

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

CERTIFIED MODEL
PRE-APPROVED
FOR PERMIT APPLICATION AS PER THE
ONTARIO BUILDING CODE
TOWN OF CALEDON BUILDING DIVISION
REVIEWED BY AS S. SVELL
DATE AUG 7/19
FILE # KINMOUNT 6A-ELEV123
OPT. DECK OPT. W.O.B.

APPLICANT COPY

9/12 PITCHES (TYP.)
UNLESS NOTED

HARDWARE:

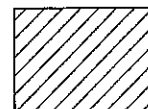
HUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

DENOTE:

ALL B- 2-2X10 (FLUSH)

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

DENOTES:
CONVENTIONAL
FRAMING



6.5

6.5

6.5

4.0

6.0

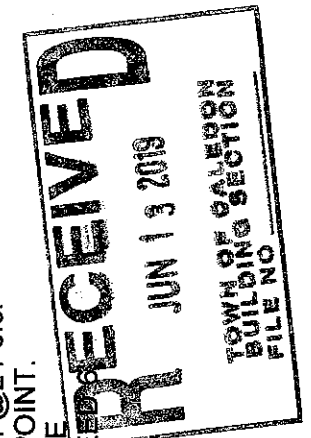
12" HIGHER
FASCIA

5/12 CATH.
CEILING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

GROUND SNOW LOAD $S_s = 1.8 \text{ kPa}$
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



M 10/25/2



Job Track: **50120**
Plan Log: **200598**
Layout ID: **403045**

Builder / Location:
GREEN PARK HOMES / Caledon
Project: **LAMBERTS LANE HOME CORP.**
Date: 5/8/2019 Sales: **Mario DiCano** Designer: JG

Model / Elevation:
KINMOUNT 6A/1

Mike ver 8.2.3.229

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



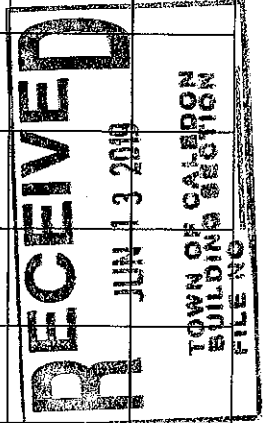
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #:
 Elevation: 1

Job Track: 50120
 PlanLog: 200598
 Layout ID: 403045
 Ref #
 Page: 1 of 2
 Date: 05/08/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	31-11-00	4-04-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	317.54 198.00		
	1 2-ply	T1Z Hip Girder	9/12	31-11-00	4-04-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	317.54 198.00		
	2	T2 Hip	9/12	31-11-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	267.53 170.33		
	2	T3 Hip	9/12	31-11-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.12 177.00		
	2	T4 Hip	9/12	31-11-00	7-07-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	294.24 187.33		
	2	T5 Hip	9/12	31-11-00	8-08-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	307.46 193.00		
	6	T6 Hip	9/12	31-11-00	9-09-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	966.5 610.00		
	2	T7 Hip	9/12	31-11-00	10-10-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	334.93 208.00		
	4	T8 Piggyback Base	9/12	26-03-00	9-00-00	2 x 4	1-03-08	1-06-04 2-06-04	502.56 320.67		
	3	T9 Scissor	9/12 5/12	15-01-00	7-02-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	193.33 124.00		
	1	T10 Hip Girder	9/12	14-09-00	4-04-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	71.7 45.00		
	1	T11 Hip	9/12	14-09-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	64.67 42.50		
	1	T12 Hip	9/12	14-09-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	69.72 46.67		
	2	T91 Common	6/12	7-09-00	2-10-15	2 x 4	1-03-08 1-03-08	6-12 6-12	50.42 31.33		



DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #:
 Elevation: 1

Job Track: 50120
 PlanLog: 200598
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 Page: 2 of 2
 Date: 05/08/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

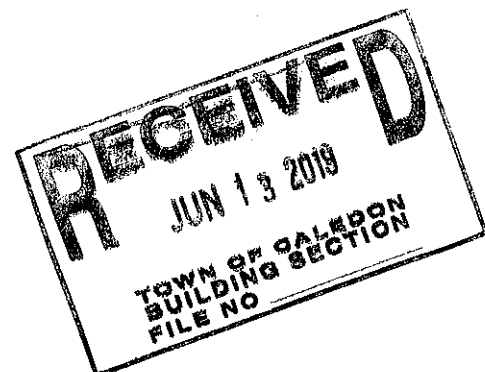
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T91Z Common Girder	6 /12	7-09-00	2-10-15	2 x 4	1-03-08 1-03-08	6-12 6-12	25.21 15.67		
	2	P1 Piggyback	9 /12	9-09-05	2-02-00	2 x 4			55.64 36.00		
	2	P2 Piggyback	9 /12	9-09-05	3-03-00	2 x 4			54.88 36.67		
	4	P3 Piggyback	9 /12	7-07-11	2-10-06	2 x 4			72.19 45.33		
	17	J1 Jack-Open	6.5 /12	5-10-08	4-04-14	2 x 4	1-03-08	1-02-11 4-04-14	289.69 181.33		
	4	J90 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	57.79 37.33		
	3	J91 Jack-Open	10 /12	1-09-02	2-11-01	2 x 4	1-03-08	1-00-07 2-06-00	22.51 16.00		

TOTAL # TRUSS= 65 TOTAL BFT OF ALL TRUSSES= 2920.16 BFT. TOTAL WEIGHT OF ALL TRSSES 4619.36 LBS

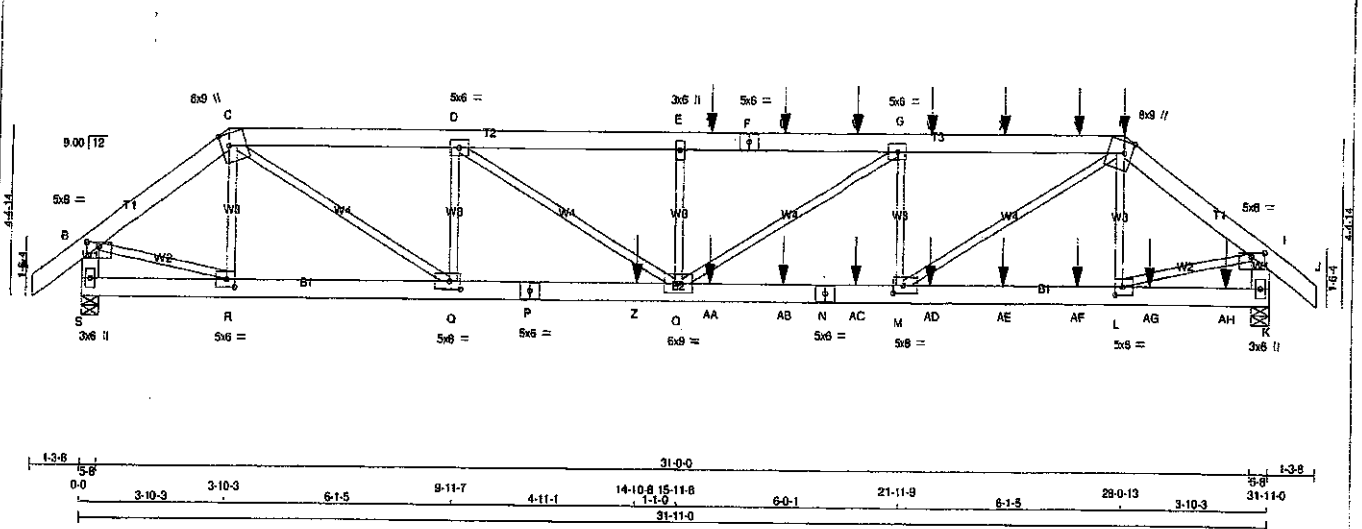
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
6	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 8



JOB NAME 403045	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 5 Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:19 2019 Page 1	ID: D7ulPnW97DzGkx1HTFaFreyNCCCH-TNB_NexVtBrnTACJOfndcoKmgXziCVuKB9bFN62GdDk
Scale = 1:53.3					



TOTAL WEIGHT = 2 X 170 = 340 lb

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF
C - F	2x6 DRY	No.2	SPF
F - H	2x6 DRY	No.2	SPF
H - J	2x6 DRY	No.2	SPF
S - B	2x6 DRY	No.2	SPF
K - I	2x6 DRY	No.2	SPF
S - P	2x6 DRY	1650F 1.5E	SPF
P - N	2x6 DRY	1650F 1.5E	SPF
N - K	2x6 DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A-C 2	12	TOP SIDE(0.0)
C-F 2	12	SIDE(61.0)
F-H 2	12	SIDE(122.0)
H-J 2	12	TOP
S-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
S-P 2	12	TOP
P-N 2	12	SIDE(0.0)
N-K 2	12	SIDE(0.0)
WEBS : (0.122'X3') SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
BT TYPE PLATES				
B TMVW-p MT20	5.0	8.0	1.25	4.00
C TTWW-m MT20	8.0	9.0	3.75	2.50
D TMVW-t MT20	5.0	6.0		
E TMVW-w MT20	3.0	6.0		
F TS-t MT20	5.0	6.0		
G TMVW-t MT20	5.0	6.0		
H TTWW-m MT20	8.0	9.0	3.75	2.50
I TMVW-p MT20	5.0	8.0	1.25	4.00
K BMVW-t MT20	3.0	6.0		
L BMVW-t MT20	5.0	8.0	2.50	2.50
M BMVW-t MT20	5.0	8.0	2.50	3.50
N BS-t MT20	5.0	6.0		
O BMVW-w MT20	6.0	9.0		
P BS-t MT20	5.0	6.0		
Q BMVW-t MT20	5.0	8.0	2.50	3.50
R BMVW-t MT20	5.0	8.0	2.50	2.50
S BMVW-t MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION	FACTORED	GROSS REACTION	FACTORED	BRG	BRG	IN-SX	IN-SX
JT VERT	4021	4021	0	0	5-8	5-8	5-8
S	4021	4021	0	0	5-8	5-8	5-8
K	4602	4602	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS	
1ST CASE	2ND CASE	1ST CASE	2ND CASE
JT COMBINED	1720 / 0	543 / 0	0 / 0
S	1720 / 0	543 / 0	0 / 0
K	1946 / 0	643 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

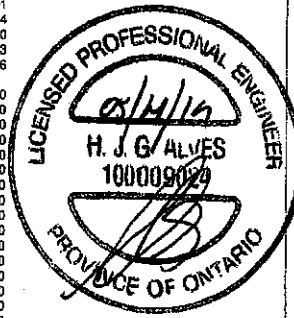
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 44	R-C	-763 / 0
B-C	-4484 / 0	C-Q	0 / 5495
C-D	-8178 / 0	Q-D	-2320 / 0
D-E	-10720 / 0	D-D	0 / 3041
E-F	-10720 / 0	O-E	-709 / 0
F-T	-10720 / 0	O-G	0 / 2701
F-U	-10720 / 0	M-G	-2599 / 0
U-V	-10720 / 0	M-H	0 / 5137
V-G	-10720 / 0	L-H	-585 / 38
G-W	-8463 / 0	B-R	0 / 3688
W-X	-8463 / 0	L-I	0 / 4288
X-Y	-8463 / 0		
Y-H	-8463 / 0		
H-I	-5213 / 0		
I-J	0 / 44		
S-B	-3959 / 0		
K-I	-4547 / 0		
S-R	0 / 0		
R-Q	0 / 3554		
Q-P	0 / 8178		
P-Z	0 / 8178		
Z-O	0 / 8178		
O-AA	0 / 8463		
AA-AB	0 / 8463		
AB-N	0 / 8463		
N-AC	0 / 8463		
AC-M	0 / 8463		
M-AD	0 / 4140		
AD-AE	0 / 4140		
AE-AF	0 / 4140		
AF-L	0 / 4140		
L-AG	0 / 0		
AG-AH	0 / 0		
AH-K	0 / 0		

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LOC.	MAX.	LOC.	MAX.	LOC.	MAX.	LOC.	MAX.	LOC.	MAX.
H	28-0-13	-49	-55	---	---	FRONT	VERT	DEAD	---	---	---
H	28-0-13	-282	-282	---	---	FRONT	VERT	SNOW	---	---	---
T	16-9-12	-123	-123	---	---	FRONT	VERT	TOTAL	---	---	---
U	18-9-12	-123	-123	---	---	FRONT	VERT	TOTAL	---	---	---



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 7.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = L/360 (1.06")
CALCULATED VERT. DEFL (LL) = L/999 (0.25")
ALLOWABLE DEFL (TL) = L/360 (1.06")
CALCULATED VERT. DEFL (TL) = L/921 (0.42")

CSI: TC=0.44/1.00 (E-G:1), BC=0.70/1.00 (O-Q:1), WB=0.88/1.00 (C-Q:1), SS=0.69/1.00 (O-Q:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	618	354	1867
			1867

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

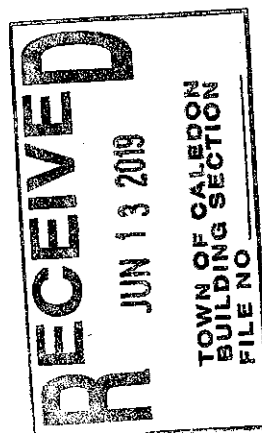
JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.91 (N) (INPUT = 1.00)

Structural component only
DWG# T-1911482 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:19 2019 Page 2
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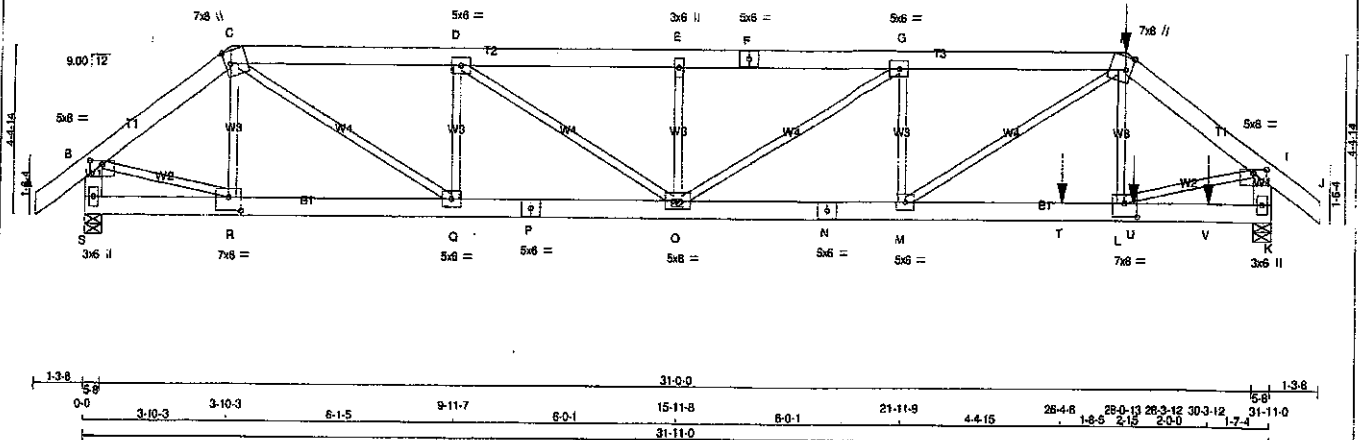
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	20-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
W	22-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
X	24-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
Y	26-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
Z	14-10-8	-2332	-2332	---	FRONT	VERT	TOTAL	---	---
AA	16-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	18-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AC	20-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AD	22-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AE	24-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AF	26-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AG	28-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AH	30-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---



Structural component only
DWG# T-1911482

JOB NAME 403045	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 MTak Industries, Inc. Tue May 14 10:41:20 2019 Page 1
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 1-3-8 0-0 3-10-3 3-10-3 6-1-5 9-11-7 6-0-1 15-11-8 6-0-1 21-11-9 6-1-5 29-0-13 3-10-3 1-3-8
 Scale = 1:50.0



TOTAL WEIGHT = 2 X 170 = 340 lb

CHORDS	SIZE	DRY	SPF	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
P - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 2 12	TOP	
C-F 2 12	TOP	
F-H 2 12	SIDE(61.0)	
H-J 2 12	SIDE(122.0)	
S-B 2 12	TOP	
K-I 2 12	TOP	
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S-P 2 12	TOP	
P-N 2 12	TOP	
N-K 2 12	SIDE(183.1)	
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	4.00
C	TTVW+m	MT20	7.0	8.0	4.00	1.75
D	TMVW-i	MT20	5.0	8.0		
E	TMVW+w	MT20	3.0	6.0		
F	TS-i	MT20	5.0	6.0		
G	TMVW-i	MT20	5.0	6.0		
H	TTVW+m	MT20	7.0	8.0	4.00	1.75
I	TMVW-p	MT20	5.0	8.0	1.25	4.00
J	BMV1+p	MT20	3.0	6.0		
L	BMVW-i	MT20	7.0	8.0	4.25	4.00
M	BMVW-i	MT20	5.0	6.0		
N	BS-i	MT20	5.0	6.0		
O	BMVW-w	MT20	5.0	6.0		
P	BS-i	MT20	5.0	6.0		
Q	BMVW-i	MT20	5.0	6.0		
R	BMVW-i	MT20	7.0	8.0	4.25	4.00
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION			BRG	BRG
JT VERT	3010	0	0	5-8
S VERT	5542	0	0	5-8
K VERT	5542	0	0	5-8

UNFACTORED REACTIONS	MAX/MIN COMPONENT REACTIONS
1ST LOASE	
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
S 2236	1277 / 0 417 / 0 0 / 0 0 / 0 542 / 0 0 / 0
K 4112	2362 / 0 755 / 0 0 / 0 0 / 0 995 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 44	-102.1 -102.1	0.04 (1)	10.00	R-C	-427 / 0	0.06 (1)
B-C	-3254 / 0	-102.1 -102.1	0.09 (1)	6.06	C-Q	0 / 3532	0.44 (1)
C-D	-5555 / 0	-102.1 -102.1	0.19 (1)	4.85	D-O	-1641 / 0	0.22 (1)
D-E	-7045 / 0	-102.1 -102.1	0.21 (1)	4.28	O-D	0 / 1733	0.22 (1)
E-F	-7045 / 0	-102.1 -102.1	0.19 (1)	4.40	O-E	-594 / 0	0.08 (1)
F-G	-7045 / 0	-102.1 -102.1	0.19 (1)	4.40	O-G	-683 / 0	0.33 (1)
G-H	-7615 / 0	-102.1 -102.1	0.25 (1)	4.21	M-G	-280 / 65	0.03 (1)
H-I	-8846 / 0	-102.1 -102.1	0.14 (1)	4.49	M-H	0 / 183	0.31 (1)
I-J	0 / 44	-102.1 -102.1	0.04 (1)	10.00	L-H	0 / 183	0.31 (1)
S-B	-2967 / 0	0.0 0.0	0.11 (1)	7.81	B-R	0 / 183	0.31 (1)
K-I	-5984 / 0	0.0 0.0	0.21 (1)	6.09	L-K	0 / 183	0.31 (1)
S-R	0 / 0	-38.5 -38.5	0.04 (2)	10.00			
R-Q	0 / 2582	-38.5 -38.5	0.14 (1)	10.00			
Q-P	0 / 5555	-38.5 -38.5	0.29 (1)	10.00			
P-O	0 / 5555	-38.5 -38.5	0.29 (1)	10.00			
O-N	0 / 7615	-38.5 -38.5	0.52 (1)	10.00			
N-M	0 / 7615	-38.5 -38.5	0.52 (1)	10.00			
M-T	0 / 5506	-38.5 -38.5	0.83 (1)	10.00			
T-L	0 / 5506	-38.5 -38.5	0.83 (1)	10.00			
L-U	0 / 0	-38.5 -38.5	0.44 (1)	10.00			
U-V	0 / 0	-38.5 -38.5	0.44 (1)	10.00			
V-K	0 / 0	-38.5 -38.5	0.44 (1)	10.00			

05/14/16

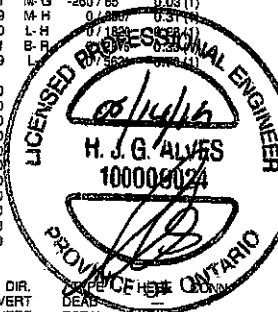
H. J. G. ALVE

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	DEAD
H	28-0-13	-49	-55	---	FRONT	VERT	---
H	28-0-13	-129	-129	---	BACK	VERT	---
H	28-0-13	-282	-282	---	FRONT	VERT	---
T	28-4-8	-3203	-3203	---	BACK	VERT	---
U	28-3-12	-55	-70	---	BACK	VERT	---
V	30-3-12	-55	-70	---	BACK	VERT	---



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.25/1.00 (G-H:1) , BC=0.83/1.00 (L-M:1) ,
 WB=0.70/1.00 (L-L:1) , SS=0.87/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MT20	618	354 1657 789 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
 JSI METAL= 0.79 (N) (INPUT = 1.00)

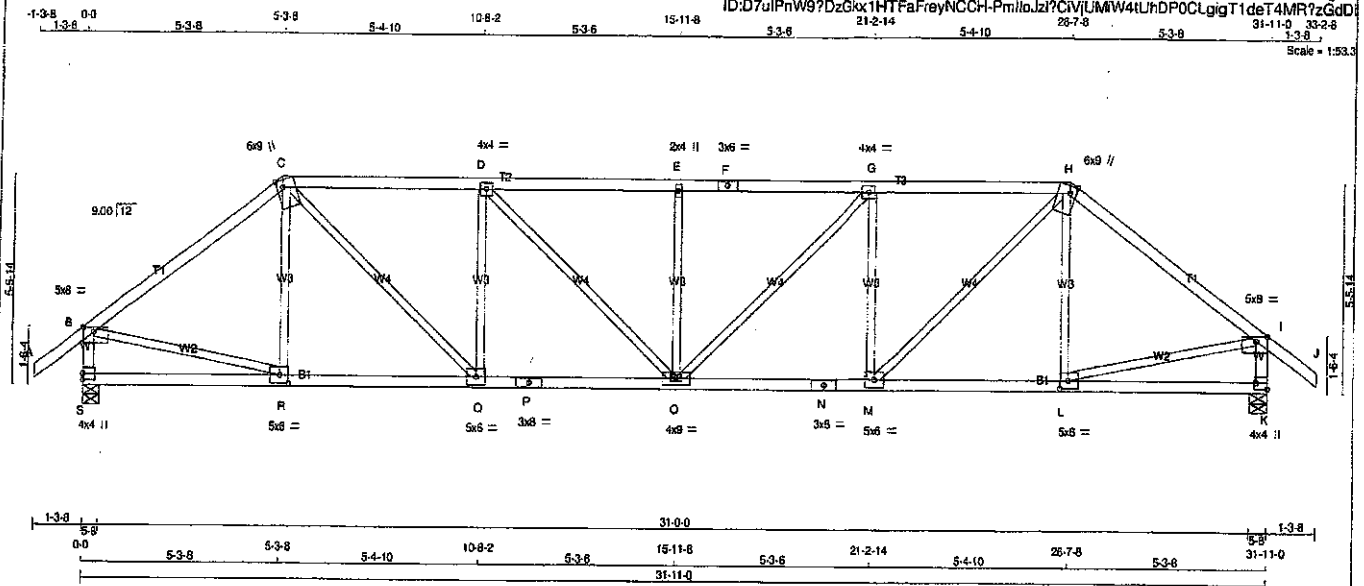
Structural component only
 DWG# T-1911483

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T2	2	1	GREEN PARK HOMES	
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 134 = 268 lb

N. L. & A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW+m	MT20	6.0	9.0	Edge 2.00	
D	TMVW-i	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TS-i	MT20	3.0	6.0		
G	TMVW-i	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge 2.00	
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
K	BMV1-p	MT20	4.0	4.0	2.00	Edge
L	BMVW-i	MT20	5.0	8.0	2.50	2.75
M	BMVW-i	MT20	5.0	8.0		
N	BS-i	MT20	3.0	8.0		
O	BMVW+i	MT20	4.0	9.0		
P	BS-i	MT20	3.0	8.0		
Q	BMVW-i	MT20	5.0	8.0		
R	BMVW-i	MT20	5.0	8.0	2.50	2.75
S	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	HORIZ	UPLIFT
S	2385	0	2385	0
K	2385	0	2385	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
S	1773	1007 / 0 335 / 0 0 / 0 0 / 0 431 / 0 0 / 0
K	1773	1007 / 0 335 / 0 0 / 0 0 / 0 431 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			FACTORED			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)		VERT. LOAD (PLF)	LC1 MAX. CSI (L/C)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (L/C)	
FR-TO			FROM TO			FR-TO			
A-B	0 / 42		-102.1	-102.1	0.14 (1)	10.00	R-C	-195 / 98	0.09 (1)
B-C	-2429 / 0		-102.1	-102.1	0.73 (1)	3.67	C-Q	0 / 1667	0.38 (1)
C-D	-3138 / 0		-102.1	-102.1	0.65 (1)	3.34	Q-D	-951 / 0	0.43 (1)
D-E	-3494 / 0		-102.1	-102.1	0.69 (1)	3.14	D-O	0 / 500	0.11 (1)
E-F	-3494 / 0		-102.1	-102.1	0.69 (1)	3.14	O-E	-497 / 0	0.22 (1)
F-G	-3494 / 0		-102.1	-102.1	0.69 (1)	3.14	G-O	0 / 500	0.11 (1)
G-H	-3138 / 0		-102.1	-102.1	0.65 (1)	3.34	M-G	-981 / 0	0.43 (1)
H-I	-2429 / 0		-102.1	-102.1	0.73 (1)	3.67	M-H	0 / 1667	0.38 (1)
I-J	0 / 42		-102.1	-102.1	0.14 (1)	10.00	L-H	-195 / 98	0.09 (1)
S-B	-2301 / 0	0.0	0.0	0.0	0.24 (1)	5.62	B-R	0 / 1987	0.45 (1)
K-I	-2301 / 0	0.0	0.0	0.0	0.24 (1)	5.62	L-I	0 / 1987	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/999 (0.16")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/999 (0.29")

CSI: TC=0.73/1.00 (H-I), BC=0.60/1.00 (O-Q), I: WB=0.45/1.00 (H-L), SI=0.25/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (L) (INPUT = 0.90)

JSI METAL = 0.96 (P) (INPUT = 1.00)

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JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

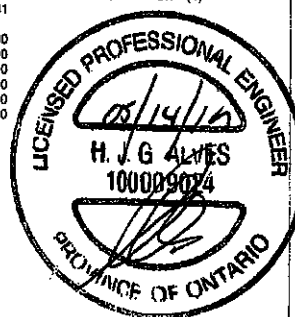
LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
1000091024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1911484

Tamarack Roof Truss, Burlington

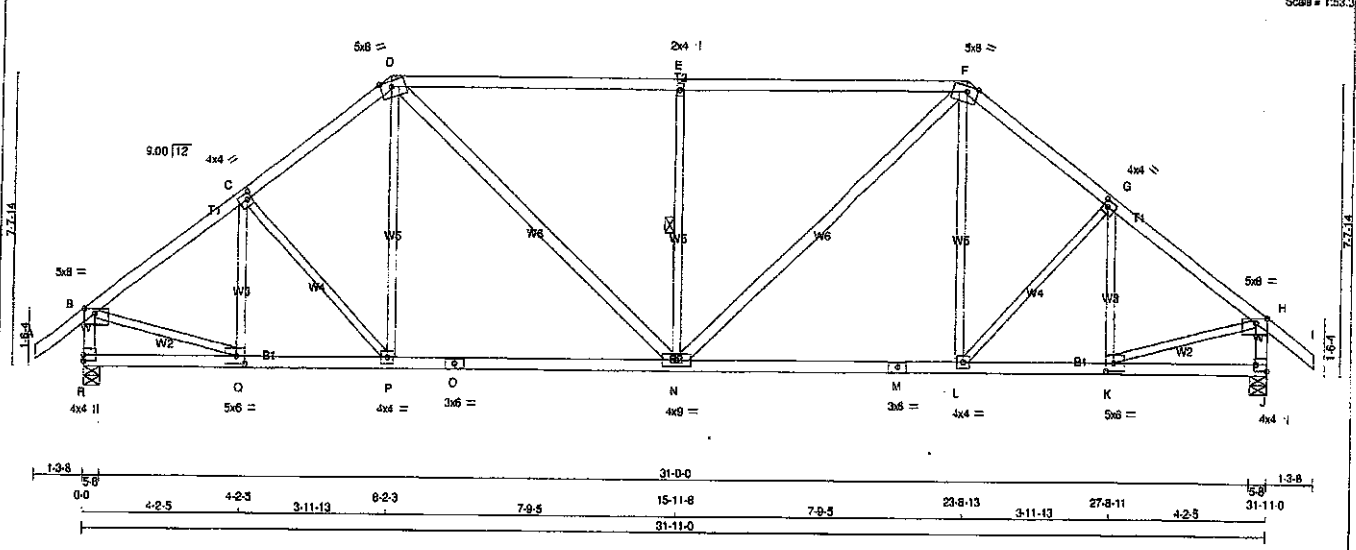
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Scale = 1:50.0



JOB NAME 403045	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:23 2019 Page 1
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 1-3-8 0-0 4-2-5 4-2-5 3-11-13 8-2-3 7-9-5 15-11-8 7-9-5 23-8-13 3-11-13 27-8-11 4-2-5 31-11-0 31-2-8
 Scale = 1:23.3



TOTAL WEIGHT = 2 X 147 = 294 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								DESIGN CRITERIA			
ALL WEBS EXCEPT D - N N - F				SIZE	LUMBER	DESCR.	BEARINGS				SPECIFIED LOADS:				
				2x4	DRY	No.2	SPF	FACTORED		MAXIMUM FACTORED		INPUT		RECRD	
				2x4	DRY	No.2	SPF	GROSS REACTION		GROSS REACTION		BRG		BRG	
				2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
				2x4	DRY	No.2	SPF	R	2385	0	2385	0	0	5-8	5-8
				2x4	DRY	No.2	SPF	J	2385	0	2385	0	0	5-8	5-8
				2x4	DRY	No.2	SPF	UNFACTORED REACTIONS							
				2x4	DRY	No.2	SPF	1ST LOASE MAX./MIN. COMPONENT REACTIONS							
				2x4	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
				2x4	DRY	No.2	SPF	R	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
				2x4	DRY	No.2	SPF	J	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J															
BRACING															
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9.															

DRY: SEASONED LUMBER.

PLATES (width is in inches)
 JT TYPE PLATES W LEN Y X
 B TMWW-p MT20 5.0 8.0 1.50 3.50
 C TMWW-t MT20 4.0 4.0 2.00 1.50
 D TTWW-m MT20 5.0 8.0 Edge 3.50
 E TMWW-w MT20 2.0 4.0
 F TTWW-m MT20 5.0 8.0 Edge 3.50
 G TMWW-t MT20 4.0 4.0 2.00 1.50
 H TMWW-p MT20 5.0 8.0 1.50 3.50
 J BMV1-p MT20 4.0 4.0 2.00 Edge
 K BMWW-t MT20 5.0 6.0 2.50 2.50
 L BMWW-t MT20 4.0 4.0
 M BS-t MT20 3.0 8.0
 N BMWWW-t MT20 4.0 9.0
 O BS-t MT20 3.0 6.0
 P BMWW-t MT20 4.0 4.0
 Q BMWW-t MT20 5.0 6.0 2.50 2.50
 R BMV1-p MT20 5.0 6.0 4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MEMB.	MAX. FACTORED	FACTORED	MEMB.	MAX. FACTORED
FR-TO	FORCE (LBS)	VERT. LOAD (LBS)	FR-TO	FORCE (LBS)	VERT. LOAD (LBS)	FR-TO	FORCE (LBS)
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	Q-C	-410/0
B-C	-2391/0	-102.1	-102.1	0.29 (1)	4.21	C-P	-89/0
C-D	-2377/0	-102.1	-102.1	0.28 (1)	4.22	P-D	0/353
D-E	-2561/0	-102.1	-102.1	1.00 (1)	2.73	D-N	0/939
E-F	-2581/0	-102.1	-102.1	1.00 (1)	2.73	N-E	-977/0
F-G	-2377/0	-102.1	-102.1	0.28 (1)	4.22	N-F	0/939
G-H	-2391/0	-102.1	-102.1	0.29 (1)	4.21	L-F	0/353
H-I	0/42	-102.1	-102.1	0.14 (1)	10.00	L-G	-89/0
R-B	-2311/0	0.0	0.0	0.24 (1)	5.82	K-G	-410/0
J-H	-2311/0	0.0	0.0	0.24 (1)	5.82	B-Q	0/2012
R-Q	0/0	-38.5	-38.5	0.12 (3)	10.00	K-H	0/2012
D-R	0/1835	-38.5	-38.5	0.50 (2)	10.00		
O-P	0/1879	-38.5	-38.5	0.82 (2)	10.00		
N-M	0/1879	-38.5	-38.5	0.82 (2)	10.00		
M-L	0/1879	-38.5	-38.5	0.82 (2)	10.00		
L-K	0/1935	-38.5	-38.5	0.50 (2)	10.00		
K-J	0/0	-38.5	-38.5	0.12 (3)	10.00		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.23")
 CSI: TC=1.00/1.00 (E-F:1), BC=0.82/1.00 (N-P:2), WB=0.45/1.00 (B-Q:1), SS=0.39/1.00 (E-F:1)
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 798 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (K) (INPUT = 0.90)
 JSI METAL = 0.65 (C) (INPUT = 1.00)

Structural component only
 DWG# T-1911486

RECEIVED
 JUN 13 2019
 TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.

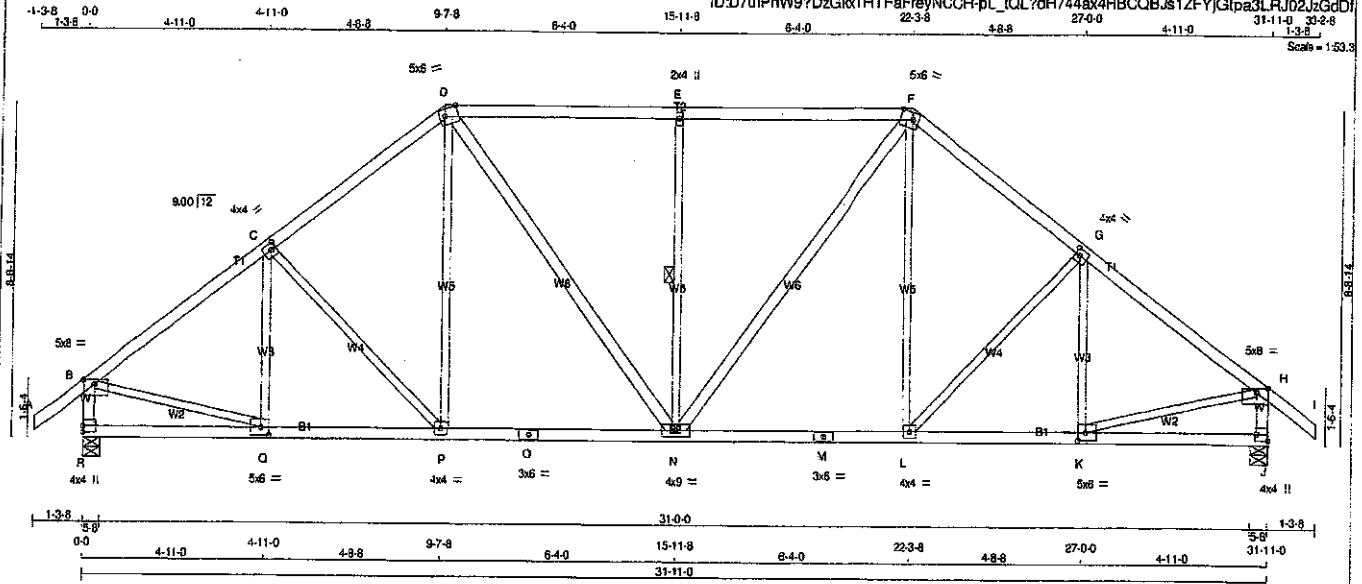
LICENSED PROFESSIONAL ENGINEER
 H. J. G. ALVES
 100009024
 PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T5	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 154 = 307 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	1.50	3.50
C TMVW-l	MT20	4.0	4.0	2.00	1.50
D TTWW-m	MT20	5.0	6.0	Edge	4.25
E TMVW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	6.0	Edge	4.25
G TMVW-t	MT20	4.0	4.0	2.00	1.50
H TMVW-p	MT20	5.0	8.0	1.50	3.50
J BMVt+p	MT20	4.0	4.0	2.00	Edge
K BMVW-t	MT20	5.0	6.0	2.50	2.50
L BMVW-t	MT20	4.0	4.0		
M BS-t	MT20	3.0	6.0		
N BMVW-w	MT20	4.0	9.0		
O BS-t	MT20	3.0	6.0		
P BMVW-t	MT20	4.0	4.0		
Q BMVW-l	MT20	5.0	8.0	2.50	2.50
R BMVt+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	2385	0	5-8	5-8
J	2385	0	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

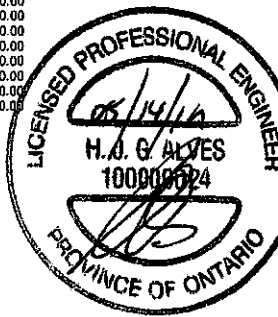
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LBS)	LC1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	Q-C	-285 / 38	0.11 (1)
B-C	-2445 / 0	-102.1 -102.1	0.40 (1)	4.05	G-P	-285 / 0	0.23 (1)
C-D	-2286 / 0	-102.1 -102.1	0.38 (1)	4.18	P-D	0 / 425	0.10 (2)
D-E	-2193 / 0	-102.1 -102.1	0.61 (1)	3.89	D-N	0 / 652	0.10 (1)
E-F	-2193 / 0	-102.1 -102.1	0.61 (1)	4.18	N-E	-791 / 0	0.39 (1)
F-G	-2286 / 0	-102.1 -102.1	0.38 (1)	4.18	N-F	0 / 652	0.10 (1)
G-H	-2445 / 0	-102.1 -102.1	0.40 (1)	4.05	L-F	0 / 425	0.10 (2)
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	L-G	-285 / 0	0.23 (1)
R-B	-2304 / 0	0.0	0.24 (1)	5.62	K-G	-285 / 38	0.11 (1)
J-H	-2304 / 0	0.0	0.24 (1)	5.62	B-Q	0 / 2041	0.46 (1)
					K-H	0 / 2041	0.46 (1)
R-O	0 / 0	-38.5	-38.5	0.15 (3)	10.00		
Q-P	0 / 1983	-38.5	-38.5	0.45 (1)	10.00		
P-O	0 / 1802	-38.5	-38.5	0.49 (2)	10.00		
O-N	0 / 1802	-38.5	-38.5	0.49 (2)	10.00		
N-M	0 / 1802	-38.5	-38.5	0.49 (2)	10.00		
M-L	0 / 1802	-38.5	-38.5	0.49 (2)	10.00		
L-K	0 / 1983	-38.5	-38.5	0.45 (1)	10.00		
K-J	0 / 0	-38.5	-38.5	0.15 (3)	10.00		



DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS:				
TOP CH.	LL	=	29.0	PS
	DL	=	6.0	PS
BOT CH.	LL	=	10.5	PS
	DL	=	7.0	PS
TOTAL LOAD		=	52.5	PS

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 LBS OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.61/1.00 (D-E:1), BC=0.48/1.00 (N-P:2), WB=0.48/1.00 (H-K:1), SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (Q) (INPUT = 0.90)

JSI METAL= 0.68 (Q) (INPUT = 1.00)

Structural component only

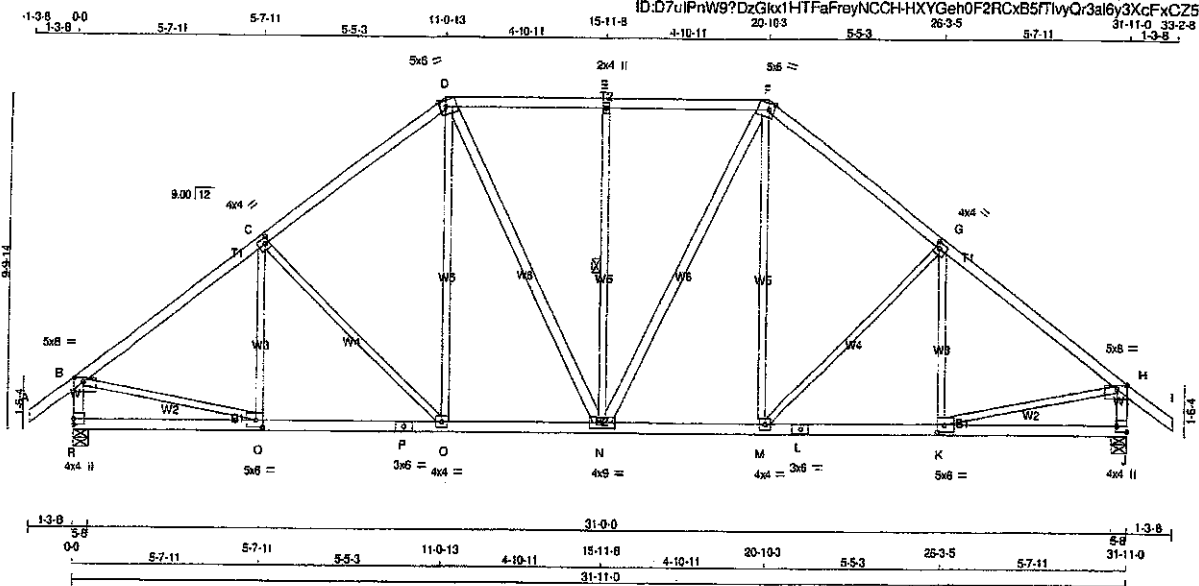
DWG# T-1911487

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403045	T6	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 6 X 161 = 966 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
L - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50 3.50
C	TMVW-i	MT20	4.0	4.0	2.00 1.50
D	TTVW-m	MT20	5.0	8.0	Edge 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TTVW-m	MT20	5.0	8.0	Edge 2.00
G	TMVW-i	MT20	4.0	4.0	2.00 1.50
H	TMVW-p	MT20	5.0	8.0	1.50 3.50
J	BMV1-p	MT20	4.0	4.0	2.00 Edge
K	BMVW-i	MT20	5.0	6.0	2.50 2.50
L	BS-i	MT20	3.0	6.0	
M	BMVW-i	MT20	4.0	4.0	
N	BMVW-w	MT20	4.0	9.0	
O	BMVW-i	MT20	4.0	4.0	
P	BS-i	MT20	3.0	6.0	
Q	BMVW-i	MT20	5.0	6.0	2.50 2.50
R	BMV1-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
R	2385	0	0	5-8
J	2385	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
R	1773	1007/0	335/0	0/0	0/0	0/0	431/0	0/0
J	1773	1007/0	335/0	0/0	0/0	0/0	431/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	Q-C	-185/119	0.09 (1)	
B-C	-2474/0	-102.1	-102.1	0.54 (1)	3.88	C-D	-422/0	0.52 (1)	
C-D	-2181/0	-102.1	-102.1	0.49 (1)	4.13	O-D	0/498	0.11 (2)	
D-E	-1916/0	-102.1	-102.1	0.35 (1)	4.50	D-N	0/440	0.07 (1)	
E-F	-1916/0	-102.1	-102.1	0.35 (1)	4.50	N-E	-603/0	0.39 (1)	
F-G	-2181/0	-102.1	-102.1	0.49 (1)	4.13	N-F	0/440	0.07 (1)	
G-H	-2474/0	-102.1	-102.1	0.54 (1)	3.88	M-F	0/498	0.11 (2)	
H-I	0/42	-102.1	-102.1	0.14 (1)	10.00	M-G	-422/0	0.52 (1)	
R-B	-2296/0	0.0	0.0	0.24 (1)	5.63	K-G	-185/119	0.09 (1)	
J-H	-2296/0	0.0	0.0	0.24 (1)	5.63	B-Q	0/2055	0.46 (1)	
R-Q	0/0	-38.5	-38.5	0.22 (3)	10.00	K-H	0/2055	0.46 (1)	
Q-P	0/2011	-38.5	-38.5	0.46 (2)	10.00				
P-O	0/2011	-38.5	-38.5	0.46 (2)	10.00				
O-N	0/1715	-38.5	-38.5	0.38 (1)	10.00				
N-M	0/1715	-38.5	-38.5	0.38 (1)	10.00				
M-L	0/2011	-38.5	-38.5	0.46 (2)	10.00				
L-K	0/2011	-38.5	-38.5	0.46 (2)	10.00				
K-J	0/0	-38.5	-38.5	0.22 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

15% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/960$ (1.06")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/960$ (1.08")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.15")

CSI: TC=0.54/1.00 (B-C:1), BC=0.48/1.00 (O-Q:2), WB=0.52/1.00 (C-O:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

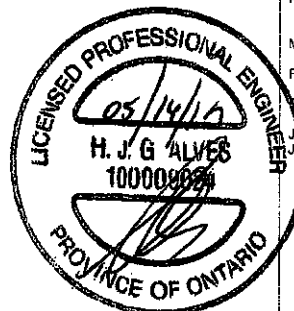
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1867
	788	1937	1856

PLATE PLACEMENT TOL = 0.250 inches

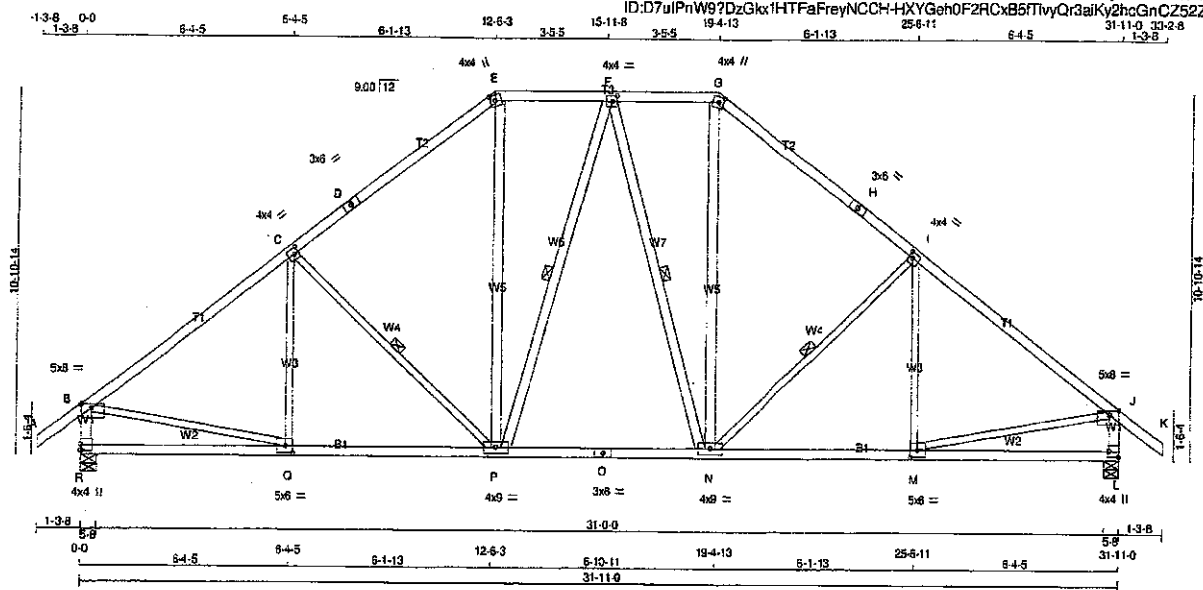
PLATE ROTATION TOL = 5.0 Deg.

JG GRIP= 0.85 (B) (INPUT = 0.90)
JG METAL= 0.60 (P) (INPUT = 1.00)



Structural component only
DWG# T-1911488

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T7	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.300 S Apr 23 2019 MiTek Industries, Inc. Tue May 14 10:41:25 2019 Page 1			
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		Scale = 1:60.7			



TOTAL WEIGHT = 2 X 187 = 335 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
R - B	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
P - E	2x4	DRY	No.2	SPF	
P - F	2x4	DRY	No.2	SPF	
F - N	2x4	DRY	No.2	SPF	
N - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-i	MT20	4.0	4.0	2.00	1.50
D	TS-t		3.0	6.0		
E	TTW+m	MT20	4.0	4.0	2.00	1.75
F	TMWW-i	MT20	4.0	4.0	2.00	1.75
G	TTW+m	MT20	4.0	4.0	2.00	1.75
H	TS-t		3.0	6.0		
I	TMWW-i	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-i	MT20	5.0	8.0	2.50	2.75
N	BMWW-i	MT20	4.0	4.0		
O	BS-t		3.0	6.0		
P	BMWW-i	MT20	4.0	4.0		
Q	BMWW-i	MT20	5.0	8.0	2.50	2.75
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
R	2385	0	2385	0	0	5-8	5-8		
L	2385	0	2385	0	0	5-8	5-8		
UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL	
R	1773	1097/0	335/0	0/0	0/0	0/0	431/0	0/0	
L	1773	1097/0	335/0	0/0	0/0	0/0	431/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L.
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LBS)	CS1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0/42	-102.1	-102.1 0.14 (1)	10.00	Q-C	-111/174	0.07 (1)
B-C	-2481/0	-102.1	-102.1 0.72 (1)	3.65	C-P	-559/0	0.29 (1)
C-D	-2070/0	-102.1	-102.1 0.64 (1)	4.02	P-E	0/832	0.13 (1)
D-E	-2070/0	-102.1	-102.1 0.64 (1)	4.02	F-N	-212/0	0.15 (1)
E-F	-1827/0	-102.1	-102.1 0.15 (1)	5.04	F-N	-216/0	0.15 (1)
F-G	-1827/0	-102.1	-102.1 0.15 (1)	5.04	N-G	0/837	0.13 (1)
G-H	-2070/0	-102.1	-102.1 0.64 (1)	4.02	N-I	-559/0	0.29 (1)
H-I	-2070/0	-102.1	-102.1 0.64 (1)	4.02	M-I	-111/174	0.07 (1)
I-J	-2481/0	-102.1	-102.1 0.72 (1)	3.65	B-Q	0/2057	0.46 (1)
J-K	0/42	-102.1	-102.1 0.14 (1)	10.00	M-J	0/2057	0.46 (1)
R-B	-2284/0	0.0	0.0 0.24 (1)	5.64			
L-J	-2284/0	0.0	0.0 0.24 (1)	5.64			
R-Q	0/0	-38.5	-38.5 0.28 (3)	10.00			
O-P	0/2022	-38.5	-38.5 0.54 (2)	10.00			
P-O	0/1891	-38.5	-38.5 0.45 (2)	10.00			
O-N	0/1891	-38.5	-38.5 0.45 (2)	10.00			
N-M	0/2022	-38.5	-38.5 0.54 (2)	10.00			
M-L	0/0	-38.5	-38.5 0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CS1: TC=0.72/1.00 (B-C:1), GC=0.54/1.00 (P-Q:2), WB=0.46/1.00 (B-Q:1), SS=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

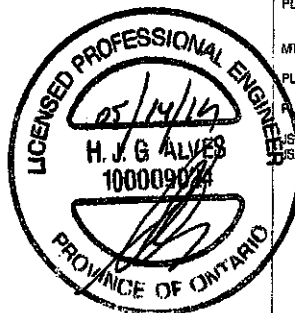
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

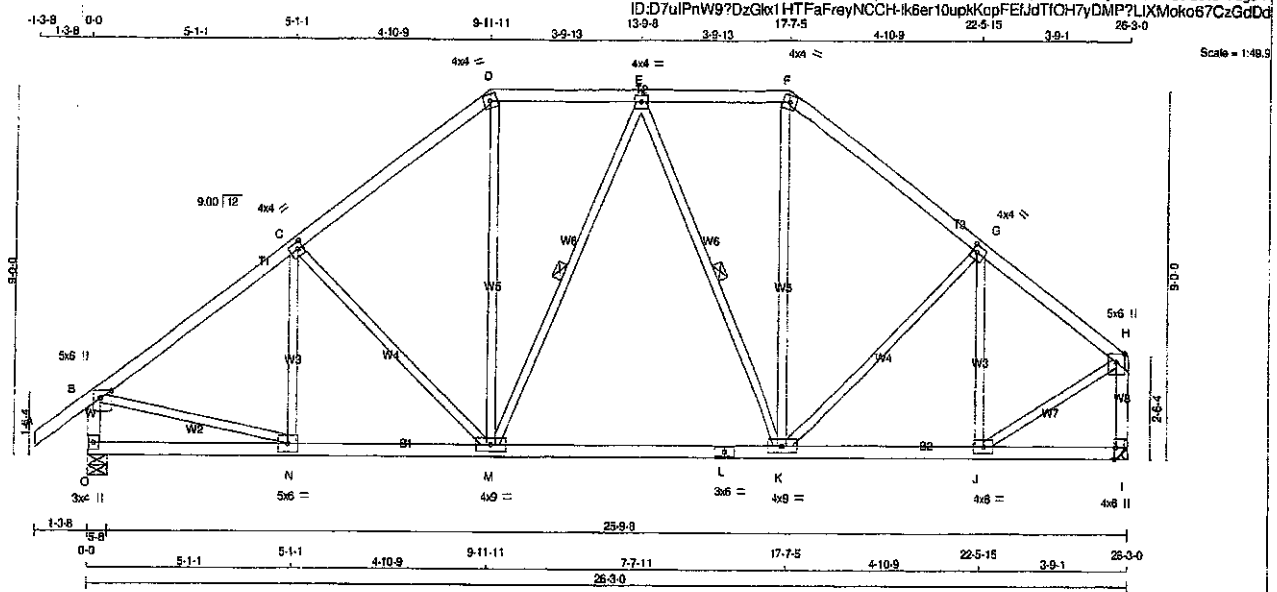
GRIP=0.88 (Q) (INPUT = 0.90)
METAL=0.69 (Q) (INPUT = 1.00)



Structural component only
DWG# T-1911489

JOB NAME TRUSS NAME QUANTITY PLY JOB DESC. GREEN PARK HOMES DRWG NO.
403045 T8 4 1 TRUSS DESC.

Tamarack Roof Truss, Burlington Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:26 2019 Page 1
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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	8.0	Edge	3.00
C TMVW-1	MT20	4.0	4.0	2.00	1.50
D TTW-m	MT20	4.0	4.0		
E TMVW-1	MT20	4.0	4.0		
F TTW-m	MT20	4.0	4.0		
G TMVW-1	MT20	4.0	4.0	2.00	1.50
H TMVW+p	MT20	5.0	8.0	Edge	
I BMV1-t	MT20	4.0	8.0	Edge	0.50
J BMVW-1	MT20	4.0	8.0		
K BMVW-1	MT20	4.0	9.0		
L BS-1	MT20	3.0	6.0		
M BMVW-1	MT20	4.0	9.0		
N BMVW-1	MT20	5.0	8.0		
O BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	DOWN	GROSS REACTION	DOWN		
O	1888	0	1888	0	5-8	5-8
I	1845	0	1845	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1476	842 / 0	276 / 0	0 / 0	0 / 0	358 / 0	0 / 0
I	1379	762 / 0	276 / 0	0 / 0	0 / 0	341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K.

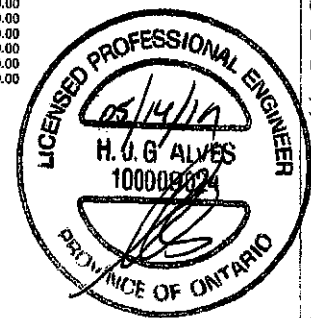
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

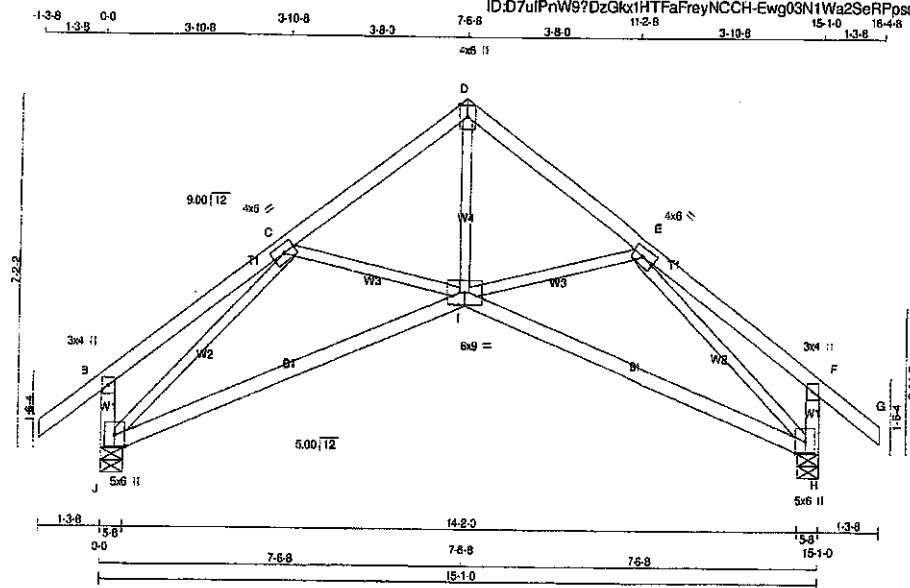
MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	WEBS		MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
				FROM TO	LENGTH (FT)		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	N-C	-178 / 83
B-C	-1946 / 0	-102.1	-102.1	0.39 (1)	4.46	C-M	-369 / 0
C-D	-1697 / 0	-102.1	-102.1	0.37 (1)	4.73	M-D	0 / 654
D-E	-1333 / 0	-102.1	-102.1	0.20 (1)	5.41	M-E	-152 / 12
E-F	-1253 / 0	-102.1	-102.1	0.19 (1)	5.54	E-K	-354 / 0
F-G	-1590 / 0	-102.1	-102.1	0.32 (1)	4.91	K-F	0 / 582
G-H	-1514 / 0	-102.1	-102.1	0.28 (1)	5.05	K-G	0 / 32
H-I	-1902 / 0	0.0	0.0	0.20 (1)	8.09	J-G	-566 / 0
I-J	-1785 / 0	0.0	0.0	0.24 (1)	6.25	B-N	0 / 1528
						J-H	0 / 1438
		0 / 0	-38.5	-38.5	0.16 (3)	10.00	
		0 / 1585	-38.5	-38.5	0.47 (2)	10.00	
		0 / 1392	-38.5	-38.5	0.45 (2)	10.00	
		0 / 1392	-38.5	-38.5	0.45 (2)	10.00	
		0 / 1242	-38.5	-38.5	0.43 (2)	10.00	
		0 / 0	-38.5	-38.5	0.09 (3)	10.00	

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FILE NO.



Structural component only
DWG# T-1911490

JOB NAME 403045	TRUSS NAME T9	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 S Apr 23 2019 M.Tek Industries, Inc. Tue May 14 10:41:27 2019 Page 1			



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BVMW1+p	MT20	5.0	6.0	Edge
I	BSWWW-p	MT20	6.0	6.0	
J	BVMW1+p	MT20	5.0	6.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	RECORD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1201	0	1201	0	0	5-8	5-8
H	1201	0	1201	0	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	889	518 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0
H	889	518 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	D	0 / 1239	0.28 (1)	
B-C	0 / 24	-102.1 -102.1	0.21 (1)	E	-68 / 79	0.02 (1)	
C-D	-1310 / 0	-102.1 -102.1	0.18 (1)	G	-68 / 79	0.02 (1)	
D-E	-1310 / 0	-102.1 -102.1	0.18 (1)	H	-1637 / 0	0.85 (1)	
E-F	0 / 24	-102.1 -102.1	0.21 (1)	I	-1637 / 0	0.85 (1)	
F-G	0 / 42	-102.1 -102.1	0.14 (1)				
J-B	-294 / 0	0.0 0.0	0.03 (1)				
H-F	-294 / 0	0.0 0.0	0.03 (1)				
J-I	0 / 1185	-38.5 -38.5	0.63 (2)				
I-H	0 / 1185	-38.5 -38.5	0.63 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/742 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/444 (0.41")

CS: TC=0.21/1.00 (B-C:1), BC=0.63/1.00 (H-J:2), WB=0.85/1.00 (E-H:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (J) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

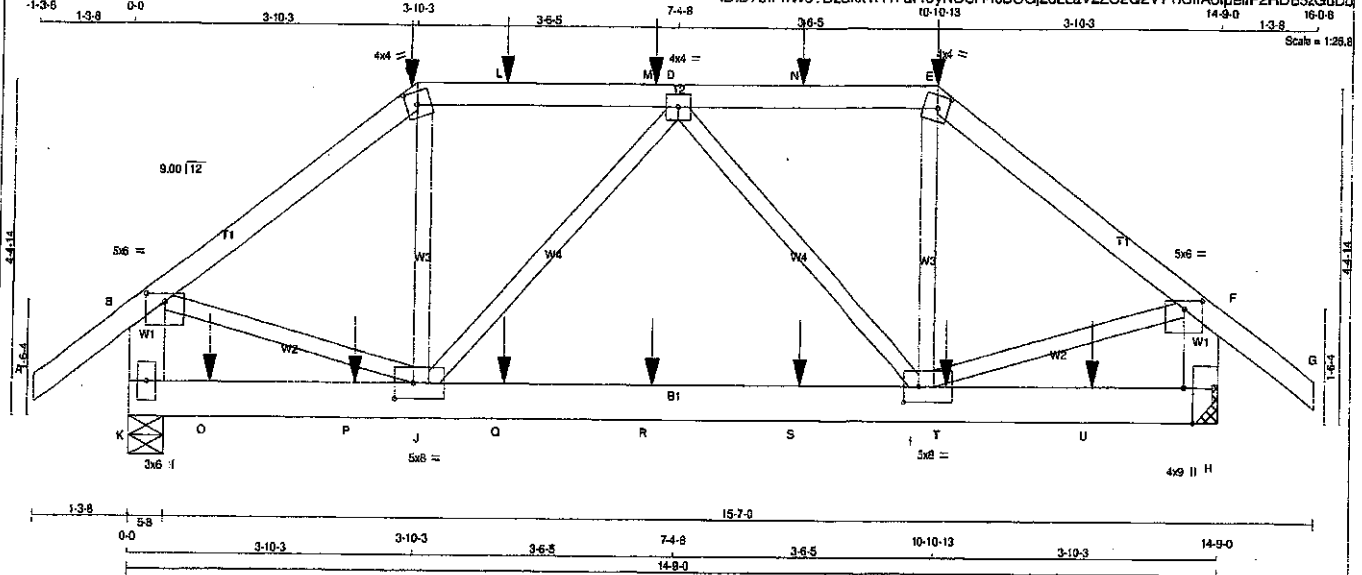
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TOWN OF CALEDON
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FILE NO.



Structural component only
DWG# T-1911491

JOB NAME 403045	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 10:41:28 2019 Page 1
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TOTAL WEIGHT = 73 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
K - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
K - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTW-m	MT20	4.0	4.0	2.00	1.75
D	TMWW-i	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	2.00	1.75
F	TMVW-p	MT20	5.0	6.0	1.25	3.00
H	BMV1-i	MT20	4.0	9.0	Edge	1.50
I	BMWW-i	MT20	5.0	6.0	2.50	2.50
J	BMWW-i	MT20	5.0	6.0	2.50	3.00
K	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
K	1936	0	1936	0	5-8
H	1966	0	1966	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	1437	824 / 0	265 / 0	0 / 0	0 / 0	348 / 0	0 / 0
H	1454	848 / 0	256 / 0	0 / 0	0 / 0	350 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 42	J-C	0 / 427
B-C	-1837 / 0	I-E	0 / 379
C-L	-1482 / 0	B-J	0 / 1521
L-M	-1482 / 0	I-F	0 / 1568
M-D	-1482 / 0	J-D	-463 / 0
D-N	-1522 / 0	O-I	-420 / 0
N-E	-1522 / 0		
E-F	-1891 / 0		
F-G	0 / 42		
K-B	-1849 / 0		
H-F	-1893 / 0		

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX.	MAX+	MAX-	TYPE	HEEL	CONN.	TYPE	HEEL	CONN.
C	3-10-3	-49	-55	---	---	FRONT	VERT	DEAD	---	---	---
C	3-10-3	-282	-282	---	---	FRONT	VERT	SNOW	---	---	---
E	10-10-13	-49	-55	---	---	FRONT	VERT	DEAD	---	---	---
E	10-10-13	-129	-129	---	---	FRONT	VERT	TOTAL	---	---	---
E	10-10-13	-282	-282	---	---	FRONT	VERT	SNOW	---	---	---
L	5-0-12	-123	-123	---	---	FRONT	VERT	TOTAL	---	---	---
M	7-0-12	-123	-123	---	---	FRONT	VERT	TOTAL	---	---	---
N	9-0-12	-123	-123	---	---	FRONT	VERT	TOTAL	---	---	---
O	1-0-12	-55	-70	---	---	FRONT	VERT	TOTAL	---	---	---
P	3-0-12	-55	-70	---	---	FRONT	VERT	TOTAL	---	---	---
Q	5-0-12	-55	-70	---	---	FRONT	VERT	TOTAL	---	---	---
R	7-0-12	-55	-70	---	---	FRONT	VERT	TOTAL	---	---	---
S	9-0-12	-55	-70	---	---	FRONT	VERT	TOTAL	---	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDT. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.49")
CALCULATED VERT. DEFL. (LL) = U/689 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.49")
CALCULATED VERT. DEFL. (TL) = U/989 (0.08")

CSI: TC=0.39/1.00 (D-E:1), BC=0.39/1.00 (I-J:1),
WB=0.39/1.00 (F-I:1), SS=0.34/1.00 (C-D:1)

COLLUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	788	1987	1656

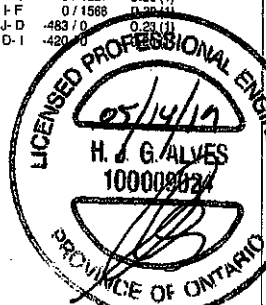
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (I) (INPUT = 0.90)
JSI METAL= 0.41 (F) (INPUT = 1.00)

Structural component only

DWG# T-1911492 CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T10	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 M-Tek Industries, Inc. Tue May 14 10:41:28 2019 Page 2
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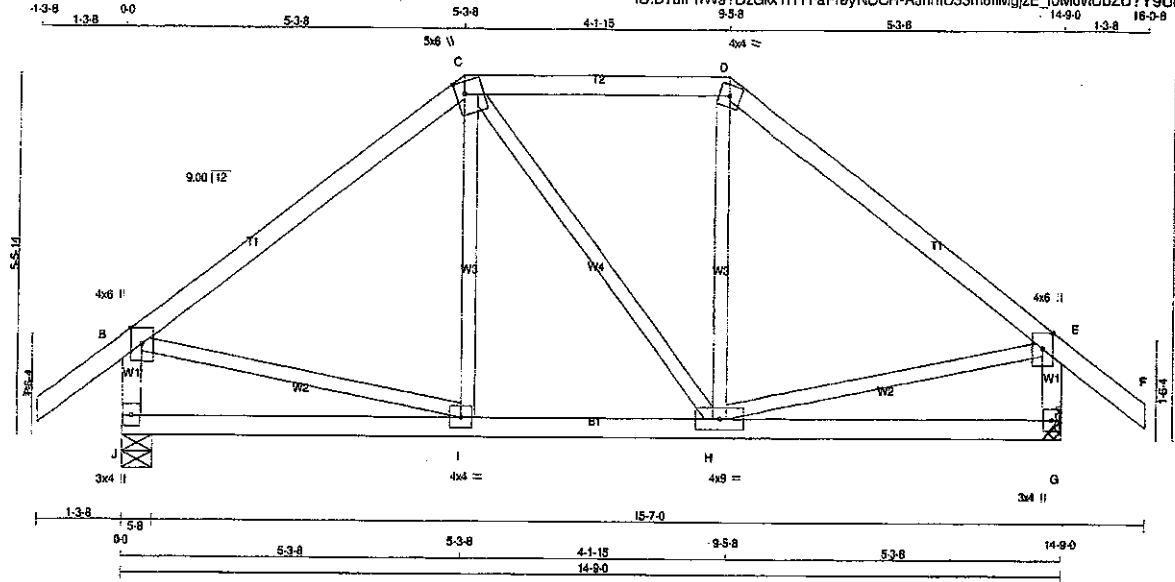
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	11-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
U	13-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---



Structural component only
 DWG# T-1911492

JOB NAME 403045	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitel Industries, Inc. Tue May 14 10:41:29 2019 Page 1
ID:D7uIPnW97DzGlx1HTFaFreyNCCCH-AJnmU33m6fIMgje_10M0vUlbZU7Y9UoU10njXzGdDa



TOTAL WEIGHT = 65 lb (M/F)

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TTW-m	MT20	4.0	4.0	
E	TMVW+p	MT20	4.0	6.0	Edge
G	BMVW+p	MT20	3.0	4.0	
H	BMVW-w	MT20	4.0	9.0	
I	BMVW-w	MT20	4.0	4.0	
J	BMVW+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
J	1178	0	1178	0
G	1178	0	1178	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED
J	872
G	872

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.09 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCT (LC)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO	FROM	TO	CSI (LC)	10.00	FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	I-C	0 / 174	0.04 (3)
B-C	872 / 0	-102.1	-102.1	0.38 (1)	C-H	0 / 4	0.00 (3)
C-D	-999 / 0	-102.1	-102.1	0.23 (1)	H-D	0 / 177	0.04 (3)
D-E	-873 / 0	-102.1	-102.1	0.38 (1)	B-I	0 / 713	0.16 (1)
E-F	0 / 42	-102.1	-102.1	0.14 (1)	H-E	0 / 714	0.16 (1)
J-B	-1090 / 0	0.0	0.0	0.11 (1)			
G-E	-1089 / 0	0.0	0.0	0.11 (1)			
J-I	0 / 0	-38.5	-38.5	0.18 (3)	10.00		
I-H	0 / 897	-38.5	-38.5	0.26 (2)	10.00		
H-G	0 / 0	-38.5	-38.5	0.18 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 085-14
- TPIC 2011, TPIC 2014

55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.38/1.00 (D-E), BC=0.26/1.00 (H-I), WB=0.16/1.00 (E-H), SS=0.17/1.00 (D-E)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

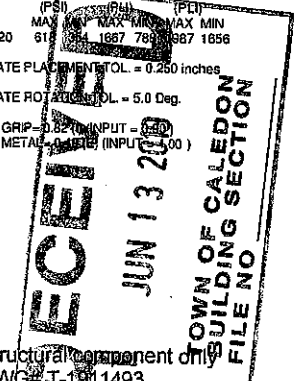
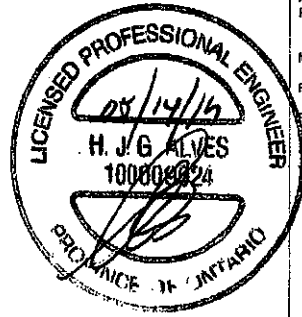
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
MT20	61	1687	78

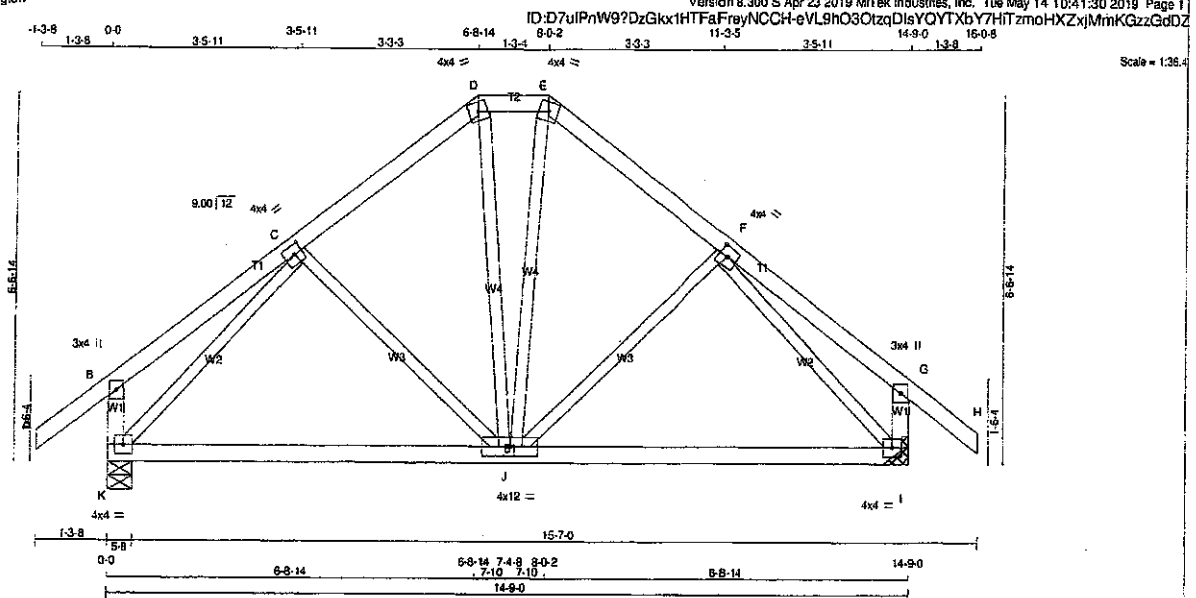
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.

SI GRIP = 3.0 INPUT = 3.0
SI METAL = 0.00 INPUT = 0.00



Structural component only
DWG# T-1911493

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
403045	T12	1	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - H	2x4	DRY	No.2	SPF		
K - B	2x4	DRY	No.2	SPF		
I - G	2x4	DRY	No.2	SPF		
K - I	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+P	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWWWW-I	MT20	4.0	12.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
K	1178	0	1178	0
I	1178	0	1178	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 5-8.

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
K	872	508 / 0	155 / 0	0 / 0	208 / 0	0 / 0
I	872	508 / 0	155 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LIFT (LC)	MAX. CSF (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LIFT (LC)	MAX. CSF (LC)
FR-TO		FROM	TO			FR-TO		FROM	TO	
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	C-J	-137 / 33	0.07 (1)		
B-C	0 / 23	-102.1	-102.1	0.18 (1)	10.00	J-F	-137 / 33	0.07 (1)		
C-D	-900 / 0	-102.1	-102.1	0.14 (1)	6.25	K-C	-1074 / 0	0.43 (1)		
D-E	-847 / 0	-102.1	-102.1	0.03 (1)	6.25	F-I	-1074 / 0	0.43 (1)		
E-F	-800 / 0	-102.1	-102.1	0.14 (1)	6.25	D-J	0 / 275	0.08 (2)		
F-G	0 / 23	-102.1	-102.1	0.18 (1)	10.00	J-E	0 / 275	0.08 (2)		
G-H	0 / 42	-102.1	-102.1	0.14 (1)	10.00					
K-B	-273 / 0	0.0	0.0	0.03 (1)	7.81					
I-G	-273 / 0	0.0	0.0	0.03 (1)	7.81					
K-J	0 / 721	-38.5	-38.5	0.54 (2)	10.00					
J-I	0 / 721	-38.5	-38.5	0.54 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 085-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.49")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.49")
CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CSI: TC=0.19/1.00 (B-C:1), SC=0.54/1.00 (J-K:2),
WB=0.43/1.00 (C-K:1), SSI=0.19/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

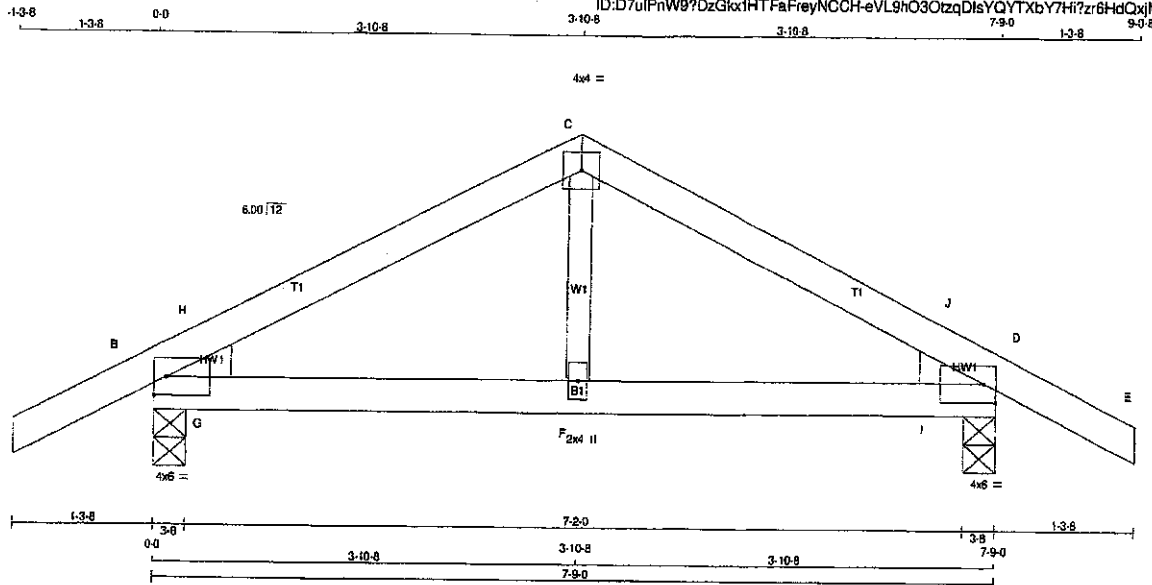
SSI GRIP = 0.87 (K) (INPUT = 0.90)
SSI METAL = 0.39 (F) (INPUT = 1.00)



Structural component only
DWG# T-1911494

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403045	T91	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

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TOTAL WEIGHT = 2 X 25 = 50 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
B - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0	Edge
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-I	MT20	4.0	6.0	Edge
F	BMW-w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	HEEL
B	883	0	0	883	0	0	0	3-8	2x4 L
D	883	0	0	883	0	0	0	3-8	2x4 R

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	502	304 / 0	81 / 0	0 / 0	0 / 0	117 / 0	0 / 0	0 / 0
D	502	304 / 0	81 / 0	0 / 0	0 / 0	117 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS			
MEMB.	MAX. FORCE (LBS)	VERT.	LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM				FR-TO			
A-B	0 / 21	-102.1	-102.1	0.13 (1)	10.00	F-C	0 / 235	0.05 (2)	
B-H	-649 / 0	-102.1	-102.1	0.07 (4)	6.25	G-H	-80 / 97	0.00 (1)	
H-C	-590 / 0	-102.1	-102.1	0.15 (1)	6.25	I-J	-80 / 97	0.00 (1)	
C-J	-590 / 0	-102.1	-102.1	0.15 (1)	6.25				
J-D	-649 / 0	-102.1	-102.1	0.07 (1)	6.25				
D-E	0 / 21	-102.1	-102.1	0.13 (1)	10.00				
B-G	0 / 521	-38.5	-38.5	0.16 (1)	10.00				
G-F	0 / 521	-38.5	-38.5	0.20 (1)	10.00				
F-I	0 / 521	-38.5	-38.5	0.20 (1)	10.00				
I-D	0 / 521	-38.5	-38.5	0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.26")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.26")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (C-H:1), BC=0.20/1.00 (F-I:1), WB=0.05/1.00 (C-F:2), SSI=0.13/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (OBY) SHEAR SECTION

(PSI) (PL) (GL)

MAX. MIN. MAX. MIN.

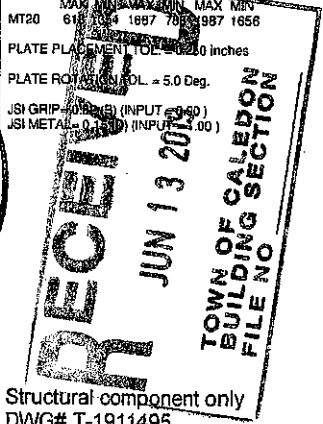
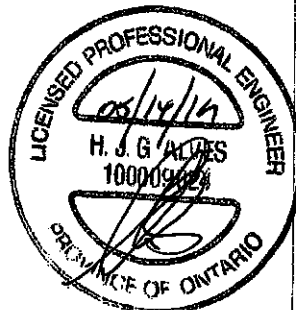
MT20 618 1854 1887 783 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP (OBY) (INPUT 0.00)

JSI METAL (OBY) (INPUT 0.00)

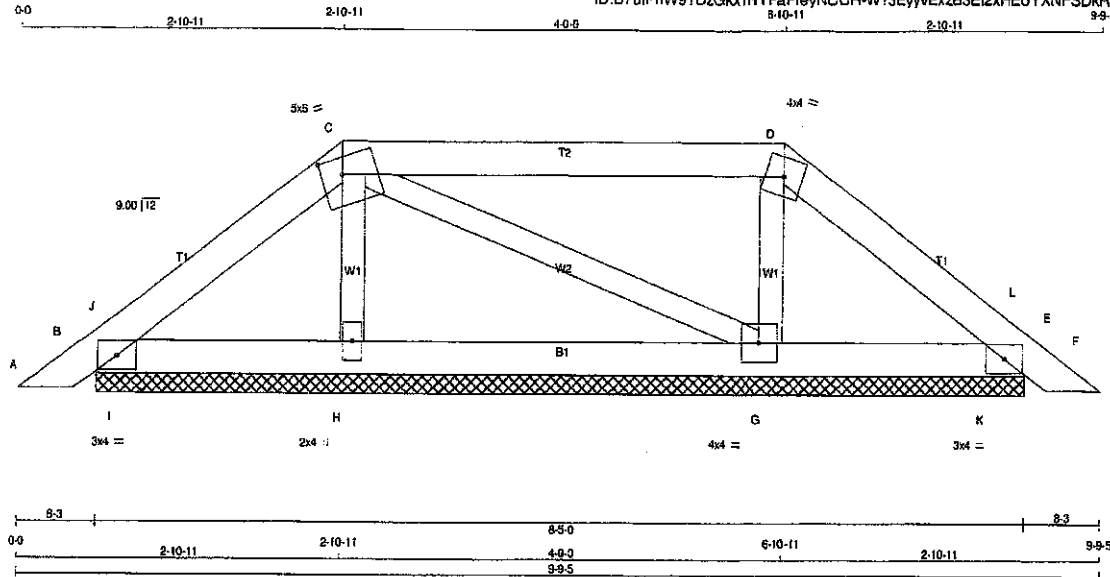


Structural component only
DWG# T-1911495

JOB NAME 403045	TRUSS NAME P1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:17 2019 Page 1
ID: D7ulPnW97DzGkx1HTFaFreyNCCCH-W73EyyvExzB3E12xHEoYXNFSDkRvKIR1kr68/DzGdDm
9-9-5

Scale = 1:17.5



TOTAL WEIGHT = 2 X 28 = 56 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
JT	TMB14	MT20	3.0	4.0		
B	TW-m	MT20	5.0	6.0	1.75	2.25
C	TW-m	MT20	4.0	4.0		
D	TMB14	MT20	3.0	4.0		
E	BMWW1-t	MT20	4.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMWW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
B	257	0	257	0	0	0	8-5-0	8-5-0	
E	243	0	243	0	0	0	8-5-0	8-5-0	
H	386	0	386	0	0	0	8-5-0	8-5-0	
G	417	0	417	0	0	0	8-5-0	8-5-0	

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS					
JT	1ST CASE COMBINED	SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
B	183	130 / 0	16 / 0	0 / 0	0 / 0	37 / 0	0 / 0
E	179	122 / 0	16 / 0	0 / 0	0 / 0	36 / 0	0 / 0
H	295	144 / 0	73 / 0	0 / 0	0 / 0	78 / 0	0 / 0
G	316	161 / 0	73 / 0	0 / 0	0 / 0	82 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.0 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	LC1 (LC)	MAX. LENGTH (FT)	UNBRACED	MEMB.	FORCE (LBS)	MAX. LENGTH (FT)	UNBRACED
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	H-C	-232 / 0	0.03 (1)	
B-J	-75 / 0	-102.1	-102.1	0.02 (1)	6.25	C-G	-21 / 0	0.01 (1)	
J-C	-98 / 0	-102.1	-102.1	0.08 (1)	6.25	G-D	-255 / 0	0.04 (1)	
C-D	-42 / 0	-102.1	-102.1	0.28 (1)	6.25	I-J	-146 / 0	0.03 (1)	
D-L	-75 / 0	-102.1	-102.1	0.06 (1)	6.25	K-L	-148 / 0	0.00 (1)	
L-E	-51 / 0	-102.1	-102.1	0.02 (1)	5.25				
E-F	0 / 17	-102.1	-102.1	0.03 (1)	10.00				
B-I	0 / 75	-38.5	-38.5	0.07 (1)	10.00				
I-H	0 / 75	-38.5	-38.5	0.08 (2)	10.00				
H-G	0 / 61	-38.5	-38.5	0.08 (2)	10.00				
G-K	0 / 57	-38.5	-38.5	0.08 (2)	10.00				
K-E	0 / 57	-38.5	-38.5	0.06 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
OL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLES WITH:
- PART 9 OF BCBC 2010, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (C-D:1), BC=0.08/1.00 (H-I:2), WB=0.04/1.00 (D-G:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

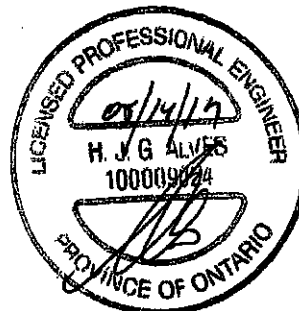
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.05 (H) (INPUT = 1.00)



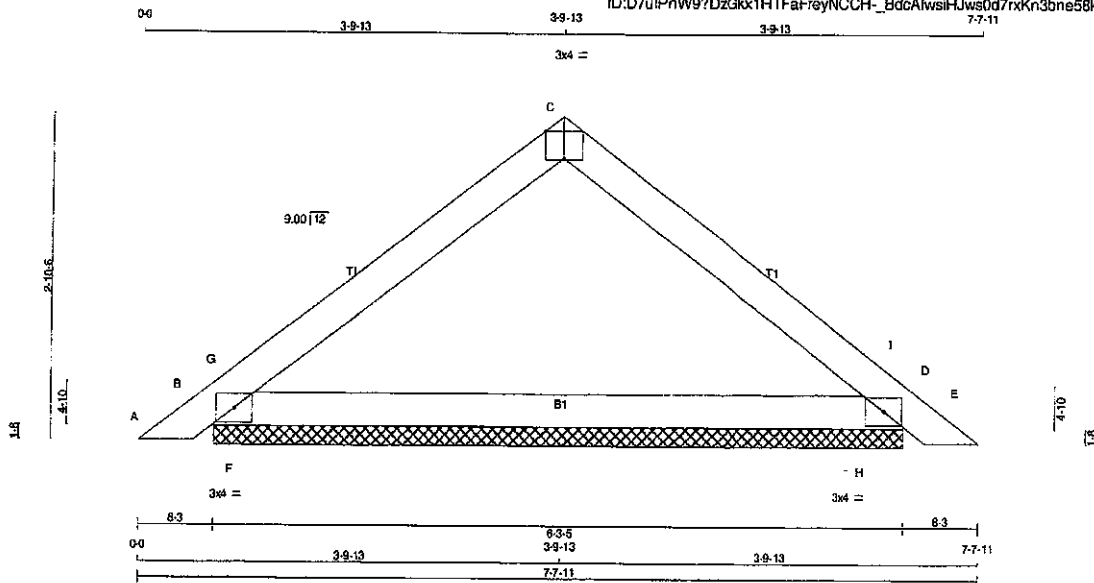
Structural component only
DWG# T-1911479

Structural component only
DWG# T-1911480

JOB NAME 403045	TRUSS NAME P3	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: D7uIPnW9?DzGkx1HTFaFreyNCCH_-BdcAlwsitJws0d7rxKn3bne58kvTCGAYVshrgzGdD

Scale = 1:18.0



TOTAL WEIGHT = 4 X 18 = 72 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP/LFT	IN-SX
B	502	0	0	6-3-5
D	502	0	0	6-3-5

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	371	217/0	66/0	0/0	0/0	89/0	0/0
D	371	217/0	66/0	0/0	0/0	89/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
A-B	0/17	-102.1 -102.1	0.03 (1)	10.00	F-G	0/437	0.00 (1)
B-G	-537/0	-102.1 -102.1	0.18 (3)	6.25	H-I	0/437	0.00 (1)
G-C	-275/0	-102.1 -102.1	0.21 (1)	6.25			
C-I	-275/0	-102.1 -102.1	0.21 (1)	6.25			
I-D	-537/0	-102.1 -102.1	0.16 (3)	6.25			
D-E	0/17	-102.1 -102.1	0.03 (1)	10.00			
B-F	0/233	-38.5 -38.5	0.13 (3)	10.00			
F-H	0/233	-38.5 -38.5	0.23 (2)	10.00			
H-D	0/233	-38.5 -38.5	0.13 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 52.5 PSF	

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-I), EC=0.23/1.00 (F-H), WB=0.00/1.00 (F-G), SS=0.25/1.00 (B-F)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

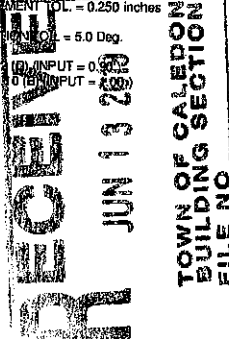
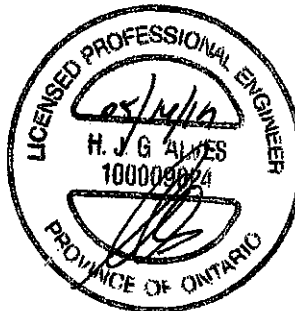
(PS)	(PL)	(PL)
MAX. MIN. MAX. MIN. MAX. MIN.		
MT20 618	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.3 (D) INPUT = 0.3 (D) INPUT = 0.3 (D) INPUT = 0.3 (D)

JSI METAL = 0.3 (D) INPUT = 0.3 (D) INPUT = 0.3 (D) INPUT = 0.3 (D)



Structural component only
DWG# T-1911481

Tamarack Roof Truss, Burlington



Structural component only
DWG# T-1911476

CHORDS		FACTORED				WE
MEMB	FORCE (LBS)	VERT. LOAD	LC1: MAX (PLF)	MAX. CSI (LC)	UNBRAC LENGTH	MEMB.
FR-TO		FROM	TO			FR-TO
E-B	-514 / 0	0.0	0.0	0.21 (3)	7.91	
A-B	0 / 33	-102.1	-102.1	0.13 (1)	10.00	
B-C	-36 / 0	-192.1	-102.1	0.60 (1)	6.25	
E-D	0 / 0	-38.5	-38.5	0.21 (3)	10.00	

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PROFESSOR



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H. J. G. ALVES

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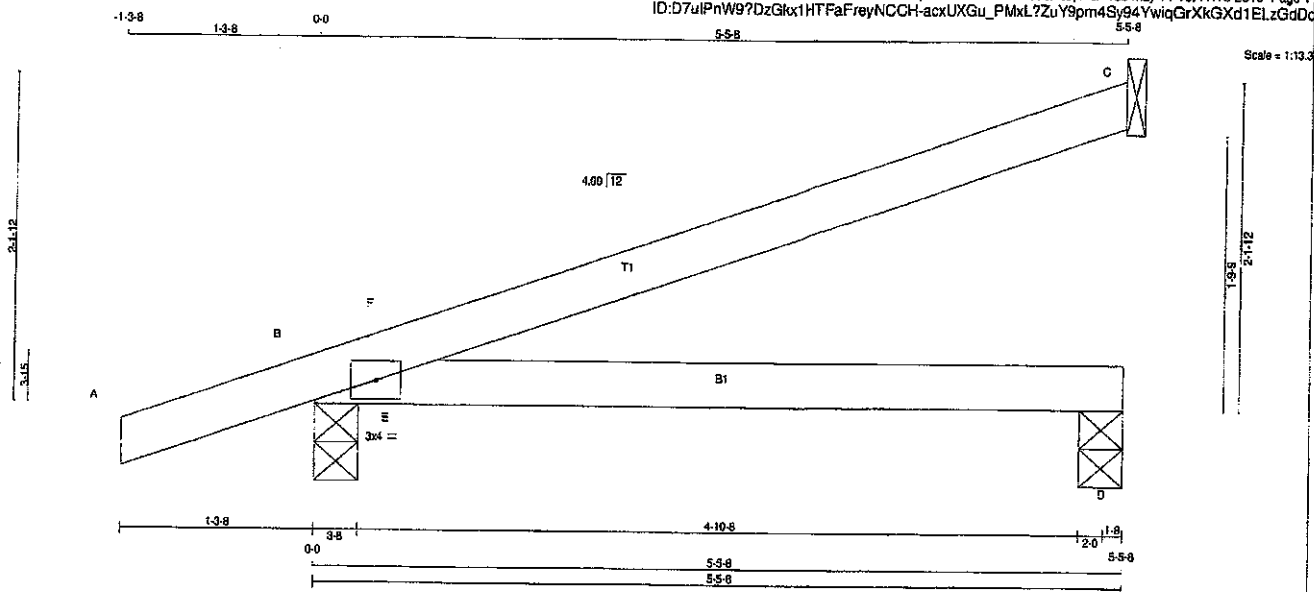


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JOB NAME 403045	TRUSS NAME J90	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:15 2019 Page 1			
		ID: D7uIPnW9?DzGkx1HTFaFreyNCCH-axUXGu_PmXL?ZuY9pm4Sy94YwiqGrXkGXcd1ElzGdDc			



TOTAL WEIGHT = 4 X 14 = 56 lb

LUMBER

N. L. & A. RULES
CHORDS SIZE LUMBER
A - C 2x4 DRY No.2
B - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X
B TMB1-M720 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
C	250	0	250	0
B	520	0	520	0
D	133	0	145	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0	0 / 0
B	350	236 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	FR-TO			
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	E-F	-254 / 122
B-F	-51 / 0	-102.1	-102.1	0.13 (3)	6.25		0.00 (1)
F-C	0 / 6	-102.1	-102.1	0.42 (1)	10.00		
B-E	0 / 0	-38.5	-38.5	0.26 (1)	10.00		
E-D	0 / 0	-38.5	-38.5	0.31 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/842$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/489$ (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SS=0.28/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

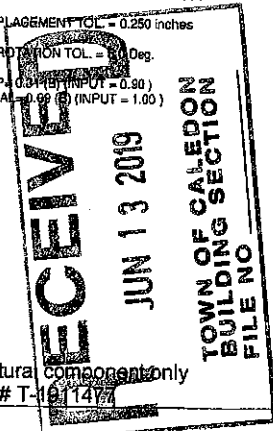
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
M720 918 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

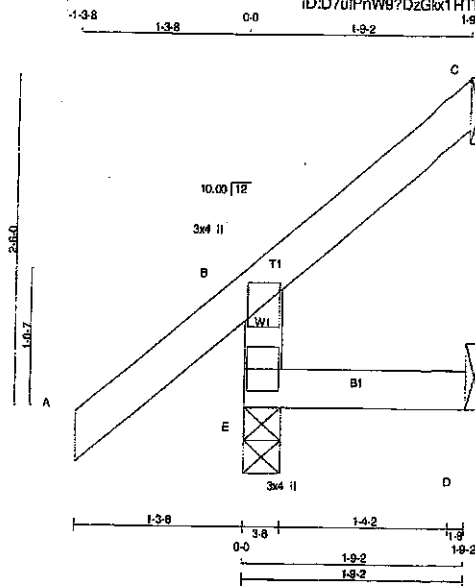
PLATE ROTATION TOL = 3.0 Deg.

JSI GRIP = 0.31 (B) (INPUT = 0.90)
JSI METAL = 0.99 (B) (INPUT = 1.00)



Structural components only
DWG# T-161147

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	J91	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:16 2018 Page 1					
ID: D7uIPnW8?DzGx1HTFaFreyNCCH-2pVslcvAg3CcTkjWHJ_AJdK8C?ImuVBnmnzGdDn					
Scale = 1:15.5					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0	
E	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT	IN-SX	RECD	BRG	IN-SX
E	321	0	321	0	0	3-8	3-8		
C	49	0	49	0	-25	1-8	1-8		
D	19	0	38	0	-10	1-8	1-8		

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	228	183/0	18/0	0/0	46/0	0/0
C	34	27/-20	1/0	0/0	8/0	0/0
D	20	0/-12	17/0	0/0	10/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS	MAX. FACTORED	FORCE	MAX
		(LBS)	(PLF)			CSI (LC)	ENBRAC			(LBS)		CSI (LC)
FR-TO				FROM		TO	LENGTH	FR-TO				
E-B	-272/0		0.0	0.0	0.0	0.05 (5)	7.81					
A-B	0/45		-102.1	-102.1	0.14 (1)	10.00						
B-C	-26/0		-102.1	-102.1	0.10 (1)	6.25						
E-D	0/0		-38.5	-38.5	0.05 (5)	10.00						

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/989 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/989 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.05/1.00 (D-E:5),
WS=0.00/1.00 (A-A:0), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

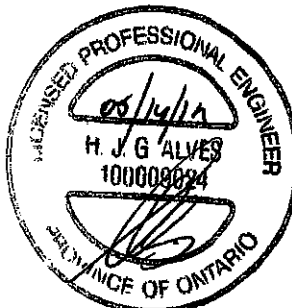
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911478



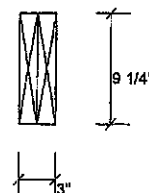
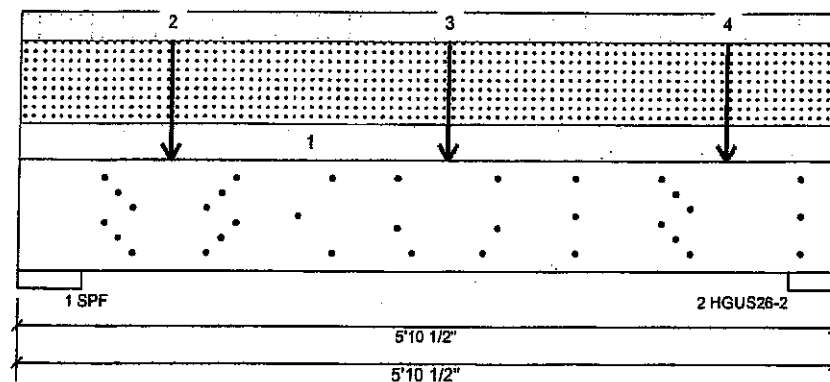
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 1 of 13

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	376	503	1210	0
2	314	415	1001	0

Bearings and Factored Reactions

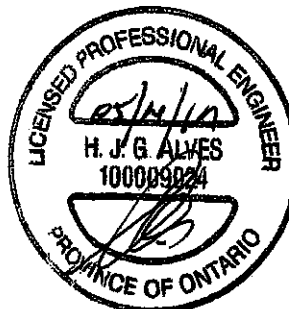
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	28%	829 / 2190	2819 L 1.25D+1.5S +L
2 - HGUS...	4.000"	32%	519 / 1815	2334 L 1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2982 ft-lb	3'1"	6039 ft-lb	0.494 (49%)	1.25D+1.5S +L	L
Unbraced	2982 ft-lb	3'1"	5236 ft-lb	0.570 (57%)	1.25D+1.5S +L	L
Shear	2763 lb	1'2"	3984 lb	0.694 (69%)	1.25D+1.5S +L	L
LL Defl inch	0.025 (L/2459)	2'11 9/16"	0.174 (L/360)	0.150 (15%)	S+0.5L	L
TL Defl inch	0.035 (L/1809)	2'11 9/16"	0.174 (L/360)	0.200 (20%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

DWG NO. TAM11157
STRUCTURAL
COMPONENT ONLY 1/3

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	1-1-0		Far Face	350 lb	256 lb	648 lb	0 lb	
3	Point	3-1-0		Far Face	208 lb	155 lb	508 lb	0 lb	
4	Point	5-1-0		Far Face	208 lb	155 lb	508 lb	0 lb	

Manufacturer InfoTamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

This design is valid until 12/11/2021





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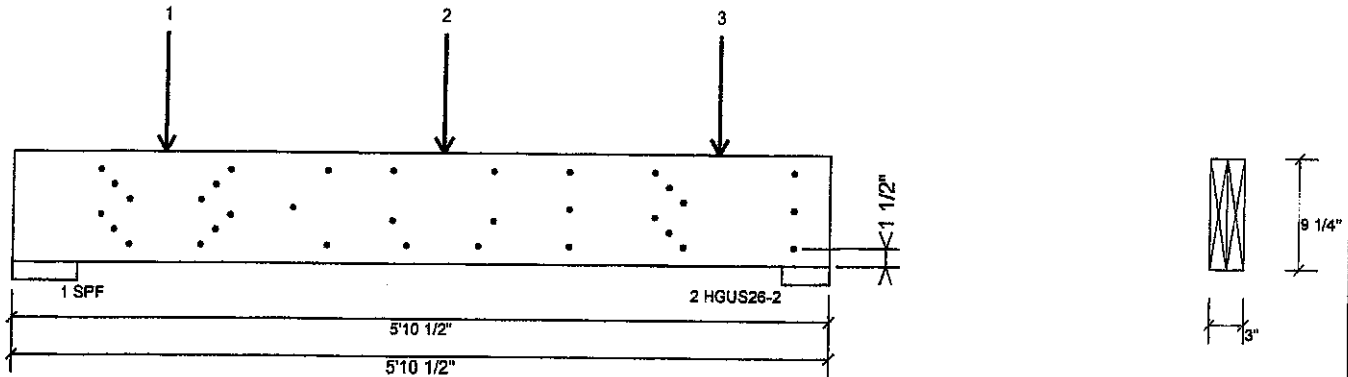
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 2 of 13

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 1-1-0 with a minimum of (12) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

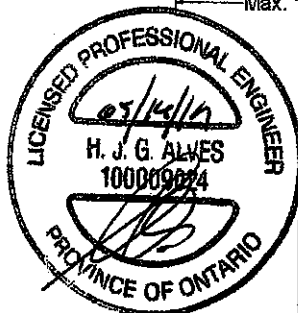
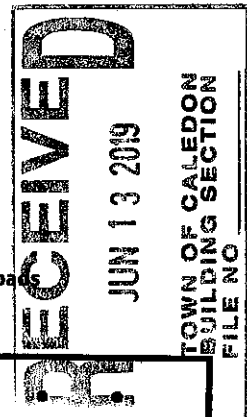
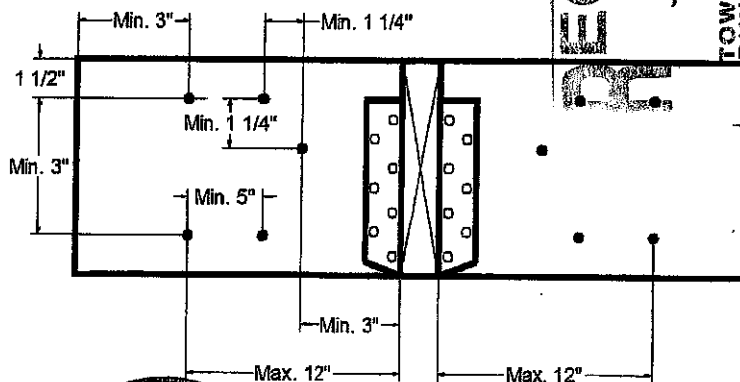
Capacity	72.3 %
Load	982.8lb.
Total Yield Limit	1360.2 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 3-1-0 with a minimum of (6) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	88.5 %
Load	588.5lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Load



DWG NO. TAM 1911517
STRUCTURAL
COMPONENT ONLY 2/3

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021



isDesign™

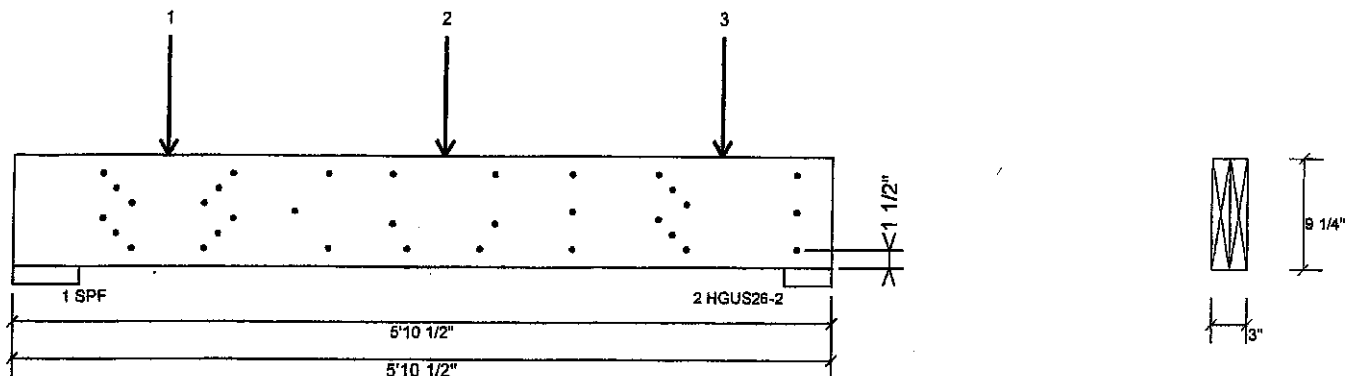
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 3 of 13

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

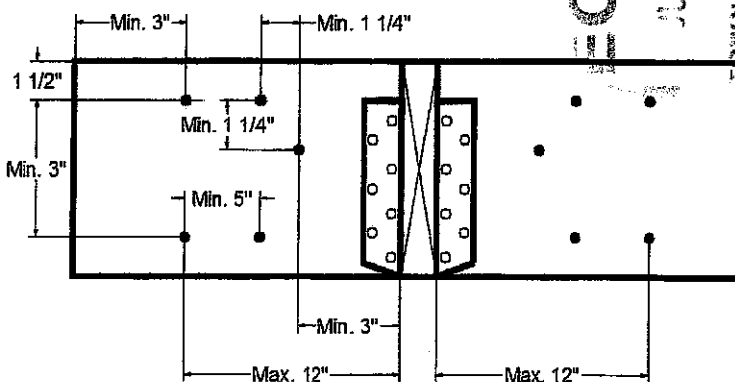
Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 5-1-0 with a minimum of (6) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	86.5 %
Load	588.5lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



DWG NO. TAM 11911 517
STRUCTURAL
COMPONENT ONLY 7/3

Manufacturer Info

Tamarack Roof Trusses
3259 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





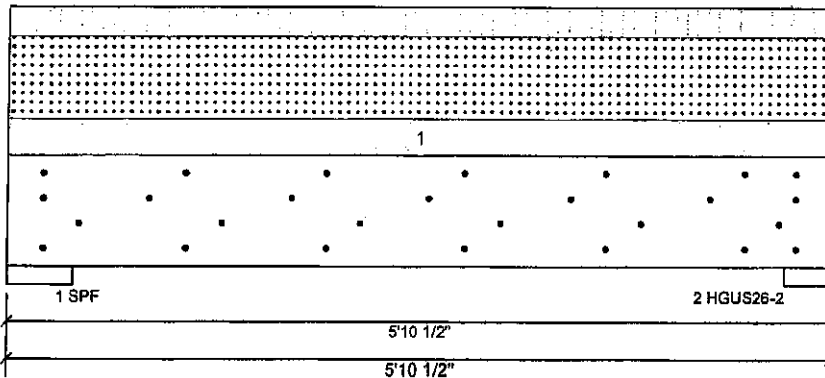
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 4 of 13

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	445	551	1250	0
2	426	526	1198	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	30%	689 / 2320	3009 L
2 - HGUS...	4.000"	39%	660 / 2223	2883 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3401 ft-lb	3'	6039 ft-lb	0.563 (56%)	1.25D+1.5S	L
Unbraced	3401 ft-lb	3'	5236 ft-lb	0.650 (65%)	1.25D+1.5S	L
Shear	2612 lb	1'2"	3964 lb	0.656 (66%)	1.25D+1.5S	L
LL Defl inch	0.030 (L/2096)	3'	0.174 (L/360)	0.170 (17%)	S+0.5L	L
TL Defl inch	0.041 (L/1525)	3'	0.174 (L/360)	0.240 (24%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

DWG NO. TAM 1911518
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform	14-1-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF		

Manufacturer InfoTamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

This design is valid until 12/11/2021





isDesign™

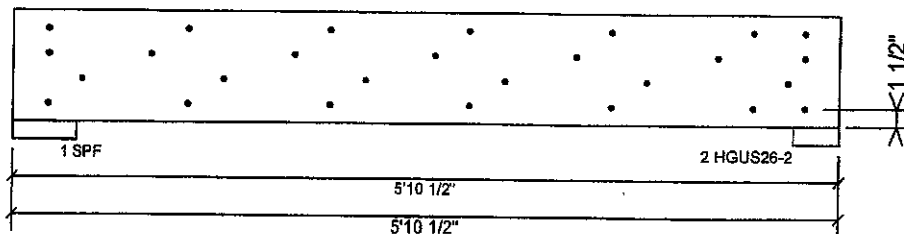
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 5 of 13

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

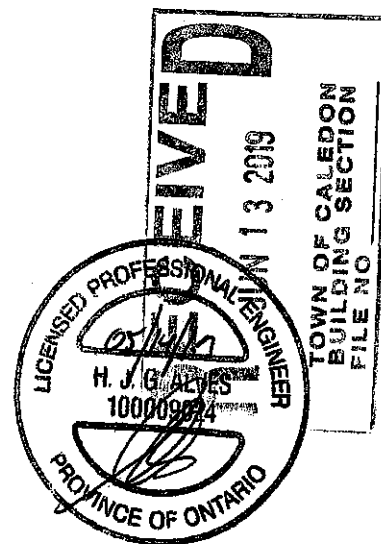
Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	98.1 %
Load	501.4 PLF
Yield Limit per Foot	511.3 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 71911513
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

TAMARACK
LUMBER INC.
ALFA LUMBER GROUP

This design is valid until 12/11/2021

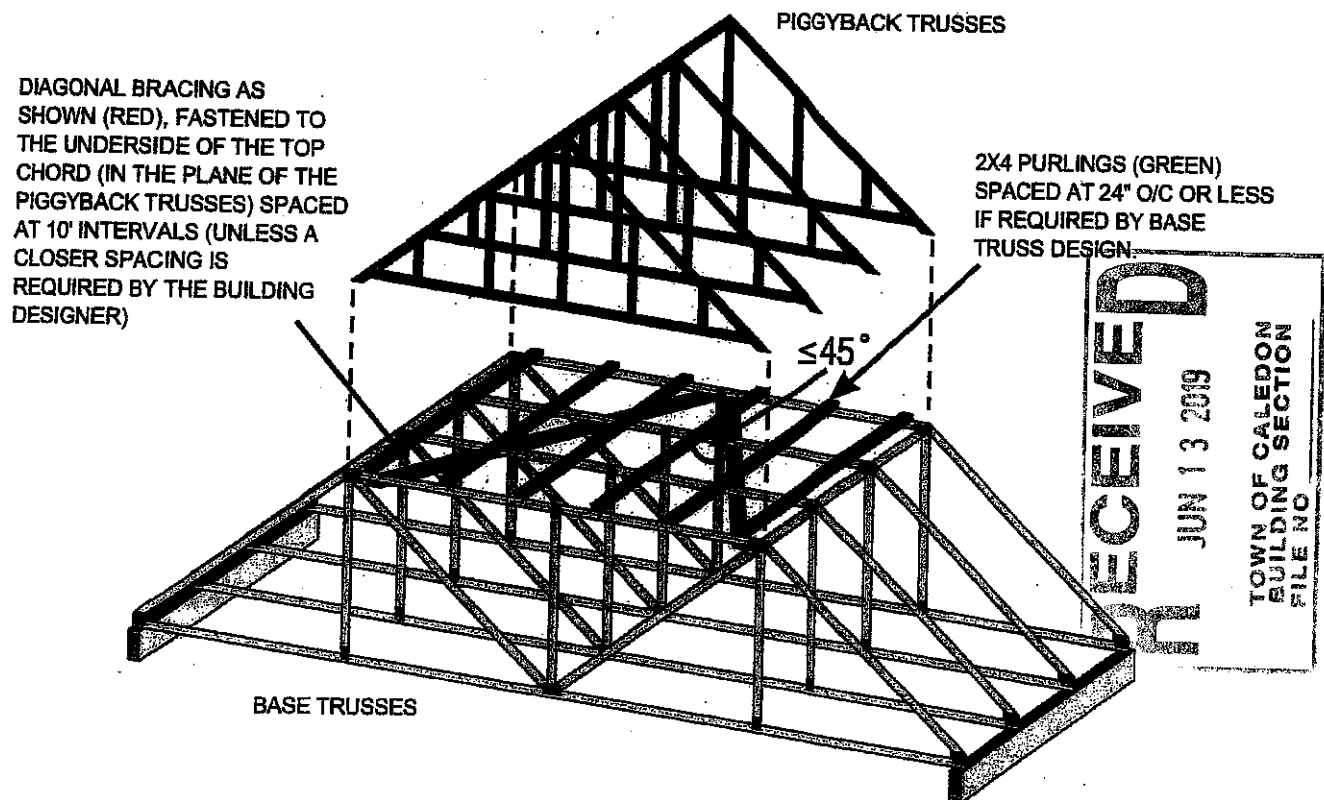


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

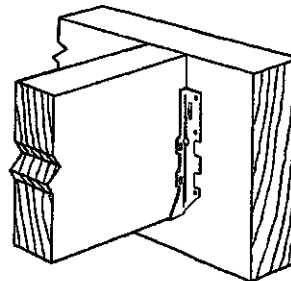
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

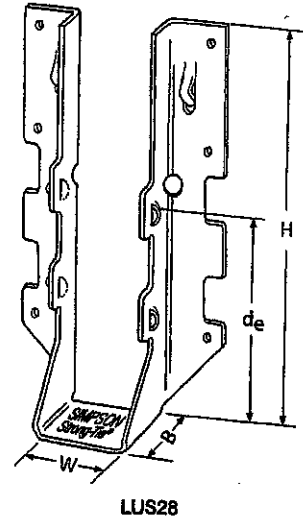
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

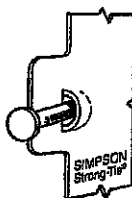


Typical LUS Installation



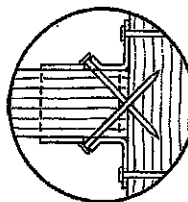
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 1/8	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 7/8	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 1/8	1 3/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.

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JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DESIGN

This technical bulletin is a revision of the previous edition, which was published in March 2010.

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strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

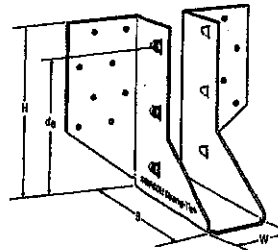
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

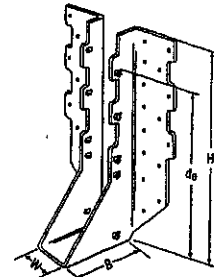
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

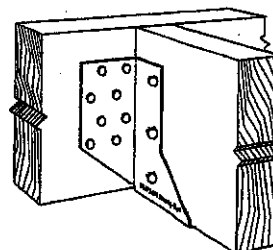
- See current catalogue for options



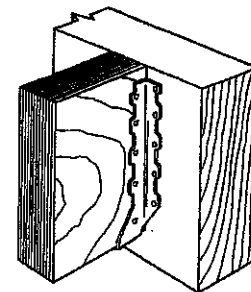
LJS26DS



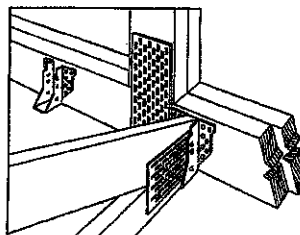
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



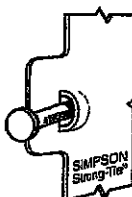
Typical HUS
Installation



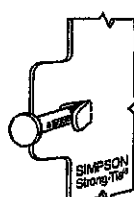
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist		D-Fr-L		S-P-F	
									Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/4	(16) 16d	(6) 16d		2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/8	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d		3605	5365	2875	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

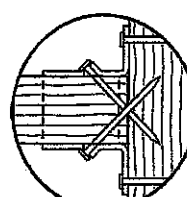
1. d_o is the distance from the seat of the hanger to the highest joist nail.



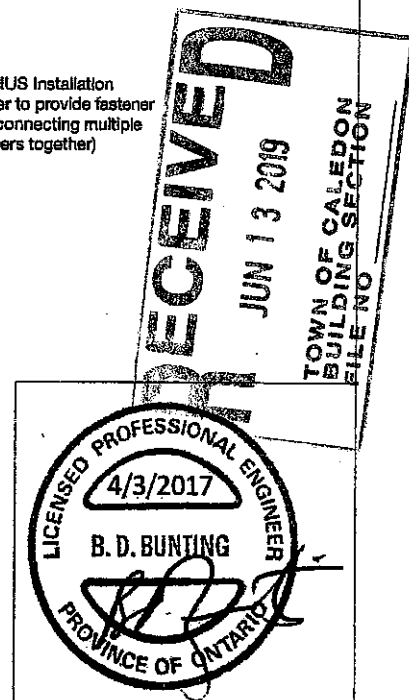
Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,803,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



DESIGN

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HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

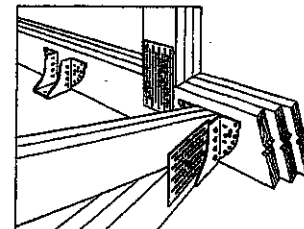
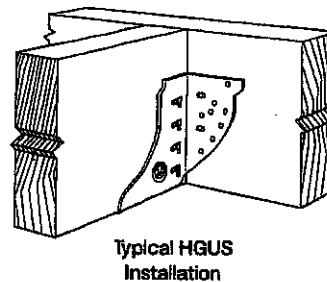
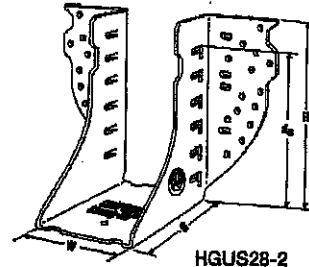
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

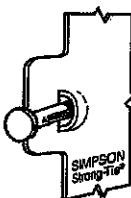
- See current catalogue for options



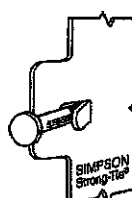
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _a ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

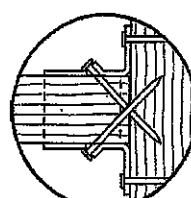
1. d_a is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



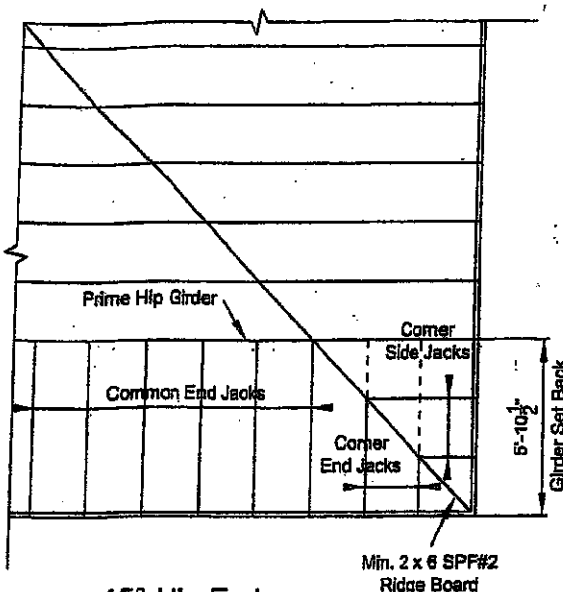
Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



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45° Hip End

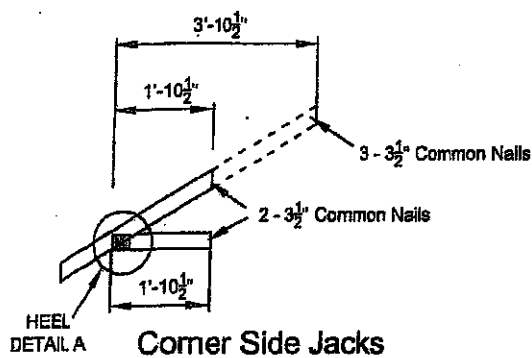
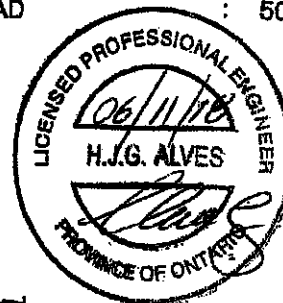
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

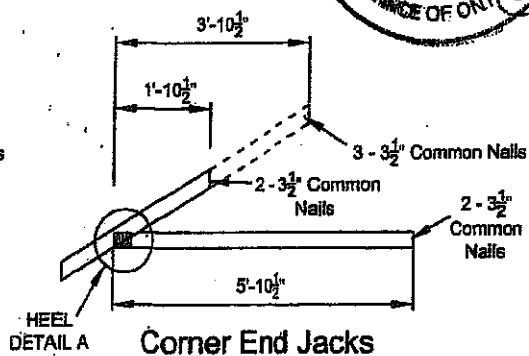
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

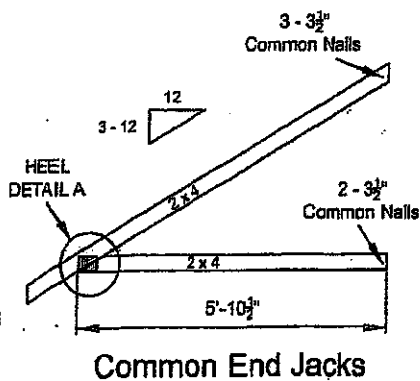
TOTAL LOAD : 50.5 P.S.F



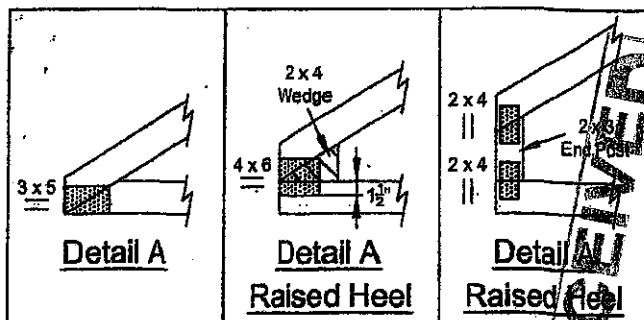
Corner Side Jacks



Corner End Jacks

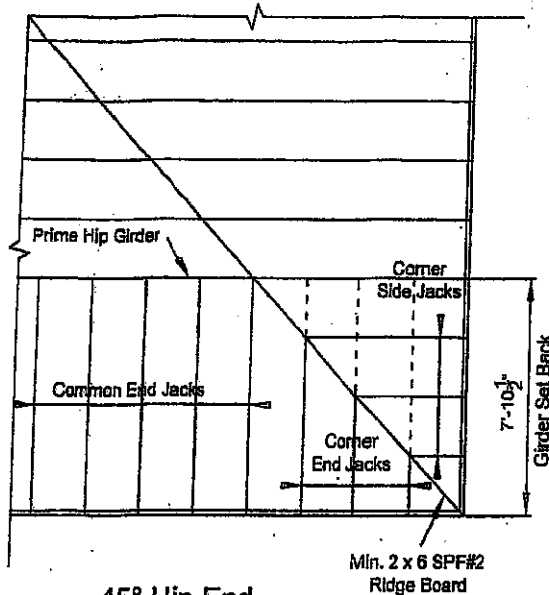


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

APPROVED
 JUN 13 2019
 T-1590216
 TOWN OF GARRISON
 BUILDING SECTION
 FILE NO.



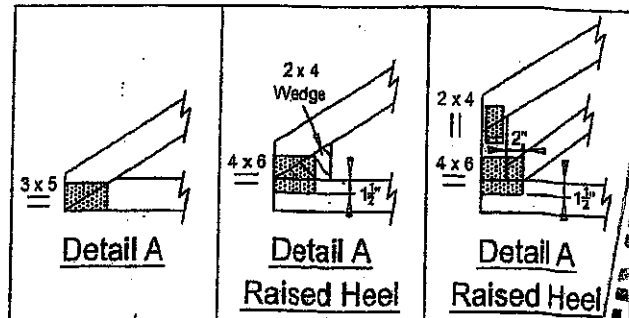
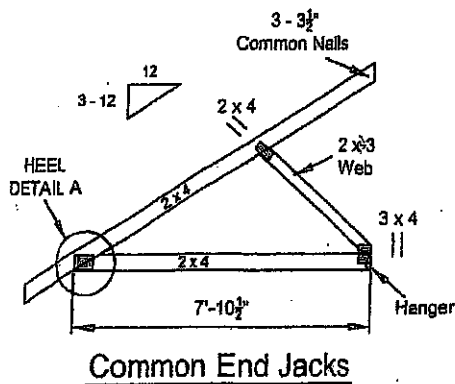
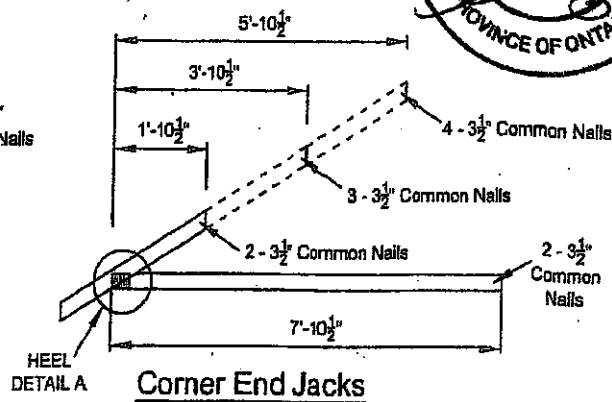
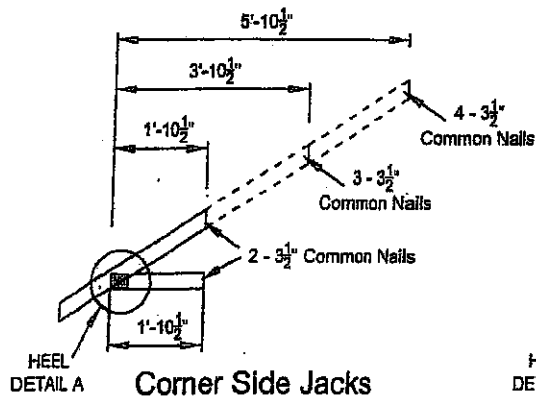
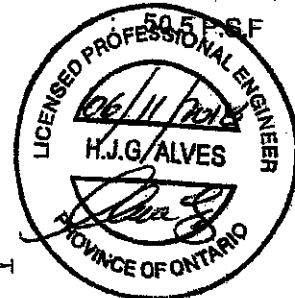
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

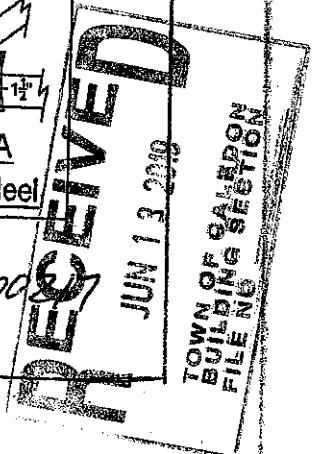
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD

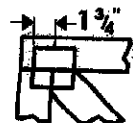


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

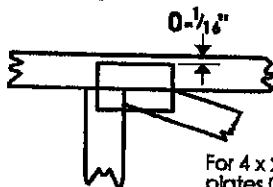


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

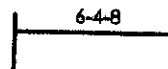


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

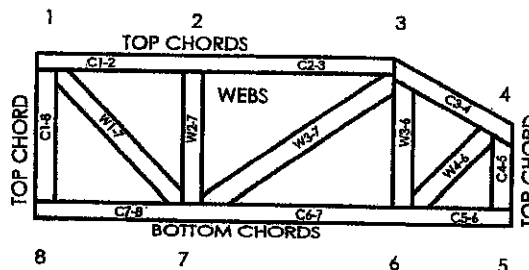
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal-Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
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MiTek Engineering Reference Sheet: MIL-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

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(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300213

Feb 09, 2018

