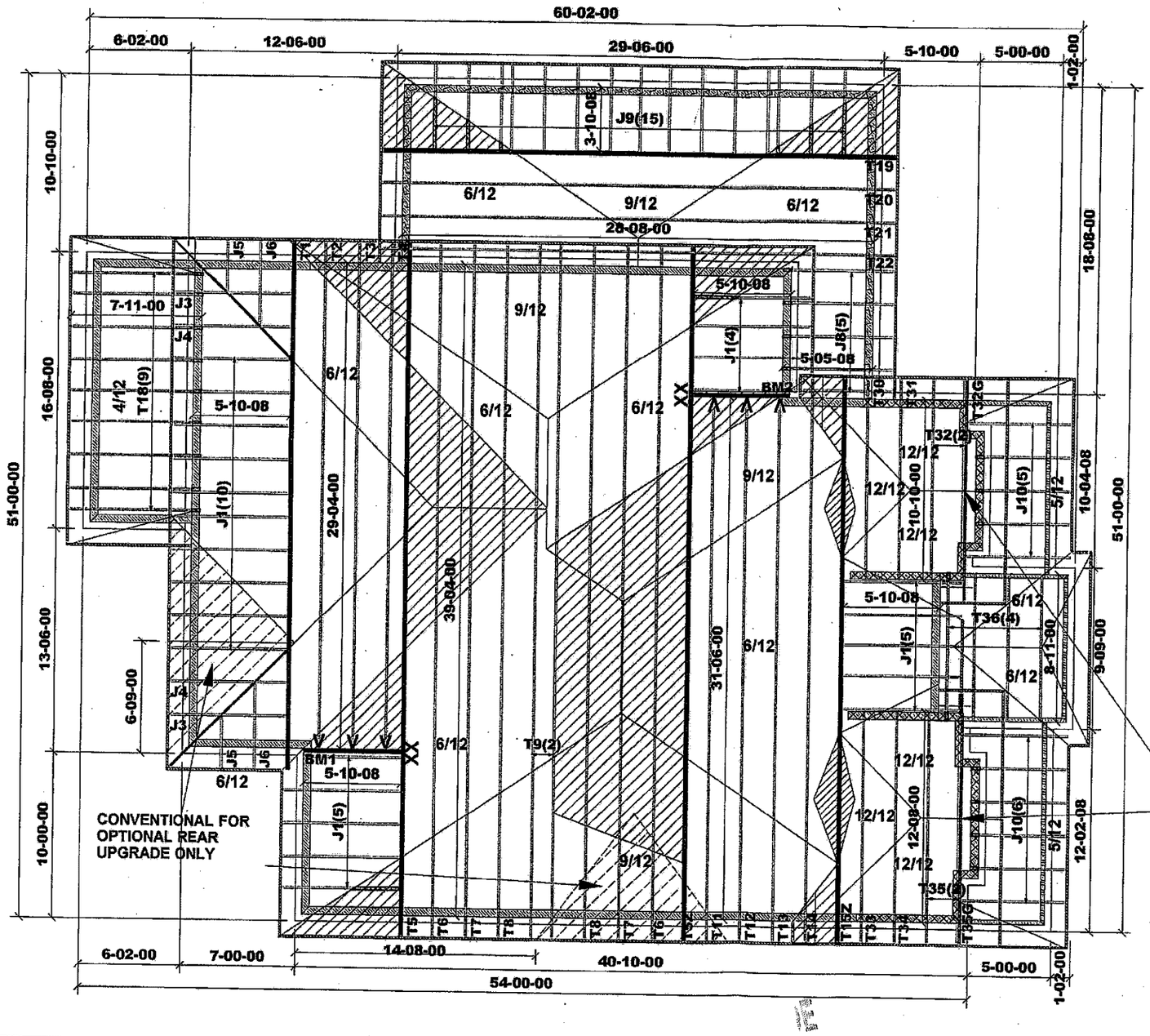


ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
GROUND SNOW LOAD Ss=37.6psf Sr=8.4psf
DESIGN LOADS
TCDL (6psf)
BCLL (10.5psf)
BCDL (7psf)

DENOTES:
CONVENTIONAL FRAMING

ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C

HARDWARE:
LJS26DS - (V) 6pcs
HGUS26-2 - (XX) 2pcs

BEAMS:
ALL BEAMS: 2 - 2x10 SPF #2

NAIL (T32 TO T32G) & (T35 TO T35G) FOR CONTINOUS SUPPORT WITH WITH ONE ROW OF STAGGERED 3" SPIRAL NAILS 4" O.C. -

DENOTES: 1' RAISED PLATE

DENOTES: STD HEEL : BOTTOM OF FASCIA 8-1/2" BELOW TOP OF PLATE

DENOTES: STD HEEL : BOTTOM OF FASCIA 9-3/4" BELOW TOP OF PLATE

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

TAMARACK LUMBER INC.

Job Track:	50033
Plan Log:	200234
Layout ID:	400669

Builder / Location: **GREEN PARK HOMES / CALEDON**

Project: **LAMBERT LANE PH.2**

Date: 2019-01-11 Designer: Brian

Model / Elevation: **BARTON 8 / ELV. 2**
STD & OPT REAR UPGRADE LOT 6

Mitek ver 8.2.3.229

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200234
Project:	LAMBERT LANE PH.2	Layout ID:	400669
Location:	CALEDON	Ref #	
Model:	BARTON 8	Page:	1 of 3
Lot #:		Date:	01-11-2019
Elevation:	2 STD & OPT REAR UPGRADE	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

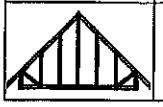
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	6 /12	29-04-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	268.97 170.67		
	1	T2 Hip	6 /12	29-04-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	134.01 82.50		
	1	T3 Hip	6 /12	29-04-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	139.47 85.33		
	1	T4 Hip	6 /12	29-04-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	136.82 85.17		
	1 2-ply	T5 Hip Girder	9 /12	39-04-00	4-01-04	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	423.12 262.33		
	1 2-ply	T5Z Hip Girder	9 /12	39-04-00	4-01-04	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	423.12 262.33		
	2	T6 Hip	9 /12	39-04-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	398.06 239.00		
	2	T7 Hip	9 /12	39-04-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	414.99 254.67		
	2	T8 Hip	9 /12	39-04-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	428.76 263.67		
	2	T9 Hip	9 /12	39-04-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	451.02 273.33		
	1	T11 Hip	9 /12	31-06-00	8-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	160.95 98.00		
	1	T12 Hip	9 /12	31-06-00	7-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	163.27 102.17		
	1	T13 Hip	9 /12	31-06-00	6-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	153.11 96.17		
	1	T14 Hip	9 /12	31-06-00	5-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	148.71 90.83		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200234
Project:	LAMBERT LANE PH.2	Layout ID:	400669
Location:	CALEDON	Ref #	
Model:	BARTON 8	Page:	2 of 3
Lot #:		Date:	01-11-2019
Elevation:	2 STD & OPT REAR UPGRADE	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

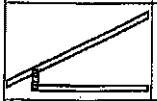
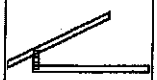
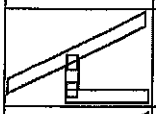
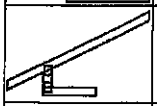
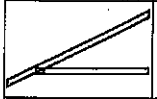
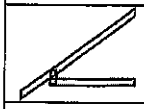
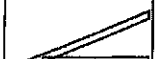
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T15Z Hip Girder	9 /12	31-06-00	4-01-04	2 x 4 2 x 6	1-05-00 1-05-00	1-06-04 1-06-04	295.66 182.67		
	9	T18 Monopitch	4 /12	6-00-04	3-01-11	2 x 4	1-03-08 7-04	11-03 2-11-04	237.68 159.00		
	1 2-ply	T19 Hip Girder	6 /12	28-08-00	4-02-13	2 x 4 2 x 6	1-03-08 1-03-08	4-03 4-03	259.51 156.00		
	1	T20 Hip	6 /12	28-08-00	5-08-13	2 x 4	1-03-08 1-03-08	4-03 4-03	104.82 65.83		
	1	T21 Hip	6 /12	28-08-00	7-02-13	2 x 4	1-03-08 1-03-08	4-03 4-03	103.51 65.50		
	1	T22 Common	6 /12	28-08-00	8-01-11	2 x 4	1-03-08 1-03-08	4-03 4-03	102.53 63.83		
	1	T30 Hip	12 /12	10-10-00	4-07-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	51.76 32.50		
	1	T31 Hip	12 /12	10-10-00	6-07-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	59.63 37.83		
	2	T32 Common	12 /12	10-10-00	7-03-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	103.75 65.67		
	1	T32G GABLE	12 /12	10-10-00	7-03-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	58.32 36.83		
	1	T33 Hip	12 /12	12-08-00	4-07-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	59.3 37.67		
	1	T34 Hip	12 /12	12-08-00	6-07-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	64.26 40.50		
	2	T35 Common	12 /12	12-08-00	8-02-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	132.84 89.33		
	1	T35G GABLE	12 /12	12-08-00	8-02-08	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	68.12 45.67		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200234
Project:	LAMBERT LANE PH.2	Layout ID:	400669
Location:	CALEDON	Ref #	
Model:	BARTON 8	Page:	3 of 3
Lot #:		Date:	01-11-2019
Elevation:	2 STD & OPT REAR UPGRADE	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	T36 Common	6 /12	8-11-00	2-08-03	2 x 4		5-07 5-07	94.2 62.00		
	24	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	403.07 256.00		
	2	J3 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J4 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J5 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J6 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	5	J8 Jack-Open	6 /12	5-05-08	3-08-07	2 x 4	1-03-08	4-03 3-00-15	74.98 46.67		
	15	J9 Jack-Open	9 /12	3-10-08	4-02-14	2 x 4	1-03-08	8-07 3-07-05	185 120.00		
	11	J10 Jack-Open	5 /12	4-05-08	2-08-09	2 x 4	1-03-08	4-01 2-02-05	135.58 88.00		

TOTAL # TRUSS= **114** TOTAL BFT OF ALL TRUSSES= **4071** BFT. TOTAL WEIGHT OF ALL TRSSES **6523.49** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **8**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T1	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Jan 16 09:41:04 2019 Page 1
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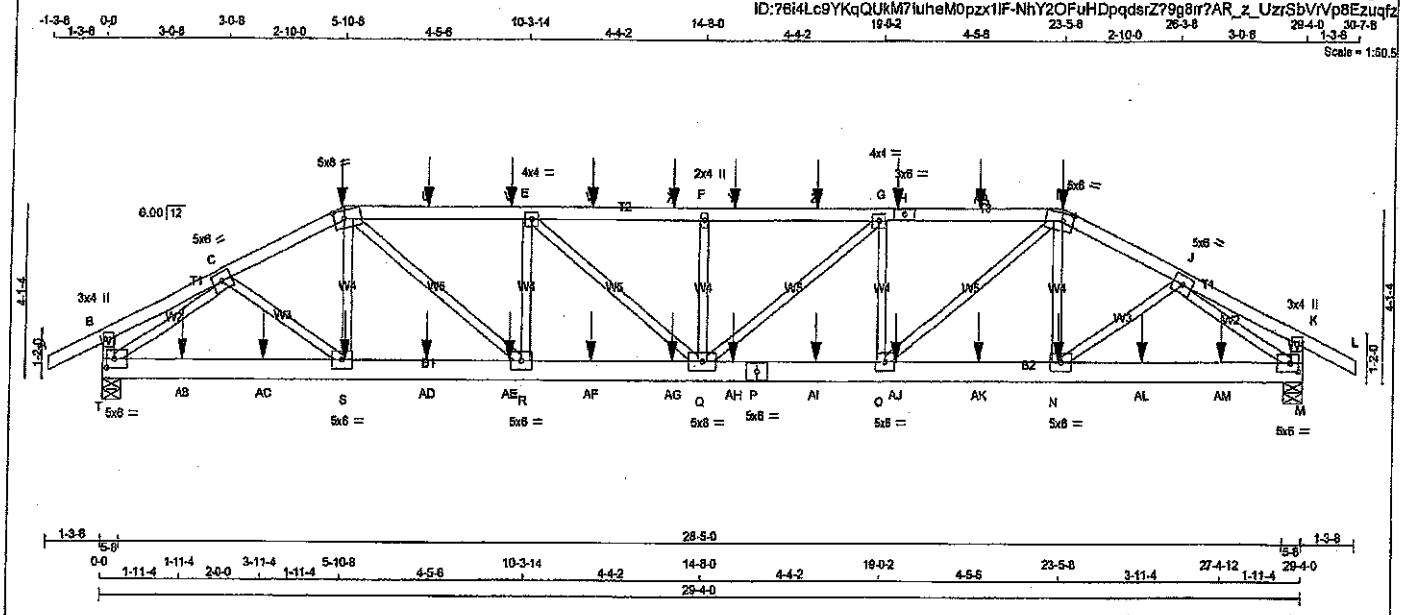


TOTAL WEIGHT = 2 X 134 = 269 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
D- H	1	12	SIDE(61.0)
H- I	1	12	SIDE(61.0)
I- L	1	12	SIDE(61.0)
T- B	1	12	TOP
M- K	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- P	2	12	SIDE(183.1)
P- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	6.0	2.25	3.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TTWW-m	MT20	5.0	6.0	2.25	3.00
J	TMWW-t	MT20	5.0	6.0		
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.25
N, O, R, S						
N	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
T	BMVW1-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	3445	0	3445	0	0	5-8	5-8
M	3446	0	3446	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2580	1408 / 0	527 / 0	0 / 0	0 / 0	645 / 0	0 / 0
M	2581	1409 / 0	527 / 0	0 / 0	0 / 0	645 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

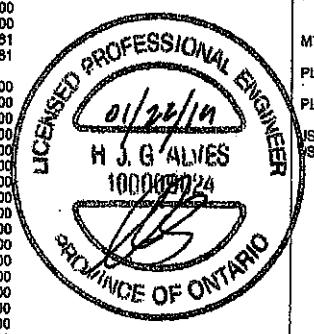
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX UNBRAC LENGTH
FR-TO					
A- B	0 / 31	-102.1	-102.1	0.07 (1)	10.00
B- C	-2 / 6	-102.1	-102.1	0.07 (1)	10.00
C- D	-5070 / 0	-102.1	-102.1	0.16 (1)	4.18
D- U	-6366 / 0	-102.1	-102.1	0.42 (1)	3.53
U- V	-6366 / 0	-102.1	-102.1	0.42 (1)	3.53
V- E	-6366 / 0	-102.1	-102.1	0.42 (1)	3.53
E- W	-6929 / 0	-102.1	-102.1	0.44 (1)	3.36
W- X	-6929 / 0	-102.1	-102.1	0.44 (1)	3.36
X- F	-6929 / 0	-102.1	-102.1	0.44 (1)	3.36
F- Y	-6929 / 0	-102.1	-102.1	0.45 (1)	3.36
Y- Z	-6929 / 0	-102.1	-102.1	0.45 (1)	3.36
Z- G	-6929 / 0	-102.1	-102.1	0.45 (1)	3.36
G- H	-6366 / 0	-102.1	-102.1	0.42 (1)	3.53
H-AA	-6366 / 0	-102.1	-102.1	0.42 (1)	3.53
AA- I	-6366 / 0	-102.1	-102.1	0.42 (1)	3.53
I- J	-5072 / 0	-102.1	-102.1	0.16 (1)	4.18
J- K	-2 / 6	-102.1	-102.1	0.07 (1)	10.00
K- L	0 / 31	-102.1	-102.1	0.07 (1)	10.00
T- B	-275 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-275 / 0	0.0	0.0	0.02 (1)	7.81
T-AB	0 / 3962	-38.5	-38.5	0.34 (1)	10.00
AB-AC	0 / 3962	-38.5	-38.5	0.34 (1)	10.00
AC- S	0 / 3962	-38.5	-38.5	0.34 (1)	10.00
S-AD	0 / 4523	-38.5	-38.5	0.34 (1)	10.00
AD-AE	0 / 4523	-38.5	-38.5	0.34 (1)	10.00
AE- R	0 / 4523	-38.5	-38.5	0.34 (1)	10.00
R-AF	0 / 6366	-38.5	-38.5	0.48 (1)	10.00
AF-AG	0 / 6366	-38.5	-38.5	0.48 (1)	10.00
AG- Q	0 / 6366	-38.5	-38.5	0.48 (1)	10.00
Q-AH	0 / 6366	-38.5	-38.5	0.48 (1)	10.00
AH- P	0 / 6366	-38.5	-38.5	0.48 (1)	10.00
P-AI	0 / 6366	-38.5	-38.5	0.48 (1)	10.00
AI- O	0 / 6366	-38.5	-38.5	0.48 (1)	10.00
O-AJ	0 / 4525	-38.5	-38.5	0.34 (1)	10.00
AJ-AK	0 / 4525	-38.5	-38.5	0.34 (1)	10.00
AK- N	0 / 4525	-38.5	-38.5	0.34 (1)	10.00
N-AL	0 / 3963	-38.5	-38.5	0.34 (1)	10.00
AL-AM	0 / 3963	-38.5	-38.5	0.34 (1)	10.00
AM- M	0 / 3963	-38.5	-38.5	0.34 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- S	0 / 700	0.09 (1)
S- D	-19 / 258	0.03 (3)
D- R	0 / 2403	0.30 (1)
R- E	-1280 / 0	0.16 (1)
E- Q	0 / 742	0.09 (1)
Q- F	-688 / 0	0.09 (1)
Q- G	0 / 743	0.09 (1)
O- G	-1277 / 0	0.16 (1)
O- I	0 / 2400	0.30 (1)
N- I	-20 / 258	0.03 (3)
N- J	0 / 700	0.09 (1)
T- C	-4982 / 0	0.62 (1)
J- M	-4984 / 0	0.62 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.45/1.00 (F-G:1) , BC=0.48/1.00 (Q-R:1) , WB=0.62/1.00 (J-M:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (D) (INPUT = 0.90)
SI METAL= 0.85 (C) (INPUT = 1.00)

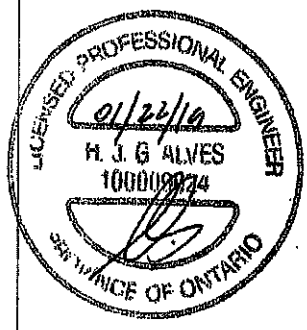
DWG NO. T1401913 1/2
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T1	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	5-10-8	-369	-369	--	FRONT	VERT	TOTAL	--	--
H	19-4-12	-123	-123	--	FRONT	VERT	TOTAL	--	--
I	23-5-8	-369	-369	--	FRONT	VERT	TOTAL	--	--
N	23-4-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
S	5-11-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
U	7-11-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
V	9-11-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
W	11-11-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
X	13-11-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
Y	15-4-12	-123	-123	--	FRONT	VERT	TOTAL	--	--
Z	17-4-12	-123	-123	--	FRONT	VERT	TOTAL	--	--
AA	21-4-12	-123	-123	--	FRONT	VERT	TOTAL	--	--
AB	1-11-4	-55	-74	--	FRONT	VERT	TOTAL	--	--
AC	3-11-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
AD	7-11-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
AE	9-11-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
AF	11-11-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
AG	13-11-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
AH	15-4-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
AI	17-4-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
AJ	19-4-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
AK	21-4-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
AL	25-4-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
AM	27-4-12	-55	-74	--	FRONT	VERT	TOTAL	--	--



DWG NO. T1901913
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T2	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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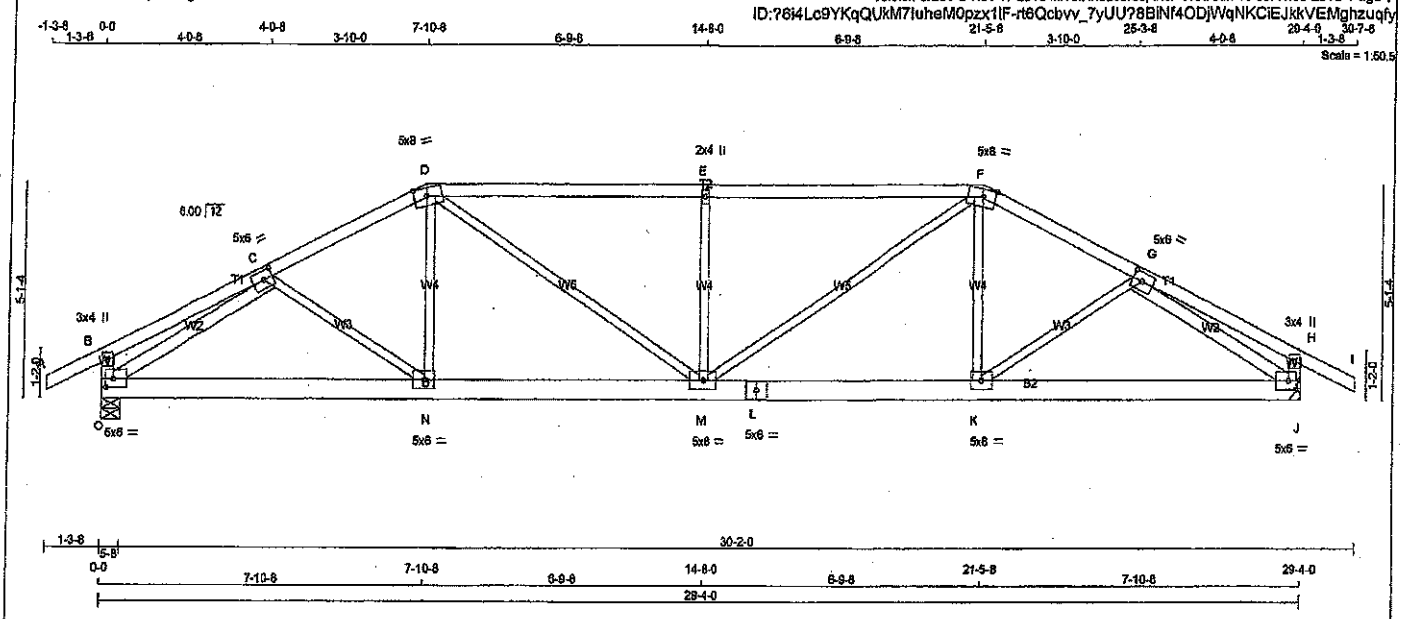


TOTAL WEIGHT = 134 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	2.25	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.25	3.50
G	TMWW-t	MT20	5.0	8.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	8.0	2.50	2.25
K	BMWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	8.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	8.0		
O	BMVW1-t	MT20	5.0	8.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2201	0	2201	0	0	5-8	5-8
J	2201	0	2201	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1636	930 / 0	308 / 0	0 / 0	0 / 0	398 / 0	0 / 0
J	1636	930 / 0	308 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

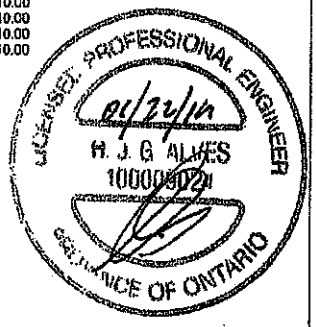
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	
B- C	0 / 19	-102.1	-102.1	0.23 (1)	10.00	
C- D	-2881 / 0	-102.1	-102.1	0.28 (1)	3.69	
D- E	-3321 / 0	-102.1	-102.1	0.83 (1)	2.95	
E- F	-3321 / 0	-102.1	-102.1	0.83 (1)	2.95	
F- G	-2881 / 0	-102.1	-102.1	0.28 (1)	3.69	
G- H	0 / 19	-102.1	-102.1	0.23 (1)	10.00	
H- I	0 / 31	-102.1	-102.1	0.13 (1)	10.00	
O- B	-299 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-299 / 0	0.0	0.0	0.03 (1)	7.81	
O- N	0 / 2552	-38.5	-38.5	0.44 (2)	10.00	
N- M	0 / 2565	-38.5	-38.5	0.45 (2)	10.00	
M- L	0 / 2565	-38.5	-38.5	0.45 (2)	10.00	
L- K	0 / 2565	-38.5	-38.5	0.45 (2)	10.00	
K- J	0 / 2552	-38.5	-38.5	0.44 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 145	0.03 (3)
N- D	0 / 364	0.09 (2)
D- M	0 / 921	0.21 (1)
M- E	-851 / 0	0.30 (1)
M- F	0 / 921	0.21 (1)
K- F	0 / 364	0.09 (2)
K- G	0 / 145	0.03 (3)
O- C	-3108 / 0	0.84 (1)
G- J	-3108 / 0	0.84 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.83/1.00 (D-E:1) , BC=0.45/1.00 (K-M:2) , WB=0.84/1.00 (G-J:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.77 (G) (INPUT = 1.00)

DWG NO. TAM T1901914
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T3	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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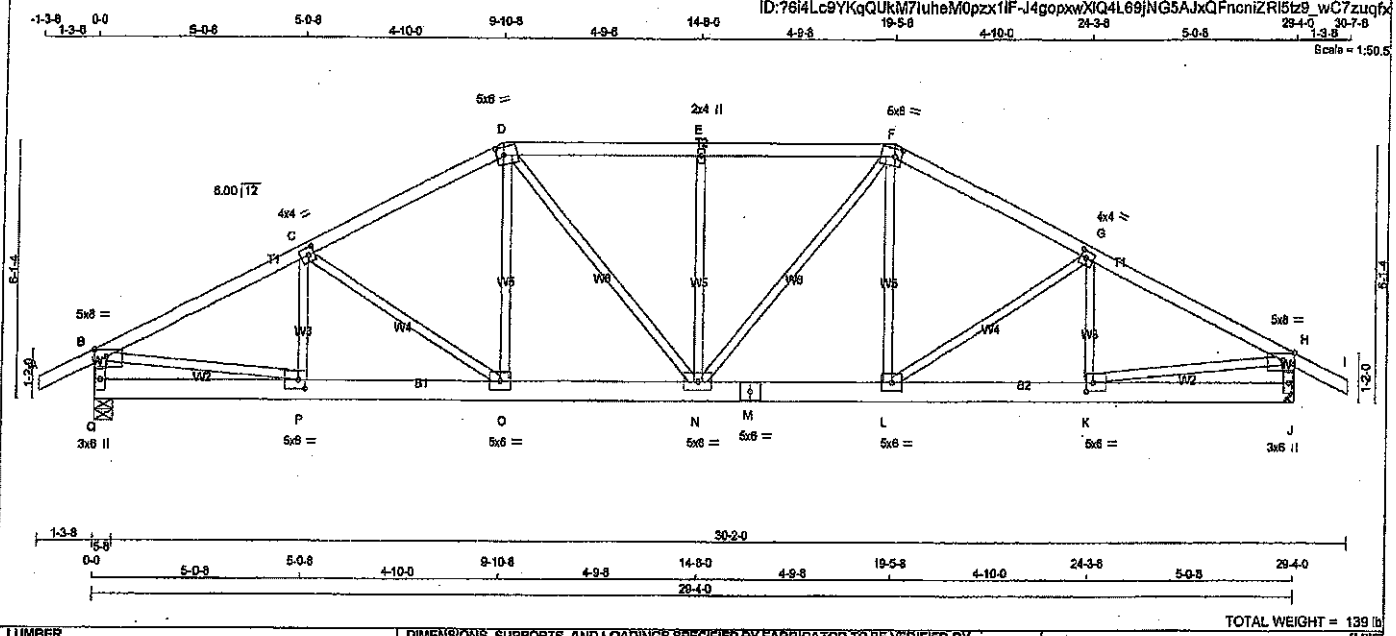


TOTAL WEIGHT = 139 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.00
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2201	0	2201	0	0	5-8	5-8
J	2201	0	2201	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-10.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1636	930 / 0	308 / 0	0 / 0	0 / 0	398 / 0	0 / 0
J	1636	930 / 0	308 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

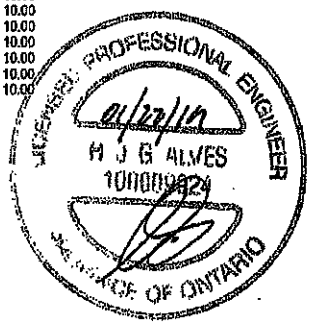
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	
B-C	-2982 / 0	-102.1	-102.1	0.44 (1)	3.67	
C-D	-2676 / 0	-102.1	-102.1	0.40 (1)	3.88	
D-E	-2689 / 0	-102.1	-102.1	0.38 (1)	3.88	
E-F	-2689 / 0	-102.1	-102.1	0.38 (1)	3.88	
F-G	-2678 / 0	-102.1	-102.1	0.40 (1)	3.88	
G-H	-2982 / 0	-102.1	-102.1	0.44 (1)	3.67	
H-I	0 / 31	-102.1	-102.1	0.13 (1)	10.00	
Q-B	-2098 / 0	0.0	0.0	0.21 (1)	5.84	
J-H	-2098 / 0	0.0	0.0	0.21 (1)	5.84	
Q-P	0 / 0	-38.5	-38.5	0.10 (2)	10.00	
P-O	0 / 2689	-38.5	-38.5	0.38 (1)	10.00	
O-N	0 / 2379	-38.5	-38.5	0.34 (1)	10.00	
N-M	0 / 2379	-38.5	-38.5	0.34 (1)	10.00	
M-L	0 / 2379	-38.5	-38.5	0.34 (1)	10.00	
L-K	0 / 2689	-38.5	-38.5	0.38 (1)	10.00	
K-J	0 / 0	-38.5	-38.5	0.10 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-250 / 67	0.05 (1)
C-O	-383 / 0	0.23 (1)
O-D	0 / 447	0.10 (2)
D-N	0 / 483	0.11 (1)
N-E	-593 / 0	0.32 (1)
N-F	0 / 483	0.11 (1)
L-F	0 / 447	0.10 (2)
L-G	-383 / 0	0.23 (1)
K-G	-250 / 67	0.05 (1)
B-P	0 / 2721	0.61 (1)
K-H	0 / 2721	0.61 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.44/1.00 (G-H:1) , BC=0.38/1.00 (K-L:1) , WB=0.61/1.00 (H-K:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	619	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.87 (P) (INPUT = 1.00)

DWG NO. TAM T1901915
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T4	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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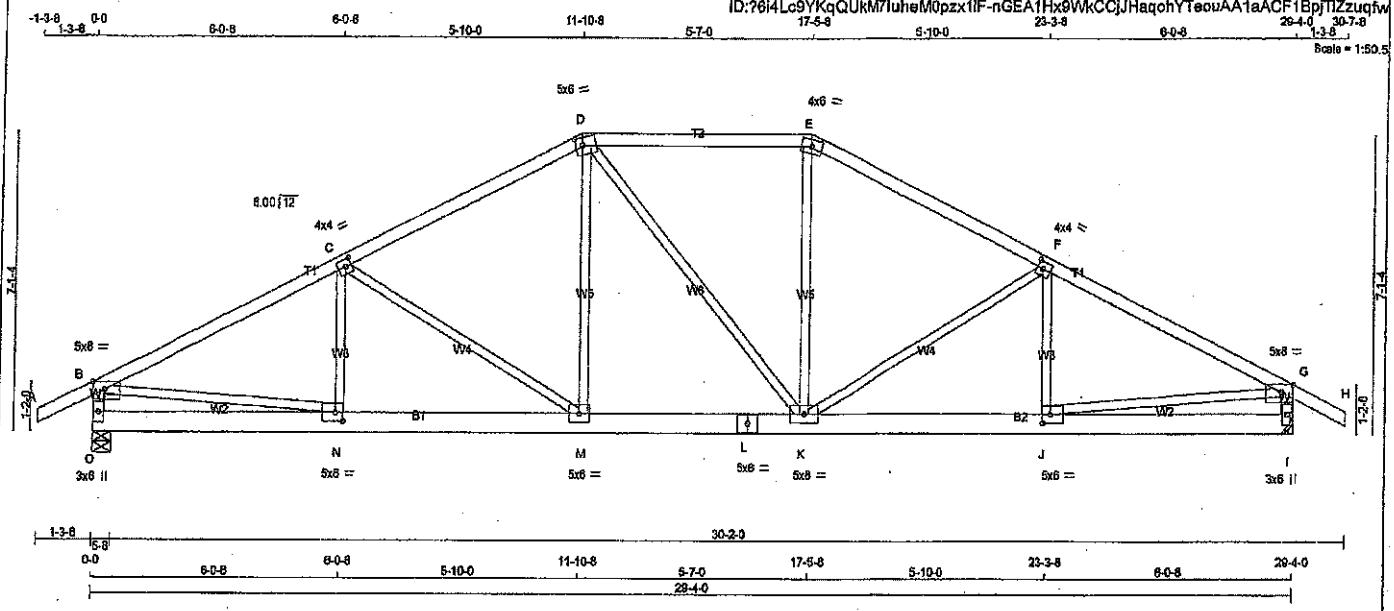
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Scale = 1:50.5
5x6 = 4x6 = 4x4 = 4x4 = 5x8 = 5x8 = 3x6 = 3x6 =
8.00 12
7-1-4
1-2-0
T1 T2 T3 T4
W1 W2 W3 W4 W5 W6
B1 B2
O N M L K J I
A B C D E F G H

TOTAL WEIGHT = 137 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW-p	MT20	5.0	8.0	Edge	3.50
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.25
K	BMWWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2201	0	2201	0	0	5-8	5-8
I	2201	0	2201	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-10.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1636	930 / 0	308 / 0	0 / 0	0 / 0	398 / 0	0 / 0
I	1636	930 / 0	308 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

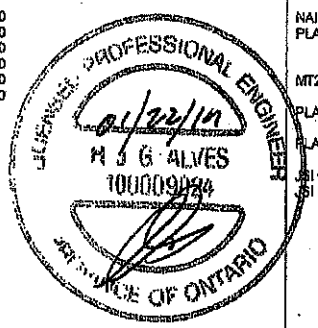
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.00
B- C	-3017 / 0	-102.1	-102.1	0.64 (1)	3.42
C- D	-2465 / 0	-102.1	-102.1	0.56 (1)	3.81
D- E	-2186 / 0	-102.1	-102.1	0.48 (1)	4.10
E- F	-2468 / 0	-102.1	-102.1	0.56 (1)	3.81
F- G	-3016 / 0	-102.1	-102.1	0.64 (1)	3.42
G- H	0 / 31	-102.1	-102.1	0.13 (1)	10.00
O- B	-2090 / 0	0.0	0.0	0.21 (1)	5.86
I- G	-2089 / 0	0.0	0.0	0.21 (1)	5.86
O- N	0 / 0	-36.5	-36.5	0.12 (2)	10.00
N- M	0 / 2726	-36.5	-36.5	0.39 (1)	10.00
M- L	0 / 2184	-36.5	-36.5	0.33 (1)	10.00
L- K	0 / 2184	-36.5	-36.5	0.33 (1)	10.00
K- J	0 / 2725	-36.5	-36.5	0.39 (1)	10.00
J- I	0 / 0	-36.5	-36.5	0.12 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-123 / 161	0.04 (3)
C- M	-655 / 0	0.61 (1)
M- D	0 / 561	0.13 (2)
D- K	0 / 4	0.00 (2)
K- E	0 / 564	0.13 (2)
K- F	-651 / 0	0.61 (1)
J- F	-127 / 158	0.04 (3)
B- N	0 / 2749	0.62 (1)
J- G	0 / 2748	0.62 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.64/1.00 (B-C:1) , BC=0.39/1.00 (M-N:1) , WB=0.62/1.00 (B-N:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (N) (INPUT = 0.90)
SSI METAL= 0.68 (N) (INPUT = 1.00)

DWG NO. TAM T4 01/16/19
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T5	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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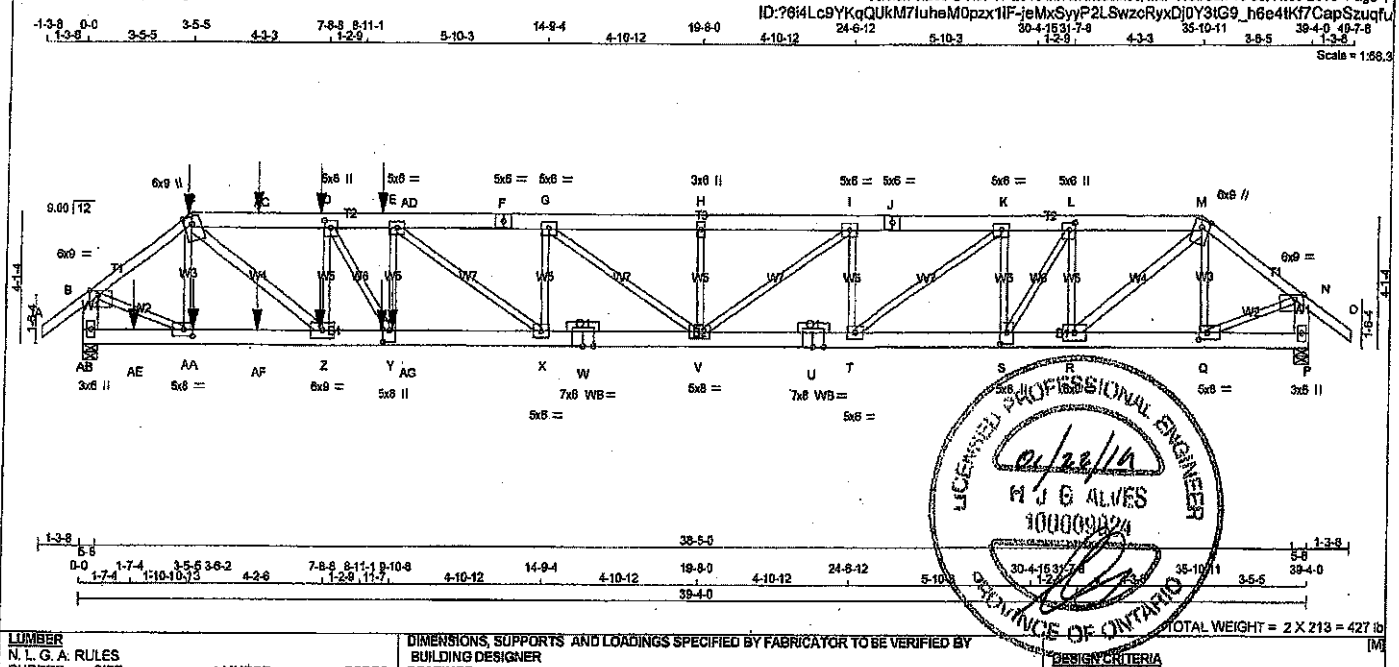

TOTAL WEIGHT = 2 X 213 = 427 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
M - O	2x4	DRY	No.2	SPF
AB- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AB- W	2x6	DRY	2100F 1.8E	SPF
W - U	2x6	DRY	2100F 1.8E	SPF
U - P	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
C - Z	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
M- O	1	12	TOP
C- F	2	12	SIDE(183.1)
F- J	2	12	TOP
J- M	2	12	TOP
AB- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AB- W	2	12	SIDE(197.8)
W- U	2	12	TOP
U- P	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
E- Y	1	2	SIDE(692.0)
K- S	1	2	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	3.25
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW+t	MT20	5.0	6.0	2.75	2.25
E, G, I, K						
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	3.0	6.0		
J	TS-t	MT20	5.0	6.0		
L	TMWW+t	MT20	5.0	6.0	2.75	2.25
M	TTWW+m	MT20	6.0	9.0	Edge	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AB	6498	0	6498	0	0	5-8	5-8
P	3920	0	3920	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	4826	2758 / 0	695 / 0	0 / 0	0 / 0	1172 / 0	0 / 0
P	2916	1653 / 0	553 / 0	0 / 0	0 / 0	711 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.08 (1)		10.00
B- C	-7060 / 0	-102.1	-102.1	0.31 (1)		3.48
C-AC	-11424 / 0	-102.1	-102.1	0.37 (1)		3.42
AC- D	-11424 / 0	-102.1	-102.1	0.37 (1)		3.42
D-AD	-13834 / 0	-102.1	-102.1	0.39 (1)		3.13
AD- E	-13834 / 0	-102.1	-102.1	0.39 (1)		3.13
E- F	-13862 / 0	-102.1	-102.1	0.46 (1)		3.03
F- G	-13862 / 0	-102.1	-102.1	0.46 (1)		3.03
G- H	-12893 / 0	-102.1	-102.1	0.40 (1)		3.22
H- I	-12893 / 0	-102.1	-102.1	0.40 (1)		3.22
I- J	-11118 / 0	-102.1	-102.1	0.32 (1)		3.52
J- K	-11118 / 0	-102.1	-102.1	0.32 (1)		3.52
K- L	-8347 / 0	-102.1	-102.1	0.16 (1)		4.13
L- M	-6884 / 0	-102.1	-102.1	0.16 (1)		4.47
M- N	-4111 / 0	-102.1	-102.1	0.18 (1)		4.54
N- O	0 / 42	-102.1	-102.1	0.06 (1)		10.00
AB- B	-6409 / 0	0.0	0.0	0.23 (1)		5.87
P- N	-3884 / 0	0.0	0.0	0.14 (1)		7.20
AB-AE	0 / 0	-38.5	-38.5	0.02 (2)		10.00
AE-AA	0 / 0	-38.5	-38.5	0.02 (2)		10.00
AA-AF	0 / 5713	-38.5	-38.5	0.17 (1)		10.00
AF- Z	0 / 5713	-38.5	-38.5	0.17 (1)		10.00
Z-AG	0 / 11424	-38.5	-38.5	0.40 (1)		10.00
AG- Y	0 / 11424	-38.5	-38.5	0.40 (1)		10.00
Y- X	0 / 13834	-38.5	-38.5	0.45 (1)		10.00
X- W	0 / 13862	-38.5	-38.5	0.41 (1)		10.00
W- V	0 / 13862	-38.5	-38.5	0.41 (1)		10.00
V- U	0 / 11118	-38.5	-38.5	0.34 (1)		10.00
U- T	0 / 11118	-38.5	-38.5	0.34 (1)		10.00
T- S	0 / 8347	-38.5	-38.5	0.26 (1)		10.00
S- R	0 / 6884	-38.5	-38.5	0.20 (1)		10.00
R- Q	0 / 3326	-38.5	-38.5	0.10 (1)		10.00
Q- P	0 / 0	-38.5	-38.5	0.01 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- C	-1640 / 0	0.20 (1)
Y- E	-493 / 0	0.06 (1)
S- K	-2391 / 0	0.29 (1)
R- M	0 / 4694	0.41 (1)
Q- M	-943 / 0	0.12 (1)
B-AA	0 / 5970	0.74 (1)
Q- N	0 / 3476	0.43 (1)
T- K	0 / 3455	0.43 (1)
E- X	0 / 284	0.04 (1)
T- I	-1847 / 0	0.22 (1)
X- G	0 / 204	0.03 (3)
V- I	0 / 2212	0.27 (1)
G- V	-1208 / 0	0.38 (1)
V- H	-466 / 0	0.06 (1)
Z- D	-4408 / 0	0.53 (1)
R- L	-2873 / 0	0.34 (1)
C- Z	0 / 7535	0.67 (1)
D- Y	0 / 4325	0.54 (1)
S- L	0 / 2893	0.35 (1)

DWG NO. TAM T1901A17
STRUCTURAL COMPONENT ONLY 1/2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-63	-70	---	FRONT	VERT	DEAD	---	---
C	3-5-5	-130	-130	---	BACK	VERT	TOTAL	---	---
C	3-5-5	-360	-360	---	FRONT	VERT	SNOW	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADOT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.36")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 782 (0.60")

CSI: TC=0.48/1.00 (E-G:1) , BC=0.45/1.00 (X-Y:1) , WB=0.74/1.00 (B-AA:1) , SSI=0.11/1.00 (Y-Z:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 1.00 (W) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:76i4Lc9YKqQUkM7IuheM0pzx1lF-jeMxSyyP2LSwzcRyxDj0Y3IG9_h6e4tKf7CapSzuqfu

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	TMVW-p	MT20	6.0	9.0	Edge	3.25
P	BMV1+p	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	3.25
R	BMWW-t	MT20	6.0	9.0		
S	BMWW+t	MT20	5.0	8.0	4.25	2.50
T	BMWW-t	MT20	5.0	6.0		
U	BS-t	MT20	7.0	8.0		
V	BMWWW-t	MT20	5.0	8.0		
W	BS-t	MT20	7.0	8.0		
X	BMWW-t	MT20	5.0	6.0		
Y	BMWW+t	MT20	5.0	8.0	4.25	2.50
Z	BMWW-t	MT20	6.0	9.0		
AA	BMWW-t	MT20	5.0	8.0	2.50	3.25
AB	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	7-6-2	-123	-123	--	BACK	VERT	TOTAL	--	--
Y	9-10-8	-3411	-3411	--	BACK	VERT	TOTAL	--	--
Z	7-6-2	-55	-70	--	BACK	VERT	TOTAL	--	--
AA	3-6-2	-55	-70	--	BACK	VERT	TOTAL	--	--
AC	5-6-2	-123	-123	--	BACK	VERT	TOTAL	--	--
AD	9-6-2	-123	-123	--	BACK	VERT	TOTAL	--	--
AE	1-7-4	-55	-70	--	BACK	VERT	TOTAL	--	--
AF	5-6-2	-55	-70	--	BACK	VERT	TOTAL	--	--
AG	9-6-2	-55	-70	--	BACK	VERT	TOTAL	--	--

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
01/22/19
H. J. G ALVES
100090124
PROVINCE OF ONTARIO

RECEIVED
AUG 08 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM T190117
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T5Z	1	2	TRUSS DESC.	

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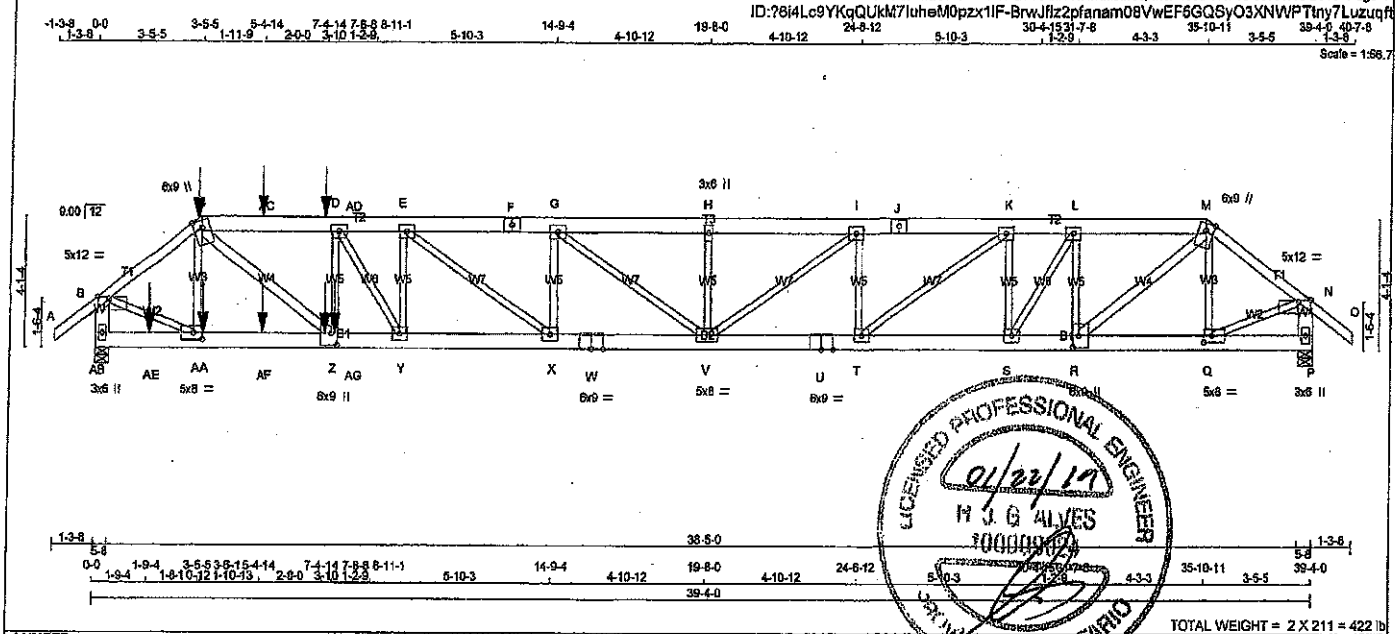

TOTAL WEIGHT = 2 X 211 = 422 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
M - O	2x4	DRY	No.2	SPF
AB- B	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
AB- W	2x6	DRY	2100F 1.8E	SPF
W - U	2x6	DRY	2100F 1.8E	SPF
U - P	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
C - Z	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
M- O	1	12	TOP
C- F	2	12	SIDE(183.1)
F- J	2	12	TOP
J- M	2	12	TOP
AB- B	2	12	TOP
P- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AB- W	2	12	SIDE(197.8)
W- U	2	12	TOP
U- P	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
Z- D	1	2	SIDE(784.5)
R- L	1	2	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	12.0	Edge	4.75
C	TTWW+m	MT20	6.0	9.0	Edge	
D, E, G, I, K, L						
D	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	3.0	6.0		
J	TS-t	MT20	5.0	6.0		
M	TTWW+m	MT20	6.0	9.0	Edge	
N	TMVW-p	MT20	5.0	12.0	Edge	4.75
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AB	6838	0	6838	0	0	5-8	5-8
P	3757	0	3757	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AB	5076	2903 / 0	941 / 0	0 / 0	0 / 0	1233 / 0	0 / 0
P	2795	1584 / 0	530 / 0	0 / 0	0 / 0	681 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.08 (1)		10.00
B- C	-7484 / 0	-102.1	-102.1	0.34 (1)		3.36
C-AC	-11982 / 0	-102.1	-102.1	0.42 (1)		3.29
AC-AD	-11982 / 0	-102.1	-102.1	0.42 (1)		3.29
AD- D	-11982 / 0	-102.1	-102.1	0.42 (1)		3.29
D- E	-12484 / 0	-102.1	-102.1	0.30 (1)		3.34
E- F	-12736 / 0	-102.1	-102.1	0.40 (1)		3.24
F- G	-12736 / 0	-102.1	-102.1	0.40 (1)		3.24
G- H	-12027 / 0	-102.1	-102.1	0.36 (1)		3.36
H- I	-12027 / 0	-102.1	-102.1	0.36 (1)		3.36
I- J	-10460 / 0	-102.1	-102.1	0.29 (1)		3.64
J- K	-10460 / 0	-102.1	-102.1	0.29 (1)		3.64
K- L	-7908 / 0	-102.1	-102.1	0.14 (1)		4.24
L- M	-6539 / 0	-102.1	-102.1	0.15 (1)		4.58
M- N	-3922 / 0	-102.1	-102.1	0.18 (1)		4.63
N- O	0 / 42	-102.1	-102.1	0.08 (1)		10.00
AB- B	-6784 / 0	0.0	0.0	0.25 (1)		5.72
P- N	-3701 / 0	0.0	0.0	0.13 (1)		7.32
AB-AE	0 / 0	-38.5	-38.5	0.03 (2)		10.00
AE-AA	0 / 0	-38.5	-38.5	0.03 (2)		10.00
AA-AF	0 / 6060	-38.5	-38.5	0.22 (1)		10.00
AF-AG	0 / 6060	-38.5	-38.5	0.22 (1)		10.00
AG- Z	0 / 6060	-38.5	-38.5	0.22 (1)		10.00
Z- Y	0 / 11981	-38.5	-38.5	0.38 (1)		10.00
Y- X	0 / 12464	-38.5	-38.5	0.37 (1)		10.00
X- W	0 / 12736	-38.5	-38.5	0.38 (1)		10.00
W- V	0 / 12736	-38.5	-38.5	0.38 (1)		10.00
V- U	0 / 10460	-38.5	-38.5	0.32 (1)		10.00
U- T	0 / 10460	-38.5	-38.5	0.32 (1)		10.00
T- S	0 / 7908	-38.5	-38.5	0.24 (1)		10.00
S- R	0 / 6539	-38.5	-38.5	0.19 (1)		10.00
R- Q	0 / 3173	-38.5	-38.5	0.09 (1)		10.00
Q- P	0 / 0	-38.5	-38.5	0.01 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AA- C	-1598 / 0	0.20 (1)
Y- E	-595 / 0	0.07 (1)
S- K	-2228 / 0	0.27 (1)
R- M	0 / 4440	0.39 (1)
Q- M	-892 / 0	0.11 (1)
B-AA	0 / 6337	0.78 (1)
Q- N	0 / 3317	0.41 (1)
T- K	0 / 3182	0.39 (1)
E- X	0 / 339	0.04 (1)
T- I	-1691 / 0	0.20 (1)
X- G	0 / 140	0.02 (3)
V- I	0 / 1953	0.24 (1)
G- V	-885 / 0	0.26 (1)
V- H	-475 / 0	0.06 (1)
Z- D	-1320 / 0	0.16 (1)
R- L	-2712 / 0	0.32 (1)
C- Z	0 / 7812	0.69 (1)
D- Y	0 / 945	0.12 (1)
S- L	0 / 2680	0.33 (1)

DWG NO. TAM 7101918
STRUCTURAL COMPONENT ONLY

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-63	-70	—	FRONT	VERT	DEAD	—	—
C	3-5-5	-130	-130	—	BACK	VERT	TOTAL	—	—
C	3-5-5	-360	-360	—	FRONT	VERT	SNOW	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	28.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 842 (0.56")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.38/1.00 (V-X:1) , WB=0.78/1.00 (B-AA:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (S) (INPUT = 0.90)
JSI METAL= 0.96 (W) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T5Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	5.0	8.0	2.50	3.00
R	BMWW+t	MT20	6.0	9.0	4.50	2.25
S, T, X, Y						
S	BMWW-t	MT20	5.0	8.0		
U	BS-t	MT20	6.0	9.0		
V	BMWWW-t	MT20	5.0	8.0		
W	BS-t	MT20	6.0	9.0		
Z	BMWW+t	MT20	6.0	9.0	4.50	2.25
AA	BMWW-t	MT20	5.0	8.0	2.50	3.00
AB	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	3-6-1	-55	-70	--	BACK	VERT	TOTAL	--	--
AC	5-4-14	-123	-123	--	BACK	VERT	TOTAL	--	--
AD	7-4-14	-123	-123	--	BACK	VERT	TOTAL	--	--
AE	1-8-4	-55	-70	--	BACK	VERT	TOTAL	--	--
AF	5-4-14	-55	-70	--	BACK	VERT	TOTAL	--	--
AG	7-4-14	-55	-70	--	BACK	VERT	TOTAL	--	--
AG	7-8-8	-3765	-3765	--	BACK	VERT	TOTAL	--	--

LICENSED PROFESSIONAL ENGINEER
01/16/19
H. J. G. ALVES
100088024
PROVINCE OF ONTARIO

DWG NO. TAM 7190191B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T6	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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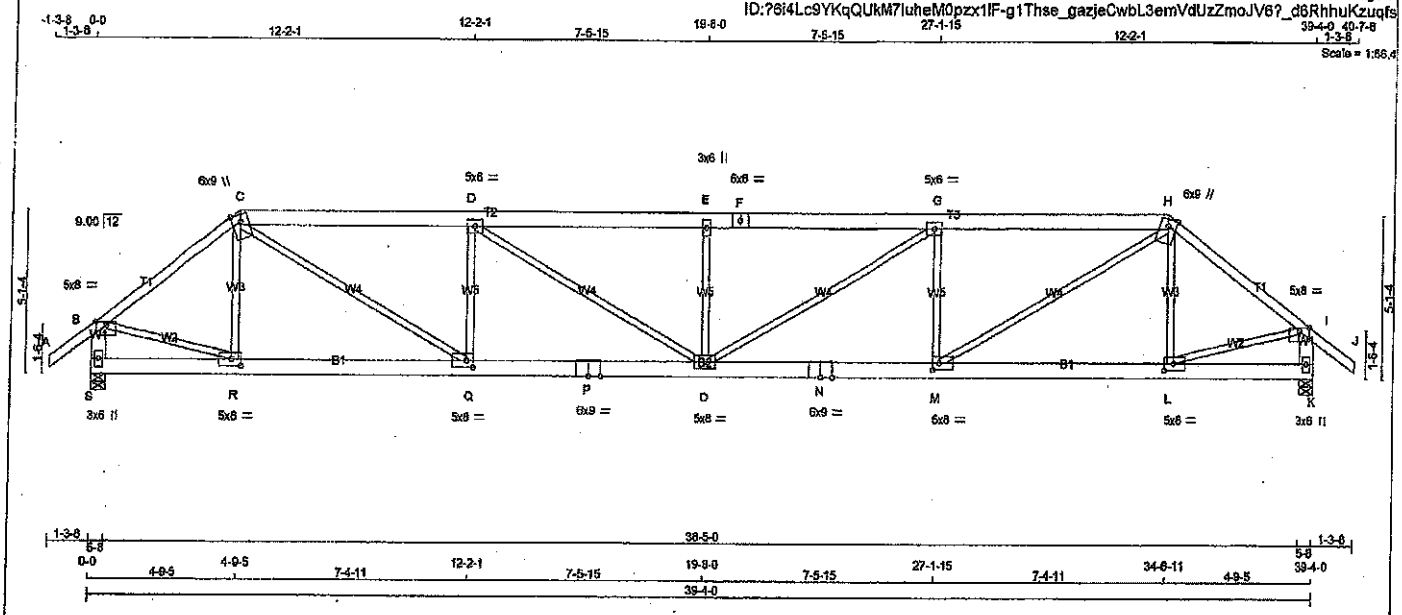


TOTAL WEIGHT = 2 X 197 = 394 lb [M]

LUMBER
N. L. G. A. RULES.

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	1.25	4.00
C	TTWW+m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMWW-p	MT20	5.0	8.0	1.25	4.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.50
M	BMWW-t	MT20	5.0	8.0	2.50	2.50
N	BS-t	MT20	6.0	9.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	2.50
R	BMWW-t	MT20	5.0	8.0	2.50	3.50
S	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2906	0	2906	0	0	5-8	5-8
K	2906	0	2906	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2163	1222 / 0	413 / 0	0 / 0	0 / 0	528 / 0	0 / 0
K	2163	1222 / 0	413 / 0	0 / 0	0 / 0	528 / 0	0 / 0

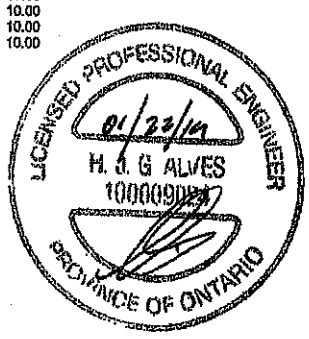
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	LC1 TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	R-C	-322 / 58	0.11 (1)	
B-C	-3129 / 0	-102.1	-102.1	0.67 (1)	3.36	L-H	-322 / 58	0.11 (1)	
C-D	-5139 / 0	-102.1	-102.1	0.59 (1)	3.40	B-R	0 / 2579	0.58 (1)	
D-E	-5949 / 0	-102.1	-102.1	0.67 (1)	3.12	L-I	0 / 2579	0.58 (1)	
E-F	-5949 / 0	-102.1	-102.1	0.67 (1)	3.12	M-H	0 / 3102	0.70 (1)	
F-G	-5949 / 0	-102.1	-102.1	0.67 (1)	3.12	C-Q	0 / 3102	0.70 (1)	
G-H	-5139 / 0	-102.1	-102.1	0.59 (1)	3.40	M-G	-1371 / 0	0.46 (1)	
H-I	-3129 / 0	-102.1	-102.1	0.67 (1)	3.36	Q-D	-1371 / 0	0.46 (1)	
I-J	0 / 42	-102.1	-102.1	0.14 (1)	10.00	C-G	0 / 952	0.21 (1)	
S-B	-2850 / 0	0.0	0.0	0.19 (1)	6.21	D-Q	0 / 952	0.21 (1)	
K-I	-2850 / 0	0.0	0.0	0.19 (1)	6.21	G-E	-720 / 0	0.24 (1)	
S-R	0 / 0	-38.5	-38.5	0.13 (3)	10.00				
R-Q	0 / 2516	-38.5	-38.5	0.40 (1)	10.00				
Q-P	0 / 5139	-38.5	-38.5	0.72 (1)	10.00				
P-O	0 / 5139	-38.5	-38.5	0.72 (1)	10.00				
O-N	0 / 5139	-38.5	-38.5	0.72 (1)	10.00				
N-M	0 / 5139	-38.5	-38.5	0.72 (1)	10.00				
M-L	0 / 2516	-38.5	-38.5	0.40 (1)	10.00				
L-K	0 / 0	-38.5	-38.5	0.13 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 660 (0.55")

CSI: TC=0.67/1.00 (E-G:1) , BC=0.72/1.00 (M-O:1) , WB=0.70/1.00 (C-Q:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.84 (P) (INPUT = 1.00)

DWG NO. TAM T190 19/9
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T7	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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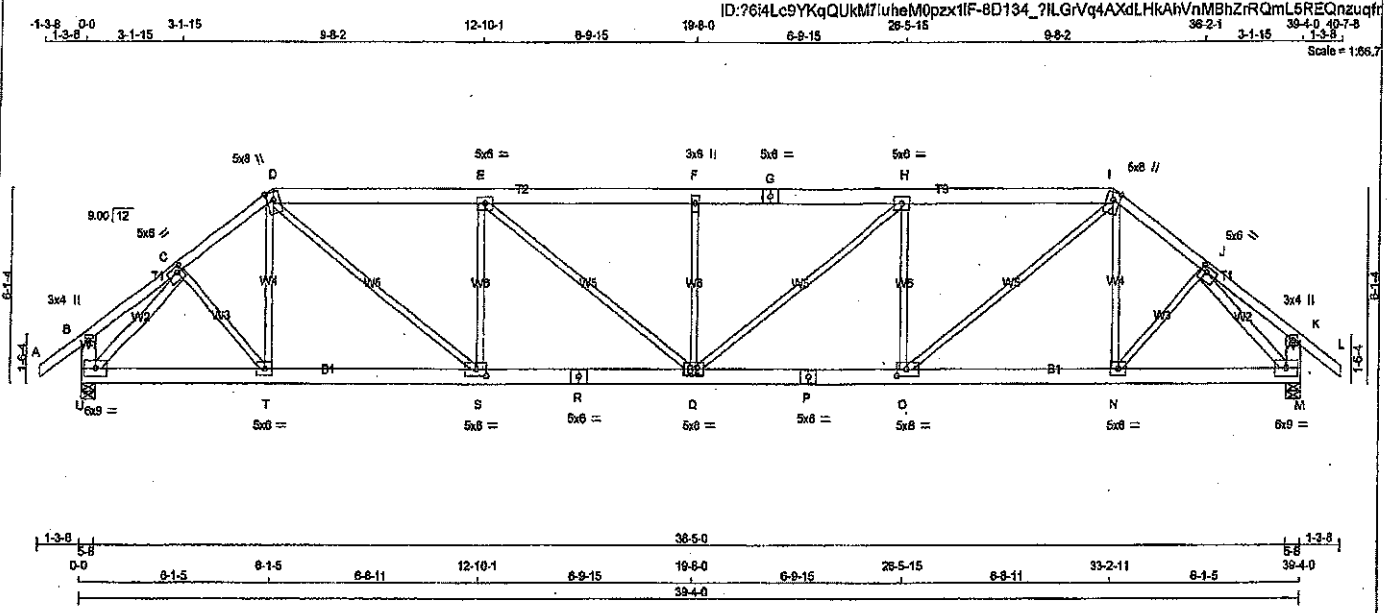


TOTAL WEIGHT = 2 X 208 = 416 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	2.00
D	TTWW+m	MT20	5.0	8.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW+m	MT20	5.0	8.0	Edge	
J	TMWW-t	MT20	5.0	6.0	2.00	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	6.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	8.0	2.50	3.75
P	BS-t	MT20	5.0	8.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	3.75
T	BMWW-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2906	0	2906	0	0	5-8	5-8
M	2906	0	2906	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
	MAX./MIN. COMPONENT REACTIONS						
U	2163	1222 / 0	413 / 0	0 / 0	0 / 0	528 / 0	0 / 0
M	2163	1222 / 0	413 / 0	0 / 0	0 / 0	528 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

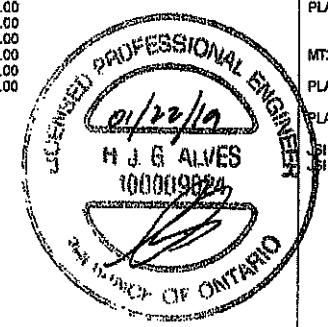
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	0 / 18	-102.1	-102.1	0.12 (1)	10.00
C-D	-3183 / 0	-102.1	-102.1	0.25 (1)	3.74
D-E	-4332 / 0	-102.1	-102.1	0.45 (1)	3.60
E-F	-4883 / 0	-102.1	-102.1	0.49 (1)	3.58
F-G	-4883 / 0	-102.1	-102.1	0.49 (1)	3.58
G-H	-4883 / 0	-102.1	-102.1	0.49 (1)	3.56
H-I	-4332 / 0	-102.1	-102.1	0.45 (1)	3.60
I-J	-3183 / 0	-102.1	-102.1	0.25 (1)	3.74
J-K	0 / 18	-102.1	-102.1	0.12 (1)	10.00
K-L	0 / 42	-102.1	-102.1	0.14 (1)	10.00
U-B	-266 / 0	0.0	0.0	0.02 (1)	7.81
M-K	-266 / 0	0.0	0.0	0.02 (1)	7.81
U-T	0 / 2284	-38.5	-38.5	0.37 (1)	10.00
T-S	0 / 2542	-38.5	-38.5	0.40 (1)	10.00
S-R	0 / 4331	-38.5	-38.5	0.60 (1)	10.00
R-Q	0 / 4331	-38.5	-38.5	0.60 (1)	10.00
Q-P	0 / 4331	-38.5	-38.5	0.60 (1)	10.00
P-O	0 / 4331	-38.5	-38.5	0.60 (1)	10.00
O-N	0 / 2542	-38.5	-38.5	0.40 (1)	10.00
N-M	0 / 2284	-38.5	-38.5	0.37 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-T	0 / 390	0.09 (1)
T-D	-13 / 154	0.04 (3)
N-I	-13 / 154	0.04 (3)
N-J	0 / 390	0.09 (1)
U-C	-3423 / 0	0.75 (1)
J-M	-3423 / 0	0.75 (1)
O-I	0 / 2340	0.53 (1)
D-S	0 / 2340	0.53 (1)
O-H	-1249 / 0	0.64 (1)
S-E	-1249 / 0	0.64 (1)
Q-H	0 / 716	0.16 (1)
E-Q	0 / 716	0.16 (1)
Q-F	-656 / 0	0.33 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.49/1.00 (E-F:1) , BC=0.60/1.00 (O-Q:1) , WB=0.75/1.00 (J-M:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (S) (INPUT = 0.90)
SI METAL= 0.85 (P) (INPUT = 1.00)

DWG NO. TAM T190192W
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T8	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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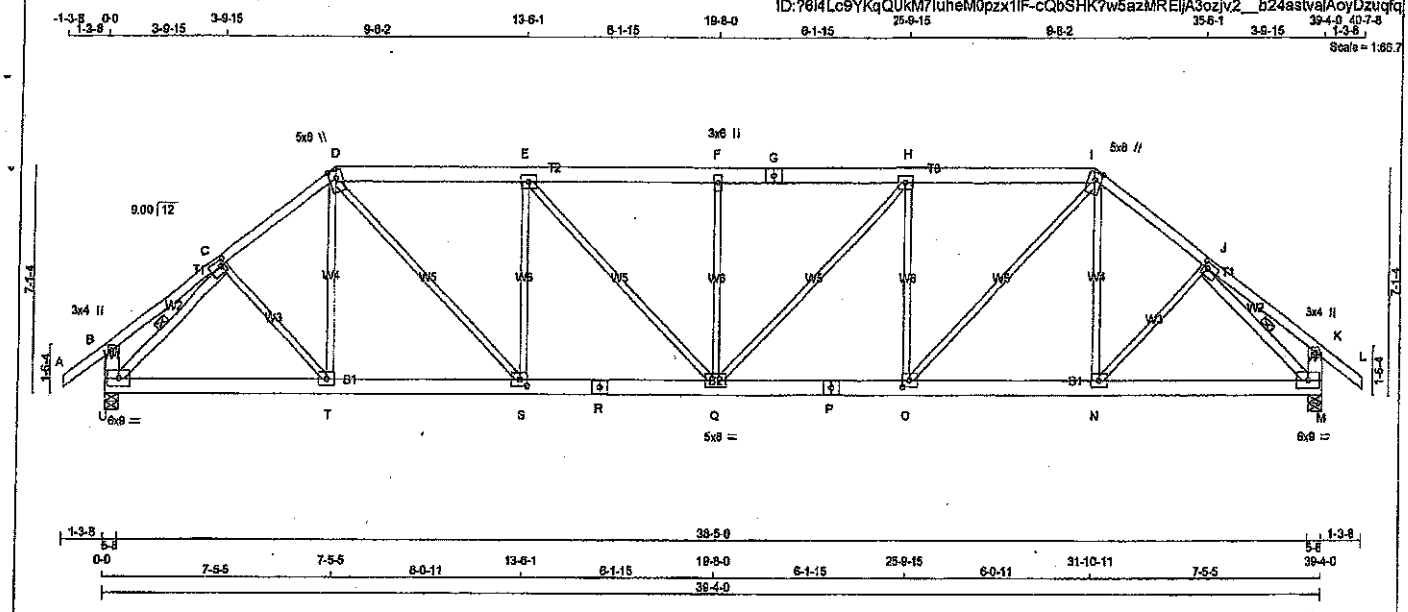


TOTAL WEIGHT = 2 X 216 = 432 lb

(M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.75
D	TTWW+m	MT20	5.0	8.0	Edge	
E	TMWW-t	MT20	5.0	8.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	8.0		
I	TTWW+m	MT20	5.0	8.0	Edge	
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	6.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMWW-t	MT20	5.0	6.0		
U	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2906	0	2906	0	0	5-8	5-8
M	2906	0	2906	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2163	1222 / 0	413 / 0	0 / 0	0 / 0	528 / 0	0 / 0
M	2163	1222 / 0	413 / 0	0 / 0	0 / 0	528 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	0 / 24	-102.1	-102.1	0.21 (1)	10.00	
C-D	-3176 / 0	-102.1	-102.1	0.37 (1)	3.63	
D-E	-3755 / 0	-102.1	-102.1	0.34 (1)	4.15	
E-F	-4137 / 0	-102.1	-102.1	0.36 (1)	3.97	
F-G	-4137 / 0	-102.1	-102.1	0.36 (1)	3.97	
G-H	-4137 / 0	-102.1	-102.1	0.36 (1)	3.97	
H-I	-3755 / 0	-102.1	-102.1	0.34 (1)	4.15	
I-J	-3176 / 0	-102.1	-102.1	0.37 (1)	3.63	
J-K	0 / 24	-102.1	-102.1	0.21 (1)	10.00	
K-L	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
U-B	-289 / 0	0.0	0.0	0.02 (1)	7.81	
M-K	-289 / 0	0.0	0.0	0.02 (1)	7.81	
U-T	0 / 2381	-38.5	-38.5	0.40 (1)	10.00	
T-S	0 / 2527	-38.5	-38.5	0.42 (1)	10.00	
S-R	0 / 3755	-38.5	-38.5	0.52 (1)	10.00	
R-Q	0 / 3755	-38.5	-38.5	0.52 (1)	10.00	
Q-P	0 / 3755	-38.5	-38.5	0.52 (1)	10.00	
P-O	0 / 3755	-38.5	-38.5	0.52 (1)	10.00	
O-N	0 / 2527	-38.5	-38.5	0.42 (1)	10.00	
N-M	0 / 2381	-38.5	-38.5	0.40 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-T	0 / 241	0.05 (2)
T-D	0 / 234	0.06 (3)
N-I	0 / 234	0.06 (3)
N-J	0 / 241	0.05 (2)
U-C	-3452 / 0	0.52 (1)
J-M	-3452 / 0	0.52 (1)
O-I	0 / 1827	0.41 (1)
O-S	0 / 1827	0.41 (1)
O-H	-1128 / 0	0.87 (1)
S-E	-1128 / 0	0.87 (1)
Q-H	0 / 562	0.13 (1)
E-Q	0 / 562	0.13 (1)
Q-F	-592 / 0	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.37/1.00 (I-J:1) , BC=0.52/1.00 (Q-S:1) , WB=0.87/1.00 (H-O:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

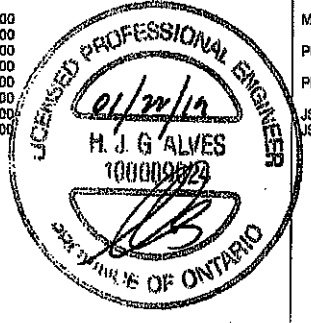
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.86 (J) (INPUT = 1.00)



DWG NO. TAM 5901921
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T9	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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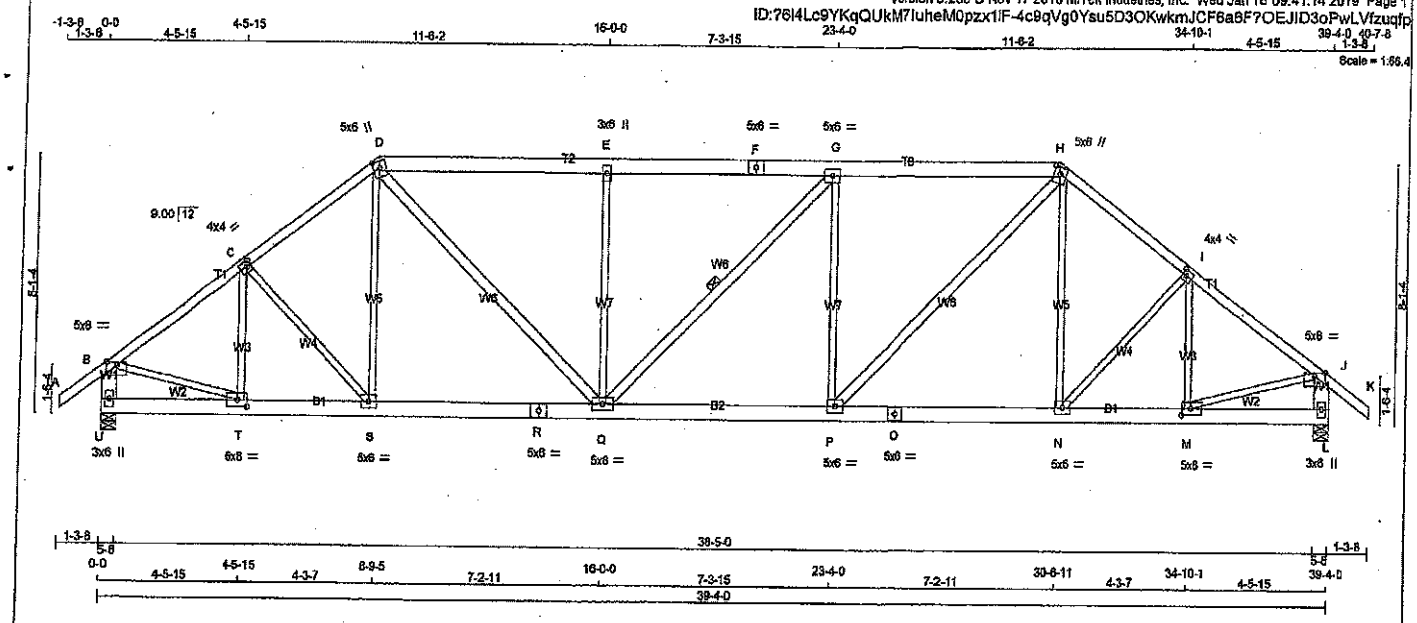


TOTAL WEIGHT = 2 X 223 = 447 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	4.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.50	2.50
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	6.0	2.50	2.50
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.25	4.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	8.0	2.50	3.50
N, P, S						
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	8.0	2.50	3.50
U	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2906	0	2906	0	0	5-8	5-8
L	2906	0	2906	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2163	1222 / 0	413 / 0	0 / 0	0 / 0	528 / 0	0 / 0
L	2163	1222 / 0	413 / 0	0 / 0	0 / 0	528 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

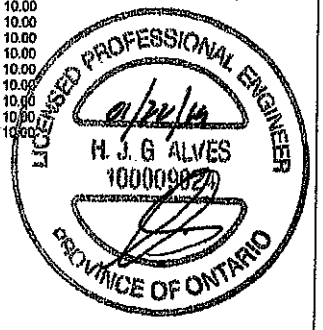
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)
A-B	0 / 42	-102.1	-102.1	0.14 (1)
B-C	-3095 / 0	-102.1	-102.1	0.53 (1)
C-D	-3121 / 0	-102.1	-102.1	0.51 (1)
D-E	-3515 / 0	-102.1	-102.1	0.44 (1)
E-F	-3515 / 0	-102.1	-102.1	0.46 (1)
F-G	-3515 / 0	-102.1	-102.1	0.46 (1)
G-H	-3520 / 0	-102.1	-102.1	0.45 (1)
H-I	-3120 / 0	-102.1	-102.1	0.51 (1)
I-J	-3095 / 0	-102.1	-102.1	0.53 (1)
J-K	0 / 42	-102.1	-102.1	0.14 (1)
U-B	-2812 / 0	0.0	0.0	0.19 (1)
L-J	-2812 / 0	0.0	0.0	0.19 (1)
U-T	0 / 0	-38.5	-38.5	0.08 (2)
T-S	0 / 2500	-38.5	-38.5	0.39 (1)
S-R	0 / 2474	-38.5	-38.5	0.39 (2)
R-Q	0 / 2474	-38.5	-38.5	0.39 (2)
Q-P	0 / 3520	-38.5	-38.5	0.52 (1)
P-O	0 / 2474	-38.5	-38.5	0.40 (1)
O-N	0 / 2474	-38.5	-38.5	0.40 (1)
N-M	0 / 2500	-38.5	-38.5	0.38 (1)
M-L	0 / 0	-38.5	-38.5	0.08 (2)

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX UNBRAC LENGTH	MAX CSI (LC)
T-C	-498 / 0	10.00	0.16 (1)
C-S	-31 / 15	3.53	0.02 (1)
S-D	0 / 329	3.52	0.07 (2)
N-H	0 / 326	4.15	0.07 (2)
N-I	-33 / 15	4.13	0.02 (1)
M-I	-496 / 0	4.13	0.16 (1)
B-T	0 / 2575	10.00	0.58 (1)
M-J	0 / 2575	10.00	0.58 (1)
P-H	0 / 1526	4.13	0.25 (1)
D-Q	0 / 1518	4.13	0.24 (1)
P-G	-822 / 0	6.24	0.92 (1)
Q-E	-820 / 0	6.24	0.92 (1)
Q-G	-7 / 0	10.00	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.53/1.00 (I-J:1) , BC=0.52/1.00 (P-Q:1) , WB=0.92/1.00 (G-P:1) , SSI=0.26/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (M) (INPUT = 0.90)
JSI METAL= 0.65 (J) (INPUT = 1.00)

DWG NO. TAM 190/928
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T11	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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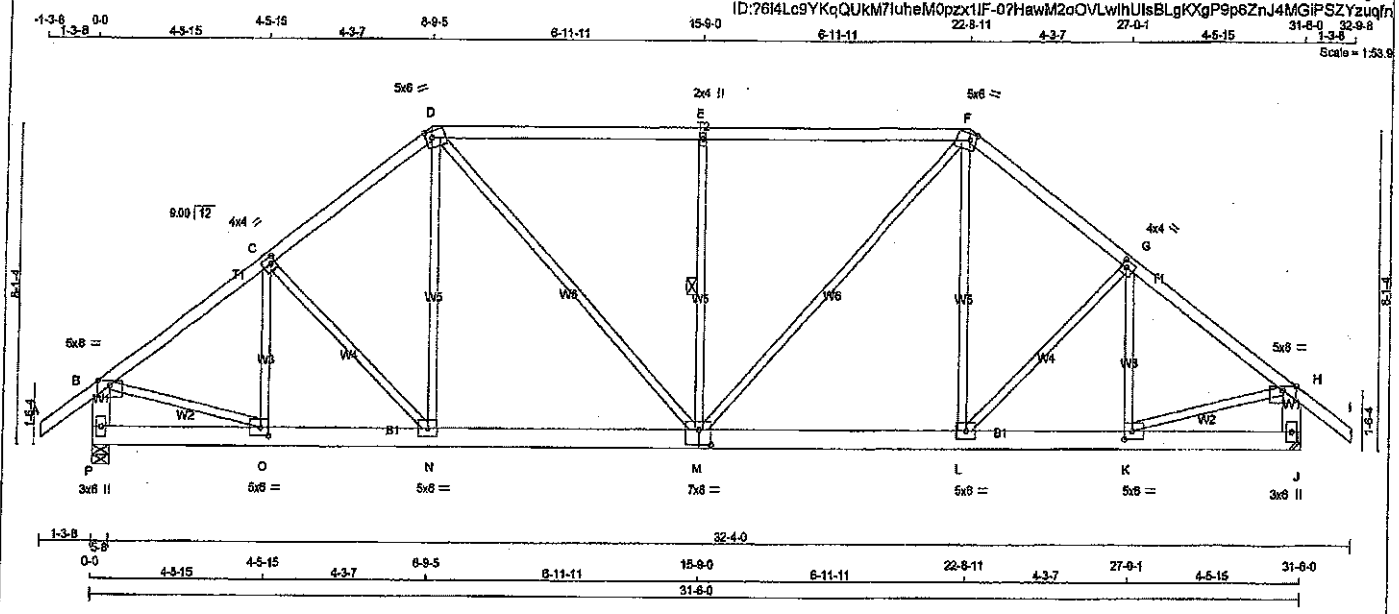


TOTAL WEIGHT = 162 lb (M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	5.0	6.0		
M	BSWWW-t	MT20	7.0	8.0	4.50	4.00
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2355	0	2355	0	0	5-8	5-8
J	2355	0	2355	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1751	995 / 0	331 / 0	0 / 0	0 / 0	426 / 0	0 / 0
J	1751	995 / 0	331 / 0	0 / 0	0 / 0	426 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

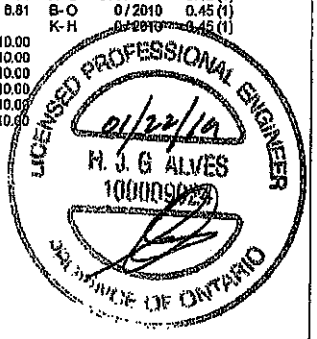
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2408 / 0	-102.1	-102.1	0.33 (1)	4.15	
C-D	-2332 / 0	-102.1	-102.1	0.32 (1)	4.21	
D-E	-2334 / 0	-102.1	-102.1	0.75 (1)	3.53	
E-F	-2334 / 0	-102.1	-102.1	0.75 (1)	3.53	
F-G	-2332 / 0	-102.1	-102.1	0.32 (1)	4.21	
G-H	-2408 / 0	-102.1	-102.1	0.33 (1)	4.15	
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
P-B	-2264 / 0	0.0	0.0	0.15 (1)	8.81	
J-H	-2264 / 0	0.0	0.0	0.15 (1)	8.81	
P-O	0 / 0	-36.5	-36.5	0.08 (2)	10.00	
O-N	0 / 1951	-36.5	-36.5	0.34 (1)	10.00	
N-M	0 / 1841	-36.5	-36.5	0.33 (2)	10.00	
M-L	0 / 1841	-36.5	-36.5	0.33 (2)	10.00	
L-K	0 / 1951	-36.5	-36.5	0.34 (1)	10.00	
K-J	0 / 0	-36.5	-36.5	0.08 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-356 / 0	0.12 (1)
C-N	-164 / 0	0.11 (1)
N-D	0 / 441	0.10 (2)
D-M	0 / 735	0.17 (1)
M-E	-873 / 0	0.35 (1)
M-F	0 / 735	0.17 (1)
L-F	0 / 441	0.10 (2)
L-G	-164 / 0	0.11 (1)
K-G	-356 / 0	0.12 (1)
B-O	0 / 2010	0.45 (1)
K-H	0 / 2010	0.45 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.75/1.00 (E-F:1) , BC=0.34/1.00 (N-O:1) , WB=0.45/1.00 (B-O:1) , SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1687	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.49 (F) (INPUT = 1.00)

DWG NO. TAM 1901924
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T12	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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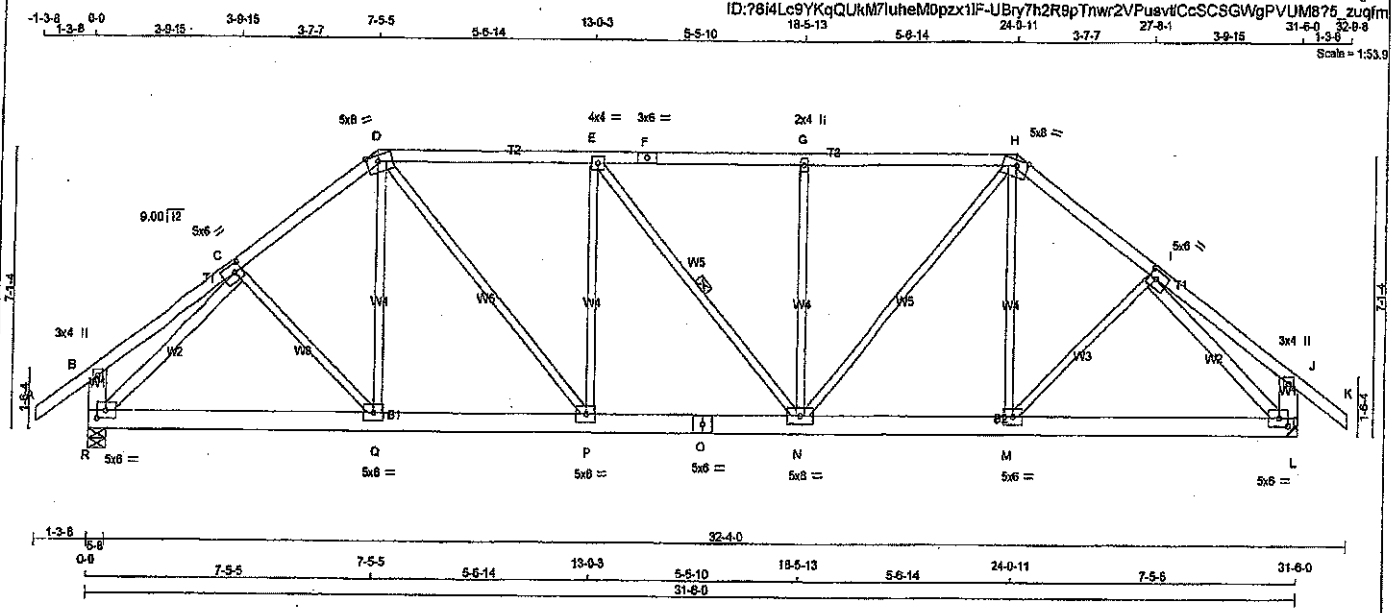

TOTAL WEIGHT = 165 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.50
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMWW1-t	MT20	5.0	8.0	2.50	2.75
M, P, Q						
M	BMWW-t	MT20	5.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
R	BMWW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2355	0	2355	0	0	5-8	5-8
L	2355	0	2355	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1751	995 / 0	331 / 0	0 / 0	0 / 0	426 / 0	0 / 0
L	1751	995 / 0	331 / 0	0 / 0	0 / 0	426 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
A- B	0 / 42	-102.1	-102.1	1	0.14	(1)	10.00	C- Q	0 / 138	0.03	(3)
B- C	0 / 25	-102.1	-102.1	1	0.21	(1)	10.00	Q- D	0 / 301	0.07	(2)
C- D	-2406 / 0	-102.1	-102.1	1	0.31	(1)	4.15	D- P	0 / 1038	0.23	(1)
D- E	-2566 / 0	-102.1	-102.1	1	0.59	(1)	3.71	P- E	-604 / 0	0.49	(1)
E- F	-2561 / 0	-102.1	-102.1	1	0.59	(1)	3.71	E- N	-9 / 0	0.00	(1)
F- G	-2561 / 0	-102.1	-102.1	1	0.59	(1)	3.71	N- G	-607 / 0	0.50	(1)
G- H	-2561 / 0	-102.1	-102.1	1	0.58	(1)	3.73	N- H	0 / 1026	0.23	(1)
H- I	-2407 / 0	-102.1	-102.1	1	0.31	(1)	4.15	M- H	0 / 306	0.07	(2)
I- J	0 / 25	-102.1	-102.1	1	0.21	(1)	10.00	M- I	0 / 136	0.03	(3)
J- K	0 / 42	-102.1	-102.1	1	0.14	(1)	10.00	R- C	-2667 / 0	0.83	(1)
R- B	-288 / 0	0.0	0.0	1	0.02	(1)	7.81	I- L	-2888 / 0	0.83	(1)
L- J	-288 / 0	0.0	0.0	1	0.02	(1)	7.81				
R- Q	0 / 1853	-38.5	-38.5	2	0.35	(2)	10.00				
Q- P	0 / 1905	-38.5	-38.5	2	0.35	(2)	10.00				
P- O	0 / 2567	-38.5	-38.5	1	0.37	(1)	10.00				
O- N	0 / 2567	-38.5	-38.5	1	0.37	(1)	10.00				
N- M	0 / 1905	-38.5	-38.5	2	0.36	(2)	10.00				
M- L	0 / 1854	-38.5	-38.5	2	0.35	(2)	10.00				

LICENSED PROFESSIONAL ENGINEER
01/22/19
H. J. G. ALVES
100009924
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.59/1.00 (D-E:1) , BC=0.37/1.00 (N-P:1) , WB=0.83/1.00 (I-L:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (L) (INPUT = 0.90)
SI METAL= 0.88 (I) (INPUT = 1.00)

DWG NO. TAM T1901925
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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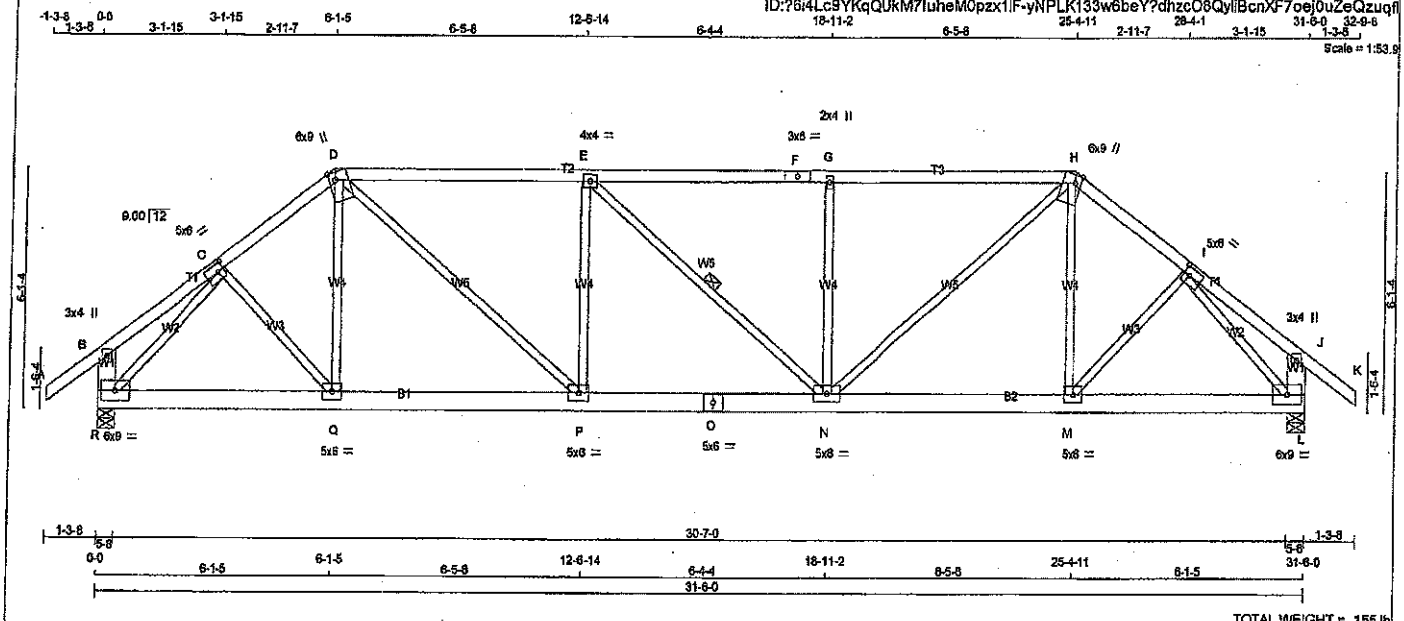

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	6.0	9.0		
M, P, Q						
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	8.0		
R	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2355	0	2355	0	0	5-8	5-8
L	2355	0	2355	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1751	995 / 0	331 / 0	0 / 0	0 / 0	426 / 0	0 / 0
L	1751	995 / 0	331 / 0	0 / 0	0 / 0	426 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)		10.00
B-C	0 / 17	-102.1	-102.1	0.12 (1)		10.00
C-D	-2442 / 0	-102.1	-102.1	0.20 (1)		4.24
D-E	-3006 / 0	-102.1	-102.1	0.91 (1)		3.10
E-F	-3001 / 0	-102.1	-102.1	0.90 (1)		3.11
F-G	-3001 / 0	-102.1	-102.1	0.90 (1)		3.11
G-H	-3000 / 0	-102.1	-102.1	0.90 (1)		3.13
H-I	-2444 / 0	-102.1	-102.1	0.20 (1)		4.24
I-J	0 / 17	-102.1	-102.1	0.12 (1)		10.00
J-K	0 / 42	-102.1	-102.1	0.14 (1)		10.00
R-B	-267 / 0	0.0	0.0	0.02 (1)		7.81
L-J	-267 / 0	0.0	0.0	0.02 (1)		7.81
R-Q	0 / 1785	-38.5	-38.5	0.30 (1)		10.00
Q-P	0 / 1937	-38.5	-38.5	0.32 (2)		10.00
P-O	0 / 3007	-38.5	-38.5	0.43 (1)		10.00
O-N	0 / 3007	-38.5	-38.5	0.43 (1)		10.00
N-M	0 / 1938	-38.5	-38.5	0.33 (2)		10.00
M-L	0 / 1786	-38.5	-38.5	0.31 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
C-Q	0 / 232	0.05 (2)
Q-D	0 / 212	0.05 (3)
D-P	0 / 1428	0.32 (1)
P-E	-705 / 0	0.38 (1)
E-N	-8 / 0	0.00 (1)
N-G	-707 / 0	0.39 (1)
N-H	0 / 1419	0.32 (1)
M-H	0 / 213	0.05 (3)
M-I	0 / 232	0.05 (2)
R-C	-2674 / 0	0.82 (1)
I-L	-2676 / 0	0.82 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.91/1.00 (D-E:1) , BC=0.43/1.00 (N-P:1) , WB=0.82/1.00 (I-L:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.66 (I) (INPUT = 1.00)

DWG NO. TAM 1901926
STRUCTURAL COMPONENT ONLY

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO B

RECEIVED
AUG 08 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
01/22/19
H. J. G. ALVES
100000024
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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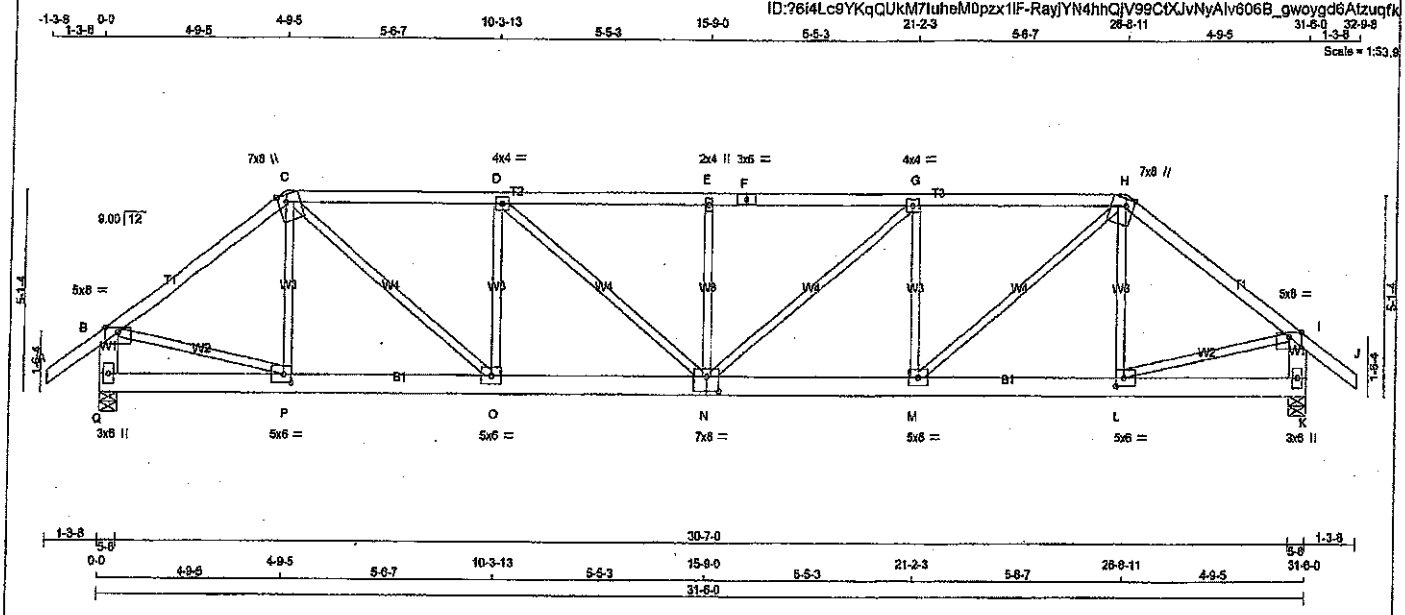


TOTAL WEIGHT = 150 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.50
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BSWWW-I	MT20	7.0	8.0	4.50	4.00
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2355	0	2355	0	0	5-8	5-8
K	2355	0	2355	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1751	995 / 0	331 / 0	0 / 0	0 / 0	426 / 0	0 / 0
K	1751	995 / 0	331 / 0	0 / 0	0 / 0	426 / 0	0 / 0

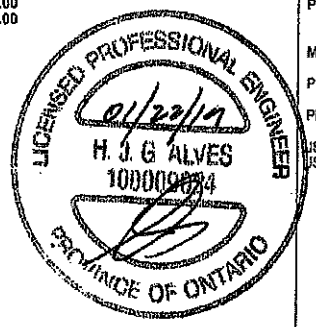
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	P-C	-213 / 74	0.08 (1)
B-C	-2420 / 0	-102.1	-102.1	0.57 (1)	3.85	L-H	-213 / 74	0.08 (1)
C-D	-3325 / 0	-102.1	-102.1	0.72 (1)	3.19	B-P	0 / 1982	0.45 (1)
D-E	-3728 / 0	-102.1	-102.1	0.77 (1)	2.96	L-I	0 / 1982	0.45 (1)
E-F	-3728 / 0	-102.1	-102.1	0.77 (1)	2.96	M-H	0 / 1835	0.41 (1)
F-G	-3728 / 0	-102.1	-102.1	0.77 (1)	2.96	C-O	0 / 1835	0.41 (1)
G-H	-3325 / 0	-102.1	-102.1	0.72 (1)	3.19	M-G	-979 / 0	0.35 (1)
H-I	-2420 / 0	-102.1	-102.1	0.57 (1)	3.85	O-D	-979 / 0	0.35 (1)
I-J	0 / 42	-102.1	-102.1	0.14 (1)	10.00	N-G	0 / 534	0.12 (1)
Q-B	-2280 / 0	0.0	0.0	0.15 (1)	6.79	D-N	0 / 534	0.12 (1)
K-I	-2280 / 0	0.0	0.0	0.15 (1)	6.79	N-E	-510 / 0	0.18 (1)
Q-P	0 / 0	-38.5	-38.5	0.08 (3)	10.00			
P-O	0 / 1928	-38.5	-38.5	0.29 (1)	10.00			
O-N	0 / 3325	-38.5	-38.5	0.47 (1)	10.00			
N-M	0 / 3325	-38.5	-38.5	0.47 (1)	10.00			
M-L	0 / 1928	-38.5	-38.5	0.29 (1)	10.00			
L-K	0 / 0	-38.5	-38.5	0.08 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.77/1.00 (D-E:1) , BC=0.47/1.00 (N-O:1) , WB=0.45/1.00 (B-P:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.89 (O) (INPUT = 0.90)
(SI METAL= 0.52 (N) (INPUT = 1.00)

DWG NO. TAM T1901927
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400669	T15Z	1	2	GREEN PARK HOMES TRUSS DESC.	

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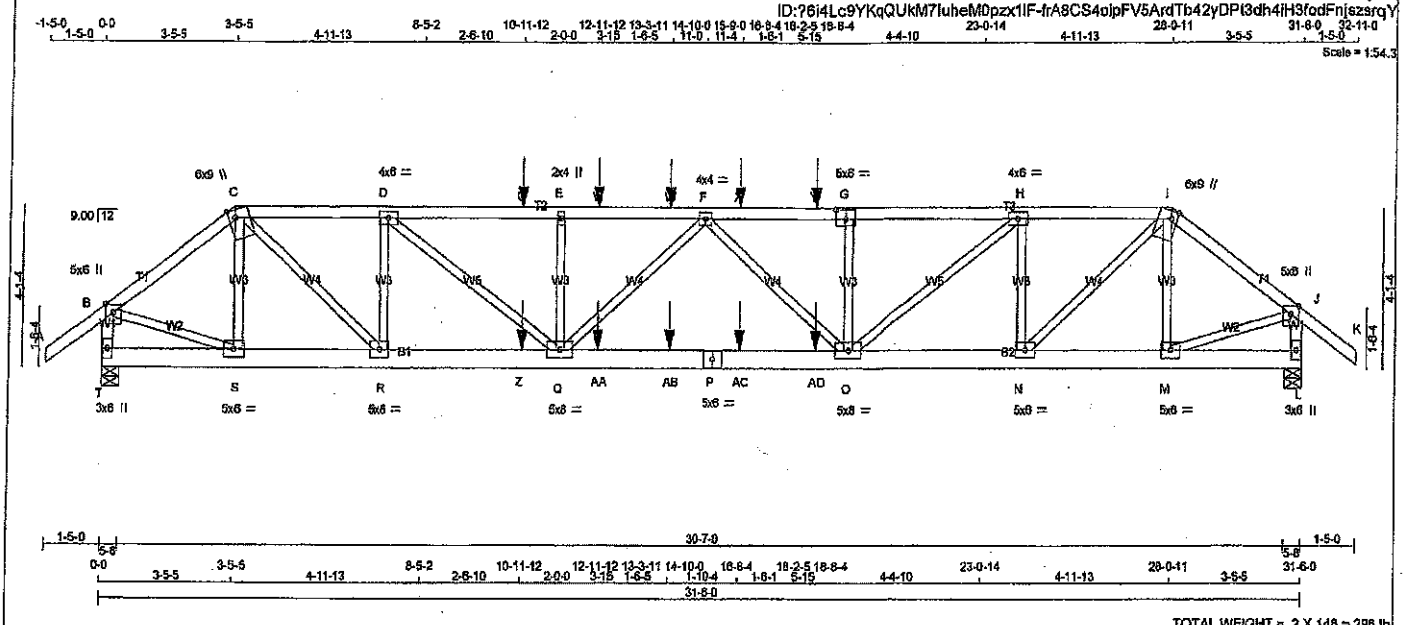

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-G	1	12	SIDE(0.0)
G-I	1	12	TOP
I-K	1	12	TOP
T-B	1	12	TOP
L-J	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T-P	2	12	SIDE(0.0)
P-L	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TSW-I	MT20	5.0	6.0	3.00	3.00
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMVW+p	MT20	5.0	6.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M, N, R, S						
M	BMWW-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2838	0	2838	0	0	5-8	5-8
L	2787	0	2787	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2116	1187 / 0	410 / 0	0 / 0	0 / 0	519 / 0	0 / 0
L	2076	1166 / 0	401 / 0	0 / 0	0 / 0	509 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 46	-102.1	-102.1	0.09 (1)	10.00	S-C	-530 / 0	0.07 (1)
B-C	-2867 / 0	-102.1	-102.1	0.15 (1)	5.27	C-R	0 / 2812	0.35 (1)
C-D	-4343 / 0	-102.1	-102.1	0.23 (1)	4.39	R-D	-1613 / 0	0.23 (1)
D-U	-5959 / 0	-102.1	-102.1	0.32 (1)	3.77	D-Q	0 / 2086	0.26 (1)
U-E	-5959 / 0	-102.1	-102.1	0.32 (1)	3.77	Q-E	-625 / 0	0.08 (1)
E-V	-5959 / 0	-102.1	-102.1	0.33 (1)	3.74	Q-G	-447 / 0	0.06 (1)
V-W	-5959 / 0	-102.1	-102.1	0.33 (1)	3.74	Q-H	0 / 1992	0.25 (1)
W-F	-5959 / 0	-102.1	-102.1	0.33 (1)	3.74	N-H	-1780 / 0	0.23 (1)
F-X	-5797 / 0	-102.1	-102.1	0.32 (1)	3.79	N-I	0 / 2739	0.34 (1)
X-Y	-5797 / 0	-102.1	-102.1	0.32 (1)	3.79	M-I	-511 / 0	0.06 (1)
Y-G	-5797 / 0	-102.1	-102.1	0.32 (1)	3.79	B-S	0 / 2414	0.30 (1)
G-H	-5786 / 0	-102.1	-102.1	0.33 (1)	3.81	M-J	0 / 2364	0.29 (1)
H-I	-4243 / 0	-102.1	-102.1	0.24 (1)	4.42	Q-F	-382 / 0	0.09 (1)
I-J	-2827 / 0	-102.1	-102.1	0.15 (1)	5.31	F-O	-609 / 0	0.16 (1)
J-K	0 / 46	-102.1	-102.1	0.09 (1)	10.00			
T-B	-2789 / 0	0.0	0.0	0.16 (1)	6.65			
L-J	-2738 / 0	0.0	0.0	0.15 (1)	6.80			
T-S	0 / 0	-38.5	-38.5	0.03 (2)	10.00			
S-R	0 / 2289	-38.5	-38.5	0.17 (1)	10.00			
R-Z	0 / 4343	-38.5	-38.5	0.33 (1)	10.00			
Z-Q	0 / 4343	-38.5	-38.5	0.33 (1)	10.00			
Q-AA	0 / 6230	-38.5	-38.5	0.53 (1)	10.00			
AA-AB	0 / 6230	-38.5	-38.5	0.53 (1)	10.00			
AB-P	0 / 6230	-38.5	-38.5	0.53 (1)	10.00			
P-AC	0 / 6230	-38.5	-38.5	0.53 (1)	10.00			
AC-AD	0 / 6230	-38.5	-38.5	0.53 (1)	10.00			
AD-O	0 / 6230	-38.5	-38.5	0.53 (1)	10.00			
O-N	0 / 4243	-38.5	-38.5	0.33 (1)	10.00			
N-M	0 / 2242	-38.5	-38.5	0.17 (1)	10.00			
M-L	0 / 0	-38.5	-38.5	0.03 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	10-11-12	-123	-123	—	BACK	VERT	TOTAL	—	—
V	12-11-12	-123	-123	—	BACK	VERT	TOTAL	—	—
W	14-10-0	-123	-123	—	BACK	VERT	TOTAL	—	—
X	16-8-4	-123	-123	—	BACK	VERT	TOTAL	—	—
Y	18-8-4	-123	-123	—	BACK	VERT	TOTAL	—	—
Z	10-11-12	-55	-70	—	BACK	VERT	TOTAL	—	—

TOTAL WEIGHT = 2 X 148 = 296 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.05")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.29")

CSI: TC=0.33/1.00 (G-H:1) , BC=0.53/1.00 (C-Q:1) , WB=0.35/1.00 (C-R:1) , SSI=0.16/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.82 (J) (INPUT = 0.90)
SSI METAL= 0.69 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H. S. G. ALVES
PROVINCE OF ONTARIO

DWG NO. TAM T1901947
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400669	T15Z	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	12-11-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AB	14-10-0	-55	-70	---	BACK	VERT	TOTAL	---	---
AC	16-8-4	-55	-70	---	BACK	VERT	TOTAL	---	---
AD	16-8-4	-55	-70	---	BACK	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
01/22/19
H. O. G. ALVES
100009[illegible]
PROVINCE OF ONTARIO

DWG NO. TAM 1790/1947
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T18	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Jan 16 09:41:24 2019 Page 1
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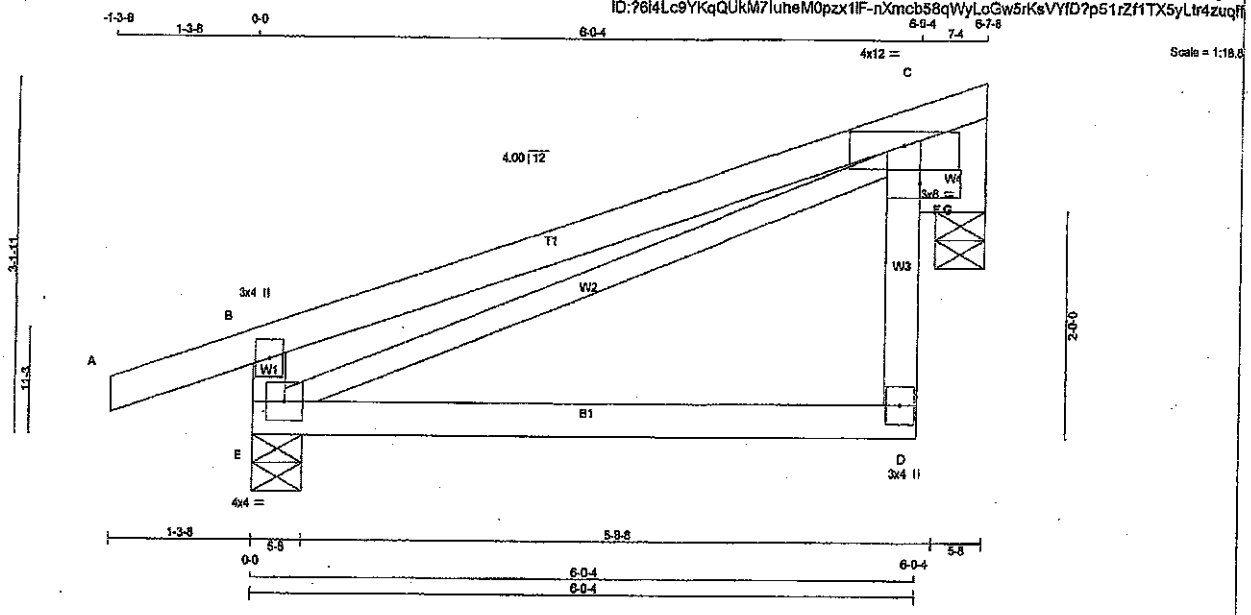


Scale = 1:18.6

TOTAL WEIGHT = 9 X 26 = 238 lb

(M)IF

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-p	MT20	4.0	12.0	1.50	6.00
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	KP-p	MT20	3.0	8.0	1.50	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	585	0	585	0	0	5-8	5-8
G	398	0	398	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	429	253 / 0	67 / 0	0 / 0	0 / 0	99 / 0	0 / 0
G	298	165 / 0	60 / 0	0 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 22	-102.1	-102.1	0.13 (1)	10.00	E-C	-62 / 0	0.04 (1)
B-C	0 / 0	-102.1	-102.1	0.63 (1)	10.00	C-G	-441 / 0	0.00 (1)
D-F	0 / 148	0.0	0.0	0.16 (1)	10.00	F-G	0 / 180	0.00 (1)
F-C	0 / 148	0.0	0.0	0.16 (1)	10.00			
E-B	-444 / 0	0.0	0.0	0.04 (1)	7.81			
E-D	0 / 57	-38.5	-38.5	0.33 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 847 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 508 (0.14")

CSI: TC=0.63/1.00 (B-C:1) , BC=0.33/1.00 (D-E:3) , WB=0.04/1.00 (C-E:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

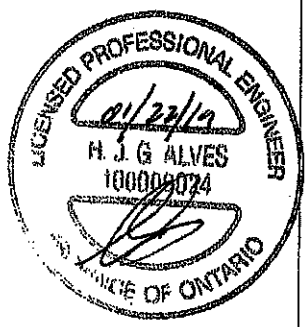
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	766	1687	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DWG NO. TAM T190 1931
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T19	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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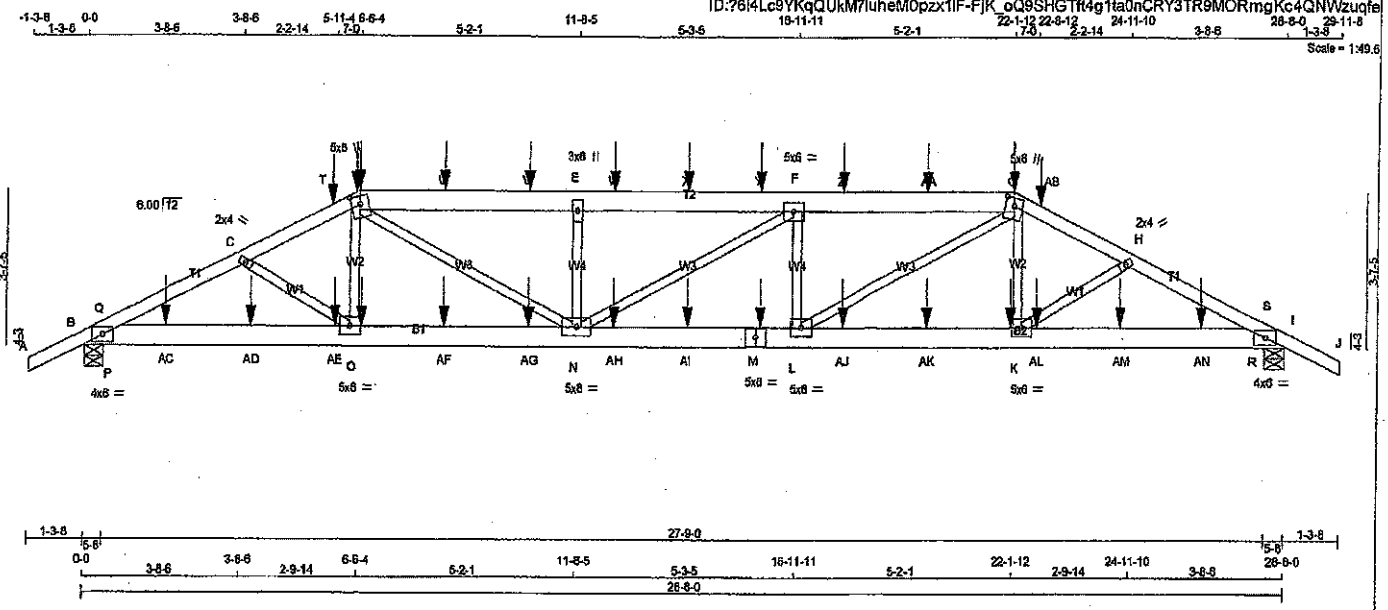


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
B - M	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- D 1	12	SIDE(61.0)
G- J 1	12	SIDE(61.0)
D- G 2	12	SIDE(61.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
B- M 2	12	SIDE(183.1)
M- I 2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	2.50
E	TMW+w	MT20	3.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.25	2.50
H	TMW+w	MT20	2.0	4.0		
I	TMB1-I	MT20	4.0	6.0		
K, L, O						
K	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2985	0	2985	0	0	5-8	5-8
I	2985	0	2985	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2194	1283 / 0	390 / 0	0 / 0	0 / 0	521 / 0	0 / 0
I	2193	1283 / 0	389 / 0	0 / 0	0 / 0	521 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

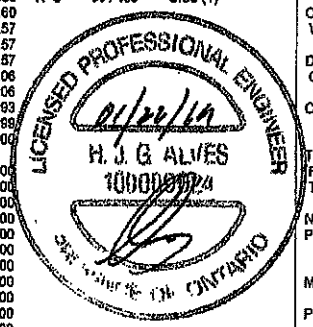
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 31	-102.1	-102.1	0.07 (1)	10.00
B- Q	-5734 / 0	-102.1	-102.1	0.13 (1)	3.99
Q- C	-5891 / 0	-102.1	-102.1	0.21 (1)	3.93
C- T	-5389 / 0	-102.1	-102.1	0.17 (1)	4.06
T- D	-5389 / 0	-102.1	-102.1	0.17 (1)	4.06
D- U	-6405 / 0	-102.1	-102.1	0.19 (1)	4.57
U- V	-6405 / 0	-102.1	-102.1	0.19 (1)	4.57
V- E	-6405 / 0	-102.1	-102.1	0.19 (1)	4.57
E- W	-6405 / 0	-102.1	-102.1	0.15 (1)	4.60
W- X	-6405 / 0	-102.1	-102.1	0.15 (1)	4.60
X- Y	-6405 / 0	-102.1	-102.1	0.15 (1)	4.60
Y- F	-6405 / 0	-102.1	-102.1	0.15 (1)	4.60
F- Z	-6413 / 0	-102.1	-102.1	0.18 (1)	4.57
Z- AA	-6413 / 0	-102.1	-102.1	0.18 (1)	4.57
AA- G	-6413 / 0	-102.1	-102.1	0.18 (1)	4.57
G- AB	-5388 / 0	-102.1	-102.1	0.17 (1)	4.06
AB- H	-5388 / 0	-102.1	-102.1	0.17 (1)	4.06
H- S	-5689 / 0	-102.1	-102.1	0.21 (1)	3.93
S- I	-5733 / 0	-102.1	-102.1	0.13 (1)	3.99
I- J	0 / 31	-102.1	-102.1	0.07 (1)	10.00
B- P	0 / 5080	-38.5	-38.5	0.45 (1)	10.00
P- AC	0 / 5102	-38.5	-38.5	0.47 (1)	10.00
AC- AD	0 / 5102	-38.5	-38.5	0.47 (1)	10.00
AD- AE	0 / 5102	-38.5	-38.5	0.47 (1)	10.00
AE- O	0 / 5102	-38.5	-38.5	0.47 (1)	10.00
O- AF	0 / 4824	-38.5	-38.5	0.37 (1)	10.00
AF- AG	0 / 4824	-38.5	-38.5	0.37 (1)	10.00
AG- N	0 / 4824	-38.5	-38.5	0.37 (1)	10.00
N- AH	0 / 6412	-38.5	-38.5	0.49 (1)	10.00
AH- AI	0 / 6412	-38.5	-38.5	0.49 (1)	10.00
AI- M	0 / 6412	-38.5	-38.5	0.49 (1)	10.00
M- L	0 / 6412	-38.5	-38.5	0.49 (1)	10.00
L- AJ	0 / 4823	-38.5	-38.5	0.36 (1)	10.00
AJ- AK	0 / 4823	-38.5	-38.5	0.36 (1)	10.00
AK- K	0 / 4823	-38.5	-38.5	0.36 (1)	10.00
K- AL	0 / 5101	-38.5	-38.5	0.47 (1)	10.00
AL- AM	0 / 5101	-38.5	-38.5	0.47 (1)	10.00
AM- AN	0 / 5101	-38.5	-38.5	0.47 (1)	10.00
AN- R	0 / 5101	-38.5	-38.5	0.47 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	-313 / 0	0.04 (1)
O- D	0 / 602	0.07 (2)
K- G	0 / 600	0.07 (2)
K- H	-314 / 0	0.04 (1)
L- G	0 / 1866	0.23 (1)
D- N	0 / 1858	0.23 (1)
L- F	-741 / 0	0.08 (1)
N- E	-738 / 0	0.08 (1)
N- F	-8 / 0	0.00 (1)
P- Q	-94 / 105	0.00 (1)
R- S	-93 / 106	0.00 (1)



DWG NO. TAM 7901932
STRUCTURAL COMPONENT ONLY 1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.21/1.00 (H-S:1) , BC=0.49/1.00 (L-N:1) , WB=0.23/1.00 (G-L:1) , SSI=0.17/1.00 (B-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.60 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T19	1	2	TRUSS DESC.	

Tamarack Roof Trues, Burlington

ID:?6l4Lc9YKqQUkM7luheM0pzx1lF-FiK_oQ9SHGTft4g1ta0nCRY3TR9MORmqKc4QNWzuqfe

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH
R-I	0 / 5079	-38.5	-38.5	0.45 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-8-4	-44	-49	--	FRONT	VERT	DEAD	--	--
D	6-8-4	-53	-53	--	BACK	VERT	TOTAL	--	--
D	6-8-4	-323	-323	--	FRONT	VERT	SNOW	--	--
G	22-1-12	-44	-49	--	FRONT	VERT	DEAD	--	--
G	22-1-12	-53	-53	--	BACK	VERT	TOTAL	--	--
G	22-1-12	-323	-323	--	FRONT	VERT	SNOW	--	--
K	22-1-0	-22	-28	--	BACK	VERT	TOTAL	--	--
M	16-1-0	-22	-28	--	BACK	VERT	TOTAL	--	--
O	6-7-0	-22	-28	--	BACK	VERT	TOTAL	--	--
T	5-11-4	-68	-68	--	BACK	VERT	TOTAL	--	--
U	8-7-0	-46	-46	--	BACK	VERT	TOTAL	--	--
V	10-7-0	-46	-46	--	BACK	VERT	TOTAL	--	--
W	12-7-0	-46	-46	--	BACK	VERT	TOTAL	--	--
X	14-4-0	-46	-46	--	BACK	VERT	TOTAL	--	--
Y	16-1-0	-46	-46	--	BACK	VERT	TOTAL	--	--
Z	18-1-0	-46	-46	--	BACK	VERT	TOTAL	--	--
AA	20-1-0	-46	-46	--	BACK	VERT	TOTAL	--	--
AB	22-8-12	-68	-68	--	BACK	VERT	TOTAL	--	--
AC	1-11-4	-22	-28	--	BACK	VERT	TOTAL	--	--
AD	3-11-4	-22	-28	--	BACK	VERT	TOTAL	--	--
AE	5-11-4	-22	-28	--	BACK	VERT	TOTAL	--	--
AF	8-7-0	-22	-28	--	BACK	VERT	TOTAL	--	--
AG	10-7-0	-22	-28	--	BACK	VERT	TOTAL	--	--
AH	12-7-0	-22	-28	--	BACK	VERT	TOTAL	--	--
AI	14-4-0	-22	-28	--	BACK	VERT	TOTAL	--	--
AJ	16-1-0	-22	-28	--	BACK	VERT	TOTAL	--	--
AK	20-1-0	-22	-28	--	BACK	VERT	TOTAL	--	--
AL	22-8-12	-22	-28	--	BACK	VERT	TOTAL	--	--
AM	24-8-12	-22	-28	--	BACK	VERT	TOTAL	--	--
AN	26-8-12	-22	-28	--	BACK	VERT	TOTAL	--	--

DWG NO. TAM T1901932
STRUCTURAL
COMPONENTS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T20	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Jan 16 09:41:26 2019 Page 1

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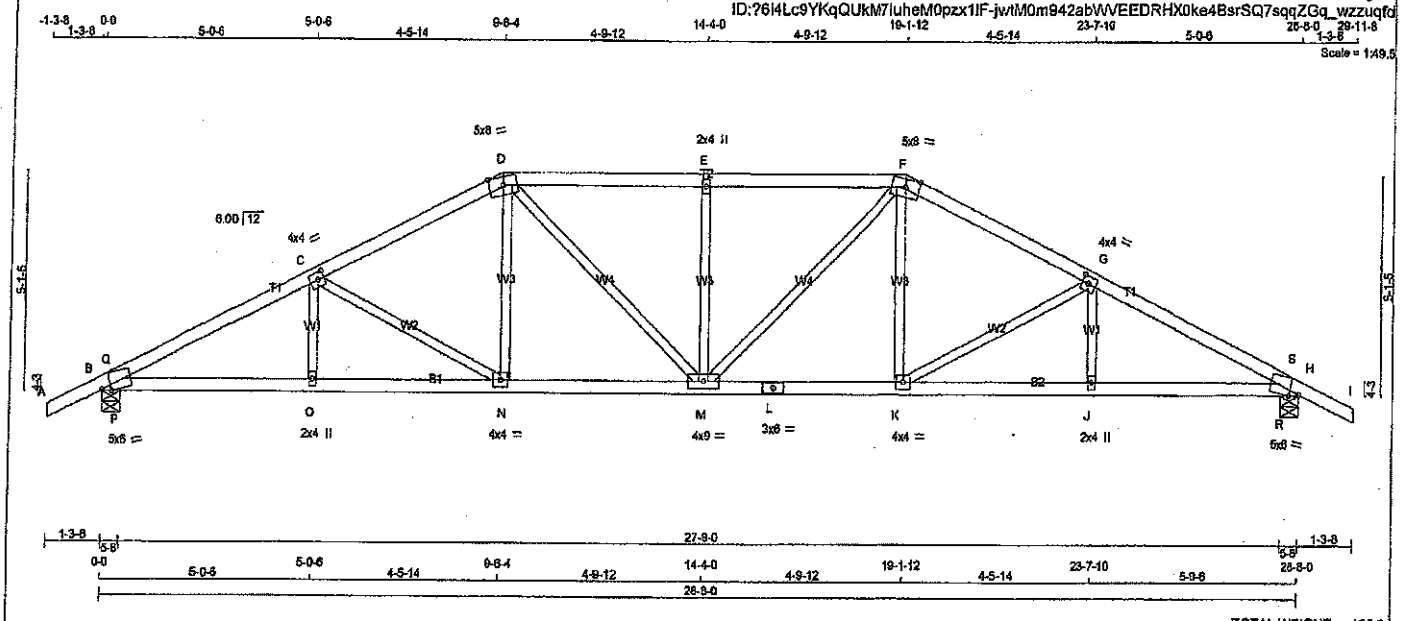


TOTAL WEIGHT = 105 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-m	MT20	5.0	6.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TMB1-m	MT20	5.0	6.0	Edge	2.75
J	BMW+w	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
B	2154	0	2154	0	0	IN-SX	IN-SX
H	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1601	911 / 0	301 / 0	0 / 0	0 / 0	389 / 0	0 / 0
H	1601	911 / 0	301 / 0	0 / 0	0 / 0	389 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

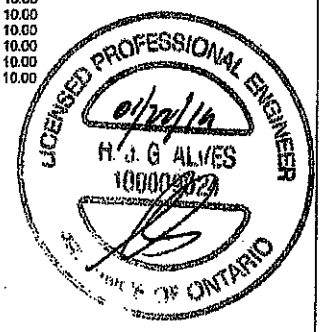
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 30	-102.1	-102.1	0.13 (1)	10.00
B-Q	-3857 / 0	-102.1	-102.1	0.20 (1)	3.46
Q-C	-3754 / 0	-102.1	-102.1	0.42 (1)	3.33
C-D	-3007 / 0	-102.1	-102.1	0.35 (1)	3.75
D-E	-3067 / 0	-102.1	-102.1	0.41 (1)	3.63
E-F	-3067 / 0	-102.1	-102.1	0.41 (1)	3.63
F-G	-3007 / 0	-102.1	-102.1	0.35 (1)	3.75
G-S	-3754 / 0	-102.1	-102.1	0.42 (1)	3.33
S-H	-3857 / 0	-102.1	-102.1	0.20 (1)	3.46
H-I	0 / 30	-102.1	-102.1	0.13 (1)	10.00
B-P	0 / 3371	-38.5	-38.5	0.60 (1)	10.00
P-O	0 / 3371	-38.5	-38.5	0.69 (1)	10.00
O-N	0 / 3371	-38.5	-38.5	0.64 (1)	10.00
N-M	0 / 2682	-38.5	-38.5	0.53 (1)	10.00
M-L	0 / 2682	-38.5	-38.5	0.53 (1)	10.00
L-K	0 / 2682	-38.5	-38.5	0.53 (1)	10.00
K-J	0 / 3371	-38.5	-38.5	0.64 (1)	10.00
J-R	0 / 3371	-38.5	-38.5	0.69 (1)	10.00
R-H	0 / 3371	-38.5	-38.5	0.60 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	0 / 240	0.06 (3)
N-C	-803 / 0	0.37 (1)
N-D	0 / 578	0.13 (1)
D-M	0 / 544	0.12 (1)
M-E	-593 / 0	0.23 (1)
M-F	0 / 544	0.12 (1)
K-F	0 / 578	0.13 (1)
K-G	-803 / 0	0.37 (1)
J-G	0 / 240	0.06 (3)
P-Q	-33 / 178	0.00 (1)
R-S	-33 / 178	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.42/1.00 (C-Q:1) , BC=0.69/1.00 (O-P:1) , WB=0.37/1.00 (C-N:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.60)
JSI METAL= 0.97 (H) (INPUT = 1.00)

DWG NO. TAM T40/933
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	T21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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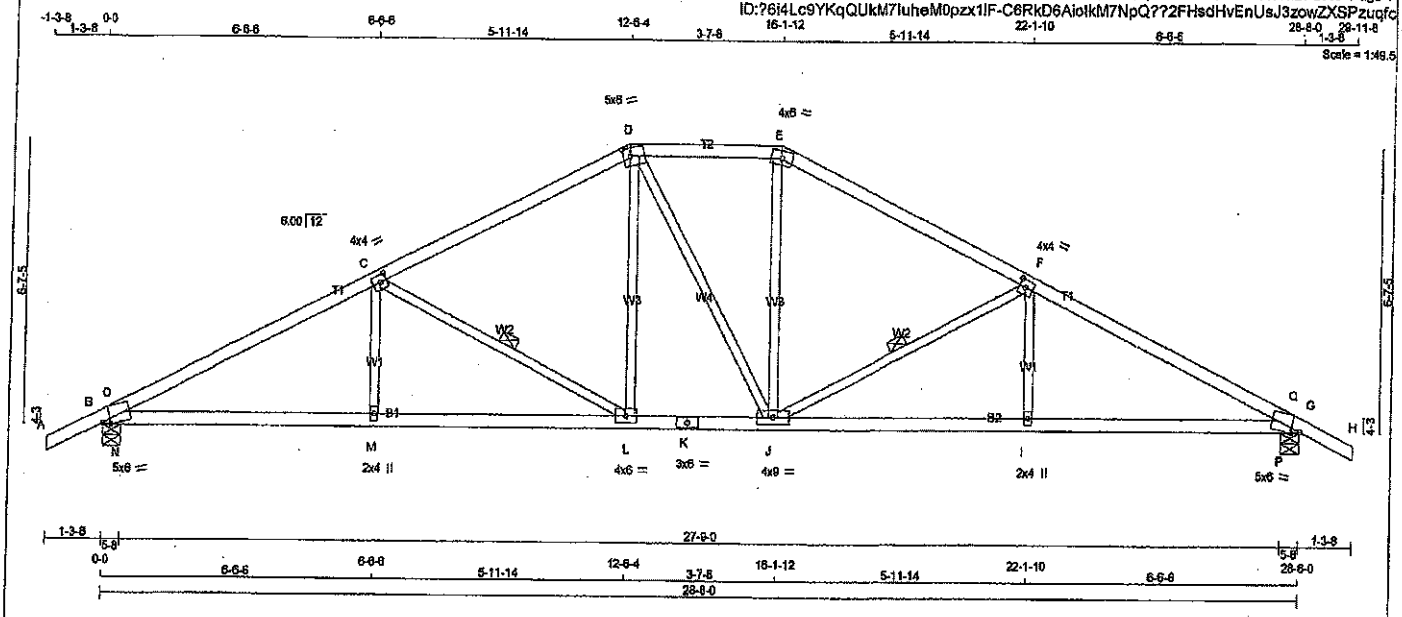


TOTAL WEIGHT = 104 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
B - K	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-m	MT20	5.0	6.0	Edge	2.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-m	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMB1-m	MT20	5.0	6.0	Edge	2.50
I	BMW+w	MT20	2.0	4.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2154	0	2154	0	0	5-8	5-8
G	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1601	911 / 0	301 / 0	0 / 0	0 / 0	389 / 0	0 / 0
G	1601	911 / 0	301 / 0	0 / 0	0 / 0	389 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, F-J.

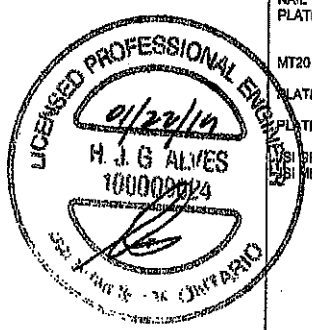
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 30	-102.1	-102.1	0.13 (1)		10.00
B- O	-3681 / 0	-102.1	-102.1	0.22 (1)		3.52
O- C	-3607 / 0	-102.1	-102.1	0.72 (1)		3.10
C- D	-2551 / 0	-102.1	-102.1	0.58 (1)		3.75
D- E	-2266 / 0	-102.1	-102.1	0.23 (1)		4.35
E- F	-2553 / 0	-102.1	-102.1	0.58 (1)		3.75
F- Q	-3606 / 0	-102.1	-102.1	0.72 (1)		3.10
Q- G	-3681 / 0	-102.1	-102.1	0.22 (1)		3.52
G- H	0 / 30	-102.1	-102.1	0.13 (1)		10.00
B- N	0 / 3248	-38.5	-38.5	0.66 (1)		10.00
N- M	0 / 3248	-38.5	-38.5	0.76 (1)		10.00
M- L	0 / 3248	-38.5	-38.5	0.73 (1)		10.00
L- K	0 / 2264	-38.5	-38.5	0.46 (1)		10.00
K- J	0 / 2264	-38.5	-38.5	0.46 (1)		10.00
J- I	0 / 3247	-38.5	-38.5	0.73 (1)		10.00
I- P	0 / 3247	-38.5	-38.5	0.76 (1)		10.00
P- G	0 / 3247	-38.5	-38.5	0.66 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	0 / 354	0.08 (3)
C- L	-1132 / 0	0.37 (1)
L- D	0 / 705	0.16 (1)
D- J	0 / 4	0.00 (1)
J- E	0 / 709	0.16 (1)
J- F	-1130 / 0	0.37 (1)
I- F	0 / 351	0.08 (3)
N- O	-163 / 228	0.00 (1)
P- Q	-161 / 229	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.72/1.00 (F-Q:1) , BC=0.76/1.00 (I-P:1) , WB=0.37/1.00 (C-L:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1657	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.86 (B) (INPUT = 0.90)
SSI METAL= 0.94 (B) (INPUT = 1.00)

DWG NO. TAM T190934
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	T22	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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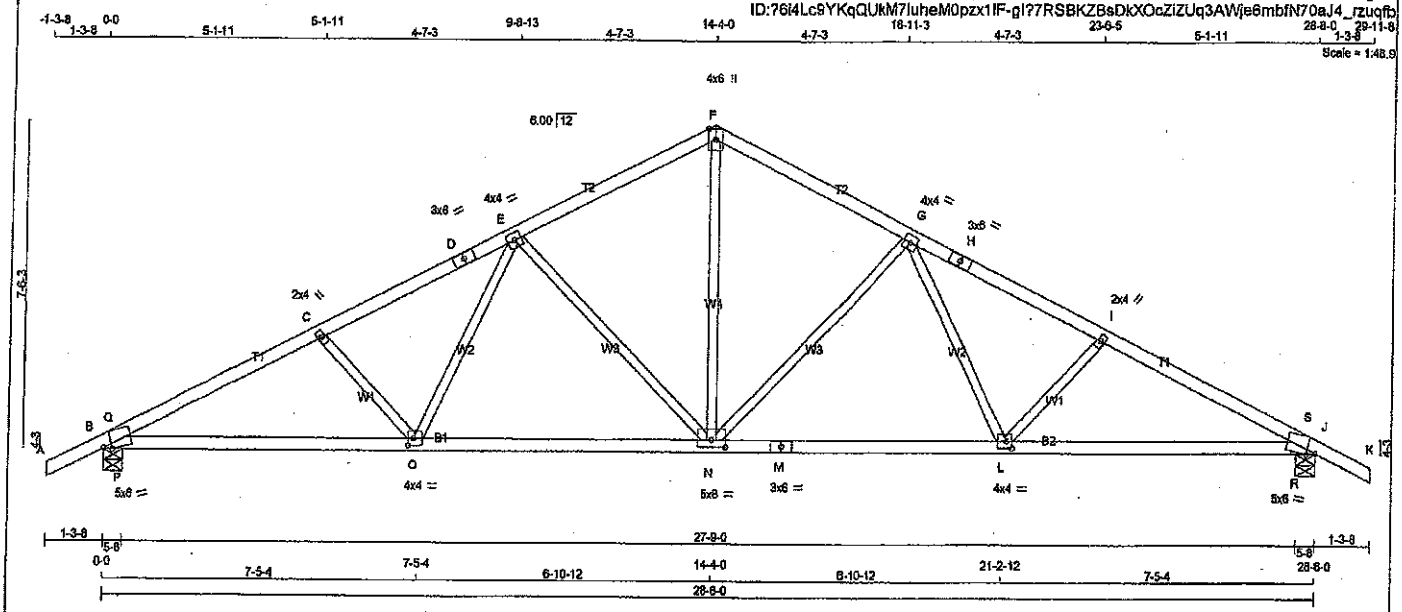

TOTAL WEIGHT = 103 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
B - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-m	MT20	5.0	6.0	Edge	2.50
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0		
J	TMB1-m	MT20	5.0	6.0	Edge	2.50
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0	2.00	4.00
O	BMWW-t	MT20	4.0	4.0	2.00	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	2154	0	2154	0	0	5-8	5-8
J	2154	0	2154	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1601	911 / 0	301 / 0	0 / 0	0 / 0	389 / 0	0 / 0
J	1601	911 / 0	301 / 0	0 / 0	0 / 0	389 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 30	-102.1	-102.1	0.13 (1)	10.00
B-Q	-3841 / 0	-102.1	-102.1	0.17 (2)	3.52
Q-C	-3673 / 0	-102.1	-102.1	0.46 (1)	3.33
C-D	-3341 / 0	-102.1	-102.1	0.38 (1)	3.56
D-E	-3341 / 0	-102.1	-102.1	0.38 (1)	3.56
E-F	-2263 / 0	-102.1	-102.1	0.33 (1)	4.24
F-G	-2263 / 0	-102.1	-102.1	0.33 (1)	4.24
G-H	-3341 / 0	-102.1	-102.1	0.38 (1)	3.56
H-I	-3341 / 0	-102.1	-102.1	0.38 (1)	3.56
I-S	-3673 / 0	-102.1	-102.1	0.46 (1)	3.33
S-J	-3841 / 0	-102.1	-102.1	0.17 (2)	3.52
J-K	0 / 30	-102.1	-102.1	0.13 (1)	10.00
B-P	0 / 3301	-38.5	-38.5	0.54 (1)	10.00
P-O	0 / 3301	-38.5	-38.5	0.76 (1)	10.00
O-N	0 / 2689	-38.5	-38.5	0.67 (2)	10.00
N-M	0 / 2689	-38.5	-38.5	0.67 (2)	10.00
M-L	0 / 2689	-38.5	-38.5	0.67 (2)	10.00
L-R	0 / 3301	-38.5	-38.5	0.76 (1)	10.00
R-J	0 / 3301	-38.5	-38.5	0.54 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-F	0 / 1632	0.37 (1)
N-G	-955 / 0	0.82 (1)
G-L	0 / 713	0.16 (1)
L-I	-467 / 0	0.11 (1)
E-N	-955 / 0	0.82 (1)
O-E	0 / 713	0.16 (1)
C-O	-467 / 0	0.11 (1)
P-Q	0 / 397	0.09 (1)
R-S	0 / 397	0.09 (1)

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009[illegible]
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.46/1.00 (I-S:1) , BC=0.76/1.00 (L-R:1) , WB=0.82/1.00 (G-N:1) , SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.95 (B) (INPUT = 1.00)

DWG NO. TAM T90 1935
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400669	T30	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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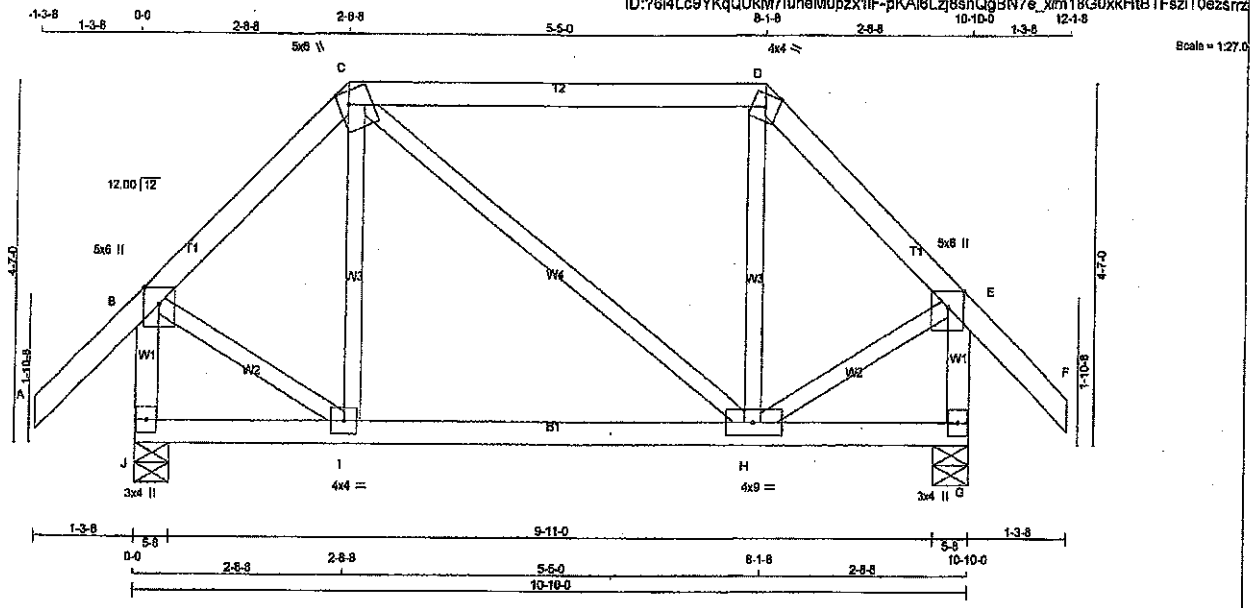

TOTAL WEIGHT = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	2.50
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	904	0	904	0	0	5-8	5-8
G	904	0	904	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	667	395 / 0	114 / 0	0 / 0	0 / 0	158 / 0	0 / 0
G	667	395 / 0	114 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00	I-C	-28 / 128	0.03 (3)
B-C	-544 / 0	-102.1	-102.1	0.13 (1)	6.25	C-H	0 / 0	0.00 (1)
C-D	-382 / 0	-102.1	-102.1	0.52 (1)	6.25	H-D	-28 / 128	0.03 (3)
D-E	-544 / 0	-102.1	-102.1	0.13 (1)	6.25	B-I	0 / 435	0.10 (1)
E-F	0 / 50	-102.1	-102.1	0.14 (1)	10.00	H-E	0 / 435	0.10 (1)
J-B	-875 / 0	0.0	0.0	0.10 (1)	7.81			
G-E	-875 / 0	0.0	0.0	0.10 (1)	7.81			
J-I	0 / 0	-38.5	-38.5	0.13 (3)	10.00			
I-H	0 / 382	-38.5	-38.5	0.18 (2)	10.00			
H-G	0 / 0	-38.5	-38.5	0.14 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.52/1.00 (C-D:1) , BC=0.18/1.00 (H-I:2) , WB=0.10/1.00 (B-I:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (D) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RECEIVED
AUG 08 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
01/22/19
H. J. G. ALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM T1901948
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400669	T31	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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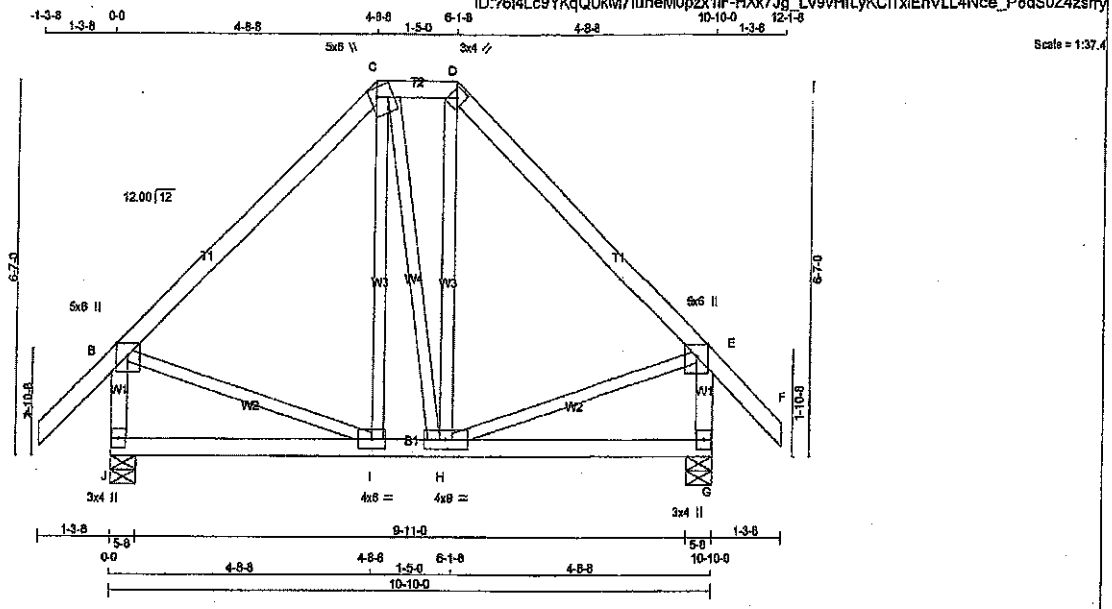


TOTAL WEIGHT = 60 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW+h	MT20	3.0	4.0		
E	TMVW+p	MT20	5.0	6.0	Edge	2.50
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	904	0	904	0	0	5-8	5-8
G	904	0	904	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	667	395 / 0	114 / 0	0 / 0	0 / 0	158 / 0	0 / 0
G	667	395 / 0	114 / 0	0 / 0	0 / 0	158 / 0	0 / 0

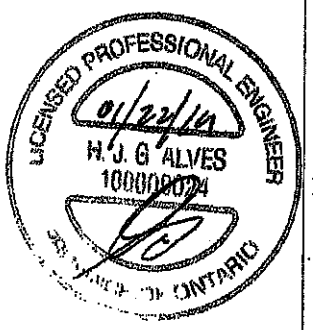
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-102.1	-102.1	0.14 (1)	10.00	I- C	0 / 112	0.03 (3)
B- C	-477 / 0	-102.1	-102.1	0.29 (1)	6.25	C- H	0 / 5	0.00 (3)
C- D	-337 / 0	-102.1	-102.1	0.03 (1)	6.25	H- D	0 / 116	0.03 (3)
D- E	-477 / 0	-102.1	-102.1	0.29 (1)	6.25	B- I	0 / 352	0.08 (1)
E- F	0 / 50	-102.1	-102.1	0.14 (1)	10.00	H- E	0 / 353	0.08 (1)
J- B	-827 / 0	0.0	0.0	0.09 (1)	7.81			
G- E	-828 / 0	0.0	0.0	0.09 (1)	7.81			
J- I	0 / 0	-38.5	-38.5	0.14 (3)	10.00			
I- H	0 / 336	-38.5	-38.5	0.19 (2)	10.00			
H- G	0 / 0	-38.5	-38.5	0.15 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.29/1.00 (D-E:1) , BC=0.19/1.00 (H-I:2) , WB=0.08/1.00 (E-H:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (D) (INPUT = 0.90)
JSI METAL= 0.22 (E) (INPUT = 1.00)

DWG NO. TAM T1901949
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400669	T32	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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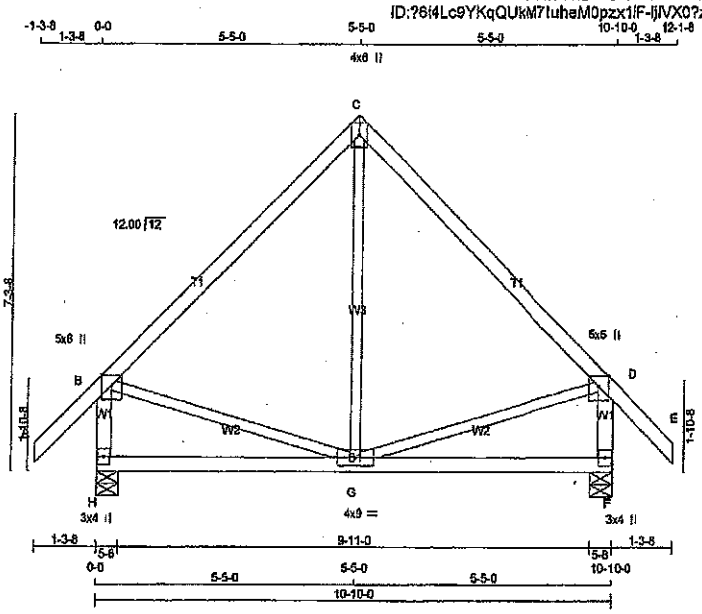


TOTAL WEIGHT = 2 X 52 = 104 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TTW+p	MT20	4.0	6.0		
D	TMVW+p	MT20	5.0	6.0	Edge	2.50
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
H	904	0	904	0	0	IN-SX 5-8	IN-SX 5-8
F	904	0	904	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	667	395 / 0	114 / 0	0 / 0	0 / 0	158 / 0	0 / 0
F	667	395 / 0	114 / 0	0 / 0	0 / 0	158 / 0	0 / 0

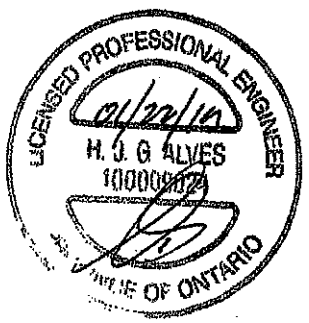
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00	G-C	0 / 240	0.06 (3)	
B-C	-448 / 0	-102.1	-102.1	0.39 (1)	6.25	B-G	0 / 329	0.07 (1)	
C-D	-448 / 0	-102.1	-102.1	0.39 (1)	6.25	G-D	0 / 329	0.07 (1)	
D-E	0 / 50	-102.1	-102.1	0.14 (1)	10.00				
H-B	-825 / 0	0.0	0.0	0.09 (1)	7.81				
F-D	-825 / 0	0.0	0.0	0.09 (1)	7.81				
H-G	0 / 0	-38.5	-38.5	0.26 (3)	10.00				
G-F	0 / 0	-38.5	-38.5	0.26 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.26/1.00 (F-G:3) , WB=0.07/1.00 (D-G:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.44 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

DWG NO. TAM 5401952
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400669	T32G	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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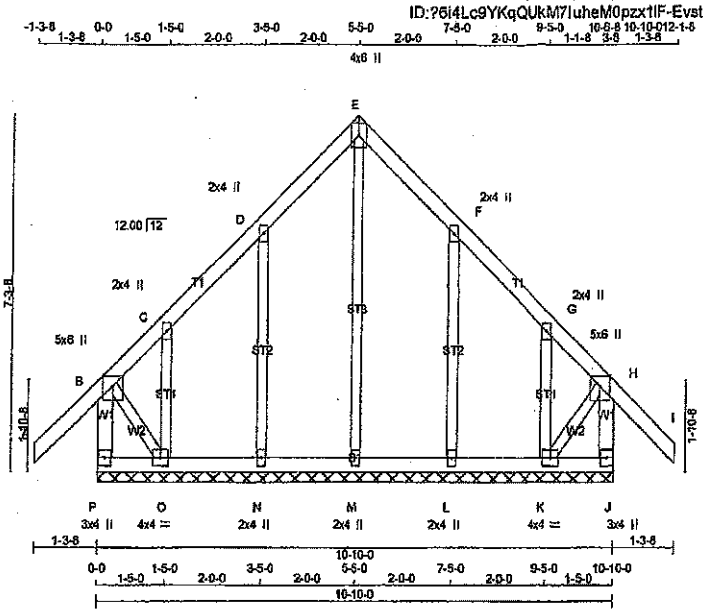


TOTAL WEIGHT = 58 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0		
H	TMVW+p	MT20	5.0	6.0	Edge	2.50
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00
B-C	-72 / 0	-102.1	-102.1	0.14 (1)	6.25
C-D	-5 / 0	-102.1	-102.1	0.07 (1)	10.00
D-E	-28 / 0	-102.1	-102.1	0.07 (1)	6.25
E-F	-28 / 0	-102.1	-102.1	0.07 (1)	6.25
F-G	-5 / 0	-102.1	-102.1	0.07 (1)	10.00
G-H	-72 / 0	-102.1	-102.1	0.14 (1)	6.25
H-I	0 / 50	-102.1	-102.1	0.14 (1)	10.00
P-B	-313 / 0	0.0	0.0	0.03 (1)	7.81
J-H	-313 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 0	-38.5	-38.5	0.02 (3)	10.00
O-N	0 / 14	-38.5	-38.5	0.03 (3)	10.00
N-M	0 / 9	-38.5	-38.5	0.02 (3)	10.00
M-L	0 / 9	-38.5	-38.5	0.02 (3)	10.00
L-K	0 / 14	-38.5	-38.5	0.03 (3)	10.00
K-J	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-151 / 0	0.14 (1)
N-D	-247 / 0	0.10 (1)
O-C	-76 / 0	0.01 (1)
L-F	-247 / 0	0.10 (1)
K-G	-76 / 0	0.01 (1)
B-O	0 / 24	0.01 (1)
K-H	0 / 24	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	26.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (K-L:3) , WB=0.14/1.00 (E-M:1) , SSI=0.08/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

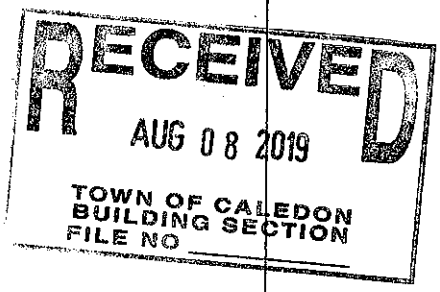
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (F) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)



DWG NO. TAM T7901957
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400669	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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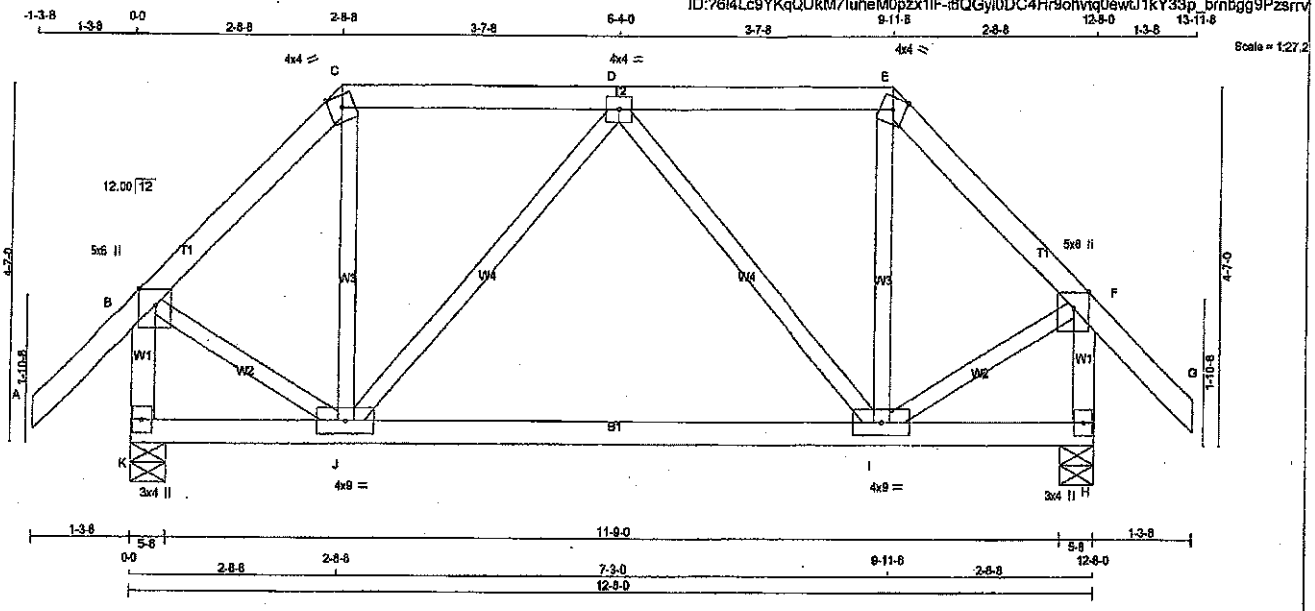


TOTAL WEIGHT = 59 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMVW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMVW+p	MT20	5.0	6.0	Edge	2.50
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1033	0	1033	0	0	5-8	5-8
H	1033	0	1033	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	763	449 / 0	133 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	763	449 / 0	133 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		LENGTH	FR-TO		
A- B	0 / 50	-102.1	-102.1	0.14 (1)	10.00	J- C	0 / 228	0.05 (2)
B- C	-681 / 0	-102.1	-102.1	0.14 (1)	6.25	I- E	0 / 228	0.05 (2)
C- D	-484 / 0	-102.1	-102.1	0.22 (1)	6.25	B- J	0 / 545	0.12 (1)
D- E	-484 / 0	-102.1	-102.1	0.22 (1)	6.25	I- F	0 / 545	0.12 (1)
E- F	-681 / 0	-102.1	-102.1	0.14 (1)	6.25	J- D	-290 / 0	0.15 (1)
F- G	0 / 50	-102.1	-102.1	0.14 (1)	10.00	D- I	-290 / 0	0.15 (1)
K- B	-1024 / 0	0.0	0.0	0.11 (1)	7.75			
H- F	-1024 / 0	0.0	0.0	0.11 (1)	7.75			
K- J	0 / 0	-38.5	-38.5	0.24 (3)	10.00			
J- I	0 / 668	-38.5	-38.5	0.32 (2)	10.00			
I- H	0 / 0	-38.5	-38.5	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.32/1.00 (I-J:2) , WB=0.15/1.00 (D-J:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

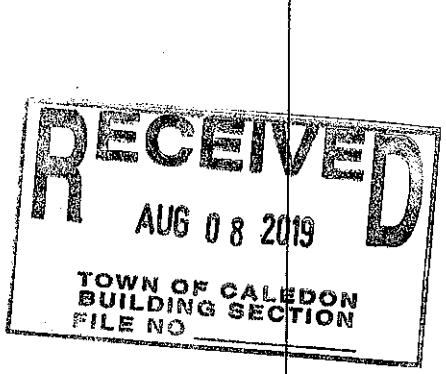
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (F) (INPUT = 0.90)
JSI METAL= 0.26 (F) (INPUT = 1.00)



DWG NO. TAM 7190 1952
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400669	T34	1	1	TRUSS DESC.	

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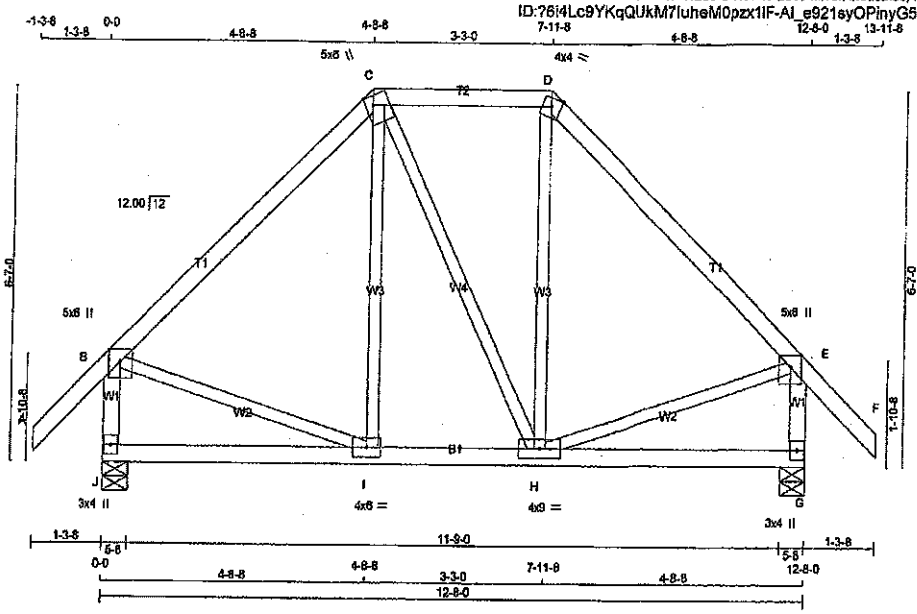


TOTAL WEIGHT = 84 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	5.0	6.0	Edge	2.50
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1033	0	1033	0	0	5-8	5-8
G	1033	0	1033	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	763	449 / 0	133 / 0	0 / 0	0 / 0	181 / 0	0 / 0
G	763	449 / 0	133 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

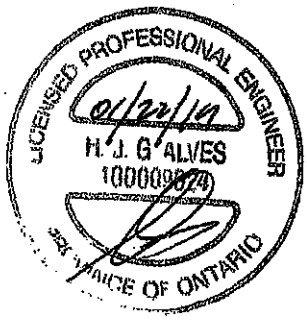
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00
B-C	-613 / 0	-102.1	-102.1	0.30 (1)	6.25
C-D	-432 / 0	-102.1	-102.1	0.14 (1)	6.25
D-E	-613 / 0	-102.1	-102.1	0.30 (1)	6.25
E-F	0 / 50	-102.1	-102.1	0.14 (1)	10.00
J-B	-954 / 0	0.0	0.0	0.11 (1)	7.81
G-E	-954 / 0	0.0	0.0	0.11 (1)	7.81
J-I	0 / 0	-38.5	-38.5	0.15 (3)	10.00
I-H	0 / 432	-38.5	-38.5	0.18 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-C	0 / 145	0.04 (3)
C-H	0 / 1	0.00 (3)
H-D	0 / 146	0.04 (3)
B-I	0 / 453	0.10 (1)
H-E	0 / 453	0.10 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.18/1.00 (H-I:2) , WB=0.10/1.00 (E-H:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES:

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (E) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)

DWG NO. TAM 190/953
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400669	T35	2	1	TRUSS DESC.	

Tamarack Roof Trusses, Burlington

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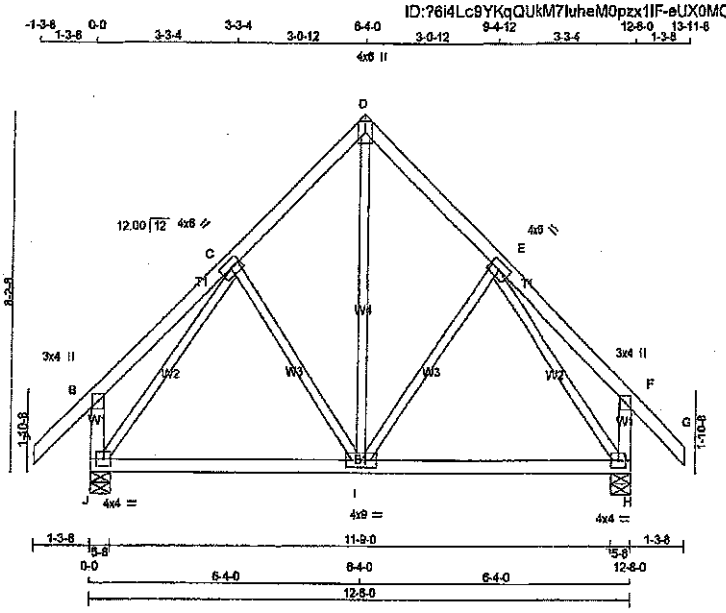

TOTAL WEIGHT = 2 X 66 = 133 lb (M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	6.0	
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	4.0	
I	BMWWW-t	MT20	4.0	9.0	
J	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1033	0	1033	0	0	5-8	5-8
H	1033	0	1033	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	763	449 / 0	133 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	763	449 / 0	133 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00
B-C	0 / 26	-102.1	-102.1	0.16 (1)	10.00
C-D	-566 / 0	-102.1	-102.1	0.12 (1)	6.25
D-E	-566 / 0	-102.1	-102.1	0.12 (1)	6.25
E-F	0 / 26	-102.1	-102.1	0.16 (1)	10.00
F-G	0 / 50	-102.1	-102.1	0.14 (1)	10.00
J-B	-285 / 0	0.0	0.0	0.03 (1)	7.81
H-F	-285 / 0	0.0	0.0	0.03 (1)	7.81
J-I	0 / 454	-38.5	-38.5	0.38 (2)	10.00
I-H	0 / 454	-38.5	-38.5	0.38 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
I-D	0 / 506	0.11 (1)
I-E	-125 / 24	0.07 (1)
C-I	-125 / 24	0.07 (1)
J-C	-813 / 0	0.42 (1)
E-H	-813 / 0	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.38/1.00 (H-I:2) , WB=0.42/1.00 (C-J:1) , SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (H) (INPUT = 0.90)
JSI METAL= 0.20 (E) (INPUT = 1.00)

DWG NO. TAM B90/954
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400669	T35G	1	1	TRUSS DESC.	

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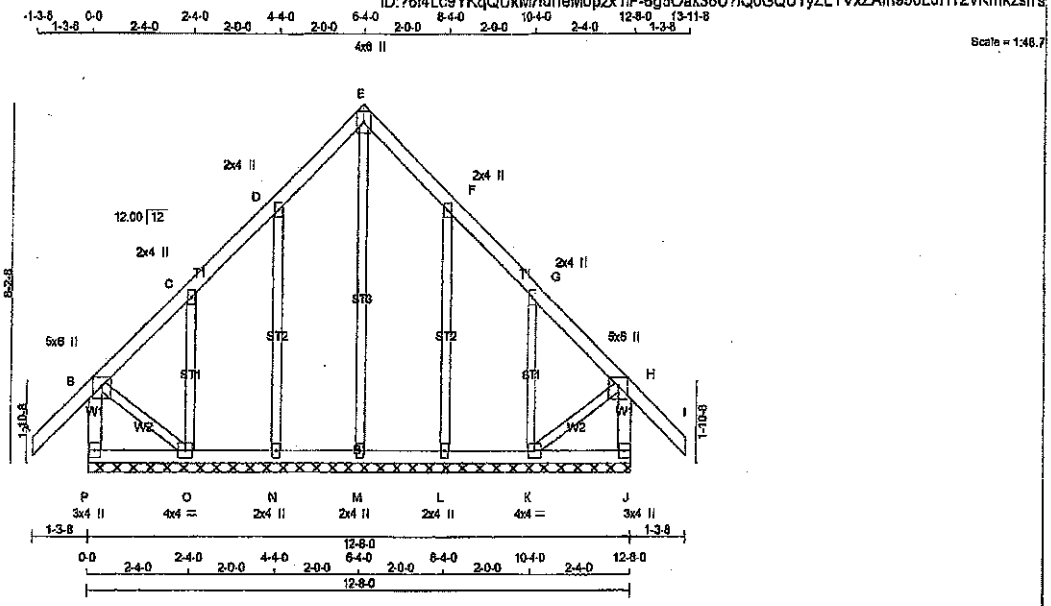


TOTAL WEIGHT = 68 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.50
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0		
H	TMVW+p	MT20	5.0	6.0	Edge	2.50
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

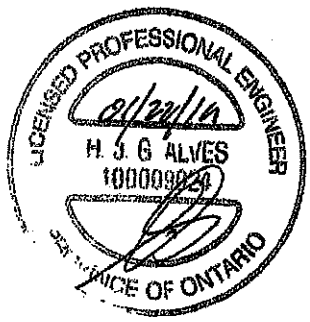
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00
B-C	-30 / 0	-102.1	-102.1	0.06 (1)	6.25
C-D	-38 / 0	-102.1	-102.1	0.06 (1)	6.25
D-E	-44 / 0	-102.1	-102.1	0.06 (1)	6.25
E-F	-44 / 0	-102.1	-102.1	0.06 (1)	6.25
F-G	-38 / 0	-102.1	-102.1	0.06 (1)	6.25
G-H	-30 / 0	-102.1	-102.1	0.06 (1)	6.25
H-I	0 / 50	-102.1	-102.1	0.14 (1)	10.00
P-B	-287 / 0	0.0	0.0	0.03 (1)	7.81
J-H	-287 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 0	-38.5	-38.5	0.04 (3)	10.00
O-N	0 / 25	-38.5	-38.5	0.04 (3)	10.00
N-M	0 / 21	-38.5	-38.5	0.03 (2)	10.00
M-L	0 / 21	-38.5	-38.5	0.03 (2)	10.00
L-K	0 / 25	-38.5	-38.5	0.04 (3)	10.00
K-J	0 / 0	-38.5	-38.5	0.04 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-E	-133 / 0	0.17 (1)
N-D	-221 / 0	0.13 (1)
O-C	-234 / 0	0.06 (1)
L-F	-221 / 0	0.13 (1)
K-G	-234 / 0	0.06 (1)
B-O	0 / 38	0.01 (1)
K-H	0 / 38	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.04/1.00 (K-L:3) , WB=0.17/1.00 (E-M:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (C) (INPUT = 0.60)
JSI METAL= 0.12 (C) (INPUT = 1.00)

DWG NO. TAM 1790/1955
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400669	T36	4	1	TRUSS DESC.	

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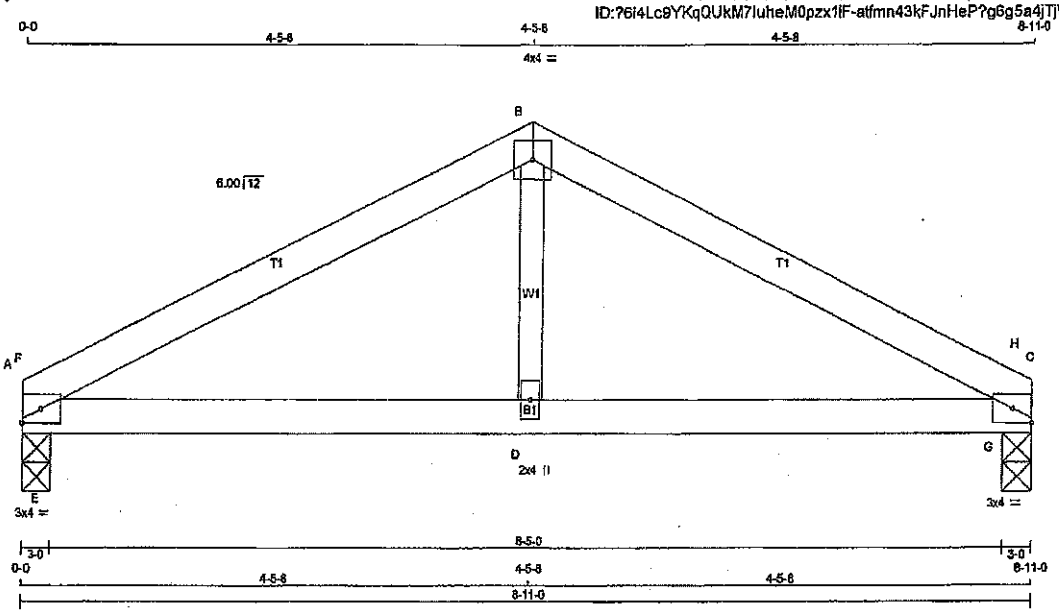


TOTAL WEIGHT = 4 X 24 = 94 lb (M)[F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	3.0	4.0		Edge
B	TTW-p	MT20	4.0	4.0		
C	TMB1-I	MT20	3.0	4.0		Edge
D	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	627	0	627	0	0	3-0	3-0
C	627	0	627	0	0	3-0	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	468	259 / 0	94 / 0	0 / 0	0 / 0	116 / 0	0 / 0
C	468	259 / 0	94 / 0	0 / 0	0 / 0	116 / 0	0 / 0

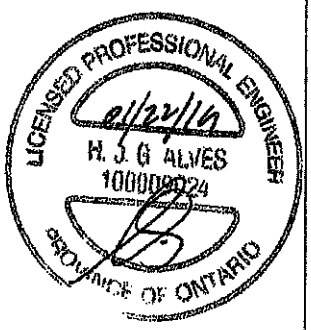
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- F	-624 / 0	-102.1	-102.1	0.12 (1)		6.25	D- B	0 / 291	0.07 (2)
F- B	-730 / 0	-102.1	-102.1	0.23 (1)		6.25	E- F	-332 / 162	0.00 (1)
B- H	-730 / 0	-102.1	-102.1	0.23 (1)		6.25	G- H	-332 / 162	0.00 (1)
H- C	-624 / 0	-102.1	-102.1	0.12 (1)		6.25			
A- E	0 / 642	-38.5	-38.5	0.22 (1)		10.00			
E- D	0 / 642	-38.5	-38.5	0.28 (1)		10.00			
D- G	0 / 642	-38.5	-38.5	0.28 (1)		10.00			
G- C	0 / 642	-38.5	-38.5	0.22 (1)		10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.23/1.00 (B-F:1) , BC=0.28/1.00 (D-E:1) , WB=0.07/1.00 (B-D:2) , SSI=0.29/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (A) (INPUT = 0.90)
JSI METAL= 0.23 (A) (INPUT = 1.00)

DWG NO. 1AM 7901958
STRUCTURAL COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	J1	26	1	GREEN PARK HOMES TRUSS DESC.	

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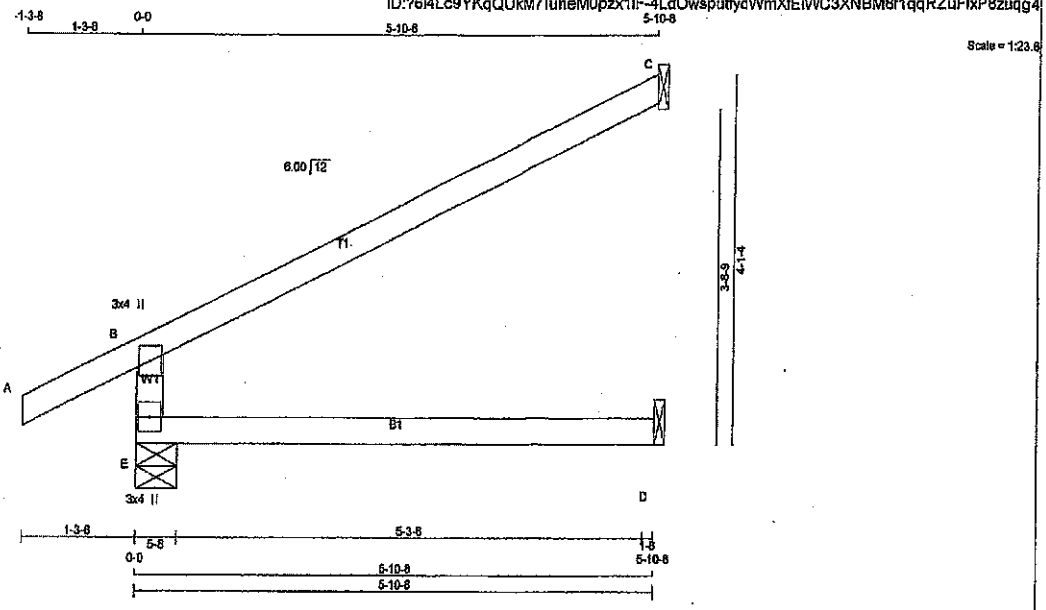

TOTAL WEIGHT = 26 X 17 = 437 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	646	0	646	0	0	5-8	5-8
C	225	0	225	0	0	1-8	1-8
D	93	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37621H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	292 / 0	72 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
E- B	-513 / 0	0.0	0.0	0.22 (3)	7.81			
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.00			
B- C	-34 / 0	-102.1	-102.1	0.60 (1)	6.25			
E- D	0 / 0	-38.5	-38.5	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 942 (0.07")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.22/1.00 (D-E:3) , WB=0.00/1.00 (n/a:0) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

DWG NO. TAM T190 1936
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	J3	2	1	TRUSS DESC.	

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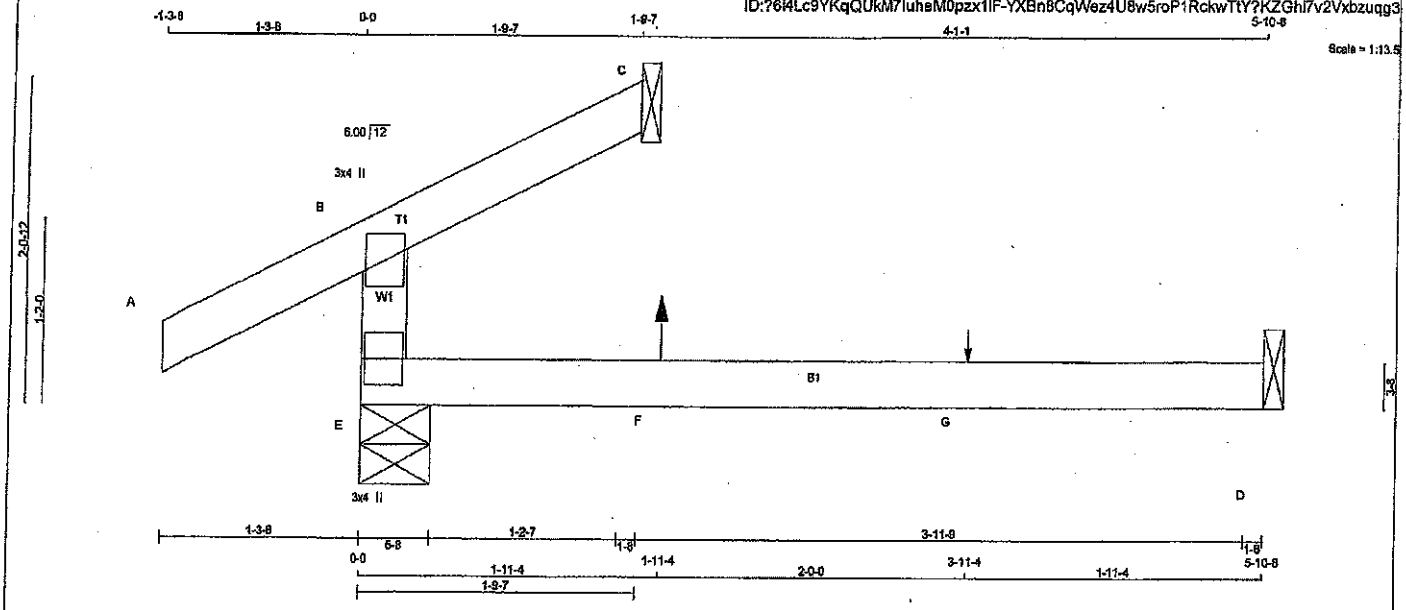

TOTAL WEIGHT = 2 X 12 = 23 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	352	0	352	0	0	5-8	5-8
C	94	0	97	0	0	1-8	1-8
D	94	0	123	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	256	156 / 0	42 / 0	0 / 0	0 / 0	91 / 0	0 / 0
C	76	24 / 0	28 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	66	0 / -3	53 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CSI	MAX (LC)	MAX. LINBRAC LENGTH	MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO						
E- B	-229 / 26	0.0 0.0	0.17	(3)	7.81			
A- B	0 / 31	-102.1 -102.1	0.13	(1)	10.00			
B- C	0 / 27	-102.1 -102.1	0.15	(3)	10.00			
B- F	0 / 0	-38.5 -38.5	0.21	(3)	10.00			
F- G	0 / 0	-38.5 -38.5	0.21	(3)	10.00			
G- D	0 / 0	-38.5 -38.5	0.21	(3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	6	1	13	BACK	VERT	TOTAL	—	—
G	3-11-4	1	1	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 839 (0.08")

CSI: TC=0.17/1.00 (B-E:3) , BC=0.21/1.00 (D-E:3) , WB=0.00/1.00 (n/a:0) , SSI=0.13/1.00 (D-E:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DWG NO. TAM 1790 1937
STRUCTURAL COMPONENTS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	J4	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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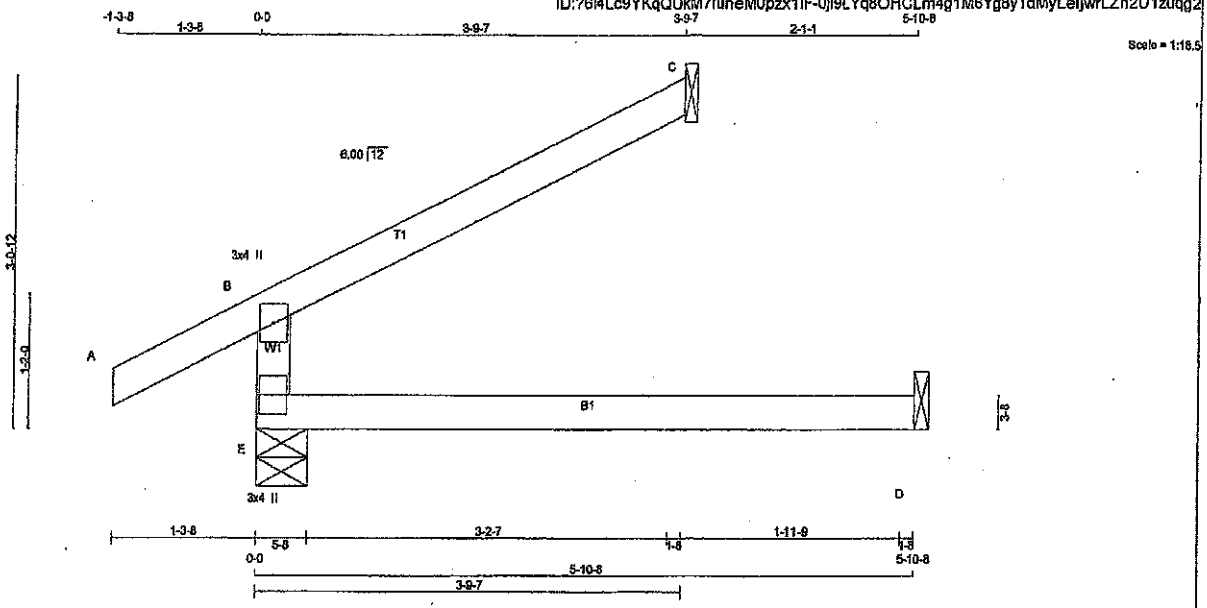


TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	513	0	513	0	0	5-8	5-8
C	145	0	145	0	0	1-8	1-8
D	93	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	361	216 / 0	72 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	100	83 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

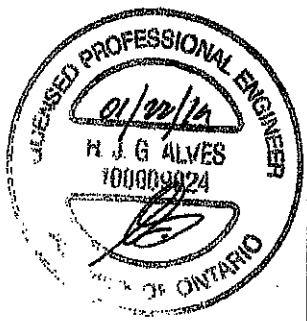
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
E- B	-380 / 0	0.0	0.0	0.21 (3)	7.61
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.00
B- C	-22 / 0	-102.1	-102.1	0.25 (1)	6.25
E- D	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 942 (0.07")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.21/1.00 (D-E:3) , WB=0.00/1.00 (n/a:0) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1957	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

DWG NO. TAM T190 1938
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
400517	J5	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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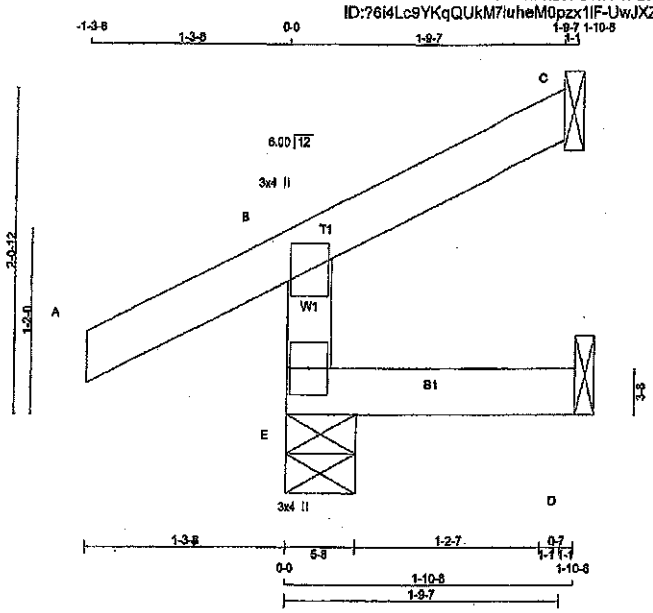

TOTAL WEIGHT = 2 X 7 = 14 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	318	0	318	0	0	5-8	5-8
C	51	0	51	0	-26	1-8	1-8
D	24	0	41	0	-4	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	226	160 / 0	20 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	36	27 / -21	2 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	24	0 / -9	18 / 0	0 / 0	0 / 0	11 / 0	0 / 0

SEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
E- B	-270 / 0	0.0	0.0	0.04 (5)	7.81			
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.09			
B- C	-18 / 0	-102.1	-102.1	0.10 (1)	6.25			
E- D	0 / 0	-38.5	-38.5	0.04 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.04/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM 1901939
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	J8	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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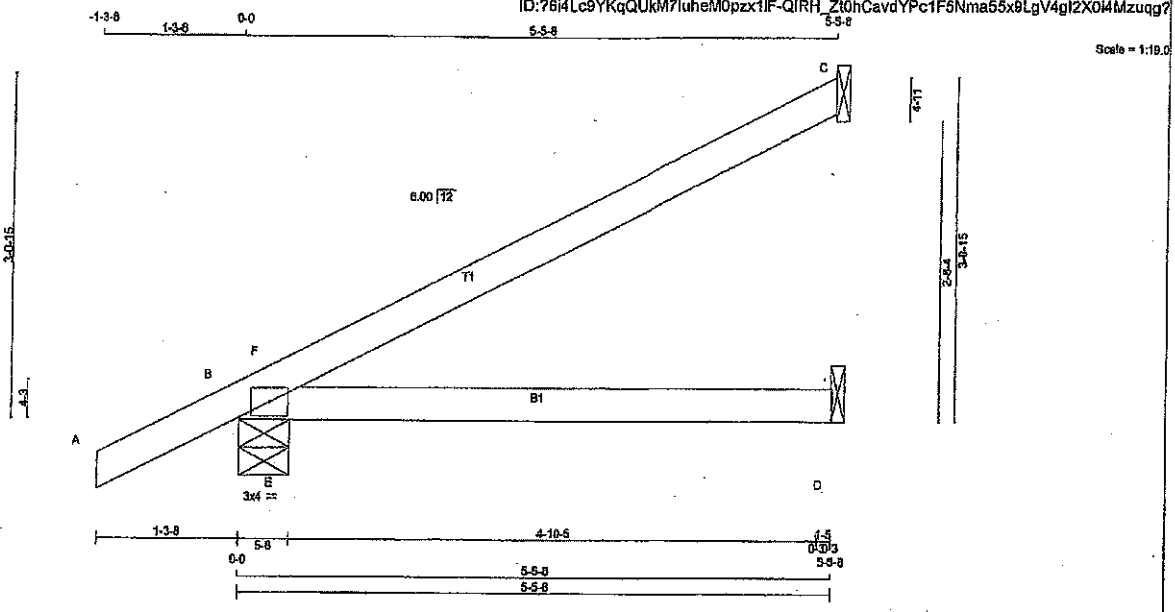

TOTAL WEIGHT = 5 X 15 = 75 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	251	0	251	0	0	1-8	1-8
B	522	0	522	0	0	5-8	5-8
D	133	0	146	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	175	134 / 0	8 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	382	237 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	112	24 / 0	50 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 30	-102.1	-102.1	0.13 (1)		10.00
B- F	-95 / 46	-102.1	-102.1	0.11 (3)		6.25
F- C	-3 / 7	-102.1	-102.1	0.42 (1)		10.00
B- E	0 / 0	-38.5	-38.5	0.25 (1)		10.00
E- D	0 / 0	-38.5	-38.5	0.31 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-377 / 160	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/854 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/492 (0.13")

CSI: TC=0.42/1.00 (C-F:1) , BC=0.31/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.35/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

DWG NO. TAM 7190/19 41
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400517	J9	15	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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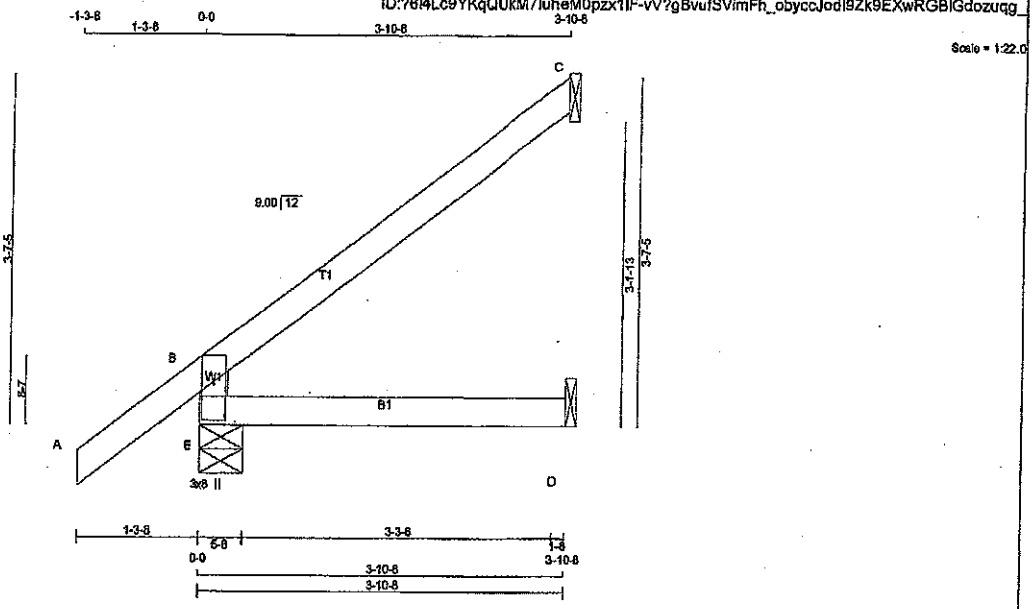


TOTAL WEIGHT = 15 X 12 = 185 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
E						
E	TMBMV1+p	MT20	3.0	8.0	4.50	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	477	0	477	0	0	5-8	5-8
C	148	0	148	0	0	1-8	1-8
D	80	0	77	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37621H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	347	221 / 0	49 / 0	0 / 0	0 / 0	78 / 0	0 / 0
C	102	64 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	55	0 / 0	33 / 0	0 / 0	0 / 0	22 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

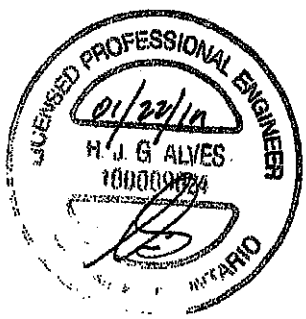
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-368 / 0	0.0	0.0	0.11	(3)	7.81			
A-B	0 / 42	-102.1	-102.1	0.14	(1)	10.00			
B-C	-30 / 0	-102.1	-102.1	0.28	(1)	6.25			
E-D	0 / 0	-36.9	-36.5	0.11	(3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.11/1.00 (D-E:3) , WB=0.00/1.00 (n/a:0) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)

DWG NO. TAM 17901942
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
400669	J10	11	1	TRUSS DESC.	

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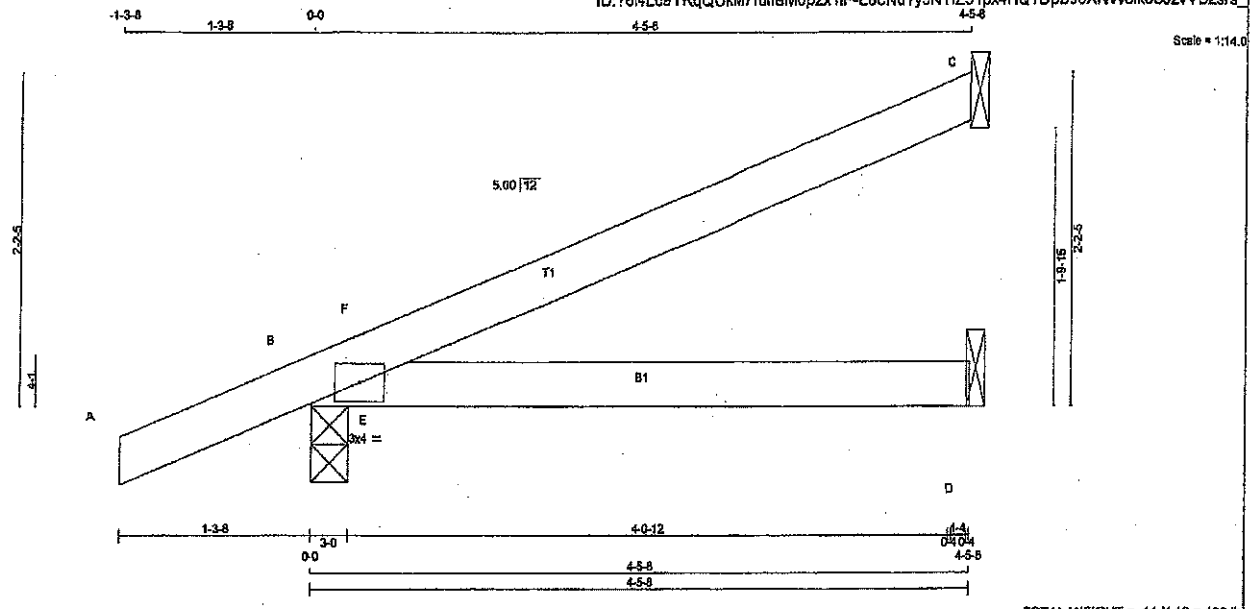


TOTAL WEIGHT = 11 X 12 = 136 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	204	0	204	0	0	1-8	1-8
B	451	0	451	0	0	3-0	3-0
D	110	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	143	109 / 0	7 / 0	0 / 0	0 / 0	27 / 0	0 / 0
B	329	208 / 0	47 / 0	0 / 0	0 / 0	74 / 0	0 / 0
D	91	21 / 0	40 / 0	0 / 0	0 / 0	31 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 25	-102.1	-102.1	0.13 (1)	10.00	E-F	-217 / 98	0.00 (1)
B-F	-51 / 0	-102.1	-102.1	0.08 (3)	6.25			
F-C	0 / 6	-102.1	-102.1	0.28 (1)	10.00			
B-E	0 / 0	-38.5	-38.5	0.18 (1)	10.00			
E-D	0 / 0	-38.5	-38.5	0.21 (1)	10.00			

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.28/1.00 (C-F:1) , BC=0.21/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.22/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

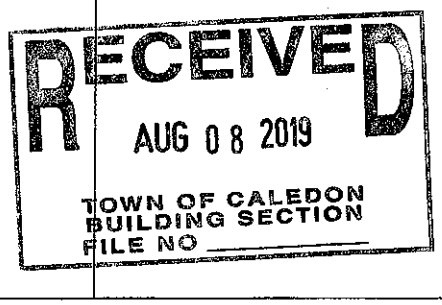
NAIL VALUES

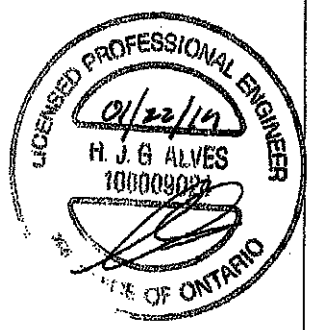
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (B) (INPUT = 0.80)
JSI METAL= 0.07 (B) (INPUT = 1.00)





DWG NO. TAM T90/957
STRUCTURAL COMPONENT

Boise Cascade

Double 2 x 10 SPF #2

BM1 (Roof Beam)

Dry | 1 span | No cant.

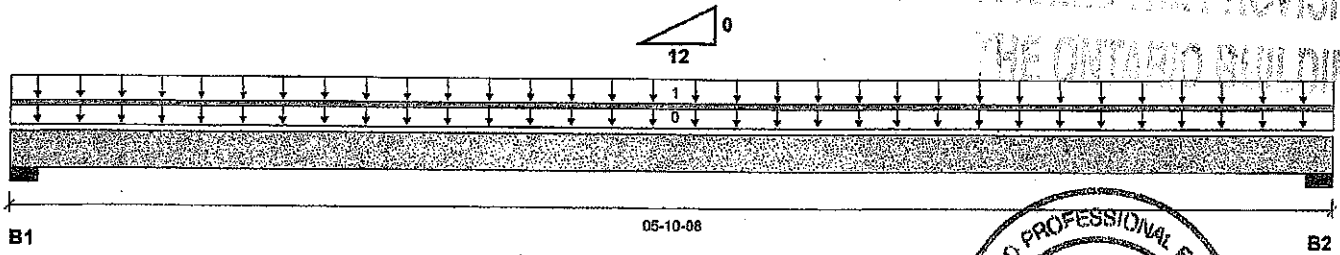
PASSED

BC CALC® Member Report
Build 6782

January 22, 2019 09:17:04

THE STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

Job name:	Greenpark Homes-Barton 8 Elv 1-2	File name:	
Address:		Description:	
City, Province, Postal Code:		Specifier:	
Builder:		Designer:	Herculano Alves
Code reports:	NLGA	Company:	



Total Horizontal Product Length = 05-10-08

LICENSED PROFESSIONAL ENGINEER
01/22/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Reaction Summary (Down / Uplift) (lbs)

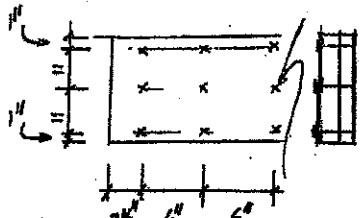
Bearing	Live	Dead	Snow	Wind
B1, 3-1/2"	485 / 0	591 / 0	1674 / 0	
B2, 3-1/2"	485 / 0	591 / 0	1674 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	05-10-08	Top		6			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	05-10-08	Front	11	13	38		15-00-00

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4664 ft-lbs	6040 ft-lbs	77.2 %	8	02-11-04
End Shear	2384 lbs	3985 lbs	59.8 %	8	01-00-12
Total Load Deflection	L/999 (0.067")	n\a	n\a	14	02-11-04
Live Load Deflection	L/999 (0.052")	n\a	n\a	21	02-11-04
Max Defl.	0.067"	n\a	n\a	14	02-11-04
Span / Depth	7.0				



CONNECT W/ 3" COMMON SPIRAL NAILS @ 6" o/c IN 3 ROWS

Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur. NA

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load. NA

Distributed side-load exceeds allowable magnitude for connection design. Please consult a technical representative or Professional Engineer for the design of the connection. OK W/ NAILING PATTERN

DWG NO. TAM T1901943
STRUCTURAL
COMPONENT ONLY

Notes

Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/360) Live load deflection criteria.
Design meets User specified (1") Maximum Total load deflection criteria.
Minimum bearing length for B1 is 2".
Minimum bearing length for B2 is 2".
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
Design based on Dry Service Condition. CONFORMS TO OBC 2012
The analysis of solid sawn wood members is in accordance with CSA 086 and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.
Importance Factor : Normal Part code : Part 9

Disclosure

Boise Cascade

Double 2 x 10 SPF #2

BM2 (Roof Beam)

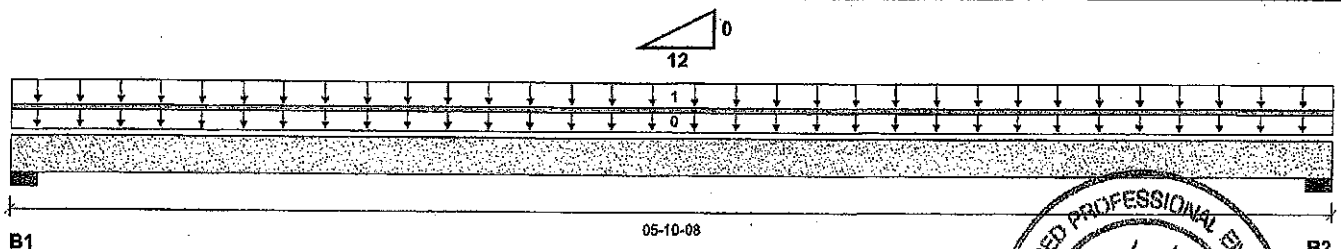
Dry | 1 span | No cant.

PASSED

BC CALC® Member Report
Build 6782

January 22, 2019 09:17:04

Job name:	Greenpark Homes-Barton 8 Elv 1-2	File name:	
Address:		Description:	
City, Province, Postal Code:		Specifier:	
Builder:		Designer:	Herculano Alves
Code reports:	NLGA	Company:	



Total Horizontal Product Length = 05-10-08

LICENSED PROFESSIONAL ENGINEER
01/22/19
H. J. G. ALVES
PROVINCE OF ONTARIO

Reaction Summary (Down / Uplift) (lbs)

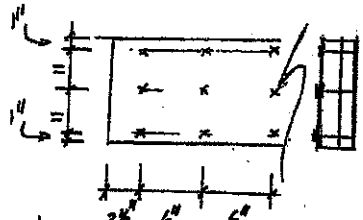
Bearing	Live	Dead	Snow	Wind
B1, 3-1/2"	517 / 0	630 / 0	1786 / 0	
B2, 3-1/2"	517 / 0	630 / 0	1786 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	05-10-08	Top		6			00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	05-10-08	Back	11	13	38		16-00-00

Controls Summary

	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	4973 ft-lbs	6040 ft-lbs	82.3 %	8	02-11-04
End Shear	2542 lbs	3985 lbs	63.8 %	8	01-00-12
Total Load Deflection	L/999 (0.071")	n\a	n\a	14	02-11-04
Live Load Deflection	L/999 (0.056")	n\a	n\a	21	02-11-04
Max Defl.	0.071"	n\a	n\a	14	02-11-04
Span / Depth	7.0				



connect w/ 3" common spiral nails @ 6" o/c in 3 rows

Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur. NR

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load. NR

Distributed side-load exceeds allowable magnitude for connection design. Please consult a technical representative or Professional Engineer for the design of the connection. ok w/ manufacturer

DWG NO. TAM T190 1944
STRUCTURAL
COMPONENT ONLY

Notes

- Design meets User specified (L/360) Total load deflection criteria.
- Design meets User specified (L/360) Live load deflection criteria.
- Design meets User specified (1") Maximum Total load deflection criteria.
- Minimum bearing length for B1 is 2-3/16".
- Minimum bearing length for B2 is 2-3/16".
- Calculations assume member is fully braced.
- Resistance Factor phi has been applied to all presented results per CSA O86.
- BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2015 and CSA O86.
- Design based on Dry Service Condition. conforms to OBC 2012
- The analysis of solid sawn wood members is in accordance with CSA 086 and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.
- Importance Factor : Normal Part code : Part 9

Disclosure

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

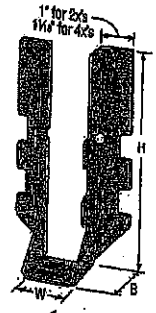
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

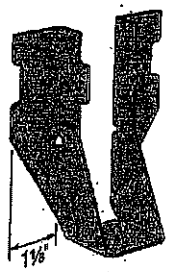
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

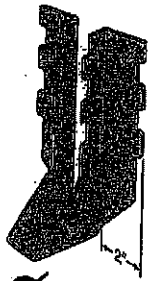
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



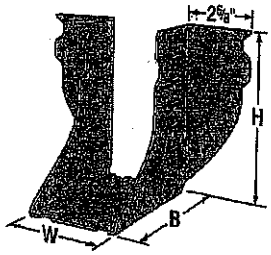
LUS28



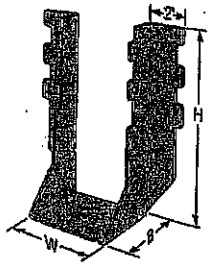
LU26L



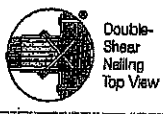
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2



Double-Shear Nailing Top View

Double-Shear Nailing Side View; Do not bend tab

Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

(Truss Designer to provide fastener quantity for connecting multiple members together)

LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie

HHUS/HGUS

See Hanger Options information on pp. 125–127.

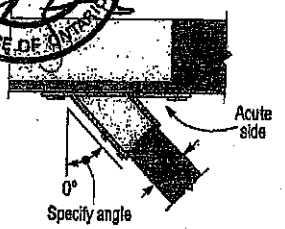
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance (C-C-D) Uplift (K_D = 1.15) lb. / kN	Normal (K_D = 1.00) lb. / kN
Single 2x Sizes									
LUS24	18	1⅛	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅛	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅛	5	1⅝	4⅛	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
SS LUS26	18	1⅛	4¼	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅞	3	5⅛	(14) 16d	(6) 16d	2705 / 12.03	4940 / 21.98	2085 / 9.28	3875 / 17.24
LJS260S	18	1⅛	5	3½	4⅝	(16) 16d	(6) 16d	2056 / 9.15	4265 / 18.97	1460 / 6.49	4115 / 16.31
HGUS26	12	1⅝	5⅞	5	4⅞	(28) 16d	(8) 16d	2685 / 11.95	6625 / 29.47	2685 / 11.96	5700 / 25.35
LU26L	20	1⅛	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
SS LUS28	18	1⅛	6¾	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5385 / 23.95	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.73	7675 / 34.14	3310 / 14.74	6900 / 30.73
LU210L	20	1⅛	8	1⅝	7⅛	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
SS LUS210	18	1⅛	7⅞	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

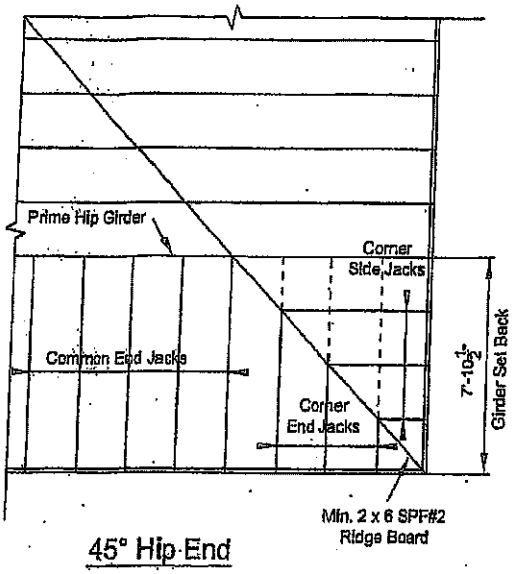
SIMPSON Strong-Tie®

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_s[1]	Fasteners Header	Joist	Factored Resistance (D.F.) Uplift lb.	Normal lb.	S-P-F Uplift (K_D = 1.15) lb.	Normal (K_D = 1.00) lb.
										Uplift kN	Normal kN
Double 2x Sizes											
LUS24-2	18	3¼	9⅛	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.98	2.62	6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3⅝	5⅜	3	3⅛	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.67	32.62	9.20	23.15
HGUS26-2	12	3⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.50	39.81	13.83	28.27
LUS28-2	18	3¼	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3⅝	7⅛	4	6⅛	(30) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.75	19.17	40.99
LUS210-2	18	3¼	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3⅝	9⅛	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3⅝	9⅛	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4⅜	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.50	39.81	13.83	28.27
HGUS28-3	12	4⅜	7¼	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.75	19.17	40.99
HHUS210-3	14	4⅜	9	3	7⅝	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4⅜	9¼	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.15	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	5⅜	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.50	39.81	13.83	28.27
HGUS28-4	12	5⅜	7⅛	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.75	19.17	40.99
HHUS210-4	14	5⅜	8⅞	3	7⅝	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	5⅜	9⅛	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.15	21.60	46.26
HGUS212-4	12	5⅜	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.72	24.13	47.35
HGUS214-4	12	5⅜	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
LUS46	18	3⅜	4⅛	2	3⅛	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS46	14	3⅝	5¼	3	3⅛	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.29	32.62	9.20	23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.50	39.81	13.83	28.27
LUS48	18	3⅜	6⅛	2	3⅞	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.75	19.17	40.99
LUS410	18	3⅜	8¼	2	5⅜	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.72	24.13	47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80

See footnotes on p. 258.



45° Hip End

LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F.

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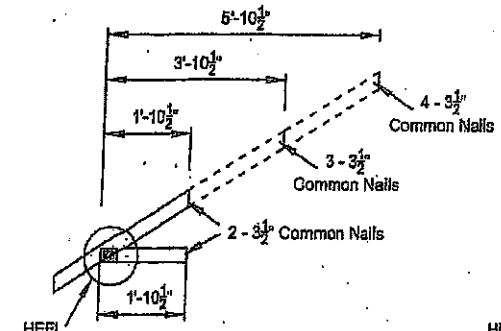
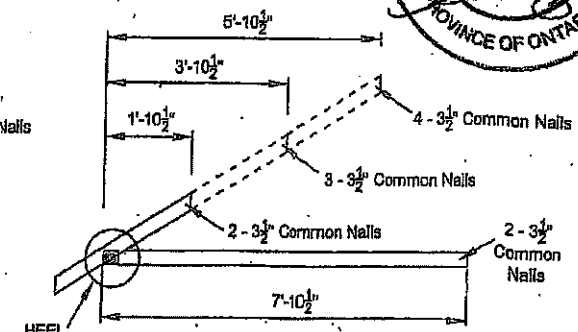
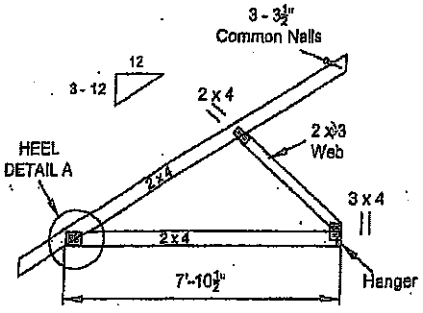


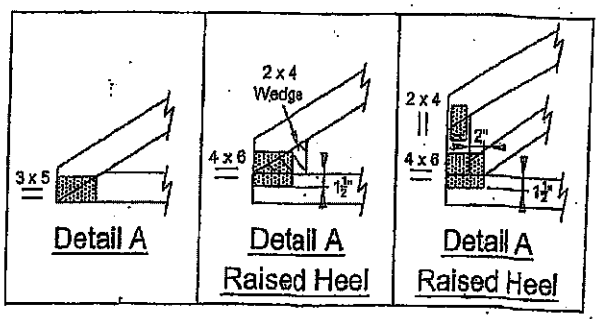
Corner Side Jacks



Corner End Jacks



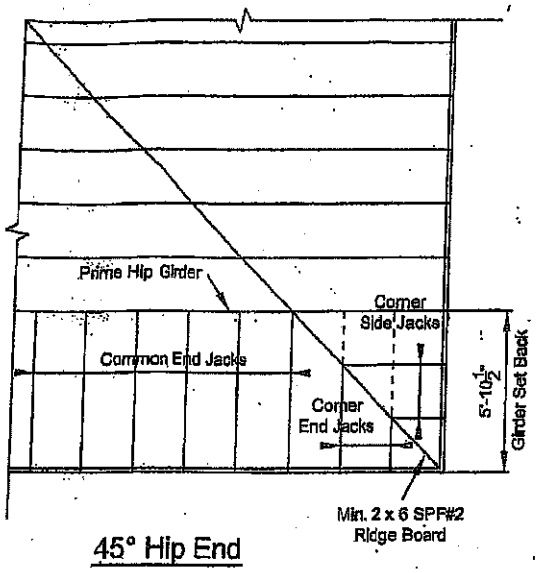
Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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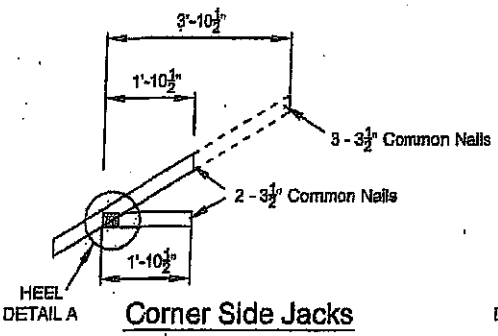
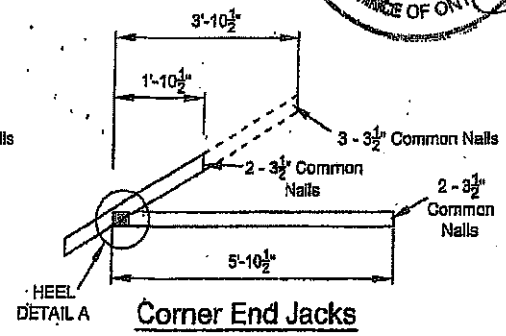
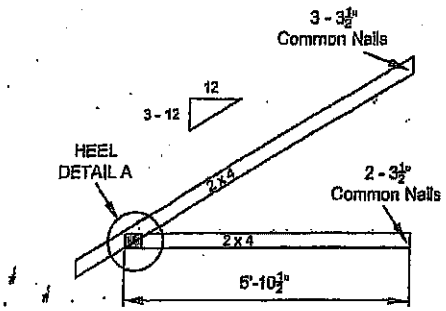


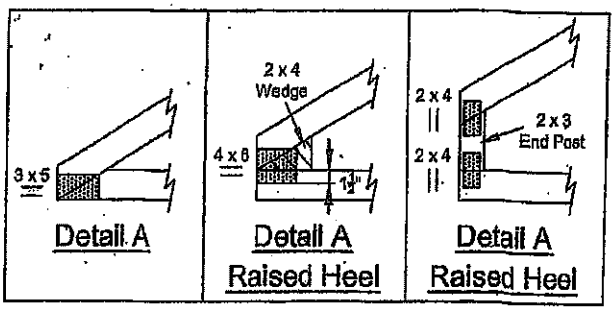
Corner Side Jacks



Corner End Jacks



Common End Jacks



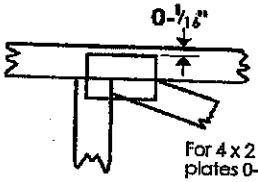
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

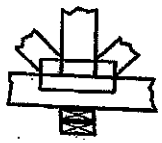
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

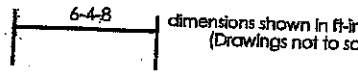


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

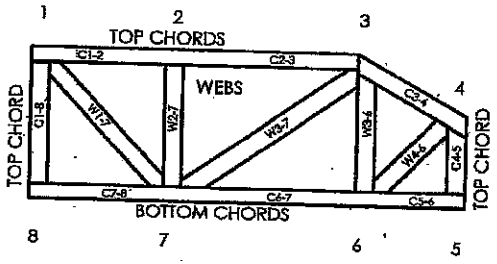
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS
CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

LICENSED PROFESSIONAL ENGINEER
06/11/18
H.J.G. ALVES
PROVINCE OF ONTARIO

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

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BUILDING SECTION
FILE NO

T-1800218 Feb 09, 2018