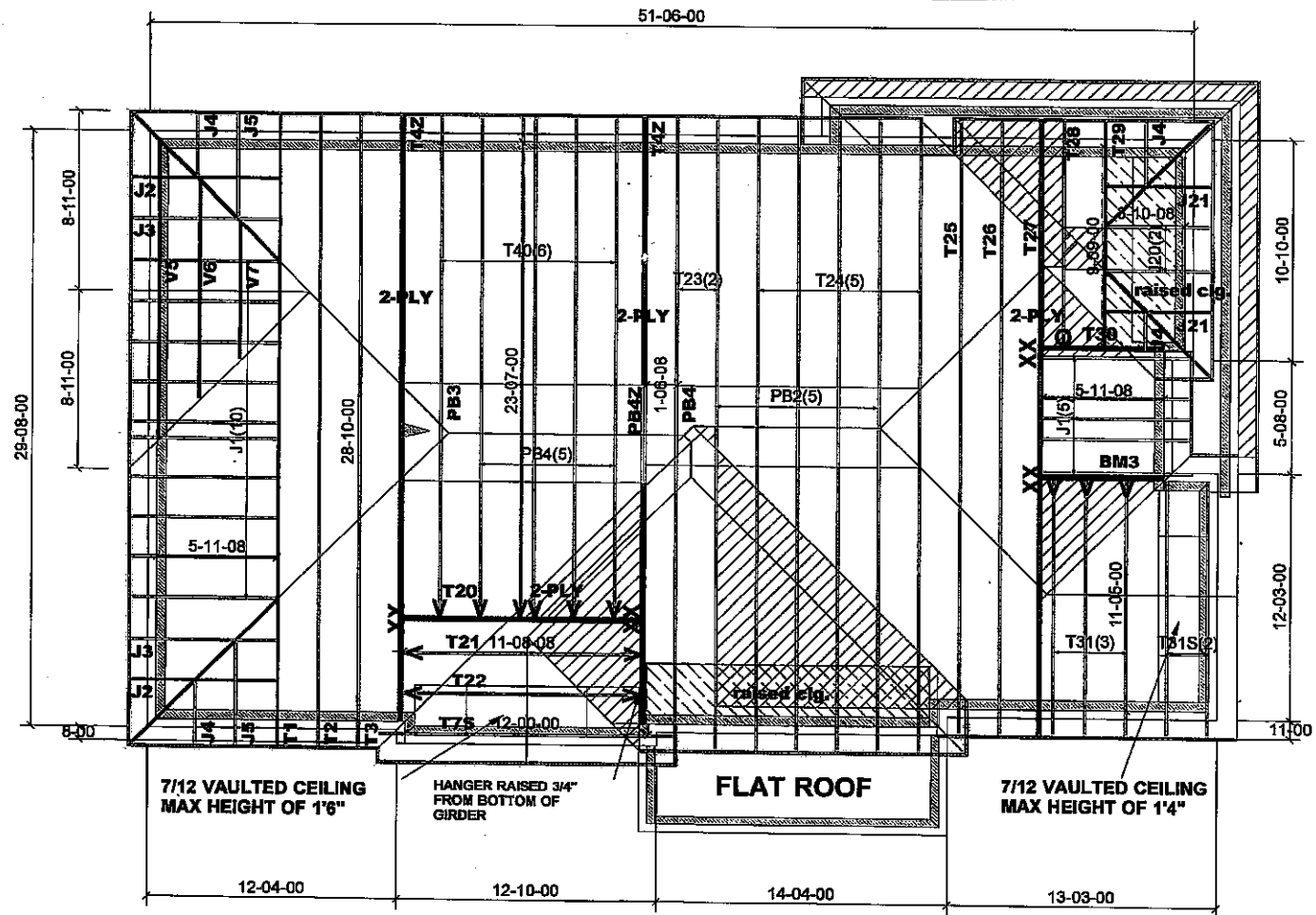


1/4" RAISED
CEILING/TOP PLATE

9/12 PITCH UNLESS
NOTED OTHERWISE
BM3: 2-2"x10" SPF #2



**TRUSS PROFILES TO BE VERIFIED BY
BUILDING DESIGNER**

ALL CONVENTIONAL ROOF FRAMING TO CONFORM WITH PART 9 OF THE OBC. ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"O.C. WITH 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED CONFORM WITH:
ONTARIO BUILDING CODE (2012)
OCCUPANCY: RESIDENTIAL | PART: 9

DESIGN LOADS:
CITY: CALEDON
G.S.L= 37.6 psf
TC DL= 6 psf
BC LL= 10.50 psf
BC DL= 7.00 psf

NOTES:
FIN. OH.: 12"
HEEL TYPE: R.T.M. CANT.
EXT. WALLS: 2X6
CLAD. TYPE 1: BRICK/5"
CLAD. TYPE 2: SIDING/0"
FSC SIZE: 2X6
SHEATHING: ASPHALT SHINGLE

IF DESIGNED COMMERCIAL, REFER TO
SEALED TRUSS DOCS FOR UPLIFT DESIGN

HARDWARE:

LJS26DS (V) 6pcs
HGUS26-2 (XX) 4pcs
LUS24 (O) 8pcs

CONV FRM BY OTHERS

T- 180737

COMMENTS:



Job Track: **50033**
Plan Log: **200664**
Layout ID: **401823**

Builder / Location:
GREEN PARK HOMES / CALEDON
Project: **LAMBERT LANE PH.2**
Date: 2019-03-20 Sales: Mario DiCano Designer: Brian/JG

Model / Elevation:
PRESTON 11 / 2- OPT. 5 BR.
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
Mitek ver 8.2.3.229



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 11
 Lot #:
 Elevation: 2 OPT 5 BED

Job Track: 50120
 Plan Log: 200172
 Layout ID: 400375
 Ref #
 Page: 1 of 3
 Date: 03/12/2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T1 Hip Girder	9/12	28-10-00	6-00-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	162.3 100.00		
	1	T2 Hip	9/12	28-10-00	7-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	126.61 79.17		
	1	T3 Hip	9/12	28-10-00	8-11-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	140.45 88.50		
	2 2-ply	T4Z Hip Girder	9/12	28-10-00	10-05-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	700.14 428.67		
	6	T40 Piggyback Base	9/12	23-07-00	10-05-14	2 x 4	1-03-08	1-06-04 5-05-08	754.34 467.00		
	1	T7S Roof Special	9/12 7/12	12-00-00	6-00-04	2 x 3 2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	50.01 33.33		
	1 2-ply	T20 Flat Girder	0/12	11-08-08	5-03-04	2 x 6		5-03-04 5-03-04	156.49 96.00		
	1	T21 Common	9/12	11-08-08	6-00-04	2 x 4		1-05-02 1-10-00	52.3 33.83		
	1	T22 Roof Special	9/12 7/12	11-08-08	6-00-04	2 x 3 2 x 4		1-05-02 1-10-00	45.6 30.00		
	2	T23 Hip	9/12	28-10-00	10-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	336.43 207.33		
	5	T24 Roof Special	9/12	28-10-00	10-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	796.49 494.17		
	1	T25 Hip	9/12	27-11-00	8-11-14	2 x 4	1-05-00 1-03-08	1-06-04 1-06-04	131.45 82.67		
	1	T26 Hip	9/12	27-11-00	7-05-14	2 x 4	1-05-00 1-03-08	1-06-04 1-06-04	124.22 76.83		
	1 2-ply	T27 Hip Girder	9/12	27-11-00	5-11-14	2 x 4 2 x 6	1-05-00 1-03-08	1-06-04 1-06-04	296.59 187.33		

THIS STRUCTURE MUST BE
 CONSTRUCTED TO MEET OR
 EXCEED THE PROVISIONS OF
 THE CANADIAN BUILDING CODE



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 11
 Lot #:
 Elevation: 2 OPT 5 BED

Job Track: 50120
 PlanLog: 200172
 Layout ID: 400375
 Ref #
 Page: 2 of 3
 Date: 03/12/2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T28 Hip	9 /12	9-09-00	5-10-02	2 x 4	1-03-08	2-10-04 3-00-08	52.67 34.67		
	1	T29 Hip Girder	9 /12	10-00-00	4-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	47.49 31.17		
	1 2-ply	T30 Roof Special Girder	0 /12	5-11-08	1-04-00	2 x 4		1-04-00 1-04-00	40.58 25.67		
	3	T31 Common	9 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	146.9 95.00		
	2	T31S Roof Special	9 /12 7 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	106.56 68.00		
	1	V5 Valley	9 /12	14-07-09	5-05-13	2 x 4			48.89 30.50		
	1	V6 Valley	9 /12	10-07-09	3-11-13	2 x 4			28.75 18.67		
	1	V7 Valley	9 /12	6-07-09	2-05-13	2 x 4			17.26 11.00		
	1	PB3 Piggyback	9 /12	4-11-00	1-06-00	2 x 4			12.09 8.33		
	6	PB4 Piggyback	9 /12	4-11-00	1-10-02	2 x 4			66.37 48.00		
	1 2-ply	PB4Z Piggyback	9 /12	4-11-00	1-10-02	2 x 4			22.12 16.00		
	15	J1 Jack-Open	9 /12	5-11-08	5-11-14	2 x 4	1-03-08	1-06-04 5-11-14	279.2 180.00		
	2	J2 Jack-Open Girder	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 4-02-01	1-06-04 2-10-05	28.62 20.33		
	2	J3 Jack-Open	9 /12	3-09-07	4-04-05	2 x 4	1-03-08 2-02-01	1-06-04 4-04-05	34.33 23.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 11
 Lot #:
 Elevation: 2 OPT 5 BED

Job Track: 50120
 PlanLog: 200172
 Layout ID: 400375
 Ref #
 Page: 3 of 3
 Date: 03/12/2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	J4 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	37.25 28.00		
	2	J5 Jack-Open	9 /12	1-10-08	4-04-05	2 x 4	1-03-08 1-10-15	1-06-04 2-11-02	24.44 16.67		
	2	J20 Jack-Open	9 /12	3-10-08	4-05-02	2 x 4	1-03-08	1-06-04 4-05-02	29.83 20.33		
	2	J21 Jack-Open Girder	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 2-01-01	1-06-04 2-10-05	23.87 17.67		

TOTAL # TRUSS= 79

TOTAL BFT OF ALL TRUSSES= 3097.84 BFT.

TOTAL WEIGHT OF ALL TRSSES 4920.64 LBS

HARDWARE

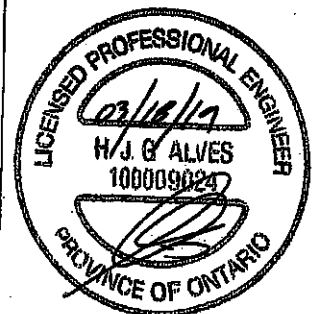
QTY	TYPE	MODEL	LENGTH
13	Hardware	LJS28DS	
1	Hardware	LUS24	
4	Hardware	HGUS28-2	

TOTAL NUMBER OF ITEMS= 18

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T1	1	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Mon Mar 18 20:52:57 2019 Page 2
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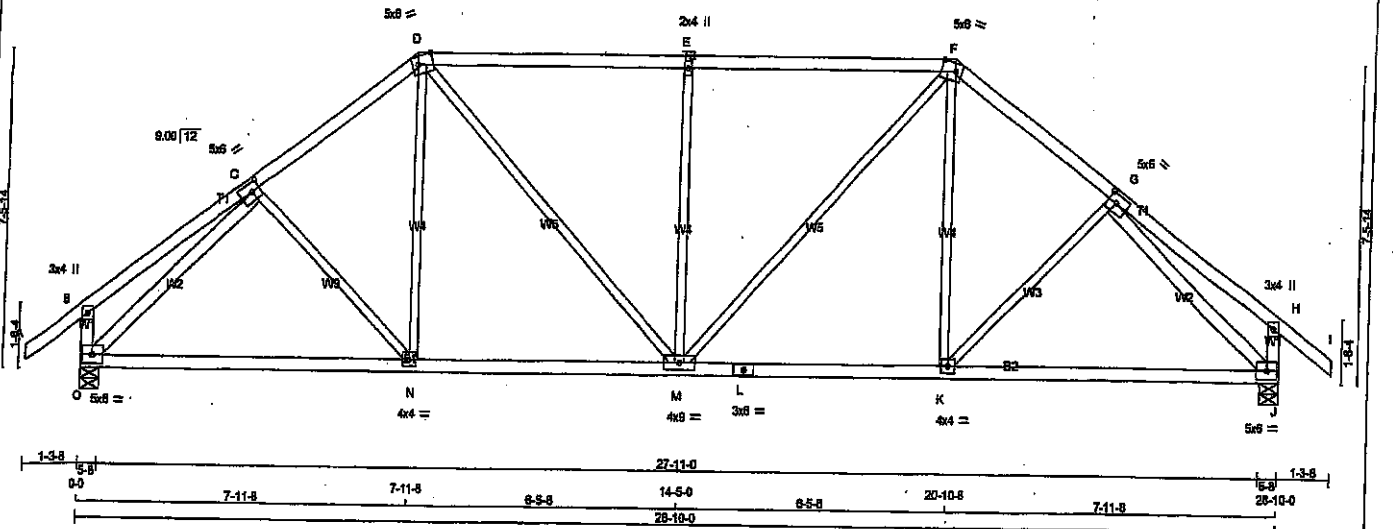
FACTORED CONCENTRATED LOADS (LBS)									
LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	
5-11-8	-483	-483	---	BACK	VERT	TOTAL	---	---	
22-10-8	-483	-483	---	BACK	VERT	TOTAL	---	---	
22-9-12	-28	-28	---	BACK	VERT	TOTAL	---	---	
8-0-4	-28	-28	---	BACK	VERT	TOTAL	---	---	
8-0-4	-128	-128	---	BACK	VERT	TOTAL	---	---	
10-0-4	-128	-128	---	BACK	VERT	TOTAL	---	---	
12-0-4	-128	-128	---	BACK	VERT	TOTAL	---	---	
14-0-4	-128	-128	---	BACK	VERT	TOTAL	---	---	
14-9-12	-128	-128	---	BACK	VERT	TOTAL	---	---	
18-9-12	-128	-128	---	BACK	VERT	TOTAL	---	---	
18-9-12	-128	-128	---	BACK	VERT	TOTAL	---	---	
20-9-12	-128	-128	---	BACK	VERT	TOTAL	---	---	
1-11-4	-78	-97	---	BACK	VERT	TOTAL	---	---	
3-11-4	-78	-97	---	BACK	VERT	TOTAL	---	---	
8-0-4	-28	-28	---	BACK	VERT	TOTAL	---	---	
10-0-4	-28	-28	---	BACK	VERT	TOTAL	---	---	
12-0-4	-28	-28	---	BACK	VERT	TOTAL	---	---	
14-0-4	-28	-28	---	BACK	VERT	TOTAL	---	---	
14-9-12	-28	-28	---	BACK	VERT	TOTAL	---	---	
18-9-12	-28	-28	---	BACK	VERT	TOTAL	---	---	
18-9-12	-28	-28	---	BACK	VERT	TOTAL	---	---	
20-9-12	-28	-28	---	BACK	VERT	TOTAL	---	---	
24-10-12	-78	-97	---	BACK	VERT	TOTAL	---	---	
28-10-12	-78	-97	---	BACK	VERT	TOTAL	---	---	



DWG NO. TAM 71905520
 STRUCTURAL
 COMPONENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T2	1	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Mon Mar 18 20:52:55 2019 Page 1
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 Scale = 1:49.7



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPFF
 A - D 2x4 DRY No.2 SPFF
 D - F 2x4 DRY No.2 SPFF
 F - I 2x4 DRY No.2 SPFF
 I - H 2x4 DRY No.2 SPFF
 H - J 2x4 DRY No.2 SPFF
 J - K 2x4 DRY No.2 SPFF
 K - L 2x4 DRY No.2 SPFF
 L - M 2x4 DRY No.2 SPFF
 M - N 2x4 DRY No.2 SPFF
 N - O 2x4 DRY No.2 SPFF
 O - P 2x4 DRY No.2 SPFF
 P - Q 2x4 DRY No.2 SPFF
 Q - R 2x4 DRY No.2 SPFF
 R - S 2x4 DRY No.2 SPFF
 S - T 2x4 DRY No.2 SPFF
 T - U 2x4 DRY No.2 SPFF
 U - V 2x4 DRY No.2 SPFF
 V - W 2x4 DRY No.2 SPFF
 W - X 2x4 DRY No.2 SPFF
 X - Y 2x4 DRY No.2 SPFF
 Y - Z 2x4 DRY No.2 SPFF
 Z - A 2x4 DRY No.2 SPFF
 ALL WEBS 2x3 DRY No.2 SPFF
 EXCEPT
 O - C 2x4 DRY No.2 SPFF
 G - J 2x4 DRY No.2 SPFF
 DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
JOINT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	FACTORED GROSS REACTION
JOINT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
JT	2168	0	2168	0	0	5-8	5-8	5-8	5-8
O	2168	0	2168	0	0	5-8	5-8	5-8	5-8
J	2168	0	2168	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS									
JOINT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW
JT	1811	917 / 0	303 / 0	0 / 0	0 / 0	391 / 0	0 / 0	1811	917 / 0
O	1811	917 / 0	303 / 0	0 / 0	0 / 0	391 / 0	0 / 0	1811	917 / 0
J	1811	917 / 0	303 / 0	0 / 0	0 / 0	391 / 0	0 / 0	1811	917 / 0

BEARING MATERIAL TO BE SPFF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

TMWV-1	MT20	5.0	8.0	2.50	2.50	CHORDS										WEBS										ROOF LIVE LOAD															
TMV-1	MT20	3.0	4.0			MAX. FACTORED					FACTORED					MAX. FACTORED					MAX. FACTORED					ALLOWABLE DEF. (LL)= L/360 (0.96")															
BMVWV-1	MT20	5.0	8.0			MEMB. FORCE (LBS)					VERT. LOAD LC1 (PLF)					MAX. CSI (LC)					UNBRACED LENGTH					MEMB. FORCE (LBS)					MAX. CSI (LC)					CALCULATED VERT. DEF. (LL) = L/999 (0.17")					
BMVWV-1	MT20	4.0	4.0																												ALLOWABLE DEF. (TL) = L/360 (0.96")										
BS-1	MT20	3.0	8.0																												CALCULATED VERT. DEF. (TL) = L/999 (0.26")										
BMVWV-1	MT20	4.0	8.0																																						
BMVWV-1	MT20	4.0	8.0																																						
BMVWV-1	MT20	4.0	8.0																																						
BMVWV-1	MT20	5.0	8.0																																						
- INDICATES REFERENCE CORNER OF PLATE																																									
- INDICATES EDGE OF CHORD.																																									
FR-TO						FROM TO						FR-TO						FR-TO						FR-TO						FR-TO											
A-B 0 / 42						-102.1 -102.1 0.14 (1)						10.00 C-N -3 / 112 0.03 (3)						C-N -3 / 112 0.03 (3)						C-N -3 / 112 0.03 (3)						C-N -3 / 112 0.03 (3)											
B-C 0 / 27						-102.1 -102.1 0.25 (1)						10.00 N-D 0 / 387 0.09 (2)						N-D 0 / 387 0.09 (2)						N-D 0 / 387 0.09 (2)						N-D 0 / 387 0.09 (2)											
C-D -2103 / 0						-102.1 -102.1 0.25 (1)						4.47 D-M 0 / 571 0.15 (1)						D-M 0 / 571 0.15 (1)						D-M 0 / 571 0.15 (1)						D-M 0 / 571 0.15 (1)											
D-E -2110 / 0						-102.1 -102.1 0.83 (1)						3.93 M-E -807 / 0 0.82 (1)						M-E -807 / 0 0.82 (1)						M-E -807 / 0 0.82 (1)						M-E -807 / 0 0.82 (1)											
E-F -2110 / 0						-102.1 -102.1 0.83 (1)						3.93 F-F 0 / 671 0.15 (1)						F-F 0 / 671 0.15 (1)						F-F 0 / 671 0.15 (1)						F-F 0 / 671 0.15 (1)											
F-G -2103 / 0						-102.1 -102.1 0.25 (1)						4.47 K-F 0 / 387 0.09 (2)						K-F 0 / 387 0.09 (2)						K-F 0 / 387 0.09 (2)						K-F 0 / 387 0.09 (2)											
G-H 0 / 27						-102.1 -102.1 0.25 (1)						10.00 K-G -3 / 112 0.03 (3)						K-G -3 / 112 0.03 (3)						K-G -3 / 112 0.03 (3)						K-G -3 / 112 0.03 (3)											
H-I 0 / 42						-102.1 -102.1 0.14 (1)						10.00 O-C -2408 / 0 0.84 (1)						O-C -2408 / 0 0.84 (1)						O-C -2408 / 0 0.84 (1)						O-C -2408 / 0 0.84 (1)											
O-B -297 / 0						0.0 0.0 0.03 (1)						7.81 G-J -2408 / 0 0.84 (1)						G-J -2408 / 0 0.84 (1)						G-J -2408 / 0 0.84 (1)						G-J -2408 / 0 0.84 (1)											
J-H -297 / 0						0.0 0.0 0.03 (1)						7.81																													
O-N 0 / 1659						-38.5 -38.5 0.84 (2)						10.00																													
N-M 0 / 1682						-38.5 -38.5 0.85 (2)						10.00																													
M-L 0 / 1662						-38.5 -38.5 0.85 (2)						10.00																													
L-K 0 / 1682						-38.5 -38.5 0.85 (2)						10.00																													
K-J 0 / 1659						-38.5 -38.5 0.84 (2)						10.00																													
AUTOSOLVE HEELS OFF																																									
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.																																									
NAIL VALUES																																									

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

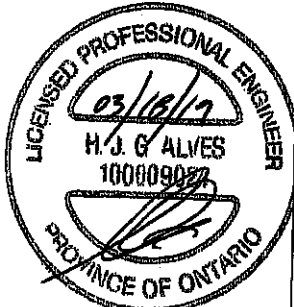
THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012
 - CSA 086-08, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.96")
 CALCULATED VERT. DEFL. (LL) = 1 / 999 (0.17")
 ALLOWABLE DEFL. (TL) = L/360 (0.96")
 CALCULATED VERT. DEFL. (TL) = 1 / 999 (0.28")

CS1: TO=0.63/1.00 (D-E-1), BC=0.65/1.00 (M-N-2),
 WB=0.94/1.00 (C-O-1), SB=0.32/1.00 (E-F-1)

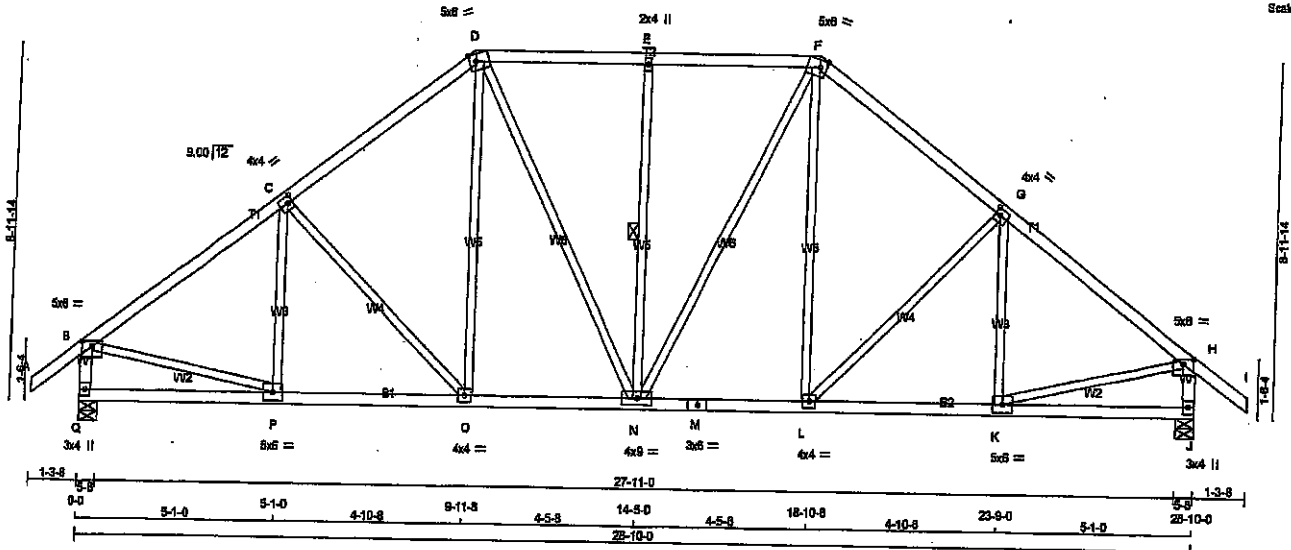
DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 1.00
 AUTOSOLVE HEELS OFF
 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
 NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656
 PLATE PLACEMENT TOL = 0.250 inches
 PLATE ROTATION TOL = 5.0 Deg.
 JSI GRIP = 0.88 (O) (INPUT = 0.90)
 JSI METAL = 0.59 (G) (INPUT = 1.00)



DWG NO. TAM 1905521
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T3	1	1	Preston 11	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Mon Mar 18 20:52:58 2019 Page 1
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 1-3-8 0-0 5-1-0 5-1-0 4-10-8 9-11-8 4-5-8 14-5-0 4-5-8 18-10-8 23-9-0 5-1-0 28-10-0 1-3-8
 Scale = 1/32"



TOTAL WEIGHT = 140 lb

LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
 EXCEPT
 DRY, SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	6.0	Edge	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	8.0	Edge	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-w	MT20	4.0	9.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX	IN-SX
Q	2168	0	2168	0	5-8
J	2168	0	2168	0	5-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1811	917 / 0	303 / 0	0 / 0	0 / 0	381 / 0	0 / 0
J	1811	917 / 0	303 / 0	0 / 0	0 / 0	381 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	P-C	-201 / 91 0.09 (1)	
B-C	-2181 / 0	-102.1 -102.1	0.41 (1)	4.24	C-D	-350 / 0 0.33 (1)	
C-D	-1948 / 0	-102.1 -102.1	0.38 (1)	4.48	D-E	0 / 438 0.10 (2)	
D-E	-1713 / 0	-102.1 -102.1	0.28 (1)	4.80	E-F	0 / 389 0.09 (1)	
E-F	-1713 / 0	-102.1 -102.1	0.28 (1)	4.80	F-G	-547 / 0 0.28 (1)	
F-G	-1948 / 0	-102.1 -102.1	0.38 (1)	4.48	G-H	0 / 389 0.09 (1)	
G-H	-2181 / 0	-102.1 -102.1	0.41 (1)	4.24	H-I	0 / 438 0.10 (2)	
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	I-J	-350 / 0 0.33 (1)	
I-J	-2087 / 0	0.0 0.0	0.22 (1)	5.86	J-K	-201 / 91 0.09 (1)	
J-K	-2087 / 0	0.0 0.0	0.22 (1)	5.86	K-L	0 / 1821 0.41 (1)	
K-L					L-M	0 / 1821 0.41 (1)	
L-M	0 / 0	-38.5 -38.5	0.18 (3)	10.00			
M-N	0 / 1773	-38.5 -38.5	0.41 (2)	10.00			
N-O	0 / 1531	-38.5 -38.5	0.33 (1)	10.00			
O-P	0 / 1531	-38.5 -38.5	0.33 (1)	10.00			
P-Q	0 / 1531	-38.5 -38.5	0.33 (1)	10.00			
Q-R	0 / 1773	-38.5 -38.5	0.41 (2)	10.00			
R-S	0 / 0	-38.5 -38.5	0.18 (3)	10.00			

DESIGN CRITERIA
 SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012
 - CSA 085-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.98")
 CALCULATED VERT. DEFL. (LL) = L/998 (0.07")
 ALLOWABLE DEFL. (TL) = L/360 (0.98")
 CALCULATED VERT. DEFL. (TL) = L/998 (0.12")

CSI: TC=0.41/1.00 (E-C:1), BC=0.41/1.00 (K-L:2), WB=0.41/1.00 (H-K:1), SEI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

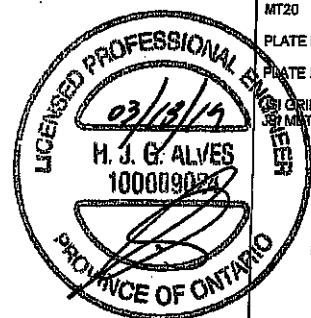
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1567 758 1997 1658

PLATE PLACEMENT TOL. = 0.250 inches

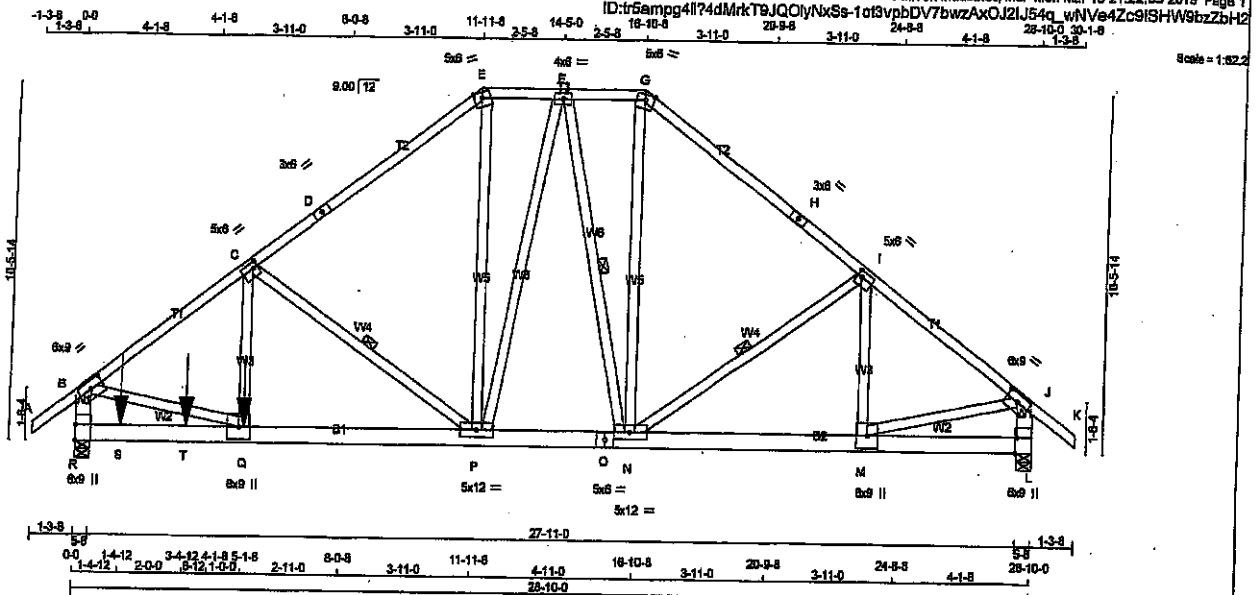
PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.90 (P) (INPUT = 0.90)
 SEI = 0.50 (N) (INPUT = 1.00)



DWG NO. TAM 71905522
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T4Z	2	2	Preston 11	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 4 X 184 = 738 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
K - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
R - O	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
D-A	12	TOP
D-E	12	TOP
E-G	12	TOP
G-H	12	TOP
H-K	12	TOP
K-B	12	TOP
L-J	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
R-O	12	SIDE(183.1)
O-L	12	TOP
WEBS : (0.122'X3') SPIRAL NAILS		
C-Q	12	SIDE(135.1)
2x4	2	
L-M	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMWW-t	MT20	6.0	9.0	2.75 4.50
C TMWW-t	MT20	5.0	6.0	2.00 1.50
D TS-t	MT20	3.0	6.0	
E TTW-m	MT20	5.0	6.0	Edge
F TMWW-t	MT20	4.0	6.0	
G TTW-m	MT20	5.0	6.0	Edge
H TS-t	MT20	3.0	6.0	
I TMWW-t	MT20	5.0	6.0	2.00 1.50
J TMWW-t	MT20	6.0	9.0	2.75 4.50
K BMV1-t	MT20	6.0	9.0	Edge 0.50
L BMWW-t	MT20	8.0	9.0	
M BMWW-t	MT20	5.0	12.0	
O BS-t	MT20	5.0	6.0	
P BMWW-t	MT20	5.0	12.0	
Q BMWW-t	MT20	8.0	9.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
R 8455	0	5-8	5-8
L 3346	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS	DEAD	SCIL
JT COMBINED	SNOW	LIVE	PERM LIVE
R 6277	3597 / 0	1162 / 0	0 / 0
L 2486	1416 / 0	465 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.55 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, H-N, F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

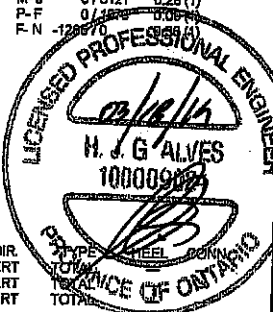
LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO			FR-TO	
A-B	0 / 42		Q-C	0 / 5359
B-C	-9361 / 0	-102.1 -102.1 0.08 (1)	C-P	-5147 / 0
C-D	-4208 / 0	-102.1 -102.1 1.00 (1)	P-E	0 / 2074
D-E	-4208 / 0	-102.1 -102.1 0.85 (1)	N-G	0 / 1881
E-F	-3382 / 0	-102.1 -102.1 0.09 (1)	N-I	-293 / 0
F-G	-2838 / 0	-102.1 -102.1 0.08 (1)	M-I	-430 / 32
G-H	-3580 / 0	-102.1 -102.1 0.80 (1)	B-Q	0 / 7704
H-I	-3580 / 0	-102.1 -102.1 0.80 (1)	M-J	0 / 3121
I-J	-3758 / 0	-102.1 -102.1 0.48 (1)	P-F	0 / 1587
J-K	0 / 42	-102.1 -102.1 0.08 (1)	F-N	-1287 / 0
R-B	-7584 / 0	0.0 0.0 0.28 (1)		
L-J	-3259 / 0	0.0 0.0 0.12 (1)		
R-S	0 / 0	-38.5 -38.5 0.43 (1)		
S-T	0 / 0	-38.5 -38.5 0.43 (1)		
T-Q	0 / 0	-38.5 -38.5 0.43 (1)		
Q-P	0 / 7530	-38.5 -38.5 0.63 (1)		
P-O	0 / 3123	-38.5 -38.5 0.28 (1)		
O-N	0 / 3123	-38.5 -38.5 0.28 (1)		
N-M	0 / 3051	-38.5 -38.5 0.26 (1)		
M-L	0 / 0	-38.5 -38.5 0.08 (3)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.
Q	5-1-8	-5898	-5898	BACK	VERT
S	1-4-12	-785	-785	BACK	VERT
T	3-4-12	-785	-785	BACK	VERT



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012

- CSA 086-09, CSA 088-14

- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.98")

CALCULATED VERT. DEFL. (LL) = L/999 (0.10")

ALLOWABLE DEFL. (TL) = L/360 (0.98")

CALCULATED VERT. DEFL. (TL) = L/999 (0.17")

CSH: TC=1.00/1.00 (B-C-1), BC=0.83/1.00 (P-Q-1),

WB=0.93/1.00 (C-P-1), SS=0.30/1.00 (Q-R-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1867 1865

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.84 (I) (INPUT = 0.90)

SI METAL= 0.85 (I) (INPUT = 1.00)

DRWG NO. TAM 71905569
STRUCTURAL
COMPONENT ONLY 1/2

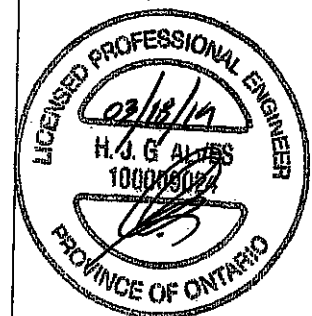
CONTINUED ON PAGE 2

JOB NAME 200172-400374	TRUSS NAME T4Z	QUANTITY 2	PLY 2	JOB DESC. Preston 11	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Mon Mar 18 21:22:03 2018 Page 2 ID:tr5ampg4j74dMrkT9JQOlvNxSs-1c13vpbDV7bwzAxOJ2N54a_wNVe4Zc9ISHW8bzZbH2	

PLATES (table is in inches)

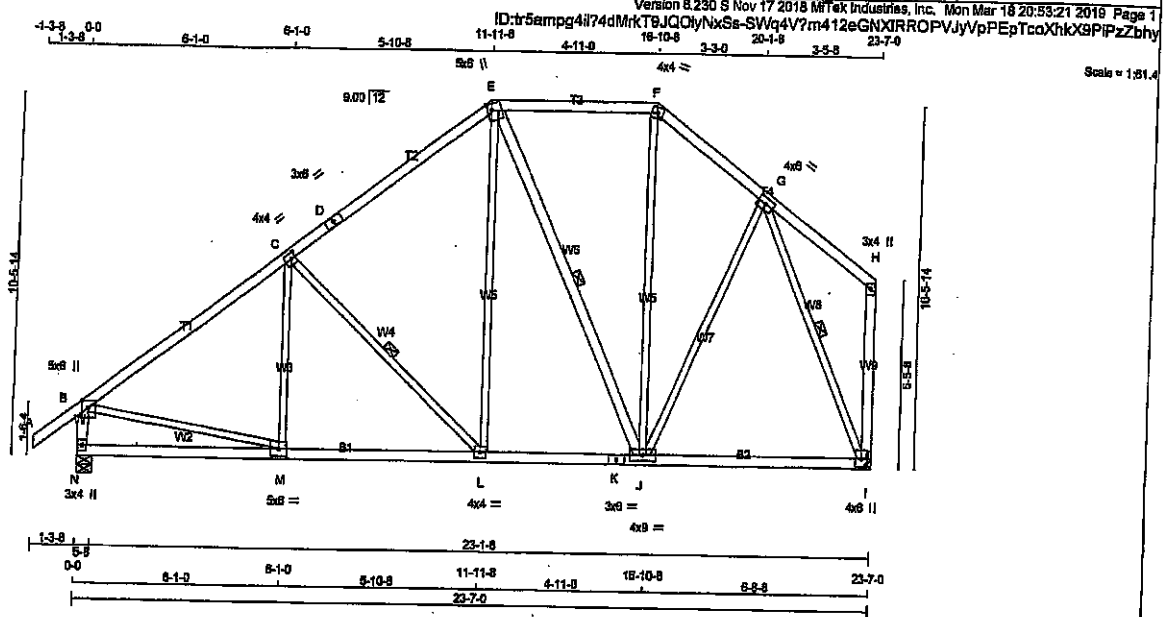
JT TYPE	PLATES	W	LEN	Y	X
R	BMV14	MT20	6.0	9.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DWG NO. TAM 11905569
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T40	6	1	Preston 11	
Tamarack Roof Truss, Burlington					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
N - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+P	MT20	5.0	6.0	2.50	2.25
C	TMVW+I	MT20	4.0	4.0	2.00	1.50
D	TS+I	MT20	3.0	8.0		
E	TTWV+m	MT20	5.0	8.0	2.25	1.50
F	TTW+m	MT20	4.0	4.0		
G	TMVW+I	MT20	4.0	4.0		
H	TMV+P	MT20	3.0	4.0		
I	BMVW+P	MT20	4.0	8.0		
J	BMVW+I	MT20	4.0	8.0		
K	BS+I	MT20	3.0	6.0		
L	BMVW+I	MT20	4.0	4.0		
M	BMVW+I	MT20	5.0	8.0		
N	BMV+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	1789	0	1789	0
I	1858	0	1858	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3'-8".

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1336	785/0	248/0	0/0	0/0	323/0	0/0
I	1239	685/0	248/0	0/0	0/0	307/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
		FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)		FORCE (LBS)
FR-TO			CS1 (LC)	UNBRAC	
A-B	0/42		FROM TO	LENGTH FR-TO	
B-C	-1710/0	-102.1	-102.1	0.14 (1)	10.00
C-D	-1248/0	-102.1	-102.1	0.56 (1)	4.45
D-E	-1248/0	-102.1	-102.1	0.51 (1)	5.10
E-F	-802/0	-102.1	-102.1	0.51 (1)	5.10
F-G	-1026/0	-102.1	-102.1	0.32 (1)	8.25
G-H	0/723	-102.1	-102.1	0.15 (1)	8.03
H-I	-1706/0	-102.1	-102.1	0.18 (1)	10.00
I-H	-136/0	0.0	0.0	0.18 (1)	8.36
		0.0	0.0	0.08 (1)	7.81
N-M	0/0	-38.5	-38.5	0.28 (3)	10.00
M-L	0/1403	-38.5	-38.5	0.45 (2)	10.00
L-K	0/967	-38.5	-38.5	0.41 (2)	10.00
K-J	0/967	-38.5	-38.5	0.41 (2)	10.00
J-I	0/626	-38.5	-38.5	0.38 (2)	10.00

TOTAL WEIGHT = 6 X 126 = 754 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CS1: TC=0.56/1.00 (B-C-1), BC=0.45/1.00 (L-M-2), WB=0.72/1.00 (G-I), SS1=0.24/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

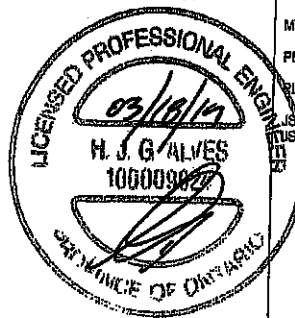
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 793 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

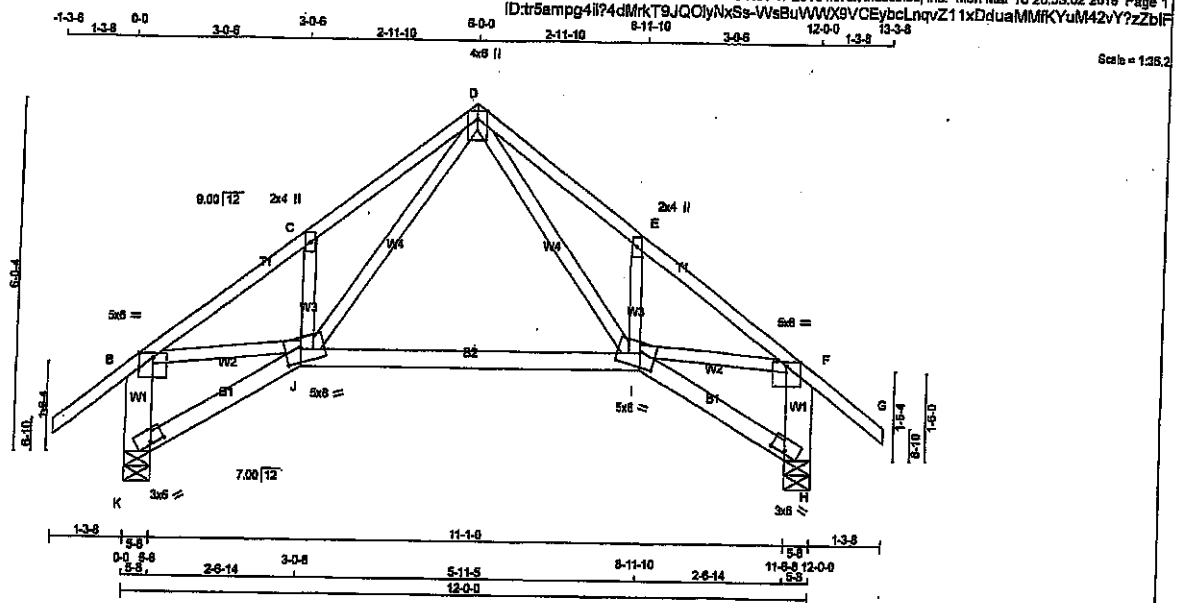
PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.89 (B) (INPUT = 0.90)
TENS METAL= 0.62 (K) (INPUT = 1.00)



DRWG NO. TAM 71905538
STRUCTURAL
CONCEPTS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T7S	1	1	Preston 11	
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 50 lb

N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER
A - D	2x3	DRY	No.2	SPF
D - G	2x3	DRY	No.2	SPF
K - B	2x3	DRY	No.2	SPF
H - F	2x3	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-w	MT20	2.0	4.0		
D	TMVW-p	MT20	4.0	6.0	Edge	
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BVM1-I	MT20	3.0	8.0	0.50	3.00
I	BBVWVW-m	MT20	5.0	8.0	2.75	3.25
J	BBVWVW-m	MT20	5.0	8.0	2.75	3.25
K	BVM1-I	MT20	3.0	8.0	0.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
K 982	0	982	0
H 982	0	982	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
K	725	427 / 0	128 / 0
H	725	427 / 0	128 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
(LBS)	(PLF)	LC1 (LC)	(LBS)	(LBS)
FR-TO	FROM TO	LENGTH	FR-TO	FROM TO
A-B	0 / 41	-102.1 -102.1	D-I	0 / 780
B-C	-1228 / 0	-102.1 -102.1	E-J	-378 / 0
C-D	-1277 / 0	-102.1 -102.1	F-K	0 / 780
D-E	-1277 / 0	-102.1 -102.1	G-L	-378 / 0
E-F	-1228 / 0	-102.1 -102.1	H-I	0 / 1002
F-G	0 / 41	-102.1 -102.1	J-K	0 / 1002
G-H	-924 / 0	0.0 0.0	L-M	0 / 1002
H-I	-924 / 0	0.0 0.0		
K-J	0 / 0	-38.5 -38.5		
J-I	0 / 558	-38.5 -38.5		
I-H	0 / 0	-38.5 -38.5		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
BOT CH. LL = 10.5 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/988 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/827 (0.17")

CSI: TC=0.28/1.00 (A-B-1), BC=0.37/1.00 (I-J-2), WB=0.23/1.00 (B-J-1), SSI=0.17/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HELLS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

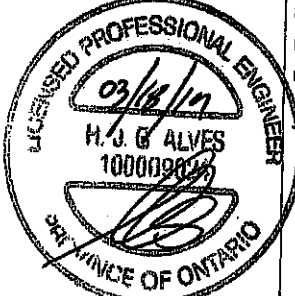
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

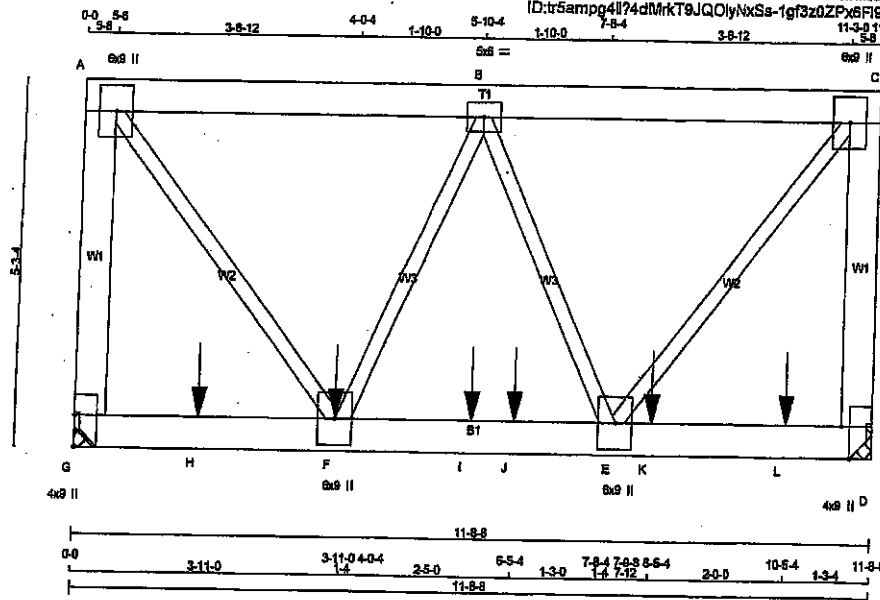
SI GRIP= 0.74 (B) (INPUT = 0.80)

SI METAL= 0.31 (B) (INPUT = 1.00)



DWG NO. TAM 914055 24
STRUCTURAL
COMPONENT ONLY

JOB NAME 200172-400374	TRUSS NAME T20	QUANTITY 1	PLY 2	JOB DESC. Preston 11	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 74 = 148 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
G - A	2x6 DRY	No.2	SPF		
A - C	2x6 DRY	No.2	SPF		
D - C	2x6 DRY	No.2	SPF		
G - D	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
G-A 2	12	TOP
A-C 2	12	TOP
C-D 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G-D 2	12	SIDE (0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	6.0	8.0			
B	TMVW+L	MT20	5.0	8.0			
C	TMVW+p	MT20	6.0	8.0			
D	BMV1+t	MT20	4.0	9.0	Edge	1.50	
E	BMVW+H	MT20	6.0	8.0			
F	BMVW+H	MT20	6.0	8.0			
G	BMV1+t	MT20	4.0	9.0	5.50		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
G 5458	0	0	MECHANICAL
D 5906	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G, D. MINIMUM BEARING LENGTH AT JOINT G = 4-0, JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
G	4060	2289 / 0	771 / 0	0 / 0	989 / 0	0 / 0
D	4394	2469 / 0	834 / 0	0 / 0	1071 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

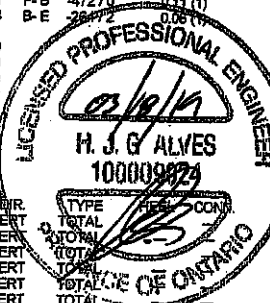
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO	G-A	-4832 / 0	0.0	0.0	0.84 (1)	6.59	E-C	0 / 5867	0.74 (1)
	A-B	-3625 / 0	-102.1	-102.1	0.14 (1)	8.78	A-F	0 / 5836	0.72 (1)
	B-C	-3707 / 0	-102.1	-102.1	0.15 (1)	5.71	F-B	-472 / 0	0.31 (1)
	D-C	-4935 / 0	0.0	0.0	0.85 (1)	6.54	B-E	-2847 / 0	0.88 (1)
	G-H	0 / 0	-38.5	-38.5	0.42 (1)	10.00			
	H-F	0 / 0	-38.5	-38.5	0.42 (1)	10.00			
	F-I	0 / 3809	-38.5	-38.5	0.83 (1)	10.00			
	I-J	0 / 3809	-38.5	-38.5	0.83 (1)	10.00			
	J-E	0 / 3809	-38.5	-38.5	0.83 (1)	10.00			
	E-K	0 / 0	-38.5	-38.5	0.58 (1)	10.00			
	K-L	0 / 0	-38.5	-38.5	0.58 (1)	10.00			
	L-D	0 / 0	-38.5	-38.5	0.58 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	W/R	TYPE	RES. CONC.
F	3-8-12	-1819	-1819	---	BACK	VERT	TOTAL	---
H	1-9-12	-1819	-1819	---	BACK	VERT	TOTAL	---
I	5-9-12	-1819	-1819	---	BACK	VERT	TOTAL	---
J	6-5-4	-1819	-1819	---	BACK	VERT	TOTAL	---
K	8-5-4	-1819	-1819	---	BACK	VERT	TOTAL	---
L	10-5-4	-1819	-1819	---	BACK	VERT	TOTAL	---



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBCC 2015
- CSA C88-09, CSA C88-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.39")
CALCULATED VERT. DEFL. (LL) = L/999 (0.06")
ALLOWABLE DEFL. (TL) = L/360 (0.39")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TO=0.65/1.00 (C-D:1), BO=0.83/1.00 (E-F:1), WB=0.74/1.00 (C-E:1), SS=0.80/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

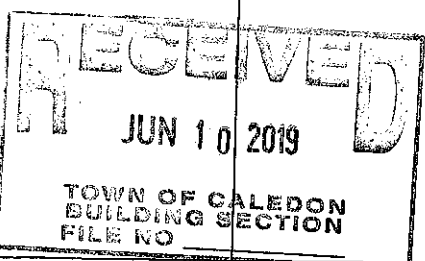
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1667 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.99)
JSI METAL= 0.57 (F) (INPUT = 1.00)

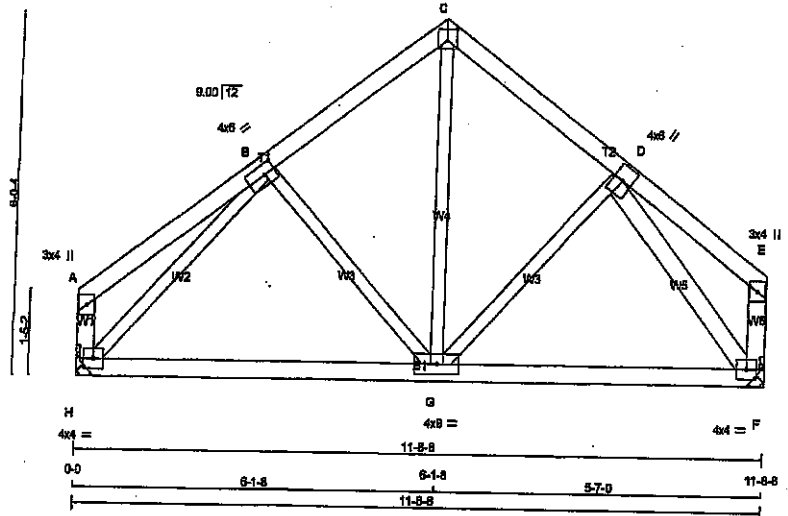


DWG NO. TAM 71905570
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T21	1	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:tr5ampg41174dMrkT9JQOlyNxSs-RNZZYre6n2zVqdfz_AsojSIWbbfH4bbOQWAmwzZbH7

Scale = 1/32



TOTAL WEIGHT = 52 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	823	0	0	0
F	823	0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
H	815 340/0 123/0 0/0 0/0 152/0 0/0
F	815 340/0 123/0 0/0 0/0 152/0 0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO							FR-TO			
A-B	0/21						B-G	-148/19	0.05 (1)	
B-C	-583/0	-102.1	-102.1	0.15 (1)	10.00		G-C	0/425	0.10 (1)	
C-D	-580/0	-102.1	-102.1	0.11 (1)	8.25		G-D	-48/54	0.02 (1)	
D-E	0/22	-102.1	-102.1	0.13 (1)	10.00		H-B	-820/0	0.27 (1)	
H-A	-122/0	0.0	0.0	0.01 (1)	7.81		D-F	-812/0	0.17 (1)	
F-E	-81/0	0.0	0.0	0.01 (1)	7.81					
H-G	0/549	-38.5	-38.5	0.35 (2)	10.00					
G-F	0/482	-38.5	-38.5	0.34 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012
 - CSA 088-08, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.39")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.15/1.00 (A-B:1), BC=0.35/1.00 (G-H:2),
 WB=0.27/1.00 (B-H:1), 6SI=0.15/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

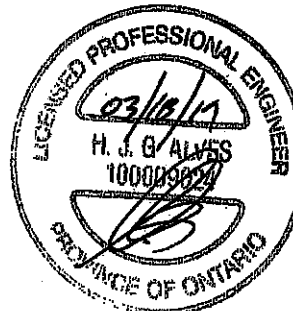
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 788 1887 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
 JSI METAL= 0.34 (D) (INPUT = 1.00)

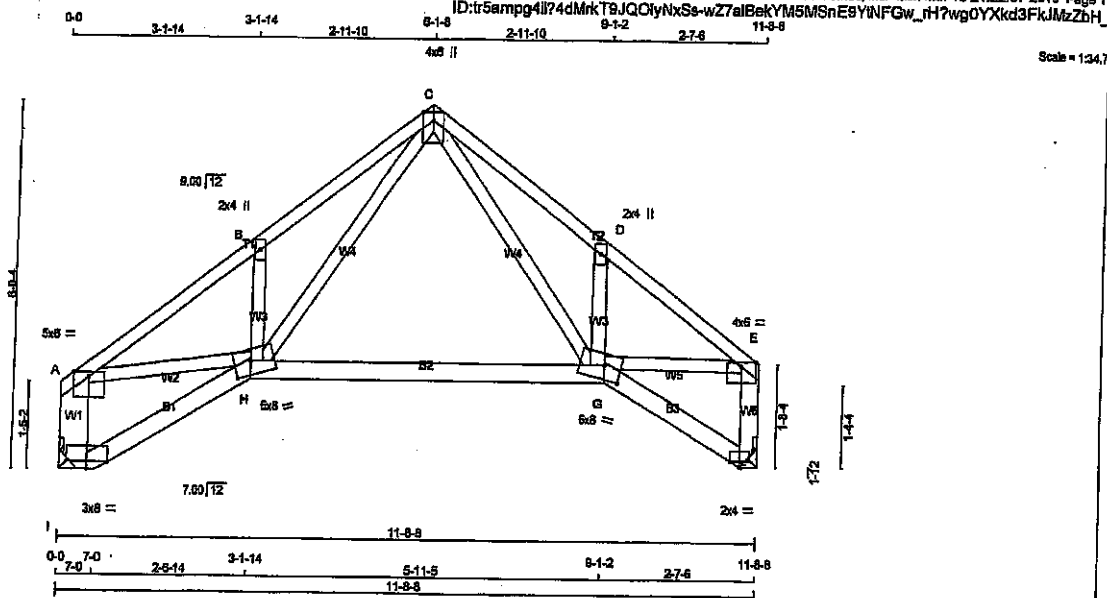


DRWG NO. TAM 1705571
 STRUCTURAL
 COMPONENT - W. V.

JOB NAME 200172-400374	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. Preston 11	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:tr5ampg4ll74dMkT9JQOlyNxSe-wZ7alBekYMSMSneSYNFGw_rH7wgOYXkd3FkJMzZbH



Scale = 1/4" = 1'-0"

LUMBER	N L G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x3	DRY	No.2	SPF		
C - E	2x3	DRY	No.2	SPF		
F - H	2x4	DRY	No.2	SPF		
I - J	2x4	DRY	No.2	SPF		
K - L	2x4	DRY	No.2	SPF		
M - N	2x4	DRY	No.2	SPF		
O - P	2x4	DRY	No.2	SPF		
Q - R	2x4	DRY	No.2	SPF		
S - T	2x4	DRY	No.2	SPF		
U - V	2x4	DRY	No.2	SPF		
W - X	2x4	DRY	No.2	SPF		
Y - Z	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMVW-w	MT20	2.0	4.0		
C	TTWW-p	MT20	4.0	8.0	Edge	
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-p	MT20	4.0	8.0	Edge	
F	BVM1-p	MT20	2.0	4.0	Edge	2.00
G	BBWWW-m	MT20	5.0	8.0	2.75	3.25
H	BBWWW-m	MT20	5.0	8.0	2.75	3.25
I	BVM1-p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED			MAXIMUM FACTORED			INPUT	REQD
GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	823	0	823	0	0	MECHANICAL	
F	823	0	823	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, F. MINIMUM BEARING LENGTH AT JOINT I = 3'-8", JOINT F = 3'-8".

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	815	340 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0
F	815	340 / 0	123 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	UNBRAC LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
A-B	-1223 / 0	-102.1	-102.1	0.27 (1)	4.83	H-B	-388 / 0	0.08 (1)		
B-C	-1273 / 0	-102.1	-102.1	0.28 (1)	4.77	H-C	0 / 825	0.19 (1)		
C-D	-1177 / 0	-102.1	-102.1	0.22 (1)	5.08	C-G	0 / 813	0.14 (1)		
D-E	-1071 / 0	-102.1	-102.1	0.21 (1)	5.18	G-D	-352 / 0	0.08 (1)		
E-F	-762 / 0	0.0	0.0	0.05 (1)	7.81	A-H	0 / 1000	0.23 (1)		
F-G	-773 / 0	0.0	0.0	0.08 (1)	7.81	G-E	0 / 873	0.20 (1)		
I-H	0 / 0	-38.5	-38.5	0.09 (3)	10.00					
H-G	0 / 528	-38.5	-38.5	0.37 (2)	10.00					
G-F	0 / 0	-38.5	-38.5	0.06 (3)	10.00					

TOTAL WEIGHT = 48 lb

(MUF)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 82.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/831 (0.17")

CSI: TC=0.27/1.00 (A-B-1), BC=0.37/1.00 (G-H-2),
WB=0.23/1.00 (A-H-1), SS=0.17/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

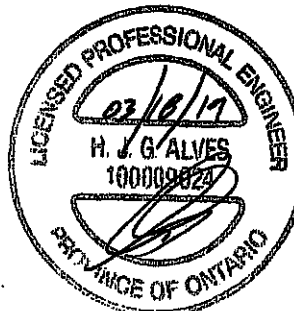
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.280 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (A) (INPUT = 0.80)
JSI METAL= 0.28 (A) (INPUT = 1.00)

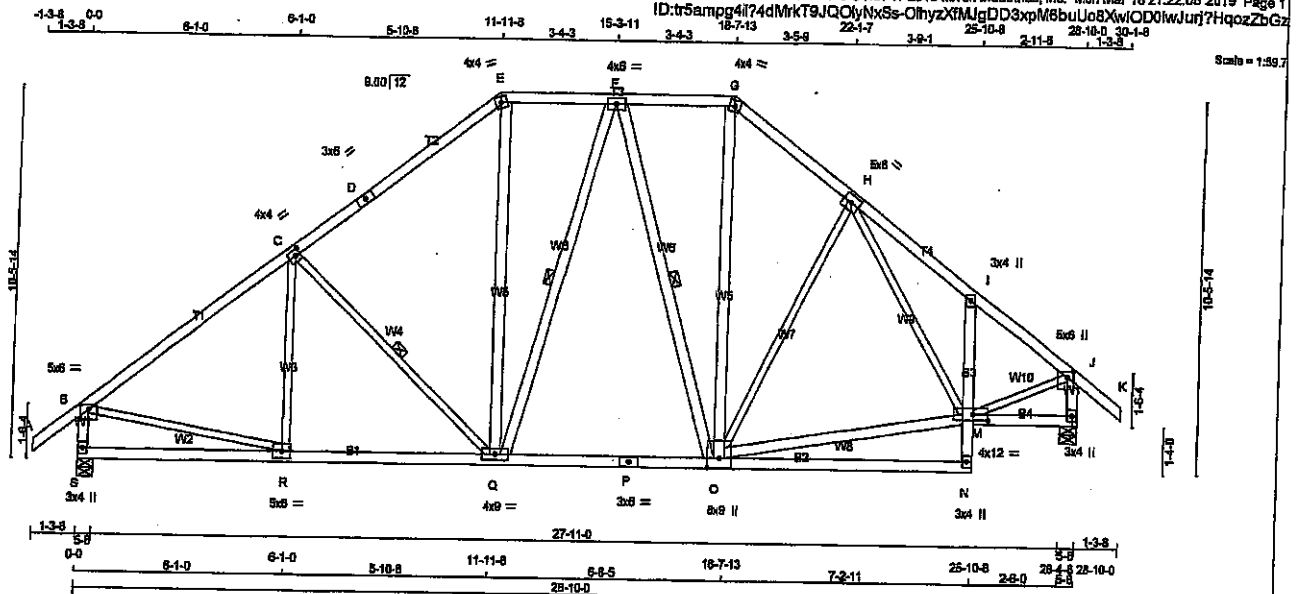


DRWG NO. TAM 11405572
STRUCTURAL
COMPONENT CIVIL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400874	T23	2	1	Preston 11	
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 168 = 336 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
S - J	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
R - C	2x3	DRY	No.2	SPF
C - Q	2x3	DRY	No.2	SPF
B - R	2x3	DRY	No.2	SPF
O - H	2x3	DRY	No.2	SPF
H - M	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	TMVW+p	MT20	5.0	6.0	2.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMVWVW-t	MT20	4.0	12.0	2.00	5.25
N	BMV+p	MT20	3.0	4.0		
O	BMVWVW-t	MT20	6.0	9.0	3.25	4.00
P	BS-t	MT20	3.0	6.0		
Q	BMVWVW-t	MT20	4.0	9.0		
R	BMVW-t	MT20	5.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
S	2168	0	0	2168	0	0	5-8	5-8	5-8
L	2168	0	0	2168	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1811	917 / 0	303 / 0	0 / 0	0 / 0	381 / 0	0 / 0
L	1811	917 / 0	303 / 0	0 / 0	0 / 0	381 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-Q, F-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. CS1 (LC)	UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	R-C	-89 / 178
B-C	-2195 / 0	-102.1	-102.1	0.61 (1)	3.97	C-Q	-547 / 0
C-D	-1793 / 0	-102.1	-102.1	0.55 (1)	4.38	Q-E	0 / 685
D-E	-1793 / 0	-102.1	-102.1	0.55 (1)	4.38	C-G	0 / 720
E-F	-1408 / 0	-102.1	-102.1	0.18 (1)	5.35	B-R	0 / 1825
F-G	-1341 / 0	-102.1	-102.1	0.15 (1)	5.45	Q-M	0 / 1485
G-H	-1683 / 0	-102.1	-102.1	0.17 (1)	4.97	O-H	-416 / 0
H-I	-1989 / 0	-102.1	-102.1	0.19 (1)	4.66	H-M	0 / 179
I-J	-2013 / 0	-102.1	-102.1	0.14 (1)	4.88	M-J	0 / 1740
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	Q-F	-98 / 8
S-B	-2073 / 0	0.0	0.0	0.22 (1)	5.87	F-O	-320 / 0
L-J	-2108 / 0	0.0	0.0	0.22 (1)	5.84		
S-R	0 / 0	-38.5	-38.5	0.27 (3)	10.00		
R-Q	0 / 1791	-38.5	-38.5	0.49 (2)	10.00		
Q-P	0 / 1435	-38.5	-38.5	0.55 (2)	10.00		
P-O	0 / 1435	-38.5	-38.5	0.55 (2)	10.00		
O-N	0 / 47	-38.5	-38.5	0.38 (3)	10.00		
N-M	0 / 144	0.0	0.0	0.10 (1)	7.00		
M-I	-340 / 0	0.0	0.0	0.08 (1)	10.00		
M-L	0 / 0	-38.5	-38.5	0.09 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA C89-08, CSA C89-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.22")

CS1: TO=0.61/1.00 (B-C-1), BO=0.55/1.00 (O-Q-2), WB=0.58/1.00 (H-O-1), SB=0.24/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

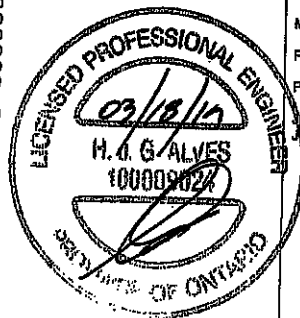
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (L) (INPUT = 0.80)
SI METAL= 0.46 (P) (INPUT = 1.00)



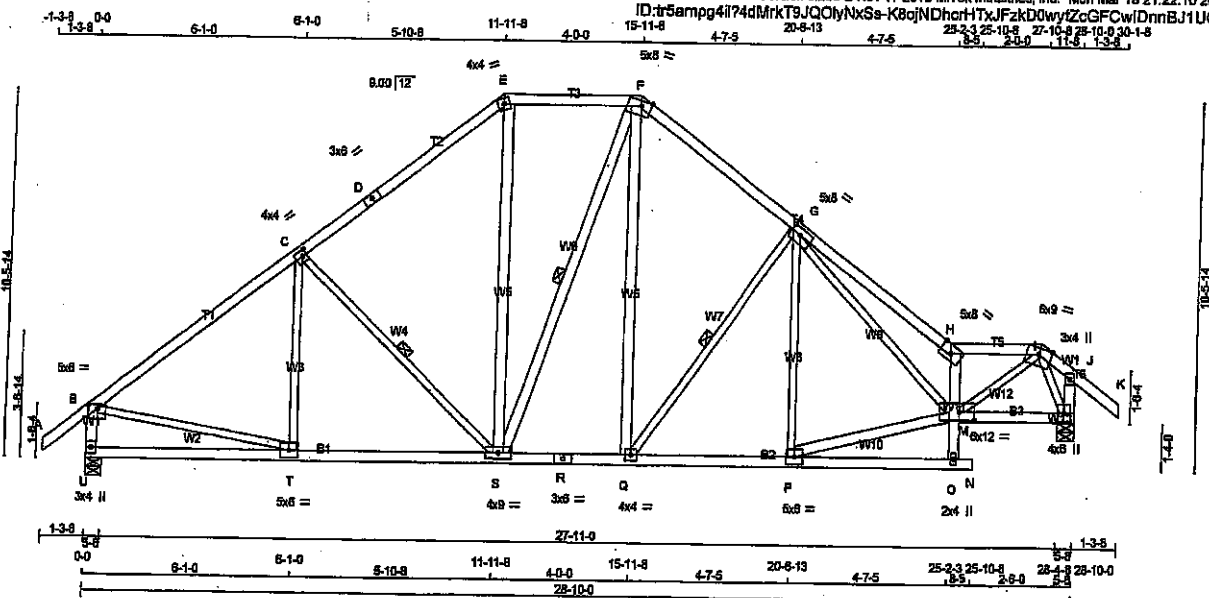
DRWG NO. TAM 17405373
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T24	5	1	Preston 11	

Tamarack Roof Truss, Burlington

TRUSS DESC.

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ID:tr5ampg4i74dMrkT9JQOlyNxSs-K8ojNDhorHTXJFzkDwyZzGFCwIDnnBJ1UOuHzZbGx
25-2-3 25-10-8 27-10-8 28-10-8 30-1-8
8-5 2-0-0 11-5 1-3-8



Scale = 1/80.3

TOTAL WEIGHT = 5 X 158 = 795 lb

LUMBER	SIZE	DRY	NO.2	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - E	2x4	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TTWV-m	MT20	5.0	6.0	1.75	3.25
G	TMVWV-t	MT20	5.0	6.0	2.25	3.50
H	TTW-h	MT20	5.0	6.0	2.75	4.00
I	TTWV-m	MT20	6.0	9.0	Edge	
J	TMV+p	MT20	3.0	4.0		
L	BMVWV+p	MT20	4.0	6.0		
M	BMVWVWV-t	MT20	6.0	12.0	3.00	5.25
O	BMVW-w	MT20	2.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVWV-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
S	BMVWVW-t	MT20	4.0	6.0		
T	BMVWV-t	MT20	5.0	6.0		
U	BMVW+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
U	2187	0	2187	0	5-8	5-8
L	2189	0	2189	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1811	915 / 0	304 / 0	0 / 0	0 / 0	392 / 0	0 / 0
L	1830	919 / 0	312 / 0	0 / 0	0 / 0	398 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 4.75 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-S, F-S, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	CH1 (PLF)	CH2 (CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	CH1 (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	T-C	-87 / 182	0.05 (1)	
B-C	-2195 / 0	-102.1	-102.1	0.61 (1)	3.97	C-S	-549 / 0	0.28 (1)	
C-D	-1790 / 0	-102.1	-102.1	0.55 (1)	4.38	S-E	0 / 1821	0.10 (1)	
D-E	-1780 / 0	-102.1	-102.1	0.55 (1)	4.38	S-F	-53 / 0	0.03 (1)	
E-F	-1403 / 0	-102.1	-102.1	0.23 (1)	5.28	Q-F	0 / 1731	0.12 (1)	
F-G	-1803 / 0	-102.1	-102.1	0.34 (1)	4.64	Q-G	-735 / 0	0.33 (1)	
G-H	-4074 / 0	-102.1	-102.1	0.58 (1)	3.08	P-G	-235 / 66	0.20 (1)	
H-I	-3185 / 0	-102.1	-102.1	0.20 (1)	3.78	P-M	0 / 1882	0.27 (1)	
I-J	-58 / 0	-102.1	-102.1	0.10 (1)	6.25	M-I	0 / 2850	0.64 (1)	
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	I-L	-2084 / 0	0.32 (1)	
U-B	-2072 / 0	0.0	0.0	0.22 (1)	5.87	B-T	0 / 1825	0.41 (1)	
L-J	-289 / 0	0.0	0.0	0.03 (1)	7.81	O-M	0 / 121	0.40 (1)	
U-T	0 / 0	-38.5	-38.5	0.28 (3)	10.00	M-H	-2786 / 0	0.38 (1)	
T-S	0 / 1791	-38.5	-38.5	0.50 (2)	10.00	G-M	0 / 1825	0.35 (1)	
S-R	0 / 1422	-38.5	-38.5	0.34 (1)	10.00				
R-Q	0 / 1422	-38.5	-38.5	0.34 (1)	10.00				
Q-P	0 / 1841	-38.5	-38.5	0.44 (1)	10.00				
P-O	0 / 212	-38.5	-38.5	0.20 (2)	10.00				
O-N	0 / 0	-38.5	-38.5	0.01 (3)	10.00				
M-L	0 / 948	-38.5	-38.5	0.23 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
CL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/G

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA C88-08, CSA C86-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.08")
CALCULATED VERT. DEFL. (LL) = L/999 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (0.08")
CALCULATED VERT. DEFL. (TL) = L/999 (0.23")

CSI: TO=0.61/1.00 (B-C:1), BO=0.50/1.00 (S-T:2), WE=0.84/1.00 (I-M:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

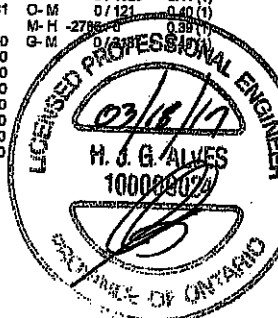
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 785 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

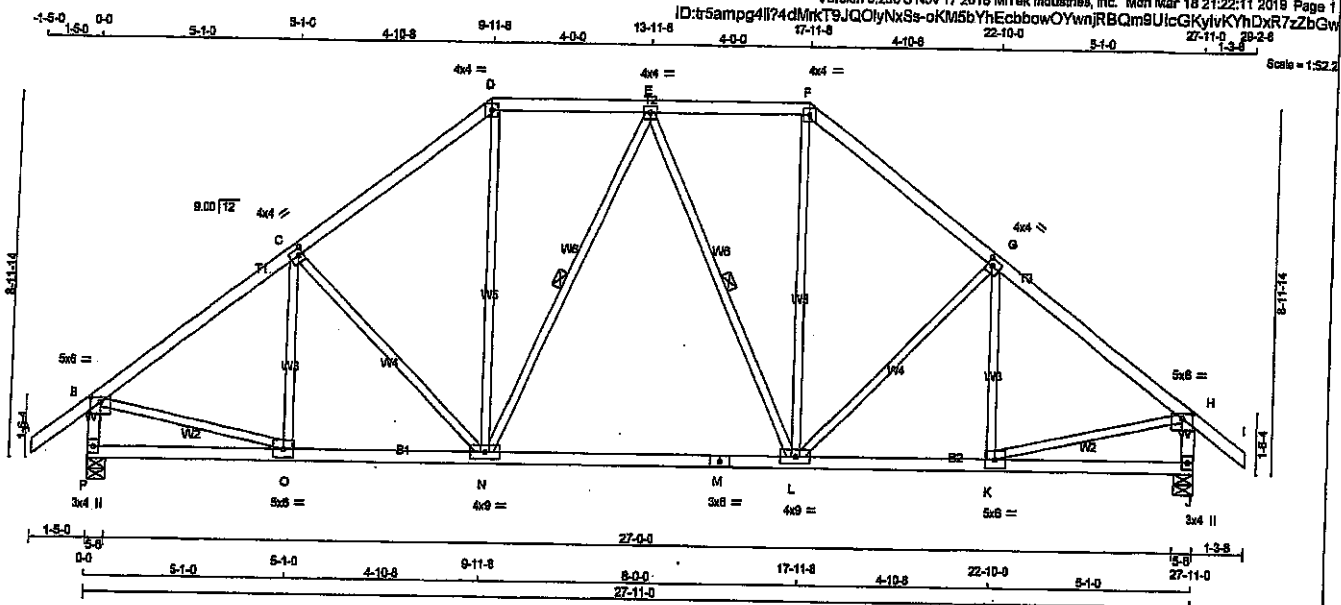
PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.89 (M) (INPUT = 0.90)
SI METAL= 0.65 (F) (INPUT = 1.00)

DRWG NO. TAM 1905574
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T25	1	1	Preston 11	
Tamarack Roof Truss, Burlington					
TRUSS DESC.					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
P - B	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
P - M	2x4 DRY	No.2	SPF		
M - J	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVVW-p	MT20	5.0	8.0	1.50	3.00
C	TMWWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-t	MT20	4.0	4.0		
E	TMWWW-t	MT20	4.0	4.0		
F	TTW-t	MT20	4.0	4.0		
G	TMWWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVVW-p	MT20	5.0	8.0	1.50	3.00
J	BMV-t+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	8.0		
L	BMWWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	8.0		
P	BMV-t+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
P	2116	0	2116	0
J	2103	0	2103	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1572	888 / 0	293 / 0	0 / 0	0 / 0	381 / 0	0 / 0
J	1563	891 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
-------	---------------------------	---------------------------	---------------------------	---------------------------	---------------------------	---------------------------

FR-TO	FROM	TO	CS1 (LC)	MAX. UNBRACED LENGTH (FT)	FR-TO	FROM	TO	CS1 (LC)	MAX. UNBRACED LENGTH (FT)
-------	------	----	----------	---------------------------	-------	------	----	----------	---------------------------

A-B	0 / 46	-102.1	-102.1	0.18 (1)	10.00	C-C	-212 / 64	0.09 (1)	10.00
B-C	-2093 / 0	-102.1	-102.1	0.40 (1)	4.32	C-N	-341 / 0	0.32 (1)	10.00
C-D	-1868 / 0	-102.1	-102.1	0.38 (1)	4.54	N-D	0 / 750	0.17 (1)	10.00
D-E	-1471 / 0	-102.1	-102.1	0.22 (1)	5.18	L-F	0 / 750	0.17 (1)	10.00
E-F	-1471 / 0	-102.1	-102.1	0.22 (1)	5.18	L-G	-341 / 0	0.32 (1)	10.00
F-G	-1868 / 0	-102.1	-102.1	0.38 (1)	4.54	K-G	-212 / 64	0.09 (1)	10.00
G-H	-2093 / 0	-102.1	-102.1	0.40 (1)	4.32	B-O	0 / 1749	0.39 (1)	10.00
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	K-H	0 / 1749	0.39 (1)	10.00
P-B	-2031 / 0	0.0	0.0	0.21 (1)	5.93	N-E	-267 / 0	0.17 (1)	10.00
J-H	-2018 / 0	0.0	0.0	0.21 (1)	5.94	E-L	-267 / 0	0.17 (1)	10.00

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
-------	---------------------------	---------------------------	---------------------------	---------------------------	---------------------------	---------------------------

P-O	0 / 0	-38.5	-38.5	0.16 (3)	10.00				
O-N	0 / 1703	-38.5	-38.5	0.51 (2)	10.00				
N-M	0 / 1580	-38.5	-38.5	0.49 (2)	10.00				
M-L	0 / 1580	-38.5	-38.5	0.49 (2)	10.00				
L-K	0 / 1703	-38.5	-38.5	0.51 (2)	10.00				
K-J	0 / 0	-38.5	-38.5	0.16 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CS1: TC=0.40/1.00 (B-C:1), BC=0.51/1.00 (N-O:2), WB=0.39/1.00 (H-K:1), SS1=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

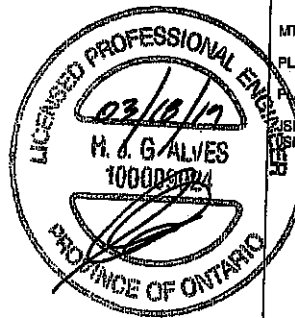
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (FL) (PL) (FL)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1867 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

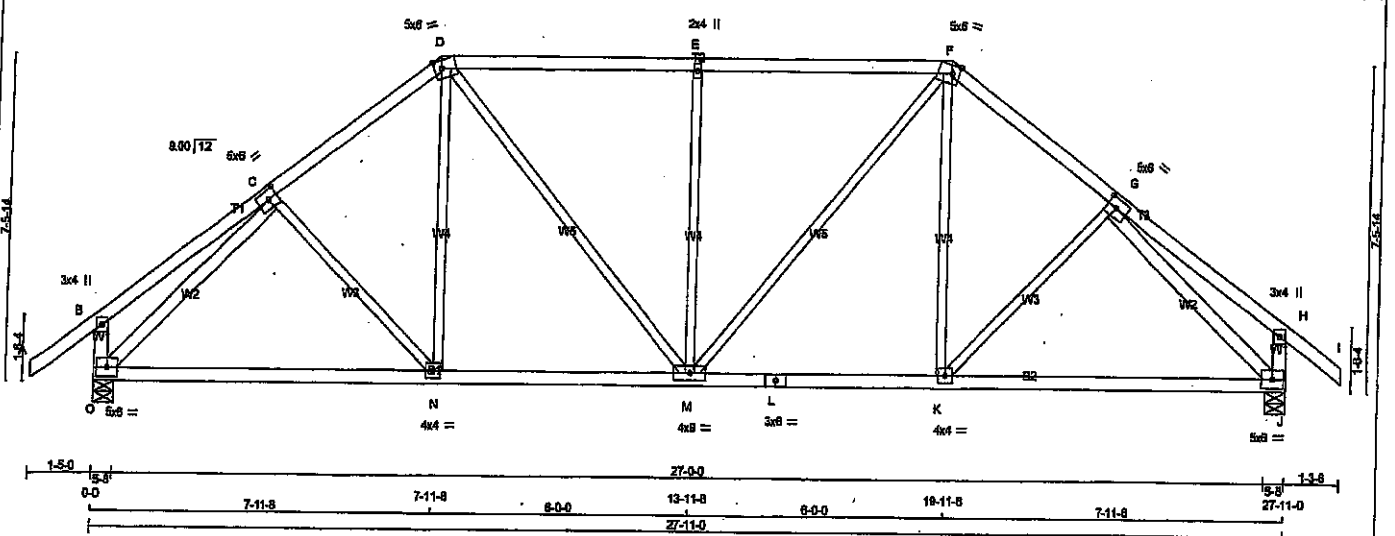
GRIP= 0.90 (D) (INPUT = 0.90)
METAL= 0.59 (M) (INPUT = 1.00)



DRWG NO. TAM 71905575
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T26	1	1	Preston 11	
Tamarack Roof Truss, Burlington					

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ID: tr5ampg4il74dMkt8JQOlyNxSs-kjUroEjV8CvAlu8UvBEokPwvQ4Qd??72V0zZbGu
18-11-8 23-10-9 27-11-0 28-2-8
Scale: 1/4"=1'



TOTAL WEIGHT = 126 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.75
D	TTVW-m	MT20	5.0	6.0	Edge	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	Edge	2.00
G	TMVW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	9.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2116	0	2116	0	0	5-8	5-8
J	2103	0	2103	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS		PERM LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM LIVE
O	1572	888 / 0	283 / 0	0 / 0	0 / 0	361 / 0	0 / 0	1572	888 / 0	283 / 0	0 / 0
J	1593	891 / 0	283 / 0	0 / 0	0 / 0	379 / 0	0 / 0	1593	891 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX. (LC)	UNBRAC
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 46	-102.1	-102.1	0.16 (1)	10.00	C-N	-18 / 105	0.03 (3)	
B-C	0 / 27	-102.1	-102.1	0.25 (1)	10.00	N-D	0 / 392	0.09 (2)	
C-D	-2013 / 0	-102.1	-102.1	0.25 (1)	4.55	D-M	0 / 593	0.13 (1)	
D-E	-1970 / 0	-102.1	-102.1	0.53 (1)	4.20	M-E	-748 / 0	0.76 (1)	
E-F	-1970 / 0	-102.1	-102.1	0.53 (1)	4.20	M-F	0 / 593	0.13 (1)	
F-G	-2013 / 0	-102.1	-102.1	0.25 (1)	4.55	K-F	0 / 392	0.09 (2)	
G-H	0 / 27	-102.1	-102.1	0.25 (1)	10.00	K-G	-18 / 105	0.03 (3)	
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	O-C	-2318 / 0	0.80 (1)	
O-B	-310 / 0	0.0	0.0	0.03 (1)	7.61	G-J	-2318 / 0	0.80 (1)	
J-H	-287 / 0	0.0	0.0	0.03 (1)	7.61				
O-N	0 / 1598	-38.5	-38.5	0.63 (2)	10.00				
N-M	0 / 1590	-38.5	-38.5	0.63 (2)	10.00				
M-L	0 / 1590	-38.5	-38.5	0.63 (2)	10.00				
L-K	0 / 1590	-38.5	-38.5	0.63 (2)	10.00				
K-J	0 / 1598	-38.5	-38.5	0.63 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.53/1.00 (D-E-1), BC=0.63/1.00 (N-N-2),
WB=0.90/1.00 (C-C-1), SS=0.30/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

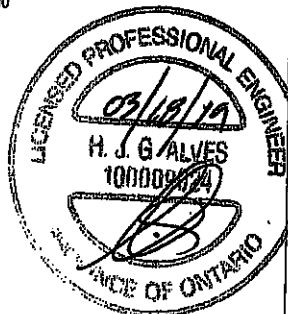
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

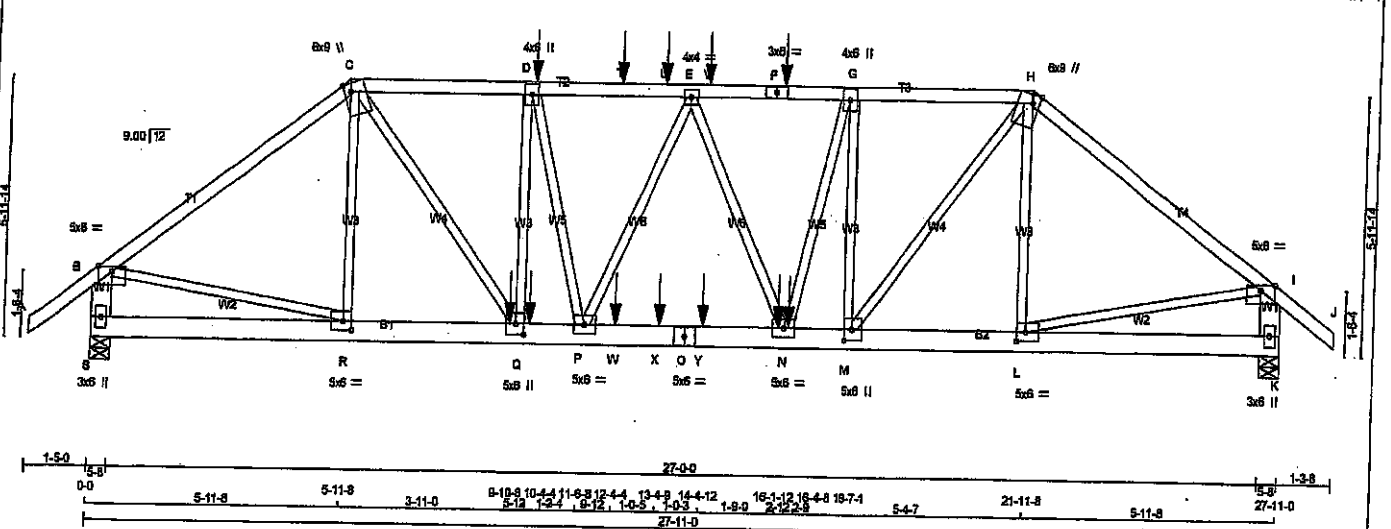
SI GRIP= 0.88 (C) (INPUT = 0.80)
SI METAL= 0.57 (G) (INPUT = 1.00)



DRWG NO. TAM 71905576
STRUCTURAL
CONSEQUENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T27	1	2	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID: t5ampg4f74dMkT9JQCvNxs-JQC81XPJwHEFaVxGISAMVWuIM0qO5FW8yShqzZbC7
 1-5-0 6-0 3-1-0 3-1-0 2-10-8 5-11-8 4-4-12 10-4-4 11-8-8 12-4-4 13-4-8 13-11-8 14-0-2 14-4-12 15-1-12 16-4-8 16-7-1 21-11-8 24-10-0 27-5-8 27-11-0 29-2-8
 1-5-0 6-0 3-1-0 3-1-0 2-10-8 5-11-8 4-4-12 10-4-4 11-8-8 12-4-4 13-4-8 13-11-8 14-0-2 14-4-12 15-1-12 16-4-8 16-7-1 21-11-8 24-10-0 27-5-8 27-11-0 29-2-8
 Scale: 1/4"=1'



TOTAL WEIGHT = 2 X 145 = 297 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
O - K	2x6	DRY	No.2
O - L	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A-C 1	12	TOP
C-F 1	12	SIDE(61.0)
F-H 1	12	SIDE(0.0)
H-J 1	12	TOP
S-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
S-O 2	12	SIDE(163.1)
O-K 2	12	SIDE(0.0)
WEBS : (0.122'X3') SPIRAL NAILS		
D-Q 1	6	SIDE(233.7)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge
C	TTWW-m	MT20	6.0	9.0	Edge 2.00
D	TMWV-t	MT20	4.0	6.0	
E	TMWV-t	MT20	4.0	4.0	
F	TS-1	MT20	3.0	6.0	
G	TMWV-t	MT20	4.0	6.0	
H	TTWW-m	MT20	6.0	9.0	Edge 2.00
I	TMWV-p	MT20	5.0	8.0	Edge
K	BMV1-p	MT20	3.0	8.0	
L	BMWV-t	MT20	5.0	6.0	2.50 2.50
M	BMWV-t	MT20	5.0	6.0	3.00 2.25
N	BMWV-t	MT20	5.0	6.0	
O	BS-1	MT20	5.0	6.0	
P	BMWV-t	MT20	5.0	6.0	
Q	BMWV-t	MT20	5.0	6.0	3.00 2.25
R	BMWV-t	MT20	5.0	6.0	2.50 2.50
S	BMV1-p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
JT					
S	4438 0	4438 0	0 0	5-8	5-8
K	4305 0	4305 0	0 0	5-8	5-8

UNFACTORED REACTIONS					
	1ST LCASE	MAX/MIN	COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM/LIVE	WIND
S	3291	1882 / 0	589 / 0	0 / 0	821 / 0
K	3190	1839 / 0	569 / 0	0 / 0	782 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 48	-102.1 -102.1	0.09 (1)	R-C	-503 / 0	0.13 (1)	
B-C	-5191 / 0	-102.1 -102.1	0.58 (1)	C-Q	0 / 3989	0.48 (1)	
C-D	-8460 / 0	-102.1 -102.1	0.29 (1)	Q-D	-1534 / 0	0.41 (1)	
D-T	-6703 / 0	-102.1 -102.1	0.33 (1)	M-G	-2588 / 0	0.69 (1)	
T-U	-6703 / 0	-102.1 -102.1	0.33 (1)	M-H	0 / 3834	0.47 (1)	
U-E	-6703 / 0	-102.1 -102.1	0.33 (1)	L-H	-580 / 0	0.15 (1)	
E-V	-8840 / 0	-102.1 -102.1	0.34 (1)	D-P	0 / 980	0.12 (1)	
V-F	-8840 / 0	-102.1 -102.1	0.34 (1)	P-E	-382 / 0	0.19 (1)	
F-G	-8840 / 0	-102.1 -102.1	0.34 (1)	E-N	-240 / 0	0.08 (1)	
G-H	-8280 / 0	-102.1 -102.1	0.28 (1)	N-G	0 / 2170	0.27 (1)	
H-I	-5021 / 0	-102.1 -102.1	0.55 (1)	B-R	0 / 4218	0.52 (1)	
I-J	0 / 42	-102.1 -102.1	0.08 (1)	L-I	0 / 4078	0.50 (1)	
S-B	-4340 / 0	0.0 0.0	0.16 (1)				
K-I	-4200 / 0	0.0 0.0	0.15 (1)				
S-R	0 / 0	-38.5 -38.5	0.08 (3)				
R-Q	0 / 4138	-38.5 -38.5	0.31 (1)				
Q-P	0 / 8460	-38.5 -38.5	0.44 (1)				
P-W	0 / 6933	-38.5 -38.5	0.60 (1)				
W-X	0 / 6933	-38.5 -38.5	0.60 (1)				
X-O	0 / 6933	-38.5 -38.5	0.60 (1)				
O-Y	0 / 6933	-38.5 -38.5	0.60 (1)				
Y-N	0 / 6933	-38.5 -38.5	0.60 (1)				
N-M	0 / 6291	-38.5 -38.5	0.62 (1)				
M-L	0 / 4001	-38.5 -38.5	0.33 (1)				
L-K	0 / 0	-38.5 -38.5	0.06 (2)				

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX-	MAX+	FACE DIR.
D	10-4-4	-128	-128		BACK VERT
F	16-1-12	-128	-128		BACK VERT
N	16-1-12	-58	-74		BACK VERT
Q	16-4-8	-2062	-2062		BACK VERT
N	8-10-8	-1539	-1539		BACK VERT
Q	10-4-4	-58	-74		BACK VERT
T	12-4-4	-128	-128		BACK VERT
U	13-4-9	-128	-128		BACK VERT
V	14-4-12	-128	-128		BACK VERT
W	12-4-4	-58	-74		BACK VERT
X	13-4-8	-58	-74		BACK VERT

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.9 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2015, CBC 2012
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.93")
 CALCULATED VERT. DEFL. (LL) = L/989 (0.14")
 ALLOWABLE DEFL. (TL) = L/360 (0.93")
 CALCULATED VERT. DEFL. (TL) = L/989 (0.23")

CSI: TC=0.68/1.00 (B-C-1), BC=0.60/1.00 (N-P-1), WB=0.69/1.00 (G-M-1), SS=0.17/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.80)
 JSI METAL= 0.76 (C) (INPUT = 1.00)

DRWG NO. TAM 1105577
 STRUCTURAL
 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T27	1	2	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

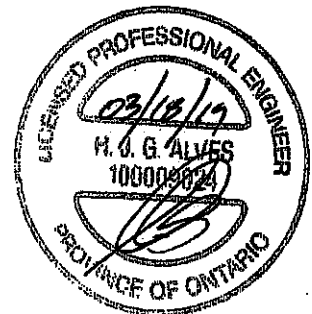
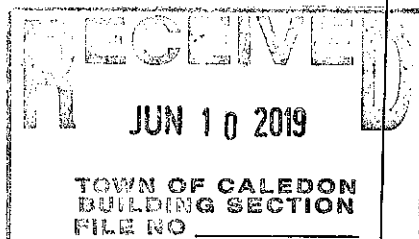
Version 8.230 © Nov 17 2018 Mitak Industries, Inc. Mon Mar 18 21:27:18 2019 Page 2
ID:tr5ampg4l74dMkT9JQOlyNxSe-JQQ81XPpJwHEFaVxGISAMVNUIM0qO5FW8y5hkgZz6C7

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

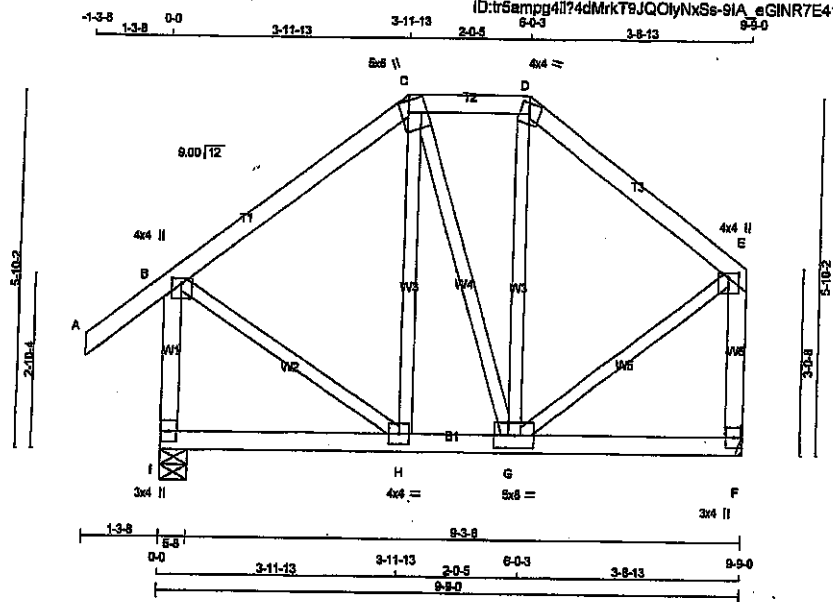
JT	LOC.	LC1	MAX	MAX*	FACE	DIR.	TYPE	HEEL	CONN.
Y	14-4-12	-58	-74	—	BACK	VERT	TOTAL	—	—

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



DWG NO. TAM 11905577
STRUCTURAL
CONSULTANT C. H. Y.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Mon Mar 18 21:22:18 2018 Page 1	DRWG NO.
200172-400374	T28	1	1	TRUSS DESC.	ID:tr5ampg4i74dMrkT8JQOlyNxSe-9IA_eGINR7E41AQuaG1M7qsO1d2Sde03hZx8LzZbG	
Tamarack Roof Truss, Burlington						



Scale = 1/32"

LUMBER	SIZE	LUMBER
A - C	2x4	DRY
C - D	2x4	DRY
D - E	2x4	DRY
E - F	2x4	DRY
F - G	2x4	DRY
G - H	2x4	DRY
H - I	2x4	DRY
I - J	2x4	DRY
J - K	2x4	DRY
K - L	2x4	DRY
L - M	2x4	DRY
M - N	2x4	DRY
N - O	2x4	DRY
O - P	2x4	DRY
P - Q	2x4	DRY
Q - R	2x4	DRY
R - S	2x4	DRY
S - T	2x4	DRY
T - U	2x4	DRY
U - V	2x4	DRY
V - W	2x4	DRY
W - X	2x4	DRY
X - Y	2x4	DRY
Y - Z	2x4	DRY
Z - A	2x4	DRY

PLATES (table is in inches)	W	LEN	Y	X
B - TMVW+p	MT20	4.0	4.0	1.00 2.00
C - TTWW+m	MT20	5.0	6.0	2.25 1.50
D - TTW-m	MT20	4.0	4.0	
E - TMVW+p	MT20	4.0	4.0	1.00 2.00
F - BMV1+p	MT20	3.0	4.0	
G - BMVWW-t	MT20	5.0	6.0	
H - BMVWW-t	MT20	4.0	4.0	
I - BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
I 826	0	826	5-8
F 885	0	885	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
I 609	363 / 0	102 / 0
F 512	283 / 0	102 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (L.C)	MAX	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
FR-TO	MEMB.	FORCE (LBS)	FROM	TO	PLF	CSI (L.C)	LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX	CSI (L.C)	MEMB.	FORCE (LBS)	MAX
A-B	0 / 42		-102.1	-102.1	0.14 (1)	10.00	H-C	-55 / 76	0.03 (1)				H-C	-55 / 76	0.03 (1)
B-C	-378 / 0		-102.1	-102.1	0.21 (1)	8.25	C-G	-24 / 0	0.01 (1)				C-G	-24 / 0	0.01 (1)
C-D	-294 / 0		-102.1	-102.1	0.05 (1)	8.25	G-D	-72 / 82	0.04 (1)				G-D	-72 / 82	0.04 (1)
D-E	-370 / 0		-102.1	-102.1	0.18 (1)	8.25	B-H	0 / 356	0.08 (1)				B-H	0 / 356	0.08 (1)
E-F	-760 / 0		0.0	0.0	0.12 (1)	7.81	G-E	0 / 361	0.08 (1)				G-E	0 / 361	0.08 (1)
F-G	-622 / 0		0.0	0.0	0.10 (1)	7.81									
H-I	0 / 0		-38.5	-38.5	0.11 (3)	10.00									
I-G	0 / 302		-38.5	-38.5	0.13 (2)	10.00									
G-F	0 / 0		-38.5	-38.5	0.09 (3)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.33")
CALCULATED VERT. DEFL. (LL) = L/699 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.33")
CALCULATED VERT. DEFL. (TL) = L/699 (0.02")

CSI: TC=0.21/1.00 (B-C:1), BC=0.13/1.00 (G-H:2), WB=0.09/1.00 (E-G:1), SI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

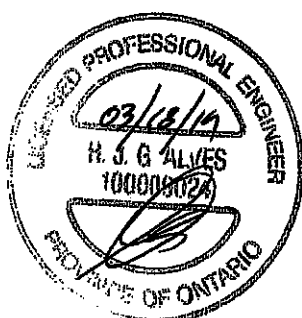
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE - GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

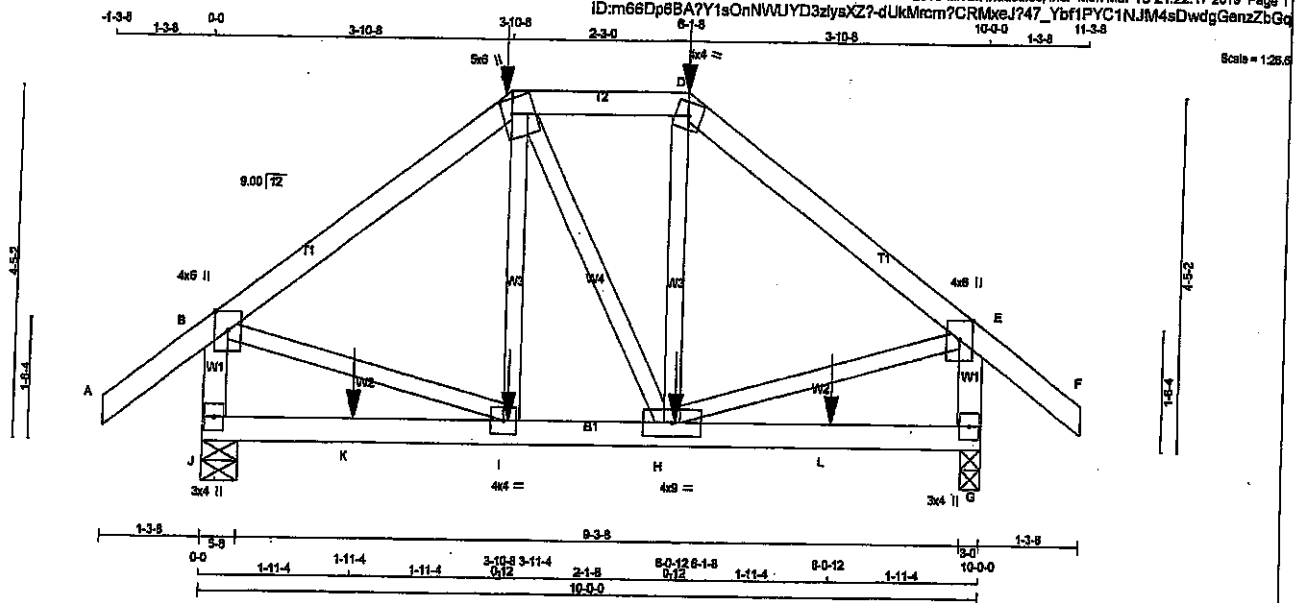
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.60 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)



DRWG NO. TAM 1905578
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T29	1	1	Preston 11	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 47 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
J - B	2x4 DRY	No.2	SPF		
G - E	2x4 DRY	No.2	SPF		
J - G	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		
DRY, SEASONED LUMBER.					

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TTW+m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVWV+1	MT20	4.0	9.0		
I	BMVWV+1	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	DOWN	UP	IN-SX
JT	1120	0	0	0
J	1120	0	0	0
G	1119	0	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	828	482 / 0	144 / 0	0 / 0	0 / 0	202 / 0	0 / 0
G	828	482 / 0	144 / 0	0 / 0	0 / 0	201 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)
FR-TO								
A-B	0 / 42	-102.1	-102.1	0.15 (1)	10.00	I-C	-18 / 140	0.04 (3)
B-C	-832 / 0	-102.1	-102.1	0.31 (1)	6.22	C-H	0 / 6	0.00 (3)
C-D	-665 / 0	-102.1	-102.1	0.10 (1)	6.25	H-D	-14 / 145	0.04 (3)
D-E	-833 / 0	-102.1	-102.1	0.31 (1)	6.21	B-I	0 / 663	0.17 (1)
E-F	0 / 42	-102.1	-102.1	0.15 (1)	10.00	H-E	0 / 684	0.17 (1)
J-B	-1039 / 0	0.0	0.0	0.12 (1)	7.68			
G-E	-1037 / 0	0.0	0.0	0.12 (1)	7.68			
J-K	0 / 0	-38.5	-38.5	0.17 (3)	10.00			
K-I	0 / 0	-38.5	-38.5	0.17 (3)	10.00			
I-H	0 / 863	-38.5	-38.5	0.22 (2)	10.00			
H-L	0 / 0	-38.5	-38.5	0.17 (3)	10.00			
L-G	0 / 0	-38.5	-38.5	0.17 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-204	-204		BACK	VERT	TOTAL		
D	6-1-8	-204	-204		BACK	VERT	TOTAL		
H	8-0-12	-36	-46		BACK	VERT	TOTAL		
I	3-11-4	-36	-46		BACK	VERT	TOTAL		
K	1-11-4	-36	-46		BACK	VERT	TOTAL		
L	8-0-12	-36	-46		BACK	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.31/1.00 (D-E-1), BC=0.22/1.00 (H-2),
WB=0.17/1.00 (E-H-1), SSI=0.14/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

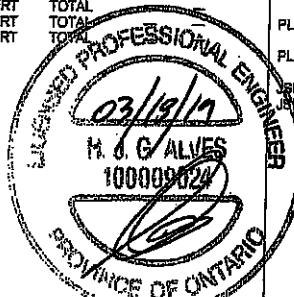
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 786 1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

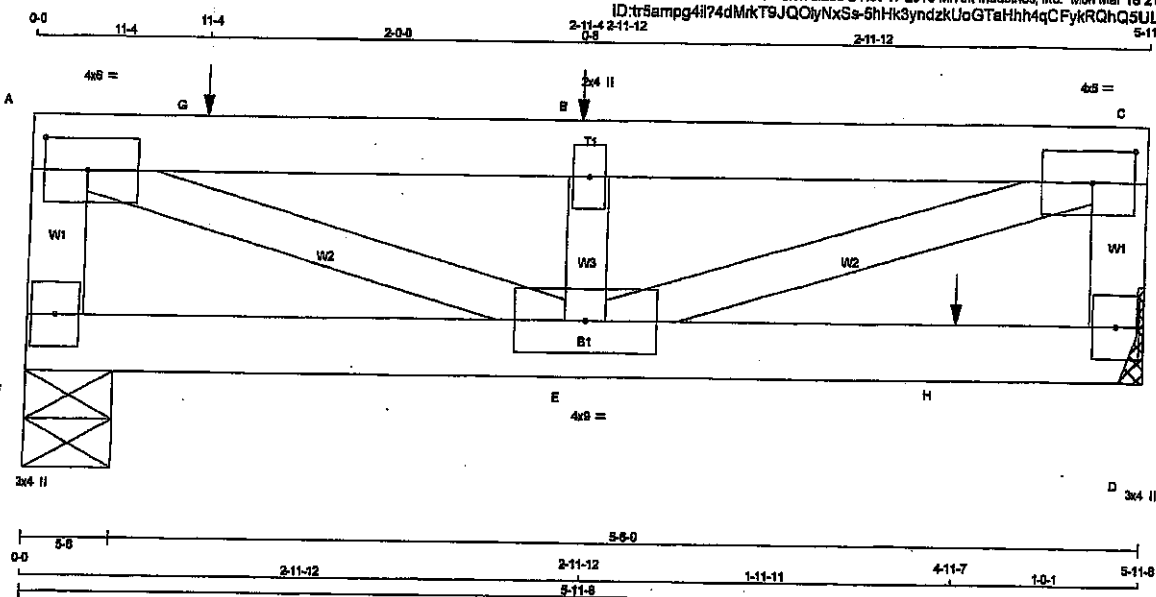
PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.86 (I) (INPUT = 0.90)
METAL = 0.46 (E) (INPUT = 1.00)



DRWG NO. TAM 1705579
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T30	1	2	Preston 11 TRUSS DESC.	
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
D - C	2x4	DRY No.2	SPF
F - D	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122'X3") SPIRAL NAILS		
F-A 1	12	TOP
A-C 1	12	SIDE(0.0)
C-D 1	12	TOP
BOTTOM CHORDS : (0.122'X3") SPIRAL NAILS		
F-D 1	12	SIDE(14.0)
WEBS : (0.122'X3") SPIRAL NAILS		
2x3 1	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-t	MT20	4.0	6.0	2.00 2.75
B	TMVW-w	MT20	2.0	4.0	
C	TMVW-t	MT20	4.0	6.0	2.00 2.75
D	BMV1+p	MT20	3.0	4.0	
E	BMVWWW-t	MT20	4.0	9.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1317	0	1317	0	0	5-8	5-8
D	1578	0	1578	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	980	533 / 0	165 / 0	0 / 0	0 / 0	283 / 0	0 / 0
D	1176	639 / 0	205 / 0	0 / 0	0 / 0	331 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBB			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (L.C.)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (L.C.)
FR-TO		FROM TO			FR-TO		
F-A	-1245 / 0	0.0	0.0 0.07 (1)	7.81	A-E	0 / 2820	0.35 (1)
A-G	-2662 / 0	-102.1	-102.1 0.21 (1)	5.33	E-B	-1408 / 0	0.11 (1)
G-B	-2662 / 0	-102.1	-102.1 0.21 (1)	5.33	E-C	0 / 2820	0.35 (1)
B-C	-2662 / 0	-102.1	-102.1 0.10 (1)	5.49			
D-C	-1078 / 0	0.0	0.0 0.08 (1)	7.81			
F-E	0 / 0	-63.5	-63.5 0.06 (3)	10.00			
E-H	0 / 0	-63.5	-63.5 0.35 (1)	10.00			
H-D	0 / 0	-63.5	-63.5 0.35 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-11-4	-1017	-1017	—	TOP	VERT	TOTAL	—	—
G	11-4	-244	-244	—	BACK	VERT	TOTAL	—	—
H	4-11-7	-647	-647	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	20.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.21/1.00 (A-B:1), BC=0.35/1.00 (D-E:1),
WB=0.35/1.00 (A-E:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(FLI)		(FLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

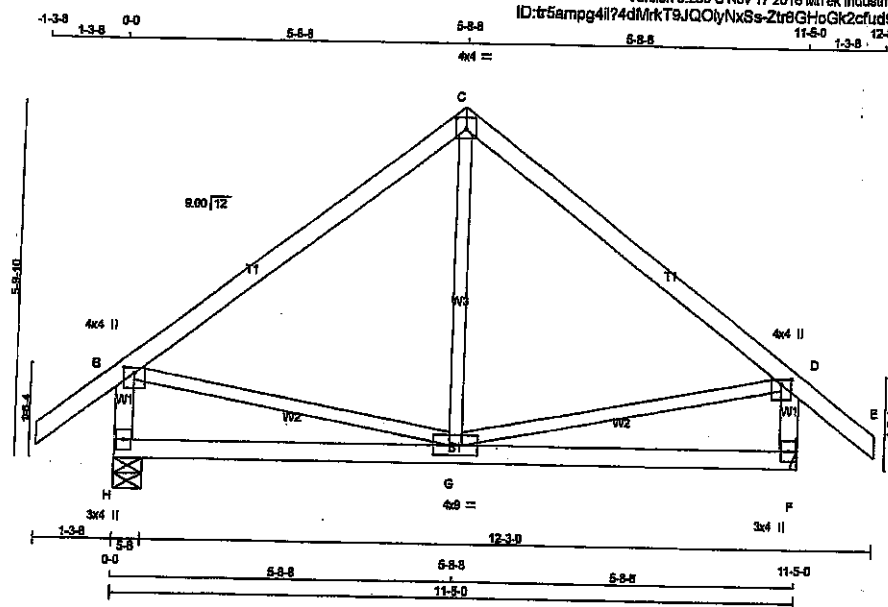
JSI GRIP= 0.82 (C) (INPUT = 0.80)
JSI METAL= 0.33 (C) (INPUT = 1.00)

DWG NO. TAM 17205580
STRUCTURAL
CONCRETE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400374	T31	3	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Mon Mar 18 21:22:19 2018 Page 1
ID:tr5ampg4il74dMkT9JQOlyNxSs-Ztr8GHoGk2cfud9TFFb3ISUsnq2jq7RVWNx9NjgzZbGo

Scale = 1/32"



TOTAL WEIGHT = 3 X 49 = 147 lb
(MIF)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
H - B	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	943	0	0	5-8
F	943	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	696	412/0	120/0	0/0	0/0	165/0	0/0
F	696	412/0	120/0	0/0	0/0	165/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0/42	-102.1 -102.1	0.14 (1)	10.00	G-C	0/254	0.08 (3)
B-C	-557/0	-102.1 -102.1	0.43 (1)	6.25	B-G	0/455	0.10 (1)
C-D	-557/0	-102.1 -102.1	0.43 (1)	6.25	G-D	0/455	0.10 (1)
D-E	0/42	-102.1 -102.1	0.14 (1)	10.00			
H-B	-860/0	0.0	0.0 0.09 (1)	7.81			
F-D	-860/0	0.0	0.0 0.09 (1)	7.81			
H-G	0/0	-36.5 -36.5	0.29 (3)	10.00			
G-F	0/0	-36.5 -36.5	0.29 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 20.0	PSF
	DL = 8.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.39")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CS1: TC=0.43/1.00 (B-C:1), BC=0.29/1.00 (G-H:3),
WB=0.10/1.00 (D-G:1), SS=0.18/1.00 (B-C:1)

DOI: LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

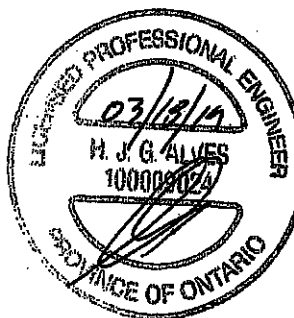
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1657 1656

PLATE PLACEMENT TOL. = 0.250 inches

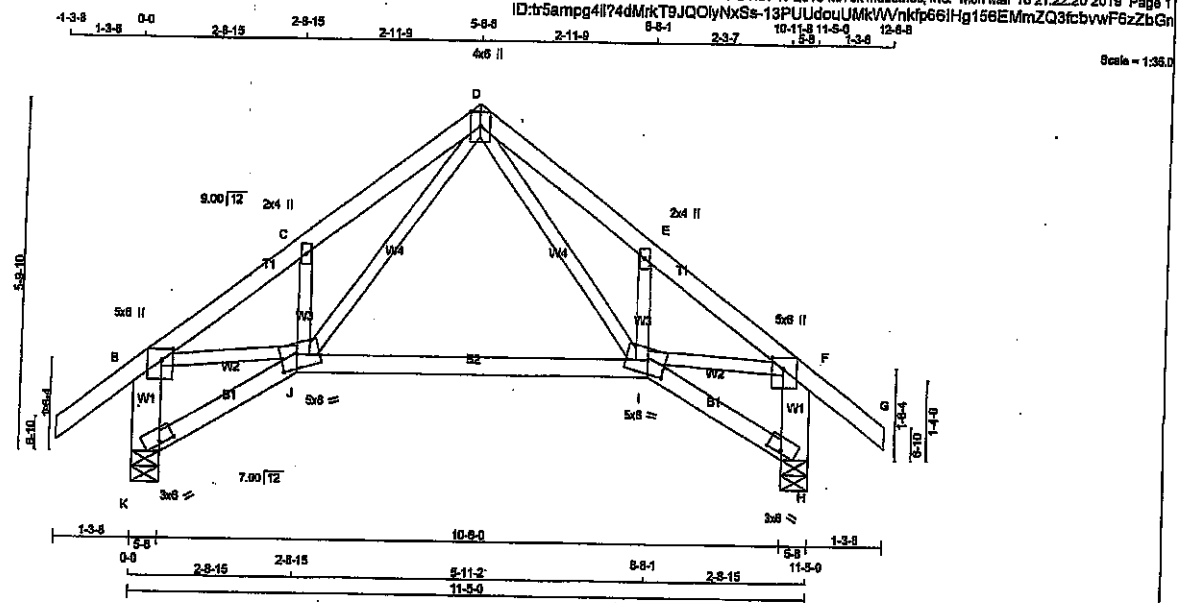
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (D) (INPUT = 0.80)
JSI METAL= 0.22 (D) (INPUT = 1.00)



DRWG NO. TAMI 19905581
STRUCTURAL
EX. 100008024

JOB NAME 200172-400374	TRUSS NAME T31S	QUANTITY 2	PLY 1	JOB DESC. Preston 11	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



TOTAL WEIGHT = 2 X 53 = 107 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
K - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	8.0	2.25 2.25
C	TMVW+w	MT20	2.0	4.0	
D	TTWV+p	MT20	4.0	6.0	Edge
E	TMVW+w	MT20	2.0	4.0	
F	TMVW+p	MT20	5.0	8.0	2.25 2.25
H	BVM1-I	MT20	3.0	6.0	0.50 3.00
I	BBWWW-m	MT20	5.0	8.0	2.75 3.50
J	BBWWW-m	MT20	5.0	8.0	2.75 3.50
K	BVM1-I	MT20	3.0	6.0	0.50 3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	943	0	943	0	0	5-8	5-8
H	943	0	943	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	586	412 / 0	120 / 0	0 / 0	0 / 0	185 / 0	0 / 0
H	586	412 / 0	120 / 0	0 / 0	0 / 0	185 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	D-I	0 / 688	0.15 (1)	
B-C	-1125 / 0	-102.1 -102.1	0.10 (1)	E-J	-341 / 0	0.05 (1)	
C-D	-1157 / 0	-102.1 -102.1	0.11 (1)	J-K	0 / 688	0.15 (1)	
D-E	-1157 / 0	-102.1 -102.1	0.11 (1)	K-L	-341 / 0	0.05 (1)	
E-F	-1125 / 0	-102.1 -102.1	0.10 (1)	L-M	0 / 613	0.21 (1)	
F-G	0 / 42	-102.1 -102.1	0.14 (1)	M-N	0 / 613	0.21 (1)	
G-H	-881 / 0	0.0 0.0	0.08 (1)				
H-I	-881 / 0	0.0 0.0	0.08 (1)				
K-J	0 / 0	-38.5 -38.5	0.07 (3)				
J-I	0 / 514	-38.5 -38.5	0.36 (2)				
I-H	0 / 0	-38.5 -38.5	0.07 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/858 (0.18")

CSI: TO=0.141/0.0 (F-G:1), BO=0.381/0.0 (I-J:2),
WB=0.211/0.0 (F-I:1), SS=0.121/0.0 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1887 1668

PLATE PLACEMENT TOL. = 0.250 inches

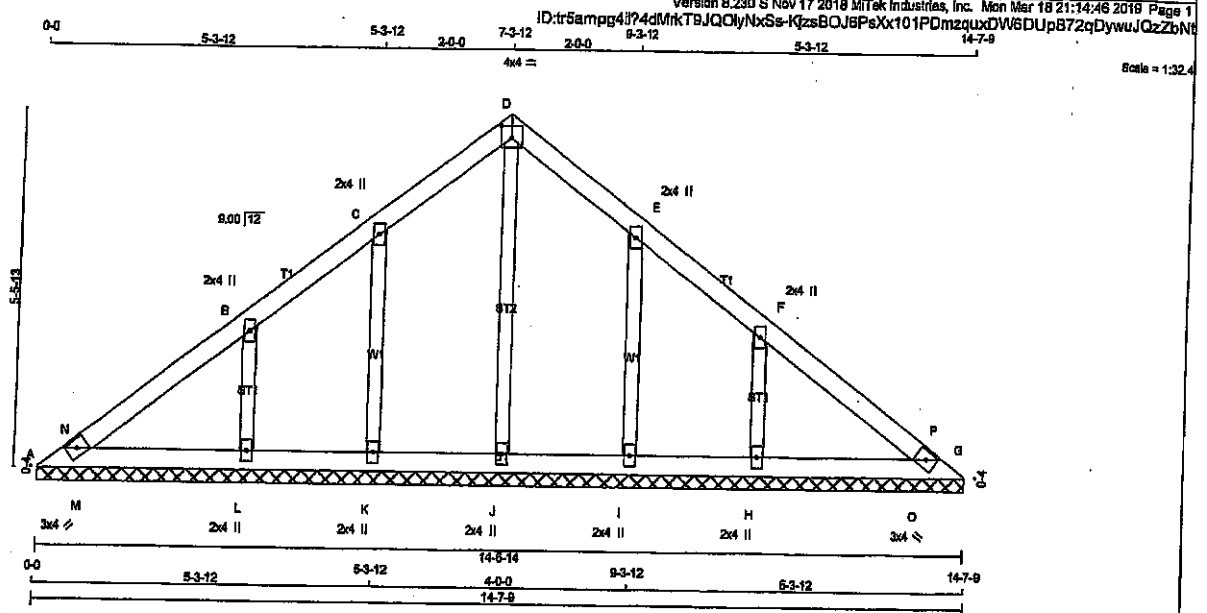
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)



DRWG NO. TAM T19055.82
STRUCTURAL
CITY OF BURLINGTON

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Version 8.230 S Nov 17 2018 MITek Industries, Inc. Mon Mar 18 21:14:46 2018 Page 1	DRWG NO.
200172-400373	V5	1	1	TRUSS DESC.	ID:tr5ampg4j74dMkT9JQOlyNxSs-KQzsBOJ8P5Xx101PDmzquxOW6DUUp872qDywuJQzZbNt	
Tamarack Roof Truss, Burlington						



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
A - G	2x4	DRY	No.2	SPF	
ALL WEBS 2x3 DRY					
DRY: SEASONED LUMBER.					

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, C, E, F						
B	TBW-h	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
G	TBM1-h	MT20	3.0	4.0		
H, I, J, K, L						
H	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS						
JT	FACTORED		MAXIMUM FACTORED		INPUT BRG	REQD BRG
	GROSS REACTION		GROSS REACTION			
	VERT	HORZ	DOWN	HORZ	UPLIFT	
A	158	0	158	0	0	14-8-14
G	158	0	158	0	0	14-8-14
J	384	0	384	0	0	14-8-14
L	438	0	438	0	0	14-8-14
H	438	0	438	0	0	14-8-14
K	245	0	245	0	0	14-8-14
I	245	0	245	0	0	14-8-14

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	UNFACTORED REACTIONS	
								1ST LCASE	MAX. MIN. COMPONENT REACTIONS
A	117	89/0	20/0	0/0	0/0	28/0	0/0		
G	117	89/0	20/0	0/0	0/0	28/0	0/0		
J	278	140/0	85/0	0/0	0/0	72/0	0/0		
L	328	178/0	85/0	0/0	0/0	82/0	0/0		
H	328	178/0	85/0	0/0	0/0	82/0	0/0		
K	182	108/0	32/0	0/0	0/0	43/0	0/0		
I	182	108/0	32/0	0/0	0/0	43/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, L, H, K, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	WEBS	
											MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO												
A-N	0/38	-102.1	-102.1	0.04 (2)	10.00	J-D	-288/0	0.12 (1)				
N-B	0/78	-102.1	-102.1	0.11 (1)	10.00	L-B	-278/0	0.04 (1)				
B-C	0/88	-102.1	-102.1	0.11 (1)	10.00	H-F	-278/0	0.04 (1)				
C-D	0/65	-102.1	-102.1	0.06 (1)	10.00	K-C	-205/0	0.05 (1)				
D-E	0/65	-102.1	-102.1	0.06 (1)	10.00	I-E	-205/0	0.05 (1)				
E-F	0/88	-102.1	-102.1	0.11 (1)	10.00	M-N	-84/33	0.00 (1)				
F-P	0/78	-102.1	-102.1	0.11 (1)	10.00	O-P	-84/33	0.00 (1)				
P-G	0/38	-102.1	-102.1	0.04 (2)	10.00							
A-M	-63/0	-38.5	-38.5	0.09 (1)	8.25							
M-L	-44/0	-38.5	-38.5	0.09 (1)	8.25							
L-K	-58/0	-38.5	-38.5	0.08 (2)	8.25							
K-J	-61/0	-38.5	-38.5	0.03 (2)	8.25							
J-I	-61/0	-38.5	-38.5	0.03 (2)	8.25							
I-H	-58/0	-38.5	-38.5	0.08 (2)	8.25							
H-O	-44/0	-38.5	-38.5	0.09 (1)	8.25							
O-G	-63/0	-38.5	-38.5	0.09 (1)	8.25							

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 240 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA D88-08, CSA D88-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.11/1.00 (F-P:1), SC=0.09/1.00 (H-O:1), WB=0.12/1.00 (D-L:1), SB=0.10/1.00 (F-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

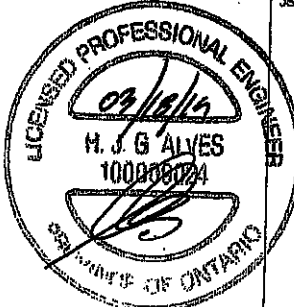
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 510 354 1657 788 1987 1636

PLATE PLACEMENT TOL. = 0.250 inches

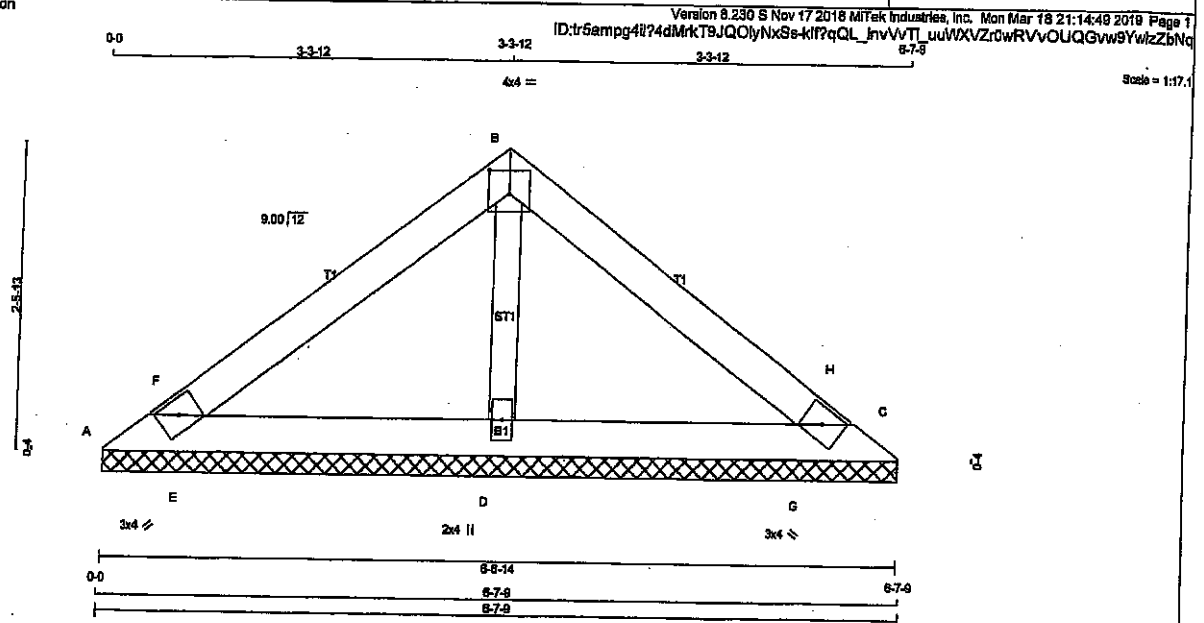
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (D) (INPUT = 0.80)
JSI METAL= 0.15 (F) (INPUT = 1.00)



DWG NO. TAM 1905566
STRUCTURAL
COMPONENT ONLY

JOB NAME 200172-400373	TRUSS NAME N7	QUANTITY 1	PLY 1	JOB DESC. Preston 11	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0			
B	TTW-p	MT20	4.0	4.0	2.25	2.00	
C	TBM1-h	MT20	3.0	4.0			
D	BMW1-w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		RECRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX	IN-SX
A	85	0	95	0	0	8-6-14	8-6-14	8-6-14	8-6-14
C	95	0	95	0	0	8-6-14	8-6-14	8-6-14	8-6-14
D	734	0	734	0	0	8-6-14	8-6-14	8-6-14	8-6-14

UNFACTORED REACTIONS							
1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
A	70	41/0	13/0	0/0	0/0	17/0	0/0
C	70	41/0	13/0	0/0	0/0	17/0	0/0
D	550	301/0	112/0	0/0	0/0	137/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
A-F	0/183	-102.1	-102.1 0.05 (1)	10.00	D-B	-488/0	0.08 (1)
F-B	0/202	-102.1	-102.1 0.14 (1)	10.00	E-F	-153/12	0.00 (1)
B-H	0/202	-102.1	-102.1 0.14 (1)	10.00	G-H	-153/12	0.00 (1)
H-C	0/183	-102.1	-102.1 0.05 (1)	10.00			
A-E	-180/0	-38.5	-38.5 0.11 (1)	8.25			
E-D	-164/0	-38.5	-38.5 0.13 (1)	8.25			
D-G	-164/0	-38.5	-38.5 0.13 (1)	8.25			
G-C	-180/0	-38.5	-38.5 0.11 (1)	8.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
CL = 6.0 PSF
BOT CH. LL = 10.5 PSF
CL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (B-H:1), BC=0.13/1.00 (D-E:1), WB=0.08/1.00 (B-D:1), SS=0.10/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

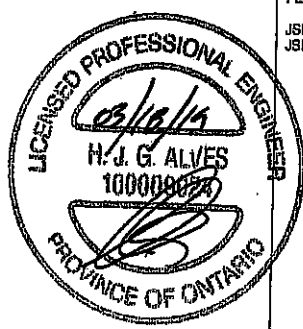
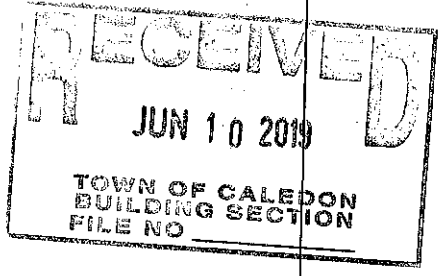
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.80)
JSI METAL= 0.14 (B) (INPUT = 1.00)



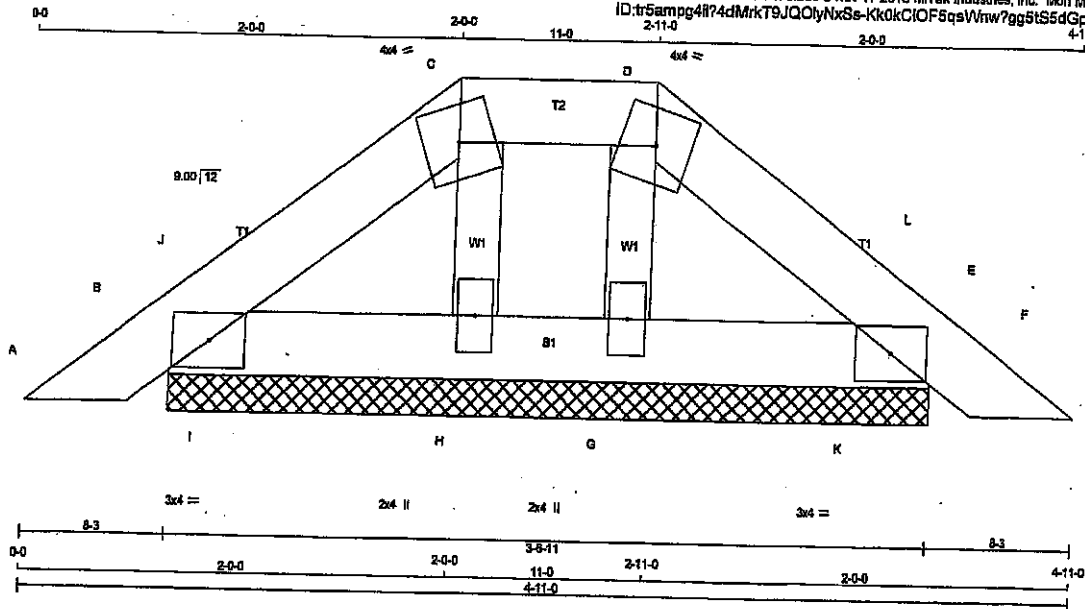
DWG NO. TAM 17905568
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	PRSTON 11	DRWG NO.
200172-400371	PB3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:51 2019 Page 1
ID:tr5ampg4#74dMkT9JQOlyNxSs-Kk0kCfOF5qsVWw?gg5tS5dGpH8h9aPAHqtug8zZblQ

Scale = 1/8" = 1'-0"



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2
C - D	2x4 DRY	No.2
D - F	2x4 DRY	No.2
B - E	2x4 DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	172	0	172	0	3-8-11	3-8-11
E	172	0	172	0	3-8-11	3-8-11
H	138	0	138	0	3-8-11	3-8-11
G	138	0	138	0	3-8-11	3-8-11

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
B	124	83/0	14/0	0/0	0/0	0/0	27/0	0/0
E	124	83/0	14/0	0/0	0/0	0/0	27/0	0/0
H	104	54/0	23/0	0/0	0/0	0/0	27/0	0/0
G	104	54/0	23/0	0/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED (LBS)	FACTORED VERT. LOAD (LBS)	LC1 PLF	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0/17	-102.1	-102.1	0.03 (1)	10.00	H-C	-80/0	0.01 (1)	
B-J	-48/0	-102.1	-102.1	0.01 (3)	6.25	G-D	-80/0	0.01 (1)	
J-C	-35/0	-102.1	-102.1	0.02 (1)	8.25	I-J	-52/11	0.00 (1)	
C-D	-21/0	-102.1	-102.1	0.01 (1)	6.25	K-L	-52/11	0.00 (1)	
D-L	-35/0	-102.1	-102.1	0.02 (1)	6.25				
L-E	-48/0	-102.1	-102.1	0.01 (3)	6.25				
E-F	0/17	-102.1	-102.1	0.03 (1)	10.00				
B-I	0/29	-38.5	-38.5	0.03 (1)	10.00				
I-H	0/29	-38.5	-38.5	0.03 (1)	10.00				
H-G	0/21	-38.5	-38.5	0.02 (1)	10.00				
G-K	0/29	-38.5	-38.5	0.03 (1)	10.00				
K-E	0/29	-38.5	-38.5	0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (E-F-1), BC=0.03/1.00 (G-K-1), WB=0.01/1.00 (D-G-1), SS=0.05/1.00 (E-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

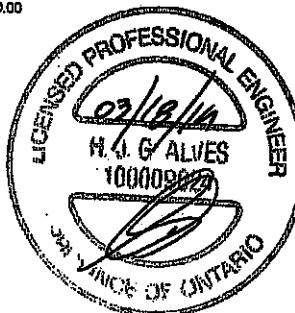
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1657

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

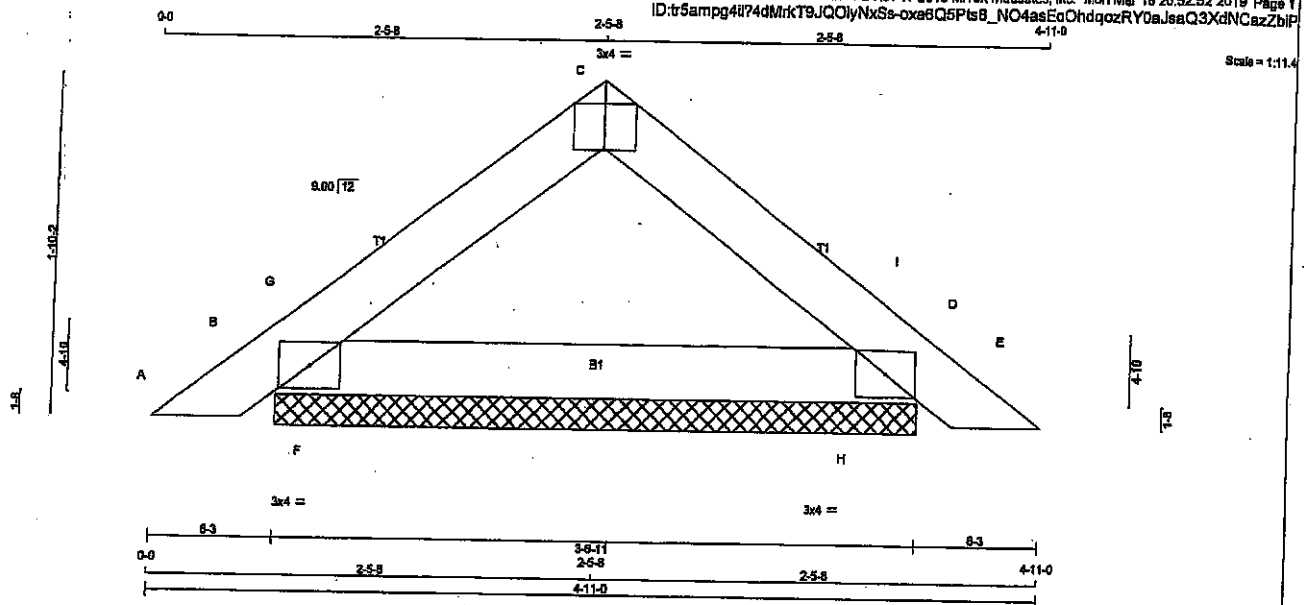
JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.03 (E) (INPUT = 1.00)



DRWG NO. 11705544
STRUCTURAL
CROSS-SECTION (S/P)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	PB4	6	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Mon Mar 18 20:52:52 2019 Page 1
 ID:tr5ampg4#74dMkT9JQOlyNxSs-oxa6Q5Pts8_NO4asEoOhdqozRY0aJsaQ3XdNCazZbIP



TOTAL WEIGHT = 6 X 11 = 66 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TT-p	MT20	3.0	4.0	Edge 2.00
D	TMB1-I	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	311	0	311	0	3-6-11	3-6-11
D	311	0	311	0	3-6-11	3-6-11

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
B	228	138 / 0	37 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0
D	228	138 / 0	37 / 0	0 / 0	0 / 0	0 / 0	53 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		VERT. LOAD	LC1	MAX	MAX	MEMB.	WEBS		MAX	MAX
	MAX.	FACTORED						MAX.	FACTORED		
FR-TO	FORCE (LBS)	(PLF)	FROM	TO	CSI (LC)	UNBRAC LENGTH	FR-TO	FORCE (LBS)	(PLF)	CSI (LC)	UNBRAC LENGTH
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	6.25	F-G	0 / 148	0.00 (1)	10.00	6.25
B-G	-262 / 0	-102.1	-102.1	0.05 (3)	6.25	6.25	H-I	0 / 148	0.00 (1)	10.00	6.25
G-C	-151 / 0	-102.1	-102.1	0.08 (1)	6.25	6.25					
C-I	-151 / 0	-102.1	-102.1	0.08 (1)	6.25	6.25					
I-D	-262 / 0	-102.1	-102.1	0.05 (3)	6.25	6.25					
D-E	0 / 17	-102.1	-102.1	0.03 (1)	10.00	6.25					
B-F	0 / 130	-38.5	-38.5	0.04 (3)	10.00	6.25					
F-H	0 / 130	-38.5	-38.5	0.08 (2)	10.00	6.25					
H-D	0 / 130	-38.5	-38.5	0.04 (3)	10.00	6.25					

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 28.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24" IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2015, CBC 2012
 - CSA 085-08, CSA 085-14
 - TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-G:1), BC=0.08/1.00 (F-H:2), WB=0.00/1.00 (F-G:1), SS=0.08/1.00 (D-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

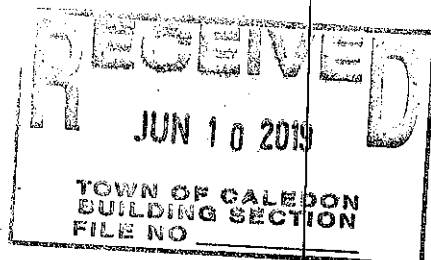
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 788 1887 1656

PLATE PLACEMENT TOL = 0.250 inches

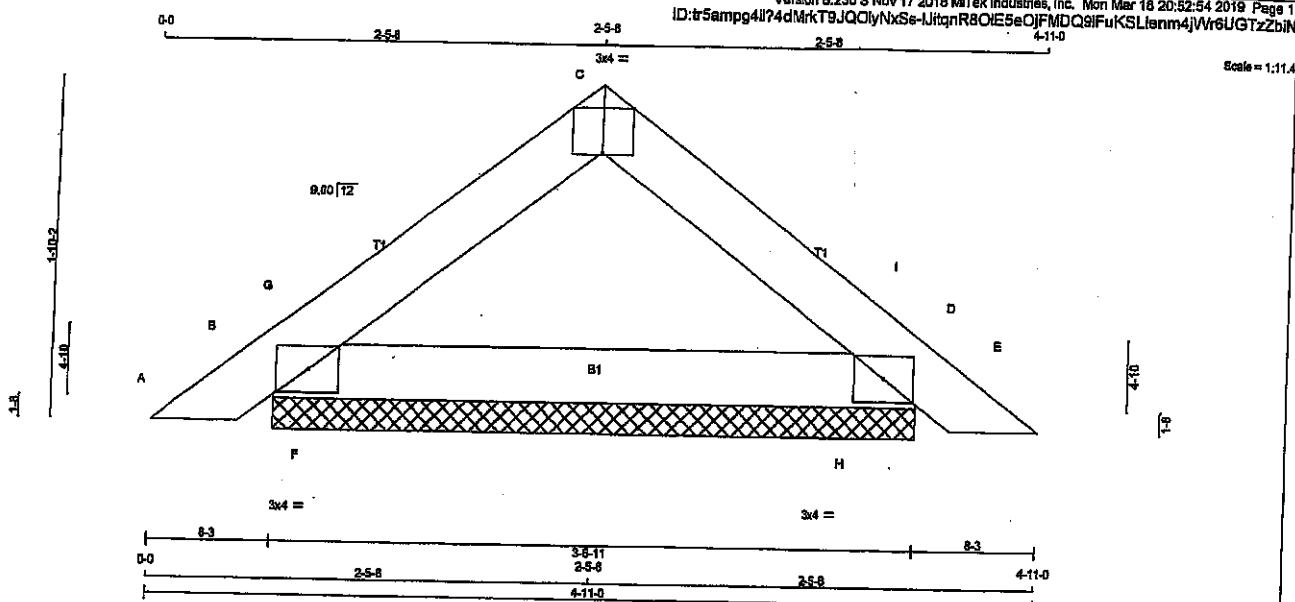
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.50)
 JSI METAL= 0.06 (B) (INPUT = 1.00)



DRWG NO. TAM T1905545
 STRUCTURAL
 (C) 2018 MITEK INDUSTRIES, INC.

JOB NAME 200172-400371	TRUSS NAME PB4Z	QUANTITY 1	PLY 2	JOB DESC. Preston 11	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Mon Mar 18 20:52:54 2019 Page 1 ID:tr5ampg4ll74dMkT9JQOlyNxSe-UJtgnR8OIE5eOJFMDQ9iFuKSLenm4jVr6UGTzZbIn	



TOTAL WEIGHT = 2 X 11 = 22 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A-C	12	TOP
C-E	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
B-D	12	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
B	311	311	3-8-11	3-8-11
D	311	311	3-8-11	3-8-11

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	WIND	DEAD	SOIL
B	COMBINED	SNOW	LIVE	PERM.LIVE	
B	228	138 / 0	37 / 0	0 / 0	53 / 0
D	228	138 / 0	37 / 0	0 / 0	53 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 117	-102.1	-102.1	0.01 (1)	10.00		
B-G	-281 / 0	-102.1	-102.1	0.03 (3)	8.25		
G-C	-150 / 0	-102.1	-102.1	0.03 (1)	8.25		
C-I	-150 / 0	-102.1	-102.1	0.03 (1)	8.25		
I-D	-281 / 0	-102.1	-102.1	0.03 (3)	8.25		
D-E	0 / 117	-102.1	-102.1	0.01 (1)	10.00		
B-F	0 / 130	-38.5	-38.5	0.02 (3)	10.00		
F-H	0 / 130	-38.5	-38.5	0.04 (2)	10.00		
H-D	0 / 130	-38.5	-38.5	0.02 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (C-I-1), BC=0.04/1.00 (F-H-2), WS=0.00/1.00 (F-G-1), SS=0.04/1.00 (D-I-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

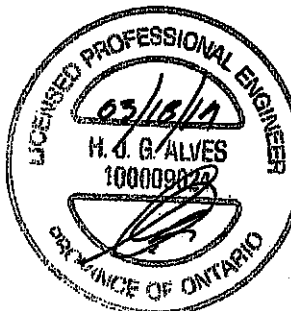
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788

PLATE PLACEMENT TOL. = 0.250 inches

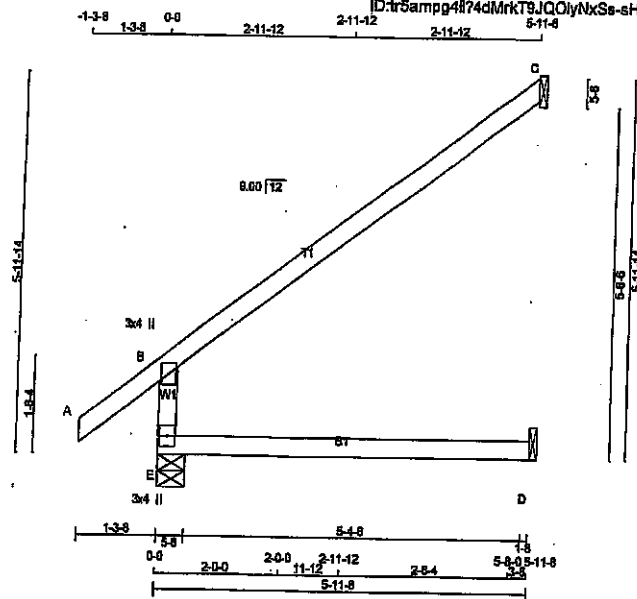
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (D) (INPUT = 0.90)
JSI METAL= 0.03 (D) (INPUT = 1.00)



DRWG NO. TAM **T7905546**
STRUCTURAL
CONSENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	J1	15	1	Preston 11	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
E - B	2x4	DRY No.2
A - C	2x4	DRY No.2
E - D	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SK	REQD BRG IN-SK
	VERT	HORZ	DOWN	HORZ		
E	581	0	581	0	5-8	5-8
C	228	0	228	0	1-8	1-8
D	44	0	49	0	1-8	1-8

SEE MITEK STANDARD DETAIL BS7821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	406	296/0	0/0	0/0	0/0	109/0	0/0
C	157	130/0	0/0	0/0	0/0	27/0	0/0
D	35	0/0	0/0	0/0	0/0	35/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM	TO	FR-TO			
E-B	-521/0	0.0	0.0	0.11 (4)	7.81		
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00		
B-C	-48/0	-102.1	-102.1	0.62 (1)	6.25		
E-D	0/0	-17.5	-17.5	0.13 (4)	10.00		

TOTAL WEIGHT = 15 X 19 = 279 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.62/1.00 (B-C-1), BC=0.13/1.00 (D-E-4),
WB=0.00/1.00 (V-A-0), SS=0.24/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	518 354	1687 788	1987 1556

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

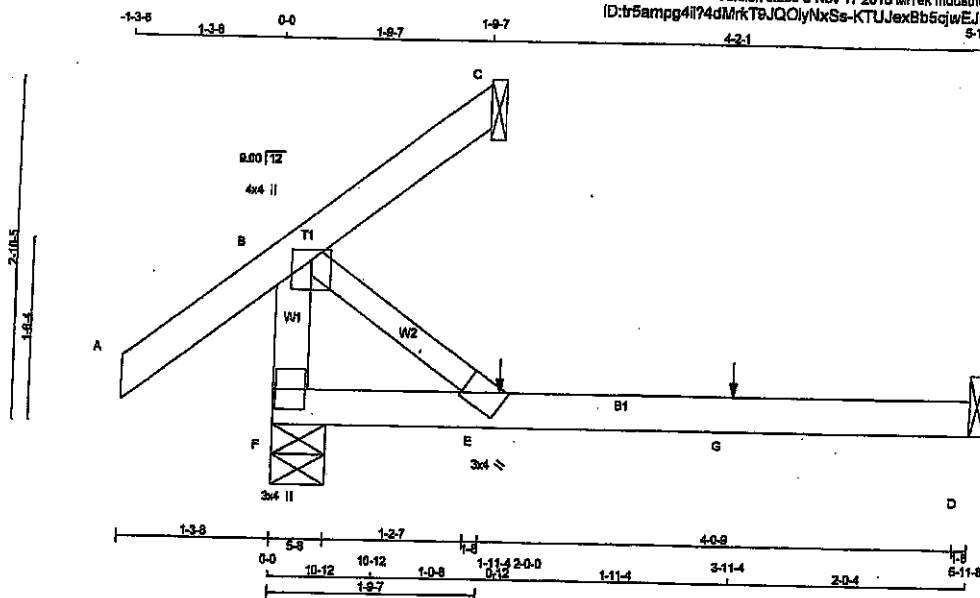


ENGR. NO. TAM 71905550
STRUCTURAL
CALCULATIONS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	J2	2	1	Preston 11	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.290 S Nov 17 2018 MTEK Industries, Inc. Mon Mar 18 20:52:34 2019 Page 1
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 5-11-8

Scale = 1:17.7



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMVW+p	MT20	4.0	4.0	1.00	2.00
2	BMVW+p	MT20	3.0	4.0	2.00	1.25
3	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
F	401	0	0	5-8
C	37	0	0	5-8
D	115	0	0	1-8

SEE MTEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	301	163/0	63/0	0/0	0/0	75/0	0/0
C	25	21/0	0/0	0/0	0/0	4/0	0/0
D	104	0/0	63/0	0/0	0/0	42/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO		
F-B	-288/0	0.0	0.0	0.03 (1)	7.81	B-E	0/0	0.00 (1)
A-B	0/42	-102.1	-102.1	0.15 (1)	10.00			
B-C	-33/0	-102.1	-102.1	0.15 (1)	6.25			
F-E	0/0	-38.5	-38.5	0.28 (2)	10.00			
E-G	0/0	-38.5	-38.5	0.33 (2)	10.00			
G-D	0/0	-38.5	-38.5	0.33 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11-4	1	1	1	FRONT	VERT	TOTAL	—	—
G	3-11-4	1	1	1	FRONT	VERT	TOTAL	—	—

TOTAL WEIGHT = 2 X 14 = 28 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2015, OBC 2012
 - CSA 088-09, CSA 088-14
 - TPC 2011, TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/887 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/532 (0.13")

CSI: TC=0.15/1.00 (A-B-1), BC=0.33/1.00 (D-E-2), WB=0.00/1.00 (B-E-1), GSI=0.13/1.00 (E-F-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1657

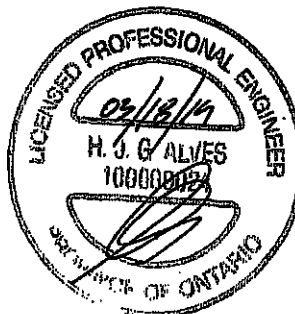
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.00)
 JSI METAL= 0.06 (B) (INPUT = 1.00)

ENGINEER: TAM 77905551
 STRUCTURAL
 CONSULTANT ONLY

THIS STRUCTURE MUST BE
 CONSTRUCTED TO MEET OR
 EXCEED THE PROVISIONS OF
 THE ONTARIO BUILDING CODE



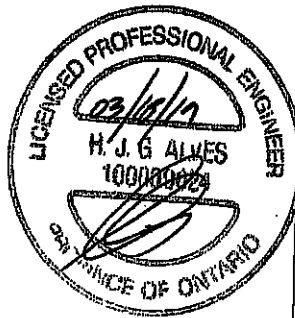
Tamarack Roof Truss, Burlington

Verilon 8.230 S Nov 17 2018 Mitek Industries, Inc. Mon Mar 18 20:52:36 2019 Page 1
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3-8-7 5-11-8

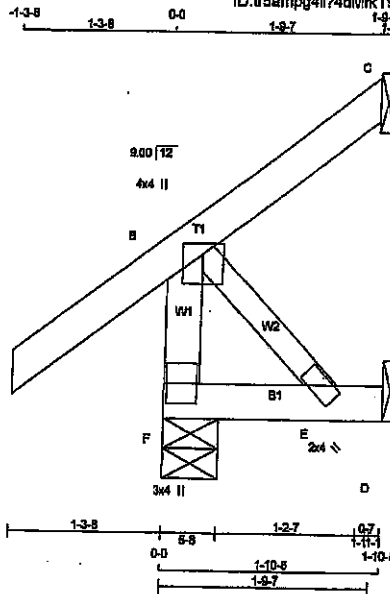
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DISC NO. TAM 77905552
STRUCTURAL
CONTAINER ONLY

CHORDS				WEBBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
R-TO		FROM TO			FR-TO		
F-B	-334 / 0	0.0 0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00			
B-C	0 / 0	-102.1 -102.1	0.25 (1)	10.00			
F-E	0 / 0	-38.5 -38.5	0.28 (3)	10.00			
E-D	0 / 0	-38.5 -38.5	0.31 (3)	10.00			



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	J4	4	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Scale = 1:17.7

TOTAL WEIGHT = 4 X 9 = 37 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMMW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REGRD	BRG	IN-SX
F	322	0	322	0	0	5-8	5-8		
C	37	0	37	0	45	1-8	1-8		
D	36	0	45	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
F	229	163 / 0	20 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	25	21 / -31	0 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	33	0 / 0	20 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (PLF)
FR-TO									
F-B	-288 / 0	0.0	0.0	0.03 (1)	7.81	0 / 0	0.00 (1)		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00				
B-C	-33 / 0	-102.1	-102.1	0.13 (1)	6.25				
F-E	0 / 0	-38.5	-38.5	0.03 (3)	10.00				
E-D	0 / 0	-38.5	-38.5	0.03 (3)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:					
TOP CH.	LL	=	29.0	PSF	
	DL	=	8.0	PSF	
BOT CH.	LL	=	10.5	PSF	
	DL	=	7.0	PSF	
TOTAL LOAD		=	52.5	PSF	

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBCC 2015
- CSA 089-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.18")
CALCULATED VERT. DEFL. (LL) = $L/699$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.18")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SB=0.00/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

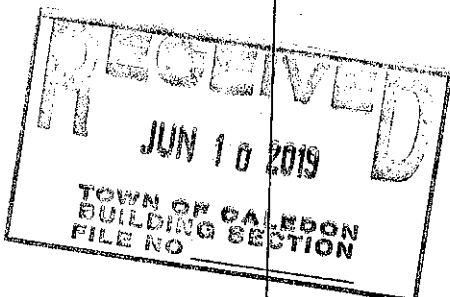
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1997 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

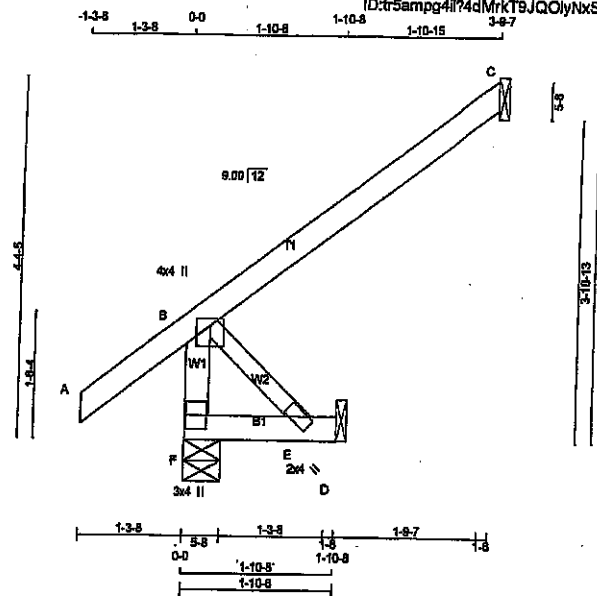


DESIGNATION 71905563
STRUCTURAL
CONSULTING ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	J5	4	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Mon Mar 18 20:52:39 2019 Page 1
ID:tr5ampg4ll74dMrKT9JQOlyNxSs-hQICheFkw8LDL44Mzafec5mjrtb8m_MV3?_BErzZblc

Scale = 1/25.0



TOTAL WEIGHT = 4 X 12 = 48 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

F - B 2x4 DRY No.2

A - C 2x4 DRY No.2

F - D 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

SPF

SPF

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/699 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/898 (0.00")

CSI: TC=0.25/1.00 (B-C:1), BC=0.04/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

MAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.60)

JSI METAL= 0.07 (B) (INPUT = 1.00)

BURGESS TAM 7405554
STRUCTURAL
GOOD ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT	370	0	370	0
F	370	0	370	0
C	193	0	193	0
D	39	0	45	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	282	190/0	20/0	0/0	0/0	52/0	0/0
C	133	110/0	0/0	0/0	0/0	23/0	0/0
D	33	0/0	20/0	0/0	0/0	13/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

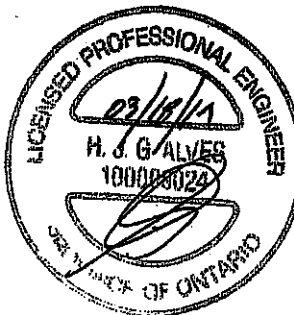
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
F-B	-334/0	0.0 0.0 0.03 (1)	7.81	B-E	0/0	0.09 (1)	
A-B	0/42	-102.1 -102.1 0.14 (1)	10.00				
B-C	0/0	-102.1 -102.1 0.25 (1)	10.00				
F-E	0/0	-38.5 -38.5 0.04 (3)	10.00				
E-D	0/0	-38.5 -38.5 0.03 (3)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN





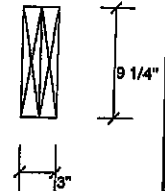
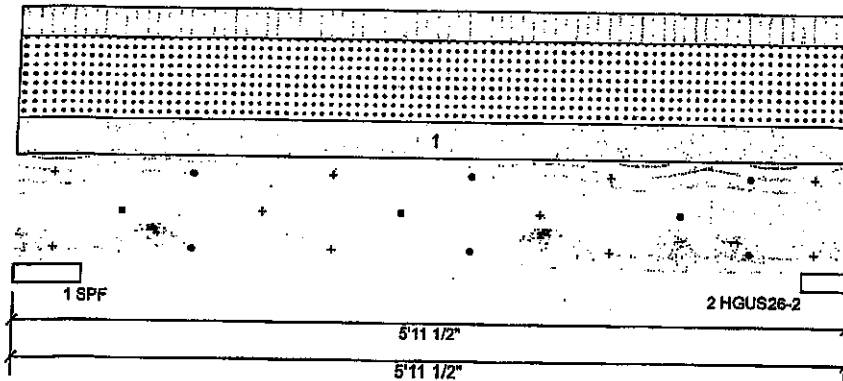
isDesign™

Client:
Project:
Address:Date: 3/18/2019
Designer:
Job Name: 200172
Project #:

Page 3 of 4

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	214	265	592	0
2	205	254	567	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	332 / 1102	1433 L
2 - HGUS...	4.000"	19%	318 / 1057	1375 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1650 ft-lb	3' 1/2"	6039 ft-lb	0.273 (27%)	1.25D+1.5S	L
Unbraced	1650 ft-lb	3' 1/2"	5210 ft-lb	0.317 (32%)	1.25D+1.5S	L
Shear	1247 lb	1'2"	3984 lb	0.313 (31%)	1.25D+1.5S	L
LL Defl inch	0.015 (L/4270)	3' 1/2"	0.176 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.021 (L/3095)	3' 1/2"	0.176 (L/360)	0.120 (12%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Nail from opposite sides as indicated by + and • symbols.
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

DWG NO. TAM 1105565
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-8-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

Manufacturer InfoTamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

This design is valid until 12/11/2021





isDesign™

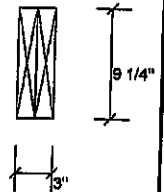
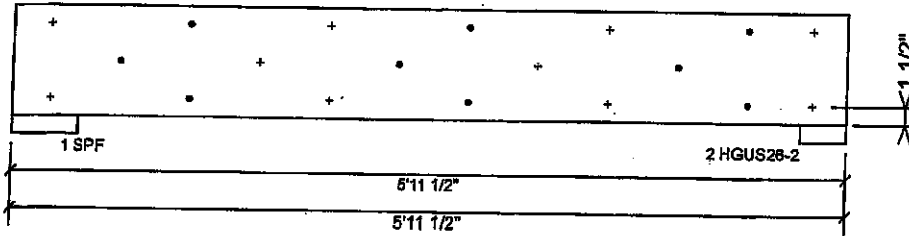
Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 200172
Project #:

Page 4 of 4

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	69.3 %
Load	235.6 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**



DWG NO. TAM 7905565
STRUCTURAL
FOR EMENT ONLY 792

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021



TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

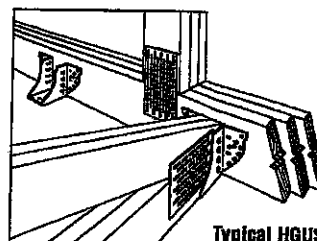
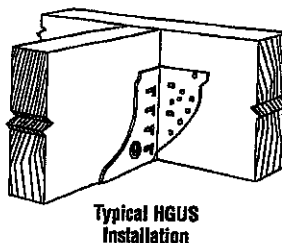
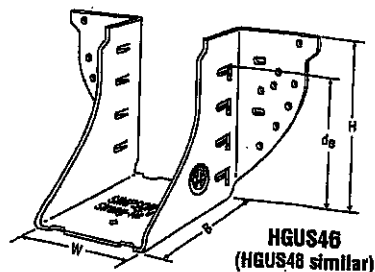
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/4" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

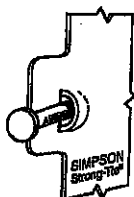
- See current catalogue for options



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

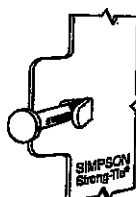
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
HGUS46	12	3 3/4	7 1/4	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS410	12	3 3/4	9	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGUS412	12	3 3/4	10 7/8	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGUS414	12	3 3/4	12 7/8	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645
HGUS5.50/8	12	5 1/4	6 3/8	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS5.50/10	12	5 1/4	8 1/8	4	8 1/8	46-16d	16-16d	6840	14645	4855	10400
HGUS5.50/12	12	5 1/4	10 1/8	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGUS5.50/14	12	5 1/4	12 1/8	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645
HGUS7.25/8	12	7 1/4	7 1/2	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS7.25/10	12	7 1/4	8 1/2	4	8 1/8	46-16d	16-16d	6840	15760	4855	11190
HGUS7.25/12	12	7 1/4	10 1/2	4	10 1/8	56-16d	20-16d	7640	16110	5425	11435
HGUS7.25/14	12	7 1/4	12 1/2	4	11 1/8	66-16d	22-16d	10130	18200	7195	12920

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

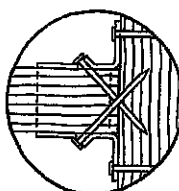


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

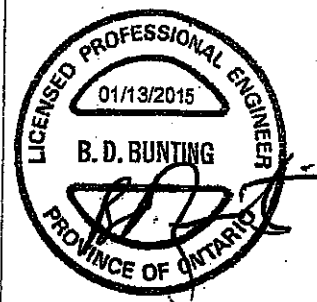
U.S. Patent
5,803,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPEC-SHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

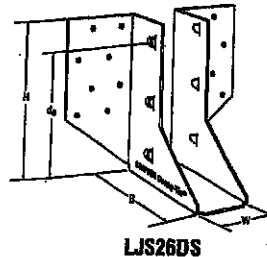
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

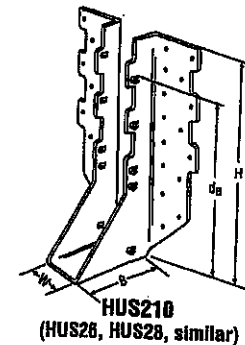
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

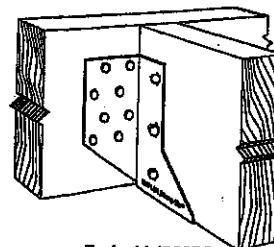
- See current catalogue for options



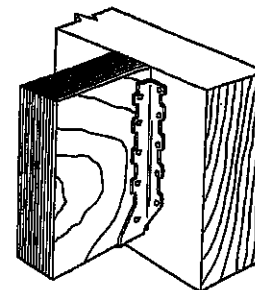
LJS26DS



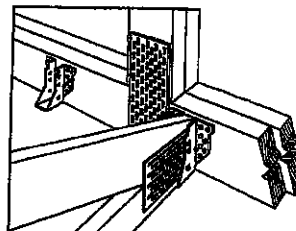
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



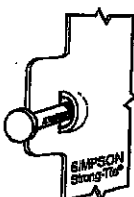
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

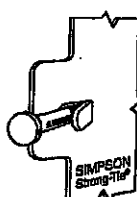
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.16)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS28DS	18	1 1/8	5	3 1/2	4 1/2	16-16d	8-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/8	14-16d	6-16d	2705	4940	2085	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	22-16d	8-16d	3605	5385	2875	4345
HUS210	18	1 1/8	9 1/2	3	7 3/8	30-16d	10-16d	4805	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

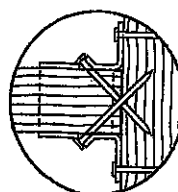


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

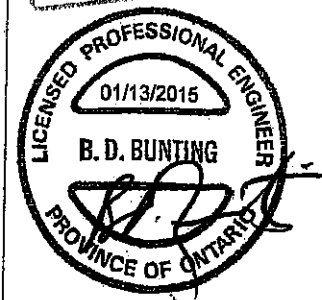
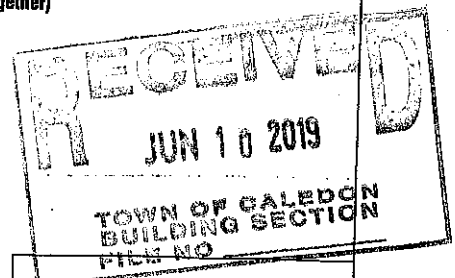
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPEC-HUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

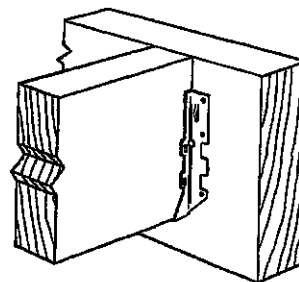
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

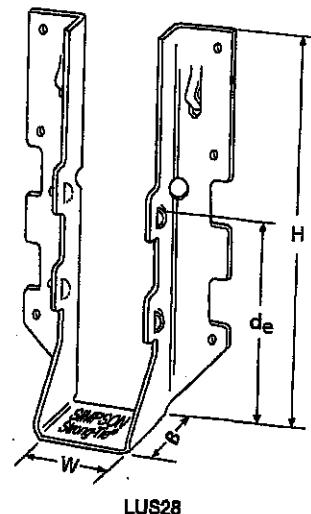
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

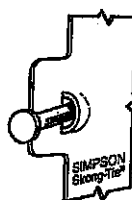


Typical LUS Installation



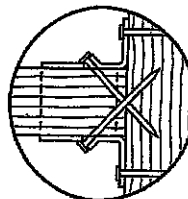
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
LUS24	18	1 1/8	3 1/2	1 1/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/2	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 1/4	3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/2	1 1/4	3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/4	3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

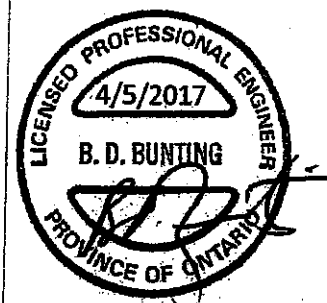


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



(800) 999-5099
strongtie.com



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

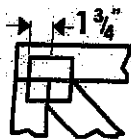
- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

F-1300218

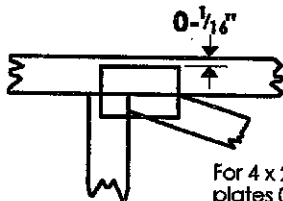
Feb 09, 2018

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

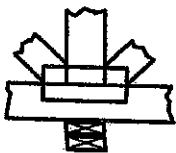
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

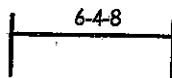


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

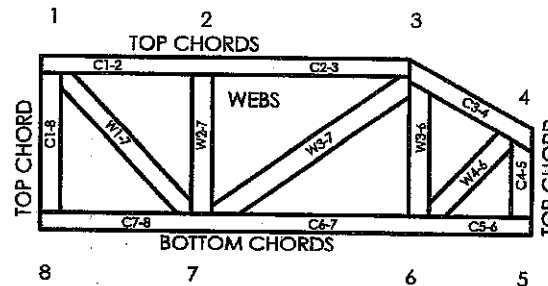
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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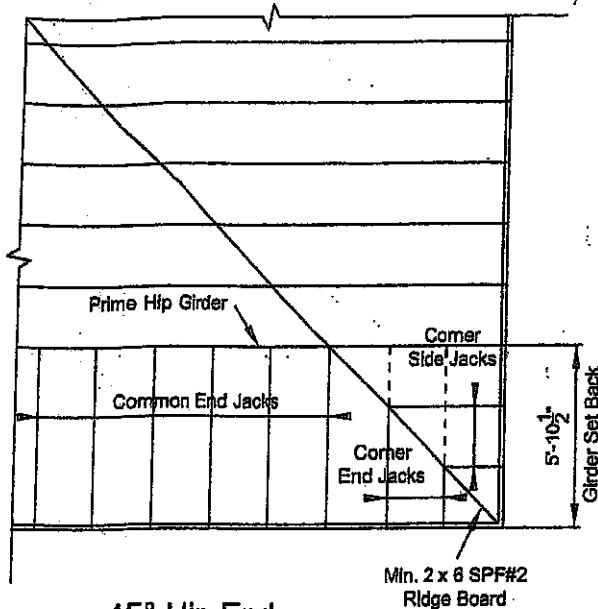
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



45° Hip End

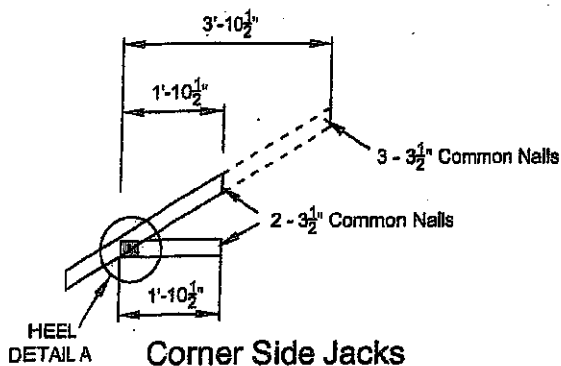
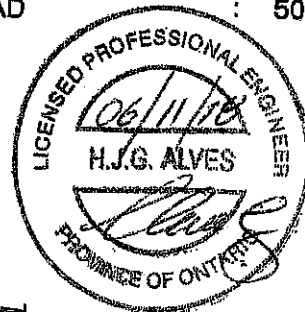
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

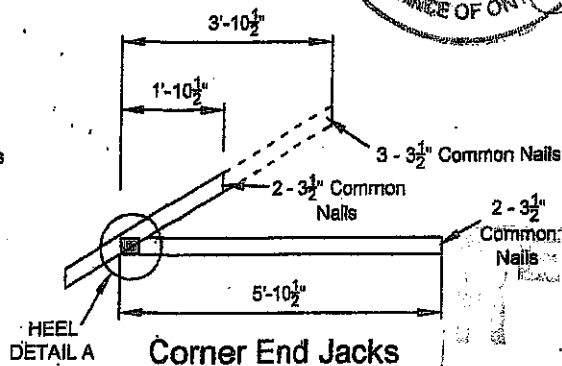
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

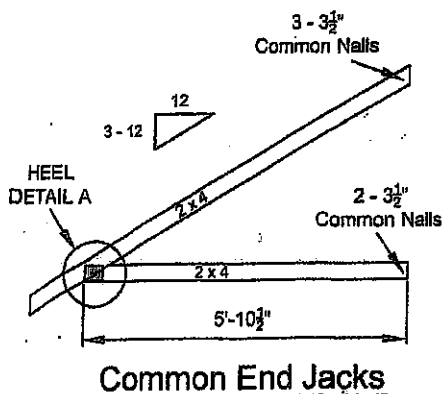
TOTAL LOAD : 50.5 P.S.F



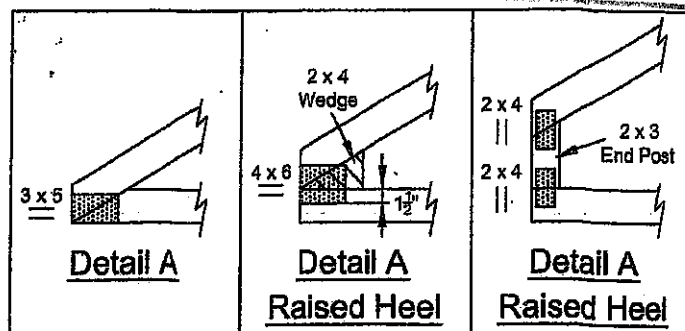
Corner Side Jacks



Corner End Jacks



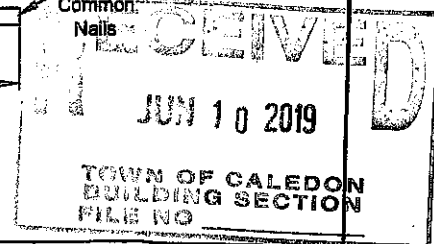
Common End Jacks



Detail A

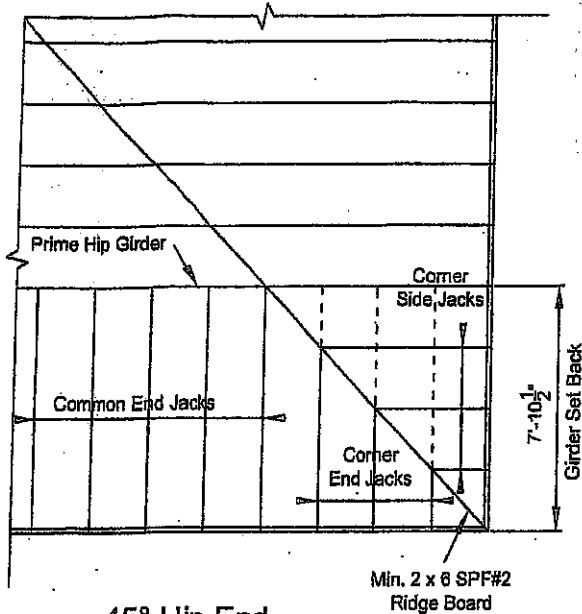
**Detail A
Raised Heel**

**Detail A
Raised Heel**



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

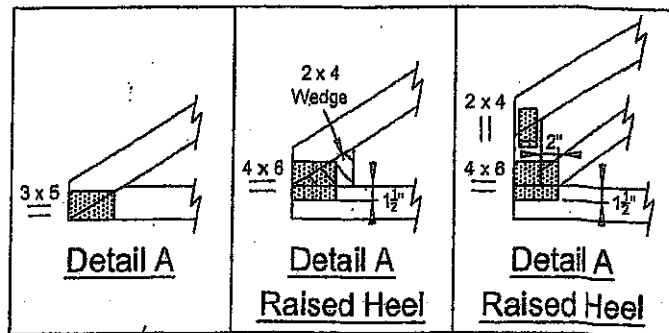
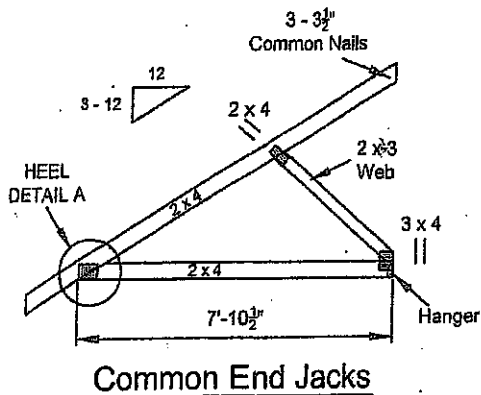
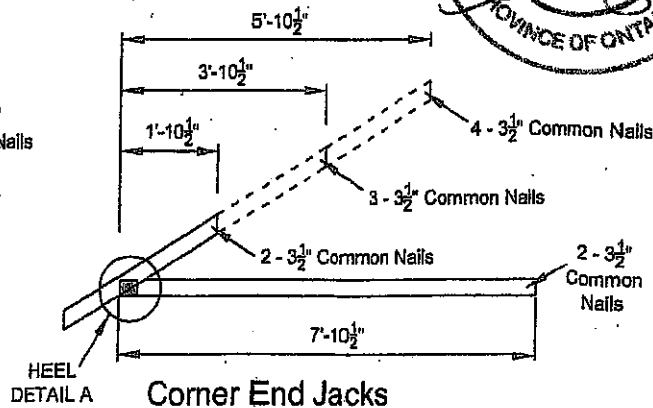
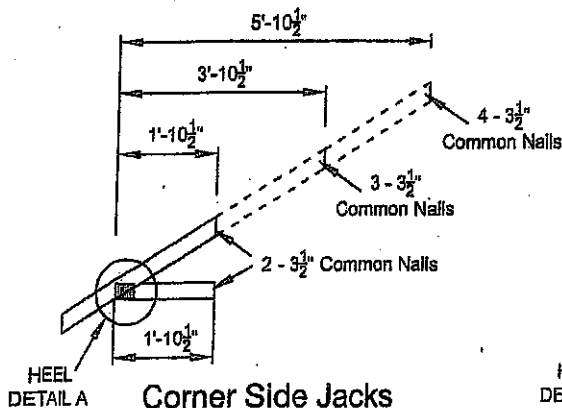
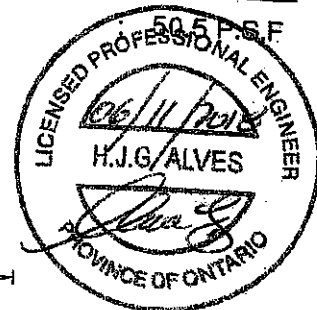
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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