

43-06-00

2-09-00

4-07-00

** NAIL 2-2X10 BESIDE G10 FOR
CONTINUOUS BEARING SUPPORT

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

GSL 37.6 PSF

DESIGN CONFORMS
WITH O.B.C. 2018 PART 9
DESIGN LOADS:

TC LIVE 29 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

BM1,2= 2- 2x10

ALL CONV. FRAMINGS TO
CONFORM WITH PART 9 OF
O.B.C. 2012 ROOF RAFTERS THAT
CROSS MEET OVER TRUSSES TO
BE 2x4 SPF @ 24" O.C. WITH A 2x4
VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS
PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO
THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF
BRACING DOES NOT EXCEED 6'

STRUCTURE MUST BE
STRUCTURED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

33-03-00

24-03-00

RECEIVED
JUN 25 2019

TOWN OF CALEDON
BUILDING DEPARTMENT

Job Track: 50033

Plan Log: 200666

Layout ID: 401828

GREEN PARK HOMES / CALEDON

Project: LAMBERT LANE PH.2

Date: 2019-03-21

Sales: Mario DiCano

Designer: Chris/
JC

Model / Elevation:

KINMOUNT 11B / 2

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE.

Mitek ver 2.3/229



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: KINMOUNT 11B
 Lot #:
 Elevation: 2

Job Track: 50120
 PlanLog: 200659
 Layout ID: 401812
 Ref #
 Page: 1 of 3
 Date: 03/13/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1Z2 Hip Girder	9/12	32-05-00	4-08-13	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	382.78 224.67		
	1 2-ply	T1Z Hip Girder	9/12	32-05-00	4-08-13	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	382.78 224.67		
	2	T2 Hip	9/12	32-05-00	5-10-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	284.98 186.00		
	2	T3 Hip	9/12	32-05-00	7-00-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	286.62 182.33		
	2	T4 Hip	9/12	32-05-00	8-02-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	303.91 191.33		
	3	T5 Hip	9/12	32-05-00	9-04-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	478.29 299.50		
	4	T5A Hip	9/12	32-00-00	9-04-09	2 x 4	1-03-08	1-06-04 1-06-04	626.45 392.00		
	1	T6 Hip	9/12	32-05-00	10-06-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	167.74 104.00		
	1	T6A Hip	9/12	32-00-00	10-06-09	2 x 4	1-03-08	1-06-04 1-06-04	164.91 103.33		
	3	T8S Scissor	9/12 4/12	12-06-00	6-02-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	170.08 113.00		
	4	T10 Common	9/12	23-07-00	9-11-14	2 x 4	1-03-08	9-04 1-06-04	436.07 272.67		
	1	G10 GABLE	9/12	23-07-00	9-11-14	2 x 4	1-03-08	9-04 1-06-04	112.07 70.33		
	2	T14 Common	9/12	21-05-00	9-06-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	194.58 125.33		
	1	G14 GABLE	9/12	21-05-00	9-06-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	107.06 68.17		

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TOWN OF CALEDON
 BUILDING SECTION



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: KINMOUNT 11B
 Lot #:
 Elevation: 2

Job Track: 50120
 PlanLog: 200659
 Layout ID: 401812
 Ref #
 Page: 2 of 3
 Date: 03/13/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T17S Scissor	9/12 6/12	9-00-00	4-10-12	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	79.74 53.00		
	1 3-ply	T18 Common Girder	9/12	9-00-00	4-10-12	2 x 4 2 x 6		1-06-04 1-06-04	129.41 88.50		
	1	V10 Valley	9/12	23-03-09	8-08-13	2 x 4			93.83 60.50		
	1	V11 Valley	9/12	19-03-09	7-02-13	2 x 4			68.17 42.83		
	1	V12 Valley	9/12	15-03-09	5-08-13	2 x 4			52.77 35.00		
	1	V13 Valley	9/12	11-03-09	4-02-13	2 x 4			30.66 19.33		
	1	V14 Valley	9/12	7-03-09	2-08-13	2 x 4			19.18 13.50		
	2	PB1 Piggyback	9/12	11-05-09	2-04-00	2 x 4			66.31 43.33		
	2	PB2 Piggyback	9/12	11-05-09	3-06-00	2 x 4			68.93 44.67		
	1	PB3 Piggyback	9/12	11-05-09	4-03-09	2 x 4			30.72 20.00		
	9	J7 Jack-Open	7/12	5-10-08	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	167.53 105.00		
	5	J8 Jack-Open	8/12	4-01-08	3-11-09	2 x 4	1-03-08	4-07 3-01-07	64.24 43.33		

TOTAL # TRUSS= 59

TOTAL BFT OF ALL TRUSSES= 3126.32

BFT.

TOTAL WEIGHT OF ALL TRUSSES= 4969.8 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS28DS	
2	Hardware	HGUS26-2	

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TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
Builder: GREEN PARK HOMES
Project: Lamberts Lane Home Corp.
Location: Caledon
Model: KINMOUNT 11B
Lot #:
Elevation: 2

Job Track: 50120
PlanLog: 200659
Layout ID: 401812
Ref #
Page: 3 of 3
Date: 03/13/2019
Designer: Jane Gong
Sales Rep: Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
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TOTAL NUMBER OF ITEMS= 12

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TOWN OF CALEDON
BUILDING SECTION
FILE NO. _____

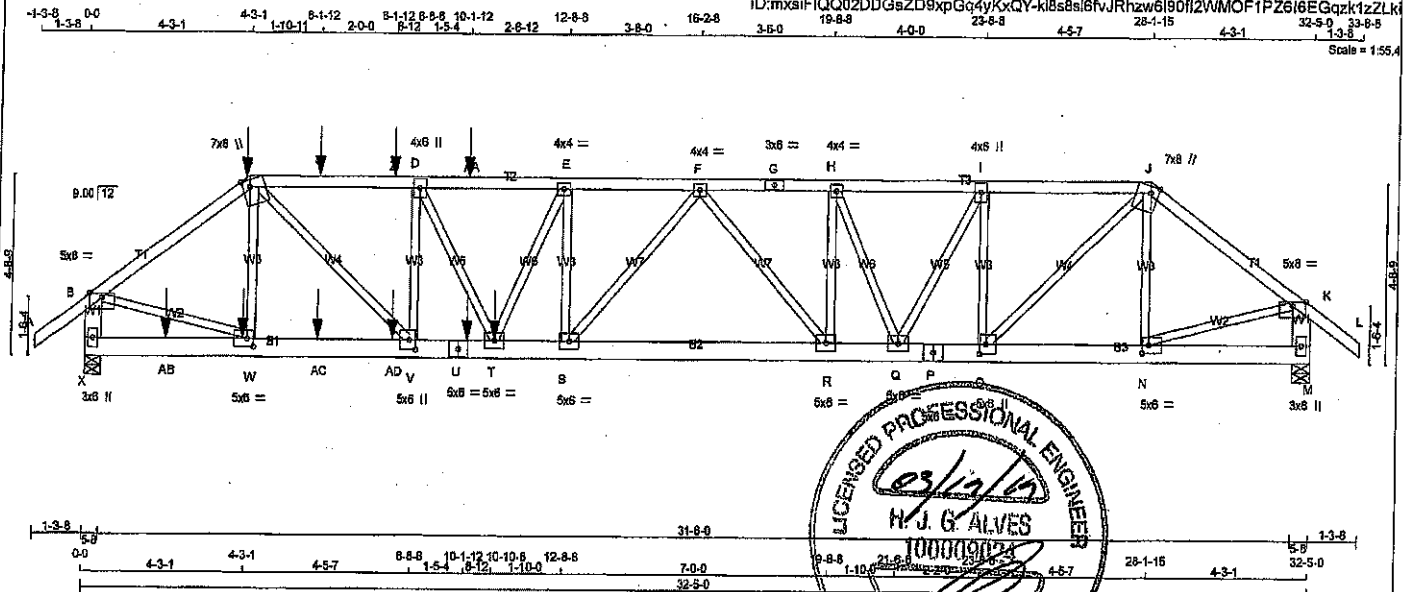
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	T122	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:mxsiFIQQ02DDGaZD9xpGq4yKxQY-k18s8s16VJRhw6190112VMOF1PZ61EGqzk1zZLk

Scale = 1:55.4



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE	LOAD(PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
A - C	1	12	SIDE(61.0)
C - G	1	12	SIDE(61.0)
G - J	1	12	TOP
J - L	1	12	TOP
X - B	2	12	TOP
M - K	2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
X - U	2	12	SIDE(183.1)
U - P	2	12	SIDE(0.0)
P - M	2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS			
W - C	1	6	SIDE(24.1)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (tablets in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMVW-p	MT20	5.0	8.0	Edge
C TTVW+m	MT20	7.0	8.0	Edge 2.50
D TMVW+H	MT20	4.0	8.0	
E, F, H				
G TMVW-I	MT20	4.0	4.0	
I TS-I	MT20	3.0	6.0	
J TMVW+I	MT20	4.0	6.0	
K TTVW+m	MT20	7.0	8.0	Edge 2.50
L TMVW-p	MT20	5.0	8.0	Edge
M BMVW+I	MT20	3.0	6.0	
N BMVW+I	MT22	5.0	6.0	2.50 2.00
O BMVW+I	MT20	5.0	6.0	3.00 2.00
P BS-I	MT20	5.0	6.0	
Q, R, S, T				
U BMVW-I	MT20	5.0	8.0	
V BS-I	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT GROSS REACTION				
VERT				
HORIZ				
X 5018	0	5018	0	5-8
M 3401	0	3401	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED		
X 3718	2147 / 0	674 / 0
M 2524	1450 / 0	464 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING	CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
TOTAL LOAD CASES: (4)					
	MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
	FR-TO		FROM TO	FR-TO	
	A-B	0 / 42	-102.1 -102.1 0.08 (1)	W-C	-781 / 0
	B-C	-5663 / 0	-102.1 -102.1 0.34 (1)	C-V	0 / 4444
	C-Y	-7692 / 0	-102.1 -102.1 0.57 (1)	V-D	-2813 / 0
	Y-Z	-7692 / 0	-102.1 -102.1 0.57 (1)	D-T	0 / 1884
	Z-D	-7692 / 0	-102.1 -102.1 0.57 (1)	T-E	-90 / 0
	D-AA	-8534 / 0	-102.1 -102.1 0.59 (1)	E-F	-328 / 27
	AA-E	-8534 / 0	-102.1 -102.1 0.59 (1)	F-R	0 / 1052
	E-F	-8580 / 0	-102.1 -102.1 0.35 (1)	R-H	-1510 / 0
	F-G	-6951 / 0	-102.1 -102.1 0.26 (1)	H-Q	0 / 1378
	G-H	-6951 / 0	-102.1 -102.1 0.26 (1)	Q-I	-1891 / 0
	H-I	-5215 / 0	-102.1 -102.1 0.30 (1)	I-J	0 / 1818
	I-J	-5357 / 0	-102.1 -102.1 0.27 (1)	J-K	-2185 / 0
	J-K	-3696 / 0	-102.1 -102.1 0.25 (1)	K-L	0 / 3371
	K-L	0 / 42	-102.1 -102.1 0.09 (1)	L-M	-549 / 0
	X-B	-4919 / 0	0.0 0.0 0.18 (1)	B-W	0 / 4585
	M-K	-3335 / 0	0.0 0.0 0.12 (1)	N-K	0 / 3045
	X-AB	0 / 0	-38.5 -38.5 0.06 (3)		
	AB-W	0 / 0	-38.5 -38.5 0.05 (3)		
	W-AC	0 / 4502	-38.5 -38.5 0.33 (1)		
	AC-AD	0 / 4502	-38.5 -38.5 0.33 (1)		
	AD-V	0 / 4502	-38.5 -38.5 0.39 (1)		
	V-U	0 / 7692	-38.5 -38.5 0.87 (1)		
	U-T	0 / 7692	-38.5 -38.5 0.87 (1)		
	T-S	0 / 8580	-38.5 -38.5 0.73 (1)		
	S-R	0 / 7900	-38.5 -38.5 0.58 (1)		
	R-Q	0 / 6961	-38.5 -38.5 0.48 (1)		
	Q-P	0 / 5358	-38.5 -38.5 0.39 (1)		
	P-O	0 / 5358	-38.5 -38.5 0.39 (1)		
	O-N	0 / 2837	-38.5 -38.5 0.22 (1)		
	N-M	0 / 0	-38.5 -38.5 0.04 (3)		

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
	C	4-3-1	-53	-59		FRONT	VERT	DEAD		
	C	4-3-1	-205	-205		FRONT	VERT	TOTAL		
	C	4-3-1	-303	-303		FRONT	VERT	SNOW		
	T	10-10-8	-2051	-2051		FRONT	VERT	TOTAL		
	U	10-1-12	-75	-95		FRONT	VERT	TOTAL		
	W	4-1-12	-75	-95		FRONT	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2016.

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2016
- CSA 086-14
- TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.59/1.00 (D-E:1), BC=0.73/1.00 (S-T:1),
WB=0.59/1.00 (B-W:1), SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (V) (INPUT = 0.90)
JSI METAL= 0.78 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME:	TRUSS NAME:	QUANTITY:	PLY:	JOB DESC:	DRWG NO.
401812	T1Z2	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 15:02:41 2019 Page 2
 ID:mxslFIQQ02DDG6ZD9xpG04yKxQY-kI8s8sl6fvJRhzW6180f12WMOF1P26I6EG0zk1zZ1k

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWWH	MT20	5.0	6.0	3.00	2.00
W	BMWWH	MT20	5.0	6.0	2.50	2.00
X	BMW1+p	MT20	3.0	6.0		

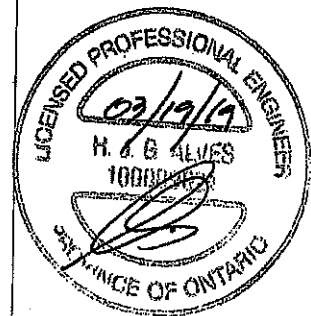
Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	6-1-12	-188	-188	---	FRONT	VERT	TOTAL	---	---
Z	8-1-12	-188	-188	---	FRONT	VERT	TOTAL	---	---
AA	10-1-12	-188	-188	---	FRONT	VERT	TOTAL	---	---
AB	2-1-12	-75	-85	---	FRONT	VERT	TOTAL	---	---
AC	6-1-12	-75	-85	---	FRONT	VERT	TOTAL	---	---
AD	8-1-12	-75	-85	---	FRONT	VERT	TOTAL	---	---

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 JUN 25 2019

TOWN OF CALEDON
 BUILDING SECTION
 FILE NO. _____



DWG NO. TAM 11905717
 STRUCTURAL
 CALCULATION ONLY 2/2

Tamarack Roof Truss, Burlington

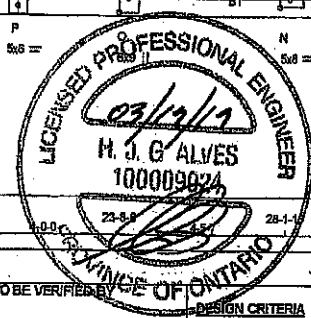
Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 19 14:03:22 2019 Page 1

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1-3-8 0-0 4-3-1 4-3-1 1-10-11 8-1-12 8-1-12 8-8 12-8-8 12-8-8 18-2-8 3-8-0 19-8-8 4-0-0 23-8-6 4-8-7 26-1-16 4-3-1 32-5-0 33-8-4

1-3-8 1-3-8 2-0-0 8-12 4-0-0 3-8-0 1-3-8

Scale = 1:55.4



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP . CH. LL = 28.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 8, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018
- CSA 069-14
- TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 26.0 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL(LL)= L/360 (1.08")
CALCULATED VERT. DEFL(LL) = L/999 (0.21")

ALLOWABLE DEFL(TL) = $L/280$ (1.08")
CALCULATED VERT. DEFL(TL) = $L/999$ (0.35")

CS: TC=0.71/1.00 (C-D:1), BC=0.68/1.00 (R-T:1),
WB=0.67/1.00 (B-U:1), SS=0.23/1.00 (C-D:1)

COMP=1.00 SHEAR=1.00 TENS= 1.00
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF

RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP (RY)	SHEAR SECTION (FS)	(FL)	(PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 798	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 6.0 Deg.

JSI GRIP = 0.60 (B) (INPUT = 0.90)
JSI METAL = 0.81 (S) (INPUT = 1.00)

JOB NAME 401811	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO. T1Z
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 19 14:03:22 2019 Page 2 ID:mxsIFIQQ02DDGsZD9xpGg4yKxQY-R7bPAvgppPvQC0L6JCf3ZXc8ibMZBTIF3C8KozZMcJ	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWH-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	8.0		
T	BMWWH	MT20	6.0	9.0	4.50	2.75
U	BMWW-t	MT20	5.0	8.0	2.50	3.25
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

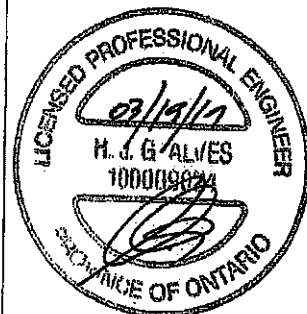
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	8-1-12	-75	-95		BACK	VERT	TOTAL		

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**

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TOWN OF CALEDON
BUILDING SECTION
FILE NO

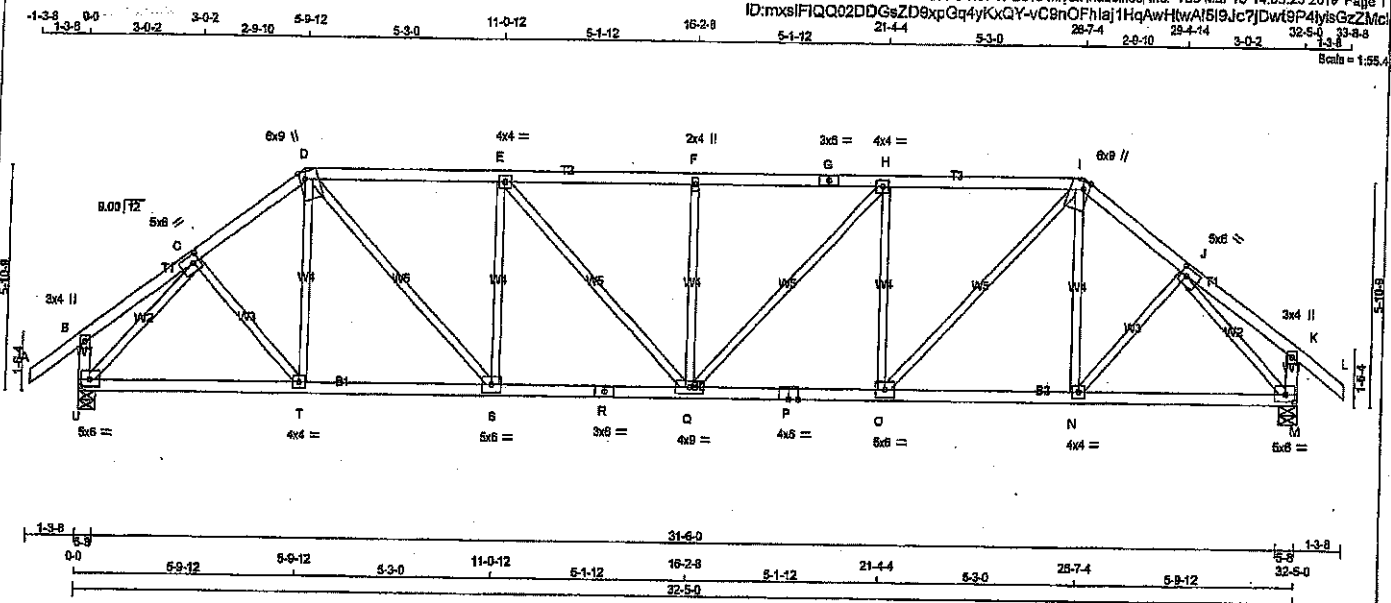


DWG NO. TAM 71905691
STRUCTURAL
COMPONENT ONLY 2/0

JOB NAME:	TRUSS NAME:	QUANTITY:	PLY:	JOB DESC.:	GREEN PARK HOMES	DRWG NO.:	T2
401811	T2	2	1	TRUSS DESC.:			

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:23 2018 Page 1
ID:mxslFIQQ02DDGszD9xpGq4yKxQY-vC8nOFh1j1HqAwHwA1519Jc7Jdw19P4lyisGzZMc1



TOTAL WEIGHT = 2 X 142 = 285 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	DOWN	UP	IN-SX
JT	2420	0	0	5-8
U	2420	0	0	5-8
M	2420	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0
U	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0
M	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	CS1 (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	MAX. FACTORED	FORCE	MAX	CS1 (LC)
A-B	0 / 42	-102.1	-102.1	0.14	(1)	10.00	C-T	0 / 301	0.07	(1)					
B-C	0 / 15	-102.1	-102.1	0.10	(1)	10.00	D-T	0 / 153	0.04	(3)					
C-D	-2482 / 0	-102.1	-102.1	0.18	(1)	4.22	D-S	0 / 1542	0.35	(1)					
D-E	-3033 / 0	-102.1	-102.1	0.80	(1)	3.44	S-E	-938 / 0	0.60	(1)					
E-F	-3349 / 0	-102.1	-102.1	0.84	(1)	3.25	E-Q	0 / 488	0.10	(1)					
F-G	-3349 / 0	-102.1	-102.1	0.84	(1)	3.25	Q-F	-484 / 0	0.28	(1)					
G-H	-3349 / 0	-102.1	-102.1	0.84	(1)	3.25	Q-H	0 / 488	0.10	(1)					
H-I	-3033 / 0	-102.1	-102.1	0.80	(1)	3.44	O-H	-938 / 0	0.60	(1)					
I-J	-2492 / 0	-102.1	-102.1	0.18	(1)	4.22	O-I	0 / 1542	0.35	(1)					
J-K	0 / 15	-102.1	-102.1	0.10	(1)	10.00	N-I	0 / 153	0.04	(3)					
K-L	0 / 42	-102.1	-102.1	0.14	(1)	10.00	N-J	0 / 301	0.07	(1)					
U-B	-263 / 0	0.0	0.0	0.03	(1)	7.81	U-C	-2722 / 0	0.85	(1)					
M-K	-263 / 0	0.0	0.0	0.03	(1)	7.81	J-M	-2722 / 0	0.85	(1)					
U-T	0 / 1773	-38.5	-38.5	0.46	(2)	10.00									
T-S	0 / 1878	-38.5	-38.5	0.50	(2)	10.00									
S-R	0 / 3033	-38.5	-38.5	0.59	(1)	10.00									
R-Q	0 / 3033	-38.5	-38.5	0.59	(1)	10.00									
Q-P	0 / 3033	-38.5	-38.5	0.59	(1)	10.00									
P-O	0 / 3033	-38.5	-38.5	0.59	(1)	10.00									
O-N	0 / 1878	-38.5	-38.5	0.50	(2)	10.00									
N-M	0 / 1773	-38.5	-38.5	0.46	(2)	10.00									

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	5.0	8.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMVW+1	MT20	4.0	4.0		
F	TMVW+w	MT20	2.0	4.0		
G	TS-1	MT20	3.0	6.0		
H	TMVW+1	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMVW+1	MT20	5.0	6.0	2.50	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVW+1	MT20	5.0	6.0	2.25	2.75
N	BMVW+1	MT20	4.0	4.0		
O	BMVW+1	MT20	6.0	6.0		
P	BS-1	MT20	4.0	6.0		
Q	BMVW+1	MT20	4.0	6.0		
R	BS-1	MT20	3.0	6.0		
S	BMVW+1	MT20	5.0	6.0		
T	BMVW+1	MT20	4.0	4.0		
U	BMVW+1	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015
- CSA 085-14
- TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.08")
CALCULATED VERT. DEFL. (LL) = U/999 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (1.08")
CALCULATED VERT. DEFL. (TL) = U/999 (0.28")

CS1: TC=0.84/1.00 (E-F-1), BC=0.59/1.00 (O-Q-1),
WB=0.85/1.00 (C-U-1), SS1=0.25/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

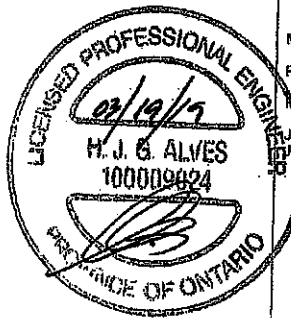
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 756 1987 1673

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JS GRIP=0.90 (M) (INPUT = 0.80)
JS METAL=1.00 (R) (INPUT = 1.00)



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TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DWG NO. TAM 1190 5692
STRUCTURAL
COMPONENT ONLY

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

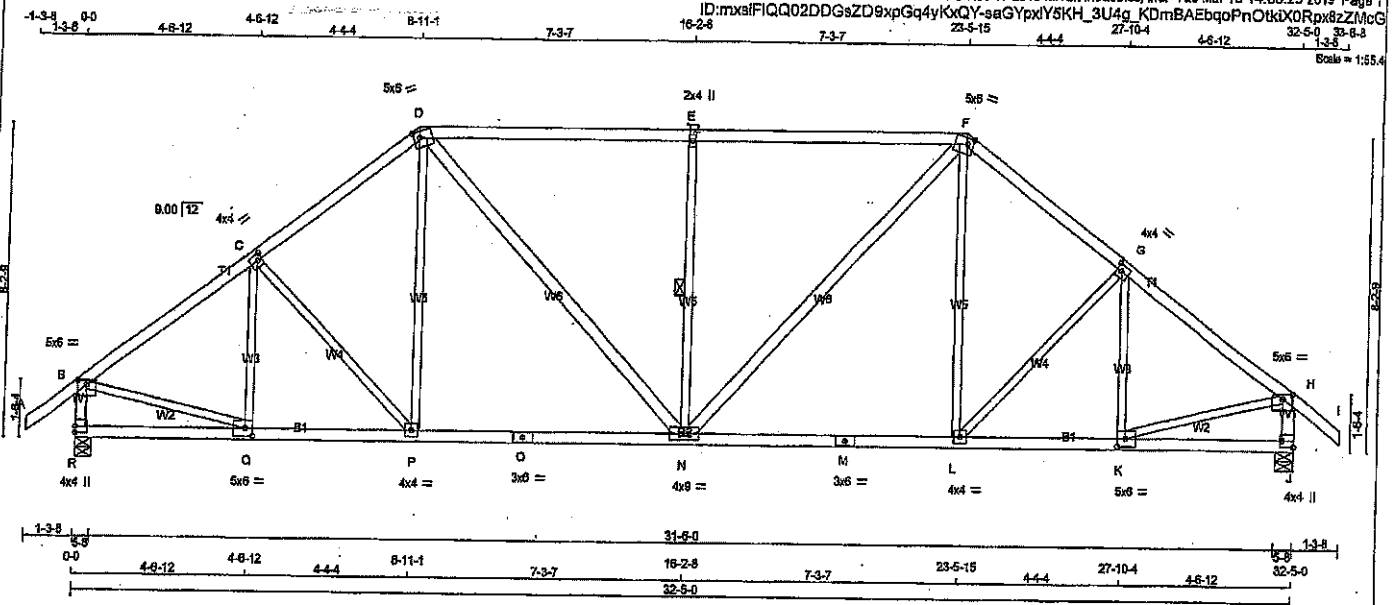
JOB NAME 401811	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO. T4
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Tamarack Roof Truss, Burlington

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ID:mxalFIQQ02DDGsZD9xpGq4yKxQY-saGypxiY5KH_3U4g_KDmBAEbqoPnOtkX0Rpx8zZMcG

Scale = 1:55.4



TOTAL WEIGHT = 2 X 162 = 324 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
R	2420	0	2420	0	5-8
J	2420	0	2420	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
R	1800	1021	0	340	0	0	0
J	1800	1021	0	340	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	MAX. UNBRACED	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	MAX. UNBRACED
MEMB.	FORCE (LBS)	PLF	CS1 (LC)	LENGTH	MEMB.	FORCE (LBS)	PLF	CS1 (LC)	LENGTH
FR-TO		FROM	TO		FR-TO		FROM	TO	
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	Q-C	-355/0	0.13 (1)	
B-C	-2485/0	-102.1	-102.1	0.35 (1)	4.10	C-P	-173/0	0.13 (1)	
C-D	-2383/0	-102.1	-102.1	0.33 (1)	4.18	P-D	0/392	0.09 (2)	
D-E	-2437/0	-102.1	-102.1	0.85 (1)	3.28	D-N	0/821	0.13 (1)	
E-F	-2437/0	-102.1	-102.1	0.85 (1)	3.28	N-E	-914/0	0.38 (1)	
F-G	-2383/0	-102.1	-102.1	0.33 (1)	4.16	N-F	0/821	0.13 (1)	
G-H	-2465/0	-102.1	-102.1	0.35 (1)	4.10	L-F	0/392	0.09 (2)	
H-I	0/42	-102.1	-102.1	0.14 (1)	10.00	L-G	-173/0	0.13 (1)	
R-B	-2342/0	0.0	0.0	0.24 (1)	5.58	K-G	-355/0	0.13 (1)	
J-H	-2342/0	0.0	0.0	0.24 (1)	5.58	B-Q	0/2084	0.48 (1)	
					K-H	0/2084	0.48 (1)		
R-Q	0/0	-38.5	-38.5	0.13 (3)	10.00				
Q-P	0/1897	-38.5	-38.5	0.48 (2)	10.00				
P-O	0/1881	-38.5	-38.5	0.58 (2)	10.00				
O-N	0/1881	-38.5	-38.5	0.58 (2)	10.00				
N-M	0/1881	-38.5	-38.5	0.58 (2)	10.00				
M-L	0/1881	-38.5	-38.5	0.58 (2)	10.00				
L-K	0/1897	-38.5	-38.5	0.48 (2)	10.00				
K-J	0/0	-38.5	-38.5	0.13 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.12")
ALLOWABLE DEFL. (TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.21")

CS1: TC=0.85/1.00 (E-F:1), BC=0.58/1.00 (L-N:2), WB=0.48/1.00 (H-K:1), SS=0.39/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
MT20	650	371
	1747	789
	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

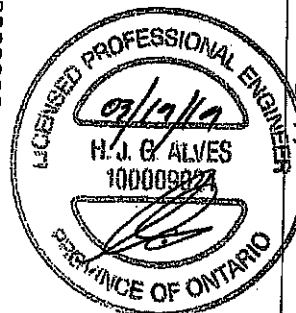
PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP=0.88 (H) (INPUT = 0.80)
SSI METAL=0.78 (C) (INPUT = 1.00)

DRWG NO. TAM 77905894
STRUCTURAL
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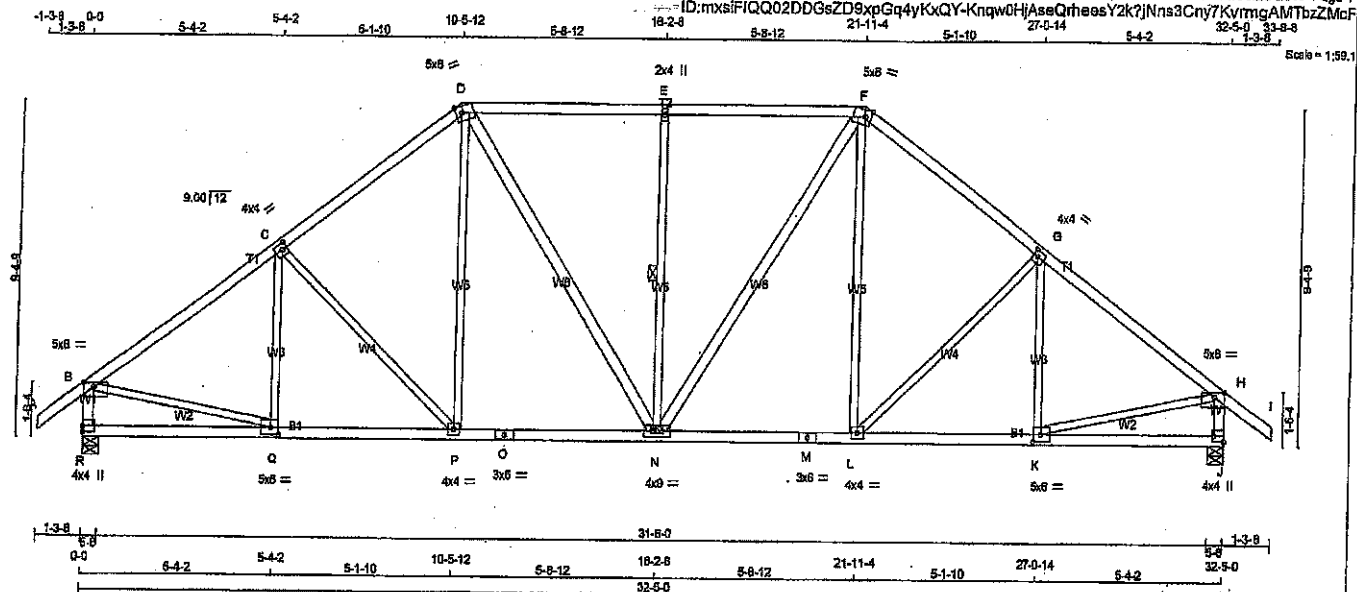
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	T5
401811	T5	7	1	TRUSS DESC.			

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 7 X 159 = 1116 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
R - B	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
R - O	2x4 DRY	No.2	SPF		
O - M	2x4 DRY	No.2	SPF		
M - J	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
D - N	2x4 DRY	No.2	SPF		
N - F	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
R	2420	0	0	5-8
J	2420	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED				
R	1800	1021 / 0	340 / 0	0 / 0	438 / 0
J	1800	1021 / 0	340 / 0	0 / 0	438 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
A-B	0 / 42	-102.1 -102.1	Q-C	-233 / 82
B-C	-2509 / 0	-102.1 -102.1	C-P	-352 / 0
C-D	-2277 / 0	-102.1 -102.1	P-D	0 / 489
D-E	-2088 / 0	-102.1 -102.1	D-N	0 / 553
E-F	-2088 / 0	-102.1 -102.1	N-E	-713 / 0
F-G	-2277 / 0	-102.1 -102.1	N-F	0 / 553
G-H	-2509 / 0	-102.1 -102.1	L-F	0 / 489
H-I	0 / 42	-102.1 -102.1	L-G	-352 / 0
R-B	-2334 / 0	0.0 0.0	K-G	-233 / 82
J-H	-2334 / 0	0.0 0.0	B-Q	0 / 2087
			K-H	0 / 2087
R-Q	0 / 0	-38.5 -38.5		
Q-P	0 / 2037	-38.5 -38.5		
P-D	0 / 1782	-38.5 -38.5		
O-N	0 / 1782	-38.5 -38.5		
N-M	0 / 1782	-38.5 -38.5		
M-L	0 / 1782	-38.5 -38.5		
L-K	0 / 2037	-38.5 -38.5		
K-J	0 / 0	-38.5 -38.5		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015
- CSA 088-14
- TRC 2014

(66 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = 1/869 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = 1/669 (0.15")

CSI: TO=0.49/1.00 (D-E-1), BC=0.48/1.00 (K-L-2),
WB=0.47/1.00 (H-K-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

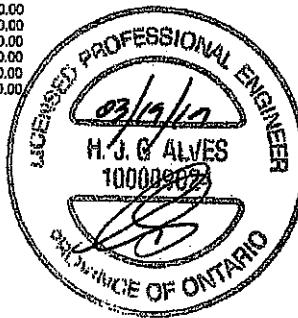
JSI GRIP= 0.86 (Q) (INPUT = 0.90)
JSI METAL= 0.55 (Q) (INPUT = 1.00)

DRWG NO. TAM T1405695
STRUCTURAL
COMPONENT ONLY

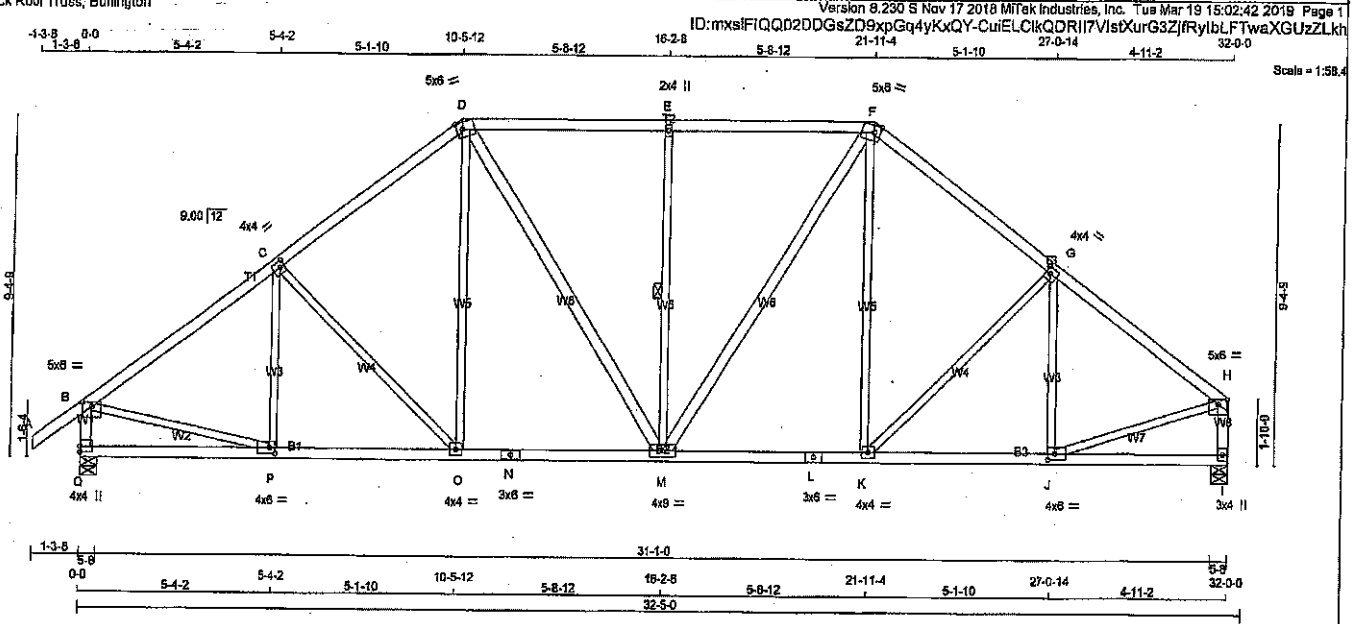
THIS STRUCTURE MUST BE
CONSTRUCTED IN ACCORDANCE
WITH THE PROVISIONS OF
THE ONTARIO BUILDING CODE

RECEIVED
JUN 25 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.



JOB NAME 401812	TRUSS NAME T5A	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 4 X 157 = 628 lb
(MTF)

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - M	2x4	DRY	No.2
M - F	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	5.0	6.0 1.50 3.00
C	TMVW-v	MT20	4.0	4.0 2.00 1.50
D	TMVW-m	MT20	5.0	6.0 Edge 2.00
E	TMVW-w	MT20	2.0	4.0
F	TMVW-m	MT20	5.0	6.0 Edge 2.00
G	TMVW-v	MT20	4.0	4.0 2.00 1.50
H	TMVW-p	MT20	5.0	6.0 1.50 3.00
I	BMV1+p	MT20	3.0	4.0
J	BMVW-v	MT20	4.0	6.0 2.00 2.25
K	BMVW-v	MT20	4.0	4.0
L	BS-t	MT20	3.0	6.0
M	BMVW-v	MT20	4.0	9.0
N	BS-t	MT20	3.0	6.0
O	BMVW-v	MT20	4.0	4.0
P	BMVW-v	MT20	4.0	6.0 2.00 2.00
Q	BMV1+p	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	HORZ	UPLIFT
Q	2390	0	0
I	2280	0	0

UNFACTORED REACTIONS							
1ST CASE	MAX	MIN	COMPONENT	REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD
Q	1778	1008 / 0	336 / 0	0 / 0	0 / 0	493 / 0	0 / 0
I	1681	929 / 0	336 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I.

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING			
TOTAL LOAD CASES: (4)			

CHORDS			
MEMB.	MAX. FACTORED	FACTORED	WEBS
	FORCE	VERT. LOAD	MAX. FACTORED
	(LBS)	(PLF)	FORCE
FR-TO		FROM TO	LENGTH
A-B	0 / 42	-102.1 -102.1	0.14 (1)
B-C	-2471 / 0	-102.1 -102.1	0.48 (1)
C-D	-2234 / 0	-102.1 -102.1	0.45 (1)
D-E	-2035 / 0	-102.1 -102.1	0.48 (1)
E-F	-2035 / 0	-102.1 -102.1	0.49 (1)
F-G	-2187 / 0	-102.1 -102.1	0.42 (1)
G-H	-2310 / 0	-102.1 -102.1	0.42 (1)
H-I	-2305 / 0	0.0 0.0	0.24 (1)
I-J	-2172 / 0	0.0 0.0	0.24 (1)
Q-P	0 / 0	-38.5 -38.5	0.19 (3)
P-O	0 / 2007	-38.5 -38.5	0.45 (2)
O-N	0 / 1759	-38.5 -38.5	0.43 (2)
N-M	0 / 1759	-38.5 -38.5	0.43 (2)
M-L	0 / 1722	-38.5 -38.5	0.42 (2)
L-K	0 / 1722	-38.5 -38.5	0.42 (2)
K-J	0 / 1878	-38.5 -38.5	0.42 (1)
J-I	0 / 0	-38.5 -38.5	0.17 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CS: TC=0.49/1.00 (D-E-1), BC=0.45/1.00 (O-P-2), WB=0.48/1.00 (B-P-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN
MT20 650 371 1747 768 1997 1873

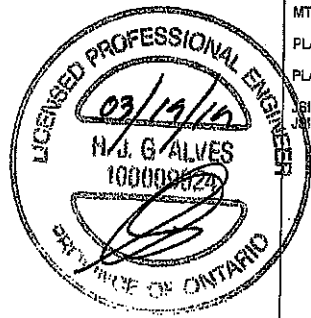
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP=0.89 (P) (INPUT = 0.90)
METAL=0.55 (N) (INPUT = 1.00)

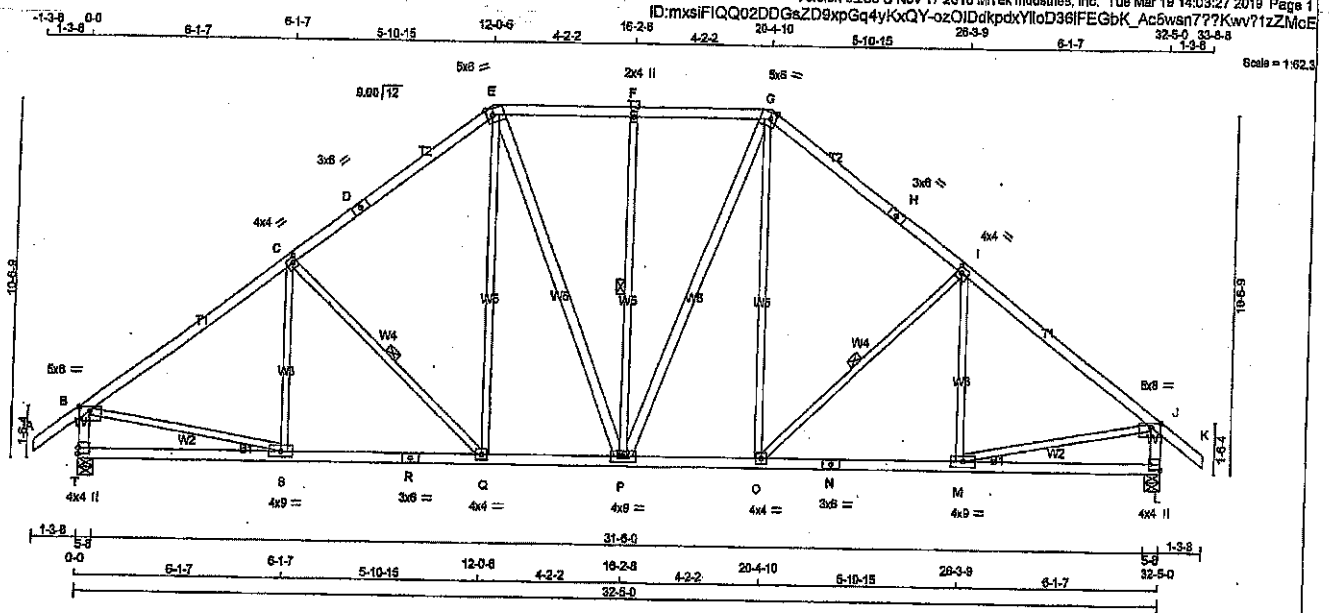
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JUN 25 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO



ENG. H.J.G. ALVES 17905718
STRUCTURAL
COMPONENT ONLY

JOB NAME 401811	TRUSS NAME T6	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. T6
Tamarack Roof Truss, Burlington				Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:27 2019 Page 1	



TOTAL WEIGHT = 2 X 168 = 336 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - R	2x4	DRY	No.2
R - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	1.50 3.50
C	TMWW-4	MT20	4.0	4.0	2.00 1.50
D	TS-4	MT20	3.0	6.0	
E	TTWW-m	MT20	5.0	6.0	Edge 2.00
F	TMWW-w	MT20	2.0	4.0	
G	TTWW-m	MT20	5.0	6.0	Edge 2.00
H	TS-4	MT20	3.0	6.0	
I	TMWW-4	MT20	4.0	4.0	2.00 1.50
J	TMWW-p	MT20	5.0	8.0	1.50 3.50
L	BMV1-p	MT20	4.0	4.0	2.00 Edge
M	BMWW-4	MT20	4.0	6.0	
N	BS-4	MT20	3.0	6.0	
O	BMWW-4	MT20	4.0	4.0	
P	BMWW-4	MT20	4.0	4.0	
Q	BMWW-4	MT20	4.0	4.0	
R	BS-4	MT20	3.0	6.0	
S	BMWW-4	MT20	4.0	4.0	
T	BMV1-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX
T	2420 0	2420 0	5-8
L	2420 0	2420 0	5-8

UNFACTORED REACTIONS			
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	
T	1800	1021 / 0	340 / 0
L	1800	1021 / 0	340 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

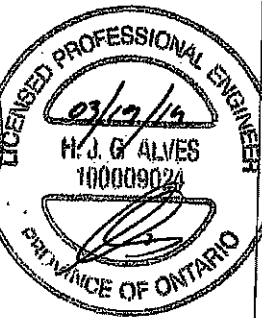
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)
B-C	-2528 / 0	-102.1 -102.1	0.66 (1)
C-D	-2158 / 0	-102.1 -102.1	0.59 (1)
D-E	-2158 / 0	-102.1 -102.1	0.59 (1)
E-F	-1625 / 0	-102.1 -102.1	0.28 (1)
F-G	-1625 / 0	-102.1 -102.1	0.28 (1)
G-H	-2158 / 0	-102.1 -102.1	0.59 (1)
H-I	-2158 / 0	-102.1 -102.1	0.59 (1)
I-J	-2528 / 0	-102.1 -102.1	0.66 (1)
J-K	0 / 42	-102.1 -102.1	0.14 (1)
K-L	-2324 / 0	0.0 0.0	0.24 (1)
L-M	-2324 / 0	0.0 0.0	0.24 (1)
T-S	0 / 0	-38.5 -38.5	0.27 (3)
S-R	0 / 2058	-38.5 -38.5	0.54 (2)
R-Q	0 / 2058	-38.5 -38.5	0.54 (2)
Q-P	0 / 1893	-38.5 -38.5	0.38 (1)
P-O	0 / 1893	-38.5 -38.5	0.38 (1)
O-N	0 / 2058	-38.5 -38.5	0.54 (2)
N-M	0 / 2058	-38.5 -38.5	0.54 (2)
M-L	0 / 2058	-38.5 -38.5	0.54 (2)



RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015
- CSA 088-14
- TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.16")

CSI: TC=0.66/1.00 (B-C:1), BC=0.54/1.00 (M-Q:2),
WB=0.47/1.00 (J-M:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR= 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI)
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP= 0.88 (S) (INPUT = 0.90)
ISI METAL= 0.74 (R) (INPUT = 1.00)

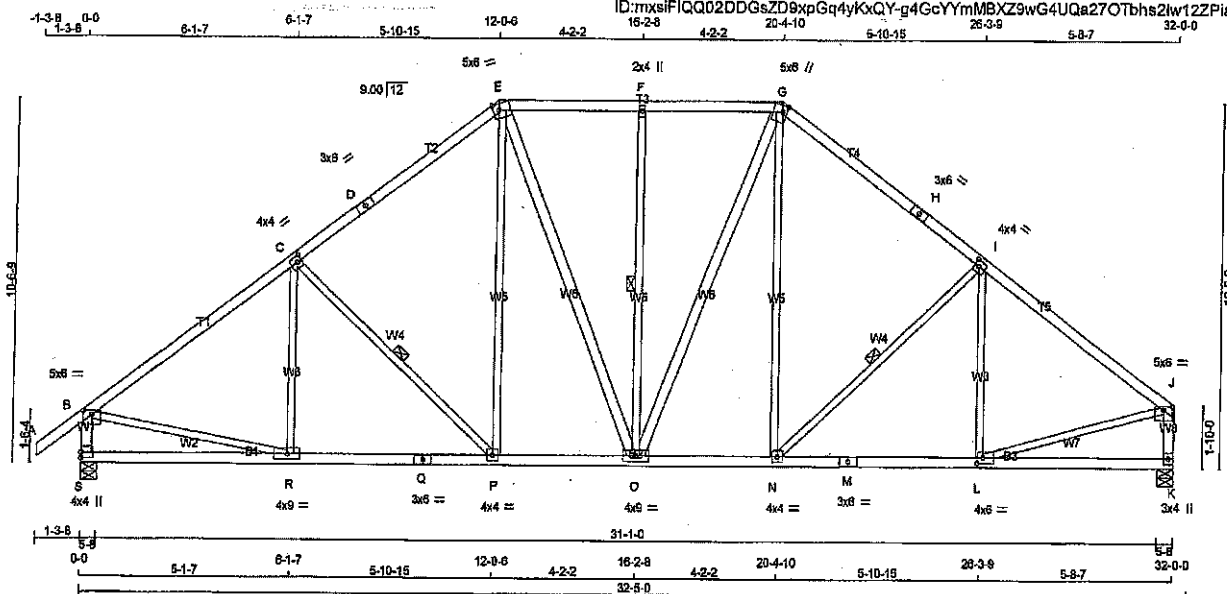
DRWG NO. TAM 71905696
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	T6A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:21.5

TOTAL WEIGHT = 165 lb

LUMBER

N.L.G.A. RULES

CHORDS

SIZE

DRY

LUMBER

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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SPF

SPF

SPF

SPF

ALL WEBS

EXCEPT

E - O

O - G

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

B TMWW-p

MT20

5.0

6.0

1.50

3.00

C TMWW-I

MT20

4.0

4.0

2.00

1.50

D TS-I

MT20

3.0

6.0

Edge 2.00

E TTWW-m

MT20

2.0

4.0

F TTWW-w

MT20

5.0

6.0

2.25

1.50

G TTWW+m

MT20

5.0

6.0

2.00

1.50

H TS-I

MT20

3.0

6.0

2.00

1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

GROSS REACTION

VERT

HORZ

DOWN

HORZ

UPLIFT

IN-SX

IN-SX

BRG

BRG

5-8

5-8

5-8

5-8

5-8

5-8

5-8

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5-8

UNFACTORED REACTIONS

1ST LOASE

COMBINED

SNOW

LIVE

PERM.LIVE

WIND

DEAD

SOIL

1778

1009 / 0

336 / 0

0 / 0

0 / 0

433 / 0

0 / 0

1681

929 / 0

336 / 0

0 / 0

0 / 0

416 / 0

0 / 0

0 / 0

0 / 0

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BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-O, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN

THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

MEMB.

FORCE

(LBS)

FACTORED

VERT. LOAD

LC1

MAX

CS1 (LC)

MAX.

MEMB.

MAX. FACTORED

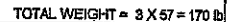
FORCE

(LBS)

CS1 (LC)

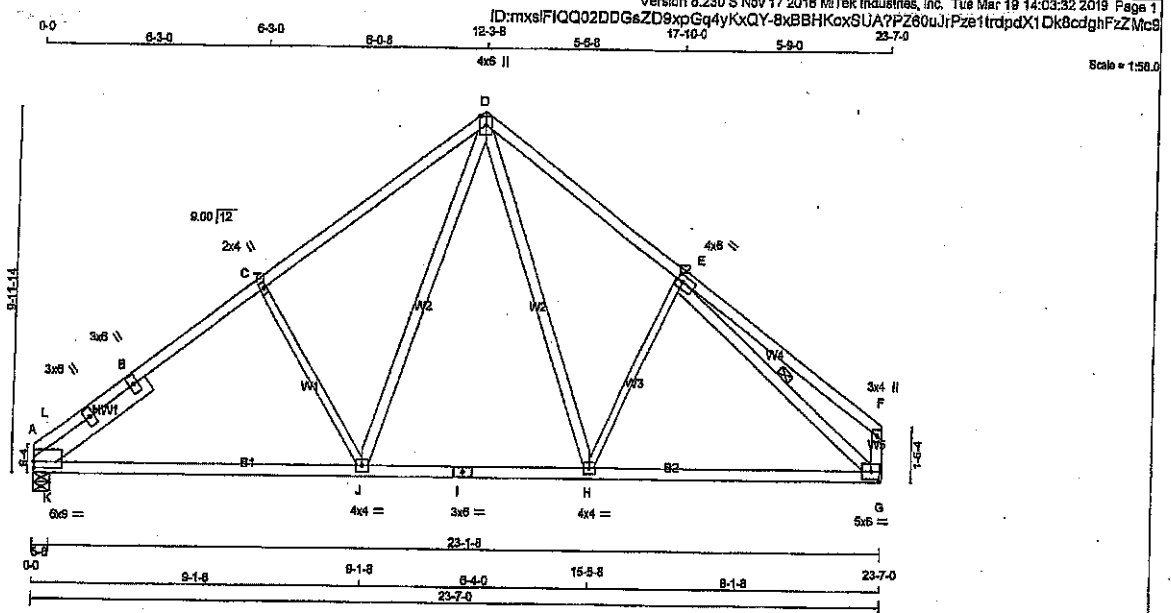
MAX.

ନିର୍ଦ୍ଦେଶ - 135.6



DWG NO. TAM 77905699
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	T10	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 3 X 109 = 326 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
A - I	2x4 DRY	No.2	SPF		
I - G	2x4 DRY	No.2	SPF		
REINFORCING MEMBERS					
HW1	2x8 DRY	No.2	SPF		
ALL WEBS EXCEPT					
C - J	2x3 DRY	No.2	SPF		
H - E	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
A	1658 0	1658 0	0	0
G	1658 0	1658 0	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-13.

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	1239	885 / 0	248 / 0	0 / 0	0 / 0	307 / 0	0 / 0
G	1239	885 / 0	248 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO									
A-L	-2387 / 0	-102.1	-102.1	0.10 (1)	4.38	C-J	-552 / 0	0.33 (1)	
L-B	-1696 / 0	-102.1	-102.1	0.47 (1)	4.60	J-D	0 / 789	0.13 (1)	
B-C	-1696 / 0	-102.1	-102.1	0.47 (1)	4.60	D-H	0 / 656	0.11 (1)	
C-D	-1612 / 0	-102.1	-102.1	0.52 (1)	4.62	H-E	-347 / 16	0.22 (1)	
D-E	-1560 / 0	-102.1	-102.1	0.47 (1)	4.74	E-G	-1691 / 0	0.54 (1)	
E-F	0 / 42	-102.1	-102.1	0.54 (1)	10.00	K-L	0 / 829	0.00 (1)	
G-F	-218 / 0	0.0	0.0	0.02 (1)	7.81	K-B	-438 / 173	0.05 (1)	
A-K	0 / 1242	-38.5	-38.5	0.54 (2)	10.00				
K-J	0 / 1538	-38.5	-38.5	0.56 (2)	10.00				
J-I	0 / 1010	-38.5	-38.5	0.52 (2)	10.00				
I-H	0 / 1010	-38.5	-38.5	0.52 (2)	10.00				
H-G	0 / 1362	-38.5	-38.5	0.57 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.78")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.78")
CALCULATED VERT. DEFL.(TL) = L/932 (0.30")

CSI: TC=0.54/1.00 (E-F:1), BC=0.57/1.00 (G-H:2), WB=0.54/1.00 (E-G:1), SS=0.50/1.00 (A-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLD)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1657 788 1957 1656

PLATE PLACEMENT TOL = 0.250 inches

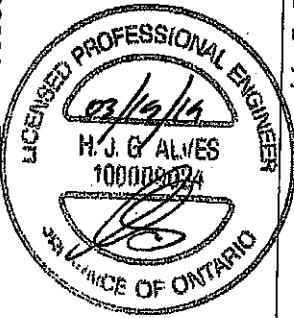
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.62 (A) (INPUT = 1.00)

DWG NO. 17905702
STRUCTURAL
COMPLEMENT ONLY

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TOWN OF CALEDON
BUILDING SECTION
FILE NO



THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE CANADIAN BUILDING CODE

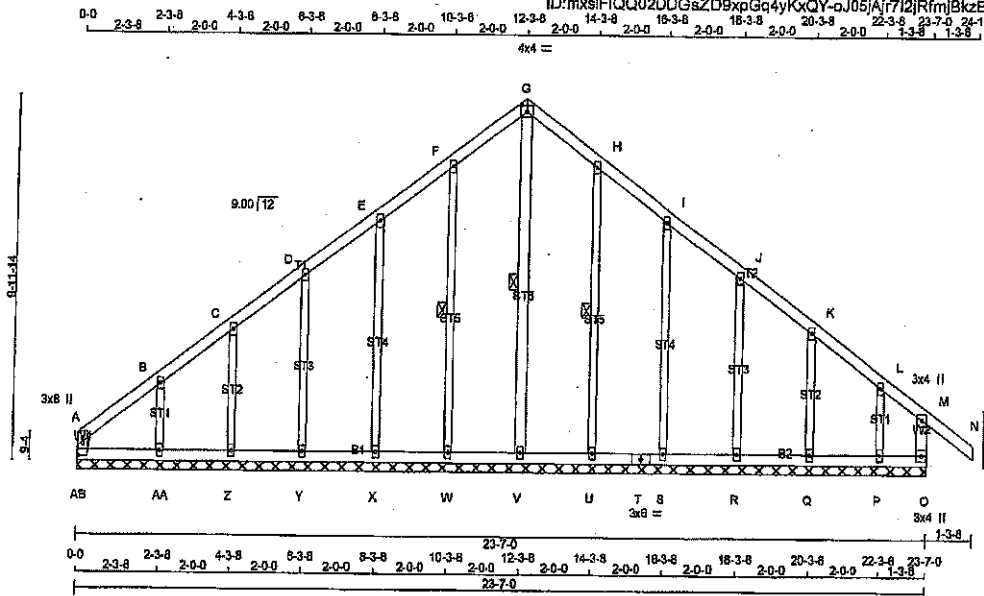
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	G10	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:mxsIFQQ02DDGsZD9xpGq4yKxQY-aJ05A/r712/RfmBkzBDdR7_RWg5J9pnyLsg9zZLkk

Scale = 1:50.2



TOTAL WEIGHT = 115 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - N	2x4	DRY	No.2	SPF
AB- A	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
AB- T	2x4	DRY	No.2	SPF
T - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
V - G	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ST2	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-V, F-W, H-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMBM/v1+p MT20	3.0	8.0	4.00	1.50
B, C, D, E, F, H, I, J, K, L					
G	TMW+w MT20	2.0	4.0		
H	TTW+p MT20	4.0	4.0	2.25	2.00
M	TMW+p MT20	3.0	4.0		
O	BMV1+p MT20	3.0	4.0		
P, Q, R, S, U, V, W, X, Y, Z, AA					
P	BMW1+w MT20	2.0	4.0		
T	BS-1 MT20	3.0	6.0		
AB					

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO	
A-B	-14/0	-102.1	-102.1	0.08 (1)	6.25	V-G	-226/0	0.11 (1)
B-C	-14/0	-102.1	-102.1	0.08 (1)	6.25	W-F	-204/0	0.09 (1)
C-D	-5/0	-102.1	-102.1	0.05 (1)	10.00	X-E	-202/0	0.17 (1)
D-E	-4/0	-102.1	-102.1	0.05 (1)	10.00	Y-D	-203/0	0.09 (1)
E-F	-4/4	-102.1	-102.1	0.05 (1)	10.00	Z-C	-193/0	0.05 (1)
F-G	-3/8	-102.1	-102.1	0.05 (1)	10.00	AA-B	-227/0	0.04 (1)
G-H	-3/8	-102.1	-102.1	0.05 (1)	10.00	U-H	-204/0	0.09 (1)
H-I	-4/4	-102.1	-102.1	0.05 (1)	10.00	S-I	-203/0	0.17 (1)
I-J	-4/0	-102.1	-102.1	0.05 (1)	10.00	R-J	-189/0	0.09 (1)
J-K	-6/0	-102.1	-102.1	0.08 (1)	10.00	Q-K	-216/0	0.05 (1)
K-L	-5/0	-102.1	-102.1	0.05 (1)	10.00	P-L	-90/0	0.01 (1)
L-M	-50/0	-102.1	-102.1	0.11 (1)	6.25			
M-N	0/42	-102.1	-102.1	0.14 (1)	10.00			
AB-A	-113/0	0.0	0.0	0.02 (1)	7.81			
O-M	-270/0	0.0	0.0	0.04 (1)	7.81			
AB-AA	0/17	-38.5	-38.5	0.04 (2)	10.00			
AA-Z	0/8	-38.5	-38.5	0.04 (2)	10.00			
Z-Y	0/4	-38.5	-38.5	0.02 (3)	10.00			
Y-X	0/13	-38.5	-38.5	0.02 (3)	10.00			
X-W	-3/3	-38.5	-38.5	0.02 (3)	10.00			
W-V	-5/3	-38.5	-38.5	0.02 (3)	10.00			
V-U	-6/3	-38.5	-38.5	0.02 (3)	10.00			
U-T	-3/3	-38.5	-38.5	0.02 (3)	10.00			
T-S	-3/3	-38.5	-38.5	0.02 (3)	10.00			
S-R	0/3	-38.5	-38.5	0.02 (3)	10.00			
R-Q	0/4	-38.5	-38.5	0.03 (3)	10.00			
Q-P	0/8	-38.5	-38.5	0.03 (2)	10.00			
P-O	0/12	-38.5	-38.5	0.04 (2)	10.00			

LICENSED PROFESSIONAL

03/19/16

H. J. G. ALVE

100009234

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 085-08, CSA 085-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (M-N:1), BC=0.04/1.00 (O-P:1)

, WB=0.17/1.00 (S-1), SS=0.08/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

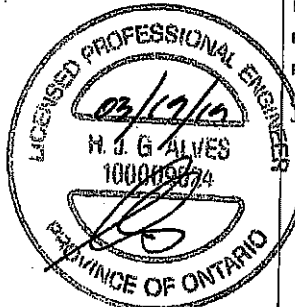
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.40 (F) (INPUT = 0.80)
SI METAL=0.12 (B) (INPUT = 1.00)

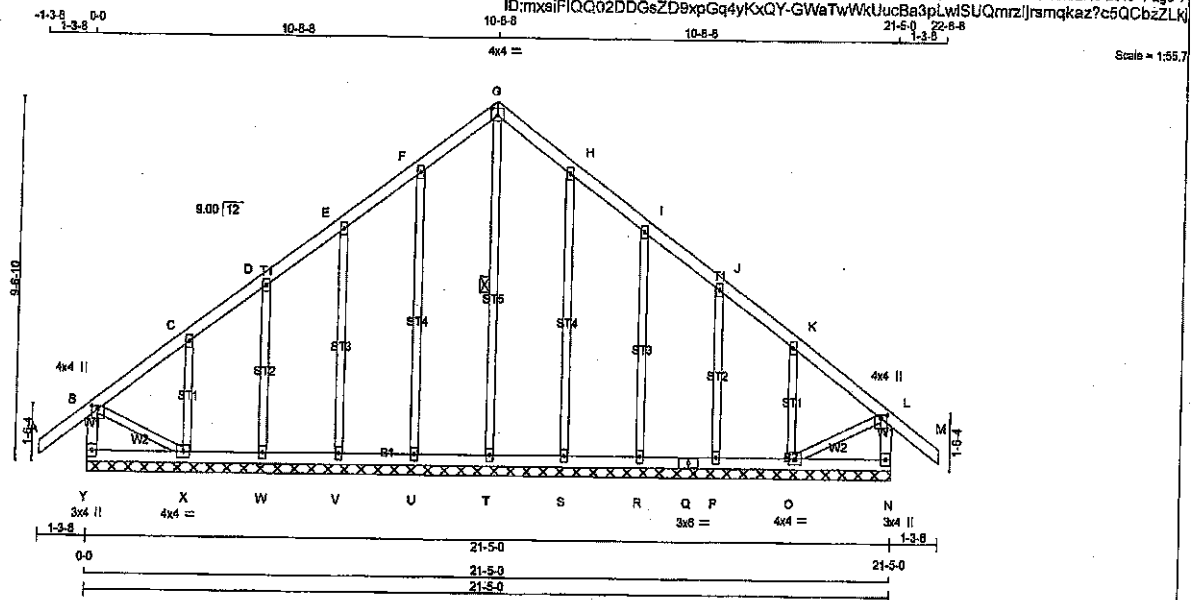


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JUN 25 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DWG NO. TAM 71905715
STRUCTURAL
CONSTRUCTION

JOB NAME 401812	TRUSS NAME G14	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 107 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
Y - B	2x4	DRY	No.2	SPF	
A - G	2x4	DRY	No.2	SPF	
G - M	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
Y - Q	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2'-0" O.C.					

PLATES (table in inches)	W	LEN	Y	X
JT TYPE				
B TMVW+p	MT20	4.0	4.0	1.00 2.00
C D, E, F, H, I, J, K				
C TMVW+w	MT20	2.0	4.0	
G TMVW+p	MT20	4.0	4.0	2.25 2.00
L TMVW+p	MT20	4.0	4.0	1.00 2.00
N BMV1+p	MT20	3.0	4.0	
O BMVW1-l	MT20	4.0	4.0	
P R, S, T, U, V, W				
P BMV1+w	MT20	2.0	4.0	
Q BS-l	MT20	3.0	6.0	
X BMVW1-l	MT20	4.0	4.0	
Y BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
Y-B	-284 / 0	0.0 0.0	0.03 (1)	T-G	-142 / 0	0.08 (1)	
A-B	0 / 42	-102.1 -102.1	0.14 (1)	U-F	-231 / 0	0.29 (1)	
B-C	-31 / 0	-102.1 -102.1	0.08 (1)	V-E	-201 / 0	0.14 (1)	
C-D	-48 / 0	-102.1 -102.1	0.08 (1)	W-D	-180 / 0	0.07 (1)	
D-E	-31 / 0	-102.1 -102.1	0.08 (1)	X-C	-273 / 0	0.09 (1)	
E-F	-26 / 0	-102.1 -102.1	0.08 (1)	S-H	-231 / 0	0.29 (1)	
F-G	-38 / 0	-102.1 -102.1	0.08 (1)	R-I	-201 / 0	0.14 (1)	
G-H	-38 / 0	-102.1 -102.1	0.08 (1)	P-J	-180 / 0	0.07 (1)	
H-I	-26 / 0	-102.1 -102.1	0.08 (1)	Q-K	-273 / 0	0.09 (1)	
I-J	-31 / 0	-102.1 -102.1	0.08 (1)	B-X	0 / 40	0.01 (1)	
J-K	-48 / 0	-102.1 -102.1	0.09 (1)	O-L	0 / 40	0.01 (1)	
K-L	-31 / 0	-102.1 -102.1	0.08 (1)				
L-M	0 / 42	-102.1 -102.1	0.14 (1)				
N-L	-284 / 0	0.0 0.0	0.03 (1)				
Y-X	0 / 0	-38.5 -38.5	0.05 (3)				
X-W	0 / 29	-38.5 -38.5	0.05 (3)				
W-V	0 / 26	-38.5 -38.5	0.03 (2)				
V-U	0 / 23	-38.5 -38.5	0.03 (2)				
U-T	0 / 20	-38.5 -38.5	0.02 (2)				
T-S	0 / 20	-38.5 -38.5	0.02 (2)				
S-R	0 / 23	-38.5 -38.5	0.03 (2)				
R-Q	0 / 26	-38.5 -38.5	0.03 (2)				
Q-P	0 / 26	-38.5 -38.5	0.03 (2)				
P-O	0 / 29	-38.5 -38.5	0.05 (3)				
O-N	0 / 0	-38.5 -38.5	0.05 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.14/1.00 (L-M:1), BC=0.05/1.00 (O-P:3), WB=0.29/1.00 (F-U:1), SS=0.10/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

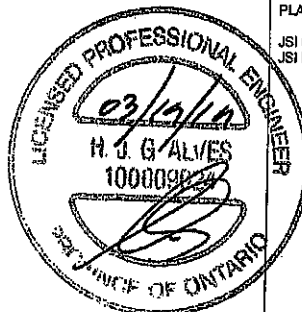
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.58 (C) (INPUT = 0.80)
JSI METAL= 0.14 (C) (INPUT = 1.00)



RECEIVED
JUN 25 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

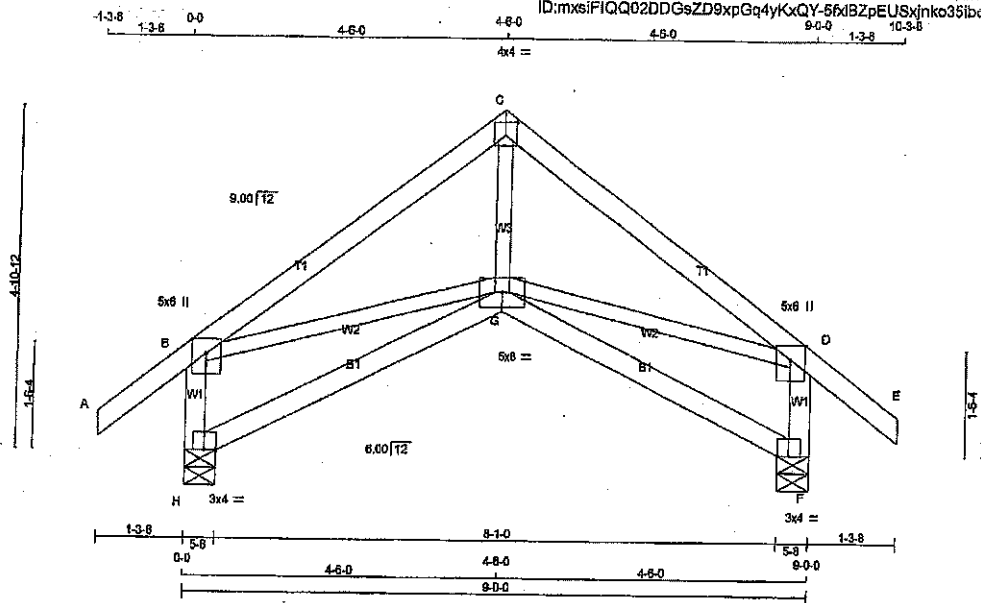
DWG NO. TAM T1905716
STRUCTURAL
COUNCIL OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	T17S	2	1	TRUSS DESC.		

Farmack Roof Truss, Burlington

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ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-56MBZpEUSXlnko35ibq76DH8G4EUYOYYKPFzZLkd

Scale = 1:30.4



TOTAL WEIGHT = 2 X 40 = 80 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM/VW+p	MT20	5.0	6.0	Edge	
C	TM/VW+p	MT20	4.0	4.0	2.25	2.00
D	TM/VW+p	MT20	5.0	5.0	Edge	
F	BVM1-p	MT20	3.0	4.0		
G	BVM1-p	MT20	5.0	6.0		
H	BVM1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX	IN-SX
JT VERT	JT VERT	JT VERT	JT VERT	BRG	BRG	IN-SX	IN-SX
H 774	0	774	0	5-8	5-8		
F 774	0	774	0	5-8	5-8		

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
H 569	341/0	95/0
F 569	341/0	95/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (L/C)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (L/C)
FR-TO		FROM	TO		FR-TO		
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	G-C	0/404
B-C	-703/0	-102.1	-102.1	0.27 (1)	8.25	B-G	0/573
C-D	-703/0	-102.1	-102.1	0.27 (1)	8.25	G-D	0/573
D-E	0/42	-102.1	-102.1	0.14 (1)	10.00		
H-B	-687/0	0.0	0.0	0.08 (1)	7.81		
F-D	-687/0	0.0	0.0	0.08 (1)	7.81		
H-G	0/0	-38.5	-38.5	0.18 (3)	10.00		
G-F	0/0	-38.5	-38.5	0.18 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TCIC 2011, TCIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.30")
CALCULATED VERT. DEFL. (LL) = L/989 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.30")
CALCULATED VERT. DEFL. (TL) = L/989 (0.06")

CSI: TC=0.27/1.00 (B-C:1), BC=0.18/1.00 (G-H:3), WB=0.13/1.00 (D-G:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

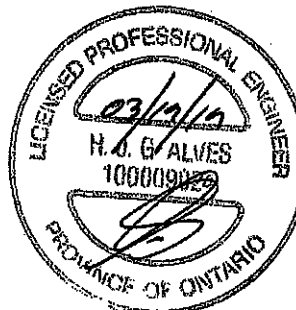
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	618	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (C) (INPUT = 0.80)
JSI METAL= 0.40 (H) (INPUT = 1.00)



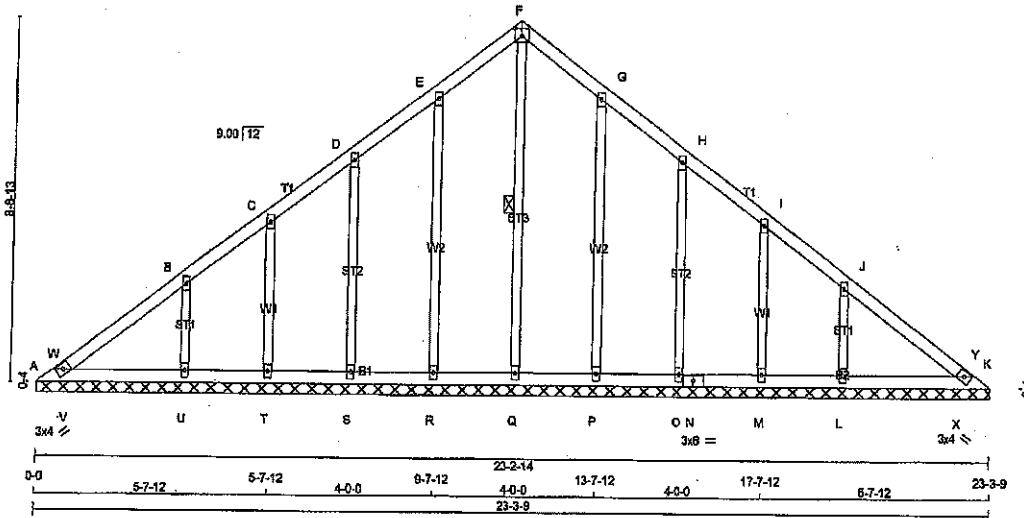
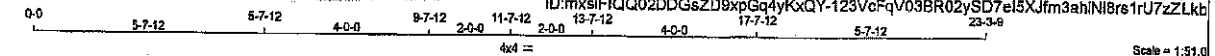
DWG NO. TAM 71905724
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	V10	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:mxsIFQQ02DDGsZD9xpGq4yKxQY-123VcFqV03BR02ySD7e15XJfm3ahINi8r1rU7zZLkb



TOTAL WEIGHT = 84 lb

LUMBER

N.L.G.A. RULES

CHORDS SIZE

A - F 2x4 DRY

F - K 2x4 DRY

A - N 2x4 DRY

N - K 2x4 DRY

ALL WEBS 2x3 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

No.2

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

PLATES (table is in inches)

JT TYPE PLATES

A TBM1-h MT20

B, C, D, E, G, H, I, J

B TMW+w MT20

F TTW+p MT20

K TBM1-h MT20

L, M, O, P, Q, R, S, T, U

L BMW1+w MT20

N BS4 MT20

W

LEN

Y

X

3.0

4.0

2.0

4.0

4.0

2.25

2.00

3.0

4.0

2.0

4.0

3.0

6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

DESIGNER									
BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
A	186	0	186	0	0	23-2-14	(7-3-23-2-14		
K	186	0	186	0	0	23-2-14	(7-3-23-2-14		
Q	325	0	325	0	0	23-2-14	(7-3-23-2-14		
S	292	0	292	0	0	23-2-14	(7-3-23-2-14		
U	494	0	494	0	0	23-2-14	(7-3-23-2-14		
O	292	0	292	0	0	23-2-14	(7-3-23-2-14		
L	494	0	494	0	0	23-2-14	(7-3-23-2-14		
T	192	0	192	0	0	23-2-14	(7-3-23-2-14		
R	308	0	308	0	0	23-2-14	(7-3-23-2-14		
P	308	0	308	0	0	23-2-14	(7-3-23-2-14		
M	192	0	192	0	0	23-2-14	(7-3-23-2-14		

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS					SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD		
A	138	81/0	24/0	0/0	0/0	33/0	0/0	
K	138	81/0	24/0	0/0	0/0	33/0	0/0	
Q	247	124/0	58/0	0/0	0/0	65/0	0/0	
S	219	119/0	45/0	0/0	0/0	55/0	0/0	
U	370	202/0	75/0	0/0	0/0	82/0	0/0	
O	219	118/0	45/0	0/0	0/0	55/0	0/0	
L	370	202/0	75/0	0/0	0/0	82/0	0/0	
T	143	79/0	29/0	0/0	0/0	35/0	0/0	
R	227	131/0	42/0	0/0	0/0	55/0	0/0	
P	227	131/0	42/0	0/0	0/0	55/0	0/0	
M	143	79/0	29/0	0/0	0/0	35/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K, Q, S, U, O, L, T, R, P, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-W	0/17	-102.1	-102.1 0.04 (2)	10.00	Q-F	-247/0	0.12 (1)	
W-B	0/81	-102.1	-102.1 0.13 (1)	10.00	S-D	-203/0	0.10 (1)	
B-C	0/34	-102.1	-102.1 0.13 (1)	10.00	U-B	-313/0	0.05 (1)	
C-D	0/60	-102.1	-102.1 0.08 (1)	10.00	O-H	-203/0	0.10 (1)	
D-E	0/63	-102.1	-102.1 0.07 (1)	10.00	L-J	-313/0	0.05 (1)	
E-F	0/51	-102.1	-102.1 0.07 (1)	10.00	T-C	-160/0	0.04 (1)	
F-G	0/51	-102.1	-102.1 0.07 (1)	10.00	R-E	-233/0	0.21 (1)	
G-H	0/83	-102.1	-102.1 0.07 (1)	10.00	P-G	-233/0	0.21 (1)	
H-I	0/60	-102.1	-102.1 0.08 (1)	10.00	M-I	-160/0	0.04 (1)	
I-J	0/34	-102.1	-102.1 0.13 (1)	10.00	V-W	-98/39	0.00 (1)	
J-Y	0/81	-102.1	-102.1 0.13 (1)	10.00	X-Y	-98/39	0.00 (1)	
Y-K	0/17	-102.1	-102.1 0.04 (2)	10.00				
A-V	-52/0	-38.5	-38.5 0.11 (1)	6.25				
V-U	-32/0	-38.5	-38.5 0.11 (1)	6.25				
U-T	-41/0	-38.5	-38.5 0.09 (1)	6.25				
T-S	-45/0	-38.5	-38.5 0.03 (2)	6.25				
S-R	-48/0	-38.5	-38.5 0.03 (2)	6.25				
R-Q	-51/0	-38.5	-38.5 0.02 (3)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012

- CSA 086-09, CSA 088-14

- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (J-Y:1), BC=0.11/1.00 (L-X:1),

WB=0.21/1.00 (G-P:1), SS=0.12/1.00 (J-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 789 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

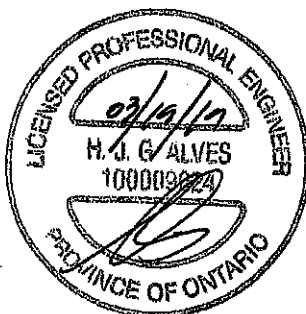
JSI GRIP= 0.40 (G) (INPUT = 0.90)

JSI METAL= 0.17 (J) (INPUT = 1.00)

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JUN 25 2019

TOWN OF CAMBODIA
BUILDING SECTION
FILE NO. 792573

CONTINUED ON PAGE 2



JOB NAME 401812	TRUSS NAME V10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

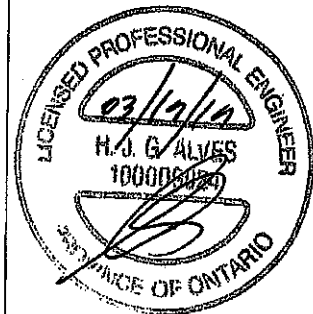
Version 8.250 S Nov 17 2016 Mitak Industries, Inc. Tue Mar 19 15:02:48 2016 Page 2
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LOADING TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
Q-P	-51 / 0	-38.5	-38.5 0.02 (3)	6.25			
P-O	-48 / 0	-38.5	-38.5 0.03 (2)	6.25			
O-N	-45 / 0	-38.5	-38.5 0.03 (2)	6.25			
N-M	-45 / 0	-38.5	-38.5 0.03 (2)	6.25			
M-L	-41 / 0	-38.5	-38.5 0.09 (1)	6.25			
L-X	-32 / 0	-38.5	-38.5 0.11 (1)	6.25			
X-K	-52 / 0	-38.5	-38.5 0.11 (1)	6.25			

RECEIVED
JUN 25 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO. _____



ENGINE NO. IAM 17905723
STRUCTURAL
CHECKED ONLY 72

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	V11	1	1	TRUSS DESC.		

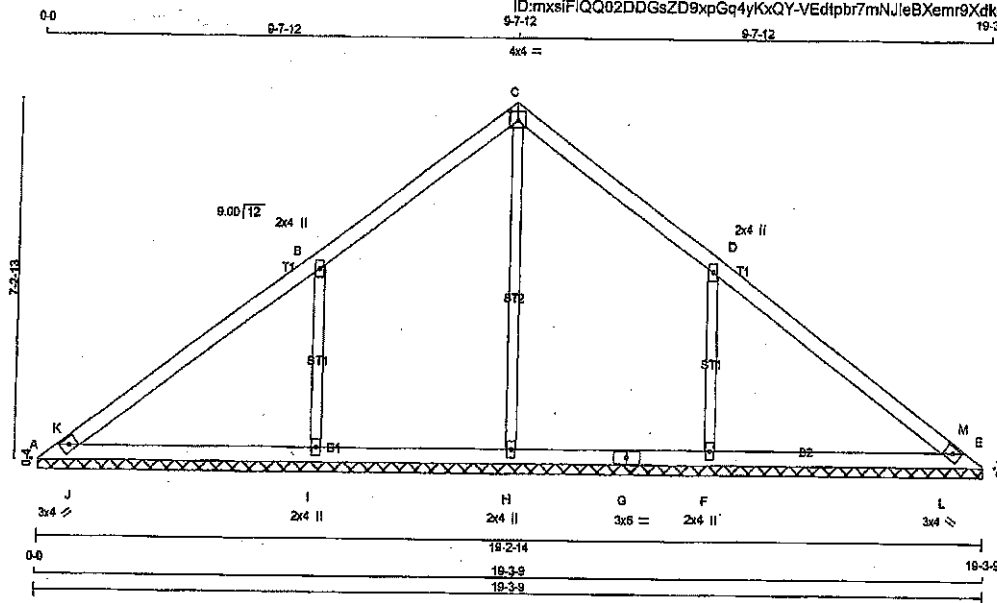
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 15:02:46 2018 Page 1

ID:mxsIF1QQ02DDGsZD9xpGq4yKxQY-VEdtpr7mNjleBXemr9Xdkm4TuGRkuH4WmO0azZLka

19-3-9

Scale = 1/4" = 1'-0"



TOTAL WEIGHT = 60 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

A - G 2x4 DRY No.2

G - E 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

DRY, SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
A	146	0	146	0
E	146	0	146	0
H	756	0	756	0
I	829	0	829	0
F	829	0	829	0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL
A	106	68/0 18/0 0/0 0/0 24/0 0/0
E	106	68/0 18/0 0/0 0/0 24/0 0/0
H	574	290/0 134/0 0/0 0/0 150/0 0/0
I	617	347/0 119/0 0/0 0/0 151/0 0/0
F	617	347/0 119/0 0/0 0/0 151/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	FACTORED	WEBS	FACTORED
	MEMB.	FORCE	MEMB.	FORCE
		(LBS)		(LBS)
FR-TO				
A-K	0/281	-102.1 -102.1 0.09 (3)	H-C	-674/0 0.60 (1)
K-B	0/340	-102.1 -102.1 0.42 (1)	I-B	-665/0 0.15 (1)
B-C	0/284	-102.1 -102.1 0.41 (1)	F-D	-665/0 0.15 (1)
C-D	0/284	-102.1 -102.1 0.41 (1)	J-K	-180/80 0.00 (1)
D-M	0/340	-102.1 -102.1 0.42 (1)	L-M	-180/80 0.00 (1)
M-E	0/281	-102.1 -102.1 0.09 (3)		
A-J	-287/0	-38.5 -38.5 0.18 (1)		
J-I	-248/0	-38.5 -38.5 0.22 (2)		
I-H	-259/0	-38.5 -38.5 0.21 (2)		
H-G	-259/0	-38.5 -38.5 0.21 (2)		
G-F	-259/0	-38.5 -38.5 0.21 (2)		
F-L	-246/0	-38.5 -38.5 0.22 (2)		
L-E	-287/0	-38.5 -38.5 0.18 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2016, OBC 2012

- CSA 086-09, CSA 088-14

- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42/1.00 (B-K:1), BC=0.22/1.00 (J-K:2), WB=0.60/1.00 (C-H:1), SSI=0.18/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

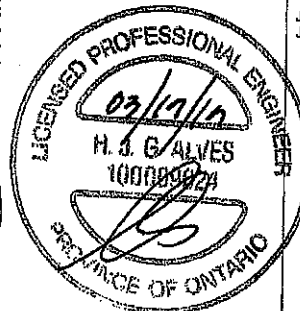
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.60 (C) (INPUT = 0.60)

JSI METAL= 0.30 (D) (INPUT = 1.00)



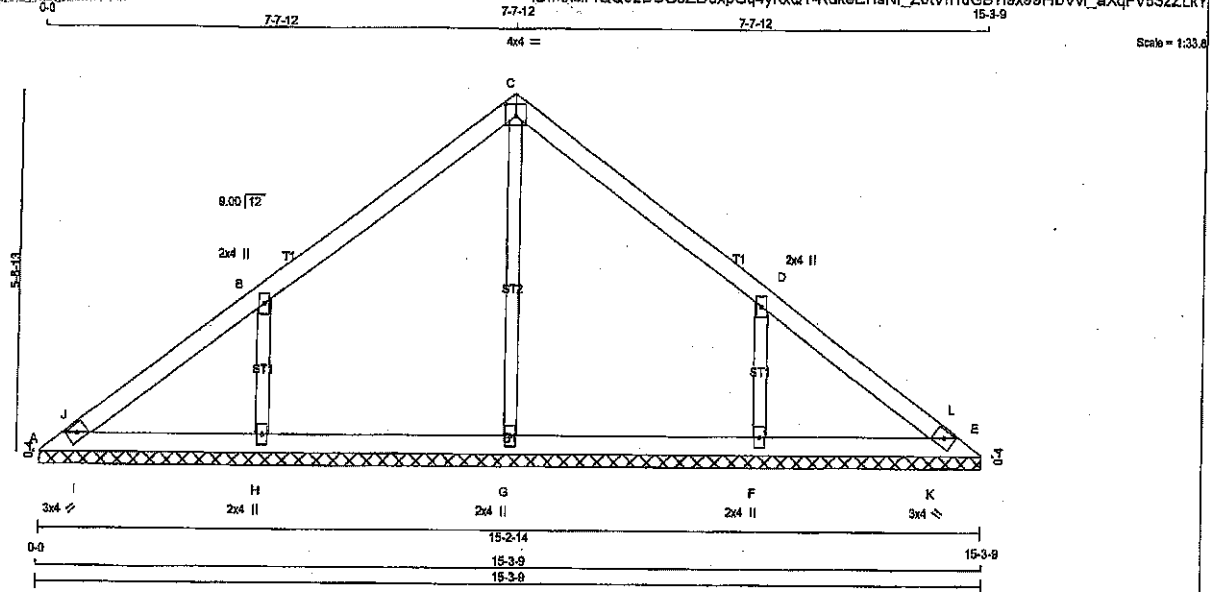
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TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DESIGN NO. TAM 171905724
STRUCTURAL
CONSULTANT ONLY

JOB NAME 401812	TRUSS NAME V12	QUANTITY 1	PLY. 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 6.230 S Nov 17 2016 MITek Industries, Inc. Tue Mar 19 15:02:51 2019 Page 1
ID:mxslFIQQ02DDGsZD9xpGq4yKxQY-RdkeEHsNI_Z0Vh1uGB7i8x99HbVvl_aXqFV5SzZLKY



TOTAL WEIGHT = 48 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW1-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMW1-w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
		GROSS REACTION		GROSS REACTION		BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	175	0	175	0	0	15-2-14	15-2-14
E	175	0	175	0	0	15-2-14	15-2-14
G	512	0	512	0	0	15-2-14	15-2-14
H	641	0	641	0	0	15-2-14	15-2-14
F	641	0	641	0	0	15-2-14	15-2-14

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
A	128	78 / 0	20 / 0	0 / 0	30 / 0	0 / 0
E	128	78 / 0	20 / 0	0 / 0	30 / 0	0 / 0
G	394	182 / 0	104 / 0	0 / 0	107 / 0	0 / 0
H	475	273 / 0	87 / 0	0 / 0	115 / 0	0 / 0
F	475	273 / 0	87 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED	FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	LC1 MAX (CSF)	UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO						FR-TO	
A-J	-14 / 17	-102.1	-102.1	0.05 (1)	6.25	G-C	-369 / 0
J-B	0 / 82	-102.1	-102.1	0.25 (1)	10.00	H-B	-481 / 0
B-C	0 / 38	-102.1	-102.1	0.25 (1)	10.00	F-D	-481 / 0
C-D	0 / 38	-102.1	-102.1	0.25 (1)	10.00	I-J	-48 / 36
D-L	0 / 62	-102.1	-102.1	0.25 (1)	10.00	K-L	-48 / 36
L-E	-14 / 17	-102.1	-102.1	0.05 (1)	6.25		
A-I	-34 / 0	-38.5	-38.5	0.07 (1)	6.25		
I-H	-30 / 0	-38.5	-38.5	0.10 (2)	6.25		
H-G	-33 / 0	-38.5	-38.5	0.10 (2)	6.25		
G-F	-33 / 0	-38.5	-38.5	0.10 (2)	6.25		
F-K	-30 / 0	-38.5	-38.5	0.10 (2)	6.25		
K-E	-34 / 0	-38.5	-38.5	0.07 (1)	6.25		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (D-L:1), BC=0.10/1.00 (F-G:2), WB=0.18/1.00 (C-G:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

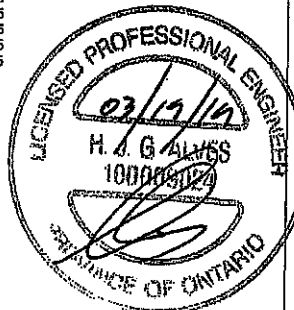
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)

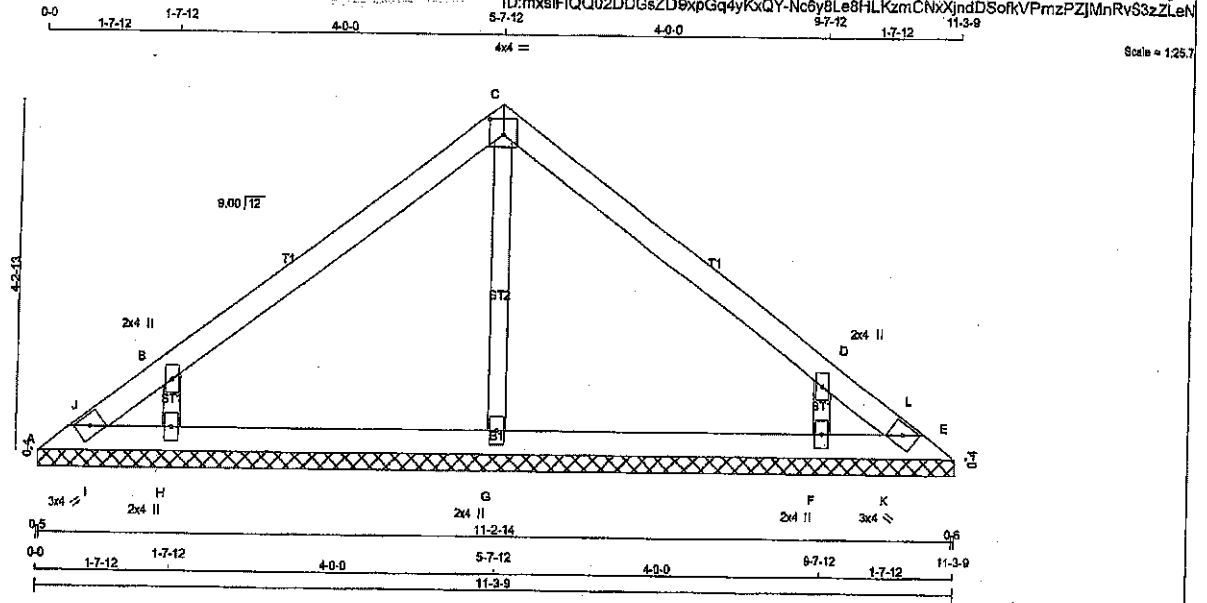
JSI METAL= 0.24 (D) (INPUT = 1.00)



DRWG NO. TAM 71905725
STRUCTURAL
CONSTRUCTION

JOB NAME 401812	TRUSS NAME V13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 5 Nov 17 2018 MTEK Industries, Inc. Tue Mar 19 15:09:28 2019 Page 1
ID:mxslFIQQ02DDGsZD9xpGq4yKxQY-Nc8y8Le8HLKzmCNxXjndDSofKVPmzPZJmRnVS3zZLeN



TOTAL WEIGHT = 32 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
C - E 2x4 DRY No.2
A - E 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TBM1-h	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TTW-p	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F	BMV1-h	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	43	0	43	0	11-3-9 (11-2-11) 3-9	11-3-9
E	43	0	43	0	11-3-9 (11-2-11) 3-9	11-3-9
G	394	0	394	0	11-3-9 (11-2-11) 3-9	11-3-9
H	550	0	550	0	11-3-9 (11-2-11) 3-9	11-3-9
F	550	0	550	0	11-3-9 (11-2-11) 3-9	11-3-9

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	32	19/0	5/0	0/0	0/0	7/0	0/0
E	32	19/0	5/0	0/0	0/0	7/0	0/0
G	305	134/0	85/0	0/0	0/0	85/0	0/0
H	406	240/0	70/0	0/0	0/0	98/0	0/0
F	406	240/0	70/0	0/0	0/0	98/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)	MAX. UNBRAC LENGTH (G)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)		
FR-TO		FROM TO			FR-TO				
A-J	-135/0	-102.1 -102.1	0.01 (3)	6.25	G-C	-224/0	0.08 (1)		
J-B	-26/0	-102.1 -102.1	0.23 (1)	6.25	H-B	-488/0	0.07 (1)		
B-C	-118/0	-102.1 -102.1	0.24 (1)	6.25	F-D	-488/0	0.07 (1)		
C-D	-116/0	-102.1 -102.1	0.24 (1)	6.25	I-J	-4/101	0.00 (1)		
D-L	-28/0	-102.1 -102.1	0.23 (1)	6.25	K-L	-4/101	0.00 (1)		
L-E	-135/0	-102.1 -102.1	0.01 (3)	6.25					
A-I	0/132	-38.5 -38.5	0.06 (1)	10.00					
I-H	0/121	-38.5 -38.5	0.07 (3)	10.00					
H-G	0/73	-38.5 -38.5	0.11 (2)	10.00					
G-F	0/73	-38.5 -38.5	0.11 (2)	10.00					
F-K	0/121	-38.5 -38.5	0.07 (3)	10.00					
K-E	0/132	-38.5 -38.5	0.06 (1)	10.00					

A circular seal with the word "PROFESSIONAL" curved along the bottom edge. The center of the seal contains a stylized graphic of a bridge or arch structure.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (B-C:1), BC=0.11/1.00 (F-G:2), WB=0.07/1.00 (B-H:1), GS=0.16/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

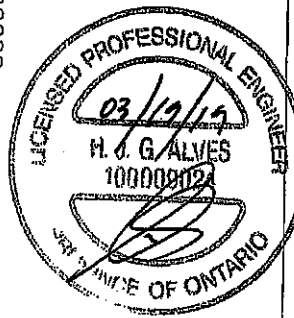
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 789 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

RECEIVED
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TOWN OF CALEDON
BUILDING SECTION
FILE NO.



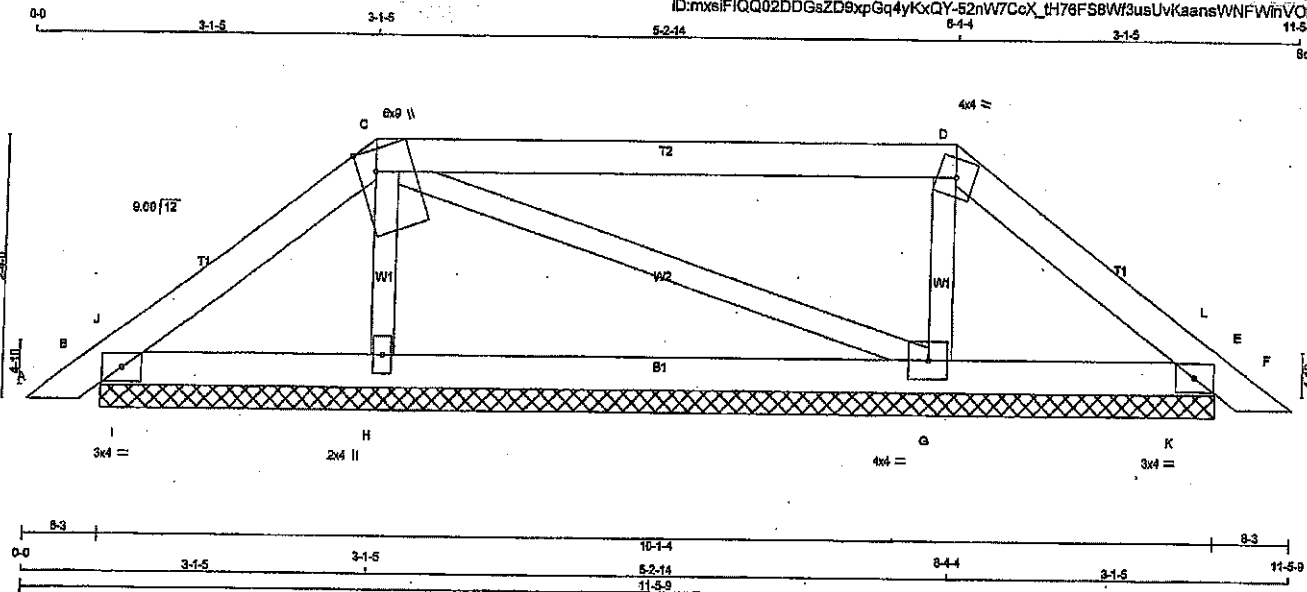
DAVID TAM 71905726
STRUCTURAL
CALCULATION ONLY

JOB NAME 401811	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO. PB1
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:17 2019 Page 1
ID:mxsiFIQQ02DDGsZD8xpGq4yKxQY-52nV7CoX_th76FSBW3usUvKaansWVNFVhVOfczzMcO

Scale = 1:10.0



TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

C - D 2x4 DRY

D - E 2x4 DRY

B - E 2x4 DRY

ALL WEBS 2x3 DRY

DRY, SEASONED LUMBER.

LUMBER

No.2

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ
B	270	0	270	0
E	258	0	258	0
H	494	0	494	0
G	521	0	521	0

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	190	142/0	11/0	0/0	0/0	0/0
E	181	134/0	11/0	0/0	0/0	0/0
H	378	182/0	95/0	0/0	0/0	0/0
G	395	168/0	95/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0/17	-102.1 -102.1	0.03 (1)	H-C	-298/0	0.05 (1)	
B-J	-83/0	-102.1 -102.1	0.03 (1)	C-G	-20/0	0.01 (1)	
J-C	-111/0	-102.1 -102.1	0.07 (1)	G-D	-319/0	0.05 (1)	
C-D	-48/0	-102.1 -102.1	0.46 (1)	I-J	-194/0	0.00 (1)	
D-L	-89/0	-102.1 -102.1	0.07 (1)	K-L	-195/0	0.00 (1)	
L-E	-39/1	-102.1 -102.1	0.03 (1)				
E-F	0/17	-102.1 -102.1	0.03 (1)				
B-I	0/84	-38.5 -38.5	0.03 (1)				
I-H	0/84	-38.5 -38.5	0.13 (2)				
H-G	0/67	-38.5 -38.5	0.13 (2)				
G-K	0/68	-38.5 -38.5	0.13 (2)				
K-E	0/68	-38.5 -38.5	0.08 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2015
- CSA 086-14
- TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.48/1.00 (C-D:1), BC=0.13/1.00 (H-I:2), WB=0.05/1.00 (D-G:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

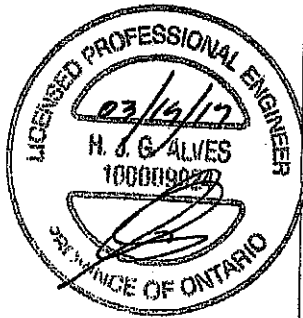
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)

JSI METAL= 0.08 (H) (INPUT = 1.00)

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JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



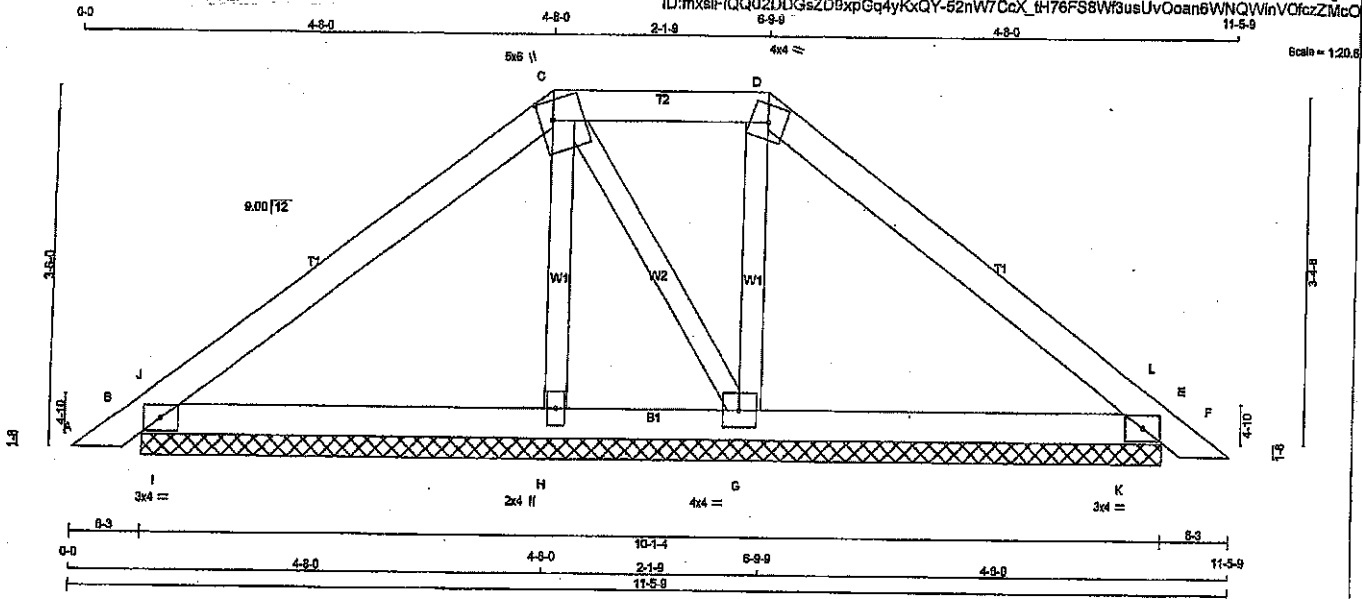
DRWG NO. TAM 17205705
STRUCTURAL
CLIMATE ZONE ONLY

JOB NAME 401811	TRUSS NAME PB2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. PB2
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Tamarack Roof Truss, Burlington

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ID:mxslFIQQ02DDGsZD8xpGq4yKxQY-52nW7CcX_tH76FS8Wf3usUvOan6WNQWVnVOfczZMcC



TOTAL WEIGHT = 2 X 34 = 68 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DESCR.	SPF
SPF	
SPF	
SPF	
SPF	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
B	439	0	439	0	0	10-1-4	10-1-4	10-1-4	10-1-4
E	418	0	418	0	0	10-1-4	10-1-4	10-1-4	10-1-4
H	285	0	285	0	0	10-1-4	10-1-4	10-1-4	10-1-4
G	398	0	398	0	0	10-1-4	10-1-4	10-1-4	10-1-4

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
B	320	202 / 0	46 / 0
E	306	189 / 0	46 / 0
H	221	98 / 0	62 / 0
G	296	166 / 0	59 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 17	-102.1	-102.1	H-C	-125 / 0	0.02 (1)	10.00
B-J	-122 / 0	-102.1	-102.1	C-G	-55 / 0	0.01 (1)	10.00
J-C	-197 / 0	-102.1	-102.1	G-D	-192 / 0	0.04 (1)	10.00
C-D	-118 / 0	-102.1	-102.1	I-J	-373 / 71	0.00 (1)	10.00
D-L	-162 / 0	-102.1	-102.1	K-L	-377 / 70	0.00 (1)	10.00
L-E	-118 / 0	-102.1	-102.1				
E-F	0 / 17	-102.1	-102.1				
B-I	0 / 151	-38.5	-38.5				
I-H	0 / 151	-38.5	-38.5				
H-G	0 / 147	-38.5	-38.5				
G-K	0 / 123	-38.5	-38.5				
K-E	0 / 123	-38.5	-38.5				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015
- CSA 086-14
- TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.21/1.00 (C-J-1), BC=0.18/1.00 (H-I-1), WB=0.04/1.00 (D-G-1), SS=0.32/1.00 (E-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

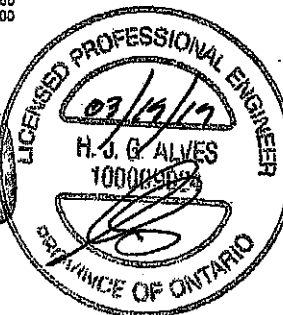
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1857 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.80)
JSI METAL= 0.08 (B) (INPUT = 1.00)

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM T1905706
STRUCTURAL
COMPANY ONLY

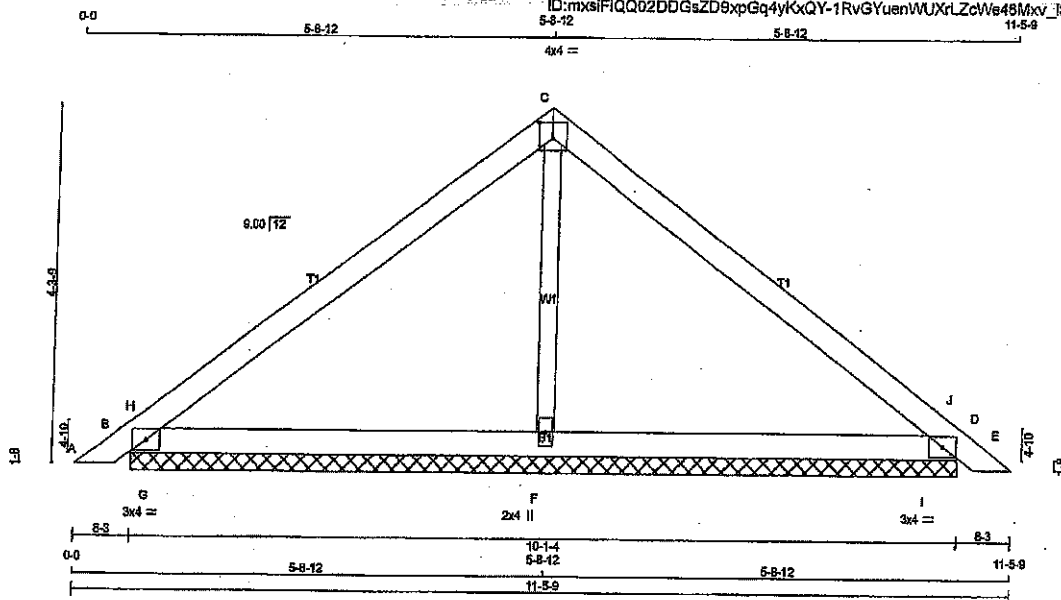
JOB NAME 401811	TRUSS NAME PB3	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO. PB3
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Tamarack Roof Truss, Burlington

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ID:mxsiFIQQ02DDGsZD8xpGq4yKxQY-1RvGYuenWUXrLZcVw46Mxv_ISNQL_G_p95_UjVzZMcM

Scale = 1:25.4



TOTAL WEIGHT = 31 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	528	0	528	0	0	10-1-4	10-1-4
D	528	0	528	0	0	10-1-4	10-1-4
F	485	0	485	0	0	10-1-4	10-1-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERALIVE	WIND	DEAD	SOIL
B	384	246/0	53/0	0/0	0/0	88/0	0/0
D	384	246/0	53/0	0/0	0/0	88/0	0/0
F	377	164/0	107/0	0/0	0/0	105/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO
FR-TO							
A-B	0/17	-102.1	-102.1 0.03 (1)	F-C	-132/10	0.03 (1)	
B-H	-128/25	-102.1	-102.1 0.17 (1)	G-H	-683/45	0.00 (1)	
H-C	-270/0	-102.1	-102.1 0.33 (1)	I-J	-683/45	0.00 (1)	
C-J	-270/0	-102.1	-102.1 0.33 (1)				
J-D	-128/25	-102.1	-102.1 0.17 (1)				
D-E	0/17	-102.1	-102.1 0.03 (1)				
B-G	0/204	-38.5	-38.5 0.29 (1)				
G-F	0/204	-38.5	-38.5 0.33 (1)				
F-I	0/204	-38.5	-38.5 0.33 (1)				
I-D	0/204	-38.5	-38.5 0.29 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015
- CSA 085-14
- TPIC 2014

(55% OF 37.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (C-J:1), BC=0.33/1.00 (F-G:1), WB=0.03/1.00 (C-F:1), SSI=0.55/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

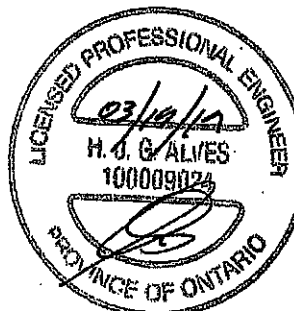
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	650	371	1747 785 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.80)
JSI METAL= 0.11 (B) (INPUT = 1.00)



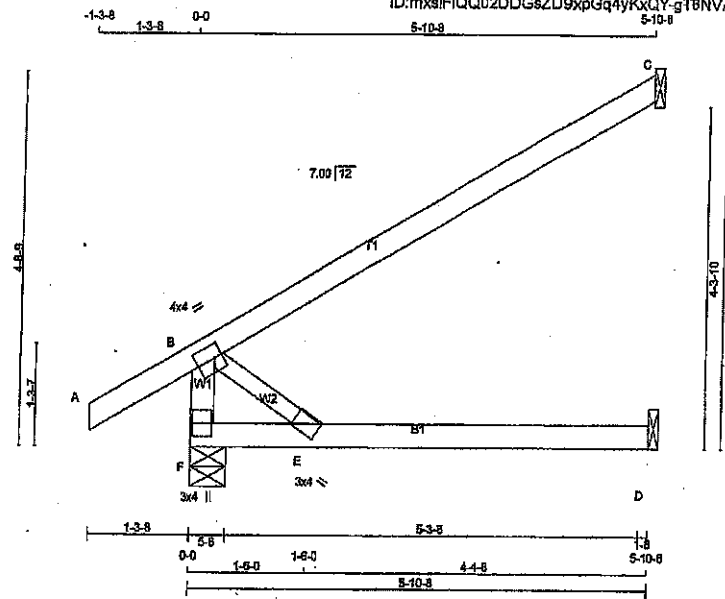
DRWG NO. TAM 77905707
STRUCTURAL
COMPONENT ONLY

JOB NAME 401811	TRUSS NAME J7	QUANTITY 10	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO. J7
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Tamereck Roof Truss, Burlington

Version 5.230 5 Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:14 2019 Page 1
ID:mxslFIQQ02DDGsZD9xpGq4yKxQY-gT8NVAAafhyvYFokZrXWBEHnSMJQJ0G47pGj2HzZMcR

Scale = 1:25.7



TOTAL WEIGHT = 10 X 19 = 185 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE			LUMBER
F - B	2x4	DRY		No.2
A - C	2x4	DRY		No.2
F - D	2x4	DRY		No.2
ALL WEBS	2x3	DRY		No.2
DRY: SEASONED LUMBER.				

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMVW-w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
F	552	0	552	0	5-8	5-8
C	300	0	300	0	1-8	1-8
D	113	0	144	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	404	250 / 0	62 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	208	171 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	103	0 / 0	62 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
F-B	-439 / 0	0.0	0.0	0.04 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 35	-102.1	-102.1	0.14 (1)	10.00				
B-C	0 / 0	-102.1	-102.1	0.60 (1)	10.00				
F-E	0 / 0	-38.5	-38.5	0.24 (3)	10.00				
E-D	0 / 0	-38.5	-38.5	0.31 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015
- CSA 086-14
- TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/625 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/555 (0.13")

CSI: TC=0.60/1.00 (B-C:1), BC=0.31/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	650	371	1747
	788	1887	1873

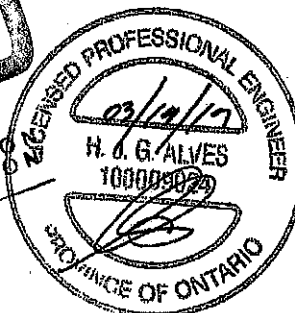
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.80)
JSI METAL= 0.05 (B) (INPUT = 1.00)

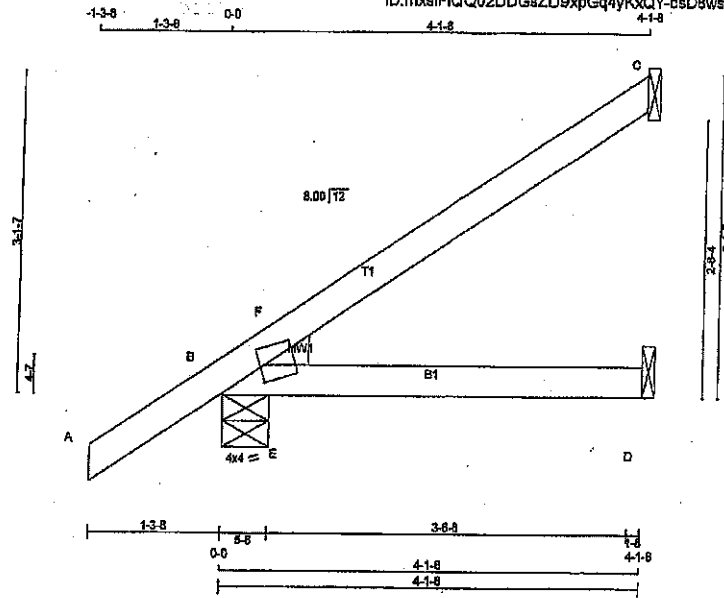
DRWG NO. TAM 71905713
STRUCTURAL
COMPONENT ONLY

RECEIVED
JUN 25 2019
TOWN OF CALEDONIA
BUILDING SECTION
FILE NO.



JOB NAME 401811	TRUSS NAME J8	QUANTITY 5	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 6 Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:16 2019 Page 1
ID:mxsiFIQ02DDGaZD9xpGq4yKxQY-csD8wscvDZ8GUSbyyYfJGMDnAQenwmNT7Iq6AZMcP



TOTAL WEIGHT = 5 X 13 = 64 lb

LUMBER

N.L.G.A. RULES
CHORDS SIZE
A - C 2x4 DRY No.2
B - D 2x4 DRY No.2

LUMBER
No.2

DESCR.
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m MT20	4.0	4.0	2.00	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
C	184	0	184	0	1-8	1-8	
B	430	0	430	0	5-8	5-8	2x4 L
D	108	0	113	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	129	99/0	7/0	0/0	0/0	25/0	0/0
B	313	199/0	43/0	0/0	0/0	70/0	0/0
D	88	22/0	37/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/37	-102.1 -102.1	0.14 (1)	E-F	-171/82	0.00 (1)	
B-F	-47/0	-102.1 -102.1	0.07 (3)				
F-C	0/8	-102.1 -102.1	0.23 (1)				
B-E	0/0	-38.5 -38.5	0.18 (1)				
E-D	0/0	-38.5 -38.5	0.20 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
LL	29.0	PSF
DL	6.0	PSF
BOT CH.	LL	PSF
LL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.23/1.00 (C-F:1), BC=0.20/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM **1905714**
STRUCTURAL
COMPLEMENT ONLY



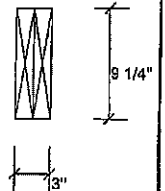
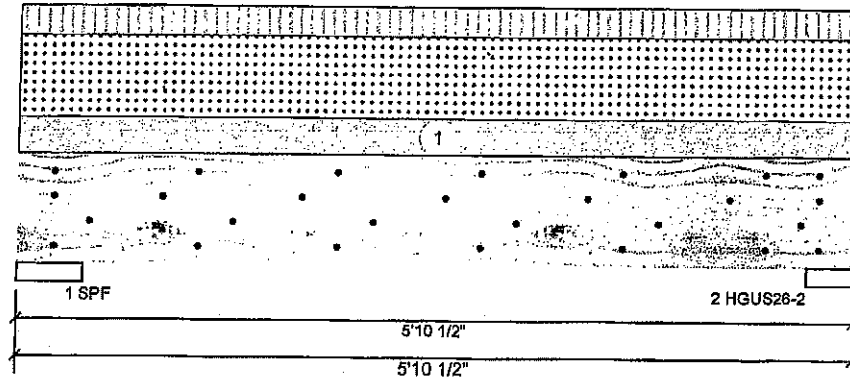
isDesign™

Client:
Project:
Address:Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 5 of 6

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
 Piles: 2
 Moisture Condition: Dry
 Deflection LL: 360
 Deflection TL: 360
 Importance: Normal

Application: Roof (Residential)
 Slope: 0/12
 Design Method: LSD
 Building Code: NBCC 2015
 Load Sharing: No
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	369	457	1019	0
2	353	438	976	0

Bearings and Factored Reactions

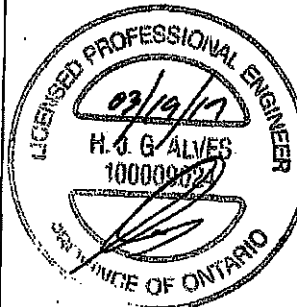
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	24%	571 / 1897	2466 L
2 - HGUS...	4.000"	32%	547 / 1818	2365 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2789 ft-lb	3'	6039 ft-lb	0.462 (46%)	1.25D+1.5S	L
Unbraced	2789 ft-lb	3'	5236 ft-lb	0.533 (53%)	1.25D+1.5S	L
Shear	2142 lb	1'2"	3984 lb	0.536 (54%)	1.25D+1.5S	L
LL Defl inch	0.024 (L/2566)	3'	0.174 (L/360)	0.140 (14%)	S+0.5L	L
TL Defl inch	0.034 (L/1860)	3'	0.174 (L/360)	0.190 (19%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



ENGINEER 7905730
 STRUCTURAL
 COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		11-8-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
 3289 North Service Rd., ON
 Canada
 L7N3G2
 (905) 335-1115



This design is valid until 12/11/2021





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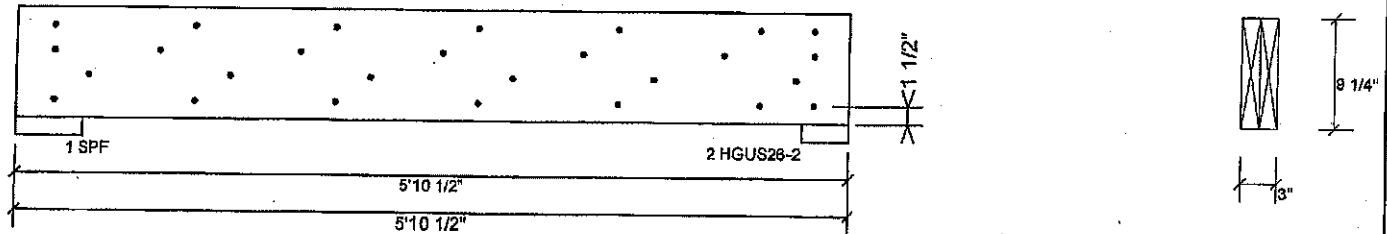
Client:
Project:
Address:

Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 6 of 6

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	90.7 %
Load	411.3 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



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17:05:30
STRUCTURAL
1/2

Manufacturer Info

TOWN &
BUILDING
FILE NO

Tamarack Rod, Tubes
6288 North Service Rd. N.
Canada
L7N3G2
(905) 335-1115

TAMARACK
LUMBER INC
ALPHA LUMBER GROUP

This design is valid until 12/11/2021





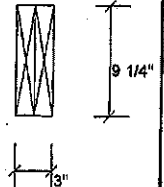
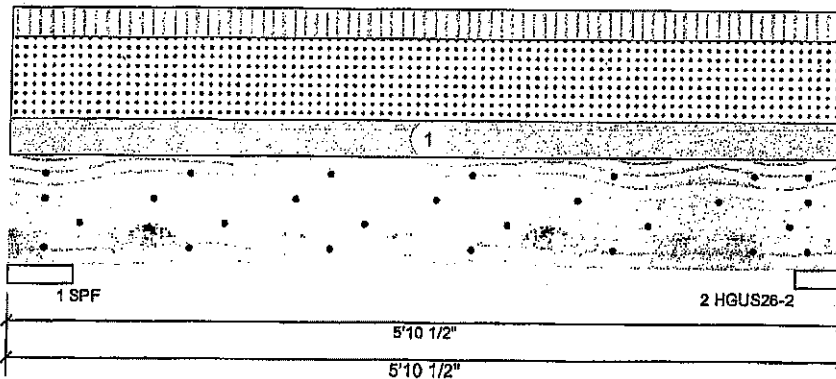
isDesign™

Client:
Project:
Address:Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 3 of 6

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	403	498	1113	0
2	388	478	1067	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	27% 624 / 2072	2696 L		1.25D+1.5S +L
2 - HGUS...	4.000"	35% 598 / 1986	2584 L		1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3047 ft-lb	3'	6039 ft-lb	0.505 (60%)	1.25D+1.5S +L	L
Unbraced	3047 ft-lb	3'	5236 ft-lb	0.582 (58%)	1.25D+1.5S +L	L
Shear	2340 lb	4'10"	3984 lb	0.587 (59%)	1.25D+1.5S +L	L
LL Defl inch	0.027 (L/2348)	3'	0.174 (L/360)	0.150 (15%)	S+0.5L	L
TL Defl inch	0.037 (L/1702)	3'	0.174 (L/360)	0.210 (21%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1905124
STRUCTURAL
APPROVED ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		12-8-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/31/2021





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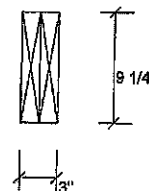
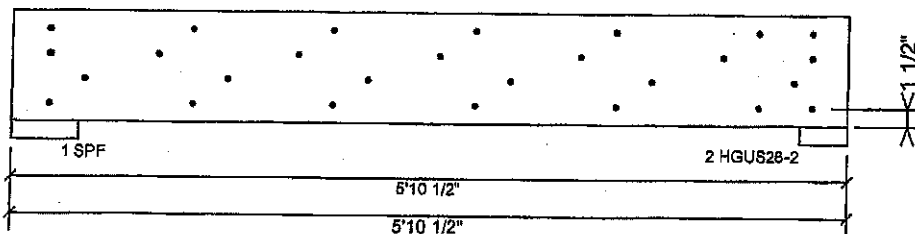
Client:
Project:
Address:

Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 4 of 6

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

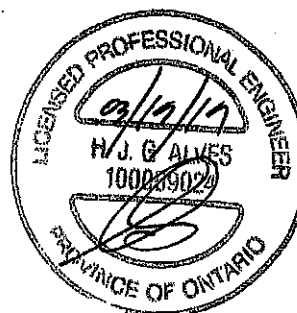


Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	99.1 %
Load	449.3 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	II
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**



RECEIVED
JUN 25 2019

Manufacturer Info	Tamarack Roof Trusses ON 6289 North Road, #100 Canada Centre E7N3G2 (905) 385-1115
TOWN OF BUILDING FILE NO	TAMARACK LUMBER INC ALPHA LUMBER GROUP

This design is valid until: 12/11/2021



LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

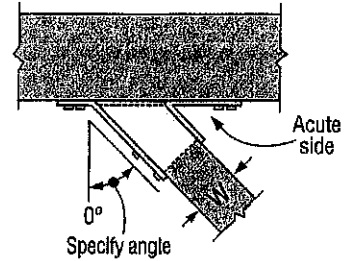
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger

Skewed Right

(Joist must be bevel cut)

All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _B ^a	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
Single 2x Sizes											
LUS24	18	1 1/8	3 1/2	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								3.16	7.23	2.87	5.14
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								1.60	4.54	1.42	3.22
LU26L	22	1 1/8	5	1 1/4	4 3/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								3.20	7.14	2.87	5.07
LUS26	18	1 1/8	4 3/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								6.32	9.85	5.74	7.25
HUS26	16	1 1/8	5 3/4	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
								11.30	21.97	9.20	17.24
LJS26DS	18	1 1/8	5	3 1/2	4 3/4	(16) 16d	(6) 16d	2055	4265	1460	4115
								9.14	18.97	6.49	18.31
HGUS26	12	1 1/8	5 3/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
								11.96	29.51	11.96	25.35
LU28L	20	1 1/8	6 3/4	1 1/4	5 3/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								5.07	9.72	4.54	6.89
LUS28	18	1 1/8	6 3/4	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								6.32	11.21	5.74	7.96
HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345
								16.04	23.86	11.90	19.33
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(38) 16d	(12) 16d	3310	7675	3310	6900
								14.74	34.19	14.74	30.73
LU210L	20	1 1/8	8	1 1/4	7 3/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								5.07	11.10	4.54	7.87
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								6.32	12.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_g is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply glider truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.182" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

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TOWN OF CALEDON
 BUILDING SECTION
 FILE NO

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

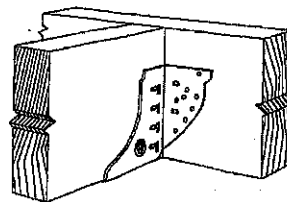
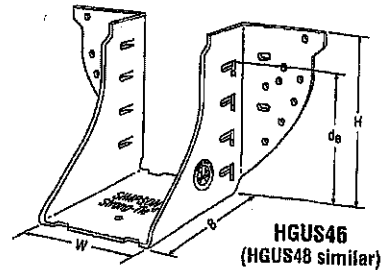
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

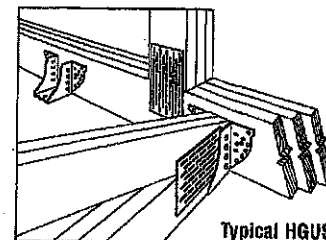
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



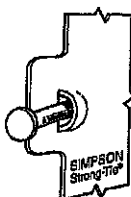
Typical HGUS Installation



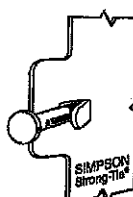
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)					Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₁	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS48	12	3%	7 1/4	4	6 1/8	36-16d	12-16d		6070	12980	4310	9215
HGUS410	12	3%	9	4	8 1/8	46-16d	16-16d		6840	14645	4855	10400
HGUS412	12	3%	10 1/8	4	10 1/8	56-16d	20-16d		7640	14995	5425	10645
HGUS414	12	3%	12 1/8	4	11 1/8	66-16d	22-16d		10130	16400	7195	11645
HGUS5.50/8	12	5 1/2	6 1/8	4	6%	36-16d	12-16d		6070	12980	4310	9215
HGUS5.50/10	12	5 1/2	8 1/8	4	8%	46-16d	16-16d		6840	14645	4855	10400
HGUS5.50/12	12	5 1/2	10 1/8	4	10%	56-16d	20-16d		7640	14995	5425	10645
HGUS5.50/14	12	5 1/2	12 1/8	4	11%	66-16d	22-16d		10130	16400	7195	11645
HGUS7.25/8	12	7 1/4	7 1/8	4	6 1/8	36-16d	12-16d		6070	12980	4310	9215
HGUS7.25/10	12	7 1/4	8%	4	8%	46-16d	16-16d		6840	15760	4855	11190
HGUS7.25/12	12	7 1/4	10%	4	10%	56-16d	20-16d		7640	16110	5425	11435
HGUS7.25/14	12	7 1/4	12%	4	11%	66-16d	22-16d		10130	18200	7195	12920

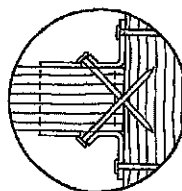
1. d_g is the distance from the seat of the hanger to the highest joist nail.



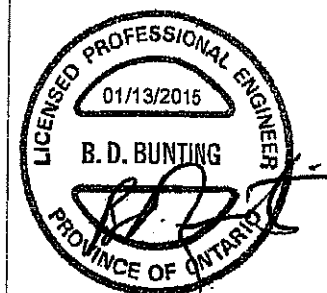
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



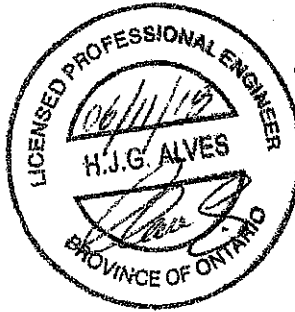
LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2016. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECDSHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

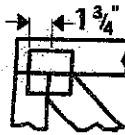
- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

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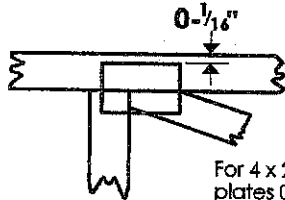
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BUILDING SECTION
FILE NO _____

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

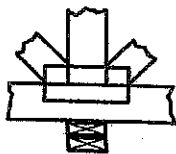
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



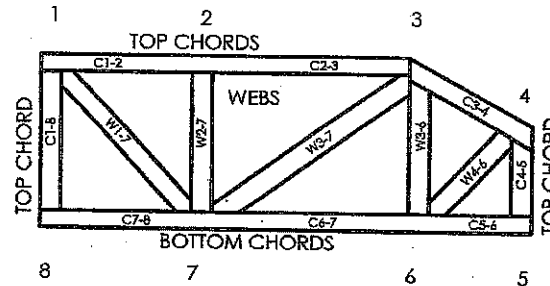
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

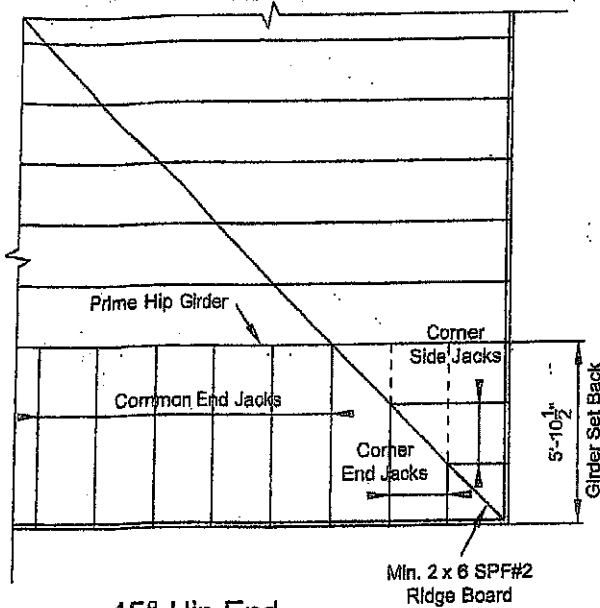
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



45° Hip End

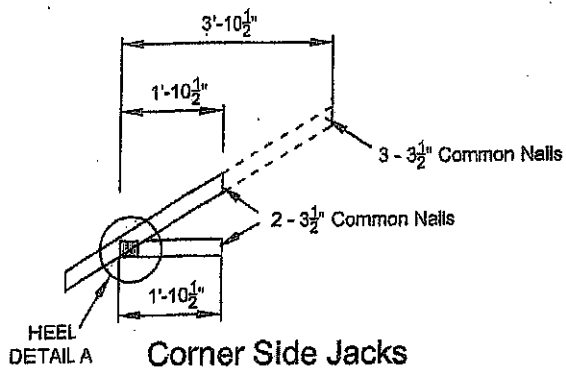
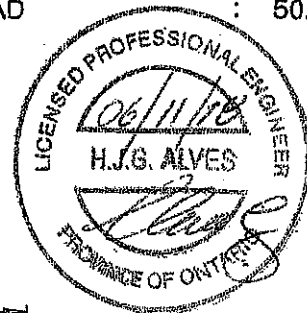
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

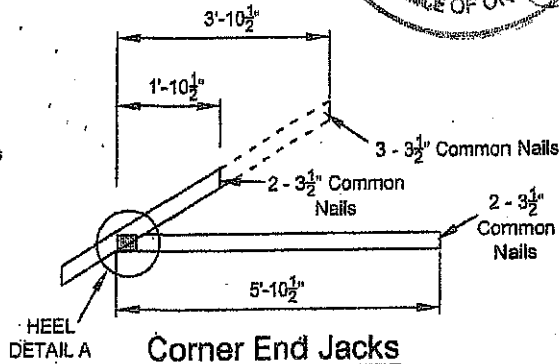
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

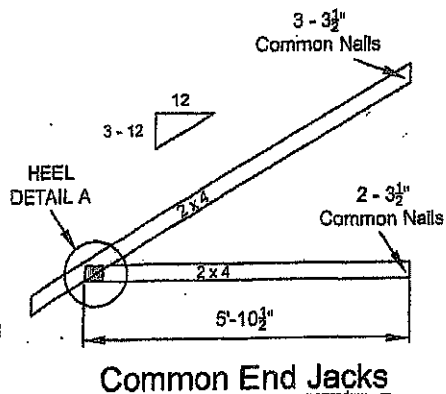
TOTAL LOAD : 50.5 P.S.F



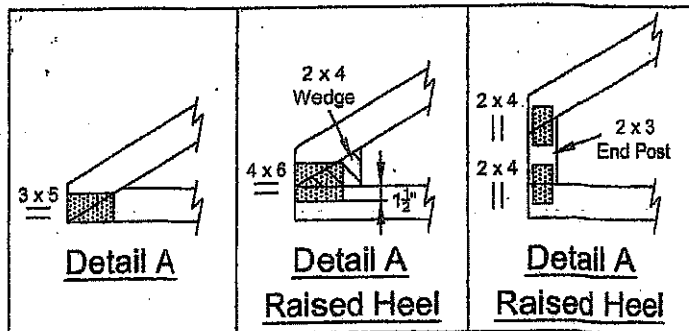
Corner Side Jacks



Corner End Jacks

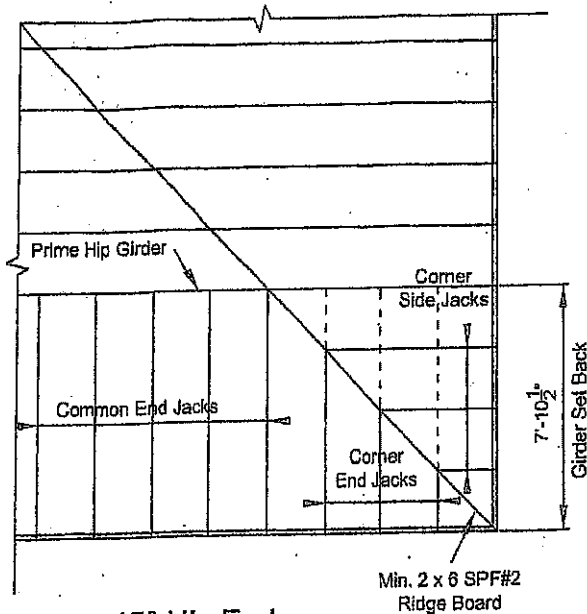


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

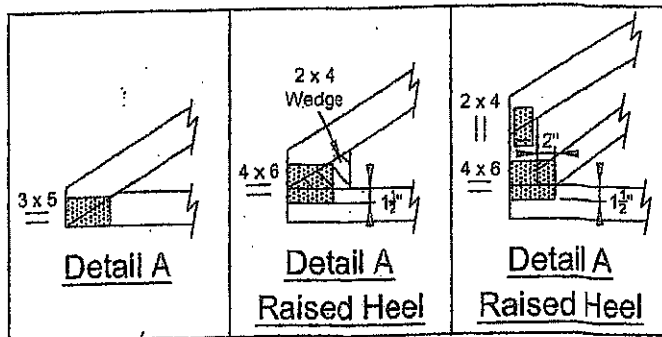
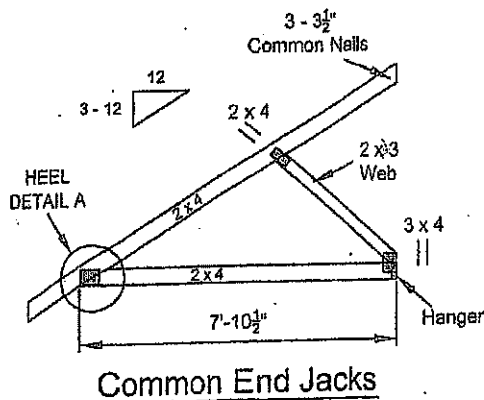
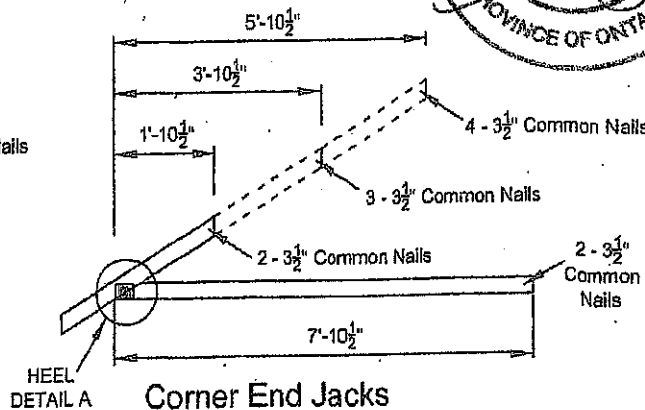
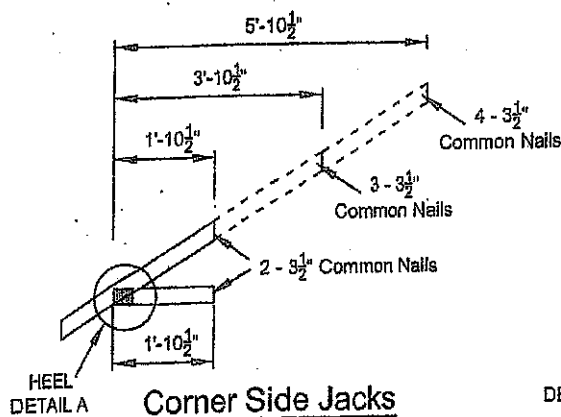
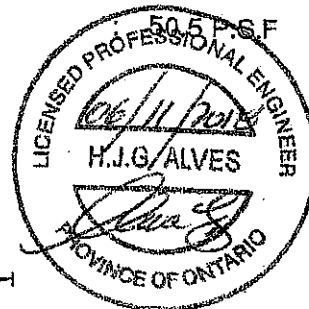
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217