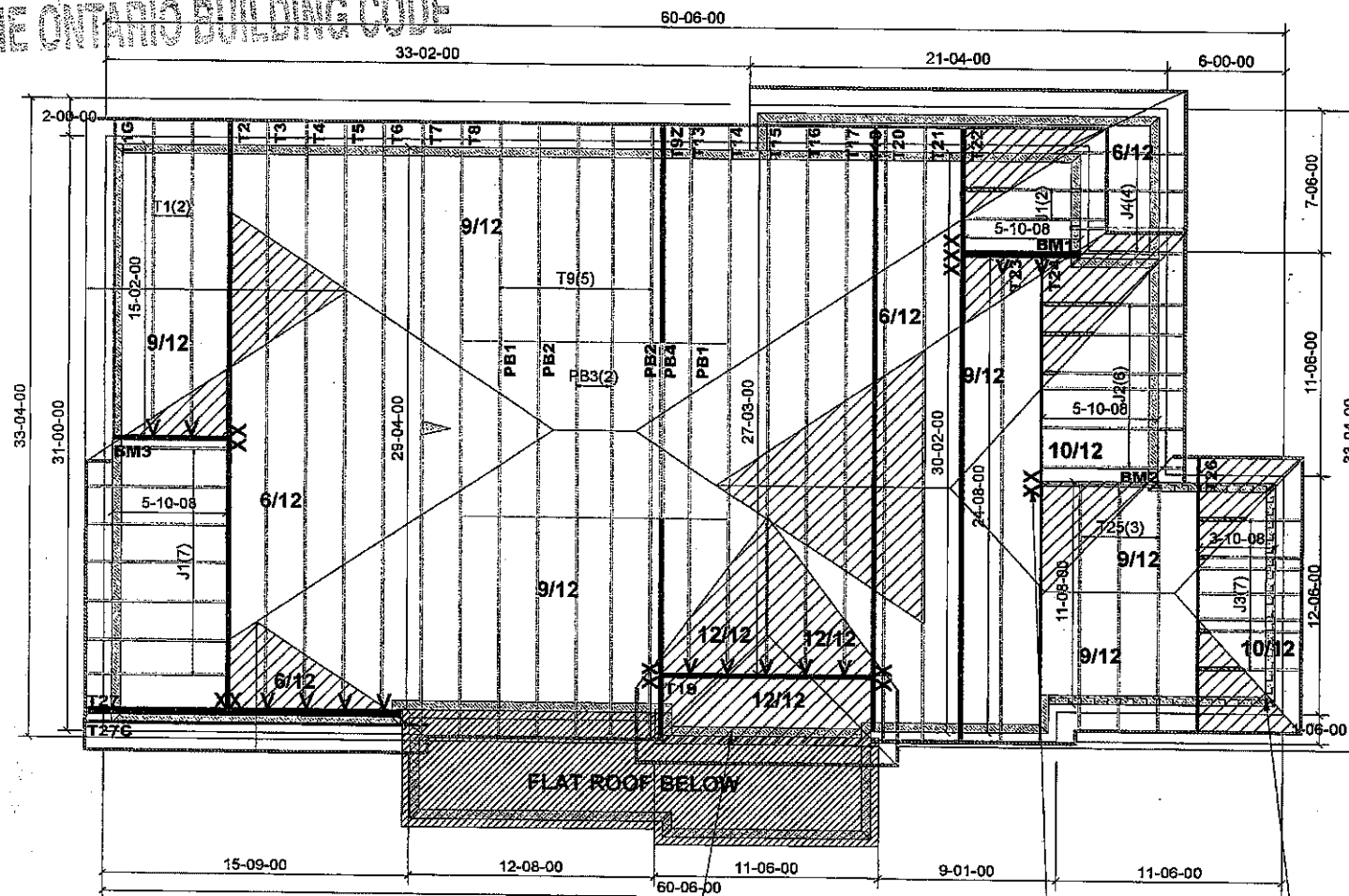
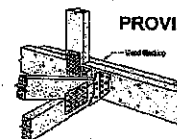


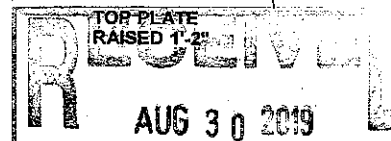
THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



TOP PLATE
RAISED 2"



PROVIDE 2" BLOCK



TOWN OF CALEDON
BUILDING SECTION

FILE NO
Barton 12 / Elv 1 Lot 79

TRUSS PROFILES TO BE VERIFIED BY
BUILDING DESIGNER

ALL CONVENTIONAL ROOF FRAMING TO
CONFORM WITH PART 9 OF THE OBC.
ROOF RAFTERS THAT MEET OR CROSS OVER
TRUSSES ARE TO BE 2"x4"SPF@24"O.C.
WITH 2"x4"SPF VERTICAL POST TO THE
TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally
BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS
OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED CONFORM WITH:
ONTARIO BUILDING CODE (2012)
OCCUPANCY: RESIDENTIAL | PART: 9

DESIGN LOADS:

CITY: CALEDON
G.S.L = 37.6 psf
TC DL = 6 psf
BC LL = 10.50 psf
BC DL = 7.00 psf

NOTES:

FIN. OH.: 12"
HEEL TYPE: R.T.M. CANT.
EXT. WALLS: 2X6
CLAD. TYPE 1: BRICK/5"
CLAD. TYPE 2: SIDING/0"
FSC SIZE: 2X6
SHEATHING: ASPHALT SHINGLE

IF DESIGNED COMMERCIAL, REFERENCE TO
SEALED TRUSS DOCS FOR UPLIFT DESIGN

HARDWARE:

HGUS26-2 (XX) 5pcs
HGUS26-2 (XXX) 1pcs
LSJ26DS (V) 13pcs

CONV FRM BY OTHERS

T- 180777

COMMENTS:

-BM1-3: 2-2X10 SPF #2
-BM2 NEEDS TO BE BUILT UP TO
REACH TOP HEIGHT OF 12"



Job Track: 50120

Plan Log: 200164

Layout ID: 400357

Builder / Location:

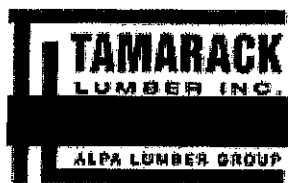
Greenpark / Caledon

Project: Lamberts Lane Home Corp.

Date: 2019-03-18 Designer: Brian

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED,
OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE.

Mitek ver 8.2.3.229



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Barton 12
 Lot #: 079
 Elevation: 1

Job Track: 50120
 PlanLog: 200164
 Layout ID: 400357
 Ref #
 Page: 1 of 3
 Date: 03-18-2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T1 Common	9/12	15-02-00	7-02-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	135.52 89.67		
	1	T1G GABLE	9/12	15-02-00	7-02-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	71.26 47.83		
	1 2-ply	T2 Half Hip Girder	9/12	29-04-00	4-01-04	2 x 4 2 x 6	1-03-08	1-06-04 4-01-04	289.1 166.67		
	1	T3 Hip	9/12	29-04-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	129.71 80.33		
	1	T4 Hip	9/12	29-04-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	127.96 82.00		
	1	T5 Hip	9/12	29-04-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	129.18 83.17		
	1	T6 Hip	9/12	29-04-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	143.12 91.00		
	1	T7 Hip	9/12	29-04-00	9-01-04	2 x 4	1-03-08 1-05-00	1-06-04 1-06-04	142.74 89.83		
	1	T8 Hip	9/12	29-04-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	142.52 90.33		
	5	T9 Piggyback Base	9/12	29-04-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	843.58 535.83		
	1 2-ply	T9Z Piggyback Base Girder	9/12	29-04-00	9-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	337.43 214.33		
	1	T13 Piggyback Base	9/12	27-03-00	9-01-04	2 x 4	1-03-08	1-06-04 3-01-00	137.49 87.00		
	1	T14 Hip	9/12	27-03-00	10-01-04	2 x 4	1-03-08	1-06-04 3-01-00	137.36 86.83		
	1	T15 Hip	9/12	27-03-00	9-01-04	2 x 4	1-03-08	1-06-04 3-01-00	137.49 87.00		



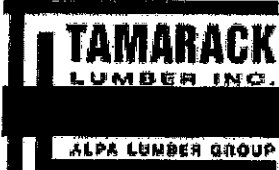
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Barton 12
 Lot #: 079
 Elevation: 1

Job Track: 50120
 PlanLog: 200164
 Layout ID: 400357
 Ref #
 Page: 2 of 3
 Date: 03-18-2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T16 Hip	9/12	27-03-00	8-01-04	2 x 4	1-03-08	1-06-04 3-01-00	137.38 86.17		
	1	T17 Hip	9/12	27-03-00	7-01-04	2 x 4	1-03-08	1-06-04 3-01-00	126.05 79.67		
	1 2-ply	T18 Hip Girder	9/12	30-02-00	6-04-00	2 x 4 2 x 6	1-03-08	10-12 1-06-04	305.37 191.00		
	1 2-ply	T19 Hip Girder	12/12	10-08-00	6-10-08	2 x 4 2 x 6		3-10-08 3-10-08	139.59 92.67		
	1	T20 Hip	9/12	30-02-00	6-01-04	2 x 4	1-03-08 5-08	1-06-04 10-12	129.77 82.50		
	1	T21 Hip	9/12	30-02-00	5-01-04	2 x 4	1-03-08 5-08	1-06-04 10-12	129.23 82.17		
	1 2-ply	T22 Hip Girder	9/12	30-02-00	4-01-08	2 x 4 2 x 6	1-03-08 5-08	1-06-04 10-12	283.03 179.33		
	1	T23 Hip	9/12	24-08-00	8-02-07	2 x 4	5-08	1-06-04 10-12	109.2 70.50		
	1	T24 Hip Girder	9/12	24-08-00	6-06-07	2 x 4 2 x 6	5-08	1-06-04 10-12	136.51 86.33		
	3	T25 Common	9/12	11-08-00	5-08-08	2 x 4	1-03-08 1-03-08	10-04 10-04	151.52 95.50		
	1	T26 Hip Girder	9/12	11-08-00	4-08-03	2 x 4	1-03-08 1-03-08	10-04 10-04	50.51 33.50		
	1 3-ply	T27 Common Girder	6/12	14-11-00	4-10-12	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	212.75 137.50		
	1	T27G GABLE	6/12	14-11-00	4-10-12	2 x 4 2 x 6	1-03-08 1-05-00	1-02-00 1-02-00	66.84 43.50		
	2	PB1 Piggyback	9/12	9-01-05	2-00-00	2 x 4			45.7 30.67		

		DELIVERY SHIPLIST	
 TAMARACK LUMBER INC. ALPA LUMBER GROUP	Lumber Yard:	TAMARACK LUMBER	Job Track: 50120
	Builder:	Greenpark	PlanLog: 200164
	Project:	Lamberts Lane Home Corp.	Layout ID: 400357
	Location:	Caledon	Ref #
	Model:	Barton 12	Page: 3 of 3
	Lot #:	079	Date: 03-18-2019
Elevation:	1	Designer: Brian Faneca	Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB2 Piggyback	9 / 12	9-01-05	3-00-00	2 x 4			50.67 32.67		
	2	PB3 Piggyback	9 / 12	9-01-05	3-05-00	2 x 4			47.94 29.67		
	1 2-ply	PB4 Piggyback	9 / 12	9-01-05	2-08-04	2 x 4			49.69 32.67		
	9	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	6	J2 Jack-Open	10 / 12	5-10-08	6-06-07	2 x 4	1-03-08	1-07-11 6-06-07	157.52 102.00		
	7	J3 Jack-Open	10 / 12	3-10-08	4-08-04	2 x 4	1-03-08	11-11 4-02-07	90.9 56.00		
	4	J4 Jack-Open	6 / 12	4-05-08	3-02-07	2 x 4	1-03-08	4-03 2-06-15	50.32 32.00		

TOTAL # TRUSS= 75

TOTAL BFT OF ALL TRUSSES= 3503.84 BFT.

TOTAL WEIGHT OF ALL TRSSES 5504.89 LBS

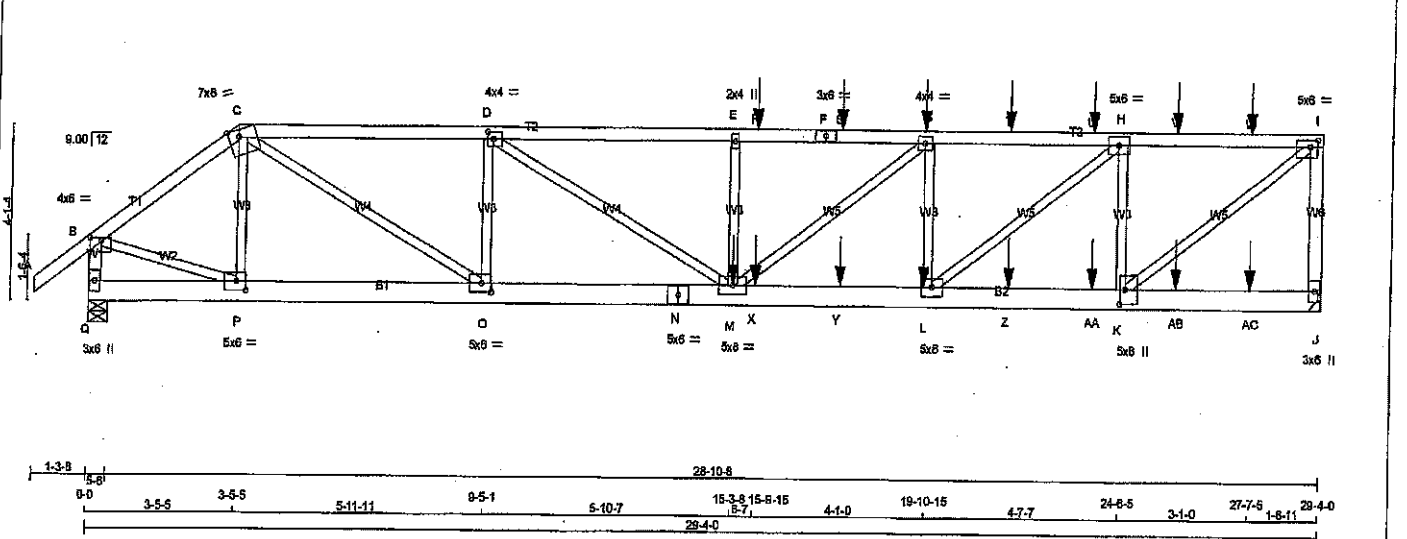
HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	HGUS26-2	
1	Hardware	HGUS26-3	
13	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 19

DWG NO. TAM *7706457*
STRUCTURAL
COMPONENT ONLY

JOB NAME 200164-400357	TRUSS NAME T2	QUANTITY 1	PLY 2	JOB DESC. Barlow 12	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 26 17:49:16 2019 Page 1 ID: ILZa865hX_OC9BuuR0TOTVWjZpO-F9yk7b?QZl?Bln8nEbLM1uzQHjoadmY7k1jUSzX?eX 15-9-15 4-1-0 19-10-15 4-7-7 24-8-5 3-1-0 27-7-5 1-8-11 29-4-0 Scale = 1:49.6						



TOTAL WEIGHT = 2 X 135 = 268 lb

CHORDS	SIZE	DRY	LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	SIDE(0.0)
F - I	12	SIDE(81.0)
I - J	12	TOP
Q - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q - N	12	TOP
N - J	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
E - M	6	SIDE(183.0)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
CB TMW+p	MT20	4.0	6.0	1.00 3.25
OC TMW+m	MT20	7.0	8.0	Edge 3.00
CD TMW+w	MT20	4.0	4.0	2.00 1.50
TE TMW+v	MT20	2.0	4.0	
TS-t	MT20	3.0	6.0	
CG TMW+w	MT20	4.0	4.0	
H TMW+w	MT20	5.0	6.0	
I TMW+v	MT20	5.0	6.0	2.00 2.00
J BMW+p	MT20	3.0	6.0	
K BMW+w	MT20	5.0	6.0	4.00 1.50
L BMW+w	MT20	5.0	6.0	
M BMW+w	MT20	5.0	6.0	
N BS-t	MT20	5.0	6.0	
O BMW+w	MT20	5.0	6.0	2.50 2.75
P BMW+w	MT20	5.0	6.0	2.50 2.75
Q BMW+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					
BEARINGS					
FACTORED	MAXIMUM FACTORED	INPUT	REQRD		
GROSS REACTION	GROSS REACTION	BRG	BRG		
JT VERT	DOWN	UP	IN-SX	IN-SX	
J 3721	0	3721	0	0	MECHANICAL
Q 3196	0	3196	0	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS					
1ST CASE	MAX	MIN	COMPONENT REACTIONS		
JT	FACTORED	LIVE	PERALIVE	WIND	DEAD
J 2761	1534 / 0	568 / 0	0 / 0	0 / 0	689 / 0
Q 2376	1350 / 0	448 / 0	0 / 0	0 / 0	578 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 42	-102.1 -102.1 0.08 (1)	10.00	P-C	-596 / 0	0.08 (1)	
B-C	-3943 / 0	-102.1 -102.1 0.16 (1)	4.98	M-E	-572 / 0	0.09 (1)	
C-D	-6148 / 0	-102.1 -102.1 0.55 (1)	3.54	B-P	0 / 2795	0.35 (1)	
D-E	-8137 / 0	-102.1 -102.1 0.73 (1)	2.98	D-M	0 / 2359	0.29 (1)	
E-F	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12	C-O	0 / 4114	0.51 (1)	
F-G	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12	O-D	-1935 / 0	0.25 (1)	
G-H	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12	K-I	0 / 5400	0.67 (1)	
H-I	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12	M-G	0 / 1838	0.20 (1)	
I-J	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12	K-H	-2977 / 0	0.38 (1)	
J-K	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12	L-G	-1768 / 0	0.22 (1)	
K-L	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12	L-H	0 / 3340	0.41 (1)	
L-M	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12				
M-N	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12				
N-O	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12				
O-P	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12				
P-Q	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12				
Q-R	-8137 / 0	-102.1 -102.1 0.50 (1)	3.12				

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	MAX	FACE	DIR.	TYPE
G	19-9-15	-123	FRONT	VERT	TOTAL
L	19-9-15	-55	FRONT	VERT	TOTAL
M	15-3-8	-1407	FRONT	VERT	TOTAL

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	29.0 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		=	52.5 PSF
SPACING		=	24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TO=0.73/1.00 (D-E:1), BC=0.54/1.00 (L-M:1), WB=0.67/1.00 (H-I:1), SS=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

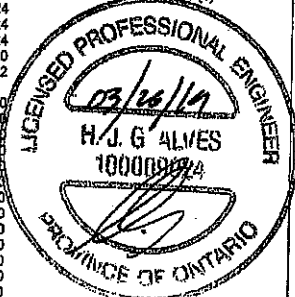
NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354 1667 738 1987 1658		

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.60 (K) (INPUT = 0.60)
JSI METAL= 0.77 (K) (INPUT = 1.00)

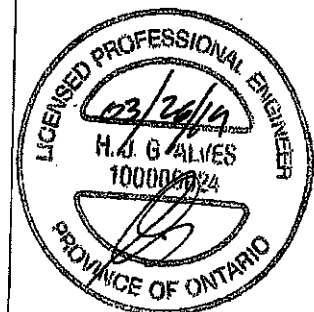
DWG NO. TAM 17906457
STRUCTURAL
COMPONENT ONLY



JOB NAME 200164-400357	TRUSS NAME T2	QUANTITY 1	PLY 2	JOB DESC. Barton 12	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mittek Industries, Inc. Tue Mar 26 17:49:16 2019 Page 2
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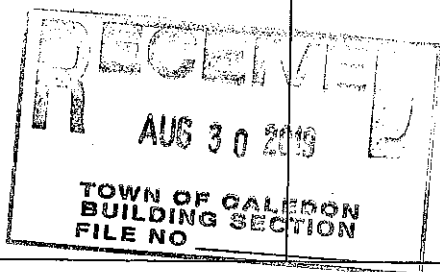
FACTORED CONCENTRATED LOADS (LBS)						FACE	DIR.	TYPE	HEEL	CONN.
JT	LOC.	LC1	MAX-	MAX+						
R	15-9-15	-123	-123	—	—	FRONT	VERT	TOTAL	—	—
S	17-8-15	-123	-123	—	—	FRONT	VERT	TOTAL	—	—
T	21-9-15	-123	-123	—	—	FRONT	VERT	TOTAL	—	—
U	23-8-15	-123	-123	—	—	FRONT	VERT	TOTAL	—	—
V	25-8-15	-123	-123	—	—	FRONT	VERT	TOTAL	—	—
W	27-7-5	-123	-123	—	—	FRONT	VERT	TOTAL	—	—
X	15-9-15	-55	-70	—	—	FRONT	VERT	TOTAL	—	—
Y	17-8-15	-55	-70	—	—	FRONT	VERT	TOTAL	—	—
Z	21-9-15	-55	-70	—	—	FRONT	VERT	TOTAL	—	—
AA	23-9-15	-55	-70	—	—	FRONT	VERT	TOTAL	—	—
AB	25-9-15	-55	-70	—	—	FRONT	VERT	TOTAL	—	—
AC	27-7-5	-55	-70	—	—	FRONT	VERT	TOTAL	—	—

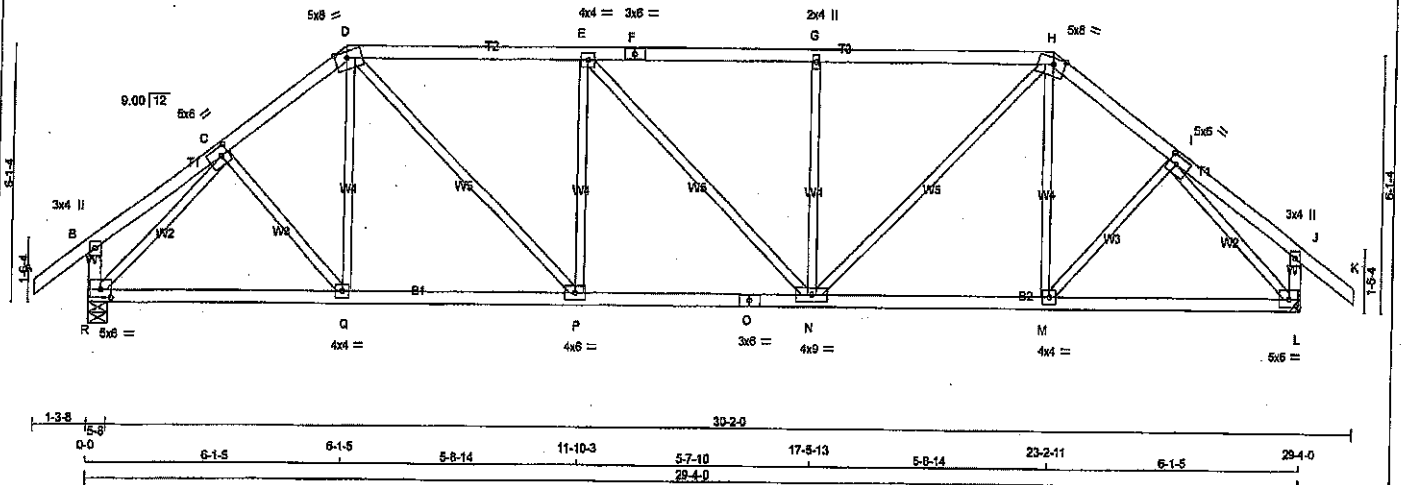


DWG NO. TAM 1906459
 STRUCTURAL
 COMPONENT ONLY 2/2

Scale = 1:51 0

DWG NO. TAM 72906460
STRUCTURAL
COMPONENT ONLY





TOTAL WEIGHT = 128 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
R - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVvp	MT20	3.0	4.0		
C TMVWv-l	MT20	5.0	8.0	2.50	2.25
D TMVWv-m	MT20	5.0	8.0	Edge 3.50	
E TMVWv-t	MT20	4.0	4.0		
F TS-t	MT20	3.0	8.0		
G TMVWv-w	MT20	2.0	4.0		
H TMVWv-m	MT20	5.0	8.0	Edge 3.50	
I TMVWv-l	MT20	5.0	8.0	2.50	2.25
J TMVv-p	MT20	3.0	4.0		
L BMVWv-l-t	MT20	5.0	6.0	2.25	3.00
M BMVWv-l	MT20	4.0	4.0		
N BMVWv-w-l	MT20	4.0	9.0		
O BS-t	MT20	3.0	6.0		
P BMVWv-l	MT20	4.0	6.0		
Q BMVWv-l	MT20	4.0	4.0		
R BMVWv-l	MT20	5.0	6.0	2.25	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT BRG	REQRD BRG
	GROSS REACTION	GROSS REACTION	DOWN	HORZ		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
R	2203	0	2203	0	0	5-8
L	2203	0	2203	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
JT	1938	932/0	308/0	0/0	0/0	398/0
R	1938	932/0	308/0	0/0	0/0	398/0
L	1938	932/0	308/0	0/0	0/0	398/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. FACTORED FORCE (LBS)	MEMB.	FACTORED	
		VERT. LOAD	LC1 MAX			MAX. FACTORED FORCE (LBS)	MAX. FACTORED
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	C-Q	0/218
B-C	0/18	-102.1	-102.1	0.12 (1)	10.00	Q-D	0/202
C-D	-2206/0	-102.1	-102.1	0.19 (1)	4.44	D-E	0/1175
D-E	-2575/0	-102.1	-102.1	0.63 (1)	3.68	E-F	-927/0
E-F	-2574/0	-102.1	-102.1	0.62 (1)	3.68	F-G	-2/0
F-G	-2574/0	-102.1	-102.1	0.62 (1)	3.68	G-H	-826/0
G-H	-2573/0	-102.1	-102.1	0.62 (1)	3.67	H-I	0/1172
H-I	-2207/0	-102.1	-102.1	0.19 (1)	4.44	I-J	0/202
I-J	0/18	-102.1	-102.1	0.12 (1)	10.00	J-K	0/218
J-K	0/42	-102.1	-102.1	0.14 (1)	10.00	K-L	-2444/0
R-B	-267/0	0.0	0.0	0.03 (1)	7.81	L-L	-2444/0
L-J	-267/0	0.0	0.0	0.03 (1)	7.81		
R-Q	0/1609	-38.5	-38.5	0.47 (2)	10.00		
Q-P	0/1748	-38.5	-38.5	0.49 (2)	10.00		
P-O	0/2575	-38.5	-38.5	0.53 (1)	10.00		
O-N	0/2575	-38.5	-38.5	0.53 (1)	10.00		
N-M	0/1748	-38.5	-38.5	0.49 (2)	10.00		
M-L	0/1609	-38.5	-38.5	0.47 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
OL = 6.0 PSF
BOT CH. LL = 10.5 PSF
OL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09/CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/989 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/989 (0.20")

CSI: TC=0.83/1.00 (D-E-1), BC=0.53/1.00 (N-P-1), WB=0.83/1.00 (L-L-1), SSI=0.27/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

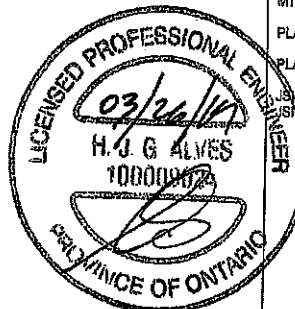
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP=0.88 (L) (INPUT = 0.80)
SMETAL=0.83 (O) (INPUT = 1.00)

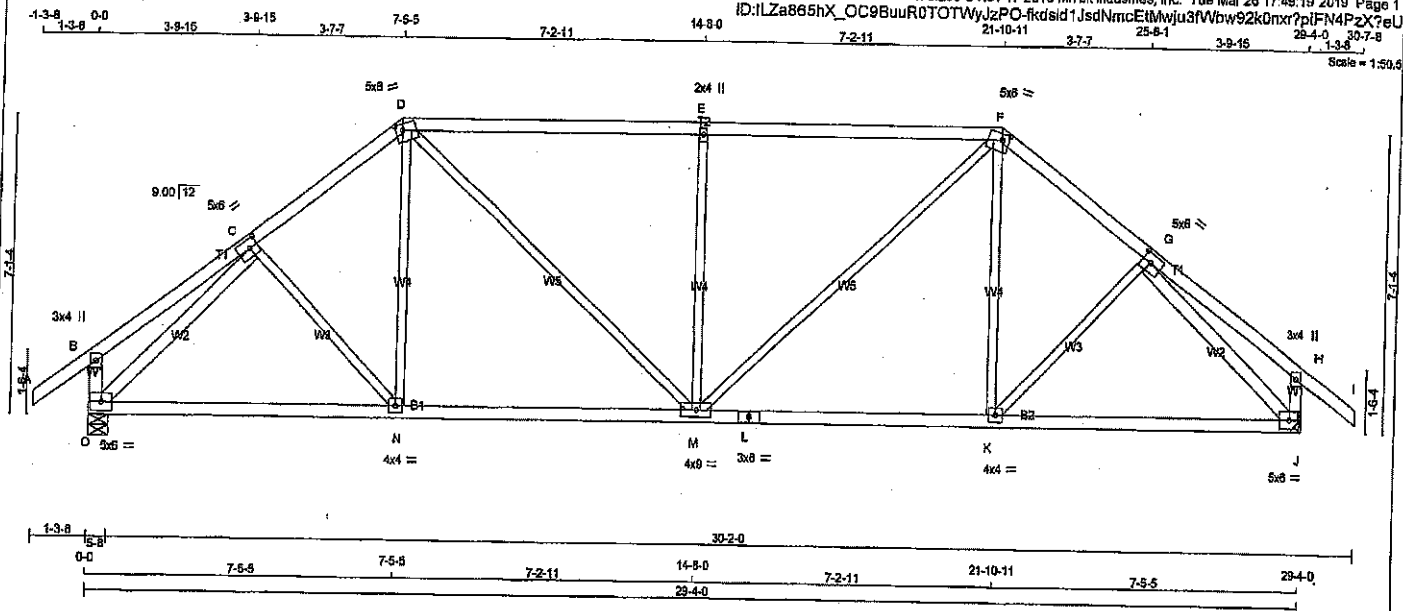


DRWG NO. TAM 71906461
STRUCTURAL
COMPONENT ONLY

JOB NAME 200164-400357	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. Barton 12	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: ILZa865hX_OC9BuuR0TOTWYJzPO-fkdsid1JsdNmcEtMwju3fWbw92k0nxr?plFN4PzX7eU
21-10-11 3-7-7 25-8-1 3-9-15 28-4-0 30-7-8
Scale = 1:50.0



TOTAL WEIGHT = 129 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - C	2x4	DRY	No.2
G - J	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-1	MT20	5.0	6.0	2.50 2.50
D	TMVW-m	MT20	5.0	6.0	1.75 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TMVW-m	MT20	5.0	6.0	1.75 2.00
G	TMVW-t	MT20	5.0	6.0	2.50 2.50
H	TMV+p	MT20	3.0	4.0	
J	BMVW-1-t	MT20	5.0	6.0	
K	BMVW-1	MT20	4.0	4.0	
L	BS-1	MT20	3.0	6.0	
M	BMVW-1	MT20	4.0	8.0	
N	BMVW-1	MT20	4.0	4.0	
O	BMVW-1	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX
O	2203	0	2203	0	0	0	5-8	5-8	IN-SX
J	2203	0	2203	0	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1638	932 / 0	308 / 0	0 / 0	0 / 0	398 / 0	0 / 0
J	1638	932 / 0	308 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	C-N	0 / 131	0.03 (3)	
B-C	0 / 25	-102.1 -102.1	0.21 (1)	10.00	N-D	0 / 342	0.08 (2)	
C-D	-2176 / 0	-102.1 -102.1	0.22 (1)	4.44	D-M	0 / 839	0.19 (1)	
D-E	-2332 / 0	-102.1 -102.1	0.82 (1)	3.39	M-E	-905 / 0	0.79 (1)	
E-F	-2332 / 0	-102.1 -102.1	0.82 (1)	3.39	M-F	0 / 839	0.19 (1)	
F-G	-2176 / 0	-102.1 -102.1	0.22 (1)	4.44	K-F	0 / 342	0.08 (2)	
G-H	0 / 25	-102.1 -102.1	0.21 (1)	10.00	K-G	0 / 131	0.03 (3)	
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	O-C	-2460 / 0	0.84 (1)	
O-B	-288 / 0	0.0	0.0	0.03 (1)	7.81	G-J	-2460 / 0	0.84 (1)
J-H	-288 / 0	0.0	0.0	0.03 (1)	7.81			
O-N	0 / 1877	-38.5	-38.5	0.82 (2)	10.00			
N-M	0 / 1721	-38.5	-38.5	0.83 (2)	10.00			
M-L	0 / 1721	-38.5	-38.5	0.83 (2)	10.00			
L-K	0 / 1721	-38.5	-38.5	0.83 (2)	10.00			
K-J	0 / 1877	-38.5	-38.5	0.82 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00H/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/699 (0.22")

CSI: TC=0.82/1.00 (D-E:1), BC=0.83/1.00 (M-N:2),
WB=0.84/1.00 (C-G:1), SB=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

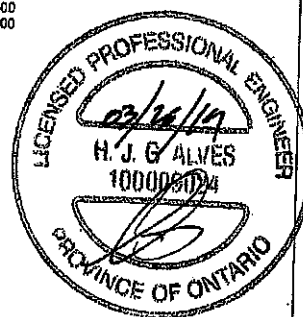
NAIL VALUES
PLATE GRIP(OR)Y SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1667 1666

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.87 (G) (INPUT = 0.80)

SI METAL= 0.81 (G) (INPUT = 1.00)



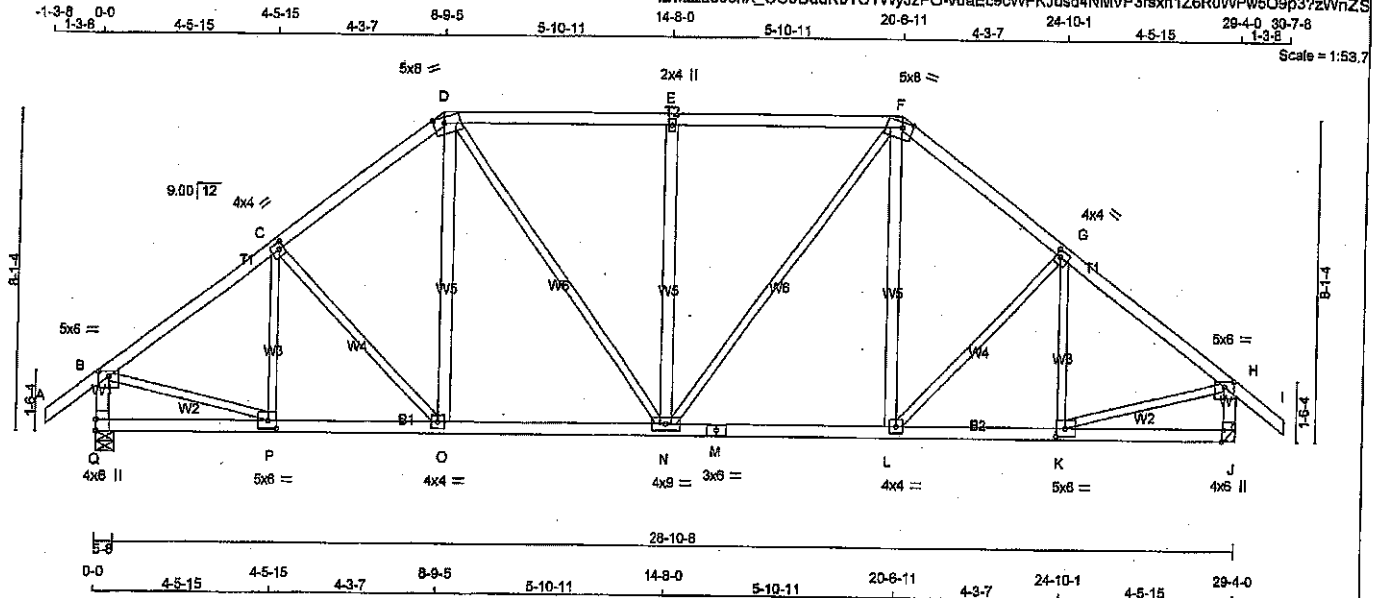
DRWG NO. TAM 11906462
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
200164-400357	T6	1	1	Barton 12		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 143 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
F - I	2x4	DRY	No.2	SPF		
Q - B	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		
Q - M	2x4	DRY	No.2	SPF		
M - J	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
Q - D	2x4	DRY	No.2	SPF		
N - E	2x4	DRY	No.2	SPF		
L - F	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
Q	2203	0	2203	0
J	2203	0	2203	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
Q	1638
J	1638

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	MAX	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
FR-TO	FORCE (LBS)	VERT. (PLF)	CSI (L)	UNBRACED LENGTH	FR-TO	FORCE (LBS)	MAX. FACTORED	CSI (L)
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	P-C	-288/19	0.10 (1)
B-C	-2187/0	-102.1	-102.1	0.32 (1)	4.33	C-O	-213/0	0.15 (1)
C-D	-2079/0	-102.1	-102.1	0.30 (1)	4.43	O-D	0/377	0.08 (2)
D-E	-2004/0	-102.1	-102.1	0.52 (1)	4.20	D-N	0/803	0.14 (1)
E-F	-2004/0	-102.1	-102.1	0.52 (1)	4.20	N-E	-734/0	0.65 (1)
F-G	-2079/0	-102.1	-102.1	0.30 (1)	4.43	N-F	0/803	0.14 (1)
G-H	-2197/0	-102.1	-102.1	0.32 (1)	4.33	L-F	0/377	0.08 (2)
H-I	0/42	-102.1	-102.1	0.14 (1)	10.00	L-G	-213/0	0.15 (1)
Q-B	-2129/0	0.0	0.0	0.22 (1)	5.81	K-G	-288/19	0.10 (1)
J-H	-2129/0	0.0	0.0	0.22 (1)	5.81	B-P	0/1844	0.41 (1)
Q-P	0/0	-38.5	-38.5	0.13 (3)	10.00	K-H	0/1844	0.41 (1)
P-O	0/1782	-38.5	-38.5	0.40 (1)	10.00			
O-N	0/1840	-38.5	-38.5	0.42 (2)	10.00			
N-M	0/1840	-38.5	-38.5	0.42 (2)	10.00			
M-L	0/1840	-38.5	-38.5	0.42 (2)	10.00			
L-K	0/1782	-38.5	-38.5	0.40 (1)	10.00			
K-J	0/0	-38.5	-38.5	0.13 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.08")
CALCULATED VERT. DEFL. (LL) = $L/998$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.08")
CALCULATED VERT. DEFL. (TL) = $L/998$ (0.14")

CSI: TC=0.52/1.00 (D-E:1), BC=0.42/1.00 (L-N:2), WB=0.68/1.00 (E-N:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

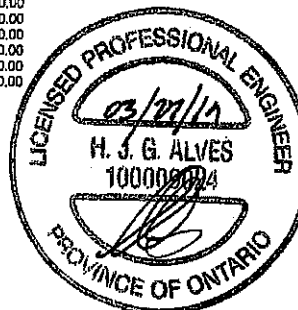
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
	1857	789
	1857	1857

PLATE PLACEMENT TOL. = 0.250 inches

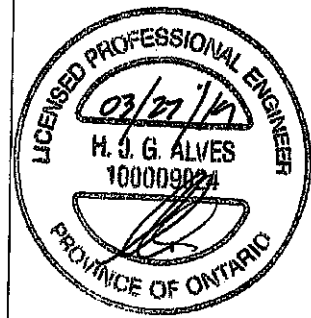
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.80)
JSI METAL= 0.48 (M) (INPUT = 1.00)

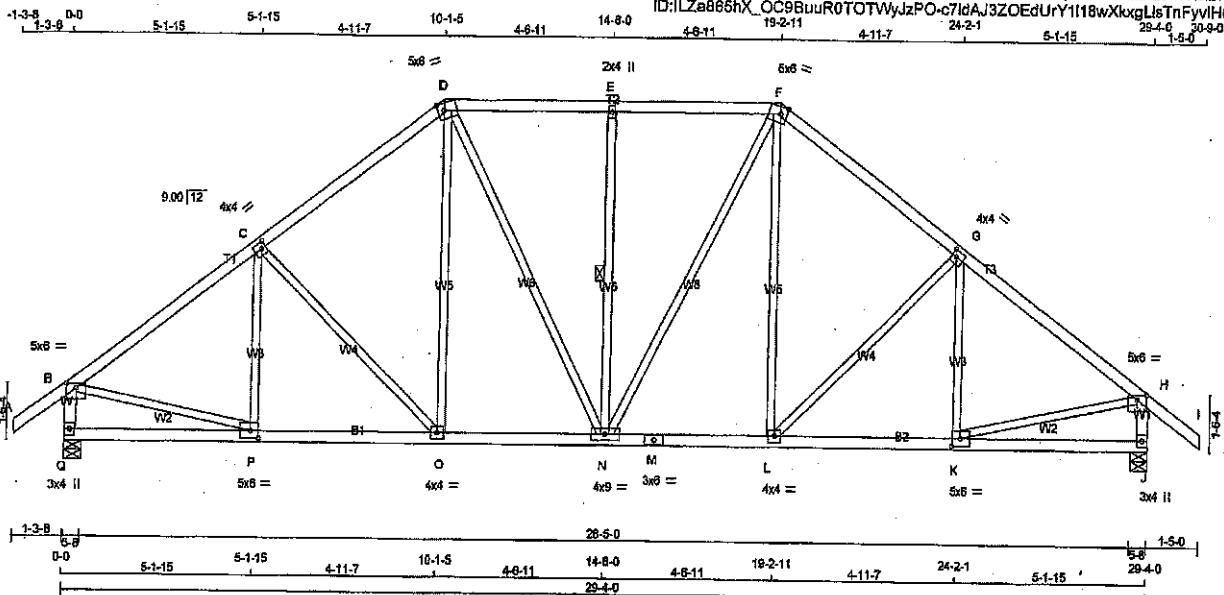


DWG NO. TAM T1906463
STRUCTURAL
COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME 200164-400357	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. Barton 12	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Wed Mar 27 09:50:25 2018 Page 2 ID:ILZa865hX_OC9BuuR0TOTWVjzPO-vdaEc9cWPKJusd4NMVP3rsxh1Z6R0WPw5O8p3?zWn2S	
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.					
 DWG NO. TAM T1906463 STRUCTURAL COMPONENT ONLY 7/2					

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	T7	1	1	Barton 12	
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
Q - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	Edge	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-w-t	MT20	4.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
Q	2203	0	0	2203	0	0	0	5-8	5-8
J	2216	0	0	2216	0	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1638	932 / 0	308 / 0	0 / 0	0 / 0	0 / 0	398 / 0	0 / 0
J	1646	939 / 0	308 / 0	0 / 0	0 / 0	0 / 0	399 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CS1 (LC)	MAX. UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	P-C	-200 / 94	0.09 (1)
B-C	-2228 / 0	-102.1 -102.1	0.42 (1)	4.18	C-O	-358 / 0	0.35 (1)
C-D	-1988 / 0	-102.1 -102.1	0.40 (1)	4.41	O-D	0 / 444	0.10 (2)
D-E	-1750 / 0	-102.1 -102.1	0.30 (1)	4.74	D-N	0 / 409	0.09 (1)
E-F	-1750 / 0	-102.1 -102.1	0.30 (1)	4.74	N-E	-980 / 0	0.30 (1)
F-G	-1988 / 0	-102.1 -102.1	0.40 (1)	4.41	N-F	0 / 409	0.09 (1)
G-H	-2228 / 0	-102.1 -102.1	0.42 (1)	4.18	L-F	0 / 444	0.10 (2)
H-I	0 / 46	-102.1 -102.1	0.16 (1)	10.00	L-G	-358 / 0	0.35 (1)
Q-B	-2121 / 0	0.0 0.0	0.22 (1)	5.82	K-G	-200 / 94	0.09 (1)
J-H	-2134 / 0	0.0 0.0	0.22 (1)	5.81	B-P	0 / 1859	0.42 (1)
					K-H	0 / 1859	0.42 (1)
Q-P	0 / 0	-38.5 -38.5	0.19 (3)	10.00			
P-O	0 / 1811	-38.5 -38.5	0.42 (2)	10.00			
C-N	0 / 1562	-38.5 -38.5	0.34 (1)	10.00			
N-M	0 / 1562	-38.5 -38.5	0.34 (1)	10.00			
M-L	0 / 1562	-38.5 -38.5	0.34 (1)	10.00			
L-K	0 / 1811	-38.5 -38.5	0.42 (2)	10.00			
K-J	0 / 0	-38.5 -38.5	0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/998 (0.07")
ALLOWABLE DEFL.(TL) = L/980 (0.88")
CALCULATED VERT. DEFL.(TL) = L/998 (0.12")

CSI: TC=0.42/1.00 (B-C:1), BC=0.42/1.00 (O-P:2), WS=0.42/1.00 (B-P:1), SSI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

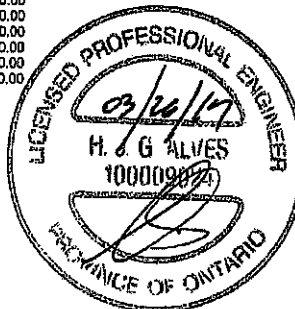
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 334 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

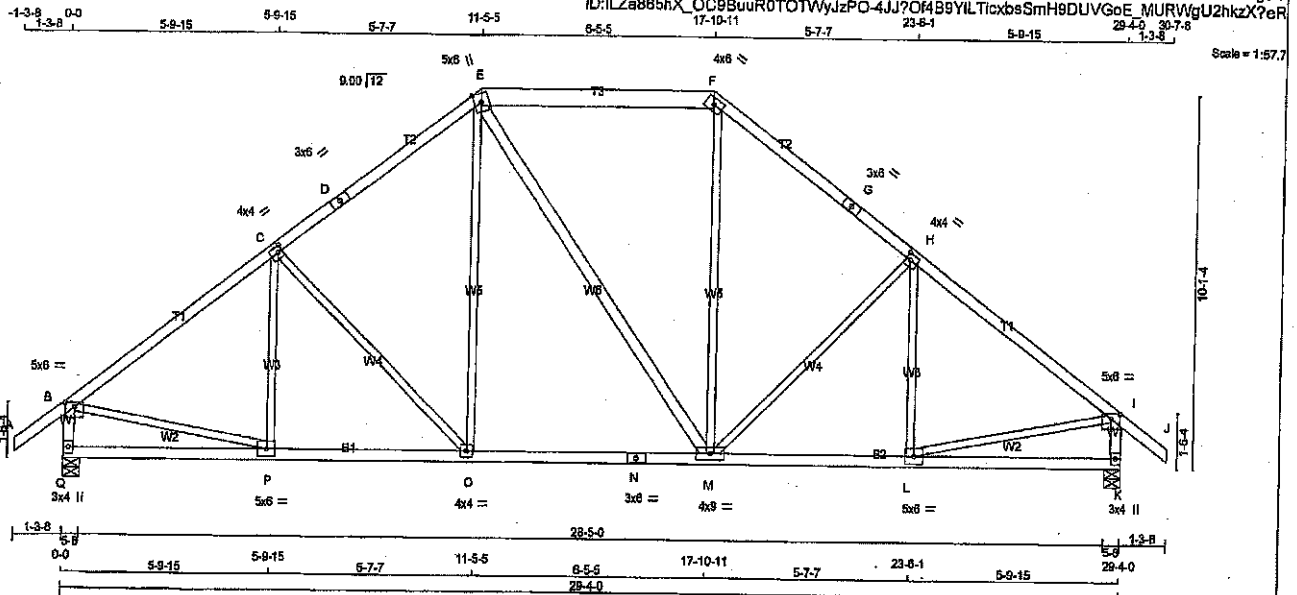
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.49 (M) (INPUT = 1.00)



DWG NO. TAM 71906464
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	T8	1	1	Barton 12.	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	1.50 3.00
C	TMVW-i	MT20	4.0	4.0	2.00 1.50
D	TS-i	MT20	3.0	6.0	
E	TTWV-m	MT20	5.0	6.0	Edge
F	TTW-h	MT20	4.0	6.0	
G	TS-i	MT20	3.0	6.0	
H	TMVW-i	MT20	4.0	4.0	2.00 1.50
I	TMVW-p	MT20	5.0	8.0	1.50 3.00
K	BMV1-p	MT20	3.0	4.0	
L	BMVW-i	MT20	5.0	6.0	
M	BMVWV-i	MT20	4.0	9.0	
N	BS-i	MT20	3.0	6.0	
O	BMVW-i	MT20	4.0	4.0	
P	BMVW-i	MT20	5.0	6.0	
Q	BMV1-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	Q	VERT	HORIZ	DOWN	UPLIFT
JT	Q	2203	0	2203	0
K	Q	2203	0	2203	0

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	Q	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
JT	Q	1638	932 / 0 308 / 0 0 / 0 0 / 0 368 / 0 0 / 0
K	Q	1638	932 / 0 308 / 0 0 / 0 0 / 0 368 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX. UNBRACED LENGTH	FR-TO	WEBS	MAX. FACTORED	FORCE	MAX
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (LBS)	MAX (LBS)	MAX (LBS)	MAX (LBS)	MAX (LBS)	MAX (LBS)	MEMB.	FORCE (LBS)	MAX (LBS)	MAX (LBS)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	10.00	10.00	10.00	P-C	-138 / 135	0.07 (1)	0.07 (1)
B-C	-2236 / 0	-102.1	-102.1	0.55 (1)	4.02	4.02	4.02	4.02	C-O	-488 / 0	0.58 (1)	0.58 (1)
C-D	-1905 / 0	-102.1	-102.1	0.50 (1)	4.35	4.35	4.35	4.35	O-E	0 / 579	0.13 (2)	0.13 (2)
D-E	-1905 / 0	-102.1	-102.1	0.50 (1)	4.35	4.35	4.35	4.35	E-M	0 / 1	0.00 (2)	0.00 (2)
E-F	-1487 / 0	-102.1	-102.1	0.28 (1)	6.03	6.03	6.03	6.03	M-F	0 / 580	0.13 (2)	0.13 (2)
F-G	-1905 / 0	-102.1	-102.1	0.50 (1)	4.35	4.35	4.35	4.35	M-H	-488 / 0	0.59 (1)	0.59 (1)
G-H	-1905 / 0	-102.1	-102.1	0.50 (1)	4.35	4.35	4.35	4.35	H-L	-140 / 134	0.07 (1)	0.07 (1)
H-I	-2236 / 0	-102.1	-102.1	0.55 (1)	4.02	4.02	4.02	4.02	B-P	0 / 1860	0.42 (1)	0.42 (1)
I-J	0 / 42	-102.1	-102.1	0.14 (1)	10.00	10.00	10.00	10.00	L-I	0 / 1859	0.42 (1)	0.42 (1)
Q-B	-2109 / 0	0.0	0.0	0.22 (1)	5.84	5.84	5.84	5.84				
K-I	-2109 / 0	0.0	0.0	0.22 (1)	5.84	5.84	5.84	5.84				
Q-P	0 / 0	-38.5	-38.5	0.22 (3)	10.00	10.00	10.00	10.00				
P-O	0 / 1822	-38.5	-38.5	0.45 (2)	10.00	10.00	10.00	10.00				
O-N	0 / 1488	-38.5	-38.5	0.42 (2)	10.00	10.00	10.00	10.00				
N-M	0 / 1488	-38.5	-38.5	0.42 (2)	10.00	10.00	10.00	10.00				
M-L	0 / 1822	-38.5	-38.5	0.47 (2)	10.00	10.00	10.00	10.00				
L-K	0 / 0	-38.5	-38.5	0.22 (3)	10.00	10.00	10.00	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 20.6 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CS: TC=0.55/1.00 (B-C:1), BC=0.47/1.00 (L-M:2),
WB=0.59/1.00 (C-O:1), SS=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

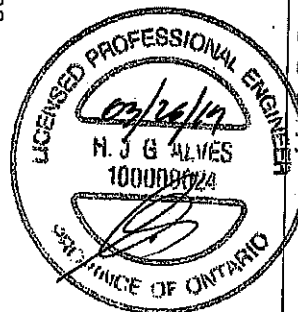
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FSI) (FLI) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.89 (P) (INPUT = 0.90)
METAL= 0.57 (N) (INPUT = 1.00)

DWG NO. TAM 11906465
STRUCTURAL
COMPONENT ONLY



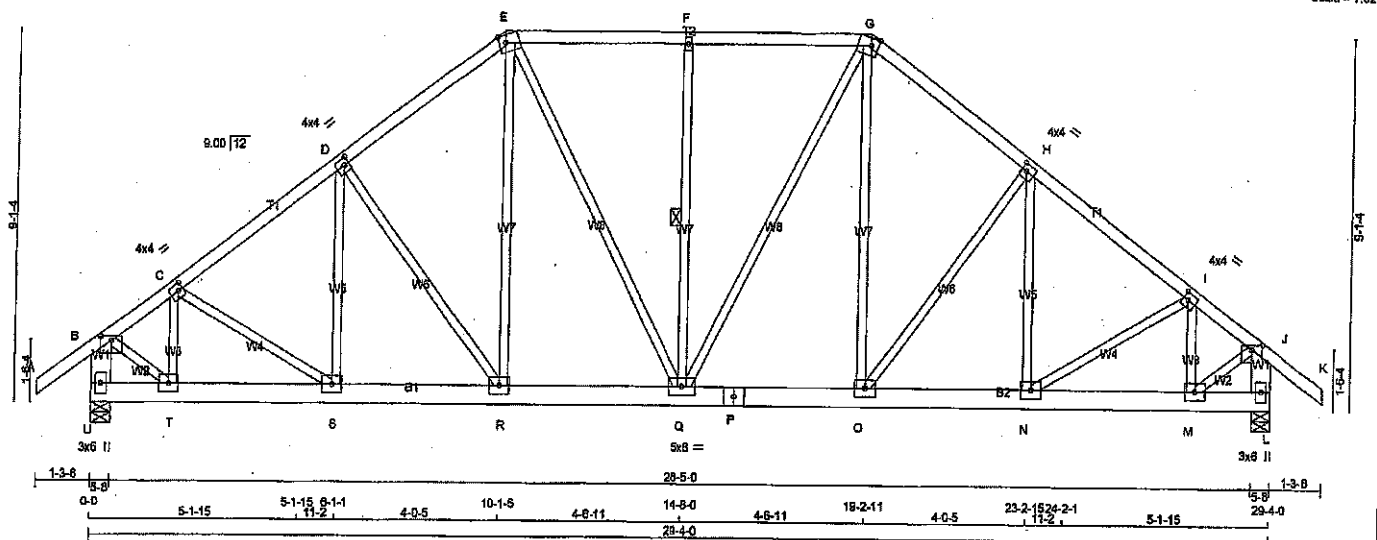
THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

RECEIVED
AUG 30 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Barton 12	DRWG NO.
200164-400357	T9	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 26 17:48:24 2019 Page 1
ID:ILZa865hX_OC9Buur0TOTWYJzPO-0IRioL5Rn9727IKIHUEMatuy3X7SJ9kz_z8mdzX7eP
1-3-8 0-0 5-1-15 5-1-1 11-2 4-0-5 10-1-5 4-6-11 14-8-0 4-6-11 18-2-11 4-0-5 23-2-1524-2-1 11-2 5-1-15 28-4-0 30-7-8 1-3-8
Scale = 1:52.0



TOTAL WEIGHT = 5 X 189 = 844 lb
(M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - K	2x4	DRY	No.2	SPF	
U - S	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
U - P	2x6	DRY	No.2	SPF	
P - L	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.25	3.00
C, D, H, I					
C TMVW-t	MT20	4.0	4.0	2.00	1.50
E TMVW-m	MT20	5.0	6.0	Edge	2.00
F TMVW-w	MT20	2.0	4.0		
G TMVW-m	MT20	5.0	6.0	Edge	2.00
J TMVW-p	MT20	5.0	6.0	1.25	3.00
L BMV1+p	MT20	3.0	6.0		
M, N, O, R, S, T					
M BMVW-t	MT20	5.0	6.0		
P BS-t	MT20	5.0	6.0		
Q BMVW-w	MT20	5.0	6.0		
U BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS	REACTION	GROSS	REACTION		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
U	2203	0	2203	0	5-8	5-8
L	2203	0	2203	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD
U	1638	932 / 0	308 / 0	0 / 0	0 / 0	398 / 0
L	1638	932 / 0	308 / 0	0 / 0	0 / 0	398 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (LBS)	
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)	
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	T-C	-735 / 0	0.12 (1)	
B-C	-1841 / 0	-102.1 -102.1	0.13 (1)	4.85	C-S	0 / 345	0.08 (1)	
C-D	-2220 / 0	-102.1 -102.1	0.26 (1)	4.37	R-E	0 / 475	0.11 (1)	
D-E	-1994 / 0	-102.1 -102.1	0.24 (1)	4.58	E-Q	0 / 410	0.09 (1)	
E-F	-1768 / 0	-102.1 -102.1	0.30 (1)	4.72	Q-F	-559 / 0	0.29 (1)	
F-G	-1768 / 0	-102.1 -102.1	0.30 (1)	4.72	Q-G	0 / 410	0.09 (1)	
G-H	-1994 / 0	-102.1 -102.1	0.24 (1)	4.58	O-G	0 / 475	0.11 (1)	
H-I	-2220 / 0	-102.1 -102.1	0.28 (1)	4.37	N-I	0 / 345	0.08 (1)	
I-J	-1841 / 0	-102.1 -102.1	0.13 (1)	4.85	M-I	-735 / 0	0.12 (1)	
J-K	0 / 42	-102.1 -102.1	0.14 (1)	10.00	B-T	0 / 1717	0.39 (1)	
U-B	-2164 / 0	0.0	0.0	0.14 (1)	8.93	M-J	0 / 1717	0.36 (1)
L-J	-2164 / 0	0.0	0.0	0.14 (1)	6.93	S-D	-30 / 122	0.03 (3)
U-T	0 / 0	-38.5	-38.5	0.02 (3)	10.00	D-R	-367 / 0	0.33 (1)
T-S	0 / 1494	-38.5	-38.5	0.23 (1)	10.00	N-H	-30 / 122	0.03 (3)
S-R	0 / 1787	-38.5	-38.5	0.25 (1)	10.00	O-H	-367 / 0	0.33 (1)
R-Q	0 / 1576	-38.5	-38.5	0.22 (1)	10.00			
Q-P	0 / 1576	-38.5	-38.5	0.22 (1)	10.00			
P-O	0 / 1576	-38.5	-38.5	0.22 (1)	10.00			
O-N	0 / 1787	-38.5	-38.5	0.25 (1)	10.00			
N-M	0 / 1494	-38.5	-38.5	0.23 (1)	10.00			
M-L	0 / 0	-38.5	-38.5	0.02 (3)	10.00			

ENGINEERED PROFESSIONAL

02/26/10

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/800 (0.98")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/800 (0.98")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.30/1.00 (E-F:1), BC=0.25/1.00 (R-S:1),
WB=0.39/1.00 (B-T:1), SI=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

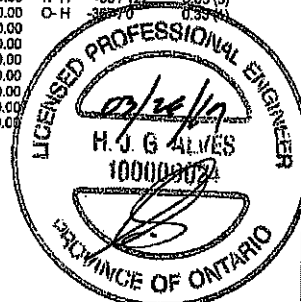
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

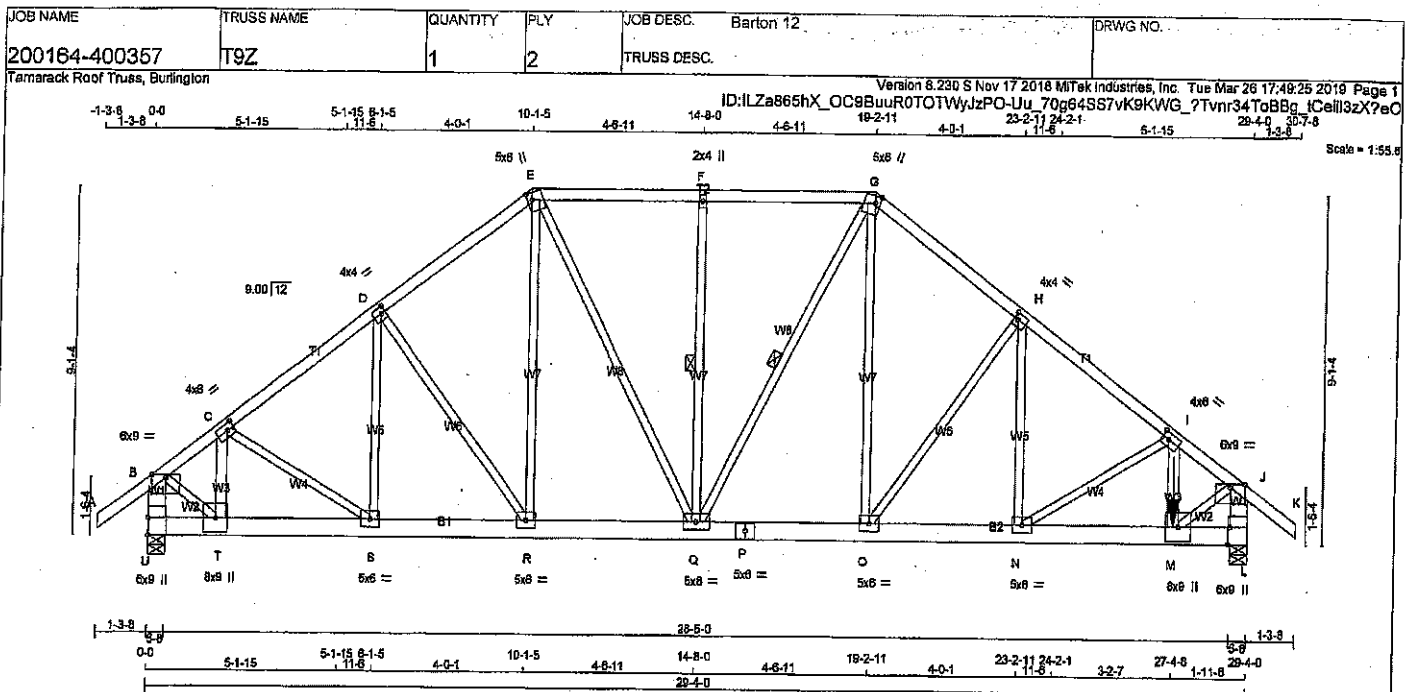
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (M) (INPUT = 0.60)
JSI METAL = 0.39 (E) (INPUT = 1.00)



DWG NO. TAM 71706466
STRUCTURAL
COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - K	2x4	DRY	No.2	SPF	
U - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
U - P	2x6	DRY	No.2	SPF	
P - L	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
T - C	2x4	DRY	No.2	SPF	
M - I	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-E	12	TOP
E-G	12	TOP
G-K	12	TOP
U-B	12	TOP
L-J	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
U-P	6	TOP
P-L	6	SIDE(427.3)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	8	SIDE(1068.1)
I-M	2	
2x4	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	6.0	9.0	Edge	
C TMVW-i	MT20	4.0	6.0	2.00	2.25
D TMVW-i	MT20	4.0	4.0	2.00	1.25
E TTWV+m	MT20	5.0	6.0	2.25	1.50
F TMVW+w	MT20	2.0	4.0		
G TTWV+m	MT20	5.0	6.0	2.25	1.50
H TMVW-i	MT20	4.0	4.0	2.00	1.25
I TMVW-p	MT20	4.0	6.0	2.00	2.25
J TMVW-i	MT20	6.0	9.0	Edge	
L BMVW-i	MT20	6.0	9.0	Edge	0.50
M BMVW-i	MT20	6.0	9.0		
N O, R, S					
N BMVW-i	MT20	5.0	6.0		
P BS-i		5.0	6.0		
Q BMVW-i	MT20	5.0	6.0		
T BMVW-i	MT20	6.0	9.0		
U BMVW-i	MT20	6.0	9.0	5.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
U	2808	0	0	2808	0	0	0	5-8	5-8
L	8187	0	0	8187	0	0	0	5-8	5-8
UNFACTORED REACTIONS		1ST LOASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
U	1939	1103 / 0	385 / 0	0 / 0	0 / 0	471 / 0	0 / 0		
L	8099	3455 / 0	1152 / 0	0 / 0	0 / 0	1482 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MEMB.		WEBS		MEMB.		WEBS	
MAX. FACTORED	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. FACTORED	FORCE (LBS)	MAX. FACTORED	FORCE (LBS)	MAX. FACTORED	FORCE (LBS)
FR-TO		FROM TO	CS1 (LC)	UNBRACED LENGTH	FR-TO				
A-B	0 / 42	-102.1 -102.1	0.08 (1)	10.00	T-C	-915 / 0	0.08 (1)		
B-C	-2219 / 0	-102.1 -102.1	0.08 (1)	5.90	C-S	0 / 498	0.08 (1)		
C-D	-2782 / 0	-102.1 -102.1	0.17 (1)	5.33	R-E	0 / 426	0.05 (2)		
D-E	-2583 / 0	-102.1 -102.1	0.17 (1)	5.48	E-Q	0 / 889	0.11 (1)		
E-F	-2448 / 0	-102.1 -102.1	0.22 (1)	5.52	Q-F	-553 / 0	0.15 (1)		
F-G	-2448 / 0	-102.1 -102.1	0.22 (1)	5.52	Q-G	-37 / 0	0.01 (1)		
G-H	-3094 / 0	-102.1 -102.1	0.19 (1)	5.08	O-G	0 / 1533	0.19 (1)		
H-I	-4287 / 0	-102.1 -102.1	0.23 (1)	4.43	N-I	-2537 / 0	0.50 (1)		
I-J	-6999 / 0	-102.1 -102.1	0.19 (1)	3.59	M-I	0 / 2875	0.24 (1)		
J-K	0 / 42	-102.1 -102.1	0.08 (1)	10.00	B-T	0 / 2065	0.26 (1)		
U-B	-2562 / 0	0.0 0.0	0.09 (1)	7.81	M-J	0 / 6440	0.80 (1)		
L-J	-7809 / 0	0.0 0.0	0.28 (1)	5.44	S-D	-108 / 67	0.03 (1)		
					D-R	-305 / 0	0.14 (1)		
U-T	0 / 0	-38.5 -38.5	0.01 (3)	10.00	N-H	0 / 1624	0.20 (1)		
T-S	0 / 1787	-38.5 -38.5	0.15 (1)	10.00	O-H	-1711 / 0	0.09 (1)		
S-R	0 / 2220	-38.5 -38.5	0.17 (1)	10.00					
R-Q	0 / 2046	-38.5 -38.5	0.16 (1)	10.00					
Q-P	0 / 2489	-38.5 -38.5	0.19 (1)	10.00					
P-O	0 / 2486	-38.5 -38.5	0.19 (1)	10.00					
O-N	0 / 3448	-38.5 -38.5	0.25 (1)	10.00					
N-M	0 / 5603	-38.5 -38.5	0.52 (1)	10.00					
M-L	0 / 0	-373.9 -373.9	0.16 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR
M	27-4-8	-5732	-5732	-	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/998 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.28/1.00 (J-L1), BC=0.52/1.00 (M-N1),
WB=0.80/1.00 (J-M1), SS=0.19/1.00 (L-M1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (T) (INPUT = 0.90)
SI METAL= 0.83 (T) (INPUT = 1.00)

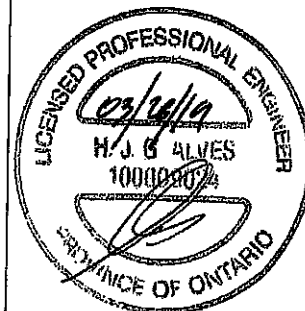
DWG NO. TAM 1906467
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	T9Z	1	2	Barton 12	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

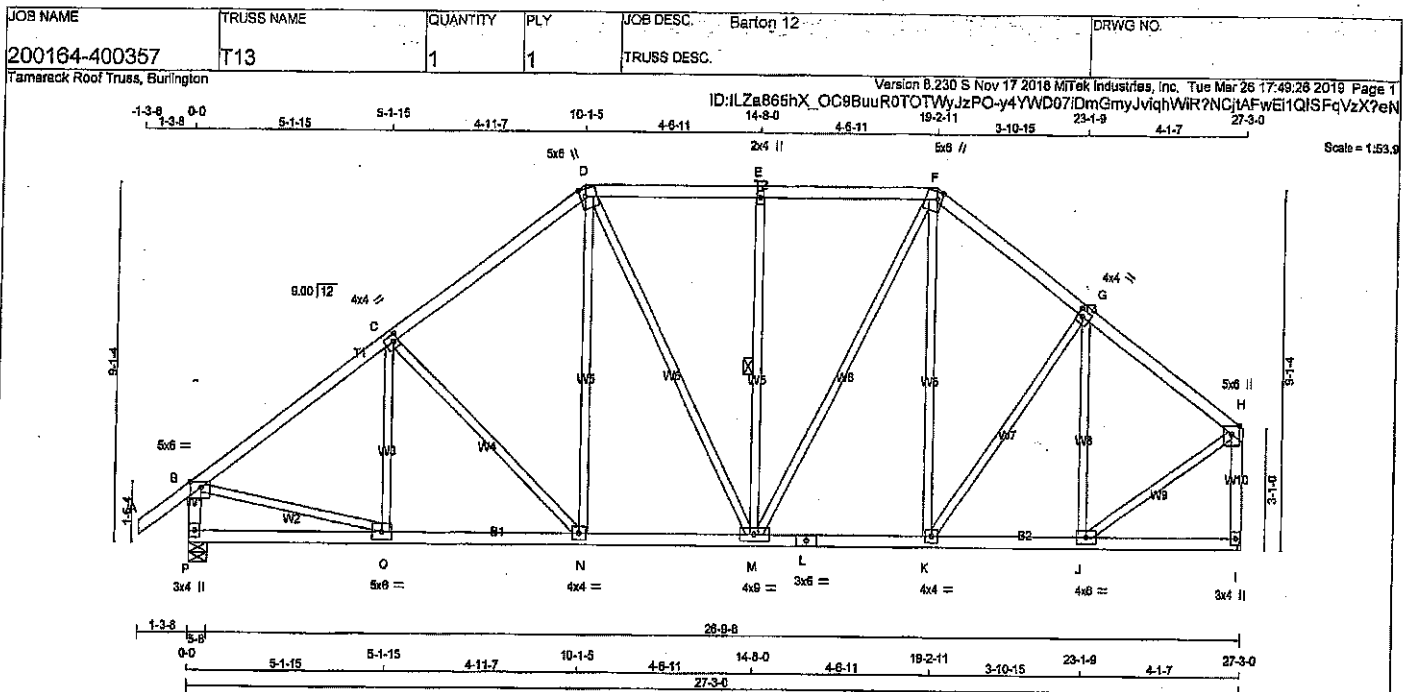
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 26 17:49:25 2019 Page 2
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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



DWG NO. TAM 17906467
STRUCTURAL
COMPONENT ONLY

7/2



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TMVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW+p	MT20	5.0	6.0	2.25	1.50
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-w	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
P	2057	2057	5-8	5-8
I	1918	1918	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-12.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX / MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1528	871 / 0	286 / 0	0 / 0	0 / 0	371 / 0	0 / 0
I	1432	791 / 0	286 / 0	0 / 0	0 / 0	354 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	O-C	-185 / 111	0.07 (1)		
B-C	-2042 / 0	-102.1 -102.1	0.41 (1)	4.35	C-N	-387 / 0	0.38 (1)		
C-D	-1778 / 0	-102.1 -102.1	0.38 (1)	4.82	N-D	0 / 460	0.10 (2)		
D-E	-1507 / 0	-102.1 -102.1	0.29 (1)	5.04	D-M	0 / 246	0.08 (1)		
E-F	-1507 / 0	-102.1 -102.1	0.29 (1)	5.04	M-E	-562 / 0	0.30 (1)		
F-G	-1583 / 0	-102.1 -102.1	0.24 (1)	5.02	M-F	0 / 579	0.13 (1)		
G-H	-1488 / 0	-102.1 -102.1	0.23 (1)	5.18	K-F	0 / 159	0.04 (3)		
P-B	-1975 / 0	0.0 0.0	0.21 (1)	6.00	K-G	0 / 89	0.02 (1)		
I-H	-1850 / 0	0.0 0.0	0.31 (1)	6.16	J-G	-825 / 0	0.38 (1)		
P-O	0 / 0	-38.5 -38.5	0.19 (3)	10.00	B-O	0 / 1708	0.38 (1)		
O-N	0 / 1682	-38.5 -38.5	0.40 (2)	10.00	J-H	0 / 1431	0.38 (1)		
N-M	0 / 1394	-38.5 -38.5	0.31 (1)	10.00					
M-L	0 / 1241	-38.5 -38.5	0.28 (2)	10.00					
L-K	0 / 1241	-38.5 -38.5	0.28 (2)	10.00					
K-J	0 / 1189	-38.5 -38.5	0.27 (2)	10.00					
J-I	0 / 0	-38.5 -38.5	0.11 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.91")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.91")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CS: TC=0.41/1.00 (B-C:1), BC=0.40/1.00 (N-O:2), WB=0.38/1.00 (B-O:1), SS=0.22/1.00 (D-E:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

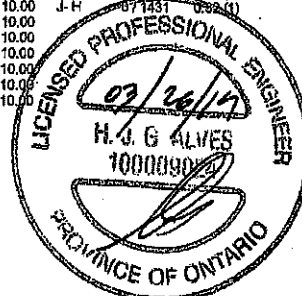
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1067 788 1987 1855

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (B) (INPUT=0.90)
JSI METAL=0.98 (H) (INPUT=1.00)



DWG NO. TAM T1206468
STRUCTURAL
COMPONENT ONLY

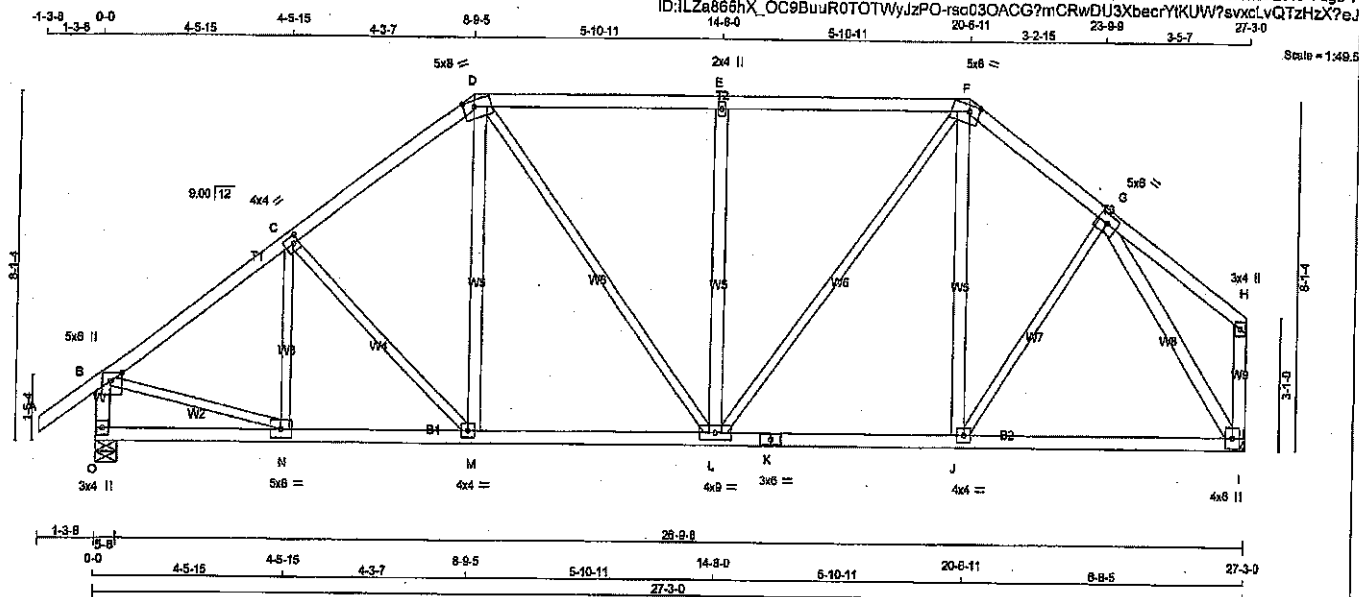
DWG NO. TAM 77906469
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 77906470
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	T16	1	1	Barton 12	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 137 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
M - D	2x4	DRY	No.2
L - E	2x4	DRY	No.2
J - F	2x4	DRY	No.2
G - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	Edge 3.00
C	TMVW+1	MT20	4.0	4.0	2.00 1.50
D	TTVW+m	MT20	5.0	8.0	1.75 3.25
E	TMVW+w	MT20	2.0	4.0	
F	TTVW+m	MT20	5.0	8.0	1.75 3.00
G	TMVW+1	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
I	BMVW+1+p	MT20	4.0	6.0	
J	BMVW+1	MT20	4.0	4.0	
K	BS+1	MT20	3.0	6.0	
L	BMVW+1	MT20	4.0	9.0	
M	BMVW+1	MT20	4.0	4.0	
N	BMVW+1	MT20	5.0	6.0	
O	BMV+1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
		FACTORED		MAXIMUM FACTORED			INPUT	REQD	
		GROSS REACTION		GROSS REACTION			BRG	BRG	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
O		2057	0	2057	0	0	5-8	5-8	
I		1916	0	1916	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-1.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM/LIVE	WIND	DEAD	SOIL
O	1528	871 / 0	288 / 0	0 / 0	371 / 0	0 / 0
I	1452	791 / 0	288 / 0	0 / 0	354 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	N-C	-259 / 34	0.09 (1)
B-C	-2017 / 0	-102.1	-102.1	0.31 (1)	4.49	C-M	-242 / 0	0.17 (1)
C-D	-1873 / 0	-102.1	-102.1	0.29 (1)	4.63	M-D	0 / 403	0.08 (2)
D-E	-1726 / 0	-102.1	-102.1	0.50 (1)	4.49	D-L	0 / 415	0.09 (1)
E-F	-1726 / 0	-102.1	-102.1	0.50 (1)	4.49	L-E	-736 / 0	0.87 (1)
F-G	-1610 / 0	-102.1	-102.1	0.17 (1)	5.07	L-F	0 / 784	0.17 (1)
G-H	0 / 23	-102.1	-102.1	0.18 (1)	10.00	J-F	-17 / 150	0.03 (3)
O-B	-1982 / 0	0.0	0.0	0.21 (1)	5.89	J-G	0 / 359	0.08 (1)
I-H	-134 / 0	0.0	0.0	0.02 (1)	7.81	B-N	0 / 1695	0.38 (1)
						G-I	-1987 / 0	0.94 (1)
O-N	0 / 0	-38.5	-38.5	0.12 (3)	10.00			
N-M	0 / 1838	-38.5	-38.5	0.39 (1)	10.00			
M-L	0 / 1476	-38.5	-38.5	0.36 (2)	10.00			
L-K	0 / 1266	-38.5	-38.5	0.48 (2)	10.00			
K-J	0 / 1266	-38.5	-38.5	0.48 (2)	10.00			
J-I	0 / 1069	-38.5	-38.5	0.44 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.50/1.00 (D-E:1), BC=0.48/1.00 (J-L:2), WB=0.94/1.00 (G-I:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

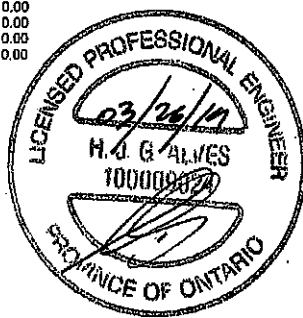
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

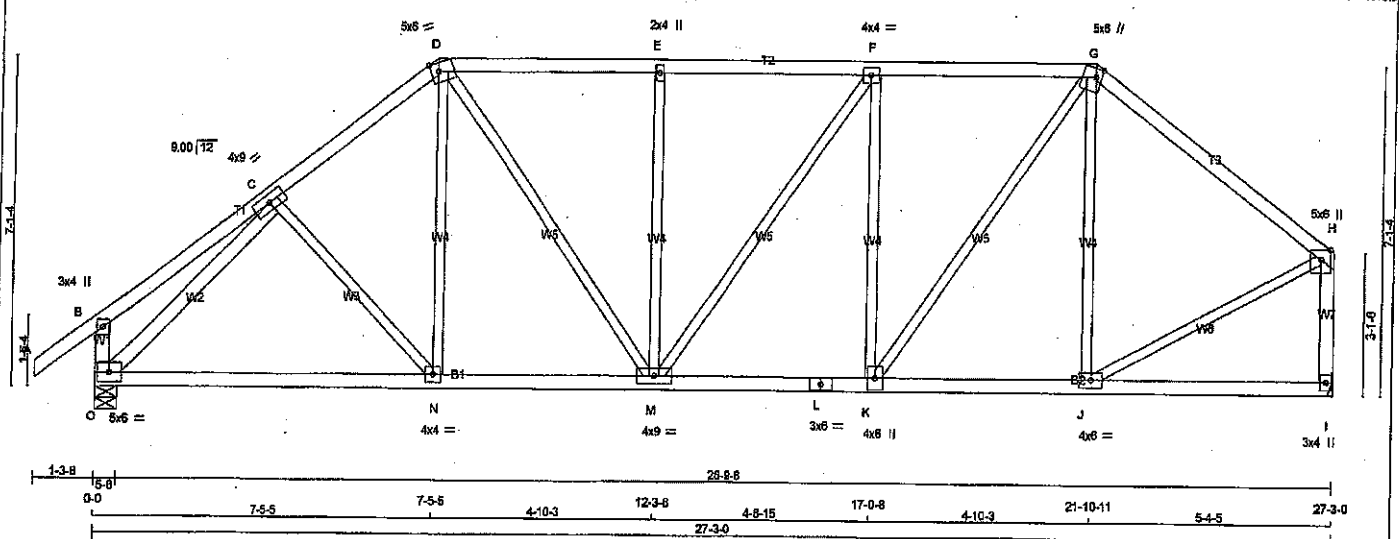
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.48 (I) (INPUT = 1.00)



DWG NO. TAM 1906471
STRUCTURAL
COMPONENT ONLY

JOB NAME 200164-400357	TRUSS NAME T17	QUANTITY 1	PLY 1	JOB DESC. Barton 12	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 26 17:46:31 2019 Page 1	
ID: ILZa865hX_OC9BuuR0TOTWJzPO-J2MPHkBg1lu324ogdF682507urvOumaZ90WjzX7el					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT O - C	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TTWW-t	MT20	4.0	8.0	
D	TTWW-m	MT20	5.0	6.0	Edge 2.00
E	TMV+w	MT20	2.0	4.0	
F	TMVW-t	MT20	4.0	4.0	
G	TTWW-m	MT20	5.0	6.0	2.25 1.50
H	TMVW-p	MT20	5.0	6.0	Edge
I	BMV1+p	MT20	3.0	4.0	
J	BMVW-t	MT20	4.0	6.0	
K	BMVW-m	MT20	4.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMVW-t	MT20	4.0	6.0	
N	BMVW-t	MT20	4.0	4.0	
O	BMVW-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	IN-SX	IN-SX	MECHANICAL		
O	2057	0	2057	0	0	5-8	5-8		
I	1916	0	1916	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 2-12.

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
O	1528	871/0	286/0	0/0	0/0	371/0	0/0
I	1492	791/0	286/0	0/0	0/0	354/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0/42	-102.1	-102.1 0.14 (1)	10.00	C-N	0/114	0.03 (3)
B-C	0/25	-102.1	-102.1 0.22 (1)	10.00	N-D	0/228	0.07 (2)
C-D	-1989/0	-102.1	-102.1 0.28 (1)	4.53	J-G	-398/0	0.35 (1)
D-E	-1934/0	-102.1	-102.1 0.38 (1)	4.44	O-C	-2255/0	0.77 (1)
E-F	-1934/0	-102.1	-102.1 0.38 (1)	4.43	J-H	0/1387	0.31 (1)
F-G	-1838/0	-102.1	-102.1 0.38 (1)	4.52	K-G	0/1043	0.23 (1)
G-H	-1550/0	-102.1	-102.1 0.62 (1)	4.50	D-M	0/651	0.15 (1)
O-B	-288/0	0.0	0.0 0.03 (1)	7.81	K-F	-898/0	0.59 (1)
I-H	-1831/0	0.0	0.0 0.31 (1)	6.19	M-E	-527/0	0.48 (1)
					M-F	0/169	0.04 (1)
O-N	0/1539	-38.5	-38.5 0.55 (2)	10.00			
N-M	0/1556	-38.5	-38.5 0.56 (2)	10.00			
M-L	0/1838	-38.5	-38.5 0.38 (1)	10.00			
L-K	0/1838	-38.5	-38.5 0.38 (1)	10.00			
K-J	0/1232	-38.5	-38.5 0.38 (2)	10.00			
J-I	0/0	-38.5	-38.5 0.20 (3)	10.00			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH. LL =	29.0	PSF	
DL =	6.0	PSF	
BOT CH. LL =	10.5	PSF	
DL =	7.0	PSF	
TOTAL LOAD =	52.5	PSF	
SPACING =	24.0	IN. GIG	

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/998 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/589 (0.28")

CSI: TO=0.62/1.00 (G-H:1), BC=0.58/1.00 (M-N:2), WB=0.77/1.00 (O-C:1), SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

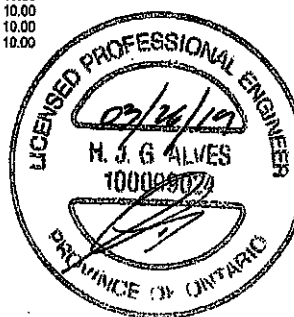
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 788 1987 1636

PLATE PLACEMENT TOL = 0.250 inches

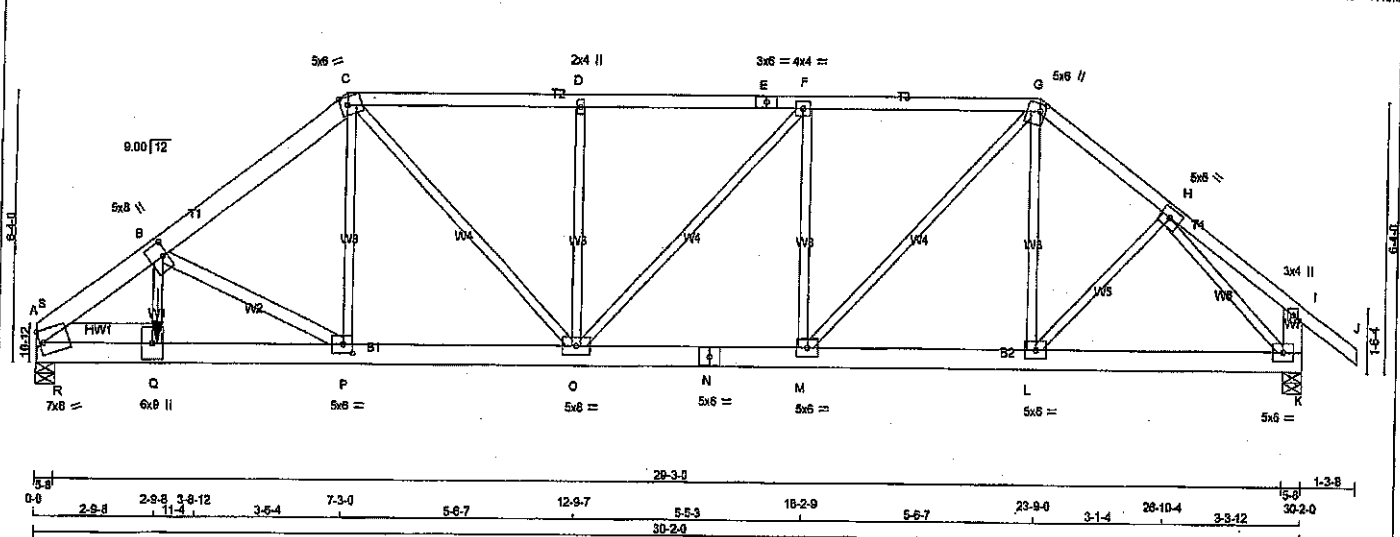
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.80 (C) (INPUT = 0.88)
JSI METAL = 0.61 (H) (INPUT = 1.00)



DWG NO. TAM 7906472
STRUCTURAL
COMPONENT ONLY

JOB NAME 200164-400357	TRUSS NAME T18	QUANTITY 1	PLY 2	JOB DESC. Barton 12	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 26 17:48:33 2019 Page 1	
ID:ILZa865hX_OC9Buur0T0TWjZpO-FRT9hPC5Zw8nIOx2kg8LDTAQ6V13lc31te7abzX7aG				Scale = 1:49.6	



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
K - L	2x6	DRY	No.2
A - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
REINFORCING MEMBERS			
HW1	2x6	DRY	No.2
ALL WEBS EXCEPT B - P			
2x3	DRY	No.2	SPF
2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	2	12
K-I	2	12
C-E	1	12
E-G	1	12
G-I	1	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
A-N	2	2
N-K	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
B-Q	1	6
2x3	1	6
2x4	1	6
2x6	2	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
A	TMBW1-m	MT20	7.0	8.0 Edge
B	TMBW1-m	MT20	5.0	8.0 4.0 1.25
C	TTWW-m	MT20	5.0	8.0 Edge 2.00
D	TMBW-w	MT20	2.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMBW1-m	MT20	4.0	4.0
G	TTWW-m	MT20	5.0	8.0 2.00 1.50
H	TMBW1-m	MT20	5.0	6.0
I	TMBW1-m	MT20	3.0	4.0
K	TMBW1-m	MT20	5.0	6.0
L	TMBW1-m	MT20	5.0	6.0
M	TMBW1-m	MT20	5.0	6.0
N	BS-t	MT20	5.0	6.0
O	TMBW1-m	MT20	5.0	8.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG
A	8195 0	8195 0	5-8
K	2833 0	2833 0	5-8

UNFACTORED REACTIONS			
JT	1ST LOASE COMBINED	MAX/MIN. COMPONENT REACTIONS	
A	6105 3435 / 0	1176 / 0	PERM. LIVE WIND DEAD SOIL
K	2107 1197 / 0	397 / 0	0 / 0 0 / 0 512 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

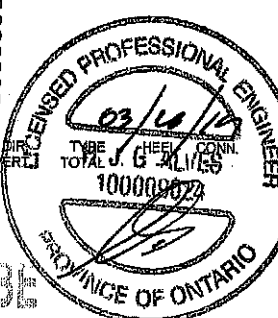
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
A-S	-9571 / 0	-102.1 -102.1	0.22 (1)
S-B	-10311 / 0	-102.1 -102.1	0.24 (1)
B-C	-5272 / 0	-102.1 -102.1	0.16 (1)
C-D	-4341 / 0	-102.1 -102.1	0.33 (1)
D-E	-4341 / 0	-102.1 -102.1	0.34 (1)
E-F	-4341 / 0	-102.1 -102.1	0.34 (1)
F-G	-3773 / 0	-102.1 -102.1	0.31 (1)
G-H	-3082 / 0	-102.1 -102.1	0.11 (1)
H-I	0 / 18	-102.1 -102.1	0.07 (1)
I-J	0 / 42	-102.1 -102.1	0.08 (1)
K-L	-274 / 0	0.0 0.0	0.01 (1)
WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
B-P	-4655 / 0	3.79	0.74 (1)
P-C	0 / 2573	3.54	0.32 (1)
C-O	0 / 145	4.96	0.02 (1)
O-D	-555 / 0	4.30	0.17 (1)
D-F	0 / 843	4.28	0.10 (1)
F-M	-1225 / 0	4.57	0.38 (1)
M-G	0 / 1946	5.18	0.24 (1)
L-G	0 / 177	4.57	0.02 (1)
O-B	0 / 4822	10.00	0.60 (1)
L-H	0 / 307	10.00	0.04 (2)
H-K	-3320 / 0	7.81	0.57 (1)
R-S	0 / 555	10.00	0.00 (1)
S-Q	0 / 3121	10.00	0.20 (1)

FACTORED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1	MAX- MAX+
Q	2-9-8	-5711	-5711



TOTAL WEIGHT = 2 X 158 = 317 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.01")

CALCULATED VERT. DEFL.(LL) = L/998 (0.08")

ALLOWABLE DEFL.(TL) = L/360 (1.01")

CALCULATED VERT. DEFL.(TL) = L/998 (0.14")

CSI: TC=0.34/1.00 (D-F:1), BC=0.88/1.00 (P-Q:1).

WB=0.74/1.00 (S-P:1), SS=0.19/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 768 1987 1665

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.89 (G) (INPUT = 0.90)

ISI METAL= 0.85 (B) (INPUT = 1.00)

DWG NO. TAM T906473

STRUCTURAL COMPONENT ONLY

1/2

CONTINUED ON PAGE 2

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	T18	1	2	Barton 12	

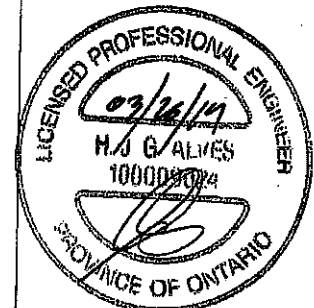
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 M/Tek Industries, Inc. Tue Mar 26 17:48:33 2019 Page 2
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PLATES (table is in inches)

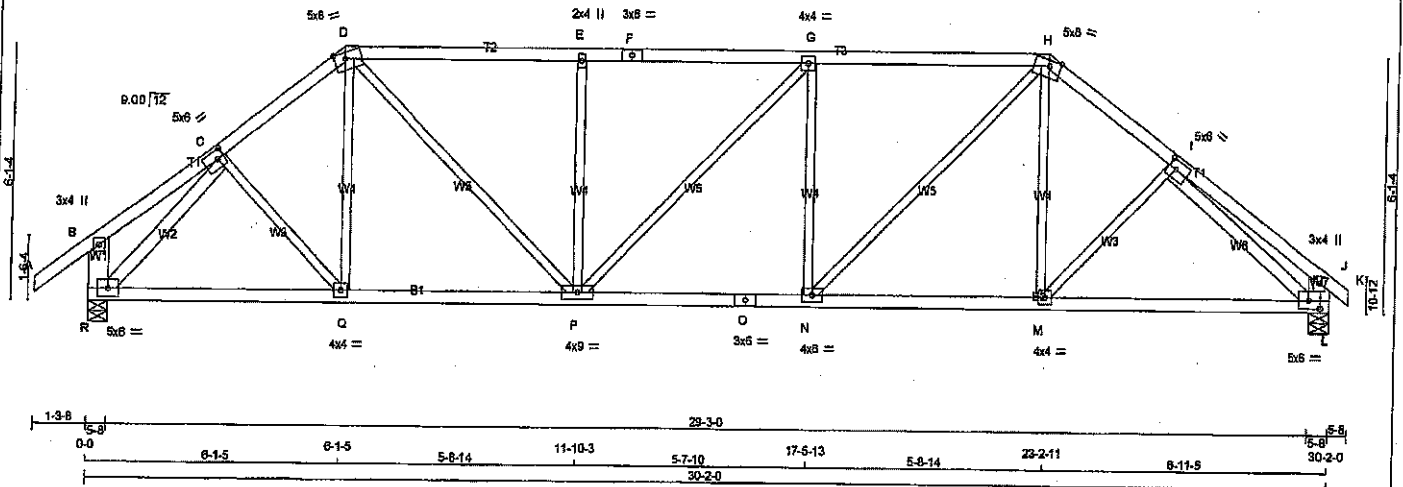
JT TYPE	PLATES	W	LEN	Y	X
P BMVW+1	MT20	5.0	6.0	2.50	2.75
Q BMVW+1	MT20	6.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.



DWG NO. TAM **T1906473**
STRUCTURAL
COMPONENT ONLY **7/2**

DWG NO. TAM 7906474
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x6	DRY	No.2
L - J	2x6	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	5.0	8.0	2.50	2.00
D TTWW-m	MT20	5.0	8.0	1.75	3.25
E TMW+w	MT20	2.0	4.0		
F TS-t	MT20	3.0	8.0		
G TMWW-t	MT20	4.0	4.0		
H TTWW-m	MT20	5.0	8.0	Edge	3.50
I TMWW-t	MT20	5.0	8.0	2.50	2.25
J TMV+p	MT20	3.0	4.0		
L BMVW1-t	MT20	5.0	8.0	2.25	3.00
M BMWW-t	MT20	4.0	4.0		
N BMWW-t	MT20	4.0	6.0		
O BS-t	MT20	3.0	6.0		
P BMWWW-t	MT20	4.0	9.0		
Q BMWW-t	MT20	4.0	4.0		
R BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD
	VERT	DOWN	BRG
R	2282	0	5-8
L	2176	0	5-8

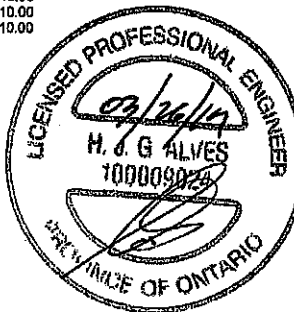
UNFACTORED REACTIONS			
JT	1ST CASE	MAX/MIN COMPONENT REACTIONS	
		SNOW	LIVE
R	1881	958 / 0	317 / 0
L	1623	908 / 0	317 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING			
TOTAL LOAD CASES: (4)			
CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LC)
FR-TO		FROM TO	LENGTH FR-TO
A-B	0 / 42	-102.1 -102.1	0.14 (1)
B-C	0 / 19	-102.1 -102.1	0.13 (1)
C-D	-2285 / 0	-102.1 -102.1	0.20 (1)
D-E	-2693 / 0	-102.1 -102.1	0.64 (1)
E-F	-2693 / 0	-102.1 -102.1	0.64 (1)
F-G	-2693 / 0	-102.1 -102.1	0.64 (1)
G-H	-2750 / 0	-102.1 -102.1	0.65 (1)
H-I	-2508 / 0	-102.1 -102.1	0.28 (1)
I-J	0 / 20	-102.1 -102.1	0.17 (1)
J-K	0 / 17	-102.1 -102.1	0.02 (1)
R-B	-258 / 0	0.0 0.0	0.02 (1)
L-J	-214 / 0	0.0 0.0	0.01 (1)
R-Q	0 / 1648	-38.5 -38.5	0.48 (2)
Q-P	0 / 1811	-38.5 -38.5	0.50 (2)
P-O	0 / 2750	-38.5 -38.5	0.55 (1)
O-N	0 / 2750	-38.5 -38.5	0.55 (1)
N-M	0 / 1889	-38.5 -38.5	0.58 (2)
M-L	0 / 2054	-38.5 -38.5	0.59 (2)



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/380$ (1.01")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.13")
ALLOWABLE DEFL. (TL) = $L/380$ (1.01")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.22")

CS: TC=0.65/1.00 (G-H-1), BC=0.59/1.00 (L-M-2), WB=0.77/1.00 (L-1), SS=0.27/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.89 (O) (INPUT = 1.00)

DWG NO. TAM R906475
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	T21	1	1	Barton 12	

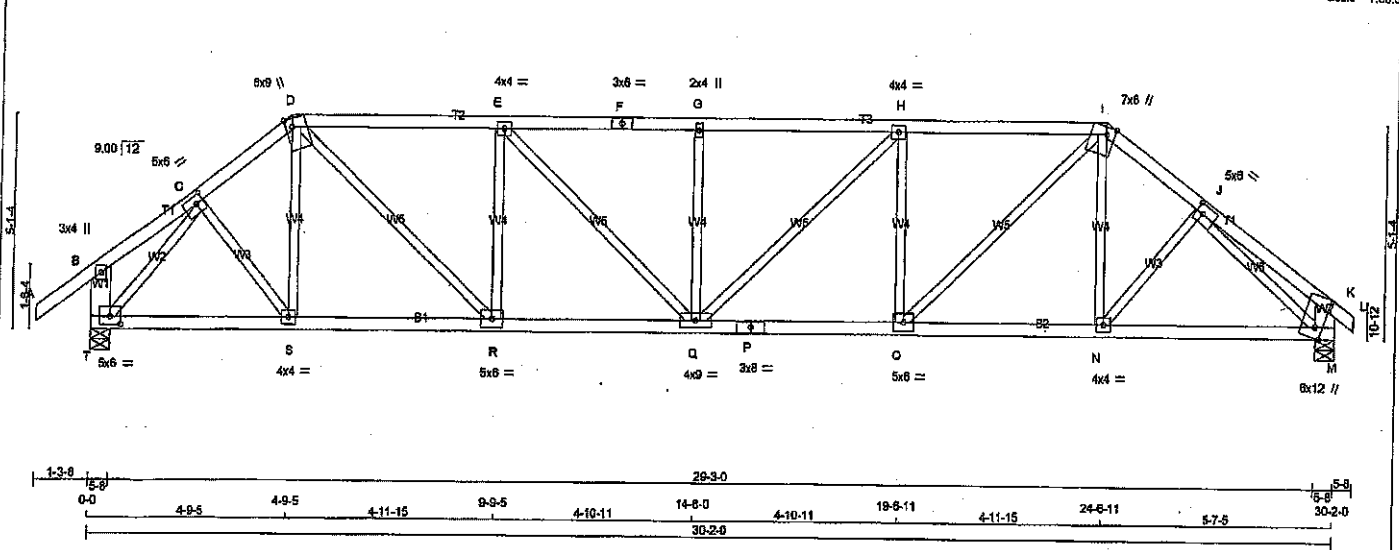
Tamarack Roof Truss, Burlington

Version 6.230 S Nov 17 2016 Mitek Industries, Inc. Tue Mar 26 17:49:36 2019 Page 1

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1-3-8 0-0 2-5-15 2-5-15 4-9-5 4-11-15 9-9-5 4-10-11 14-8-0 4-10-11 19-8-11 4-11-15 24-6-11 2-3-7 2-5 3-1-8 5-8

Scale = 1:50.8



TOTAL WEIGHT = 129 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - D	2x4	DRY	No.2	SPF	FACTORED	GROSS REACTION	MAXIMUM FACTORED	BRG	REQD	TOP CH. LL = 29.0 PSF	
D - F	2x4	DRY	No.2	SPF	JT	VERT	DOWN	UPLIFT	IN-SX	DL = 6.0 PSF	
F - I	2x4	DRY	No.2	SPF	T	2262	0	5-8	5-8	BOT CH. LL = 10.5 PSF	
I - L	2x4	DRY	No.2	SPF	M	2176	0	5-8	5-8	DL = 7.0 PSF	
T - B	2x6	DRY	No.2	SPF						TOTAL LOAD = 52.5 PSF	
M - K	2x6	DRY	No.2	SPF							
T - P	2x4	DRY	No.2	SPF							
P - M	2x4	DRY	No.2	SPF							

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

UNFACTORED REACTIONS

1ST CASE	MAX. LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1681	956 / 0	317 / 0	0 / 0	409 / 0
T	1623	908 / 0	317 / 0	0 / 0	399 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

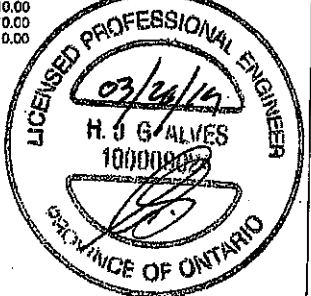
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MEMB.	MAX. FACTORED	FACTORED
	FORCE (LBS)	VERT. (PLF)	CSI (LC)	UNBRAC		FORCE (LBS)	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	C-S	0 / 393
B-C	-1 / 10	-102.1	-102.1	0.07 (1)	10.00	S-D	-85 / 88
C-D	-2270 / 0	-102.1	-102.1	0.14 (1)	4.44	N-I	0 / 261
D-E	-2978 / 0	-102.1	-102.1	0.63 (1)	3.55	T-C	-2482 / 0
E-F	-3368 / 0	-102.1	-102.1	0.57 (1)	3.31	N-J	0 / 89
F-G	-3559 / 0	-102.1	-102.1	0.57 (1)	3.31	J-M	-2774 / 0
G-H	-3559 / 0	-102.1	-102.1	0.57 (1)	3.31	O-I	0 / 1484
H-I	-3095 / 0	-102.1	-102.1	0.55 (1)	3.47	D-R	0 / 1630
I-J	-2578 / 0	-102.1	-102.1	0.16 (1)	4.18	O-H	-831 / 0
J-K	0 / 12	-102.1	-102.1	0.14 (1)	10.00	R-E	-947 / 0
K-L	0 / 17	-102.1	-102.1	0.02 (1)	10.00	Q-H	0 / 360
L-B	-246 / 0	0.0	0.0	0.02 (1)	7.81	E-Q	0 / 546
M-K	-199 / 0	0.0	0.0	0.01 (1)	7.81	Q-G	-460 / 0
T-S	0 / 1548	-38.5	-38.5	0.38 (2)	10.00		
S-R	0 / 1820	-38.5	-38.5	0.41 (1)	10.00		
R-Q	0 / 2877	-38.5	-38.5	0.58 (1)	10.00		
Q-P	0 / 3095	-38.5	-38.5	0.58 (1)	10.00		
P-O	0 / 3095	-38.5	-38.5	0.58 (1)	10.00		
O-N	0 / 2048	-38.5	-38.5	0.49 (2)	10.00		
N-M	0 / 2034	-38.5	-38.5	0.48 (2)	10.00		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



ALLOWABLE DEFL.(LL) = L/360 (1.01")

CALCULATED VERT. DEFL.(LL) = L/989 (0.16")

ALLOWABLE DEFL.(TL) = L/360 (1.01")

CALCULATED VERT. DEFL.(TL) = L/989 (0.27")

CSI: TC=0.57/1.00 (E-G:1), BC=0.58/1.00 (O-Q:1), WB=0.81/1.00 (J-H:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1587

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (T) (INPUT = 0.80)

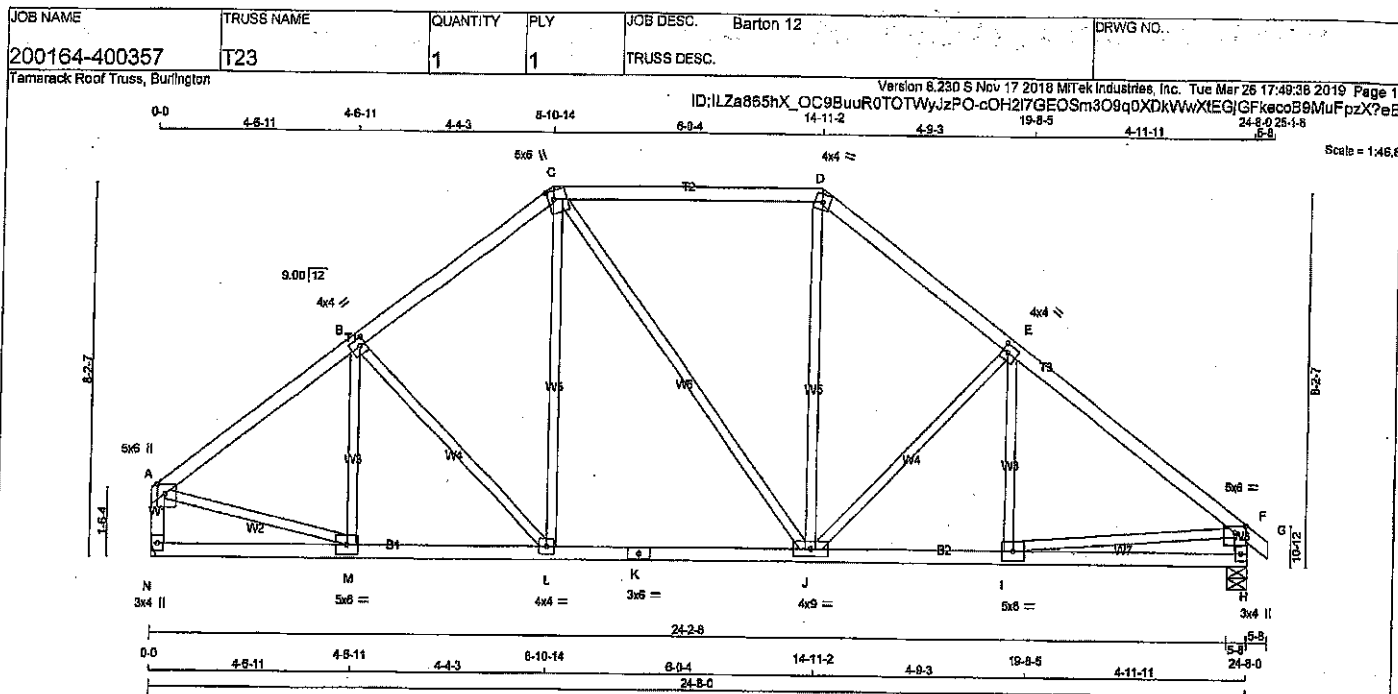
JSI METAL= 0.95 (P) (INPUT = 1.00)

DWG NO. TAM 7906476

STRUCTURAL

COMPONENT ONLY

TOWN OF CALEDON
BUILDING SECTION
FILE NO _____



TOTAL WEIGHT = 108 lb

LUMBER
N L G A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - A	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - K	2x4	DRY	No.2
K - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.50	2.00
B	TMVW-i	MT20	4.0	4.0	2.00	1.50
C	TTWV+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-i	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	5.0	6.0	1.75	3.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-i	MT20	5.0	6.0		
J	BMVWV-i	MT20	4.0	6.0		
K	BS-i	MT20	3.0	6.0		
L	BMVW-i	MT20	4.0	4.0		
M	BMVW-i	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	1734	0	1734	0	IN-SX	IN-SX
H	1790	0	1790	0	MECHANICAL	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N. MINIMUM BEARING LENGTH AT JOINT N = 2-2.

UNFACTORED REACTIONS

JT	1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM			
N	1296	716 / 0	259 / 0	0 / 0	0 / 0	321 / 0	0 / 0
H	1334	748 / 0	259 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED	FORCE	FACTORED	MAX.	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX.		
		(LBS)	VERT. LOAD	LC1	UNBRAC			(LBS)	CSI (LC)		
			(PLF)	CSI (LC)							
FR-TO			FROM	TO	LENGTH	FR-TO					
A-B	-1796 / 0		-102.1	-102.1	0.30 (1)	4.71	M-B	-203 / 60	0.07 (1)		
B-C	-1608 / 0		-102.1	-102.1	0.28 (1)	4.93	B-L	-293 / 0	0.22 (1)		
C-D	-1307 / 0		-102.1	-102.1	0.50 (1)	4.69	L-C	0 / 436	0.10 (2)		
D-E	-1689 / 0		-102.1	-102.1	0.35 (1)	4.77	C-J	0 / 73	0.02 (1)		
E-F	-2049 / 0		-102.1	-102.1	0.43 (1)	4.33	J-D	0 / 523	0.12 (2)		
F-G	0 / 17		-102.1	-102.1	0.02 (1)	10.00	J-E	-525 / 0	0.39 (1)		
N-A	-1659 / 0		0.0	0.0	0.17 (1)	6.44	I-E	0 / 185	0.04 (3)		
H-F	-1701 / 0		0.0	0.0	0.17 (1)	6.38	A-M	0 / 1511	0.34 (1)		
							I-F	0 / 1674	0.38 (1)		
N-M	0 / 0		-38.5	-38.5	0.13 (3)	10.00					
M-L	0 / 1482		-38.5	-38.5	0.36 (2)	10.00					
L-K	0 / 1263		-38.5	-38.5	0.33 (2)	10.00					
K-J	0 / 1263		-38.5	-38.5	0.33 (2)	10.00					
J-I	0 / 1885		-38.5	-38.5	0.39 (1)	10.00					
I-H	0 / 0		-38.5	-38.5	0.18 (3)	10.00					

A circular seal with the word "PROFESSIONAL" in a bold, serif font, arched across the top. The rest of the seal is partially cut off and obscured by the bottom edge of the image.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2010, OBC 2012
- CSA 086-08, CSA 089-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.82")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.82")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.50/1.00 (C-D:1), BC=0.39/1.00 (I-J:1), WB=0.39/1.00 (E-J:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

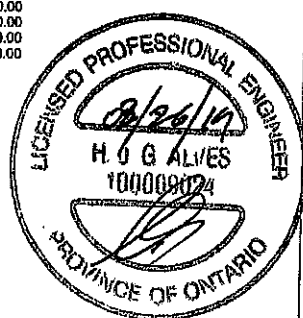
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 786 1967 1656

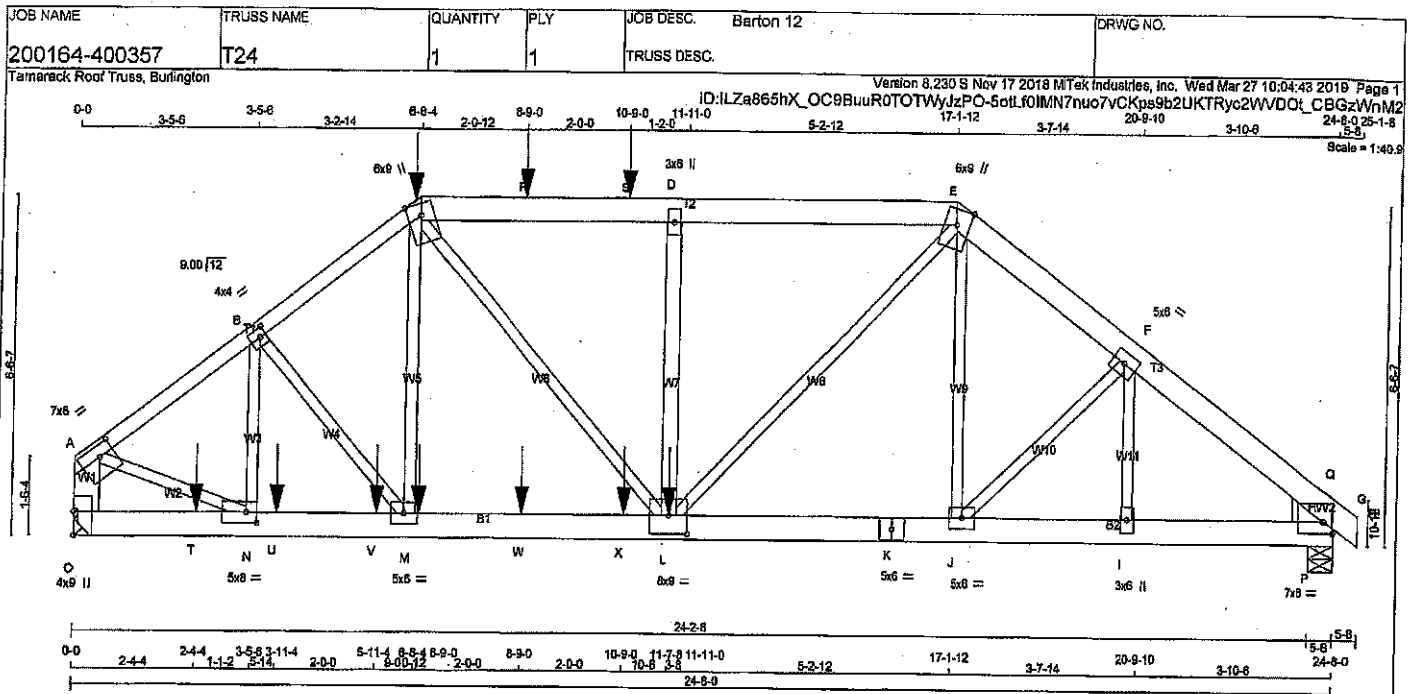
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.84 (D) (INPUT=0.80)
JSI METAL=0.51 (K) (INPUT=1.00)



DWG NO. TAM 11906478
STRUCTURAL
COMPONENT ONLY



LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - C	2x4	DRY No.2	SPF	
C - E	2x6	DRY No.2	SPF	
E - H	2x6	DRY No.2	SPF	
O - A	2x6	DRY No.2	SPF	
O - K	2x6	DRY 2100F 1.8E	SPF	
K - G	2x6	DRY 2100F 1.8E	SPF	
ALL WEBS EXCEPT L - D	2x3	DRY No.2	SPF	
	2x4	DRY No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	7.0	8.0	2.50	3.50
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMVW-t	MT20	3.0	6.0		
E	TTWW-m	MT20	6.0	9.0	Edge	
F	TMVW-t	MT20	5.0	6.0		
G	TMBH-t	MT20	7.0	8.0	2.75	2.00
I	BMVW-t	MT20	3.0	6.0		
J	BMVW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMVWW-t	MT20	8.0	8.0	4.25	4.50
M	BMVW-t	MT20	5.0	6.0		
N	BMVW-t	MT20	5.0	8.0	2.50	2.50
O	BMV-t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQD BRG IN-SX	HEEL WEDGE
O	3912	0	3912	0	5-8	5-8	2x6 R
G	2882	0	2882	0	5-8	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O. MINIMUM BEARING LENGTH AT JOINT O = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX MIN SNOW	MAX MIN LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	2899	1677 / 0	523 / 0	0 / 0	0 / 0	700 / 0	0 / 0
G	2141	1223 / 0	398 / 0	0 / 0	0 / 0	520 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO				FR-TO			
A-B	-4191 / 0	-102.1	-102.1 0.44 (1)	N-B	-810 / 0	0.15 (1)	
B-C	-4383 / 0	-102.1	-102.1 0.45 (1)	B-M	0 / 185	0.05 (1)	
C-R	-4148 / 0	-102.1	-102.1 0.29 (1)	M-C	0 / 855	0.21 (1)	
R-S	-4148 / 0	-102.1	-102.1 0.29 (1)	C-L	0 / 1073	0.27 (1)	
S-D	-4148 / 0	-102.1	-102.1 0.29 (1)	L-D	-641 / 0	0.29 (1)	
D-E	-4148 / 0	-102.1	-102.1 0.32 (1)	E-E	0 / 1893	0.47 (1)	
E-F	-3600 / 0	-102.1	-102.1 0.21 (1)	J-E	0 / 359	0.09 (2)	
F-Q	-3724 / 0	-102.1	-102.1 0.39 (1)	J-F	-58 / 0	0.02 (1)	
Q-G	-4778 / 0	-102.1	-102.1 0.42 (1)	I-F	-169 / 31	0.04 (1)	
G-H	0 / 3	-102.1	-102.1 0.01 (1)	A-N	0 / 3538	0.88 (1)	
O-A	-3755 / 0	0.0	0.0 0.27 (1)	P-Q	0 / 1282	0.00 (1)	
C-T	0 / 0	-38.5	-38.5 0.11 (1)				
T-N	0 / 0	-38.5	-38.5 0.11 (1)				
N-U	0 / 3358	-38.5	-38.5 0.29 (1)				
U-V	0 / 3358	-38.5	-38.5 0.29 (1)				
V-M	0 / 3358	-38.5	-38.5 0.29 (1)				
M-W	0 / 3473	-38.5	-38.5 0.31 (1)				
W-X	0 / 3473	-38.5	-38.5 0.31 (1)				
X-L	0 / 3473	-38.5	-38.5 0.31 (1)				
L-K	0 / 2871	-38.5	-38.5 0.20 (1)				
K-J	0 / 2871	-38.5	-38.5 0.20 (1)				
J-I	0 / 2871	-38.5	-38.5 0.25 (1)				
I-P	0 / 2917	-38.5	-38.5 0.33 (1)				
P-G	0 / 2917	-38.5	-38.5 0.33 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	6-8-4	-83	-92	FRONT	VERT	DEAD		
C	6-8-4	-18	-18	BACK	VERT	TOTAL		
C	6-8-4	-477	-477	FRONT	VERT	SNOW		
L	11-7-8	-1093	-1093	BACK	VERT	TOTAL		
M	6-9-0	-282	-282	BACK	VERT	TOTAL		
R	6-9-0	-11	-11	BACK	VERT	TOTAL		
S	10-6-0	-11	-11	BACK	VERT	TOTAL		
T	2-4-4	-282	-282	BACK	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

***** NON STANDARD GIRDER *****

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCRC 2018, OBC 2012
- CSA 089-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.J. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.82")
CALCULATED VERT. DEFL. (LL) = L/989 (0.11")
ALLOWABLE DEFL. (TL) = L/360 (0.82")
CALCULATED VERT. DEFL. (TL) = L/989 (0.18")

CSI: TC=0.45/1.00 (B-C:1), BC=0.33/1.00 (I-P:1), WB=0.88/1.00 (A-N:1), SSI=0.89/1.00 (G-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

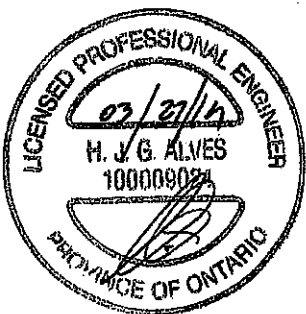
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1667
	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.81 (A) (INPUT = 1.00)

CONTINUED ON PAGE 2



DWG NO. TAM T120 6479
STRUCTURAL
COMPONENT ONLY

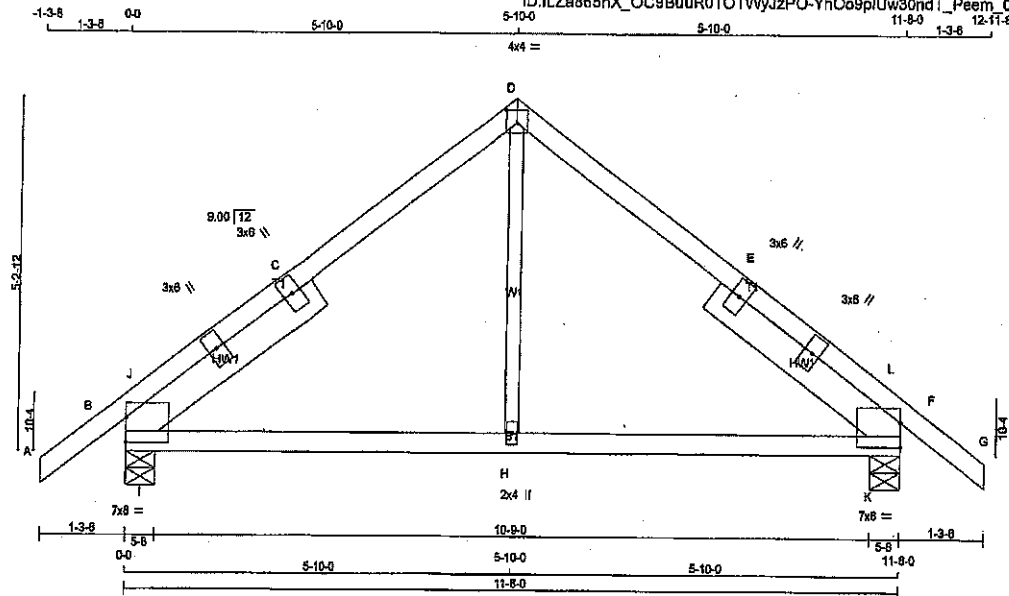
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																																												
200164-400357	T24	1	1	Barton 12																																																													
Taniareck Roof Truss, Burlington																																																																	
Version 8.230 S Nov 17 2018 MTek Industries, Inc. Wed Mar 27 10:04:44 2019 Page 2																																																																	
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<table border="1"><thead><tr><th colspan="10">FACTORED CONCENTRATED LOADS (LBS)</th></tr><tr><th>JT</th><th>LOC.</th><th>LC1</th><th>MAX-</th><th>MAX+</th><th>FACE</th><th>DIR.</th><th>TYPE</th><th>HEEL</th><th>CONN.</th></tr></thead><tbody><tr><td>U</td><td>3-11-4</td><td>-282</td><td>-282</td><td>—</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>—</td><td>—</td></tr><tr><td>V</td><td>5-11-4</td><td>-282</td><td>-282</td><td>—</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>—</td><td>—</td></tr><tr><td>W</td><td>8-9-0</td><td>-282</td><td>-282</td><td>—</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>—</td><td>—</td></tr><tr><td>X</td><td>10-9-0</td><td>-282</td><td>-282</td><td>—</td><td>BACK</td><td>VERT</td><td>TOTAL</td><td>—</td><td>—</td></tr></tbody></table>						FACTORED CONCENTRATED LOADS (LBS)										JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	U	3-11-4	-282	-282	—	BACK	VERT	TOTAL	—	—	V	5-11-4	-282	-282	—	BACK	VERT	TOTAL	—	—	W	8-9-0	-282	-282	—	BACK	VERT	TOTAL	—	—	X	10-9-0	-282	-282	—	BACK	VERT	TOTAL	—	—
FACTORED CONCENTRATED LOADS (LBS)																																																																	
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																																																								
U	3-11-4	-282	-282	—	BACK	VERT	TOTAL	—	—																																																								
V	5-11-4	-282	-282	—	BACK	VERT	TOTAL	—	—																																																								
W	8-9-0	-282	-282	—	BACK	VERT	TOTAL	—	—																																																								
X	10-9-0	-282	-282	—	BACK	VERT	TOTAL	—	—																																																								
LICENSED PROFESSIONAL ENGINEER 03/27/19 H. G. G. ALVES 100009024 PROVINCE OF ONTARIO DWG NO. TAM 7906479 STRUCTURAL COMPONENT ONLY 42																																																																	

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	T25	3	1	Barton 12	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 26 17:48:40 2019 Page 1

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Scale = 1:31.4

TOTAL WEIGHT = 3 X 51 = 152 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
B - F	2x4	DRY	No.2	SPF	
REINFORCING MEMBERS					
HW1	2x6	DRY	No.2	SPF	
HW2	2x6	DRY	No.2	SPF	
ALL WEBS 2x3 DRY: SEASONED LUMBER.					
	2x3	DRY	No.2	SPF	

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBMR1-I	7.0	8.0	4.25	
B, B, F, F	MT20	3.0	6.0		
D	RTW-p	4.0	4.0	2.25	2.00
F	TMBMR1-I	7.0	8.0	4.50	
H	BMW+w	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
B	981	0	0	5-8
F	981	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	710	419 / 0	123 / 0	0 / 0	0 / 0	168 / 0	0 / 0
F	710	419 / 0	123 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)	MAX. VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 24	-102.1	-102.1	0.14 (1)	10.00	H-D	0 / 389
B-J	-548 / 0	-102.1	-102.1	0.11 (1)	6.25	I-J	-138 / 284
J-C	-520 / 0	-102.1	-102.1	0.34 (1)	6.25	I-C	-388 / 0
C-D	-520 / 0	-102.1	-102.1	0.34 (1)	6.25	K-L	-138 / 284
D-E	-520 / 0	-102.1	-102.1	0.34 (1)	6.25	K-E	-388 / 0
E-L	-520 / 0	-102.1	-102.1	0.34 (1)	6.25		
L-F	-548 / 0	-102.1	-102.1	0.11 (1)	6.25		
F-G	0 / 24	-102.1	-102.1	0.14 (1)	10.00		
B-I	0 / 302	-38.5	-38.5	0.28 (1)	10.00		
I-H	0 / 540	-38.5	-38.5	0.43 (1)	10.00		
H-K	0 / 540	-38.5	-38.5	0.43 (1)	10.00		
K-F	0 / 302	-38.5	-38.5	0.28 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.6 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.38")
CALCULATED VERT. DEFL.(LL) = $L/889$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.38")
CALCULATED VERT. DEFL.(TL) = $L/889$ (0.07")

CSI: TC=0.34/1.00 (D-J:1), BC=0.43/1.00 (H-I:1),
WB=0.09/1.00 (D-H:2), SB=0.37/1.00 (F-K:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1655

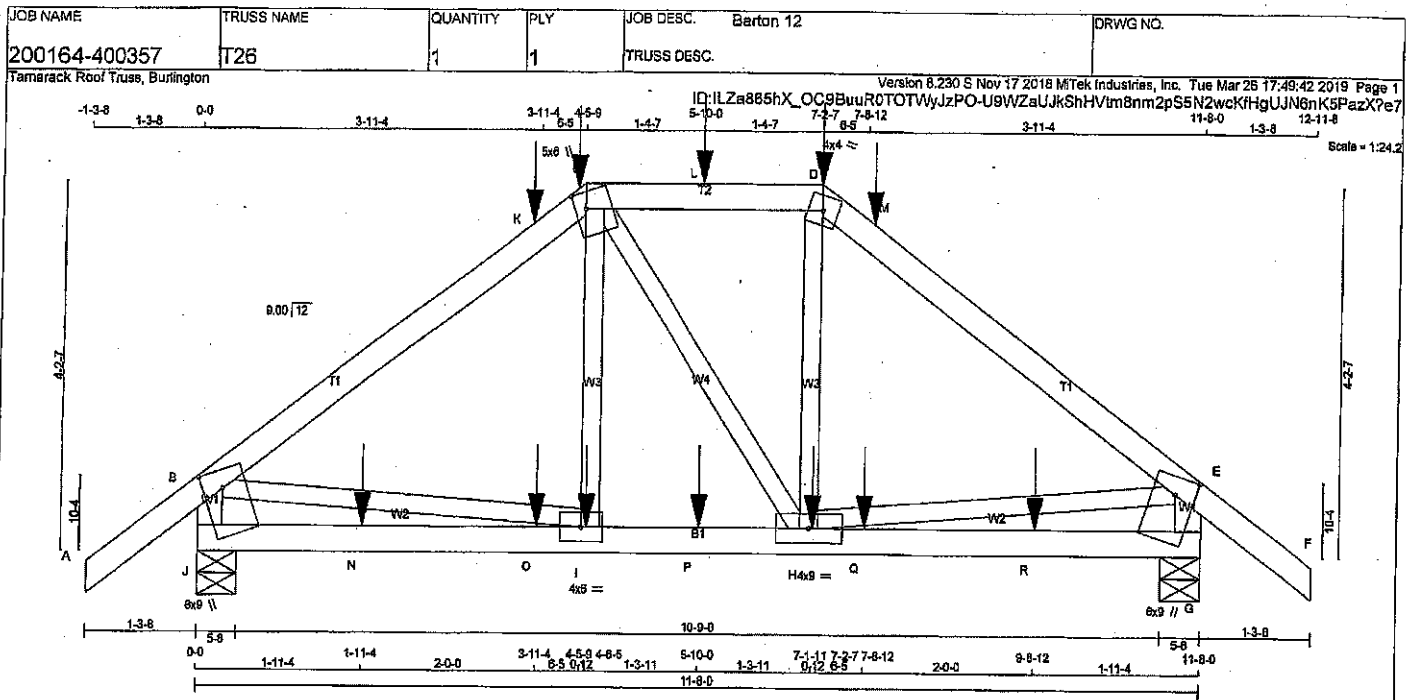
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (D) (INPUT = 0.50)
JSI METAL= 0.28 (F) (INPUT = 1.00)



DWG NO. TAM 77906480
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 51 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	E, G, J					
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
G	TMBWVW+m	MT20	6.0	9.0	2.50	2.75
H	BMWVW-l	MT20	4.0	9.0		
I	BMWVW-l	MT20	4.0	6.0		
J	TMBWVW+m	MT20	6.0	9.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
J	1433	0	1433	0	0	0	5-8	5-8	5-8
G	1432	0	1432	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	1053	636 / 0	168 / 0	0 / 0	0 / 0	249 / 0	0 / 0
G	1052	636 / 0	167 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MEMB.	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 42	-102.1 -102.1	0.15 (1)	10.00	I-C	0 / 222	0.05 (3)
B-K	-1376 / 0	-102.1 -102.1	0.48 (1)	4.89	C-H	0 / 5	0.00 (3)
K-C	-1378 / 0	-102.1 -102.1	0.48 (1)	4.89	H-D	0 / 227	0.06 (3)
C-L	-1118 / 0	-102.1 -102.1	0.21 (1)	5.71	B-I	0 / 1119	0.28 (1)
L-D	-1116 / 0	-102.1 -102.1	0.21 (1)	5.71	H-E	0 / 1119	0.28 (1)
D-M	-1377 / 0	-102.1 -102.1	0.48 (1)	4.89			
M-E	-1377 / 0	-102.1 -102.1	0.48 (1)	4.89			
E-F	0 / 42	-102.1 -102.1	0.15 (1)	10.00			
J-B	-1343 / 0	0.0 0.0	0.15 (1)	6.86			
G-E	-1342 / 0	0.0 0.0	0.15 (1)	6.86			
J-N	0 / 0	-38.5 -38.5	0.20 (3)	10.00	DING NO. TAM 11906 STRUCTURAL COMPONENT ONLY		
N-O	0 / 0	-38.5 -38.5	0.20 (3)	10.00			
O-I	0 / 0	-38.5 -38.5	0.20 (3)	10.00			
I-P	0 / 1114	-38.5 -38.5	0.32 (2)	10.00			
P-H	0 / 1114	-38.5 -38.5	0.32 (2)	10.00			
H-Q	0 / 0	-38.5 -38.5	0.20 (3)	10.00			
Q-R	0 / 0	-38.5 -38.5	0.20 (3)	10.00			
R-G	0 / 0	-38.5 -38.5	0.20 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-5-9	-38	-41		FRONT	VERT	DEAD		
C	4-5-9	-53	-53		BACK	VERT	TOTAL		
C	4-5-9	-206	-206		FRONT	VERT	SNOW		
D	7-2-7	-38	-41		FRONT	VERT	DEAD		
D	7-2-7	-53	-53		BACK	VERT	TOTAL		
D	7-2-7	-206	-206		FRONT	VERT	SNOW		
H	7-1-11	-24	-30		BACK	VERT	TOTAL		
H	4-8-5	-24	-30		BACK	VERT	TOTAL		
K	3-11-4	-71	-71		BACK	VERT	TOTAL		
L	5-10-0	-46	-46		BACK	VERT	TOTAL		
M	7-8-12	-71	-71		BACK	VERT	TOTAL		
N	1-11-4	-24	-30		BACK	VERT	TOTAL		
O	3-11-4	-24	-30		BACK	VERT	TOTAL		
P	5-10-0	-24	-30		BACK	VERT	TOTAL		
Q	7-8-12	-24	-30		BACK	VERT	TOTAL		
R	9-8-12	-24	-30		BACK	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.39")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.39")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.48/1.00 (D-E:1), BC=0.32/1.00 (H-I:2),
WB=0.28/1.00 (E-H:1), SI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

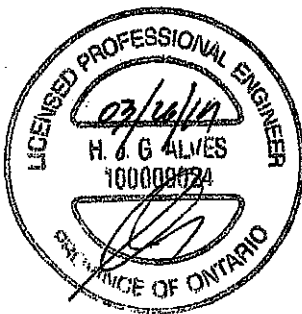
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.77 (D) (INPUT = 0.90)
JSI METAL= 0.48 (G) (INPUT = 1.00)



DWG NO. TAM 7906481
STRUCTURAL
COMPANION ONLY

Tamarack Roof Truss, Burlington

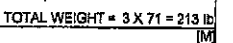
Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Mar 28 17:48:43 2019 Page 1

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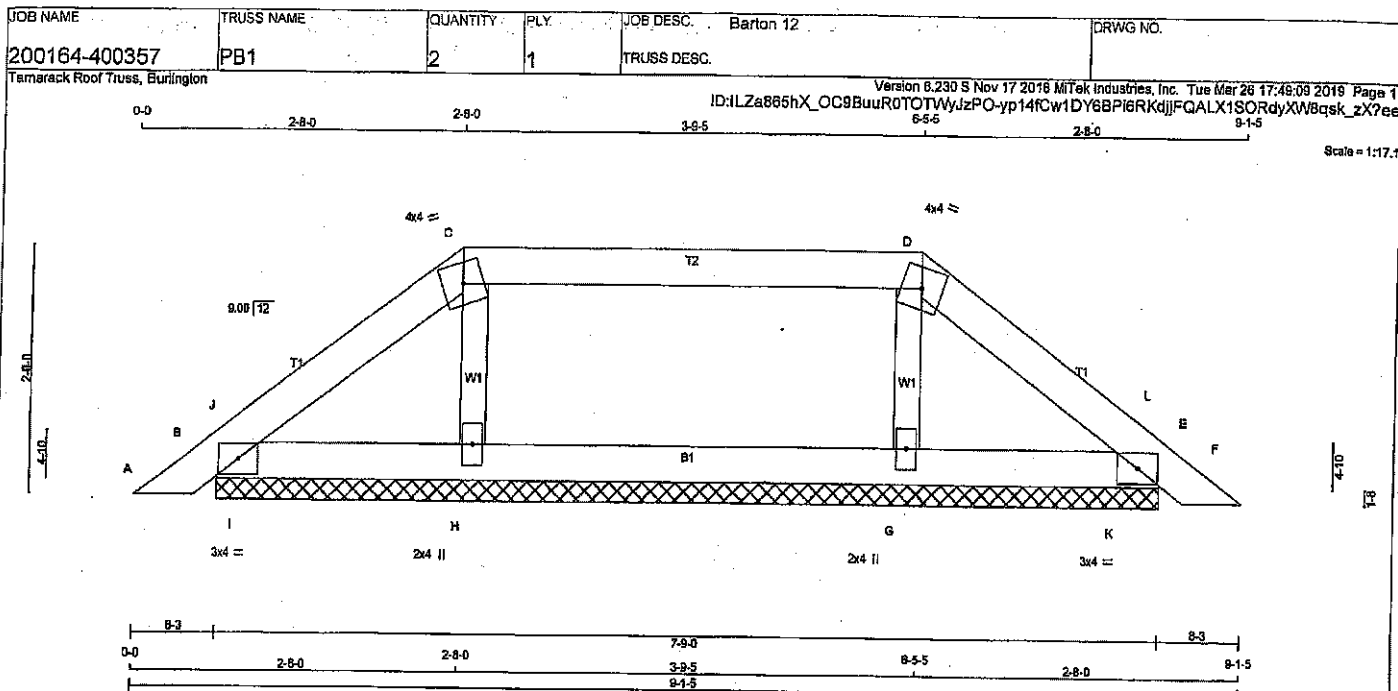
1-3-8 1-3-8 0-0 3-10-0 3-10-0 3-7-8 7-5-8 3-7-8 11-1-0 3-10-0 14-11-0 16-2-8 1-3-8

SW6 H

Scale = 1:30.1



DWG NO. TAM 77906483
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 23 = 46 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER,			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TM81-I	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
B	231	0	0	7-9-0
E	231	0	0	7-9-0
H	375	0	0	7-9-0
G	375	0	0	7-9-0

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	164	117/0	13/0	0/0	0/0	0/0	33/0	0/0
E	164	117/0	13/0	0/0	0/0	0/0	33/0	0/0
H	285	142/0	68/0	0/0	0/0	0/0	75/0	0/0
G	285	142/0	68/0	0/0	0/0	0/0	75/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLP)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO						FR-TO		
A-B	0/17	-102.1	-102.1	0.03 (1)	10.00	H-C	-231/0	0.03 (1)
B-J	-81/0	-102.1	-102.1	0.01 (1)	6.25	G-D	-231/0	0.03 (1)
J-C	-77/0	-102.1	-102.1	0.05 (1)	6.25	I-J	-125/0	0.00 (1)
C-D	-44/0	-102.1	-102.1	0.25 (1)	6.25	K-L	-125/0	0.00 (1)
D-L	-77/0	-102.1	-102.1	0.05 (1)	6.25			
L-E	-81/0	-102.1	-102.1	0.01 (1)	6.25			
E-F	0/17	-102.1	-102.1	0.03 (1)	10.00			
B-I	0/59	-38.5	-38.5	0.06 (1)	10.00			
I-H	0/59	-38.5	-38.5	0.07 (2)	10.00			
H-G	0/44	-38.5	-38.5	0.07 (2)	10.00			
G-K	0/59	-38.5	-38.5	0.07 (2)	10.00			
K-E	0/59	-38.5	-38.5	0.06 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2015
- CSA C88-09, CSA C88-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-D-1), BC=0.07/1.00 (H-I-2), WB=0.03/1.00 (C-H-1), SS=0.15/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

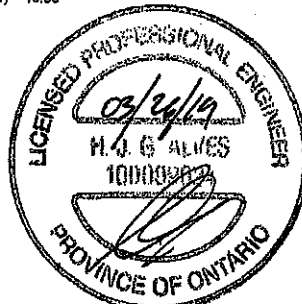
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.18 (B) (INPUT = 0.90)
JSI METAL = 0.05 (H) (INPUT = 1.00)

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



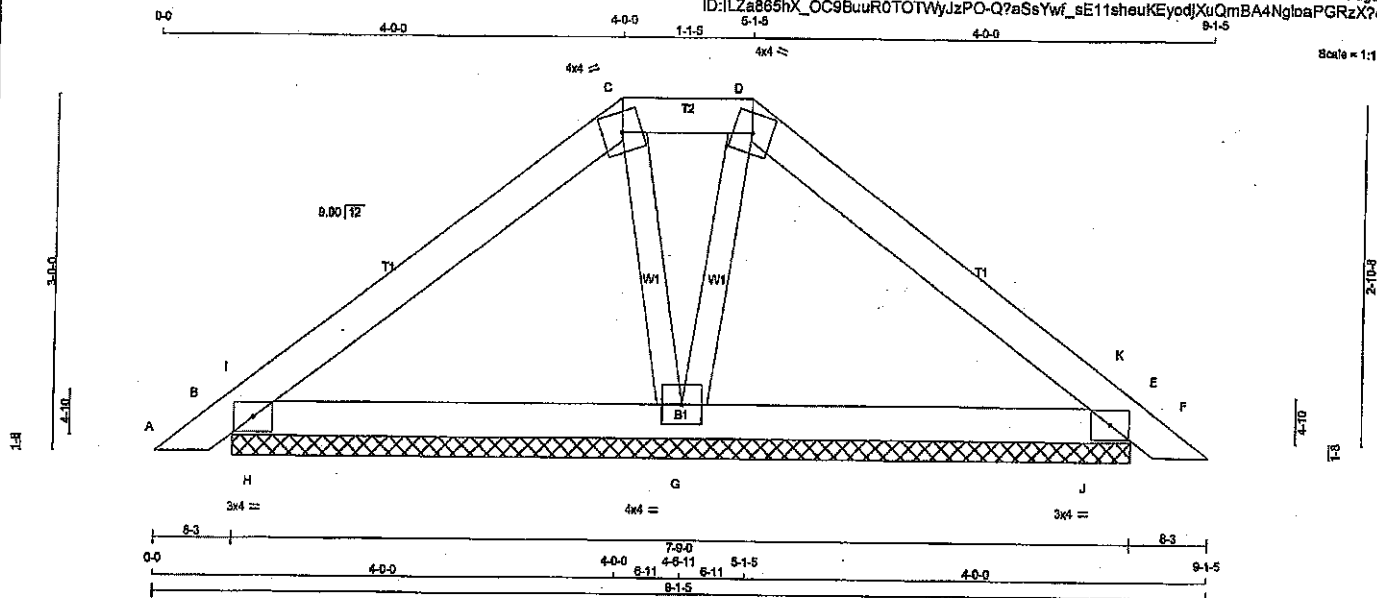
DWG NO. TAM T1206453
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	PB2	2	1	Barton 12	

Tamarack Roof Truss, Burlington

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ID: ILZa865hX_OC9BuuR0T0TWyJzPO-Q7aSSywf_se11sheuKeyodjXuQmBA4NgbaPGRzX7ed

Scale = 1:18.0



TOTAL WEIGHT = 2 X 25 = 51 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	SPF
C - D	2x4	DRY	SPF
D - F	2x4	DRY	SPF
B - E	2x4	DRY	SPF
ALL WEBS	2x3	DRY	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT		
B	368	0	368	0	7-9/0	7-9/0
E	357	0	357	0	7-9/0	7-9/0
G	486	0	486	0	7-9/0	7-9/0

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND		
B	288	166 / 0	39 / 0	0 / 0	81 / 0	0 / 0
E	280	184 / 0	38 / 0	0 / 0	59 / 0	0 / 0
G	369	187 / 0	66 / 0	0 / 0	95 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	MEMB.	WEBS		MAX. FACTORED
	FORCE (LBS)	VERT. LOAD LC1	PLF (PLF)	CS1 (LC)			FORCE (LBS)	CS1 (LC)	
FR-TO						FR-TO			
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	C-G	-124 / 0	0.02 (1)	
B-I	-108 / 0	-102.1	-102.1	0.03 (1)	6.25	G-D	-127 / 0	0.02 (1)	
I-C	-143 / 0	-102.1	-102.1	0.15 (1)	6.25	H-I	-249 / 89	0.00 (1)	
C-D	-80 / 0	-102.1	-102.1	0.02 (1)	6.25	J-K	-270 / 49	0.00 (1)	
D-K	-135 / 0	-102.1	-102.1	0.14 (1)	6.25				
K-E	-80 / 0	-102.1	-102.1	0.04 (1)	6.25				
E-F	0 / 17	-102.1	-102.1	0.03 (1)	10.00				
B-H	0 / 110	-38.5	-38.5	0.12 (1)	10.00				
H-G	0 / 110	-38.5	-38.5	0.15 (2)	10.00				
G-J	0 / 103	-38.5	-38.5	0.16 (2)	10.00				
J-E	0 / 103	-38.5	-38.5	0.12 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-K), BC=0.16/1.00 (G-H-2), WB=0.02/1.00 (D-G-1), SSI=0.23/1.00 (E-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

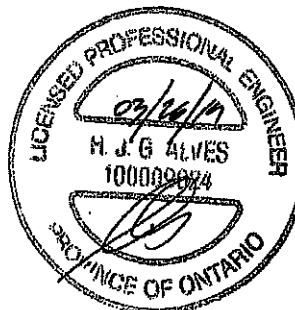
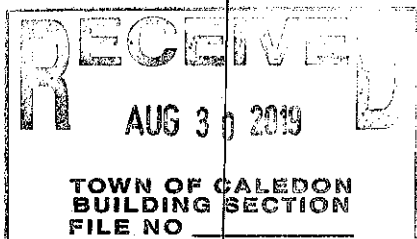
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 786

PLATE PLACEMENT TOL. = 0.250 inches

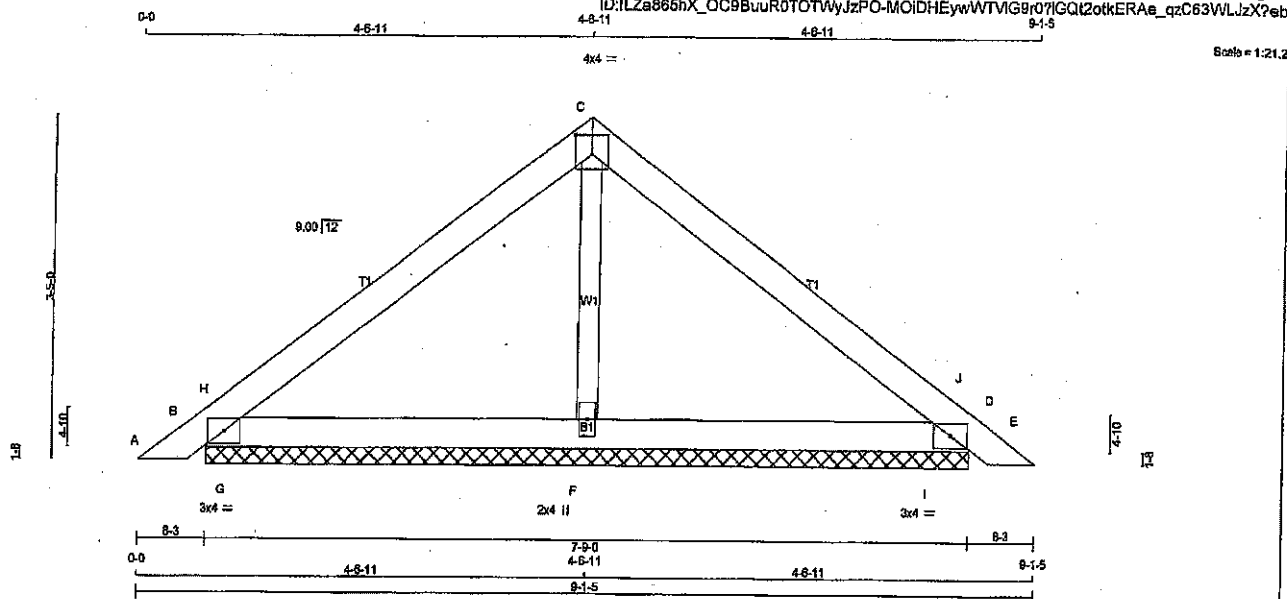
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.00)
JSI METAL= 0.07 (B) (INPUT = 1.00)



DWG NO. TAM 17106454
STRUCTURAL
COMPONENT ONLY

JOB NAME 200164-400357	TRUSS NAME PB3	QUANTITY 2	PLY. 1	JOB DESC. Barton 12	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 26 17:48:12 2018 Page 1	



TOTAL WEIGHT = 2 X 24 = 48 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	UP	BRG	BRG
B	402	0	0	7-9-0	7-9-0
D	402	0	0	7-9-0	7-9-0
F	406	0	0	7-9-0	7-9-0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	PERM	LIVE	WIND	DEAD	SOIL
B	282	189/0	39/0	0/0	0/0	0/0	85/0	0/0
D	292	189/0	39/0	0/0	0/0	0/0	85/0	0/0
F	314	142/0	86/0	0/0	0/0	0/0	86/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LC1 MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO				FR-TO			
A-B	0/17	-102.1	-102.1 0.03 (1)	F-C	-133/0	0.03 (1)	
B-H	-83/0	-102.1	-102.1 0.08 (1)	G-H	-415/27	0.00 (1)	
H-C	-178/0	-102.1	-102.1 0.19 (1)	I-J	-415/27	0.00 (1)	
C-J	-178/0	-102.1	-102.1 0.18 (1)				
J-D	-83/0	-102.1	-102.1 0.08 (1)				
D-E	0/17	-102.1	-102.1 0.03 (1)				
B-G	0/134	-38.5	-38.5 0.18 (1)				
G-F	0/134	-38.5	-38.5 0.20 (1)				
F-I	0/134	-38.5	-38.5 0.20 (1)				
I-D	0/134	-38.5	-38.5 0.18 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 28.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.B.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD.

CS1: TC=0.19/1.00 (C-H:1), BC=0.20/1.00 (F-I:1), WB=0.03/1.00 (C-F:1), SS=0.33/1.00 (D-I:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

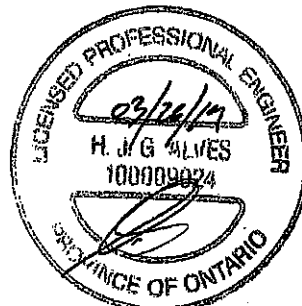
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

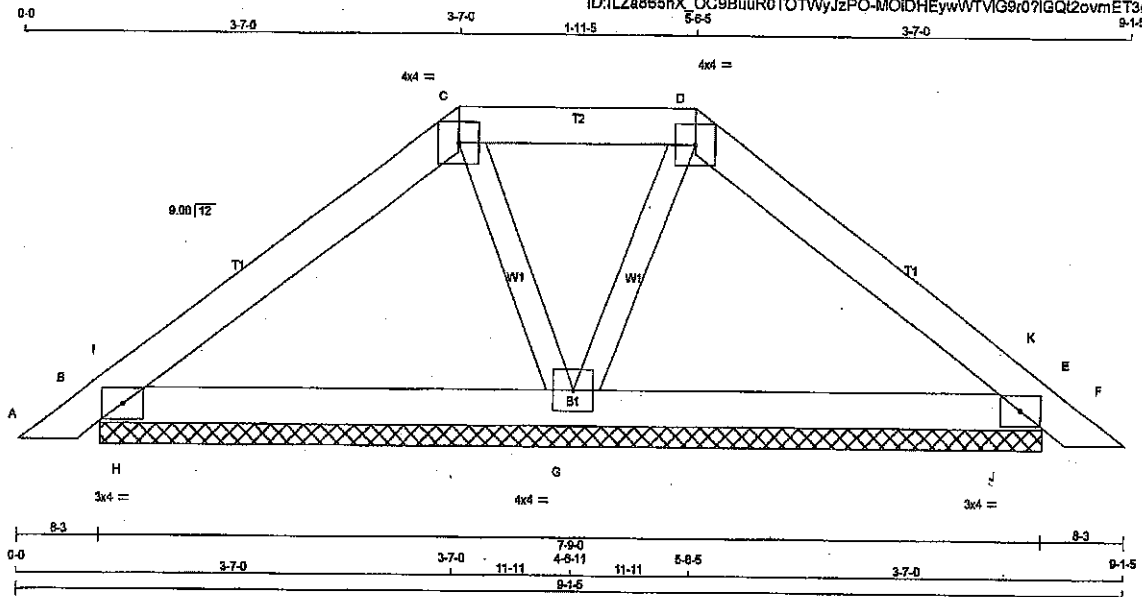
JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DRWG NO. TAM **T1906455**
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	PB4	1	2	Barton 12	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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Scale = 1:17.1

TOTAL WEIGHT = 2 X 25 = 50 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY No.2 SPF					
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	TOP
C-D 1	12	TOP
D-F 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
B-E 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW4	MT20	4.0	4.0		
D	TTW4	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	UNFACTORED REACTIONS		MAXIMUM FACTORED REACTIONS		INPUT BRG	REQD BRG
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION		
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
B	346	0	346	0	7-9-0	7-9-0
E	346	0	346	0	7-9-0	7-9-0
G	519	0	519	0	7-9-0	7-9-0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	253	158/0	38/0	0/0	0/0	58/0	0/0
E	253	158/0	38/0	0/0	0/0	58/0	0/0
G	392	204/0	87/0	0/0	0/0	100/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRAC	MEMB.	WEBS		MAX. FACTORED
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	VERT. LOAD (PLF)	CS1 (LC)			MAX. FORCE (LBS)	MAX. CS1 (LC)	
FR-TO						FR-TO			
A-B	0/17	-102.1	-102.1	0.01 (1)	10.00	C-G	-163/0	0.01 (1)	
B-I	-138/0	-102.1	-102.1	0.02 (3)	6.25	G-D	-163/0	0.01 (1)	
I-C	-138/0	-102.1	-102.1	0.08 (1)	6.25	H-I	-158/81	0.00 (1)	
C-D	-43/0	-102.1	-102.1	0.03 (1)	6.25	J-K	-158/81	0.00 (1)	
D-K	-136/0	-102.1	-102.1	0.06 (1)	6.25				
K-E	-138/0	-102.1	-102.1	0.02 (3)	6.25				
E-F	0/17	-102.1	-102.1	0.01 (1)	10.00				
B-H	0/107	-38.5	-38.5	0.04 (1)	10.00				
H-G	0/107	-38.5	-38.5	0.07 (2)	10.00				
G-J	0/107	-38.5	-38.5	0.07 (2)	10.00				
J-E	0/107	-38.5	-38.5	0.04 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.08/1.00 (C-I), EC=0.07/1.00 (G-H/2), WB=0.01/1.00 (C-G), SS=0.03/1.00 (B-H/1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

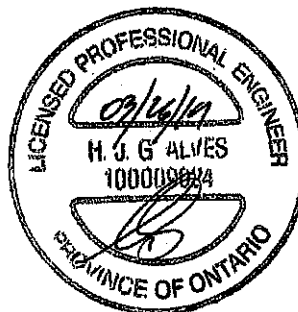
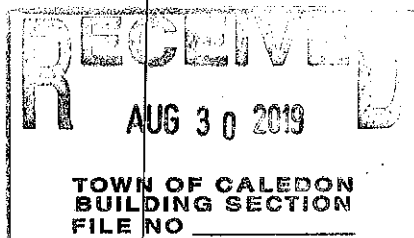
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1687 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.80)
JSI METAL= 0.03 (B) (INPUT = 1.00)

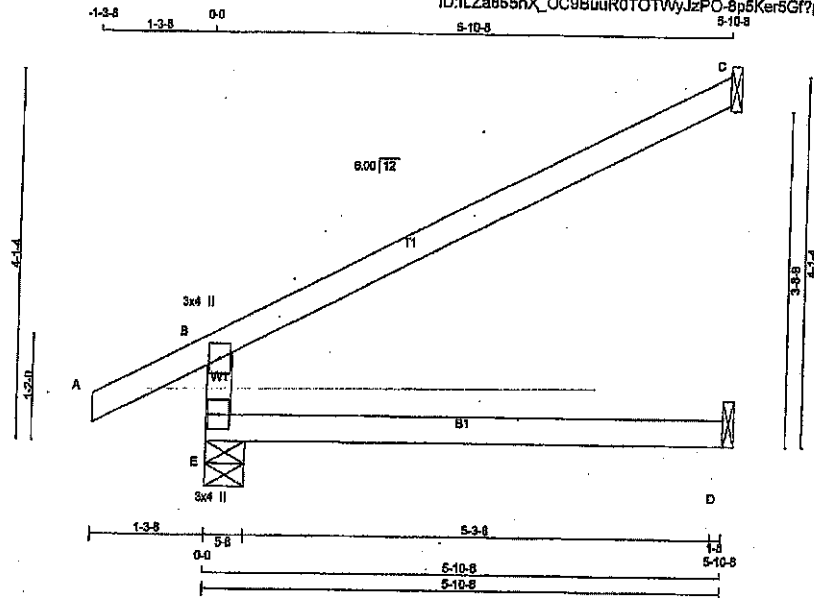


DWG NO. TAM 11906456
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	J1	9	1	Barton 12	
Tamarack Roof Truss, Burlington					
TRUSS DESC.					

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ID:ILZa865hX_OC9BuuR0T0TWyJzPO-8p5Ker5Gf?pgnzaDly_m1F9m2Y9asIZFn7Yt5WzWgz1

Scale = 1/2" = 1'-0"



TOTAL WEIGHT = 5 X 17 = 151 LB

LUMBER

N. L. G. A. RULES:
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	646	0	646	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	93	0	119	0	1-8	1-8

SEE MITEK STANDARD DETAIL 837821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	282 / 0	72 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. CSI (LC)
		FROM	TO				
E-B	-513 / 0	0.0	0.0	0.22 (3)	7.81		
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00		
B-C	-34 / 0	-102.1	-102.1	0.60 (1)	8.25		
E-D	0 / 0	-38.5	-38.5	0.22 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/942 (0.07")

CSI.TC=0.80/1.00 (B-C-1), BC=0.22/1.00 (D-E-3),
WS=0.00/1.00 (n/a-0), CSI=0.28/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

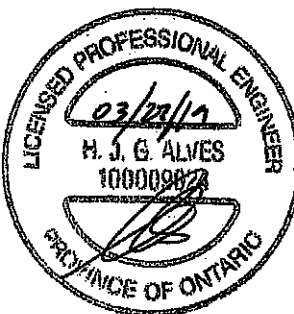
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PS)	SECTION (PLI)
MT20	818	354	1687

PLATE PLACEMENT TOL = 0.250 inches

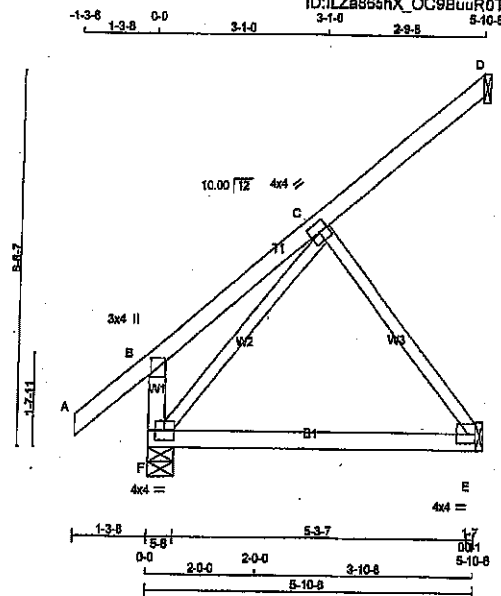
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.21 (E) (INPUT = 0.90)
JSI METAL=0.14 (B) (INPUT = 1.00)



DRWG NO. TAM 17406517
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	J2	6	1	Barton 12	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed Mar 27 17:21:01 2019 Page 1	



Scale = 1:37.0

TOTAL WEIGHT = 6 X 28 = 168 lb

LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS	2x4	No.2	SPF
F - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
F - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			SPF

PLATES (table in inches)	TYPE	PLATES	W	LEN	Y	X
JT	TMV-p	MT20	3.0	4.0		
B	TMV-p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
E	BMW1-t	MT20	4.0	4.0		Edge
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX
JT	VERT	DOWN	HORIZ	UPLIFT
F	551	0	0	5-8
D	113	0	0	1-8
E	301	0	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT	REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	403	280 / 0	61 / 0	0 / 0	0 / 0	92 / 0	0 / 0
D	78	64 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
E	231	107 / 0	61 / 0	0 / 0	0 / 0	63 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	FORCE
(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO		FROM	TO	LENGTH	FR-TO		
F-B	-258 / 0	0.0	0.0	0.03 (1)	7.81	F-C	-233 / 0
A-B	0 / 45	-102.1	-102.1	0.14 (1)	10.00	C-E	-237 / 0
B-C	0 / 23	-102.1	-102.1	0.15 (1)	10.00		
C-D	-23 / 0	-102.1	-102.1	0.14 (1)	6.25		
F-E	0 / 144	-38.5	-38.5	0.31 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOY CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/957$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/574$ (0.12")

CSI: TO=0.15/1.00 (B-C1), BC=0.31/1.00 (E-F:3), WB=0.09/1.00 (C-E:1), SSI=0.12/1.00 (E-F:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

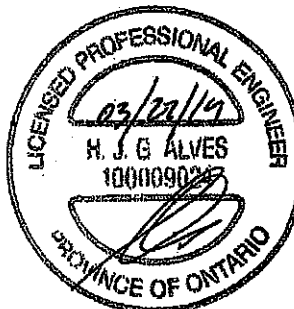
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 768

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

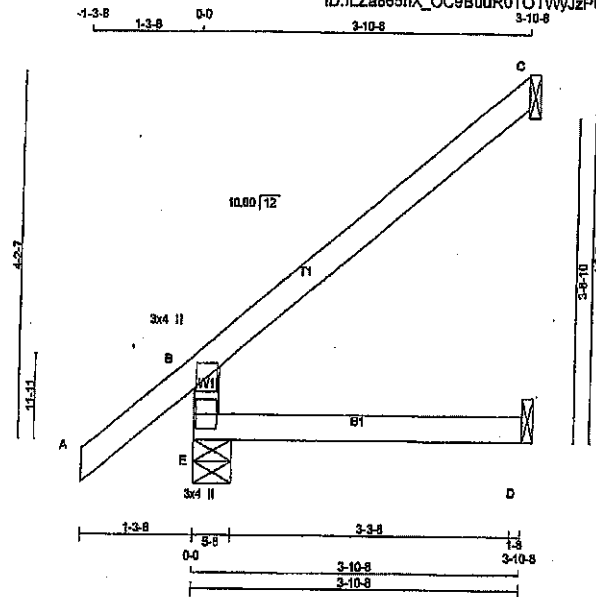
JSI GRIP= 0.27 (C) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DRWG NO. TAM 1906518
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	J3	7	1	Barton 12	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 6.230 S Nov 17 2018 Mitek Industries, Inc. Wed Mar 27 17:21:03 2019 Page 1
 ID: ILZa865hX_OC9BuR0TOTWJzPO-ZOnTGs79ywBFsQJo_4XTumMbiDA36JhTznY3rzWgz



Scale = 1/24.4

TOTAL WEIGHT = 7 X 13 = 91 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	478	0	478	0	5-8	5-8
C	148	0	148	0	1-8	1-8
D	62	0	78	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	348	221 / 0	48 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	102	84 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	55	0 / 0	34 / 0	0 / 0	0 / 0	23 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	MEMB.	WEBS	
	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	UNBRAC			FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO		
E-B	-389 / 0	0.0	0.0	0.08 (3)	7.81			
A-B	0 / 45	-102.1	-102.1	0.14 (1)	10.00			
B-C	-32 / 0	-102.1	-102.1	0.26 (1)	6.25			
E-D	0 / 0	-38.5	-38.5	0.09 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
 ALLOWABLE DEFL. (TL) = L/360 (0.19")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CS1: TC=0.28/1.00 (B-C-1), BC=0.09/1.00 (D-E-3), WB=0.00/1.00 (n/a), SS=0.15/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

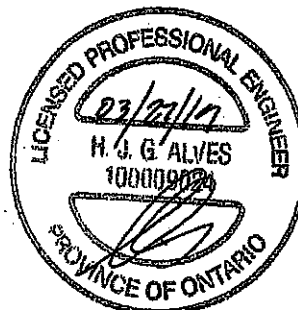
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788	1887 1888

PLATE PLACEMENT TOL. = 0.250 inches

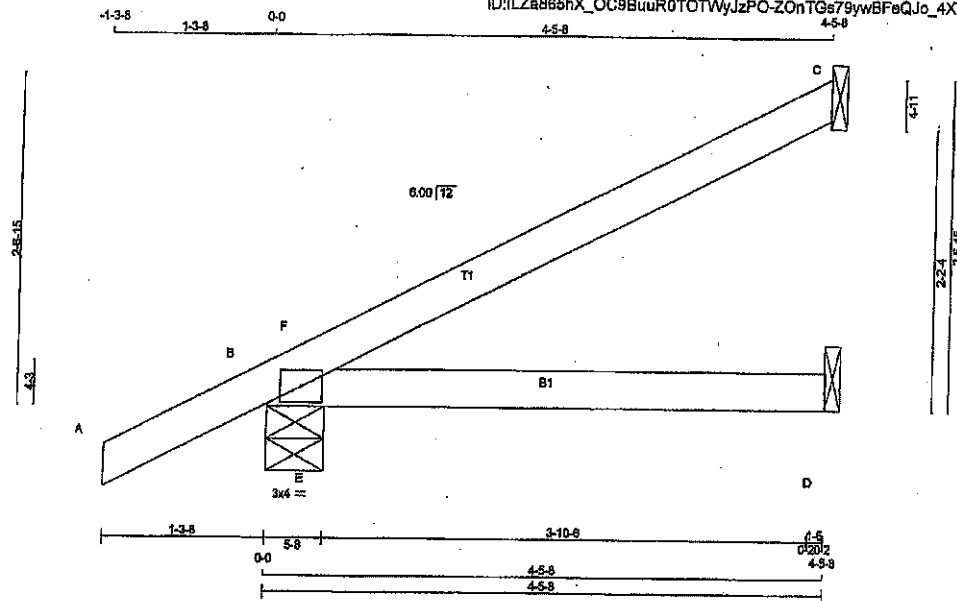
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
 JSI METAL= 0.14 (B) (INPUT = 1.00)



DWG NO. TAM 77906519
 STRUCTURAL
 CONSULTANT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200164-400357	J4	4	1	Barton 12	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Wed Mar 27 17:21:03 2019 Page 1	



TOTAL WEIGHT = 4 X 13 = 50 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	204	0	204	0	1-8	1-8
B	452	0	452	0	5-8	5-8
D	108	0	119	0	1-8	1-8

SEE MTEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
C	143	109/0	7/0	0/0	0/0	27/0	0/0
B	329	208/0	47/0	0/0	0/0	74/0	0/0
D	92	20/0	40/0	0/0	0/0	31/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/30	-102.1 -102.1	0.13 (1)	10.00	E-F	-259 / 110	
B-F	-67 / 15	-102.1 -102.1	0.08 (3)	6.25			
F-C	-1/6	-102.1 -102.1	0.28 (1)	10.00			
B-E	0/0	-38.5 -38.5	0.17 (1)	10.00			
E-D	0/0	-38.5 -38.5	0.21 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCG 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19')
CALCULATED VERT. DEFL. (LL) = $L/998$ (0.03')
ALLOWABLE DEFL. (TL) = $L/360$ (0.19')
CALCULATED VERT. DEFL. (TL) = $L/891$ (0.08')

CSI: TC=0.28/1.00 (C-F:1), BC=0.21/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



DWG NO. TAM 11906520
STRUCTURAL
CONSTRUCTION ONLY



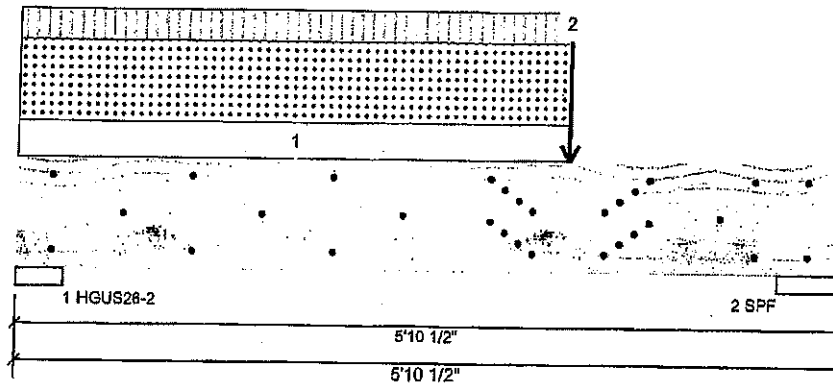
isDesign™

Client: Greenpark
 Project:
 Address:

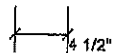
Date: 2019-03-18
 Designer:
 Job Name: Lamberts Lane Home Corp.
 Project #: 200164

BM1 S-P-F #2 2.000" X 10.000" 3-Ply - PASSED

Level: Level



8 1/4"



4 1/2"

Member Information

Type: Girder
 Plies: 3
 Moisture Condition: Dry
 Deflection LL: 480
 Deflection TL: 240
 Importance: Normal

Application: Floor (Residential)
 Design Method: LSD
 Building Code: NBCC 2015 / OBC 2012
 Load Sharing: Yes
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	619	637	1457	0
2	546	703	1651	0

Bearings and Factored Reactions

Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - HGUS...	4.000"	32%	797 / 2704	3501	L	1.25D+1.5S +L
2 - SPF	5.500"	26%	878 / 3022	3900	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6013 ft-lb	3'11 1/4"	9964 ft-lb	0.603 (60%)	1.25D+1.5S +L	L
Unbraced	6013 ft-lb	3'11 1/4"	8360 ft-lb	0.719 (72%)	1.25D+1.5S +L	L
Shear	4582 lb	4'8 1/2"	6573 lb	0.697 (70%)	1.25D+1.5S +L	L
Perm Defl in.	0.012 (L/5150)	2'11 7/8"	0.174 (L/360)	0.070 (7%)	D	Uniform
LL Defl inch	0.033 (L/1892)	2'11 15/16"	0.130 (L/480)	0.250 (26%)	S+0.5L	L
TL Defl inch	0.045 (L/1383)	2'11 15/16"	0.260 (L/240)	0.170 (17%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of 18d Common nails (.162x3.5") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 179085/15
 STRUCTURAL
 COMPONENT ONLY

1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-11-4	12-6-0	Near Face	13 PSF	11 PSF	29 PSF	0 PSF	
2	Point	3-11-4		Near Face	700 lb	524 lb	1680 lb	0 lb	

Manufacturer Info

TAMARACK LUMBER
 NORTH SERVICE RD, ON
 CANADA
 (905)-335-1115



This design is valid until 2021-12-11



isDesign™

Client: Greenpark

Project:

Address:

Date: 2019-03-18

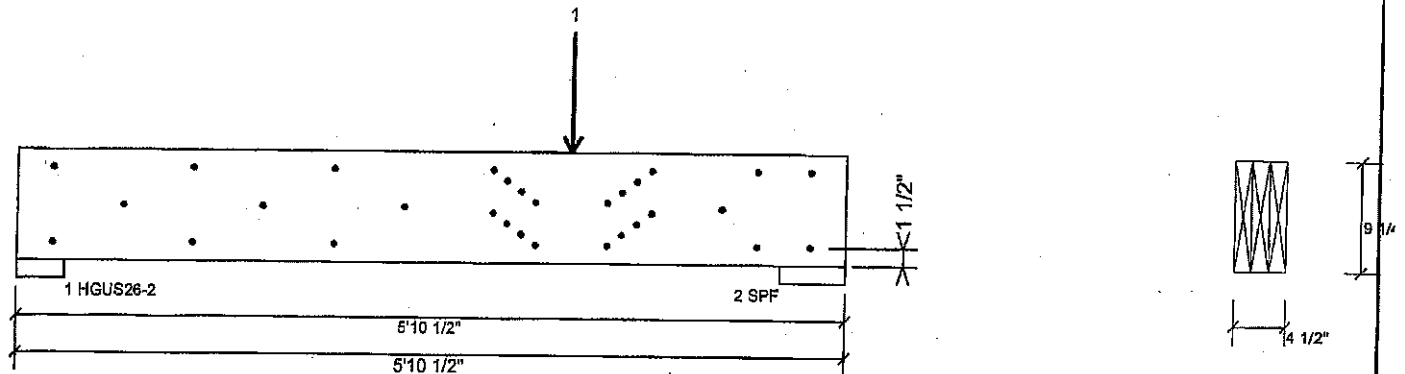
Designer:

Job Name: Lamberts Lane Home Corp.

Project #: 200164

BM1 S-P-F #2 2.000" X 10.000" 3-Ply - PASSED

Level: Level

**Multi-Ply Analysis**

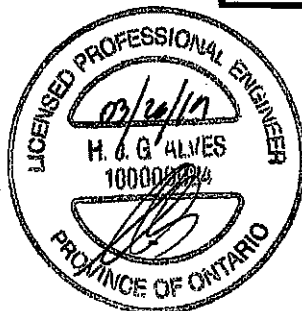
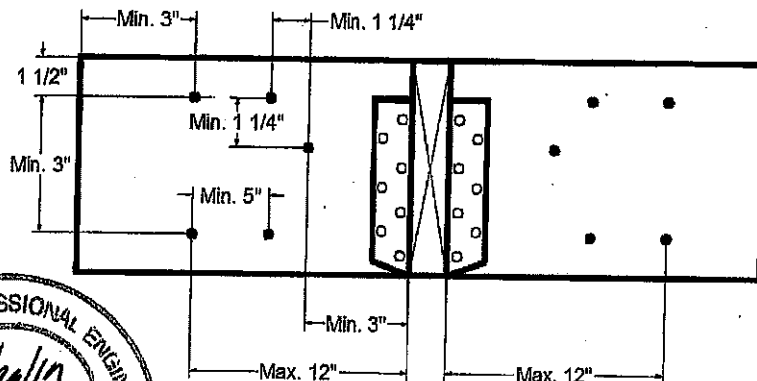
Fasten all plies using 3 rows of 16d Common nails (.162x3.5") at 12" o.c. except for regions covered by concentrated load fastening. Nail from both sides. Maximum end distance not to exceed 6"

Capacity	99.9 %
Load	589.6 PLF
Yield Limit per Foot	590.4 PLF
Yield Limit per Fastener	196.8 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 3-11-4 with a minimum of (16) – 16d Common nails (.162x3.5") in the pattern shown. Repeat fasteners on both sides.

Capacity	83.0 %
Load	2612.7 lb.
Total Yield Limit	3148.7 lb.
Yield Limit per Fastener	196.8 lb.
Yield Mode	9
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads

DWG NO. TAM 7706515
STRUCTURAL
COMPONENT ONLY 2/2

This design is valid until 2021-12-11

Manufacturer Info

TAMARACK LUMBER
NORTH SERVICE RD, ON
CANADA
(805)-335-1115





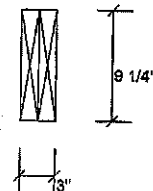
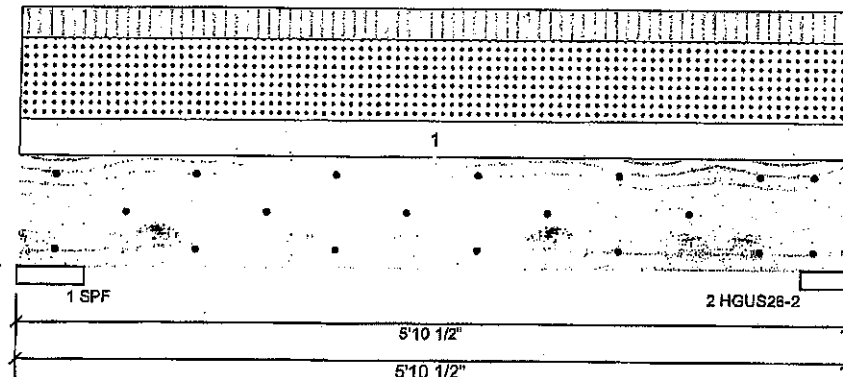
isDesign™

Client: Greenpark
Project:
Address:

Date: 2019-03-18
Designer:
Job Name: Lamberts Lane Home Corp.
Project #: 200164

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 480
Deflection TL: 240
Importance: Normal

Application: Floor (Residential)
Design Method: LSD
Building Code: NBCC 2015 / OBC 2012
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	198	234	522	0
2	190	224	500	0

Bearings and Factored Reactions

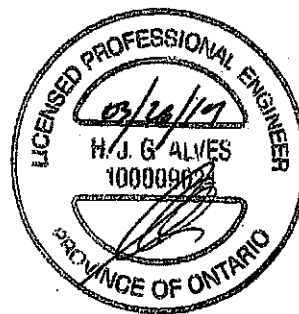
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.600"	13%	293 / 681	1274 L	1.25D+1.5S +L
2 - HGUS...	4.000"	17%	280 / 940	1220 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1439 ft-lb	3'	6039 ft-lb	0.238 (24%)	1.25D+1.5S +L	L
Unbraced	1439 ft-lb	3'	5236 ft-lb	0.275 (27%)	1.25D+1.5S +L	L
Shear	778 lb	1'2"	3984 lb	0.195 (20%)	1.25D+1.5S +L	L
Perm Defl in.	0.065 (L/13191)	3'	0.174 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.013 (L/4970)	3'	0.130 (L/480)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.017 (L/3610)	3'	0.260 (L/240)	0.070 (7%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



DWG NO. TAM T1906512
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-0-0	Top	13 PSF	11 PSF	29 PSF	0 PSF	

Manufacturer Info

TAMARACK LUMBER
NORTH SERVICE RD, ON
CANADA
(905)-335-1115



This design is valid until 2021-12-11





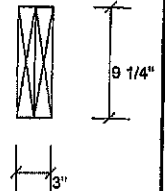
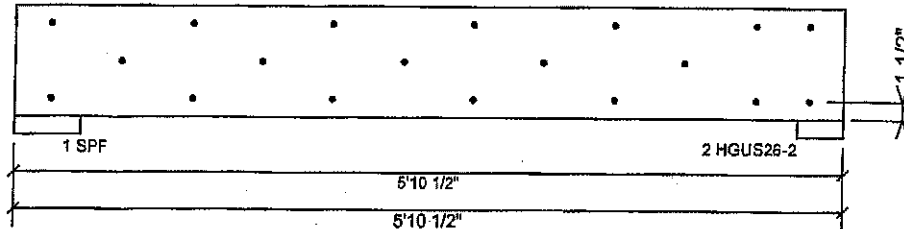
isDesign™

Client: Greenpark
Project:
Address:

Date: 2019-03-18
Designer:
Job Name: Lamberts Lane Home Corp.
Project #: 200184

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c., Maximum end distance not to exceed 6"

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00



DWG NO. TAM 17906512
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info

TAMARACK LUMBER
NORTH SERVICE RD, ON
CANADA
(805)-335-1115



This design is valid until 2021-12-11





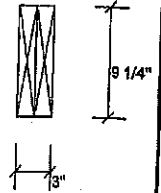
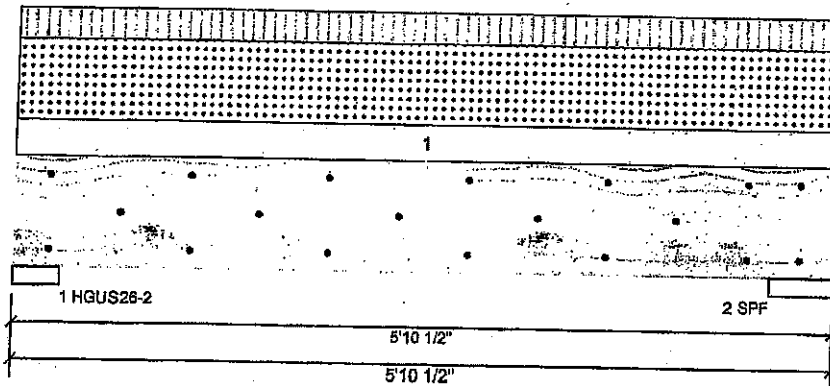
isDesign™

Client: Greenpark
Project:
Address:

Date: 2019-03-18
Designer:
Job Name: Lamberts Lane Home Corp.
Project #: 200164

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 480
Deflection TL: 240
Importance: Normal

Application: Floor (Residential)
Design Method: LSD
Building Code: NBCC 2015 / OBC 2012
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	253	289	867	0
2	284	312	698	0

Bearings and Factored Reactions

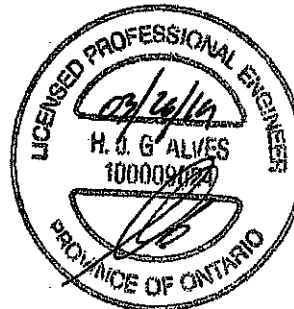
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - HGUS...	4.000"	22%	374 / 1254	1627 L	1.25D+1.5S +L
2 - SPF	6.500"	17%	390 / 1308	1698 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1919 ft-lb	2'10 1/2"	6039 ft-lb	0.318 (32%)	1.25D+1.5S +L	L
Unbraced	1919 ft-lb	2'10 1/2"	5236 ft-lb	0.367 (37%)	1.25D+1.5S +L	L
Shear	1474 lb	4'8 1/2"	3984 lb	0.370 (37%)	1.25D+1.5S +L	L
Perm Defl in.	0.006 (L/8893)	2'10 1/2"	0.174 (L/880)	0.040 (4%)	D	Uniform
LL Defl inch	0.017 (L/3728)	2'10 1/2"	0.130 (L/480)	0.130 (13%)	S+0.5L	L
TL Defl inch	0.023 (L/2708)	2'10 1/2"	0.260 (L/240)	0.090 (9%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1906513
STRUCTURAL
COMPONENT ONLY

1/2

ID	Load Type	Location	Trib Width	Slide	Dead	Live	Snow	Wind	Comments
1	Uniform		8-0-0	Near Face	13 PSF	11 PSF	29 PSF	0 PSF	

Manufacturer Info

TAMARACK LUMBER
NORTH SERVICE RD, ON
CANADA
(905)-335-1115



This design is valid until 2021-12-11





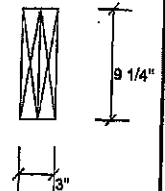
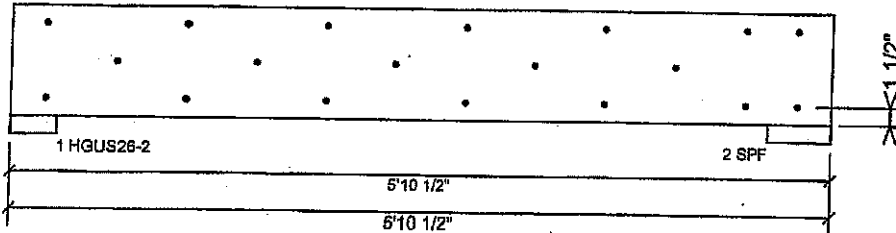
isDesign™

Client: Greenpark
Project:
Address:

Date: 2019-03-18
Designer:
Job Name: Lamberts Lane Home Corp.
Project #: 200164

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

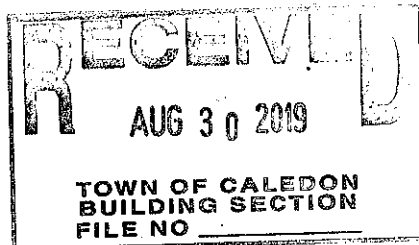
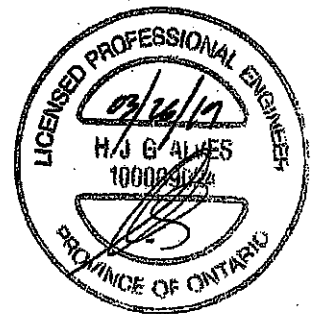
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	83.2 %
Load	283.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 71906513
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	TAMARACK LUMBER NORTH SERVICE RD, ON CANADA (905)-335-1115
	TAMARACK LUMBER INC ALPHA LUMBER GROUP

This design is valid until 2021-12-11



THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

Simpson Strong-Tie Wood Construction Connectors — Canadian Limit States Design

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

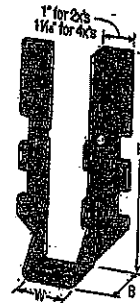
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



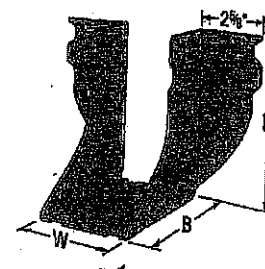
✓ LUS28



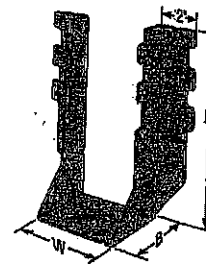
LU26L



✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



Double-Shear
Nailing
Top View

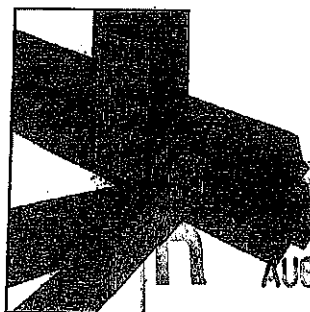


Double-Shear
Nailing
Side View;
Do not
bend tab



Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,803,580

Typical HUS28
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options Information on pp. 125–127.

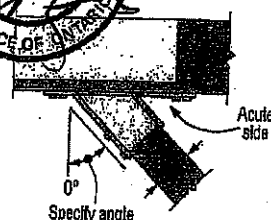
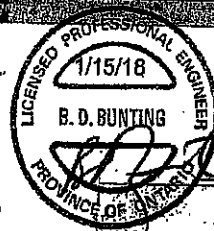
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	F	B	dg ¹	Header	Joist	Uplift	Uplift	Uplift	Normal
								lb.	lb.	lb.	lb.
Single 2x Sizes											
LUS24	18	1 1/4	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
LU24L	22	1 1/4	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
LU24H	22	1 1/4	5	1 1/4	4 1/4	(6) 10d	(3) 10d x 1 1/2"	720	1805	645	1140
LUS26	18	1 1/4	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
HUS26	16	1 1/4	6 1/4	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2085	3875
LUS26S	18	1 1/4	5	3 1/2	4 1/4	(18) 10d	(6) 16d	2055	4265	1460	4115
HUS26S	12	1 1/4	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
LU28L	20	1 1/4	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1650
LUS28	18	1 1/4	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	1420	2520	1290	1790
HUS28	16	1 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3605	6365	2675	4345
HGUS28	12	1 1/4	7 1/4	5	6 1/4	(38) 16d	(12) 16d	3310	7675	3310	6900
LU240L	20	1 1/4	8	1 1/4	7 1/4	(10) 10d	(8) 10d x 1 1/2"	1140	2495	1020	1770
LUS210	18	1 1/4	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1280	2210

- Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
- Designer must ensure that hanger is compatible with truss when reduced heel height is used.
- d_g is the distance from the bearing seat to the top joist nail.
- Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
- Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

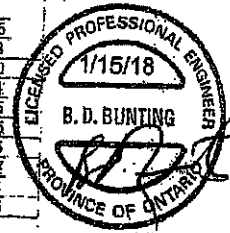
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

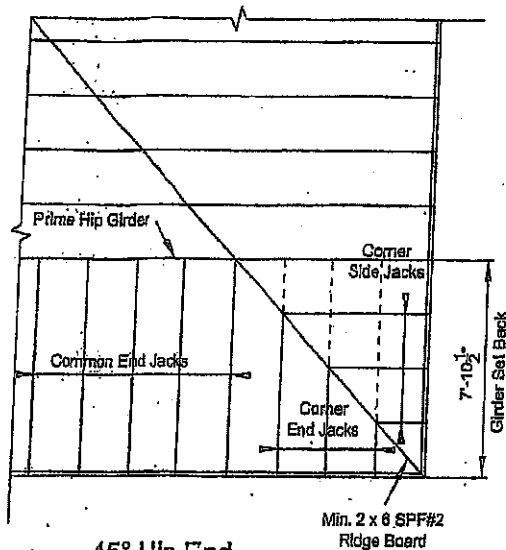
These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance					
		W	H	B	d _s	Header	Joist	T & P		S-P-F			
								lb.	lb.	lb.	lb.		
												(k _f = 3.15)	(k _f = 1.80)
										lb.	lb.	lb.	lb.
Double 2x Sizes												kN	kN
LUS24-2	18	3 1/8	3 1/8	2	1 1/4	(4) 16d	(2) 16d	895	2020	590	1435		
LUS28-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920		
HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 16d	2850	7335	2085	5205		
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
LUS28-3	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575		
HHUS28-2	14	3 1/8	7 1/8	3	4 1/8	(22) 16d	(8) 16d	3785	8940	2875	6945		
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(30) 16d	(12) 16d	6070	12980	4310	9215		
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(8) 16d	2580	4500	2320	3195		
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000		
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270		
Triple 2x Sizes													
HGUS28-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(30) 16d	(12) 16d	6070	12980	4310	9215		
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865		
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400		
Quadruple 2x Sizes													
HGUS28-4	12	5 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
HGUS28-4	12	5 1/8	7 1/8	4	6 1/8	(30) 16d	(12) 16d	6070	12980	4310	9215		
HHUS210-4	14	5 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210		
HGUS210-4	12	5 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400		
HGUS212-4	12	5 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10845		
HGUS214-4	12	5 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645		
4x Sizes													
LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920		
HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 16d	2850	7335	2085	5205		
HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
LUS48	18	3 1/8	6 1/8	2	3 1/8	(6) 16d	(4) 16d	1720	3325	1545	2575		
HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3785	8940	2875	6945		
HGUS48	12	3 1/8	7 1/8	4	8 1/8	(30) 16d	(12) 16d	6070	12980	4310	9215		
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195		
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270		
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10845		
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645		



Plated Truss Connectors

FILED
30 2019
OFFICE OF CALEDON BUILDING DEPARTMENT
FILE NO.



45° Hip End

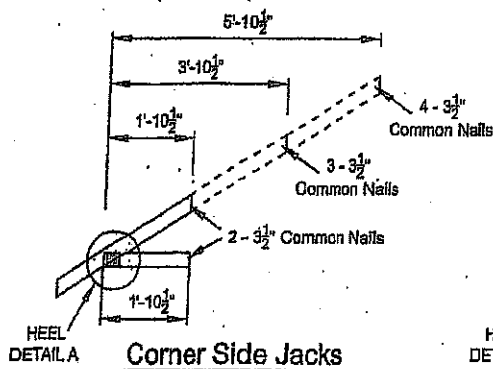
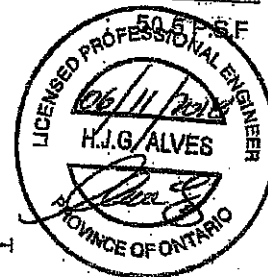
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

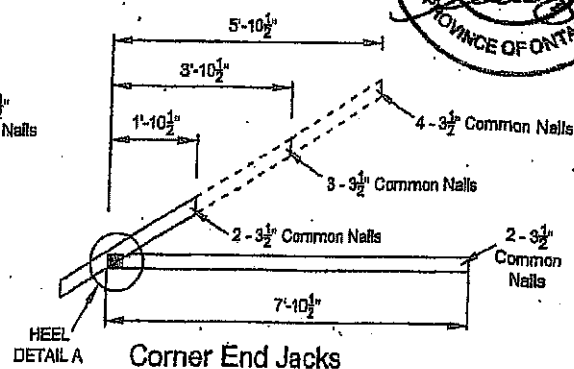
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

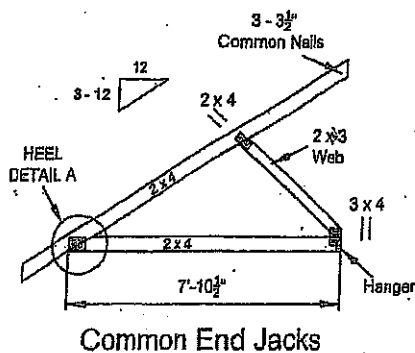
TOTAL LOAD



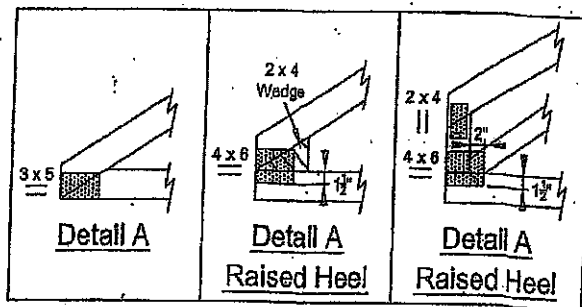
Corner Side Jacks



Corner End Jacks

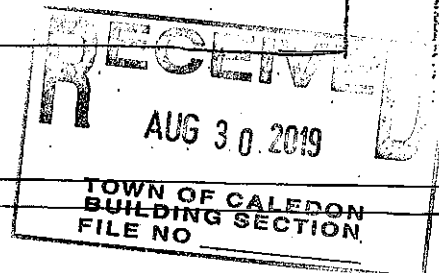


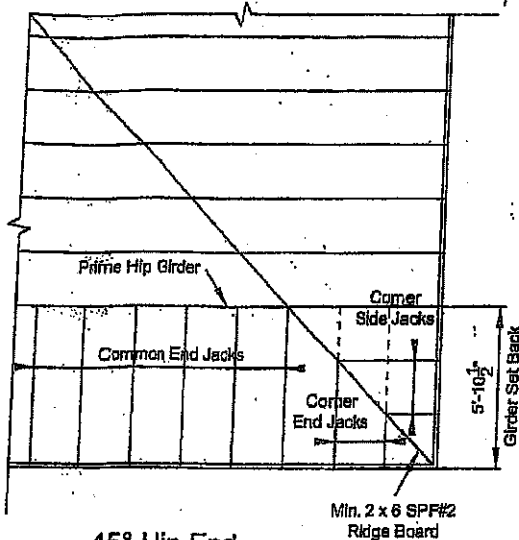
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 8, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217





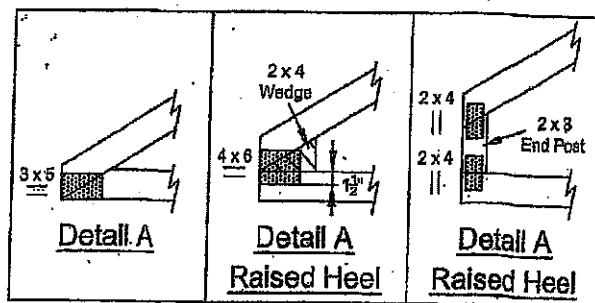
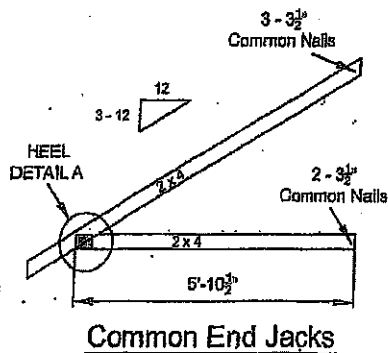
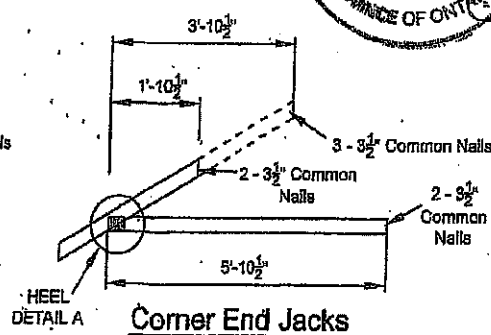
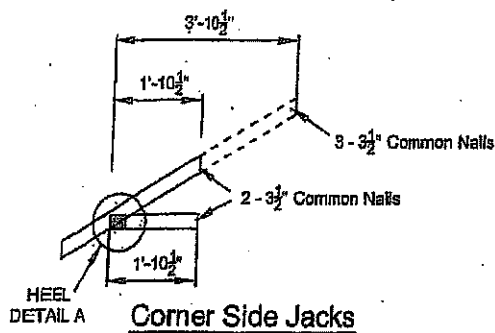
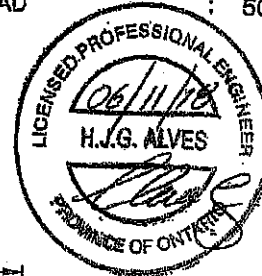
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

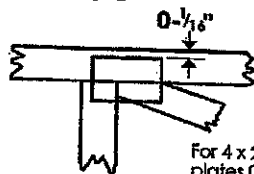
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in fractions or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/2\"/>



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



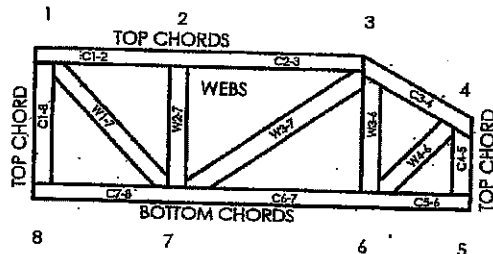
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-fractions or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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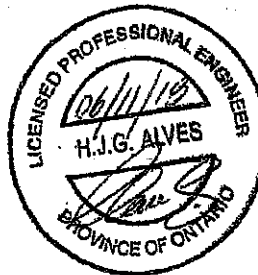
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MU-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

T-1800218

Feb 09, 2018