



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 3
 Lot #:
 Elevation: ELV.1-REAR UPGRADE

Job Track: 50120
 PlanLog: 200444
 Layout ID: 401584
 Ref #
 Page: 1 of 2
 Date: 02/25/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T2 Hip	9 /12	29-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	116.51 73.67		
	1	T3 Hip	9 /12	29-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	127.01 82.00		
	1	T4 Hip	9 /12	29-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	128.23 83.17		
	1	T5 Hip	9 /12	29-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	134.97 87.00		
	13	T6 Piggyback Base	9 /12	29-00-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1841.9 1159.17		
	1	T7 Hip	9 /12	29-00-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	137.62 88.00		
	1 2-ply	T8 Hip Girder	9 /12	29-00-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	301.66 193.33		
	1	T9 Hip	9 /12	29-00-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	130.19 82.50		
	1	T10 Hip	9 /12	29-00-00	8-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	140.46 89.83		
	1	T11 Common	9 /12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	56.46 37.67		
	1	T11G GABLE	9 /12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	56.56 38.67		
	2	T12 Common	9 /12	12-03-00	7-07-06	2 x 4	1-03-08 1-03-08	3-00-04 3-00-04	130.14 83.00		
	2	T13 Common	9 /12	12-03-00	6-10-06	2 x 4	1-03-08	1-06-04 3-00-04	110.73 71.00		
	4	T14 Common Supported Gable	6 /12	7-04-00	2-09-11	2 x 4	1-03-08 1-03-08	4-03 4-03	90.44 57.33		



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Job Track: 50120
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 Layout ID: 401584
 Ref #
 Page: 2 of 2
 Date: 02/25/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T100 Hip Girder	9 /12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	268.14 173.33		
	2	T101 Common	9 /12	17-06-00	8-01-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	162.67 102.00		
	1	G101 GABLE	9 /12	17-06-00	8-01-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	83.84 55.00		
	1	PB1 Piggyback	9 /12	8-09-05	2-00-00	2 x 4			22.09 15.33		
	1	PB2 Piggyback	9 /12	8-09-05	3-00-00	2 x 4			25.07 16.33		
	8	PB3 Piggyback	9 /12	8-09-05	3-03-08	2 x 4			184.11 118.67		
	1	PB4 Piggyback	9 /12	8-09-05	1-03-14	2 x 4			20.97 14.83		
	1	PB5 Piggyback	9 /12	8-09-05	2-09-14	2 x 4			25.91 17.83		
	3	J4 Jack-Open	4 /12	5-04-00	2-06-03	2 x 4	1-03-08	3-15 1-10-04	42.47 28.00		
	5	J6 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	8	J9 Jack-Open	9 /12	5-10-08	5-11-02	2 x 4	1-03-08	1-06-04 5-11-02	160.37 108.00		

TOTAL # TRUSS= 65

TOTAL BFT OF ALL TRUSSES= 2928.99

BFT.

TOTAL WEIGHT OF ALL TRSSES 4582.5 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
2	Hardware	LJS26DS	
3	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 7

Version 8.230 S Nov 17 2018 MTek Industries, Inc. Tue Feb 5 20:29:07 2019 Page 1
ID: XxNdf4mUPyxd3LURX25ZQKvOFG0-Sy0JpkyogTV44uNm0AhSyvd2CEaD7mAJdT87BWz05IQ
-1-3-8 0-4 4-9-5 4-9-5 8-9-3 11-3-8 8-4-13 17-8-9 8-5-3 24-2-11 4-9-5 28-0-0 30-3-8 1-3-8
Scale = 1:50.0



ALL WEBS 2x3 DRY
EXCEPT
DRY: SEASONED LUMBER

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD

BEARING

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P. 1

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	28.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD		52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 8.07/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 088-14
- TFCG 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.97")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.15")
 ALLOWABLE DEFL.(TL)= $L/360$ (0.97")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.26")

CSI: TC=0.93/1.00 (F-G:1), BC=0.65/1.00 (L-N:1),
WB=0.40/1.00 (B-O:1), SS=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

	NAIL VALUES		PLATE GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987	1873		

PLATE PLACEMENT TOL = 0.250 inches

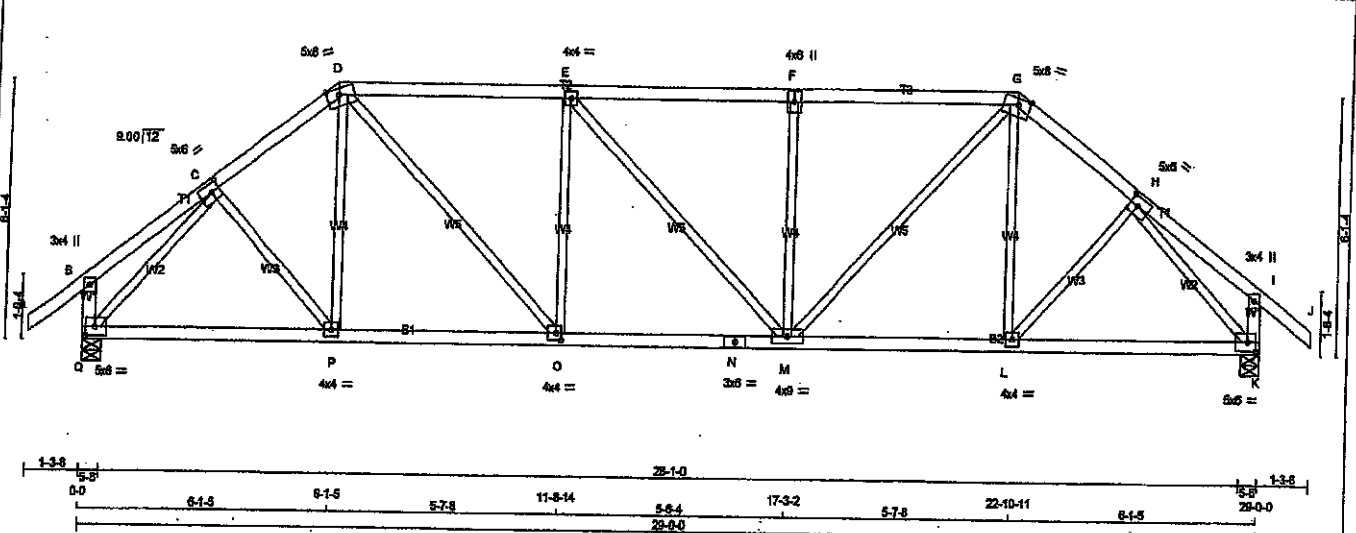
PLATE ROTATION TOL = 5.0 Deg

SI GRIP= 0.90 (B) (INPUT = 0.90)
SI METAL= 0.85 (M) (INPUT = 1.00)

DWG NO. TAM 17502709
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. Freston 3	TRUSS DESC.	DRWG NO. T3
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Tamarack Roof Truss, Burlington
Version 8.230 5 Nov 17 2018 MITek Industries, Inc. Tue Feb 5 20:25:07 2019 Page 1
ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-8y0JPakyogTW4uNmoAhSyvd55Eck7gfJdT87BWzo5IQ
Scale = 1:50.0



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
Q	VERT	DOWN	DOWN	IN-SX	IN-SX
K	2180	0	2180	0	5-8
Q	2180	0	2180	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
K	1820	922 / 0	305 / 0
Q	1820	922 / 0	305 / 0

BEARING MATERIAL TO BE 6PF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
FR-TO	MAX. FACTORED (LBS)	FR-TO	MAX. FACTORED (LBS)
A-B	0 / 142	C-P	0 / 212
B-C	0 / 18	P-D	0 / 202
C-D	-2175 / 0	D-O	0 / 1157
D-E	-2530 / 0	O-E	-628 / 0
E-F	-2484 / 0	E-M	-96 / 0
F-G	-2484 / 0	M-F	-482 / 0
G-H	-2175 / 0	M-G	0 / 1063
H-I	0 / 18	L-G	0 / 202
I-J	0 / 142	L-H	0 / 212
Q-R	-258 / 0	Q-C	-2413 / 0
K-I	-258 / 0	H-K	-2413 / 0
Q-P	0 / 1589		
P-O	0 / 1723		
O-N	0 / 2530		
N-M	0 / 2530		
M-L	0 / 1723		
L-K	0 / 1589		

TOTAL WEIGHT = 127 lb

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015
- CSA 088-14
- TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.87")
CALCULATED VERT. DEF. (LL) = L/599 (0.11")
ALLOWABLE DEF. (TL) = L/360 (0.87")
CALCULATED VERT. DEF. (TL) = L/599 (0.18")

CSK: TC=0.74/1.00 (F-G-1), BC=0.51/1.00 (M-O-1)
WB=0.82/1.00 (H-K-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE	GRIP (DRY) SHEAR SECTION
(PSD)	(PLI) (PLI)
MAX MIN MAX MIN MAX MIN	
MT20	650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.78 (N) (INPUT = 1.00)

RECEIVED
JUN 13 2019

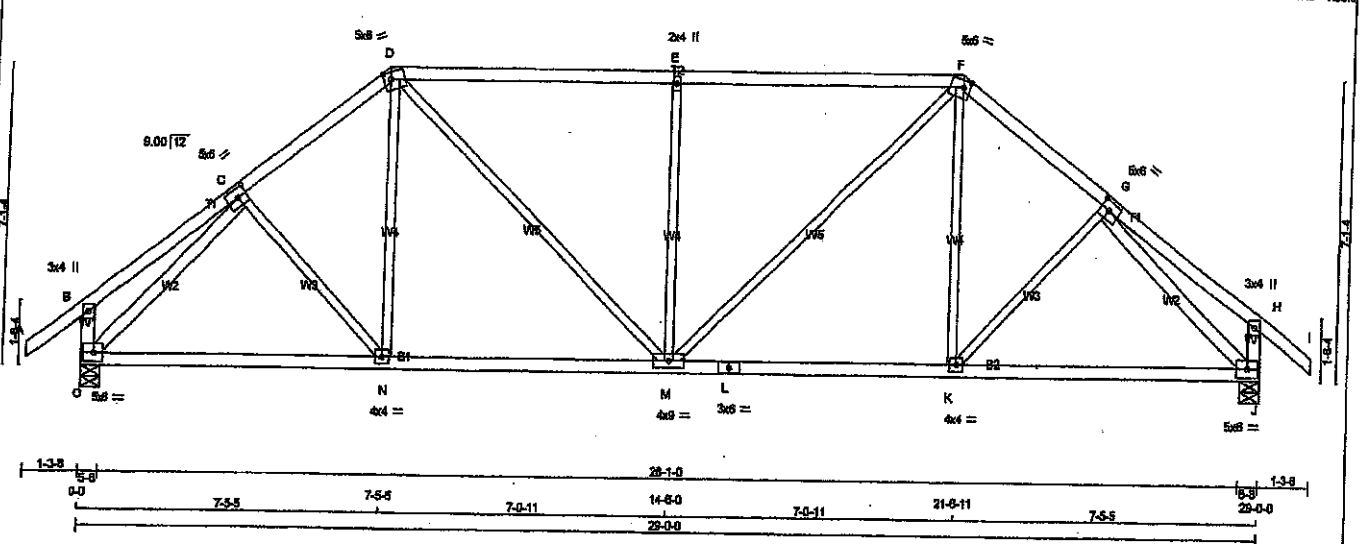
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM 17720 2710
STRUCTURAL
COMPONENT ONLY

JOB NAME: 200444-401156 TRUSS NAME: T4 QUANTITY: 1 PLY: 1 JOB DESC: Preston 3 TRUSS DESC: DRWG NO. T4

Tamarack Roof Truss, Burlington Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:29:08 2019 Page 1
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Scale = 1:50.0



LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 128 lb	
L.G.A. RULES																				[M/F]	
CHORDS																					
SIZE																					
LUMBER																					
DESCR.																					
BEARINGS																				DESIGN CRITERIA	
FACTORED										MAXIMUM FACTORED										SPECIFIED LOADS:	
GROSS REACTION										GROSS REACTION										TOP CH. LL = 28.0 PSF	
JT VERT										DOWN										DL = 8.0 PSF	
O 2180 0										UP										BOT CH. LL = 10.5 PSF	
J 2180 0										IN-SX										DL = 7.0 PSF	
SPF										5-8										TOTAL LOAD = 52.5 PSF	
SPF										5-8											
UNFACTORED REACTIONS																				SPACING = 24.0 IN. OC	
1ST LCASE										MAX./MIN. COMPONENT REACTIONS										LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12	
JT COMBINED										SNOW											
O 1620										LIVE										DEAD	
J 1620										PERM LIVE										SOIL	
SPF										0/0										394/0	
SPF										0/0										0/0	

DRY: SEASONED LUMBER. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING: TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING: TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0/42	-102.1	-102.1 0.14 (1)	C-N	0/129	0.03 (3)	10.00
B-C	0/25	-102.1	-102.1 0.21 (1)	N-D	0/343	0.08 (2)	10.00
C-D	-2143/0	-102.1	-102.1 0.22 (1)	D-M	0/807	0.18 (1)	10.00
D-E	-2276/0	-102.1	-102.1 0.77 (1)	M-E	-594/0	0.77 (1)	10.00
E-F	-2276/0	-102.1	-102.1 0.77 (1)	M-F	0/807	0.18 (1)	10.00
F-G	-2143/0	-102.1	-102.1 0.22 (1)	K-F	0/343	0.08 (2)	10.00
G-H	0/25	-102.1	-102.1 0.21 (1)	K-G	0/129	0.03 (3)	10.00
H-I	0/42	-102.1	-102.1 0.14 (1)	O-C	-2427/0	0.83 (1)	10.00
O-B	-288/0	0.0	0.0 0.03 (1)	G-J	-2427/0	0.83 (1)	10.00
J-H	-288/0	0.0	0.0 0.03 (1)				
O-N	0/1655	-38.5	-38.5 0.61 (2)				
N-M	0/1695	-38.5	-38.5 0.62 (2)				
M-L	0/1695	-38.5	-38.5 0.62 (2)				
L-K	0/1695	-38.5	-38.5 0.62 (2)				
K-J	0/1655	-38.5	-38.5 0.61 (2)				

DESIGN CRITERIA: SPECIFIED LOADS: TOP CH. LL = 29.0 PSF, DL = 6.0 PSF, BOT CH. LL = 10.5 PSF, DL = 7.0 PSF, TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, - CSA 088-14, - TFC 2014

(55 % OF 37.6 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/980 (0.97"), CALCULATED VERT. DEFL.(LL) = L/999 (0.13"), ALLOWABLE DEFL.(TL) = L/980 (0.97"), CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.77/1.00 (E-F-1), SC=0.62/1.00 (K-M-2), WB=0.83/1.00 (G-J-1), SSI=0.35/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES: PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLJ) (PLJ) MAX MIN MAX MIN MAX MIN

MT20 650 371 1747 788 1987 1878

PLATE PLACEMENT TOL = 0.250 inches

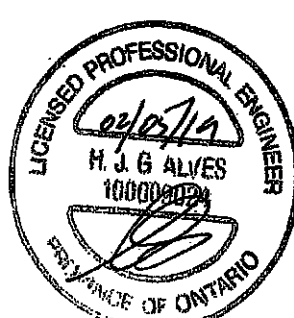
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)

JSI METAL= 0.54 (C) (INPUT = 1.00)

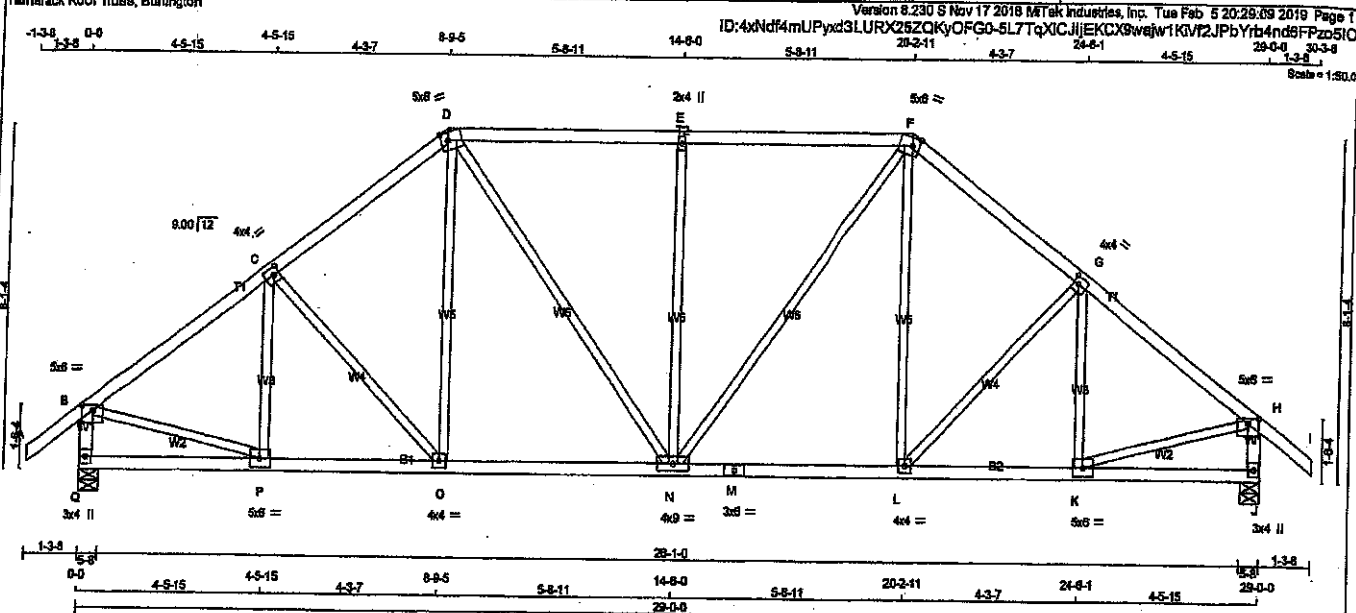
RECEIVED JUN 13 2019

TOWN OF CALEDON BUILDING SECTION FILE NO.



DWG NO. TAM 1790 2711 STRUCTURAL COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. Preston 3	DRWG NO. T5
Tamarack Roof Truss, Burlington		Version 8.230 8 Nov 17 2018 MTak Industries, Inc. Tue Feb 5 20:29:09 2019 Page 1			
ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-5L7TqXICJHJEKX9wajw1KvT2JPbYrb4nd6FPz0510					



TOTAL WEIGHT = 135 lb
[M/F]

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
Q	2180	0	2180	0	5-8	5-8
J	2180	0	2180	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
Q	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF
DL = 8.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2015
- CSA 086-14
- TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVVW-m	MT20	5.0	8.0	Edge	2.00
E	TMVVW-m	MT20	2.0	4.0		
F	TTVVW-m	MT20	5.0	8.0	Edge	2.00
G	TMVVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVVW-p	MT20	5.0	8.0	1.50	3.00
J	BMV1-p	MT20	3.0	4.0		
K	BMVVW-t	MT20	5.0	8.0		
L	BMVVW-t	MT20	4.0	4.0		
N	BMVVW-t	MT20	3.0	6.0		
O	BMVVW-t	MT20	4.0	8.0		
P	BMVVW-t	MT20	4.0	4.0		
Q	BMV1-p	MT20	5.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CSF (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LBS)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	P-C	-288 / 23	0.10 (1)		
B-C	-2180 / 0	-102.1 -102.1	0.31 (1)	4.35	C-D	-218 / 0	0.16 (1)		
C-D	-2048 / 0	-102.1 -102.1	0.30 (1)	4.46	O-D	0 / 375	0.06 (2)		
D-E	-1956 / 0	-102.1 -102.1	0.46 (1)	4.28	D-N	0 / 581	0.13 (1)		
E-F	-1956 / 0	-102.1 -102.1	0.48 (1)	4.29	N-E	-712 / 0	0.90 (1)		
F-G	-2048 / 0	-102.1 -102.1	0.30 (1)	4.46	N-F	0 / 581	0.13 (1)		
G-H	-2180 / 0	-102.1 -102.1	0.31 (1)	4.35	L-F	0 / 375	0.06 (2)		
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	L-G	-218 / 0	0.16 (1)		
Q-B	-2105 / 0	0.0 0.0	0.22 (1)	5.84	K-G	-288 / 23	0.10 (1)		
J-H	-2105 / 0	0.0 0.0	0.22 (1)	5.84	B-P	0 / 1820	0.41 (1)		
					K-H	0 / 1820	0.41 (1)		
Q-P	0 / 0	-38.5 -38.5	0.13 (3)	10.00					
P-O	0 / 1759	-38.5 -38.5	0.39 (1)	10.00					
O-N	0 / 1613	-38.5 -38.5	0.41 (2)	10.00					
N-M	0 / 1613	-38.5 -38.5	0.41 (2)	10.00					
M-L	0 / 1613	-38.5 -38.5	0.41 (2)	10.00					
L-K	0 / 1759	-38.5 -38.5	0.39 (1)	10.00					
K-J	0 / 0	-38.5 -38.5	0.13 (3)	10.00					

ALLOWABLE DEFL.(LL) = L/860 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/860 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSF: TC=0.48/1.00 (D-E-1), 8C=0.41/1.00 (L-N-2),
WB=0.80/1.00 (E-N-1), 5S=0.25/1.00 (O-E-1)

DOL LUMBER=1.00 NAIL=1.00 L8 BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788	1987

PLATE PLACEMENT TOL = 0.250 inches

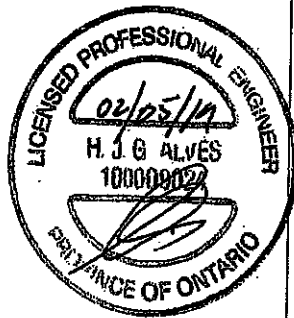
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (K) (INPUT = 0.80)
JSI METAL = 0.51 (M) (INPUT = 1.00)

RECEIVED

JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO



DWG NO. TAM 17902712
STRUCTURAL
COMPONENT ONLY

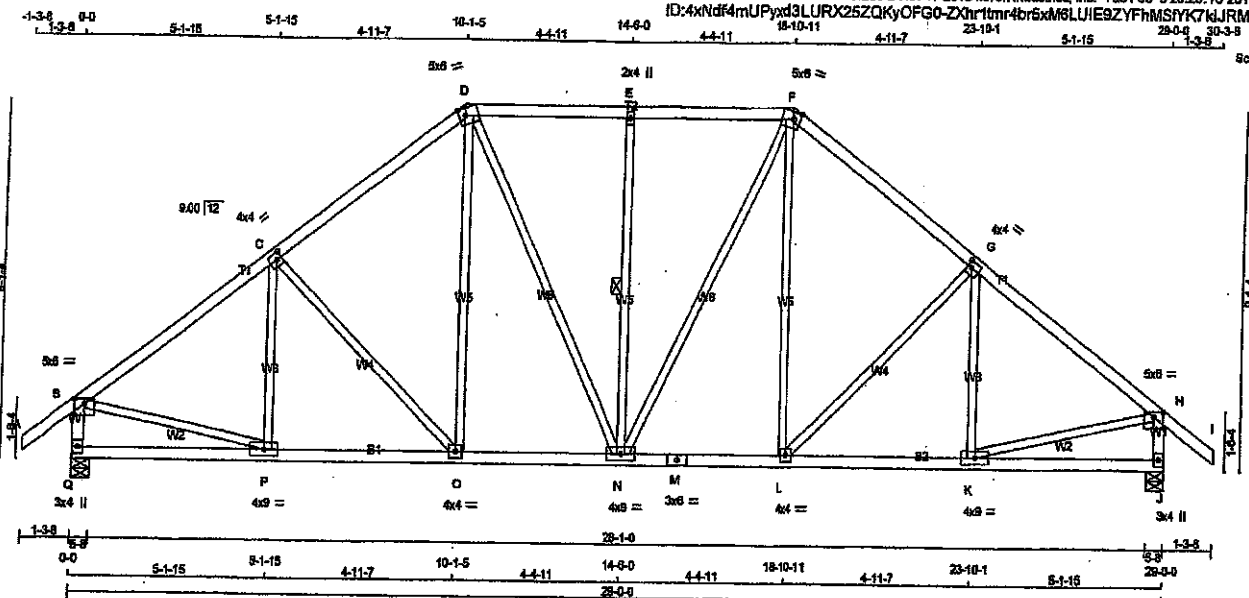
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401156	T6	13	1	Preston 3	T6

Tamarack Roof Truss, Burlington

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ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-ZXhr1mr4br6xM8LUIE9ZYFhMSYK7WJRMhrzo5IN

Scale = 1:54.0



TOTAL WEIGHT = 13 X 142 = 1842 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMAV+p	MT20	5.0	8.0	1.50 3.00
C	TMAV+4	MT20	4.0	4.0	2.00 1.50
D	TTMAV+m	MT20	5.0	8.0	Edge 2.00
E	TMAV+m	MT20	2.0	4.0	
F	TTMAV+m	MT20	5.0	8.0	Edge 2.00
G	TMAV+4	MT20	4.0	4.0	2.00 1.50
H	TMAV+p	MT20	5.0	8.0	1.50 3.00
J	BMV+1+p	MT20	3.0	4.0	
K	BMVW+4	MT20	4.0	4.0	
L	BMVW+4	MT20	4.0	4.0	
M	BS+4	MT20	3.0	8.0	
N	BMVW+4	MT20	4.0	4.0	
O	BMVW+4	MT20	4.0	4.0	
P	BMVW+4	MT20	4.0	4.0	
Q	BMV+1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
Q	2180	0	2180	0	5-8	5-8
J	2180	0	2180	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	
Q	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF EN.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	LENGTH PR-TO UNBRAC
FR-TO				PR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	P-C	-194 / 98	0.08 (1)	10.00
B-C	-2188 / 0	-102.1	-102.1 0.42 (1)	C-D	-364 / 0	0.36 (1)	4.21
C-D	-1954 / 0	-102.1	-102.1 0.39 (1)	D-E	0 / 444	0.10 (2)	4.44
D-E	-1708 / 0	-102.1	-102.1 0.28 (1)	E-F	0 / 389	0.09 (1)	4.82
E-F	-1708 / 0	-102.1	-102.1 0.28 (1)	F-G	-538 / 0	0.29 (1)	4.82
F-G	-1954 / 0	-102.1	-102.1 0.39 (1)	G-H	0 / 389	0.09 (1)	4.44
G-H	-2188 / 0	-102.1	-102.1 0.42 (1)	H-I	0 / 444	0.10 (2)	4.21
H-I	0 / 42	-102.1	-102.1 0.14 (1)	I-J	-364 / 0	0.36 (1)	10.00
Q-B	-2088 / 0	0.0	0.0 0.22 (1)	J-K	-194 / 98	0.08 (1)	5.84
J-H	-2088 / 0	0.0	0.0 -0.22 (1)	K-P	0 / 1834	0.41 (1)	5.84
Q-P	0 / 0	-38.5	-38.5 0.19 (3)	P-K	0 / 1834	0.41 (1)	10.00
P-O	0 / 1787	-38.5	-38.5 0.42 (2)				10.00
O-N	0 / 1535	-38.5	-38.5 0.34 (1)				10.00
N-M	0 / 1535	-38.5	-38.5 0.34 (1)				10.00
M-L	0 / 1535	-38.5	-38.5 0.34 (1)				10.00
L-K	0 / 1787	-38.5	-38.5 0.42 (2)				10.00
K-J	0 / 0	-38.5	-38.5 0.19 (3)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 92.5 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 088-08, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.12")

CSI: TC=0.42/1.00 (G-H-1), BC=0.42/1.00 (K-L-2), WB=0.41/1.00 (H-K-1), SS=0.22/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

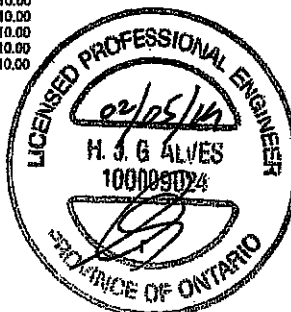
NAIL VALUES:

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1857 788

PLATE PLACEMENT TOL. = 0.250 inches

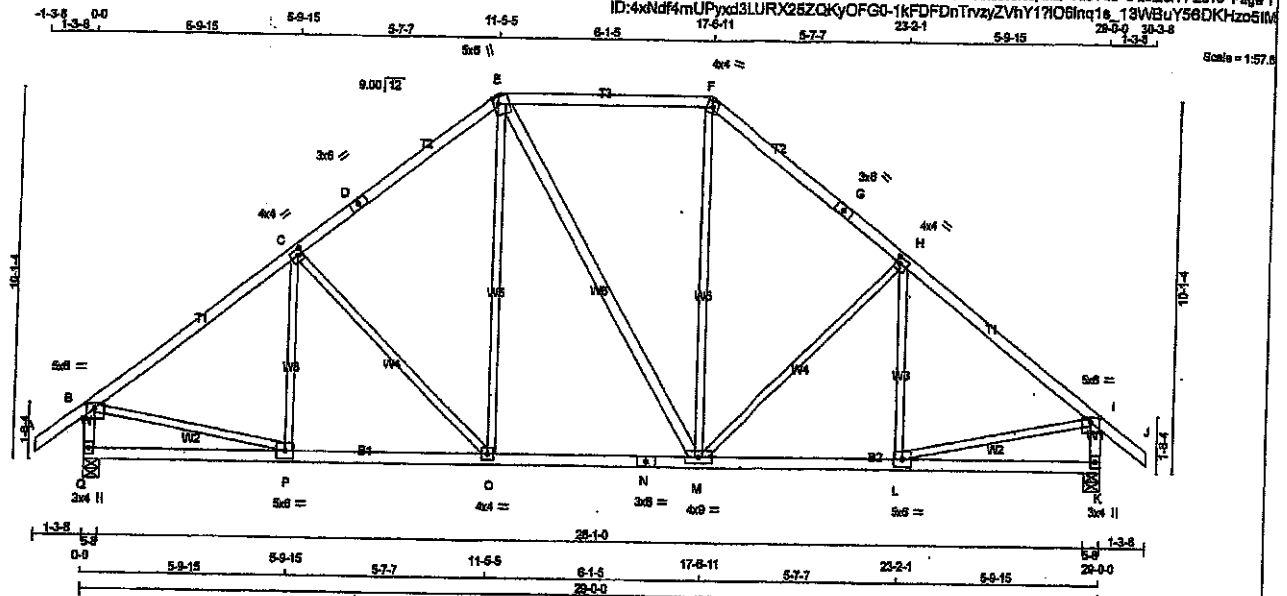
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (P) (INPUT = 0.90)
JSI METAL = 0.50 (M) (INPUT = 1.00)



DWG NO. TAM 17902713
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME 17	QUANTITY 1	PLY 1	JOB DESC. Preston 3	DRWG NO. T7
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	6.0	1.50	3.00
C TMWV-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	6.0		
E TTWV-m	MT20	5.0	6.0	2.25	1.50
F TTW-m	MT20	4.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMWV-t	MT20	4.0	4.0	2.00	1.50
I TMWV-p	MT20	5.0	6.0	1.50	3.00
K BMV1-p	MT20	3.0	4.0		
L BMWV-t	MT20	5.0	6.0		
M BMWVW-t	MT20	4.0	9.0		
N BS-t	MT20	3.0	6.0		
O BMWV-t	MT20	4.0	4.0		
P BMWV-t	MT20	5.0	6.0		
Q BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
Q	2180	0	0	5-8
K	2180	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1620	922 / 0	305 / 0	0 / 0	0 / 0	384 / 0	0 / 0
K	1620	922 / 0	305 / 0	0 / 0	0 / 0	384 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (CSI (LC))	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (CSI (LC))	
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	10.00	P-C	-123 / 150	0.07 (1)
B-C	-2208 / 0	-102.1	-102.1 0.55 (1)	4.04	C-O	-492 / 0	0.65 (1)
C-D	-1855 / 0	-102.1	-102.1 0.50 (1)	4.38	O-E	0 / 571	0.13 (2)
D-E	-1855 / 0	-102.1	-102.1 0.50 (1)	4.38	E-M	0 / 1	0.00 (2)
E-F	-1454 / 0	-102.1	-102.1 0.52 (1)	4.75	M-F	0 / 573	0.13 (2)
F-G	-1858 / 0	-102.1	-102.1 0.50 (1)	4.38	M-H	-481 / 0	0.65 (1)
G-H	-1858 / 0	-102.1	-102.1 0.50 (1)	4.38	L-H	-125 / 149	0.07 (1)
H-I	-2208 / 0	-102.1	-102.1 0.55 (1)	4.04	B-P	0 / 1838	0.41 (1)
I-J	0 / 42	-102.1	-102.1 0.14 (1)	10.00	L-I	0 / 1838	0.41 (1)
Q-B	-2088 / 0	0.0	0.0 0.22 (1)	5.88			
K-I	-2087 / 0	0.0	0.0 0.22 (1)	5.88			
Q-P	0 / 0	-38.5	-38.5 0.24 (3)	10.00			
P-O	0 / 1801	-38.5	-38.5 0.46 (2)	10.00			
O-N	0 / 1453	-38.5	-38.5 0.39 (2)	10.00			
N-M	0 / 1453	-38.5	-38.5 0.39 (2)	10.00			
M-L	0 / 1800	-38.5	-38.5 0.46 (2)	10.00			
L-K	0 / 0	-38.5	-38.5 0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015
- CSA S86-14
- TRC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.97)
CALCULATED VERT. DEFL. (LL) = L/999 (0.08)
ALLOWABLE DEFL. (TL) = L/360 (0.97)
CALCULATED VERT. DEFL. (TL) = L/999 (0.14)

CSI: TC=0.55/1.00 (B-C-1), BC=0.48/1.00 (O-P-2), WB=0.85/1.00 (C-O-1), SS=0.24/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

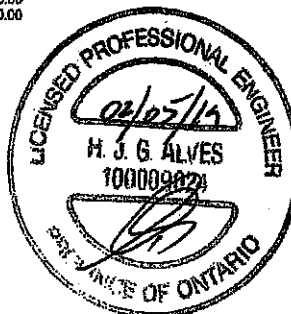
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (FLI) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (F) (INPUT = 0.80)
JSI METAL=0.46 (N) (INPUT = 1.00)



DRWG NO. TAM 1702714
STRUCTURAL
COMPONENT ONLY

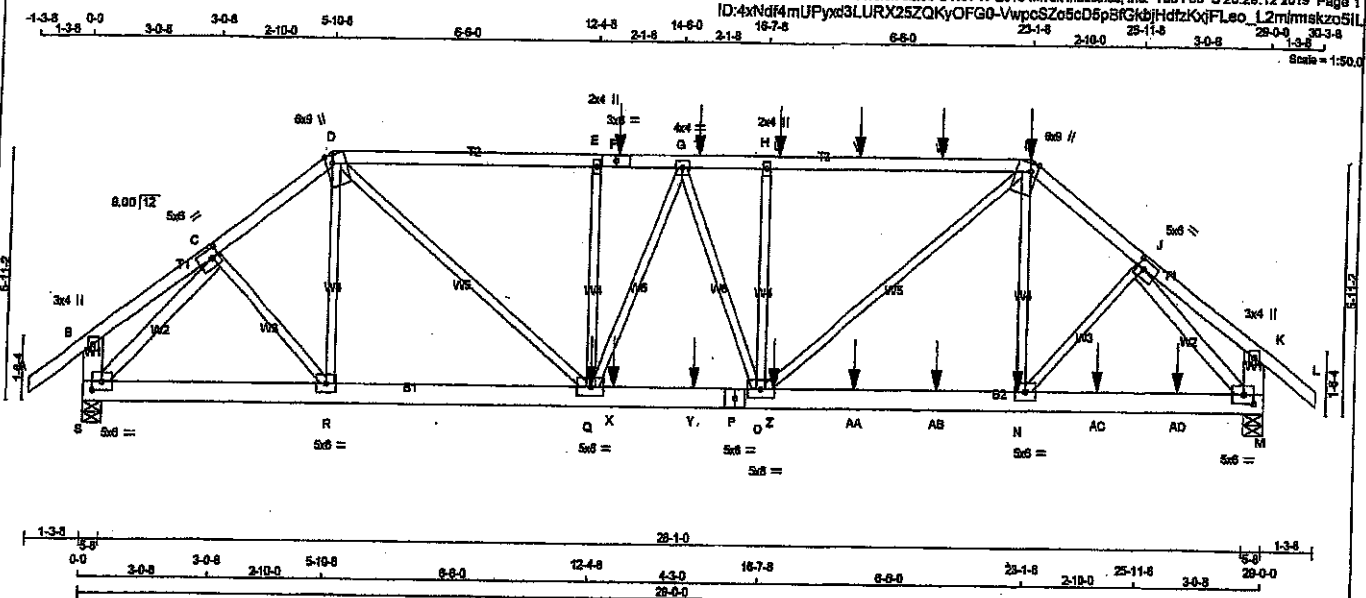
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401156	T8	1	2	Preston 3	T8

Tamarack Roof Truss, Burlington

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ID:4xNd4mUPyxd3LURX2S2QKYOFG0-VwpcSzo5cD5p8fGkbjHdfzKvjFLao_L2mimaskzo5IL

Scale = 1:50.0



TOTAL WEIGHT = 2 X 151 = 302 lb

MEMBER	SIZE	DRY	NO. 2	SPF
A - D	2x4	DRY	No. 2	SPF
D - F	2x4	DRY	No. 2	SPF
F - I	2x4	DRY	No. 2	SPF
I - L	2x4	DRY	No. 2	SPF
L - K	2x6	DRY	No. 2	SPF
M - N	2x6	DRY	No. 2	SPF
S - P	2x6	DRY	No. 2	SPF
P - M	2x6	DRY	No. 2	SPF

MEMBER	SIZE	DRY	NO. 2	SPF
ALL WEBS	2x3	DRY	No. 2	SPF
EXCEPT				
S - C	2x4	DRY	No. 2	SPF
J - M	2x4	DRY	No. 2	SPF

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
--------------	----------------------	------------

TOP CHORDS : (0.122"x3") SPIRAL NAILS

MEMBER	SIZE	DRY	NO. 2	SPF
A-D	1	12		
D-F	1	12		
F-I	1	12		
I-L	1	12		
L-K	2	12		
M-N	2	12		

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

MEMBER	SIZE	DRY	NO. 2	SPF
S-P	2	12		
P-M	2	12		

WEBS : (0.122"x3") SPIRAL NAILS

MEMBER	SIZE	DRY	NO. 2	SPF
N-I	1	6		
E-Q	1	6		
2x8	1	6		
2x4	1	6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	8.0	4.0	
C	TMVW+P	MT20	5.0	8.0	2.50 2.00
D	TMVW+m	MT20	8.0	9.0	Edge 2.00
E	TMVW+P	MT20	2.0	4.0	
F	TS-1	MT20	3.0	8.0	
G	TMVW+P	MT20	4.0	4.0	
H	TMVW+P	MT20	2.0	4.0	
I	TTVW+m	MT20	6.0	9.0	Edge 2.00
J	TMVW+P	MT20	5.0	8.0	2.50 2.00
K	TMV+P	MT20	3.0	4.0	
M	BMVW+P	MT20	5.0	8.0	2.50 2.75
N	BMVW+P	MT20	5.0	8.0	
O	BMVW+P	MT20	5.0	8.0	
P	BS-1	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	0	5-8
M	4521	0	0	5-8
S	3690	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERALIVE WIND DEAD SOIL
S	3346 1947 / 0 595 / 0 0 / 0 0 / 0 804 / 0 0 / 0
S	2743 1562 / 0 514 / 0 0 / 0 0 / 0 667 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, S

BRACING

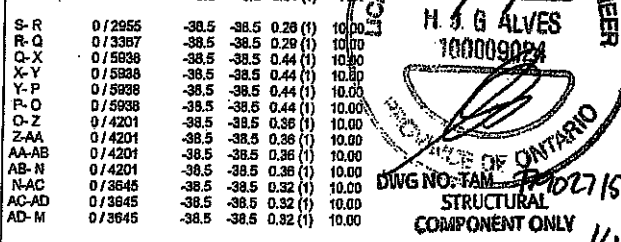
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX. UNBRAC	MEMB.	FORCE
FR-TO				FR-TO	
A-B	0 / 42	-102.1 -102.1 0.09 (1)	10.00	R-D	-137 / 106 0.04 (1)
B-C	0 / 15	-102.1 -102.1 0.06 (1)	10.00	D-Q	0 / 3500 0.43 (1)
C-D	-4238 / 0	-102.1 -102.1 0.12 (1)	4.55	Q-I	0 / 2289 0.28 (1)
D-E	-6031 / 0	-102.1 -102.1 0.59 (1)	3.54	N-I	-120 / 302 0.04 (3)
E-F	-6031 / 0	-102.1 -102.1 0.42 (1)	3.54	Q-E	-823 / 0 0.21 (1)
F-G	-6031 / 0	-102.1 -102.1 0.42 (1)	3.54	Q-H	-1137 / 0 0.30 (1)
G-T	-5844 / 0	-102.1 -102.1 0.59 (1)	3.25	Q-G	0 / 281 0.03 (1)
T-H	-5844 / 0	-102.1 -102.1 0.59 (1)	3.25	Q-O	-85 / 16 0.03 (3)
H-U	-5843 / 0	-102.1 -102.1 0.81 (1)	3.25	S-C	-4481 / 0 0.48 (1)
U-V	-5843 / 0	-102.1 -102.1 0.81 (1)	3.25	C-R	0 / 808 0.03 (1)
V-W	-5843 / 0	-102.1 -102.1 0.81 (1)	3.25	N-I	0 / 2289 0.28 (1)
W-I	-5843 / 0	-102.1 -102.1 0.81 (1)	3.25		
I-J	-5283 / 0	-102.1 -102.1 0.16 (1)	4.11		
J-K	-1 / 13	-102.1 -102.1 0.08 (1)	10.00		
K-L	0 / 42	-102.1 -102.1 0.08 (1)	10.00		
S-B	-286 / 0	0.0 0.0 0.01 (1)	7.81		
M-K	-288 / 0	0.0 0.0 0.01 (1)	7.81		
S-R	0 / 2955	-38.5 -38.5 0.28 (1)	10.00		
R-Q	0 / 3367	-38.5 -38.5 0.29 (1)	10.00		
Q-X	0 / 5838	-38.5 -38.5 0.44 (1)	10.00		
X-Y	0 / 5838	-38.5 -38.5 0.44 (1)	10.00		
Y-P	0 / 5838	-38.5 -38.5 0.44 (1)	10.00		
P-O	0 / 5838	-38.5 -38.5 0.44 (1)	10.00		
O-Z	0 / 4201	-38.5 -38.5 0.36 (1)	10.00		
Z-AA	0 / 4201	-38.5 -38.5 0.36 (1)	10.00		
AA-AB	0 / 4201	-38.5 -38.5 0.36 (1)	10.00		
AB-N	0 / 4201	-38.5 -38.5 0.36 (1)	10.00		
N-AC	0 / 3645	-38.5 -38.5 0.32 (1)	10.00		
AC-AD	0 / 3645	-38.5 -38.5 0.32 (1)	10.00		
AD-M	0 / 3645	-38.5 -38.5 0.32 (1)	10.00		



FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
F	12-11-4	-123	-123		FRONT	VERT	TOTAL		
I	23-1-8	-75	-84		FRONT	VERT	DEAD		
I	23-1-8	-798	-798		FRONT	VERT	TOTAL		
N	23-1-8	-428	-428		FRONT	VERT	SNOW		
I	22-11-4	-57	-73		FRONT	VERT	TOTAL		
Q	12-4-8	-1481	-1481		FRONT	VERT	TOTAL		

DESIGN CRITERIA

SPECIAL LOADS ANALYSIS --- GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.8 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

NON STANDARD GIRDER --- ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH: - PART 9 OF NBC 2015 - CSA S88-14 - TPC 2014

55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/860 (0.97) CALCULATED VERT. DEFL. (LL) = L/989 (0.11) ALLOWABLE DEFL. (TL) = L/860 (0.97) CALCULATED VERT. DEFL. (TL) = L/989 (0.19)

CSI: TC=0.81/1.00 (H=1), BC=0.44/1.00 (O-Q:1), WB=0.60/1.00 (L=1), SS=0.29/1.00 (H=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.88 (C) (INPUT = 0.90) JSI METAL=0.82 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401156	T8	1	2	Preston 3	
Tamarack Roof Truss, Burlington		TRUSS DESC.		T8	

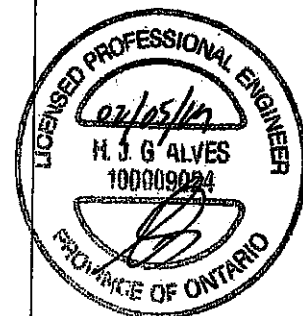
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:29:12 2019 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWWW-1	MT20	5.0	8.0		
R	BMWWW-1	MT20	5.0	8.0		
S	BMWWW-1	MT20	5.0	8.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	MAX*	FACE	DIR.	TYPE	HEEL	CONN.
T	14-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
U	18-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
V	18-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
W	20-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
X	12-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
Y	14-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
Z	18-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AA	18-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AB	20-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AC	24-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—
AD	28-11-4	-57	-73	—	FRONT	VERT	TOTAL	—	—



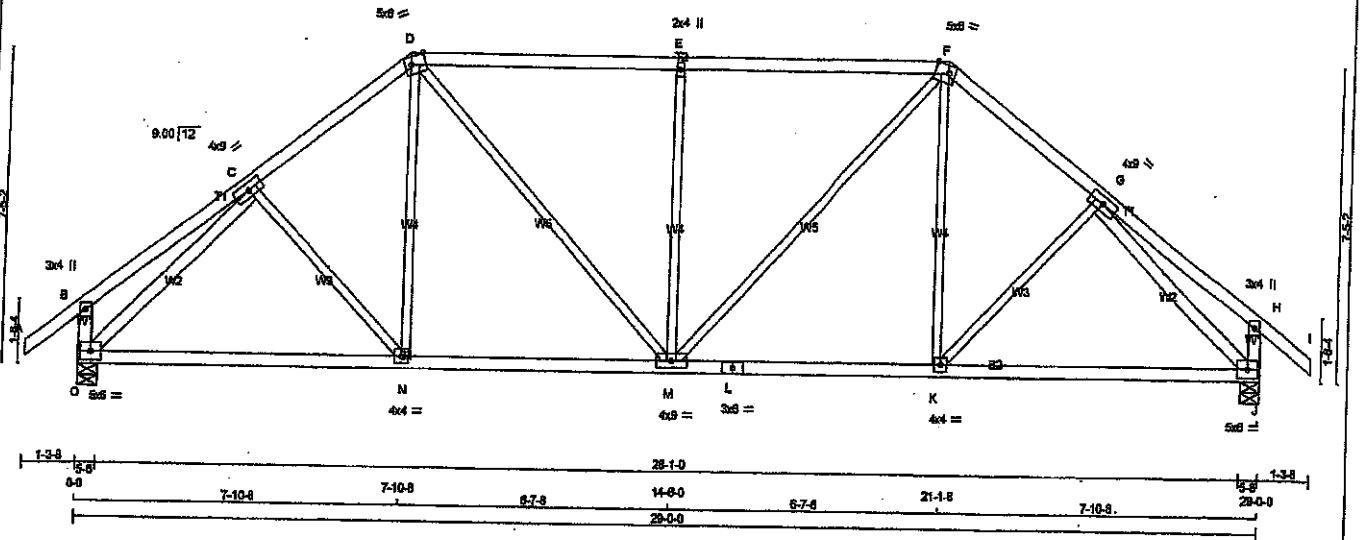
DWG NO. TAM 719021K
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME 200444-401156	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. Preston 3	TRUSS DESC.	DRWG NO. T9
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:28:12 2019 Page 1
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Scale = 1/8" = 1'-0"



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - C	2x4	DRY	No.2
G - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+1	MT20	4.0	9.0	
D	TTWV+m	MT20	5.0	8.0	Edge 4.25
E	TMVW+1	MT20	2.0	4.0	
F	TTWV+m	MT20	5.0	8.0	Edge 4.25
G	TMVW+1	MT20	4.0	9.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW+1	MT20	5.0	8.0	
K	BMVW+1	MT20	4.0	4.0	
L	BS-1	MT20	2.0	8.0	
M	BMVW+1	MT20	4.0	9.0	
N	BMVW+1	MT20	4.0	4.0	
O	BMVW+1	MT20	5.0	8.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
O	2180	0	2180	0	5-8
J	2180	0	2180	0	5-8

UNFACTORED REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
O	1620	922 / 0	305 / 0	0 / 0	0 / 0
J	1620	922 / 0	305 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	C-N	0 / 116	10.00	0.03 (3)
B-C	0 / 27	-102.1	-102.1 0.24 (1)	N-D	0 / 379	10.00	0.09 (2)
C-D	-2123 / 0	-102.1	-102.1 0.25 (1)	D-M	0 / 704	4.46	0.16 (1)
D-E	-2158 / 0	-102.1	-102.1 0.86 (1)	M-E	-829 / 0	3.83	0.82 (1)
E-F	-2158 / 0	-102.1	-102.1 0.86 (1)	M-F	0 / 704	3.83	0.16 (1)
F-G	-2123 / 0	-102.1	-102.1 0.25 (1)	K-F	0 / 379	4.46	0.08 (2)
G-H	0 / 27	-102.1	-102.1 0.24 (1)	K-G	0 / 116	10.00	0.03 (3)
H-I	0 / 42	-102.1	-102.1 0.14 (1)	O-C	-2425 / 0	10.00	0.83 (1)
O-B	-298 / 0	0.0	0.0 0.03 (1)	G-J	-2425 / 0	7.81	0.83 (1)
J-H	-298 / 0	0.0	0.0 0.03 (1)				
O-N	0 / 1688	-38.5	-38.5 0.64 (2)				
N-M	0 / 1678	-38.5	-38.5 0.65 (2)				
M-L	0 / 1678	-38.5	-38.5 0.85 (2)				
L-K	0 / 1678	-38.5	-38.5 0.65 (2)				
K-J	0 / 1688	-38.5	-38.5 0.64 (2)				

TOTAL WEIGHT = 130 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015
- CSA 088-14
- TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.97")
CALCULATED VERT. DEFL. (LL) = 1/998 (0.16")
ALLOWABLE DEFL. (TL) = 1/360 (0.97")
CALCULATED VERT. DEFL. (TL) = 1/998 (0.27")

CSI: TC=0.88/1.00 (D-E:1), BC=0.85/1.00 (M-N:2), WS=0.83/1.00 (O-C:1), SS=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

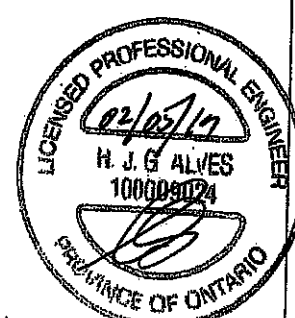
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.80 (C) (INPUT= 0.80)
SI METAL= 0.55 (L) (INPUT= 1.00)



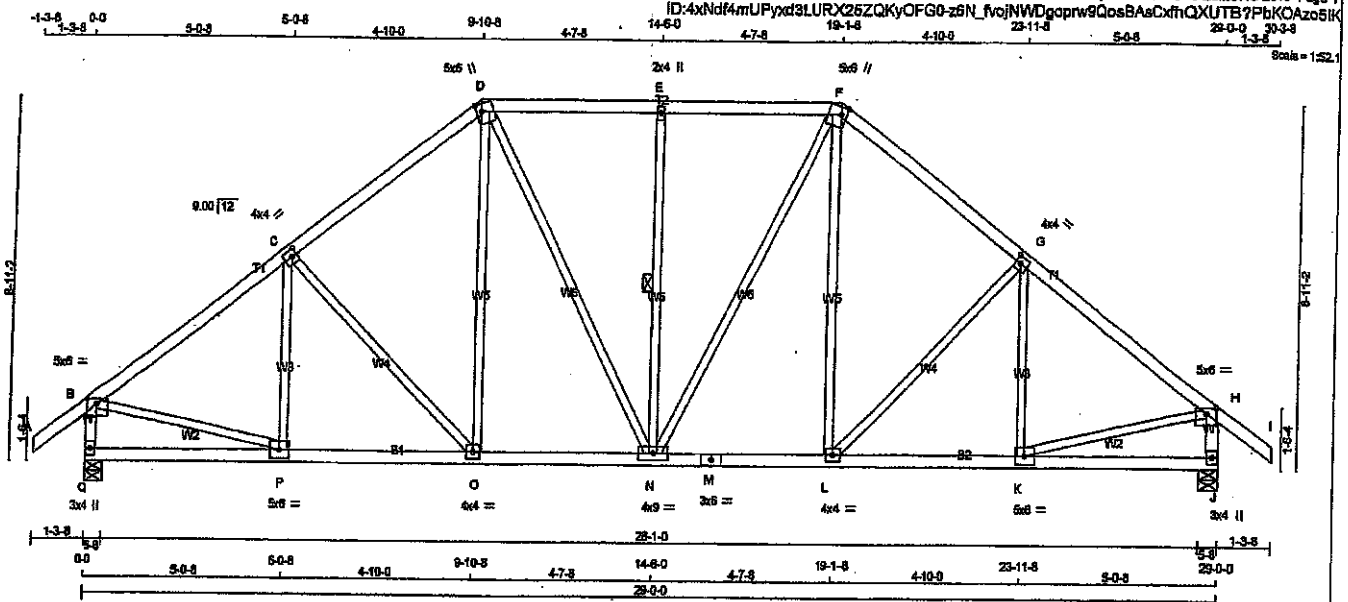
DRWG NO. TAM 19902716
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401156	T10	1	1	Preston 3	T10

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 140 lb

LUMBERS	N.L.G.A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY; SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.50	3.00
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TTWV+m	MT20	5.0	6.0	2.25	1.50
E TMVW-w	MT20	2.0	4.0		
F TTWV+m	MT20	5.0	6.0	2.25	1.50
G TMVW-t	MT20	4.0	4.0	2.00	1.50
H TMVW-p	MT20	5.0	6.0	1.50	3.00
J BMV1+p	MT20	3.0	4.0		
K BMVW-t	MT20	5.0	6.0		
L BMVW-t	MT20	4.0	4.0		
M BS-t	MT20	3.0	6.0		
N BMVW-t	MT20	4.0	9.0		
O BMVW-t	MT20	4.0	4.0		
P BMVW-t	MT20	5.0	6.0		
Q BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
Q	2180	0	2180	0
J	2180	0	2180	0

UNFACTORED REACTIONS							
	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1620	822 / 0	306 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1620	822 / 0	306 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX
MEMB.	FORCE (LBS)	FROM	TO	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	FROM	TO	CS1 (LC)	UNBRAC
FR-TO						FR-TO					
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	P-C	-208 / 85	0.09 (1)			
B-C	-2194 / 0	-102.1	-102.1	0.40 (1)	4.24	C-D	-338 / 0	0.32 (1)			
C-D	-1971 / 0	-102.1	-102.1	0.38 (1)	4.44	D-E	0 / 432	0.10 (2)			
D-E	-1748 / 0	-102.1	-102.1	0.31 (1)	4.74	E-F	0 / 420	0.09 (1)			
E-F	-1748 / 0	-102.1	-102.1	0.31 (1)	4.74	F-G	-389 / 0	0.29 (1)			
F-G	-1971 / 0	-102.1	-102.1	0.38 (1)	4.44	G-H	0 / 420	0.09 (1)			
G-H	-1971 / 0	-102.1	-102.1	0.40 (1)	4.24	H-I	0 / 431	0.10 (2)			
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	I-J	-338 / 0	0.32 (1)			
I-J	-2099 / 0	0.0	0.0	0.22 (1)	5.84	J-K	-208 / 85	0.09 (1)			
J-K	-2099 / 0	0.0	0.0	0.22 (1)	5.84	K-L	0 / 1833	0.41 (1)			
K-L	0 / 0	-38.5	-38.5	0.18 (3)	10.00	L-M	0 / 1833	0.41 (1)			
L-M	0 / 1783	-38.5	-38.5	0.40 (2)	10.00	M-N	0 / 1850	-38.5	-38.5	0.34 (1)	10.00
M-N	0 / 1850	-38.5	-38.5	0.34 (1)	10.00	N-O	0 / 1550	-38.5	-38.5	0.34 (1)	10.00
N-O	0 / 1550	-38.5	-38.5	0.34 (1)	10.00	O-P	0 / 1783	-38.5	-38.5	0.40 (2)	10.00
O-P	0 / 1783	-38.5	-38.5	0.40 (2)	10.00	P-Q	0 / 0	-38.5	-38.5	0.18 (3)	10.00
P-Q	0 / 0	-38.5	-38.5	0.18 (3)	10.00	Q-R	0 / 0	-38.5	-38.5	0.18 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015
- CSA 088-14
- TRC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/989 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/989 (0.12")

CS1: TC=0.40/1.00 (B-C-1), SC=0.40/1.00 (D-F-2), WB=0.41/1.00 (B-P-1), SS=0.23/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

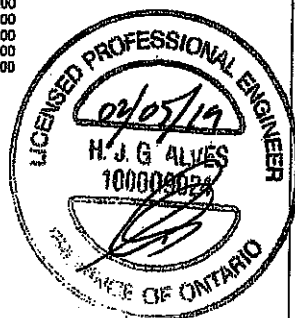
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.60 (M) (INPUT = 1.00)



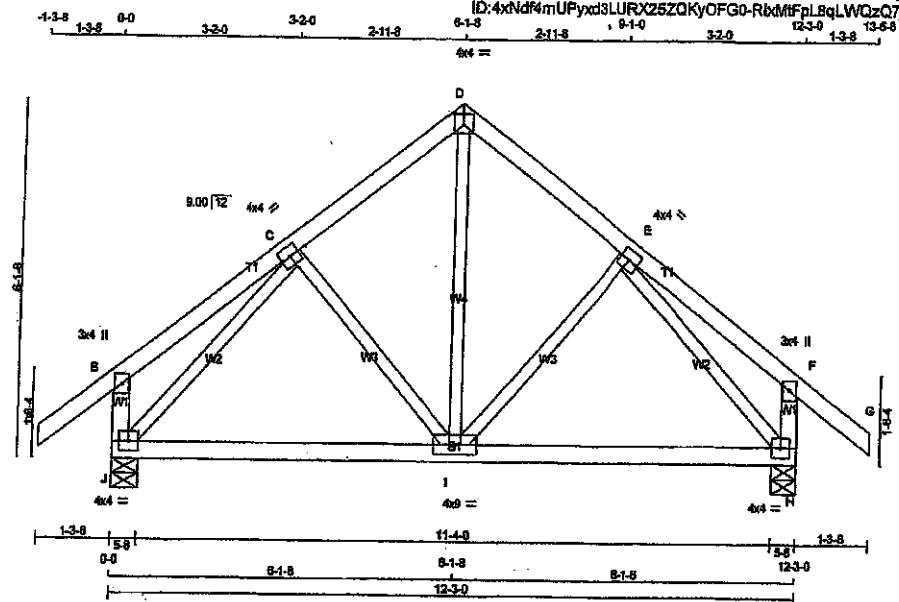
DRWG NO. TAM 1902717
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. Preston 3	DRWG NO. T11
TRUSS DESC.					

Farmersack Roof Truss, Burlington

Version 8.230 8 Nov 17 2018 MTak Industries, Inc. Tue Feb 5 20:29:14 2019 Page 1
ID:4xNd4mUPyxd3LURX25ZQKyOFG0-RbdMfPL8qLWQzQ7J8J5kOPRg313GzaKE3Ktwcz05LJ

Scale = 1:36.3



TOTAL WEIGHT = 58 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-1	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	4.0		
I	BMVW-1	MT20	4.0	9.0		
J	BMVW-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
J	1002	0	1002	0	0	5-8	5-8	5-8	5-8
H	1002	0	1002	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
J	740	436 / 0	129 / 0	0 / 0	0 / 0	178 / 0	0 / 0
H	740	436 / 0	129 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. UNBRACED LENGTH (FT)	MAX. MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	I-D	0 / 481	0.11 (1)
B-C	0 / 21	-102.1 -102.1	0.15 (1)	10.00	I-E	-127 / 29	0.04 (1)
C-D	-827 / 0	-102.1 -102.1	0.11 (1)	6.25	C-I	-127 / 29	0.04 (1)
D-E	-827 / 0	-102.1 -102.1	0.11 (1)	6.25	J-C	-864 / 0	0.29 (1)
E-F	0 / 21	-102.1 -102.1	0.15 (1)	10.00	E-H	-864 / 0	0.29 (1)
F-G	0 / 42	-102.1 -102.1	0.14 (1)	10.00			
J-B	-262 / 0	0.0	0.03 (1)	7.81			
H-F	-262 / 0	0.0	0.03 (1)	7.81			
J-I	0 / 569	-38.5 -38.5	0.38 (2)	10.00			
I-H	0 / 569	-38.5 -38.5	0.38 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.15/1.00 (E-F-1), BC=0.38/1.00 (H-I-2),
WB=0.29/1.00 (E-H-1), SS=0.16/1.00 (H-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

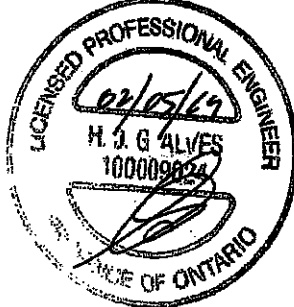
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1887 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.80)
JSI METAL= 0.28 (C) (INPUT = 1.00)



DWG NO. TAM T902718
STRUCTURAL
COMPONENT ONLY

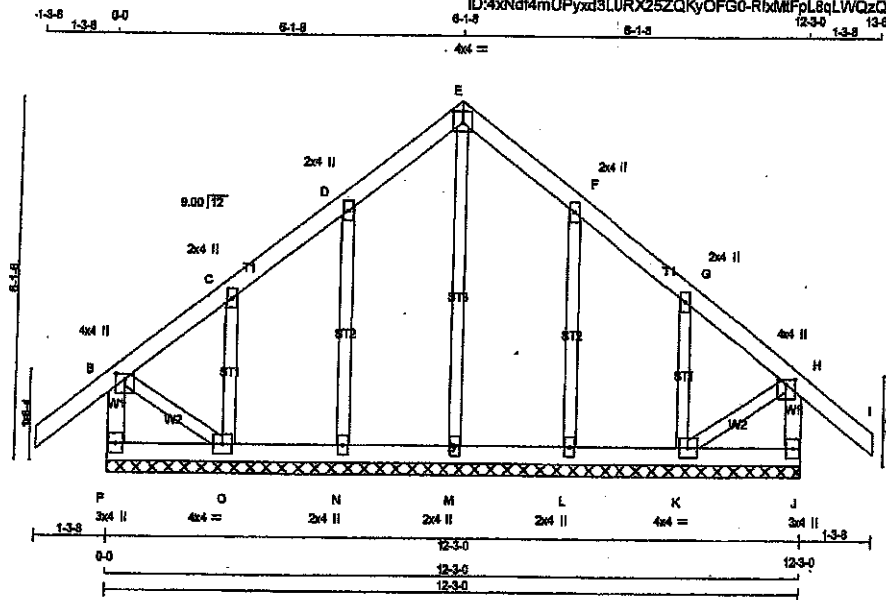
JOB NAME 200444-401156	TRUSS NAME T11G	QUANTITY 1	PLY 1	JOB DESC. Preston 3	TRUSS DESC.	DRWG NO. T11G
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTek Industries, Inc. Tue Feb 5 20:28:14 2019 Page 1

ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-RbMfPpL8qLWQzQ778J5kOPRn36SG0skE3Ktwcz051j

Scale = 1/32"



TOTAL WEIGHT = 57 lb

LUMBER

N.L.G.A. RULES

CHORDS

P - B 2x4 DRY No.2

A - E 2x4 DRY No.2

E - I 2x4 DRY No.2

J - H 2x4 DRY No.2

P - J 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2

DRY, SEASONED LUMBER.

GABLE STUDS SPACED AT 24-0 OC.

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.

MAX. FACTORED

FORCE

(LBS)

FR-TO

VERT. LOAD

LC1

MAX

MAX.

CS1 (LC)

UNBRAC

LENGTH

FR-TO

MEMB.

MAX. FACTORED

FORCE

(LBS)

CS1 (LC)

MEMB.

MAX. FACTORED

FORCE

(LBS)

CS1 (LC)

MEMB.

MAX. FACTORED

FORCE

(LBS)

CS1 (LC)

MEMB.

MAX. FACTORED

FORCE

(LBS)

CS1 (LC)

MEMB.

MAX. FACTORED

FORCE

(LBS)

CS1 (LC)

MEMB.

MAX. FACTORED

FORCE

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CS1 (LC)

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MAX. FACTORED

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CS1 (LC)

MEMB.

MAX. FACTORED

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CS1 (LC)

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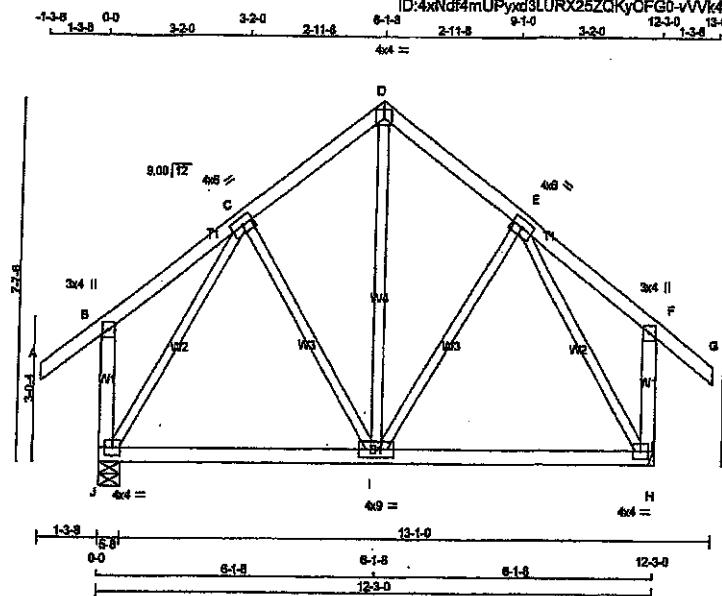
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401156	T12-Cond1	2	1	Preston 3	T12

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MitTek Industries, Inc. Tue Feb 5 20:28:15 2019 Page 1

ID:4xNd4mUPyxd3LURX25ZQKyOFG0-VVvK4bzv8UN277JGrtqKGbydLTNe?OfUTJ4Q73zo5I

Scale = 1/4" = 1'



TOTAL WEIGHT = 2 X 65 = 130 lb

[M/F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESOR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	HORIZ	UPLIFT
J 1002 0	1002 0	0	5-8
H 1002 0	1002 0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOL
J 740	438 / 0 129 / 0 0 / 0 0 / 0 178 / 0 0 / 0
H 740	438 / 0 129 / 0 0 / 0 0 / 0 178 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS) MAX CSI (LC)
FR-TO		FROM TO		FR-TO	
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00	I-D	0 / 346 0.08 (2)
B-C	0 / 21	-102.1 -102.1 0.15 (1)	10.00	I-E	-28 / 68 0.02 (3)
C-D	-503 / 0	-102.1 -102.1 0.12 (1)	6.25	C-I	-28 / 68 0.02 (3)
D-E	-503 / 0	-102.1 -102.1 0.12 (1)	6.25	J-C	-762 / 0 0.43 (1)
E-F	0 / 21	-102.1 -102.1 0.15 (1)	10.00	E-H	-762 / 0 0.43 (1)
F-G	0 / 42	-102.1 -102.1 0.14 (1)	10.00		
J-B	-284 / 0	0.0 0.0 0.04 (1)	7.81		
H-F	-284 / 0	0.0 0.0 0.04 (1)	7.81		
J-I	0 / 400	-38.5 -38.5 0.36 (3)	10.00		
I-H	0 / 400	-38.5 -38.5 0.36 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 28.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015
- CSA 086-14
- TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")
CALCULATED VERT. DEFL. (LL) = L/899 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.41")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.15/1.00 (E-F-1), BC=0.38/1.00 (H-I-3),
WB=0.43/1.00 (C-I-1), SS=0.16/1.00 (H-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

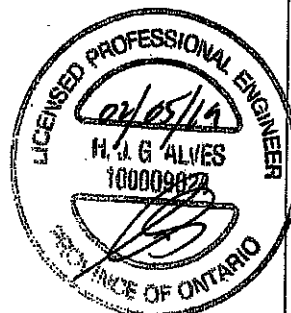
PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 1.00)

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



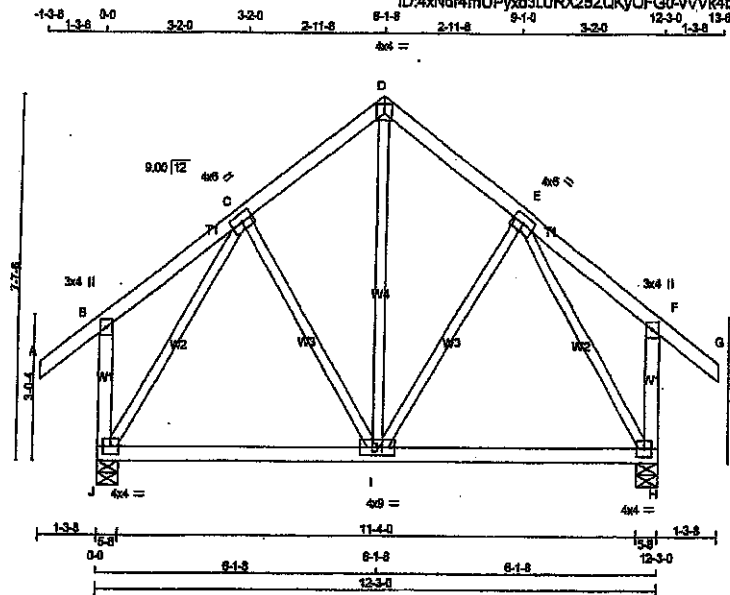
DRWG NO. TAM 17902220
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401156	T12-Cond2	2	1	Preston 3	T12
Tamarack Roof Truss, Burlington					

Version 8.230 8 Nov 17 2016 Mitek Industries, Inc. Tue Feb 5 20:29:15 2018 Page 1

ID:4xNd4mUPyxd3LURX26ZQKyOFG0-vVvK4bzv8UN27?JGrqKGbycLTNo?OfUTj4QT3zo5fj

Scale = 1:64.3



TOTAL WEIGHT = 2 X 65 = 130 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW+L	MT20	4.0	6.0		
D TTW+p	MT20	4.0	4.0	2.25	2.00
E TMVW+L	MT20	4.0	6.0		
F TMV+p	MT20	3.0	4.0		
H BMVW+L	MT20	4.0	4.0		
I BMVW+L	MT20	4.0	9.0		
J BMVW+L	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UPLIFT
JT	1002	0	1002	0
J	1002	0	1002	0
H	1002	0	1002	0

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED
J	740
H	740

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0/42	-102.1 -102.1 0.14 (1)	I-D	0/345
B-C	0/21	-102.1 -102.1 0.15 (1)	I-E	-28/68
C-D	-503/0	-102.1 -102.1 0.12 (1)	C-I	-28/68
D-E	-503/0	-102.1 -102.1 0.12 (1)	J-C	-782/0
E-F	0/21	-102.1 -102.1 0.15 (1)	E-H	-782/0
F-G	0/42	-102.1 -102.1 0.14 (1)		
J-B	-284/0	0.0 0.0 0.04 (1)		
H-F	-284/0	0.0 0.0 0.04 (1)		
J-I	0/400	-38.5 -38.5 0.36 (3)		
I-H	0/400	-38.5 -38.5 0.36 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015
- CSA 086-14
- TPIC 2014

(65% OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.41")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.04")
ALLOWABLE DEFL.(TL) = 1/360 (0.41")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.07")

CSI: TC=0.16/1.00 (E-F:1), BC=0.38/1.00 (H-I:3), WB=0.43/1.00 (C-I:1), SS=0.18/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

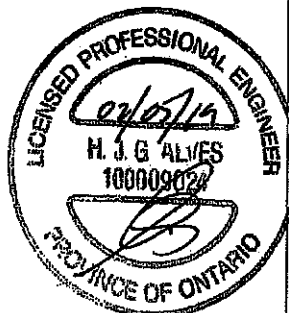
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (D) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 1.00)



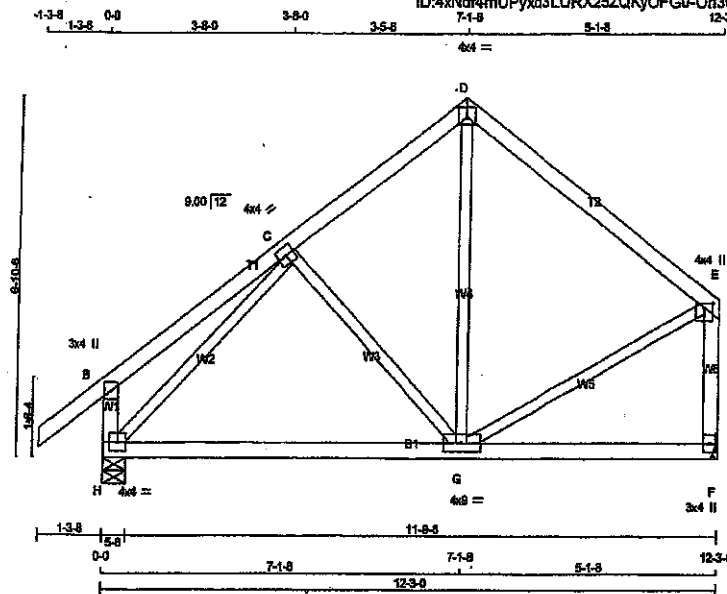
DWG NO. TAM T202721
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME T13	QUANTITY 2	PLY 1	JOB DESC. Preston 3	TRUSS DESC.	DRWG NO. T13
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Tamarack Roof Truss, Burlington

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ID:4xNd4mUPyxd3LURX25ZQKyOFG0-Qh36brogRcEHZVqZLZppUk?68ksxdhNp_7Vzo5IH



TOTAL WEIGHT = 2 X 55 = 111 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - E	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWV+1	MT20	4.0	9.0		
H	BMVW1+1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
F	881	0	881	0	0	MECHANICAL			
H	1002	0	1002	0	0	5-8	5-8		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	844	358 / 0	128 / 0	0 / 0	0 / 0	159 / 0	0 / 0
H	740	438 / 0	128 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	C-G	-208 / 6	0.10 (1)
B-C	0 / 25	-102.1 -102.1	0.21 (1)	10.00	G-D	0 / 278	0.08 (3)
C-D	-562 / 0	-102.1 -102.1	0.18 (1)	6.25	H-C	-832 / 0	0.37 (1)
D-E	-527 / 0	-102.1 -102.1	0.35 (1)	6.25	G-E	0 / 478	0.11 (1)
H-B	-279 / 0	0.0	0.03 (1)	7.81			
F-E	-799 / 0	0.0	0.03 (1)	7.81			
H-G	0 / 583	-38.5 -38.5	0.40 (2)	10.00			
G-F	0 / 0	-38.5 -38.5	0.35 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/G

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/989 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/948 (0.16")

CSI: TO=0.35/1.00 (D-E:1), BC=0.40/1.00 (G-H:2),
WB=0.37/1.00 (C-H:1), SS=0.17/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

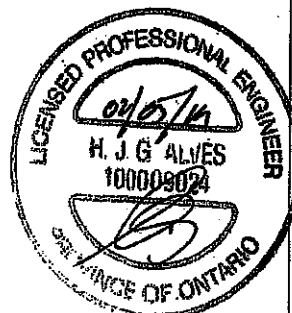
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 680 371 1747 789 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.99)
JSI METAL= 0.27 (C) (INPUT = 1.00)



DWG NO. TAM 1790 2722
STRUCTURAL
COMPONENT ONLY

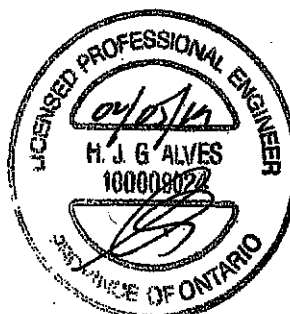
Version 8.230 S Nov 17 2018 Mtek Industries, Inc. Tue Feb 5 20:28:16 2019 Page 1
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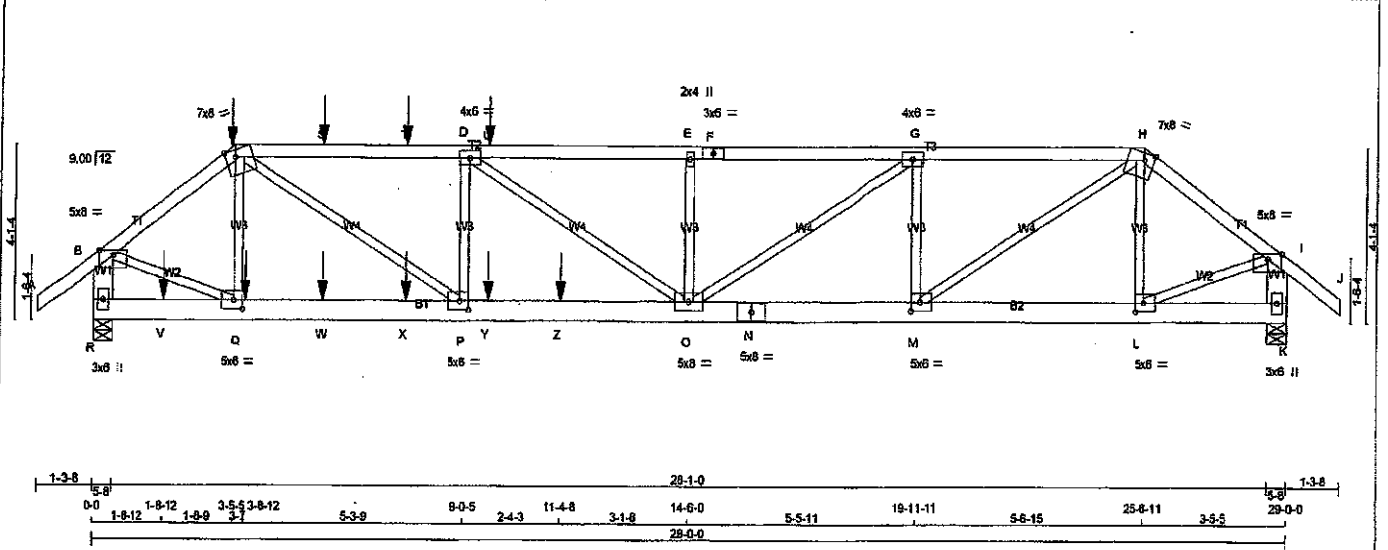


DWG NO. TAM 1750 2723
STRUCTURAL
COMPONENT ONLY

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE CURRENT BUILDING CODE



JOB NAME 401584	TRUSS NAME T100	QUANTITY 1	PLY 2	JOB DESC. Greenpark	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 M/Tek Industries, Inc. Wed Feb 27 16:23:17 2019 Page 1	
ID:4xNd4mUPyxd3LURX25ZQKyOFG0-G7VQynjvMcu?OLmVvMgAJk5dmSI_g6IM9e2UGCzquu				29-0-0 30-3-8	
1-3-8 0-0 3-5-5 3-5-5 5-8-15 9-0-5 5-5-11 14-6-0 5-5-11 19-11-11 5-8-15 25-4-11 3-5-5 1-3-8				Scale = 1/50.0	



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - F	2x4 DRY	No.2	SPF		
F - H	2x4 DRY	No.2	SPF		
H - J	2x4 DRY	No.2	SPF		
R - B	2x6 DRY	No.2	SPF		
K - I	2x6 DRY	No.2	SPF		
R - N	2x6 DRY	1850F 1.5E	SPF		
N - K	2x6 DRY	1850F 1.5E	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-F	12	SIDE(61.0)
F-H	12	TOP
H-J	12	TOP
R-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-N	12	SIDE(183.1)
N-K	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	8.0	Edge		
C TMWV-m	MT20	7.0	8.0	Edge	3.00	
D TMWV-l	MT20	4.0	8.0			
E TMWV-v	MT20	2.0	4.0			
F TS-l	MT20	3.0	8.0			
G TMWV-l	MT20	4.0	8.0			
H TMWV-m	MT20	7.0	8.0	Edge	3.00	
I TMWV-p	MT20	5.0	8.0	Edge		
K BMV1-p	MT20	3.0	6.0			
L, M, P, Q						
N BMWV-l	MT20	5.0	8.0	2.50	2.50	
BS-l	MT20	5.0	8.0			
O BMWV-l	MT20	5.0	8.0			
R BMV1-p	MT20	3.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	4446	0	4446	0
K	3118	0	3118	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	JT	COMBINED	SNOW
R	3277	1948 / 0	553 / 0
K	2308	1338 / 0	418 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	CS1 (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	CS1 (LC)
FR-TO							FR-TO				
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	Q-C	-1031 / 0	0.13 (1)			
B-C	-4717 / 0	-102.1	-102.1	0.21 (1)	4.27	C-P	0 / 4631	0.57 (1)			
C-S	-7591 / 0	-102.1	-102.1	0.71 (1)	3.02	P-D	-1080 / 0	0.13 (1)			
S-T	-7591 / 0	-102.1	-102.1	0.71 (1)	3.02	D-O	0 / 249	0.03 (1)			
T-D	-7591 / 0	-102.1	-102.1	0.71 (1)	3.02	O-E	-480 / 0	0.08 (1)			
D-U	-7797 / 0	-102.1	-102.1	0.72 (1)	2.96	E-G	0 / 2524	0.31 (1)			
U-E	-7797 / 0	-102.1	-102.1	0.72 (1)	2.96	G-S	0 / 2349	0.48 (1)			
E-F	-7797 / 0	-102.1	-102.1	0.61 (1)	3.11	S-T	0 / 3943	0.49 (1)			
F-G	-7797 / 0	-102.1	-102.1	0.61 (1)	3.11	T-U	0 / 3943	0.49 (1)			
G-H	-5703 / 0	-102.1	-102.1	0.45 (1)	3.71	U-V	0 / 3943	0.49 (1)			
H-I	-3262 / 0	-102.1	-102.1	0.16 (1)	5.01	V-W	0 / 3943	0.49 (1)			
I-J	0 / 42	-102.1	-102.1	0.08 (1)	10.00	W-X	0 / 3943	0.49 (1)			
J-K	-4336 / 0	0.0	0.0	0.16 (1)	6.88	X-Y	0 / 3943	0.49 (1)			
K-L	-3037 / 0	0.0	0.0	0.11 (1)	7.81	Y-Z	0 / 3943	0.49 (1)			
R-V	0 / 0	-38.5	-38.5	0.03 (1)	10.00	Z-A	0 / 3943	0.49 (1)			
V-Q	0 / 0	-38.5	-38.5	0.03 (1)	10.00	A-B	0 / 3943	0.49 (1)			
Q-W	0 / 3735	-38.5	-38.5	0.34 (1)	10.00	B-C	0 / 3943	0.49 (1)			
W-X	0 / 3735	-38.5	-38.5	0.34 (1)	10.00	C-D	0 / 3943	0.49 (1)			
X-P	0 / 3735	-38.5	-38.5	0.34 (1)	10.00	D-E	0 / 3943	0.49 (1)			
P-Y	0 / 7591	-38.5	-38.5	0.78 (1)	10.00	E-F	0 / 3943	0.49 (1)			
Y-Z	0 / 7591	-38.5	-38.5	0.78 (1)	10.00	F-G	0 / 3943	0.49 (1)			
Z-A	0 / 7591	-38.5	-38.5	0.78 (1)	10.00	G-H	0 / 3943	0.49 (1)			
O-N	0 / 5703	-38.5	-38.5	0.36 (1)	10.00	H-I	0 / 3943	0.49 (1)			
N-M	0 / 5703	-38.5	-38.5	0.36 (1)	10.00	I-J	0 / 3943	0.49 (1)			
M-L	0 / 2598	-38.5	-38.5	0.16 (1)	10.00	J-K	0 / 3943	0.49 (1)			
L-K	0 / 0	-38.5	-38.5	0.05 (2)	10.00	K-L	0 / 3943	0.49 (1)			

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-50	-56			FRONT	VERT	DEAD		
C	3-5-5	-460	-460			BACK	VERT	TOTAL		
C	3-5-5	-285	-285			FRONT	VERT	SNOW		
Q	3-8-12	-55	-70			BACK	VERT	TOTAL		
S	5-7-4	-123	-123			BACK	VERT	TOTAL		
T	7-7-4	-123	-123			BACK	VERT	TOTAL		
U	9-7-4	-123	-123			BACK	VERT	TOTAL		
V	1-8-12	-55	-70			BACK	VERT	TOTAL		
W	5-7-4	-55	-70			BACK	VERT	TOTAL		
X	7-7-4	-55	-70			BACK	VERT	TOTAL		

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.97")
CALCULATED VERT. DEFL. (LL) = L/600 (0.23")
ALLOWABLE DEFL. (TL) = L/360 (0.97")
CALCULATED VERT. DEFL. (TL) = L/821 (0.38")

CS1: TC=0.72/1.00 (D-E:1), BC=0.78/1.00 (O-P:1), WB=0.57/1.00 (C-P:1), SS=0.40/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
			788
			1987
			1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)

JSI METAL= 0.94 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Greenpark	DRWG NO.
401584	T100	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed Feb 27 16:23:17 2019 Page 2
ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-G?VQynivMcu?OLmVvMgAJk5dmSi q8IM9e2UGCzguu

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	9-7-4	-55	-70	—	BACK	VERT	TOTAL	—	—
Z	11-4-8	-1764	-1764	—	BACK	VERT	TOTAL	—	—



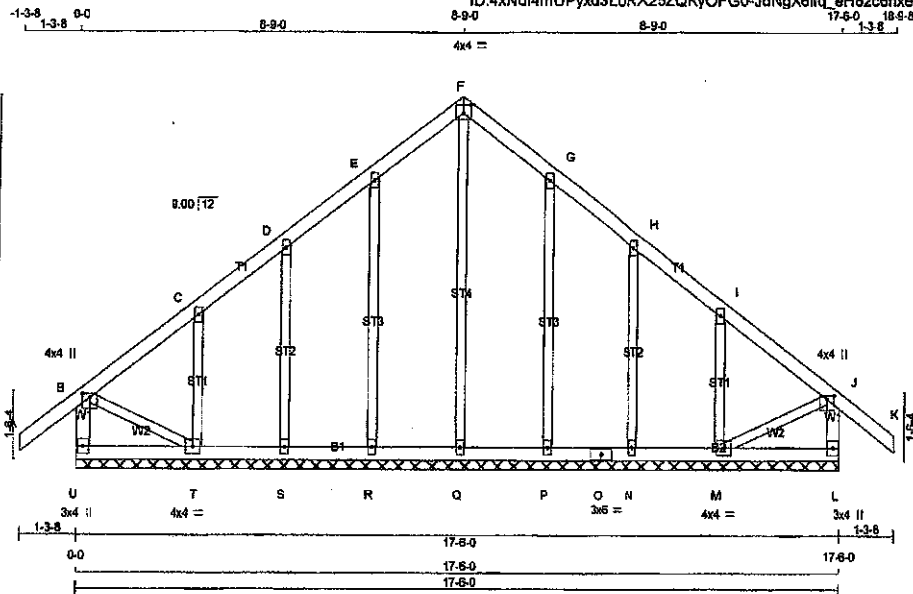
DRWG NO. TAM T1904516
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Greenpark	DRWG NO.
401584	G101	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-JdNgX8ifq_eH82c6nxeiDJOQLfCwMIW3hKZOBjzguqw



Scale = 1:47.5

TOTAL WEIGHT = 84 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
U - B	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
F - K	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
U - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I					
C TMVW+w	MT20	2.0	4.0		
F TTW-p	MT20	4.0	4.0	2.25	2.00
J TMVW+p	MT20	4.0	4.0	1.00	2.00
L BMV1+p	MT20	3.0	4.0		
M BMVW1-l	MT20	4.0	4.0		
N, P, Q, R, S					
N BMV1+w	MT20	2.0	4.0		
O BS-t	MT20	3.0	6.0		
T BMVW1-t	MT20	4.0	4.0		
U BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
U-B	-291/0		0.0	0.0	0.03 (1)	7.81	Q-F	-142/0	0.17 (1)
A-B	0/42		-102.1	-102.1	0.14 (1)	10.00	R-E	-237/0	0.17 (1)
B-C	-27/0		-102.1	-102.1	0.09 (1)	6.25	S-D	-172/0	0.07 (1)
C-D	-45/0		-102.1	-102.1	0.09 (1)	6.25	T-C	-278/0	0.06 (1)
D-E	-23/0		-102.1	-102.1	0.06 (1)	6.25	P-G	-237/0	0.17 (1)
E-F	-37/0		-102.1	-102.1	0.06 (1)	6.25	N-H	-172/0	0.07 (1)
F-G	-37/0		-102.1	-102.1	0.06 (1)	6.25	M-I	-278/0	0.06 (1)
G-H	-23/0		-102.1	-102.1	0.06 (1)	6.25	B-T	0/36	0.01 (1)
H-I	-45/0		-102.1	-102.1	0.09 (1)	6.25	M-J	0/36	0.01 (1)
I-J	-27/0		-102.1	-102.1	0.09 (1)	6.25			
J-K	0/42		-102.1	-102.1	0.14 (1)	10.00			
L-J	-291/0		0.0	0.0	0.03 (1)	7.81			
U-T	0/0		-38.5	-38.5	0.05 (3)	10.00			
T-S	0/26		-38.5	-38.5	0.05 (3)	10.00			
S-R	0/23		-38.5	-38.5	0.03 (2)	10.00			
R-Q	0/19		-38.5	-38.5	0.03 (2)	10.00			
Q-P	0/19		-38.5	-38.5	0.03 (2)	10.00			
P-O	0/23		-38.5	-38.5	0.03 (2)	10.00			
O-N	0/23		-38.5	-38.5	0.03 (2)	10.00			
N-M	0/26		-38.5	-38.5	0.05 (3)	10.00			
M-L	0/0		-38.5	-38.5	0.05 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	28.0	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.14/1.00 (J-K-1), BC=0.06/1.00 (S-T-3), WB=0.17/1.00 (F-Q-1), SS=0.10/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

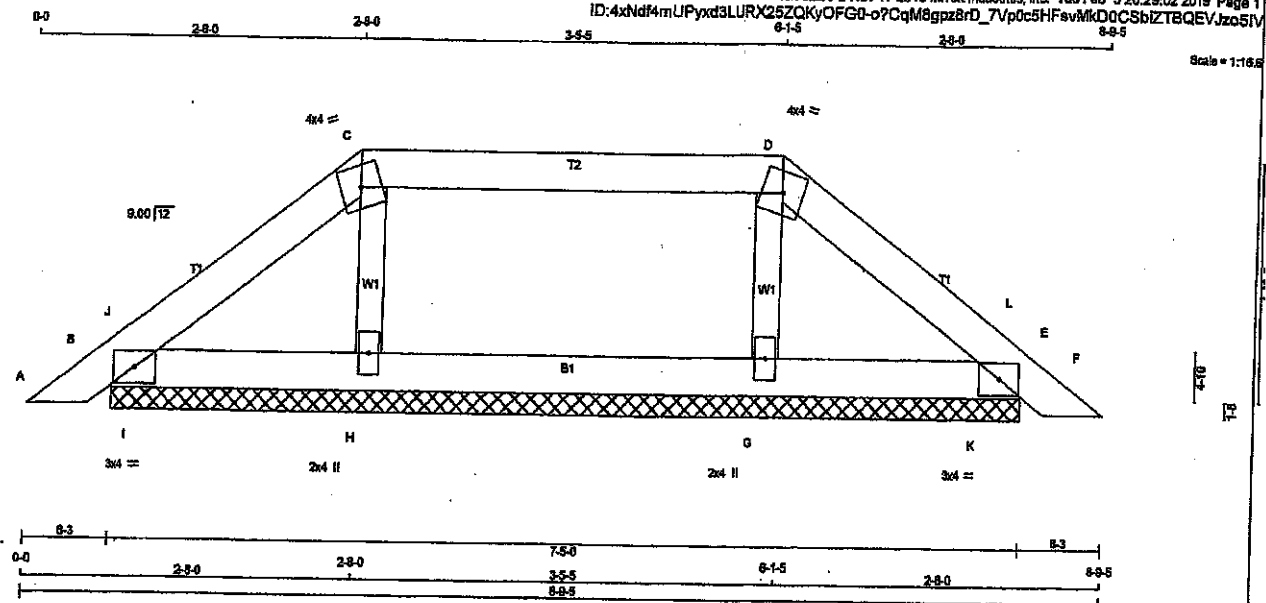
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (7) (INPUT = 0.90)
JSI METAL= 0.15 (1) (INPUT = 1.00)



DRWG NO. TAM 11904515
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME PB1	QUANTITY 1	PLY 1	JOB DESC. Preston 3	DRWG NO. PB1
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Tue Feb 5 20:28:02 2019 Page 1 ID:4xNd4mUPyxd3LURX25ZQKyOFG0-o7CqM8gpz8rD_7Vp0c5HFavMkD0CSbIZTBQEVJzo5IV			



TOTAL WEIGHT = 22 lb

LUMBER
N.L.G.A. RULES
CHORDS
A - C 2x4 DRY
C - D 2x4 DRY
D - E 2x4 DRY
B - F 2x4 DRY
ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

LUMBER
No.2
No.2
No.2
No.2
No.2

DESCR.
SPF
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT	232	0	232	0	7-5-0	7-5-0
B-E	232	0	232	0	7-5-0	7-5-0
H	349	0	349	0	7-5-0	7-5-0
G	349	0	349	0	7-5-0	7-5-0

UNFACTORED REACTIONS

	1ST LCASE MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	168	117/0	15/0	0/0	0/0	34/0	0/0
B	168	117/0	15/0	0/0	0/0	34/0	0/0
E	266	133/0	63/0	0/0	0/0	89/0	0/0
H	266	133/0	63/0	0/0	0/0	89/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
A-B	0/17	-102.1	H-C	-215/0
B-J	-64/0	-102.1	G-D	-215/0
J-C	-76/0	-102.1	I-J	-119/0
C-D	-44/0	-102.1	K-L	-119/0
D-L	-76/0	-102.1		
L-E	-64/0	-102.1		
E-F	0/17	-102.1		
B-I	0/58	-38.5		
I-H	0/58	-38.5		
H-G	0/44	-38.5		
G-K	0/58	-38.5		
K-E	0/58	-38.5		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 RLCIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(56 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI TO=0.21/1.00 (C-D:1), BC=0.06/1.00 (H-I:2), WB=0.03/1.00 (C-H:1), SSI=0.14/1.00 (G-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1856

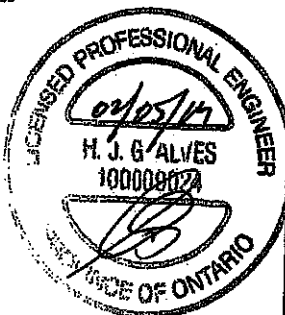
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.05 (H) (INPUT = 1.00)

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JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO



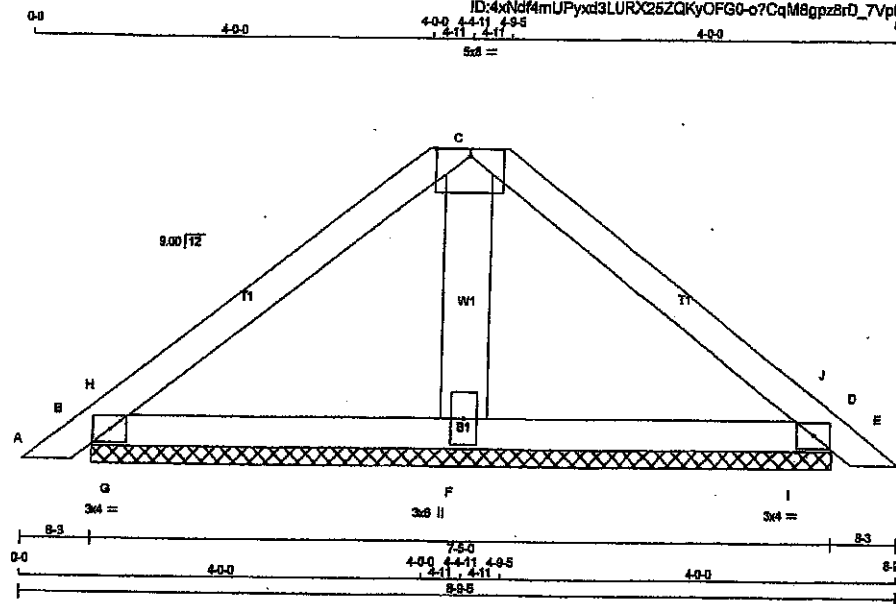
DWG NO. TAM 17902724
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME PB2	QUANTITY 1	PLY 1	JOB DESC. Preston 3	DRWG NO. PB2
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Tamarack Roof Truss, Burlington

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ID:4xNd4mUPyxd3LURX26ZQKyOFG0-o7CqM8gpzBrD_7Vp0c5HFsvNID_PSB7ZTBQEVJzo5IV



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 25 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

ALL WEBS 2x6 DRY No.2 SPF
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
B	373	0	373	0	0	7-5-0	7-5-0	7-5-0	7-5-0
D	373	0	373	0	0	7-5-0	7-5-0	7-5-0	7-5-0
F	418	0	418	0	0	7-5-0	7-5-0	7-5-0	7-5-0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	271	173 / 0	37 / 0	0 / 0	0 / 0	61 / 0	0 / 0
D	271	173 / 0	37 / 0	0 / 0	0 / 0	61 / 0	0 / 0
F	320	153 / 0	81 / 0	0 / 0	0 / 0	86 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LOCH (CSI (LC))	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	F-C	-158 / 0	0.01 (1)	
B-H	-78 / 0	-102.1	-102.1	0.07 (1)	6.25	G-H	-384 / 24	0.00 (1)	
H-C	-148 / 0	-102.1	-102.1	0.17 (1)	6.25	I-J	-384 / 24	0.00 (1)	
C-J	-148 / 0	-102.1	-102.1	0.17 (1)	6.25				
J-D	-78 / 0	-102.1	-102.1	0.07 (1)	8.25				
D-E	0 / 17	-102.1	-102.1	0.03 (1)	10.00				
B-G	0 / 108	-38.5	-38.5	0.16 (1)	10.00				
G-F	0 / 108	-38.5	-38.5	0.16 (1)	10.00				
F-I	0 / 108	-38.5	-38.5	0.16 (1)	10.00				
I-D	0 / 109	-38.5	-38.5	0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. GC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-H:1), BC=0.18/1.00 (F-I:1), WB=0.01/1.00 (C-F:1), GS=0.31/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 788	1987 1858

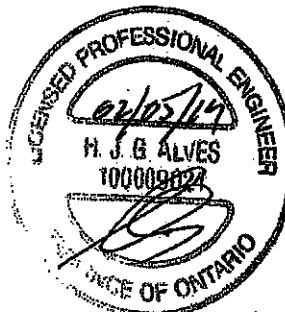
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.30 (D) (INPUT = 0.80)
JSI METAL=0.07 (D) (INPUT = 1.00)

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JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO

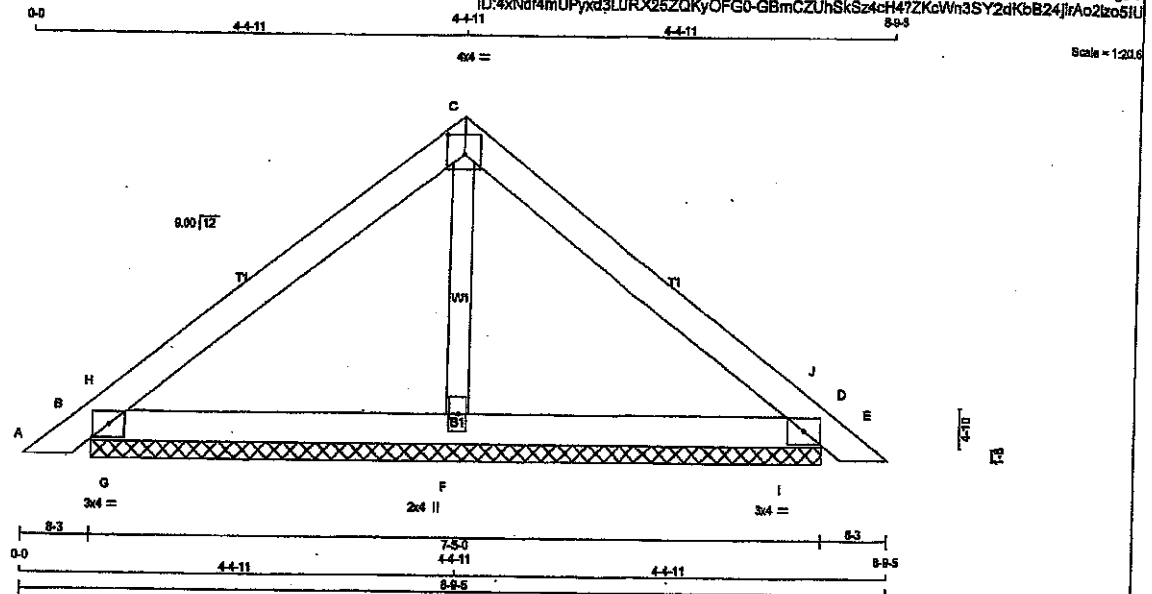


DRWG NO. TAM 17902725
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200444-401156	PB3	8	1	Preston 3	PB3

Tamarack Roof Truss, Burlington

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ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-GBmCZUhSkSz4cH47ZKcWn3SY2dKbB24JlrAo2bo5IU



TOTAL WEIGHT = 8 X 23 = 184 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW4-P	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+W	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	DOWN
B	385	0	0	7-5-0
D	385	0	0	7-5-0
F	394	0	0	7-5-0

UNFACTORED REACTIONS

	1ST LOSE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	279	181 / 0	37 / 0	0 / 0	0 / 0	62 / 0	0 / 0
D	279	181 / 0	37 / 0	0 / 0	0 / 0	62 / 0	0 / 0
F	304	138 / 0	82 / 0	0 / 0	0 / 0	83 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-102.1 -102.1	0.03 (1)	10.00	F-C	-151 / 0	0.02 (1)
B-H	-77 / 0	-102.1 -102.1	0.07 (1)	8.25	G-H	-382 / 25	0.00 (1)
H-C	-168 / 0	-102.1 -102.1	0.17 (1)	8.25	I-J	-382 / 25	0.00 (1)
C-J	-168 / 0	-102.1 -102.1	0.17 (1)	8.25			
J-D	-77 / 0	-102.1 -102.1	0.07 (1)	8.25			
D-E	0 / 17	-102.1 -102.1	0.03 (1)	10.00			
B-G	0 / 125	-38.5 -38.5	0.18 (1)	10.00			
G-F	0 / 125	-38.5 -38.5	0.18 (1)	10.00			
F-I	0 / 125	-38.5 -38.5	0.18 (1)	10.00			
I-D	0 / 125	-38.5 -38.5	0.18 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-J:1), BC=0.18/1.00 (F-I:1),
WB=0.02/1.00 (C-F:1), SS=0.31/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

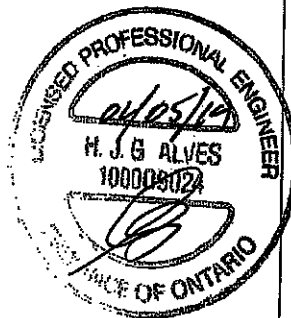
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1867
	788	1967	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



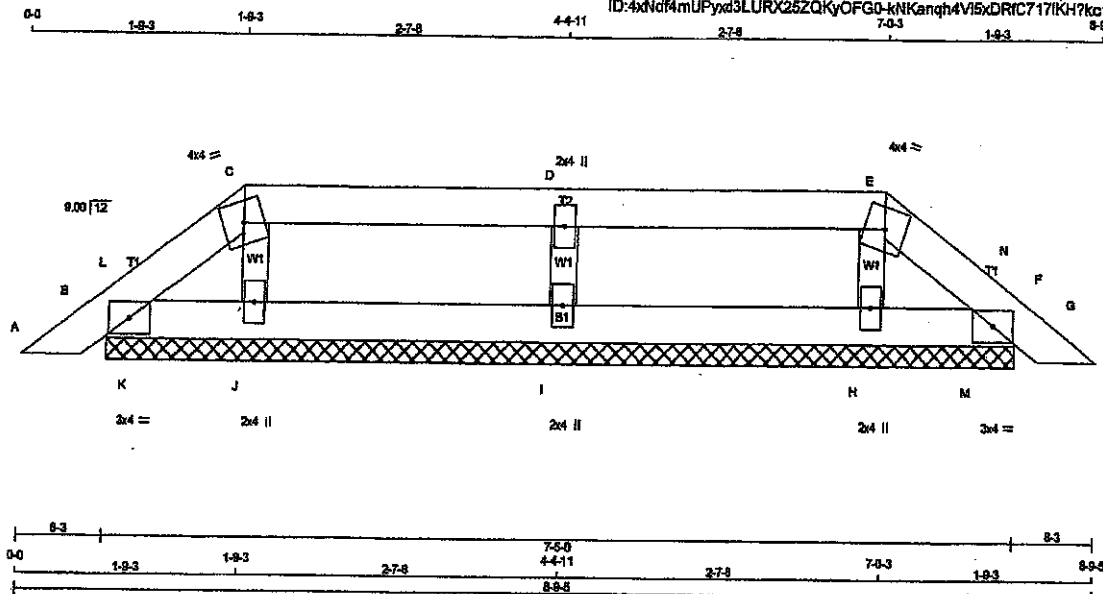
DWG NO. TAM 17202726
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
200444-401156	PB4	1	1	Preston 3		PB4

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:28:04 2019 Page 1
ID: 4xNdf4mUPyxd3LURX25ZQKyOFG0-kNKangh4V5xDRIC717IKH7ko1j_wVzaxVvLaBzo5IT

Scale = 1:16.5



TOTAL WEIGHT = 21 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TMW-w	MT20	2.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H, I, J	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
B	145	0	145	0	0	7-5-0	7-5-0		
F	145	0	145	0	0	7-5-0	7-5-0		
J	220	0	220	0	0	7-5-0	7-5-0		
H	220	0	220	0	0	7-5-0	7-5-0		
I	434	0	434	0	0	7-5-0	7-5-0		

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE		MAX/MIN COMPONENT REACTIONS		DEAD	SOIL
		SNOW	LIVE	PERM	LIVE		
B	102	77 / 0	5 / 0	0 / 0	0 / 0	19 / 0	0 / 0
F	102	77 / 0	5 / 0	0 / 0	0 / 0	19 / 0	0 / 0
J	189	79 / 0	44 / 0	0 / 0	0 / 0	48 / 0	0 / 0
H	189	79 / 0	44 / 0	0 / 0	0 / 0	48 / 0	0 / 0
I	321	188 / 0	56 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 17	-102.1	-102.1 0.03 (1)	J-C	-123 / 0
B-L	-32 / 0	-102.1	-102.1 0.00 (2)	H-E	-123 / 0
C-D	-32 / 0	-102.1	-102.1 0.01 (1)	I-D	-334 / 0
D-E	-10 / 0	-102.1	-102.1 0.12 (1)	K-L	-55 / 0
E-N	-10 / 0	-102.1	-102.1 0.12 (1)	M-N	-55 / 0
N-F	-32 / 0	-102.1	-102.1 0.01 (1)		
F-G	0 / 17	-102.1	-102.1 0.00 (2)		
		-102.1	-102.1 0.03 (1)		
B-K	0 / 25	-38.5	-38.5 0.02 (1)		
K-J	0 / 25	-38.5	-38.5 0.03 (2)		
J-I	0 / 10	-38.5	-38.5 0.04 (3)		
I-H	0 / 10	-38.5	-38.5 0.04 (3)		
H-M	0 / 25	-38.5	-38.5 0.03 (2)		
M-F	0 / 25	-38.5	-38.5 0.02 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.12/1.00 (C-D:1), BC=0.04/1.00 (J-I:3), WB=0.05/1.00 (D-I:1), SS1=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

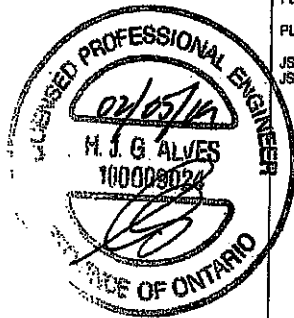
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1867 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (D) (INPUT = 0.80)
JSI METAL= 0.07 (D) (INPUT = 1.00)



DRWG NO. TAM 1790772
STRUCTURAL
COMPONENT ONLY

PB5

Scale = 1:17.1

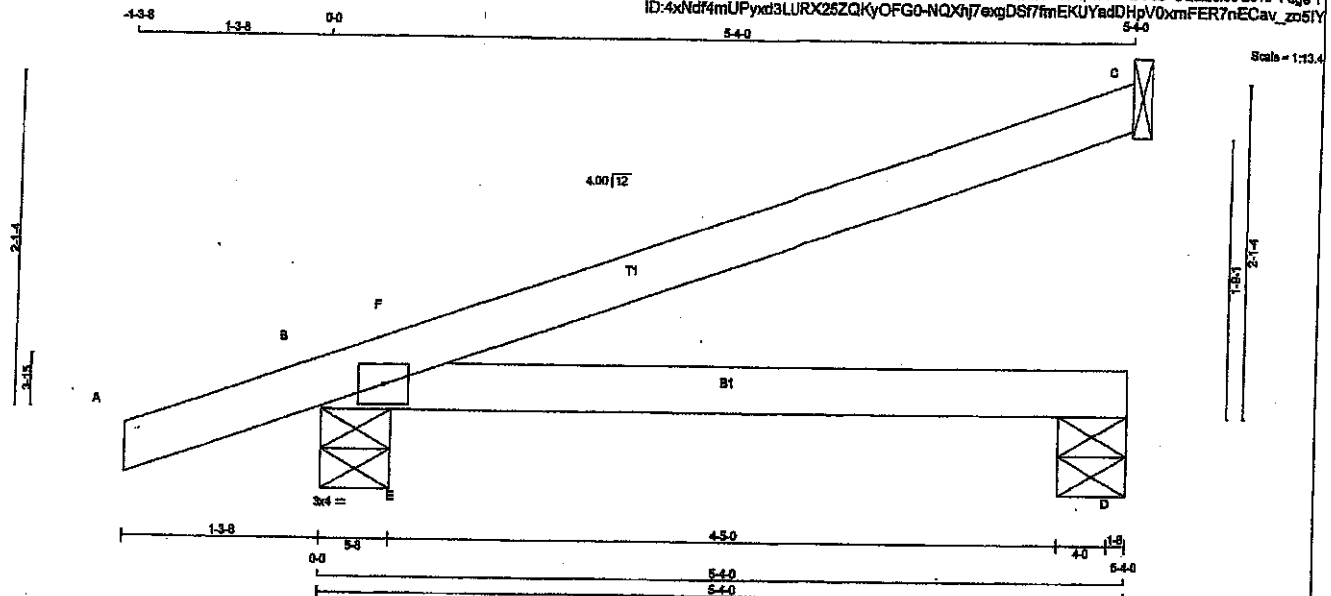
TOTAL WEIGHT = 25 #

DWG NO. TAM 17902728
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME J4	QUANTITY 3	PLY 1	JOB DESC. Preston 3	TRUSS DESC.	DRWG NO. J4
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Tamarack Roof Truss, Burlington

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:28:59 2019 Page 1
ID:4xNd4mUPyxd3LURX25ZQKyOFG0-NQXh7exgDS7fmEKUYadDhpV0xmFER7nECav_zo51Y



Scale = 1:13.4

TOTAL WEIGHT = 3 X 14 = 42 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

B - D 2x4 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

DESCR.

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
C	244	0	244	0
B	512	0	512	0
D	131	0	142	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
C	171	130 / 0	8 / 0	0 / 0	33 / 0	0 / 0
B	374	233 / 0	56 / 0	0 / 0	65 / 0	0 / 0
D	109	25 / 0	48 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)
FR-TO								
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	E-F	-244 / 117	0.00 (1)
B-F	-49 / 0	-102.1	-102.1	0.12 (3)	6.25			
F-C	0 / 8	-102.1	-102.1	0.40 (1)	10.00			
B-E	0 / 0	-38.5	-38.5	0.25 (1)	10.00			
E-D	0 / 0	-38.5	-38.5	0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015
- CSA 086-14
- TPO 2014

(56 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/500 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/523 (0.12")

CS: TC=0.40/1.00 (C-F-1), SC=0.30/1.00 (D-E-1), WB=0.00/1.00 (E-F-1), SS=0.25/1.00 (B-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 880 371 1747 788 1987 1879

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 6.0 Deg.

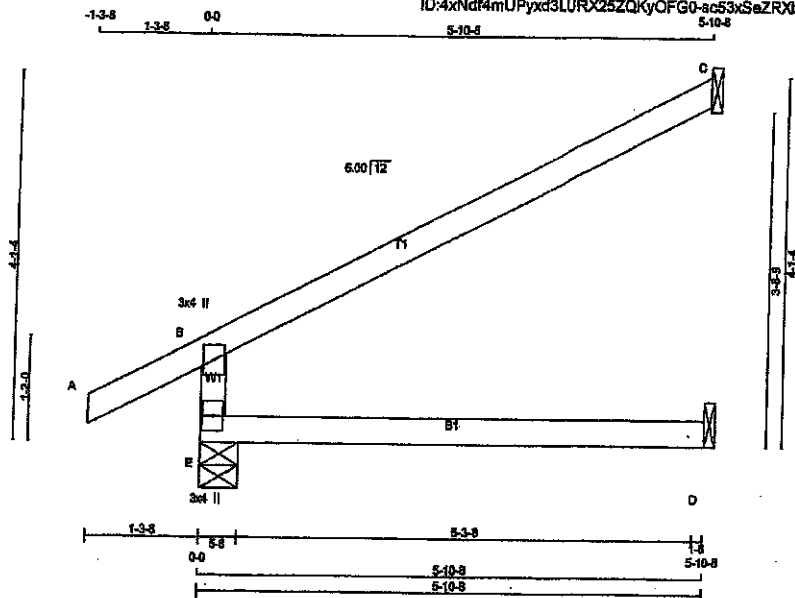
JSI GRIP=0.30 (B) (INPUT = 0.90)
JSI METAL=0.08 (B) (INPUT = 1.00)

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING DEPARTMENT



DRWG NO. TAM 174.00729
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME J6	QUANTITY 14	PLY 1	JOB DESC. Preston 3	DRWG NO. J6
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 14 X 17 = 235 lb

LUMBER

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	646	0	646	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	93	0	119	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37321H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	292/0	72/0	0/0	0/0	109/0	0/0
C	154	128/0	0/0	0/0	0/0	26/0	0/0
D	85	0/0	51/0	0/0	0/0	34/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	WEBS		MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
			FR-TO	FR-TO		
E-B	-513/0	0.0	0.0	0.22 (3)	7.81	
A-B	0/31	-102.1	-102.1	0.13 (1)	10.00	
B-C	-34/0	-102.1	-102.1	0.60 (1)	8.25	
E-D	0/0	-38.5	-38.5	0.22 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.6 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/942 (0.07")

CSI: TC=0.80/1.00 (B-C:1), BC=0.22/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 789 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.80)
JSI METAL= 0.14 (B) (INPUT = 1.00)

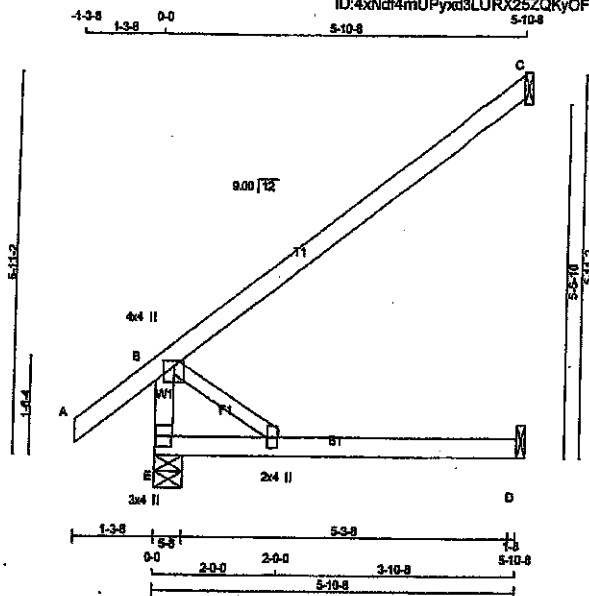


DWG NO. TAM 17902130
STRUCTURAL
COMPONENT ONLY

JOB NAME 200444-401156	TRUSS NAME J9	QUANTITY 8	PLY 1	JOB DESC. Preston 3	DRWG NO. J9
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:28:01 2018 Page 1
ID:4xNdf4mUPyxd3LURX25ZQKyOFG0-KoeS8ofBCqjNMzwdSva2leN6qqeVj8xQFXhhztzo5iW



Scale = 1:32.5

TOTAL WEIGHT = 8 X 20 = 160 lb

LUMBER

N. L. G. A. RULES			LUMBER	DESCR
CHORDS	SIZE			
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMAVA+p	MT20	4.0	4.0	1.00 2.00
E	BMV1+p	MT20	3.0	4.0	
F	NP+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
E	648	0	648	0	0	5-8
C	225	0	225	0	0	1-8
D	98	0	122	0	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	293/0	71/0	0/0	0/0	108/0	0/0
C	154	128/0	0/0	0/0	0/0	26/0	0/0
D	87	0/0	52/0	0/0	0/0	35/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		LENGTH FR-TO
E-B	-516/0	0.0 0.0 0.19 (3)	7.81	
A-B	0/42	-102.1 -102.1 0.14 (1)	10.00	
B-C	-45/0	-102.1 -102.1 0.60 (1)	6.25	
E-D	0/0	-38.5 -38.5 0.22 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/878 (0.08")

CSI: TC=0.60/1.00 (B-C:1), BC=0.22/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

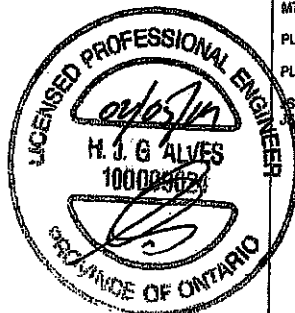
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP = 0.39 (B) (INPUT = 0.80)
SSI METAL = 0.10 (B) (INPUT = 1.00)



RECEIVED
JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DRWG NO. TAM **19402731**
STRUCTURAL
COMPONENT ONLY



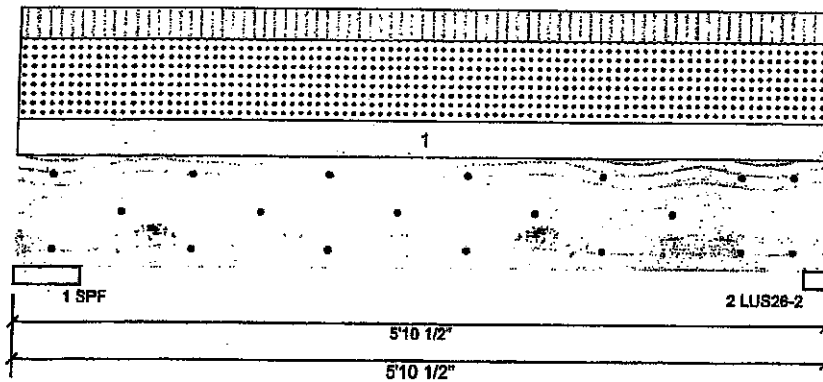
isDesign™

Client: Greenpark Homes
Project: Preston 3
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200444

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor (Residential)
Piles:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	237	281	626	0
2	215	254	567	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	15% 351 / 1176	1527 L	1.25D+1.5S +L
2 - LUS26-2	2.000"	37% 318 / 1065	1383 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1789 ft-lb	3'1"	6039 ft-lb	0.298 (30%)	1.25D+1.5S +L	L
Unbraced	1789 ft-lb	3'1"	5183 ft-lb	0.345 (35%)	1.25D+1.5S +L	L
Shear	1331 lb	1'2"	3984 lb	0.334 (33%)	1.25D+1.5S +L	L
Perm Defl in. (L/10287)	0.006	3'1"	0.179 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch (L/3676)	0.017	3'1"	0.179 (L/360)	0.090 (9%)	S+0.5L	L
TL Defl inch (L/2815)	0.023	3'1"	0.179 (L/360)	0.130 (13%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 77902732
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		7'-0"	Near Face	13 PSF	11 PSF	29 PSF	0 PSF	

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

This design is valid until 2021-12-11

TAMARACK LUMBER
3255 NORTH SERVICE RD, ON
CANADA
(905) 335-1115



RECEIVED
JUN 13 2019

Version 18.80.245 Powered by iStruct

TOWN OF CALEDON
BUILDING SECTION
FILE NO



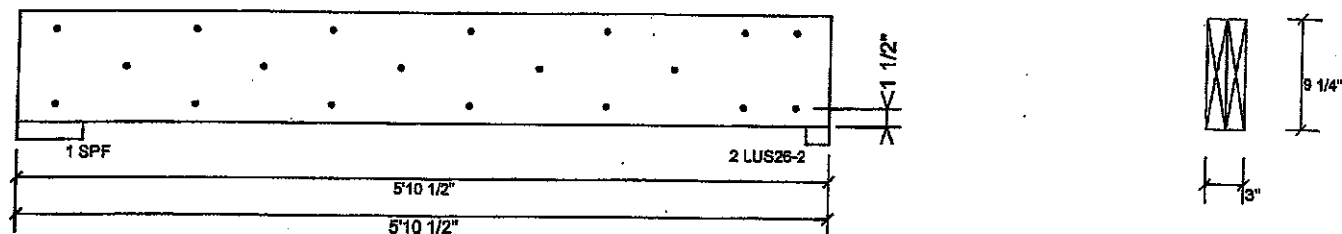
isDesign™

Client: Greenpark Homes
Project: Preston 3
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200444

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	84.6 %
Load	247.6 PLF
Yield Limit per Foot	383.4 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17902732
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	TAMARACK LUMBER 3255 NORTH SERVICE RD, ON CANADA (905) 335-1115

This design is valid until 2021-12-11





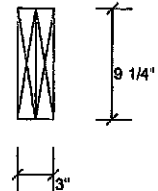
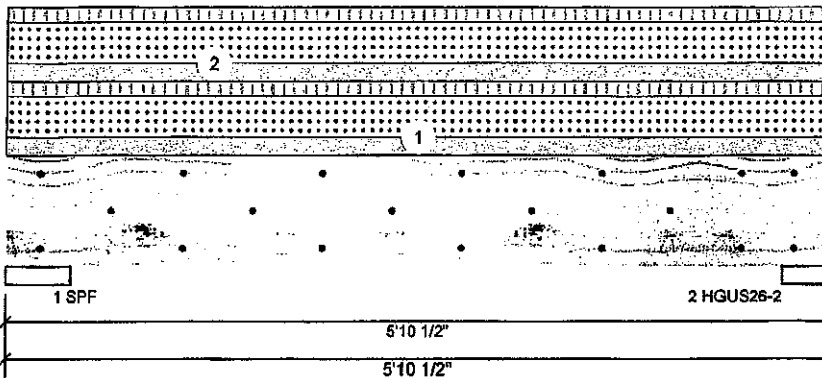
isDesign™

Client:
Project:
Address:Date: 2/27/2019
Designer: JG
Job Name: 401596 / GREENPARK/LAMBERTS LANE PH.2
Project #:

Page 1 of 3

B100 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Ply:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	10.5 PSF		
Dead:	6 PSF		
Snow:	29 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	308	382	852	0
2	296	366	816	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	20%	477 / 1586	2084 L	1.25D+1.5S +L
2 - HGUS...	4.000"	27%	457 / 1520	1978 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2332 ft-lb	3'	6039 ft-lb	0.386 (39%)	1.25D+1.5S +L	L
Unbraced	2332 ft-lb	3'	5236 ft-lb	0.445 (45%)	1.25D+1.5S +L	L
Shear	1791 lb	1'2"	3984 lb	0.450 (45%)	1.25D+1.5S +L	L
LL Defl inch	0.020 (L/3068)	3'	0.174 (L/360)	0.120 (12%)	S+0.5L	L
TL Defl inch	0.028 (L/2224)	3'	0.174 (L/360)	0.160 (16%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 7904518
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-9-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	
2	Uniform		1-0-0	Far Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3288 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





isDesign™

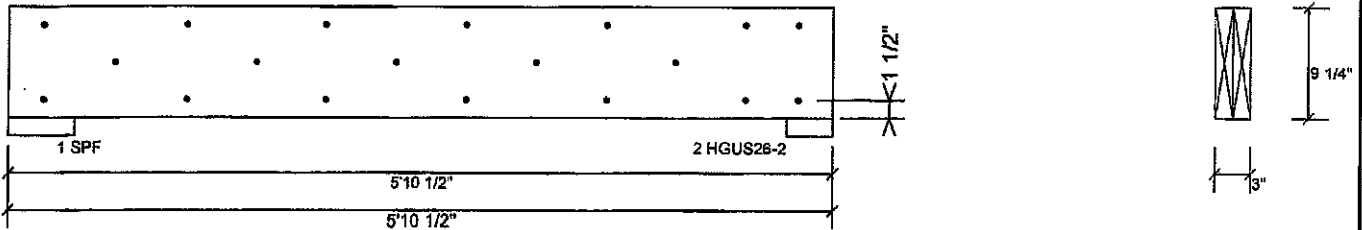
Client:
Project:
Address:

Date: 2/27/2019
Designer: JG
Job Name: 401596 / GREENPARK/LAMBERTS LANE PH.2
Project #:

Page 2 of 3

B100 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	90.8 %
Load	308.8 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 71704514
STRUCTURAL
COMPONENT ONLY 4/2

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021



LUL/LJS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258-259.

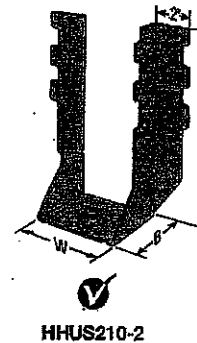
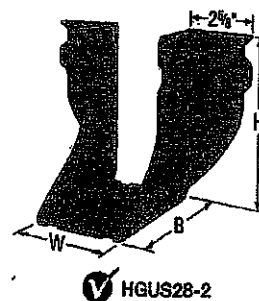
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp.20-24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/4" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



Double-Shear Nailing Top View

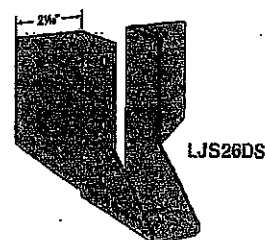
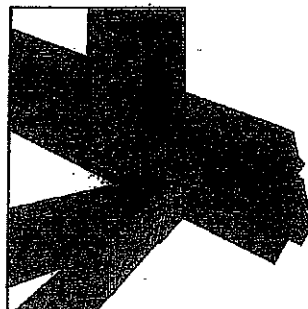


Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View (available on some models)
U.S. Patent 5,803,580

Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for connecting multiple members together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options Information on pp. 125–127.

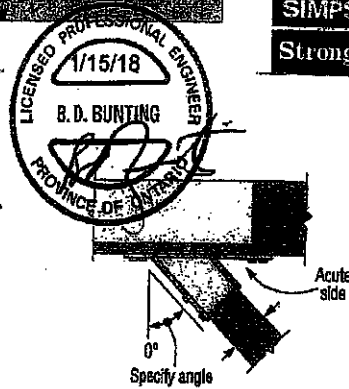
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	GA	Dimensions (in.)				Fastenings		Factored Resistances			
		W	H	B	d _o ¹	Header	Joist	S.D.F.		Wind	
								Down	Uplift	R _u = 1.55	R _u = 1.90
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
										2.87	5.14
LUS24L	22	1 1/8	3	1 3/4	2 3/4	(4) 10d	(2) 10d x 1 1/2"	380	1020	320	725
										3.42	3.72
LUS24L	22	1 1/8	5	1 3/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
										2.87	5.57
LUS26	18	1 1/8	4 1/4	1 3/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
										5.74	7.25
HUS26	16	1 1/8	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2085	3875
										9.29	17.24
LUS26DS	18	1 1/8	5	3 1/4	4 1/4	(18) 16d	(8) 16d	2055	4265	1480	4115
										6.43	16.31
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2885	5625	2685	5700
										11.96	25.35
LUS28L	20	1 1/8	8 1/4	1 3/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1660
										4.54	6.46
LUS28	18	1 1/8	6 1/4	1 3/4	3 1/4	(6) 10d	(4) 10d	1420	2520	1290	1790
										5.74	7.98
HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3605	5385	2675	4345
										11.80	19.33
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(38) 16d	(12) 16d	3310	7675	3310	6900
										14.74	30.73
LUS24L	20	1 1/8	8	1 3/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
										4.64	7.87
LUS210	18	1 1/8	7 1/4	1 3/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
										5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_o is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nail: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.

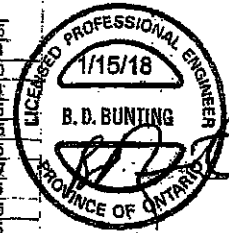
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d ₁	d ₂	Header	Joist	Uplift		S-P-F	
									(K ₁ = 1.15)	(K ₂ = 1.00)	(K ₃ = 1.00)	(K ₄ = 1.00)
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Double 2x Sizes												
LUS24-2	18	3 1/4	3 1/4	2	1 1/4	(4) 16d	(2) 16d		835	2020	590	1435
											2.62	8.38
LUS26-2	18	3 1/4	4 1/4	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
											6.87	8.38
HHUS26-2	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(8) 16d		2850	7335	2065	5205
											9.20	23.15
HGUS26-2	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
											13.83	28.27
LUS28-2	18	3 1/4	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2575
											6.87	11.45
HHUS28-2	14	3 1/4	7 1/4	3	3 1/4	(22) 16d	(8) 16d		3785	8940	2875	6345
											11.90	28.22
HGUS28-2	12	3 1/4	7 1/4	4	4 1/4	(30) 16d	(12) 16d		6070	12980	4310	9215
											19.17	40.99
LUS210-2	18	3 1/4	9	2	6	(8) 16d	(6) 16d		2580	4500	2320	3185
											10.32	14.21
HHUS210-2	14	3 1/4	9 1/4	3	8	(30) 16d	(10) 16d		4670	9660	4235	7000
											18.84	31.14
HGUS210-2	12	3 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14015	4855	10270
											21.80	45.69
Triple 2x Sizes												
HGUS26-3	12	4 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
											13.83	28.27
HGUS28-3	12	4 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
											19.17	40.99
HHUS210-3	14	4 1/4	9	3	7 1/4	(30) 16d	(10) 16d		4670	9670	4235	6865
											18.84	30.54
HGUS210-3	12	4 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14645	4855	10400
											21.80	46.28
Quadruple 2x Sizes												
HGUS26-4	12	6 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
											13.83	28.27
HGUS28-4	12	6 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
											19.17	40.99
HHUS210-4	14	6 1/4	9	3	7 1/4	(30) 16d	(10) 16d		4670	10155	4235	7210
											18.84	32.07
HGUS210-4	12	6 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14645	4855	10400
											21.80	46.28
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d		7640	14995	5425	10645
											24.13	47.35
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d		10130	16400	7195	11645
											32.00	51.80
4x Sizes												
LUS48	18	3 1/4	4 1/4	2	3 1/4	(4) 16d	(4) 16d		1720	2595	1545	1920
											6.87	8.38
HHUS48	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(8) 16d		2850	7335	2065	5205
											9.20	23.15
HGUS48	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
											13.83	28.27
LUS48	18	3 1/4	6 1/4	2	3 1/4	(8) 16d	(4) 16d		1720	3325	1545	2575
											6.87	11.45
HHUS48	14	3 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d		3785	8940	2875	6345
											11.90	28.22
HGUS48	12	3 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
											19.17	40.99
LUS410	18	3 1/4	8 1/4	2	5 1/4	(8) 16d	(6) 16d		2580	4500	2320	3185
											10.32	14.21
HGUS410	12	3 1/4	9	4	8 1/4	(46) 16d	(16) 16d		6840	14015	4855	10270
											21.80	45.69
HGUS412	12	3 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d		7640	14995	5425	10645
											24.13	47.35
HGUS414	12	3 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d		10130	16400	7195	11645
											32.00	51.80

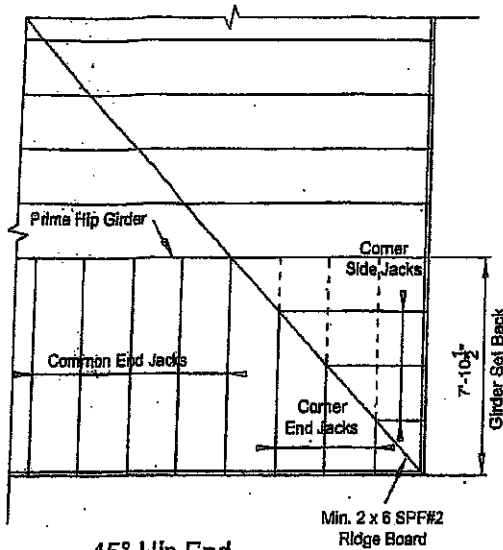


Plated Truss Connectors

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See footnotes on p. 258.



45° Hip End

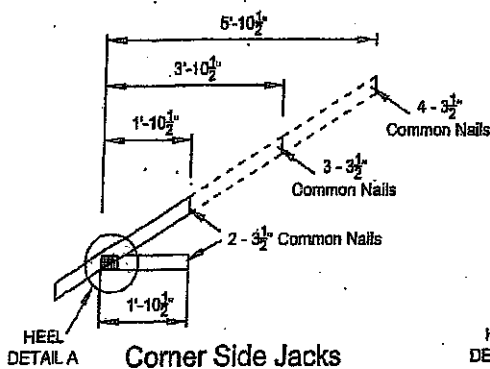
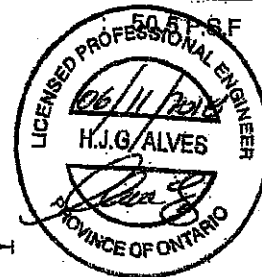
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

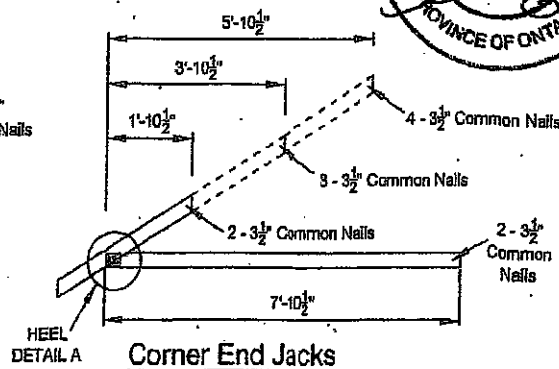
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

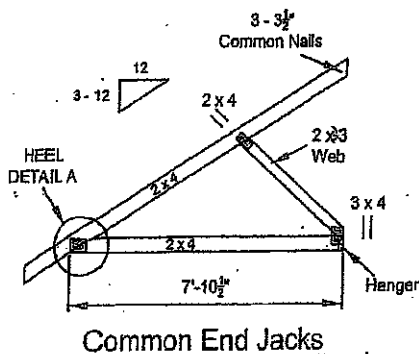
TOTAL LOAD



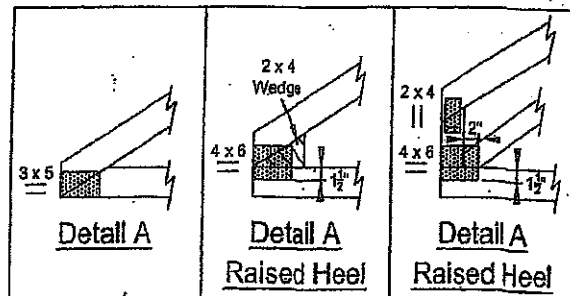
Corner Side Jacks



Corner End Jacks

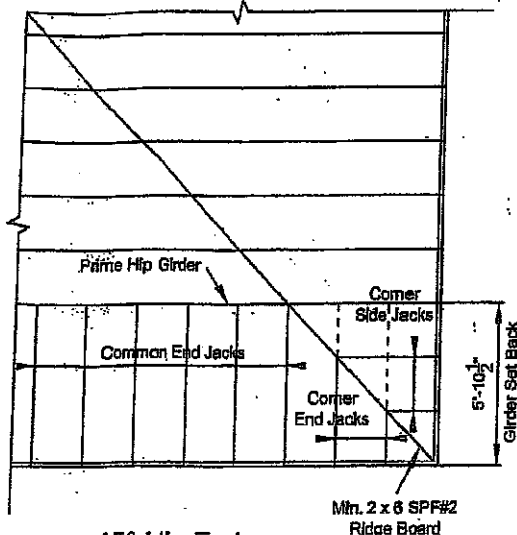


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



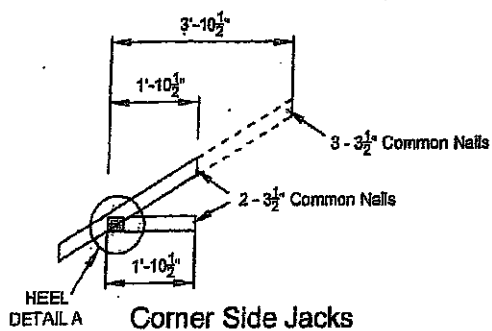
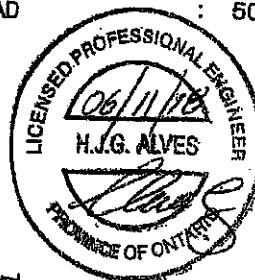
45° Hip End

LUMBER SPECIFICATION

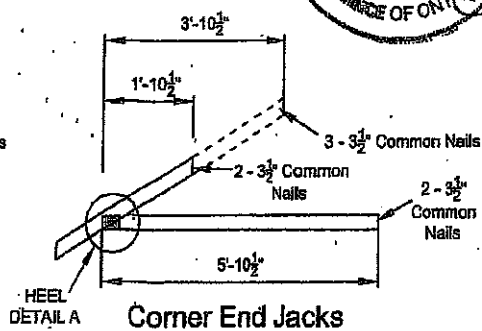
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

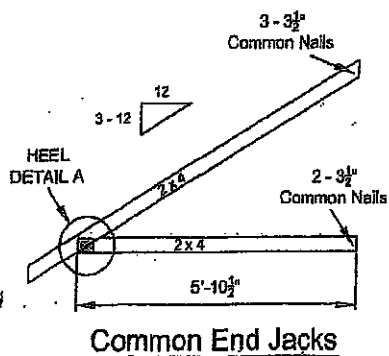
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 50.5 P.S.F.



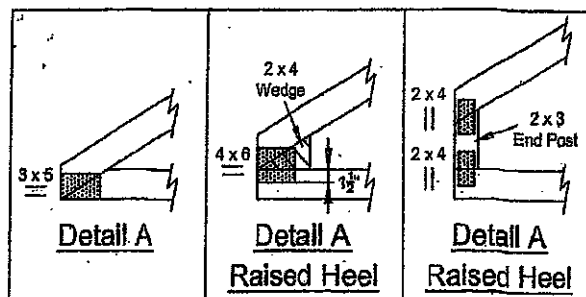
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

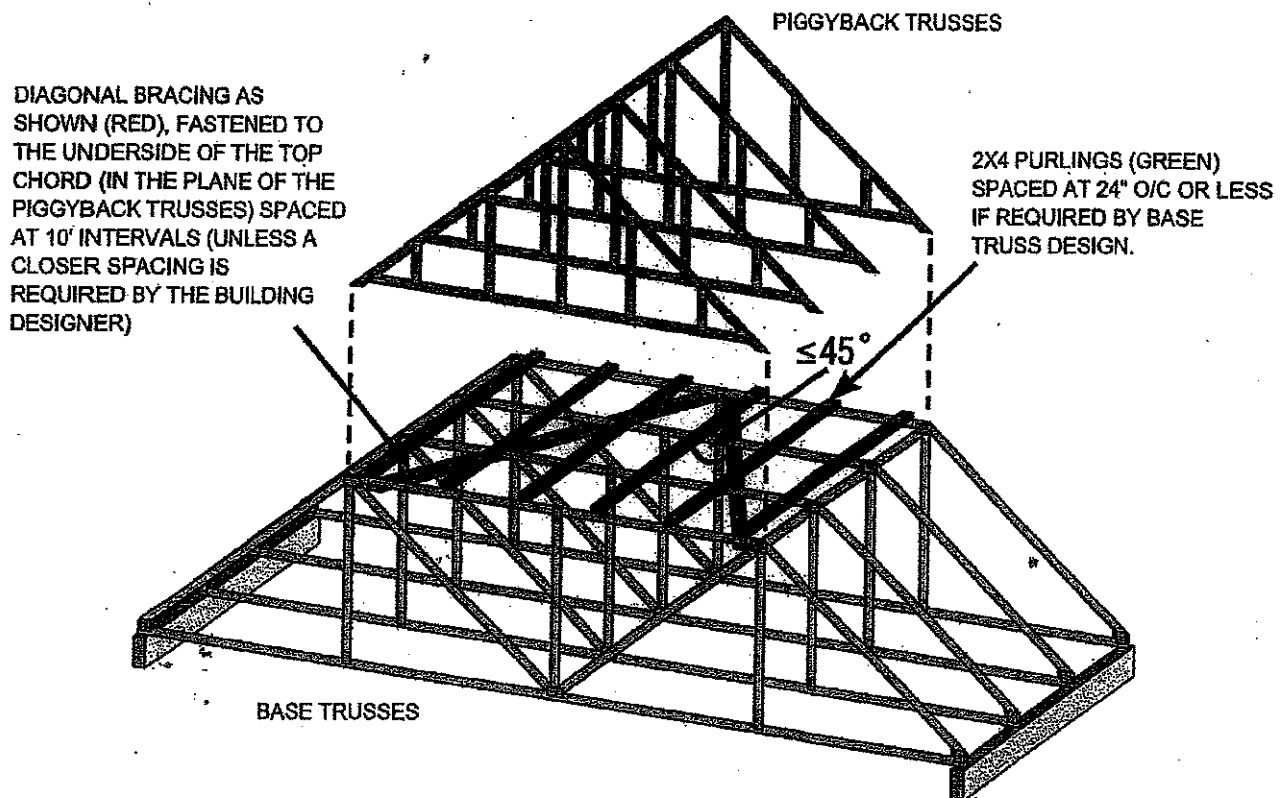
T-1800216

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

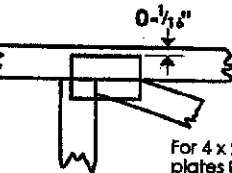
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in fractions or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0 1/2" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



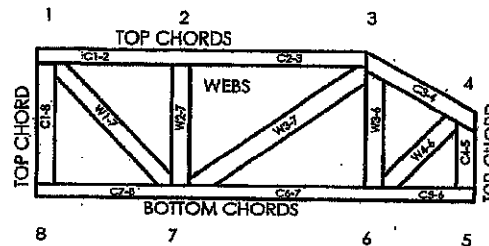
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate-Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE APPLICABLE BUILDING CODE



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RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

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