	6¾	2	3⅞	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3⅝	7 1/16	4	6 1/16	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3 9/16	8¾	2	5 9/16	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3⅝	9	4	8 1/16	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3⅝	10 7/16	4	10 1/16	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3⅝	12 5/16	4	11 7/16	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80

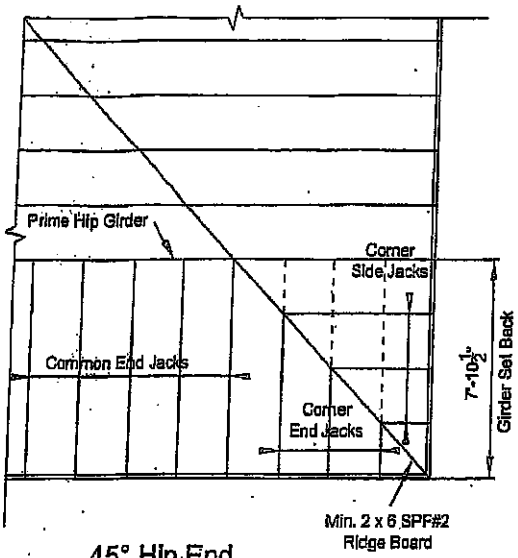
See footnotes p. 258.

Plated Truss Connectors

LICENSED PROFESSIONAL ENGINEER 1/15/18 B. D. BUNTING PROVINCE OF ONTARIO

RECEIVED AUG 02 2019 TOWN OF CALEDON BUILDING SECTION FILE NO

C-C-CAN2018 ©2017 SIMPSON STRONG-TIE COMPANY INC.



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

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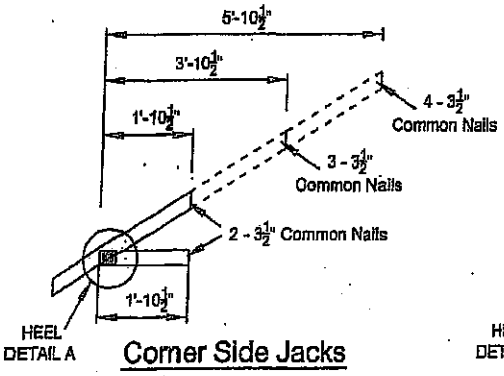
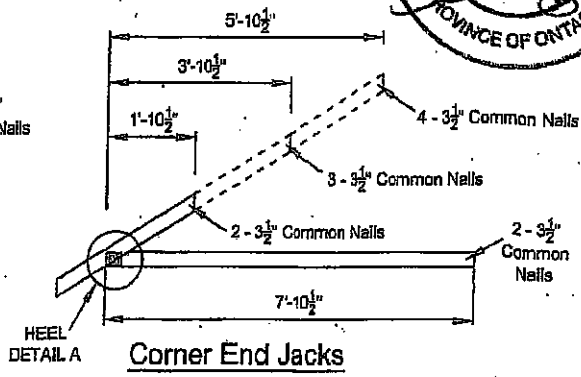
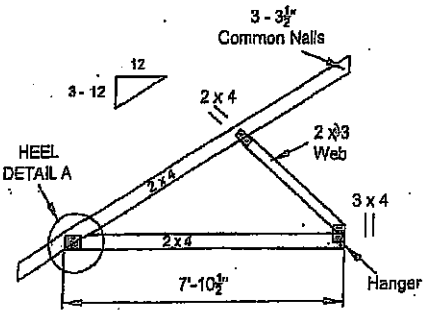
26/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



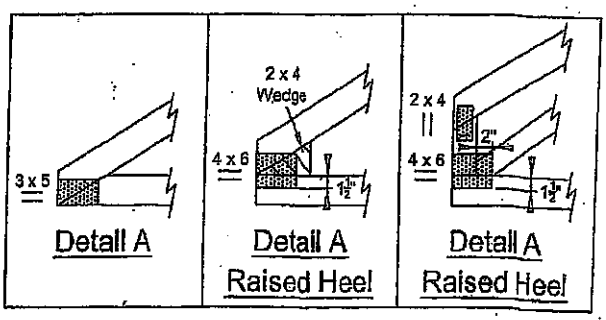
Corner Side Jacks



Corner End Jacks



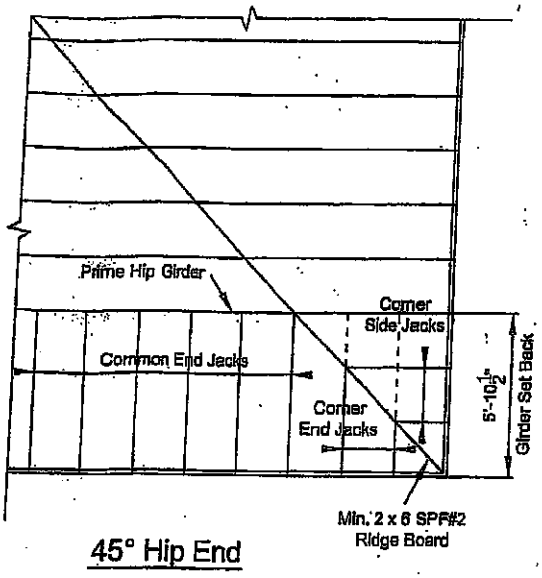
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE



45° Hip End

LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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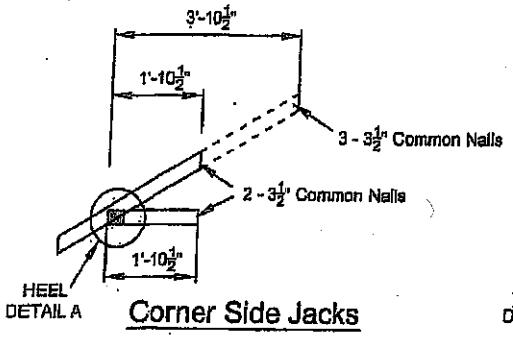
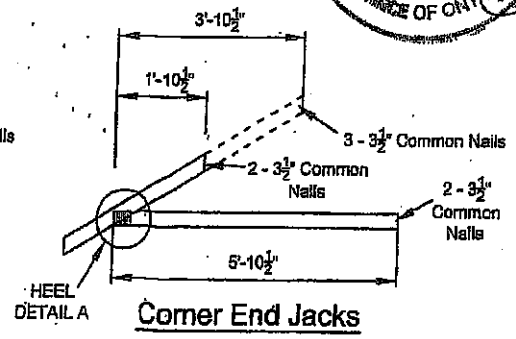
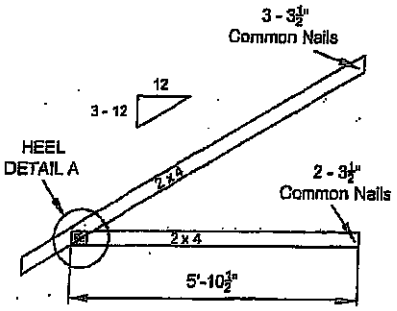
06/11/18
H.J.G. ALVES
PROVINCE OF ONTARIO



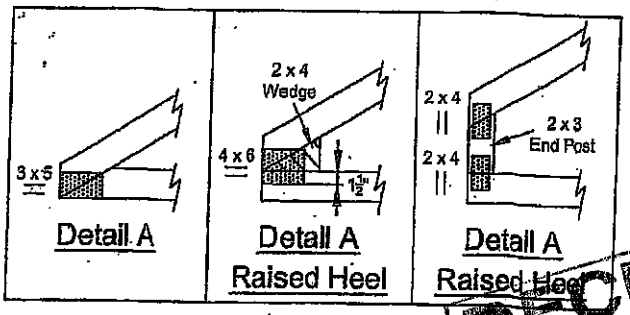
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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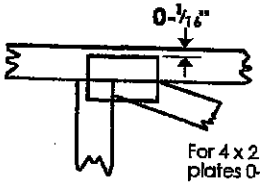
BUILDING SECTION
FILE NO

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

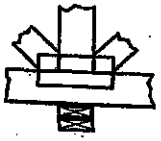
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

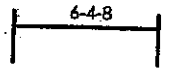


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

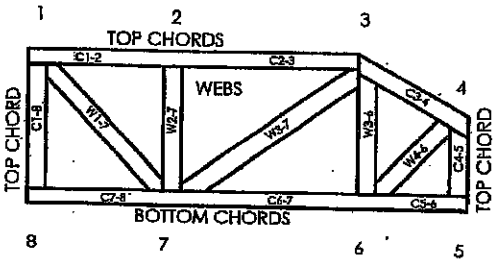
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

LICENSED PROFESSIONAL ENGINEER
06/11/18
H.J.G. ALVES
PROVINCE OF ONTARIO

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218 Feb 09, 2018