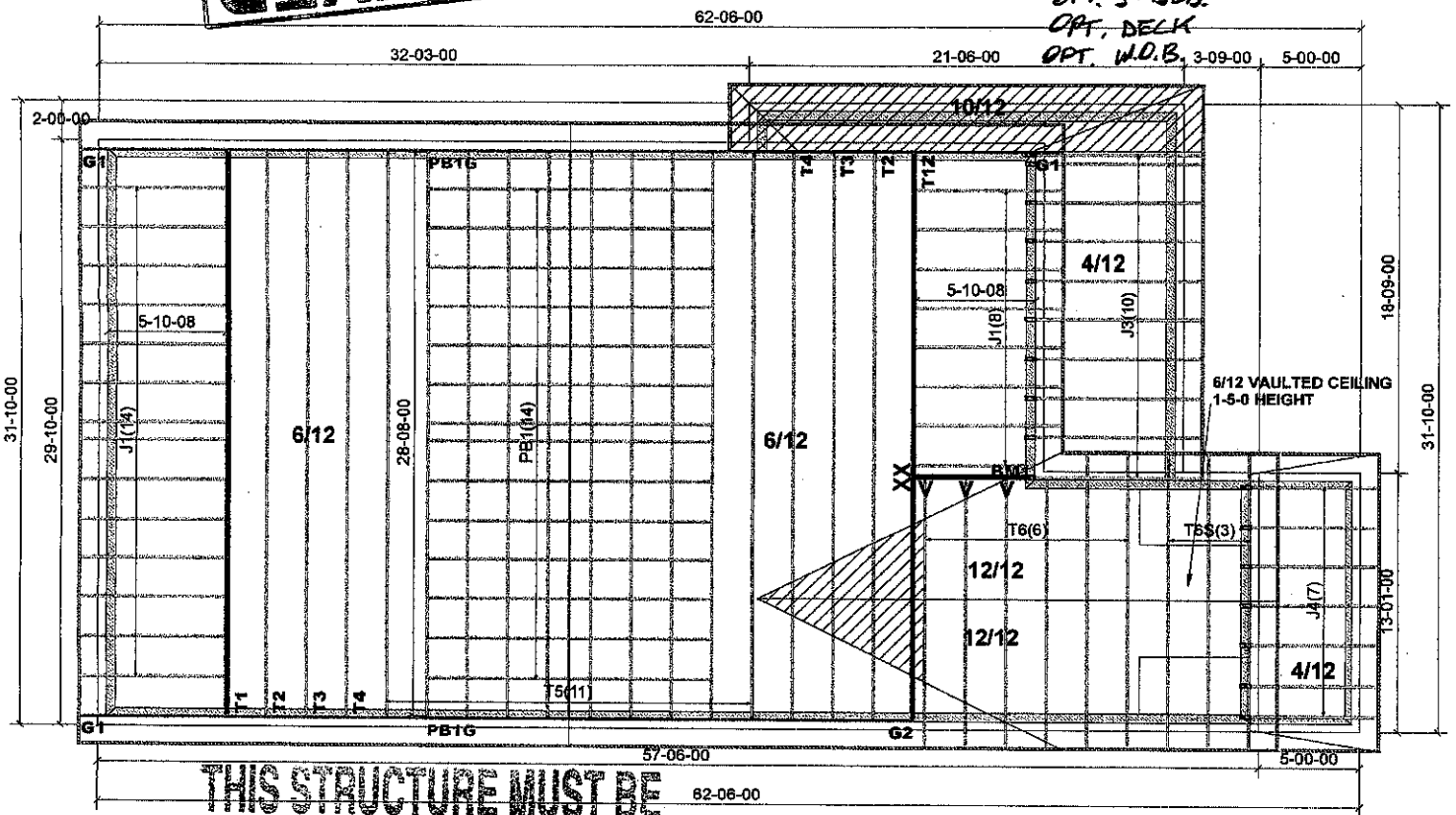
OPT. 5-BED.
OPT. DECK
OPT. W.O.B.



LOWER ROOF HAS STD HEEL JACKS 7-1/4" DROP FROM TOP PLATE TO BOTTOM OF FASCIA

TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONVENTIONAL ROOF FRAMING TO CONFORM WITH PART 9 OF THE OBC.
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"O.C. WITH 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED CONFORM WITH:
ONTARIO BUILDING CODE (2012)
OCCUPANCY: **RESIDENTIAL | PART: 9**

DESIGN LOADS:
CITY: CALEDON
G.S.L.= 37.6 psf
TC DL.= 6 psf
BC LL.= 10.50 psf
BC DL.= 7.00 psf

NOTES:

FIN. OH.:	12"
HEEL TYPE:	R.T.M. CANT.
EXT. WALLS:	2X6
CLAD. TYPE 1:	BRICK/5"
CLAD. TYPE 2:	SIDING/0"
FSC SIZE:	2X6
SHEATHING:	ASPHALT SHINGLE

IF DESIGNED COMMERCIAL, REFERE TO SEALED TRUSS DOCs FOR UPLIFT DESIGN

HARDWARE:
LJS26DS (V) 3pcs
HGUS26-2 (XX) 1pcs

CONV FRM BY OTHERS

T- 180737

COMMENTS:
BM1: 2-2"X10" SPF #2

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

APPLICANT COPY

TAMARACK

Job Track:	50120
Plan Log:	200170
Layout ID:	400367

Builder / Location: **Greenpark / Caledon**
Project: **Lamberts Lane Home Corp.**
Date: 2019-02-01 Designer: Brian

Model / Elevation: **Preston 2 / Elv. 1**

Mitek ver 8.2.3.229

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
Builder:	Greenpark	PlanLog:	200170
Project:	Lamberts Lane Home Corp.	Layout ID:	400367
Location:	Caledon	Ref #	
Model:	Preston 2	Page:	1 of 2
Lot #:		Date:	02-01-2019
Elevation:	1	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

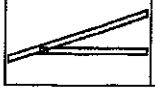
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Flat Girder	0 /12	**28-08-00**	4-01-04	2 x 6		4-01-04 4-01-04	296.21 182.67		
	1 2-ply	T1Z Flat Girder	0 /12	**28-08-00**	4-01-04	2 x 6		4-01-04 4-01-04	296.21 182.67		
	2	T2 Flat	0 /12	**28-08-00**	5-01-04	2 x 6		5-01-04 5-01-04	297.43 184.33		
	2	T3 Flat	0 /12	**28-08-00**	6-01-04	2 x 6		6-01-04 6-01-04	310.55 190.00		
	2	T4 Flat	0 /12	**28-08-00**	7-01-04	2 x 6		7-01-04 7-01-04	324.15 199.67		
	11	T5 Flat	0 /12	**28-08-00**	8-01-04	2 x 6		8-01-04 8-01-04	2334.75 1400.67		
	6	T6 Common	12 /12	**12-03-00**	8-00-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	387.79 252.00		
	3	T6S Roof Special	12 /12 6 /12	**12-03-00**	8-00-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	190.86 122.00		
	14	PB1 Common	6 /12	**14-03-00**	4-06-12	2 x 4		1-00-00 1-00-00	732.87 504.00		
	2	PB1G GABLE	6 /12	**14-03-00**	4-06-12	2 x 4		1-00-00 1-00-00	94.6 62.00		
	3	G1 GABLE	6 /12	**23-00-00**	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	348.37 224.50		
	1	G2 GABLE	6 /12	**17-01-08**	8-01-04	2 x 4		4-01-04 8-01-04	100.58 64.83		
	22	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	369.48 234.67		
	10	J3 Jack-Open	4 /12	**7-04-08**	3-02-06	2 x 4	1-03-08	3-15 2-09-07	217.51 140.00		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
Builder:	Greenpark	PlanLog:	200170
Project:	Lamberts Lane Home Corp.	Layout ID:	400367
Location:	Caledon	Ref #	
Model:	Preston 2	Page:	2 of 2
Lot #:		Date:	02-01-2019
Elevation:	1	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	7	J4 Jack-Open	4 /12	**5-05-08**	2-06-11	2 x 4	1-03-08	3-15 2-01-12	101.14 65.33		

TOTAL # TRUSS= **89** TOTAL BFT OF ALL TRUSSES= **4009.34** BFT. TOTAL WEIGHT OF ALL TRSSES **6402.5** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
3	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **4**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	T1	1	2	Preston 2 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 5 15:56:53 2019 Page 1
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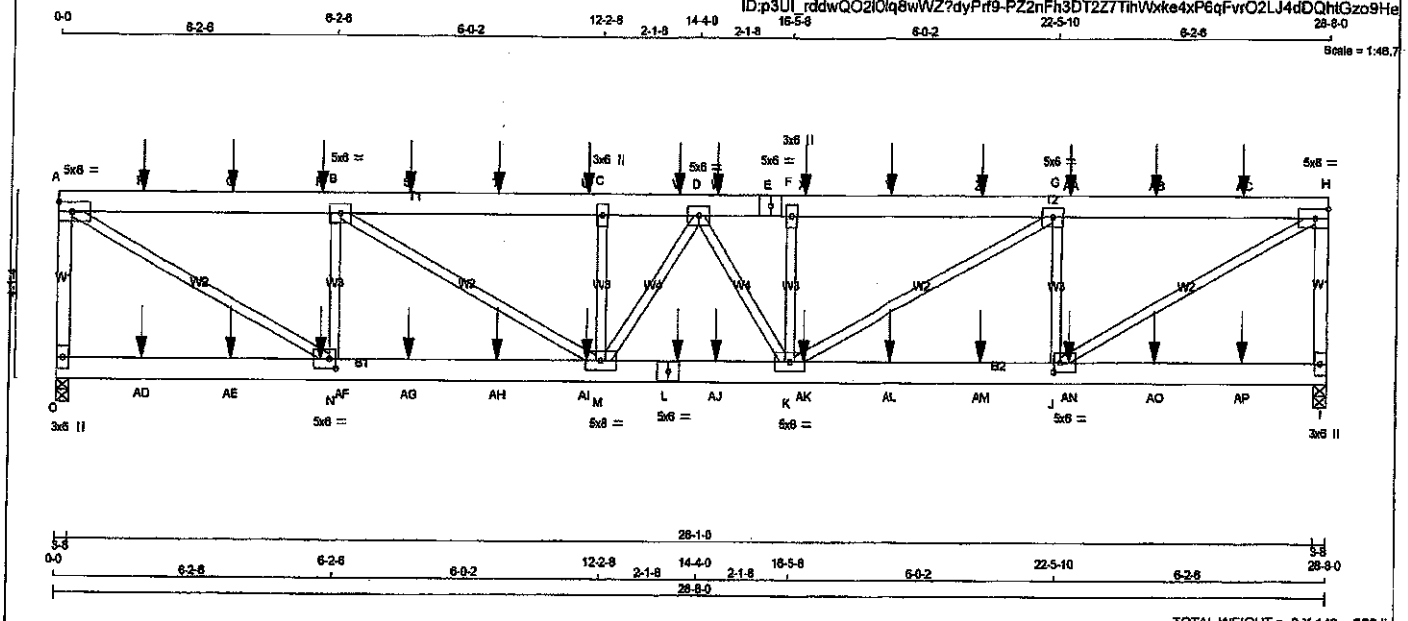

TOTAL WEIGHT = 2 X 148 = 296 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
O-A	1	12	TOP
H-I	1	12	TOP
A-E	2	12	SIDE(183.1)
E-H	2	12	SIDE(183.1)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-L	2	12	SIDE(183.1)
L-I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	8.0		Edge
B, D, G						
B	TMWW-t	MT20	5.0	8.0		
C	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	8.0		
F	TMW+w	MT20	3.0	6.0		
H	TMWW-t	MT20	5.0	8.0		Edge
I	BMV1+p	MT20	3.0	8.0		
J	BMWW-t	MT20	5.0	8.0	2.50	1.75
K	BMWWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	8.0	2.50	1.75
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TO CORNER EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	3260	0	3260	0	0	3-8	3-8
I	3260	0	3260	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2446	1322 / 0	511 / 0	0 / 0	0 / 0	614 / 0	0 / 0
I	2446	1322 / 0	511 / 0	0 / 0	0 / 0	613 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
O-A	-3106 / 0	0.0	0.0	0.36 (1)		6.57
A-P	-4664 / 0	-102.1	-102.1	0.26 (1)		5.10
P-Q	-4664 / 0	-102.1	-102.1	0.26 (1)		5.10
Q-R	-4664 / 0	-102.1	-102.1	0.26 (1)		5.10
R-B	-4664 / 0	-102.1	-102.1	0.26 (1)		5.10
B-S	-6545 / 0	-102.1	-102.1	0.30 (1)		4.41
S-T	-6545 / 0	-102.1	-102.1	0.30 (1)		4.41
T-U	-6545 / 0	-102.1	-102.1	0.30 (1)		4.41
U-C	-6545 / 0	-102.1	-102.1	0.30 (1)		4.41
C-V	-6545 / 0	-102.1	-102.1	0.14 (1)		4.58
V-D	-6545 / 0	-102.1	-102.1	0.14 (1)		4.58
D-W	-6542 / 0	-102.1	-102.1	0.14 (1)		4.58
W-E	-6542 / 0	-102.1	-102.1	0.14 (1)		4.58
E-F	-6542 / 0	-102.1	-102.1	0.14 (1)		4.58
F-X	-6542 / 0	-102.1	-102.1	0.30 (1)		4.41
X-Y	-6542 / 0	-102.1	-102.1	0.30 (1)		4.41
Y-Z	-6542 / 0	-102.1	-102.1	0.30 (1)		4.41
Z-G	-6542 / 0	-102.1	-102.1	0.30 (1)		4.41
G-AA	-4662 / 0	-102.1	-102.1	0.26 (1)		5.10
AA-AB	-4662 / 0	-102.1	-102.1	0.26 (1)		5.10
AB-AC	-4662 / 0	-102.1	-102.1	0.26 (1)		5.10
AC-H	-4662 / 0	-102.1	-102.1	0.26 (1)		5.10
I-H	-3105 / 0	0.0	0.0	0.36 (1)		6.57
O-AD	0 / 0	-38.5	-38.5	0.11 (3)		10.00
AD-AE	0 / 0	-38.5	-38.5	0.11 (3)		10.00
AE-AF	0 / 0	-38.5	-38.5	0.11 (3)		10.00
AF-N	0 / 0	-38.5	-38.5	0.11 (3)		10.00
N-AG	0 / 4664	-38.5	-38.5	0.38 (1)		10.00
AG-AH	0 / 4664	-38.5	-38.5	0.38 (1)		10.00
AH-AI	0 / 4664	-38.5	-38.5	0.38 (1)		10.00
AI-M	0 / 4664	-38.5	-38.5	0.38 (1)		10.00
M-AJ	0 / 6605	-38.5	-38.5	0.48 (1)		10.00
AJ-K	0 / 6605	-38.5	-38.5	0.48 (1)		10.00
K-AK	0 / 4662	-38.5	-38.5	0.38 (1)		10.00
AK-AL	0 / 4662	-38.5	-38.5	0.38 (1)		10.00
AL-AM	0 / 4662	-38.5	-38.5	0.38 (1)		10.00
AM-J	0 / 4662	-38.5	-38.5	0.38 (1)		10.00
J-AN	0 / 0	-38.5	-38.5	0.11 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
K-F	-704 / 0	0.08 (1)
M-C	-704 / 0	0.08 (1)
B-M	0 / 2200	0.27 (1)
A-N	0 / 5411	0.67 (1)
N-B	-2293 / 0	0.27 (1)
J-H	0 / 5409	0.67 (1)
K-G	0 / 2198	0.27 (1)
J-G	-2292 / 0	0.27 (1)
M-D	-119 / 0	0.02 (1)
D-K	-125 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.36/1.00 (A-O:1) , BC=0.48/1.00 (K-M:1) , WB=0.67/1.00 (A-N:1) , SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.62 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/05/19
H. J. G ALVES
100028824
PROVINCE OF ONTARIO

DWG NO. TAM T1202681
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T1	1	2	TRUSS DESC.	

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AN-AO	0 / 0	-38.5	-38.5	0.11 (3)	10.00			
AO-AP	0 / 0	-38.5	-38.5	0.11 (3)	10.00			
AP-I	0 / 0	-38.5	-38.5	0.11 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	22-9-4	-123	-123	—	BACK	VERT	TOTAL	—	—
J	22-9-4	-55	-70	—	BACK	VERT	TOTAL	—	—
L	13-10-12	-55	-70	—	BACK	VERT	TOTAL	—	—
P	1-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
Q	3-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
R	5-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
S	7-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
T	9-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
U	11-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
V	13-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
W	14-9-4	-123	-123	—	BACK	VERT	TOTAL	—	—
X	16-9-4	-123	-123	—	BACK	VERT	TOTAL	—	—
Y	18-9-4	-123	-123	—	BACK	VERT	TOTAL	—	—
Z	20-9-4	-123	-123	—	BACK	VERT	TOTAL	—	—
AB	24-9-4	-123	-123	—	BACK	VERT	TOTAL	—	—
AC	26-9-4	-123	-123	—	BACK	VERT	TOTAL	—	—
AD	1-10-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AE	3-10-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AF	5-10-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AG	7-10-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AH	9-10-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AI	11-10-12	-55	-70	—	BACK	VERT	TOTAL	—	—
AJ	14-9-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AK	16-9-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AL	18-9-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AM	20-9-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AO	24-9-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AP	26-9-4	-55	-70	—	BACK	VERT	TOTAL	—	—

DWG NO. TAM T1402689
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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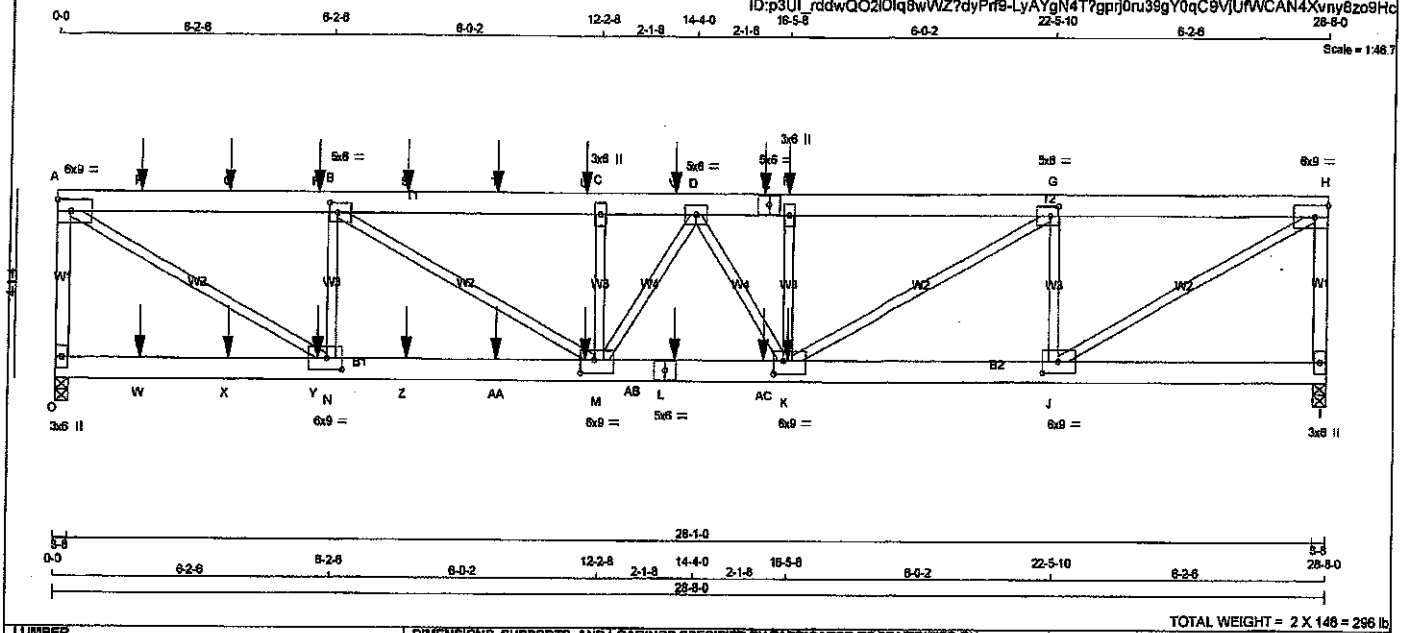

TOTAL WEIGHT = 2 X 148 = 296 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
O- A	1	12	TOP
H- I	1	12	TOP
A- E	2	12	SIDE(183.1)
E- H	2	12	SIDE(122.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	SIDE(183.1)
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
K- F	1	4	SIDE(389.8)
2x3	1	6	
M- C	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0		Edge
B	TMWW-t	MT20	5.0	6.0	2.50	2.25
C	TMW+w	MT20	3.0	6.0		
D	TMWW-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMVW-t	MT20	6.0	9.0		Edge
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	6.0	9.0	3.00	4.25
K	BMWWW-t	MT20	6.0	9.0	3.50	2.75
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	6.0	9.0	3.50	3.75
N	BMWW-t	MT20	6.0	9.0	3.00	4.25
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	3954	0	3954	0	0	3-8	3-8
I	3747	0	3747	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2942	1669 / 0	566 / 0	0 / 0	0 / 0	707 / 0	0 / 0
I	2777	1615 / 0	509 / 0	0 / 0	0 / 0	653 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)
O- A	-3797 / 0	0.0	0.0	4	0.44 (1)
A- P	-5836 / 0	-102.1	-102.1	1	0.28 (1)
P- Q	-5836 / 0	-102.1	-102.1	1	0.28 (1)
Q- R	-5836 / 0	-102.1	-102.1	1	0.28 (1)
R- B	-5836 / 0	-102.1	-102.1	1	0.28 (1)
B- S	-8648 / 0	-102.1	-102.1	1	0.37 (1)
S- T	-8648 / 0	-102.1	-102.1	1	0.37 (1)
T- U	-8648 / 0	-102.1	-102.1	1	0.37 (1)
U- C	-8648 / 0	-102.1	-102.1	1	0.37 (1)
C- V	-8648 / 0	-102.1	-102.1	1	0.18 (1)
V- D	-8648 / 0	-102.1	-102.1	1	0.18 (1)
D- E	-9610 / 0	-102.1	-102.1	1	0.21 (1)
E- F	-9610 / 0	-102.1	-102.1	1	0.21 (1)
F- G	-9610 / 0	-102.1	-102.1	1	0.31 (1)
G- H	-5750 / 0	-102.1	-102.1	1	0.20 (1)
I- H	-3643 / 0	0.0	0.0	4	0.42 (1)
O- W	0 / 0	-38.5	-38.5	1	0.11 (3)
W- X	0 / 0	-38.5	-38.5	1	0.11 (3)
X- Y	0 / 0	-38.5	-38.5	1	0.11 (3)
Y- N	0 / 0	-38.5	-38.5	1	0.11 (3)
N- Z	0 / 5836	-38.5	-38.5	1	0.46 (1)
Z- AA	0 / 5836	-38.5	-38.5	1	0.46 (1)
AA- AB	0 / 5836	-38.5	-38.5	1	0.46 (1)
AB- M	0 / 5836	-38.5	-38.5	1	0.46 (1)
M- L	0 / 9314	-38.5	-38.5	1	0.69 (1)
L- AC	0 / 9314	-38.5	-38.5	1	0.69 (1)
AC- K	0 / 9314	-38.5	-38.5	1	0.69 (1)
K- J	0 / 5750	-38.5	-38.5	1	0.46 (1)
J- I	0 / 0	-38.5	-38.5	1	0.07 (7)

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH	LC	MAX CSI (LC)
K- F	-467 / 0	6.05	1	0.06 (1)
M- C	-671 / 0	4.64	1	0.08 (1)
B- M	0 / 3522	4.64	1	0.44 (1)
A- N	0 / 6771	4.64	1	0.84 (1)
N- B	-2983 / 0	4.64	1	0.36 (1)
J- H	0 / 6671	3.82	1	0.83 (1)
K- G	0 / 4514	3.82	1	0.56 (1)
J- G	-3092 / 0	3.82	1	0.37 (1)
M- D	-927 / 0	3.82	1	0.14 (1)
D- K	-36 / 567	4.00	1	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	15-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
F	16-5-8	-52	-78	232	BACK	VERT	TOTAL	—	—
K	16-5-8	-2198	-2198	—	BACK	VERT	TOTAL	—	—
L	13-10-12	-55	-70	—	BACK	VERT	TOTAL	—	—
P	1-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
Q	3-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—
R	5-10-12	-123	-123	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 949 (0.36")

CSI: TC=0.44/1.00 (A-O:1) , BC=0.69/1.00 (K-M:1) , WB=0.84/1.00 (A-N:1) , SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.90 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/05/19
H. J. G. ALVES
100009324
PROVINCE OF ONTARIO

DWG NO. TAM T1902690
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	7-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
T	9-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
U	11-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
V	13-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
W	1-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
X	3-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
Y	5-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
Z	7-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AA	9-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AB	11-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AC	15-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--

DWG NO. TAM T1902690
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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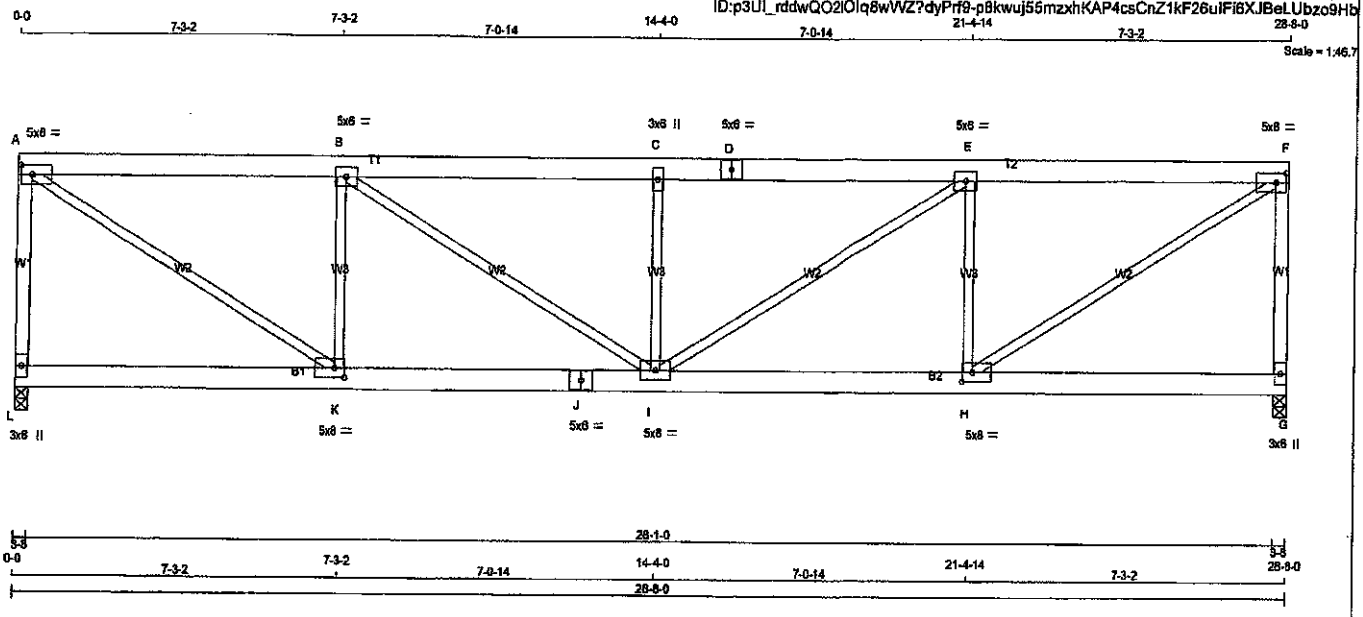

TOTAL WEIGHT = 2 X 149 = 297 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	2.75
B	TMWW-t	MT20	5.0	6.0		
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMVW-t	MT20	5.0	8.0	2.50	2.75
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	5.0	8.0	2.50	2.75
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	8.0	2.50	2.75
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	2015	0	2015	0	0	3-8	3-8
G	2015	0	2015	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0
G	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-A	-1898 / 0	0.0	0.0	0.70 (1)	6.10	
A-B	-2496 / 0	-102.1	-102.1	0.41 (1)	4.80	
B-C	-3193 / 0	-102.1	-102.1	0.44 (1)	4.32	
C-D	-3193 / 0	-102.1	-102.1	0.44 (1)	4.32	
D-E	-3193 / 0	-102.1	-102.1	0.44 (1)	4.32	
E-F	-2496 / 0	-102.1	-102.1	0.41 (1)	4.80	
G-F	-1898 / 0	0.0	0.0	0.70 (1)	6.10	
L-K	0 / 0	-38.5	-38.5	0.17 (3)	10.00	
K-J	0 / 2498	-38.5	-38.5	0.41 (2)	10.00	
J-I	0 / 2498	-38.5	-38.5	0.41 (2)	10.00	
I-H	0 / 2498	-38.5	-38.5	0.41 (2)	10.00	
H-G	0 / 0	-38.5	-38.5	0.17 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-F	0 / 2964	0.67 (1)
A-K	0 / 2964	0.67 (1)
H-E	-1288 / 0	0.43 (1)
K-B	-1288 / 0	0.43 (1)
I-E	0 / 834	0.19 (1)
B-I	0 / 834	0.19 (1)
I-C	-666 / 0	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.70/1.00 (A-L:1) , BC=0.41/1.00 (I-K:2) , WB=0.67/1.00 (F-H:1) , SSI=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.49 (J) (INPUT = 1.00)

DWG NO. TAM T1902691
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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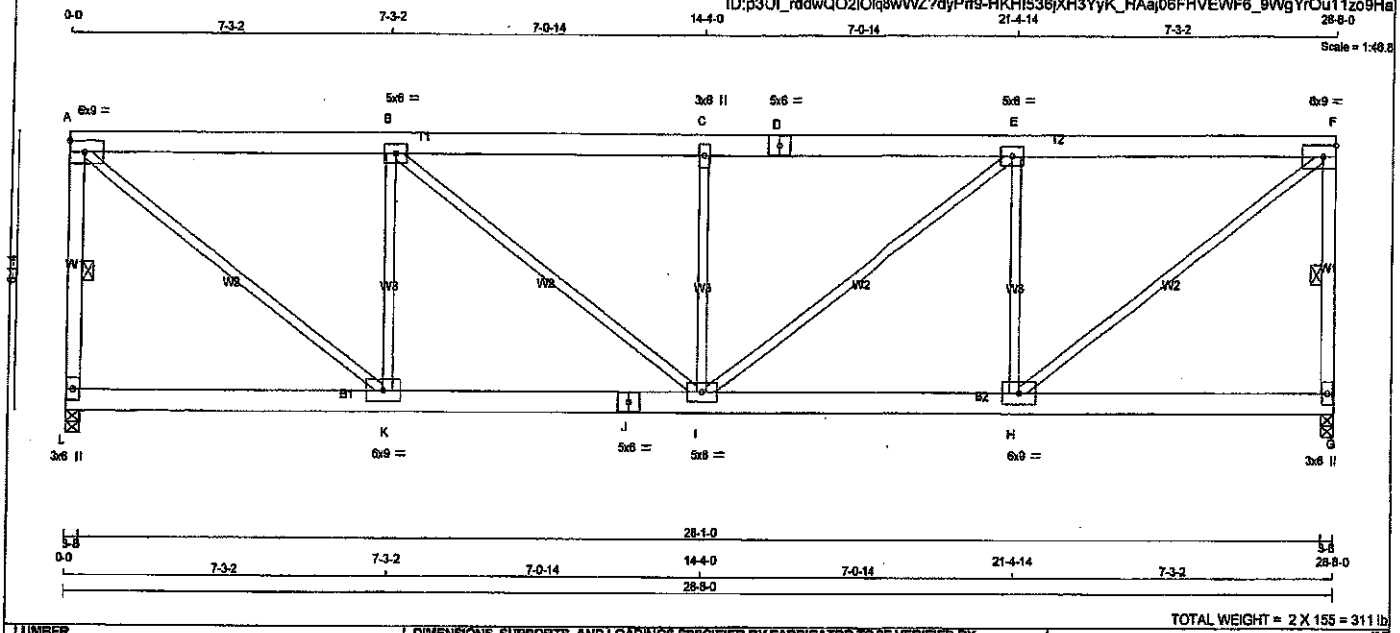

TOTAL WEIGHT = 2 X 155 = 311 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMWW-t	MT20	6.0	9.0	Edge
B	TMWW-t	MT20	5.0	6.0	
C	TMW+w	MT20	3.0	6.0	
D	TS-t	MT20	5.0	6.0	
E	TMWW-t	MT20	5.0	6.0	
F	TMWW-t	MT20	6.0	9.0	Edge
G	BMV1+p	MT20	3.0	6.0	
H	BMWW-t	MT20	6.0	9.0	
I	BMWWW-t	MT20	5.0	8.0	
J	BS-t	MT20	5.0	6.0	
K	BMWW-t	MT20	6.0	9.0	
L	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2015	0	2015	0	0	3-8	3-8
G	2015	0	2015	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0
G	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-L, F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
L-A	-1900 / 0	0.0	0.0	0.31 (1)	4.88
A-B	-2059 / 0	-102.1	-102.1	0.40 (1)	5.18
B-C	-2630 / 0	-102.1	-102.1	0.42 (1)	4.88
C-D	-2630 / 0	-102.1	-102.1	0.42 (1)	4.88
D-E	-2630 / 0	-102.1	-102.1	0.42 (1)	4.88
E-F	-2059 / 0	-102.1	-102.1	0.40 (1)	5.18
G-F	-1900 / 0	0.0	0.0	0.31 (1)	4.88
L-K	0 / 0	-38.5	-38.5	0.18 (3)	10.00
K-J	0 / 2059	-38.5	-38.5	0.37 (2)	10.00
J-I	0 / 2059	-38.5	-38.5	0.37 (2)	10.00
I-H	0 / 2059	-38.5	-38.5	0.37 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.18 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
H-F	0 / 2608	0.59 (1)
A-K	0 / 2608	0.59 (1)
H-E	-1289 / 0	0.66 (1)
K-B	-1289 / 0	0.66 (1)
I-E	0 / 731	0.16 (1)
B-I	0 / 731	0.16 (1)
I-C	-665 / 0	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.37/1.00 (I-K:2) , WB=0.66/1.00 (E-H:1) , SSI=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.77 (K) (INPUT = 0.90)
SI METAL= 0.41 (J) (INPUT = 1.00)

DWG NO. TAM T190269Z
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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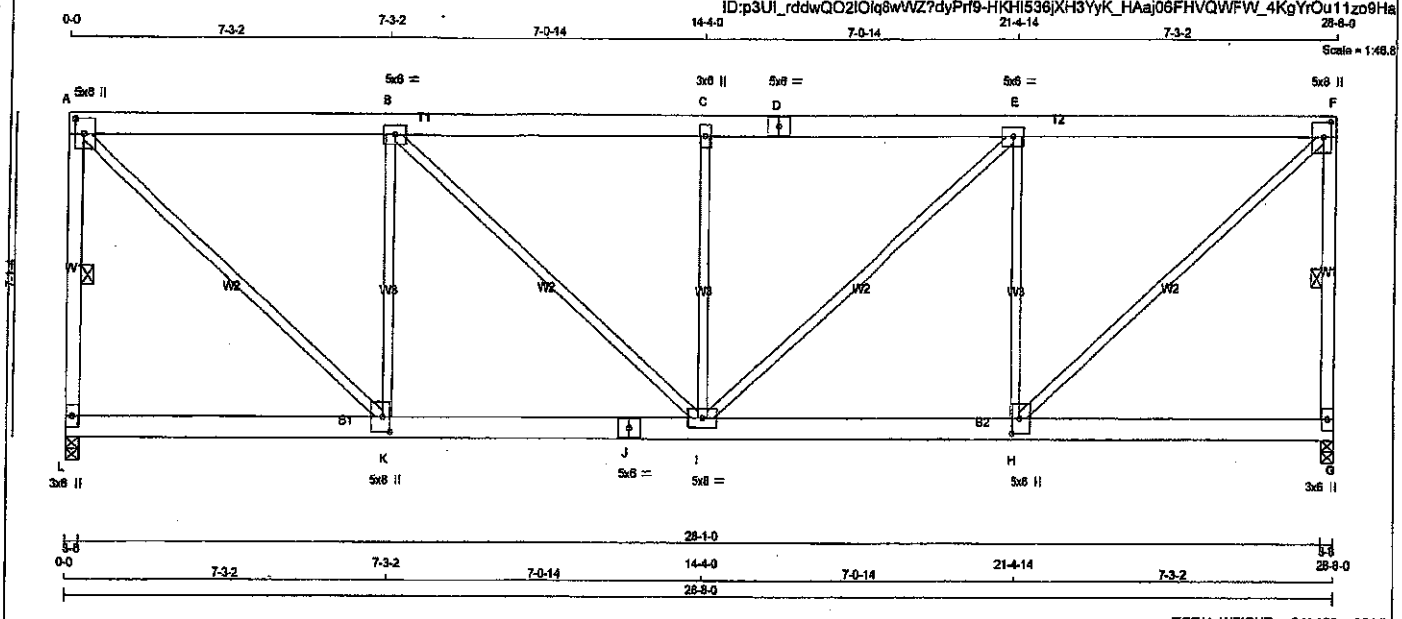

TOTAL WEIGHT = 2 X 162 = 324 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	8.0	4.00	2.00
B	TMWW-t	MT20	5.0	6.0		
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMVW+p	MT20	5.0	8.0	4.00	2.00
G	BMV1+p	MT20	3.0	6.0		
H	BMWW+t	MT20	5.0	8.0	4.00	2.00
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW+t	MT20	5.0	8.0	4.00	2.00
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	2015	0	2015	0	0	3-8	3-8
G	2015	0	2015	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0
G	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-L, F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
L- A	-1901 / 0	0.0	0.0	0.40 (1)	4.87
A- B	-1751 / 0	-102.1	-102.1	0.39 (1)	5.51
B- C	-2236 / 0	-102.1	-102.1	0.41 (1)	5.00
C- D	-2236 / 0	-102.1	-102.1	0.41 (1)	5.00
D- E	-2236 / 0	-102.1	-102.1	0.41 (1)	5.00
E- F	-1751 / 0	-102.1	-102.1	0.39 (1)	5.51
G- F	-1901 / 0	0.0	0.0	0.40 (1)	4.87
L- K	0 / 0	-38.5	-38.5	0.18 (3)	10.00
K- J	0 / 1751	-38.5	-38.5	0.34 (2)	10.00
J- I	0 / 1751	-38.5	-38.5	0.34 (2)	10.00
I- H	0 / 1751	-38.5	-38.5	0.34 (2)	10.00
H- G	0 / 0	-38.5	-38.5	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- F	0 / 2374	0.53 (1)
A- K	0 / 2374	0.53 (1)
H- E	-1289 / 0	0.99 (1)
K- B	-1289 / 0	0.99 (1)
I- E	0 / 664	0.15 (1)
B- I	0 / 664	0.15 (1)
I- C	-664 / 0	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.34/1.00 (I-K:2) , WB=0.99/1.00 (E-H:1) , SSI=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.63 (K) (INPUT = 1.00)

DWG NO. TAM T1902693
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T5	11	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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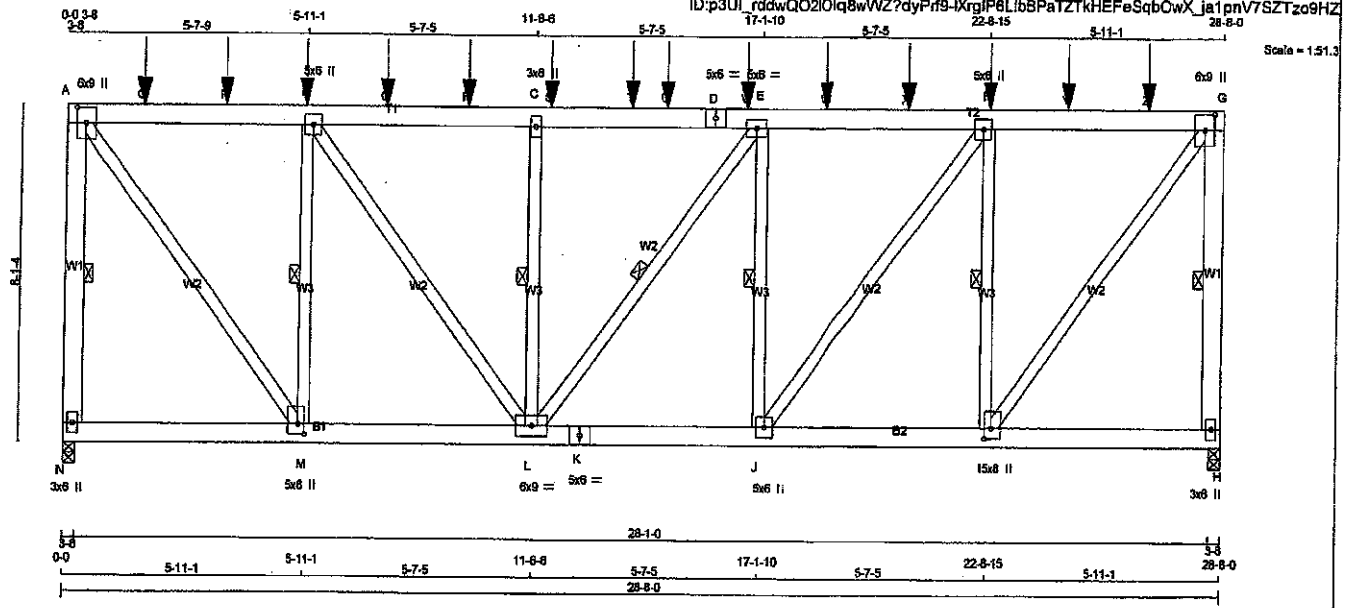

TOTAL WEIGHT = 11 X 212 = 2335 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	6.0	9.0	4.50	2.75
B	TMWW+t	MT20	5.0	6.0		
C	TMW+w	MT20	3.0	6.0		
D	TS-I	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMWW+t	MT20	5.0	6.0		
G	TMVW+p	MT20	6.0	9.0	4.50	2.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	8.0	3.00	2.00
J	BMWW+t	MT20	5.0	6.0		
K	BS-I	MT20	5.0	6.0		
L	BMWWW-t	MT20	6.0	9.0		
M	BMWW+t	MT20	5.0	8.0	3.00	2.00
N	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	3862	0	3862	0	0	3-8	3-8 & 2SBP
H	3862	0	3862	0	0	3-8	3-8 & 2SBP

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	2934	1473 / 0	694 / 0	0 / 0	0 / 0	767 / 0	0 / 0
H	2934	1473 / 0	694 / 0	0 / 0	0 / 0	767 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H
INSTALL TWO USP SBP4 BEARING PLATES AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-N, G-H, F-I, B-M, E-J, C-L, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
N-A	-3768 / 0	0.0	0.0	0.65 (1)	4.42
A-O	-2569 / 0	-102.1	-102.1	0.54 (1)	4.50
O-P	-2569 / 0	-102.1	-102.1	0.54 (1)	4.50
P-B	-2569 / 0	-102.1	-102.1	0.54 (1)	4.50
B-Q	-3707 / 0	-102.1	-102.1	0.59 (1)	3.80
Q-R	-3707 / 0	-102.1	-102.1	0.59 (1)	3.80
R-C	-3707 / 0	-102.1	-102.1	0.59 (1)	3.80
C-S	-3707 / 0	-102.1	-102.1	0.52 (1)	3.92
S-T	-3707 / 0	-102.1	-102.1	0.52 (1)	3.92
T-U	-3707 / 0	-102.1	-102.1	0.52 (1)	3.92
U-D	-3707 / 0	-102.1	-102.1	0.52 (1)	3.92
D-V	-3707 / 0	-102.1	-102.1	0.52 (1)	3.92
V-E	-3707 / 0	-102.1	-102.1	0.52 (1)	3.92
E-W	-3717 / 0	-102.1	-102.1	0.59 (1)	3.80
W-X	-3717 / 0	-102.1	-102.1	0.59 (1)	3.80
X-F	-3717 / 0	-102.1	-102.1	0.59 (1)	3.80
F-Y	-2566 / 0	-102.1	-102.1	0.54 (1)	4.51
Y-Z	-2566 / 0	-102.1	-102.1	0.54 (1)	4.51
Z-G	-2566 / 0	-102.1	-102.1	0.54 (1)	4.51
H-G	-3766 / 0	0.0	0.0	0.65 (1)	4.42
N-M	0 / 0	-38.5	-38.5	0.11 (3)	10.00
M-L	0 / 2569	-38.5	-38.5	0.38 (1)	10.00
L-K	0 / 3717	-38.5	-38.5	0.52 (1)	10.00
K-J	0 / 3717	-38.5	-38.5	0.52 (1)	10.00
J-I	0 / 2566	-38.5	-38.5	0.37 (1)	10.00
I-H	0 / 0	-38.5	-38.5	0.11 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-G	0 / 4191	0.67 (1)
A-M	0 / 4195	0.67 (1)
I-F	-3084 / 0	0.83 (1)
M-B	-3057 / 0	0.83 (1)
J-F	0 / 1945	0.31 (1)
B-L	0 / 1925	0.31 (1)
J-E	-1356 / 0	0.37 (1)
L-C	-1348 / 0	0.37 (1)
L-E	-17 / 0	0.01 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
F	22-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
O	1-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
P	3-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.65/1.00 (A-N:1) , BC=0.52/1.00 (J-L:1) , WB=0.83/1.00 (F-I:1) , SSI=0.49/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (FLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (M) (INPUT = 0.90)
SI METAL= 0.93 (M) (INPUT = 1.00)

DWG NO. TAM 17902694
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T5	11	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	7-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
R	9-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
S	11-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
T	13-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
U	14-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
V	16-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
W	18-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
X	20-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
Y	24-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
Z	28-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—

DWG NO. TAM T7702694
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T6-Cond1	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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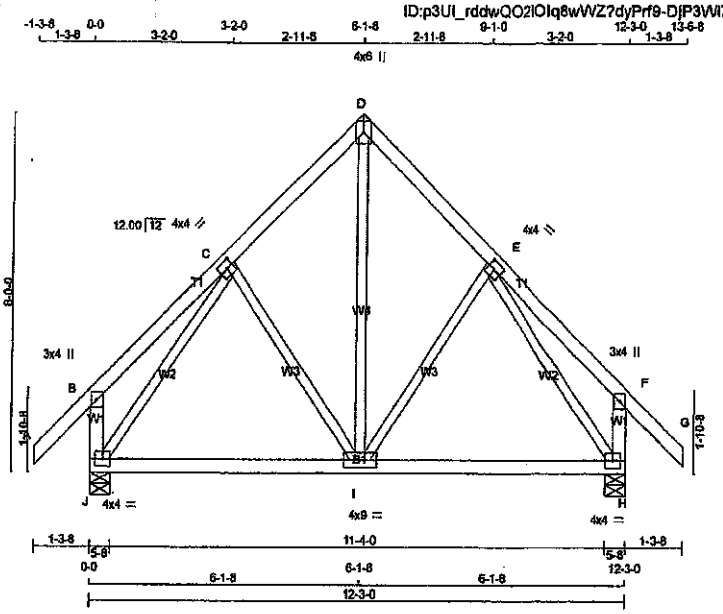

TOTAL WEIGHT = 6 X 65 = 388 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1004	0	1004	0	0	5-8	5-8
H	1004	0	1004	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	437 / 0	129 / 0	0 / 0	0 / 0	176 / 0	0 / 0
H	741	437 / 0	129 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00
B-C	0 / 25	-102.1	-102.1	0.15 (1)	10.00
C-D	-544 / 0	-102.1	-102.1	0.12 (1)	6.25
D-E	-544 / 0	-102.1	-102.1	0.12 (1)	6.25
E-F	0 / 25	-102.1	-102.1	0.15 (1)	10.00
F-G	0 / 50	-102.1	-102.1	0.14 (1)	10.00
J-B	-261 / 0	0.0	0.0	0.03 (1)	7.81
H-F	-261 / 0	0.0	0.0	0.03 (1)	7.81
J-I	0 / 434	-38.5	-38.5	0.36 (2)	10.00
I-H	0 / 434	-38.5	-38.5	0.36 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 486	0.11 (1)
I-E	-117 / 24	0.06 (1)
C-I	-117 / 24	0.06 (1)
J-C	-784 / 0	0.38 (1)
E-H	-784 / 0	0.38 (1)

LICENSED PROFESSIONAL ENGINEER
02/05/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.36/1.00 (I-J:2) , WB=0.38/1.00 (C-J:1) , SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

DWG NO. TAM T1902695
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T6-Cond2	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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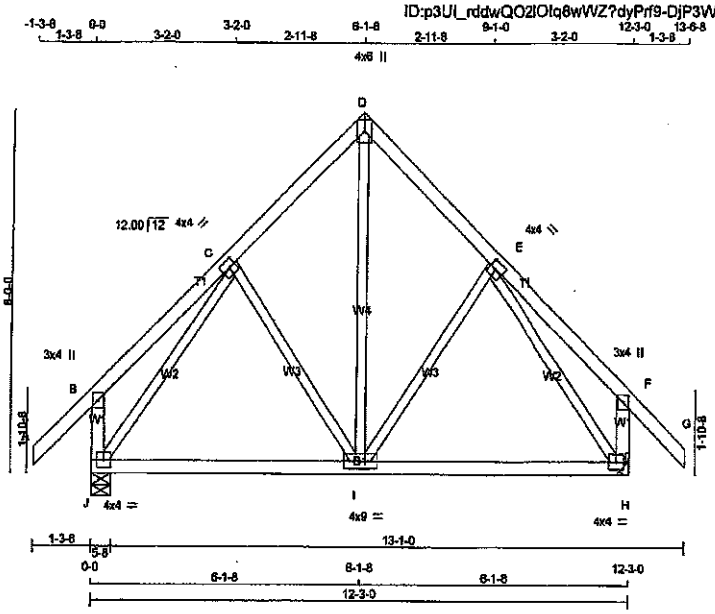

TOTAL WEIGHT = 6 X 65 = 388 lb

[M]F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1004	0	1004	0	0	5-8	5-8
H	1004	0	1004	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	741	437 / 0	129 / 0	0 / 0	0 / 0	176 / 0	0 / 0
H	741	437 / 0	129 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 50	-102.1	-102.1	0.14 (1)	10.00	I- D	0 / 466	0.11 (1)
B- C	0 / 25	-102.1	-102.1	0.15 (1)	10.00	I- E	-117 / 24	0.06 (1)
C- D	-544 / 0	-102.1	-102.1	0.12 (1)	6.25	C- I	-117 / 24	0.06 (1)
D- E	-544 / 0	-102.1	-102.1	0.12 (1)	6.25	J- C	-784 / 0	0.38 (1)
E- F	0 / 25	-102.1	-102.1	0.15 (1)	10.00	E- H	-784 / 0	0.38 (1)
F- G	0 / 50	-102.1	-102.1	0.14 (1)	10.00			
J- B	-261 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-261 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 434	-38.5	-38.5	0.36 (2)	10.00			
I- H	0 / 434	-38.5	-38.5	0.36 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.36/1.00 (I-J:2) , WB=0.38/1.00 (C-J:1) , SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (E) (INPUT = 0.90)
SI METAL= 0.30 (E) (INPUT = 1.00)

DWG NO. TAM T190 2696
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	T6S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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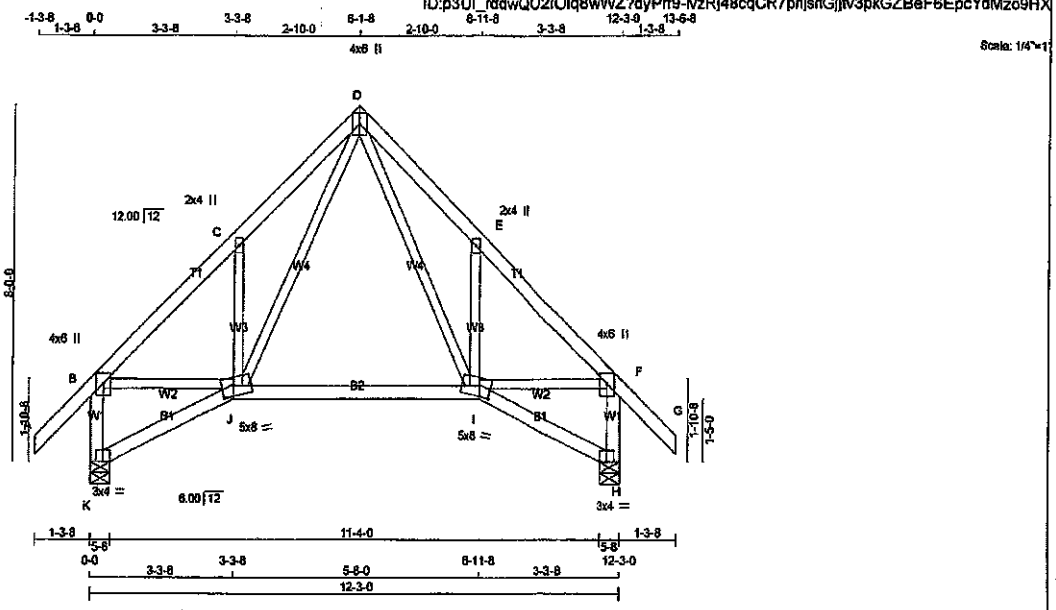

TOTAL WEIGHT = 3 X 64 = 191 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BVM1-p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BVM1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1004	0	1004	0	0	5-8	5-8
H	1004	0	1004	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	741	437 / 0	129 / 0	0 / 0	0 / 0	176 / 0	0 / 0
H	741	437 / 0	129 / 0	0 / 0	0 / 0	176 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00
B-C	-885 / 0	-102.1	-102.1	0.13 (1)	6.25
C-D	-931 / 0	-102.1	-102.1	0.11 (1)	6.25
D-E	-931 / 0	-102.1	-102.1	0.11 (1)	6.25
E-F	-885 / 0	-102.1	-102.1	0.13 (1)	6.25
F-G	0 / 50	-102.1	-102.1	0.14 (1)	10.00
K-B	-940 / 0	0.0	0.0	0.11 (1)	7.81
H-F	-940 / 0	0.0	0.0	0.11 (1)	7.81
K-J	0 / 0	-38.5	-38.5	0.10 (3)	10.00
J-I	0 / 399	-38.5	-38.5	0.32 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.10 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 / 584	0.13 (1)
I-E	-378 / 0	0.09 (1)
J-D	0 / 584	0.13 (1)
J-C	-378 / 0	0.09 (1)
B-J	0 / 641	0.14 (1)
I-F	0 / 641	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.32/1.00 (I-J:2) , WB=0.14/1.00 (B-J:1) , SSI=0.12/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.55 (K) (INPUT = 1.00)

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
02/05/19
H. J. G. ALVES
100099784
PROVINCE OF ONTARIO

DWG NO. TAM 1902697
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	PB1	14	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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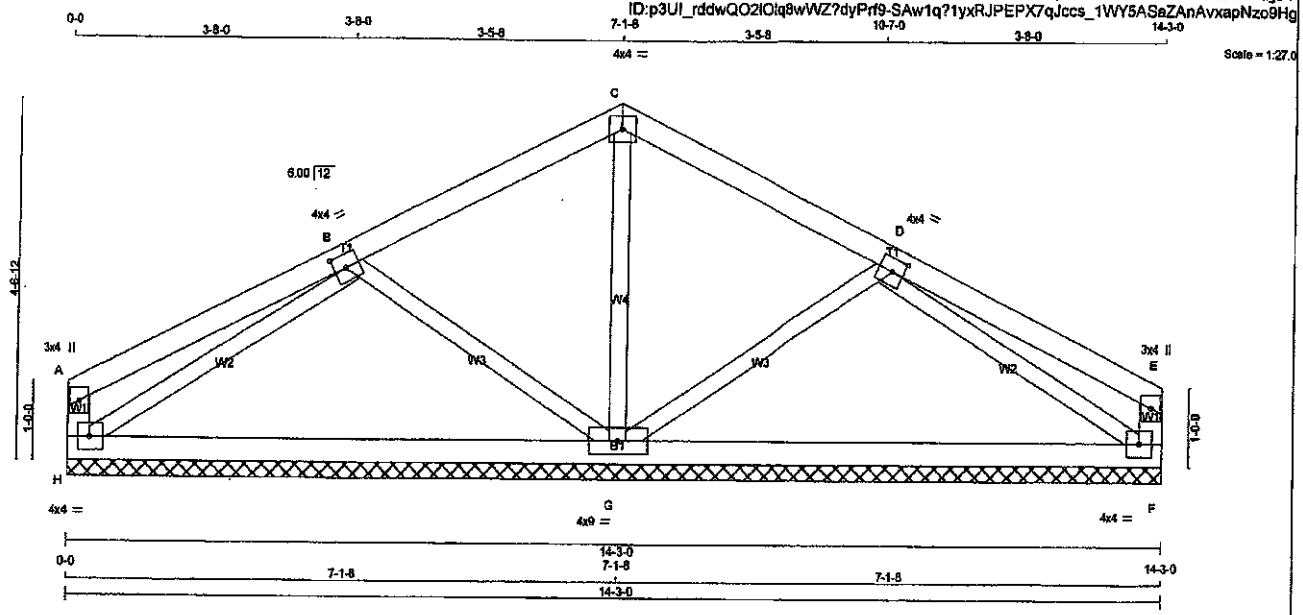

TOTAL WEIGHT = 14 X 52 = 733 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTW-p	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1272	0	1272	0	0	14-3-0	14-3-0
H	366	0	366	0	0	14-3-0	14-3-0
F	366	0	366	0	0	14-3-0	14-3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	949	528 / 0	187 / 0	0 / 0	0 / 0	234 / 0	0 / 0
H	274	150 / 0	56 / 0	0 / 0	0 / 0	68 / 0	0 / 0
F	274	150 / 0	56 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-102.1	-102.1	0.22 (1)	10.00	G- C	-442 / 0	0.13 (1)
B- C	0 / 169	-102.1	-102.1	0.24 (1)	10.00	G- D	-425 / 0	0.14 (1)
C- D	0 / 169	-102.1	-102.1	0.24 (1)	10.00	B- G	-425 / 0	0.14 (1)
D- E	0 / 20	-102.1	-102.1	0.22 (1)	10.00	H- B	-219 / 0	0.07 (1)
H- A	-138 / 0	0.0	0.0	0.01 (1)	7.81	D- F	-219 / 0	0.07 (1)
F- E	-138 / 0	0.0	0.0	0.01 (1)	7.81			
H- G	0 / 180	-38.5	-38.5	0.45 (3)	10.00			
G- F	0 / 180	-38.5	-38.5	0.45 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1) , BC=0.45/1.00 (F-G:3) , WB=0.14/1.00 (D-G:1) , SSI=0.18/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.13 (C) (INPUT = 1.00)

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
02/05/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 17902700
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	PB1G	2	1	TRUSS DESC.	

Tamarack Roof Trues, Burlington

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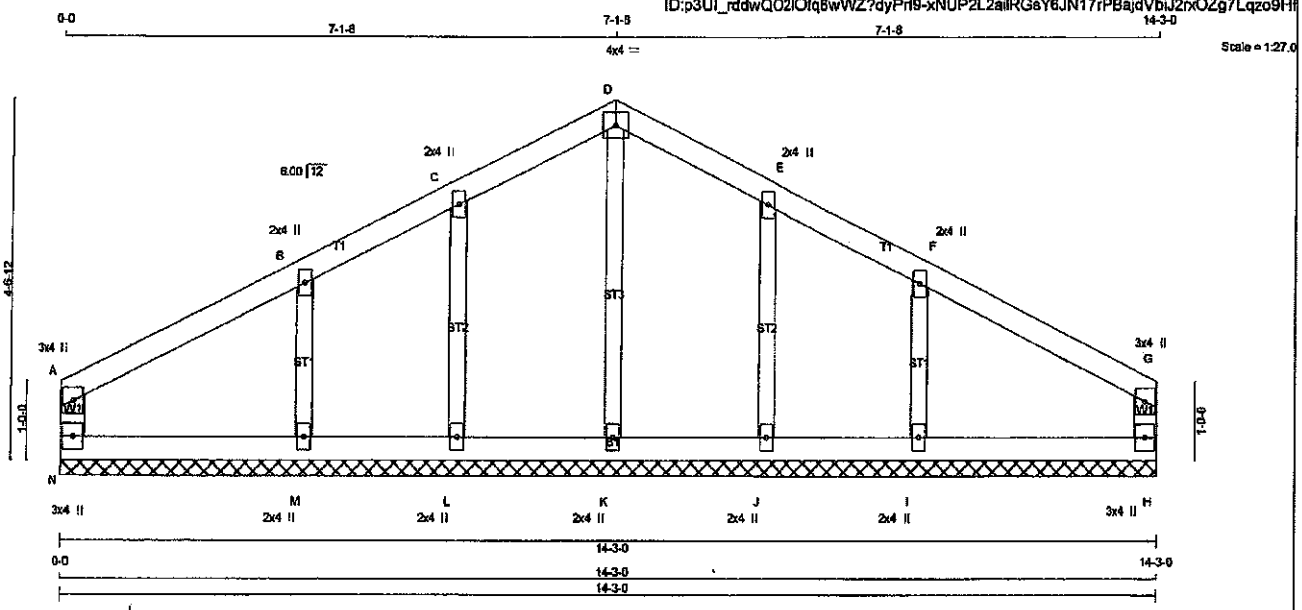

TOTAL WEIGHT = 2 X 47 = 95 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMV+p	MT20	3.0	4.0	
B, C, E, F					
B	TMW+w	MT20	2.0	4.0	
D	TTW-p	MT20	4.0	4.0	
G	TMV+p	MT20	3.0	4.0	
H	BMV1+p	MT20	3.0	4.0	
I, J, K, L, M					
I	BMW1+w	MT20	2.0	4.0	
N	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
N-A	-213 / 0	0.0	0.0	0.08 (1)		7.81
A-B	-130 / 0	-102.1	-102.1	0.09 (1)		6.25
B-C	-131 / 0	-102.1	-102.1	0.09 (1)		6.25
C-D	-114 / 0	-102.1	-102.1	0.06 (1)		6.25
D-E	-114 / 0	-102.1	-102.1	0.06 (1)		6.25
E-F	-131 / 0	-102.1	-102.1	0.09 (1)		6.25
F-G	-130 / 0	-102.1	-102.1	0.09 (1)		6.25
H-G	-213 / 0	0.0	0.0	0.08 (1)		7.81
N-M	0 / 117	-36.5	-36.5	0.06 (2)		10.00
M-L	0 / 109	-36.5	-36.5	0.06 (2)		10.00
L-K	0 / 105	-36.5	-36.5	0.04 (2)		10.00
K-J	0 / 105	-36.5	-36.5	0.04 (2)		10.00
J-I	0 / 109	-36.5	-36.5	0.06 (2)		10.00
I-H	0 / 117	-36.5	-36.5	0.06 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-D	-122 / 11	0.04 (1)
L-C	-176 / 0	0.04 (1)
M-B	-278 / 0	0.04 (1)
J-E	-176 / 0	0.04 (1)
I-F	-278 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (F-G:1) , BC=0.08/1.00 (H-I:2) , WB=0.04/1.00 (F-I:1) , SSI=0.11/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/05/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T1902701
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	G1	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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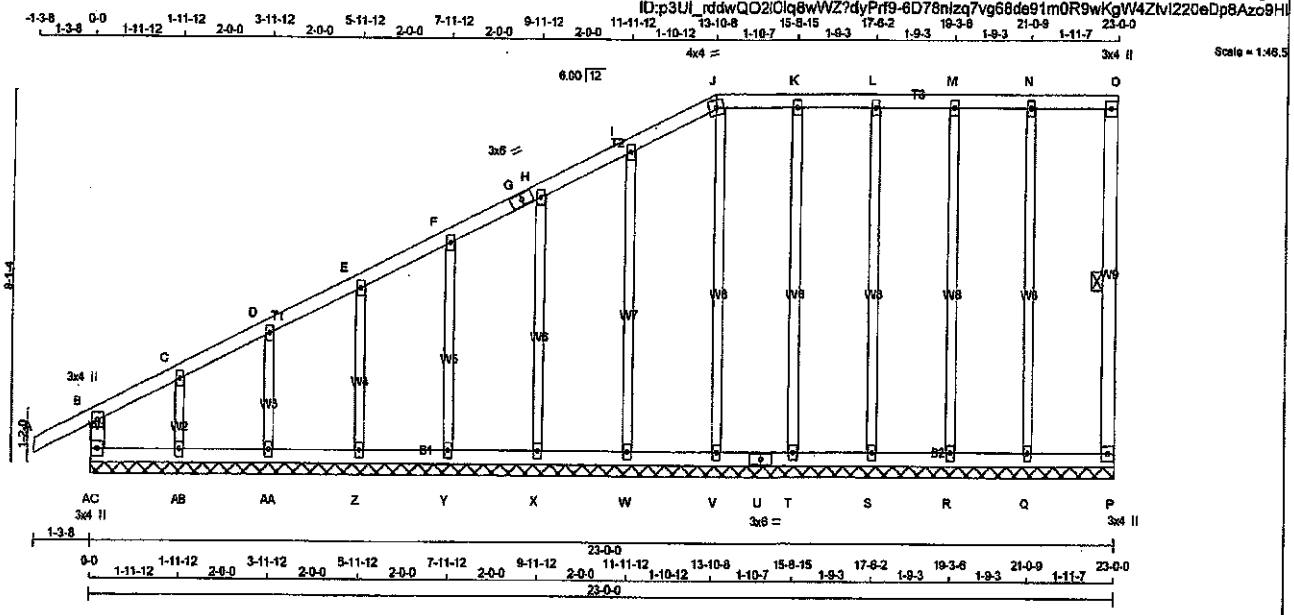

TOTAL WEIGHT = 3 X 116 = 348 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AC- B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AC- U	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, K, L, M, N						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
J	TTW-m	MT20	4.0	4.0		
O	TMV+p	MT20	3.0	4.0		
P	BMV1+p	MT20	3.0	4.0		
Q, R, S, T, V, W, X, Y, Z, AA, AB						
Q	BMW1+w	MT20	2.0	4.0		
U	BS-t	MT20	3.0	6.0		
AC	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AC- B	-250 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.00
B- C	-30 / 0	-102.1	-102.1	0.04 (1)	6.25
C- D	-22 / 0	-102.1	-102.1	0.05 (1)	6.25
D- E	-17 / 0	-102.1	-102.1	0.05 (1)	6.25
E- F	-13 / 0	-102.1	-102.1	0.05 (1)	6.25
F- G	-10 / 0	-102.1	-102.1	0.05 (1)	6.25
G- H	-10 / 0	-102.1	-102.1	0.05 (1)	6.25
H- I	-8 / 0	-102.1	-102.1	0.05 (1)	10.00
I- J	-6 / 0	-102.1	-102.1	0.05 (1)	10.00
J- K	-2 / 0	-102.1	-102.1	0.04 (1)	10.00
K- L	-2 / 0	-102.1	-102.1	0.04 (1)	10.00
L- M	-2 / 0	-102.1	-102.1	0.04 (1)	10.00
M- N	-2 / 0	-102.1	-102.1	0.04 (1)	10.00
N- O	-2 / 0	-102.1	-102.1	0.04 (1)	10.00
P- O	-90 / 0	0.0	0.0	0.02 (1)	6.25
AC-AB	0 / 27	-38.5	-38.5	0.03 (3)	10.00
AB-AA	0 / 20	-38.5	-38.5	0.03 (3)	10.00
AA-Z	0 / 15	-38.5	-38.5	0.02 (2)	10.00
Z-Y	0 / 12	-38.5	-38.5	0.02 (3)	10.00
Y-X	0 / 9	-38.5	-38.5	0.02 (3)	10.00
X- W	0 / 7	-38.5	-38.5	0.02 (3)	10.00
W- V	0 / 5	-38.5	-38.5	0.02 (3)	10.00
V- U	0 / 2	-38.5	-38.5	0.02 (3)	10.00
U- T	0 / 2	-38.5	-38.5	0.02 (3)	10.00
T- S	0 / 2	-38.5	-38.5	0.02 (3)	10.00
S- R	0 / 2	-38.5	-38.5	0.02 (3)	10.00
R- Q	0 / 2	-38.5	-38.5	0.03 (3)	10.00
Q- P	0 / 2	-38.5	-38.5	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- J	-193 / 0	0.24 (1)
Q- N	-201 / 0	0.26 (1)
T- K	-185 / 0	0.23 (1)
R- M	-179 / 0	0.23 (1)
S- L	-179 / 0	0.23 (1)
AB- C	-196 / 0	0.03 (1)
AA- D	-203 / 0	0.04 (1)
Z- E	-202 / 0	0.05 (1)
Y- F	-203 / 0	0.08 (1)
X- H	-203 / 0	0.12 (1)
W- I	-202 / 0	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1) , BC=0.03/1.00 (AB-AC:3) , WB=0.26/1.00 (N-Q:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.08 (H) (INPUT = 1.00)

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
06/05/19
H. J. G. ALVES
100099
PROVINCE OF ONTARIO

DWG NO. TAM 190269
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	G2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 5 15:58:54 2019 Page 1

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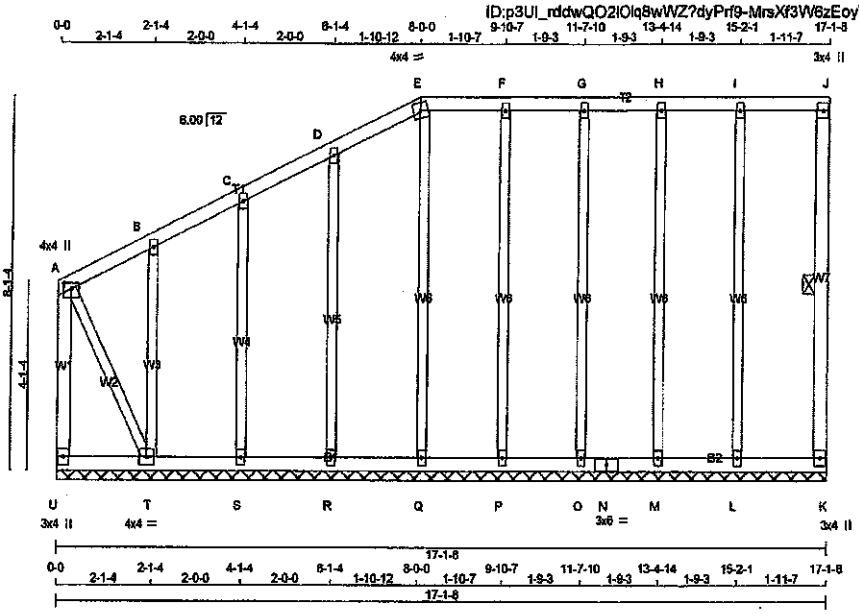

TOTAL WEIGHT = 101 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
U - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
U - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50	2.00
B, C, D, F, G, H, I						
B	TMW+w	MT20	2.0	4.0		
E	TTW-m	MT20	4.0	4.0		
J	TMV+p	MT20	3.0	4.0		
K	BMV1+p	MT20	3.0	4.0		
L, M, O, P, Q, R, S						
L	BMW1+w	MT20	2.0	4.0		
N	BS-t	MT20	3.0	6.0		
T	BMWW1-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
U- A	-108 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	-2 / 0	-102.1	-102.1	0.06 (1)	10.00	
B- C	-9 / 0	-102.1	-102.1	0.06 (1)	10.00	
C- D	-3 / 0	-102.1	-102.1	0.05 (1)	10.00	
D- E	-11 / 0	-102.1	-102.1	0.05 (1)	6.25	
E- F	0 / 0	-102.1	-102.1	0.05 (1)	10.00	
F- G	0 / 0	-102.1	-102.1	0.05 (1)	10.00	
G- H	0 / 0	-102.1	-102.1	0.04 (1)	10.00	
H- I	0 / 0	-102.1	-102.1	0.04 (1)	10.00	
I- J	0 / 0	-102.1	-102.1	0.05 (1)	10.00	
K- J	-83 / 0	0.0	0.0	0.02 (1)	6.25	
U- T	0 / 0	-38.5	-38.5	0.03 (3)	10.00	
T- S	0 / 6	-38.5	-38.5	0.03 (3)	10.00	
S- R	0 / 4	-38.5	-38.5	0.02 (3)	10.00	
R- Q	0 / 2	-38.5	-38.5	0.02 (3)	10.00	
Q- P	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
P- O	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
O- N	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
N- M	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
M- L	0 / 0	-38.5	-38.5	0.03 (3)	10.00	
L- K	0 / 0	-38.5	-38.5	0.03 (3)	10.00	

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
Q- E	-157 / 0	0.20 (1)
A- T	0 / 20	0.00 (1)
T- B	-230 / 0	0.09 (1)
S- C	-195 / 0	0.12 (1)
R- D	-225 / 0	0.20 (1)
L- I	-210 / 0	0.27 (1)
P- F	-207 / 0	0.26 (1)
M- H	-177 / 0	0.22 (1)
O- G	-176 / 0	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (B-C:1) , BC=0.03/1.00 (T-U:3) , WB=0.27/1.00 (I-L:1) , SSI=0.09/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
Feb/05/19
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

DWG NO. TAM T1902699
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	J1	22	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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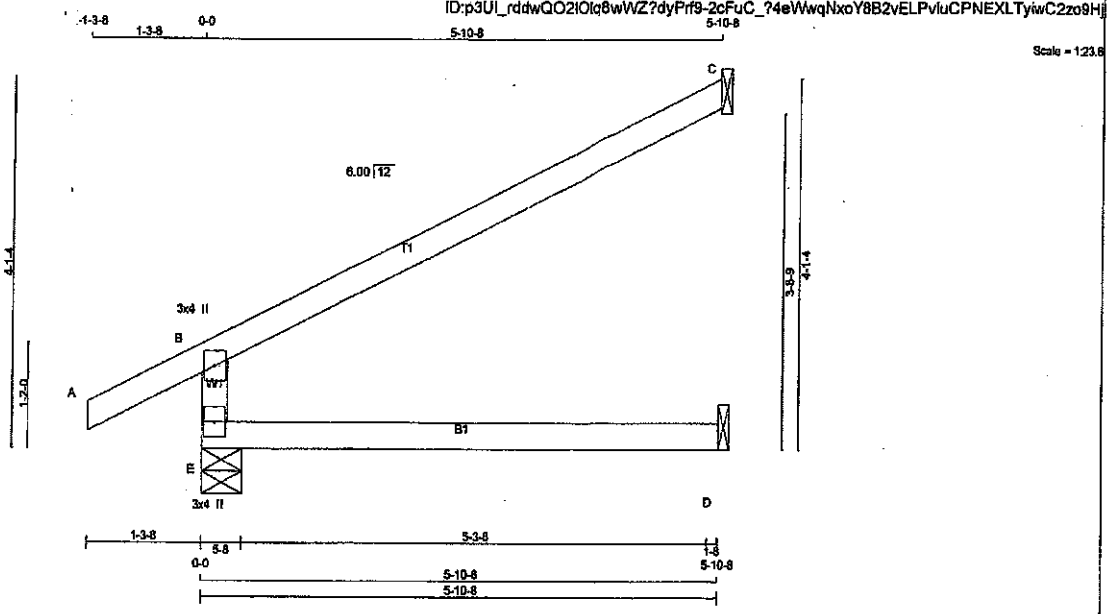


TOTAL WEIGHT = 22 X 17 = 369 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	646	0	646	0	0	5-8	5-8
C	225	0	225	0	0	1-8	1-8
D	93	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	473	292 / 0	72 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	65	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

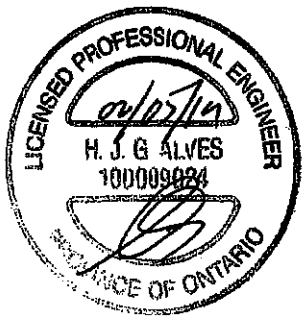
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-513 / 0	0.0	0.0	0.22 (3)	7.81			
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00			
B-C	-34 / 0	-102.1	-102.1	0.60 (1)	6.25			
E-D	0 / 0	-38.5	-38.5	0.22 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 942 (0.07")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.22/1.00 (D-E:3) , WB=0.00/1.00 (n/a:0) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

DWG NO. TAM 1190202
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	J3	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 5 15:56:49 2019 Page 1

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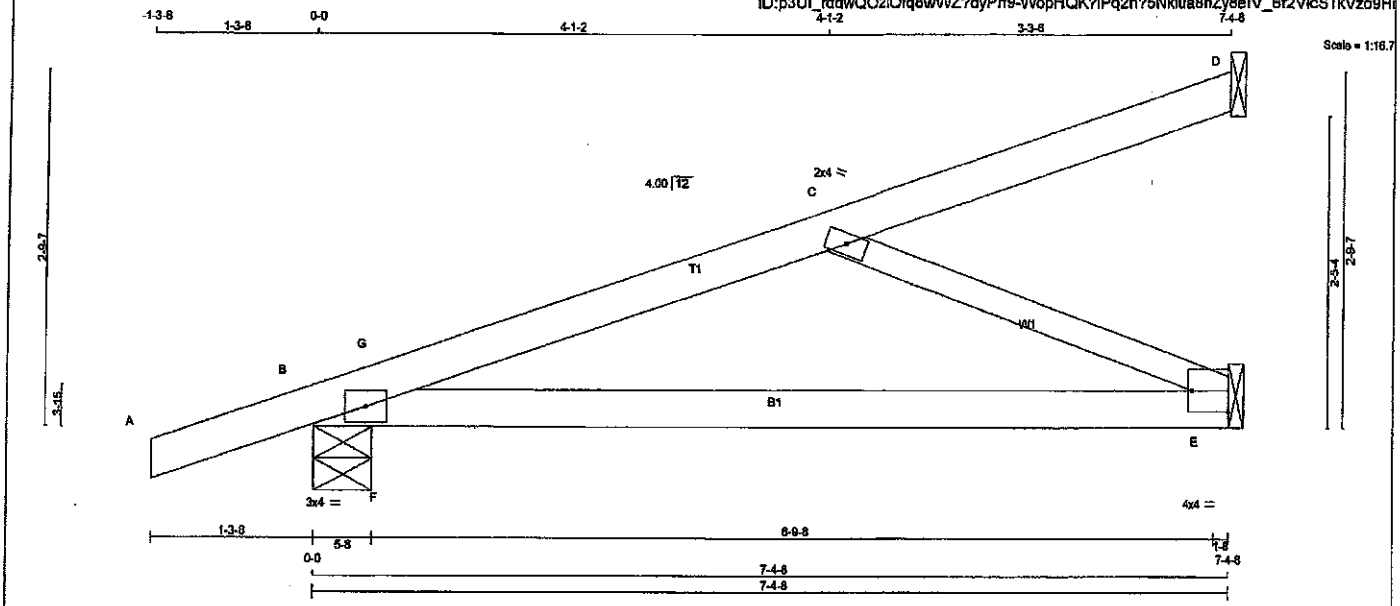

TOTAL WEIGHT = 10 X 22 = 218 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
E	BMW1-t	MT20	4.0	4.0	2.00	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	99	0	99	0	-5	1-8	1-8
B	648	0	648	0	0	5-8	5-8
E	422	0	422	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) D , E

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	64	66 / 0	0 / -9	0 / 0	0 / 0	7 / 0	0 / 0
B	474	289 / 0	76 / 0	0 / 0	0 / 0	110 / 0	0 / 0
E	325	151 / 0	85 / 0	0 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13 (1)		10.00
B- G	-787 / 0	-102.1	-102.1	0.24 (3)		6.25
G- C	-837 / 0	-102.1	-102.1	0.33 (2)		6.25
C- D	-20 / 0	-102.1	-102.1	0.21 (1)		6.25
B- F	0 / 636	-38.5	-38.5	0.23 (3)		10.00
F- E	0 / 636	-38.5	-38.5	0.39 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- E	-704 / 0	0.18 (1)
F- G	0 / 411	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/ 820 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/ 488 (0.18")

CSI: TC=0.33/1.00 (C-G:2) , BC=0.39/1.00 (E-F:2) , WB=0.18/1.00 (C-E:1) , SSI=0.28/1.00 (B-G:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (FSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (C) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 1.00)

DWG NO. TAM 19402703
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 2	DRWG NO.
200170-400367	J4	7	1	TRUSS DESC.	

Tamarack Roof Trusses, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 5 15:56:50 2019 Page 1
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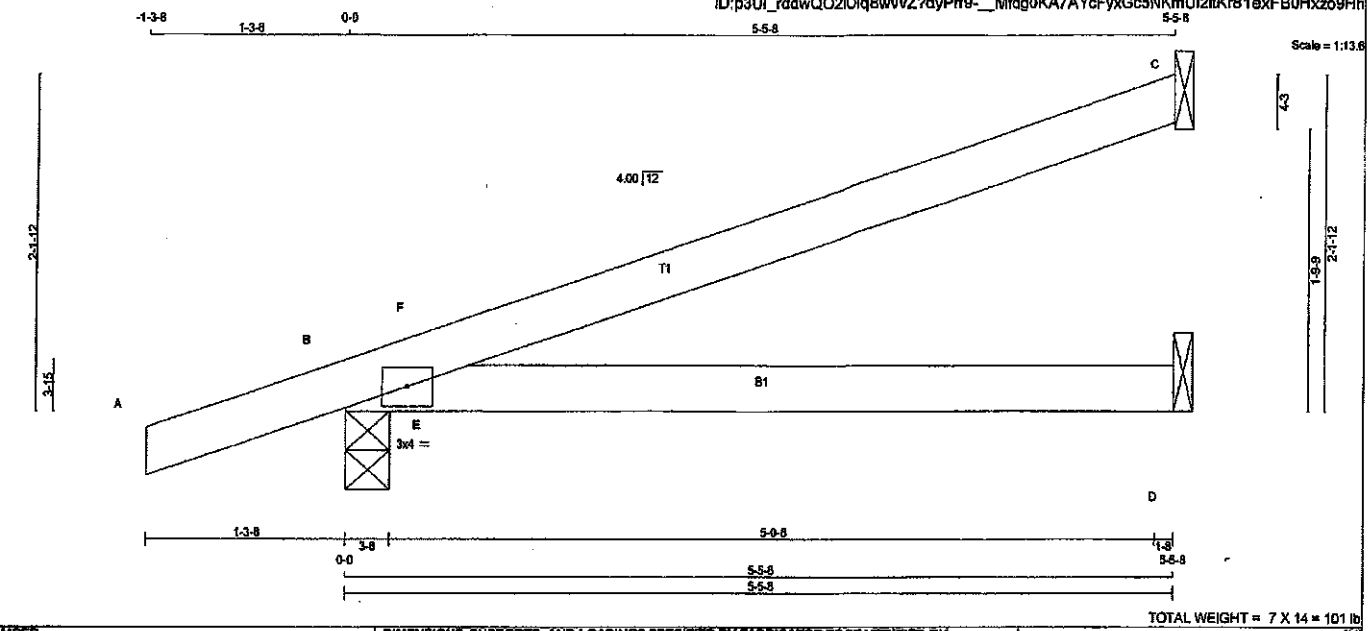


TOTAL WEIGHT = 7 X 14 = 101 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	250	0	250	0	0	1-8	1-8
B	520	0	520	0	0	3-8	3-8
D	133	0	145	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	380	236 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00		E-F	-254 / 122	0.00 (1)
B-F	-51 / 0	-102.1	-102.1	0.13 (3)	6.25				
F-C	0 / 6	-102.1	-102.1	0.42 (1)	10.00				
B-E	0 / 0	-38.5	-38.5	0.26 (1)	10.00				
E-D	0 / 0	-38.5	-38.5	0.31 (1)	10.00				

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 842 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 489 (0.13")

CSI: TC=0.42/1.00 (C-F:1) , BC=0.31/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.98)
JSI METAL= 0.09 (B) (INPUT = 1.00)

DWG NO. TAM D902704
STRUCTURAL
COMPONENT ONLY

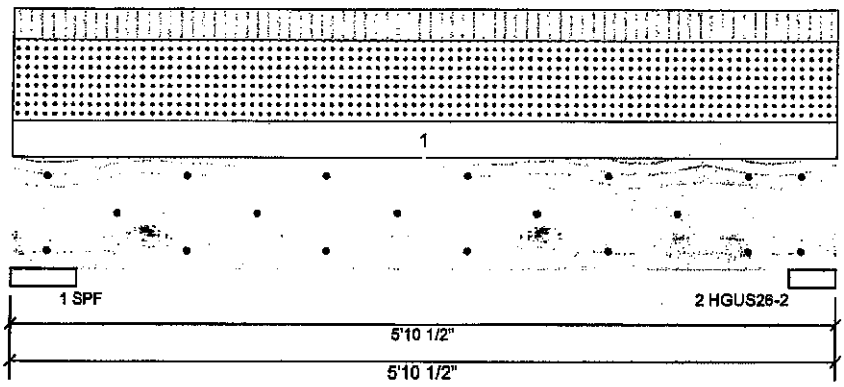
isDesign™

Client: Greenpark Homes
Project: Preston 2
Address: Caledon

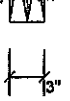
Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200170

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE



Member Information

Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	231	273	609	0
2	221	262	584	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	15%	341 / 1145	1486	L	1.25D+1.5S +L
2 - HGUS...	4.000"	19%	327 / 1097	1424	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1679 ft-lb	3'	6039 ft-lb	0.278 (28%)	1.25D+1.5S +L	L
Unbraced	1679 ft-lb	3'	5236 ft-lb	0.321 (32%)	1.25D+1.5S +L	L
Shear	1290 lb	1'2"	3984 lb	0.324 (32%)	1.25D+1.5S +L	L
Perm Defl in.	0.006 (L/11306)	3'	0.174 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.015 (L/4260)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.020 (L/3094)	3'	0.174 (L/360)	0.120 (12%)	D+S+0.5L	L

Design Notes

1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
02/05/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 17902705
STRUCTURAL COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	D+S	Live	Snow	Wind	Comments
1	Uniform		7-0-0	Near Face	73 PSF	41 PSF	29 PSF	0 PSF	

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

This design is valid until 2021-12-11

Manufacturer Info

TAMARACK LUMBER
3255 NORTH SERVICE RD , ON
CANADA
(905) 335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

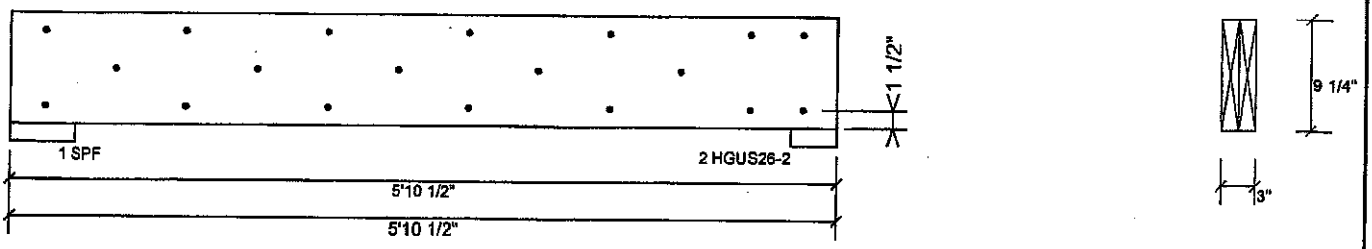
Version 18.80.245 Powered by iStruct™

isDesign™

Client:	Greenpark Homes	Date:	2019-02-05
Project:	Preston 2	Designer:	Brian
Address:	Caledon	Job Name:	Lamberts Lane Homes Corp.
		Project #:	200170

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

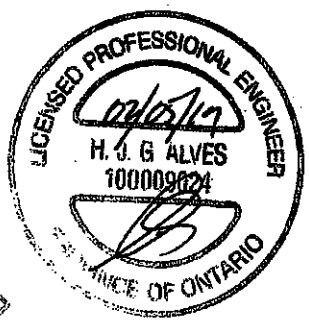
Level: Level

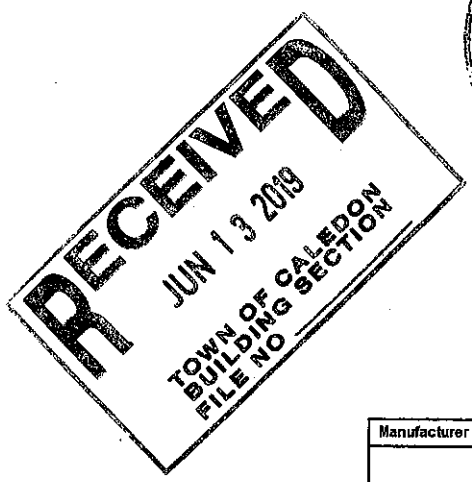


Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	64.6 %
Load	247.6 PLF
Yield Limit per Foot	383.4 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00





DWG NO. TAM T190 2705
STRUCTURAL
COMPONENT ONLY
2/2

Manufacturer Info	TAMARACK LUMBER 3255 NORTH SERVICE RD , ON CANADA (905) 335-1115

This design is valid until 2021-12-11

Version 18.80.245 Powered by iStruct™

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

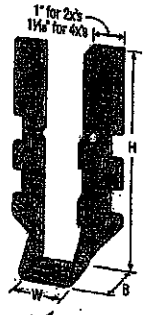
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

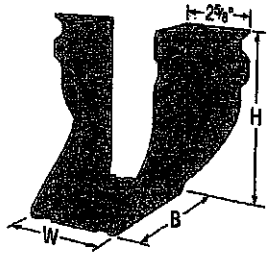
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



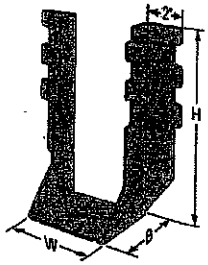
LUS28

LU26L

HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2

Double-Shear Nailing Top View

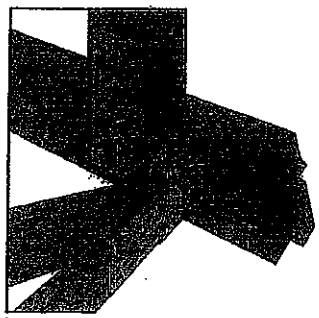
Double-Shear Nailing Side View; Do not bend tab

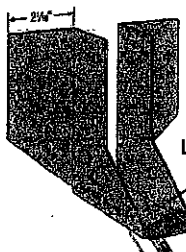
Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

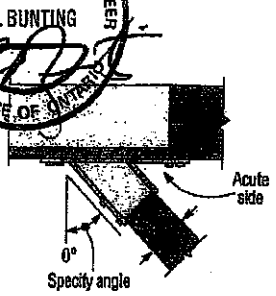
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance S-P-F Uplift (Kp = 1.15) lb. / kN	S-P-F Normal (Kp = 1.00) lb. / kN
Single 2x Sizes									
LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¼	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅜	5⅛	3	3⅛	(14) 16d	(6) 16d	2705 / 12.03	4940 / 21.98	2065 / 9.20	3875 / 17.24
LUS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅜	5⅛	5	4⅛	(20) 16d	(8) 16d	2685 / 11.94	6625 / 29.47	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2620 / 11.65	1290 / 5.74	1790 / 7.96
HUS28	16	1⅜	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.03	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅜	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.72	7675 / 34.14	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 16d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7¹⁵⁄₁₆	1¾	3¾	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

SIMPSON Strong-Tie

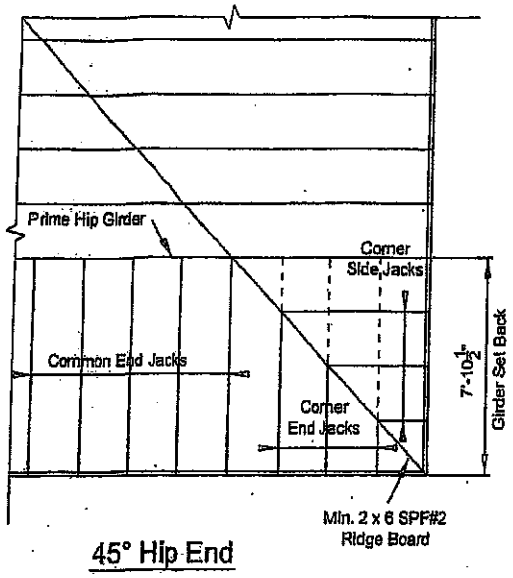
Face-Mount Hangers

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_p[3]	Fasteners Header	Joist	Factored Resistance Normal lb.	Normal kN	S-P-F Uplift (K_D = 1.15) lb.	kN	Normal (K_D = 1.00) lb.	kN
Double 2x Sizes													
■ LUS24-2	18	3⅛	8⅛	2	1⅛	(4) 16d	(2) 16d	835	2020	590	2.62	1435	6.38
SS ■ LUS26-2	18	3⅛	4⅛	2	4	(4) 16d	(4) 16d	1720	2595	1545	6.87	1920	8.54
■ HHUS26-2	14	3⅝	6⅛	3	3⅛	(14) 16d	(6) 16d	2850	7335	2065	9.20	5205	23.15
HGUS26-2	12	3⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	13.83	6355	28.27
SS LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	6.87	2575	11.45
■ HHUS28-2	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3785	8940	2675	11.90	6345	28.22
HGUS28-2	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	19.17	9215	40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	10.32	3195	14.21
■ HHUS210-2	14	3⅝	9⅛	3	8	(30) 16d	(10) 16d	4670	9660	4235	18.84	7000	31.14
HGUS210-2	12	3⅝	9⅛	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	21.60	10270	45.69
Triple 2x Sizes													
HGUS26-3	12	4⅝	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	13.83	6355	28.27
HGUS28-3	12	4⅝	7¼	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	19.17	9215	40.99
■ HHUS210-3	14	4⅝	9	3	7⅛	(30) 16d	(10) 16d	4670	9670	4235	18.84	6865	30.54
HGUS210-3	12	4⅝	9¼	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	21.60	10400	46.26
Quadruple 2x Sizes													
HGUS26-4	12	6⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	13.83	6355	28.27
HGUS28-4	12	6⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	19.17	9215	40.99
■ HHUS210-4	14	6⅝	8⅞	3	7⅛	(36) 16d	(10) 16d	4670	10155	4235	18.84	7210	32.07
HGUS210-4	12	6⅝	9⅛	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	21.60	10400	46.26
HGUS212-4	12	6⅝	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	24.13	10645	47.35
HGUS214-4	12	6⅝	12⅝	4	11⅞	(66) 16d	(22) 16d	10130	16400	7195	32.00	11645	51.80
4x Sizes													
■ LUS46	18	3⅝	4¼	2	3⅞	(4) 16d	(4) 16d	1720	2595	1545	6.87	1920	8.54
■ HHUS46	14	3⅝	5¼	3	3⅛	(14) 16d	(6) 16d	2540	7335	2065	9.20	5205	23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	13.83	6355	28.27
■ LUS48	18	3⅝	6¼	2	3⅞	(6) 16d	(4) 16d	1720	3325	1545	6.87	2575	11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3785	8940	2675	11.90	6345	28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	19.17	9215	40.99
■ LUS410	18	3⅝	8¼	2	5⅞	(6) 16d	(6) 16d	2580	4500	2320	10.32	3195	14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	21.60	10270	45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	24.13	10645	47.35
HGUS414	12	3⅝	12⅝	4	11⅞	(66) 16d	(22) 16d	10130	16400	7195	32.00	11645	51.80

See footnotes on p. 258.



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD:		7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

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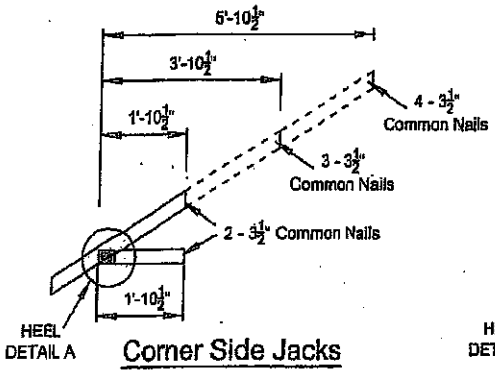
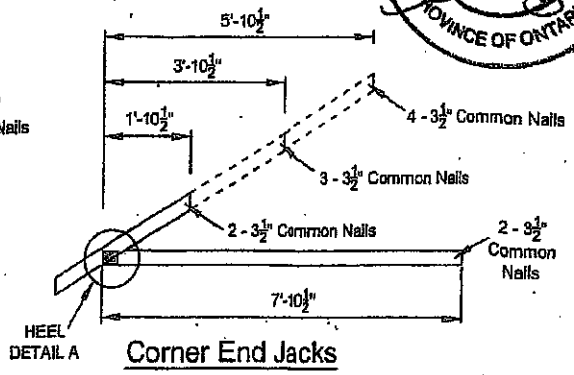
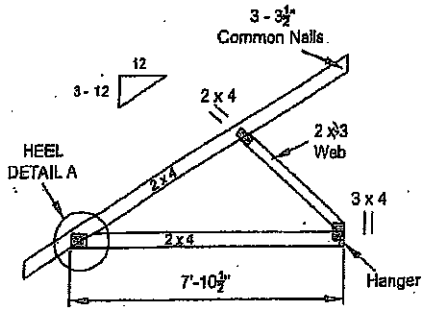


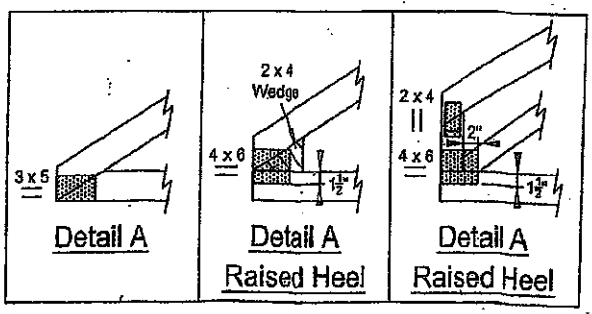
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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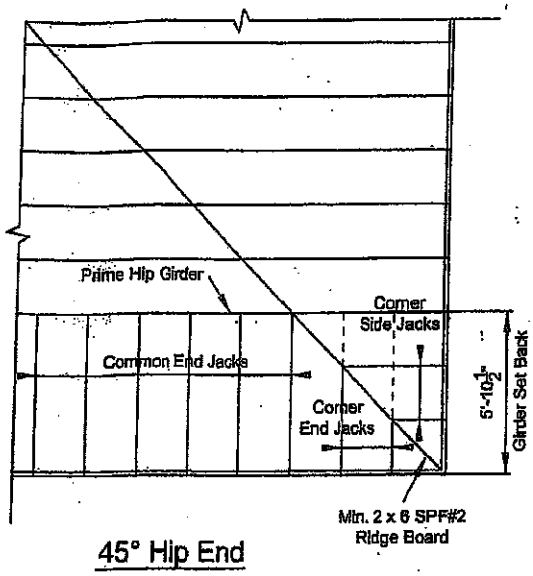

45° Hip End

LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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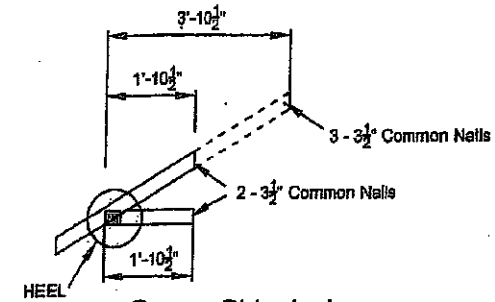
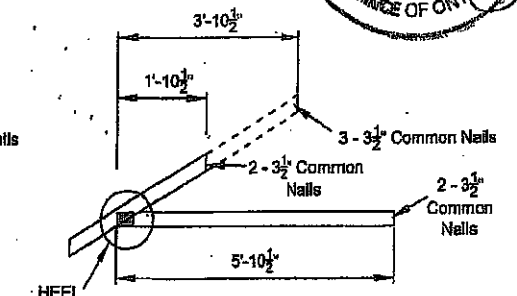
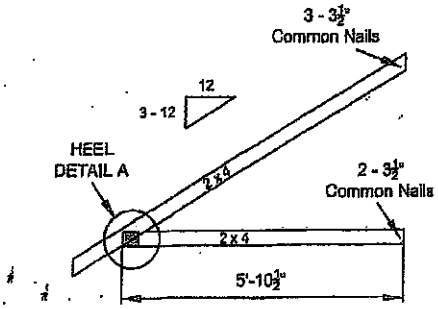


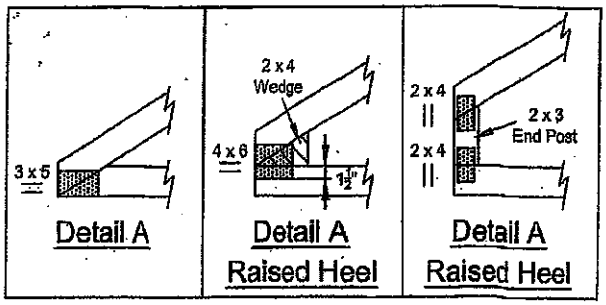
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

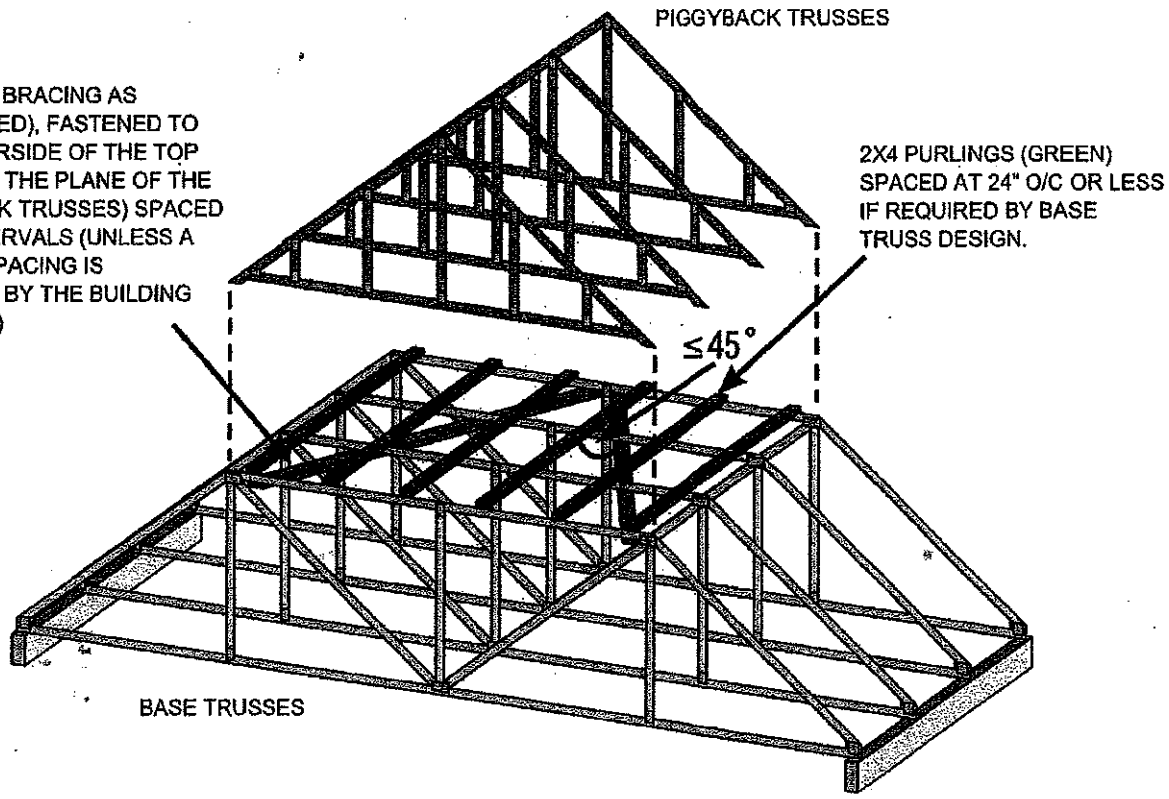
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



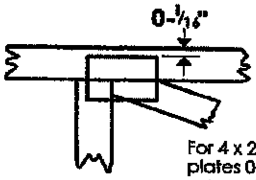
NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

= This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4 — The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

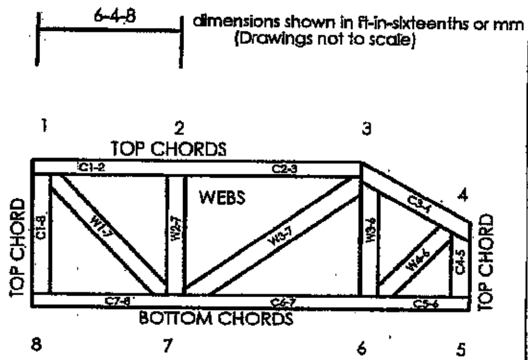
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

LICENSED PROFESSIONAL ENGINEER
06/11/19
H.J.G. ALVES
PROVINCE OF ONTARIO

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218 Feb 09, 201

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