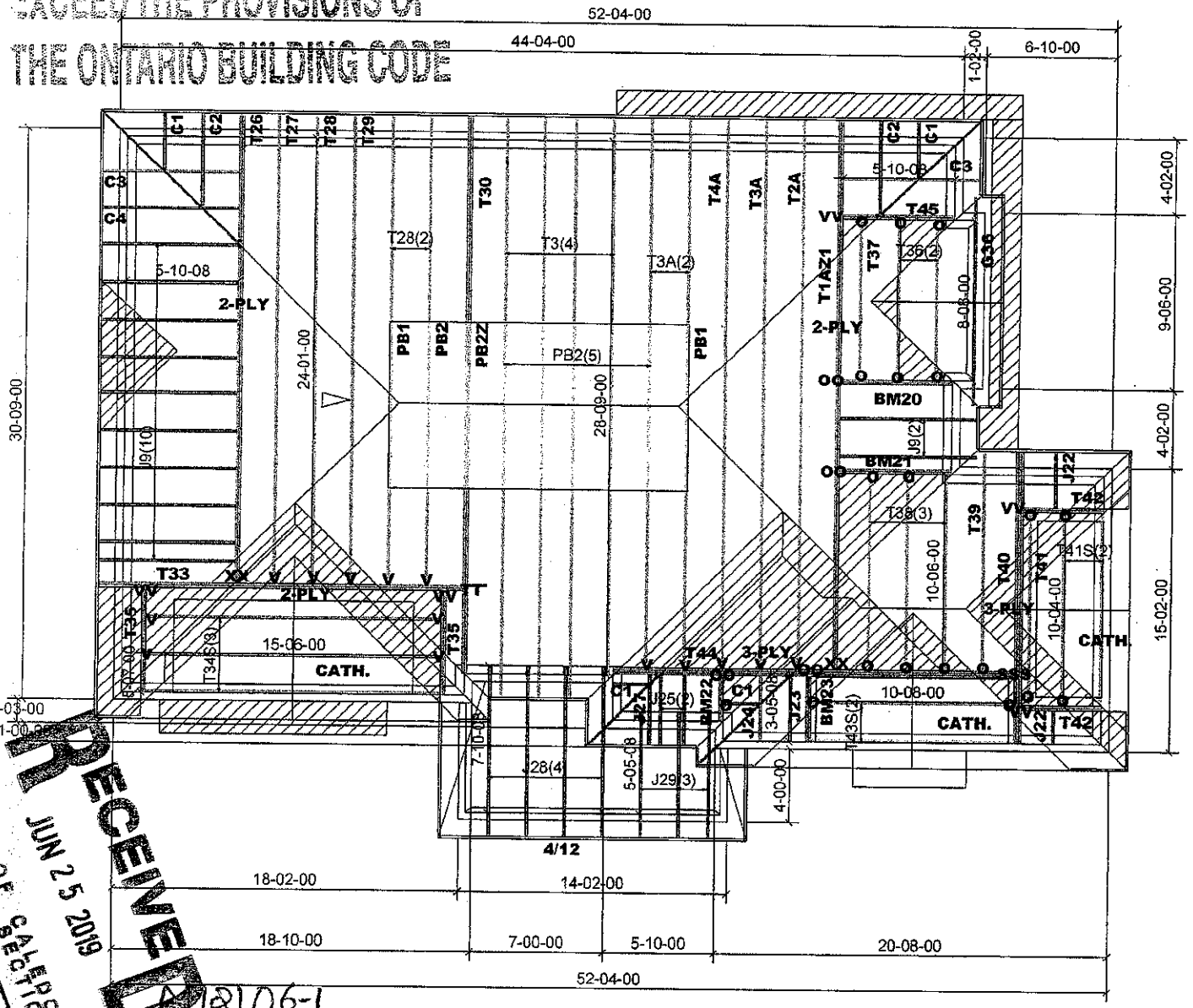
THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O).
LJS26DS - (V)
LUS26-2(VV)
LUS24-2(OO)
HGUS26-2 - (XX)
HGUS28-3(SSS)
THGQ2-SDS 3 (MAX) (TT)

DENOTES CONV. FRAMING

GSL 37.6 PSF

DESIGN CONFORMS WITH O.B.C. 2018 PART 9
DESIGN LOADS:

TC LIVE	29 PSF
TC DEAD	6 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

BM20~23 : 2- 2x10

All Pitches are 9/12 unless noted

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE A12106-1

TAMARACK LUMBER INC.

Job Track:	50120
Plan Log:	200778
Layout ID:	402114

Builder / Location: **GREEN PARK HOMES / Caledon**

Model / Elevation: **PRESTON 12 / 2**

Project: **Lamberts Lane Home Corp.PH2.**

Date: 4/5/2019 Sales: Mario DiCano Designer: chris /JG

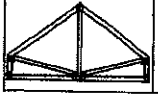
Mitek ver 8.2.3.229

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
Builder:	GREEN PARK HOMES	PlanLog:	200778
Project:	Lamberts Lane Home Corp.	Layout ID:	402114
Location:	Caledon	Ref #	
Model:	PRESTON 12	Page:	1 of 4
Lot #:		Date:	04/05/2019
Elevation:	2	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1AZ1 Hip Girder	9 /12	**28-04-00**	5-11-02	2 x 4 2 x 6	1-03-08	1-06-04 1-10-00	280.4 180.00		
	1	T2A Hip	9 /12	**28-04-00**	7-05-02	2 x 4	1-03-08	1-06-04 1-10-00	126.61 79.83		
	4	T3 Piggyback Base	9 /12	**28-09-00**	8-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	559.19 354.00		
	3	T3A Hip	9 /12	**28-04-00**	8-11-02	2 x 4	1-03-08	1-06-04 1-10-00	410.96 261.50		
	1	T4A Hip	9 /12	**28-04-00**	10-05-02	2 x 4	1-03-08	1-06-04 1-10-00	135.77 84.00		
	1 2-ply	T26 Hip Girder	9 /12	**24-01-00**	5-11-02	2 x 4 2 x 6	1-03-08	1-06-04 5-00-04	250.51 161.67		
	1	T27 Hip	9 /12	**24-01-00**	7-05-02	2 x 4	1-03-08	1-06-04 5-00-04	113.58 71.83		
	3	T28 Hip	9 /12	**24-01-00**	8-11-02	2 x 4	1-03-08	1-06-04 5-00-04	366.82 232.50		
	1	T29 Hip	9 /12	**24-01-00**	10-05-02	2 x 4	1-03-08	1-06-04 5-00-04	126.05 78.00		
	1 2-ply	T30 Piggyback Base Girder	9 /12	**28-09-00**	8-11-06	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	406.46 256.33		
	1 2-ply	T33 Hip Girder	9 /12	**17-10-08**	5-07-10	2 x 6	1-03-08	1-06-04 1-07-06	216.16 132.33		
	3	T34S Roof Special	9 /12 7 /12	**15-06-00**	6-11-14	2 x 4		1-02-02 1-02-02	194.31 130.50		
	2 2-ply	T35 Half Hip Girder	9 /12	**5-05-08**	2-03-04	2 x 4 2 x 6		1-06-04 2-03-04	99.62 68.67		
	2	T36 Common	9 /12	**8-08-00**	4-09-04	2 x 4		1-06-04 1-06-04	70.41 48.33		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
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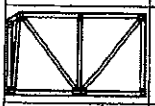
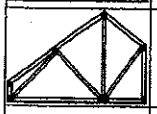
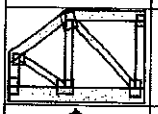
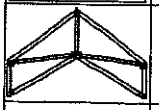
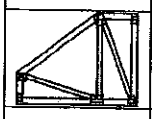
Location:	Caledon	Ref #	
Model:	PRESTON 12	Page:	2 of 4
Lot #:		Date:	04/05/2019
Elevation:	2	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G36 GABLE	9 /12	8-08-00	4-09-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	40.24 27.83		
	1	T37 Half Hip	9 /12	8-08-00	5-01-12	2 x 4		4-07-12 5-01-12	47.22 30.33		
	3	T38 Common	9 /12	10-06-00	6-10-12	2 x 4		1-06-04 4-04-12	150.88 97.00		
	1	T39 Hip	9 /12	10-06-00	6-02-02	2 x 4	1-03-08	1-06-04 4-04-12	55.25 35.50		
	1 3-ply	T40 Hip Girder	9 /12	14-04-00	4-08-02	2 x 4 2 x 8	1-03-08 1-03-08	1-06-04 1-06-04	246.58 159.00		
	1	T41 Common	9 /12	10-04-00	5-04-12	2 x 4		1-06-04 1-06-04	41.31 26.83		
	2	T41S Scissor	9 /12 7 /12	10-04-00	5-04-12	2 x 4		1-06-04 1-06-04	82.97 55.00		
	2 2-ply	T42 Half Hip Girder	9 /12	4-02-08	2-10-00	2 x 4 2 x 6		1-06-04 2-10-00	87.75 62.67		
	2	T43S Scissor	9 /12 7 /12	10-08-00	6-08-08	2 x 4		2-08-08 2-08-08	91.76 58.67		
	1 3-ply	T44 Roof Special Girder	9 /12	21-01-00	4-01-06	2 x 6		1-06-04 4-01-06	341.33 216.00		
	1 2-ply	T45 Jack-Closed Girder	9 /12	5-10-08	4-05-08	2 x 4		1-06-04 4-05-08	60.06 39.67		
	2	PB1 Piggyback	9 /12	9-00-00	3-00-00	2 x 4			50.12 31.33		
	6	PB2 Piggyback	9 /12	9-00-00	3-04-08	2 x 4			141.91 89.00		
	1 2-ply	PB2Z Piggyback	9 /12	9-00-00	3-04-08	2 x 4			47.3 29.67		

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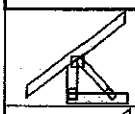
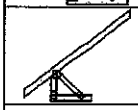
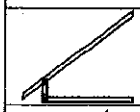
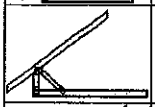
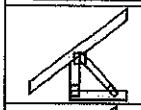
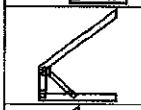
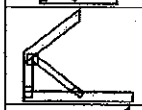
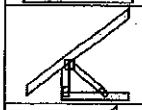
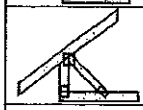
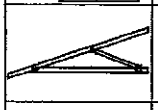
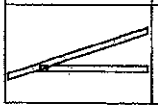
Location:	Caledon	Ref #	
Model:	PRESTON 12	Page:	3 of 4
Lot #:		Date:	04/05/2019
Elevation:	2	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	C1 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	37.25 28.00		
	2	C2 Jack-Open	9 /12	1-10-08	4-04-05	2 x 4	1-03-08 1-10-15	1-06-04 2-11-02	24.44 16.67		
	2	C3 Jack-Open Girder	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 4-01-01	1-06-04 2-10-05	27.75 19.33		
	12	J9 Jack-Open	9 /12	5-10-08	5-11-02	2 x 4	1-03-08	1-06-04 5-11-02	220.79 144.00		
	1	C4 Jack-Open	9 /12	3-09-07	4-04-05	2 x 4	1-03-08 2-01-01	1-06-04 4-04-05	16.84 11.00		
	2	J22 Jack-Open	9 /12	1-09-00	2-10-00	2 x 4	1-03-08	1-06-04 2-10-00	18.19 14.00		
	1	J23 Jack-Open	9 /12	3-05-08	4-01-06	2 x 4		1-06-04 4-01-06	11.77 8.33		
	1	J24 Jack-Open Girder	9 /12	3-05-08	2-10-05	2 x 4		1-06-04 4-01-06	9.55 7.00		
	2	J25 Jack-Open	9 /12	2-05-08	3-04-06	2 x 4	1-03-08	1-06-04 3-04-06	22.1 15.33		
	1	J27 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 8-01	1-06-04 2-10-05	9.98 7.67		
	4	J28 Jack-Open	4 /12	7-10-08	3-04-06	2 x 4	1-03-08	3-15 2-11-07	91.87 56.00		
	3	J29 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	43.35 28.00		

TOTAL # TRUSS= **99** TOTAL BFT OF ALL TRUSSES= **3453.32** BFT. TOTAL WEIGHT OF ALL TRSSES **5375.63** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
1	Hardware	HGUS28-3	

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
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Location:	Caledon	Ref #	
Model:	PRESTON 12	Page:	4 of 4
Lot #:		Date:	04/05/2019
Elevation:	2	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	LJS26DS	
19	Hardware	LUS24	
3	Hardware	LUS24-2	
5	Hardware	LUS26-2	
1	Hardware	THGQ2 SDS 3 MAX	

TOTAL NUMBER OF ITEMS= **40**

TECHNICAL BULLETIN

THGQ/THGQH – Truss Girder Hangers

SIMPSON Strong-Tie®

MATERIAL: THGQ—7 gauge, THGQH—3 gauge

FINISH: THGQ—G90 Galvanized,
THGQH—Simpson Strong-Tie® gray paint

DESIGN:

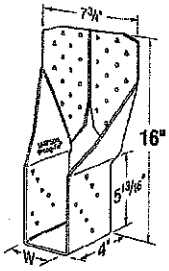
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% for short term load duration. No further increase is allowed.
- Designer must ensure that vertical web member supporting a hanger is capable of resisting loads based on net cross section
- Girder truss must be a minimum of 2 plys
- Bearing assumes Q_r/A_b and Q'_r/A'_b = 812 psi D.Fir-L and 615 psi S-P-F. Truss plates on supported member must be as per 6.5.4 and 7.5.9 TPIC 2014 to archieve values shown.
- All multiple members must be fastened together to act as a single unit independent of the hanger fasteners
- Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses

INSTALLATION:

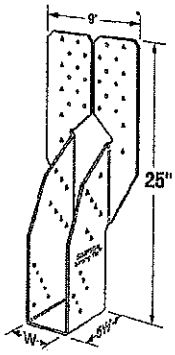
- Use all specified fasteners
- Fill all round holes for min values and all round and triangle holes for max values
- Strong-Drive® SDS screws driven through truss plates must be approved by the truss designer. Pre-drilling, using a 5⁄32" bit, is required.
- Connector must be installed, centred on girder vertical web.

OPTIONS:

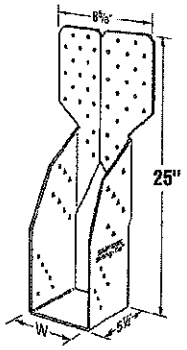
- These hangers may be skewed 45°
- See current catalogue for options



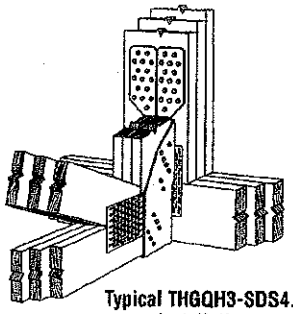
THGQ2-SDS3
(THGQ3-SDS3 Similar)



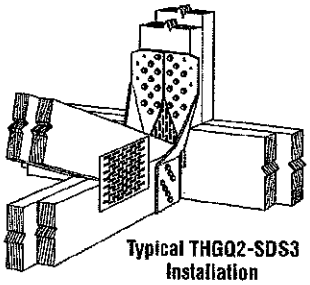
THGQH2-SDS3



THGQH3-SDS4.5
(THGQH4-SDS6 Similar)



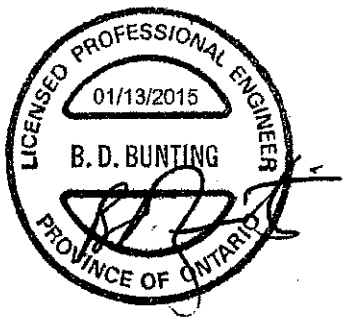
Typical THGQH3-SDS4.5 Installation



Typical THGQ2-SDS3 Installation

Model No.	Width W (in)	Max. B.C. Depth	Min Vert Web Size	Fasteners		Factored Resistance (lbs)			
						D.Fir-L		S-P-F	
				Header	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
THGQ2-SDS3 (Min)	3⁵⁄₁₆	2x6	2x8	22-SDS¼"x3"	10-SDS¼"x3"	5205	11655	3750	8395
THGQ2-SDS3 (Max)	3⁵⁄₁₆	2x6	2x10	28-SDS¼"x3"	14-SDS¼"x3"	6555	18055	4720	13000
THGQH2-SDS3 (Min)	3⁵⁄₁₆	2x10	2x8	18-SDS¼"x3"	14-SDS¼"x3"	5790	12555	4170	9040
THGQH2-SDS3 (Max)	3⁵⁄₁₆	2x10	2x10	28-SDS¼"x3"	26-SDS¼"x3"	14190	18455	10215	15160
THGQ3-SDS4.5 (Min)	4¹⁵⁄₁₆	2x6	2x8	22-SDS¼"x4½"	10-SDS¼"x4½"	5205	11655	3750	8395
THGQ3-SDS4.5 (Max)	4¹⁵⁄₁₆	2x6	2x10	28-SDS¼"x4½"	14-SDS¼"x4½"	6555	17760	4720	12785
THGQH3-SDS4.5 (Min)	4¹⁵⁄₁₆	2x10	2x10	32-SDS¼"x4½"	14-SDS¼"x4½"	5790	17860	4170	12860
THGQH3-SDS4.5 (Max)	4¹⁵⁄₁₆	2x10	2x12	38-SDS¼"x4½"	26-SDS¼"x4½"	14190	21055	10215	15160
THGQH4-SDS6 (Min)	6⁹⁄₁₆	2x12	2x10	34-SDS¼"x6"	14-SDS¼"x6"	5790	17860	4170	12860
THGQH4-SDS6 (Max)	6⁹⁄₁₆	2x12	2x12	40-SDS¼"x6"	26-SDS¼"x6"	14190	24870	10215	17905

1. Factored resistances have been increased 15% for earthquake or wind load with no further increase allowed; reduce where other loads govern.
2. Minimum 2-ply girder required.
3. Connector must be installed centred on girder vertical webs.
4. The thickness of the supporting girder must be equal to or greater than the screw length. For applications where the length of the supplied screws exceeds the thickness of the supporting girder, 3" or 4½" screws may be substituted for the longer length screws with no load reduction, or a shim block may used as approved by the Designer.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

T-SPECTHGQ15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
402114	T1AZ1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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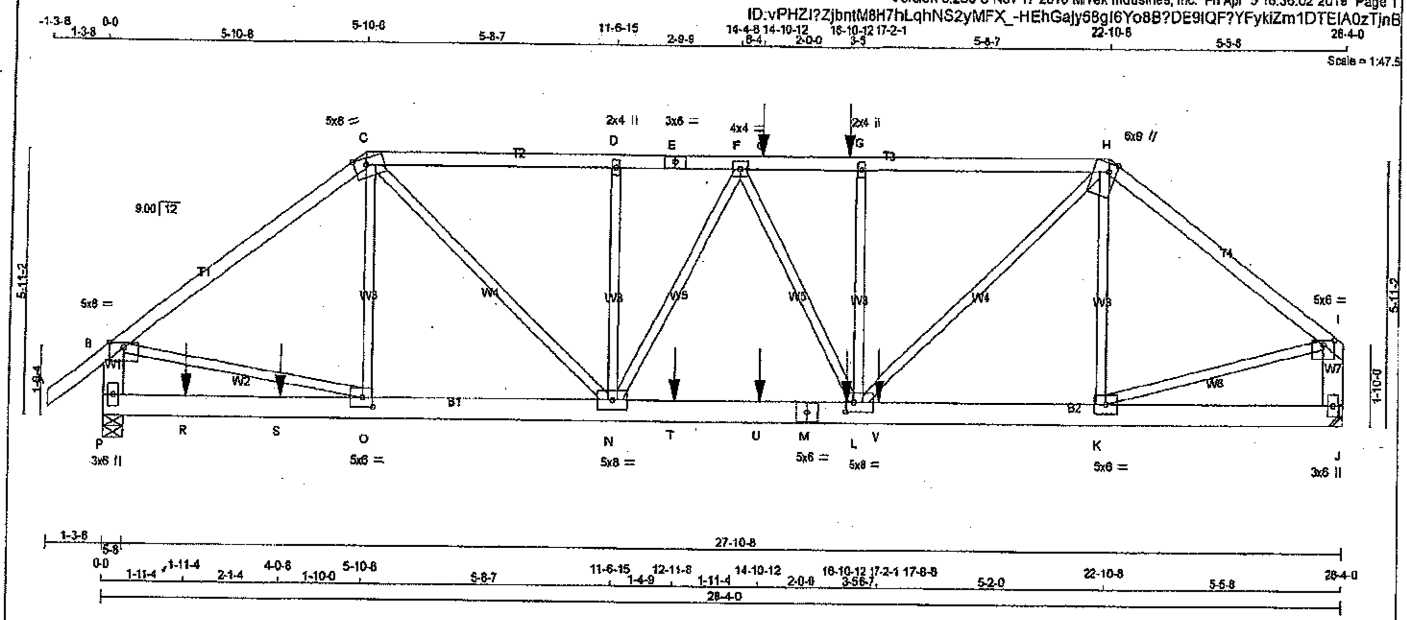

TOTAL WEIGHT = 2 X 140 = 280 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- I	1	12	TOP
P- B	2	12	TOP
J- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- M	2	12	SIDE(0.0)
M- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMVW-p	MT20	5.0	6.0	1.25	3.00
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0	2.50	2.25
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.75
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	4671	0	4671	0	0	5-8	5-8
J	3683	0	3683	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	3458	2013 / 0	618 / 0	0 / 0	0 / 0	828 / 0	0 / 0
J	2736	1563 / 0	510 / 0	0 / 0	0 / 0	663 / 0	0 / 0

(MAX./MIN. COMPONENT REACTIONS)

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.08 (1)	10.00
B- C	-4998 / 0	-102.1	-102.1	0.62 (1)	3.85
C- D	-5789 / 0	-102.1	-102.1	0.43 (1)	3.73
D- E	-5789 / 0	-102.1	-102.1	0.32 (1)	3.75
E- F	-5789 / 0	-102.1	-102.1	0.32 (1)	3.75
F- Q	-5869 / 0	-102.1	-102.1	0.33 (1)	3.72
Q- G	-5869 / 0	-102.1	-102.1	0.33 (1)	3.72
G- H	-5869 / 0	-102.1	-102.1	0.43 (1)	3.70
H- I	-4123 / 0	-102.1	-102.1	0.45 (1)	4.29
P- B	-4186 / 0	0.0	0.0	0.15 (1)	6.97
J- I	-3601 / 0	0.0	0.0	0.14 (1)	7.40
P- R	0 / 0	-38.5	-38.5	0.54 (1)	10.00
R- S	0 / 0	-38.5	-38.5	0.54 (1)	10.00
S- O	0 / 0	-38.5	-38.5	0.54 (1)	10.00
O- N	0 / 4006	-38.5	-38.5	0.57 (1)	10.00
N- T	0 / 5875	-38.5	-38.5	0.78 (1)	10.00
T- U	0 / 5875	-38.5	-38.5	0.78 (1)	10.00
U- M	0 / 5875	-38.5	-38.5	0.78 (1)	10.00
M- L	0 / 5875	-38.5	-38.5	0.78 (1)	10.00
L- V	0 / 3283	-38.5	-38.5	0.43 (1)	10.00
V- K	0 / 3283	-30.5	-38.5	0.43 (1)	10.00
K- J	0 / 0	-38.5	-38.5	0.06 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	0 / 714	0.09 (1)
C- N	0 / 2465	0.31 (1)
N- D	-564 / 0	0.15 (1)
L- G	-709 / 0	0.18 (1)
L- H	0 / 3604	0.45 (1)
K- H	-571 / 0	0.15 (1)
B- O	0 / 4061	0.50 (1)
K- I	[illegible]	[illegible] (1)
N- F	[illegible] / 0	0.06 (1)
F- L	[illegible]	[illegible]

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	16-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
L	16-10-12	-57	-73	—	BACK	VERT	TOTAL	—	—
Q	14-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
R	1-11-4	-75	-95	—	BACK	VERT	TOTAL	—	—
S	4-0-8	-1419	-1419	—	BACK	VERT	TOTAL	—	—
T	12-11-8	-1223	-1223	—	BACK	VERT	TOTAL	—	—
U	14-10-12	-57	-73	—	BACK	VERT	TOTAL	—	—
V	17-6-8	-1154	-1154	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.78/1.00 (L-N:1) , WB=0.50/1.00 (B-O:1) , SSI=0.41/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.50 (M) (INPUT = 1.00)

DWG NO. TAM 77907797
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
402114	T2A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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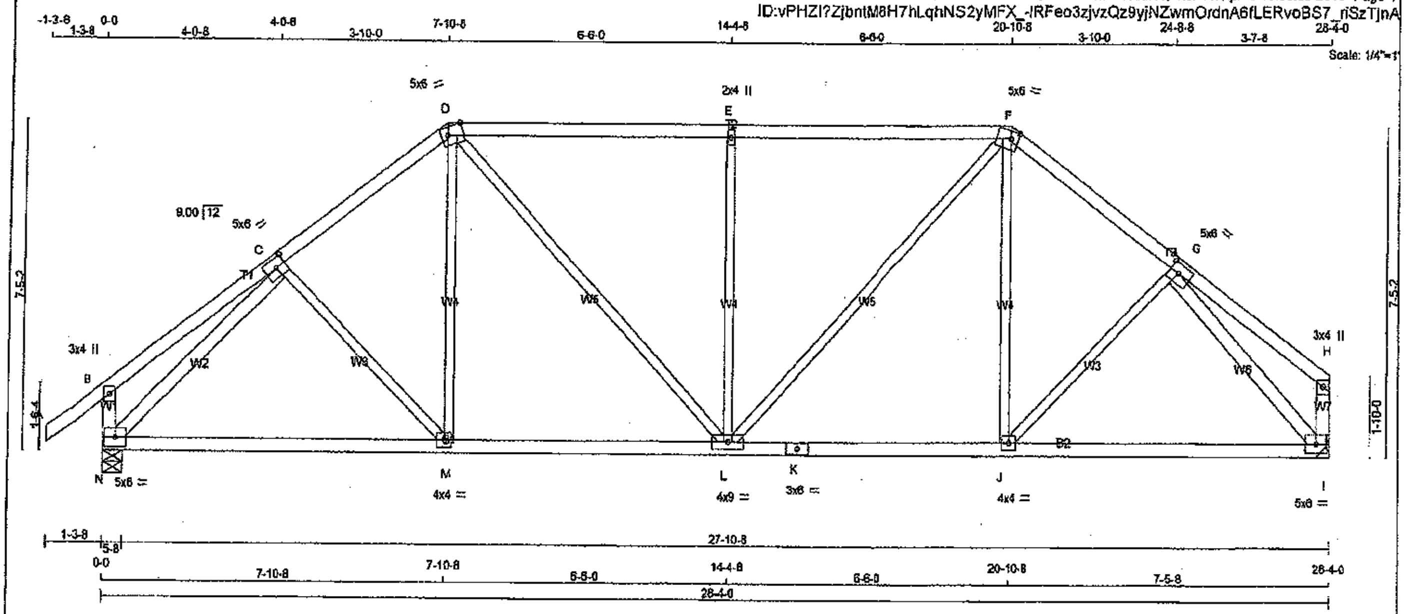

TOTAL WEIGHT = 127 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - C	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	6.0	Edge	4.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.00
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2133	0	2133	0	0	5-8	5-8
I	1992	0	1992	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1585	903 / 0	298 / 0	0 / 0	0 / 0	385 / 0	0 / 0
I	1488	823 / 0	298 / 0	0 / 0	0 / 0	388 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 27	-102.1	-102.1	0.24 (1)	10.00
C- D	-2058 / 0	-102.1	-102.1	0.24 (1)	4.52
D- E	-2060 / 0	-102.1	-102.1	0.63 (1)	3.96
E- F	-2060 / 0	-102.1	-102.1	0.63 (1)	3.96
F- G	-1987 / 0	-102.1	-102.1	0.23 (1)	4.60
G- H	0 / 27	-102.1	-102.1	0.22 (1)	10.00
N- B	-298 / 0	0.0	0.0	0.03 (1)	7.81
I- H	-133 / 0	0.0	0.0	0.01 (1)	7.81
N- M	0 / 1624	-38.5	-38.5	0.63 (2)	10.00
M- L	0 / 1626	-38.5	-38.5	0.63 (2)	10.00
L- K	0 / 1570	-38.5	-38.5	0.60 (2)	10.00
K- J	0 / 1570	-38.5	-38.5	0.60 (2)	10.00
J- I	0 / 1476	-38.5	-38.5	0.58 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-3 / 110	0.03 (3)
M- D	0 / 383	0.09 (2)
D- L	0 / 644	0.14 (1)
L- E	-813 / 0	0.80 (1)
L- F	0 / 727	0.16 (1)
J- F	0 / 291	0.07 (3)
J- G	0 / 163	0.04 (3)
N- C	-2360 / 0	0.90 (1)
G- I	-2283 / 0	0.77 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	29.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.63/1.00 (L-M:2) , WB=0.90/1.00 (C-N:1) , SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.58 (C) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402113	T3	11	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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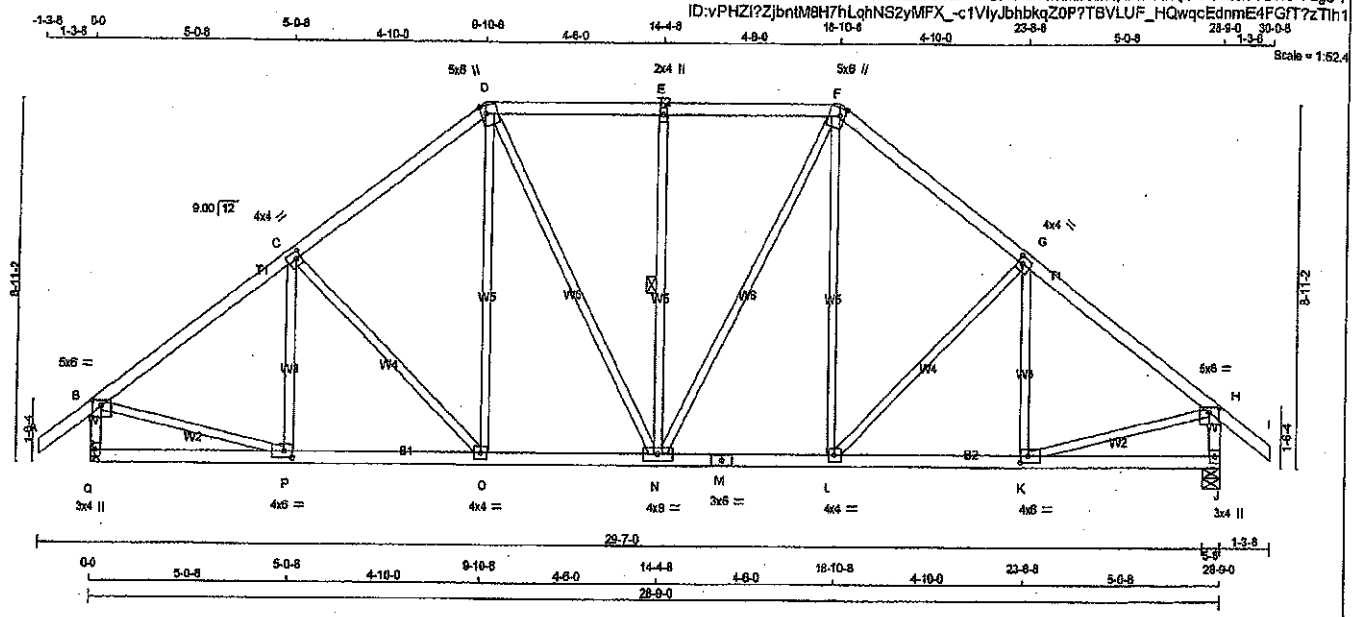

TOTAL WEIGHT = 11 X 140 = 1538 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.25
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2162	0	2162	0	0	MECHANICAL	
J	2162	0	2162	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT Q. MINIMUM BEARING LENGTH AT JOINT Q = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1607	915 / 0	302 / 0	0 / 0	0 / 0	390 / 0	0 / 0
J	1607	915 / 0	302 / 0	0 / 0	0 / 0	390 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2172 / 0	-102.1	-102.1	0.40 (1)	4.25	
C-D	-1945 / 0	-102.1	-102.1	0.38 (1)	4.47	
D-E	-1716 / 0	-102.1	-102.1	0.29 (1)	4.79	
E-F	-1716 / 0	-102.1	-102.1	0.29 (1)	4.79	
F-G	-1945 / 0	-102.1	-102.1	0.38 (1)	4.47	
G-H	-2172 / 0	-102.1	-102.1	0.40 (1)	4.25	
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
Q-B	-2082 / 0	0.0	0.0	0.22 (1)	5.67	
J-H	-2082 / 0	0.0	0.0	0.22 (1)	5.67	
Q-P	0 / 0	-38.5	-38.5	0.18 (3)	10.00	
P-O	0 / 1766	-38.5	-38.5	0.40 (2)	10.00	
O-N	0 / 1529	-38.5	-38.5	0.33 (1)	10.00	
N-M	0 / 1529	-38.5	-38.5	0.33 (1)	10.00	
M-L	0 / 1529	-38.5	-38.5	0.33 (1)	10.00	
L-K	0 / 1766	-38.5	-38.5	0.40 (2)	10.00	
K-J	0 / 0	-38.5	-38.5	0.18 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-205 / 88	0.09 (1)
C-O	-343 / 0	0.32 (1)
O-D	0 / 431	0.10 (2)
D-N	0 / 405	0.09 (1)
N-E	-553 / 0	0.26 (1)
N-F	0 / 405	0.09 (1)
L-F	0 / 431	0.10 (2)
L-G	-343 / 0	0.32 (1)
K-G	-205 / 88	0.09 (1)
B-P	0 / 1814	0.41 (1)
K-H	0 / 1814	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.40/1.00 (G-H:1) , BC=0.40/1.00 (K-L:2) , WB=0.41/1.00 (B-P:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1867	1373

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LUMBER GRIP= 0.86 (F) (INPUT = 0.90)
PLATE/METAL= 0.50 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM 1907764
STRUCTURAL
COMPONENT ONLY

RECEIVED
JUN 25 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T3A	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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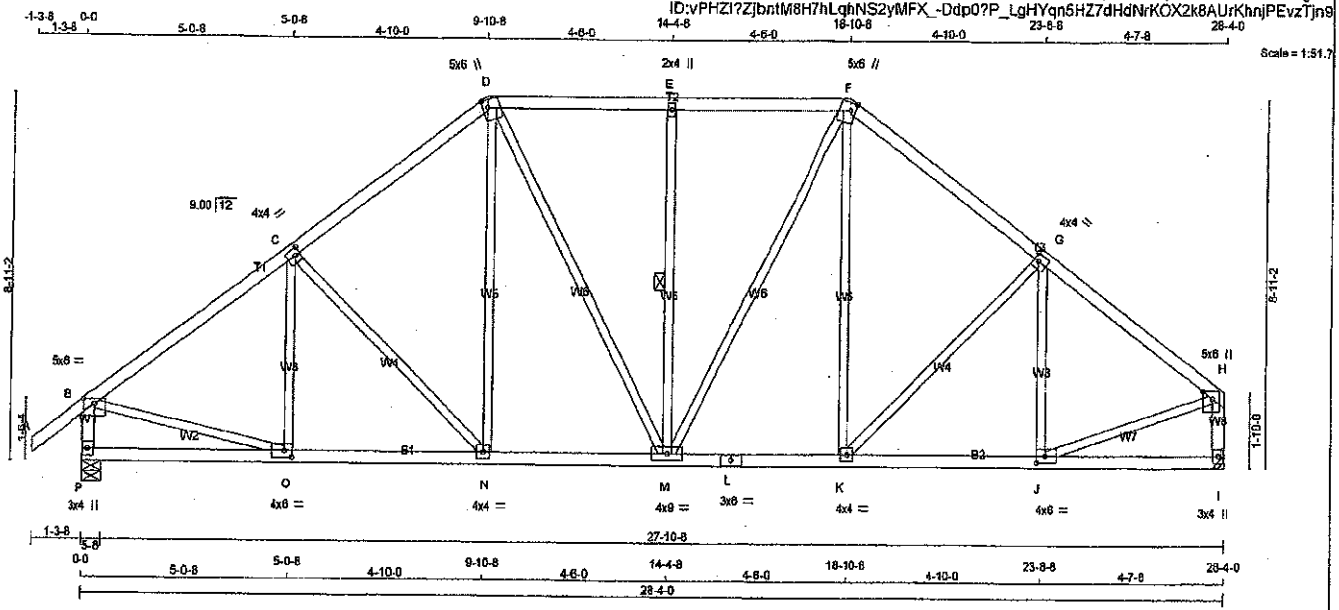

TOTAL WEIGHT = 3 X 137 = 411 lb

(M|F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.50
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	6.0	2.00	2.25
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	RECMD BRG
P	2133	0	2133	0	0	5-8	5-8
I	1992	0	1992	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1585	903 / 0	298 / 0	0 / 0	0 / 0	395 / 0	0 / 0
I	1488	823 / 0	298 / 0	0 / 0	0 / 0	366 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B- C	-2135 / 0	-102.1	-102.1	0.40 (1)	4.28	
C- D	-1903 / 0	-102.1	-102.1	0.37 (1)	4.51	
D- E	-1667 / 0	-102.1	-102.1	0.29 (1)	4.85	
E- F	-1667 / 0	-102.1	-102.1	0.29 (1)	4.85	
F- G	-1864 / 0	-102.1	-102.1	0.34 (1)	4.59	
G- H	-1989 / 0	-102.1	-102.1	0.35 (1)	4.46	
P- B	-2053 / 0	0.0	0.0	0.21 (1)	5.91	
I- H	-1919 / 0	0.0	0.0	0.21 (1)	6.07	
P- O	0 / 0	-38.5	-38.5	0.16 (3)	10.00	
O- N	0 / 1736	-38.5	-38.5	0.40 (2)	10.00	
N- M	0 / 1496	-38.5	-38.5	0.33 (1)	10.00	
M- L	0 / 1465	-38.5	-38.5	0.33 (1)	10.00	
L- K	0 / 1465	-38.5	-38.5	0.33 (1)	10.00	
K- J	0 / 1620	-38.5	-38.5	0.37 (2)	10.00	
J- I	0 / 0	-38.5	-38.5	0.16 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-198 / 91	0.08 (1)
C- N	-349 / 0	0.32 (1)
N- D	0 / 435	0.10 (2)
D- M	0 / 372	0.08 (1)
M- E	-553 / 0	0.28 (1)
M- F	0 / 437	0.10 (1)
K- F	0 / 362	0.08 (2)
K- G	-225 / 0	0.21 (1)
J- G	-325 / 24	0.14 (1)
B- O	0 / 764	0.40 (1)
J- H	0 / 1638	0.85 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.40/1.00 (N-O:2) , WB=0.40/1.00 (B-O:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.48 (L) (INPUT = 1.00)

DWG NO. TAM 1907799
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G ALVES
100009821
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T4A	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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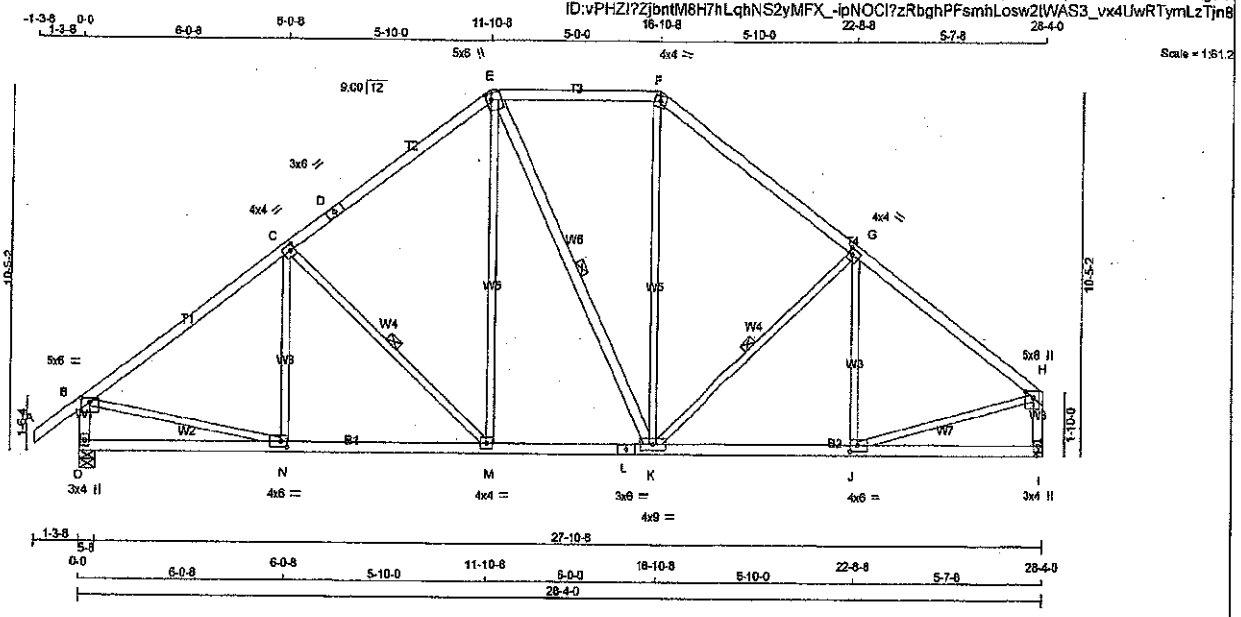


TOTAL WEIGHT = 136 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTVW+m	MT20	5.0	6.0	2.25	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0	2.00	2.50
K	BMWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0	2.00	2.00
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2133	0	2133	0	0	5-6	5-6
I	1992	0	1992	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1585	903 / 0	298 / 0	0 / 0	0 / 0	385 / 0	0 / 0
I	1488	823 / 0	298 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, E-K, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

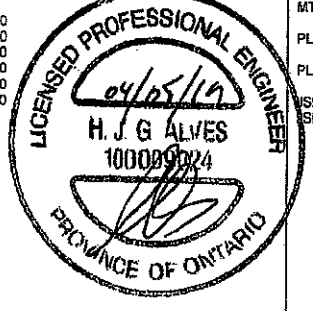
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	-2149 / 0	-102.1	-102.1	0.60 (1)	4.03
C-D	-1748 / 0	-102.1	-102.1	0.54 (1)	4.44
D-E	-1748 / 0	-102.1	-102.1	0.54 (1)	4.44
E-F	-1354 / 0	-102.1	-102.1	0.35 (1)	5.17
F-G	-1730 / 0	-102.1	-102.1	0.51 (1)	4.50
G-H	-2033 / 0	-102.1	-102.1	0.52 (1)	4.21
O-B	-2039 / 0	0.0	0.0	0.21 (1)	5.92
I-H	-1905 / 0	0.0	0.0	0.21 (1)	6.09
O-N	0 / 0	-38.5	-38.5	0.27 (3)	10.00
N-M	0 / 1754	-38.5	-38.5	0.49 (2)	10.00
M-L	0 / 1367	-38.5	-38.5	0.35 (2)	10.00
L-K	0 / 1367	-38.5	-38.5	0.35 (2)	10.00
K-J	0 / 1661	-38.5	-38.5	0.45 (2)	10.00
J-I	0 / 0	-38.5	-38.5	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-83 / 181	0.05 (1)
C-M	-547 / 0	0.25 (1)
M-E	0 / 573	0.13 (1)
E-K	-29 / 0	0.02 (1)
K-F	0 / 538	0.12 (2)
K-G	-434 / 0	0.20 (1)
J-G	-195 / 121	0.11 (1)
B-N	0 / 1788	0.40 (1)
J-H	0 / 1710	0.39 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.49/1.00 (M-N:2) , WB=0.40/1.00 (B-N:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.90 (J) (INPUT = 0.90)
SSI METAL= 0.46 (L) (INPUT = 1.00)

DWG NO. TAM 1907800
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T26	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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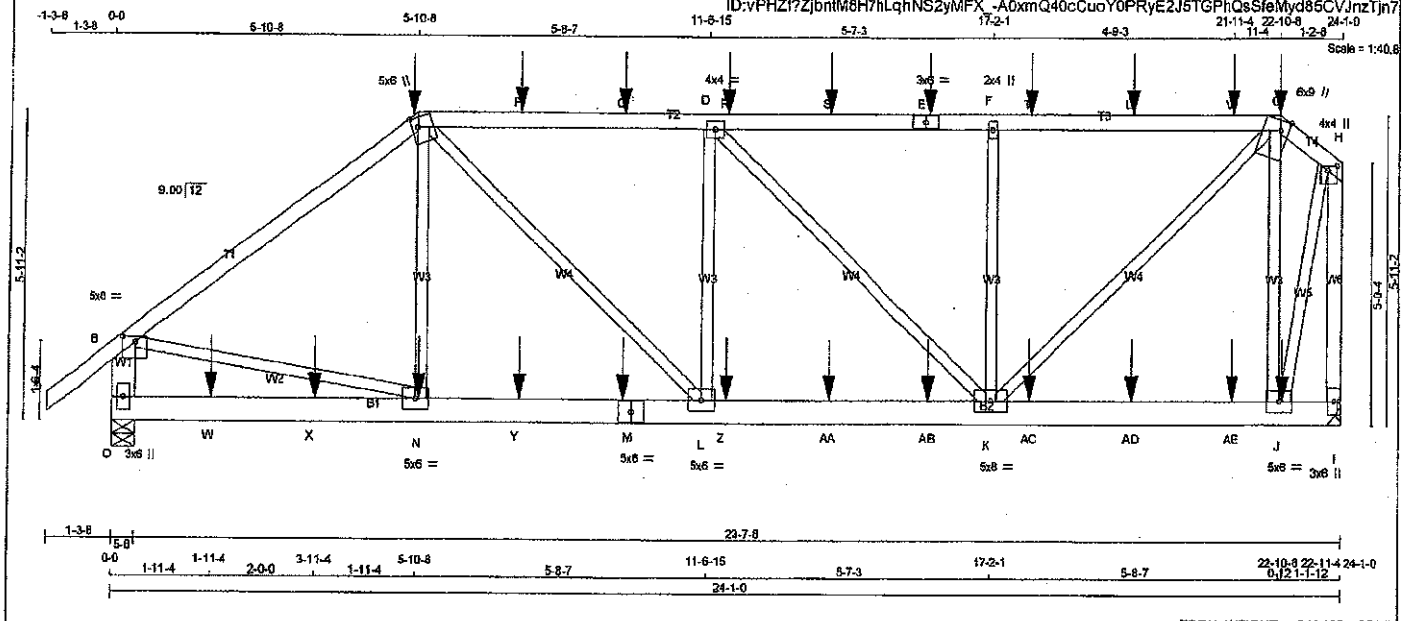

TOTAL WEIGHT = 2 X 125 = 251 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-E	1	12	SIDE(61.0)
E-G	1	12	SIDE(61.0)
G-H	1	12	SIDE(61.0)
I-H	1	12	TOP
O-B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-M	2	12	SIDE(183.1)
M-I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
N-C	1	6	SIDE(7.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	2.00
H	TMW+p	MT20	4.0	4.0	1.00	2.25
I	BMV1+p	MT20	3.0	6.0		
J, L, N						
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2938	0	2938	0	0	5-8	5-8
I	2920	0	2920	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2198	1205 / 0	443 / 0	0 / 0	0 / 0	548 / 0	0 / 0
I	2190	1182 / 0	455 / 0	0 / 0	0 / 0	554 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00
B-C	-3122 / 0	-102.1	-102.1	0.46 (1)	4.78
C-P	-3276 / 0	-102.1	-102.1	0.47 (1)	4.60
P-Q	-3276 / 0	-102.1	-102.1	0.47 (1)	4.60
Q-D	-3276 / 0	-102.1	-102.1	0.47 (1)	4.60
D-R	-2682 / 0	-102.1	-102.1	0.45 (1)	4.99
R-S	-2682 / 0	-102.1	-102.1	0.45 (1)	4.99
S-E	-2682 / 0	-102.1	-102.1	0.45 (1)	4.99
E-F	-2682 / 0	-102.1	-102.1	0.45 (1)	4.99
F-T	-2682 / 0	-102.1	-102.1	0.45 (1)	4.99
T-U	-2682 / 0	-102.1	-102.1	0.45 (1)	4.99
U-V	-2682 / 0	-102.1	-102.1	0.45 (1)	4.99
V-G	-2682 / 0	-102.1	-102.1	0.45 (1)	4.99
G-H	-751 / 0	-102.1	-102.1	0.02 (1)	6.25
O-B	-2781 / 0	0.0	0.0	0.10 (1)	7.81
I-H	-2932 / 0	0.0	0.0	0.63 (1)	6.72
O-W	0 / 0	-38.5	-38.5	0.11 (3)	10.00
W-X	0 / 0	-38.5	-38.5	0.11 (3)	10.00
X-N	0 / 0	-38.5	-38.5	0.11 (3)	10.00
N-Y	0 / 2494	-38.5	-38.5	0.24 (2)	10.00
Y-M	0 / 2494	-38.5	-38.5	0.24 (2)	10.00
M-L	0 / 2494	-38.5	-38.5	0.24 (2)	10.00
L-Z	0 / 3276	-38.5	-38.5	0.27 (1)	10.00
Z-AA	0 / 3276	-38.5	-38.5	0.27 (1)	10.00
AA-AB	0 / 3276	-38.5	-38.5	0.27 (1)	10.00
AB-K	0 / 3276	-38.5	-38.5	0.27 (1)	10.00
K-AC	0 / 623	-38.5	-38.5	0.12 (2)	10.00
AC-AD	0 / 623	-38.5	-38.5	0.12 (2)	10.00
AD-AE	0 / 623	-38.5	-38.5	0.12 (2)	10.00
AE-J	0 / 623	-38.5	-38.5	0.12 (2)	10.00
J-I	0 / 0	-38.5	-38.5	0.00 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
N-C	-3 / 371	0.05 (3)
C-L	0 / 1091	0.13 (1)
L-D	-410 / 113	0.11 (1)
D-K	-837 / 0	0.53 (1)
K-F	-1065 / 0	0.28 (1)
K-G	0 / 2670	0.36 (1)
J-G	-2125 / 0	0.55 (1)
B-N	0 / 2536	0.31 (1)
J-H	0 / 2486	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.63/1.00 (H-I:1) , BC=0.27/1.00 (K-L:1) , WB=0.55/1.00 (G-J:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.34 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

DWG NO. TAM T1907801
STRUCTURAL
CONCURRENCE ONLY
1/2

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T26	1	2	GREEN PARK HOMES TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-495	-495	—	FRONT	VERT	TOTAL	—	—
E	15-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
G	22-10-8	-130	-130	—	FRONT	VERT	TOTAL	—	—
J	22-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
M	9-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
N	5-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
P	7-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
Q	9-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
R	11-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
S	13-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
T	17-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
U	19-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
V	21-11-4	-126	-126	—	FRONT	VERT	TOTAL	—	—
W	1-11-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
X	3-11-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
Y	7-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
Z	11-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AA	13-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AB	15-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AC	17-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AD	19-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AE	21-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. GONCALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 1907801
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T27	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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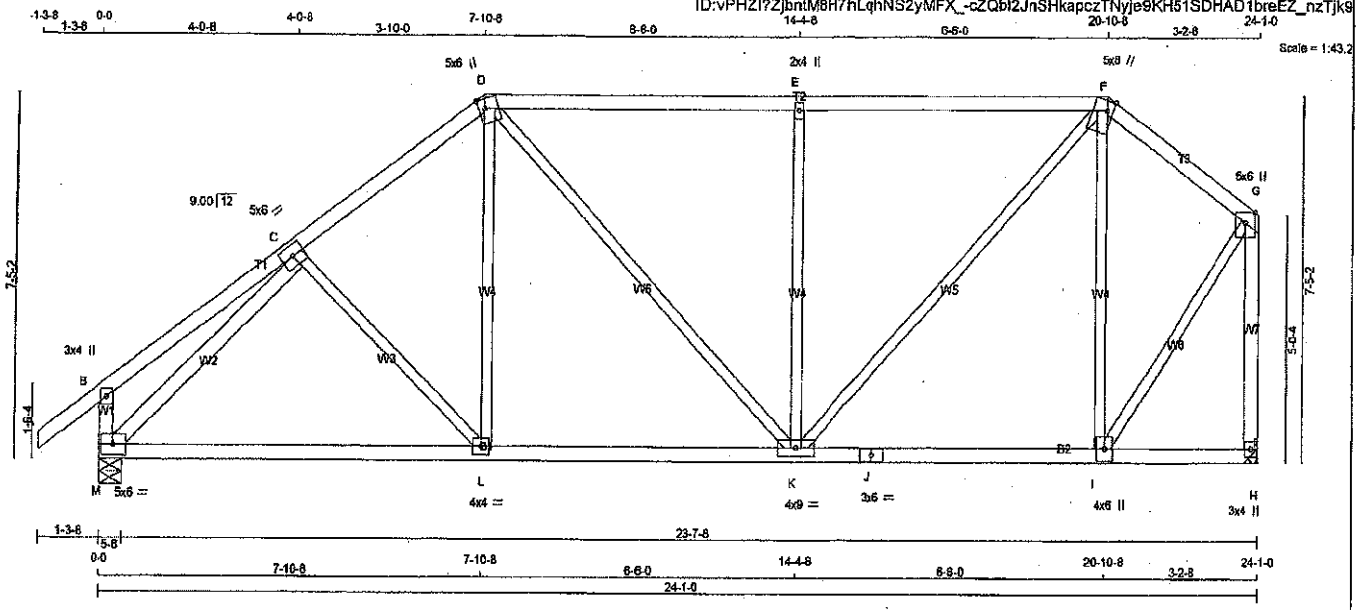

TOTAL WEIGHT = 114 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	8.0	Edge	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMVW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMVWW-t	MT20	4.0	9.0		
L	BMVW-t	MT20	4.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	RECMD BRG
M	1834	0	1834	0	0	5-8	5-8
H	1693	0	1693	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1362	779 / 0	253 / 0	0 / 0	0 / 0	330 / 0	0 / 0
H	1265	699 / 0	253 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 27	-102.1	-102.1	0.25 (1)	10.00
C- D	-1642 / 0	-102.1	-102.1	0.30 (1)	4.85
D- E	-1462 / 0	-102.1	-102.1	0.85 (1)	4.00
E- F	-1462 / 0	-102.1	-102.1	0.85 (1)	4.00
F- G	-665 / 0	-102.1	-102.1	0.19 (1)	6.25
M- B	-295 / 0	0.0	0.0	0.03 (1)	7.81
H- G	-1664 / 0	0.0	0.0	0.76 (1)	6.43
M- L	0 / 1340	-38.5	-38.5	0.58 (2)	10.00
L- K	0 / 1295	-38.5	-38.5	0.58 (2)	10.00
K- J	0 / 679	-38.5	-38.5	0.29 (2)	10.00
J- I	0 / 679	-38.5	-38.5	0.29 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.20 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-73 / 79	0.04 (1)
L- D	0 / 419	0.09 (2)
D- K	0 / 249	0.05 (1)
K- E	-917 / 0	0.81 (1)
K- F	0 / 1161	0.26 (1)
I- F	-755 / 0	0.75 (1)
M- C	-1948 / 0	0.74 (1)
I- G	0 / 1197	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.85/1.00 (D-E:1) , BC=0.58/1.00 (K-L:2) , WB=0.81/1.00 (E-K:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.87 (K) (INPUT = 0.90)
SI METAL= 0.48 (C) (INPUT = 1.00)

DWG NO. TAM T1907802
STRUCTURAL COMPONENT ONLY

THIS DESIGN IS FOR THE USE OF A BUILDING DESIGNER WHO IS RESPONSIBLE FOR DETERMINING THE SUITABILITY OF THIS DESIGN FOR THE INTENDED BUILDING. THE BUILDING DESIGNER IS RESPONSIBLE FOR THE INCORPORATION OF THE COMPONENT IN THE BUILDING DESIGN.

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T28	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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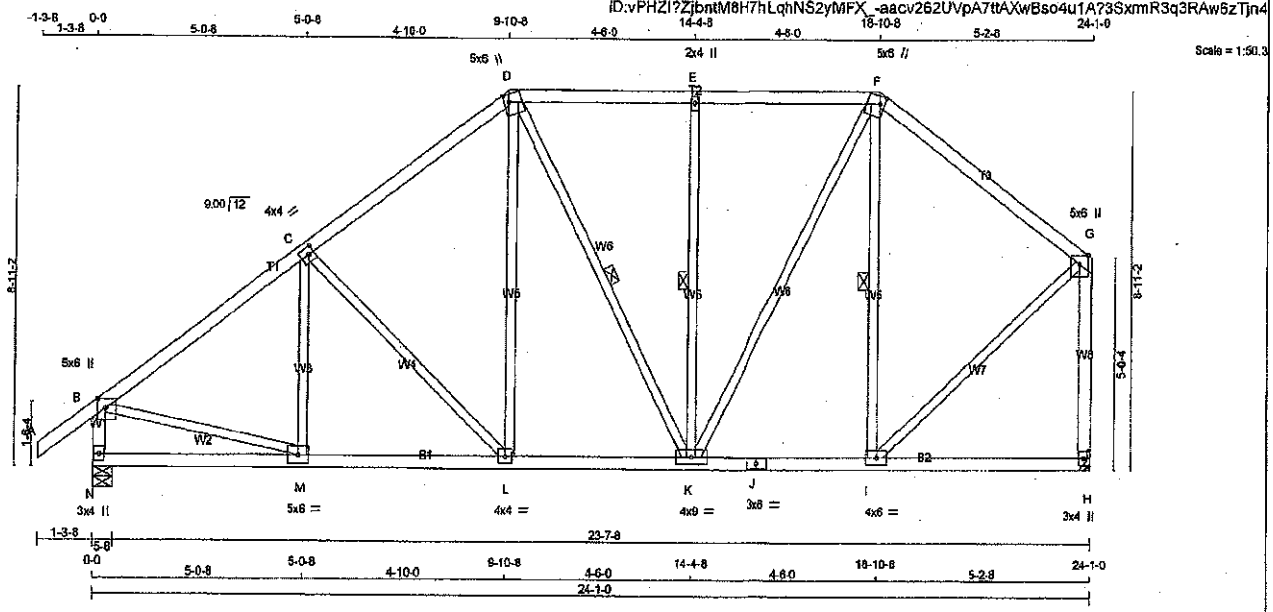

TOTAL WEIGHT = 3 X 122 = 367 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-I	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	8.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1834	0	1834	0	0	5-8	5-8
H	1693	0	1693	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1362	779 / 0	253 / 0	0 / 0	0 / 0	330 / 0	0 / 0
H	1265	699 / 0	253 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K, E-K, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B- C	-1757 / 0	-102.1	-102.1	0.37 (1)	4.66	
C- D	-1474 / 0	-102.1	-102.1	0.35 (1)	5.02	
D- E	-1169 / 0	-102.1	-102.1	0.27 (1)	5.58	
E- F	-1169 / 0	-102.1	-102.1	0.27 (1)	5.58	
F- G	-1029 / 0	-102.1	-102.1	0.37 (1)	5.71	
N- B	-1754 / 0	0.0	0.0	0.18 (1)	6.29	
H- G	-1611 / 0	0.0	0.0	0.73 (1)	8.52	
N- M	0 / 0	-38.5	-38.5	0.18 (3)	10.00	
M- L	0 / 1434	-38.5	-38.5	0.36 (2)	10.00	
L- K	0 / 1153	-38.5	-38.5	0.26 (1)	10.00	
K- J	0 / 816	-38.5	-38.5	0.30 (2)	10.00	
J- I	0 / 816	-38.5	-38.5	0.30 (2)	10.00	
I- H	0 / 0	-38.5	-38.5	0.19 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-126 / 123	0.05 (1)
C- L	-406 / 0	0.38 (1)
L- D	0 / 475	0.11 (2)
D- K	-8 / 33	0.01 (1)
K- E	-558 / 0	0.29 (1)
K- F	0 / 763	0.17 (1)
I- F	-503 / 0	0.26 (1)
B- M	0 / 1474	0.33 (1)
I- G	0 / 1097	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.73/1.00 (G-H:1) , BC=0.36/1.00 (L-M:2) , WB=0.36/1.00 (C-L:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ADJ. GRIP= 0.90 (B) (INPUT = 0.90)
ADJ. METAL= 0.42 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100099824
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DWG NO. TAM 7207803
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T29	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Fri Apr 5 16:39:48 2019 Page 1

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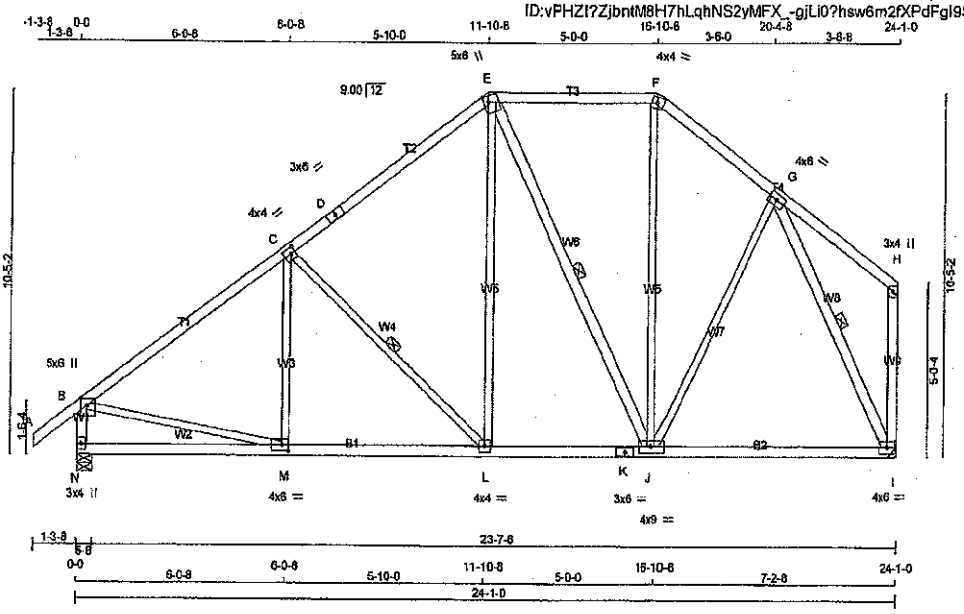

TOTAL WEIGHT = 129 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-I	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0	2.00	2.25
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1834	0	1834	0	0	5-8	5-8
I	1693	0	1693	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1362	779 / 0	253 / 0	0 / 0	0 / 0	330 / 0	0 / 0
I	1265	699 / 0	253 / 0	0 / 0	0 / 0	313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	-1757 / 0	-102.1	-102.1	0.55 (1)	4.41
C- D	-1308 / 0	-102.1	-102.1	0.50 (1)	5.02
D- E	-1308 / 0	-102.1	-102.1	0.50 (1)	5.02
E- F	-868 / 0	-102.1	-102.1	0.34 (1)	6.13
F- G	-1110 / 0	-102.1	-102.1	0.18 (1)	5.82
G- H	0 / 25	-102.1	-102.1	0.21 (1)	10.00
N- B	-1741 / 0	0.0	0.0	0.18 (1)	6.32
I- H	-144 / 0	0.0	0.0	0.07 (1)	7.81
N- M	0 / 0	-38.5	-38.5	0.28 (3)	10.00
M- L	0 / 1441	-38.5	-38.5	0.46 (2)	10.00
L- K	0 / 1013	-38.5	-38.5	0.47 (2)	10.00
K- J	0 / 1013	-38.5	-38.5	0.47 (2)	10.00
J- I	0 / 704	-38.5	-38.5	0.42 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-15 / 217	0.05 (3)
C- L	-602 / 0	0.28 (1)
L- E	0 / 581	0.13 (1)
E- J	-334 / 0	0.23 (1)
J- F	0 / 290	0.07 (2)
J- G	0 / 365	0.08 (1)
B- M	0 / 1468	0.33 (1)
G- I	-1597 / 0	0.51 (1)

LICENSED PROFESSIONAL ENGINEER
04/07/19
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.47/1.00 (J-L:2) , WB=0.51/1.00 (G-I:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.87 (G) (INPUT = 0.90)
LSI METAL= 0.60 (K) (INPUT = 1.00)

DWG NO. TAM 1907804
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
402114	T30	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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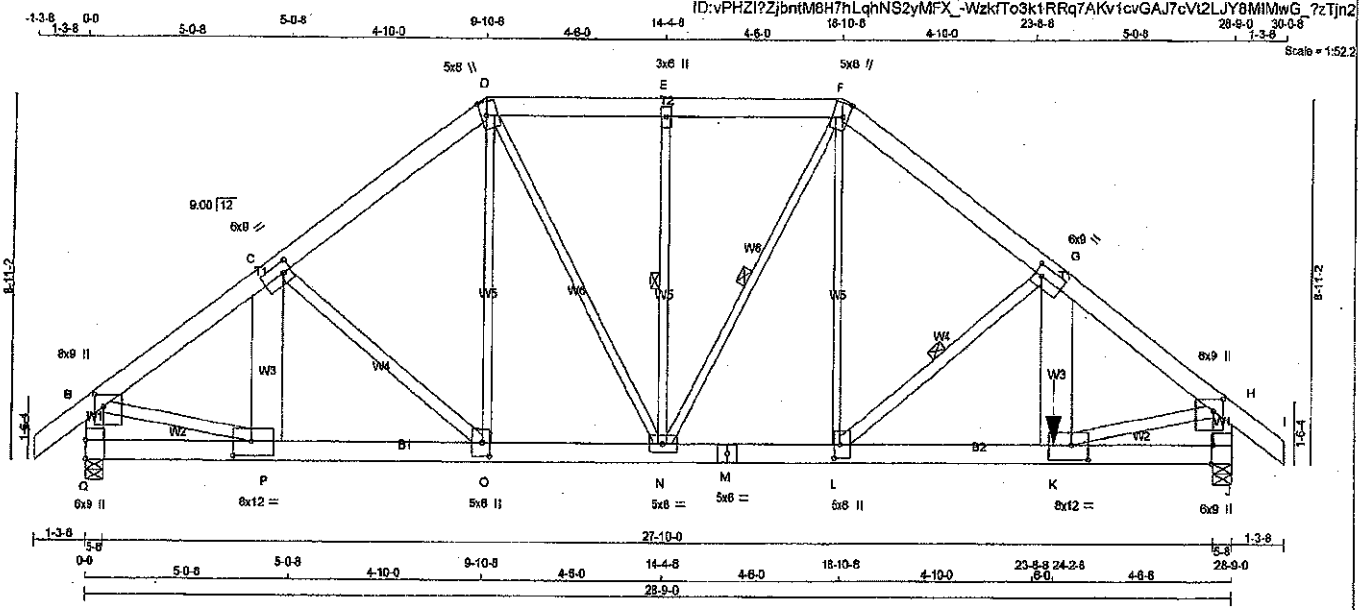

TOTAL WEIGHT = 2 X 204 = 409 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - C	2x10	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF
K - G	2x10	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	2	12	TOP
D-F	2	12	TOP
F-I	2	12	TOP
Q-B	2	12	TOP
J-H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q-M	2	12	TOP
M-J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
G-K	3	2	SIDE(2227.4)
2x10	3	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	8.0	9.0	3.50	2.75
C	TMWW-t	MT20	6.0	9.0	3.00	2.25
D	TTWW+m	MT20	5.0	8.0	4.00	1.50
E	TMW+w	MT20	3.0	6.0		
F	TTWW+m	MT20	5.0	8.0	4.00	1.50
G	TMWW-t	MT20	6.0	9.0	3.00	2.25
H	TMVW+p	MT20	8.0	9.0	3.50	2.75
J	BMV1+t	MT20	6.0	9.0	Edge	0.50
K	BMWW-t	MT20	8.0	12.0	4.25	5.25
L	BMWW+t	MT20	5.0	8.0	4.00	2.00
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3607	0	3607	0	0	5-8	5-8
J	9968	0	9968	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2680	1528 / 0	502 / 0	0 / 0	0 / 0	651 / 0	0 / 0
J	7406	4224 / 0	1384 / 0	0 / 0	0 / 0	1798 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, F-N, G-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB: FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 44	-102.1	-102.1	0.04 (1)	10.00
B-C	-4074 / 0	-102.1	-102.1	0.12 (1)	5.54
C-D	-4098 / 0	-102.1	-102.1	0.13 (1)	5.52
D-E	-4194 / 0	-102.1	-102.1	0.09 (1)	5.50
E-F	-4194 / 0	-102.1	-102.1	0.09 (1)	5.50
F-G	-5922 / 0	-102.1	-102.1	0.21 (1)	4.69
G-H	-12109 / 0	-102.1	-102.1	0.32 (1)	3.33
H-I	0 / 44	-102.1	-102.1	0.04 (1)	10.00
Q-B	-3521 / 0	0.0	0.0	0.13 (1)	7.46
J-H	-9815 / 0	0.0	0.0	0.35 (1)	4.81
Q-P	0 / 0	-38.5	-38.5	0.04 (2)	10.00
P-O	0 / 3285	-38.5	-38.5	0.26 (1)	10.00
O-N	0 / 3251	-38.5	-38.5	0.25 (1)	10.00
N-M	0 / 4778	-38.5	-38.5	0.35 (1)	10.00
M-L	0 / 4778	-38.5	-38.5	0.35 (1)	10.00
L-K	0 / 9686	-38.5	-38.5	0.75 (1)	10.00
K-J	0 / 0	-38.5	-38.5	0.11 (1)	10.00

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
P-C	-564 / 0	0.03 (1)
C-O	-47 / 0	0.01 (1)
O-D	0 / 282	0.03 (2)
D-N	0 / 2004	0.25 (1)
N-E	-468 / 0	0.12 (1)
N-F	-1240 / 0	0.39 (1)
L-F	0 / 4172	0.52 (1)
L-G	-8332 / 0	0.75 (1)
K-G	0 / 7131	0.33 (1)
B-P	0 / 3387	0.30 (1)
K-H	0 / 9827	0.88 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL CONN.
K	24-2-8	-9240	-9240	—	FRONT	VERT	TOTAL	0000000024

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.35/1.00 (H-J:1) , BC=0.75/1.00 (K-L:1) , WB=0.88/1.00 (H-K:1) , SSI=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (P) (INPUT = 0.90)
SI METAL= 0.88 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/07/17
H. J. G. ALVES
100090024
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

BWG NO. TAM 1907805
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
402114	T30	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWW+t	MT20	5.0	8.0	4.00	2.00
P	BMWW-t	MT20	8.0	12.0	4.25	5.25
Q	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100008024
PROVINCE OF ONTARIO

DWG NO. TAM 71907805
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO
402114	T33	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Fri Apr 5 16:35:12 2019 Page 1

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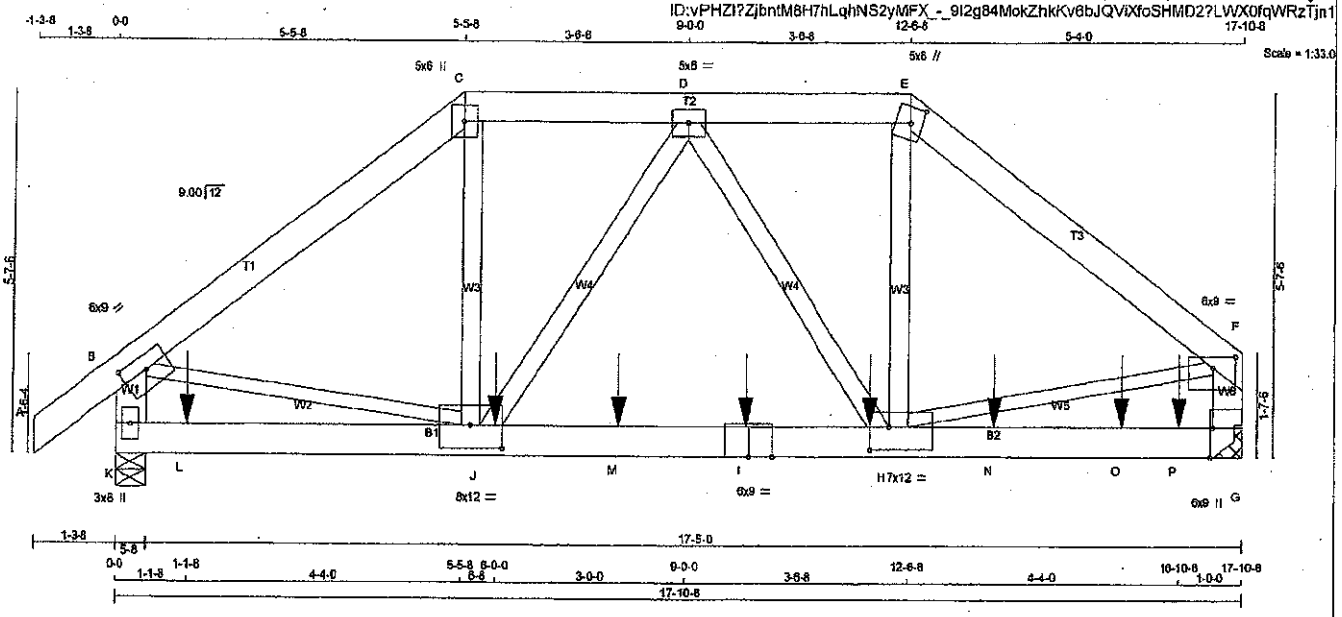

TOTAL WEIGHT = 2 X 108 = 216 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - I	2x6	DRY	2100F 1.8E	SPF
I - G	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - J	2x3	DRY	No.2	SPF
H - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
E- F	2	12	TOP
K- B	2	12	TOP
G- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- I	2	12	SIDE(197.8)
I- G	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
J- C	1	5	SIDE(553.8)
H- E	1	6	SIDE(233.9)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0	2.50	4.50
C	TTW+p	MT20	5.0	6.0		
D	TMWW-t	MT20	5.0	6.0		
E	TTW+m	MT20	5.0	6.0	3.00	2.25
F	TMWW-p	MT20	6.0	9.0	2.00	4.25
G	BMV1+t	MT20	6.0	9.0	Edge	0.50
H	BMWWW-t	MT20	7.0	12.0	4.25	3.75
I	BS-t	MT20	6.0	9.0		
J	BMWWW-t	MT20	8.0	12.0	4.25	6.00
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	7584	0	7584	0	0	5-8	5-8
G	9279	0	9279	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 5-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	5637	3209 / 0	1056 / 0	0 / 0	0 / 0	1372 / 0	0 / 0
G	6900	3917 / 0	1303 / 0	0 / 0	0 / 0	1680 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 44	-102.1	-102.1	0.04 (1)	10.00	J- C	0 / 4404	0.39 (1)
B- C	-8173 / 0	-102.1	-102.1	0.27 (1)	4.06	J- D	-576 / 0	0.13 (1)
C- D	-6797 / 0	-102.1	-102.1	0.13 (1)	4.50	H- E	0 / 4751	0.42 (1)
D- E	-7272 / 0	-102.1	-102.1	0.14 (1)	4.36	B- J	0 / 6920	0.88 (1)
E- F	-6742 / 0	-102.1	-102.1	0.27 (1)	3.93	H- F	0 / 7100	0.88 (1)
K- B	-6485 / 0	0.0	0.0	0.23 (1)	5.84	D- H	0 / 2122	0.84 (1)
G- F	-6698 / 0	0.0	0.0	0.25 (1)	5.68			
K- L	0 / 0	-38.5	-38.5	0.28 (1)	10.00			
L- J	0 / 0	-38.5	-38.5	0.28 (1)	10.00			
J- M	0 / 7101	-38.5	-38.5	0.83 (1)	10.00			
M- I	0 / 7101	-38.5	-38.5	0.83 (1)	10.00			
I- H	0 / 7101	-38.5	-38.5	0.83 (1)	10.00			
H- N	0 / 0	-38.5	-38.5	0.65 (1)	10.00			
N- O	0 / 0	-38.5	-38.5	0.65 (1)	10.00			
O- P	0 / 0	-38.5	-38.5	0.65 (1)	10.00			
P- G	0 / 0	-38.5	-38.5	0.65 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	REEL	DOWN
H	11-11-4	-1655	-1655	---	BACK	VERT	TOTAL	---	---
I	9-11-4	-1655	-1655	---	BACK	VERT	TOTAL	---	---
J	6-0-0	-2881	-2881	---	BACK	VERT	TOTAL	---	---
L	1-1-8	-1525	-1525	---	FRONT	VERT	TOTAL	---	---
M	7-11-4	-1655	-1655	---	BACK	VERT	TOTAL	---	---
N	13-11-4	-1655	-1655	---	BACK	VERT	TOTAL	---	---
O	15-11-4	-1655	-1655	---	BACK	VERT	TOTAL	---	---
P	18-10-0	-1525	-1525	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.27/1.00 (E-F:1) , BC=0.83/1.00 (H-J:1) , WB=0.88/1.00 (F-H:1) , SSI=0.89/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.95 (I) (INPUT = 1.00)

DWG NO. TAM T19078 06

STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
H. J. G ALVES
100009824
PROVINCE OF ONTARIO
04/05/19

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T34S	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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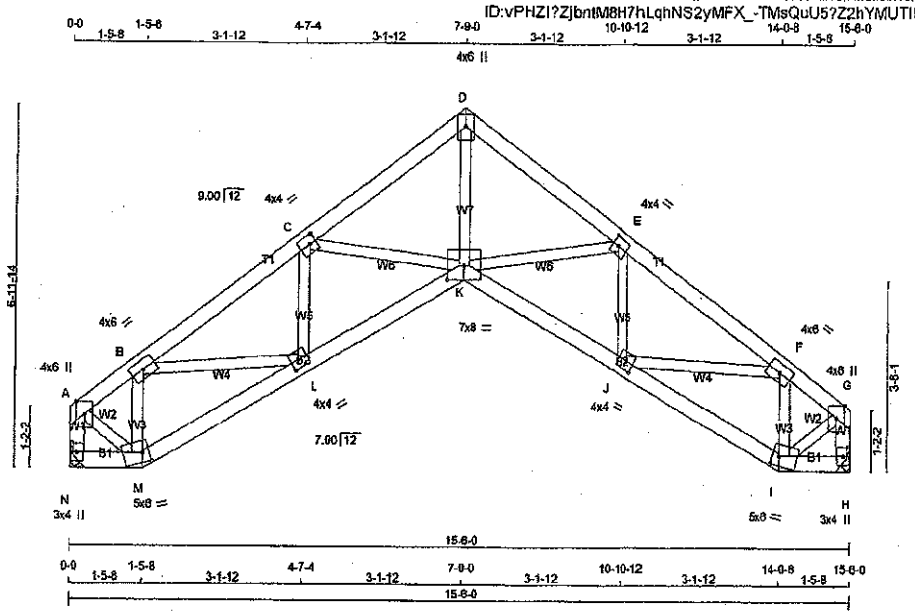


TOTAL WEIGHT = 3 X 65 = 194 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BBWW-m	MT20	5.0	6.0	2.25	1.50
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BBWWW-p	MT20	7.0	8.0	3.25	4.00
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BBWW-m	MT20	5.0	6.0	2.25	1.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1090	0	1090	0	0	IN-SX	IN-SX
H	1090	0	1090	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT N, H. MINIMUM BEARING LENGTH AT JOINT N = 3-8, JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	814	450 / 0	163 / 0	0 / 0	0 / 0	202 / 0	0 / 0
H	814	450 / 0	163 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	-951 / 0	-102.1	-102.1	0.07 (1)	6.25	K- D	0 / 1868	0.42 (1)
B- C	-1855 / 0	-102.1	-102.1	0.13 (1)	4.83	K- E	-67 / 0	0.01 (1)
C- D	-1795 / 0	-102.1	-102.1	0.15 (1)	4.88	J- E	-302 / 0	0.05 (1)
D- E	-1795 / 0	-102.1	-102.1	0.15 (1)	4.88	J- F	0 / 753	0.17 (1)
E- F	-1855 / 0	-102.1	-102.1	0.13 (1)	4.83	I- F	-800 / 0	0.12 (1)
F- G	-951 / 0	-102.1	-102.1	0.07 (1)	6.25	C- K	-67 / 0	0.01 (1)
N- A	-1062 / 0	0.0	0.0	0.11 (1)	7.64	L- C	-302 / 0	0.05 (1)
H- G	-1062 / 0	0.0	0.0	0.11 (1)	7.64	E- L	0 / 753	0.17 (1)
						M- B	-800 / 0	0.12 (1)
N- M	0 / 0	-38.5	-38.5	0.02 (3)	10.00	A- M	0 / 900	0.20 (1)
M- L	0 / 857	-38.5	-38.5	0.19 (1)	10.00	I- G	0 / 900	0.20 (1)
L- K	0 / 1719	-38.5	-38.5	0.33 (1)	10.00			
K- J	0 / 1719	-38.5	-38.5	0.33 (1)	10.00			
J- I	0 / 857	-38.5	-38.5	0.19 (1)	10.00			
I- H	0 / 0	-38.5	-38.5	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.15/1.00 (D-E:1) , BC=0.33/1.00 (K-L:1) , WB=0.42/1.00 (D-K:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

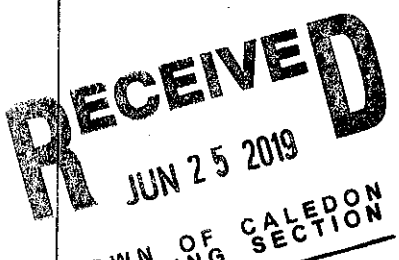
NAIL VALUES

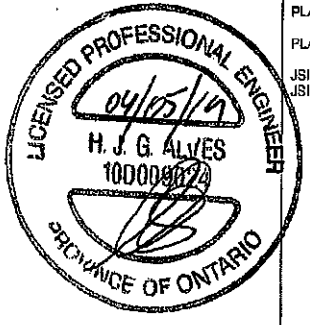
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (M) (INPUT = 0.90)
JSI METAL= 0.45 (D) (INPUT = 1.00)





DWG NO. TAM 1901807
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T35	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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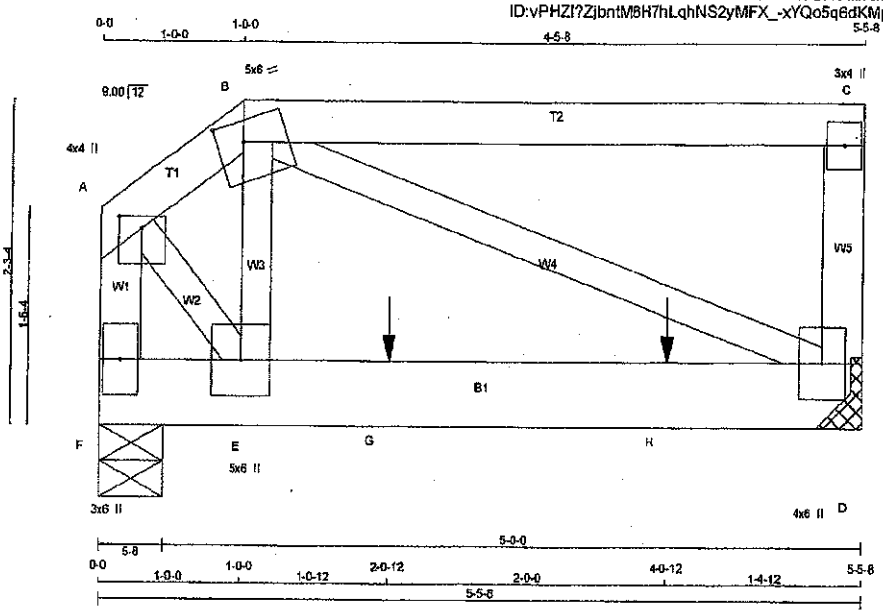


TOTAL WEIGHT = 4 X 25 = 100 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-B	1	12	TOP
B-C	1	12	TOP
C-D	1	12	TOP
F-A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW-m	MT20	5.0	6.0	1.75	2.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1563	0	1563	0	0	MECHANICAL	
F	1306	0	1306	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1161	664 / 0	216 / 0	0 / 0	0 / 0	281 / 0	0 / 0
F	971	553 / 0	182 / 0	0 / 0	0 / 0	235 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	-1476 / 0	-102.1	-102.1	1	0.02	(1)	6.25
B-C	0 / 0	-102.1	-102.1	1	0.19	(1)	10.00
D-C	-228 / 0	0.0	0.0	1	0.01	(1)	7.81
F-A	-2126 / 0	0.0	0.0	1	0.12	(1)	7.61
F-E	0 / 0	-38.5	-38.5	1	0.34	(1)	10.00
E-G	0 / 1234	-38.5	-38.5	1	0.49	(1)	10.00
G-H	0 / 1234	-38.5	-38.5	1	0.49	(1)	10.00
H-D	0 / 1234	-38.5	-38.5	1	0.49	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
E-B	0 / 1142	0.14	(1)
B-D	-1339 / 0	0.24	(1)
A-E	0 / 1659	0.21	(1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	2-0-12	-1051	-1051	—	FRONT	VERT	TOTAL	—	—
H	4-0-12	-1051	-1051	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.49/1.00 (D-E:1) , WB=0.24/1.00 (B-D:1) , SSI=0.48/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

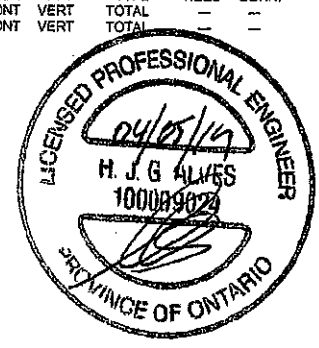
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)



DWG NO. TAM T1907B08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T36	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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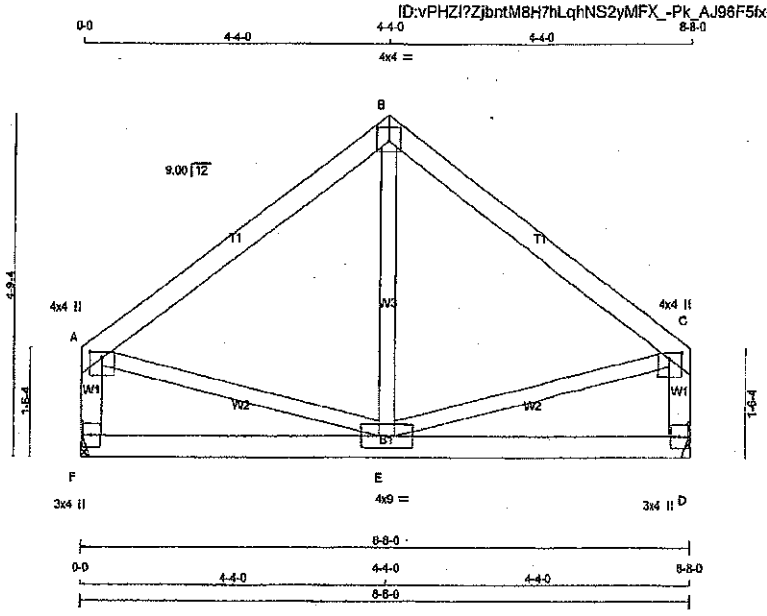


TOTAL WEIGHT = 2 X 35 = 70 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	609	0	609	0	0	MECHANICAL	
D	609	0	609	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

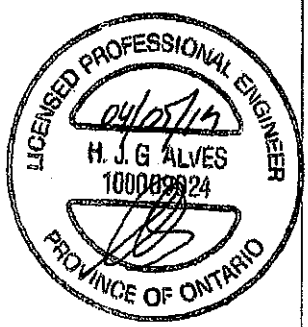
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	455	252 / 0	91 / 0	0 / 0	0 / 0	113 / 0	0 / 0
D	455	252 / 0	91 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-395 / 0	-102.1	-102.1	0.25 (1)	6.25	E- B	0 / 175	0.04 (3)
B- C	-395 / 0	-102.1	-102.1	0.25 (1)	6.25	A- E	0 / 328	0.07 (1)
F- A	-545 / 0	0.0	0.0	0.06 (1)	7.81	E- C	0 / 328	0.07 (1)
D- C	-545 / 0	0.0	0.0	0.06 (1)	7.81			
F- E	0 / 0	-38.5	-38.5	0.16 (3)	10.00			
E- D	0 / 0	-38.5	-38.5	0.16 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.16/1.00 (E-F:3) , WB=0.07/1.00 (A-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (C) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)

DWG NO. TAM T2007809
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	G36	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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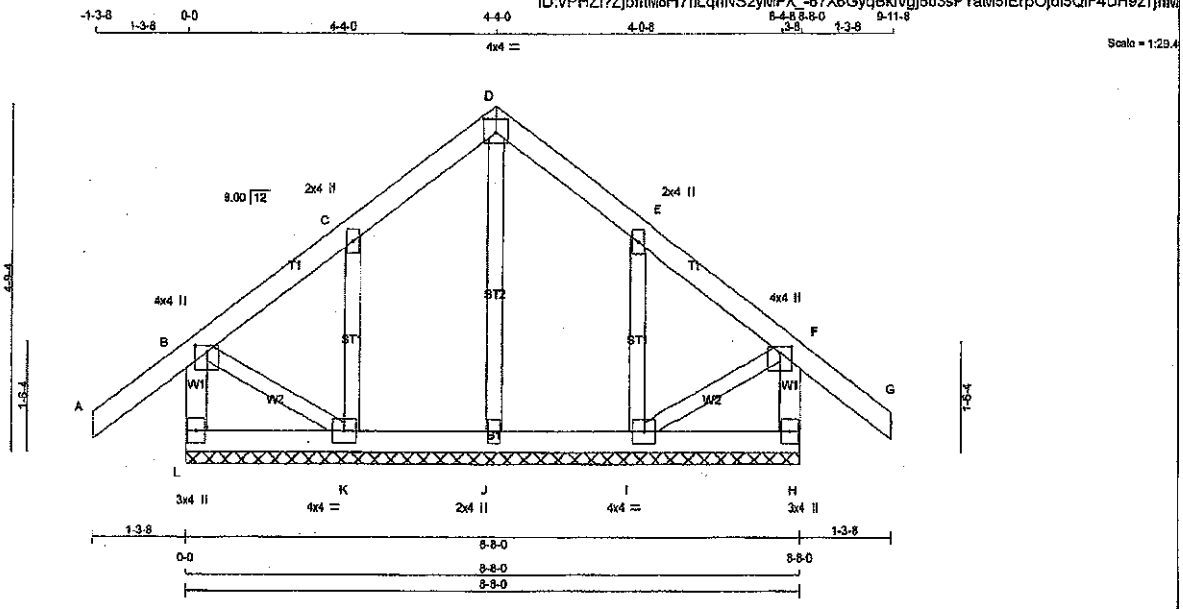

TOTAL WEIGHT = 40 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-B	-256 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	-10 / 0	-102.1	-102.1	0.08 (1)	6.25
C-D	-32 / 0	-102.1	-102.1	0.08 (1)	6.25
D-E	-32 / 0	-102.1	-102.1	0.08 (1)	6.25
E-F	-10 / 0	-102.1	-102.1	0.08 (1)	6.25
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00
H-F	-256 / 0	0.0	0.0	0.03 (1)	7.81
L-K	0 / 0	-38.5	-38.5	0.04 (3)	10.00
K-J	0 / 12	-38.5	-38.5	0.04 (3)	10.00
J-I	0 / 12	-38.5	-38.5	0.04 (3)	10.00
I-H	0 / 0	-38.5	-38.5	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-D	-139 / 0	0.04 (1)
K-C	-269 / 0	0.05 (1)
I-E	-269 / 0	0.05 (1)
B-K	0 / 23	0.01 (1)
I-F	0 / 23	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1) , BC=0.04/1.00 (J-K:3) , WB=0.05/1.00 (C-K:1) , SSI=0.09/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	766	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (F) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)

[M]

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G ALVES
100089024
PROVINCE OF ONTARIO

DWG NO. TAM 7907810
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T37	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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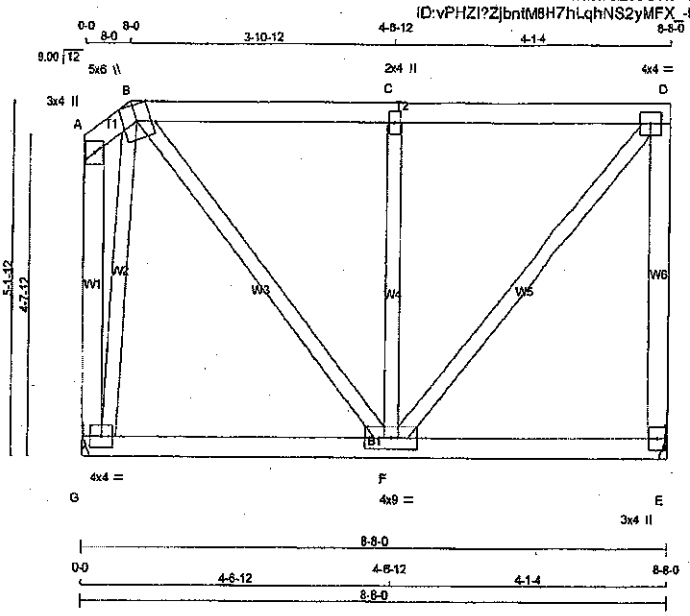


Scale = 1:30.9

TOTAL WEIGHT = 47 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TTWW+m	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	609	0	609	0	0	MECHANICAL	
G	609	0	609	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, G. MINIMUM BEARING LENGTH AT JOINT E = 1-8, JOINT G = 1-8.

UNFACTORED REACTIONS

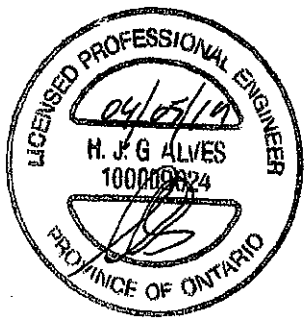
JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	455	252 / 0	91 / 0	0 / 0	0 / 0	113 / 0	0 / 0
G	455	252 / 0	91 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 0	-102.1	-102.1	0.01 (1)	10.00	B-F	0 / 407	0.09 (1)
B-C	-327 / 0	-102.1	-102.1	0.26 (1)	6.25	F-C	-501 / 0	0.20 (1)
C-D	-327 / 0	-102.1	-102.1	0.26 (1)	6.25	F-D	0 / 506	0.11 (1)
E-D	-550 / 0	0.0	0.0	0.25 (1)	7.81	G-B	-508 / 0	0.20 (1)
G-A	-37 / 0	0.0	0.0	0.01 (1)	7.81			
G-F	0 / 74	-38.5	-38.5	0.16 (3)	10.00			
F-E	0 / 0	-38.5	-38.5	0.16 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.16/1.00 (F-G:3) , WB=0.20/1.00 (B-G:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (D) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

DWG NO. TAM 1740784
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
402114	T38	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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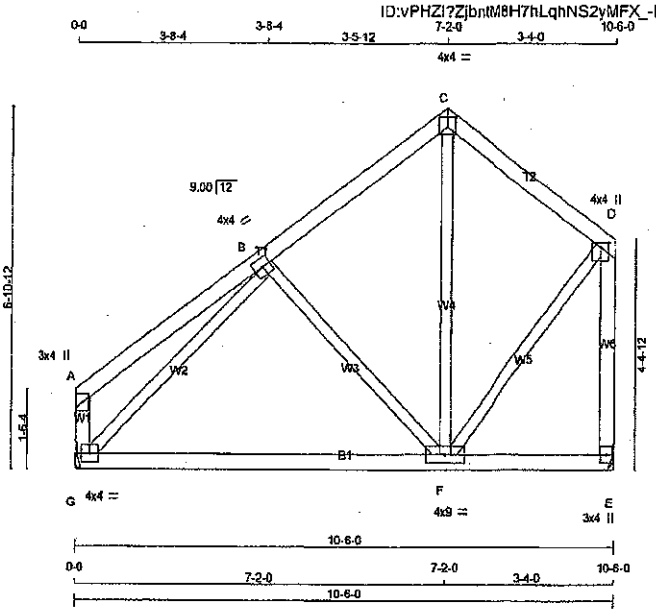


TOTAL WEIGHT = 3 X 50 = 151 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	738	0	738	0	0	MECHANICAL	
G	738	0	738	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E, G. MINIMUM BEARING LENGTH AT JOINT E = 1-8, JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	552	305 / 0	110 / 0	0 / 0	0 / 0	137 / 0	0 / 0
G	552	305 / 0	110 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-102.1	-102.1	0.21 (1)	10.00
B-C	-379 / 0	-102.1	-102.1	0.16 (1)	6.25
C-D	-354 / 0	-102.1	-102.1	0.15 (1)	6.25
G-A	-139 / 0	0.0	0.0	0.01 (1)	7.81
E-D	-727 / 0	0.0	0.0	0.24 (1)	7.81
G-F	0 / 449	-38.5	-38.5	0.37 (2)	10.00
F-E	0 / 0	-38.5	-38.5	0.33 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-F	-243 / 0	0.11 (1)
F-C	0 / 161	0.04 (3)
G-B	-662 / 0	0.29 (1)
F-D	0 / 446	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 767 (0.16")

CSI: TC=0.24/1.00 (D-E:1) , BC=0.37/1.00 (F-G:2) , WB=0.29/1.00 (B-G:1) , SSI=0.17/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

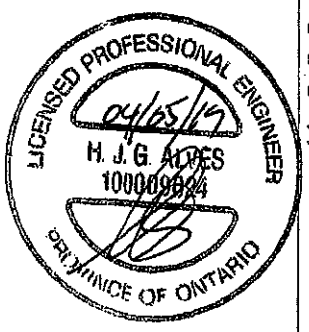
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



DWG NO. TAM 190781/2
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T39	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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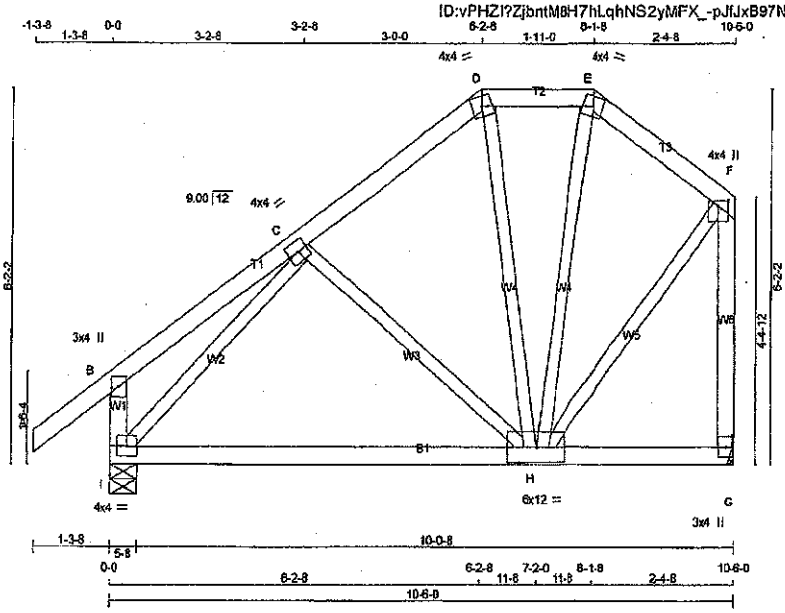


TOTAL WEIGHT = 55 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWWW*m	MT20	6.0	12.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	738	0	738	0	0	MECHANICAL	
I	879	0	879	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	552	305 / 0	110 / 0	0 / 0	0 / 0	137 / 0	0 / 0
I	648	385 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

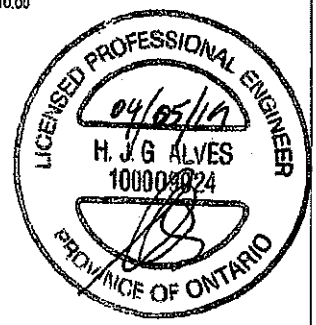
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 22	-102.1	-102.1	0.16 (1)	10.00
C- D	-407 / 0	-102.1	-102.1	0.12 (1)	6.25
D- E	-309 / 0	-102.1	-102.1	0.05 (1)	6.25
E- F	-385 / 0	-102.1	-102.1	0.08 (1)	6.25
I- B	-262 / 0	0.0	0.0	0.03 (1)	7.81
G- F	-727 / 0	0.0	0.0	0.24 (1)	7.81
I- H	0 / 444	-38.5	-38.5	0.37 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.34 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	-180 / 2	0.08 (1)
I- C	-672 / 0	0.23 (1)
H- F	0 / 483	0.11 (1)
D- H	0 / 85	0.02 (3)
H- E	0 / 86	0.02 (3)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 776 (0.16")

CSI: TC=0.24/1.00 (F-G:1) , BC=0.37/1.00 (H-I:2) , WB=0.23/1.00 (C-I:1) , SSI=0.17/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (C) (INPUT = 0.90)
JSI METAL= 0.23 (C) (INPUT = 1.00)

DWG NO. TAM: T1907813
STRUCTURAL
CONSIGNMENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T40	1	3	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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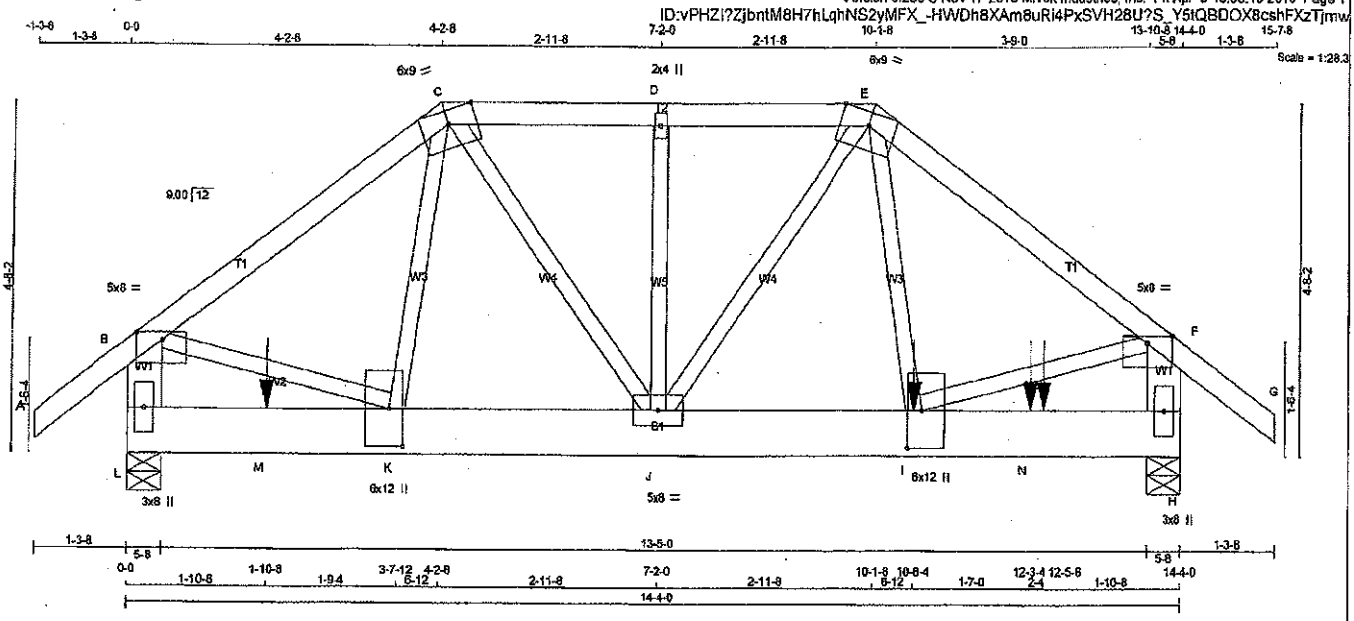

TOTAL WEIGHT = 3 X 82 = 247 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - H	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
E-G	1	12	TOP
L-B	2	12	TOP
H-F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L-H	4	4	SIDE(2105.6)
WEBS : (0.122"X3") SPIRAL NAILS			
E-I	1	6	SIDE(386.7)
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	4.00
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	6.0	9.0	Edge	
F	TMVW-p	MT20	5.0	8.0	1.25	4.00
H	BMV1+p	MT20	3.0	8.0		
I	BMWW+t	MT20	6.0	12.0	6.00	2.25
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW+t	MT20	6.0	12.0	6.00	2.25
L	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	4703	0	4703	0	0	5-8	5-8
H	9508	0	9508	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3477	2037 / 0	611 / 0	0 / 0	0 / 0	829 / 0	0 / 0
H	7035	4104 / 0	1248 / 0	0 / 0	0 / 0	1693 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 42	-102.1	-102.1	0.05 (1)	10.00	C- J	0 / 3165	0.24 (1)
B- C	-4541 / 0	-102.1	-102.1	0.15 (1)	5.21	J- D	-342 / 0	0.03 (1)
C- D	-5460 / 0	-102.1	-102.1	0.07 (1)	4.92	J- E	-2177 / 0	0.30 (1)
D- E	-5460 / 0	-102.1	-102.1	0.07 (1)	4.92	K- C	-229 / 2	0.02 (1)
E- F	-9678 / 0	-102.1	-102.1	0.28 (1)	3.73	B- K	0 / 3777	0.28 (1)
F- G	0 / 42	-102.1	-102.1	0.05 (1)	10.00	E- I	0 / 7392	0.55 (1)
L- B	-4116 / 0	0.0	0.0	0.09 (1)	7.81	I- F	0 / 8049	0.60 (1)
H- F	-8366 / 0	0.0	0.0	0.18 (1)	6.26			
L- M	0 / 0	-38.5	-38.5	0.15 (1)	10.00			
M- K	0 / 0	-38.5	-38.5	0.15 (1)	10.00			
K- J	0 / 3666	-38.5	-38.5	0.18 (1)	10.00			
J- I	0 / 6694	-38.5	-38.5	0.35 (1)	10.00			
I- N	0 / 0	-38.5	-38.5	0.34 (1)	10.00			
N- H	0 / 0	-38.5	-38.5	0.34 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	10-8-4	-8814	-8814	---	FRONT	VERT	TOTAL	---	---
M	1-10-8	-1194	-1194	---	BACK	VERT	TOTAL	---	---
N	12-3-4	-711	-711	---	FRONT	VERT	TOTAL	---	---
N	12-5-8	-1194	-1194	---	BACK	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
04/05/19
M. J. G. ALVES
100089024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.35/1.00 (I-J:1) , WB=0.60/1.00 (F-I:1) , SSI=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (C) (INPUT = 0.90)
JSI METAL= 0.67 (K) (INPUT = 1.00)

DWG NO. TAM 1907814
STRUCTURAL
CITY PERMIT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T41	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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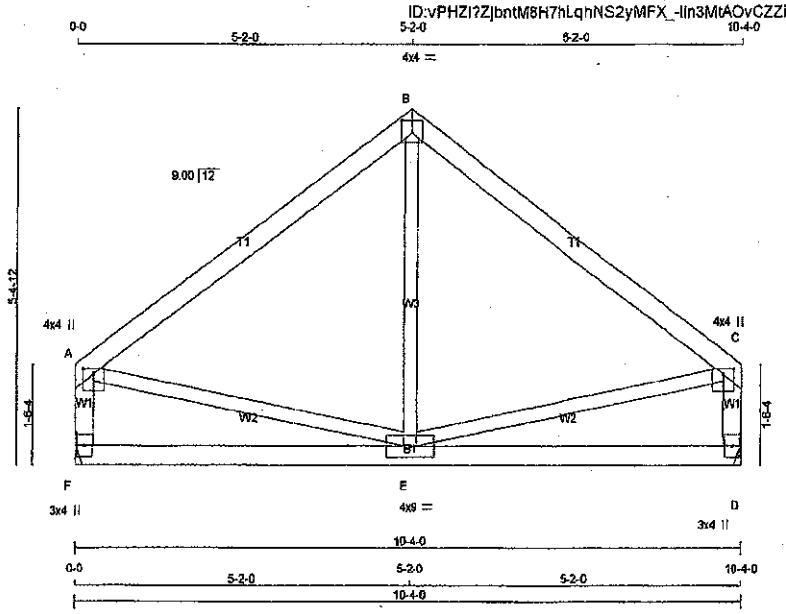


TOTAL WEIGHT = 41 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	726	0	726	0	0	MECHANICAL	
D	726	0	726	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	543	300 / 0	109 / 0	0 / 0	0 / 0	134 / 0	0 / 0
D	543	300 / 0	109 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-493 / 0	-102.1	-102.1	0.35 (1)	6.25	E- B	0 / 223	0.05 (3)
B- C	-493 / 0	-102.1	-102.1	0.35 (1)	6.25	A- E	0 / 405	0.09 (1)
F- A	-651 / 0	0.0	0.0	0.07 (1)	7.81	E- C	0 / 405	0.09 (1)
D- C	-651 / 0	0.0	0.0	0.07 (1)	7.81			
F- E	0 / 0	-38.5	-38.5	0.23 (3)	10.00			
E- D	0 / 0	-38.5	-38.5	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.23/1.00 (D-E:3) , WB=0.09/1.00 (A-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

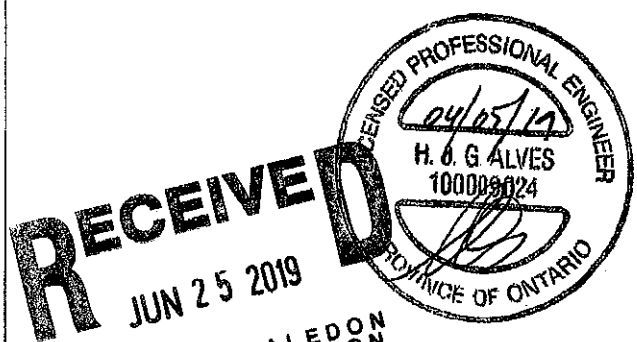
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (C) (INPUT = 0.90)
JSI METAL= 0.17 (A) (INPUT = 1.00)



RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 1740715
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T41S	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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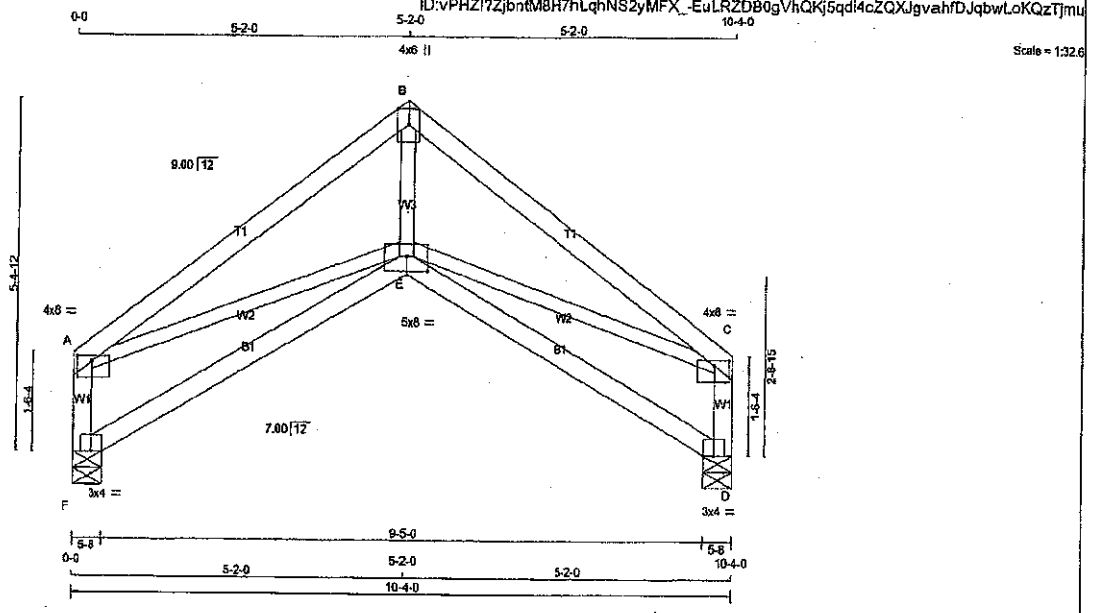

TOTAL WEIGHT = 2 X 41 = 83 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	2.75
B	TTW+p	MT20	4.0	8.0	Edge	
C	TMVW-p	MT20	4.0	6.0	1.00	2.75
D	BVM1-p	MT20	3.0	4.0		
E	BBWWW-p	MT20	5.0	8.0	2.75	4.00
F	BVM1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	726	0	726	0	0	5-8	5-8
D	726	0	726	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	543	300 / 0	108 / 0	0 / 0	0 / 0	134 / 0	0 / 0
D	543	300 / 0	108 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1021 / 0	-102.1	-102.1	0.37 (1)		5.73	E- B	0 / 697	0.16 (1)
B- C	-1021 / 0	-102.1	-102.1	0.37 (1)		5.73	A- E	0 / 854	0.19 (1)
F- A	-627 / 0	0.0	0.0	0.07 (1)		7.81	E- C	0 / 854	0.19 (1)
D- C	-627 / 0	0.0	0.0	0.07 (1)		7.81			
F- E	0 / 0	-38.5	-38.5	0.23 (3)		10.00			
E- D	0 / 0	-38.5	-38.5	0.23 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NECC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.23/1.00 (D-E:3) , WB=0.19/1.00 (C-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (A) (INPUT = 0.80)
JSI METAL= 0.37 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. O. G. ALVES
100089024
PROVINCE OF ONTARIO

DWG NO. TAM T7907816
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T42	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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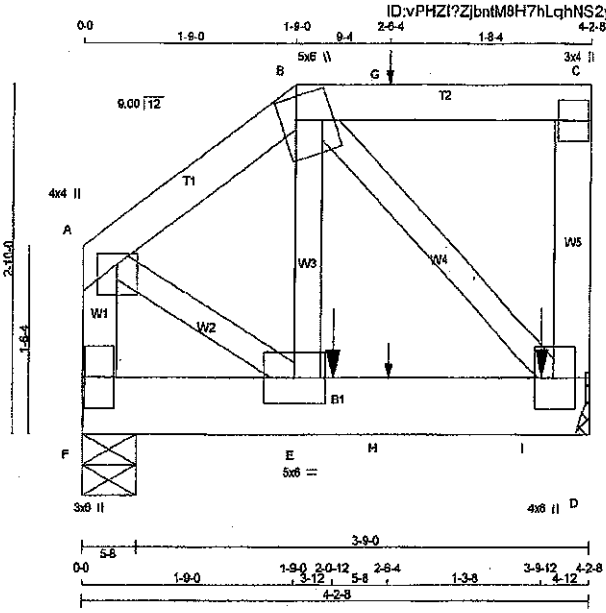

Scale = 1:17.3

TOTAL WEIGHT = 4 X 22 = 88 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	SIDE(0.0)
C- D	1	12	SIDE(60.5)
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTVW+m	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	1233	0	1233	0	0	MECHANICAL	
F	746	0	746	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	915	526 / 0	168 / 0	0 / 0	0 / 0	221 / 0	0 / 0
F	554	318 / 0	102 / 0	0 / 0	0 / 0	134 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	-589 / 0	-102.1	-102.1	0.03 (1)		6.25	E- B	0 / 605	0.07 (1)
B- G	0 / 0	-102.1	-102.1	0.06 (1)		10.00	B- D	-674 / 0	0.07 (1)
G- C	0 / 0	-102.1	-102.1	0.06 (1)		10.00	A- E	0 / 527	0.07 (1)
D- C	-126 / 0	0.0	0.0	0.01 (1)		7.81			
F- A	-704 / 0	0.0	0.0	0.04 (1)		7.81			
F- E	0 / 0	-38.5	-38.5	0.01 (1)		10.00			
E- H	0 / 478	-38.5	-38.5	0.12 (1)		10.00			
H- I	0 / 478	-38.5	-38.5	0.12 (1)		10.00			
I- D	0 / 478	-38.5	-38.5	0.12 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-0-12	-688	-688	---	BACK	VERT	TOTAL	---	---
G	2-6-4	1	1	124	FRONT	VERT	TOTAL	---	---
H	2-6-4	1	1	---	FRONT	VERT	TOTAL		
I	3-9-12	-700	-700	---	BACK	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.06/1.00 (B-C:1) , BC=0.12/1.00 (D-E:1) , WB=0.07/1.00 (B-E:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.31 (A) (INPUT = 0.90)
SSI METAL= 0.11 (D) (INPUT = 1.00)

DWG NO. TAM T1907817

STRUCTURAL
COMPONENTS

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
402114	T43S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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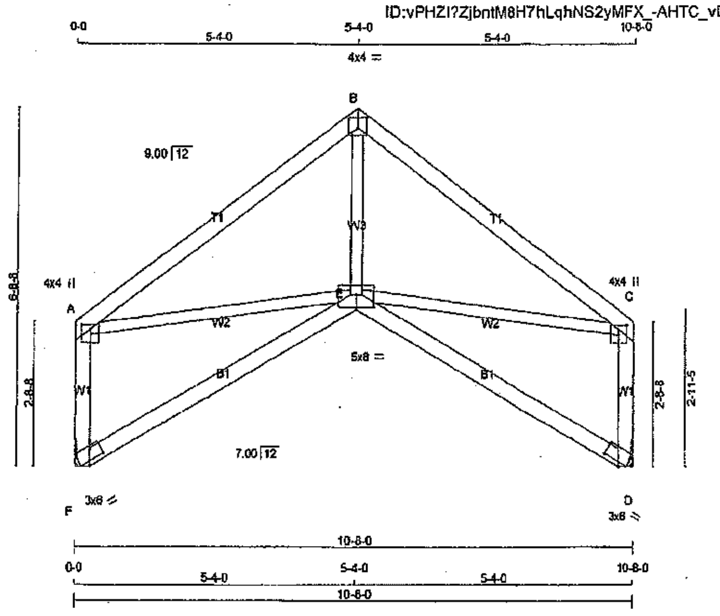

TOTAL WEIGHT = 2 X 46 = 92 lb

[M(F)]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BVM1-I	MT20	3.0	6.0	1.00	4.25
E	BBWWW-p	MT20	5.0	8.0	2.75	4.00
F	BVM1-I	MT20	3.0	6.0	1.00	4.25

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	750	0	750	0	0	MECHANICAL	
D	750	0	750	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	560	310 / 0	112 / 0	0 / 0	0 / 0	139 / 0	0 / 0
D	560	310 / 0	112 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS / WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-732 / 0	-102.1	-102.1	0.38 (1)	6.25	E- B	0 / 373	0.06 (2)
B- C	-732 / 0	-102.1	-102.1	0.38 (1)	6.25	A- E	0 / 589	0.13 (1)
F- A	-647 / 0	0.0	0.0	0.10 (1)	7.81	E- C	0 / 589	0.13 (1)
D- C	-647 / 0	0.0	0.0	0.10 (1)	7.81			
F- E	0 / 0	-38.5	-38.5	0.25 (3)	10.00			
E- D	0 / 0	-38.5	-38.5	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.25/1.00 (E-F:3) , WB=0.13/1.00 (C-E:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (C) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 71078/18
STRUCTURAL
[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T44	1	3	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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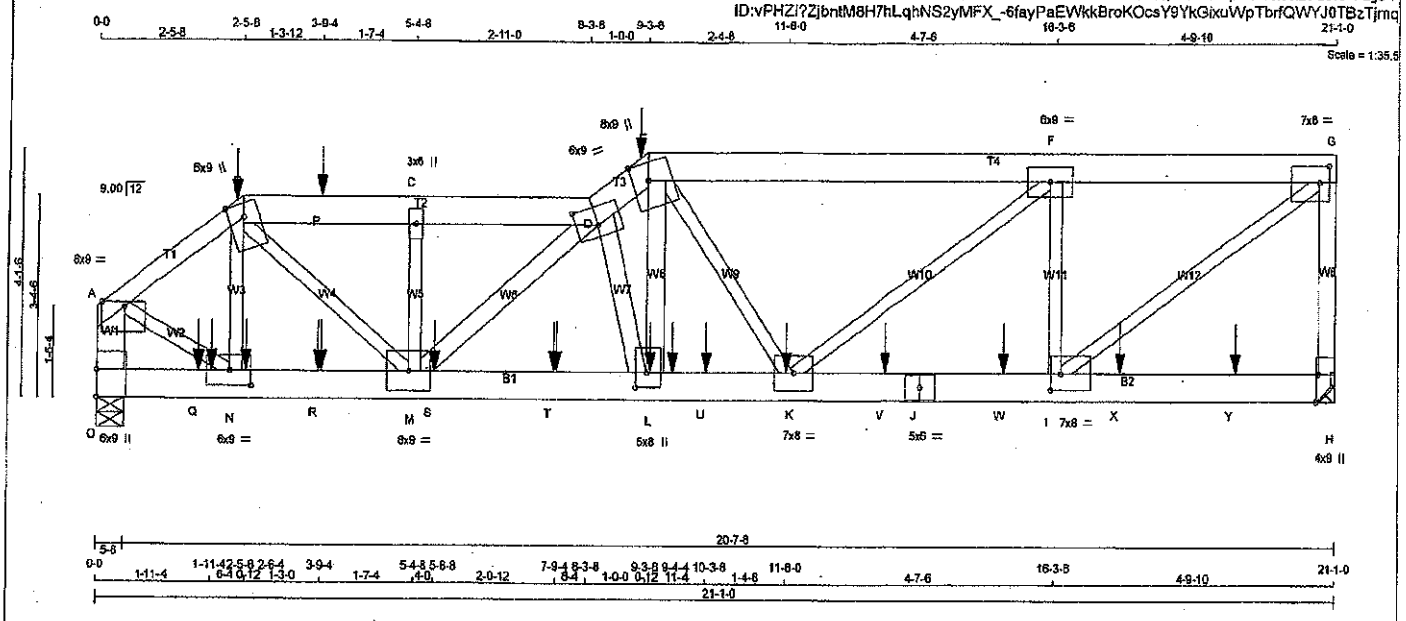

TOTAL WEIGHT = 3 X 116 = 348 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
O - J	2x6	DRY	1650F 1.5E	SPF
J - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - D	2x4	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF
D - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-B	1	12	SIDE(91.5)
G-H	1	12	TOP
B-D	2	12	SIDE(91.5)
D-E	2	12	SIDE(183.1)
E-G	2	12	SIDE(91.5)
O-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-J	2	4	SIDE(1043.3)
J-H	2	12	SIDE(182.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
C-M	1	5	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	6.0	9.0	Edge	
B	TTWW+m	MT20	6.0	9.0	Edge	
C	TMW+w	MT20	3.0	6.0		
D	TTWW-m	MT20	6.0	9.0	3.75	4.25
E	TTWW+m	MT20	8.0	9.0	3.50	3.25
F	TMWW-t	MT20	6.0	9.0		
G	TMVW-t	MT20	7.0	8.0	3.25	2.00
H	BMV1+t	MT20	4.0	9.0	Edge	0.50
I	BMWW-t	MT20	7.0	8.0	3.25	2.00
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	7.0	8.0		
L	BMWW+t	MT20	5.0	8.0	3.00	2.25
M	BMWWW-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	8852	0	8852	0	0	MECHANICAL	
O	11670	0	11670	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	6564	3787 / 0	1195 / 0	0 / 0	0 / 0	1582 / 0	0 / 0
O	8664	4963 / 0	1600 / 0	0 / 0	0 / 0	2100 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO			FROM	TO			FR-TO		
A-B	-11590 / 0		-102.1	-102.1	0.20 (1)	3.47	N-B	-1451 / 0	0.09 (1)
B-P	-17245 / 0		-102.1	-102.1	0.25 (1)	3.50	B-M	0 / 11169	0.84 (1)
P-C	-17245 / 0		-102.1	-102.1	0.25 (1)	3.50	M-C	-288 / 57	0.02 (5)
C-D	-17246 / 0		-102.1	-102.1	0.25 (1)	3.50	M-D	-5430 / 0	0.37 (1)
D-E	-22078 / 0		-102.1	-102.1	0.22 (1)	3.14	L-E	0 / 13921	0.75 (1)
E-F	-17247 / 0		-102.1	-102.1	0.33 (1)	3.49	A-N	0 / 10297	0.77 (1)
F-G	-10621 / 0		-102.1	-102.1	0.11 (1)	4.49	E-K	-1246 / 0	0.12 (1)
H-G	-8306 / 0		0.0	0.0	0.61 (1)	5.17	I-G	0 / 13349	1.00 (1)
O-A	-11412 / 0		0.0	0.0	0.25 (1)	5.50	I-F	-8000 / 0	0.45 (1)
							K-F	0 / 858	0.04 (1)
O-Q	0 / 0		-38.5	-38.5	0.11 (1)	10.00	D-L	-10868 / 0	0.60 (1)
Q-N	0 / 0		-38.5	-38.5	0.11 (1)	10.00			
N-R	0 / 9368		-38.5	-38.5	0.35 (1)	10.00			
R-M	0 / 9366		-38.5	-38.5	0.35 (1)	10.00			
M-S	0 / 21142		-38.5	-38.5	0.82 (1)	10.00			
S-T	0 / 21142		-38.5	-38.5	0.82 (1)	10.00			
T-L	0 / 21142		-38.5	-38.5	0.82 (1)	10.00			
L-U	0 / 17959		-38.5	-38.5	0.48 (1)	10.00			
U-K	0 / 17959		-38.5	-38.5	0.48 (1)	10.00			
K-V	0 / 10621		-38.5	-38.5	0.38 (1)	10.00			
V-J	0 / 10621		-38.5	-38.5	0.38 (1)	10.00			
J-W	0 / 10621		-38.5	-38.5	0.38 (1)	10.00			
W-I	0 / 10621		-38.5	-38.5	0.38 (1)	10.00			
I-X	0 / 0		-38.5	-38.5	0.13 (1)	10.00			
X-Y	0 / 0		-38.5	-38.5	0.13 (1)	10.00			
Y-H	0 / 0		-38.5	-38.5	0.13 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-5-8	-116	-116	149	FRONT	VERT	TOTAL	—	—
E	9-3-8	-245	-245	—	FRONT	VERT	TOTAL	—	—
K	11-8-0	-3645	-3645	—	BACK	VERT	TOTAL	—	—
L	9-4-4	-28	-36	—	FRONT	VERT	TOTAL	—	—
L	9-8-12	-1953	-1953	—	BACK	VERT	TOTAL	—	—
N	2-6-4	-9	-11	—	FRONT	VERT	TOTAL	—	—
P	3-9-4	-23	-23	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 758 (0.33")

CSI: TC=0.61/1.00 (G-H:1) , BC=0.82/1.00 (L-M:1) , WB=1.00/1.00 (G-I:1) , SSI=0.62/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (G) (INPUT = 0.90)
SI METAL= 0.83 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. O. G ALVES
100099024
PROVINCE OF ONTARIO

DWG NO. TAM T1907819
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
402114	T44	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWW-t	MT20	6.0	9.0	3.00	4.25
O	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	1-8-12	-1953	-1953	—	BACK	VERT	TOTAL	—	—
Q	1-11-4	-14	-18	—	FRONT	VERT	TOTAL	—	—
R	3-8-12	-1953	-1953	—	BACK	VERT	TOTAL	—	—
R	3-9-4	-9	-11	—	FRONT	VERT	TOTAL	—	—
S	5-8-8	-246	-246	—	FRONT	VERT	TOTAL	—	—
S	5-8-12	-1953	-1953	—	BACK	VERT	TOTAL	—	—
T	7-8-12	-1953	-1953	—	BACK	VERT	TOTAL	—	—
T	7-9-4	-28	-36	—	FRONT	VERT	TOTAL	—	—
U	10-3-8	-629	-629	—	FRONT	VERT	TOTAL	—	—
V	13-4-12	-700	-700	—	BACK	VERT	TOTAL	—	—
W	15-4-12	-700	-700	—	BACK	VERT	TOTAL	—	—
X	17-4-12	-700	-700	—	BACK	VERT	TOTAL	—	—
Y	19-4-12	-700	-700	—	BACK	VERT	TOTAL	—	—

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	T45	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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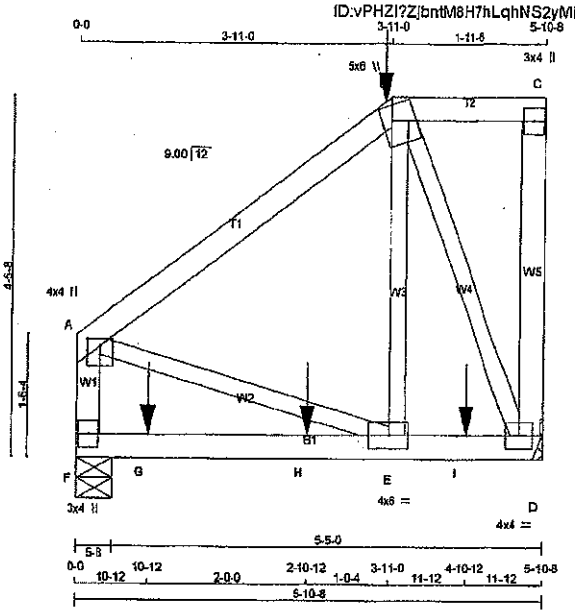

Scale = 1:26.3

TOTAL WEIGHT = 2 X 30 = 60 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
A- B	1	12	SIDE(61.0)
B- C	1	12	SIDE(61.0)
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+t	MT20	4.0	4.0		
E	BMVW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1383	0	1383	0	0	5-8	5-8
D	1457	0	1457	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1018	610 / 0	169 / 0	0 / 0	0 / 0	239 / 0	0 / 0
D	1068	656 / 0	166 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-828 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 624	0.08 (1)
A- B	-749 / 0	-102.1	-102.1	0.15 (1)	6.25	E- B	0 / 1181	0.15 (1)
B- C	0 / 0	-102.1	-102.1	0.04 (1)	10.00	B- D	-1470 / 0	0.25 (1)
D- C	-100 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-38.5	-38.5	0.38 (1)	10.00			
G- H	0 / 0	-38.5	-38.5	0.38 (1)	10.00			
H- E	0 / 0	-38.5	-38.5	0.38 (1)	10.00			
E- I	0 / 628	-38.5	-38.5	0.44 (1)	10.00			
I- D	0 / 628	-38.5	-38.5	0.44 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-11-0	-300	-300	—	FRONT	VERT	TOTAL	—	—
G	10-12	-573	-573	—	BACK	VERT	TOTAL	—	—
H	2-10-12	-571	-571	—	BACK	VERT	TOTAL	—	—
I	4-10-12	-571	-571	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.44/1.00 (D-E:1) , WB=0.25/1.00 (B-D:1) , SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (D) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

DWG NO. TAM T1907820
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402113	PB1	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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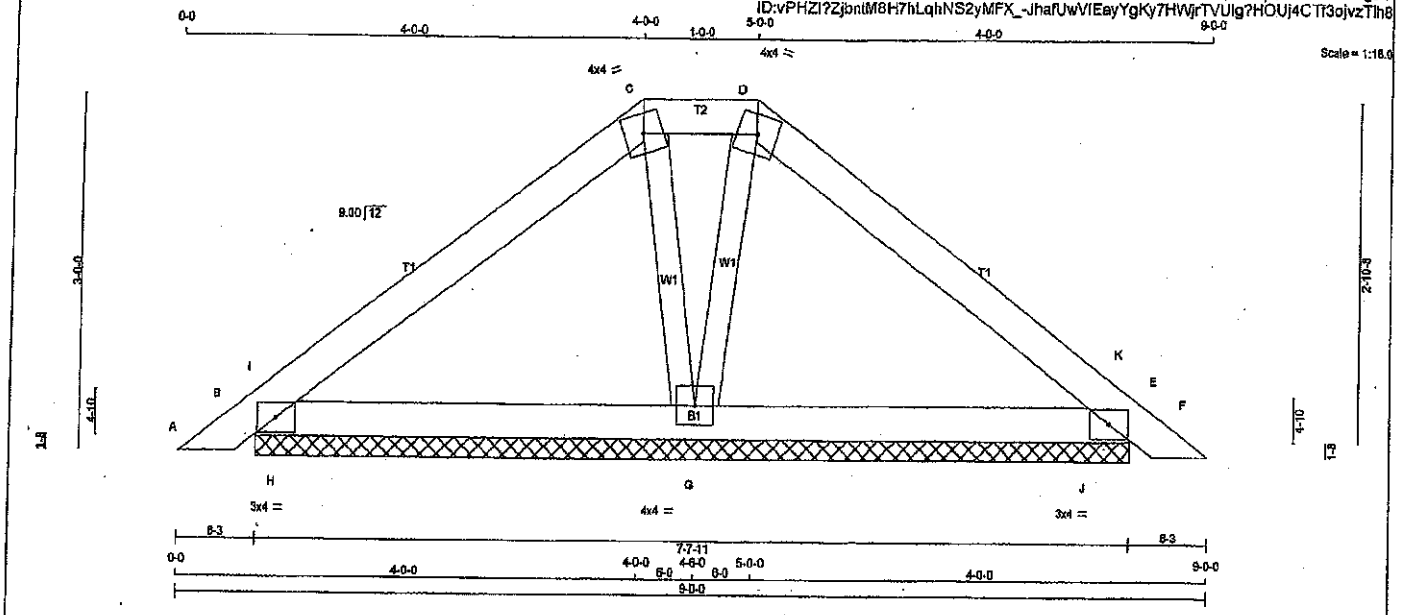

TOTAL WEIGHT = 2 X 25 = 50 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMWW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	365	0	365	0	0	7-7-11	7-7-11
E	354	0	354	0	0	7-7-11	7-7-11
G	476	0	476	0	0	7-7-11	7-7-11

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	266	167 / 0	39 / 0	0 / 0	0 / 0	60 / 0	0 / 0
E	258	163 / 0	37 / 0	0 / 0	0 / 0	58 / 0	0 / 0
G	361	183 / 0	85 / 0	0 / 0	0 / 0	94 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- I	-102 / 0	-102.1	-102.1	0.04 (1)	6.25
I- C	-141 / 0	-102.1	-102.1	0.14 (1)	6.25
C- D	-82 / 0	-102.1	-102.1	0.02 (1)	6.25
D- K	-133 / 0	-102.1	-102.1	0.14 (1)	6.25
K- E	-86 / 0	-102.1	-102.1	0.04 (1)	6.25
E- F	0 / 17	-102.1	-102.1	0.09 (1)	10.00
B- H	0 / 108	-38.5	-38.5	0.12 (1)	10.00
H- G	0 / 108	-38.5	-38.5	0.16 (2)	10.00
G- J	0 / 101	-38.5	-38.5	0.16 (2)	10.00
J- E	0 / 101	-38.5	-38.5	0.12 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-119 / 0	0.02 (1)
G- D	-121 / 0	0.02 (1)
H- I	-254 / 64	0.00 (1)
J- K	-275 / 45	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (C-I:1) , BC=0.16/1.00 (G-H:2) , WB=0.02/1.00 (D-G:1) , SSI=0.23/1.00 (E-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM 19007788
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402113	PB2	7	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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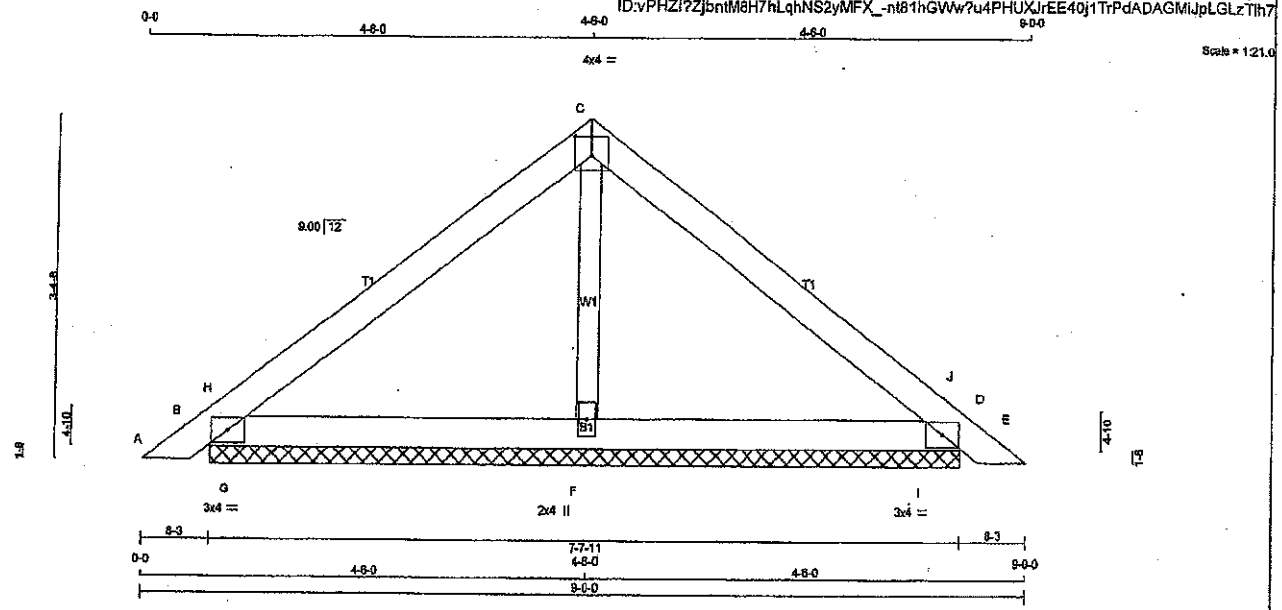


TOTAL WEIGHT = 7 X 24 = 168 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	397	0	397	0	0	7-7-11	7-7-11
D	397	0	397	0	0	7-7-11	7-7-11
F	402	0	402	0	0	7-7-11	7-7-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	288	186 / 0	38 / 0	0 / 0	0 / 0	64 / 0	0 / 0
D	288	186 / 0	38 / 0	0 / 0	0 / 0	64 / 0	0 / 0
F	310	141 / 0	85 / 0	0 / 0	0 / 0	85 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

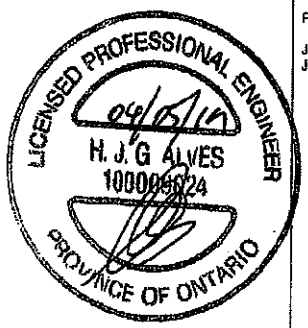
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- H	-81 / 0	-102.1	-102.1	0.08 (1)	6.25
H- C	-174 / 0	-102.1	-102.1	0.18 (1)	6.25
C- J	-174 / 0	-102.1	-102.1	0.18 (1)	6.25
J- D	-81 / 0	-102.1	-102.1	0.08 (1)	6.25
D- E	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- G	0 / 131	-38.5	-38.5	0.17 (1)	10.00
G- F	0 / 131	-38.5	-38.5	0.19 (1)	10.00
F- I	0 / 131	-38.5	-38.5	0.19 (1)	10.00
I- D	0 / 131	-38.5	-38.5	0.17 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-133 / 0	0.02 (1)
G- H	-404 / 26	0.00 (1)
I- J	-404 / 26	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	28.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TRIC 2011, TRIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-J:1) , BC=0.19/1.00 (F-G:1) , WB=0.02/1.00 (C-F:1) , SSI=0.32/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

DWG NO. TAM R907789
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	PB2Z	1	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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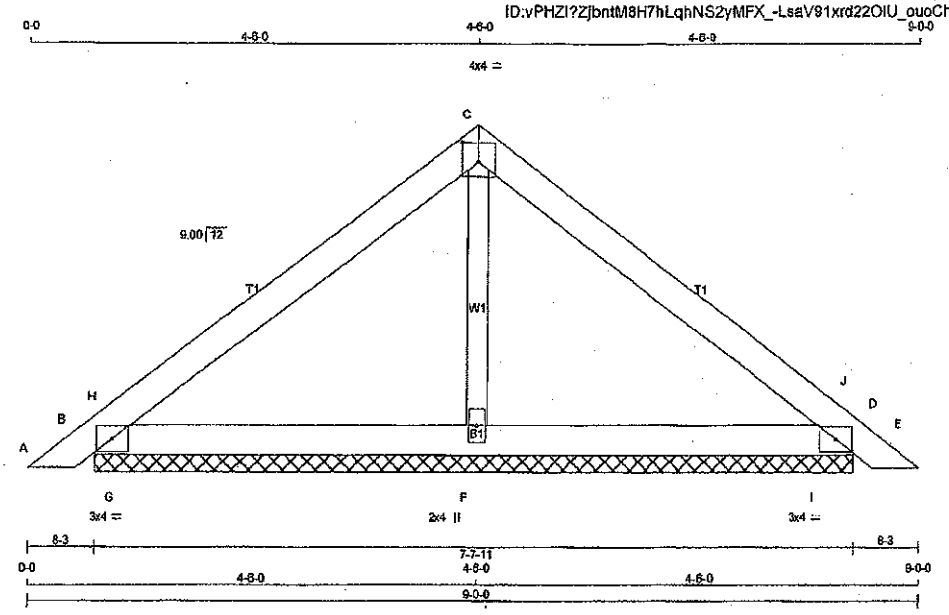

TOTAL WEIGHT = 2 X 24 = 47 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	397	0	397	0	0	7-7-11	7-7-11
D	397	0	397	0	0	7-7-11	7-7-11
F	400	0	400	0	0	7-7-11	7-7-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	288	187 / 0	38 / 0	0 / 0	0 / 0	64 / 0	0 / 0
D	288	187 / 0	38 / 0	0 / 0	0 / 0	64 / 0	0 / 0
F	309	139 / 0	85 / 0	0 / 0	0 / 0	85 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-102.1	-102.1	0.01 (1)	10.00		F- C	-132 / 0	0.01 (1)
B- H	-81 / 0	-102.1	-102.1	0.04 (1)	6.25		G- H	-399 / 26	0.00 (1)
H- C	-175 / 0	-102.1	-102.1	0.09 (1)	6.25		I- J	-399 / 26	0.00 (1)
C- J	-175 / 0	-102.1	-102.1	0.09 (1)	6.25				
J- D	-81 / 0	-102.1	-102.1	0.04 (1)	6.25				
D- E	0 / 17	-102.1	-102.1	0.01 (1)	10.00				
B- G	0 / 132	-38.5	-38.5	0.09 (1)	10.00				
G- F	0 / 132	-38.5	-38.5	0.09 (1)	10.00				
F- I	0 / 132	-38.5	-38.5	0.09 (1)	10.00				
I- D	0 / 132	-38.5	-38.5	0.09 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 26.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (C-J:1) , BC=0.09/1.00 (F-G:1) , WB=0.01/1.00 (C-F:1) , SSI=0.16/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (D) (INPUT = 0.90)
JSI METAL= 0.04 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H.J. G ALVES
100089024
PROVINCE OF ONTARIO

BCIN NO. TAM 71907821
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402113	J9	18	1	GREEN PARK HOMES TRUSS DESC.	J9

Tamarack Roof Truss, Burlington

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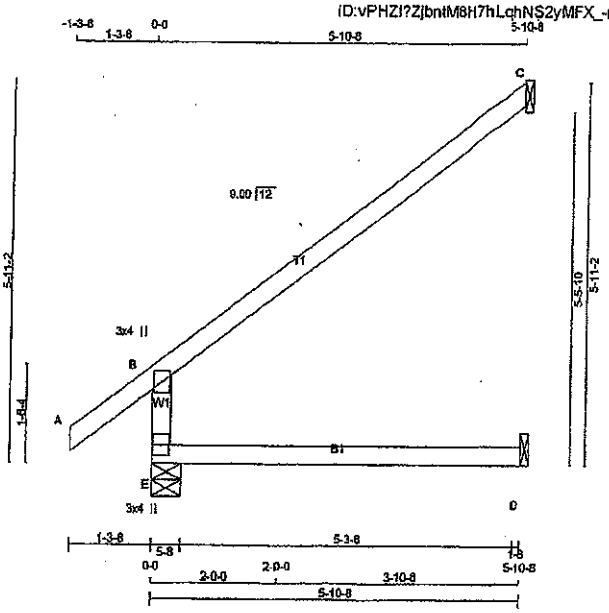

TOTAL WEIGHT = 18 X 18 = 331 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	646	0	648	0	0	5-8	5-8
C	225	0	225	0	0	1-8	1-8
D	98	0	122	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	293 / 0	71 / 0	0 / 0	0 / 0	108 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	87	0 / 0	52 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
E- B	-516 / 0	0.0	0.0	0.19 (3)		7.81			
A- B	0 / 42	-102.1	-102.1	0.14 (1)		10.00			
B- C	-45 / 0	-102.1	-102.1	0.60 (1)		6.25			
E- D	0 / 0	-38.5	-38.5	0.22 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 676 (0.08")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.22/1.00 (D-E:3) , WB=0.00/1.00 (n/a:0) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G ALVES
100009624
PROVINCE OF ONTARIO

DWG NO. TAM 7907792
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402113	C1	2	1	GREEN PARK HOMES TRUSS DESC.	C1

Tamarack Roof Truss, Burlington

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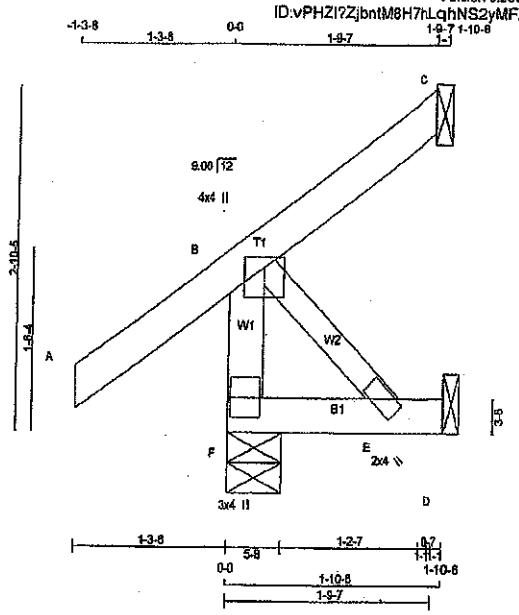


TOTAL WEIGHT = 2 X 9 = 19 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	322	0	322	0	0	5-8	5-8
C	37	0	37	0	-45	1-8	1-8
D	36	0	46	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	228	163 / 0	20 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	25	21 / -31	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	33	0 / 0	20 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

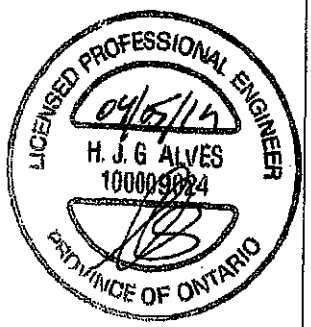
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO
F- B	-288 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	-33 / 0	-102.1	-102.1	0.13 (1)	6.25
F- E	0 / 0	-38.5	-38.5	0.03 (3)	10.00
E- D	0 / 0	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (E-F:3) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	768	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DWG NO. TAM1907793
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402113	C2	2	1	GREEN PARK HOMES TRUSS DESC.	C2

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Fri Apr 5 14:25:44 2019 Page 1
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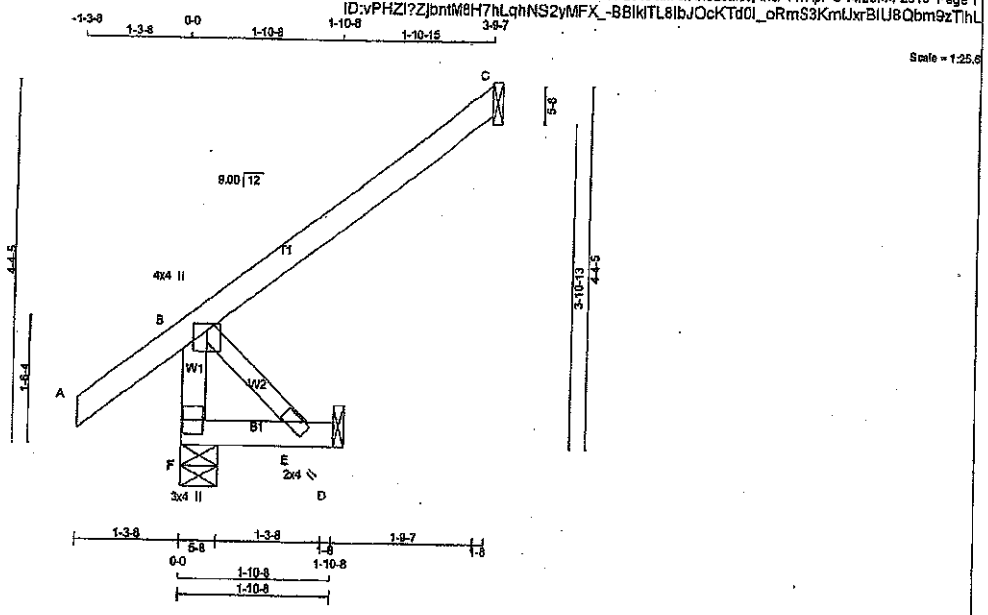

TOTAL WEIGHT = 2 X 12 = 24 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	370	0	370	0	0	5-8	5-8
C	193	0	193	0	0	1-8	1-8
D	36	0	46	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 837821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	262	190 / 0	20 / 0	0 / 0	0 / 0	52 / 0	0 / 0
C	133	110 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	33	0 / 0	20 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-334 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00			
B- C	0 / 0	-102.1	-102.1	0.25 (1)	10.00			
F- E	0 / 0	-38.5	-38.5	0.04 (3)	10.00			
E- D	0 / 0	-38.5	-38.5	0.03 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.04/1.00 (E-F:3) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1967	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
04/06/19
H. J. G ALVES
100090924
PROVINCE OF ONTARIO

DWG NO. TAM T1907794
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
402113	C3	2	1	TRUSS DESC.	C3

Tamarack Roof Truss, Burlington

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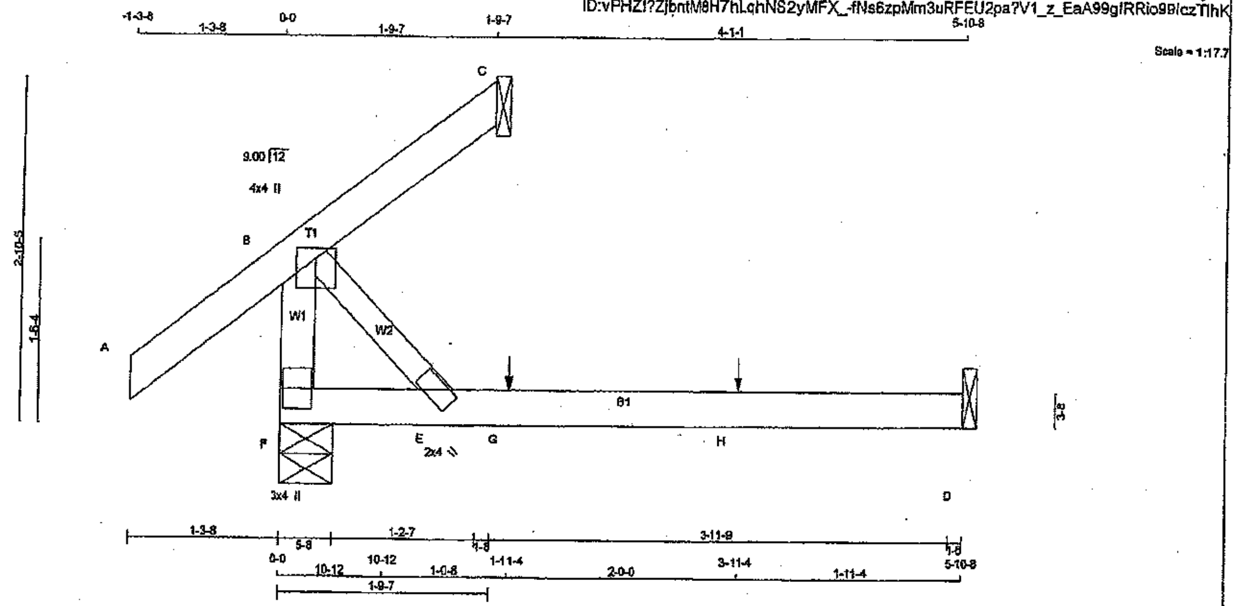

TOTAL WEIGHT = 2 X 14 = 28 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	399	0	399	0	0	5-8	5-8
C	37	0	37	0	0	1-8	1-8
D	113	0	144	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	299	163 / 0	62 / 0	0 / 0	0 / 0	75 / 0	0 / 0
C	25	21 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	103	0 / 0	62 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-286 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 42	-102.1	-102.1	0.15 (1)	10.00
B- C	-33 / 0	-102.1	-102.1	0.15 (1)	6.25
F- E	0 / 0	-38.5	-38.5	0.22 (2)	10.00
E- G	0 / 0	-38.5	-38.5	0.32 (2)	10.00
G- H	0 / 0	-38.5	-38.5	0.32 (2)	10.00
H- D	0 / 0	-38.5	-38.5	0.32 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	—	BACK	VERT	TOTAL	—	—
H	3-11-4	1	1	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J G ALVES
100009624
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 925 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.13")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.32/1.00 (D-E:2) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DWG NO. TAM 17407195
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402113	C4	2	1	GREEN PARK HOMES TRUSS DESC.	C4

Tamarack Roof Truss, Burlington

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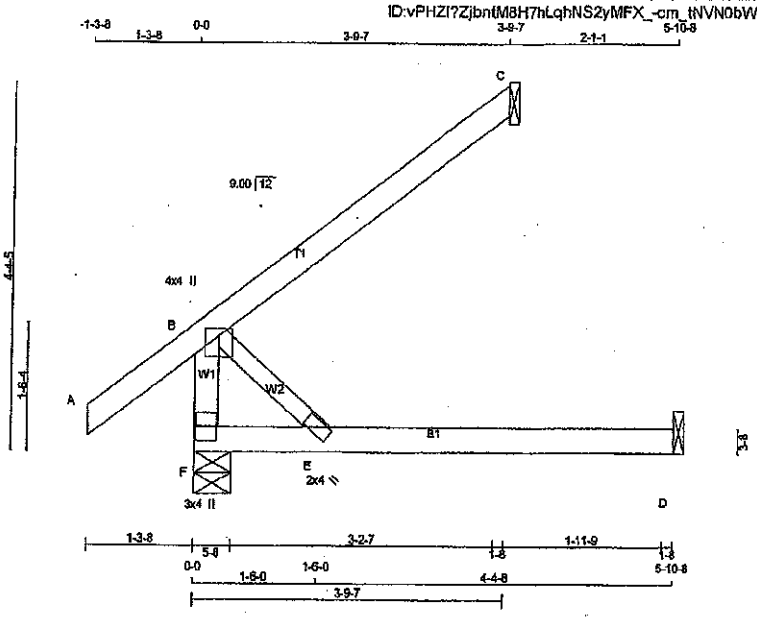

TOTAL WEIGHT = 2 X 17 = 34 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED DOWN	MAXIMUM FACTORED HORZ	MAXIMUM FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	447	0	447	0	0	5-8	5-8
C	193	0	193	0	0	1-8	1-8
D	113	0	144	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	332	190 / 0	62 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	133	110 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	103	0 / 0	62 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-334 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 0	-102.1	-102.1	0.25 (1)	10.00
F- E	0 / 0	-38.5	-38.5	0.23 (3)	10.00
E- D	0 / 0	-38.5	-38.5	0.39 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 925 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.13")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.30/1.00 (D-E:3) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM P9007796
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	J22	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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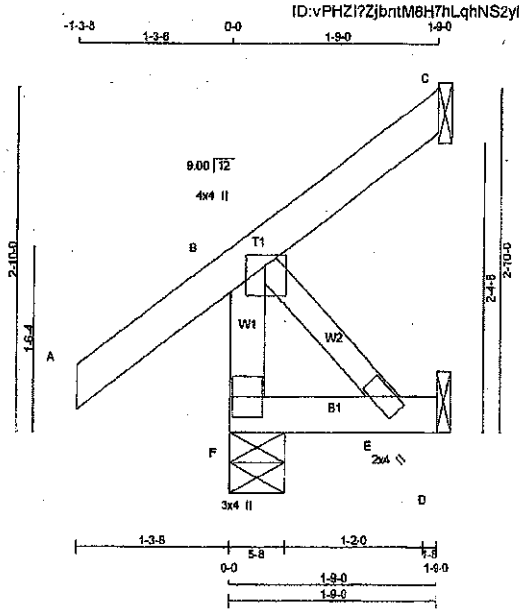


TOTAL WEIGHT = 2 X 9 = 18 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	319	0	319	0	0	5-8	5-8
C	34	0	34	0	-46	1-8	1-8
D	34	0	43	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	227	162 / 0	18 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	23	19 / -32	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	31	0 / 0	18 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-286 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	-33 / 0	-102.1	-102.1	0.13 (1)	6.25
F- E	0 / 0	-38.5	-38.5	0.03 (3)	10.00
E- D	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (E-F:3) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

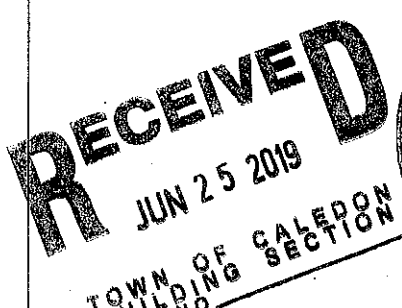
NAIL VALUES

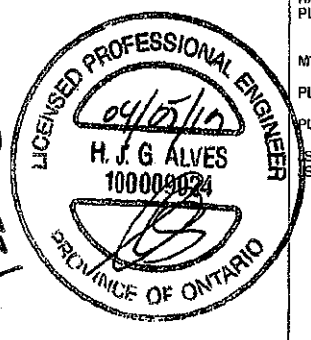
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.23 (B) (INPUT = 0.90)
SI METAL= 0.06 (B) (INPUT = 1.00)





DWG NO. T1907822
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	J23	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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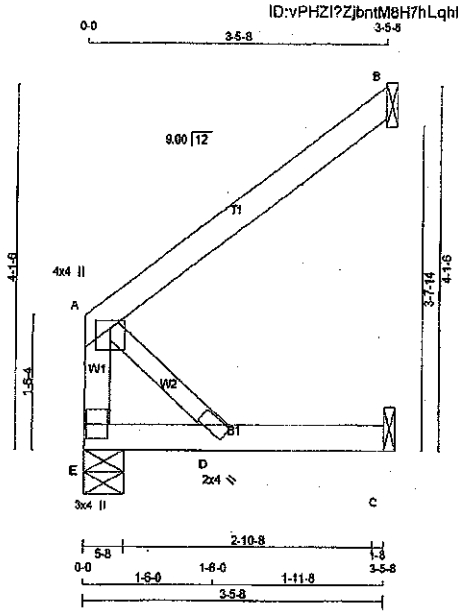
Scale: 1/2"=1'



TOTAL WEIGHT = 12 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	243	0	243	0	0	5-8	5-8
B	177	0	177	0	0	1-8	1-8
C	67	0	85	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	182	100 / 0	36 / 0	0 / 0	0 / 0	45 / 0	0 / 0
B	121	100 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0
C	61	0 / 0	36 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
E- A		-177 / 0	0.0	0.0	0.02 (1)		7.81
A- B		0 / 0	-102.1	-102.1	0.21 (1)		10.00
E- D		0 / 0	-38.5	-38.5	0.10 (3)		10.00
D- C		0 / 0	-38.5	-38.5	0.10 (3)		10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- D		0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.21/1.00 (A-B:1) , BC=0.10/1.00 (C-D:3) , WB=0.00/1.00 (A-D:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USE GRIP= 0.14 (A) (INPUT = 0.90)
USE METAL= 0.04 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

DWG NO. T.No. T1707823
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	J24	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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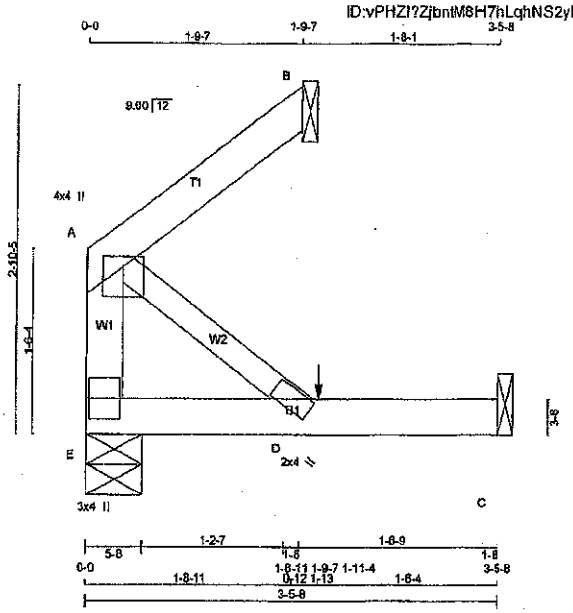

TOTAL WEIGHT = 10 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	158	0	158	0	0	5-8	5-8
B	91	0	91	0	0	1-8	1-8
C	67	0	85	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	123	52 / 0	36 / 0	0 / 0	0 / 0	35 / 0	0 / 0
B	63	52 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	61	0 / 0	36 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-91 / 0	0.0	0.0	0.01 (1)	7.81	A- D	0 / 0		0.00 (1)
A- B	0 / 0	-102.1	-102.1	0.05 (1)	10.00				
E- D	0 / 0	-38.5	-38.5	0.11 (3)	10.00				
D- C	0 / 0	-38.6	-38.6	0.11 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	1-11-4	1	1	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF SCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.06/1.00 (A-B:1) , BC=0.11/1.00 (C-D:2) , WB=0.00/1.00 (A-D:1) , SSI=0.07/1.00 (D-E:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

BCIN NO. TAM 7007824
STRUCTURAL
CERTIFICATION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	J25	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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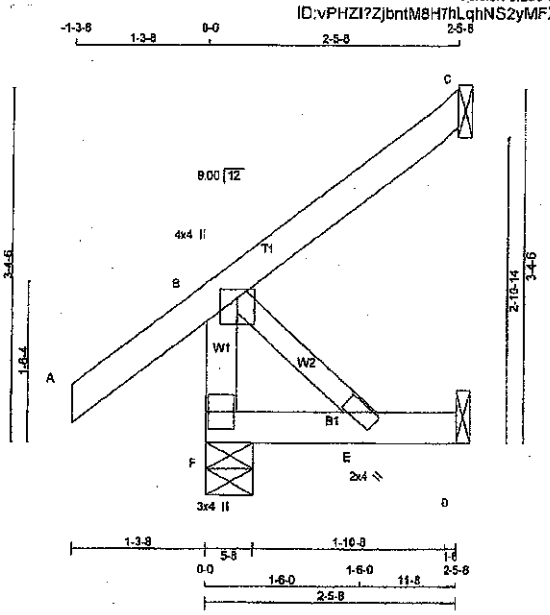

TOTAL WEIGHT = 2 X 11 = 22 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	314	0	314	0	0	5-8	5-8
C	126	0	126	0	0	1-8	1-8
D	47	0	60	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	226	151 / 0	26 / 0	0 / 0	0 / 0	49 / 0	0 / 0
C	86	71 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
D	43	0 / 0	26 / 0	0 / 0	0 / 0	17 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-266 / 0	0.0	0.0	0.03 (1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00			
B- C	0 / 0	-102.1	-102.1	0.10 (1)	10.00			
F- E	0 / 0	-36.5	-36.5	0.06 (3)	10.00			
E- D	0 / 0	-36.5	-36.5	0.05 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.06/1.00 (E-F:3) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI:GRIP= 0.21 (B) (INPUT = 0.90)
SSI:METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

DWG NO. TAM 71907825
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	J27	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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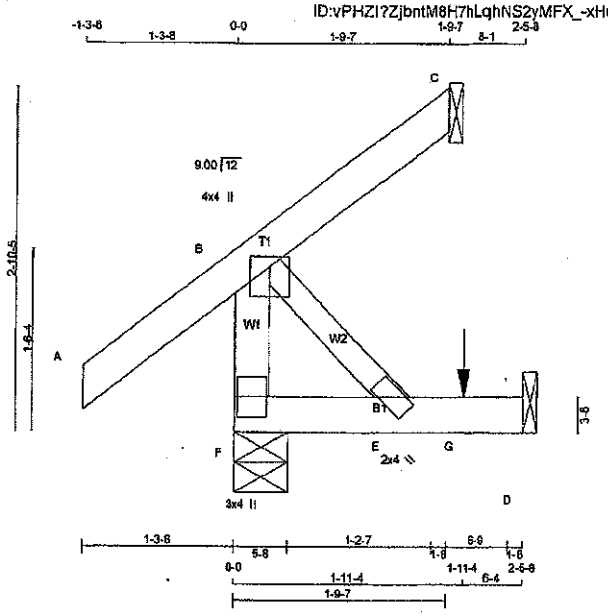

TOTAL WEIGHT = 10 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	335	0	335	0	0	5-8	5-8
C	37	0	37	0	-45	1-8	1-8
D	53	0	67	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	241	163 / 0	27 / 0	0 / 0	0 / 0	51 / 0	0 / 0
C	25	21 / -31	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	48	0 / 0	29 / 0	0 / 0	0 / 0	19 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-266 / 0	0.0	0.0	0.03	(1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 42	-102.1	-102.1	0.14	(1)	10.00			
B- C	-33 / 0	-102.1	-102.1	0.13	(1)	6.25			
F- E	0 / 0	-38.5	-38.5	0.05	(3)	10.00			
E- G	0 / 0	-38.5	-38.5	0.05	(3)	10.00			
G- D	0 / 0	-38.5	-38.5	0.05	(3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-7	-9	—	BACK	VERT	TOTAL	—	—

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.05/1.00 (E-F:3) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI:RIGID= 0.23 (B) (INPUT = 0.90)
JSI:METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/04/19
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

DWG NO. TAM 1709826
STRUCTURAL
CONSTRUCTION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	J28	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Fri Apr 5 16:35:59 2019 Page 1
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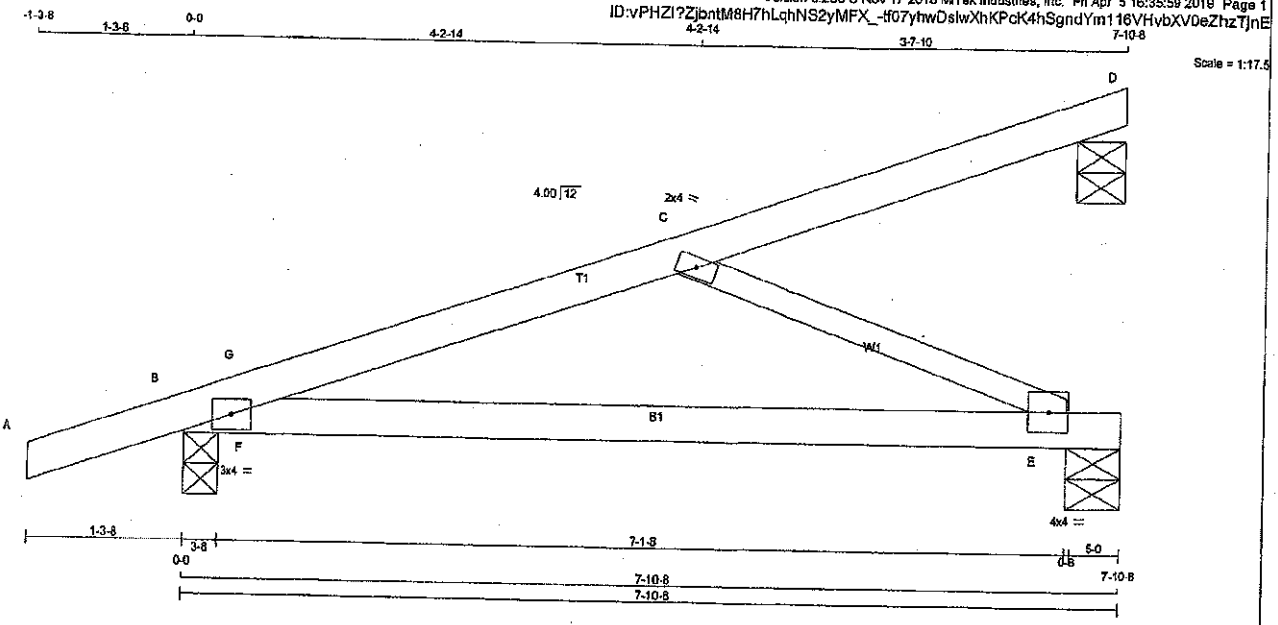


TOTAL WEIGHT = 4 X 23 = 92 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TMW+w	MT20	2.0	4.0	
E	BMW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	130	0	130	0	0	5-0	5-0
B	655	0	655	0	0	3-8	3-8
E	435	0	435	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	86	62 / 0	0 / -8	0 / 0	0 / 0	12 / 0	0 / 0
B	481	294 / 0	76 / 0	0 / 0	0 / 0	111 / 0	0 / 0
E	333	159 / 0	85 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13 (1)		10.00
B- G	-833 / 0	-102.1	-102.1	0.24 (3)		6.25
G- C	-679 / 0	-102.1	-102.1	0.33 (2)		6.25
C- D	-18 / 0	-102.1	-102.1	0.23 (1)		6.25
B- F	0 / 678	-38.5	-38.5	0.23 (3)		10.00
F- E	0 / 678	-38.5	-38.5	0.39 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- E	-747 / 0	0.19 (1)
F- G	0 / 416	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 815 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 486 (0.18")

CSI: TC=0.33/1.00 (C-G:2) , BC=0.39/1.00 (E-F:2) , WB=0.19/1.00 (C-E:1) , SSI=0.29/1.00 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

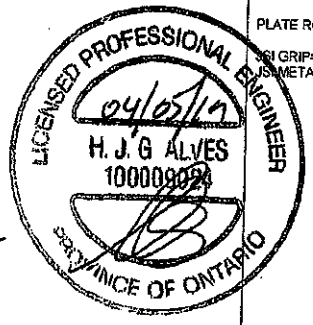
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.65 (C) (INPUT = 0.90)
SSI METAL= 0.40 (C) ((INPUT = 1.00)

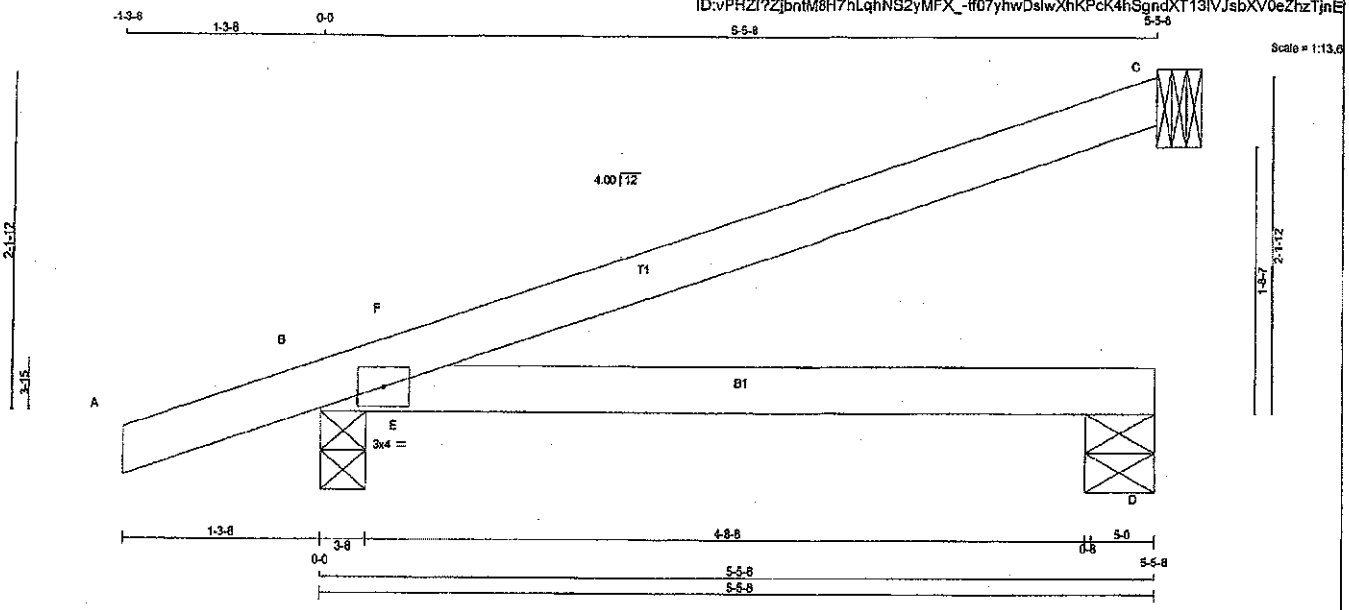


DWG NO. TAM 1901827
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402114	J29	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Fri Apr 5 16:35:59 2019 Page 1
ID:vPHZI?ZjbntM8H7hLqhNS2yMFX_-tf07yhwDslwXhKPcK4hSgndXT13IVJsbXV0eZhzTjnE



TOTAL WEIGHT = 3 X 14 = 43 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB14	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	250	0	250	0	0	5-0	5-0
B	520	0	520	0	0	3-8	3-8
D	133	0	145	0	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	380	236 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-102.1	-102.1	1	0.13 (1)	10.00	E- F	-254 / 122	0.00 (1)
B- F	-51 / 0	-102.1	-102.1	3	0.13 (3)	5.25			
F- C	0 / 6	-102.1	-102.1	1	0.42 (1)	10.00			
B- E	0 / 0	-38.5	-38.5	1	0.26 (1)	10.00			
E- D	0 / 0	-38.5	-38.5	1	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 842 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 489 (0.13")

CSI: TC=0.42/1.00 (C-F:1) , BC=0.31/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LS GRIP = 0.31 (B) (INPUT = 0.90)
SSI TOTAL = 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

BCIN NO. TAM 17907828
STRUCTURAL
CERTIFICATION BY

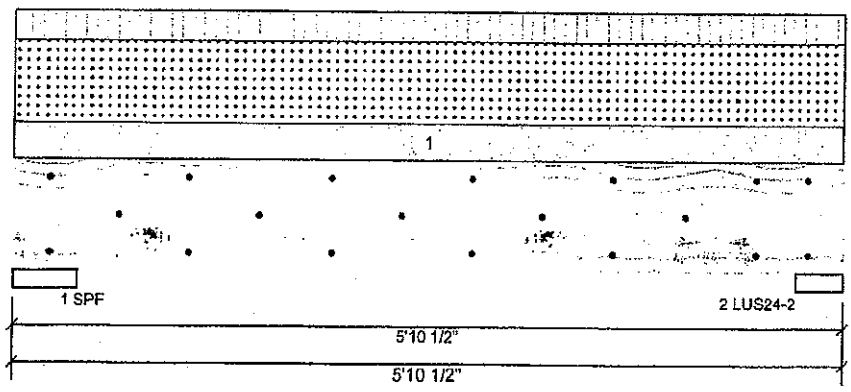
isDesign™

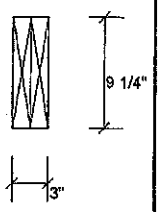
Client:
Project:
Address:

Date: 4/5/2019
Designer:
Job Name: 200778
Project #:

BM20 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	168	208	465	0
2	161	199	445	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	11%	260 / 865	1125	L	1.25D+1.5S +L
2 - LUS24-2	4.000"	15%	249 / 829	1078	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1272 ft-lb	3'	6039 ft-lb	0.211 (21%)	1.25D+1.5S +L	L
Unbraced	1272 ft-lb	3'	5236 ft-lb	0.243 (24%)	1.25D+1.5S +L	L
Shear	977 lb	1'2"	3984 lb	0.245 (25%)	1.25D+1.5S +L	L
LL Defl inch	0.011 (L/5628)	3'	0.174 (L/360)	0.060 (6%)	S+0.5L	L
TL Defl inch	0.015 (L/4079)	3'	0.174 (L/360)	0.090 (9%)	D+S+0.5L	L

Design Notes

1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

BWG NO. TAM 1907832
STRUCTURAL
COMPONENT

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		5-4-0	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RECEIVED
JUN 25 2019
TOWN OF LEDON
BUILDING SECTION
FILE NO

This design is valid until 12/11/2021

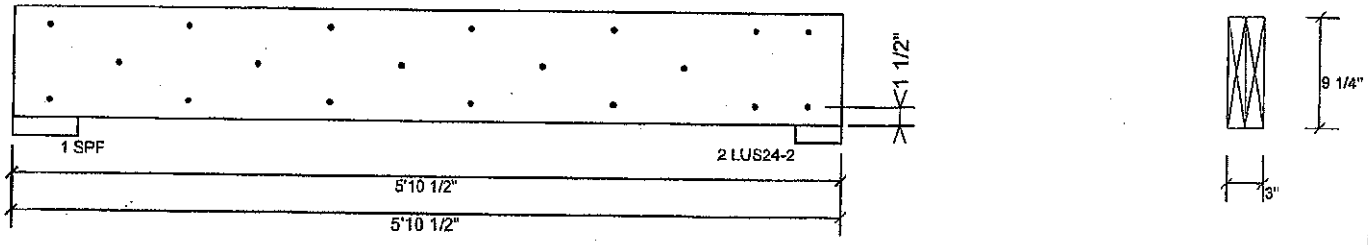
Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

BM20 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	55.1 %
Load	187.5 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

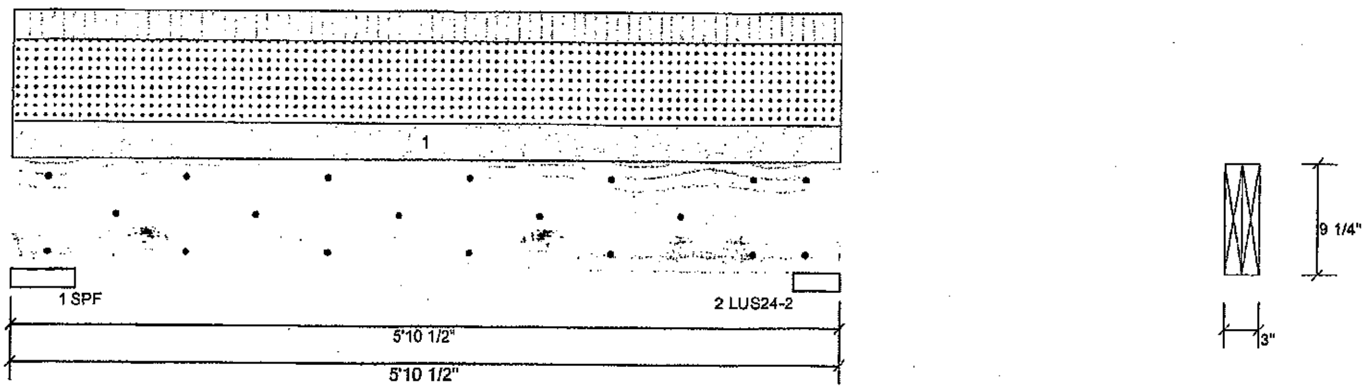
BWG NO. Info 1407832
STRUCTURAL
COMPONENT ONLY

Manufacturer Info	Tamarack Roof Trusses 3289 North Service Rd., ON Canada L7N3G2 (905) 335-1115

This design is valid until 12/11/2021

BM21 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	197	244	545	0
2	189	234	522	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	13%	305 / 1014	1318	L	1.25D+1.5S +L
2 - LUS24-2	4.000"	17%	292 / 971	1263	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1490 ft-lb	3'	6039 ft-lb	0.247 (25%)	1.25D+1.5S +L	L
Unbraced	1490 ft-lb	3'	5236 ft-lb	0.285 (28%)	1.25D+1.5S +L	L
Shear	1144 lb	1'2"	3984 lb	0.287 (29%)	1.25D+1.5S +L	L
LL Defl inch	0.013 (L/4801)	3'	0.174 (L/360)	0.070 (7%)	S+0.5L	L
TL Defl inch	0.018 (L/3481)	3'	0.174 (L/360)	0.100 (10%)	D+S+0.5L	L

Design Notes

1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM 71907833
STRUCTURAL
COMPONENT ONLY
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-3-0	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

Manufacturer Info

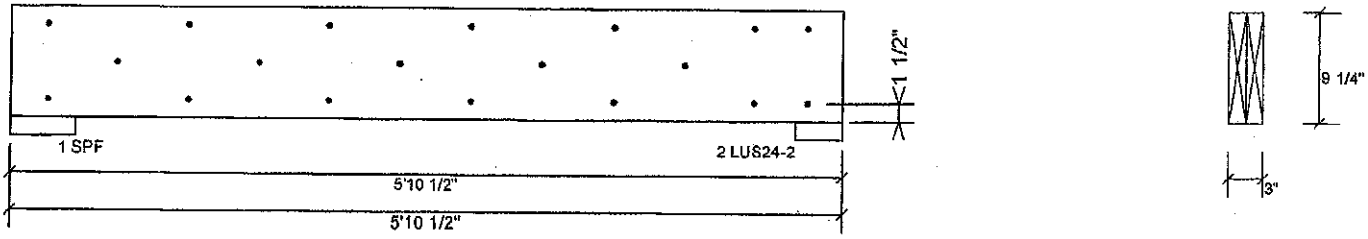
Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

TAMARACK LUMBER INC
ALFA LUMBER GROUP

This design is valid until 12/11/2021

BM21 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	64.6 %
Load	219.7 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
H. J. G ALVES
100008084
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 1907833
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	Tamarack Roof Trusses 3269 North Service Rd., ON Canada L7N3G2 (905) 335-1115

This design is valid until 12/11/2021

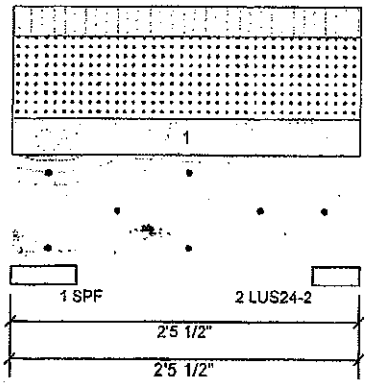
isDesign™

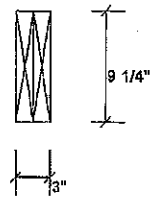
Client:
Project:
Address:

Date: 4/5/2019
Designer:
Job Name: 200778
Project #:

BM22 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	480	Building Code:	NBCC 2015
Deflection TL:	240	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	34	42	94	0
2	31	38	85	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	2%	52 / 175	227	L	1.25D+1.5S +L
2 - LUS24-2	4.000"	3%	47 / 158	205	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	71 ft-lb	1'3 1/2"	6039 ft-lb	0.012 (1%)	1.25D+1.5S +L	L
Unbraced	71 ft-lb	1'3 1/2"	5944 ft-lb	0.012 (1%)	1.25D+1.5S +L	L
Shear	157 lb	1'2"	3984 lb	0.040 (4%)	1.25D+1.5S +L	L
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		

Design Notes

1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DESIGN FOR TAM 71907834
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-6-0	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

This design is valid until 12/11/2021

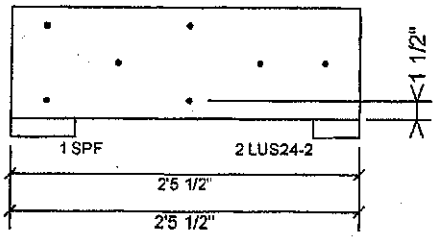
Manufacturer Info

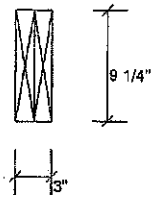
Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

BM22 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

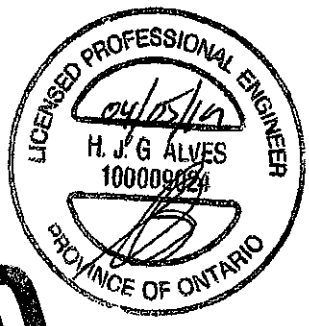




Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	25.8 %
Load	87.9 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



Manufacturer Info	Tamarack Roof Trusses 3289 North Service Rd., ON Canada L7N3G2 (905) 335-1115

This design is valid until 12/11/2021

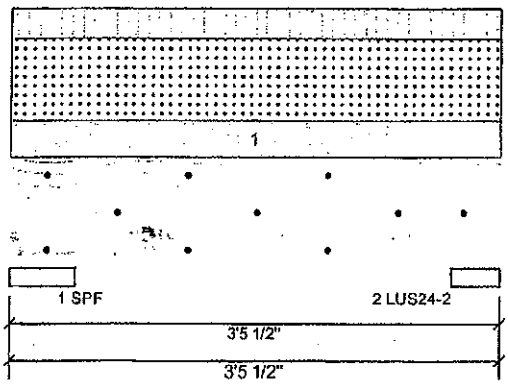
isDesign™

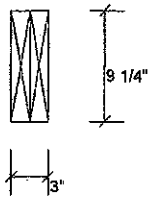
Client:
Project:
Address:

Date: 4/5/2019
Designer:
Job Name: 200778
Project #:

BM23 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	119	148	329	0
2	111	137	306	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	8%	184 / 613	797	L	1.25D+1.5S +L
2 - LUS24-2	4.000"	10%	172 / 570	742	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	433 ft-lb	1'9 1/2"	6039 ft-lb	0.072 (7%)	1.25D+1.5S +L	L
Unbraced	433 ft-lb	1'9 1/2"	5808 ft-lb	0.075 (7%)	1.25D+1.5S +L	L
Shear	621 lb	1'2"	3984 lb	0.156 (16%)	1.25D+1.5S +L	L
LL Defl inch	0.001 (L/30802)	1'9 1/2"	0.093 (L/360)	0.010 (1%)	S+0.5L	L
TL Defl inch	0.002 (L/22327)	1'9 1/2"	0.093 (L/360)	0.020 (2%)	D+S+0.5L	L

Design Notes

1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
04/05/19
H. S. G. ALVES
100089029
PROVINCE OF ONTARIO

DESIGN FOR TAM 17007835
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-4-0	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

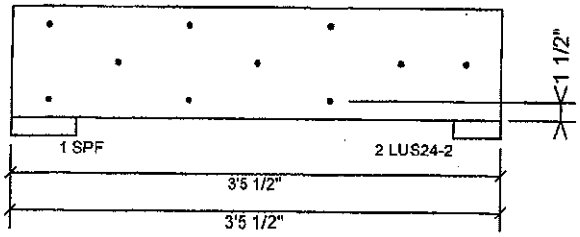
isDesign™

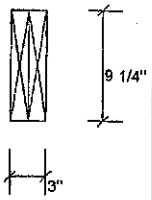
Client:
Project:
Address:

Date: 4/5/2019
Designer:
Job Name: 200778
Project #:

BM23 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	65.4 %
Load	222.5 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

LICENSED PROFESSIONAL ENGINEER
H. J. G ALVES
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

...FAM 7907835
...STRUCTURAL
...CONSULTANT ONLY

Manufacturer Info	Tamarack Roof Trusses 3269 North Service Rd., ON Canada L7N3G2 (905) 335-1115

This design is valid until 12/11/2021

TECHNICAL BULLETIN

THGQ/THGQH – Truss Girder Hangers

SIMPSON Strong-Tie®

MATERIAL: THGQ—7 gauge, THGQH—3 gauge

FINISH: THGQ—G90 Galvanized,
THGQH—Simpson Strong-Tie® gray paint

DESIGN:

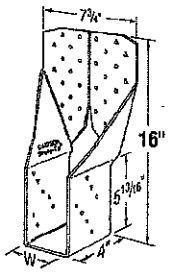
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% for short term load duration. No further increase is allowed.
- Designer must ensure that vertical web member supporting a hanger is capable of resisting loads based on net cross section
- Girder truss must be a minimum of 2 plys
- Bearing assumes Q_r/A_b and Q'_r/A'_b = 812 psi D.Fir-L and 615 psi S-P-F. Truss plates on supported member must be as per 6.5.4 and 7.5.9 TPIC 2014 to archieve values shown.
- All multiple members must be fastened together to act as a single unit independent of the hanger fasteners
- Girders must be adequately laterally braced to prevent excessive displacement due to secondary torsional stresses

INSTALLATION:

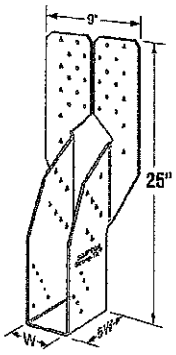
- Use all specified fasteners
- Fill all round holes for min values and all round and triangle holes for max values
- Strong-Drive® SDS screws driven through truss plates must be approved by the truss designer. Pre-drilling, using a 5⁄32" bit, is required.
- Connector must be installed, centred on girder vertical web.

OPTIONS:

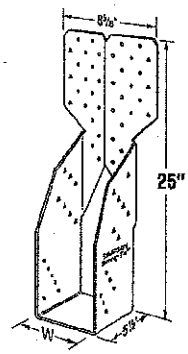
- These hangers may be skewed 45°
- See current catalogue for options



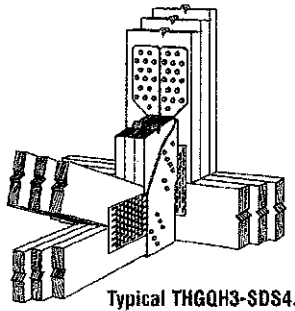
THGQ2-SDS3
(THGQ3-SDS3 Similar)



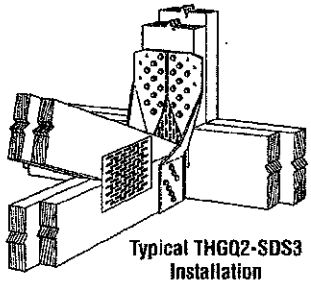
THGQH2-SDS3



THGQH3-SDS4.5
(THGQH4-SDS6 Similar)



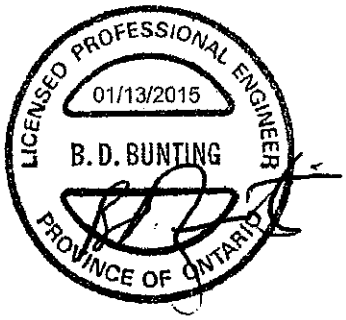
Typical THGQH3-SDS4.5 Installation



Typical THGQ2-SDS3 Installation

Model No.	Width W (in)	Max. B.C. Depth	Min Vert Web Size	Fasteners		Factored Resistance (lbs)			
						D.Fir-L		S-P-F	
				Header	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
THGQ2-SDS3 (Min)	3⁹⁄₁₆	2x6	2x8	22-SDS¼"x3"	10-SDS¼"x3"	5205	11655	3750	8395
THGQ2-SDS3 (Max)	3⁹⁄₁₆	2x6	2x10	28-SDS¼"x3"	14-SDS¼"x3"	6555	18055	4720	13000
THGQH2-SDS3 (Min)	3⁹⁄₁₆	2x10	2x8	18-SDS¼"x3"	14-SDS¼"x3"	5790	12555	4170	9040
THGQH2-SDS3 (Max)	3⁹⁄₁₆	2x10	2x10	28-SDS¼"x3"	26-SDS¼"x3"	14190	18455	10215	15160
THGQ3-SDS4.5 (Min)	4¹⁵⁄₁₆	2x6	2x8	22-SDS¼"x4½"	10-SDS¼"x4½"	5205	11655	3750	8395
THGQ3-SDS4.5 (Max)	4¹⁵⁄₁₆	2x6	2x10	28-SDS¼"x4½"	14-SDS¼"x4½"	6555	17760	4720	12785
THGQH3-SDS4.5 (Min)	4¹⁵⁄₁₆	2x10	2x10	32-SDS¼"x4½"	14-SDS¼"x4½"	5790	17860	4170	12860
THGQH3-SDS4.5 (Max)	4¹⁵⁄₁₆	2x10	2x12	38-SDS¼"x4½"	26-SDS¼"x4½"	14190	21055	10215	15160
THGQH4-SDS6 (Min)	6⁹⁄₁₆	2x12	2x10	34-SDS¼"x6"	14-SDS¼"x6"	5790	17860	4170	12860
THGQH4-SDS6 (Max)	6⁹⁄₁₆	2x12	2x12	40-SDS¼"x6"	26-SDS¼"x6"	14190	24870	10215	17905

1. Factored resistances have been increased 15% for earthquake or wind load with no further increase allowed; reduce where other loads govern.
2. Minimum 2-ply girder required.
3. Connector must be installed centred on girder vertical webs.
4. The thickness of the supporting girder must be equal to or greater than the screw length. For applications where the length of the supplied screws exceeds the thickness of the supporting girder, 3" or 4½" screws may be substituted for the longer length screws with no load reduction, or a shim block may used as approved by the Designer.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

T-SPECTHGQ15 1/15 exp 12/16

800-999-5099
www.strongtie.com

HHUS/HGUS

See Hanger Options information on pp. 125–127.

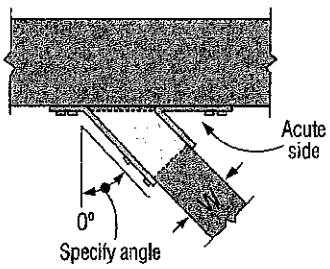
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⁹⁄₁₆	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⁹⁄₁₆	3	1⅝	2¹⁵⁄₁₆	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⁹⁄₁₆	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⁹⁄₁₆	4¾	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705 / 11.90	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.96	6625 / 29.5	2685 / 11.96	5700 / 25.35
LU28L	20	1⁹⁄₁₆	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.19	3310 / 14.74	6900 / 30.73
LU210L	20	1⁹⁄₁₆	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⁹⁄₁₆	7¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. **Nails:** 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

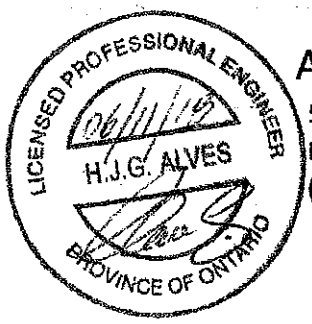
Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS26-2	14	3⁵⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS28-2	14	3⁵⁄₁₆	7⁵⁄₁₆	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HHUS210-2	14	3⁵⁄₁₆	9³⁄₁₆	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-3	14	4¹¹⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⁹⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
HHUS210-4	14	6⅛	8⅞	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⁹⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
LUS46	18	3⁹⁄₁₆	4¼	2	3⁷⁄₁₆	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
LUS48	18	3⁹⁄₁₆	6¾	2	3⁷⁄₁₆	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
LUS410	18	3⁹⁄₁₆	8¾	2	5⁹⁄₁₆	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⁷⁄₁₆	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

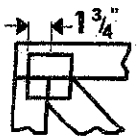
7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

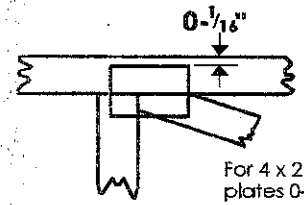
T-1800218 Feb 09, 2018

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

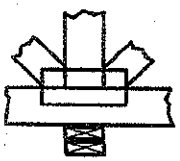
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

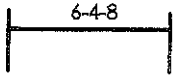
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

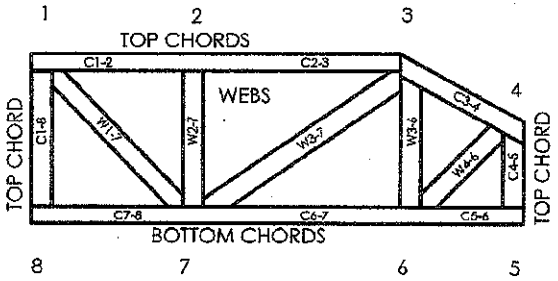
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

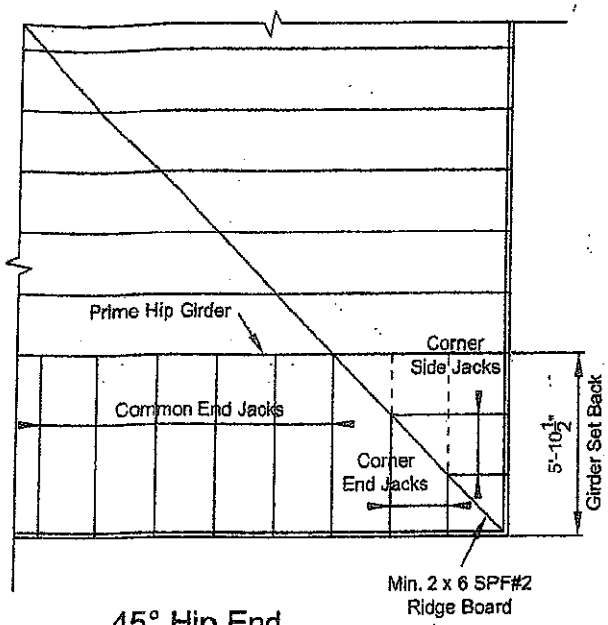
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



45° Hip End

LUMBER SPECIFICATION

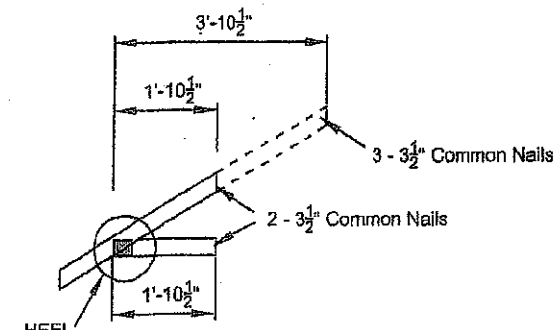
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

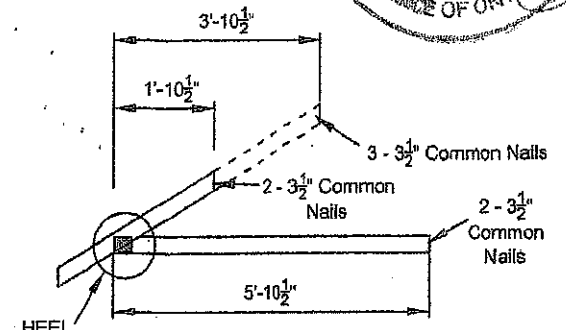
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

LICENSED PROFESSIONAL ENGINEER
06/11/19
H.J.G. ALVES
PROVINCE OF ONTARIO

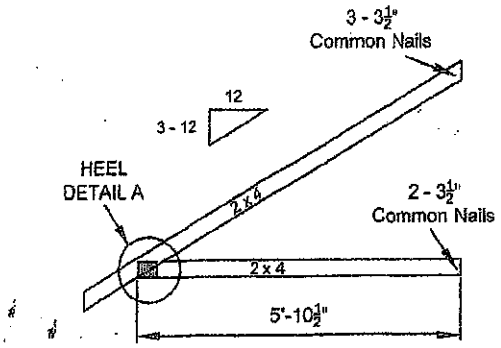
RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO



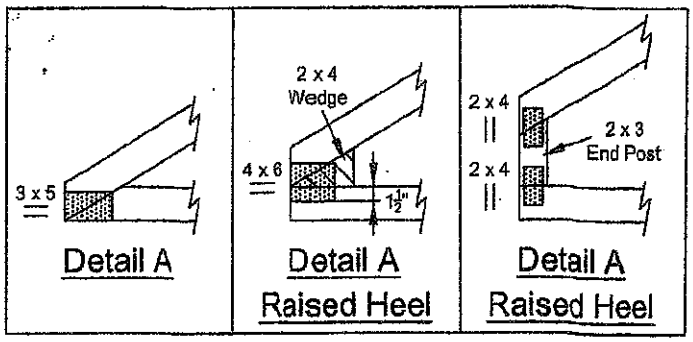
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1900216

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

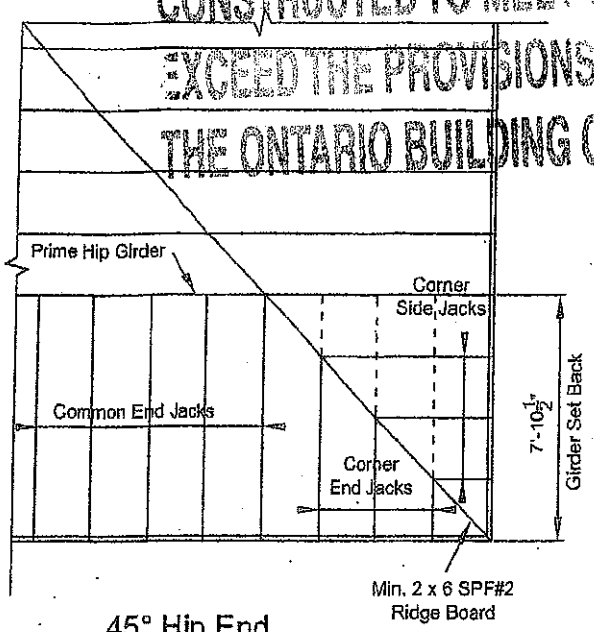
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

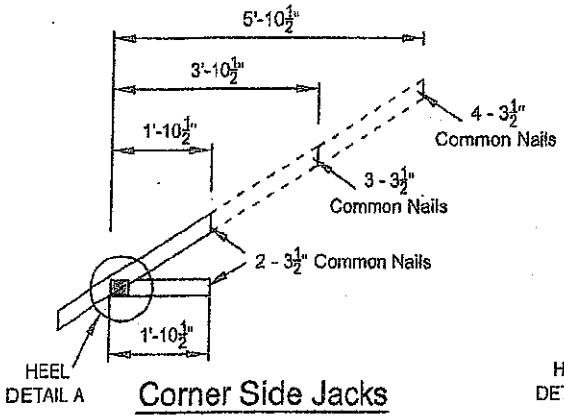
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

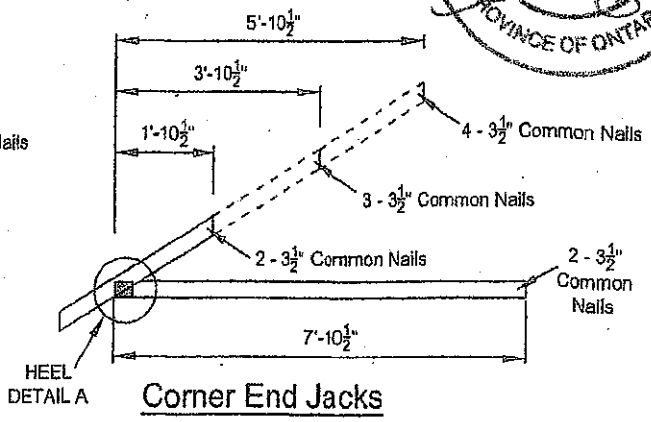
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



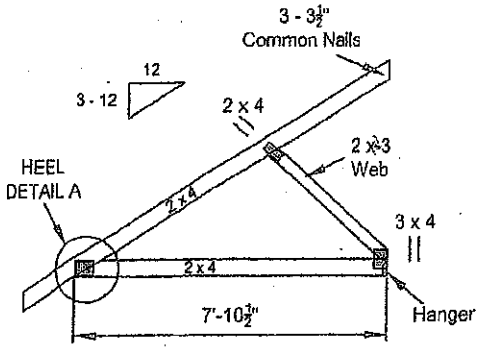
45° Hip End



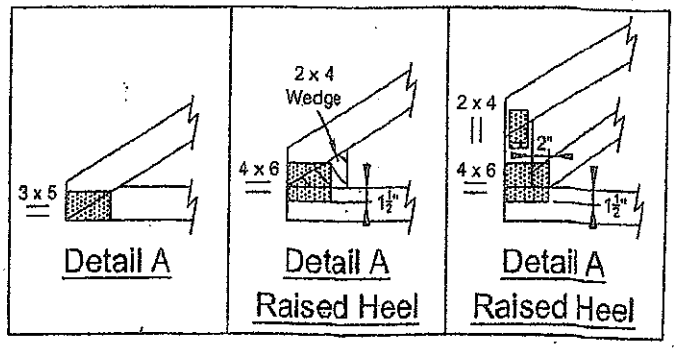
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217