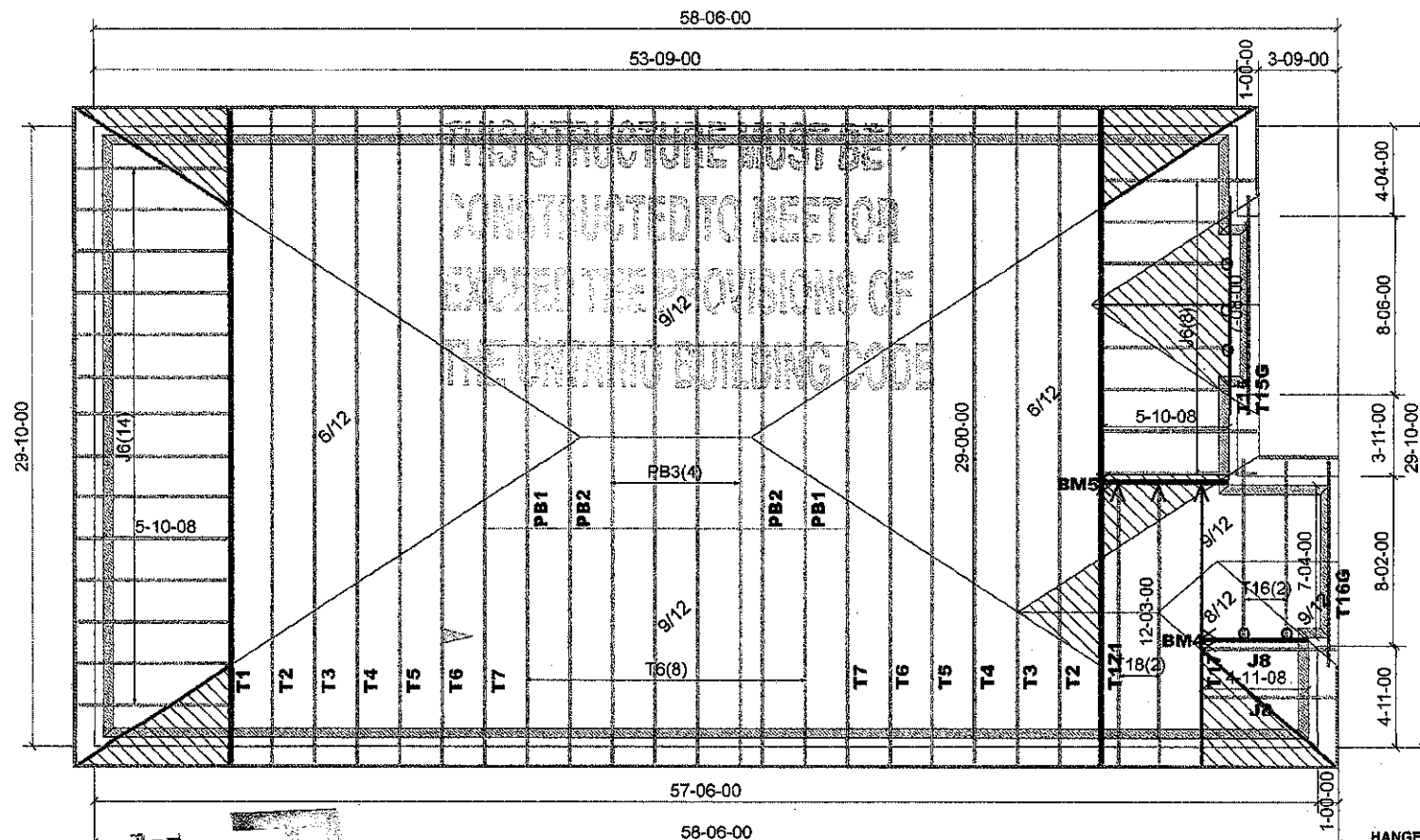




ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 (o)
LJS26DS (V)
HGUS26-2 (XX)

DENOTES
CONV.
FRAMING

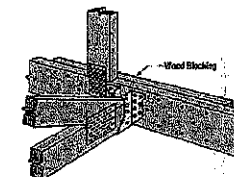
GSL 37.6 PSF

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9

DESIGN LOADS:
TC LIVE 29 PSF
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

BM4,5 = 2 2X10

HANGERS APPLIED TO A 1-PLY
GIRDERS REQUIRE A BLOCK SCABBED
BEHIND. APPROX. 2" O.C. WITH HANGER



ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

RECEIVED
AUG 28 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

	Job Track: 50120 Plan Log: 200444 Layout ID: 401157	Builder / Location: Greenpark / Caledon Project: Lamberts Lane Home Corp. Date: 2019-02-01 Designer: CS/BF	Model / Elevation: Preston 3 / ELV. 2 Lot 25 THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
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Mitek ver 8.2.3.229

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 3
 Lot #: 25
 Elevation: 2

Job Track: 50120
 PlanLog: 200444
 Layout ID: 401157
 Ref #
 Page: 1 of 2
 Date: 02-01-2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	280.72 181.33		
	1 2-ply	T1Z1 Hip Girder	9/12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	280.72 181.33		
	2	T2 Hip	9/12	29-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	233.03 147.33		
	2	T3 Hip	9/12	29-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	264.02 165.33		
	2	T4 Hip	9/12	29-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	256.45 170.33		
	2	T5 Hip	9/12	29-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	269.95 175.33		
	10	T6 Hip	9/12	29-00-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1416.85 905.00		
	2	T7 Hip	9/12	29-00-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	275.24 180.00		
	1	T15 Common Girder	9/12	7-08-00	4-04-12	2 x 4 2 x 6	1-05-00 1-05-00	1-06-04 1-06-04	40 24.67		
	1	T15G GABLE	9/12	7-08-00	4-04-12	2 x 4	1-05-00 1-05-00	1-06-04 1-06-04	36.43 24.33		
	2	T16 Common	9/12	7-04-00	4-03-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	68.05 46.67		
	1	T16G GABLE	9/12	7-04-00	4-03-04	2 x 4	1-05-00 1-03-08	1-06-04 1-06-04	34.88 24.33		
	1	T17 Hip Girder	9/12	12-03-00	4-08-08	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	64.68 42.67		
	2	T18 Hip	9/12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	112.92 75.33		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 3
 Lot #: 25
 Elevation: 2

Job Track: 50120
 PlanLog: 200444
 Layout ID: 401157
 Ref #
 Page: 2 of 2
 Date: 02-01-2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	PB1 Piggyback	9 /12	8-09-05	2-00-00	2 x 4			44.18 30.67		
	2	PB2 Piggyback	9 /12	8-09-05	3-00-00	2 x 4			45.23 29.67		
	4	PB3 Piggyback	9 /12	8-09-05	3-03-08	2 x 4			92.05 59.33		
	22	J6 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	369.48 234.67		
	2	J8 Jack-Open	8 /12	4-11-08	4-08-08	2 x 4	1-03-08	1-04-13 4-08-08	34.25 20.67		

TOTAL # TRUSS= 64 TOTAL BFT OF ALL TRUSSES= 2718.99 BFT. TOTAL WEIGHT OF ALL TRSSES 4209.11 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
2	Hardware	HGUS26-2	
2	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 7

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401156	T1	1	2	Preston 3	T1
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Feb 5 20:29:06 2019 Page 2
ID:4xNdf4mUPyxd3LURX25ZQKvOFG0-gmSKBVWIK1NLTKpaFSADP4 TrGyQJM9OpQSe42o5IF

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

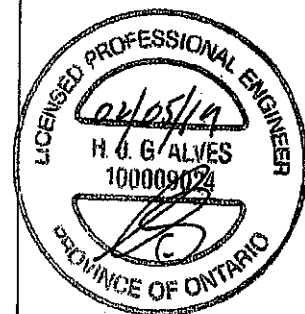
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
AD-N	0/3343	-38.5	-38.5	0.26 (1)	10.00		
N-AP	0/0	-38.5	-38.5	0.05 (3)	10.00		
AP-M	0/0	-38.5	-38.5	0.05 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-50	-56		FRONT	VERT	DEAD		
C	3-5-5	-460	-460		BACK	VERT	TOTAL		
C	3-5-5	-285	-285		FRONT	VERT	SNOW		
F	13-7-4	-123	-123		BACK	VERT	TOTAL		
J	25-6-11	-50	-56		FRONT	VERT	DEAD		
J	25-6-11	-460	-460		BACK	VERT	TOTAL		
J	25-6-11	-285	-285		FRONT	VERT	SNOW		
N	25-3-4	-55	-70		BACK	VERT	TOTAL		
T	3-8-12	-55	-70		BACK	VERT	TOTAL		
V	5-7-4	-123	-123		BACK	VERT	TOTAL		
W	7-7-4	-123	-123		BACK	VERT	TOTAL		
X	9-7-4	-123	-123		BACK	VERT	TOTAL		
Y	11-7-4	-123	-123		BACK	VERT	TOTAL		
Z	15-7-4	-123	-123		BACK	VERT	TOTAL		
AA	17-7-4	-123	-123		BACK	VERT	TOTAL		
AB	19-7-4	-123	-123		BACK	VERT	TOTAL		
AC	21-7-4	-123	-123		BACK	VERT	TOTAL		
AD	23-7-4	-123	-123		BACK	VERT	TOTAL		
AE	1-8-12	-55	-70		BACK	VERT	TOTAL		
AF	5-7-4	-55	-70		BACK	VERT	TOTAL		
AG	7-7-4	-55	-70		BACK	VERT	TOTAL		
AH	9-7-4	-55	-70		BACK	VERT	TOTAL		
AI	11-7-4	-55	-70		BACK	VERT	TOTAL		
AJ	13-7-4	-55	-70		BACK	VERT	TOTAL		
AK	15-7-4	-55	-70		BACK	VERT	TOTAL		
AL	17-7-4	-55	-70		BACK	VERT	TOTAL		
AM	19-7-4	-55	-70		BACK	VERT	TOTAL		
AN	21-7-4	-55	-70		BACK	VERT	TOTAL		
AO	23-7-4	-55	-70		BACK	VERT	TOTAL		
AP	27-3-4	-55	-70		BACK	VERT	TOTAL		



DWG NO. TAM 1902708
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401157	T1Z1	1	2	Preston 3	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mittek Industries, Inc. Tue Feb 12 09:53:26 2019 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.50
K	TMVV-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BMWWWW-t	MT20	7.0	8.0	4.25	4.00
Q	BS-t	MT20	5.0	6.0		
R	BMWWWW-t	MT20	7.0	8.0	4.25	4.00
U	BMV1+p	MT20	3.0	6.0		

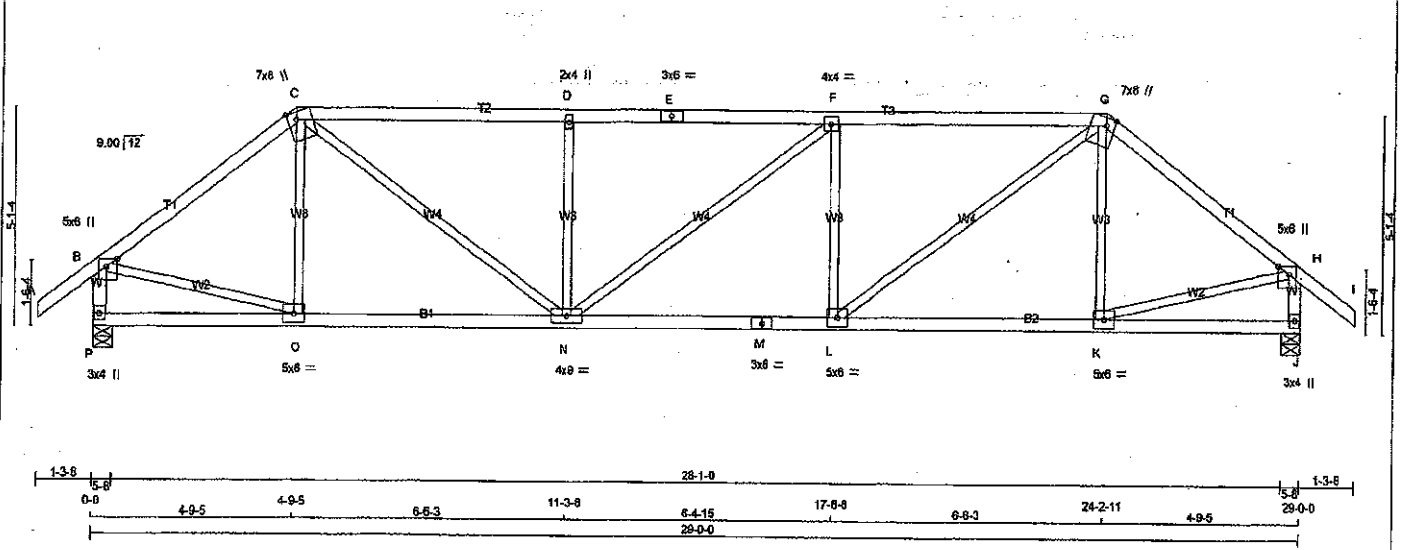
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	25-6-11	-285	-285		FRONT	VERT	SNOW		
O	20-9-12	-55	-70		FRONT	VERT	TOTAL		
P	18-9-12	-55	-70		FRONT	VERT	TOTAL		
R	12-4-8	-2097	-2097		FRONT	VERT	TOTAL		
V	12-9-12	-123	-123		FRONT	VERT	TOTAL		
W	14-9-12	-123	-123		FRONT	VERT	TOTAL		
X	18-9-12	-123	-123		FRONT	VERT	TOTAL		
Y	22-9-12	-123	-123		FRONT	VERT	TOTAL		
Z	24-9-12	-136	-136		FRONT	VERT	TOTAL		
AA	12-8-12	-55	-70		FRONT	VERT	TOTAL		
AB	14-8-12	-55	-70		FRONT	VERT	TOTAL		
AC	18-9-12	-55	-70		FRONT	VERT	TOTAL		
AD	22-9-12	-55	-70		FRONT	VERT	TOTAL		
AE	24-9-12	-55	-70		FRONT	VERT	TOTAL		
AF	28-9-12	-55	-70		FRONT	VERT	TOTAL		



DWG NO. TAM T1003211
STRUCTURAL
COMPONENT ONLY 2/2



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+P	MT20	5.0	6.0	Edge	3.00
C	TTWV+m	MT20	7.0	8.0	Edge	2.50
D	TMW+w	MT20	2.0	4.0		
E	TS-1	MT20	3.0	6.0		
F	TMWV+I	MT20	4.0	4.8		
G	TTWV+m	MT20	7.0	8.0	Edge	2.50
H	TMWV+P	MT20	5.0	6.0	Edge	3.00
J	BMV1+P	MT20	3.0	4.0		
K, L, O						
K	BMWV+I	MT20	5.0	6.0		
M	BS-1	MT20	3.0	8.0		
N	BMWV+I	MT20	4.0	9.0		
P	BMV1+P	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
P	2180	0	5-8	5-8
J	2180	0	5-8	5-6

UNFACTORED REACTIONS

1ST LCASE	MAX MIN COMPONENT REACTIONS
JT	COMBINED
P	1620 922 / 0 305 / 0 0 / 0 0 / 0 394 / 0 0 / 0
J	1620 922 / 0 305 / 0 0 / 0 0 / 0 394 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
A-B		FROM TO			FR-TO			
FR-TO								
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	O-C	-179 / 111	0.07 (1)
B-C	-2165 / 0	-102.1	-102.1	0.54 (1)	4.08	C-N	0 / 1620	0.38 (1)
C-D	-3030 / 0	-102.1	-102.1	0.92 (1)	3.09	N-D	-714 / 0	0.27 (1)
D-E	-3030 / 0	-102.1	-102.1	0.92 (1)	3.08	N-F	-2 / 0	0.00 (1)
E-F	-3030 / 0	-102.1	-102.1	0.92 (1)	3.08	L-F	-714 / 0	0.27 (1)
F-G	-3031 / 0	-102.1	-102.1	0.93 (1)	3.08	L-G	0 / 1522	0.38 (1)
G-H	-2165 / 0	-102.1	-102.1	0.54 (1)	4.08	K-G	-180 / 111	0.07 (1)
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	B-O	0 / 1780	0.40 (1)
P-B	-2111 / 0	0.0	0.0	0.22 (1)	5.83	K-H	0 / 1780	0.40 (1)
J-H	-2110 / 0	0.0	0.0	0.22 (1)	5.83			
P-O	0 / 0	-38.5	-38.5	0.23 (3)	10.00			
O-N	0 / 1725	-38.5	-38.5	0.45 (2)	10.00			
N-M	0 / 3031	-38.5	-38.5	0.65 (1)	10.00			
M-L	0 / 3031	-38.5	-38.5	0.65 (1)	10.00			
L-K	0 / 1725	-38.5	-38.5	0.46 (2)	10.00			
K-J	0 / 0	-38.5	-38.5	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015
- CSA 088-14
- TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TO=0.93/1.00 (F-G:1), BC=0.85/1.00 (L-N:1), WB=0.40/1.00 (B-C:1), SS=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 789 1887 1873

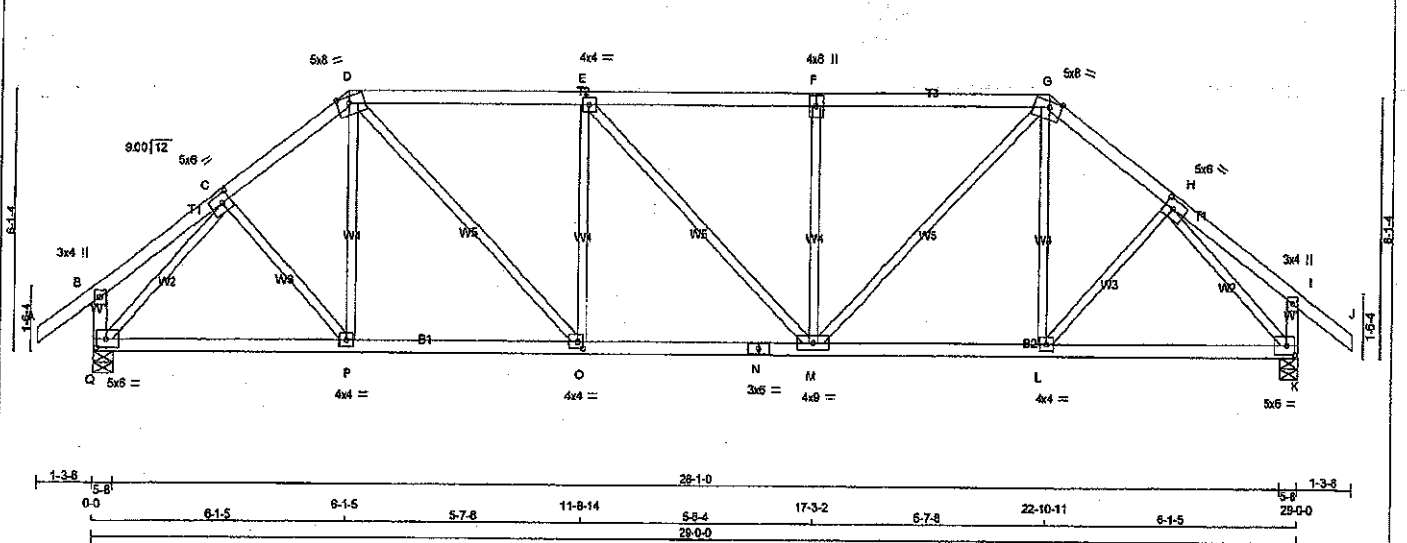
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (B) (INPUT = 0.90)
JSI METAL=0.95 (M) (INPUT = 1.00)



DRWG NO. TAM 17902709
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 127 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	SPF
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	8.0	Edge	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TSW-t	MT20	4.0	6.0		
G	TTWW-m	MT20	5.0	6.0	Edge	3.50
H	TMWW-t	MT20	5.0	6.0	2.50	2.50
I	TMV+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMVW-t	MT20	4.0	4.0		
M	BMVWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0	2.00	1.50
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ UPLIFT	IN-SX	IN-SX
Q	2180	0	2180	0	5-8	5-8
K	2180	0	2180	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
Q	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
K	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 42	-102.1	0.14 (1)	10.00	C-P	0 / 212	0.05 (2)
B-C	0 / 18	-102.1	0.12 (1)	10.00	P-D	0 / 202	0.05 (3)
C-D	-2175 / 0	-102.1	0.18 (1)	4.48	D-O	0 / 1157	0.28 (1)
D-E	-2530 / 0	-102.1	0.71 (1)	3.53	O-E	-628 / 0	0.37 (1)
E-F	-2464 / 0	-102.1	0.70 (1)	3.38	E-M	-96 / 0	0.13 (1)
F-G	-2464 / 0	-102.1	0.74 (1)	3.50	M-F	-492 / 0	0.29 (1)
G-H	-2175 / 0	-102.1	0.18 (1)	4.48	M-G	0 / 1083	0.24 (1)
H-I	0 / 18	-102.1	0.12 (1)	10.00	L-G	0 / 202	0.05 (3)
I-J	0 / 42	-102.1	0.14 (1)	10.00	L-H	0 / 212	0.05 (2)
Q-B	-268 / 0	0.0	0.03 (1)	7.81	Q-C	-2413 / 0	0.82 (1)
K-I	-268 / 0	0.0	0.03 (1)	7.81	H-K	-2413 / 0	0.82 (1)
Q-P	0 / 1589	-38.5	-38.5	0.46 (2)	10.00		
P-O	0 / 1723	-38.5	-38.5	0.49 (2)	10.00		
O-N	0 / 2530	-38.5	-38.5	0.61 (1)	10.00		
N-M	0 / 2530	-38.5	-38.5	0.51 (1)	10.00		
M-L	0 / 1723	-38.5	-38.5	0.49 (2)	10.00		
L-K	0 / 1589	-38.5	-38.5	0.46 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.6 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.74/1.00 (F-G-1), BC=0.51/1.00 (M-O-1), WB=0.82/1.00 (H-K-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.78 (N) (INPUT = 1.00)

RECEIVED
AUG 28 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.



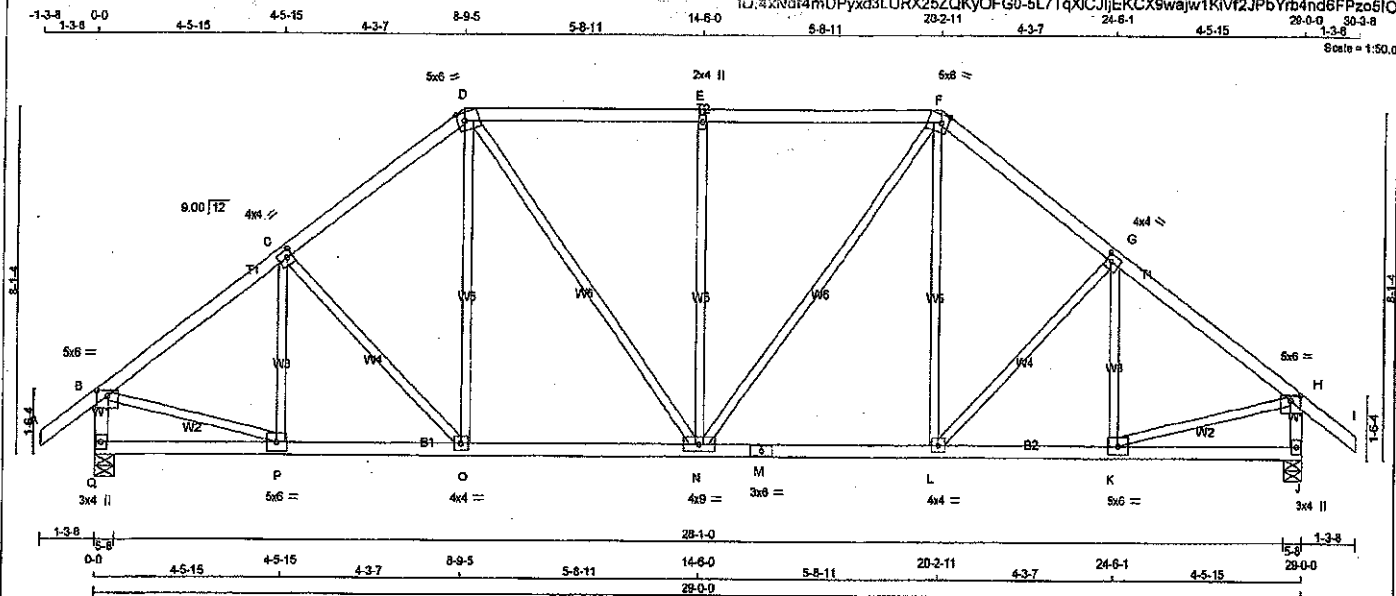
DWG NO. TAM 1790 2710
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Preston 3	DRWG NO.	T5
200444-401156	T5	1	1	TRUSS DESC.			

Tamarack Roof Truss, Burlington

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ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-5LTqXICJJEKX9wajw1Kivf2JPbYrb4nd6FPzo51Q



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
Q - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00	
C	TMVW-l	MT20	4.0	4.0	2.00	1.50	
D	TTVW-m	MT20	5.0	6.0	Edge	2.00	
E	TMVW-w	MT20	2.0	4.0			
F	TTVW-m	MT20	5.0	6.0	Edge	2.00	
G	TMVW-l	MT20	4.0	4.0	2.00	1.50	
H	TMVW-p	MT20	5.0	6.0	1.50	3.00	
J	BMV1+p	MT20	3.0	4.0			
K	BMVW-t	MT20	5.0	6.0			
L	BMVW-t	MT20	4.0	4.0			
M	BS-l	MT20	3.0	6.0			
N	BMVW-w-l	MT20	4.0	9.0			
O	BMVW-l	MT20	4.0	4.0			
P	BMVW-l	MT20	5.0	6.0			
Q	BMV1+p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD	BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2180	0	2180	0	0	5-8	5-8
J	2180	0	2180	0	0	5-8	5-8
UNFACTORED REACTIONS							
JT	1ST CASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING							
TOTAL LOAD CASES: (4)							
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	P-C	-289 / 23	0.10 (1)
B-C	-2168 / 0	-102.1 -102.1	0.31 (1)	4.35	C-O	-218 / 0	0.16 (1)
C-D	-2046 / 0	-102.1 -102.1	0.30 (1)	4.48	O-D	0 / 375	0.08 (2)
D-E	-1956 / 0	-102.1 -102.1	0.48 (1)	4.29	D-N	0 / 581	0.13 (1)
E-F	-1956 / 0	-102.1 -102.1	0.48 (1)	4.29	N-E	-712 / 0	0.90 (1)
F-G	-2046 / 0	-102.1 -102.1	0.30 (1)	4.48	N-F	0 / 581	0.13 (1)
G-H	-2168 / 0	-102.1 -102.1	0.31 (1)	4.35	L-F	0 / 375	0.08 (2)
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	L-G	-218 / 0	0.16 (1)
Q-B	-2105 / 0	0.0 0.0	0.22 (1)	5.84	K-G	-289 / 23	0.10 (1)
J-H	-2105 / 0	0.0 0.0	0.22 (1)	5.84	B-P	0 / 1820	0.41 (1)
					K-H	0 / 1820	0.41 (1)
Q-P	0 / 0	-38.5 -38.5	0.13 (3)	10.00			
P-O	0 / 1759	-38.5 -38.5	0.39 (1)	10.00			
O-N	0 / 1613	-38.5 -38.5	0.41 (2)	10.00			
N-M	0 / 1613	-38.5 -38.5	0.41 (2)	10.00			
M-L	0 / 1613	-38.5 -38.5	0.41 (2)	10.00			
L-K	0 / 1759	-38.5 -38.5	0.39 (1)	10.00			
K-J	0 / 0	-38.5 -38.5	0.13 (3)	10.00			

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 29.0 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF
SPACING	= 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00H/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.14")

CSI: TC=0.48/1.00 (D-E:1), BC=0.41/1.00 (L-N:2),
WB=0.90/1.00 (E-N:1), SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

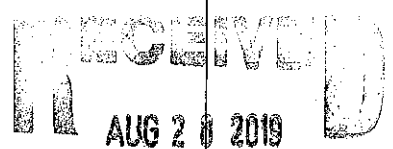
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP(DRY)	SHEAR	SECTION			
(PSI)	(PLI)	(PLI)			
MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

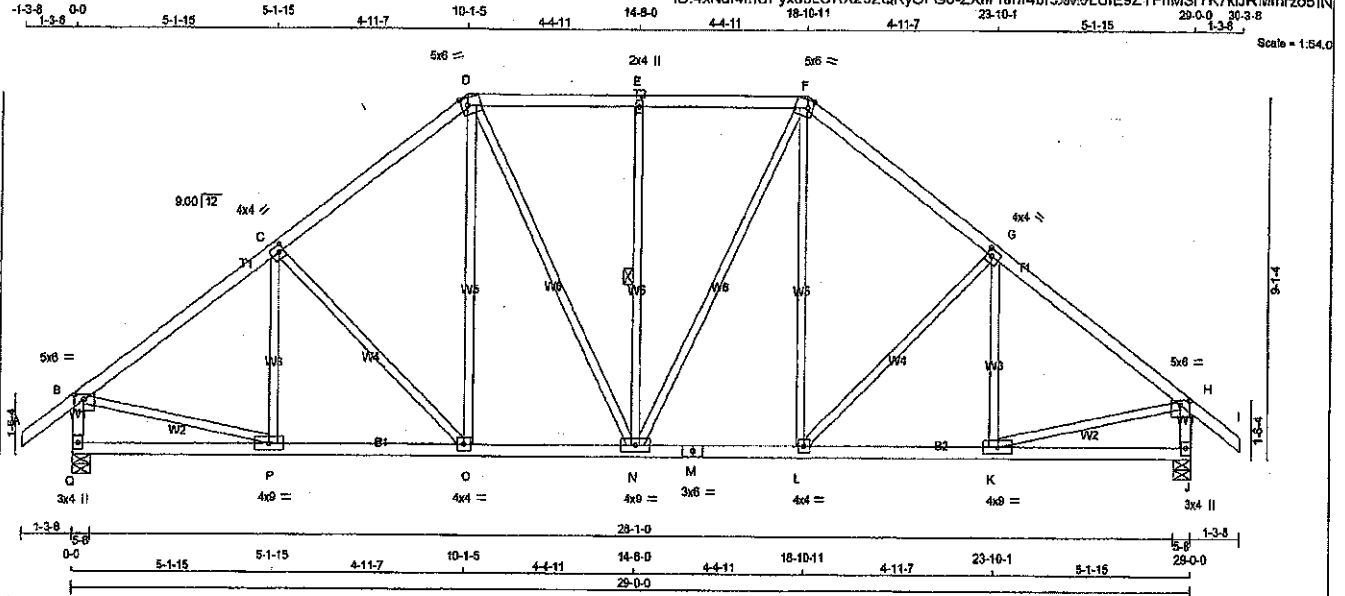
PLATE ROTATION TOL. = 5.0 Deg.

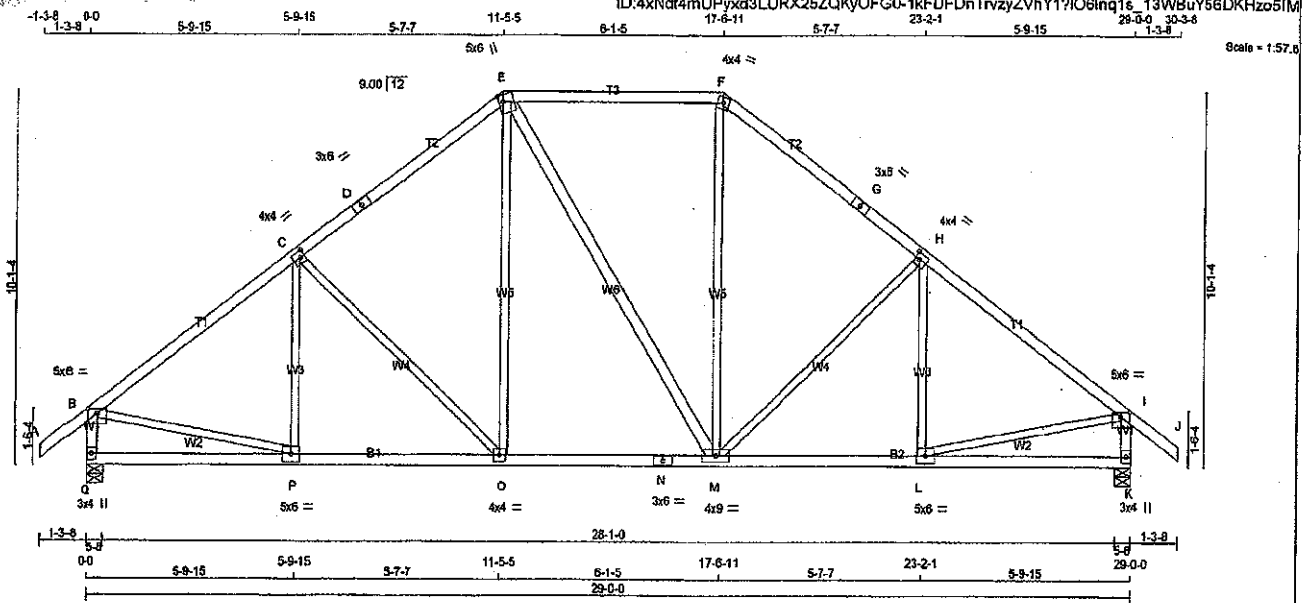
JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.51 (M) (INPUT = 1.00)



TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DRWG NO. TAM 17902712
STRUCTURAL
COMPONENT ONLY





TOTAL WEIGHT = 138 lb [M/F]

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV-p	MT20	5.0	6.0	1.50	3.00
C	TMVV-v	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTVV+m	MT20	5.0	6.0	2.25	1.50
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVV-v	MT20	4.0	4.0	2.00	1.50
I	TMVV-p	MT20	5.0	6.0	1.50	3.00
K	BMV1-p	MT20	3.0	4.0		
L	BMVV-v	MT20	5.0	6.0		
M	BMVV-v	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVV-v	MT20	4.0	4.0		
P	BMVV-v	MT20	5.0	6.0		
Q	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
Q	2180 0	2180 0	5-8	5-8
K	2180 0	2180 0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN COMPONENT REACTIONS
Q	1620 922/0	305/0 0/0 0/0 0/0
K	1620 922/0	305/0 0/0 0/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MAX. FACTORED				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/42	-102.1 -102.1 0.14 (1)	10.00	P-C	-123/150	0.07 (1)	
B-C	-2209/0	-102.1 -102.1 0.55 (1)	4.04	C-O	-492/0	0.65 (1)	
C-D	-1855/0	-102.1 -102.1 0.50 (1)	4.38	O-E	0/571	0.13 (2)	
D-E	-1855/0	-102.1 -102.1 0.50 (1)	4.38	E-M	0/1	0.00 (2)	
E-F	-1454/0	-102.1 -102.1 0.52 (1)	4.75	M-F	0/573	0.13 (2)	
F-G	-1856/0	-102.1 -102.1 0.50 (1)	4.38	M-H	-491/0	0.65 (1)	
G-H	-1856/0	-102.1 -102.1 0.50 (1)	4.38	L-H	-125/149	0.07 (1)	
H-I	-2209/0	-102.1 -102.1 0.55 (1)	4.04	B-P	0/1838	0.41 (1)	
I-J	0/42	-102.1 -102.1 0.14 (1)	10.00	L-I	0/1838	0.41 (1)	
Q-B	-2088/0	0.0 0.0 0.22 (1)	5.86				
K-I	-2087/0	0.0 0.0 0.22 (1)	5.86				
Q-P	0/0	-38.5 -38.5 0.24 (3)	10.00				
P-O	0/1801	-38.5 -38.5 0.46 (2)	10.00				
O-N	0/1453	-38.5 -38.5 0.39 (2)	10.00				
N-M	0/1453	-38.5 -38.5 0.39 (2)	10.00				
M-L	0/1800	-38.5 -38.5 0.46 (2)	10.00				
L-K	0/0	-38.5 -38.5 0.23 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.55/1.00 (B-C:1), BC=0.46/1.00 (O-P:2), WB=0.85/1.00 (C-O:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

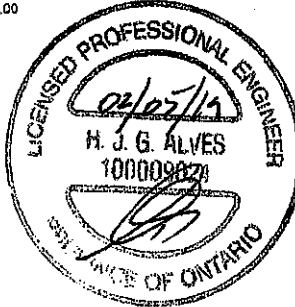
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.86 (F) (INPUT = 0.80)
JSI METAL = 0.46 (N) (INPUT = 1.00)

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



DWG NO. TAM 17902714
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 40 lb

DWG NO. TAM 7703212
STRUCTURAL
COMPONENT ONLY

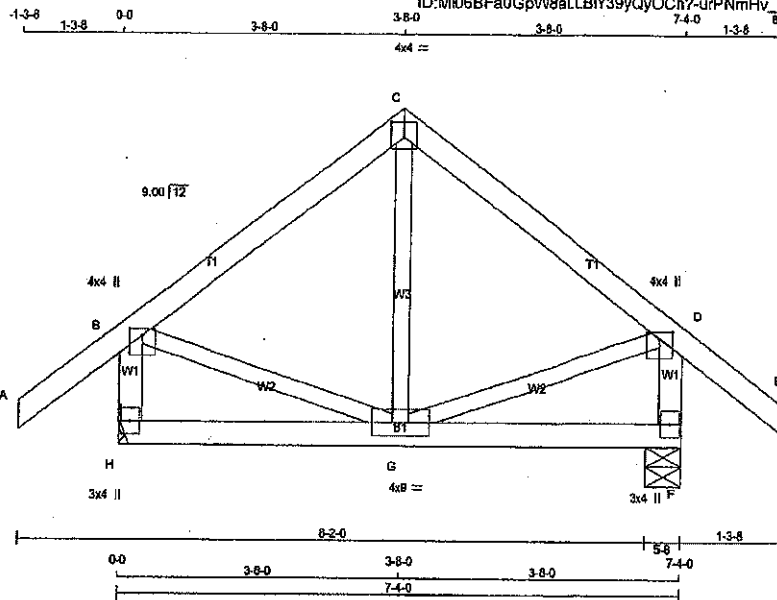
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DWG NO. TAM 72032/3
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401157	T16	2	1	Preston 3	T16

Tamereck Roof Truss, Burlington

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ID: M06BFa0GpW8aLLBiy39yCyOCh7-urPNmHv_uVLaBSEnfFOaUlxXVyzovAL6xp7nhbzlxqa



Scale = 1/27.1

TOTAL WEIGHT = 2 X 34 = 68 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVV+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVVV-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
H	856	0	656	0
F	856	0	656	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1.8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	482	293 / 0	77 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	482	293 / 0	77 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO			FROM	TO	CS (LC)	FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	G-C	0 / 139	0.03 (3)	
B-C	-318 / 0	-102.1	-102.1	0.18 (1)	6.25	B-G	0 / 268	0.06 (1)	
C-D	-318 / 0	-102.1	-102.1	0.18 (1)	6.25	G-D	0 / 268	0.06 (1)	
D-E	0 / 42	-102.1	-102.1	0.14 (1)	10.00				
H-B	-602 / 0	0.0	0.0	0.06 (1)	7.81				
F-D	-602 / 0	0.0	0.0	0.06 (1)	7.81				
H-G	0 / 0	-38.5	-38.5	0.12 (3)	10.00				
G-F	0 / 0	-38.5	-38.5	0.12 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH	LL	=	28.0	PSF
	DL	=	6.0	PSF
BOT CH	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2015
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.24")
CALCULATED VERT. DEFL. (LL) = $L/696$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.24")
CALCULATED VERT. DEFL. (TL) = $L/696$ (0.01")

CSI: TC=0.18/1.00 (B-C:1), BC=0.12/1.00 (G-H:3), WB=0.06/1.00 (D-G:1), SS=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (D) (INPUT = 0.80)

JSI METAL= 0.13 (D) (INPUT = 1.00)

RECEIVED
AUG 28 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

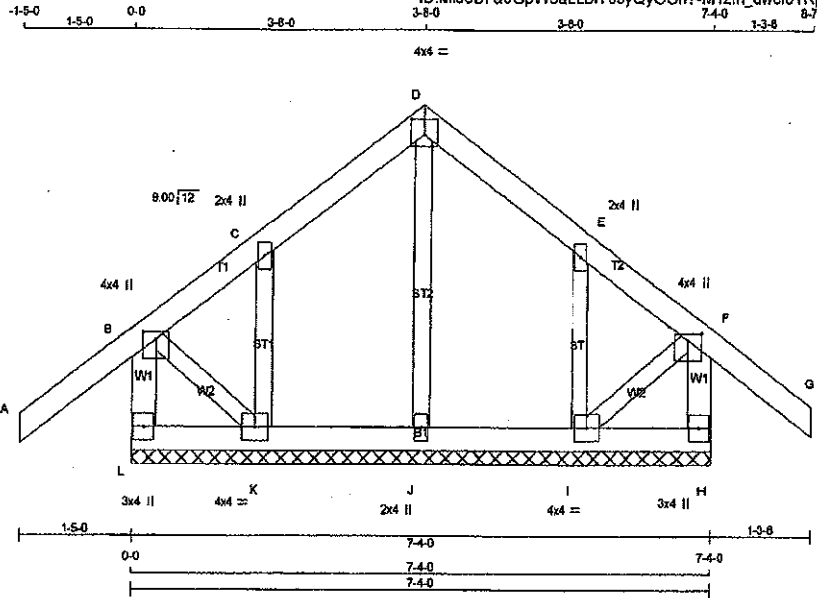


DRWG NO. TAM T16032/14
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401157	T16G	1	1	Preston 3	T16G

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 12 08:53:30 2019 Page 1
ID: M06BFa0GpW8aLLBIY39yQyOCh?M1zm_dwcf0TRpcc_Dyvp0WUQMKTednFmTKID12kxqZ



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TMVW+w	MT20	2.0	4.0	
D	TTW+p	MT20	4.0	4.0	2.25 2.00
E	TMVW+w	MT20	2.0	4.0	
F	TMVW+p	MT20	4.0	4.0	1.00 2.00
H	BMV1+p	MT20	3.0	4.0	
I	BMVW1-i	MT20	4.0	4.0	
J	BMV1+w	MT20	2.0	4.0	
K	BMVW1-i	MT20	4.0	4.0	
L	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
L-B	-311 / 0	0.0 0.0	0.03 (1)	J-D	-181 / 0	0.05 (1)	
A-B	0 / 46	-102.1 -102.1	0.18 (1)	K-C	-134 / 0	0.02 (1)	
B-C	-50 / 0	-102.1 -102.1	0.18 (1)	I-E	-150 / 0	0.03 (1)	
C-D	-14 / 0	-102.1 -102.1	0.06 (1)	B-K	0 / 14	0.00 (1)	
D-E	-16 / 0	-102.1 -102.1	0.06 (1)	I-F	0 / 15	0.00 (1)	
E-F	-43 / 0	-102.1 -102.1	0.13 (1)				
F-G	0 / 42	-102.1 -102.1	0.14 (1)				
H-F	-285 / 0	0.0 0.0	0.03 (1)				
L-K	0 / 0	-38.5 -38.5	0.02 (3)				
K-J	0 / 7	-38.5 -38.5	0.02 (3)				
J-I	0 / 7	-38.5 -38.5	0.02 (3)				
I-H	0 / 0	-38.5 -38.5	0.02 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (A-B-1), BC=0.02/1.00 (K-L-3), WB=0.05/1.00 (D-J-1), SS=0.10/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 768 1997 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.24 (F) (INPUT = 0.90)
JSI METAL= 0.03 (C) (INPUT = 1.00)

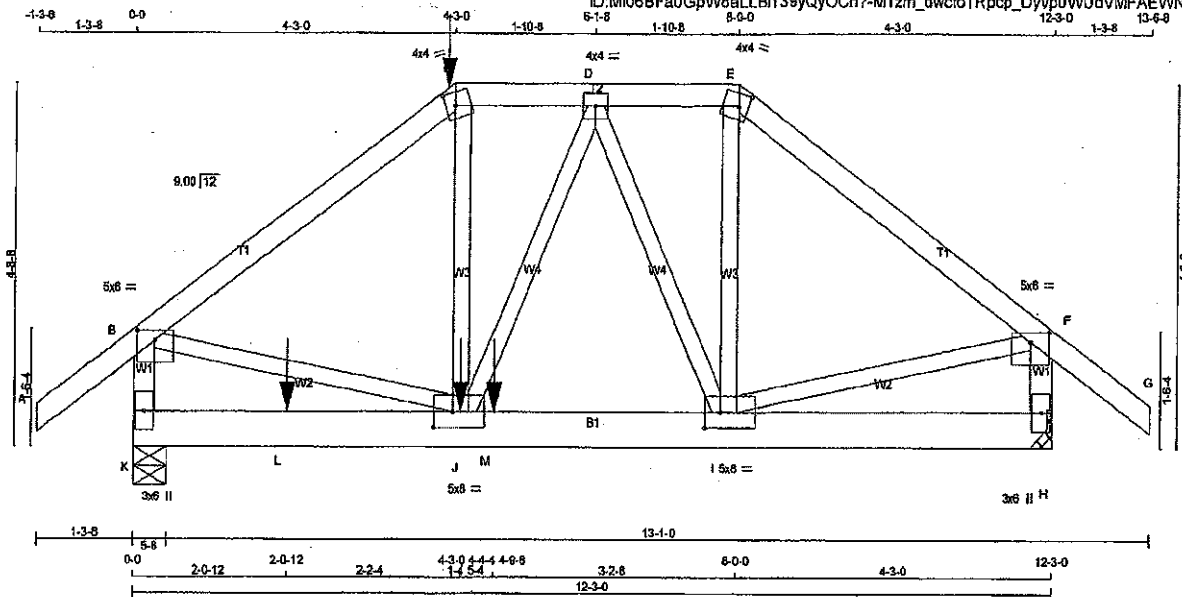


DWG NO. TAM T1603215
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MTEK Industries, Inc. Tue Feb 12 08:33:30 2019 Page 1

ID: M068Fa0GpW8aLLBIY38yQyOCh?-M1zm_dwcf0TRpcp_Dyyp0WUdV/MFAEWNFMtTKD1zbxqZ



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 85 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
K - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
K - H	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	1.50 3.00
C	TTW-m	MT20	4.0	4.0	2.00 1.50
D	TMWV-H	MT20	4.0	4.0	
E	TTW-m	MT20	4.0	4.0	2.00 1.50
F	TMWV-p	MT20	5.0	8.0	1.50 3.00
H	BMV1+p	MT20	3.0	8.0	
I	BMVWW-H	MT20	5.0	8.0	2.50 2.50
J	BMVWW-H	MT20	5.0	8.0	2.50 3.00
K	BMV1+p	MT20	3.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
K	2257	0	2257	0	0	5-8	5-8		
H	1710	0	1710	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1649	1025 / 0	245 / 0	0 / 0	0 / 0	378 / 0	0 / 0	0 / 0
H	1251	775 / 0	188 / 0	0 / 0	0 / 0	288 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO		FR-TO				
A-B	0 / 42	-102.1	-102.1	0.15 (1)	10.00	J-C	0 / 214	0.06 (3)	
B-C	-2246 / 0	-102.1	-102.1	0.48 (1)	4.03	J-D	0 / 585	0.14 (1)	
C-D	-1794 / 0	-102.1	-102.1	0.09 (1)	4.90	D-I	-768 / 0	0.28 (1)	
D-E	-1304 / 0	-102.1	-102.1	0.07 (1)	5.57	I-E	0 / 853	0.18 (1)	
E-F	-1603 / 0	-102.1	-102.1	0.42 (1)	4.69	B-J	0 / 1849	0.46 (1)	
F-G	0 / 42	-102.1	-102.1	0.15 (1)	10.00	I-F	0 / 1319	0.33 (1)	
K-B	-2181 / 0	0.0	0.0	0.24 (1)	5.72				
H-F	-1645 / 0	0.0	0.0	0.19 (1)	6.42				
K-L	0 / 0	-38.5	-38.5	0.12 (3)	10.00				
L-J	0 / 0	-38.5	-38.5	0.12 (3)	10.00				
J-M	0 / 1586	-38.5	-38.5	0.36 (1)	10.00				
M-I	0 / 1586	-38.5	-38.5	0.36 (1)	10.00				
I-H	0 / 0	-38.5	-38.5	0.07 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	4-3-0	-80	-56	FRONT	VERT	DEAD	-	-
C	4-3-0	-680	-690	FRONT	VERT	TOTAL	-	-
C	4-3-0	-285	-285	FRONT	VERT	SNOW	-	-
J	4-4-4	-57	-72	FRONT	VERT	TOTAL	-	-
L	2-0-12	-57	-72	FRONT	VERT	TOTAL	-	-
M	4-9-8	-824	-824	FRONT	VERT	TOTAL	-	-

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018
- CSA 086-14
- TPIC 2014

(58 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.41")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.48/1.00 (B-C:1), BC=0.36/1.00 (I-J:1), WB=0.48/1.00 (B-J:1), SSI=0.53/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

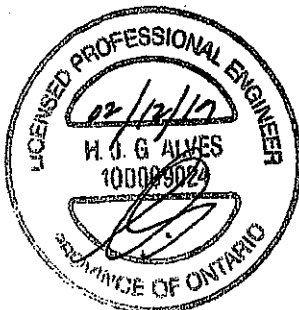
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.43 (F) (INPUT = 1.00)



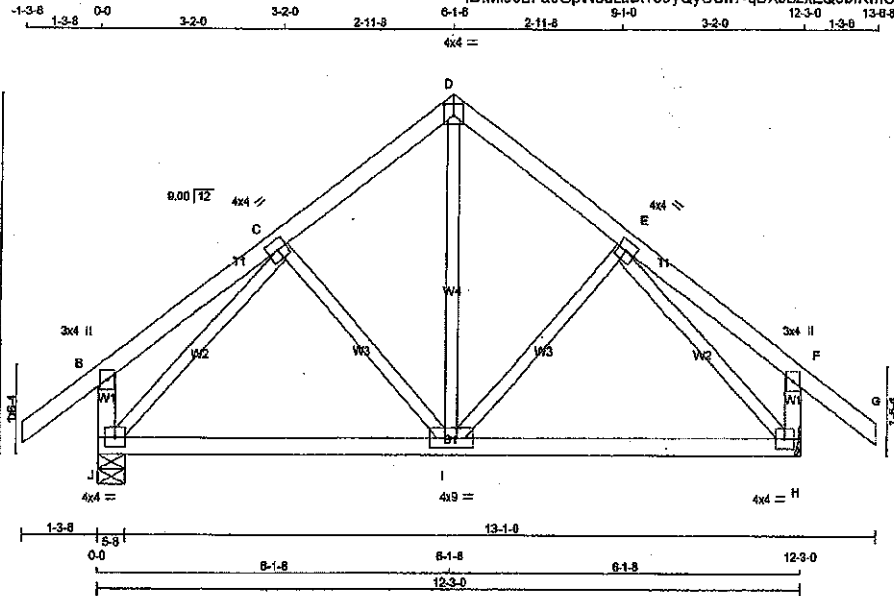
DWG NO. 1A1 T1703216
STRUCTURAL
COMPANION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401157	T18	2	1	Preston 3	T18

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Feb 12 08:53:31 2019 Page 1

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Scale = 1/32

TOTAL WEIGHT = 2 X 55 = 110 LB

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

DRY, SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
J	1002	0	1002	0
H	1002	0	1002	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	740	436 / 0	129 / 0	0 / 0	178 / 0	0 / 0
J	740	436 / 0	129 / 0	0 / 0	178 / 0	0 / 0
H	740	436 / 0	129 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	I-D	0 / 481
B-C	0 / 21	-102.1	-102.1	0.15 (1)	10.00	I-E	-127 / 28
C-D	-627 / 0	-102.1	-102.1	0.11 (1)	6.25	C-I	-127 / 28
D-E	-627 / 0	-102.1	-102.1	0.11 (1)	6.25	J-C	-884 / 0
E-F	0 / 21	-102.1	-102.1	0.15 (1)	10.00	E-H	-884 / 0
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
J-B	-262 / 0	0.0	0.0	0.03 (1)	7.81		
H-F	-262 / 0	0.0	0.0	0.03 (1)	7.81		
J-I	0 / 569	-38.5	-38.5	0.30 (2)	10.00		
I-H	0 / 569	-38.5	-38.5	0.30 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 29.0	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018
- CSA 086-14
- TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.41")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.41")
CALCULATED VERT. DEFL.(TL) = $L/899$ (0.07")

CSI: TC=0.15/1.00 (B-C:1), BC=0.38/1.00 (I-J:2), WB=0.29/1.00 (C-I:1), SSI=0.16/1.00 (J-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371
	1747	788
	1987	1873

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

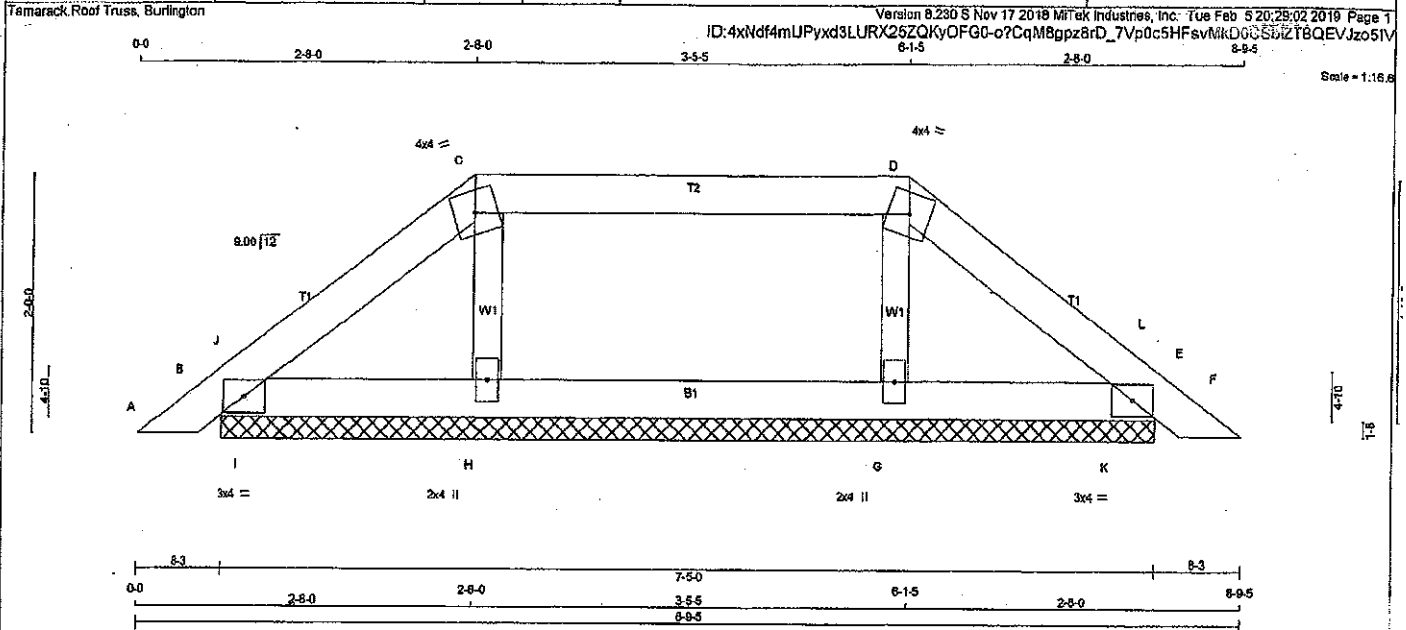
JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)

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TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. 1A1 T140321
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMW1-w	MT20	2.0	4.0	
H	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
B	232	0	7-5/0	7-5/0
E	232	0	7-5/0	7-5/0
H	349	0	7-5/0	7-5/0
G	349	0	7-5/0	7-5/0

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
B	166
E	166
H	266
G	266

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/17	-102.1 -102.1	0.03 (1)	10.00	H-C	-215/0	0.03 (1)
B-J	-64/0	-102.1 -102.1	0.01 (1)	6.25	G-D	-215/0	0.03 (1)
J-C	-76/0	-102.1 -102.1	0.05 (1)	6.25	I-J	-119/0	0.08 (1)
C-D	-44/0	-102.1 -102.1	0.21 (1)	6.25	K-L	-119/0	0.08 (1)
D-L	-76/0	-102.1 -102.1	0.05 (1)	6.25			
L-E	-64/0	-102.1 -102.1	0.01 (1)	6.25			
E-F	0/17	-102.1 -102.1	0.03 (1)	10.00			
B-I	0/58	-38.5 -38.5	0.05 (1)	10.00			
I-H	0/58	-38.5 -38.5	0.06 (2)	10.00			
H-G	0/44	-38.5 -38.5	0.06 (2)	10.00			
G-K	0/58	-38.5 -38.5	0.06 (2)	10.00			
K-E	0/58	-38.5 -38.5	0.05 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-D:1), BC=0.06/1.00 (H-L:2), WB=0.03/1.00 (C-H:1), SS=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

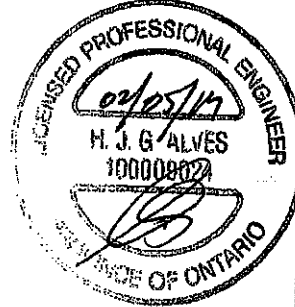
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

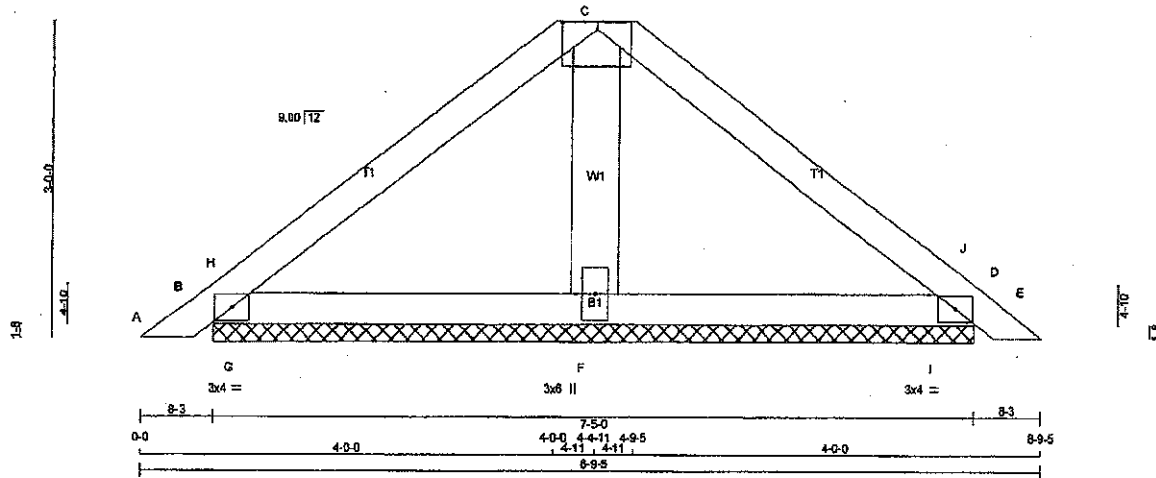
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (H) (INPUT = 1.00)



DWG NO. TAM 17902724
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 25 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS	2x6	DRY	No.2
DRY: SEASONED LUMBER			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMB1-I	MT20	3.0	4.0	
C TMTMWm	MT20	5.0	8.0	0.75 4.00
D TMB1-I	MT20	3.0	4.0	
F BMV1-w	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
B	373	0	0	7-5-0	7-5-0
D	373	0	0	7-5-0	7-5-0
F	418	0	0	7-5-0	7-5-0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	271	173 / 0	37 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
D	271	173 / 0	37 / 0	0 / 0	0 / 0	0 / 0	61 / 0	0 / 0
F	320	153 / 0	81 / 0	0 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (LC) UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC) UNBRACED LENGTH	
FR-TO				FR-TO			
A-B	0 / 17	-102.1	-102.1 0.03 (1)	10.00	F-C	-158 / 0	0.01 (1)
B-H	-78 / 0	-102.1	-102.1 0.07 (1)	6.25	G-H	-384 / 24	0.00 (1)
H-C	-148 / 0	-102.1	-102.1 0.17 (1)	6.25	I-J	-384 / 24	0.00 (1)
C-J	-148 / 0	-102.1	-102.1 0.17 (1)	6.25			
J-D	-78 / 0	-102.1	-102.1 0.07 (1)	6.25			
D-E	0 / 17	-102.1	-102.1 0.03 (1)	10.00			
B-G	0 / 109	-38.5	-38.5 0.18 (1)	10.00			
G-F	0 / 109	-38.5	-38.5 0.18 (1)	10.00			
F-I	0 / 109	-38.5	-38.5 0.18 (1)	10.00			
I-D	0 / 109	-38.5	-38.5 0.18 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-H:1), BC=0.18/1.00 (F-I:1), WS=0.01/1.00 (C-F:1), SS=0.31/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

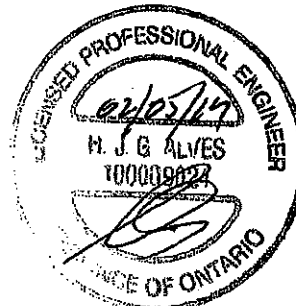
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618 354 1897 788 1897 1656	

PLATE PLACEMENT TOL = 0.250 inches

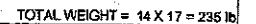
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.30 (D) (INPUT = 0.90)
JSI METAL = 0.07 (D) (INPUT = 1.00)

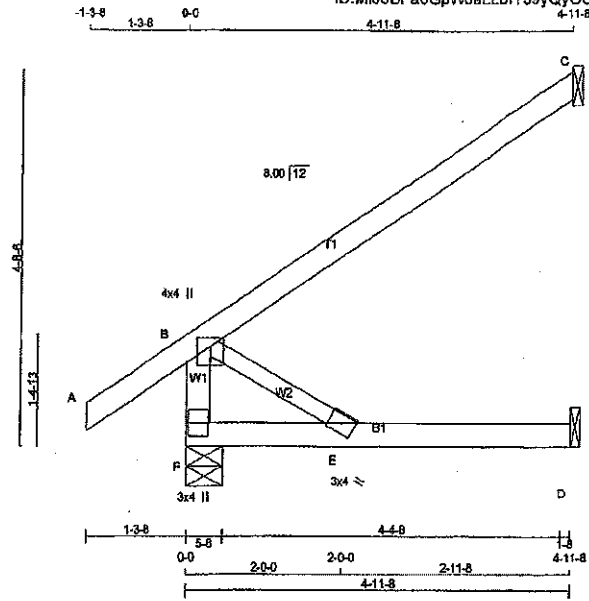


DWG NO. TAM 17902725
STRUCTURAL
COMPONENT ONLY

Scale = 123.6



DWG NO. TAM 17902730
STRUCTURAL
COMMENT ONLY



Scale = 1/25.0

TOTAL WEIGHT = 2 X 17 = 35 LB

LUMBER	SIZE	LUMBER	DESCR.
N. L. G. A. RULES			
CHORDS	2x4	DRY	No.2
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMW+p	MT20	4.0	4.0	1.25 2.00
E BMW+w	MT20	3.0	4.0	
F BMVt+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQD	
	VERT	HORZ	DOWN	HORZ	BRG	BRG
F	489	0	489	0	5-8	5-8
C	253	0	253	0	1-8	1-8
D	95	0	121	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37621H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
F	357	224 / 0	52 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	174	144 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
D	87	0 / 0	52 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO				
F-B	-393 / 0	0.0	0.0	0.04 (1)
A-B	0 / 39	-102.1	-102.1	0.14 (1)
B-C	0 / 0	-102.1	-102.1	0.43 (1)
F-E	0 / 0	-38.5	-38.5	0.21 (3)
E-D	0 / 0	-38.5	-38.5	0.22 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 28.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2015
 - CSA 086-14
 - TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = 1/869 (0.04")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = 1/823 (0.08")

CSI: TC=0.43/1.00 (B-C:1), BC=0.22/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

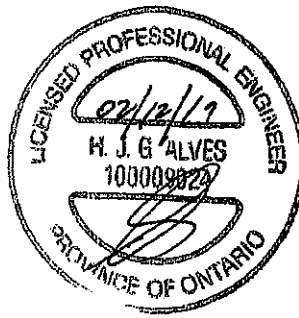
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	850	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
 JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. TAM T1703218
 STRUCTURAL
 COMPONENTS



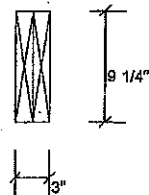
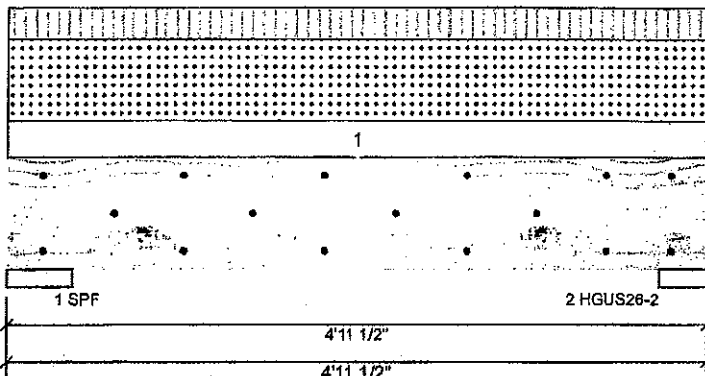
isDesign™

Client: Greenpark Homes
 Project: Preston 3
 Address: Caledon

Date: 2019-02-05
 Designer: Brian
 Job Name: Lamberts Lane Homes Corp.
 Project #: 200444

BM4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
 Piles: 2
 Moisture Condition: Dry
 Deflection LL: 360
 Deflection TL: 360
 Importance: Normal

Application: Floor (Residential)
 Design Method: LSD
 Building Code: NBCC 2015 / OBC 2012
 Load Sharing: No
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	112	132	295	0
2	106	128	280	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	7%	165 / 554	719 L 1.25D+1.5S +L
2 - HGUS...	4.000"	9%	157 / 527	684 L 1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	652 ft-lb	2'6 1/2"	6039 ft-lb	0.108 (11%)	1.25D+1.5S +L	L
Unbraced	652 ft-lb	2'6 1/2"	5493 ft-lb	0.119 (12%)	1.25D+1.5S +L	L
Shear	607 lb	1'2"	3984 lb	0.152 (15%)	1.25D+1.5S +L	L
Perm Defl in. (L/35365)	0.001	2'6 1/2"	0.143 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch (L/13326)	0.004	2'6 1/2"	0.143 (L/360)	0.030 (3%)	S+0.5L	L
TL Defl inch (L/9679)	0.005	2'6 1/2"	0.143 (L/360)	0.040 (4%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17902733
 STRUCTURAL
 COMPONENT ONLY

1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		4-0-0	Near Face	13 PSF	11 PSF	29 PSF	0 PSF	

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AUG 28 2019

TOWN OF CALEDON
 BUILDING SECTION
 FILE NO

This design is valid until 2021-12-11

Manufacturer Info

TAMARACK LUMBER
 3255 NORTH SERVICE RD, ON
 CANADA
 (905) 335-1115





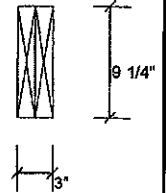
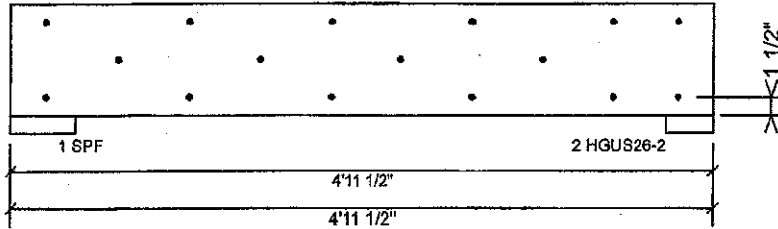
isDesign™

Client: Greenpark Homes
Project: Preston 3
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200444

BM4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	36.9 %
Load	141.5 PLF
Yield Limit per Foot	383.4 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

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AUG 28 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO. _____



DWG NO. TAM 71902733
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	TAMARACK LUMBER 3255 NORTH SERVICE RD, ON CANADA (905) 335-1115
	TAMARACK LUMBER INC. ALPHA LUMBER GROUP

This design is valid until 2021-12-11





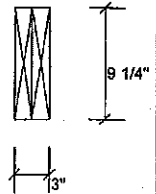
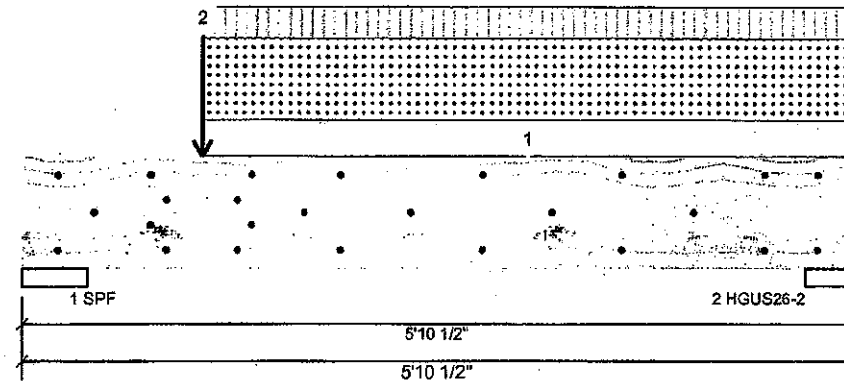
isDesign™

Client: Greenpark Homes
 Project: Preston 3
 Address: Caledon

Date: 2019-02-05
 Designer: Brian
 Job Name: Lamberts Lane Homes Corp.
 Project #: 200444

BM5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
 Plies: 2
 Moisture Condition: Dry
 Deflection LL: 360
 Deflection TL: 360
 Importance: Normal

Application: Floor (Residential)
 Design Method: LSD
 Building Code: NBCC 2015 / OBC 2012
 Load Sharing: No
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	176	251	642	0
2	62	82	200	0

Bearings and Factored Reactions

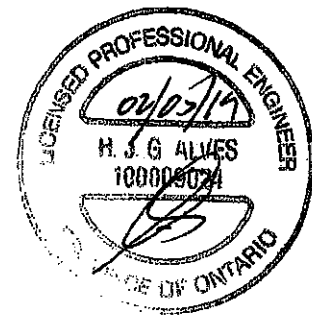
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	314 / 1139	1454 L
2 - HGUS...	4.000"	6%	103 / 363	466 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1272 ft-lb	1'3 1/4"	6039 ft-lb	0.211 (21%)	1.25D+1.5S	L
Unbraced	1272 ft-lb	1'3 1/4"	5236 ft-lb	0.243 (24%)	1.25D+1.5S	L
Shear	1508 lb	1'2"	3584 lb	0.379 (38%)	1.25D+1.5S	L
Perm Defl in.	0.003 (L/19193)	2'8 3/4"	0.174 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.009 (L/6690)	2'8 11/16"	0.174 (L/360)	0.050 (5%)	S+0.5L	L
TL Defl inch	0.013 (L/4961)	2'8 11/16"	0.174 (L/360)	0.070 (7%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 77902734
 STRUCTURAL
 COMPONENT ONLY 1/3

Manufacturer Info

TAMARACK LUMBER
 3255 NORTH SERVICE RD, ON
 CANADA
 (905) 395-1115



This design is valid until 2021-12-11





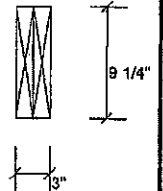
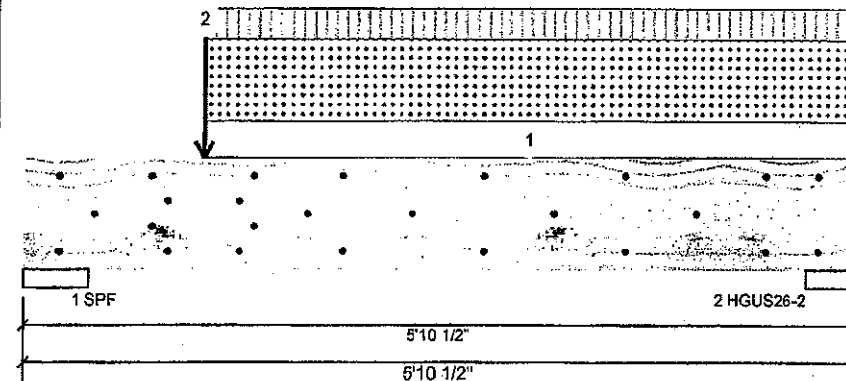
isDesign™

Client: Greenpark Homes
Project: Preston 3
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200444

BM5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	1-3-4 to 5-10-8		Near Face	13 PLF	11 PLF	29 PLF	0 PLF	
2	Point	1-3-4		Near Face	274 lb	188 lb	709 lb	0 lb	

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AUG 28 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DWG NO. TAM 1902734
STRUCTURAL
COMPONENT ONLY 2/3

Manufacturer Info

TAMARACK LUMBER
3255 NORTH SERVICE RD, ON
CANADA
(905) 335-1115



This design is valid until 2021-12-11





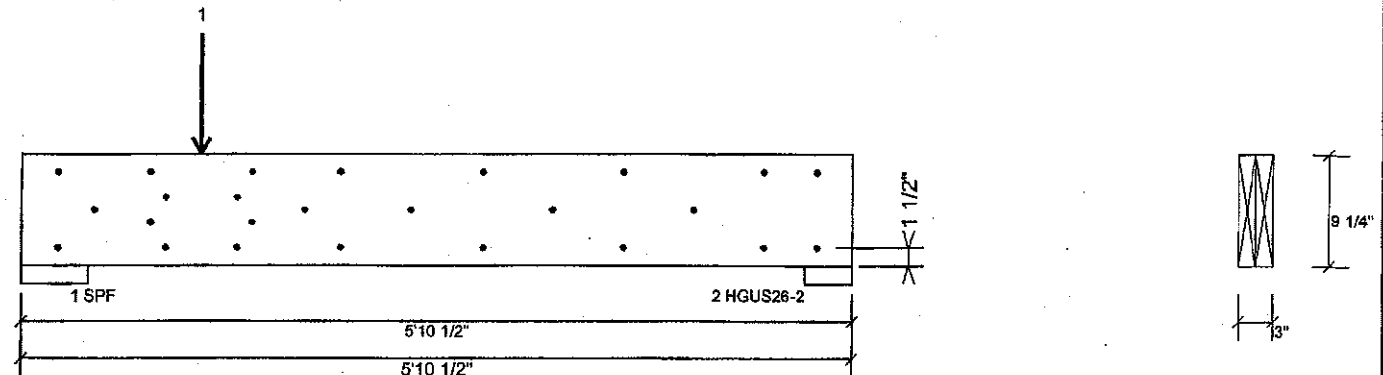
isDesign™

Client: Greenpark Homes
Project: Preston 3
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200444

BM5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c., except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

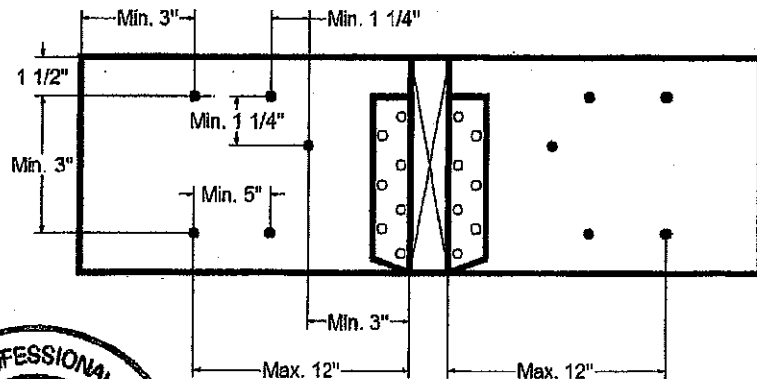
Capacity	10.4 %
Load	35.4 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 1-3-4 with a minimum of (8) – Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	87.9 %
Load	797.0 lb.
Total Yield Limit	906.8 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



DWG NO. TAM 11902734
STRUCTURAL
COMPONENT 3/3

This design is valid until 2021-12-11

Manufacturer Info

TAMARACK LUMBER
3255 NORTH SERVICE RD, ON
CANADA
(905) 335-1115



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp.20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



Double-Shear Nailing Top View



Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View (available on some models)
U.S. Patent 5,603,680



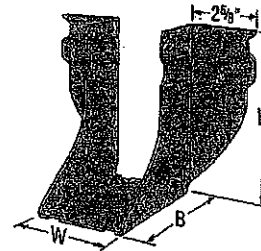
✓ LUS28



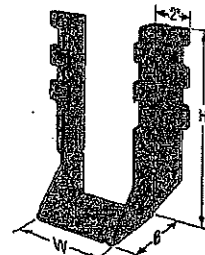
LU26L



✓ HUS210
(HUS26, HUS28, and HHUS similar)

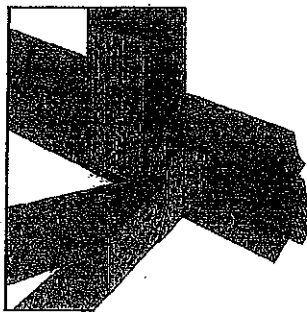


✓ HGUS28-2



✓ HHUS210-2

Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

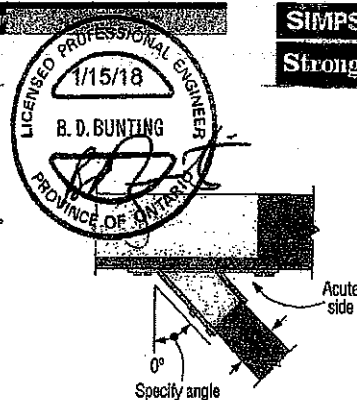
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(Joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	GA.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _e	d _e	Header	Joist	Uplift	Normal	Uplift	Normal
									(K _u = 1.15)	(K _u = 1.00)		
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Single 2x Sizes												
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d		710	1825	645	1155
LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"		360	1020	320	725
LU24L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"		720	1605	645	1140
LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d		1420	2170	1290	1630
HUS26	16	1 1/8	5 1/4	3	3 1/4	(16) 16d	(6) 16d		2705	4940	2085	3875
LJS28BS	18	1 1/8	5	3 1/4	4 1/4	(16) 16d	(6) 16d		2055	4265	1460	4115
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d		2685	6625	2685	5700
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"		1140	2185	1020	1550
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d		1420	2520	1290	1790
HUS28	16	1 1/8	7 1/4	3	8 1/4	(22) 16d	(8) 16d		3605	5365	2675	4345
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d		3310	7875	3310	6900
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"		1140	2495	1020	1770
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d		1420	2785	1290	2210

- Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
- Designer must ensure that hanger is compatible with truss when reduced heel height is used.
- d_e is the distance from the bearing seat to the top joist nail.
- Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
- Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

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BUILDING SECTION
FILE NO.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE CANADIAN BUILDING CODE

Simpson Strong-Tie® Wood Construction Connectors -- Canadian Limit States Design

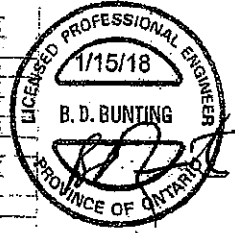
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

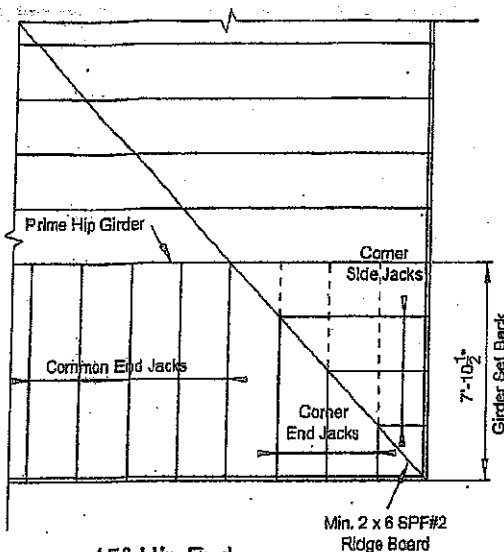
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _h	Header	Joist	D/F		S-P-F	
								Uplift (K _g = 1.15)	Normal (K _g = 1.00)	Uplift (K _g = 1.15)	Normal (K _g = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3 1/4	6 1/4	2	1 1/2	(4) 16d	(2) 16d	835	2020	690	1435
SS LUS28-2	18	3 1/4	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS26-2	14	3 3/4	5 1/4	3	3 3/4	(14) 16d	(6) 16d	2850	7335	2065	5205
HGUS26-2	12	3 3/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
SS LUS28-2	18	3 1/4	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
HHUS28-2	14	3 3/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3765	8940	2675	6345
HGUS28-2	12	3 3/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
SS LUS210-2	18	3 1/4	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
HHUS210-2	14	3 3/4	9 1/4	3	8	(30) 16d	(10) 16d	4870	9660	4235	7000
HGUS210-2	12	3 3/4	9 1/4	4	8 1/4	(48) 16d	(16) 16d	6840	14015	4855	10270
Triple 2x Sizes											
HGUS28-3	12	4 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-3	12	4 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-3	14	4 1/4	9	3	7 1/4	(36) 16d	(10) 16d	4670	9670	4235	6865
HGUS210-3	12	4 1/4	9 1/4	4	8 1/4	(48) 16d	(16) 16d	6840	14045	4855	10400
Quadruple 2x Sizes											
HGUS26-4	12	6 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS28-4	12	6 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-4	14	6 1/4	9	3	7 1/4	(36) 16d	(10) 16d	4670	10165	4235	7210
HGUS210-4	12	6 1/4	9 1/4	4	8 1/4	(48) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d	10130	16400	7195	11645
4x Sizes											
LUS48	18	3 1/4	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS48	14	3 3/4	5 1/4	3	3 3/4	(14) 16d	(6) 16d	2540	7335	2065	5205
HGUS48	12	3 3/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
LUS48	18	3 1/4	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2575
HHUS48	14	3 3/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3765	8940	2675	6345
HGUS48	12	3 3/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215
LUS410	18	3 1/4	8 1/4	2	5 1/4	(8) 16d	(6) 16d	2580	4500	2320	3195
HGUS410	12	3 3/4	9	4	8 1/4	(48) 16d	(16) 16d	6840	14015	4855	10270
HGUS412	12	3 3/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS414	12	3 3/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d	10130	16400	7195	11645



Plated Truss Connectors

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2019
See footnotes
BUILDING SECTION 259
FILE NO.



45° Hip End

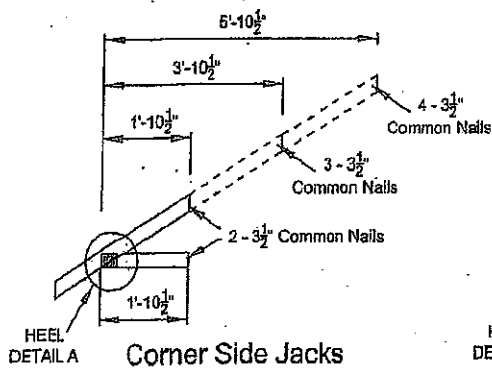
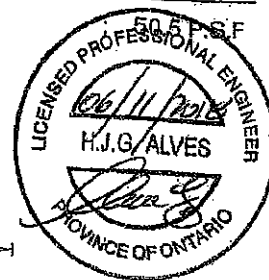
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

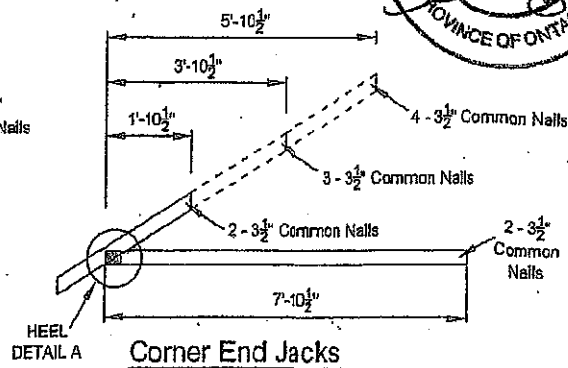
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

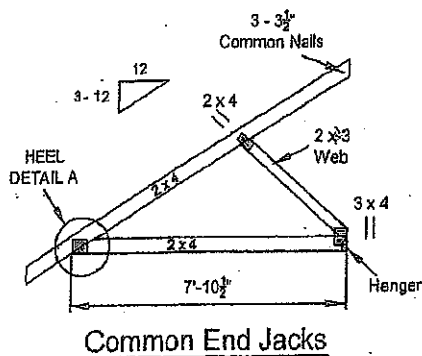
TOTAL LOAD



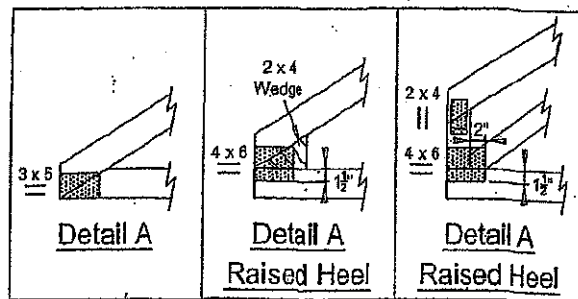
Corner Side Jacks



Corner End Jacks



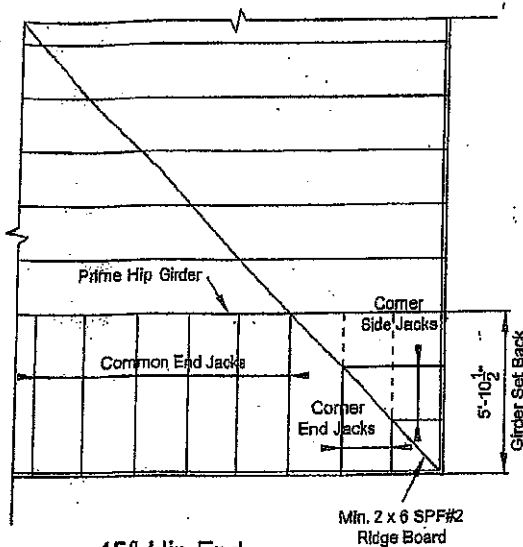
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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 BUILDING SECTION
 FILE NO



45° Hip End

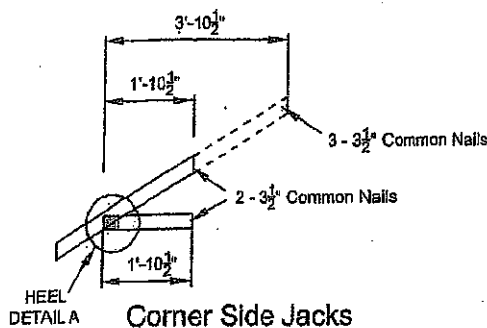
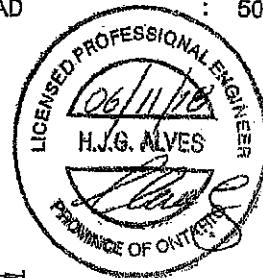
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

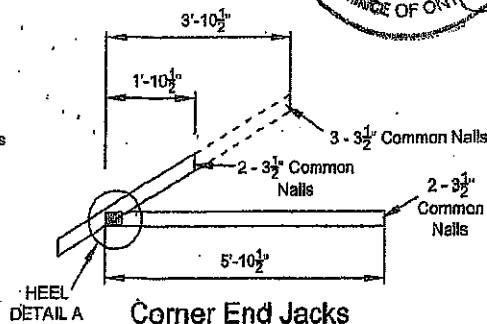
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

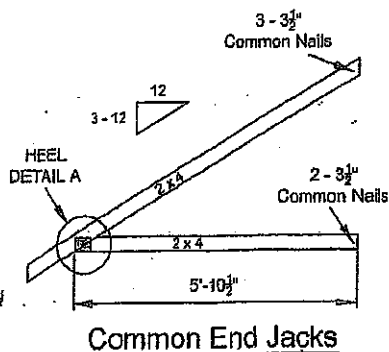
TOTAL LOAD : 50.5 P.S.F



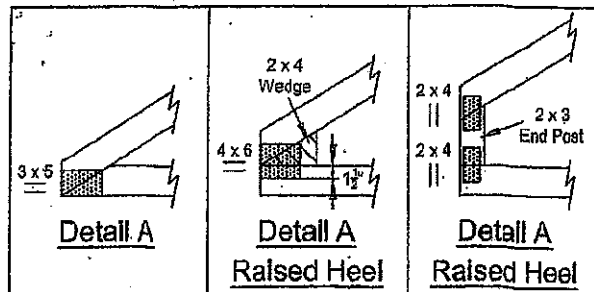
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

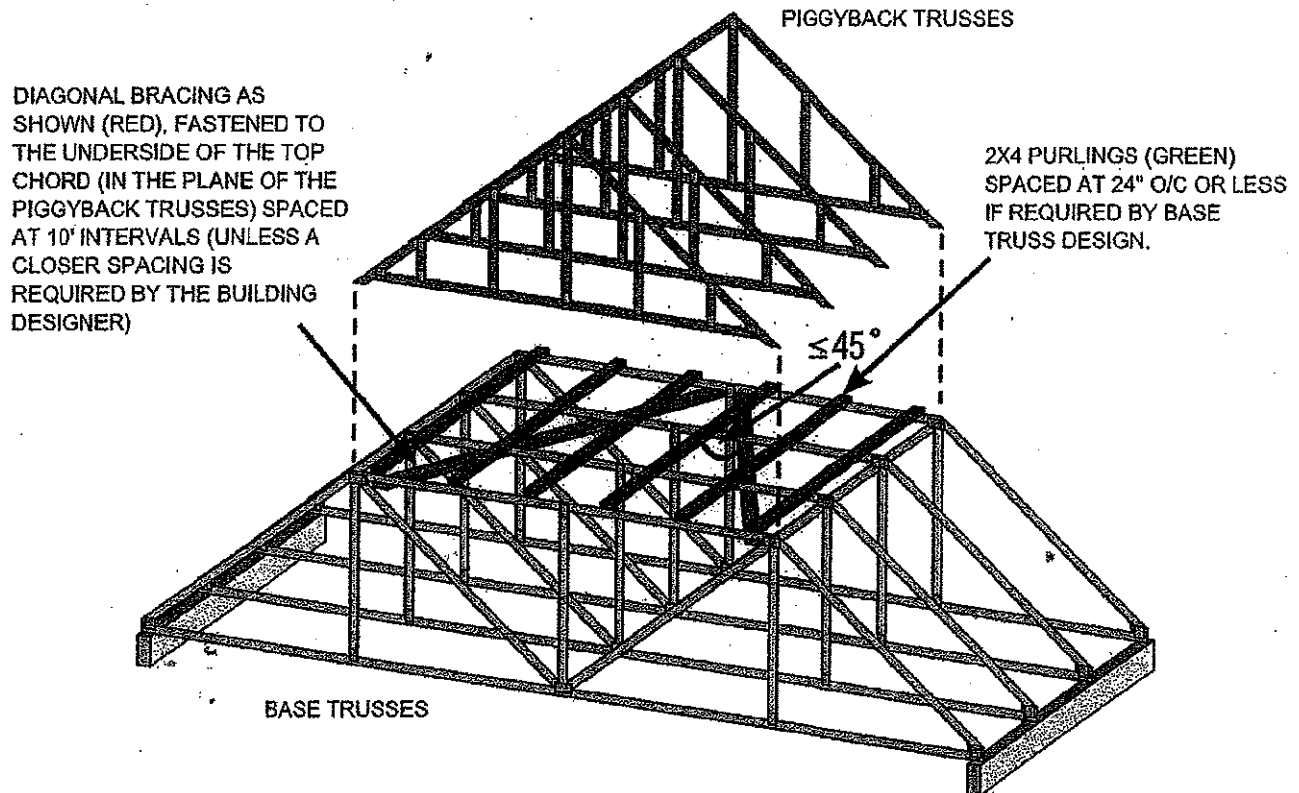
T-1800216

Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction. If this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCS CANADA 2013

AUG 28 2019

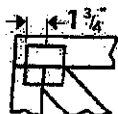
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

Disclaimer:

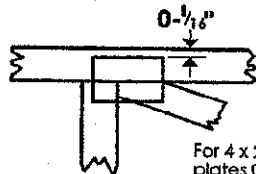
OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

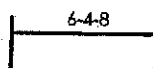


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

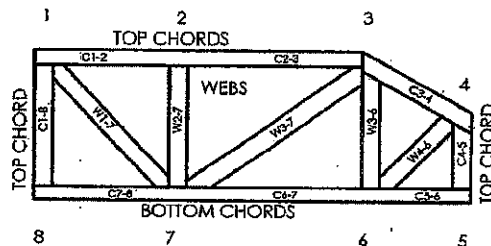
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate-Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

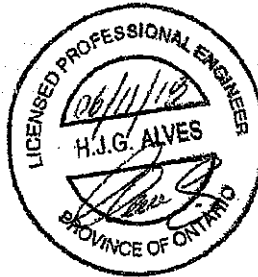
MiTek Engineering Reference Sheet: MIL-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

Feb 09, 2018