



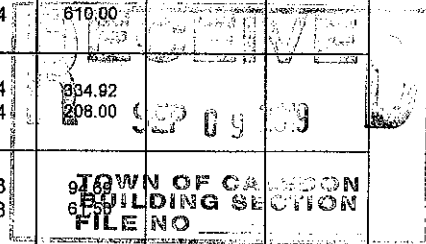
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #: Lot 73
 Elevation: 3-REAR UPGRADE

Job Track: 50120
 PlanLog: 200598
 Layout ID: 401609
 Ref #
 Page: 1 of 2
 Date: 05/09/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	31-11-00	4-05-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	339.54 204.00		
	1 2-ply	T123 Hip Girder	9/12	31-11-00	4-05-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	339.54 204.00		
	2	T2 Hip	9/12	31-11-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	254.66 160.33		
	2	T3 Hip	9/12	31-11-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.12 177.00		
	2	T4 Hip	9/12	31-11-00	7-07-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	294.24 187.33		
	2	T5 Hip	9/12	31-11-00	8-08-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	307.46 193.00		
	6	T6 Hip	9/12	31-11-00	9-09-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	988.5 610.00		
	2	T7 Hip	9/12	31-11-00	10-10-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	834.92 508.00		
	1	T18 Hip Girder	12/12	15-01-00	5-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	94.02 60.83		
	1	T19 Hip	12/12	15-01-00	7-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	94.02 60.83		
	3	T20 Hip	12/12	15-01-00	9-04-08	2 x 4	1-03-08	3-10-08 3-10-08	247.52 159.00		
	1	T21 Hip	12/12	15-01-00	7-04-08	2 x 4	1-03-08	3-10-08 3-10-08	85.26 54.83		
	1	T22 Hip Girder	12/12	10-04-00	5-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	69.54 46.33		
	1	T23 Hip	12/12	10-04-00	7-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	81.01 53.67		





DELIVERY SHIPLIST

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 Page: 2 of 2
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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T24 Hip	12/12	10-04-00	8-07-00	2 x 6 2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	68.34 43.17		
	5	T100 Common	9/12	14-09-00	7-00-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	330.72 215.83		
	1	G100 GABLE	9/12	14-09-00	7-00-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	69.06 45.17		
	2	P1 Piggyback	9/12	9-09-05	2-02-00	2 x 4			55.84 36.00		
	2	P2 Piggyback	9/12	9-09-05	3-03-00	2 x 4			54.88 36.67		
	11	J1 Jack-Open	6.5/12	5-10-08	4-04-14	2 x 4	1-03-08	1-02-11 4-04-14	187.42 117.33		
	6	J4 Jack-Open	12/12	1-06-00	5-04-08	2 x 4	1-03-08	3-10-08 5-04-08	81.06 56.00		
	9	J5 Jack-Open	12/12	1-06-00	3-04-08	2 x 4	1-03-08	1-10-08 3-04-08	86.6 63.00		
	7	J90 Jack-Open	4/12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	101.14 65.33		

TOTAL # TRUSS= 72 TOTAL BFT OF ALL TRUSSES= 3058.32 BFT. TOTAL WEIGHT OF ALL TRSSES 4827.08 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
9	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 12

JOB NAME 403045	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 Mitak Industries, Inc. Tue May 14 10:41:19 2019 Page 2 ID:07uIPhWg?DzGkx1HTFeFreyNCCH-TNB NexVTbRnTACJOH0coKmGXzICVdKB9bFN6zGdDk		

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC	LG1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	20-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
W	22-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
X	24-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
Y	26-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
Z	14-10-8	-2332	-2332	---	FRONT	VERT	TOTAL	---	---
AA	16-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	18-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AC	20-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AD	22-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AE	24-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AF	26-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AG	28-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AH	30-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



Structural component only
DWG# T-1911482

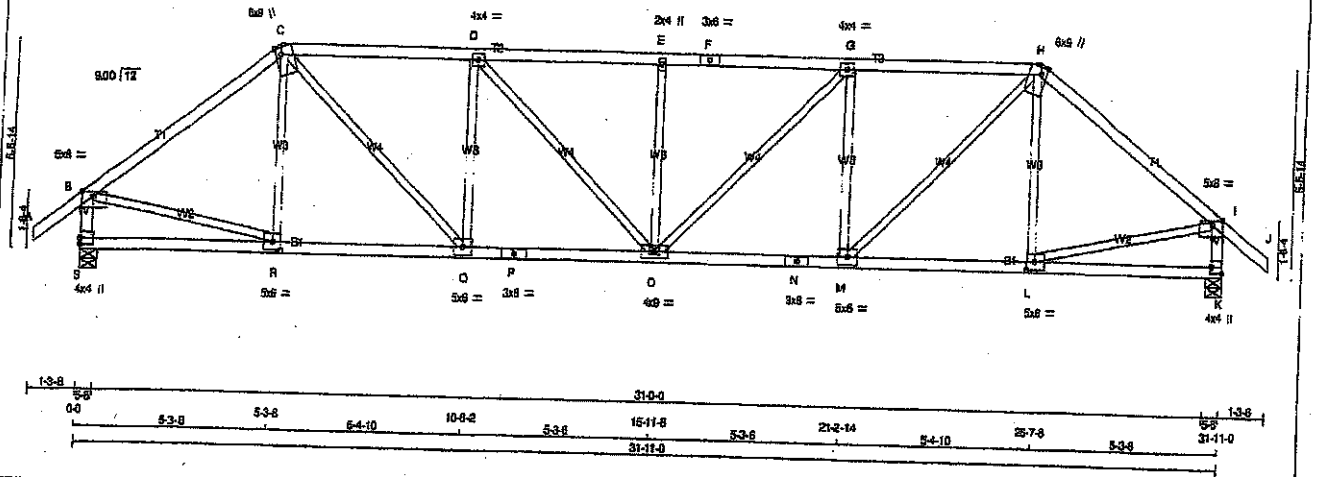
Structural drawing of a roof truss system. The top part shows a side elevation with a truss structure supported by columns. The roof is labeled "9.00 / 12". Various members are labeled with "W" and "T" followed by numbers. Columns are labeled with "Sx6" and "Sx9". The bottom part shows a plan view of the truss with dimensions and member labels.

T	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.25	4.00
C	TMWW+m	MT20	8.0	9.0	3.75	2.00
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMWW+m	MT20	8.0	9.0	3.75	2.00
I	TMWW-p	MT20	5.0	8.0	1.25	4.00
J	BMV1+u	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	6.0	9.0	3.00	4.25
M	BS-t	MT20	6.0	9.0		
N	BMWWV-t	MT20	6.0	9.0		
O	BS-t	MT20	6.0	9.0		
P	BMWWV-t	MT20	6.0	9.0	3.00	4.25
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+u	MT20	3.0	6.0		

R	0/0	-36.5	-36.5	0.04 (2)	10.00
R	0/3732	-36.5	-36.5	0.20 (1)	10.00
Q	0/8534	-36.5	-36.5	0.55 (1)	10.00
P	0/8534	-36.5	-36.5	0.55 (1)	10.00
W	0/8534	-36.5	-36.5	0.55 (1)	10.00
O	0/8534	-36.5	-36.5	0.55 (1)	10.00
X	0/9799	-36.5	-36.5	0.55 (1)	10.00
X	0/9799	-36.5	-36.5	0.55 (1)	10.00
Y	0/9799	-36.5	-36.5	0.55 (1)	10.00
N	0/9799	-36.5	-36.5	0.55 (1)	10.00
Z	0/9799	-36.5	-36.5	0.55 (1)	10.00
AA	0/9799	-36.5	-36.5	0.55 (1)	10.00
MA	0/9799	-36.5	-36.5	0.55 (1)	10.00
LA	0/4228	-36.5	-36.5	0.21 (1)	10.00
L	0/0	-36.5	-36.5	0.04 (2)	10.00

Structural component only
DWG# T-1911508 CONTINUED ON PAGE 2

JOB NAME 403045	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Trusses, Burlington						
Version 8.300 S Apr 23 2019 Mitek Industries, Inc. The May 14 10:41:21 2019 Page 1 ID:D7uIPnW9?DzGloC1HTFaFreyNCOH-PmloJd?CIVUMW4UHDPOCLgT1deT4MP?zGdD 31-11-0 33-8 1-3-8 5-3-8 5-3-8 5-4-10 10-4-2 5-3-8 15-11-8 5-3-8 21-2-14 5-4-10 25-7-8 5-3-8 31-11-0 Scale = 1/32"						



LUMBER

N.L.S.A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - F	2x4	DRY No.2
F - H	2x4	DRY No.2
H - J	2x4	DRY No.2
J - L	2x4	DRY No.2
L - N	2x4	DRY No.2
N - K	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
EXCEPT		

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	1.50	3.50
C TTVW-m	MT20	6.0	9.0	Edge	2.00
D TMVW-t	MT20	4.0	4.0		
E TMVW-w	MT20	2.0	4.0		
F TS-t	MT20	3.0	8.0		
G TMVW-t	MT20	4.0	4.0		
H TTVW-m	MT20	6.0	9.0	Edge	2.00
I TMVW-p	MT20	5.0	8.0	1.50	3.50
K BMVW-t	MT20	4.0	4.0	2.00	Edge
L BMVW-t	MT20	5.0	8.0	2.50	2.75
M BMVW-t	MT20	5.0	8.0		
N BS-t	MT20	3.0	8.0		
O BMVW-w	MT20	4.0	9.0		
P BS-t	MT20	3.0	8.0		
Q BMVW-t	MT20	3.0	8.0		
R BMVW-t	MT20	5.0	8.0	2.50	2.75
S BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
S	2385	0	2385	0
K	2385	0	2385	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO	LENGTH	FR-TO		
B-C	0 / 42	-102.1 -102.1	0.14 (1)	10.08	R-C	-195 / 98
C-D	-2429 / 0	-102.1 -102.1	0.73 (1)	3.34	C-Q	0 / 187
D-E	-3138 / 0	-102.1 -102.1	0.86 (1)	3.14	Q-D	-861 / 0
E-F	-3494 / 0	-102.1 -102.1	0.89 (1)	3.34	D-O	0 / 500
F-G	-3494 / 0	-102.1 -102.1	0.89 (1)	3.14	O-E	-497 / 0
G-H	-3138 / 0	-102.1 -102.1	0.85 (1)	3.34	E-G	0 / 500
H-I	-2429 / 0	-102.1 -102.1	0.73 (1)	3.34	G-S	-991 / 0
I-J	0 / 42	-102.1 -102.1	0.14 (1)	10.08	S-H	0 / 187
J-K	-2301 / 0	0.0 0.0	0.24 (1)	5.62	H-R	-195 / 98
K-L	-2301 / 0	0.0 0.0	0.24 (1)	5.62	R-I	0 / 187
S-R	0 / 0	-38.5 -38.5	0.21 (3)	10.00	I-L	0 / 187
R-Q	0 / 1938	-38.5 -38.5	0.47 (2)	10.00	L-I	0 / 187
Q-P	0 / 3138	-38.5 -38.5	0.80 (1)	10.00		
P-O	0 / 3138	-38.5 -38.5	0.80 (1)	10.00		
O-N	0 / 3138	-38.5 -38.5	0.80 (1)	10.00		
N-M	0 / 3138	-38.5 -38.5	0.80 (1)	10.00		
M-L	0 / 1938	-38.5 -38.5	0.47 (2)	10.00		
L-K	0 / 0	-38.5 -38.5	0.21 (3)	10.00		

PROFESSIONAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012
- CSA 086-08, CSA 086-14
- TFC 2011, TFC 2014

85% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/960$ (1.08")

CALCULATED VERT. DEFL. (LL) = $L/999$ (0.18")

ALLOWABLE DEFL. (TL) = $L/960$ (1.08")

CALCULATED VERT. DEFL. (TL) = $L/999$ (0.28")

CSI: TC=0.72/1.00 (H-H), BC=0.80/1.00 (O-Q), WB=0.45/1.00 (L-L), SS=0.26/1.00 (C-Q)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

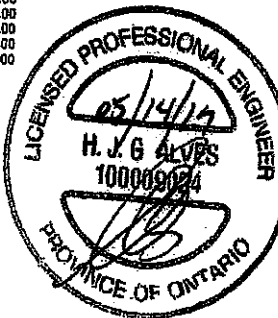
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	610 354	1867 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (L) (INPUT = 0.80)

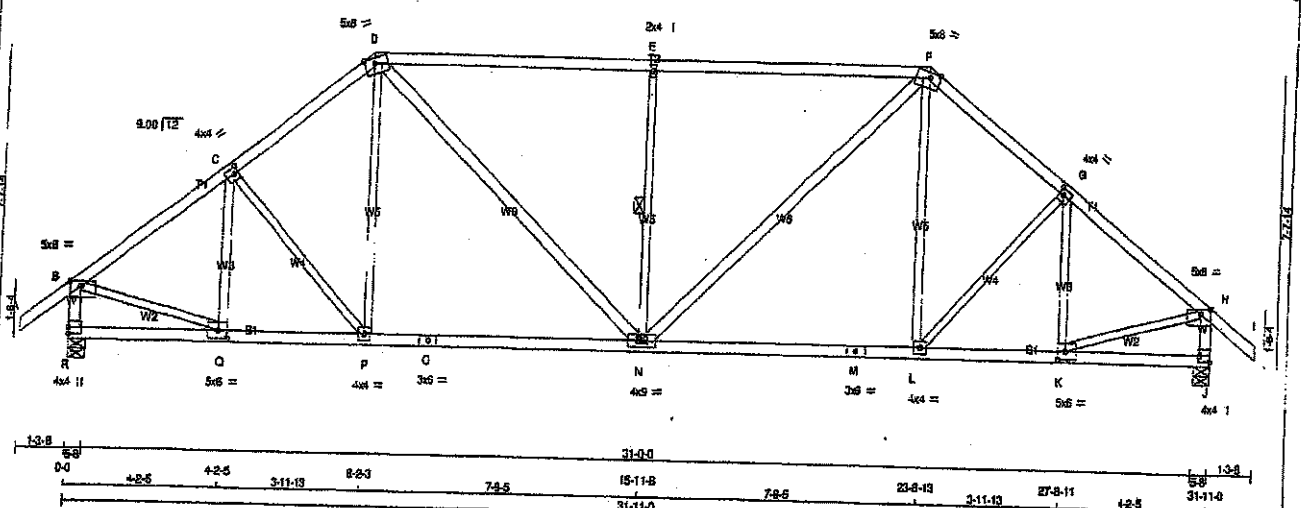
JSI METAL = 0.95 (P) (INPUT = 1.00)



Structural component only
DWG# T-1911484

JOB NAME: 403045 TRUSS NAME: T4 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington

Version 6.300 S Apr 23 2018 Mitek Industries, Inc. Tue May 14 10:41:23 2019 Page 1
 ID: D7uIPnW9?DzGkr1HTPaFreyNCCH-L9QVD?_?WpyDyoV4dUymwVIT8Ms8MRw6nZSWzGdDg
 15-11-8 23-8-13 3-11-13 27-8-11 31-11-0 3-11-0 1-3-8
 Scale = 1/2" = 1'-0"



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - I	2x4	DRY No.2
R - B	2x4	DRY No.2
J - H	2x4	DRY No.2
R - O	2x4	DRY No.2
O - M	2x4	DRY No.2
M - J	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
EXCEPT		
D - N	2x4	DRY No.2
N - F	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW-p	MT20	5.0	8.0	1.50	2.50
C TMW-w	MT20	4.0	4.0	2.00	1.50
D TTW-w	MT20	5.0	8.0	Edge	3.50
E TMW-w	MT20	2.0	4.0		
F TTW-w	MT20	5.0	8.0	Edge	3.50
G TMW-t	MT20	4.0	4.0	2.00	1.50
H TMW-p	MT20	5.0	8.0	1.50	2.50
J BMW-t	MT20	4.0	4.0	2.00	Edge
K BMW-t	MT20	5.0	8.0	2.50	2.50
L BMW-t	MT20	4.0	4.0		
M BS4	MT20	3.0	6.0		
N BMWW-t	MT20	4.0	9.0		
O BS4	MT20	3.0	6.0		
P BMW-t	MT20	4.0	4.0		
Q BMW-t	MT20	5.0	8.0	2.50	2.50
R BMW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UP/LIFT	IN-SK
R 2385	0	5-8	5-8
J 2385	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAXIMUM COMPONENT REACTIONS
COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
JT 1773	1007/0 335/0 0/0 0/0 431/0 0/0
J 1773	1007/0 335/0 0/0 0/0 431/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.73 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMS.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMS.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMS.
FR-TO				FR-TO			
A-B	0/42	-102.1	0.14 (1)	10.00	C-C	-410/0	0.13 (1)
B-C	-2381/0	-102.1	0.28 (1)	4.21	C-P	-88/0	0.05 (1)
C-D	-2377/0	-102.1	0.28 (1)	4.22	P-D	0/939	0.08 (2)
D-E	-2561/0	-102.1	1.00 (1)	2.73	D-N	0/939	0.15 (1)
E-F	-2561/0	-102.1	1.00 (1)	2.73	N-E	-977/0	0.36 (1)
F-G	-2377/0	-102.1	0.28 (1)	4.22	N-F	0/939	0.15 (1)
G-H	-2381/0	-102.1	0.28 (1)	4.21	L-F	0/939	0.08 (2)
H-I	0/42	-102.1	0.14 (1)	10.00	L-G	-89/0	0.05 (1)
I-J	-2311/0	0.0	0.24 (1)	5.82	K-G	-410/0	0.13 (1)
J-K	-2311/0	0.0	0.24 (1)	5.82	K-Q	0/2012	0.45 (1)
					K-H	0/2012	0.45 (1)
R-Q	0/0	-38.5	-38.5 0.12 (3)	10.00			
Q-P	0/1935	-38.5	-38.5 0.50 (2)	10.00			
P-O	0/1879	-38.5	-38.5 0.52 (2)	10.00			
O-N	0/1879	-38.5	-38.5 0.52 (2)	10.00			
N-M	0/1879	-38.5	-38.5 0.52 (2)	10.00			
M-L	0/1879	-38.5	-38.5 0.52 (2)	10.00			
L-K	0/1935	-38.5	-38.5 0.50 (2)	10.00			
K-J	0/0	-38.5	-38.5 0.12 (3)	10.00			

TOTAL WEIGHT = 2 X 147 = 294 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	29.0	PSF
BOT CH.	LL	PSF
DL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	52.5	PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLES WITH:

- PART 9 OF NBC 2010, NBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(5% OF 27.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (L1) = L/800 (1.06")
 CALCULATED VERT. DEF. (L1) = 1/999 (0.13")
 ALLOWABLE DEF. (L2) = L/800 (1.06")
 CALCULATED VERT. DEF. (L2) = 1/999 (0.23")

CSL TO=1.00/1.00 (E-F-1), BC=0.82/1.00 (N-P-2),
 WB=0.45/1.00 (B-C-1), SS=0.39/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

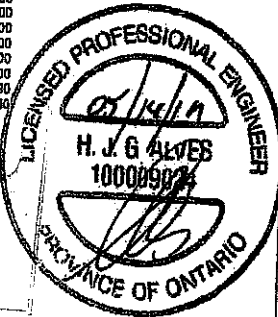
NAIL VALUES

PLATE GRP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	616	354
MT20	1667	788
MT20	1987	1665

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.80)
 JSI METAL= 0.85 (C) (INPUT = 1.00)



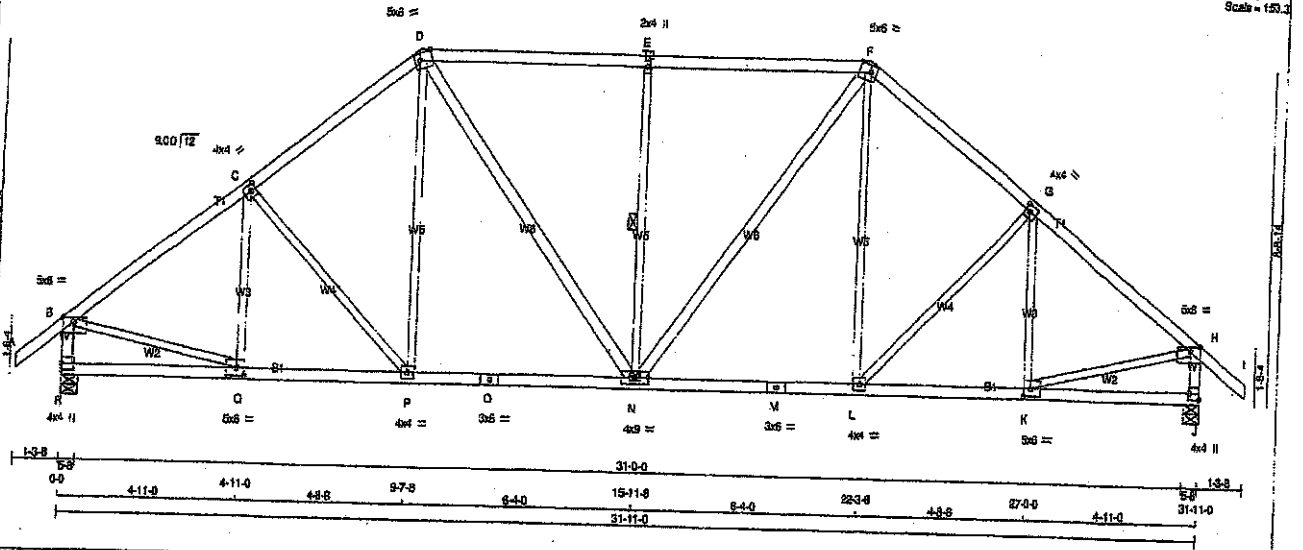
SEP 06 2019

TOWN OF CALEDON
 BUILDING DEPARTMENT
 FILE NO.

Structural component only
 DWG# T-1911486

JOB NAME 403045	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.300 S Apr 23 2019 MTEK Industries, Inc. Tue May 14 10:41:24 2019 Page 1
 ID: D7U/PnW9?DzGk1HTFaFreyNCOH-pL_7dH744ex4HBCQ8.Jst1ZFVjGpa3LRJ02JzG1Df
 1-3-8 0-0 4-11-0 4-11-0 4-8-8 9-7-8 8-4-0 15-11-8 8-4-0 22-3-8 4-8-8 27-0-0 31-11-0 1-3-8
 Scale = 1/32



LUMBER			
N. L. & A. RULES	SIZE	LUMBER	DESCR
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-p	MT20	5.0	8.0	1.50 3.50
C	TMWV-t	MT20	4.0	4.0	2.00 1.50
D	TTWV-m	MT20	5.0	6.0	Edge 4.25
E	TMW-v	MT20	2.0	4.0	
F	TTWV-m	MT20	5.0	6.0	Edge 4.25
G	TMWV-t	MT20	4.0	4.0	2.00 1.50
H	TMWV-p	MT20	5.0	8.0	1.50 3.50
J	BMV-t	MT20	4.0	4.0	2.00 Edge
K	BMWV-t	MT20	5.0	5.0	2.50 2.50
L	BMWV-t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	6.0	
N	BMWVW-t	MT20	4.0	8.0	
O	BS-t	MT20	3.0	6.0	
P	BMWV-t	MT20	4.0	4.0	
Q	BMWV-t	MT20	5.0	6.0	2.50 2.50
R	BMV-t	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	DOWN	BRG
R	2385	0	5-8
J	2385	0	5-8

UNFACTORED REACTIONS			
	1ST CASE	MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
R	1773	1007/0	335/0
J	1773	1007/0	335/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

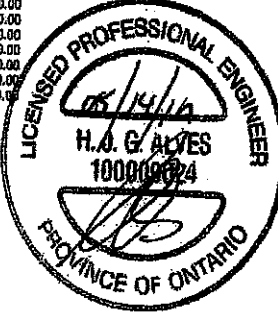
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0/42	-102.1	0.14 (1)
B-C	-2445/0	-102.1	0.40 (1)
C-D	-2288/0	-102.1	0.38 (1)
D-E	-2183/0	-102.1	0.61 (1)
E-F	-3182/0	-102.1	0.81 (1)
F-G	-2288/0	-102.1	0.38 (1)
G-H	-2445/0	-102.1	0.40 (1)
H-I	0/42	-102.1	0.14 (1)
I-J	-2304/0	0.0	0.0 (1)
J-K	-2304/0	0.0	0.0 (1)
K-L	-2304/0	0.0	0.0 (1)
L-M	-2304/0	0.0	0.0 (1)
M-N	-2304/0	0.0	0.0 (1)
N-O	-2304/0	0.0	0.0 (1)
O-P	-2304/0	0.0	0.0 (1)
P-Q	-2304/0	0.0	0.0 (1)
Q-R	-2304/0	0.0	0.0 (1)
R-S	-2304/0	0.0	0.0 (1)
S-T	-2304/0	0.0	0.0 (1)
T-U	-2304/0	0.0	0.0 (1)
U-V	-2304/0	0.0	0.0 (1)
V-W	-2304/0	0.0	0.0 (1)
W-X	-2304/0	0.0	0.0 (1)
X-Y	-2304/0	0.0	0.0 (1)
Y-Z	-2304/0	0.0	0.0 (1)
Z-A	-2304/0	0.0	0.0 (1)



DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 20.0 PSF
 TOP CH. DL = 5.0 PSF
 BOT CH. LL = 10.5 PSF
 BOT CH. DL = 7.0 PSF
 TOTAL LOAD = 32.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2010, CBC 2012
 - CSA 086-03, CSA 086-14
 - TPIC 2011, TPIC 2014

(5% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/800 (1.06")
 CALCULATED VERT. DEFL. (LL) = L/899 (0.10")
 ALLOWABLE DEFL. (TL) = L/800 (1.06")
 CALCULATED VERT. DEFL. (TL) = L/969 (0.17")

CSI: TC=0.81/1.00 (D-E), BC=0.48/1.00 (N-P), WB=0.48/1.00 (H-K), SS=0.31/1.00 (E-F)

DOL LUMBS=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PS) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 518 354 1687 788 1887 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.88 (C) (INPUT = 0.90)
 JSI METAL=0.88 (C) (INPUT = 1.00)

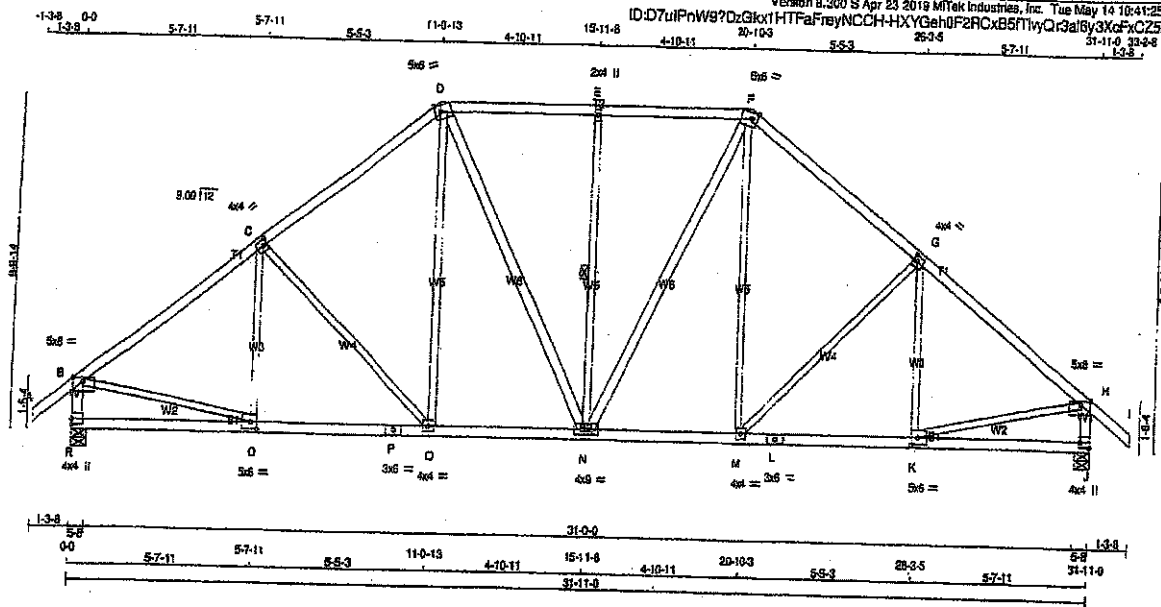
Structural component only
 DWG# T-1911487

JOB NAME: 403045 TRUSS NAME: T6 QUANTITY: 6 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Pamarack Roof Truss, Burlington

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ID: D7uIPnW8?DzGkx1HTFaFreyNCCCH-HXYGeh0F2RCx85f1tyQr3aBy3XfCZ52ZamzG0Da

Scale = 1/8" = 1'-0"



CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - P	2x4	DRY	No.2
P - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT D - N 2x4 DRY No.2

N - F 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMVW-p	MT20	5.0	6.0	1.50 3.50
C TMVW-l	MT20	4.0	4.0	2.00 1.50
D TYVW-m	MT20	5.0	6.0	Edge 2.00
E TMVW-w	MT20	2.0	4.0	
F TYVW-m	MT20	5.0	6.0	Edge 2.00
G TMVW-l	MT20	4.0	4.0	2.00 1.50
H TMVW-p	MT20	5.0	6.0	1.50 3.50
J BMVW-p	MT20	4.0	4.0	2.00 Edge
K BMVW-l	MT20	5.0	6.0	2.50 2.50
L BS-l	MT20	3.0	6.0	
M BMVW-l	MT20	4.0	4.0	
N BMVW-w	MT20	4.0	4.0	
O BMVW-l	MT20	4.0	4.0	
P BS-l	MT20	3.0	6.0	
Q BMVW-l	MT20	5.0	6.0	2.50 2.50
R BMVW-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	DOWN	GROSS REACTION	SRG	BRG
JT VERT	HORZ	UP/LIFT	IN-SX	IN-SX
R	2385	0	2385	0
J	2385	0	2385	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1773	1007/0	335/0	0/0	0/0	431/0	0/0
J	1773	1007/0	335/0	0/0	0/0	431/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 CS1 (L)	MAX CS1 (L)	MEMB. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (L)
FR-TO	FROM	TO	FR-TO	FR-TO	FR-TO			
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	Q-C	-185/119	0.06 (1)
B-C	-2474/0	-102.1	-102.1	0.54 (3)	3.68	C-D	-422/0	0.52 (1)
C-D	-2181/0	-102.1	-102.1	0.49 (1)	4.13	D-O	0/498	0.11 (2)
D-E	-1918/0	-102.1	-102.1	0.35 (1)	4.50	E-N	0/440	0.07 (1)
E-F	-1918/0	-102.1	-102.1	0.35 (1)	4.50	N-E	-603/0	0.39 (1)
F-G	-2181/0	-102.1	-102.1	0.49 (1)	4.13	D-E	0/440	0.07 (1)
G-H	-2474/0	-102.1	-102.1	0.54 (1)	3.68	M-F	0/498	0.11 (2)
H-I	0/42	-102.1	-102.1	0.14 (1)	10.00	M-G	-422/0	0.52 (1)
I-J	-2286/0	0.0	0.0	0.24 (1)	5.63	K-G	-185/119	0.06 (1)
J-K	-2298/0	0.0	0.0	0.24 (1)	5.63	R-Q	0/2055	0.49 (1)
						K-H	0/2055	0.46 (1)
Q	0/0	-38.5	-38.5	0.22 (3)	10.00			
P	0/2011	-38.5	-38.5	0.48 (2)	10.00			
O	0/2011	-38.5	-38.5	0.48 (2)	10.00			
N	0/1715	-38.5	-38.5	0.38 (1)	10.00			
M	0/1715	-38.5	-38.5	0.38 (1)	10.00			
L	0/2011	-38.5	-38.5	0.48 (2)	10.00			
K	0/2011	-38.5	-38.5	0.48 (2)	10.00			
J	0/0	-38.5	-38.5	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 8.0 PSF

BOY CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLES WITH: - PART 9 OF NBCC 2010, CSC 2012 - CSA 086-08, CSA 086-14 - TPIC 2011, TPIC 2014

(66% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06") CALCULATED VERT. DEFL. (LL) = L/999 (0.09") ALLOWABLE DEFL. (TL) = L/360 (1.06") CALCULATED VERT. DEFL. (TL) = L/999 (0.16")

CS1: TC=0.54/1.00 (B-C-1), BC=0.48/1.00 (C-D-2), WS=0.52/1.00 (C-D-1), SS=0.24/1.00 (D-E-1)

DOE LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)

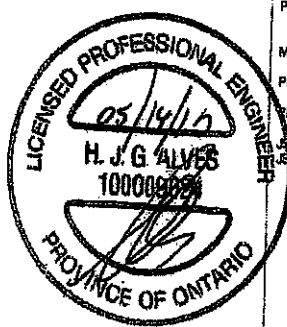
MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

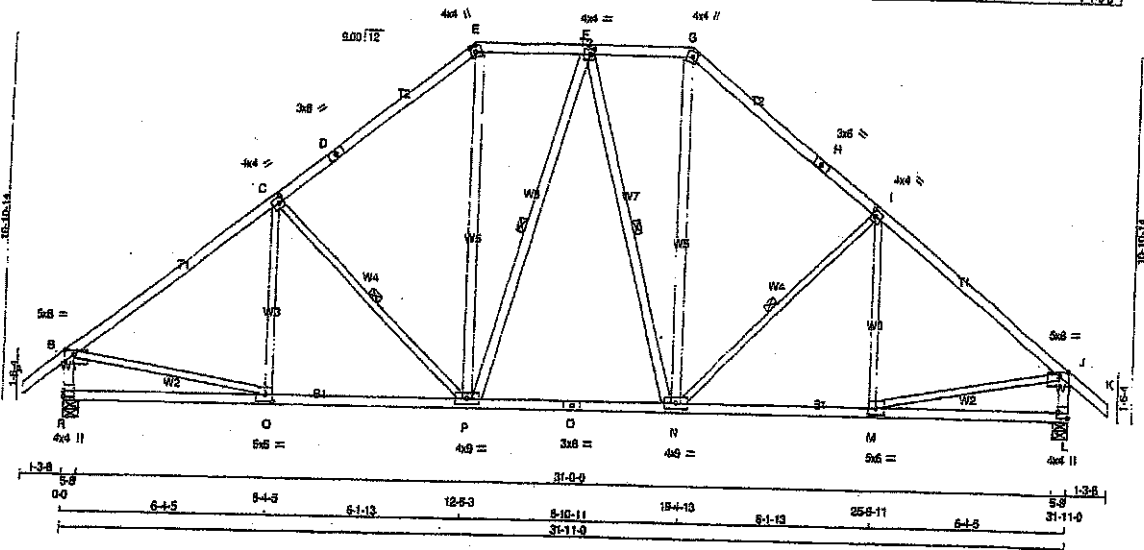
PLATE ROTATION TOL = 5.0 Deg.

GRIP=0.85 (B) (INPUT = 0.90)

METAL=0.50 (F) (INPUT = 1.00)



Structural component only DWG# T-1911488



LUMBER

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
P - E	2x4	DRY	No.2
F - N	2x4	DRY	No.2
N - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	1.50	3.50
C	TMWV-1	MT20	4.0	4.0	2.00	1.50
D	TS-1	MT20	3.0	6.0		
E	TTW+m	MT20	4.0	4.0	2.00	1.75
F	TMWV-1	MT20	4.0	4.0	2.00	1.75
G	TTW+m	MT20	4.0	4.0	2.00	1.75
H	TS-1	MT20	3.0	6.0		
I	TMWV-1	MT20	4.0	4.0	2.00	1.50
J	TMWV-p	MT20	5.0	8.0	1.50	3.50
L	BMV1-p	MT20	4.0	4.0	2.00	Edge
M	BMWV-1	MT20	5.0	8.0	2.50	2.75
N	BMWV-1	MT20	4.0	4.0		
O	BS-1	MT20	3.0	6.0		
P	BMWV-1	MT20	4.0	4.0		
Q	BMWV-1	MT20	5.0	8.0	2.50	2.75
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	RECORD BRG IN-SX
JT				
R	2385 0	2385 0	5-8	5-8
L	2385 0	2385 0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERMALIVE	WIND	DEAD	SOIL
JT								
R	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0	0 / 0
L	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CSF (LC)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	10.00	C-Q	-111 / 174	0.07 (1)
B-C	-2481 / 0	-102.1	-102.1 0.72 (1)	3.65	C-P	-559 / 0	0.29 (1)
C-D	-2070 / 0	-102.1	-102.1 0.84 (1)	4.02	P-E	0 / 832	0.13 (1)
D-E	-2070 / 0	-102.1	-102.1 0.84 (1)	4.02	P-F	-212 / 0	0.15 (1)
E-F	-1827 / 0	-102.1	-102.1 0.18 (1)	5.04	F-N	-216 / 0	0.15 (1)
F-G	-1827 / 0	-102.1	-102.1 0.18 (1)	5.04	N-G	0 / 837	0.13 (1)
G-H	-2070 / 0	-102.1	-102.1 0.84 (1)	4.02	N-I	-559 / 0	0.29 (1)
H-I	-2070 / 0	-102.1	-102.1 0.84 (1)	4.02	M-I	-111 / 174	0.07 (1)
I-J	-2481 / 0	-102.1	-102.1 0.72 (1)	3.65	B-Q	0 / 2057	0.46 (1)
J-K	0 / 42	-102.1	-102.1 0.14 (1)	10.00	M-J	0 / 2057	0.46 (1)
K-L	-2284 / 0	0.0	0.0 0.24 (1)	5.94			
L-J	-2284 / 0	0.0	0.0 0.24 (1)	5.94			
R-Q	0 / 0	-38.5	-38.5 0.28 (3)	10.00			
Q-P	0 / 2022	-38.5	-38.5 0.54 (2)	10.00			
P-O	0 / 1691	-38.5	-38.5 0.46 (2)	10.00			
O-N	0 / 1691	-38.5	-38.5 0.46 (2)	10.00			
N-M	0 / 2022	-38.5	-38.5 0.54 (2)	10.00			
M-L	0 / 0	-38.5	-38.5 0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 28.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2010, NBC 2012
 - CSA 085-09, CSA 085-14
 - TPIC 2011, TPIC 2014

(85 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/369 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/369 (0.18")

CSF: TO=0.72/1.00 (B-C1), SC=0.54/1.00 (P-C2), WS=0.48/1.00 (B-C1), SS=0.25/1.00 (B-C1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

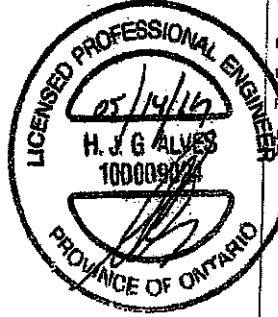
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	918 354 1987 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.88 (C) (INPUT = 0.80)
 METAL = 0.89 (C) (INPUT = 1.00)

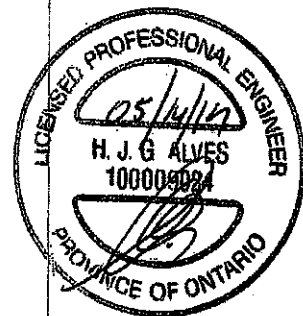


JOB NAME 403060	TRUSS NAME T18	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 MITak Industries, Inc. Tue May 14 11:25:33 2019 Page 2
 ID:vk_15VXL54fY8brNpe87hlyMICK-lzYJO73FR1pDH15OI2SNATFLPNiH1v_zT3VhXHzGcaG

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	3-0-12	1	1	144	BACK	VERT	TOTAL	---	---
T	5-10-4	1	1	144	BACK	VERT	TOTAL	---	---
U	7-10-4	1	1	144	BACK	VERT	TOTAL	---	---
V	9-2-12	1	1	144	BACK	VERT	TOTAL	---	---
W	12-0-4	1	1	144	BACK	VERT	TOTAL	---	---
X	3-0-12	1	1	---	BACK	VERT	TOTAL	---	---
Y	5-10-4	1	1	---	BACK	VERT	TOTAL	---	---
Z	7-10-4	1	1	---	BACK	VERT	TOTAL	---	---
AA	9-2-12	1	1	---	BACK	VERT	TOTAL	---	---
AB	12-0-4	1	1	---	BACK	VERT	TOTAL	---	---

THIS STRUCTURE MUST BE
 CONSTRUCTED TO MEET OR
 EXCEED THE PROVISIONS OF
 THE ONTARIO BUILDING CODE

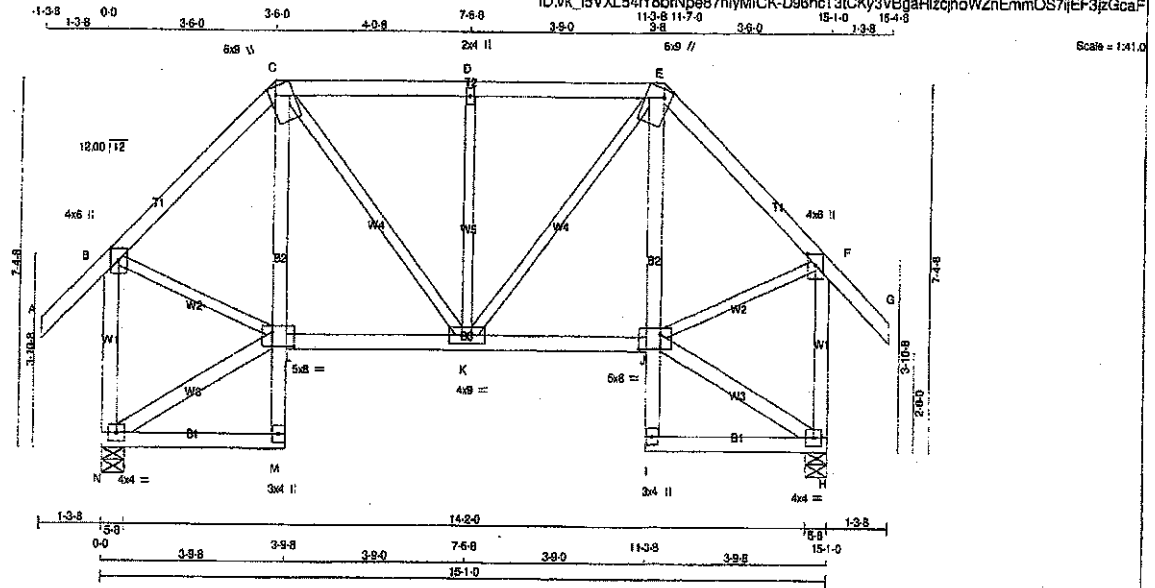


Structural component only
 DWG# T-1911509-2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403060	T19	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:vk_15VXL54Y8brNpe87hlyMICK-D98hcT3ICKy3VBgaRlzcjhoWZnEmmOS7iEF3zGcaF



TOTAL WEIGHT = 94 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTVW+m	MT20	6.0	9.0	Edge	1.75
D	TMVW+w	MT20	2.0	4.0		
E	TTVW+m	MT20	6.0	9.0	Edge	1.75
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMVW1-i	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BMVWW-i	MT20	5.0	8.0	3.50	5.50
K	BMVWW-i	MT20	4.0	9.0		
L	BMVWW-i	MT20	5.0	8.0	3.50	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORIZ	DOWN	HORIZ
N	1203	0	1203	0
H	1203	0	1203	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
N	890	518 / 0	158 / 0	0 / 0	0 / 0	213 / 0
H	890	518 / 0	158 / 0	0 / 0	0 / 0	213 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD	LC1	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00	N-L	-5 / 0	0.09 (1)	
B-C	-823 / 0	-102.1	-102.1	0.23 (1)	6.25	B-L	0 / 621	0.14 (1)	
C-D	-837 / 0	-102.1	-102.1	0.28 (1)	6.25	J-H	-5 / 0	0.00 (1)	
D-E	-837 / 0	-102.1	-102.1	0.28 (1)	6.25	J-F	0 / 621	0.14 (1)	
E-F	-823 / 0	-102.1	-102.1	0.23 (1)	6.25	K-D	-496 / 0	0.21 (1)	
F-G	0 / 50	-102.1	-102.1	0.14 (1)	10.00	C-K	0 / 414	0.09 (1)	
N-B	-1130 / 0	0.0	0.0	0.29 (1)	7.46	K-E	0 / 414	0.09 (1)	
H-F	-1130 / 0	0.0	0.0	0.29 (1)	7.46				
N-M	0 / 4	-38.5	-38.5	0.12 (3)	10.00				
M-L	0 / 89	0.0	0.0	0.02 (2)	10.00				
L-C	-82 / 67	0.0	0.0	0.02 (3)	7.81				
L-K	0 / 578	-38.5	-38.5	0.20 (2)	10.00				
K-J	0 / 578	-38.5	-38.5	0.20 (2)	10.00				
J-I	0 / 88	0.0	0.0	0.02 (2)	10.00				
J-E	-82 / 67	0.0	0.0	0.02 (3)	7.81				
I-H	0 / 4	-38.5	-38.5	0.12 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS1: TO=0.29/1.00 (B-A-E1), BC=0.20/1.00 (J-K-2), WS=0.21/1.00 (D-K-1), SS=0.20/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

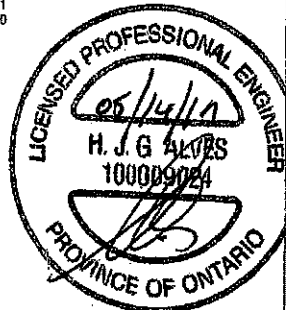
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1887 798 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.79 (F) (INPUT = 0.90)
SI METAL = 0.39 (F) (INPUT = 1.00)

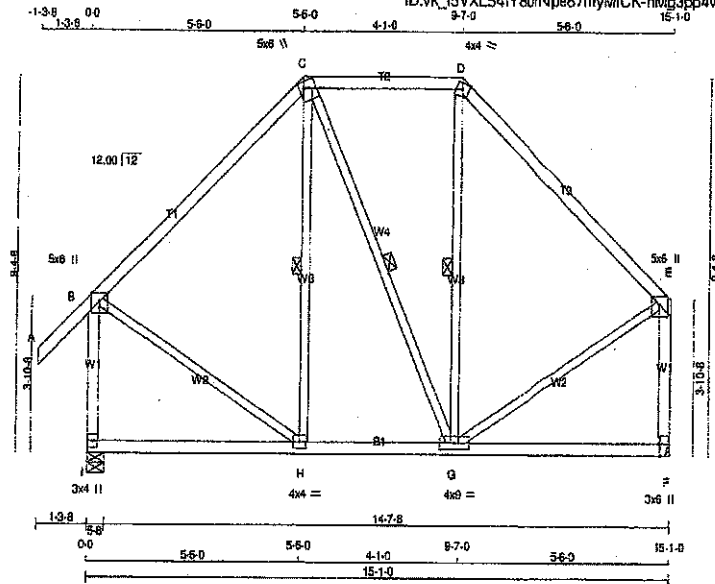


Structural component only
DWG# T-1911510

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403060	T20	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:vk_15VXL54Y8BrNpe87hlyMICK-hMg3pp4Wze4w6LFn_SUfUKTBaWVHGxN_ob9zGCaE



TOTAL WEIGHT = 3 X 83 = 249 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2	
C - D	2x4	DRY	No.2	
D - E	2x4	DRY	No.2	
E - F	2x4	DRY	No.2	
F - G	2x4	DRY	No.2	
G - H	2x4	DRY	No.2	
H - I	2x4	DRY	No.2	
I - J	2x4	DRY	No.2	
ALL WEBS	2x3	DRY	No.2	
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-n	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+H	MT20	3.0	6.0	Edge	0.50
G	BMVWW-I	MT20	4.0	9.0		
H	BMVWW-I	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
I	1203	0	1203	0
F	1060	0	1060	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	890	519 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0
F	792	438 / 0	158 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-H, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (L/C)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (L/C)
FR-TO						FR-TO			
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00	H-C	-85 / 126	0.04 (1)	
B-C	-598 / 0	-102.1	-102.1	0.40 (1)	6.25	C-G	0 / 0	0.00 (1)	
C-D	-421 / 0	-102.1	-102.1	0.22 (1)	6.25	G-D	-58 / 129	0.04 (1)	
D-E	-598 / 0	-102.1	-102.1	0.40 (1)	6.25	B-H	0 / 498	0.11 (1)	
E-F	-1112 / 0	0.0	0.0	0.28 (1)	7.51	G-E	0 / 498	0.11 (1)	
F-G	-989 / 0	0.0	0.0	0.25 (1)	7.81				
I-H	0 / 0	-38.5	-38.5	0.20 (3)	10.00				
H-G	0 / 421	-38.5	-38.5	0.23 (2)	10.00				
G-F	0 / 0	-38.5	-38.5	0.20 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF BCBC 2018, CBC 2012
-CSA 086-08, CSA 088-14
-TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/800 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/800 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.40/1.00 (B-C:1), BC=0.23/1.00 (G-H:2), WB=0.11/1.00 (B-H:1), SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

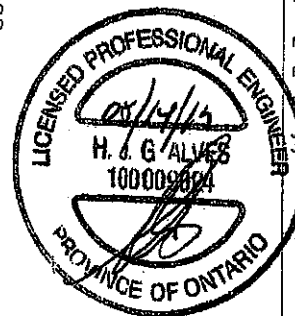
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JH GRIP=0.57 (B) (INPUT = 0.80)
JH METAL=0.27 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911511

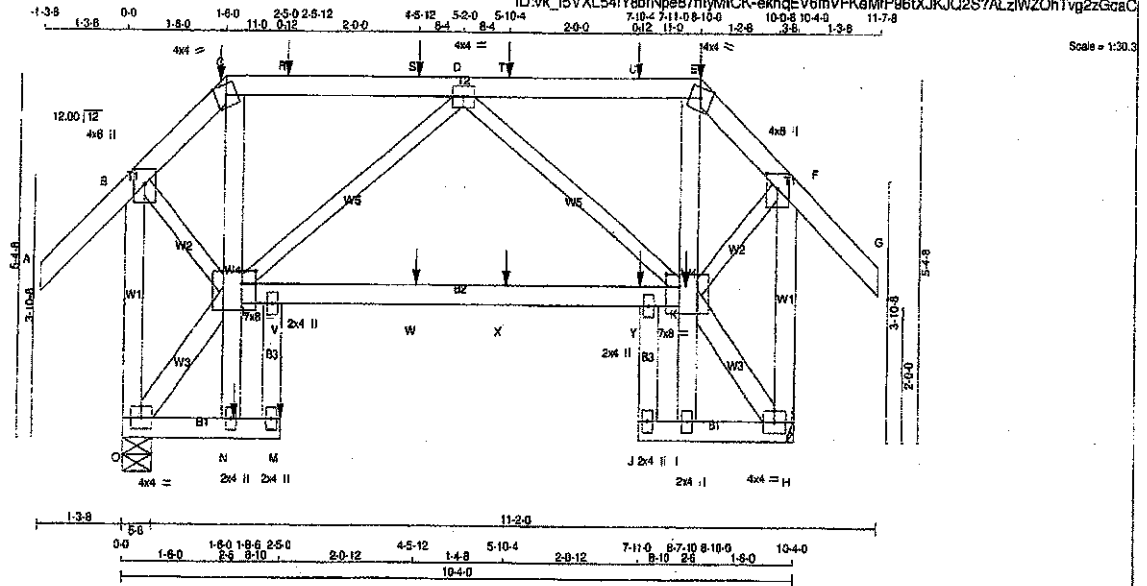
Tamarack Roof Truss, Burlington

Version 8.300 S Apr 23 2019 MTEK Industries, Inc. Tue May 14 11:25:36 2019 Page 1
ID:vk_15VXL54Y8bnppe87hlyMICK-AYES1958kyCnkVqzYA?4c6ts_bwqEDJP9jIM8czGcaD



TOTAL WEIGHT = 85 lb

Structural component only
DWG# T-1911512



TOTAL WEIGHT = 70 lb

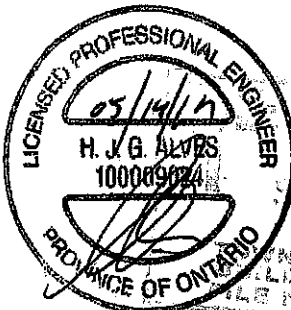
LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
O - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
O - M	2x4	DRY	No.2
L - K	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
B - L	2x3	DRY	No.2
K - F	2x3	DRY	No.2
L - D	2x3	DRY	No.2
D - K	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMVW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMVW-1	MT20	4.0	4.0		
I	BMVW-w	MT20	2.0	4.0		
J, M, P, Q	NP+w	MT20	2.0	4.0		
K	BMVWVW-1	MT20	7.0	8.0	2.25	2.50
L	BMVWVW-1	MT20	7.0	8.0	2.25	2.50
N	BMVW-w	MT20	2.0	4.0		
O	BMVW-1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT RECD
JT	VERT	DOWN	BRG
O	887	0	887
H	887	0	887

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-3.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	886	381 / 0	124 / 0	0 / 0	0 / 0	161 / 0	0 / 0
H	886	381 / 0	124 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0 / 50	-102.1	0.18 (1)
B-C	-472 / 0	-102.1	0.18 (1)
C-R	-311 / 0	-102.1	0.25 (1)
R-S	-311 / 0	-102.1	0.25 (1)
S-D	-311 / 0	-102.1	0.25 (1)
D-T	-311 / 0	-102.1	0.25 (1)
T-U	-311 / 0	-102.1	0.25 (1)
U-E	-311 / 0	-102.1	0.25 (1)
E-F	-472 / 0	-102.1	0.18 (1)
F-G	0 / 50	-102.1	0.18 (1)
G-B	-858 / 0	0.0	0.25 (1)
H-F	-858 / 0	0.0	0.25 (1)
O-N	0 / 8	-38.5	0.03 (3)
N-M	0 / 0	-38.5	0.03 (3)
L-V	0 / 588	-38.5	0.52 (2)
V-W	0 / 588	-38.5	0.52 (2)
W-X	0 / 588	-38.5	0.52 (2)
X-Y	0 / 588	-38.5	0.52 (2)
Y-K	0 / 588	-38.5	0.52 (2)
J-I	0 / 0	-38.5	0.02 (2)
I-H	0 / 8	-38.5	0.03 (3)

FACTORED CONCENTRATED LOADS (LBS)			
JT	LOC	LC1	MAX
O	1-8-0	1	144
H	5-10-0	1	144
K	8-7-10	1	144
M	2-5-0	1	144
N	1-8-6	1	144
R	2-5-12	1	143
S	4-5-12	1	144
T	5-10-4	1	144
U	7-10-4	1	143
W	4-5-12	1	144
X	5-10-4	1	144

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.6 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/614 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/488 (0.25")

CSI: TC=0.25/1.00 (C-D:1), BC=0.52/1.00 (K-L:2), WB=0.13/1.00 (D-K:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

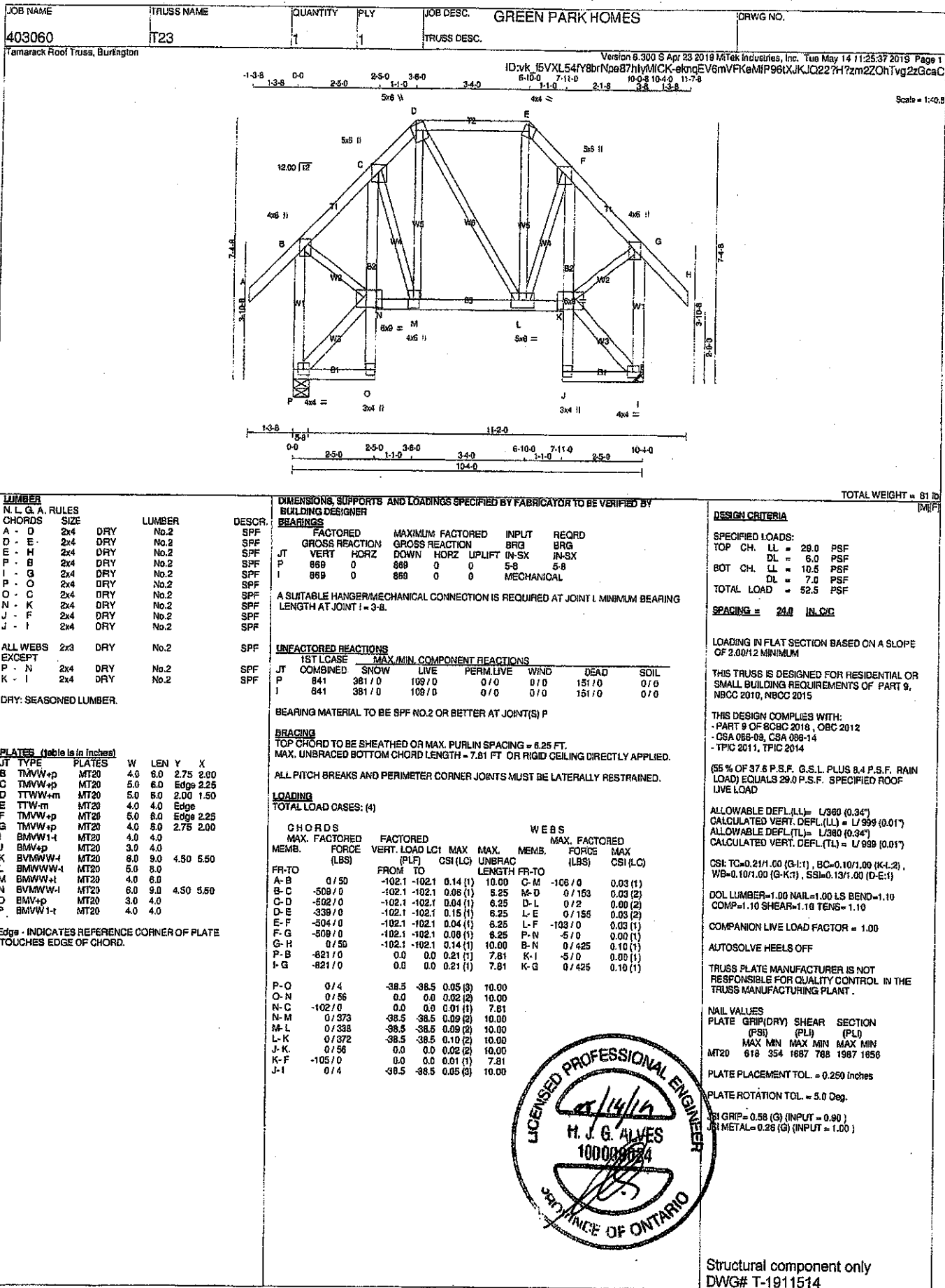
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.80)
JSI METAL= 0.23 (B) (INPUT = 1.00)

Structural component only

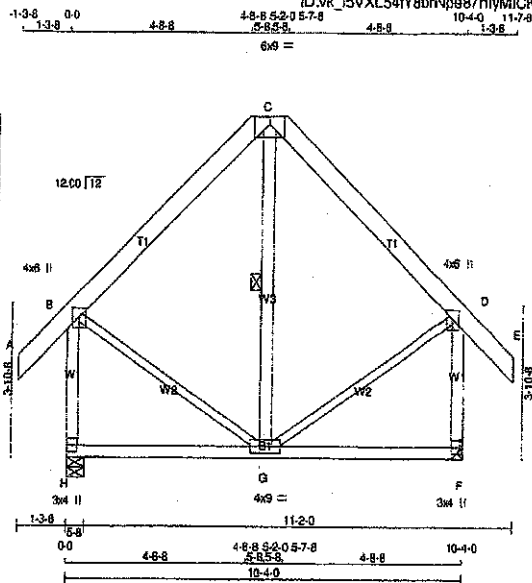
DWG# T-1911513 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																																								
403060	T22	1	1	GREEN PARK HOMES TRUSS DESC.																																									
Tamarack Roof Truss, Burlington																																													
Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:25:37 2019 Page 2 ID:vk f5VXL54fY8brNpe87htyMiCK-eknqEV8mVFKeMfP96ixJKJQ2S?ALzIWZOnTyg2xGcaC																																													
<table border="1"><thead><tr><th colspan="10">FACTORED CONCENTRATED LOADS (LBS)</th></tr><tr><th>JT</th><th>LOC.</th><th>LC1</th><th>MAX-</th><th>MAX+</th><th>FACE</th><th>DIR.</th><th>TYPE</th><th>HEEL</th><th>CONN.</th></tr><tr><th>Y</th><th>7-11-0</th><th>1</th><th>1</th><th>--</th><th>FRONT</th><th>VERT</th><th>TOTAL</th><th>--</th><th>--</th></tr></thead><tbody><tr><td colspan="10"></td></tr></tbody></table>						FACTORED CONCENTRATED LOADS (LBS)										JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	Y	7-11-0	1	1	--	FRONT	VERT	TOTAL	--	--										
FACTORED CONCENTRATED LOADS (LBS)																																													
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																																				
Y	7-11-0	1	1	--	FRONT	VERT	TOTAL	--	--																																				
LICENSED PROFESSIONAL ENGINEER 05/14/19 H. J. G ALVES 100009024 PROVINCE OF ONTARIO Structural component only DWG# T-1911513 -2																																													



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403060	T24	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington



Scale = 1/8" = 1'-0"

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF		
C - E	2x6	DRY	No.2	SPF		
H - B	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
G - C	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	8.0	2.75	2.00
C	TMTWn	MT20	6.0	9.0	2.25	4.50
D	TMVW+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWV-1	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
H	875	0	875	0
F	875	0	875	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	645	384 / 0	109 / 0	0 / 0	0 / 0	152 / 0	0 / 0
F	645	384 / 0	109 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX. FACTORED	FACTORED	VERT. LOAD LC1
	(LBS)	(PLF)	CS1 (LC)	(LBS)	(PLF)	CS1 (LC)
FR-TO		FROM TO		FR-TO		FROM TO
A-B	0 / 52	-102.1 -102.1	0.09 (1)	10.00	B-G	0 / 280
B-C	-330 / 0	-102.1 -102.1	0.17 (1)	8.25	G-D	0 / 280
C-D	-330 / 0	-102.1 -102.1	0.17 (1)	8.25	G-C	-61 / 159
D-E	0 / 52	-102.1 -102.1	0.06 (1)	10.00		
H-B	-799 / 0	0.0 0.0	0.18 (1)	7.81		
F-D	-799 / 0	0.0 0.0	0.18 (1)	7.81		
H-G	0 / 0	-38.5 -38.5	0.24 (3)	10.00		
G-F	0 / 0	-38.5 -38.5	0.24 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24" IN.CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.18/1.00 (B-H:1), BC=0.24/1.00 (G-H:3), WB=0.06/1.00 (B-G:1), SS=0.13/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

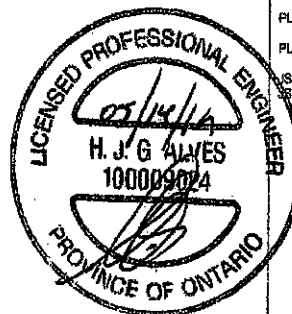
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.58 (B) (INPUT = 0.90)
SI METAL = 0.23 (D) (INPUT = 1.00)



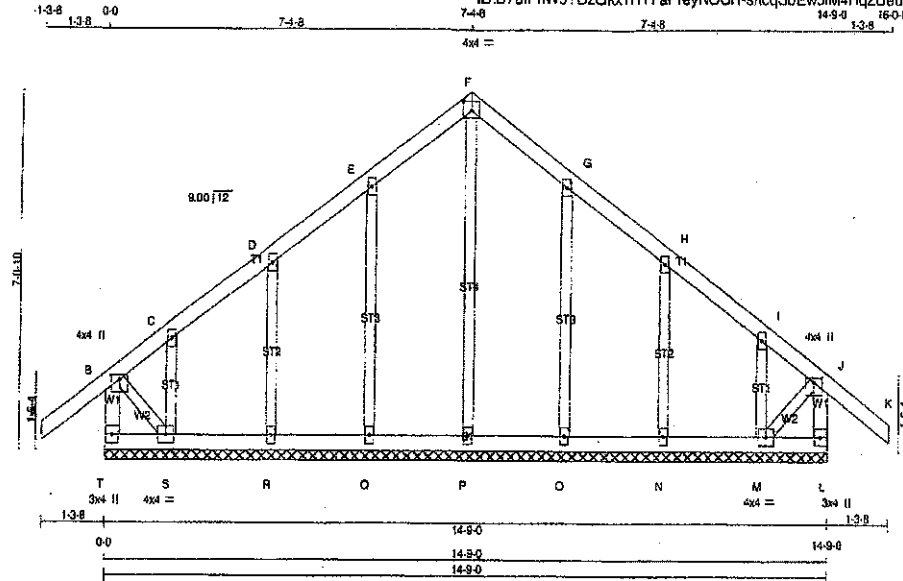
Structural component only
DWG# T-1911515

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401607	G100	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/4" = 1'-0"

LUMBER	SIZE	DRY	No.2	DESCR.
N. L. G. A. RULES				
CHORDS				
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" O.C.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMWV+p	MT20	4.0	4.0	1.00 2.00
C, D, E, G, H, I				
C TMW+w	MT20	2.0	4.0	
F TTW-p	MT20	4.0	4.0	2.25 2.00
J TMWV+p	MT20	4.0	4.0	1.00 2.00
L BMV1+p	MT20	3.0	4.0	
M BMWV1+1	MT20	4.0	4.0	
N, O, P, Q, R				
N BMW1+w	MT20	2.0	4.0	
S BMWV1+1	MT20	4.0	4.0	
T BMV1+p	MT20	3.0	4.0	

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	UNBRAC	WEBS	MAX. FACTORED	MAX
MEMB.	FORCE	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	FORCE	MAX	UNBRAC
(LBS)	(PLF)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO		FROM				FR-TO			
T-B	-310 / 0	0.0	0.0	0.03 (1)	7.81	P-F	-157 / 0	0.13 (1)	
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	Q-E	-229 / 0	0.11 (1)	
B-C	-68 / 0	-102.1	-102.1	0.13 (1)	8.25	R-D	-210 / 0	0.05 (1)	
C-D	-16 / 0	-102.1	-102.1	0.05 (1)	6.25	S-C	-86 / 0	0.01 (1)	
D-E	-18 / 0	-102.1	-102.1	0.08 (1)	8.25	O-G	-229 / 0	0.11 (1)	
E-F	-25 / 0	-102.1	-102.1	0.08 (1)	8.25	N-H	-210 / 0	0.05 (1)	
F-G	-25 / 0	-102.1	-102.1	0.05 (1)	6.25	M-I	-86 / 0	0.01 (1)	
G-H	-16 / 0	-102.1	-102.1	0.06 (1)	8.25	B-S	0 / 30	0.01 (1)	
H-I	-16 / 0	-102.1	-102.1	0.05 (1)	6.25	M-J	0 / 30	0.01 (1)	
I-J	-88 / 0	-102.1	-102.1	0.13 (1)	6.25				
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00				
L-J	-310 / 0	0.0	0.0	0.03 (1)	7.81				
T-S	0 / 0	-38.5	-38.5	0.02 (3)	10.00				
S-R	0 / 19	-38.5	-38.5	0.03 (2)	10.00				
R-Q	0 / 14	-38.5	-38.5	0.02 (3)	10.00				
Q-P	0 / 10	-38.5	-38.5	0.02 (3)	10.00				
P-O	0 / 10	-38.5	-38.5	0.02 (3)	10.00				
O-N	0 / 14	-38.5	-38.5	0.02 (3)	10.00				
N-M	0 / 19	-38.5	-38.5	0.03 (2)	10.00				
M-L	0 / 0	-38.5	-38.5	0.02 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (M-N:2), WS=0.13/1.00 (F-P:1), SI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1687 798 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

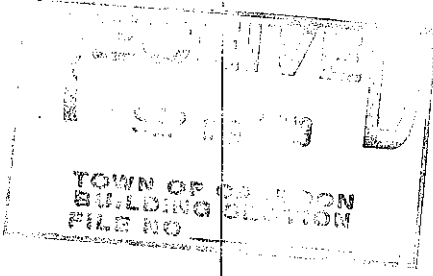
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.24 (B) (INPUT = 0.90)

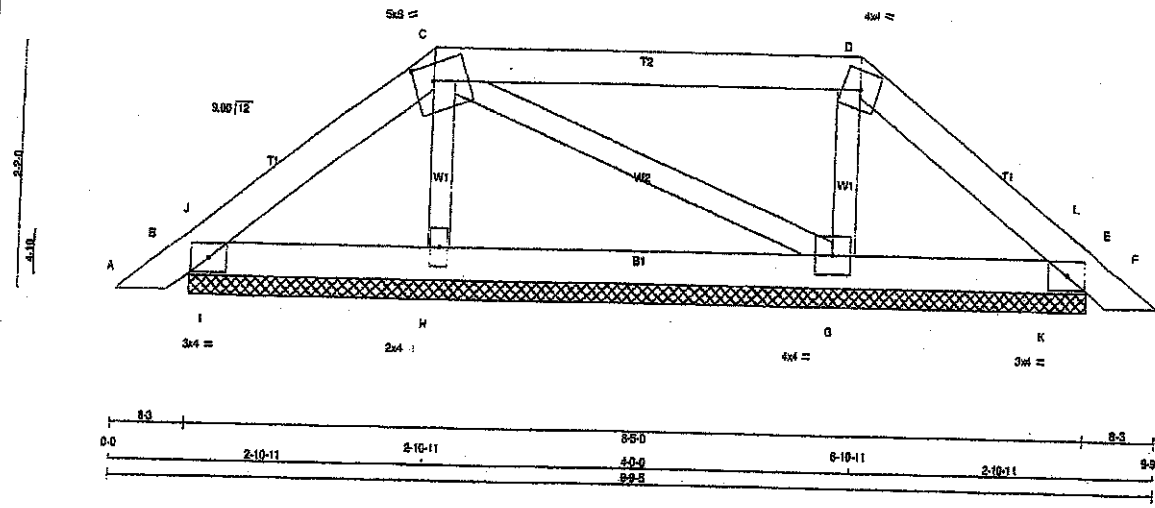
JSI METAL=0.12 (E) (INPUT = 1.00)

Structural component only

DWG# T-1911497



JOB NAME: 403045 TRUSS NAME: P1 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES DRWG NO. 1
 Truss Desc: Version 8.300 S Apr 23 2019 MTEK Industries, Inc. Tue May 14 10:41:17 2019 Page 1
 ID: D7U1PnW9?DzGkx1HTFaFreyNCC4-W73EyyvEx2B3E12xHEoYXNFSDkRwR1kr68IDzGdM
 6-10-11 9-9-5
 Scale = 1/4" = 1'-0"



LUMBER
 N.L.G.A. RULES
 CHORDS SIZE LUMBER
 A - C 2x4 DRY No.2
 C - D 2x4 DRY No.2
 D - F 2x4 DRY No.2
 B - E 2x4 DRY No.2
 ALL WEBS 2x3 DRY No.2
 DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB-1	MT20	3.0	4.0		
C	TTW-m	MT20	5.0	8.0	1.75	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMB-1	MT20	3.0	4.0		
G	BMW-1	MT20	4.0	4.0		
H	BMW-1	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT
JT	257	0	0	0
B	243	0	0	0
E	243	0	0	0
H	388	0	0	0
G	417	0	0	0

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE	2ND CASE	3RD CASE	4TH CASE	5TH CASE	6TH CASE	7TH CASE	8TH CASE	9TH CASE	10TH CASE
B	183	130/0	16/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
E	173	122/0	16/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
H	295	144/0	73/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0
G	316	161/0	73/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0	0/0

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	FACTORED MOMENT (LBS-FT)	WEBS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	FACTORED MOMENT (LBS-FT)
FR-TO										
A-B		0/17	-102.1	-102.1	0.03 (1)	H-C	-232/0	0.03 (1)		
B-J		-75/0	-102.1	-102.1	0.02 (1)	C-G	-21/0	0.01 (1)		
J-C		-98/0	-102.1	-102.1	0.08 (1)	G-D	-256/0	0.04 (1)		
C-D		-42/0	-102.1	-102.1	0.28 (1)	I-J	-148/0	0.00 (1)		
D-L		-75/0	-102.1	-102.1	0.08 (1)	K-L	-148/0	0.00 (1)		
L-E		-51/0	-102.1	-102.1	0.02 (1)					
E-F		0/17	-102.1	-102.1	0.03 (1)					
B-I		0/75	-38.5	-38.5	0.07 (1)					
I-H		0/75	-38.5	-38.5	0.08 (2)					
H-G		0/81	-38.5	-38.5	0.08 (2)					
G-K		0/57	-38.5	-38.5	0.08 (2)					
K-E		0/57	-38.5	-38.5	0.08 (1)					

DESIGN CRITERIA
 SPECIFIED LOADS:
 TOP CH. LL = 20.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C
 LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

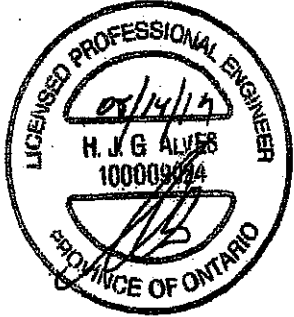
THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2010, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPC 2011, TPC 2014
 (65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.28/1.00 (C-D-1), BC=0.08/1.00 (H-I-2), WB=0.04/1.00 (D-G-1), SS=0.16/1.00 (C-D-1)
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP = 0.20 (B) (INPUT = 0.90)
 JSI METAL = 0.05 (H) (INPUT = 1.00)

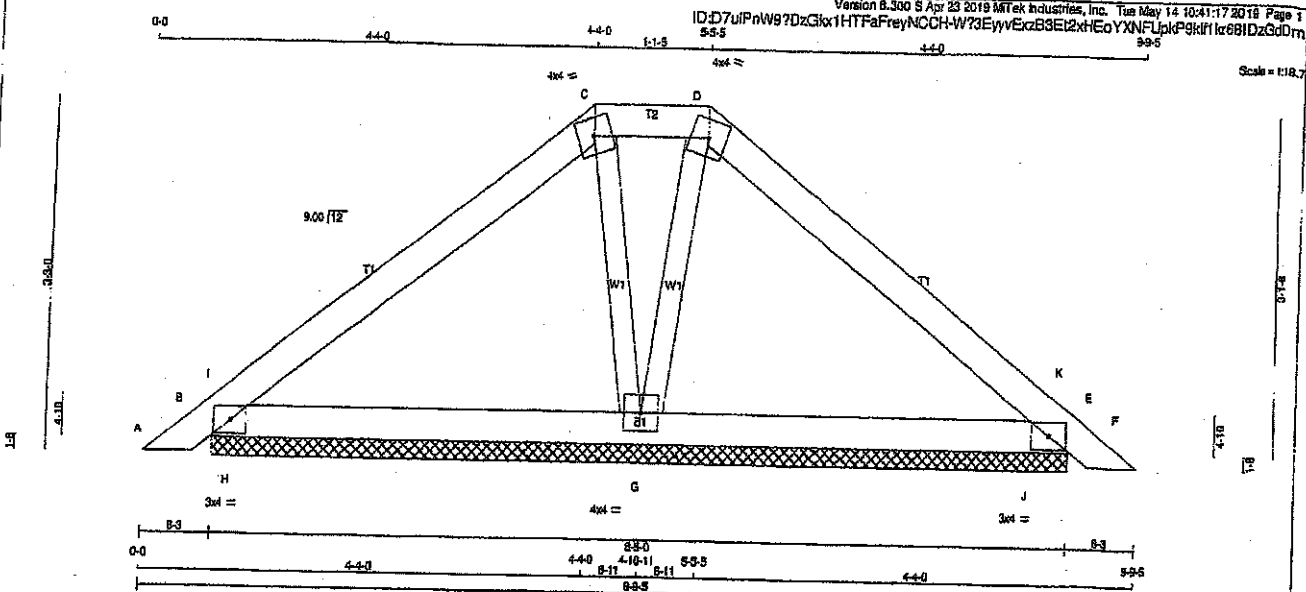


Structural component only
 DWG# T-1911479

JOB NAME 403045	TRUSS NAME P2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.300 5 Apr 23 2019 MTEK Industries, Inc. Tue May 14 10:41:17 2019 Page 1
ID: D7uIPnW9?DzGkx1HTFaFreyNCH-W73EyyvExzBSEI2xHCoYXNfLpkP9k1f1kr68IDzGd0m
55-5 9-5-5 9-5-5



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB14	MT20	3.0	4.0		
G	BMW14	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
B	401	0	401	0	8-5-0	8-5-0
E	390	0	390	0	8-5-0	8-5-0
G	514	0	514	0	8-5-0	8-5-0

UNFACTORED REACTIONS

JT	1ST LOSE	MAX/MIN COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
B	293	182/0	43/0	0/0	0/0	67/0
E	285	179/0	42/0	0/0	0/0	65/0
G	390	197/0	92/0	0/0	0/0	102/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINTS (S), (E), (G)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LBS)	
FR-TO				FR-TO			
A-B	0/17	-102.1	-102.1 0.03 (1)	10.00	C-G	-127/0	0.02 (1)
B-I	-117/0	-102.1	-102.1 0.05 (1)	8.25	G-D	-128/0	0.02 (1)
I-C	-165/0	-102.1	-102.1 0.18 (1)	8.25	H-I	-304/76	0.00 (1)
C-D	-98/0	-102.1	-102.1 0.02 (1)	8.25	J-K	-327/55	0.00 (1)
D-K	-157/0	-102.1	-102.1 0.17 (1)	8.25			
K-E	-103/0	-102.1	-102.1 0.08 (1)	8.25			
E-F	0/17	-102.1	-102.1 0.03 (1)	10.00			
B-H	0/126	-38.5	-38.5 0.14 (1)	10.00			
H-G	0/126	-38.5	-38.5 0.20 (2)	10.00			
G-J	0/119	-38.5	-38.5 0.19 (2)	10.00			
J-E	0/119	-38.5	-38.5 0.15 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TC=0.18/1.00 (C-I), BC=0.20/1.00 (G-H), WB=0.02/1.00 (D-G), SB=0.28/1.00 (E-J)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1887 768 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.30 (B) (INPUT = 0.30)
JSI METAL = 0.08 (B) (INPUT = 1.00)



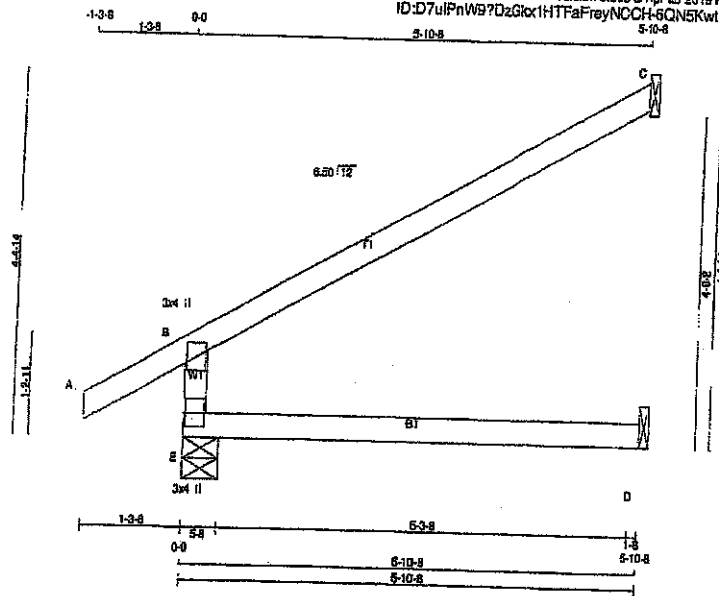
Structural component only
DWG# T-1911480

JOB NAME 403045	TRUSS NAME U1	QUANTITY 17	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.300 S Apr 23 2018 MTak Industries, Inc. Tue May 14 10:41:14 2019 Page 1
ID:D7uIPnW97DzG6c1HTFaFreyNCOCH-6CIN5KwtMe2pVNPJMc6FrvdsxWNBXOHb1wUhwzGdDp

Scale = 1/24.6



LUMBER	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE	PLATES			
B	TMV+p	MT20	3.0	4.0
E	SMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
SPF	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
SPF	DOWN	DOWN	HORIZ	UPLIFT	IN-SX
JT					
E	646	0	646	0	5-8
C	225	0	225	0	1-8
D	84	0	119	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED						
E	473	282/0	72/0	0/0	109/0	0/0
C	154	125/0	0/0	0/0	26/0	0/0
D	85	0/0	51/0	0/0	34/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS (LC)	MEMB. FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO
E-B	-514/0	0.0	0.0	0.21 (3)
A-B	0/33	-102.1	-102.1	0.13 (1)
B-C	-35/0	-102.1	-102.1	0.00 (1)
E-D	0/0	-38.5	-38.5	0.21 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 92.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, OBC 2012
- CSA 084-09, CSA 688-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/889 (0.08")
ALLOWABLE DEFL.(TL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/930 (0.08")

CS: TC=0.60/1.00 (B-C:1), SC=0.21/1.00 (D-E:3),
WB=0.00/1.00 (W&C), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

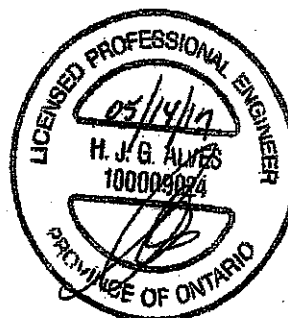
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 768 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.22 (E) (INPUT = 0.90)
JBI METAL = 0.15 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911476

JOB NAME

403060

TRUSS NAME

J4

QUANTITY

6

PLY

1

JOB DESC.

GREEN PARK HOMES

DRWG NO.

Tamarack Roof Truss, Burlington

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ID:vk_5VXL54fY8bNpe87hlyMICK-LOsAm6N98Re0aMpCwvgYrdspAvYqdoXn5G1wyzGcaJ

Scale = 1:30.0

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

LUMBER

DESCR.

SPF

SPF

SPF

SPF

ALL WEBS

2x3

DRY

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT

TYPE

PLATES

W

LEN

Y

X

B

TMVW+p

MT20

4.0

6.0

2.75

2.00

E

BMVW+p

MT20

2.0

4.0

F

BMV1+p

MT20

3.0

4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
F	314	0	314	0	5-8	5-8
C	10	0	10	0	5-8	5-8
D	29	0	37	0	1-8	1-8

SEE MITEK STANDARD DETAIL 597791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
F	222	182/0	16/0	0/0	44/0	0/0	0/0
C	7	6/-38	0/0	0/0	0/0	1/0	0/0
D	26	0/0	16/0	0/0	0/0	11/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-285/0	0.0	0.07 (1)	B-E	0/0	0.00 (1)	
A-B	0/50	-102.1	-102.1 0.14 (1)				
B-C	-47/0	-102.1	-102.1 0.14 (1)				
F-E	0/0	-38.5	-38.5 0.02 (3)				
E-D	0/0	-38.5	-38.5 0.02 (3)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

PART 9 OF BCBC 2018, OBC 2012

CSA 086-08, CSA 086-14

TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = 1/959 (0.00")

ALLOWABLE DEFL.(TL) = L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)

JSI METAL = 0.07 (B) (INPUT = 1.00)

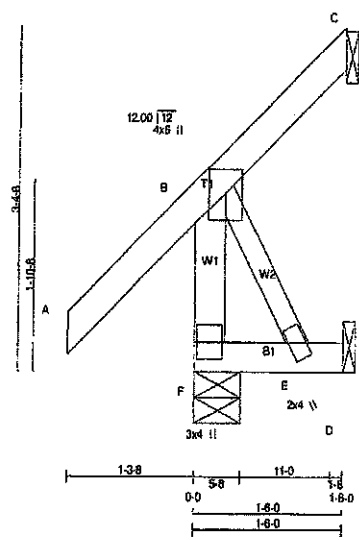
Structural component only

DWG# T-1911506

-1-3-8 1-3-8 0-0 1-6-0 1-6-0

Scale = 1:20.0

THIS TRUSS IS NOT TO BE USED FOR ANY OTHER BUILDING CODE
 EXCEPTED TO MEET THE REQUIREMENTS OF THE BUILDING CODE



TOTAL WEIGHT = 9 X 10 = 87 lb

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE		LUMBER	DESCR.	
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS		2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	314	0	314	0	5-8	5-8
C	10	0	10	0	1-8	1-8
D	28	0	27	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
F	222	162/0	16/0	0/0	0/0	44/0	0/0
C	7	61/36	0/0	0/0	0/0	1/0	0/0
D	28	0/0	16/0	0/0	0/0	11/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-285/0	0.0 0.0 0.03 (1)	7.81	B-E	0/0	0.00 (1)	
A-B	0/50	-102.1 -102.1 0.14 (1)	10.00				
B-C	-47/0	-102.1 -102.1 0.14 (1)	6.25				
F-E	0/0	-38.5 -38.5 0.02 (3)	10.00				
E-D	0/0	-38.5 -38.5 0.02 (3)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 22.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT-OFF.

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 25.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SS=0.00/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

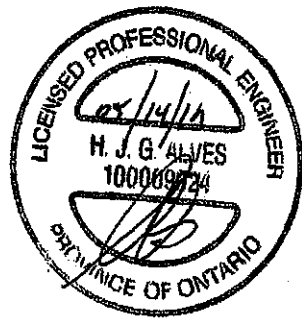
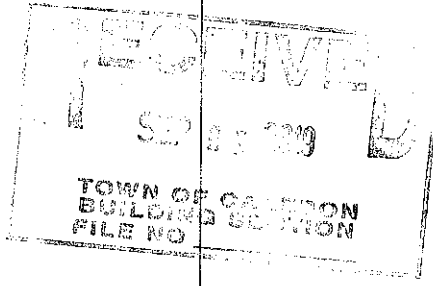
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

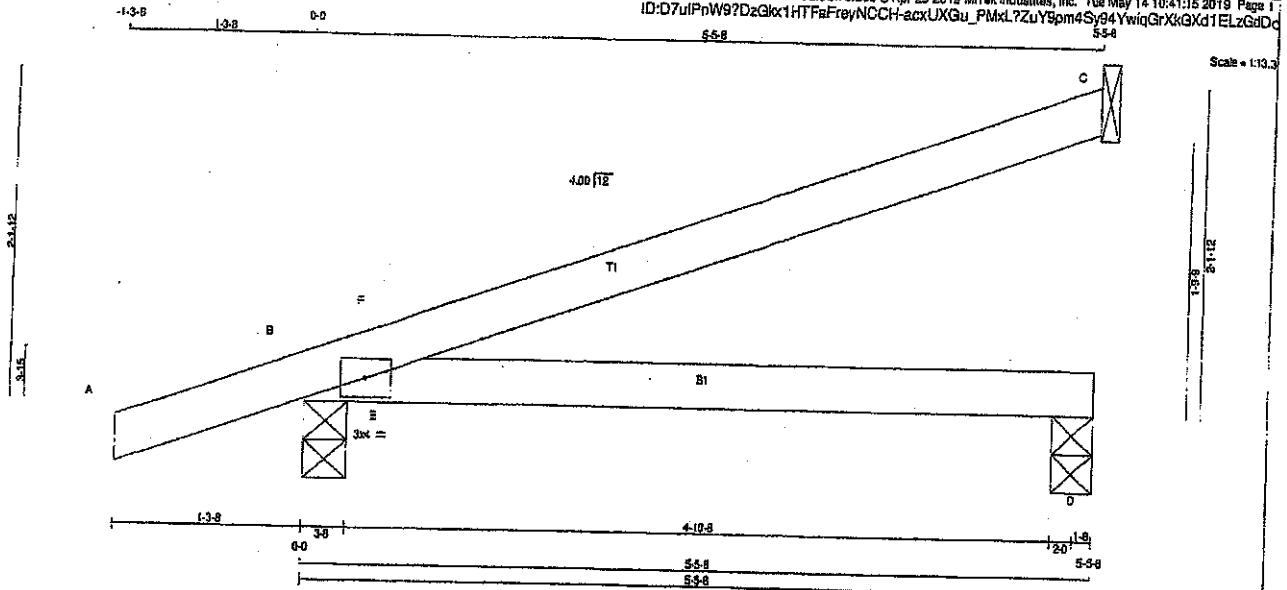
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
 JSI METAL = 0.07 (B) (INPUT = 1.00)



JOB NAME 403045	TRUSS NAME J90	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 5 Apr 23 2018 Mitek Industries, Inc. Tue May 14 10:41:15 2019 Page 1
ID:D7ulPnW9?DzGkx1HTFaFreyNCCH-accUXGu_PMXL?ZuY9pm4Sy94YwqGrXkGXdTELzGdDc



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4
B - D 2x4
LUMBER No.2
No.2
DRY: SEASONED LUMBER.

PLATES (table in inches)
JT TYPE PLATES W LEN Y X
B TMB14 MF20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HDRZ	DOWN	HDRZ		
JT	250	0	250	0	1-8	1-8
C	520	0	520	0	3-8	3-8
B	133	0	145	0	3-8	3-8

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	380	238 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	48 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 18.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/20	-102.1 -102.1	0.13 (1)	10.00	E-F	-254/ 122	0.00 (1)
B-F	-51/0	-102.1 -102.1	0.13 (3)	8.25			
F-C	0/6	-102.1 -102.1	0.42 (1)	10.00			
B-E	0/0	-38.5 -38.5	0.28 (1)	10.00			
E-D	0/0	-38.5 -38.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 5.6 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. CD

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLETES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 086-08, CSA 605-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/842 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/489 (0.13")

CS1: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SS=0.28/1.00 (B-E:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

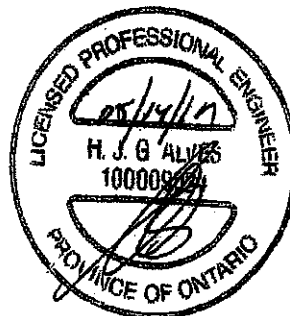
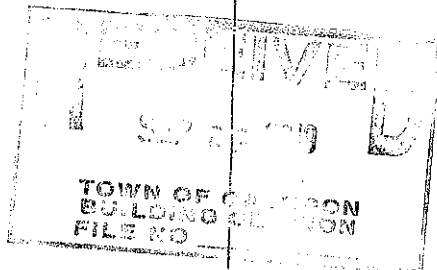
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.31 (B) (INPUT = 0.30)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911477



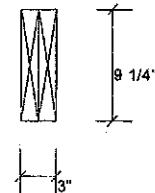
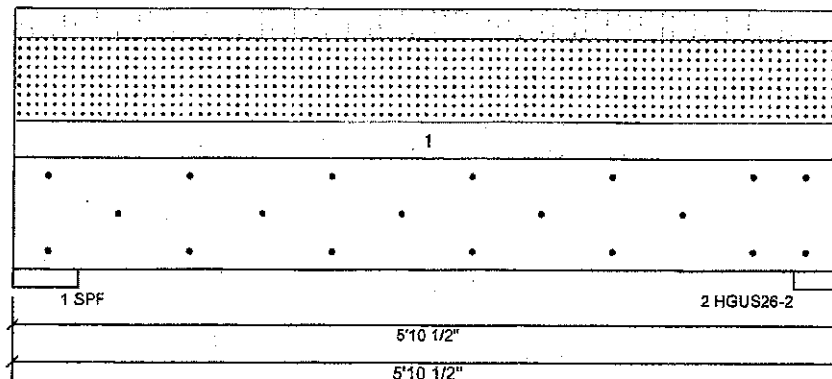
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 6 of 13

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	264	327	741	0
2	253	313	710	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	18%	408 / 1376	1784 L	1.25D+1.5S +L
2 - HGUS...	4.000"	23%	391 / 1318	1710 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2016 ft-lb	3'	6039 ft-lb	0.334 (33%)	1.25D+1.5S +L	L
Unbraced	2016 ft-lb	3'	5236 ft-lb	0.385 (39%)	1.25D+1.5S +L	L
Shear	1549 lb	1'2"	3984 lb	0.389 (39%)	1.25D+1.5S +L	L
LL Defl inch	0.018 (L/3535)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.024 (L/2573)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17911519
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-4-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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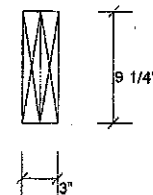
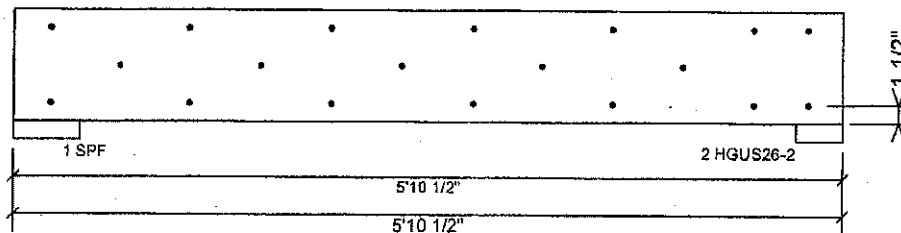
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 7 of 13

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	87.4 %
Load	297.3 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 11911519
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3250 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





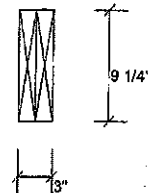
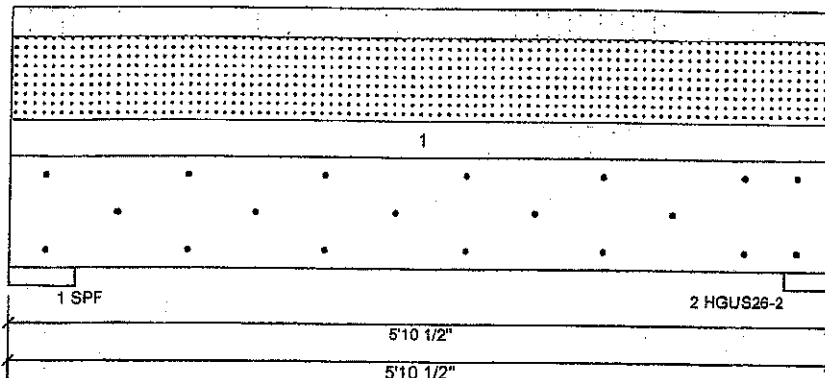
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 10 of 13

B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Piles: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brq	Live	Dead	Snow	Wind
1	269	333	756	0
2	258	319	724	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	18%	416 / 1403	1819 L	1.25D+1.5S +L
2 - HGUS...	4.000"	24%	389 / 1345	1744 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2056 ft-lb	3'	6039 ft-lb	0.341 (34%)	1.25D+1.5S +L	L
Unbraced	2056 ft-lb	3'	5236 ft-lb	0.393 (39%)	1.25D+1.5S +L	L
Shear	1579 lb	1'2"	3984 lb	0.396 (40%)	1.25D+1.5S +L	L
LL Defl inch	0.018 (U3466)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.025 (U2523)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 194521
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-6-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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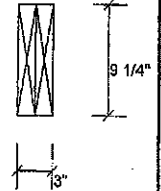
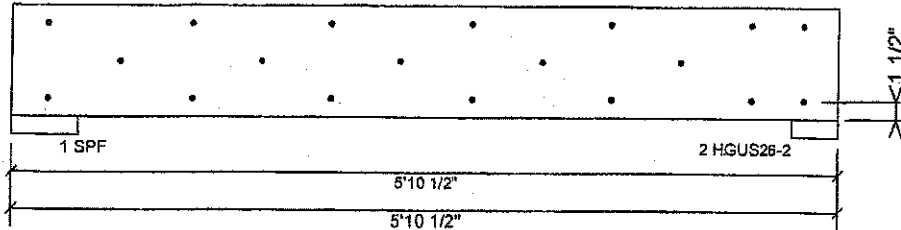
Client:
Project:
Address:

Date: 6/14/2019
Designer:
Job Name: 200598
Project #:

Page 11 of 13

B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

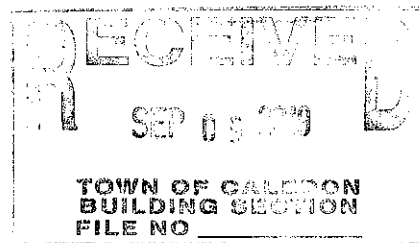
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	89.2 %
Load	303.2 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17911521
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3209 North Service Rd., ON
Canada
L7N3G2
(805) 395-1115



This design is valid until 12/11/2021





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Client:

Project:

Address:

Date: 5/14/2019

Designer:

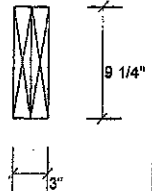
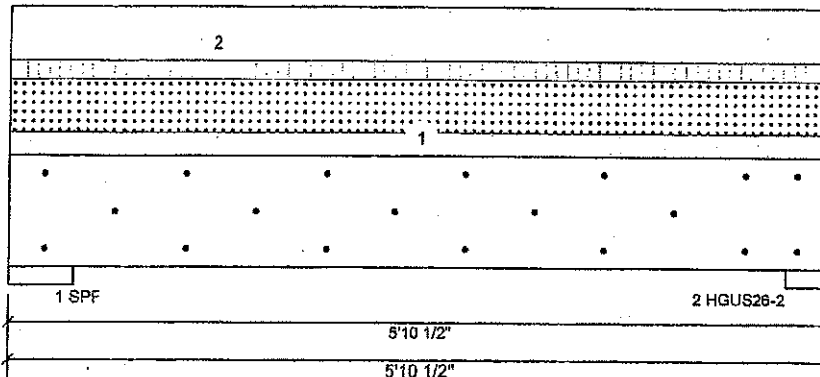
Job Name: 200598

Project #:

Page 8 of 13

B6 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
 Plies: 2
 Moisture Condition: Dry
 Deflection LL: 360
 Deflection TL: 360
 Importance: Normal

Application: Roof (Residential)
 Slope: 0/12
 Design Method: LSD
 Building Code: NBCC 2015
 Load Sharing: No
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	194	331	546	0
2	186	317	523	0

Bearings and Factored Reactions

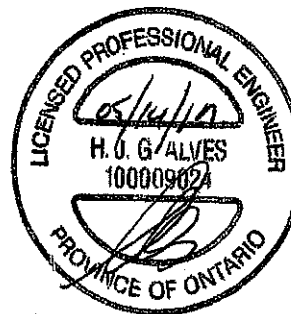
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	413 / 1013	1428 L	1.25D+1.5S +L
2 - HGUS...	4.000"	19%	386 / 971	1367 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1612 ft-lb	3'	6039 ft-lb	0.267 (27%)	1.25D+1.5S +L	L
Unbraced	1612 ft-lb	3'	-5236 ft-lb	0.308 (31%)	1.25D+1.5S +L	L
Shear	1209 lb	1'2"	3984 lb	0.303 (30%)	1.25D+1.5S +L	L
LL Def inch	0.013 (L/4801)	3'	0.174 (L/360)	0.070 (7%)	S+0.5L	L
TL Def inch	0.020 (L/3171)	3'	0.174 (L/360)	0.110 (11%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 11911520
 STRUCTURAL
 COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-2-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Uniform			Top	30 PLF	0 PLF	0 PLF	0 PLF	

**THIS STRUCTURE MUST BE
 CONSTRUCTED TO MEET OR
 EXCEED THE PROVISIONS OF
 THE ONTARIO BUILDING CODE**

This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
 3269 North Service Rd., CN
 Canada
 L7N3G2
 (905) 335-1115





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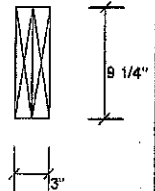
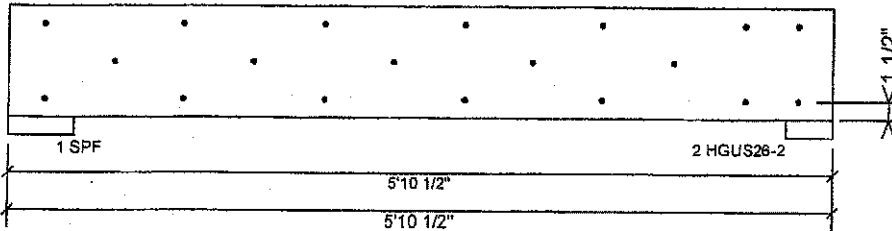
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 9 of 13

B6 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

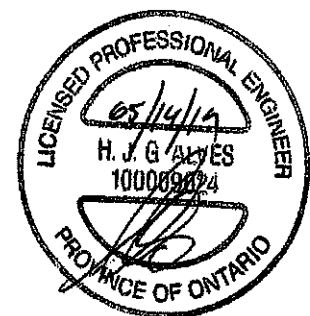
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	64.4 %
Load	218.9 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 11911520
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	Tamarack Roof Trusses 3269 North Service Rd., ON Canada L7N3G2 (905) 335-1115
	TAMARACK LUMBER INC ALPHA LUMBER GROUP

This design is valid until 12/11/2021





ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

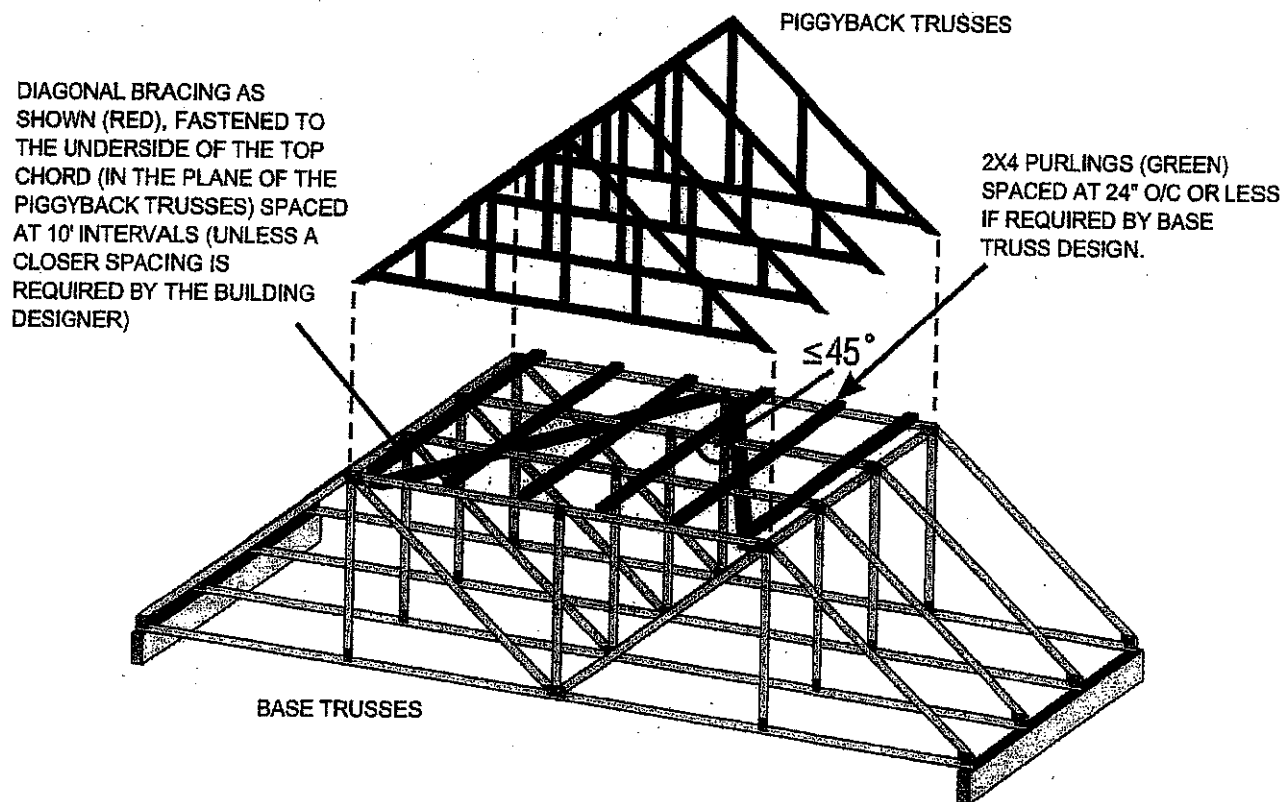
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

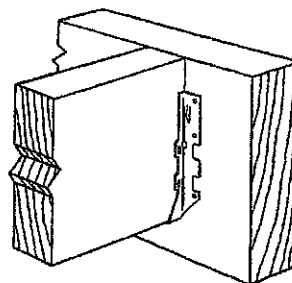
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

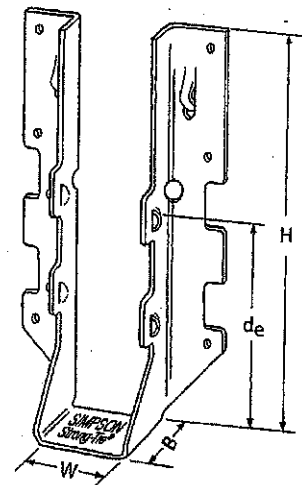
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



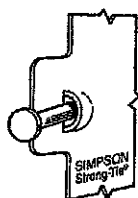
Typical LUS Installation



LUS28

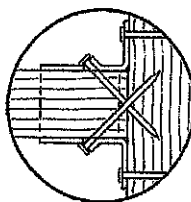
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1½	3½	1½	1½	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¾	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¾	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¾	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5½	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



UNITED STATES DESIGN

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17SPECULUS 047 exp. 5/19

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

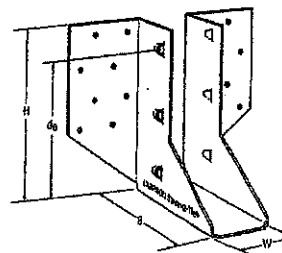
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

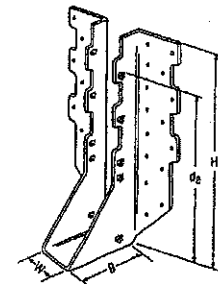
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

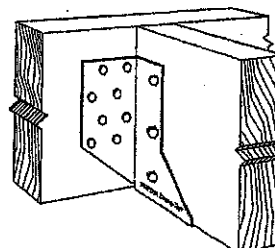
- See current catalogue for options



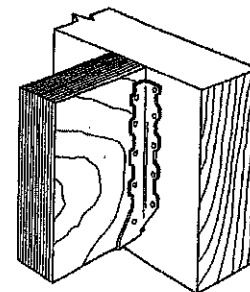
LJS26DS



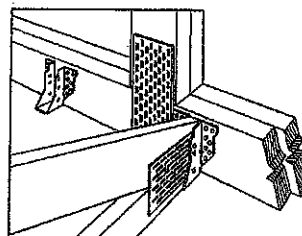
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



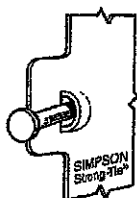
Typical HUS
Installation



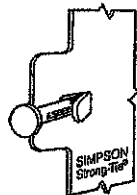
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.	Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.
LJS26DS	18	1 1/16	5	3 1/2	4 3/4	(16) 16d	(6) 16d		2055	4265	1460	4115
HUS26	16	1 1/8	5 1/4	3	3 1/16	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/32	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 1/32	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

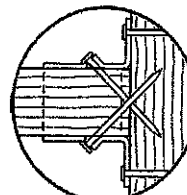
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,803,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

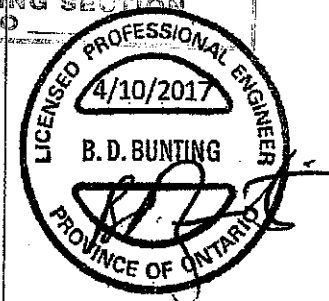
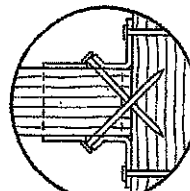
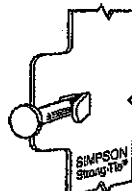
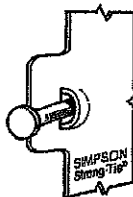
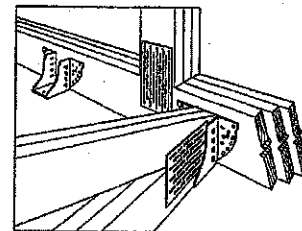
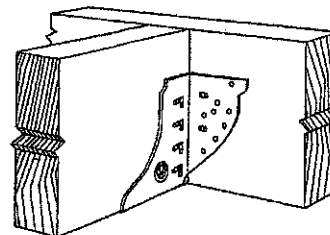
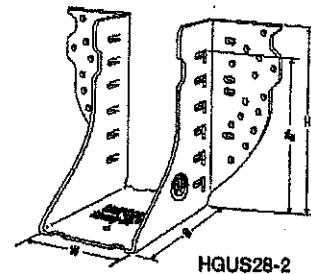


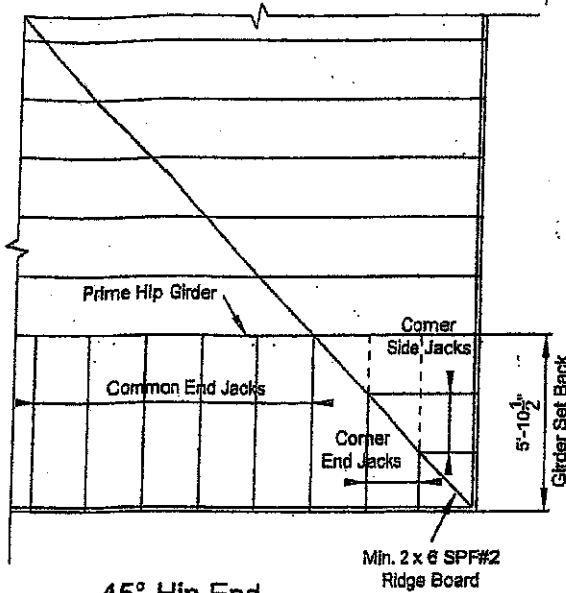
UNITED STATES DESIGN

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CS-17-001 HUS/LJS 12/17/17 exp. 6/19

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45° Hip End

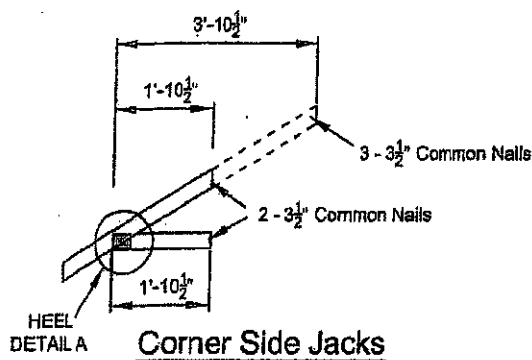
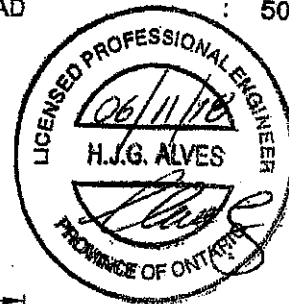
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

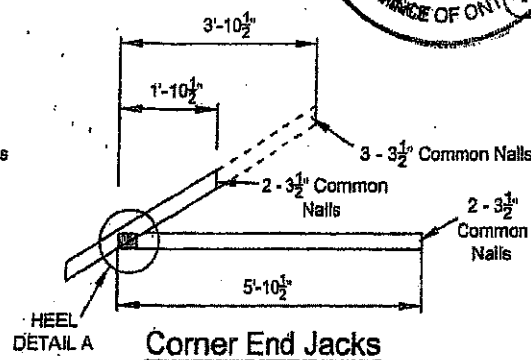
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

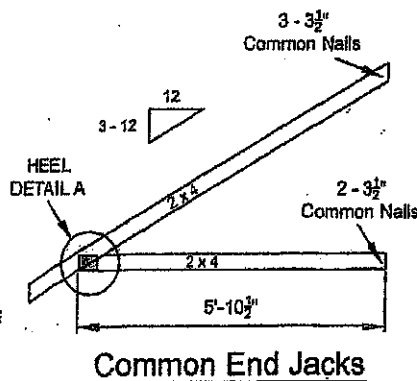
TOTAL LOAD : 50.5 P.S.F



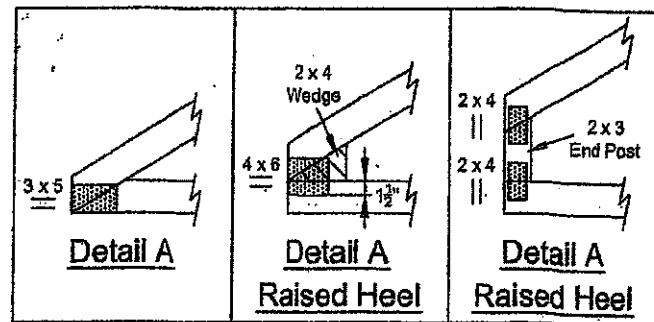
Corner Side Jacks



Corner End Jacks



Common End Jacks



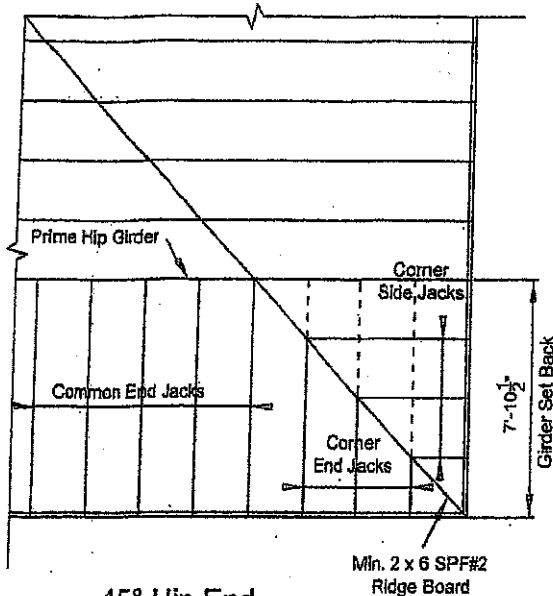
Detail A

**Detail A
Raised Heel**

**Detail A
Raised Heel**

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

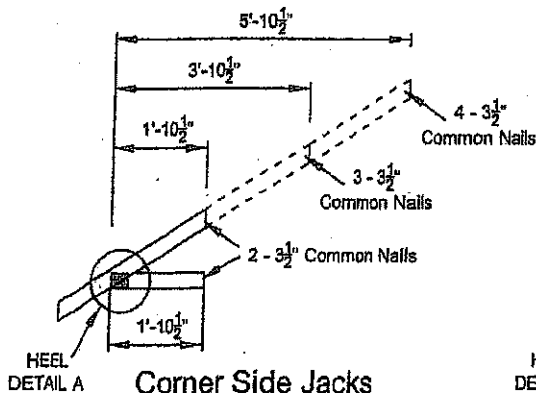
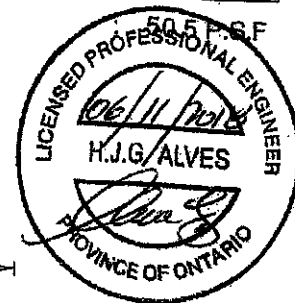
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

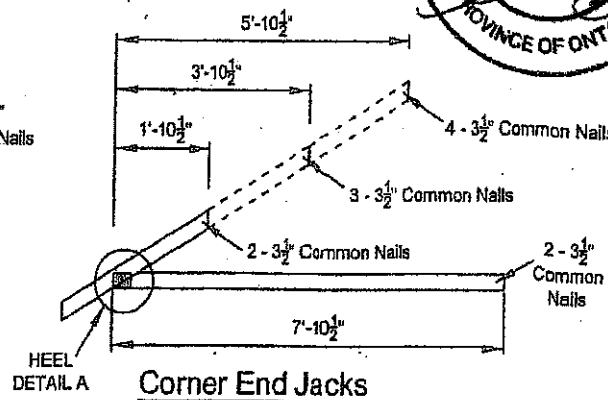
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

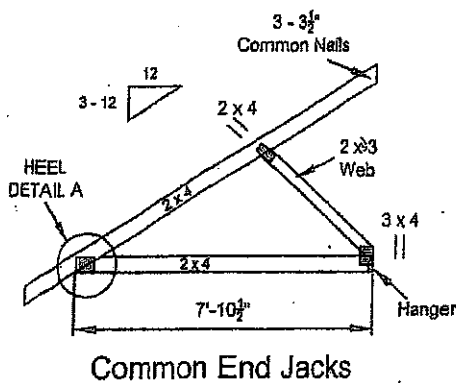
TOTAL LOAD



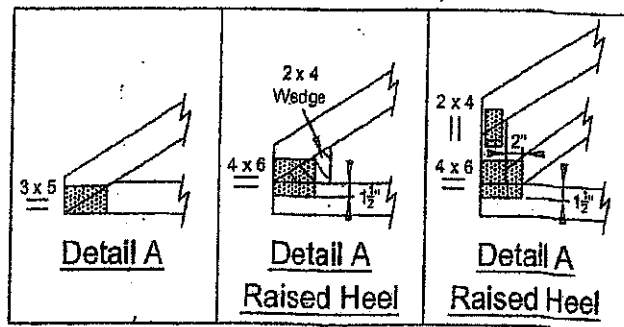
Corner Side Jacks



Corner End Jacks



Common End Jacks

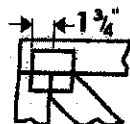


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

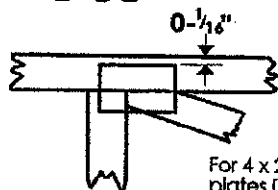
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0 1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

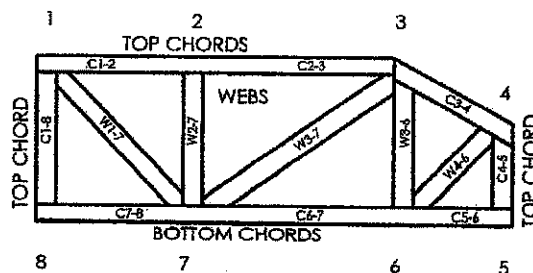


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses Design Standard for Bracing.
DSB-89: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate Institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

T-1800218

Feb 09, 2018