

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

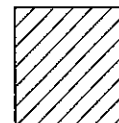
HARDWARE:

HUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

DENOTE:

ALL B- 2-2X10 (FLUSH)

DENOTES:
CONVENTIONAL
FRAMING

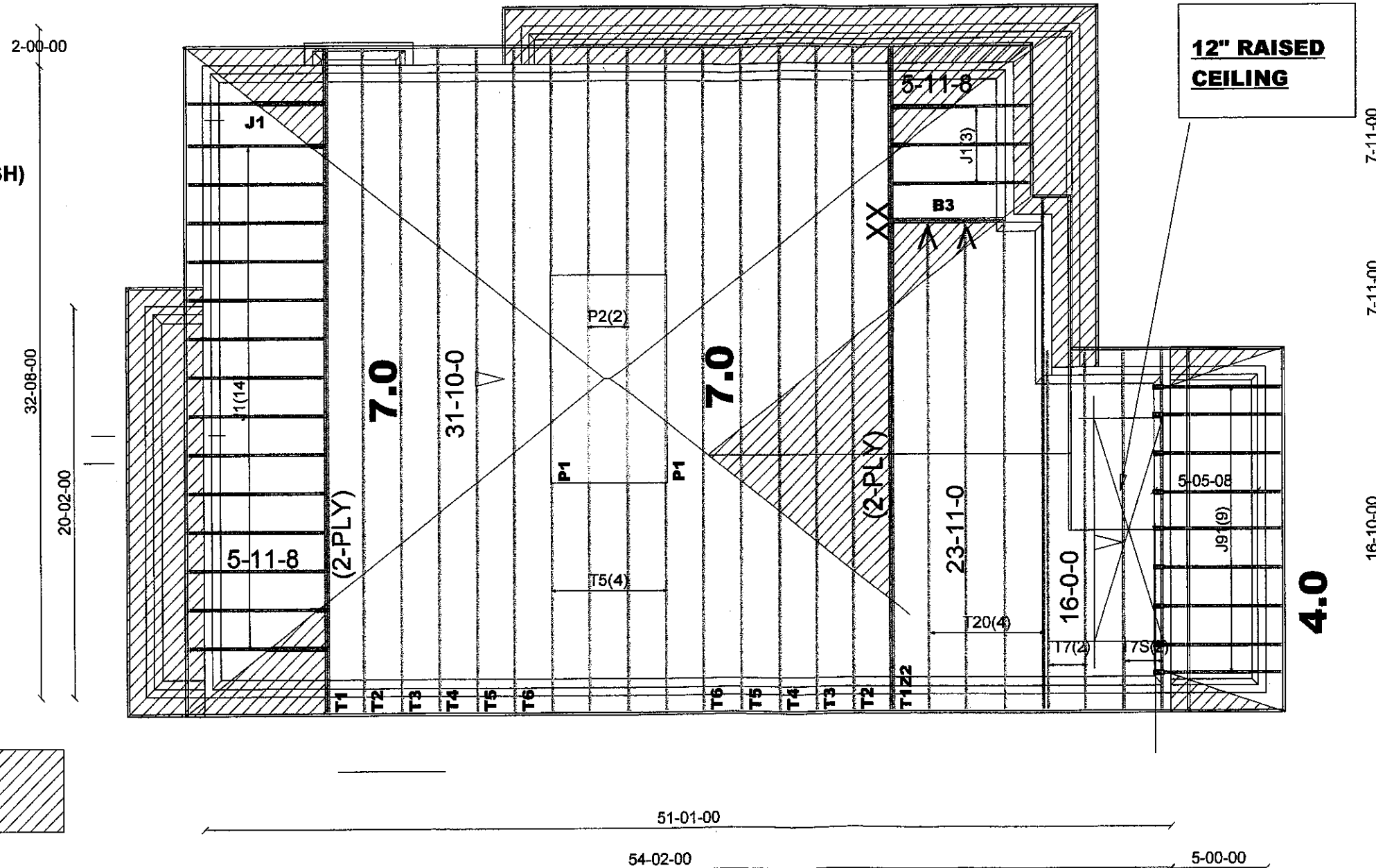


THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

9/12 PITCHES (TYP.)
UNLESS NOTED

RECEIVED
JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

GROUND SNOW LOAD $S_s = 1.8 \text{ kPa}$
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

M12265



Job Track: **50120**
Plan Log: **200595**
Layout ID: **403031**

Builder / Location:
GREEN PARK HOMES / Caledon
Project: **LAMBERTS LANE HOME CORP.**
Date: 5/7/2019 Sales: **Mario DiCano** Designer: **Johnny Dano**

Model / Elevation:
KINMOUNT 4B/2

Milek ver 8.2.3.229

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 4B
 Lot #:
 Elevation: 2

Job Track: 50120
 PlanLog: 200595
 Layout ID: 403031
 Ref #
 Page: 1 of 2
 Date: 05/07/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	31-10-00	4-09-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	326.96 199.33		
	1 2-ply	T1Z2 Hip Girder	9/12	31-10-00	4-09-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	326.96 199.33		
	2	T2 Hip	9/12	31-10-00	5-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	267.95 173.67		
	2	T3 Hip	9/12	31-10-00	7-01-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	290.58 185.67		
	2	T4 Hip	9/12	31-10-00	8-03-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	300.94 191.33		
	6	T5 Hip	9/12	31-10-00	9-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	948.51 599.00		
	2	T6 Hip	9/12	31-10-00	10-07-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	329.85 204.67		
	2	T7 Common	9/12	16-00-00	7-06-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	141.98 91.00		
	2	T7S Roof Special	9/12	16-00-00	7-06-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	150.03 96.67		
	4	T11 Common Structural Gable	9/12	23-11-00	10-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	457.42 280.00		
	2	P1 Piggyback	9/12	10-08-15	2-04-00	2 x 4			61.94 39.67		
	2	P2 Piggyback	9/12	10-08-15	3-06-00	2 x 4			60.59 38.00		
	18	J1 Jack-Open	7/12	5-11-08	4-09-02	2 x 4	1-03-08	1-03-07 4-09-02	315.05 192.00		
	9	J91 Jack-Open	4/12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	130.00 84.00		

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TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 4B
 Lot #:
 Elevation: 2

Job Track: 50120
 PlanLog: 200595
 Layout ID: 403031
 Ref #
 Page: 2 of 2
 Date: 05/07/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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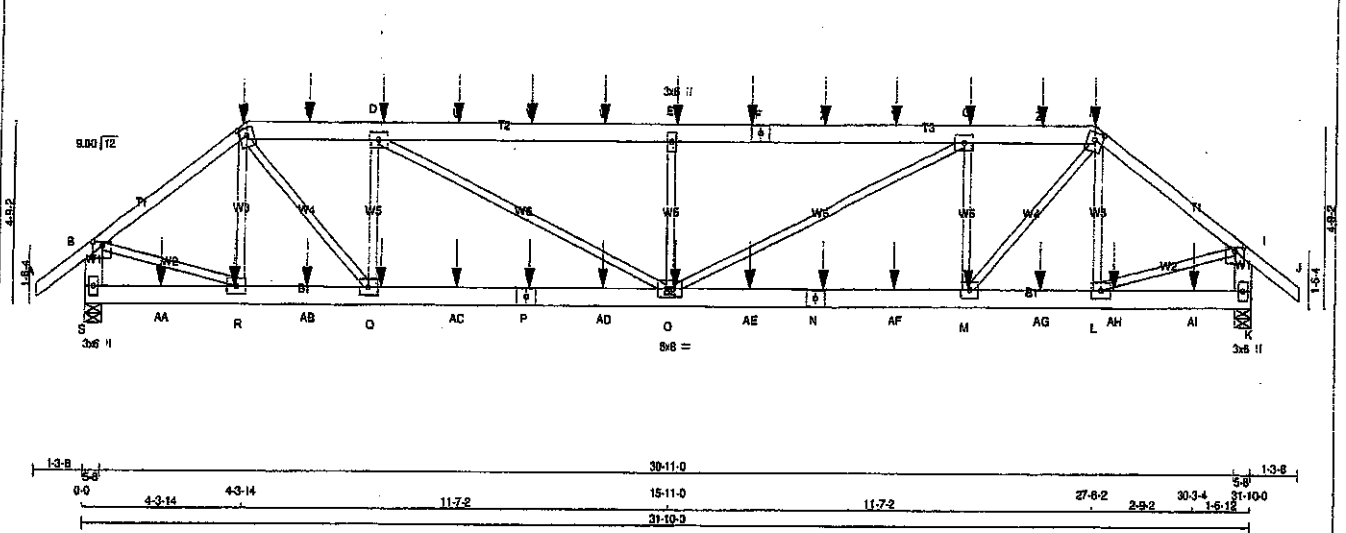
TOTAL # TRUSS= 57 TOTAL BFT OF ALL TRUSSES= 2574.34 BFT. TOTAL WEIGHT OF ALL TRSSES 4108.81 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
2	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 3

JOB NAME 401604	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 S Apr 23 2019 MTEK Industries, Inc. Tue May 14 14:00:08 2019 Page 1			
1-3-8 0-0 4-3-14 4-3-14 11-7-2 15-11-0 11-7-2 27-6-2 4-3-14 31-10-0 33-1-8		ID:biyPauQME1VecVM8aZJXxKyNDOP-hKJedat8AmnCGPezjSMPaOukWBOnhg8AFZlZBzGaJL			
1-3-8 0-0 4-3-14 4-3-14 11-7-2 15-11-0 11-7-2 27-6-2 30-3-4 31-10-0 1-3-8		Scale = 1:53.2			



LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x8	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x8	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C 1 12	SIDE(61.0)	
H - J 1 12	SIDE(61.0)	
C - F 2 12	SIDE(183.1)	
F - H 2 12	SIDE(183.1)	
S - B 2 12	TOP	
K - I 2 12	TOP	
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S - P 2 12	SIDE(183.1)	
P - N 2 12	SIDE(183.1)	
N - K 2 12	SIDE(183.1)	

WEBS: (0.122"x3") SPIRAL NAILS
2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.25	3.00
C TMVW+m	MT20	5.0	6.0	2.25	2.50
D TMVW-t	MT20	5.0	6.0		
E TMVW-w	MT20	3.0	6.0		
F TS-t	MT20	5.0	6.0		
G TMVW-t	MT20	5.0	6.0		
H TMVW+m	MT20	5.0	6.0	2.25	2.50
I TMVW-p	MT20	5.0	6.0	1.25	3.00
K BMV1-p	MT20	3.0	6.0		
L, M, Q, R					
L BMVW-t	MT20	5.0	6.0		
N BS-t	MT20	5.0	6.0		
O BMVW-t	MT20	5.0	6.0		
P BS-t	MT20	5.0	6.0		
S BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	1683 / 0	334 / 0	0 / 0	727 / 0
S	3737 / 0	3737 / 0	5-8	5-8
K	3655 / 0	3655 / 0	5-8	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	1683 / 0	334 / 0	0 / 0
S	3737 / 0	3737 / 0	5-8
K	3655 / 0	3655 / 0	5-8

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

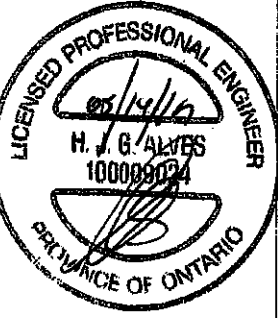
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 42	-102.1	-102.1	0.08 (1)	R-C	-695 / 0	0.11 (1)	
B-C	-4065 / 0	-102.1	-102.1	0.27 (1)	C-Q	0 / 3046	0.38 (1)	
C-T	-5198 / 0	-102.1	-102.1	0.28 (1)	Q-D	-2018 / 0	0.31 (1)	
T-D	-5198 / 0	-102.1	-102.1	0.28 (1)	M-G	-2059 / 0	0.31 (1)	
D-U	-5987 / 0	-102.1	-102.1	0.45 (1)	M-H	0 / 3085	0.38 (1)	
U-V	-5987 / 0	-102.1	-102.1	0.45 (1)	L-H	-672 / 0	0.11 (1)	
V-W	-5987 / 0	-102.1	-102.1	0.45 (1)	B-R	0 / 3371	0.42 (1)	
W-E	-5987 / 0	-102.1	-102.1	0.45 (1)	L-I	0 / 3283	0.41 (1)	
E-F	-5987 / 0	-102.1	-102.1	0.45 (1)	O-G	0 / 1955	0.24 (1)	
F-X	-5987 / 0	-102.1	-102.1	0.45 (1)	D-O	0 / 1693	0.23 (1)	
X-Y	-5987 / 0	-102.1	-102.1	0.45 (1)	O-E	-1398 / 0	0.21 (1)	
Y-G	-5987 / 0	-102.1	-102.1	0.45 (1)				
G-Z	-5143 / 0	-102.1	-102.1	0.27 (1)				
Z-H	-5143 / 0	-102.1	-102.1	0.27 (1)				
H-I	-3959 / 0	-102.1	-102.1	0.27 (1)				
I-J	0 / 42	-102.1	-102.1	0.08 (1)				
S-B	-3647 / 0	0.0	0.0	0.13 (1)				
K-I	-3561 / 0	0.0	0.0	0.13 (1)				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
		4-3-14	-56	-83	-	FRONT	VERT	DEAD	-	-
		4-3-14	-132	-132	-	BACK	VERT	TOTAL	-	-
		4-3-14	-308	-308	-	FRONT	VERT	SNOW	-	-



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/600 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.45/1.00 (D-E:1), BC=0.43/1.00 (O-Q:1), WB=0.42/1.00 (B-R:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 798 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (1) (INPUT = 0.90)
JSI METAL = 0.58 (N) (INPUT = 1.00)

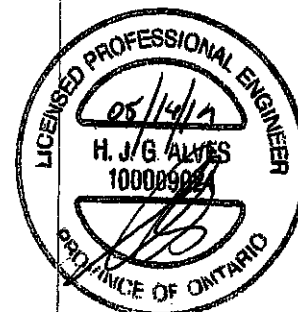
Structural component only
DWG# T-1911538 CONTINUED ON PAGE 2

TOWN OF CALEDON
BUILDING SECTION
FILE NO

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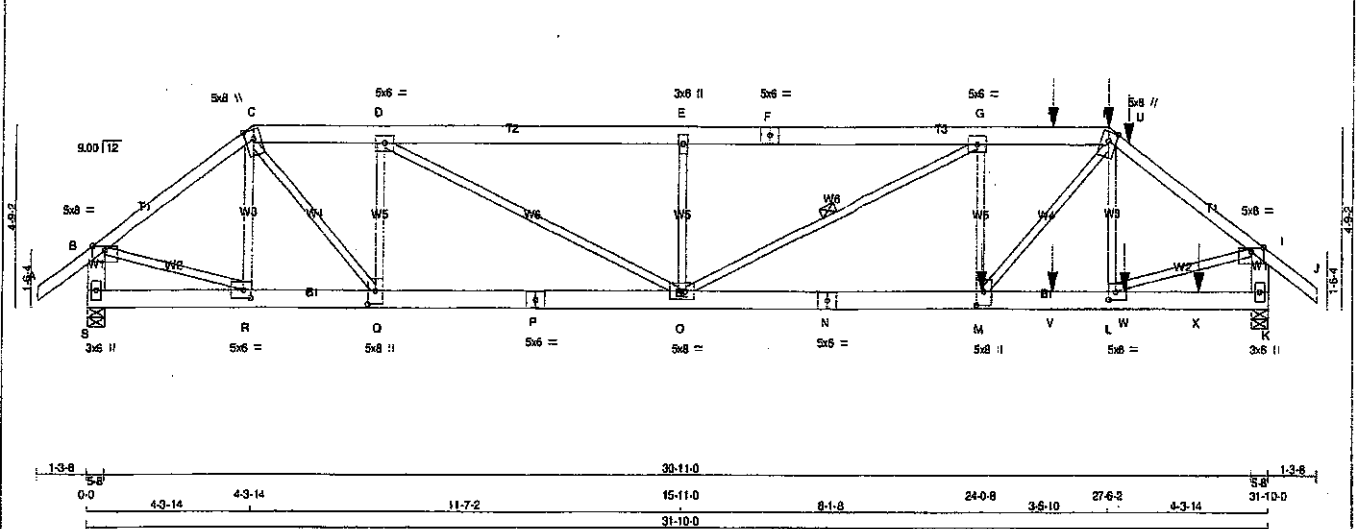
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401604	T1	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 Mitak Industries, Inc. Tue May 14 14:00:08 2019 Page 2		
IDbivPcuQME1VecVM6aZJXxKyNDOP-hKUe0a18AmNC6PezjSMPaOuKWBCnhg8AFZizZBzGaJL						

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	8-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
E	18-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
F	18-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
G	24-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
H	27-6-2	-56	-63	---	FRONT	VERT	DEAD	---	---
H	27-6-2	-308	-308	---	FRONT	VERT	SNOW	---	---
M	24-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
N	20-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
O	18-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
P	12-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
Q	8-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
R	4-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
T	8-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
U	10-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
V	12-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
W	14-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
X	20-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
Y	22-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
Z	28-0-12	-126	-126	---	BACK	VERT	TOTAL	---	---
AA	2-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
AB	8-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
AC	10-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
AD	14-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
AE	18-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
AF	22-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
AG	26-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
AH	28-0-12	-26	-26	---	BACK	VERT	TOTAL	---	---
AI	30-3-4	-26	-26	---	BACK	VERT	TOTAL	---	---



Structural component only
DWG# T-1911538 -2

JOB NAME 401604	TRUSS NAME T1Z2	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 14:00:09 2019 Page 1			
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15-11-0 27-6-2 27-11-12 31-10-0 31-1-8 1-3-8 1-3-8 0-0 4-3-14 4-3-14 11-7-2 10-0-12 1-0-8 2-10 3-10-4 1-3-8 Scale = 1:53.2					



TOTAL WEIGHT = 2 X 163 = 327 lb

LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	A - C	2x4	DRY	No.2
C - F	2x6	DRY	No.2	C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2	F - H	2x6	DRY	No.2
H - J	2x4	DRY	No.2	H - J	2x4	DRY	No.2
S - B	2x6	DRY	No.2	S - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2	K - I	2x6	DRY	No.2
S - P	2x6	DRY	No.2	S - P	2x6	DRY	No.2
P - N	2x6	DRY	No.2	P - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2	N - K	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2	ALL WEBS	2x3	DRY	No.2
EXCEPT				EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	1	12
H - J	1	12
C - F	2	12
F - H	2	12
S - B	2	12
K - I	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S - P	2	12
P - N	2	12
N - K	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1	6
G - M	1	5
D - Q	1	5

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-p	MT20	5.0	8.0	Edge
C	TTWW-m	MT20	5.0	8.0	Edge
D	TMWW-i	MT20	5.0	8.0	Edge
E	TMWW-w	MT20	5.0	8.0	Edge
F	TS-i	MT20	5.0	8.0	Edge
G	TMWW-i	MT20	5.0	8.0	Edge
H	TTWW-m	MT20	5.0	8.0	Edge
I	TMWW-p	MT20	5.0	8.0	Edge
K	BMV1-p	MT20	5.0	8.0	Edge
L	BMWW-i	MT20	5.0	8.0	Edge
M	BMWW-i	MT20	5.0	8.0	Edge
N	BS-i	MT20	5.0	8.0	Edge
O	BMWW-w	MT20	5.0	8.0	Edge
P	BS-i	MT20	5.0	8.0	Edge
Q	BMWW-i	MT20	5.0	8.0	Edge
R	BMWW-i	MT20	5.0	8.0	Edge

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED					MAXIMUM FACTORED				
GROSS REACTION					GROSS REACTION				
JT	VERT	HORIZ	UPLIFT	IN-SX	JT	VERT	HORIZ	UPLIFT	IN-SX
S	3024	0	3024	0	S	3024	0	3024	0
K	4688	0	4688	0	K	4688	0	4688	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS
S	2241	1294 / 0
K	3434	2047 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS					WEBS				
MEMB.	MAX. FACTORED (LBS)	VERT. LOAD (PLF)	LC1	MAX. UNBRAC	MEMB.	MAX. FACTORED (LBS)	FORCE (LBS)	CS1 (LC)	MAX. UNBRAC
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	R - C	-550 / 0	0.08 (1)	0.08 (1)
B-C	-3020 / 0	-102.1	-102.1	0.24 (1)	4.96	C - D	0 / 2831	0.35 (1)	0.35 (1)
C-D	-4366 / 0	-102.1	-102.1	0.17 (1)	5.33	D - E	-1941 / 0	0.30 (1)	0.30 (1)
D-E	-8681 / 0	-102.1	-102.1	0.31 (1)	4.38	M - H	-714 / 0	0.11 (1)	0.11 (1)
E-F	-6681 / 0	-102.1	-102.1	0.31 (1)	4.38	M - H	0 / 4043	0.50 (1)	0.50 (1)
F-G	-6681 / 0	-102.1	-102.1	0.31 (1)	4.38	L - R	-875 / 0	0.14 (1)	0.14 (1)
G-T	-6742 / 0	-102.1	-102.1	0.18 (1)	4.47	H - R	0 / 2831	0.35 (1)	0.35 (1)
T-H	-6742 / 0	-102.1	-102.1	0.18 (1)	4.47	L - I	0 / 2831	0.35 (1)	0.35 (1)
H-U	-1167 / 0	-102.1	-102.1	0.31 (1)	3.97	O - G	0 / 2831	0.35 (1)	0.35 (1)
U-I	-1167 / 0	-102.1	-102.1	0.31 (1)	3.97	D - C	0 / 2831	0.35 (1)	0.35 (1)
I-J	0 / 42	-102.1	-102.1	0.08 (1)	10.00	O - G	0 / 2831	0.35 (1)	0.35 (1)
S-B	-2949 / 0	0.0	0.0	0.11 (1)	7.81	O - G	0 / 2831	0.35 (1)	0.35 (1)
K-I	-4583 / 0	0.0	0.0	0.17 (1)	6.74	O - G	0 / 2831	0.35 (1)	0.35 (1)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	27-6-2	-308	-308	---	FRONT	VERT	SNOW	---	---
M	24-0-8	-2214	-2214	---	FRONT	VERT	TOTAL	---	---
T	25-11-12	-126	-126	---	FRONT	VERT	TOTAL	---	---
U	27-11-12	-153	-153	---	FRONT	VERT	TOTAL	---	---
V	25-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	---
W	27-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	---
X	23-11-12	-26	-26	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 5.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012

- CSA 088-09, CSA 088-14

- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")

CALCULATED VERT. DEFL.(LL) = L/999 (0.15")

ALLOWABLE DEFL.(TL) = L/360 (1.06")

CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.36/1.00 (H-K1), BC=0.53/1.00 (M-O1), WB=0.53/1.00 (L-1), SS=0.15/1.00 (E-G1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(OR)Y SHEAR SECTION (PSI) (PLI) (PLJ)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)

JSI METAL= 0.68 (N) (INPUT = 1.00)

Structural component only

DWG# T-1911539

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401604	T1Z2	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 14:00:09 2019 Page 2	

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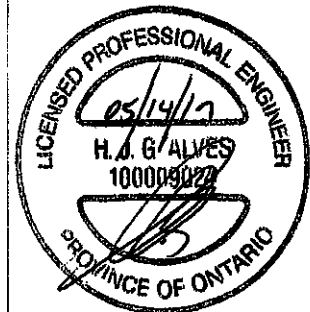
PLATES (table is in inches)

IT TYPE	PLATES	W	LEN	Y	X
S	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

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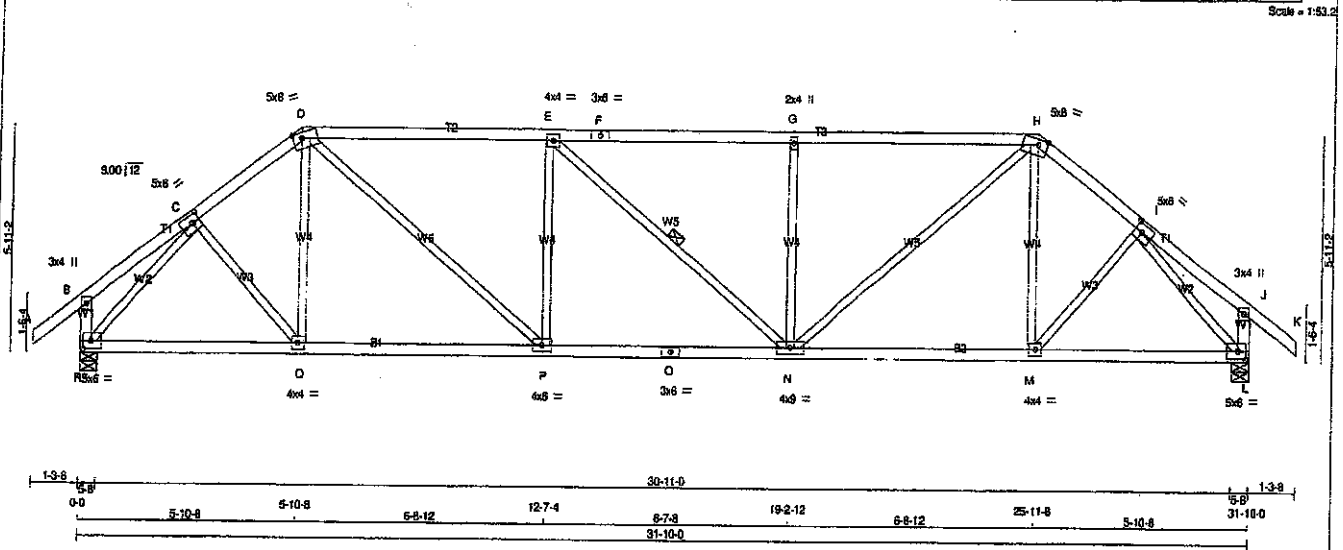
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



Structural component only
DWG# T-1911539-2

JOB NAME: 401604 TRUSS NAME: T2 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: DRWG NO. 1

Tamarack Roof Truss, Burlington Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 14:00:10 2019 Page 1
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1-3-8 0-0 3-0-8 2-10-8 5-10-8 8-8-12 12-7-4 8-7-8 19-2-12 6-8-12 25-11-8 28-9-8 31-10-0 31-1-8
Scale = 1/8" = 1'-0"



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE		LUMBER
A - D	F	2x4	DRY	No.2
D - F	H	2x4	DRY	No.2
F - H	J	2x4	DRY	No.2
H - K	L	2x4	DRY	No.2
K - L	O	2x4	DRY	No.2
L - J		2x4	DRY	No.2
J - O		2x4	DRY	No.2
O - L		2x4	DRY	No.2
ALL WEBS		2x3	DRY	No.2
EXCEPT				
DRY: SEASONED LUMBER				

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	5.0	6.0	2.50	2.50
D	TTW+1	MT20	5.0	6.0	1.75	3.00
E	TMVW+1	MT20	4.0	4.0		
F	TS-1	MT20	3.0	5.0		
G	TMVW+1	MT20	2.0	4.0		
H	TTW+1	MT20	5.0	6.0	1.75	3.00
I	TMVW+1	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW+1	MT20	5.0	6.0	2.50	2.50
M	BMVW+1	MT20	4.0	4.0		
N	BMVW+1	MT20	4.0	6.0		
O	BS-1	MT20	3.0	6.0		
P	BMVW+1	MT20	4.0	6.0		
Q	BMVW+1	MT20	4.0	4.0		
R	BMVW+1	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
R 2044	0	2044	0
L 2044	0	2044	0

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
R 1435	1004 / 0 0 / 0 0 / 0 430 / 0 0 / 0
L 1435	1004 / 0 0 / 0 0 / 0 430 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00	C-Q	0 / 200	0.04 (1)	
B-C	0 / 17	-102.1 -102.1 0.11 (1)	10.00	Q-D	-25 / 66	0.02 (4)	
C-D	-2089 / 0	-102.1 -102.1 0.17 (1)	4.57	D-P	0 / 1332	0.30 (1)	
D-E	-2859 / 0	-102.1 -102.1 0.94 (1)	3.22	P-E	-740 / 0	0.40 (1)	
E-F	-2858 / 0	-102.1 -102.1 0.92 (1)	3.22	E-N	-2 / 0	0.00 (1)	
F-G	-2858 / 0	-102.1 -102.1 0.92 (1)	3.22	N-G	-739 / 0	0.40 (1)	
G-H	-2858 / 0	-102.1 -102.1 0.93 (1)	3.23	N-H	0 / 1330	0.30 (1)	
H-I	-2089 / 0	-102.1 -102.1 0.17 (1)	4.57	M-H	-24 / 66	0.02 (4)	
I-J	0 / 17	-102.1 -102.1 0.11 (1)	10.00	M-I	0 / 200	0.04 (1)	
J-K	0 / 42	-102.1 -102.1 0.14 (1)	10.00	R-C	-2300 / 0	0.73 (1)	
K-B	-282 / 0	0.0 0.0 0.03 (1)	7.81	L-L	-2300 / 0	0.73 (1)	
L-J	-282 / 0	0.0 0.0 0.03 (1)	7.81				
R-Q	0 / 1503	-17.5 -17.5 0.33 (1)	10.00				
Q-P	0 / 1637	-17.5 -17.5 0.35 (1)	10.00				
P-O	0 / 2859	-17.5 -17.5 0.50 (1)	10.00				
O-N	0 / 2859	-17.5 -17.5 0.50 (1)	10.00				
N-M	0 / 1637	-17.5 -17.5 0.36 (1)	10.00				
M-L	0 / 1503	-17.5 -17.5 0.33 (1)	10.00				

REGISTERED PROFESSIONAL ENGINEER

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF
DL = 7.0 PSF

TOTAL LOAD = 42.0 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.94/1.00 (D-E-1), BC=0.50/1.00 (N-P-1),
WB=0.73/1.00 (L-L-1), SS=0.32/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687

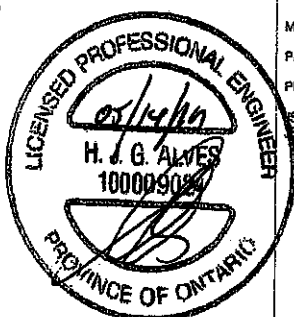
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (L) (INPUT = 0.90)
SI METAL= 0.87 (C) (INPUT = 1.00)

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TOWN OF CALEDON
BUILDING SECTION
FILE NO.

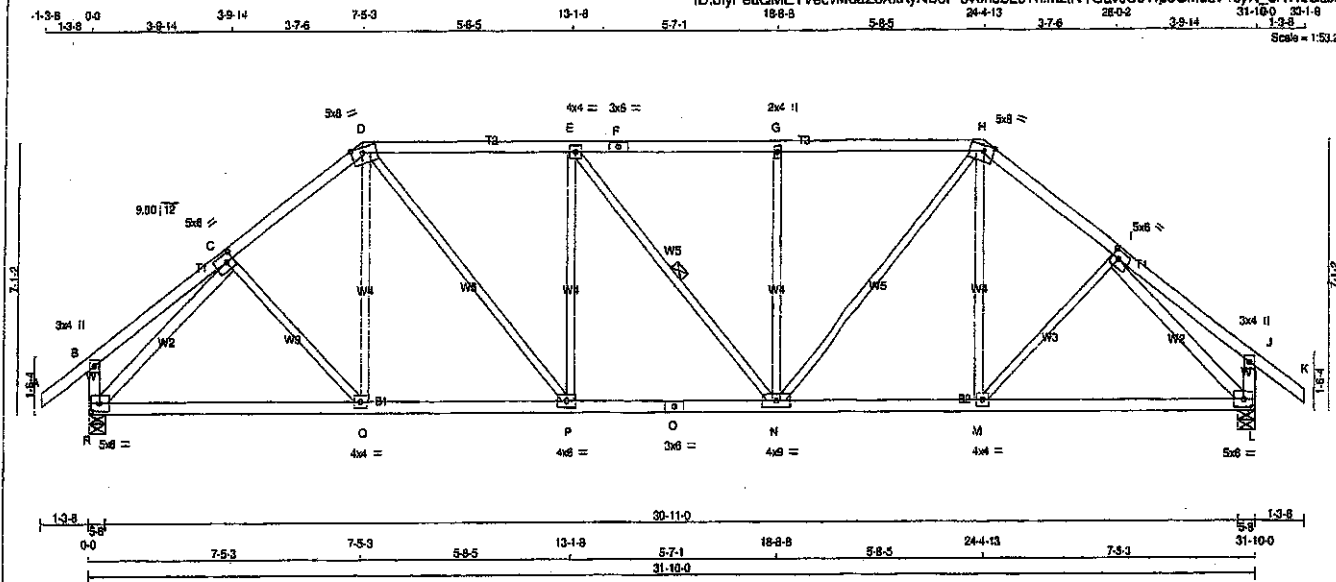
Structural component only
DWG# T-1911540

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401604	T3	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 145 = 291 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV-w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.50
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV-p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	2379	0	2379	0
L	2379	0	2379	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1789	1004 / 0
L	1789	1004 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 42	-102.1 -102.1 0.14 (1)	C-Q	0 / 150
B-C	0 / 25	-102.1 -102.1 0.21 (1)	Q-D	0 / 284
C-D	-2413 / 0	-102.1 -102.1 0.31 (1)	D-P	0 / 1065
D-E	-2594 / 0	-102.1 -102.1 0.62 (1)	P-E	-622 / 0
E-F	-2593 / 0	-102.1 -102.1 0.82 (1)	E-N	-3 / 0
F-G	-2593 / 0	-102.1 -102.1 0.82 (1)	N-G	-821 / 0
G-H	-2592 / 0	-102.1 -102.1 0.61 (1)	N-H	0 / 1062
H-I	-2413 / 0	-102.1 -102.1 0.31 (1)	M-H	0 / 265
I-J	0 / 25	-102.1 -102.1 0.21 (1)	M-I	0 / 150
J-K	0 / 42	-102.1 -102.1 0.14 (1)	R-C	-2695 / 0
R-B	-288 / 0	0.0 0.0 0.03 (1)	I-L	-2695 / 0
L-J	-288 / 0	0.0 0.0 0.03 (1)		
R-Q	0 / 1837	-38.5 -38.5 0.60 (2)		
Q-P	0 / 1910	-38.5 -38.5 0.51 (2)		
P-O	0 / 2594	-38.5 -38.5 0.53 (1)		
O-N	0 / 2594	-38.5 -38.5 0.53 (1)		
N-M	0 / 1910	-38.5 -38.5 0.81 (2)		
M-L	0 / 1837	-38.5 -38.5 0.80 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)	= L/360 (1.06")
CALCULATED VERT. DEFL.(LL)	= L/899 (0.14")
ALLOWABLE DEFL.(TL)	= L/360 (1.06")
CALCULATED VERT. DEFL.(TL)	= L/999 (0.24")

CSI: TG=0.62/1.00 (D-E:1), 8C=0.61/1.00 (M-N:2), WB=0.92/1.00 (I-L:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

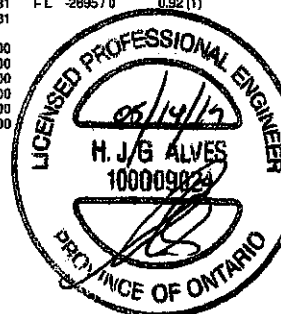
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP(DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN MAX MIN	MAX MIN
MT20	618 354 1867 788 1867 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

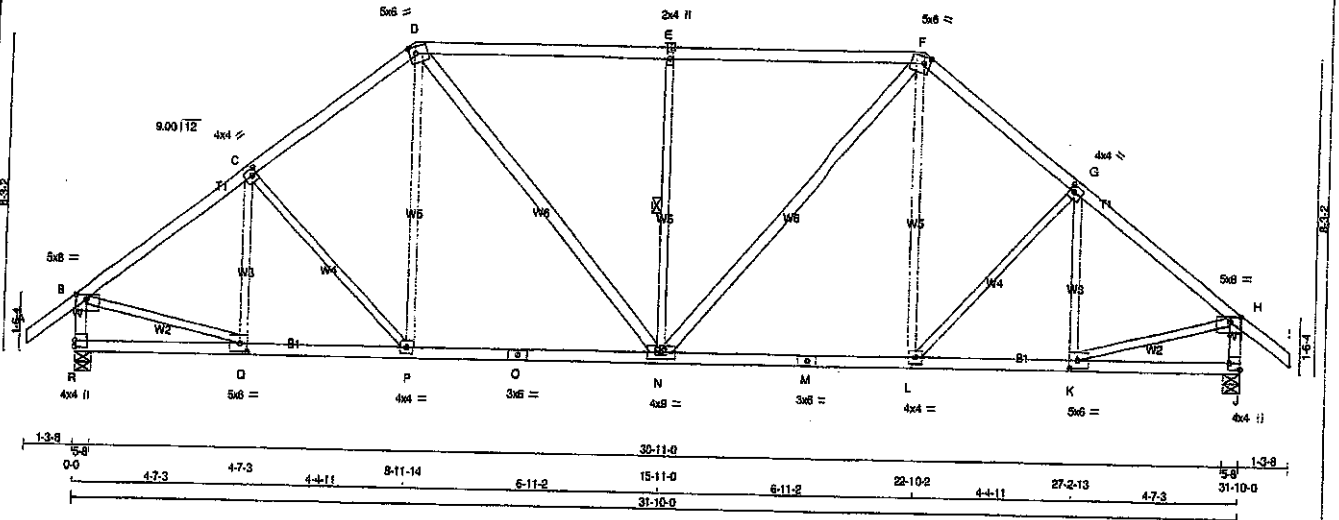
SI GRIP = 0.39 (I) (INPUT = 0.50)
SI METAL = 0.94 (C) (INPUT = 1.00)



Structural component only
DWG# T-1911541

JOB NAME: 401604 TRUSS NAME: T4 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington DRWG NO. 139

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 1-3-8 0-0 4-7-3 4-7-3 4-4-11 8-11-14 6-11-2 15-11-0 6-11-2 22-10-2 4-4-11 27-2-13 4-7-3 31-10-0 31-10-0
 Scale = 1:33.2



LUMBER

N. L. & A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x4 DRY No.2

EXCEPT

D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	1.50	3.50
C	TMW-w	MT20	4.0	4.0	2.00	1.50
D	TMW-m	MT20	5.0	8.0	2.00	2.00
E	TMW-w	MT20	2.0	4.0		
F	TMW-m	MT20	5.0	8.0	2.00	2.00
G	TMW-w	MT20	4.0	4.0	2.00	1.50
H	TMW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1-p	MT20	4.0	4.0	2.00	Edge
K	BMW-w	MT20	5.0	8.0	2.50	2.50
L	BMW-w	MT20	4.0	4.0		
M	BS-i	MT20	3.0	6.0		
N	BMW-w	MT20	4.0	4.0		
O	BS-i	MT20	3.0	6.0		
P	BMW-w	MT20	4.0	4.0		
Q	BMW-w	MT20	5.0	8.0	2.50	2.50
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION UP	IN-SP	REQD
R	2379	0	2379	0	5-8	5-8
J	2379	0	2379	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS
R	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
R	1769	1004 / 0 334 / 0 0 / 0 430 / 0 0 / 0
J	1769	1004 / 0 334 / 0 0 / 0 430 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRAC LENGTH (R-TO)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
R-TO		FROM TO			FR-TO		
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	G-P	-335 / 0	0.12 (1)
B-C	-2418 / 0	-102.1 -102.1	0.33 (1)	4.13	C-P	-192 / 0	0.14 (1)
C-D	-2320 / 0	-102.1 -102.1	0.33 (1)	4.21	P-D	0 / 393	0.09 (2)
D-E	-2320 / 0	-102.1 -102.1	0.75 (1)	3.55	D-N	0 / 759	0.12 (1)
E-F	-2320 / 0	-102.1 -102.1	0.75 (1)	3.55	N-E	-868 / 0	0.39 (1)
F-G	-2320 / 0	-102.1 -102.1	0.33 (1)	4.21	N-F	0 / 759	0.12 (1)
G-H	-2418 / 0	-102.1 -102.1	0.35 (1)	4.13	L-F	0 / 393	0.09 (2)
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	L-G	-192 / 0	0.14 (1)
R-B	-2301 / 0	0.0 0.0	0.24 (1)	5.62	K-G	-335 / 0	0.12 (1)
J-H	-2301 / 0	0.0 0.0	0.24 (1)	5.62	B-O	0 / 2024	0.46 (1)
R-O	0 / 0	-38.5 -38.5	0.13 (3)	10.00	K-H	0 / 2024	0.46 (1)
O-P	0 / 1959	-38.5 -38.5	0.46 (1)	10.00			
P-Q	0 / 1831	-38.5 -38.5	0.54 (2)	10.00			
O-N	0 / 1831	-38.5 -38.5	0.54 (2)	10.00			
N-M	0 / 1831	-38.5 -38.5	0.54 (2)	10.00			
M-L	0 / 1831	-38.5 -38.5	0.54 (2)	10.00			
L-K	0 / 1959	-38.5 -38.5	0.46 (1)	10.00			
K-J	0 / 0	-38.5 -38.5	0.13 (3)	10.00			

USED PROFESSIONAL

25/1/14

TOTAL WEIGHT = 2 X 150 = 301 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 28.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012
 - CSA 086-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(85% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.75/1.00 (D-E:1), BC=0.54/1.00 (N-P:2), WB=0.49/1.00 (B-C:1), SS=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

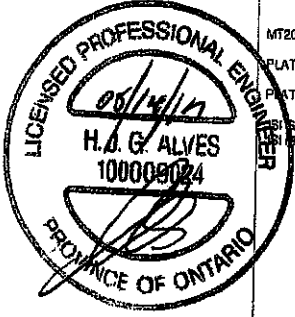
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSB) (PLI) (PLJ)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.87 (K) (INPUT = 0.90)
 METAL = 0.75 (Q) (INPUT = 1.00)



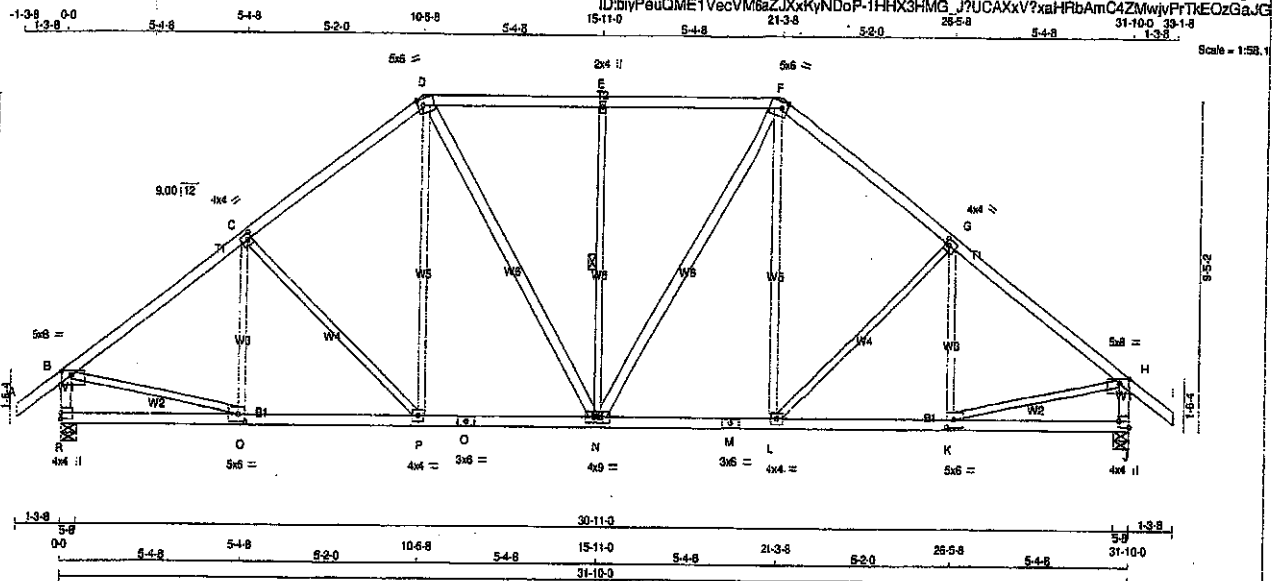
RECEIVED
 JUN 13 2019
 TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.

Structural component only
 DWG# T-1911542

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401604	T5	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS

EXCEPT	SIZE	DRY	LUMBER	DESCR.
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	8.0	Edge	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	8.0	Edge	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1-p	MT20	4.0	4.0	2.00	Edge
K	BMVW-t	MT20	5.0	8.0	2.50	2.50
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	8.0	2.50	2.50
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
R	2379	0	2379	0
J	2379	0	2379	0

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	COMBINED	1789	1004 / 0	334 / 0	0 / 0	0 / 0	430 / 0	0 / 0
J	COMBINED	1789	1004 / 0	334 / 0	0 / 0	0 / 0	430 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

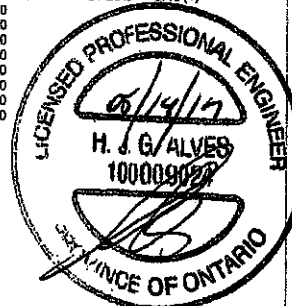
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED	MEMB.	MAX. FACTORED	FACTORED	MAX. FACTORED
	FORCE (LBS)	VERT. LOAD (LBS)	CS (LC)		FORCE (LBS)	VERT. LOAD (LBS)	CS (LC)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	10.00	C-C	-217 / 92	0.10 (1)
B-C	-2458 / 0	-102.1	-102.1 0.49 (1)	3.95	C-P	-368 / 0	0.40 (1)
C-D	-2212 / 0	-102.1	-102.1 0.45 (1)	4.16	P-D	0 / 471	0.11 (2)
D-E	-1997 / 0	-102.1	-102.1 0.43 (1)	4.33	D-N	0 / 508	0.08 (1)
E-F	-1997 / 0	-102.1	-102.1 0.43 (1)	4.33	N-E	-666 / 0	0.39 (1)
F-G	-2212 / 0	-102.1	-102.1 0.45 (1)	4.16	N-F	0 / 508	0.08 (1)
G-H	-2458 / 0	-102.1	-102.1 0.49 (1)	3.95	L-F	0 / 471	0.11 (2)
H-I	0 / 42	-102.1	-102.1 0.14 (1)	10.00	L-G	-368 / 0	0.40 (1)
R-B	-2293 / 0	0.0	0.0 0.24 (1)	5.83	K-G	-217 / 92	0.10 (1)
J-H	-2293 / 0	0.0	0.0 0.24 (1)	5.83	B-Q	0 / 2045	0.46 (1)
R-Q	0 / 0	-38.5	-38.5 0.20 (3)	10.00	K-H	0 / 2045	0.46 (1)
Q-P	0 / 1997	-38.5	-38.5 0.46 (2)	10.00			
P-O	0 / 1741	-38.5	-38.5 0.40 (2)	10.00			
O-N	0 / 1741	-38.5	-38.5 0.40 (2)	10.00			
N-M	0 / 1741	-38.5	-38.5 0.40 (2)	10.00			
M-L	0 / 1741	-38.5	-38.5 0.40 (2)	10.00			
L-K	0 / 1997	-38.5	-38.5 0.46 (2)	10.00			
K-J	0 / 0	-38.5	-38.5 0.20 (3)	10.00			



TOTAL WEIGHT = 8 X 158 = 949 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CS: TC=0.49/1.00 (B-C:1), BC=0.46/1.00 (P-Q:2), WB=0.48/1.00 (B-Q:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=-1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PS)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	518	354	1667
	788	1957	1856

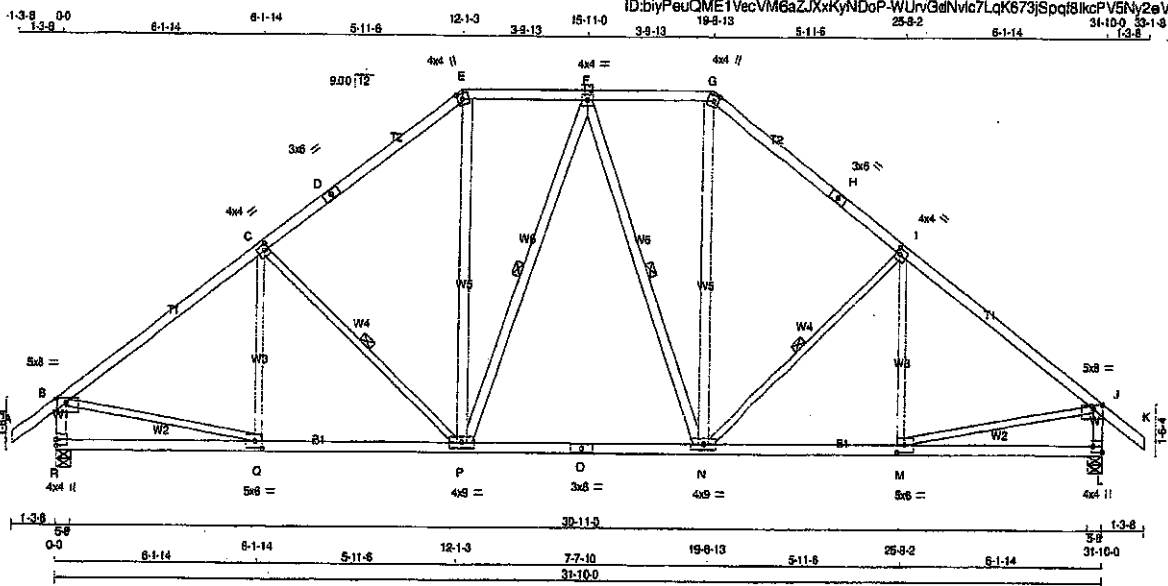
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.85 (H) (INPUT = 0.90)
SI METAL= 0.54 (O) (INPUT = 1.00)

Structural component only
DWG# T-1911543

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401604	T6	2	1	GREEN PARK HOMES	
TAMARACK ROOF TRUSS, BURLINGTON					



TOTAL WEIGHT = 2 X 165 = 330 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
I - O	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
P - E	2x4	DRY	No.2	SPF	
P - F	2x4	DRY	No.2	SPF	
F - N	2x4	DRY	No.2	SPF	
N - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+m	MT20	4.0	4.0	2.00	1.75
F	TTW-t	MT20	4.0	4.0		
G	TTW+m	MT20	4.0	4.0	2.00	1.75
H	TS-t	MT20	3.0	6.0		
I	TMVW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMVW-t	MT20	5.0	6.0	2.50	2.75
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	DOWN
R	2379	0	2379
L	2379	0	2379

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
R	1769	1004 / 0
L	1769	1004 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, H-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	FACTORED	MAX. FACTORED	VERT. LOAD	VERT. LOAD	MAX. FACTORED	MAX. FACTORED	MAX. FACTORED	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	VERT. LOAD	VERT. LOAD	VERT. LOAD	VERT. LOAD	VERT. LOAD	VERT. LOAD
FR-TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	0 / 42	-102.1	-102.1
B-C	-2471 / 0	-102.1	-102.1	0.86 (1)	3.73	0 / 42	-102.1	-102.1
C-D	-2098 / 0	-102.1	-102.1	0.59 (1)	4.06	0 / 42	-102.1	-102.1
D-E	-2098 / 0	-102.1	-102.1	0.59 (1)	4.06	0 / 42	-102.1	-102.1
E-F	-1652 / 0	-102.1	-102.1	0.21 (1)	4.97	0 / 42	-102.1	-102.1
F-G	-1652 / 0	-102.1	-102.1	0.21 (1)	4.97	0 / 42	-102.1	-102.1
G-H	-2098 / 0	-102.1	-102.1	0.59 (1)	4.06	0 / 42	-102.1	-102.1
H-I	-2098 / 0	-102.1	-102.1	0.59 (1)	4.06	0 / 42	-102.1	-102.1
I-J	-2471 / 0	-102.1	-102.1	0.86 (1)	3.73	0 / 42	-102.1	-102.1
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	0 / 42	-102.1	-102.1
K-L	-2280 / 0	0.0	0.0	0.24 (1)	5.65	0 / 42	-102.1	-102.1
L-M	-2280 / 0	0.0	0.0	0.24 (1)	5.65	0 / 42	-102.1	-102.1
M-N	0 / 0	-38.5	-38.5	0.25 (3)	10.00	0 / 42	-102.1	-102.1
N-O	0 / 2013	-38.5	-38.5	0.54 (2)	10.00	0 / 42	-102.1	-102.1
O-P	0 / 1734	-38.5	-38.5	0.50 (2)	10.00	0 / 42	-102.1	-102.1
P-Q	0 / 1734	-38.5	-38.5	0.50 (2)	10.00	0 / 42	-102.1	-102.1
Q-R	0 / 2013	-38.5	-38.5	0.54 (2)	10.00	0 / 42	-102.1	-102.1
R-S	0 / 0	-38.5	-38.5	0.25 (3)	10.00	0 / 42	-102.1	-102.1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0 PSF
DL = 8.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2016, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.86/1.00 (B-C:1), BC=0.54/1.00 (P-Q:2), WB=0.46/1.00 (B-Q:1), SS=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
			788
			1987
			1656

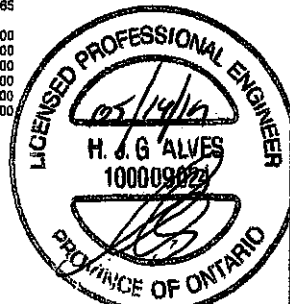
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.88 (M) (INPUT = 0.90)
ISI METAL= 0.80 (O) (INPUT = 1.00)

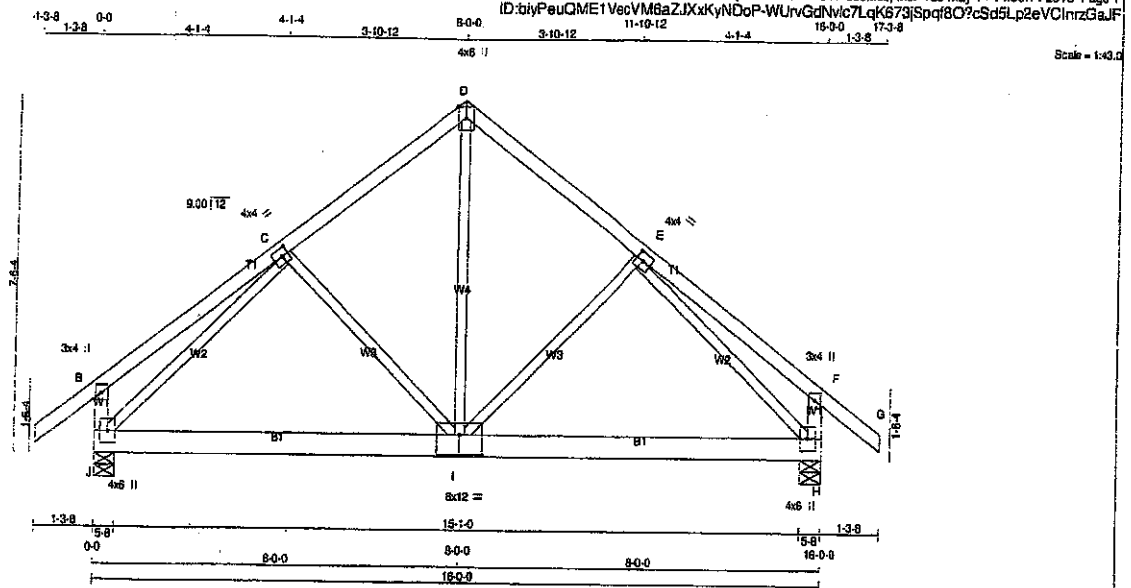
RECEIVED
JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.



Structural component only
DWG# T-1911544

JOB NAME 401604 TRUSS NAME T7 QUANTITY 2 PLY 1 JOB DESC. GREEN PARK HOMES TRUSS DESC. Tamarack Roof Truss, Burlington



LUMBER	N.L.G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2	
D - G	2x4	DRY	No.2	
J - B	2x4	DRY	No.2	
H - F	2x4	DRY	No.2	
J - I	2x6	DRY	No.2	
I - H	2x6	DRY	No.2	

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+1	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0		
I	BSWWW+1	MT20	8.0	12.0	5.00	6.00
J	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1268	0	1268	0	5-8
H	1268	0	1268	0	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
J	937	545/0 168/0 0/0 0/0 225/0 0/0
H	937	545/0 168/0 0/0 0/0 225/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0/42	-102.1 -102.1 0.14 (1)	I-D	0/633
B-C	0/29	-102.1 -102.1 0.28 (1)	I-E	-228/6
C-D	-819/0	-102.1 -102.1 0.21 (1)	C-I	-228/6
D-E	-819/0	-102.1 -102.1 0.21 (1)	J-C	-1137/0
E-F	0/29	-102.1 -102.1 0.28 (1)	E-H	-1137/0
F-G	0/42	-102.1 -102.1 0.14 (1)		
J-B	-296/0	0.0 0.0 0.03 (1)		
H-F	-296/0	0.0 0.0 0.03 (1)		
J-I	0/792	-38.5 -38.5 0.34 (2)		
I-H	0/792	-38.5 -38.5 0.34 (2)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
PART 9 OF BCBC 2018, OSC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(85 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TO=0.28/1.00 (E-F:1) , SC=0.34/1.00 (H-I:2) ,
WB=0.60/1.00 (E-H:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

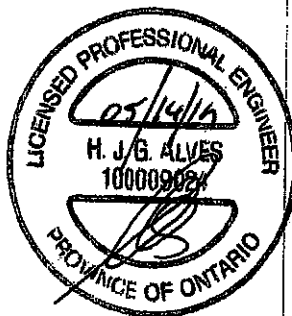
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.50)
JSI METAL= 0.41 (E) (INPUT = 1.00)

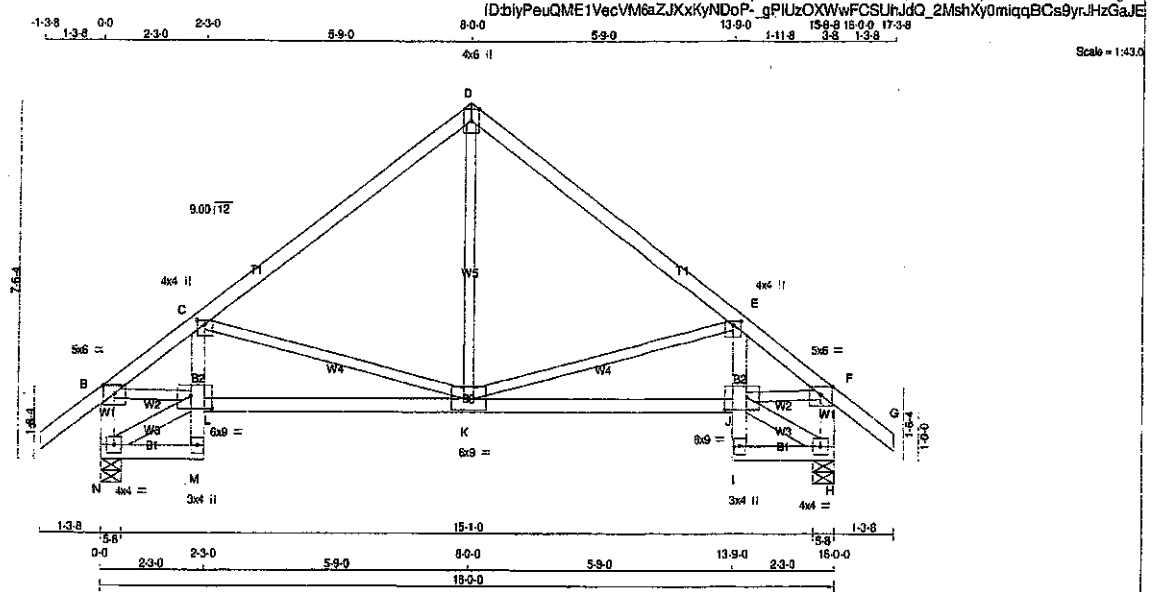


THIS TRUSS MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

Structural component only
DWG# T-1911545

JOB NAME: 401604 TRUSS NAME: T7S QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: DRWG NO. 401604

Tamareck Roof Truss, Burlington Version 8.300 S Apr 23 2019 MTek Industries, Inc. Tue May 14 14:00:15 2019 Page 1



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	1266	0	1266	0	5-8	5-8
H	1266	0	1266	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
N	937	545 / 0	168 / 0	0 / 0	0 / 0	225 / 0	0 / 0
H	937	545 / 0	168 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.78 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.44/1.00 (G-D:1), BC=0.48/1.00 (J-K:2), WB=0.48/1.00 (E-K:1), SS=0.22/1.00 (G-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-p	MT20	4.0	4.0	1.00	2.00
D	TTW-p	MT20	4.0	6.0	Edge	
E	TMVW-p	MT20	4.0	4.0	1.00	2.00
F	TMVW-p	MT20	5.0	6.0	Edge	
I	BMVW1-i	MT20	4.0	4.0		
J	BMVW-p	MT20	3.0	4.0		
K	BMVW1-i	MT20	6.0	9.0	3.25	5.50
L	BMVW1-i	MT20	6.0	9.0	3.25	5.50
M	BMVW-p	MT20	3.0	4.0		
N	BMVW1-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING

TOTAL LOAD CASES: (4)

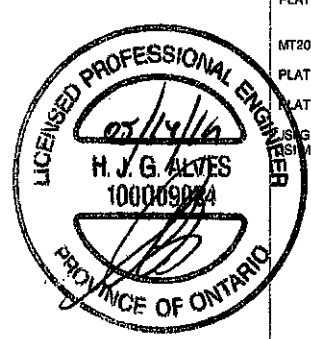
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX OSI (L)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	LC1 MAX OSI (L)	MAX. UNBRAC LENGTH
FR-TO		FROM	TO	FR-TO	FR-TO				
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	K-D	0 / 640	0.14 (2)	10.00
B-C	-1641 / 0	-102.1	-102.1	0.35 (1)	4.78	K-E	-688 / 0	0.46 (1)	10.00
C-D	-973 / 0	-102.1	-102.1	0.44 (1)	5.71	C-K	-688 / 0	0.46 (1)	10.00
D-E	-973 / 0	-102.1	-102.1	0.44 (1)	5.71	N-L	-49 / 0	0.01 (1)	10.00
E-F	-1641 / 0	-102.1	-102.1	0.35 (1)	4.78	B-L	0 / 1365	0.31 (1)	10.00
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00	J-H	-49 / 0	0.01 (1)	10.00
N-B	-1203 / 0	0.0	0.0	0.13 (1)	7.29	J-F	0 / 1365	0.31 (1)	10.00
H-F	-1203 / 0	0.0	0.0	0.13 (1)	7.29				
N-M	0 / 43	-38.5	-38.5	0.04 (2)	10.00				
M-L	0 / 52	0.0	0.0	0.07 (1)	10.00				
L-C	0 / 134	0.0	0.0	0.07 (1)	10.00				
L-K	0 / 1412	-38.5	-38.5	0.48 (2)	10.00				
K-J	0 / 1412	-38.5	-38.5	0.48 (2)	10.00				
I-J	0 / 52	0.0	0.0	0.07 (1)	10.00				
J-E	0 / 134	0.0	0.0	0.07 (1)	10.00				
I-H	0 / 43	-38.5	-38.5	0.04 (2)	10.00				

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JUN 13 2019

TOWN OF CALEDON BUILDING SECTION

FILE NO.



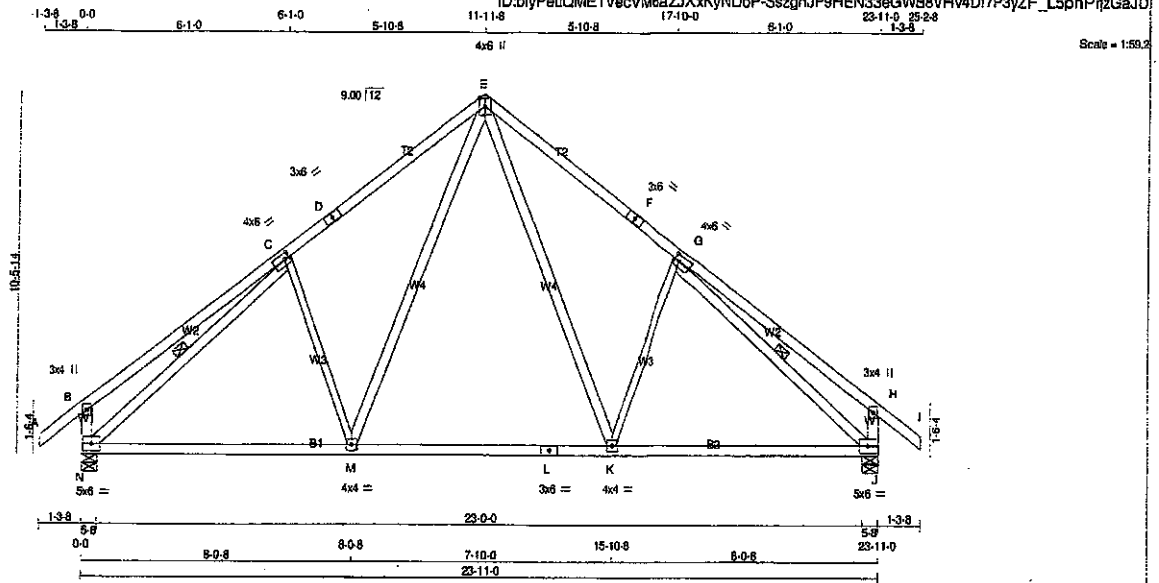
Structural component only
DWG# T-1911546

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
401604	T11	4	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:blyPauQME1VecVM6aZJXxKyNDOP-SszghJP9HEN33eGW58VHv4D7P3yZf_L5phP4zGaJD



TOTAL WEIGHT = 4 X 119 = 478 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS	2x4	DRY	No.2
EXCEPT			
K - G	2x3	DRY	No.2
C - M	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-i	MT20	4.0	6.0	2.00	1.75
D	TS-i	MT20	3.0	6.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TS-i	MT20	3.0	6.0		
G	TMVW-i	MT20	4.0	6.0	2.00	1.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW-i	MT20	5.0	6.0		
K	BMVW-i	MT20	4.0	4.0		
L	BS-i	MT20	3.0	6.0		
M	BMVW-i	MT20	4.0	4.0		
N	BMVW-i	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	1822	0	1822	0	5-8	5-8
J	1822	0	1822	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE	PERM	LIVE			
N	1353	774/0	251/0	0/0	0/0	327/0	0/0	0/0
J	1353	774/0	251/0	0/0	0/0	327/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	FORCE	FACTORED		MAX.	MAX.	MEMB.	FORCE	MAX.	
	(LBS)	VERT. LOAD	LCI	CSI (LC)	UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	E-K	0/758	0.12 (1)	
B-C	0/45	-102.1	-102.1	0.61 (1)	10.00	K-G	-397/9	0.26 (1)	
C-D	-1614/0	-102.1	-102.1	0.54 (1)	4.58	M-E	0/758	0.12 (1)	
D-E	-1614/0	-102.1	-102.1	0.54 (1)	4.58	C-M	-397/9	0.26 (1)	
E-F	-1614/0	-102.1	-102.1	0.54 (1)	4.58	N-C	-1928/0	0.62 (1)	
F-G	-1614/0	-102.1	-102.1	0.54 (1)	4.68	G-J	-1928/0	0.62 (1)	
G-H	0/45	-102.1	-102.1	0.61 (1)	10.00				
H-I	0/42	-102.1	-102.1	0.14 (1)	10.00				
N-B	-389/0	0.0	0.0	0.04 (1)	7.81				
J-H	-389/0	0.0	0.0	0.04 (1)	7.81				
N-M	0/1396	-38.5	-38.5	0.60 (2)	10.00				
M-L	0/983	-38.5	-38.5	0.55 (2)	10.00				
L-K	0/983	-38.5	-38.5	0.55 (2)	10.00				
K-J	0/1396	-38.5	-38.5	0.60 (2)	10.00				

A circular seal for a Professional Engineer. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "STATE OF TEXAS" at the bottom. The center of the seal features a large, stylized letter "E".

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.G.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.16")
ALLOWABLE DEFL.(TL) = $L/360$ (0.80")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.27")

CSI: TC=0.61/1.00 (G-H:1), BC=0.60/1.00 (J-K:2), WB=0.62/1.00 (G-J:1), SS=0.24/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

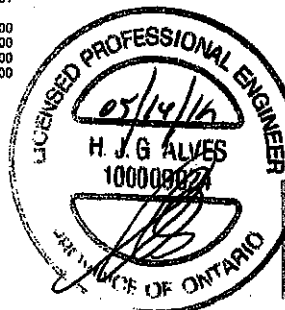
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

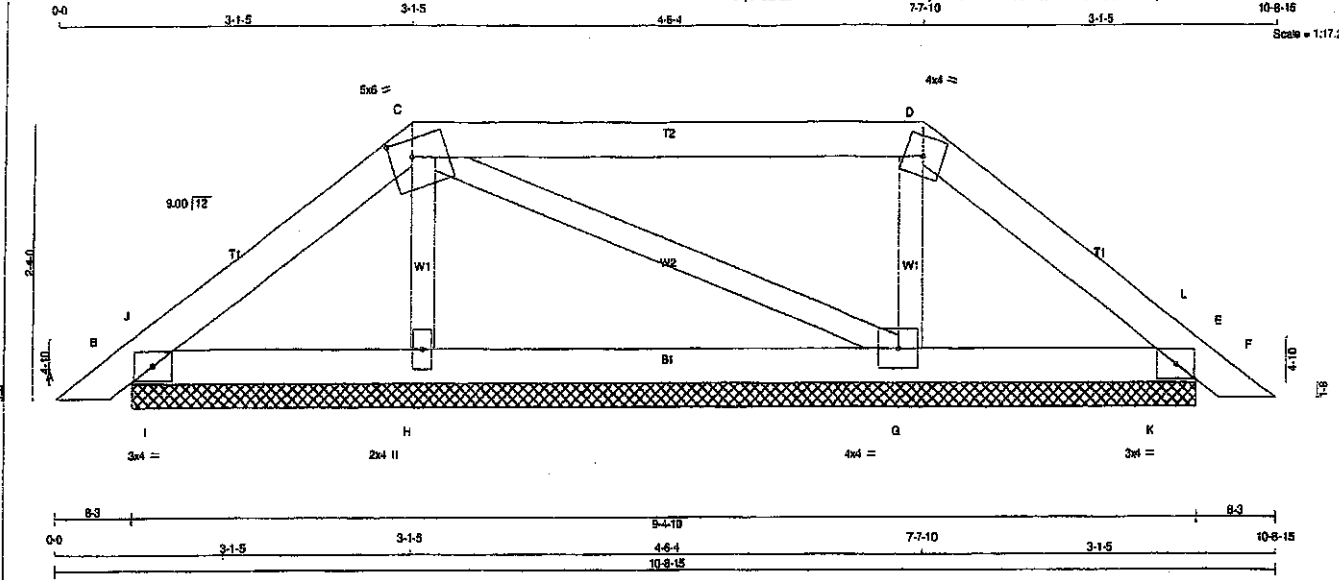
PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP = 0.86 (G) (INPUT = 0.90)
ISI METAL = 0.51 (C) (INPUT = 1.00)



TOWN OF CALEDON
BUILDING SECTION
Structural component only
DWG# T-1911547

JOB NAME 401604	TRUSS NAME P1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 14:00:06 2018 Page 1					
ID:biyPauQME1VocVM6aZJXKyNDOP-IxMubuHte97Uf5Vob1JxVzpQZNnODsVtoGGtVlzGaJN					



TOTAL WEIGHT = 2 X 31 = 62 lb

LUMBER				LUMBER			
N. L. G. A. RULES							
CHORDS	SIZE						
A - C	2x4	DRY	No.2				
C - D	2x4	DRY	No.2				
D - F	2x4	DRY	No.2				
B - E	2x4	DRY	No.2				
ALL WEBS 2x3 DRY				No.2			
DRY: SEASONED LUMBER.							

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW-m	MT20	5.0	6.0	1.75 2.25
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMWW1-I	MT20	4.0	4.0	
H	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG	
	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
B	277	0	277	0	0	9-4-10	9-4-10
E	282	0	282	0	0	9-4-10	9-4-10
H	435	0	435	0	0	9-4-10	9-4-10
G	467	0	467	0	0	9-4-10	9-4-10

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE				
B	198	141/0	16/0	0/0	0/0	40/0	0/0
E	186	132/0	16/0	0/0	0/0	38/0	0/0
H	332	161/0	82/0	0/0	0/0	88/0	0/0
G	354	180/0	82/0	0/0	0/0	92/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED	
		VERT. LOAD (PLF)	CS1 (LC)			FORCE (LBS)	CS1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/17	-102.1 -102.1	0.03 (1)	H-C	-262/0	0.04 (1)	
B-J	-74/0	-102.1 -102.1	0.02 (1)	C-G	-22/0	0.01 (1)	
J-C	-110/0	-102.1 -102.1	0.07 (1)	G-D	-285/0	0.04 (1)	
C-D	-49/0	-102.1 -102.1	0.36 (1)	I-J	-176/0	0.00 (1)	
D-L	-86/0	-102.1 -102.1	0.07 (1)	K-L	-179/0	0.00 (1)	
L-E	-48/0	-102.1 -102.1	0.02 (1)				
E-F	0/17	-102.1 -102.1	0.03 (1)				
B-I	0/84	-38.5 -38.5	0.08 (1)				
I-H	0/84	-38.5 -38.5	0.11 (2)				
H-G	0/69	-38.5 -38.5	0.10 (2)				
G-K	0/85	-38.5 -38.5	0.10 (2)				
K-E	0/85	-38.5 -38.5	0.08 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.36/1.00 (C-D:1), SC=0.11/1.00 (H-I:2), WB=0.04/1.00 (D-G:1), SS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788
		1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.90)

JSI METAL = 0.06 (H) (INPUT = 1.00)

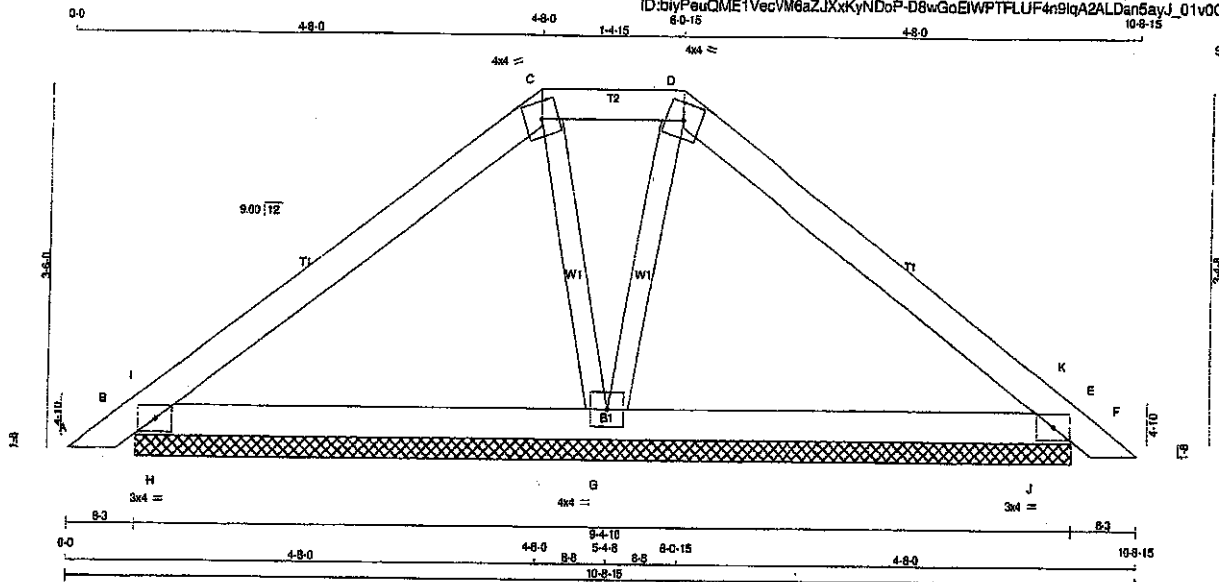
Structural component only
DWG# T-1911536



Structural component only
DWG# T-1911536

JOB NAME TRUSS NAME QUANTITY PLY JOB DESC. GREEN PARK HOMES TRUSS DESC. DRAWG NO.
 401604 P2 2 1
 Tamarack Roof Truss, Burlington

Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 14:00:07 2019 Page 1
 ID:biyPauQME1VecVM6aZJXxKyND0P-D8wGoEWPTLUF4n9lqA2ALDan5ayJ_01v0Q1zGaJM



Scale = 1:19.6

TOTAL WEIGHT = 2 X 30 = 61 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4 DRY	No.2
C - D	2x4 DRY	No.2
D - F	2x4 DRY	No.2
B - E	2x4 DRY	No.2
ALL WEBS	2x3 DRY	No.2

DRY: SEASONED LUMBER.

DESCR.

SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORIZ	DOWN	UP/LIFT		
B	437	0	437	0	9-4-10	9-4-10
E	437	0	437	0	9-4-10	9-4-10
G	567	0	567	0	9-4-10	9-4-10

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	320	198 / 0	48 / 0	0 / 0	0 / 0	73 / 0
E	320	198 / 0	48 / 0	0 / 0	0 / 0	73 / 0
G	430	218 / 0	100 / 0	0 / 0	0 / 0	112 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)
		VERT. LOAD (PLF)	CS1 (LC)					
FR-TO						FR-TO		
A-B	0 / 17	-102.1	-102.1 0.03 (1)	10.00		C-G	-143 / 0	0.03 (1)
B-I	-138 / 0	-102.1	-102.1 0.06 (1)	6.25		G-D	-143 / 0	0.03 (1)
I-C	-190 / 0	-102.1	-102.1 0.21 (1)	6.25		H-I	-380 / 87	0.00 (1)
C-D	-113 / 0	-102.1	-102.1 0.03 (1)	6.25		J-K	-380 / 87	0.00 (1)
D-K	-190 / 0	-102.1	-102.1 0.21 (1)	6.25				
K-E	-138 / 0	-102.1	-102.1 0.06 (1)	6.25				
E-F	0 / 17	-102.1	-102.1 0.03 (1)	10.00				
B-H	0 / 145	-38.5	-38.5 0.17 (1)	10.00				
H-G	0 / 145	-38.5	-38.5 0.24 (2)	10.00				
G-J	0 / 145	-38.5	-38.5 0.24 (2)	10.00				
J-E	0 / 145	-38.5	-38.5 0.17 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2015, CBC 2012
 - CSA 086-03, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.21/1.00 (G-H-1), BC=0.24/1.00 (G-H-2), WB=0.03/1.00 (D-G-1), SS=0.31/1.00 (B-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1907 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
 JSI METAL= 0.09 (B) (INPUT = 1.00)



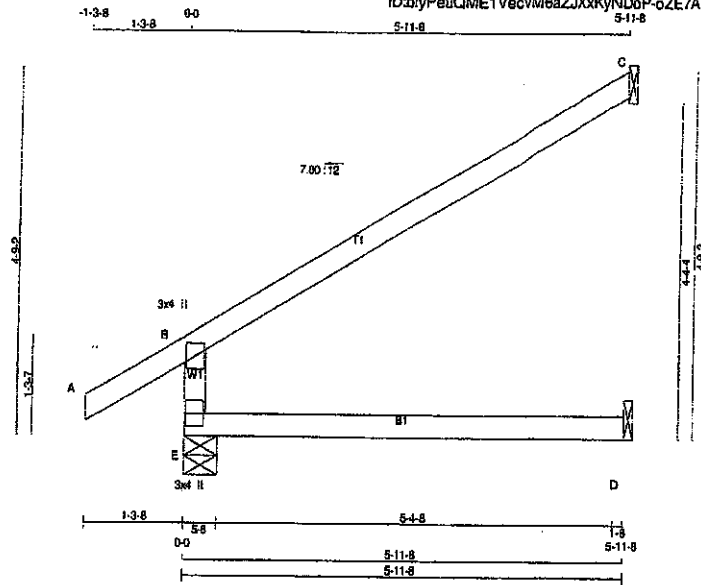
RECEIVED
 JUN 13 2019

TOWN OF CALEDON
 BUILDING SECTION
 Structural component only
 DWG# 1-1911537

JOB NAME 401604	TRUSS NAME J1	QUANTITY 18	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 14:03:04 2019 Page 1
IDblyPcuQME1VecvM8aZJXxKyNDOP-oZE7ACFd6YtmdoLCUcHTQVjca5blzgaLynnQQzGaJP

Scale = 1/25.4



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG UPLIFT IN-SX	REQD BRG IN-SX
E	580	0	0	5-8
C	228	0	0	1-8
D	43	0	0	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	405	285 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
C	157	130 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
E-B	-520 / 0	0.0	0.0	0.12 (4)	7.81		
A-B	0 / 35	-102.1	-102.1	0.14 (1)	10.00		
B-C	-38 / 0	-102.1	-102.1	0.62 (1)	6.25		
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00		

TOTAL WEIGHT = 18 X 18 = 315 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD				= 42.0 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF'L (LL) = 1/360 (0.20")
CALCULATED VERT. DEF'L (LL) = 1/999 (0.00")
ALLOWABLE DEF'L (TL) = 1/360 (0.20")
CALCULATED VERT. DEF'L (TL) = 1/999 (0.03")

CSI: TC=0.82/1.00 (B-C:1), BC=0.13/1.00 (D-E:4),
WB=0.00/1.00 (W:A:0), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

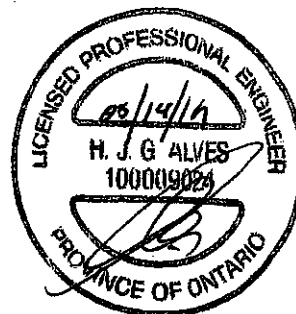
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1667	1668

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.22 (E) (INPUT = 0.90)
JSI METAL = 0.16 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911534



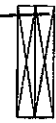
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200595
Project #:

Page 5 of 6

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

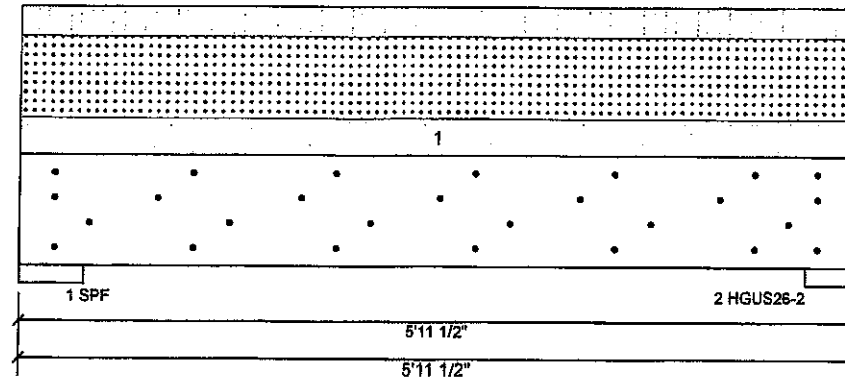
Level: Level

RECEIVED
JUN 13 2019TOWN OF CALEDON
BUILDING SECTION
FILE NO

9 1/4"



3"

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	414	512	1143	0
2	397	491	1096	0

Bearings and Factored Reactions

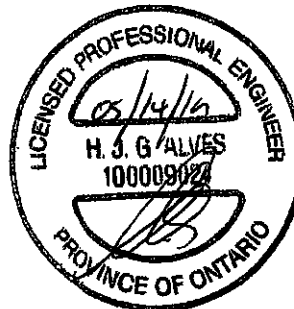
Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	27%	640 / 2128	2769	L	1.25D+1.5S +L
2 - HGUS...	4.000"	36%	614 / 2041	2655	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3186 ft-lb	3' 1/2"	6039 ft-lb	0.528 (53%)	1.25D+1.5S +L	L
Unbraced	3186 ft-lb	3' 1/2"	5210 ft-lb	0.612 (61%)	1.25D+1.5S +L	L
Shear	2409 lb	1'2"	3984 lb	0.605 (60%)	1.25D+1.5S +L	L
LL Defl inch	0.029 (L/2210)	3' 1/2"	0.176 (L/360)	0.160 (16%)	S+0.5L	L
TL Defl inch	0.040 (L/1602)	3' 1/2"	0.176 (L/360)	0.220 (22%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

DWG NO. TAM 71911561
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 5-11-8	(Span) 25-11-0	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

This design is valid until 12/11/2021

Manufacturer InfoTamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



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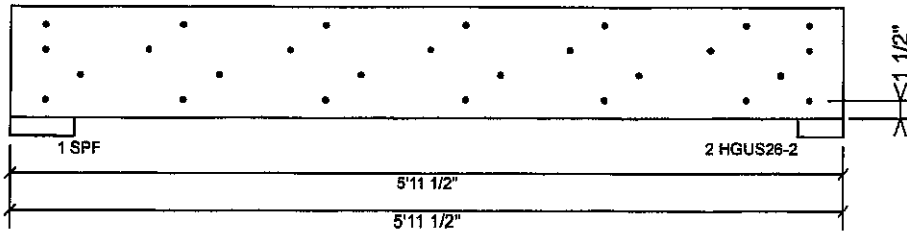
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200595
Project #:

Page 6 of 6

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

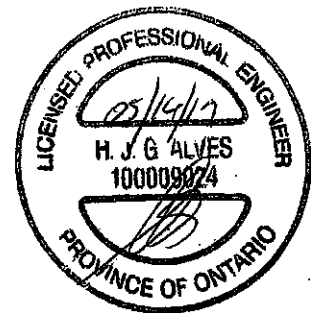
Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	89.0 %
Load	455.2 PLF
Yield Limit per Foot	511.3 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 1911561
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/11/2021

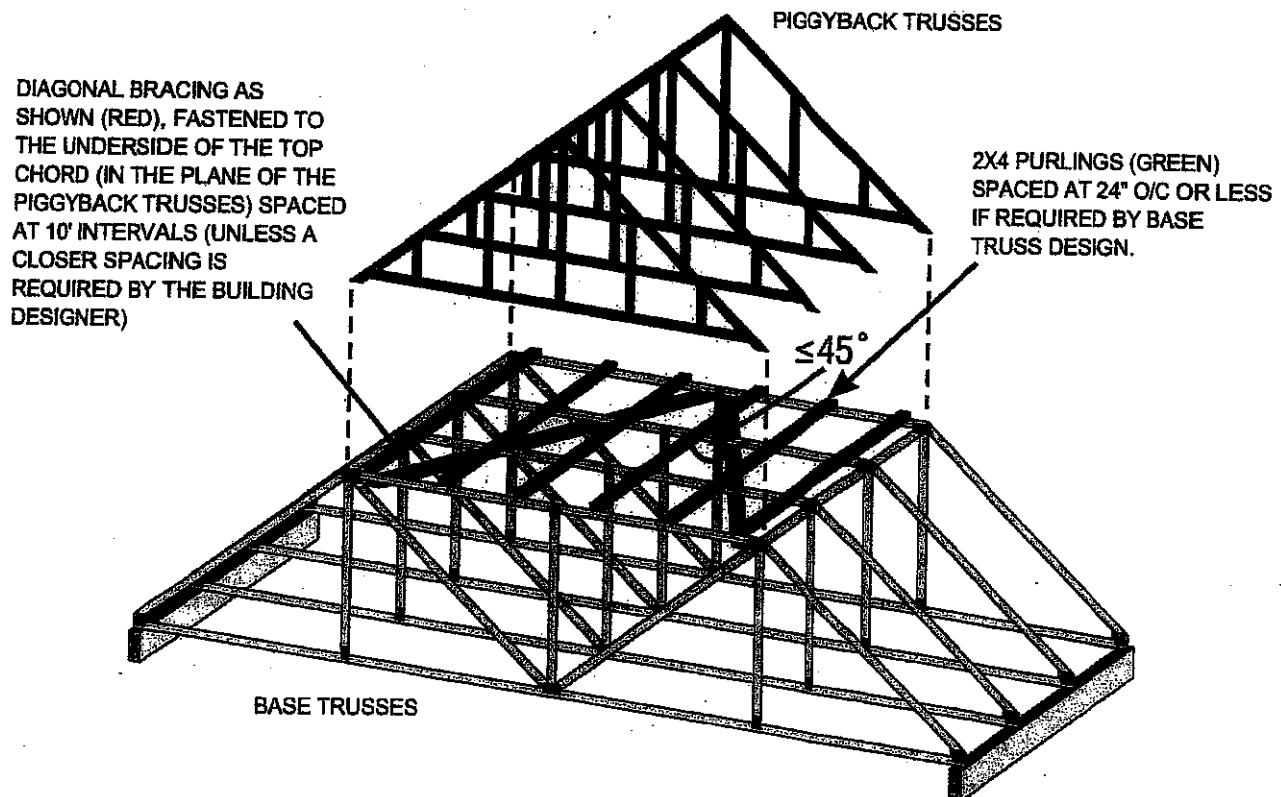


Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

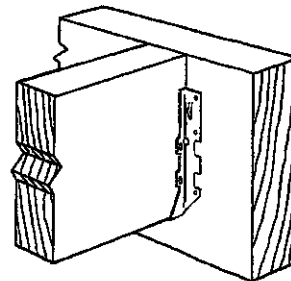
Finish: G90 galvanized

Design:

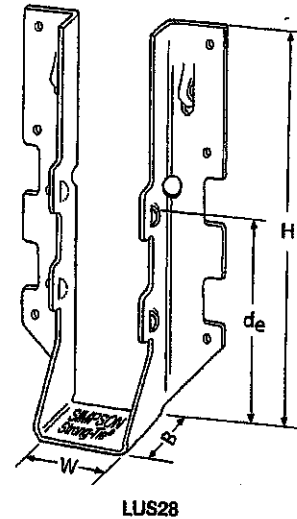
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



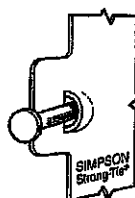
Typical LUS Installation



LUS28

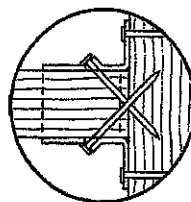
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
LUS24	18	1 1/8	3 1/8	1 1/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/8	1 1/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 1/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 13/16	1 1/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

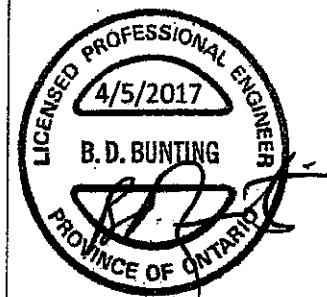


Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



DESIGN

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HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

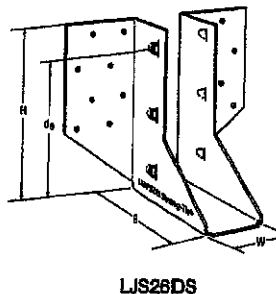
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

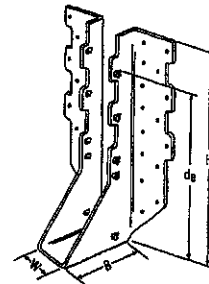
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

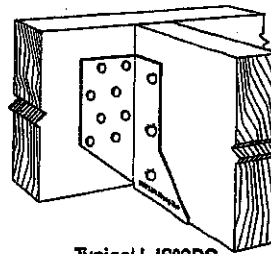
- See current catalogue for options



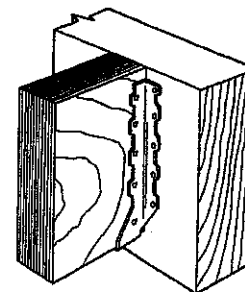
LJS26DS



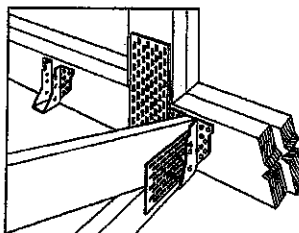
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



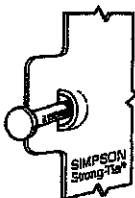
Typical HUS
Installation



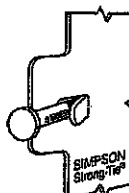
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.	Uplift (K _o =1.15) lb.	Normal (K _o =1.00) lb.
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	18	1 1/8	5 1/2	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

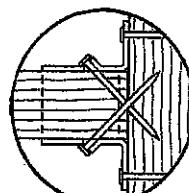
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,803,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



SIMPSON
Strong-Tie

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HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

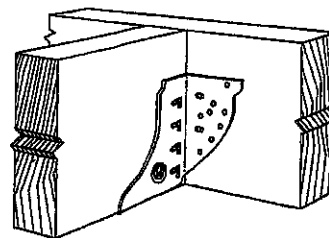
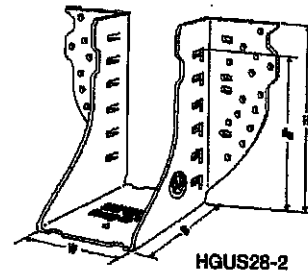
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

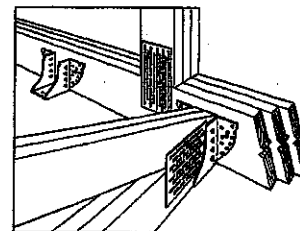
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



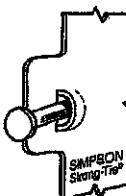
Typical HGUS
Installation



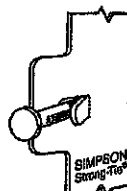
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3¼	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¼	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6¼	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	8½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3¼	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¼	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6¼	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3¼	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4¼	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6¼	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6¼	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6¼	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

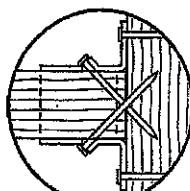
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.

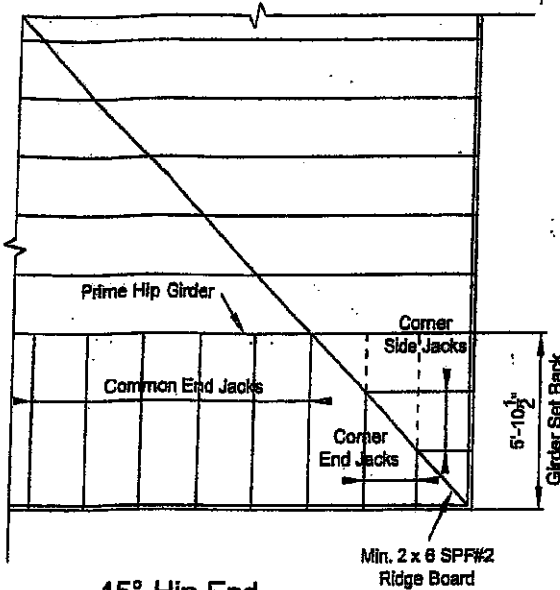


DESIGN

2017/11/15 10:00 AM

2017/11/15 10:00 AM

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45° Hip End

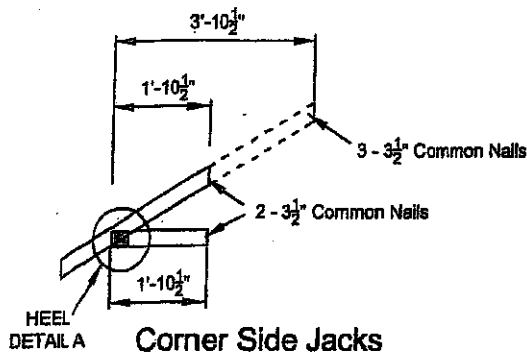
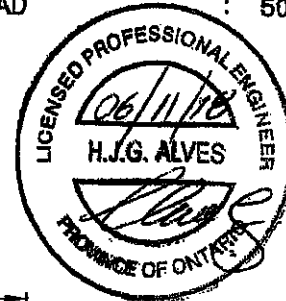
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

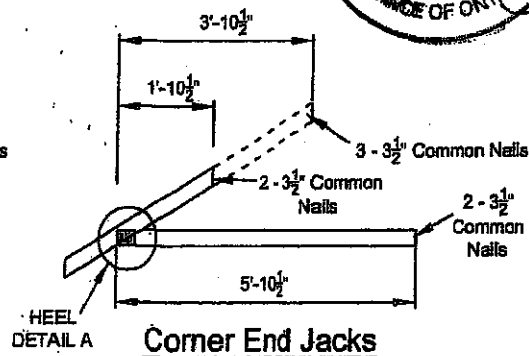
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

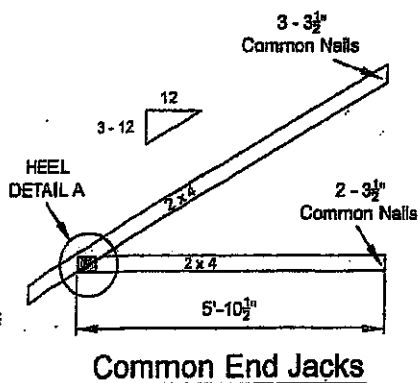
TOTAL LOAD : 50.5 P.S.F.



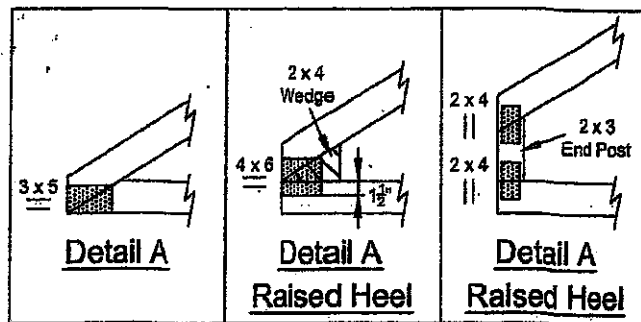
Corner Side Jacks



Corner End Jacks

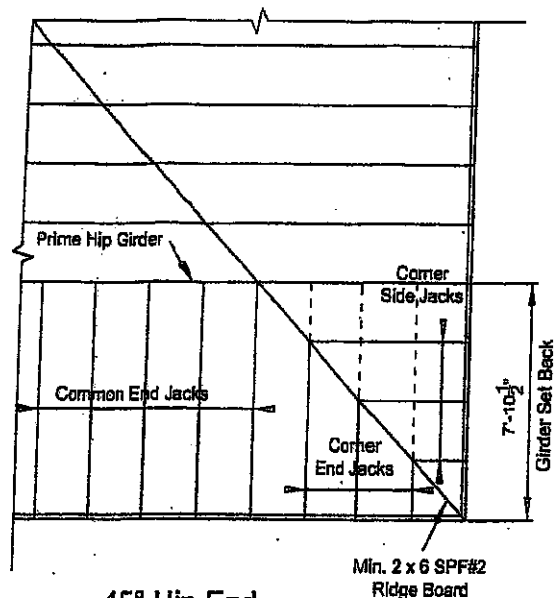


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

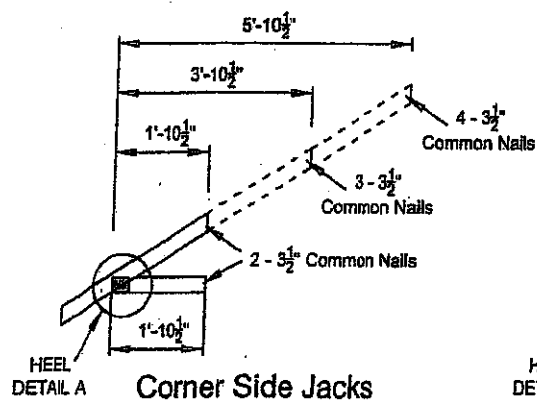
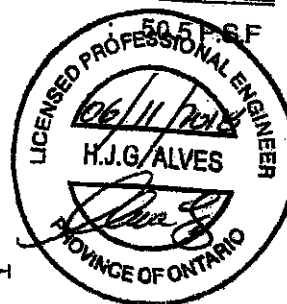
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

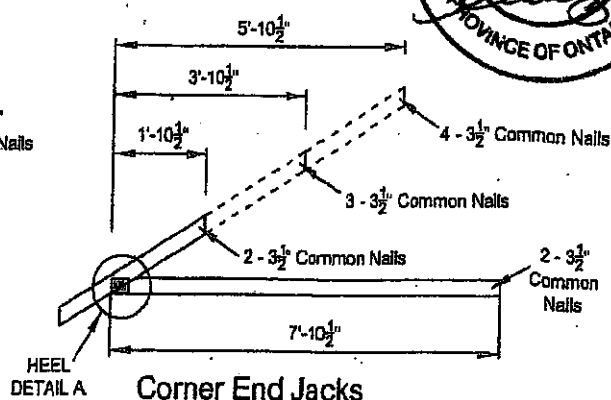
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

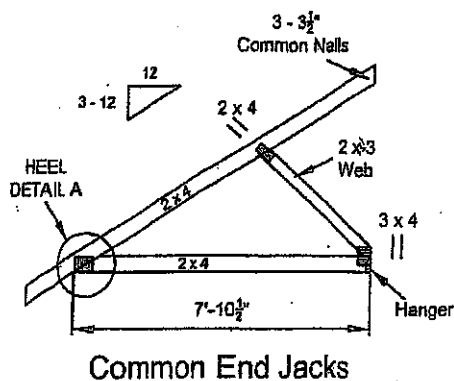
TOTAL LOAD



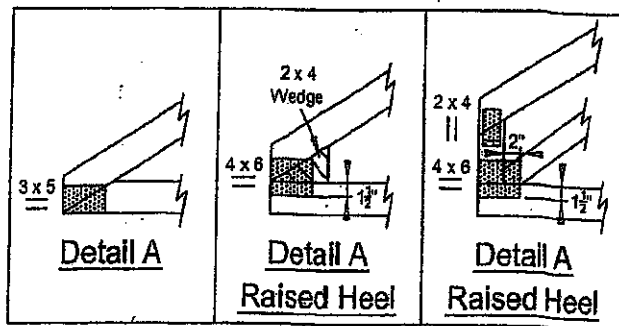
Corner Side Jacks



Corner End Jacks



Common End Jacks

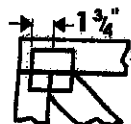


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

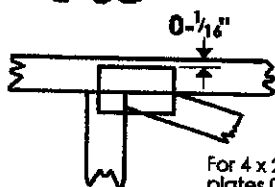
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

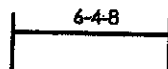


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

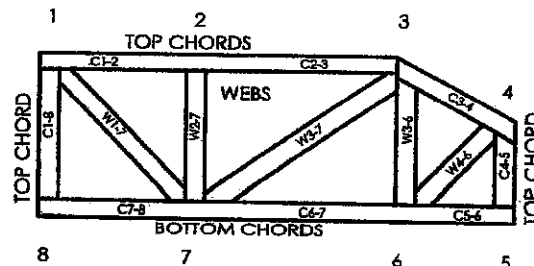
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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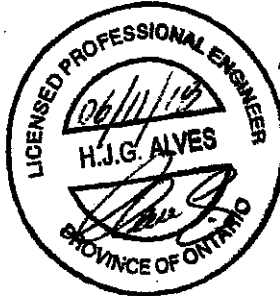
MiTek Engineering Reference Sheet: MH-7473C rev. 10-'08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

T-1300213

Feb 09, 2018