

10/12 ROOF PITCH
UNLESS OTHERWISE
SPECIFIED

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE
50-00-00

12 " FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GS LOAD 1.8 KPA
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

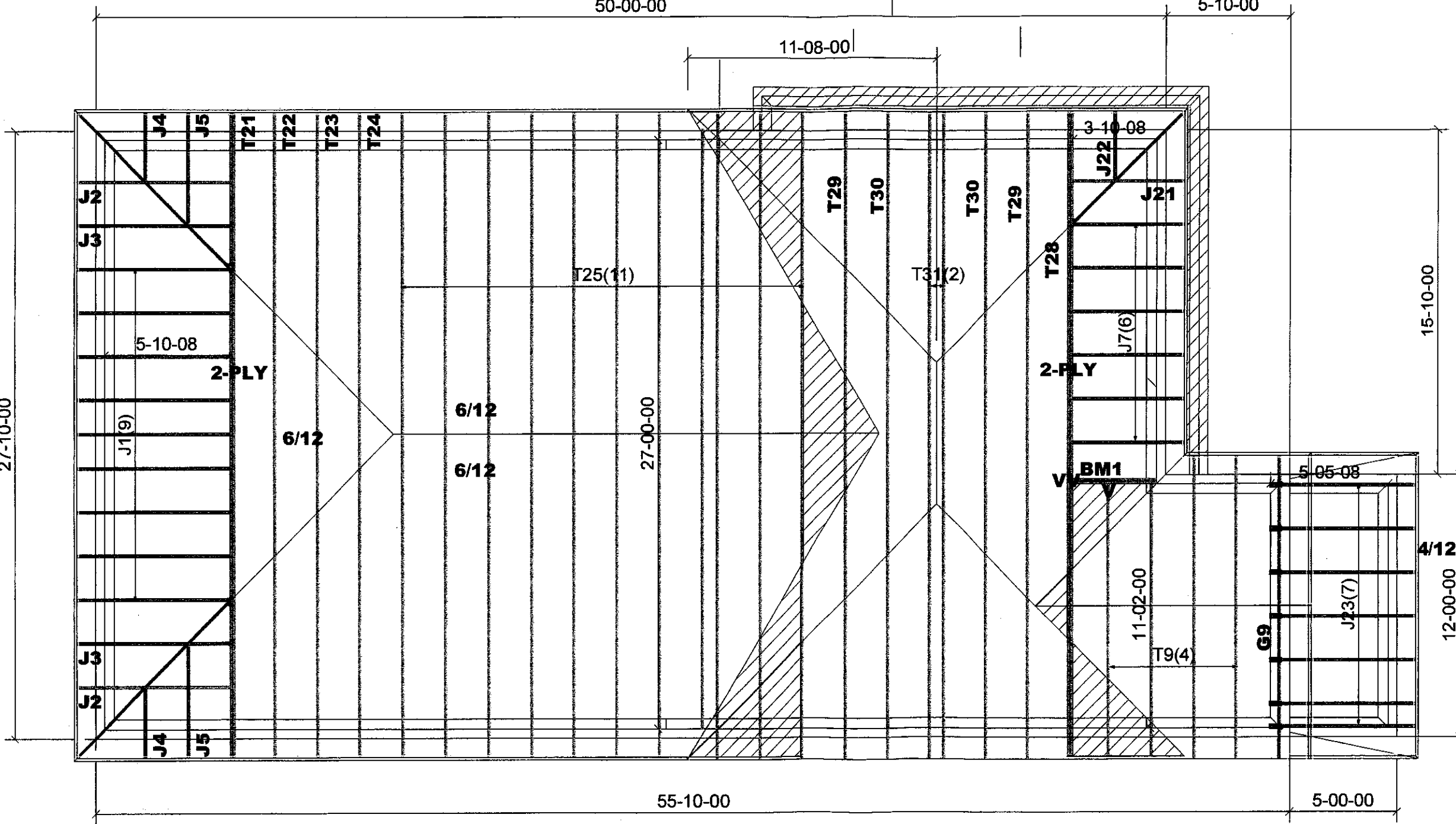
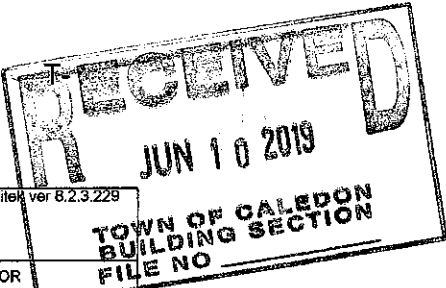
ALL CONVENTIONAL ROOF
FRAMING TO CONFORM TO
PART 9 OF THE OBC. LATEST
EDITION ROOF RAFTERS
THAT MEET OR CROSS
OVER TRUSSES ARE TO BE
2"X4" SPF @24"o.c. WITH A
2"X4"SPF VERTICAL POST
TO THE TRUSS UNDER AT
EACH CROSS POINT. POSTS
LONGER THAN 6' TO BE
LATERALLY BRACED SO
THAT THE DISTANCE
BETWEEN END POINTS AND
BETWEEN ROWS OF
BRACING DOES NOT
EXCEED 6'.

TRUSSES DESIGNED
CONFORM TO THE
RELEVANT SECTION OF THE
LATEST EDITION OF THE
ONTARIO BUILDING CODE
(RESIDENTIAL PART 9)

HARDWARE
HGUS26-2(XX)
LUS26-2(VV)
LJS26DS(V)

BM1:2-2X10

DENOTES
CONVENTIONAL
FRAMING



M12236



Job Track: 50033
Plan Log: 201018
Layout ID: 402974

Builder / Location:
GREEN PARK HOMES / CALEDON
Project: **LAMBERT LANE PH.2**
Date: 5/6/2019 Sales: **Mario DiCano** Designer: JG

Model / Elevation:
BARTON 3/2

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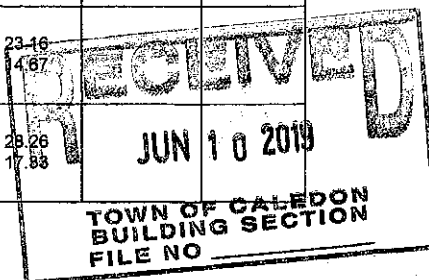
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 3
 Lot #:
 Elevation: 2

Job Track: 50033
 PlanLog: 201018
 Layout ID: 402974
 Ref #
 Page: 1 of 2
 Date: 05/06/2019
 Designer:
 Sales Rep: Mario DiCano

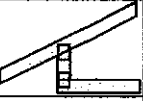
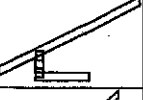
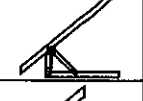
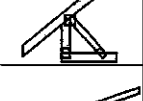
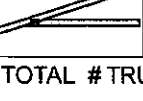
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	T9 Common	10 /12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	198.87 132.00		
	1	G9 GABLE	10 /12	11-02-00	6-03-08	2 x 4	1-05-00 1-03-08	1-07-11 1-07-11	54.45 37.00		
	1 2-ply	T21 Hip Girder	6 /12	27-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	252.71 158.00		
	1	T22 Hip	6 /12	27-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	109.31 68.83		
	1	T23 Hip	6 /12	27-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	110.98 70.67		
	1	T24 Hip	6 /12	27-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	112.6 70.67		
	11	T25 Common	6 /12	27-00-00	7-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1199.12 740.67		
	1 2-ply	T28 Hip Girder	10 /12	27-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	266.28 170.33		
	2	T29 Hip	10 /12	27-00-00	6-06-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	250.96 158.67		
	2	T30 Hip	10 /12	27-00-00	8-02-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	260.71 165.33		
	2	T31 Hip	10 /12	27-00-00	9-10-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	269.72 171.33		
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	2	J2 Jack-Open Girder	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.46 4.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	23.26 17.53		



		DELIVERY SHIPLIST	
	Lumber Yard:	TAMARACK LUMBER	Job Track: 50033
	Builder:	GREEN PARK HOMES	PlanLog: 201018
	Project:	LAMBERT LANE PH.2	Layout ID: 402974
	Location:	CALEDON	Ref #
	Model:	BARTON 3	Page: 2 of 2
	Lot #:		Date: 05/06/2019
Elevation:	2	Designer:	
			Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	6	J7 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	91.36 58.00		
	1	J21 Jack-Open Girder	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J22 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.74 7.00		
	7	J23 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	101.14 65.33		

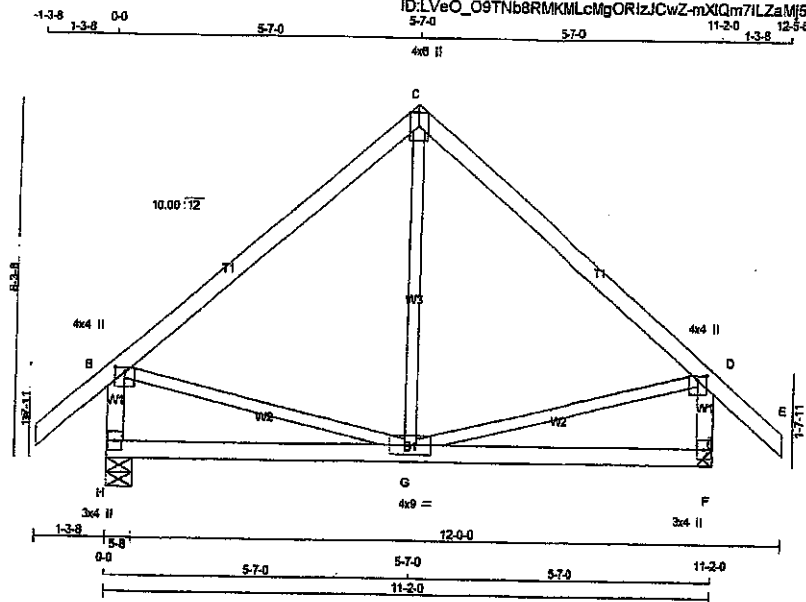
TOTAL # TRUSS= 61 TOTAL BFT OF ALL TRUSSES= 2231.49 BFT. TOTAL WEIGHT OF ALL TRSSES 3535.73 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LJS26DS	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
402973	T9	2	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						



Scale = 1:37.5

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	4.0	4.0	1.00 2.00
C	TTW+p	MT20	4.0	8.0	Edge
D	TMW+p	MT20	4.0	4.0	1.00 2.00
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW+t	MT20	4.0	9.0	
H	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	927	0	927	0	5-8
F	927	0	927	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, MINIMUM BEARING LENGTH AT JOINT F = 3-6.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
H	884	405 / 0	117 / 0	0 / 0	182 / 0	0 / 0
F	884	405 / 0	117 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 45	G-C	0 / 248
B-C	-510 / 0	B-G	0 / 403
C-D	-510 / 0	G-D	0 / 403
D-E	0 / 45		
H-B	-845 / 0		
F-D	-845 / 0		
H-G	0 / 0		
G-F	0 / 0		

TOTAL WEIGHT = 2 X 50 = 99 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, CBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSF TC=0.41/1.00 (B-C:1), BC=0.27/1.00 (G-H:3),
WB=0.09/1.00 (D-G:1), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

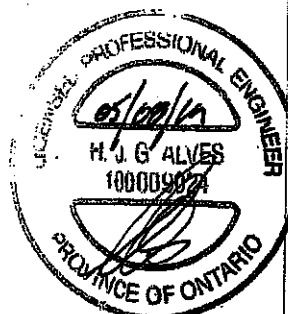
PLATE GRIP(DRY) SHEAR SECTION (PSI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 768 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)

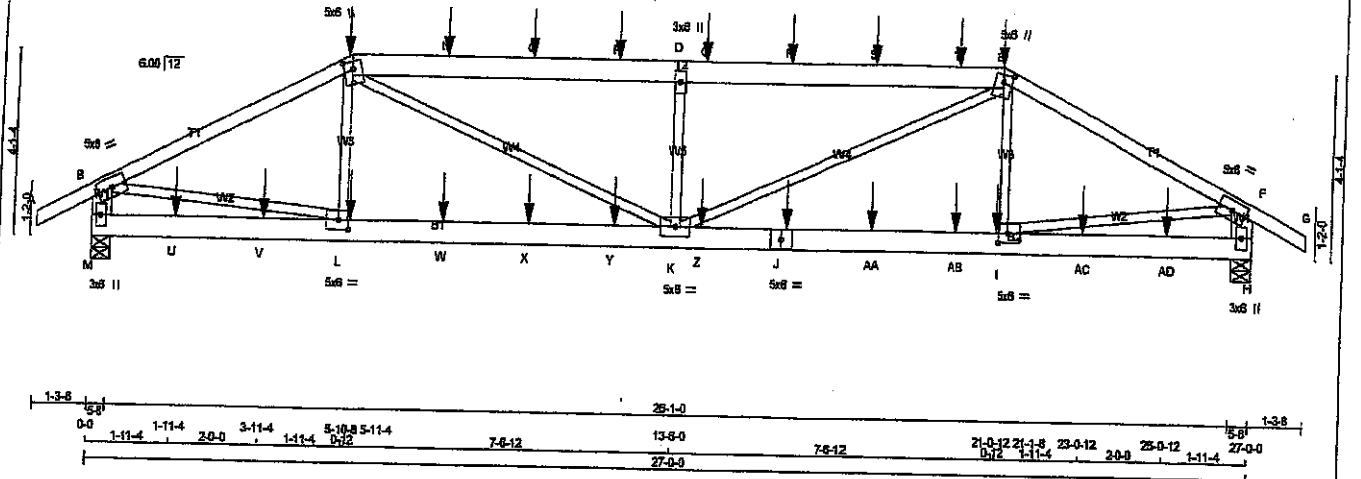
THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



DRWG NO. TAM 1741/0501
STRUCTURAL
COMPONENT ONLY

JOB NAME 402939	TRUSS NAME T21	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:48:13 2019 Page 1
 ID: yH3luQzwKBaXQAh7hsf8URzKCT-qvVnrdSag6Xyif_FsleBCy48h4xqRDaeHyczlqdw
 1-3-8 0-0 5-10-8 5-10-8 7-7-8 13-8-0 7-7-8 21-1-8 5-10-8 27-0-0 1-3-8
 Scale = 1/4" = 1'-0"



CHORDS	SIZE	LU	MBR	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF

ALL WEBS
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'x3") SPIRAL NAILS		
A-C 1	12	SIDE (61.0)
E-G 1	12	SIDE (61.0)
C-E 2	12	SIDE (61.0)
M-B 2	12	TOP
H-F 2	12	TOP
BOTTOM CHORDS: (0.122'x3") SPIRAL NAILS		
M-J 2	12	SIDE (183.1)
J-H 2	12	SIDE (183.1)
WEBS: (0.122'x3") SPIRAL NAILS		
L-C 1	8	SIDE (1.4)
I-E 1	8	SIDE (1.4)
2x3 1	8	SIDE (1.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

ORDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMW/H	MT20	5.0	8.0	
C	TTW/H	MT20	5.0	8.0	2.00 2.50
D	TMW/H	MT20	5.0	8.0	
E	TTW/H	MT20	5.0	8.0	2.00 2.50
F	TMW/H	MT20	5.0	8.0	
H	BMW/H	MT20	5.0	8.0	2.50 2.75
I	BS-1	MT20	5.0	8.0	
J	BS-1	MT20	5.0	8.0	2.50 2.75
K	BMW/H	MT20	5.0	8.0	
L	BMW/H	MT20	5.0	8.0	2.50 2.75
M	BMW/H	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
JT	VERT	3270	3270	0	5-8	5-8
M	H	3325	3325	0	5-8	5-8

UNFACTORED REACTIONS

JT	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
M	2443	1351 / 0	487 / 0	0 / 0	805 / 0	0 / 0
H	2485	1373 / 0	497 / 0	0 / 0	816 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CS (LC)	UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
			FROM	TO					
FR-TO									
A-B	0 / 31	-102.1	-102.1	0.07 (1)	10.00	L-C	-90 / 347	0.04 (3)	
B-C	-4802 / 0	-102.1	-102.1	0.54 (1)	3.92	C-K	0 / 2416	0.30 (1)	
C-N	-6483 / 0	-102.1	-102.1	0.48 (1)	4.20	K-D	-1559 / 0	0.19 (1)	
N-O	-6483 / 0	-102.1	-102.1	0.48 (1)	4.20	D-E	0 / 2317	0.29 (1)	
O-P	-6483 / 0	-102.1	-102.1	0.48 (1)	4.20	E-I	-71 / 381	0.05 (3)	
P-Q	-6483 / 0	-102.1	-102.1	0.48 (1)	4.20	I-F	0 / 4343	0.54 (1)	
Q-R	-6483 / 0	-102.1	-102.1	0.48 (1)	4.20	F-G	0 / 4343	0.55 (1)	
R-S	-6483 / 0	-102.1	-102.1	0.48 (1)	4.20				
S-T	-6483 / 0	-102.1	-102.1	0.48 (1)	4.20				
T-E	-6483 / 0	-102.1	-102.1	0.48 (1)	4.20				
E-F	-4902 / 0	-102.1	-102.1	0.55 (1)	3.88				
F-G	0 / 31	-102.1	-102.1	0.07 (1)	10.00				
M-B	-3144 / 0	0.0	0.0	0.11 (1)	7.78				
H-F	-3201 / 0	0.0	0.0	0.11 (1)	7.74				
M-U	0 / 0	-38.5	-38.5	0.13 (3)	10.00				
U-V	0 / 0	-38.5	-38.5	0.13 (3)	10.00				
V-L	0 / 0	-38.5	-38.5	0.13 (3)	10.00				
L-W	0 / 4307	-38.5	-38.5	0.38 (1)	10.00				
W-X	0 / 4307	-38.5	-38.5	0.38 (1)	10.00				
X-Y	0 / 4307	-38.5	-38.5	0.38 (1)	10.00				
Y-K	0 / 4307	-38.5	-38.5	0.38 (1)	10.00				
K-Z	0 / 4398	-38.5	-38.5	0.39 (1)	10.00				
Z-J	0 / 4398	-38.5	-38.5	0.39 (1)	10.00				
J-AA	0 / 4398	-38.5	-38.5	0.39 (1)	10.00				
AA-AB	0 / 4398	-38.5	-38.5	0.39 (1)	10.00				
AB-I	0 / 4398	-38.5	-38.5	0.39 (1)	10.00				
I-AC	0 / 0	-38.5	-38.5	0.13 (3)	10.00				
AC-AD	0 / 0	-38.5	-38.5	0.13 (3)	10.00				
AD-H	0 / 0	-38.5	-38.5	0.13 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-473	-473	FRONT	VERT	TOTAL		
E	21-1-8	-473	-473	FRONT	VERT	TOTAL		
I	21-0-12	-55	-70	FRONT	VERT	TOTAL		
J	16-1-4	-55	-70	FRONT	VERT	TOTAL		
L	5-11-4	-55	-70	FRONT	VERT	TOTAL		
N	8-1-4	-123	-123	FRONT	VERT	TOTAL		

TOTAL WEIGHT = 2 X 128 = 256 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. CC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
CALCULATED VERT. DEFL. (LL) = L/989 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (0.90")
CALCULATED VERT. DEFL. (TL) = L/989 (0.22")

CSL TC=0.55/1.00 (F-F), BC=0.38/1.00 (H-K), WB=0.55/1.00 (F-F), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE FEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

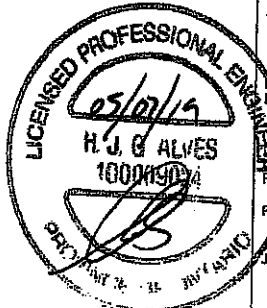
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
618 354 1667 798 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP = 0.88 (L) (INPUT = 0.90)
SI METAL = 0.55 (L) (INPUT = 1.00)

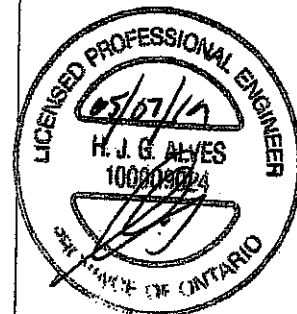


DATE: 05/07/19
BY: H. J. G. ALVES
1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402939	T21	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue May 7 17:48:13 2019 Page 2
ID:yH3luQzwK(BaXQAh?hsf8URzKCTp-gVVnxdSeg6Xyif FslaBCy48h4xqRDaElHyzdudVv

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	10-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
P	12-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
Q	14-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
R	16-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
S	18-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
T	20-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
U	1-11-4	-55	-74	-	FRONT	VERT	TOTAL	-	-
V	3-11-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
W	5-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
X	10-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
Y	12-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
Z	14-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
AA	18-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
AB	20-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
AC	23-0-12	-55	-70	-	FRONT	VERT	TOTAL	-	-
AD	25-0-12	-55	-74	-	FRONT	VERT	TOTAL	-	-

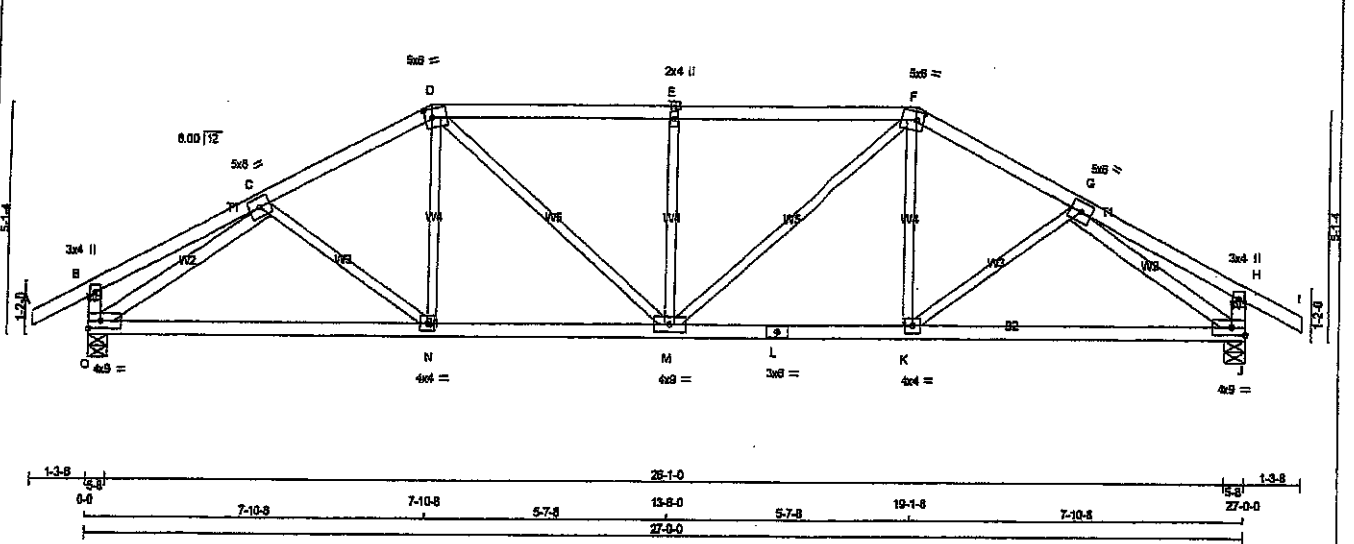


ENGINEER TAM 1910779
STRUCTURAL
CERTIFICATION ONLY

7/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402939	T22	1	1	GREEN PARK HOMES	
Trussack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:48:14 2019 Page 1
ID: yH3bQzwKBxQAh7hsf8URzKCtp-l3A8Be4LzEOasHbXyN_7OK7GYlgDEMouVqU2ldqv
1-3-8 0-0 4-0-8 4-0-8 3-10-0 7-10-8 5-7-8 13-8-0 19-1-8 3-10-0 22-11-8 4-0-8 27-0-0 1-3-8
Scale = 1/4" = 1'-0"



TOTAL WEIGHT = 109 lb (MIP)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - C	2x4	DRY	No.2
G - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-t	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	9.0		Edge
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVWW-t	MT20	4.0	9.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
O	2037	0	2037	0	0
J	2037	0	2037	0	0

UNFACTORED REACTIONS							
	1ST LOSE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0
J	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 31	-102.1 -102.1	0.13 (1)	10.00	C-N	-3 / 138	0.03 (3)
B-C	0 / 19	-102.1 -102.1	0.23 (1)	10.00	N-D	0 / 375	0.08 (2)
C-D	-2533 / 0	-102.1 -102.1	0.25 (1)	4.13	D-M	0 / 847	0.15 (1)
D-E	-2748 / 0	-102.1 -102.1	0.32 (1)	3.68	M-E	-702 / 0	0.27 (1)
E-F	-2748 / 0	-102.1 -102.1	0.32 (1)	3.68	M-F	0 / 847	0.15 (1)
F-G	-2533 / 0	-102.1 -102.1	0.25 (1)	4.13	K-F	0 / 375	0.08 (2)
G-H	0 / 19	-102.1 -102.1	0.23 (1)	10.00	K-G	-3 / 138	0.03 (3)
H-I	0 / 31	-102.1 -102.1	0.13 (1)	10.00	O-C	-2768 / 0	0.77 (1)
O-B	-299 / 0	0.0 0.0	0.03 (1)	7.81	G-J	-2768 / 0	0.77 (1)
J-H	-299 / 0	0.0 0.0	0.03 (1)	7.81			
O-N	0 / 2250	-38.5 -38.5	0.70 (2)	10.00			
N-M	0 / 2254	-38.5 -38.5	0.71 (2)	10.00			
M-L	0 / 2254	-38.5 -38.5	0.71 (2)	10.00			
L-K	0 / 2254	-38.5 -38.5	0.71 (2)	10.00			
K-J	0 / 2250	-38.5 -38.5	0.70 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF
SPACING = 24.0 IN/GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBC 2012
- CSA 088-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
CALCULATED VERT. DEFL. (LL) = L/899 (0.16")
ALLOWABLE DEFL. (TL) = L/360 (0.90")
CALCULATED VERT. DEFL. (TL) = L/899 (0.30")

CS1: TC=0.52/1.00 (D-E-1), BC=0.71/1.00 (K-M-2), WB=0.77/1.00 (G-H-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

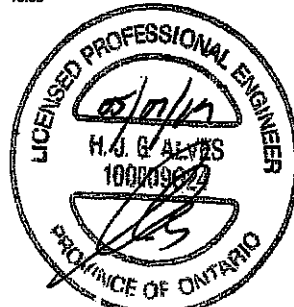
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
			788
			1997
			1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.73 (L) (INPUT = 1.00)



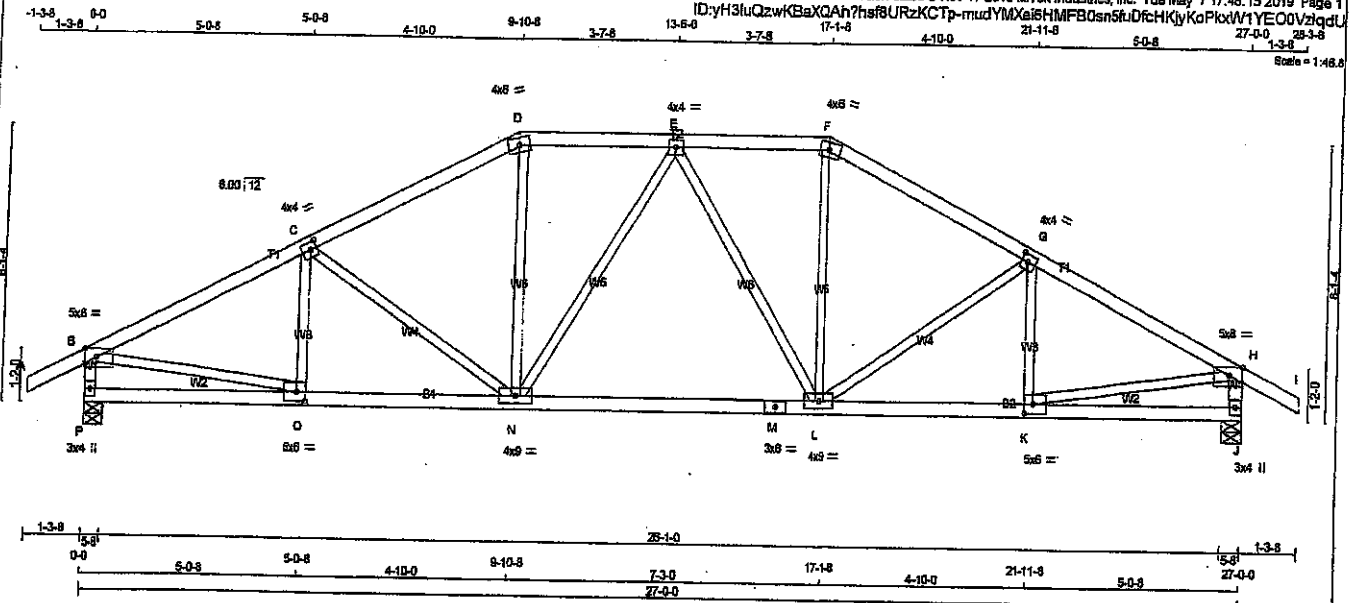
DESIGN BY TAM 176/0780
STRUCTURAL
FOR LAYOUT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402939	T23	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: yH3luQzwKBaXQAh?hsf8URzKCTp-mudYMXaishMF80an5fuDfchKlyKaPktW1YE00VzldU



TOTAL WEIGHT = 111 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - P	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge	3.50
C	TMWV-W	MT20	4.0	4.0	2.00	1.75
D	TTWV-m	MT20	4.0	6.0		
E	TMWV-W	MT20	4.0	4.0		
F	TTWV-m	MT20	4.0	8.0		
G	TMWV-W	MT20	4.0	4.0	2.00	1.75
H	TMWV-p	MT20	5.0	8.0	Edge	3.50
J	BMV1-p	MT20	3.0	4.0		
K	BMWV-W	MT20	5.0	8.0	2.50	2.25
L	BMWV-W	MT20	4.0	9.0		
M	BS-1	MT20	3.0	6.0		
N	BMWV-W	MT20	4.0	9.0		
O	BMWV-W	MT20	5.0	8.0	2.50	2.25
P	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
P	2037	0	2037	0
J	2037	0	2037	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
P	1514	863 / 0 284 / 0 0 / 0 0 / 0 367 / 0 0 / 0
J	1514	863 / 0 284 / 0 0 / 0 0 / 0 367 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 31	-102.1	-102.1 0.13 (1)	10.00	O-C	-221 / 84	0.05 (1)
B-C	-2693 / 0	-102.1	-102.1 0.41 (1)	3.68	C-N	-410 / 0	0.26 (1)
C-D	-2337 / 0	-102.1	-102.1 0.38 (1)	4.14	N-D	0 / 898	0.16 (1)
D-E	-2078 / 0	-102.1	-102.1 0.20 (1)	4.58	E-L	-258 / 0	0.22 (1)
E-F	-2078 / 0	-102.1	-102.1 0.20 (1)	4.58	L-F	0 / 898	0.16 (1)
F-G	-2337 / 0	-102.1	-102.1 0.38 (1)	4.14	F-G	-410 / 0	0.26 (1)
G-H	-2693 / 0	-102.1	-102.1 0.41 (1)	3.68	H-I	-221 / 84	0.05 (1)
H-I	0 / 31	-102.1	-102.1 0.13 (1)	10.00	I-K	0 / 2438	0.55 (1)
I-J	-1951 / 0	0.0	0.0 0.20 (1)	6.03	K-H	0 / 2438	0.55 (1)
J-H	-1951 / 0	0.0	0.0 0.20 (1)	6.03			
P-O	0 / 0	-38.5	-38.5 0.16 (3)	10.00			
O-N	0 / 2404	-38.5	-38.5 0.58 (1)	10.00			
N-M	0 / 2211	-38.5	-38.5 0.53 (2)	10.00			
M-L	0 / 2211	-38.5	-38.5 0.53 (2)	10.00			
L-K	0 / 2404	-38.5	-38.5 0.58 (1)	10.00			
K-J	0 / 0	-38.5	-38.5 0.16 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 28.1 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 10.5 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (0.90")
CALCULATED VERT. DEF. (LL) = L/999 (0.13")
ALLOWABLE DEF. (TL) = L/360 (0.90")
CALCULATED VERT. DEF. (TL) = L/999 (0.23")

CS1 TO=0.41/1.00 (G-H-I), BC=0.56/1.00 (K-L-I), WB=0.55/1.00 (H-K-I), SS=0.22/1.00 (G-H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

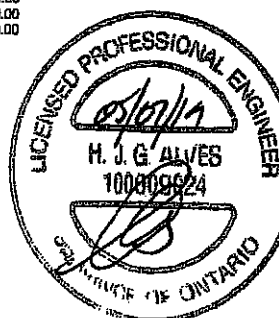
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

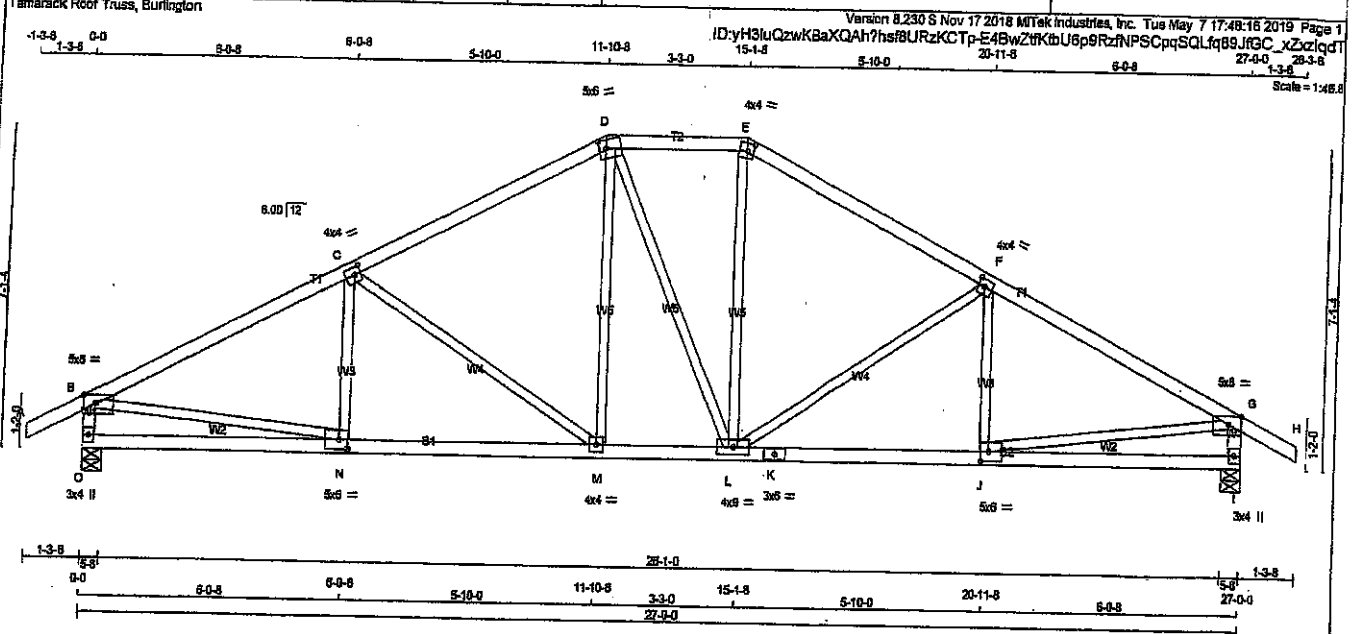
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 1.00)



DR: JES. TAM 17910781
NATURAL
ONLY

JOB NAME 402939	TRUSS NAME T24	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue May 7 17:48:16 2019 Page 1			



TOTAL WEIGHT = 113 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
JT 2037 0	2037 0	2037 0	2037 0	5-8	5-8	5-8	5-8
O 2037 0	2037 0	2037 0	2037 0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX/MIN COMPONENT REACTIONS						
	1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT		1514	883 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0
O		1514	883 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	CS1 (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX (LBS)	CS1 (LC)
FR-TO		FROM	TO			FR-TO			
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	N-C	-84 / 185	0.04 (3)	
B-C	-2694 / 0	-102.1	-102.1	0.81 (1)	3.63	C-M	-682 / 0	0.87 (1)	
C-D	-2111 / 0	-102.1	-102.1	0.53 (1)	4.11	M-D	0 / 537	0.12 (1)	
D-E	-1888 / 0	-102.1	-102.1	0.18 (1)	4.76	D-L	0 / 4	0.00 (1)	
E-F	-2113 / 0	-102.1	-102.1	0.33 (1)	4.11	L-E	0 / 541	0.12 (1)	
F-G	-2693 / 0	-102.1	-102.1	0.81 (1)	3.63	L-F	-889 / 0	0.86 (1)	
G-H	0 / 31	-102.1	-102.1	0.13 (1)	10.00	J-F	-85 / 183	0.04 (3)	
H-I	-1942 / 0	0.0	0.0	0.20 (1)	8.04	B-N	0 / 2482	0.55 (1)	
I-G	-1942 / 0	0.0	0.0	0.20 (1)	8.04	J-G	0 / 2481	0.55 (1)	
O-N	0 / 0	-38.5	-38.5	0.27 (3)	10.00				
N-M	0 / 2438	-38.5	-38.5	0.58 (2)	10.00				
M-L	0 / 1866	-38.5	-38.5	0.40 (1)	10.00				
L-K	0 / 2437	-38.5	-38.5	0.57 (2)	10.00				
K-J	0 / 2437	-38.5	-38.5	0.57 (2)	10.00				
J-I	0 / 0	-38.5	-38.5	0.27 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 086-06, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CS1: TC=0.81/1.00 (B-C-1), BC=0.58/1.00 (M-N-2),
WB=0.87/1.00 (C-M-1), SS=0.28/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 L.S. BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

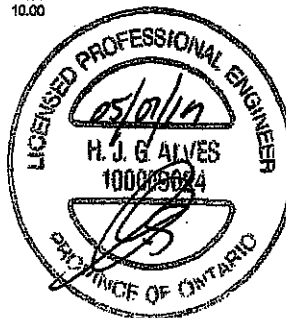
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1957 1656

PLATE PLACEMENT TOL = 0.250 inches

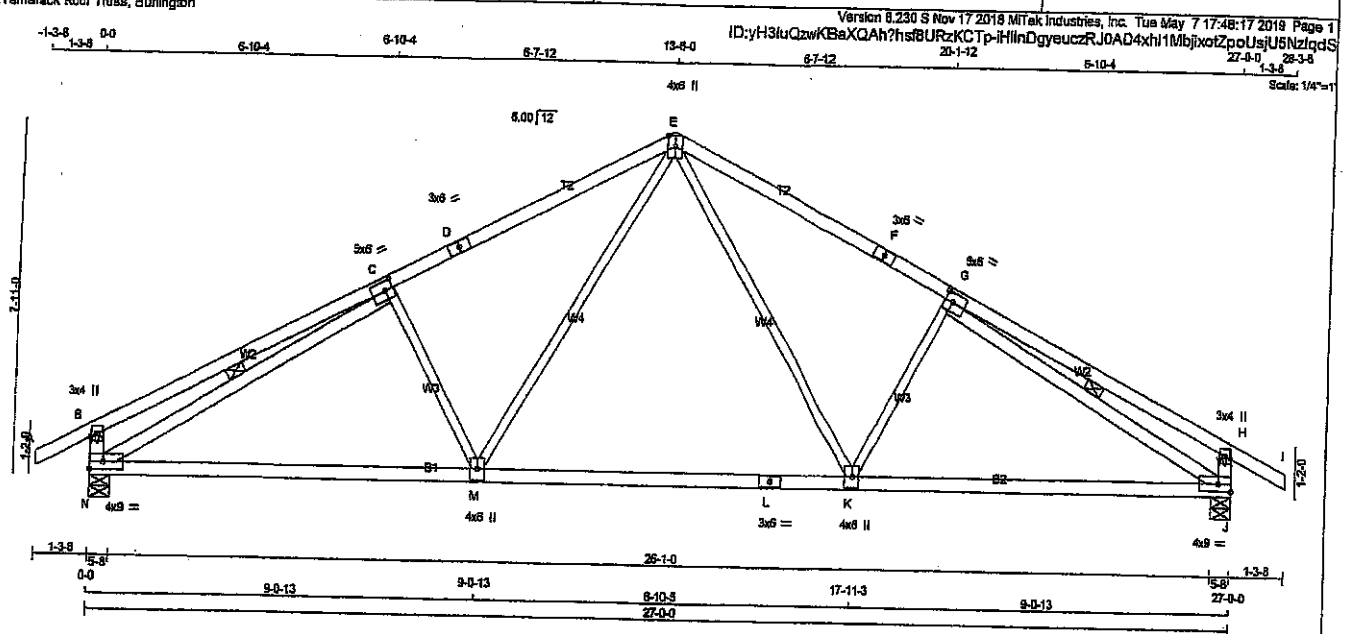
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (E) (INPUT = 0.89)
JSI METAL = 0.71 (Q) (INPUT = 1.00)



UNSAT. I AM PA/0782
ACTUAL
C. 10/10/2014

JOB NAME 402939	TRUSS NAME T25	QUANTITY 8	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			



CHORDS	SIZE	DRY	NO.2
A - D	2x4	DRY	NO.2
D - B	2x4	DRY	NO.2
E - F	2x4	DRY	NO.2
F - I	2x4	DRY	NO.2
N - B	2x4	DRY	NO.2
N - H	2x4	DRY	NO.2
N - L	2x4	DRY	NO.2
L - J	2x4	DRY	NO.2

ALL WEBS	SIZE	DRY	NO.2
EXCEPT			
N - C	2x4	DRY	NO.2
G - J	2x4	DRY	NO.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV+t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	6.0		
E	TTWV+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWV+t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	9.0	Edge	
K	BMWV+t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWV+t	MT20	4.0	6.0		
N	BMVW1-t	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	DOWN	HORIZ	UP/LIFT	IN-SX	IN-SX	IN-SX
N	2037	0	2037	0	0	5-8	5-8
J	2037	0	2037	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS				SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	
N	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0
J	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 31	-102.1	-102.1 0.13 (1)	10.00	E-K	0 / 937	0.21 (1)
B-C	0 / 37	-102.1	-102.1 0.78 (1)	10.00	K-G	-493 / 7	0.19 (1)
C-D	-2435 / 0	-102.1	-102.1 0.78 (1)	3.54	M-E	0 / 937	0.21 (1)
D-E	-2435 / 0	-102.1	-102.1 0.78 (1)	3.54	C-M	-493 / 7	0.19 (1)
E-F	-2435 / 0	-102.1	-102.1 0.78 (1)	3.54	N-C	-2813 / 0	0.84 (1)
F-G	-2435 / 0	-102.1	-102.1 0.78 (1)	3.54	G-J	-2813 / 0	0.84 (1)
G-H	0 / 37	-102.1	-102.1 0.13 (1)	10.00			
H-I	0 / 31	-102.1	-102.1 0.13 (1)	10.00			
N-B	-400 / 0	0.0	0.0 0.04 (1)	7.81			
J-H	-400 / 0	0.0	0.0 0.04 (1)	7.81			
N-M	0 / 2383	-38.5	-38.5 0.85 (2)	10.00			
M-L	0 / 1674	-38.5	-38.5 0.75 (2)	10.00			
L-K	0 / 1674	-38.5	-38.5 0.75 (2)	10.00			
K-J	0 / 2383	-38.5	-38.5 0.85 (2)	10.00			

TOTAL WEIGHT = 8 X 109 = 872 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.80")
CALCULATED VERT. DEFL.(LL) = 1/998 (0.27")
ALLOWABLE DEFL.(TL) = 1/800 (0.90")
CALCULATED VERT. DEFL.(TL) = 1/705 (0.46")

CSI: TC=0.78/1.00 (G-H-1), EC=0.85/1.00 (J-K-2),
WB=0.84/1.00 (G-J-1), SB=0.30/1.00 (E-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

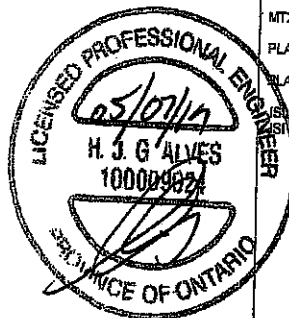
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1687 788	1987 1688

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(S) GRIP= 0.88 (G) (INPUT = 0.90)
(S) METAL= 0.69 (G) (INPUT = 1.00)

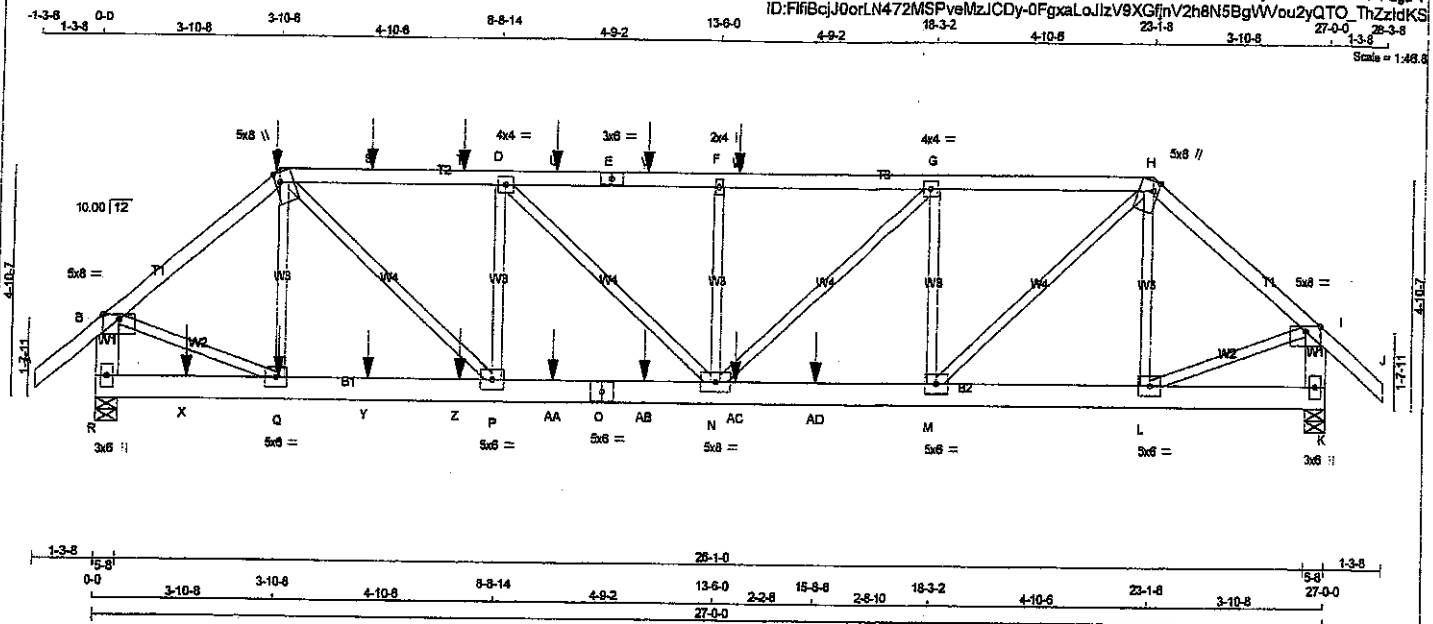


DR: [Signature] 11/10/2013
C: [Signature]
C: [Signature]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	T28	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Wed May 8 09:56:01 2019 Page 1
ID: FfBcJj0orLN472MSPveMzjCDy-0FgxaLolzv9XGfnV2h8N5BgVWou2yQTO_ThZldKs

Scale = 1:48.6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
R - O	2x6	DRY No.2	SPF
O - K	2x6	DRY No.2	SPF

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1 12		SIDE(81.0)
C-E 1 12		SIDE(81.0)
E-H 1 12		SIDE(0.0)
H-J 1 12		TOP
R-B 2 12		TOP
K-I 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-O 2 12		SIDE(183.1)
O-K 2 12		SIDE(0.0)

WEBS : (0.122"x3") SPIRAL NAILS
2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV+p	MT20	5.0	8.0	Edge	
C TTWV+m	MT20	5.0	8.0	Edge 1.25	
D TMWV-i	MT20	4.0	4.0		
E TS-i	MT20	3.0	8.0		
F TMWV+w	MT20	2.0	4.0		
G TMWV-i	MT20	4.0	4.0		
H TTWV+m	MT20	5.0	8.0	Edge 1.25	
I TMWV-p	MT20	5.0	8.0	Edge	
K BMV1+p	MT20	3.0	6.0		
L M, P, Q					
L BMWV-i	MT20	5.0	6.0		
N BMWVW-i	MT20	5.0	8.0		
O BS-i	MT20	5.0	8.0		
R BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	3087	3057	0	5-8
R HORZ	2763	2763	0	5-8
K UPLIFT	0	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
JT COMBINED	2274	1311 / 0	412 / 0	PERM. LIVE	WIND	DEAD	SOIL
R	2274	1311 / 0	412 / 0	0 / 0	0 / 0	550 / 0	0 / 0
K	2047	1188 / 0	368 / 0	0 / 0	0 / 0	491 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	WEBS	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO					FR-TO			
A-B	0 / 45	-102.1	-102.1	0.08 (1)	10.00	Q-C	-406 / 44	0.07 (1)
B-C	-3032 / 0	-102.1	-102.1	0.19 (1)	5.12	C-P	0 / 2393	0.30 (1)
C-S	-4075 / 0	-102.1	-102.1	0.37 (1)	4.34	P-D	-1438 / 0	0.24 (1)
S-T	-4075 / 0	-102.1	-102.1	0.37 (1)	4.34	D-N	0 / 919	0.11 (1)
T-D	-4075 / 0	-102.1	-102.1	0.37 (1)	4.34	N-F	-616 / 0	0.10 (1)
D-U	-4743 / 0	-102.1	-102.1	0.40 (1)	4.04	F-G	0 / 1001	0.12 (1)
U-E	-4743 / 0	-102.1	-102.1	0.40 (1)	4.04	M-H	-1228 / 0	0.21 (1)
E-V	-4743 / 0	-102.1	-102.1	0.40 (1)	4.04	M-H	0 / 2681	0.33 (1)
V-F	-4743 / 0	-102.1	-102.1	0.40 (1)	4.04	L-H	-509 / 0	0.09 (1)
F-W	-4743 / 0	-102.1	-102.1	0.29 (1)	4.18	S-Q	0 / 2430	0.30 (1)
W-G	-4743 / 0	-102.1	-102.1	0.29 (1)	4.18	L-I	0 / 2148	0.27 (1)
G-H	-4015 / 0	-102.1	-102.1	0.26 (1)	4.51			
H-I	-2680 / 0	-102.1	-102.1	0.18 (1)	5.38			
I-J	0 / 45	-102.1	-102.1	0.08 (1)	10.00			
R-B	-3002 / 0	0.0	0.0	0.11 (1)	7.81			
K-I	-2692 / 0	0.0	0.0	0.10 (1)	7.81			
R-X	0 / 0	-38.5	-38.5	0.05 (2)	10.00			
X-Q	0 / 0	-38.5	-38.5	0.05 (2)	10.00			
Q-Y	0 / 2314	-38.5	-38.5	0.19 (1)	10.00			
Y-Z	0 / 2314	-38.5	-38.5	0.19 (1)	10.00			
Z-P	0 / 2314	-38.5	-38.5	0.19 (1)	10.00			
P-A	0 / 4075	-38.5	-38.5	0.36 (1)	10.00			
AA-O	0 / 4075	-38.5	-38.5	0.36 (1)	10.00			
OA-B	0 / 4075	-38.5	-38.5	0.36 (1)	10.00			
AB-N	0 / 4075	-38.5	-38.5	0.36 (1)	10.00			
N-AC	0 / 4015	-38.5	-38.5	0.53 (1)	10.00			
AC-AD	0 / 4015	-38.5	-38.5	0.53 (1)	10.00			
AD-M	0 / 4015	-38.5	-38.5	0.53 (1)	10.00			
M-L	0 / 2041	-38.5	-38.5	0.23 (1)	10.00			
L-K	0 / 0	-38.5	-38.5	0.03 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)								
JT LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.	
C 3-10-8	-274	-274	BACK	VERT	TOTAL			
Q 3-11-4	-36	-36	BACK	VERT	TOTAL			
S 5-11-4	-96	-96	BACK	VERT	TOTAL			
T 7-11-4	-96	-96	BACK	VERT	TOTAL			
U 9-11-4	-96	-96	BACK	VERT	TOTAL			
V 11-11-4	-96	-96	BACK	VERT	TOTAL			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 52.5 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.40/1.00 (D-F:1), BC=0.53/1.00 (M-N:1), WB=0.33/1.00 (H-M:1), SS=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1697	788
	1697	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (C) (INPUT = 0.90)
SI METAL= 0.65 (O) (INPUT = 1.00)

DWG NO. TAM 77410924
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME 402975	TRUSS NAME T28	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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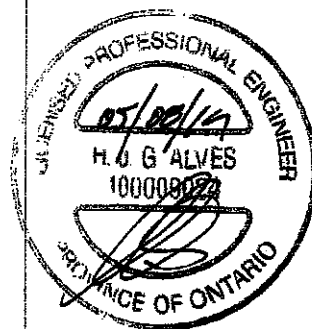
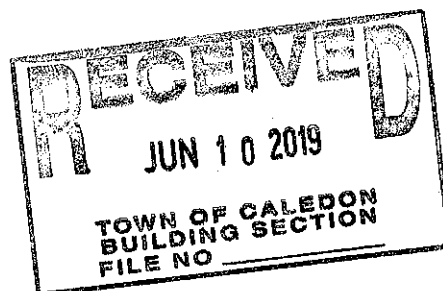
Yamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	13-11-4	-86	-86	—	BACK	VERT	TOTAL	—	—
X	1-11-4	-38	-46	—	BACK	VERT	TOTAL	—	—
Y	5-11-4	-38	-46	—	BACK	VERT	TOTAL	—	—
Z	7-11-4	-38	-46	—	BACK	VERT	TOTAL	—	—
AA	9-11-4	-38	-46	—	BACK	VERT	TOTAL	—	—
AB	11-11-4	-38	-46	—	BACK	VERT	TOTAL	—	—
AC	13-11-4	-38	-46	—	BACK	VERT	TOTAL	—	—
AD	15-8-8	-745	-745	—	BACK	VERT	TOTAL	—	—

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



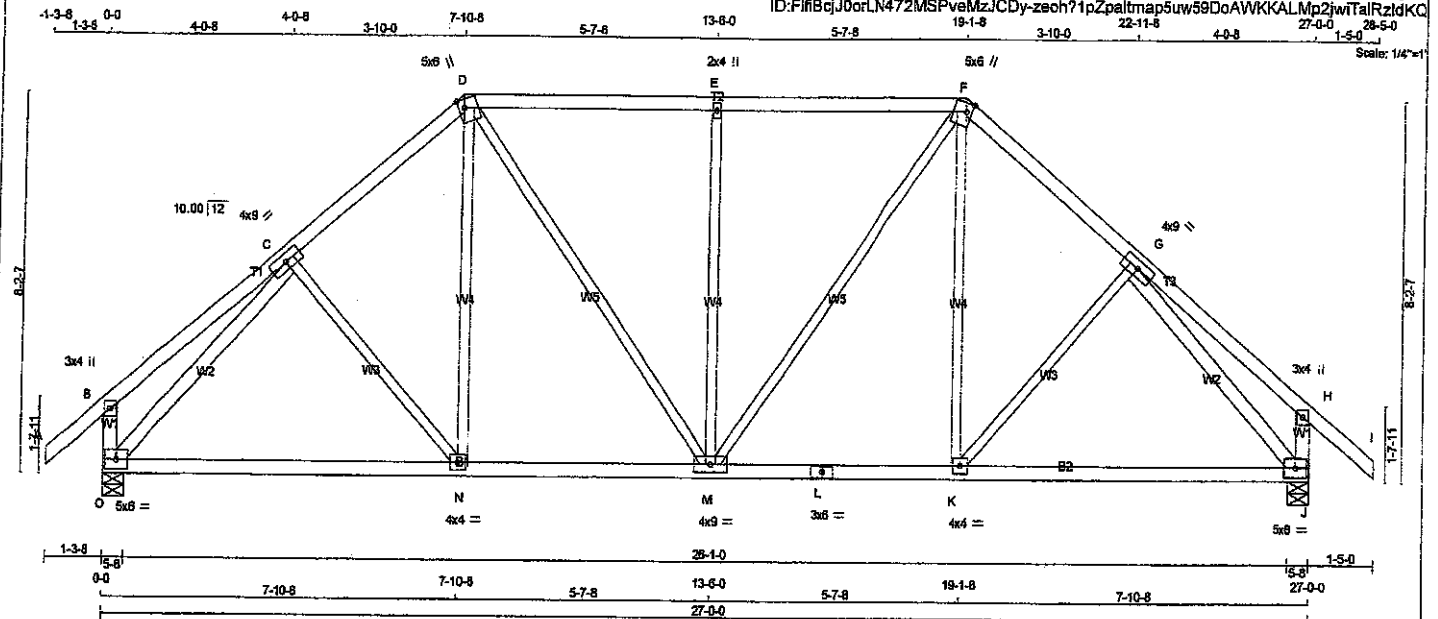
DWG NO. TAM 7910924
STRUCTURAL
COMPONENT ONLY 7/2

DWG NO. TAM **11910925**
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402975	T30	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:58:03 2019 Page 1
ID:FiBcJ0orLN472MSPveMzjCDy-zeoh?1pZpaltmap5uw59DoAWKKALMp2jwTatRzldKQ



TOTAL WEIGHT = 2 X 130 = 261 lb
(MIF)

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	SIZE	DRY	LUMBER	DESCR.
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+t	MT20	4.0	9.0	
D	TTVMW+m	MT20	5.0	6.0	2.25 1.50
E	TMVW+w	MT20	2.0	4.0	
F	TTVMW+m	MT20	5.0	6.0	2.25 1.50
G	TMVW+t	MT20	4.0	9.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW1-t	MT20	5.0	6.0	
K	BMVW-t	MT20	4.0	4.0	
L	BS-t	MT20	3.0	6.0	
M	BMVW1-t	MT20	4.0	9.0	
N	BMVW-t	MT20	4.0	4.0	
O	BMVW1-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
O	2040	0	2040	0
J	2053	0	2053	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
O	1516	865 / 0	284 / 0
J	1525	872 / 0	284 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED DIAPHRAGM (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED DIAPHRAGM (LBS)
A-B	0 / 45	-102.1	-102.1	0.14 (1)	10.00	C-N	-28 / 94	0.02 (3)	
B-C	0 / 29	-102.1	-102.1	0.25 (1)	10.00	N-D	0 / 393	0.09 (2)	
C-D	-1815 / 0	-102.1	-102.1	0.24 (1)	4.78	D-M	0 / 515	0.12 (1)	
D-E	-1872 / 0	-102.1	-102.1	0.45 (1)	4.81	M-E	-700 / 0	0.92 (1)	
E-F	-1872 / 0	-102.1	-102.1	0.45 (1)	4.81	M-F	0 / 515	0.12 (1)	
F-G	-1815 / 0	-102.1	-102.1	0.24 (1)	4.78	K-F	0 / 393	0.09 (2)	
G-H	0 / 29	-102.1	-102.1	0.25 (1)	10.00	K-G	-28 / 94	0.02 (3)	
H-I	0 / 49	-102.1	-102.1	0.17 (1)	10.00	O-C	-2133 / 0	0.93 (1)	
O-B	-295 / 0	0.0	0.0	0.03 (1)	7.81	G-J	-2133 / 0	0.93 (1)	
J-H	-308 / 0	0.0	0.0	0.03 (1)	7.81				
O-N	0 / 1387	-38.5	-38.5	0.59 (2)	10.00				
N-M	0 / 1373	-38.5	-38.5	0.59 (2)	10.00				
M-L	0 / 1373	-38.5	-38.5	0.59 (2)	10.00				
L-K	0 / 1373	-38.5	-38.5	0.59 (2)	10.00				
K-J	0 / 1387	-38.5	-38.5	0.59 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.1 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/899 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/899 (0.27")

CSI: TO=0.45/1.00 (D-E-1), BC=0.59/1.00 (K-M-2), WB=0.93/1.00 (G-J-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

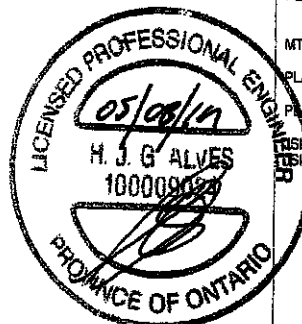
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1687 788 1987 1636	

PLATE PLACEMENT TOL = 0.250 inches

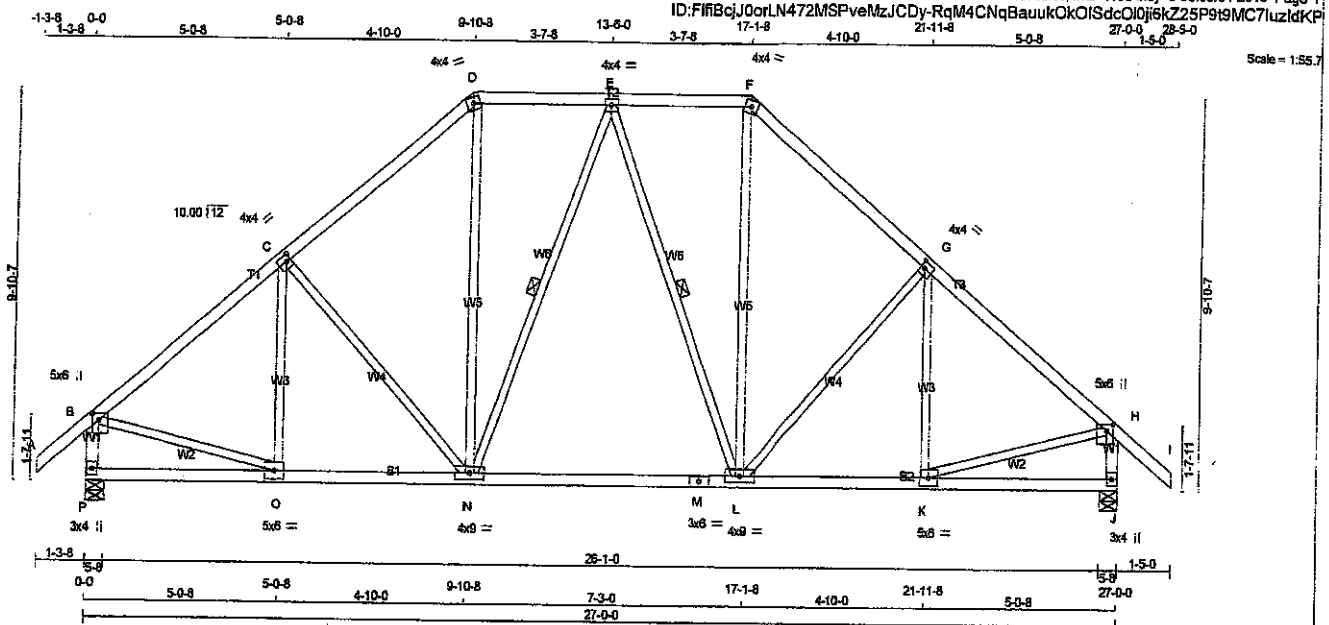
PLATE ROTATION TOL = 5.0 Deg.

SSI GRIP= 0.88 (C) (INPUT = 0.50)
SSI METAL= 0.46 (L) (INPUT = 1.00)



DRWG NO. TAM 71910.926
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	T31	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	5.0	6.0	2.00	2.00
C	TMWV-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWV-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWV-t	MT20	4.0	4.0	2.00	1.25
H	TMWV+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWV-t	MT20	5.0	6.0		
L	BMWV-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWV-t	MT20	4.0	9.0		
O	BMWV-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ
P	2040	0	2040	0
J	2053	0	2053	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1516	865 / 0	284 / 0	0 / 0	0 / 0	398 / 0	0 / 0	0 / 0
J	1525	872 / 0	284 / 0	0 / 0	0 / 0	398 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 45	-102.1 -102.1	0.14 (1)	10.00	O-C	-189 / 79	0.10 (1)
B-C	-1890 / 0	-102.1 -102.1	0.39 (1)	4.52	C-N	-339 / 0	0.37 (1)
C-D	-1678 / 0	-102.1 -102.1	0.36 (1)	4.76	N-D	0 / 712	0.16 (1)
D-E	-1262 / 0	-102.1 -102.1	0.17 (1)	5.55	E-N	-231 / 0	0.18 (1)
E-F	-1262 / 0	-102.1 -102.1	0.17 (1)	5.55	E-L	-231 / 0	0.18 (1)
F-G	-1678 / 0	-102.1 -102.1	0.38 (1)	4.76	L-F	0 / 712	0.16 (1)
G-H	-1890 / 0	-102.1 -102.1	0.39 (1)	4.52	L-G	-339 / 0	0.37 (1)
H-I	0 / 45	-102.1 -102.1	0.17 (1)	10.00	K-G	-189 / 79	0.10 (1)
P-B	-1957 / 0	0.0 0.0	0.21 (1)	6.02	B-O	0 / 1529	0.34 (1)
J-H	-1970 / 0	0.0 0.0	0.21 (1)	6.00	K-H	0 / 1529	0.34 (1)
P-O	0 / 0	-38.5 -38.5	0.16 (3)	10.00			
O-N	0 / 1481	-38.5 -38.5	0.43 (2)	10.00			
N-M	0 / 1342	-38.5 -38.5	0.41 (2)	10.00			
M-L	0 / 1342	-38.5 -38.5	0.41 (2)	10.00			
L-K	0 / 1481	-38.5 -38.5	0.43 (2)	10.00			
K-J	0 / 0	-38.5 -38.5	0.16 (3)	10.00			

TOTAL WEIGHT = 2 X 135 = 270 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.39/1.00 (G-H-1), BC=0.43/1.00 (K-L-2), WB=0.37/1.00 (G-L-1), SS=0.19/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

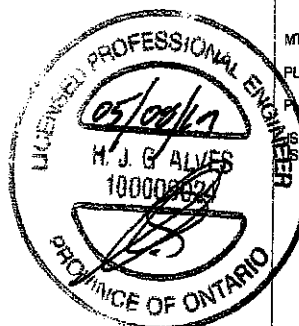
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354
	1667	788
	1967	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SIGRIP = 0.87 (B) (INPUT = 0.90)
SIGMETAL = 0.42 (B) (INPUT = 1.00)



DWG NO. TAM 19/09/27
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 08:38:21 2019 Page 1
ID:LVeO_09TNb8RMKMLcMgORlZJCwZ-3KdD6uzPwHyZMMd_ht17ArB5VEpUXkPITCOU?zldb0

Scale = 1-13 =



CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

EDITING FEATURES

D C R E T	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
	352	0	352	0	0	5-8	5-8
	94	0	87	0	0	1-8	1-8
	94	0	123	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C. D.

UNFACTORED REACTIONS

1ST CASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
E	259	156 / 0	42 / 0	0 / 0	0 / 0	61 / 0	0 / 0
C	78	24 / 0	28 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	86	0 / -3	53 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E. C.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
E-B	-229 / 26	0.0	0.0	0.16 (3)	7.81		
A-B	0 / 31	-102.1	-102.1	0.15 (1)	10.00		
B-C	0 / 27	-102.1	-102.1	0.16 (3)	10.00		
E-F	0 / 0	-38.5	-38.5	0.23 (3)	10.00		
F-G	0 / 0	-38.5	-38.5	0.23 (3)	10.00		
G-D	0 / 0	-38.5	-38.5	0.23 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

LT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	8	1	13	FRONT	VERT	TOTAL	—	—
G	3-11-4	1	1	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.1	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(TL) = $L/839$ (0.08")

CSI: TC=0.18/1.00 (B-E:3), BC=0.23/1.00 (D-E:3),
WB=0.00/1.00 (n/a:0), SS=0.14/1.00 (D-E:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

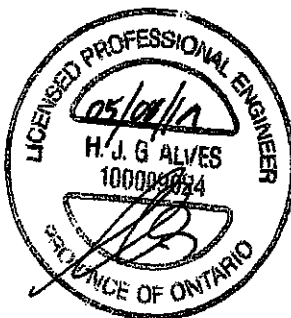
	NAIL VALUES		SHEAR (PLI)	SECTION (PLI)
	PLATE (PSI)	GRIP(DRY)		
	MAX	MIN	MAX	MIN
MT20	618	354	1867	788
			1987	1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.10 (E) (INPUT = 0.90)
SI METAL= 0.08 (B) (INPUT = 1.00)

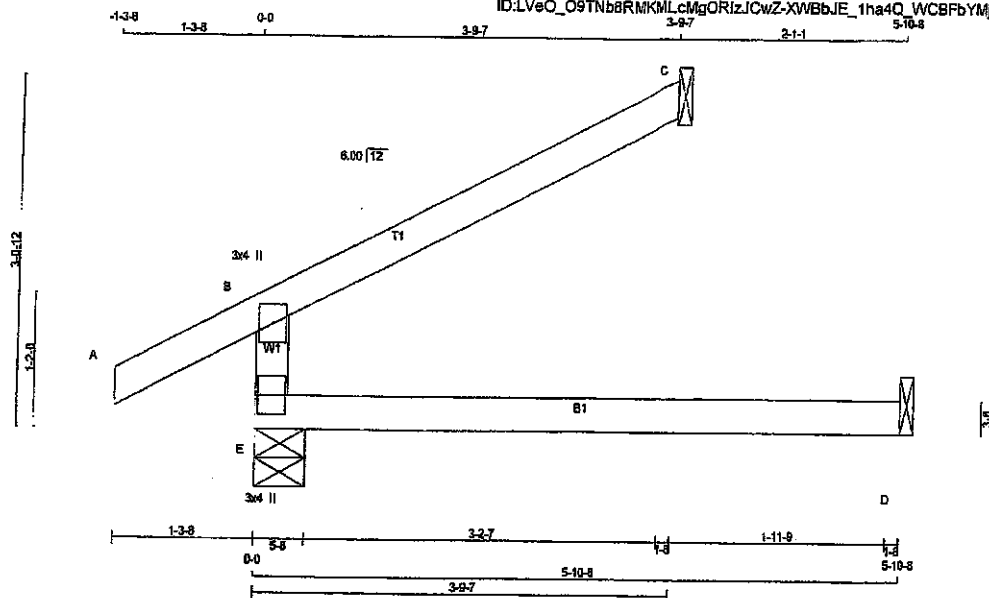
DWG NO. TAM 71910798
STRUCTURAL
COMPONENT ONLY



JOB NAME 402973	TRUSS NAME J3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:LVeO_O9TNb8RMKMLcMgORiz/CwZ-XWBbJE_1ha4Q_WCSFbYmJ3kFFe87GAYyY7yy0Rzldb?

Scale = 1:18.5



TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	513	0	513	0	5-8	5-8
C	145	0	145	0	1-8	1-8
D	83	0	119	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	382	218 / 0	72 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	100	83 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX			MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
			FROM	TO	TO				
E-B		-380 / 0	0.0	0.0	0.21 (3)	7.81			
A-B		0 / 31	-102.1	-102.1	0.13 (1)	10.00			
B-C		-22 / 0	-102.1	-102.1	0.25 (1)	6.25			
E-D		0 / 0	-38.5	-38.5	0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, OBC 2012
 - CSA 086-06, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/380$ (0.20")
 CALCULATED VERT. DEFL.(LL) = $L/988$ (0.04")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(TL) = $L/942$ (0.07")

CSI: TC=0.25/1.00 (B-C:1), BC=0.21/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), GSI=0.17/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

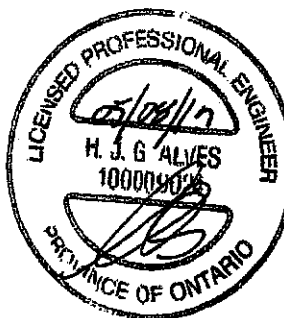
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

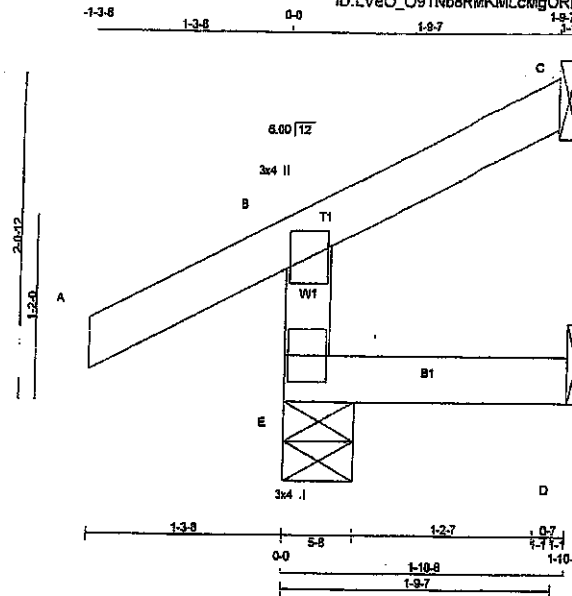
JSI GRIP= 0.18 (E) (INPUT = 0.80)
 JSI METAL= 0.11 (B) (INPUT = 1.00)



DRWG NO. TAM **17910799**
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 402973	TRUSS NAME J4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 9 Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:38:23 2019 Page 1
ID: LveO_O9TNb8RMKMLcMgORizJCwZ-7jzVla_gSuCGbgnNai3bFGHRp1Xy?dCIBnhVZuzldB



Scale = 1:13.5

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
E	318	0	318	0	5-8	5-8	
C	51	0	51	0	-28	1-8	1-8
D	24	0	41	0	-4	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	228	180 / 0	20 / 0	0 / 0	0 / 0	46 / 0	0 / 0	0 / 0
C	38	27 / -21	2 / 0	0 / 0	0 / 0	7 / 0	0 / 0	0 / 0
D	24	0 / -9	18 / 0	0 / 0	0 / 0	11 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	UNBRAC LENGTH FR-TO
FR-TO		FROM	TO				
E-B	-270 / 0	0.0	0.0	0.04 (5)	7.81		
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00		
B-C	-18 / 0	-102.1	-102.1	0.10 (1)	6.25		
E-D	0 / 0	-38.5	-38.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (n=0), SS=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

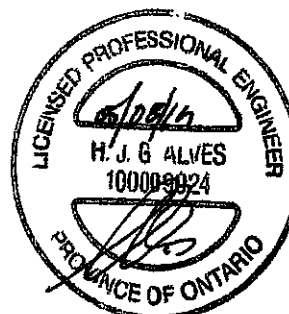
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667
		788	1987
			1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

DWG NO. TAM 191 0800
STRUCTURAL
COMPONENT ONLY

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:36:23 2019 Page 1
ID:LvO8_Q8TNb8RMKMLcMgORJzCwZ-7jizWa_gSuCGbgnNoI3bFGHQ?1X8?dCiBnhVZuzId



DRY: SEASONED LUMBER

DESCR.
SPF
SPF
SPF

TOTAL WEIGHT = $2 \times 10 = 19 \text{ lb}$

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.1	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(L) = $L/360$ (0.19")
 CALCULATED VERT. DEFL.(L) = $L/999$ (0.00")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.25/1.00 (B-C:1), BC=0.03/1.00 (D-E:3),
WB=0.00/1.00 (n/a:0), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

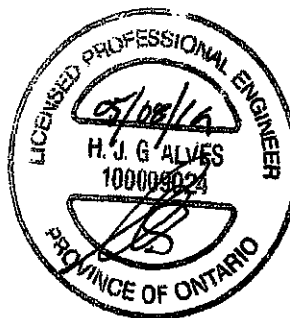
NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

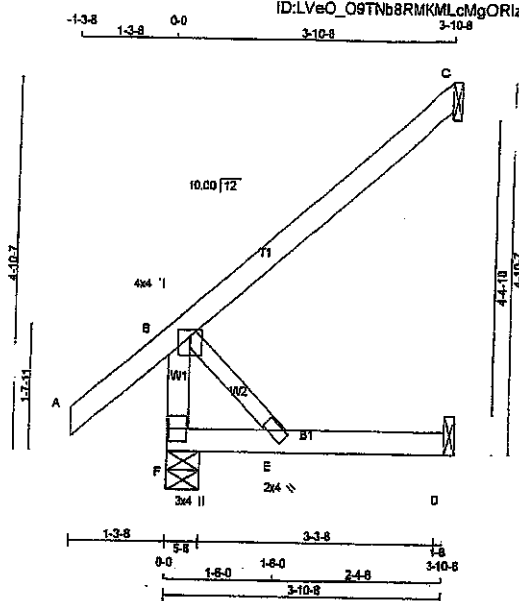
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

DWG NO. TAM 1910801
STRUCTURAL
COMPONENT ONLY



JOB NAME: 402973 TRUSS NAME: J7 QUANTITY: 4 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: DRWG NO. Tamarack Roof Truss, Burlington



Scale = 1/27.5

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
F	414	0	414	0	5-8	5-8
C	198	0	198	0	1-8	1-8
D	75	0	95	0	1-8	1-8

SEE MITEK STANDARD DETAIL 837821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	301	193 / 0	41 / 0	0 / 0	0 / 0	67 / 0	0 / 0
C	136	113 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	68	0 / 0	41 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.
FR-TO				FR-TO			
F-B	-338 / 0	0.0	0.04 (1)	F-B	0 / 0	0.00 (1)	
A-B	0 / 45	-102.1	0.14 (1)				
B-C	0 / 0	-102.1	0.28 (1)				
F-E	0 / 0	-38.5	0.13 (3)				
E-D	0 / 0	-38.5	0.14 (3)				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 4 X 15 = 61 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TO=0.26/1.00 (B-C:1), BC=0.14/1.00 (D-E:3),
WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

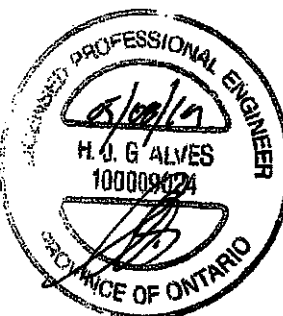
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



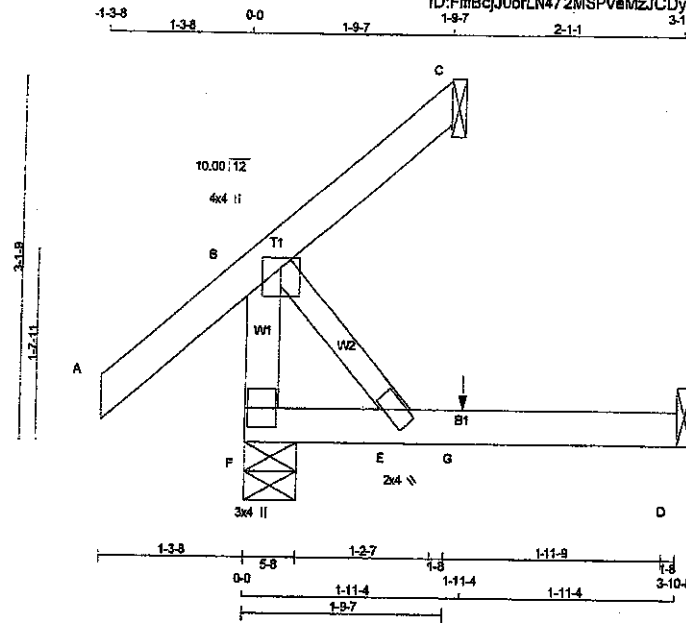
DRWG NO. TAM 10803
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	J21	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID: FfBcj0orLN472MSPvMzJCDy-cg?oxJmR?27agox86MV_VWTKDJaGhnN_nQlp4EzldKV

Scale = 1:19.1



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

LUMBER

DESCR.

SPF

SPF

SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	362	362	0	0
C	36	36	0	0
D	75	95	0	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT *OK w/ 105 MAX LAM*

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	285	164 / 0	41 / 0	0 / 0	0 / 0	0 / 0
C	25	21 / -31	0 / 0	0 / 0	4 / 0	0 / 0
D	68	0 / 0	41 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
F-B	-288 / 0	0.0	0.0	0.03 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 45	-102.1	-102.1	0.15 (1)			
B-C	-35 / 0	-102.1	-102.1	0.15 (1)			
F-E	0 / 0	-38.5	-38.5	0.12 (2)			
E-G	0 / 0	-38.5	-38.5	0.14 (3)			
G-D	0 / 0	-38.5	-38.5	0.14 (3)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	-	FRONT	VERT	TOTAL	-	-

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 52.5	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OSC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.15/1.00 (A-B:1), BC=0.14/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

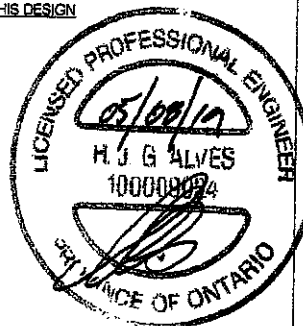
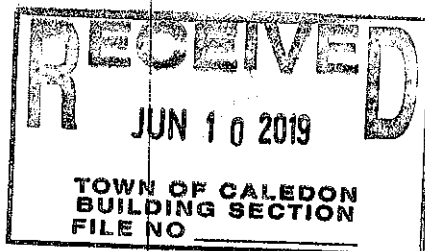
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX	MIN	MAX
MT20	618	354	1667
	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.22 (B) (INPUT = 0.90)
SI METAL= 0.08 (B) (INPUT = 1.00)

JOB NO. TAM 1910921
STRUCTURAL
COMPONENT ONLY



JOB NAME 402975	TRUSS NAME J22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:F#BcJ0orLN472MSPveMzjCDy-4tYB9fm3mLFRlyWK40D2y0vBjy8QEd804VncgzldKUL

1-3-8 1-3-8 0-0 1-9-7 1-9-7 1-10-8

Scale = 1:19.1

LUMBER

N.L.G.A. RULES

CHORDS SIZE

F - B 2x4 DRY No.2 SPF

A - C 2x4 DRY No.2 SPF

F - D 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION

MAXIMUM FACTORED GROSS REACTION

INPUT BRG UPLIFT IN-SX

REQ'D BRG IN-SX

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

C 324 0 324 0 5-8 5-8

F 36 0 36 0 -45 1-8

D 36 0 48 0 1-8 1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE MAX/MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL

F 230 164 / 0 20 / 0 0 / 0 0 / 0 47 / 0 0 / 0

C 25 21 / -31 0 / 0 0 / 0 0 / 0 4 / 0 0 / 0

D 33 0 / 0 20 / 0 0 / 0 0 / 0 13 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (PLF) MAX. LOAD LC1 MAX. MAX. CSI (LC) UNBRAC LENGTH FR-TO

F-B -288 / 0 0.0 0.0 0.03 (1) 7.81

A-B 0 / 45 -102.1 -102.1 0.14 (1) 10.00

B-C -35 / 0 -102.1 -102.1 0.13 (1) 6.25

F-E 0 / 0 -38.5 -38.5 0.03 (3) 10.00

E-D 0 / 0 -38.5 -38.5 0.03 (3) 10.00

WEBS

MEMB. MAX. FACTORED FORCE (LBS) MAX. FACTORED CSI (LC)

B-E 0 / 0 0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.1 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL DL = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012

- CSA 088-09, CSA 088-14

- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(5% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")

CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")

ALLOWABLE DEFL.(TL)= L/360 (0.19")

CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B-1), BC=0.03/1.00 (E-F-3).

WB=0.00/1.00 (B-E-1), SS=0.09/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)

JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER

05/08/19

H.J.G. ALVES

100008002

PROVINCE OF ONTARIO

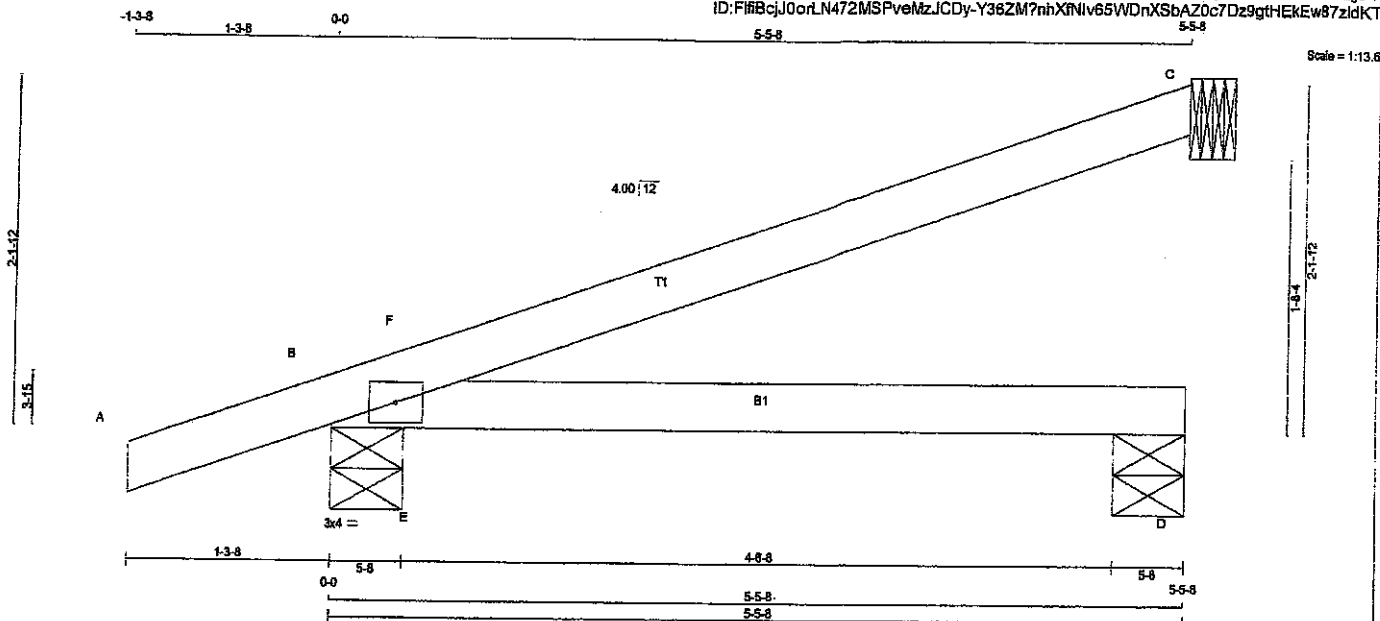
JWG NO.TAM 71910922

STRUCTURAL

COMPONENT ONLY

JOB NAME 402975	TRUSS NAME J23	QUANTITY 7	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Wed May 8 08:56:00 2019 Page 1
ID: F16BcjJ0orLN472MSPveMzJCDy-Y36ZM?nhXNiv65WDnXSbAZ0c7Dz9gtHEKEw87zldKT



TOTAL WEIGHT = 7 X 14 = 101 lb

LUMBER

N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
B - D 2x4 DRY

LUMBER
No.2
No.2

DESCR.
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-1	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	250	0	250	0	5-8	5-8
B	520	0	520	0	5-8	5-8
D	133	0	145	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	381	236 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	E-F	-254 / 122
B-F	-51 / 0	-102.1	-102.1	0.13 (3)	6.25		0.00 (1)
F-C	0 / 6	-102.1	-102.1	0.42 (1)	10.00		
B-E	0 / 0	-38.5	-38.5	0.26 (1)	10.00		
E-D	0 / 0	-38.5	-38.5	0.31 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/842 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/489 (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

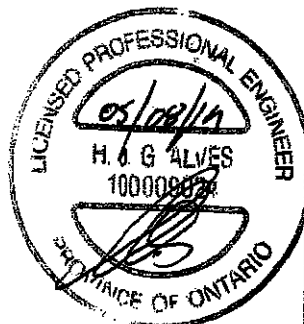
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DRWG NO. TAM 119/0923
STRUCTURAL
COMPONENT ONLY



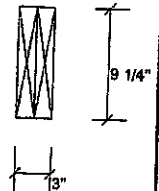
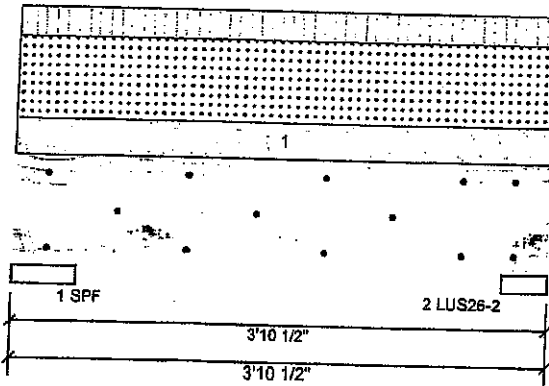
isDesign™

Client:
Project:
Address:Date: 5/8/2019
Designer:
Job Name: 201018
Project #:

Page 1 of 2

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	138	171	388	0
2	130	160	364	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	9%	214 / 721	935 L
2 - LUS26-2	4.000"	12%	201 / 676	876 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	601 ft-lb	2'	6039 ft-lb	0.100 (10%)	1.25D+1.5S +L	L
Unbraced	601 ft-lb	2'	5734 ft-lb	0.105 (10%)	1.25D+1.5S +L	L
Shear	750 lb	1'2"	3984 lb	0.188 (19%)	1.25D+1.5S +L	L
LL Defl Inch	0.002 (L/19241)	2'	0.107 (L/360)	0.020 (2%)	S+0.5L	L
TL Defl Inch	0.003 (L/14002)	2'	0.107 (L/360)	0.030 (3%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1010811
STRUCTURAL
CONSTRUCTION PLAN 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-7-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





isDesign™

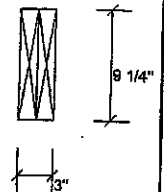
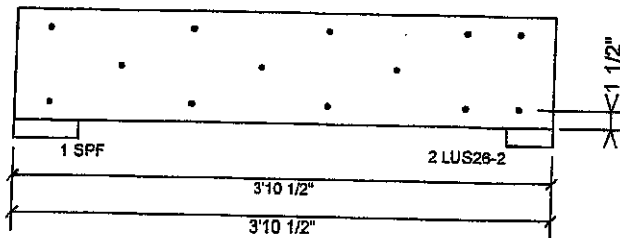
Client:
Project:
Address:

Date: 5/8/2019
Designer:
Job Name: 201018
Project #:

Page 2 of 2

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

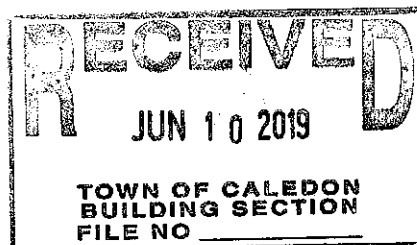
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	68.7 %
Load	233.7 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17910811
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	Tamarack Roof Trusses 3289 North Service Rd., ON Canada L7N3G2 (805) 335-1115
	TAMARACK LUMBER INC ALMA LUMBER GROUP

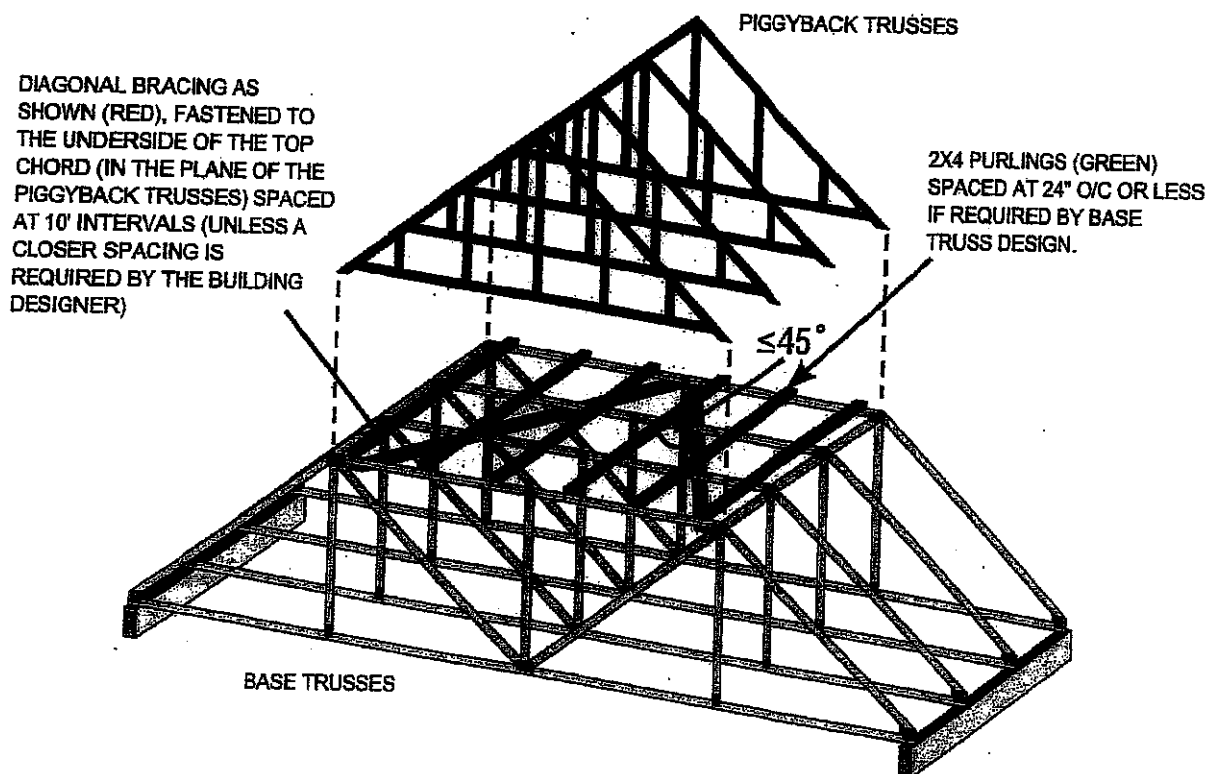


Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF

ARIO BUILDING CODE

SIMPSON

Strong-Tie

LUS - Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

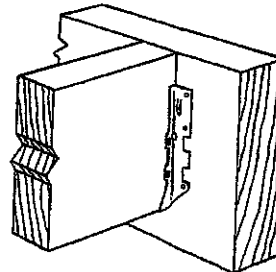
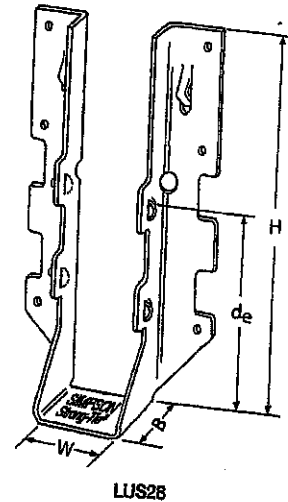
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

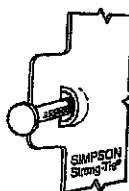
- These hangers cannot be modified



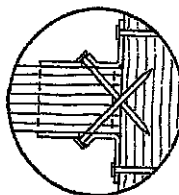
Typical LUS
Installation

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist	D.Fir-L		S.P.F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LUS24	18	1 1/8	3 1/4	1 1/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS25	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS25-2	18	3 1/8	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS25-3	18	4 1/8	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/4	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

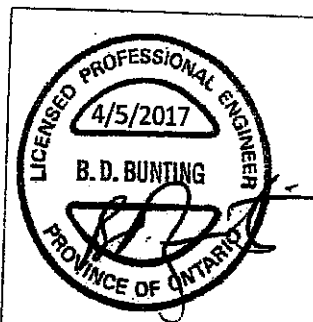
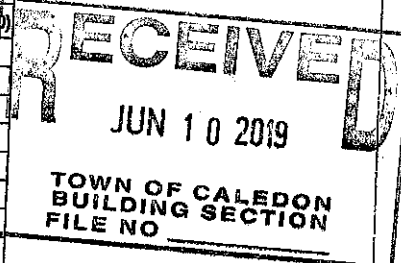
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



(200) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

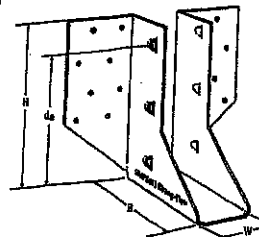
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

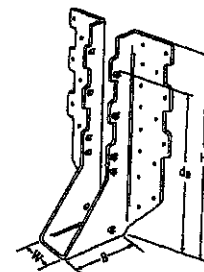
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

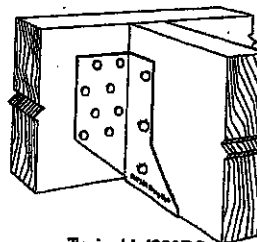
- See current catalogue for options



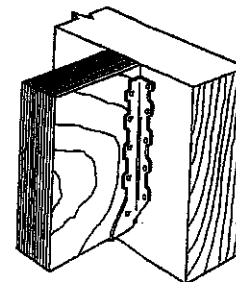
LJS26DS



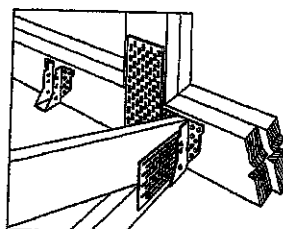
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



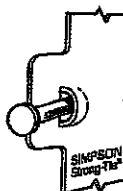
Typical HUS
Installation



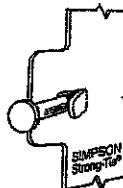
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.F.R.-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/8	(16) 16d	(8) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/4	(14) 16d	(8) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

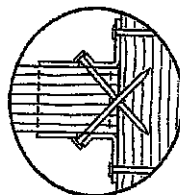
1. d₂ is the distance from the seat of the hanger to the highest joist nail.



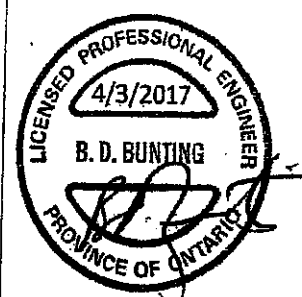
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,803,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



(800) 999-5099
strongtie.com

HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

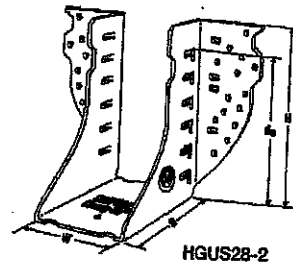
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- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

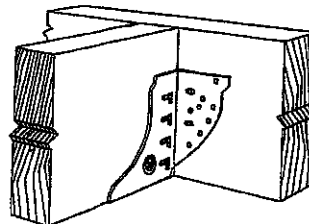
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

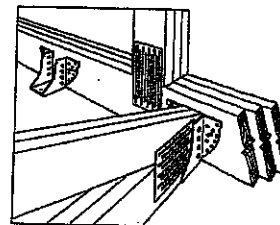
- See current catalogue for options



HGUS28-2



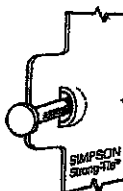
Typical HGUS
Installation



Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

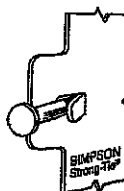
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.F.R.-L		S.P.F	
								Uplift (K _u =1.15)	Normal (K _c =1.00)	Uplift (K _u =1.15)	Normal (K _c =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

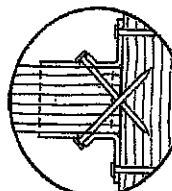


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

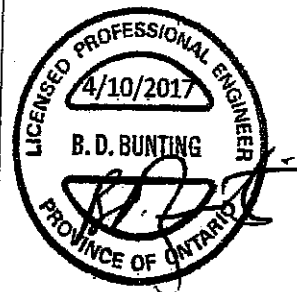
U.S. Patent
5,803,590



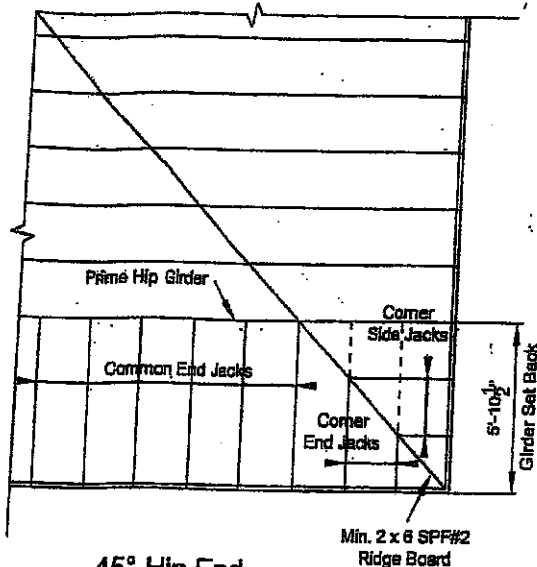
Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



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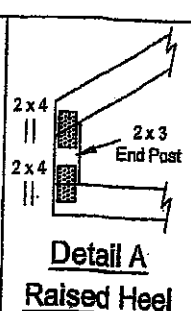
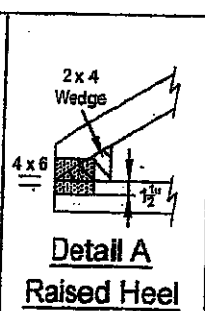
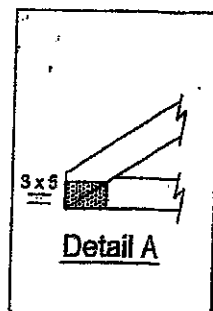
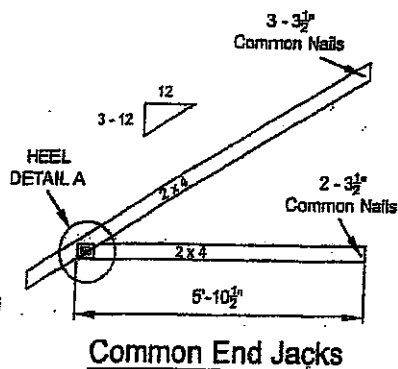
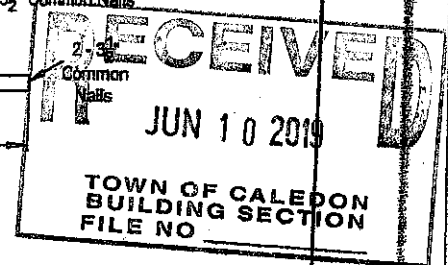
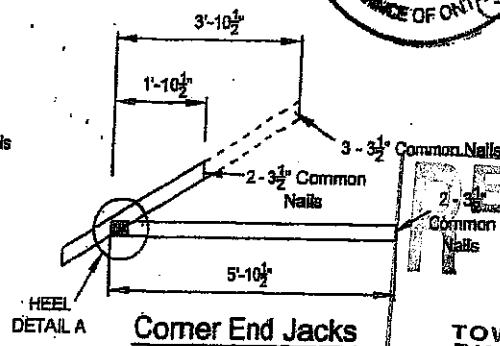
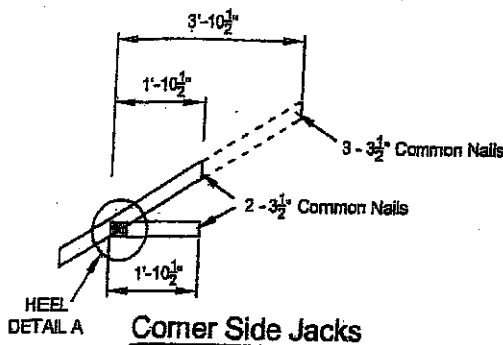
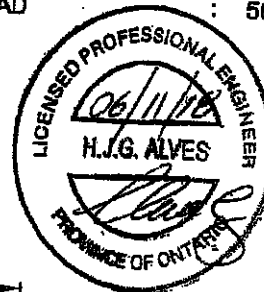


LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

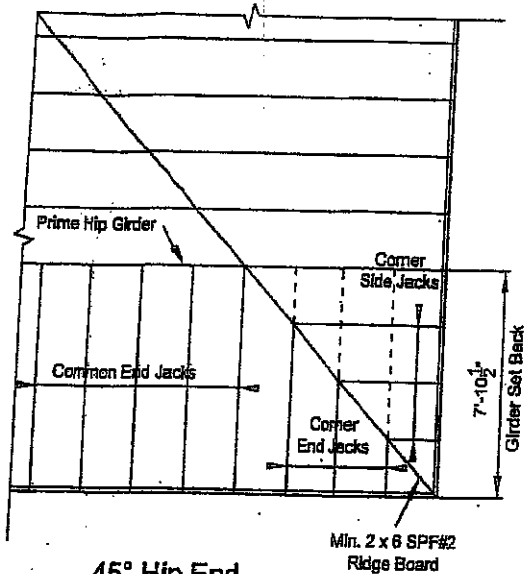
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 50.5 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C., 2012 (L.S.D. DESIGN)

T-180021



45° Hip End

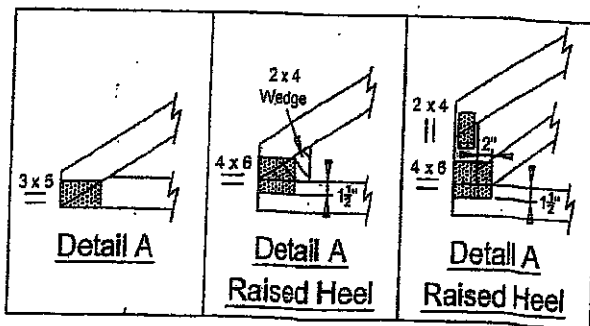
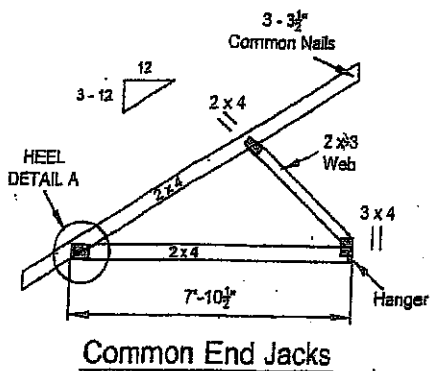
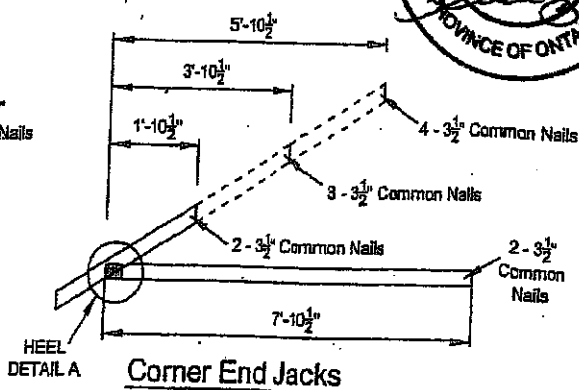
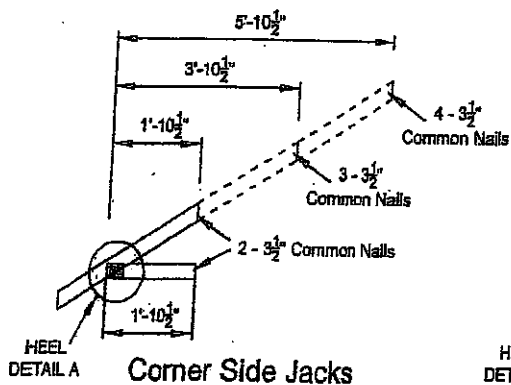
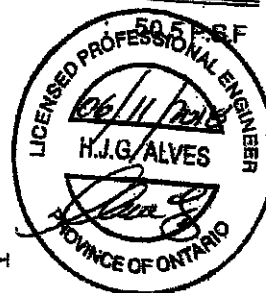
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



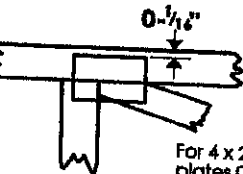
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



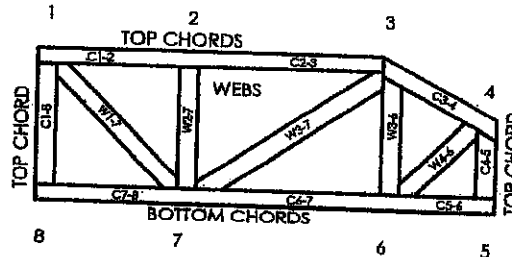
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:
11996-L, 10319-L, 13270-L, 12691-R

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MiTek
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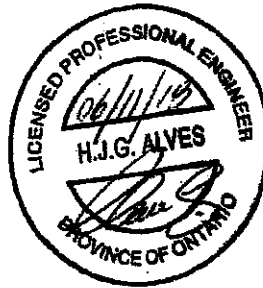
MiTek Engineering Reference Sheet: MH-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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TOWN OF CALEDON
BUILDING SECTION
FILE NO



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**

T-1300213

Feb 09, 2018