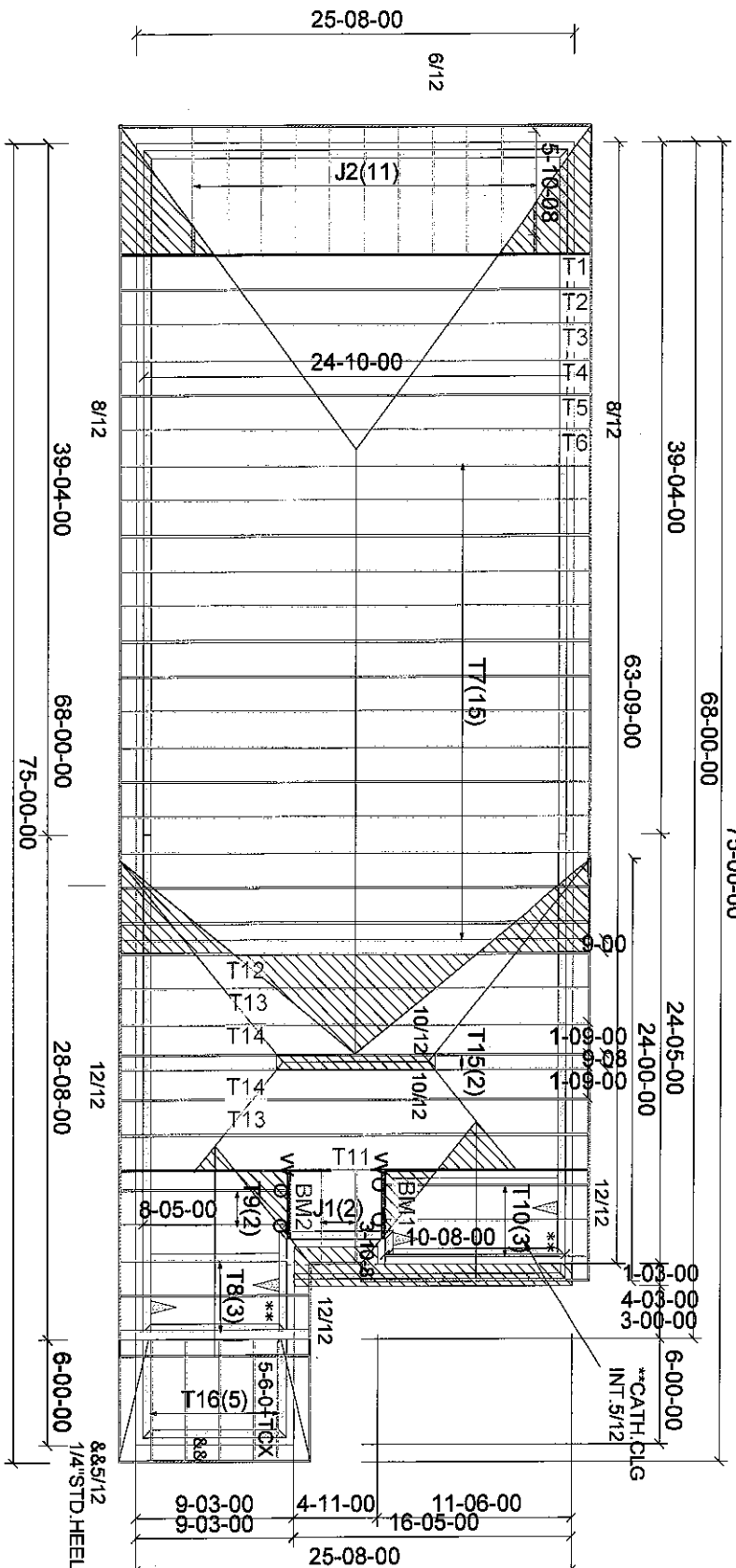


THIS TRUSS DESIGNED FOR FLAT SOLAR PANEL ARRAY TO BE INSTALLED ALONG TOP CHORDS HAVING A COMBINED DEAD LOAD (SOLAR PANEL ARRAY) NOT EXCEEDING 5.0 PSF AND IN ACCORDANCE WITH CURRENT EDITION OF T.P.C. THIS IS A LOAD READY DECLARATION ONLY AND NOT AN INSTALLATION READY DECLARATION A SEPARATE INSTALLATION DETAIL IS REQUIRED BY LICENSED PROFESSIONAL ENGINEER ONCE A SOLAR ARRAY LAYOUT IS ESTABLISHED, AND AVAILABLE ON A ROOF BY ROOF BASIS.



12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART 9

DESIGN LOADS:
GROUND SNOW LOAD
S_s = 2.3 kPa
TC DEAD 8 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LUS26-2(VV)
BM1.2: 2-2X10

T-160018

mil, 1105

		Job Track 39938		Builder / Location: GREENPARK HOMES / EAST GWILLIMBURY		Model / Elevation: NEWBERRY 6A / 1	
Layout ID: 254489D		Project: SECONDO VALLEY ESTATES		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
Plan Log: 95085		Date: 1/5/2018		Designer: JG		Milex ver 7.5.0	

[illegible]

DESIGN LOADS:
GROUND SNOW LOAD
Ss = 2.3 kPa
TC DEAD 8 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

T-160018

 TAMARACK TAMARACK, INC. A QUALITY ROOFING SERVICE		m11,105	
Job Track: 39938		Builder / Location:	
Layout ID: 254503 D		GREENPARK HOMES / EAST GWILLIMBURY	
Plan Log: 95085		Model / Elevation:	
Project: SECONDO VALLEY ESTATES		NEWBERRY 6A / 2	
Date: 1/5/2018	Designer: JG	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
Milek ver 7.5.0			

25-08-00

6/12

J2(11)

5-10-08

1

2

3

4

5

6

8/12

24-19-00

8/12

68-00-00

62-09-00

23-00-07

8-10

T21

T23

T24

T25(2)

T24

T23

T22

12/12

1-03-00

4-00-00

13-04-00

BM4

8-05-00

T19

J3(3)

3-08-26(2)

8-05-00

J4(3)

T27

9-03-00

4-11-00

11-06-00

5/12

12/12

1' RAISED CLG.

1' HIGHER

TOP OF PLATE

12" FINISH O.H.
R.T.M.C

2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN CONFORMS
WITH THE RELEVANT
SECTION OF THE
LATEST EDITION OF
O.B.C. PART.9

DESIGN LOADS:

GROUND SNOW LOAD

 $\sigma_s = 2.3 \text{ kPa}$

1C DEAD	8 PST
BC LIVE	10.6 PSE

BC DEAD 7 PSF

DENOTES

**DENOTES
CONVENTIONAL**

FRAMING

HARDWARE

LUS24(0)
11536 20AA

LU526-2(VV)
(AA)7-9787
HGIIS26-2(VV)

(vv)z-zc00011

BM4: 2-2X10

T-160018

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NOT BE REPRODUCED, PUBLISHED, OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM, WITHOUT PERMISSION IN WRITING FROM THE PUBLISHER.

MANUFACTURE OF TRUSSES BY
MECHANIZED FOR ANY OTHER

Mitek.

1

1



Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 39938 LAYOUT ID: 254489A D LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALLEY EST SUB-BUILDER:
 MODEL: NEWBERRY 6A ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T1 HIP GIRDER	8.00 0.00	24-10-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	118.39 74.67		
	1	T2 HIP	8.00 0.00	24-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	107.43 67.83		
	1	T3 HIP	8.00 0.00	24-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	106.73 67.00		
	1	T4 HIP	8.00 0.00	24-10-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	116.55 74.17		
	1	T5 HIP	8.00 0.00	24-10-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	111.99 69.50		
	1	T6 HIP	8.00 0.00	24-10-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	117.74 73.17		
	15	T7 COMMON	8.00 0.00	24-10-00	09-08-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	1597.05 985.05		
	3	T8 SCISSOR	12.00 5.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	131.55 86.01		
	2	T9 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	84.74 57.34		
	3	T10 SCISSORS	12.00 5.00	10-08-00	07-02-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	171.63 110.49		
	1	T11 HIP GIRDER	12.00 0.00	24-10-00	04-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	146.12 88.00		
	1	T12 HIP	12.00 0.00	24-10-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	104.63 67.33		
	2	T13 HIP	12.00 0.00	24-10-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	229.86 142.00		
	2	T14 HIP	12.00 0.00	24-10-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	244.66 156.00		
	2	T15 HIP	12.00 0.00	24-10-00	09-07-15	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	275.84 176.66		
	5	T16 MONOPITCH	5.00 0.00	05-06-00	02-11-15	2 X 4	2 X 4	01-03-08 00-00-00	00-04-01 02-07-09	101.25 66.65		
	2	J1 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	33.66 21.34		
	11	J2 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		

TOTAL # TRUSS= 55.00

TOTAL BFT OF ALL TRUSSES=

2500.58 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3984.51 LBS.



Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK:39938	LAYOUT ID: 254489A	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALLEY EST	SUB-BUILDER:	
MODEL: NEWBERRY 6A	ELEVATION: 1	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LUS24	
2	Hangers	LUS26-2	

TOTAL # ITEMS= 6.00



Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 39938 LAYOUT ID: 254503A LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALLEY EST SUB-BUILDER:
 MODEL: NEWBERRY 6A ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T1 HIP GIRDER	8.00 0.00	24-10-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	118.39 73.67		
	1	T2 HIP	8.00 0.00	24-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	107.43 67.83		
	1	T3 HIP	8.00 0.00	24-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	106.73 67.00		
	1	T4 HIP	8.00 0.00	24-10-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	116.55 74.17		
	1	T5 HIP	8.00 0.00	24-10-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	111.99 69.50		
	1	T6 HIP	8.00 0.00	24-10-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	117.74 73.17		
	15	T7 COMMON	8.00 0.00	24-10-00	09-08-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	1597.05 985.05		
	2	T8 SCISSOR	12.00 5.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	87.70 57.34		
	1	T11Z HIP GIRDER	12.00 0.00	24-10-00	04-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	146.12 87.00		
	1	T12 HIP	12.00 0.00	24-10-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	104.63 67.33		
	2	T13 HIP	12.00 0.00	24-10-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	229.86 142.00		
	2	T14 HIP	12.00 0.00	24-10-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	244.66 156.00		
	2	T15 HIP	12.00 0.00	24-10-00	09-07-15	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	275.84 176.66		
	1	T17 ROOF	12.00 5.00	13-04-00	08-06-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	74.66 48.00		
	2	T18 COMMON	12.00 0.00	13-04-00	08-06-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	138.56 88.00		
	5	J1 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	84.15 53.35		
	11	J2 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		

TOTAL # TRUSS= 50.00

TOTAL BFT OF ALL TRUSSES=

2403.44 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3846.75 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 39938	LAYOUT ID: 254503AD	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALLEY EST	SUB-BUILDER:	
MODEL: NEWBERRY 6A	ELEVATION: 2	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
2	Hangers	LJS26DS	

TOTAL # ITEMS= 3.00



Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 39938 LAYOUT ID: 254506A D LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALLEY EST SUB-BUILDER:
 MODEL: NEWBERRY 6A ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1	T1 HIP GIRDER	8.00 0.00	24-10-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	118.39 73.67		
	1	T2 HIP	8.00 0.00	24-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	107.43 67.83		
	1	T3 HIP	8.00 0.00	24-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	106.73 67.00		
	1	T4 HIP	8.00 0.00	24-10-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	116.55 74.17		
	1	T5 HIP	8.00 0.00	24-10-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	111.99 69.50		
	1	T6 HIP	8.00 0.00	24-10-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	117.74 73.17		
	15	T7 COMMON	8.00 0.00	24-10-00	09-08-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	1597.05 985.05		
	1	T9 COMMON	12.00 0.00	08-05-00	06-01-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	42.37 28.67		
	1	T19 HIP GIRDER	12.00 0.00	08-05-00	05-09-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	45.94 30.00		
	1	T20 HIP GIRDER	12.00 0.00	13-04-00	05-09-00	2 X 4	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08	78.07 48.83		
	1	T21 HIP	12.00 0.00	24-10-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 02-10-08	111.44 71.83		
	1	T22 HIP GIRDER	12.00 0.00	24-10-00	04-10-07	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 02-10-08	156.49 97.00		
	2	T23 HIP	12.00 0.00	24-10-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 02-10-08	231.56 143.34		
	2	T24 HIP	12.00 0.00	24-10-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 02-10-08	268.80 170.00		
	2	T25 HIP	12.00 0.00	24-10-00	09-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 02-10-08	280.14 175.66		
	2	T26 COMMON	5.00 0.00	08-05-00	02-09-10	2 X 4	2 X 4	01-03-08 01-03-08	01-00-09 01-00-09	65.62 44.34		
	1	T27 HIP GIRDER	5.00 0.00	08-05-00	02-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-00-09 01-00-09	34.19 23.00		
	1 2 Ply	T28 FLAT	0.00 0.00	03-10-08	01-00-00	2 X 4	2 X 4	00-00-00 00-00-00	01-00-00 01-00-00	24.98 17.34		



Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 39938 LAYOUT ID: 254506A LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALLEY EST SUB-BUILDER:
 MODEL: NEWBERRY 6A ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	J1 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-05-00 00-00-00	01-07-11 04-10-07	85.05 56.65		
	11	J2 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		
	5	J3 JACK-OPEN	12.00 0.00	03-10-08	05-09-00	2 X 4	2 X 4	01-03-08 00-00-00	01-10-08 05-09-00	89.50 56.65		
	3	J4 JACK-OPEN	8.00 0.00	01-10-08	02-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-07-13	27.87 17.01		

TOTAL # TRUSS= 61.00

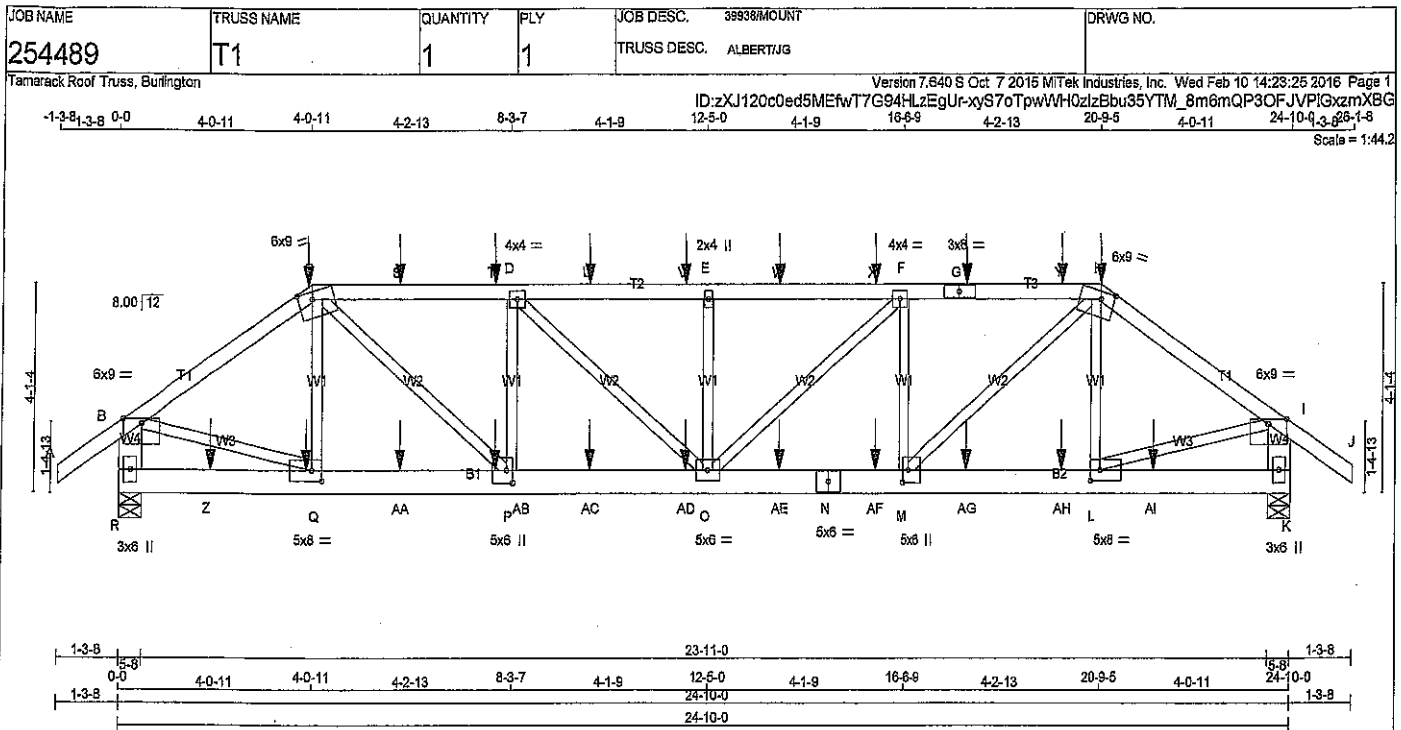
TOTAL BFT OF ALL TRUSSES=

2508.08 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4002.59 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
1	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 3.00



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E
C - G	2x4	DRY	2100F 1.8E
G - H	2x4	DRY	2100F 1.8E
H - J	2x4	DRY	2100F 1.8E
R - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
R - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
R 3272	0	5-8	5-2
K 3203	0	5-8	4-15

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	LIVE PERMLIVE WIND DEAD SOIL
R 2804	1480 / 0 470 / 0 0 / 0 0 / 0 654 / 0 0 / 0
K 2548	1450 / 0 458 / 0 0 / 0 0 / 0 639 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 47	-124.4 -124.4 0.12 (1)	10.00	Q-C	-502 / 35	0.13 (1)	
B-C	-3682 / 0	-124.4 -124.4 0.36 (1)	4.14	C-P	0 / 2383	0.59 (1)	
C-S	-4835 / 0	-124.4 -124.4 0.49 (1)	3.50	P-D	-1380 / 0	0.35 (1)	
S-T	-4835 / 0	-124.4 -124.4 0.49 (1)	3.50	D-O	0 / 882	0.17 (1)	
T-D	-4835 / 0	-124.4 -124.4 0.49 (1)	3.50	O-E	-781 / 0	0.19 (1)	
D-U	-5342 / 0	-124.4 -124.4 0.52 (1)	3.31	O-F	0 / 716	0.18 (1)	
U-V	-5342 / 0	-124.4 -124.4 0.52 (1)	3.31	M-F	-1413 / 0	0.36 (1)	
V-E	-5342 / 0	-124.4 -124.4 0.52 (1)	3.31	M-H	0 / 2434	0.60 (1)	
E-W	-5342 / 0	-124.4 -124.4 0.52 (1)	3.31	L-H	-495 / 24	0.13 (1)	
W-X	-5342 / 0	-124.4 -124.4 0.52 (1)	3.31	B-Q	0 / 3145	0.78 (1)	
X-F	-5342 / 0	-124.4 -124.4 0.52 (1)	3.31	L-I	0 / 3080	0.75 (1)	
F-G	-4911 / 0	-124.4 -124.4 0.49 (1)	3.50				
G-Y	-4911 / 0	-124.4 -124.4 0.49 (1)	3.50				
Y-H	-4911 / 0	-124.4 -124.4 0.49 (1)	3.50				
H-I	-3506 / 0	-124.4 -124.4 0.35 (1)	4.17				
I-J	0 / 47	-124.4 -124.4 0.12 (1)	10.00				
R-B	-3208 / 0	0.0 0.0 0.23 (1)	5.87				
K-I	-3148 / 0	0.0 0.0 0.23 (1)	5.91				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 80.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, CBC 2012, ABC 2014

- CSA 088-09

- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/960 (0.83")

CALCULATED VERT. DEFL.(LL) = 1/999 (0.19")

ALLOWABLE DEFL.(TL) = 1/860 (0.63")

CALCULATED VERT. DEFL.(TL) = 1/953 (0.31")

CSI: TC=0.52 (E-F:1), BC=0.73 (M-O:1), WB=0.78 (B-Q:1), SSI=0.47 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (FLI)

MT20 MAX MIN MAX MIN MAX MIN

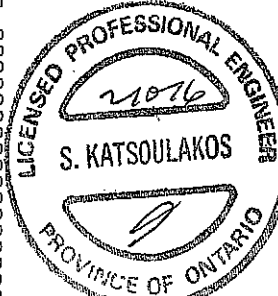
612 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (Q) (INPUT=0.90)

JSI METAL=0.82 (N) (INPUT=1.00)



DWG NO. TAM 2774-18 R1

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Tamarack Roof Truss, Burlington



DWG NO. TAM 2775 -104
STRUCTURAL
COMPONENT ONLY

JOB NAME 254489	TRUSS NAME T3	QUANTITY 1	PLY 1	JOB DESC. 39939/MOUNT	DRWG NO.
				TRUSS DESC. ALBERT/JG	

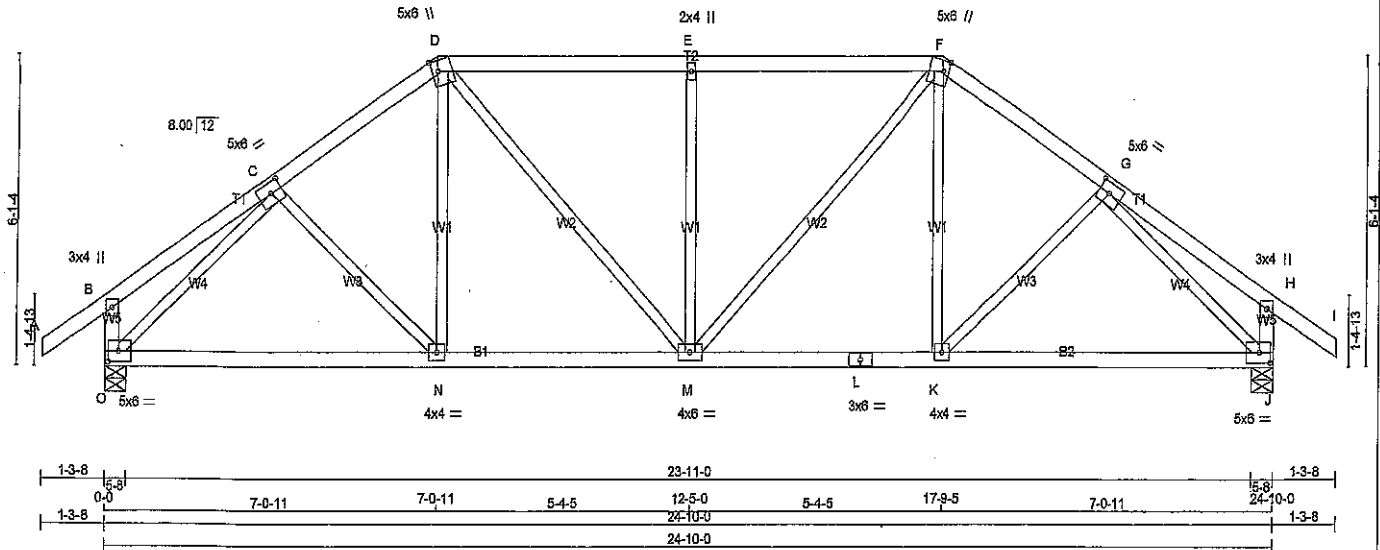
Tamarack Roof Truss, Burlington

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ID:ILZi_p3_WrOkLwnxKQE1YUzEdWU-xyS7oTpwWH0zIzBbu35YTM_8u6qvP1SFJVPiGxmXBG

-1-3-8 0-0 3-7-9 3-7-9 3-5-1 7-0-11 5-4-5 12-5-0 5-4-5 17-9-5 3-5-1 21-2-7 3-7-9 24-10-0 3-8-1-8

Scale = 1:44.2



TOTAL WEIGHT = 107 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	MAXIMUM FACTORED	INPUT	REQD			
	GROSS REACTION	GROSS REACTION	BRG	BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2063	0	2063	0	5-8	2-4	
J	2063	0	2063	0	0	5-8	2-4

UNFACTORED REACTIONS

1ST LCASE MAX/MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1615	959 / 0	281 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1615	959 / 0	281 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 47	-124.4 -124.4	0.17 (1)	10.00	C-N	-19 / 112	0.03 (3)
B-C	0 / 26	-124.4 -124.4	0.23 (1)	10.00	N-D	0 / 313	0.07 (3)
C-D	-2034 / 0	-124.4 -124.4	0.22 (1)	4.55	D-M	0 / 627	0.14 (1)
D-E	-2097 / 0	-124.4 -124.4	0.51 (1)	4.11	M-E	-815 / 0	0.48 (1)
E-F	-2097 / 0	-124.4 -124.4	0.51 (1)	4.11	M-F	0 / 627	0.14 (1)
F-G	-2034 / 0	-124.4 -124.4	0.22 (1)	4.55	K-F	0 / 313	0.07 (3)
G-H	0 / 26	-124.4 -124.4	0.23 (1)	10.00	K-G	-19 / 112	0.03 (3)
H-I	0 / 47	-124.4 -124.4	0.17 (1)	10.00	C-C	-2347 / 0	0.90 (1)
O-B	-343 / 0	0.0	0.04 (1)	7.81	G-J	-2347 / 0	0.90 (1)
J-H	-343 / 0	0.0	0.04 (1)	7.81			
O-N	0 / 1682	-28.0	-28.0	0.49 (2)	10.00		
N-M	0 / 1672	-28.0	-28.0	0.51 (2)	10.00		
M-L	0 / 1672	-28.0	-28.0	0.51 (2)	10.00		
L-K	0 / 1672	-28.0	-28.0	0.51 (2)	10.00		
K-J	0 / 1682	-28.0	-28.0	0.49 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 069-09
- TPC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.51 (D-E), BC=0.51 (K-M), WB=0.50 (C-O), CSI=0.32 (E-F)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.55 (C) (INPUT = 1.00)



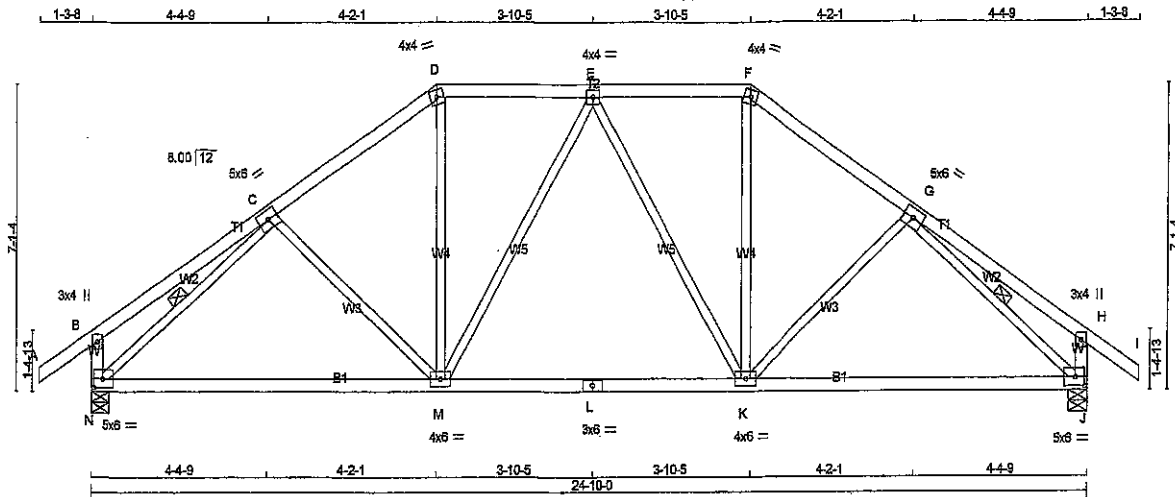
DRG NO. TAM 2776-10 14
STRUCTURAL
COMPONENT ONLY

JOB NAME 254489K	TRUSS NAME KT4	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:53.6

TOTAL WEIGHT = 108 lb (M/F)

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW-t	MT20	5.0	6.0
D	TTW-m	MT20	4.0	4.0
E	TMVW-t	MT20	4.0	4.0
F	TTW-m	MT20	4.0	4.0
G	TMVW-t	MT20	5.0	6.0
H	TMV+p	MT20	3.0	4.0
J	BMVW-t	MT20	5.0	6.0 2.50 2.75
K	BMVW-t	MT20	4.0	6.0
L	BS-t	MT20	3.0	6.0
M	BMVW-t	MT20	4.0	6.0
N	BMVW-t	MT20	5.0	6.0 2.50 2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	2063	0	2063	0	5-8
N	2063	0	2063	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
J	1615	959 / 0	261 / 0	0 / 0	0 / 0	394 / 0	0 / 0	
N	1615	959 / 0	261 / 0	0 / 0	0 / 0	394 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J. DBS = 8-0-0. CBF = 117 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED UNBRACED LENGTH FR-TO
FR-TO			
A-B	0 / 47	-124.4 -124.4 0.17 (1)	10.00
B-C	0 / 33	-124.4 -124.4 0.35 (1)	10.00
C-D	-1939 / 0	-124.4 -124.4 0.32 (1)	4.53
D-E	-1593 / 0	-124.4 -124.4 0.25 (1)	4.98
E-F	-1593 / 0	-124.4 -124.4 0.25 (1)	4.98
F-G	-1939 / 0	-124.4 -124.4 0.32 (1)	4.53
G-H	0 / 33	-124.4 -124.4 0.35 (1)	10.00
H-I	0 / 47	-124.4 -124.4 0.17 (1)	10.00
N-B	-384 / 0	0.0 0.0 0.04 (1)	7.81
J-H	-384 / 0	0.0 0.0 0.04 (1)	7.81
N-M	0 / 1727	-28.0 -28.0 0.62 (2)	10.00
M-L	0 / 1752	-28.0 -28.0 0.63 (2)	10.00
L-K	0 / 1752	-28.0 -28.0 0.63 (2)	10.00
K-J	0 / 1727	-28.0 -28.0 0.62 (2)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/761 (0.39")

CSK TC=0.35 (G-H:1), BC=0.63 (K-M:2), WB=0.58 (G-J:1), SSI=0.23 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.80)
JSI METAL= 0.57 (C) (INPUT = 1.00)



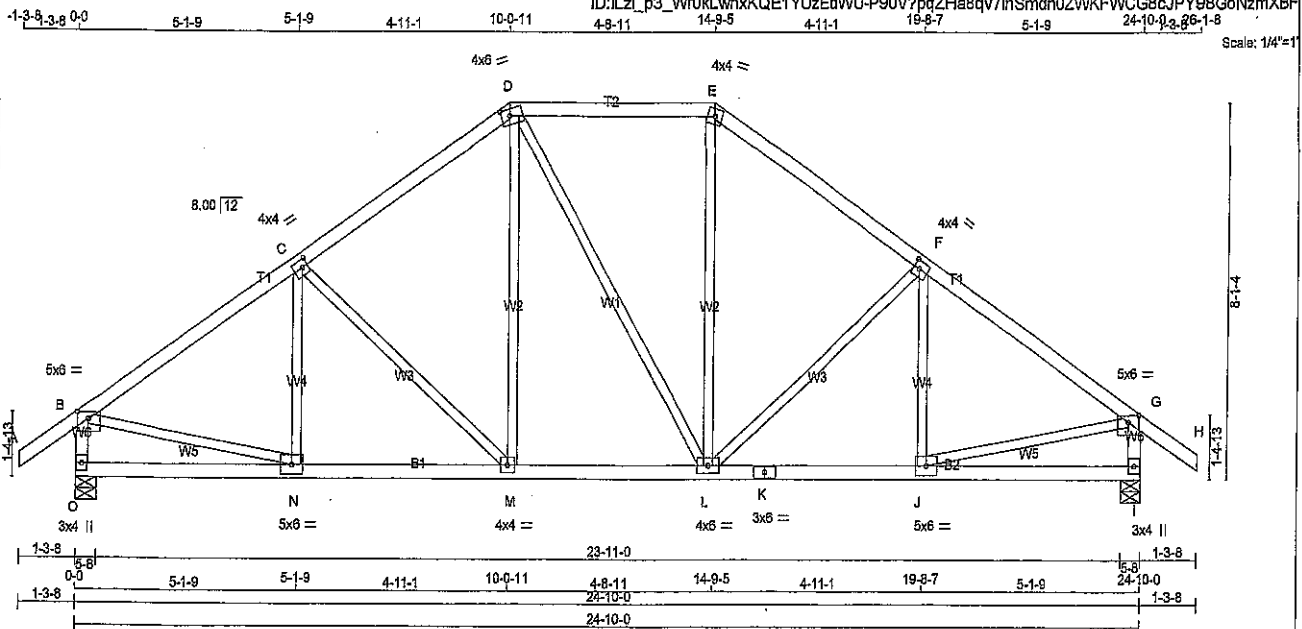
DWG NO. TAM41093-16
STRUCTURAL
COMPONENT ONLY

JOB NAME 254489	TRUSS NAME T5	QUANTITY 1	PLY 1	JOB DESC. 39958/MOUNT	DRWG NO.
				TRUSS DESC. ALBERT/JG	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 112 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED				MAXIMUM FACTORED			
GROSS REACTION				GROSS REACTION			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2083	0	2083	0	0	5-8	3-3
I	2083	0	2083	0	0	5-8	3-3

UNFACTORED REACTIONS							
JT	1ST CASE	MAX/MIN	COMPONENT	REACTIONS	1ST CASE	MAX/MIN	COMPONENT
O	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
I	1615	859 / 0	261 / 0	0 / 0	0 / 0	394 / 0	0 / 0
I	1615	859 / 0	261 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED (LC)
FR-TO				FR-TO			
A-B	0 / 47	-124.4	-124.4 0.17 (1)	10.00	N-C	-224 / 111	0.08 (1)
B-C	-2132 / 0	-124.4	-124.4 0.47 (1)	4.17	C-M	-477 / 0	0.40 (1)
C-D	-1780 / 0	-124.4	-124.4 0.45 (1)	4.50	M-D	0 / 447	0.10 (1)
D-E	-1459 / 0	-124.4	-124.4 0.38 (1)	4.97	D-L	0 / 2	0.00 (1)
E-F	-1791 / 0	-124.4	-124.4 0.45 (1)	4.50	L-E	0 / 449	0.10 (1)
F-G	-2131 / 0	-124.4	-124.4 0.47 (1)	4.17	L-F	-475 / 0	0.40 (1)
G-H	0 / 47	-124.4	-124.4 0.17 (1)	10.00	J-F	-226 / 110	0.08 (1)
O-B	-2002 / 0	0.0	0.0 0.21 (1)	5.97	B-N	0 / 1847	0.42 (1)
I-G	-2002 / 0	0.0	0.0 0.21 (1)	5.97	J-G	0 / 1847	0.42 (1)
O-N	0 / 0	-28.0	-28.0 0.19 (3)	10.00			
N-M	0 / 1807	-28.0	-28.0 0.37 (1)	10.00			
M-L	0 / 1458	-28.0	-28.0 0.30 (1)	10.00			
L-K	0 / 1807	-28.0	-28.0 0.37 (1)	10.00			
K-J	0 / 1807	-28.0	-28.0 0.37 (1)	10.00			
J-I	0 / 0	-28.0	-28.0 0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, ECBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/850 (0.83")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.47 (B-C:1), BC=0.37 (M-N:1), WB=0.42 (B-N:1), SSI=0.25 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1667 622 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

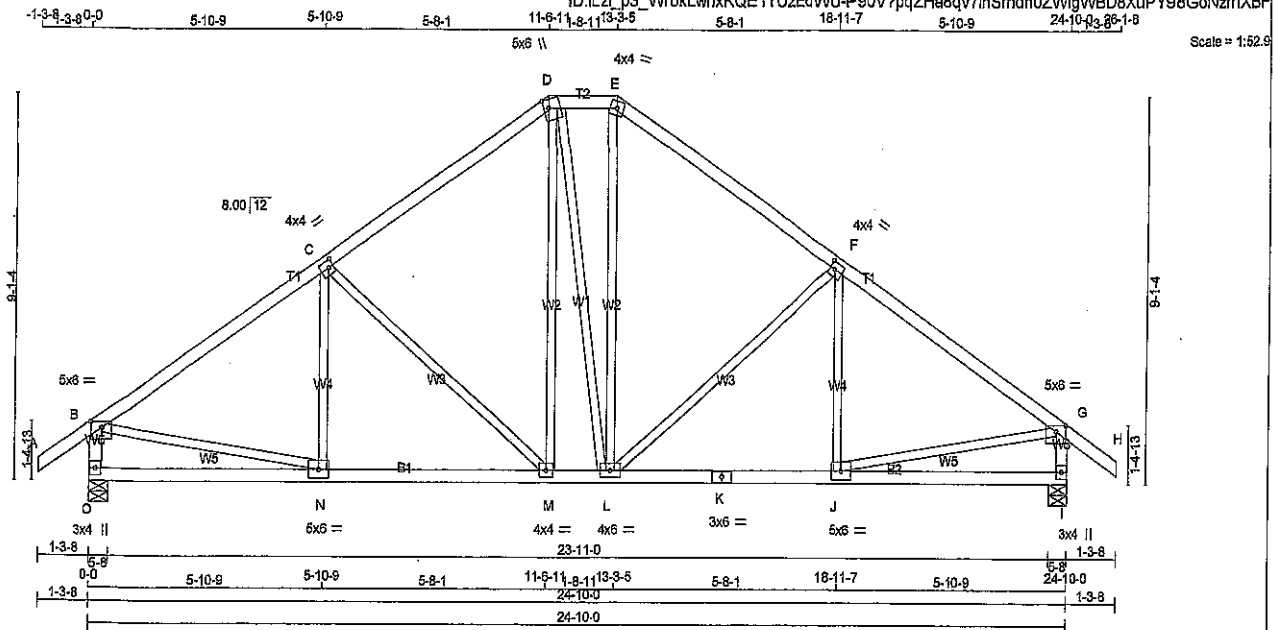
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.48 (K) (INPUT = 1.00)



DWG NO. TAM 2778-1624
STRUCTURAL
COMPONENT ONLY

JOB NAME 254489	TRUSS NAME T6	QUANTITY 1	PLY 1	JOB DESC. 39939/MOUNT	DRWG NO.
Tamarack Roof Truss, Burlington		Version 7.640 S Oct 7 2015 Mitek Industries, Inc. Wed Feb 10 14:23:26 2016 Page 1		ID: Lz_l_p3_Wr0kLwnxKQE1YUzEdWU-P90V7pqZH8qv7InSmdn0ZWigWBD8XuPY98GoNzmXB	



TOTAL WEIGHT = 118 LB [M/F]

<u>LUMBER</u>						
N. L. G. A. RULES						
CHORDS		SIZE	LUMBER		DESCR.	
A - D	2x4	DRY	No.2			SPF
D - E	2x4	DRY	No.2			SPF
E - H	2x4	DRY	No.2			SPF
O - B	2x4	DRY	No.2			SPF
I - G	2x4	DRY	No.2			SPF
O - K	2x4	DRY	No.2			SPF
K - I	2x4	DRY	No.2			SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		IN-SX	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
O	2063	0	2063	0	5-8	3-3			
I	2063	0	2063	0	5-8	3-3			

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
O	1615	959/0	261/0	0/0	0/0
I	1615	959/0	261/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO	FROM	TO	FR-TO
A-B	0/47	-124.4	-124.4
B-C	-2136/0	-124.4	-124.4
C-D	-1646/0	-124.4	-124.4
D-E	-1336/0	-124.4	-124.4
E-F	-1650/0	-124.4	-124.4
F-G	-2134/0	-124.4	-124.4
G-H	0/47	-124.4	-124.4
H-I	-1999/0	0.0	0.0
I-G	-1994/0	0.0	0.0
O-N	0/0	-28.0	-28.0
N-M	0/1816	-28.0	-28.0
M-L	0/1333	-28.0	-28.0
L-K	0/1815	-28.0	-28.0
K-J	0/1815	-28.0	-28.0
J-I	0/0	-28.0	-28.0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.64 (B-C:1), BC=0.44 (M-N:2), WB=0.76 (C-M:1), SSI=0.29 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

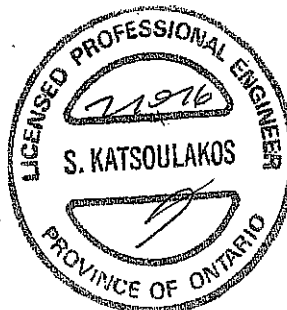
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (N) (INPUT = 0.90)
JSI METAL= 0.55 (K) (INPUT = 1.00)

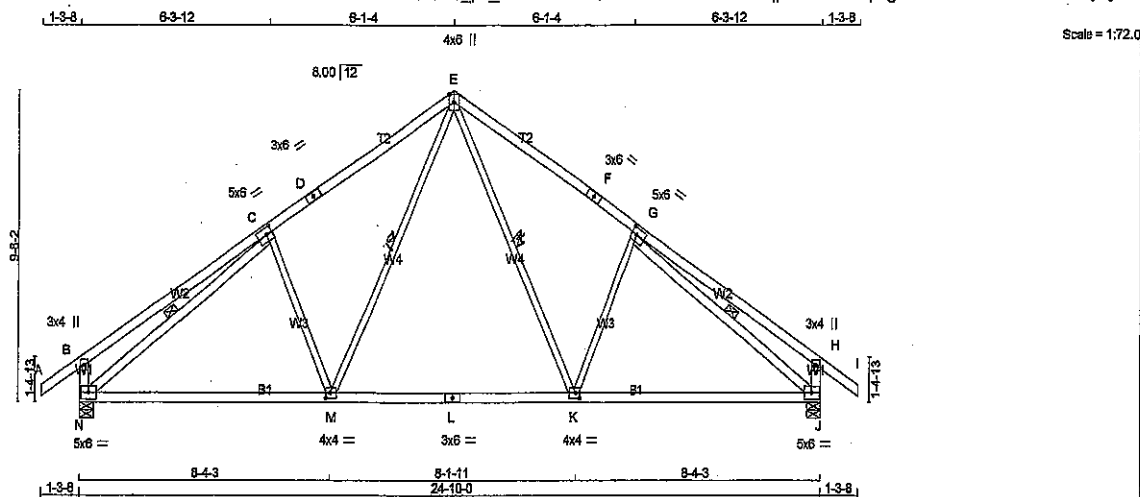


DRWG NO. **TAM 2779-16 R4**
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 8.000 S Jan 15 2016 MITek Industries, Inc. Tue Dec 06 11:07:03 2016 Page 1

ID: ILZl_p3_Wr0kLwnxKQE1YUzEdWU-3dbC2rLqptXWVWZ2b9p4g3MVAHNbOaBFCnIdAM3yBJNM



TOTAL WEIGHT = 15 X 111 = 1665 lb

LUMBER **CHORDS** **SIZE** **LUMBER** **DESCR.** **DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER** **BEARINGS** **DESIGN CRITERIA**

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - E	2x4	DRY	No.2 SPF
E - F	2x4	DRY	No.2 SPF
F - I	2x4	DRY	No.2 SPF
N - B	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
N - L	2x4	DRY	No.2 SPF
L - J	2x4	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
N	2083	0	2083	0	5-8	2-4
J	2083	0	2083	0	5-8	2-4

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
N	1615	959 / 0	261 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1615	959 / 0	261 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J. DBS = 8-0-0. CBF = 117 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX. CSI (LC)	
FR-TO		FROM TO				FR-TO			
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00	E-K	0 / 820	0.18 (1)	
B-C	0 / 52	-124.4	-124.4	0.80 (1)	10.00	K-G	-546 / 0	0.30 (1)	
C-D	-1943 / 0	-124.4	-124.4	0.72 (1)	3.92	M-E	0 / 820	0.18 (1)	
D-E	-1943 / 0	-124.4	-124.4	0.72 (1)	3.92	C-M	-546 / 0	0.30 (1)	
E-F	-1943 / 0	-124.4	-124.4	0.72 (1)	3.92	N-C	-2339 / 0	0.73 (1)	
F-G	-1943 / 0	-124.4	-124.4	0.72 (1)	3.92	G-J	-2339 / 0	0.73 (1)	
G-H	0 / 52	-124.4	-124.4	0.80 (1)	10.00				
H-I	0 / 47	-124.4	-124.4	0.17 (1)	10.00				
N-B	-460 / 0	0.0	0.0	0.05 (1)	7.81				
J-H	-460 / 0	0.0	0.0	0.05 (1)	7.81				
N-M	0 / 1789	-28.0	-28.0	0.63 (2)	10.00				
M-L	0 / 1250	-28.0	-28.0	0.59 (3)	10.00				
L-K	0 / 1250	-28.0	-28.0	0.59 (3)	10.00				
K-J	0 / 1789	-28.0	-28.0	0.63 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	34.8	PSF
DL	=	8.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	60.3	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C86-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.83")
CALCULATED VERT. DEFL.(LL) = $L/959$ (0.20")
ALLOWABLE DEFL.(TL) = $L/360$ (0.83")
CALCULATED VERT. DEFL.(TL) = $L/883$ (0.34")

CSI: TC=0.80 (G-H:1), BC=0.63 (M-N:2), WB=0.73 (G-J:1), SSI=0.31 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.81 (C) (INPUT = 1.00)



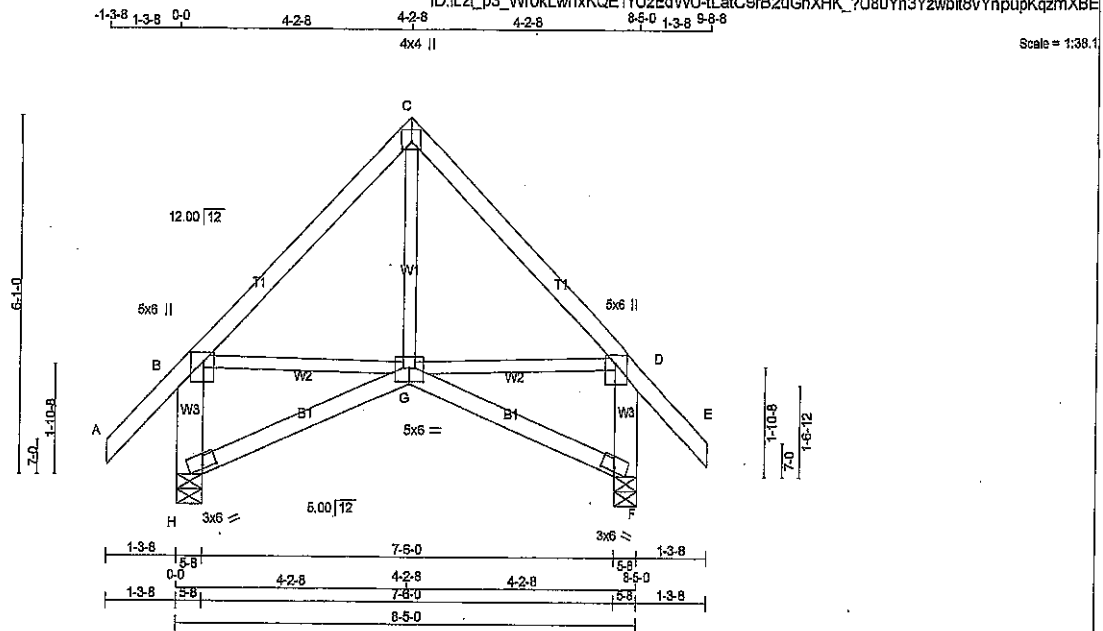
DRG NO. TAM 53662-10
STRUCTURAL
COMPONENT ONLY

JOB NAME 254489	TRUSS NAME T8	QUANTITY 3	PLY 1	JOB DESC. 39938/MOUNT	TRUSS DESC. ALBERT/JG	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 44 = 132 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x6	DRY	No.2
F - D	2x6	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED		MAXIMUM FACTORED		INPUT		REQD
	GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UP/LIFT	IN-SX	IN-SX
JT	VERT	HORIZ					
H	815	0	815	0	0	5-8	1-8
F	815	0	815	0	0	5-8	1-8

UNFACTORED REACTIONS							
	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	627	380 / 0	88 / 0	0 / 0	0 / 0	149 / 0	0 / 0
F	627	380 / 0	88 / 0	0 / 0	0 / 0	149 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED	VERT. LOAD	MAX	MEMB.	FACTORED	VERT. LOAD	MAX
	FORCE (LBS)	FROM TO	CS1 (LC)		FORCE (LBS)	FROM TO	CS1 (LC)
FR-TO				FR-TO			
A-B	0 / 61	-124.4 -124.4	0.17 (1)	10.00	G-C	0 / 212	0.05 (3)
B-C	-459 / 0	-124.4 -124.4	0.28 (1)	6.25	B-G	0 / 325	0.07 (1)
C-D	-459 / 0	-124.4 -124.4	0.28 (1)	6.25	G-D	0 / 325	0.07 (1)
D-E	0 / 61	-124.4 -124.4	0.17 (1)	10.00			
H-B	-756 / 0	0.0 0.0	0.06 (1)	7.81			
F-D	-756 / 0	0.0 0.0	0.06 (1)	7.81			
H-G	0 / 0	-28.0 -28.0	0.17 (3)	10.00			
G-F	0 / 0	-28.0 -28.0	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= 1/360 (0.28")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL)= 1/360 (0.28")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CSI: TC=0.28 (C-D:1), BC=0.17 (G-H:3), WB=0.07 (B-G:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.80)
JSI METAL= 0.14 (B) (INPUT = 1.00)



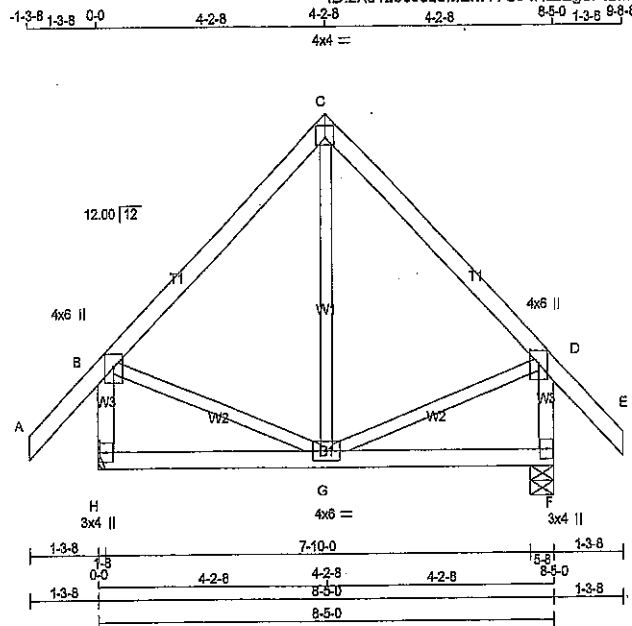
DRWG NO. TAM 2701-1672
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	39938/MOUNT	DRWG NO.
254489	T9	2	1	TRUSS DESC.	ALBERT/JG	

Tamarack Roof Truss, Burlington

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Scale = 1:38.8

TOTAL WEIGHT = 2 X 42 = 85 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	6.0 2.75 2.00
C	TTW+p	MT20	4.0	4.0 1.50 2.00
D	TMVW+p	MT20	4.0	6.0 2.75 2.00
F	BMV1+p	MT20	3.0	4.0
G	BMVWW+t	MT20	4.0	6.0
H	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	815	0	815	0	0
F	815	0	815	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	627	390 / 0	88 / 0
F	627	390 / 0	88 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX. (PLF)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 81	-124.4	-124.4 0.17 (1)	G-C	-33 / 168
B-C	-347 / 0	-124.4	-124.4 0.28 (1)	B-G	0 / 281
C-D	-347 / 0	-124.4	-124.4 0.28 (1)	G-D	0 / 281
D-E	0 / 81	-124.4	-124.4 0.17 (1)		
H-B	-769 / 0	0.0	0.0 0.09 (1)		
F-D	-769 / 0	0.0	0.0 0.09 (1)		
H-G	0 / 0	-28.0	-28.0 0.16 (3)		
G-F	0 / 0	-28.0	-28.0 0.16 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.28")
CALCULATED VERT. DEFL.(LL) = 1/899 (0.01")
ALLOWABLE DEFL.(TL) = 1/360 (0.28")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.02")

CSI: TC=0.28 (C-D:1), BC=0.16 (G-H:3), WB=0.06 (B-G:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

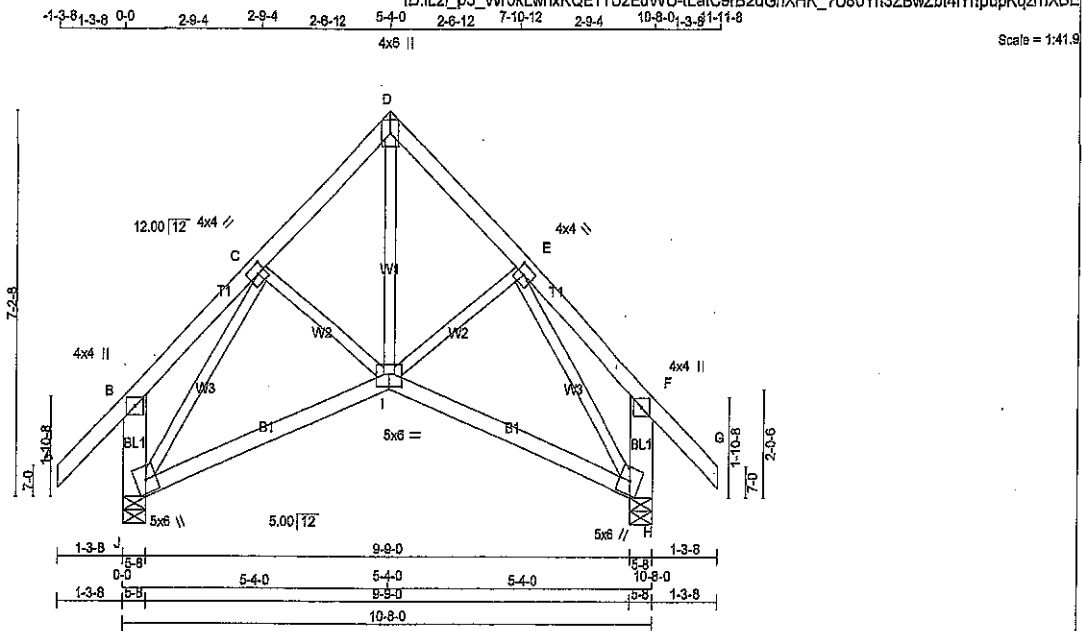
JSI GRIP = 0.74 (C) (INPUT = 0.80)
JSI METAL = 0.13 (D) (INPUT = 1.00)



DRWG NO. YAM 2782-16 (2)
STRUCTURAL
COMPONENT ONLY

JOB NAME 254489	TRUSS NAME T10	QUANTITY 3	PLY 1	JOB DESC. 39938/MOUNT	TRUSS DESC. ALBERT/JG	DRWG NO.
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TOTAL WEIGHT = 3 X 57 = 172 LB [M/F]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

BEARING BLOCKS			
BL1	2x6	DRY	No.2
BL1	2x6	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMK+p	MT20	4.0	4.0		
C	TMWV-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0		
E	TMWV-t	MT20	4.0	4.0	2.00	1.75
F	TMK+p	MT20	4.0	4.0		
H	BWKM1+m	MT20	5.0	6.0		
I	BBWVW+p	MT20	5.0	6.0	2.75	3.00
J	BWKM1+m	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	986	0	986	0	1-8
H	986	0	986	0	1-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
J	782	488 / 0	112 / 0	0 / 0
H	782	488 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 / 87	-124.4	-124.4	I-D	0 / 637	0.14	(1)
B-C	-9 / 0	-124.4	-124.4	I-E	-58 / 37	0.01	(1)
C-D	-668 / 0	-124.4	-124.4	C-I	-58 / 37	0.01	(1)
D-E	-668 / 0	-124.4	-124.4	J-C	-918 / 0	0.34	(1)
E-F	-9 / 0	-124.4	-124.4	E-H	-918 / 0	0.34	(1)
F-G	0 / 67	-124.4	-124.4				
J-B	-338 / 0	0.0	0.0				
H-F	-338 / 0	0.0	0.0				
J-I	0 / 542	-28.0	-28.0				
I-H	0 / 542	-28.0	-28.0				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C86-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/998 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.21 (F-G:1), BC=0.30 (H-I:3), WB=0.34 (C-J:1), SSI=0.11 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1657
		622	2284
			1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

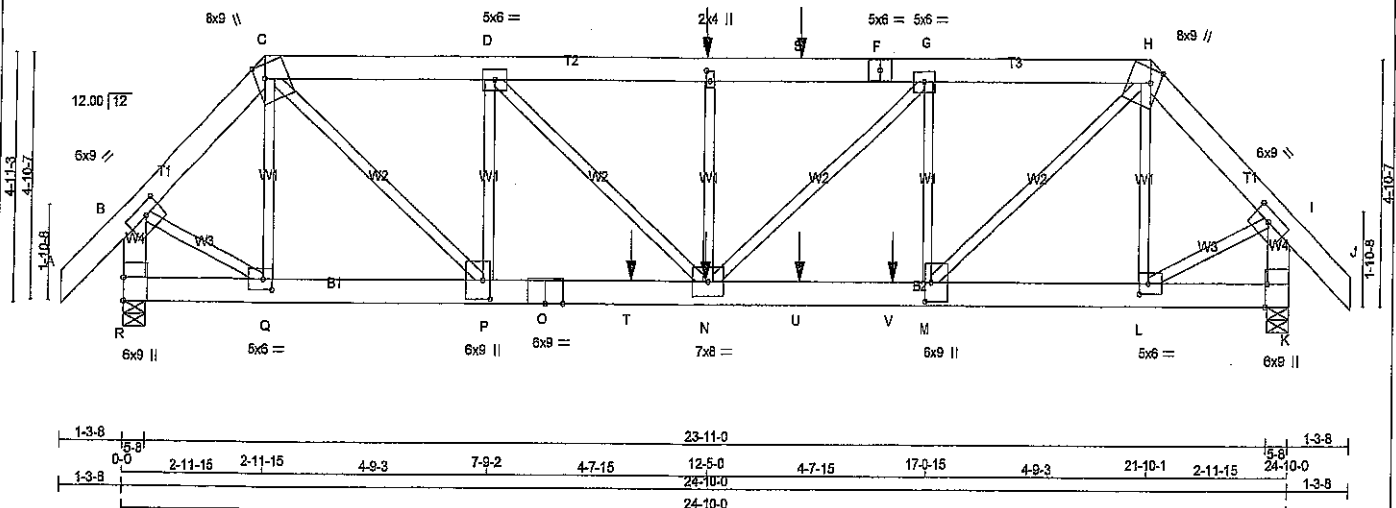


DWG NO. FAM 2783.16 (2)
STRUCTURAL
COMPONENT ONLY

JOB NAME 254489	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. 3938/MOUNT	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. ALBERTAUG	

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IDzXJ120c0ed5MEfwT7G94HLzEgUr-LX8FQVspCOY9RvAZBf5_chNKIdcOFI7TdMsGzmXBD
-1-3-8 1-3-8 0-0 2-11-15 2-11-15 4-9-3 7-9-2 4-7-15 12-5-0 4-7-15 17-9-15 4-9-3 21-10-1 2-11-15 24-10-9 3-8-5-1-8
Scale = 1:44.4



TOTAL WEIGHT = 148 lb [M]

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x6	DRY	No.2 SPF
C - F	2x6	DRY	No.2 SPF
F - H	2x6	DRY	No.2 SPF
H - J	2x6	DRY	No.2 SPF
R - S	2x6	DRY	No.2 SPF
K - I	2x6	DRY	No.2 SPF
R - O	2x6	DRY	1650F 1.5E SPF
O - K	2x6	DRY	1650F 1.5E SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-t	MT20	6.0	9.0 2.50 4.00
C	TTWW+m	MT20	8.0	9.0 Edge 2.25
D	TMVW-t	MT20	5.0	6.0
E	TMVW-w	MT20	2.0	4.0 2.50 1.00
F	TS-t	MT20	5.0	6.0
G	TMVW-t	MT20	5.0	6.0
H	TTWW+m	MT20	8.0	9.0 Edge 2.25
I	TMVW-t	MT20	6.0	9.0 2.50 4.00
K	BMV1-t	MT20	6.0	9.0 Edge 0.50
L	BMVW-t	MT20	5.0	6.0 2.50 2.25
M	BMVW-t	MT20	6.0	9.0 4.50 1.75
N	BMVW-t	MT20	7.0	8.0
O	BS-t	MT20	8.0	9.0
P	BMVW-t	MT20	6.0	9.0 4.50 1.75
Q	BMVW-t	MT20	5.0	6.0 2.50 2.25
R	BMV1-t	MT20	6.0	9.0 5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 116.8 lbs FACTORED DOWN AT 12-4-4, AND 116.8 lbs FACTORED DOWN AT 14-4-4 ON TOP CHORD, AND 1287.1 lbs FACTORED DOWN AT 10-8-8, 42.0 lbs FACTORED DOWN AT 12-4-4, AND 42.0 lbs FACTORED DOWN AT 14-4-4, AND 1003.1 lbs FACTORED DOWN AT 16-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
R	3275	0	3275	0	0	5-8	5-2	5-8	5-2
K	3442	0	3442	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	VERT	COMBINED	SNOW	VERT	COMBINED	SNOW	VERT	COMBINED	SNOW	VERT	COMBINED	SNOW	VERT
R	2532	1655/0	372/0	0/0	0/0	605/0	0/0	0/0	605/0	0/0	0/0	605/0	0/0	0/0	0/0
K	2657	1638/0	385/0	0/0	0/0	633/0	0/0	0/0	633/0	0/0	0/0	633/0	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. (LC)
FR-TO				FR-TO			
A-B	0/84	-124.4	-124.4 0.10 (1)	Q-C	-791/0	0.25 (1)	0.85 (1)
B-C	-2639/0	-124.4	-124.4 0.15 (1)	C-P	0/3451	0.85 (1)	0.85 (1)
C-D	-4517/0	-124.4	-124.4 0.33 (1)	P-D	-1745/0	0.55 (1)	0.55 (1)
D-E	-5722/0	-124.4	-124.4 0.37 (1)	D-N	0/1658	0.41 (1)	0.41 (1)
E-S	-5722/0	-124.4	-124.4 0.43 (1)	N-E	-718/0	0.23 (1)	0.23 (1)
S-F	-5722/0	-124.4	-124.4 0.43 (1)	N-G	0/1302	0.32 (1)	0.32 (1)
F-G	-5722/0	-124.4	-124.4 0.43 (1)	G-M	-1560/0	0.49 (1)	0.49 (1)
G-H	-4776/0	-124.4	-124.4 0.33 (1)	M-H	0/3928	0.80 (1)	0.80 (1)
H-I	-3022/0	-124.4	-124.4 0.15 (1)	L-H	-777/0	0.25 (1)	0.25 (1)
I-J	0/64	-124.4	-124.4 0.10 (1)	E-Q	0/2168	0.54 (1)	0.54 (1)
R-B	-3240/0	0.0	0.0 0.24 (1)	L-I	0/2305	0.57 (1)	0.57 (1)
K-I	-3425/0	0.0	0.0 0.25 (1)				
R-Q	0/0	-28.0	-28.0 0.04 (3)				
Q-P	0/1973	-28.0	-28.0 0.31 (1)				
P-O	0/4516	-28.0	-28.0 0.86 (1)				
O-T	0/4516	-28.0	-28.0 0.86 (1)				
T-N	0/4516	-28.0	-28.0 0.86 (1)				
N-U	0/4776	-28.0	-28.0 0.65 (1)				
U-V	0/4776	-28.0	-28.0 0.65 (1)				
V-M	0/4776	-28.0	-28.0 0.65 (1)				
M-L	0/2101	-28.0	-28.0 0.23 (1)				
L-K	0/0	-28.0	-28.0 0.05 (2)				

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE
E	12-4-4	-117	-117	—	BACK	VERT	TOTAL
N	12-4-4	-24	-42	—	BACK	VERT	TOTAL
S	14-4-4	-117	-117	—	BACK	VERT	TOTAL
T	10-8-8	-1287	-1287	—	BACK	VERT	TOTAL
U	14-4-4	-24	-42	—	BACK	VERT	TOTAL
V	16-3-8	-1003	-1003	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.83")
CALCULATED VERT. DEFL.(LL) = L/959 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/927 (0.32")

CSI: TC=0.43 (E-G:1), BC=0.66 (N-P:1), WB=0.80 (H-M:1), SSI=0.63 (N-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

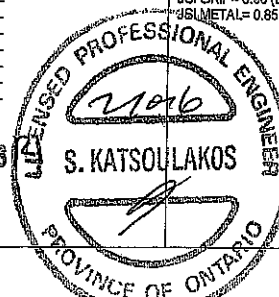
NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(FSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818	354	1687
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.90 (B) (INPUT = 0.90)
JSI METAL=0.85 (C) (INPUT = 1.00)

DWG NO. TAM 2784-16
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	39939/MOUNT	DRWG NO.
254503	T11Z	1	1	TRUSS DESC.	ALBERT/JG	

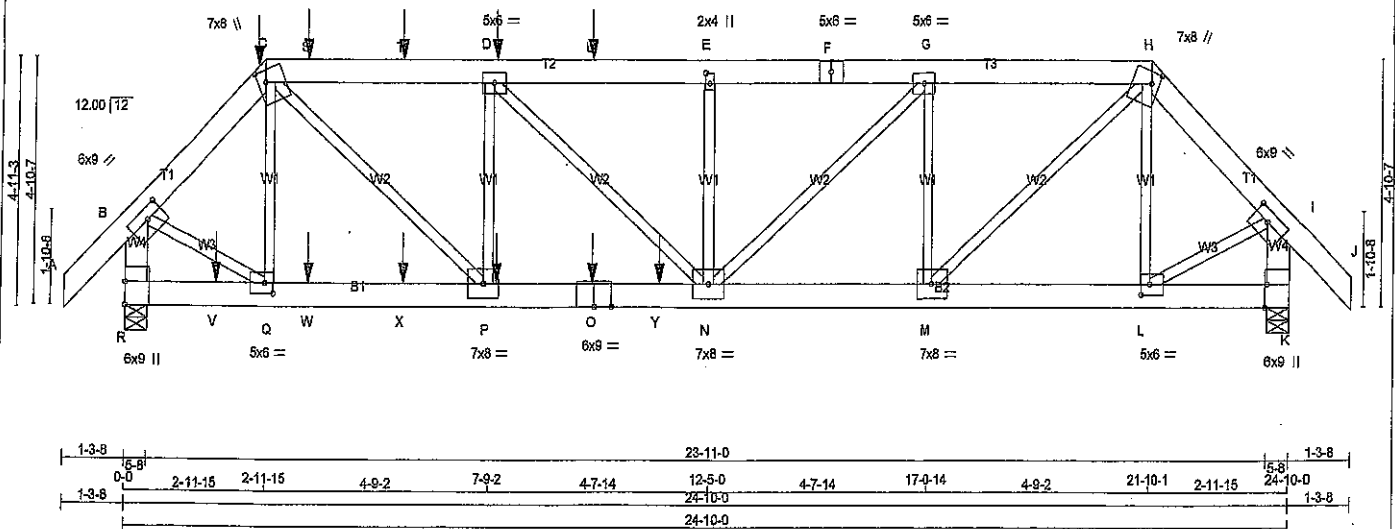
Tamarack Roof Truss, Burlington

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1-3-8 2-11-15 2-11-15 4-9-2 7-9-2 4-7-14 12-5-0 4-7-14 17-0-14 4-9-2 21-10-1 2-11-15 24-10-0 1-3-8

Scale = 1:44.4



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - O	2x6	DRY	1650F 1.5E	SPF
O - K	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMWW-t	MT20	6.0	9.0	2.50 4.00
C TTWW+m	MT20	7.0	8.0	2.75 2.00
D TMWW-t	MT20	5.0	6.0	
E TMWW-t	MT20	2.0	4.0	2.50 1.00
F TS-t	MT20	5.0	5.0	
G TMWW-t	MT20	5.0	5.0	
H TTWW+m	MT20	7.0	8.0	2.75 2.00
I TMWW-t	MT20	6.0	9.0	2.50 4.00
K BMV1-t	MT20	6.0	9.0	Edge 0.50
L BMWW-t	MT20	5.0	6.0	2.50 2.25
M BMWW-t	MT20	7.0	8.0	
N BMWWW-t	MT20	7.0	8.0	
O BS-t	MT20	6.0	9.0	
P BMWW-t	MT20	7.0	8.0	
Q BMWW-t	MT20	5.0	6.0	2.50 2.25
R BMV1-t	MT20	6.0	9.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 202.2 lbs FACTORED DOWN AT 2-11-15, 116.6 lbs FACTORED DOWN AT 3-11-4, 116.6 lbs FACTORED DOWN AT 5-11-4, AND 116.6 lbs FACTORED DOWN AT 7-11-4, AND 116.6 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, 42.0 lbs FACTORED DOWN AT 3-11-4, 42.0 lbs FACTORED DOWN AT 5-11-4, 42.0 lbs FACTORED DOWN AT 7-11-4, AND 1363.5 lbs FACTORED DOWN AT 11-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	3412	0	3412	0	5-8
K	2875	0	2875	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX./MIN. COMPONENT REACTIONS		
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
R	2886	1592 / 0	425 / 0	0 / 0	649 / 0
K	2237	1351 / 0	346 / 0	0 / 0	541 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 84	Q-C	-758 / 0
B-C	-2876 / 0	C-P	0 / 3084
C-S	-4343 / 0	P-D	-1584 / 0
S-T	-4343 / 0	D-N	0 / 999
T-D	-4343 / 0	N-E	-553 / 0
D-U	-5025 / 0	N-G	0 / 1783
U-E	-5025 / 0	M-G	-1855 / 0
E-F	-5025 / 0	M-H	0 / 2722
F-G	-5025 / 0	L-H	-544 / 0
G-H	-3730 / 0	B-Q	0 / 2269
H-I	-2476 / 0	L-I	0 / 1888
I-J	0 / 84		
R-B	-3378 / 0		
K-I	-2872 / 0		
R-V	0 / 0		
V-Q	0 / 0		
Q-W	0 / 2069		
W-X	0 / 2069		
X-P	0 / 2069		
P-O	0 / 4343		
O-Y	0 / 4343		
Y-N	0 / 4343		
N-M	0 / 3730		
M-L	0 / 1724		
L-K	0 / 0		

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	FACE	DIR.
JT						
C	2-11-15	-202	-202	---	FRONT	VERT
D	7-11-4	-117	-117	---	BACK	VERT
O	9-11-4	-24	-24	---	BACK	VERT
P	7-11-4	-24	-24	---	BACK	VERT
S	3-11-4	-117	-117	---	BACK	VERT
T	5-11-4	-117	-117	---	BACK	VERT
U	9-11-4	-117	-117	---	BACK	VERT
V	1-11-4	-24	-24	---	BACK	VERT
W	3-11-4	-24	-24	---	BACK	VERT
X	5-11-4	-24	-24	---	BACK	VERT
Y	11-4-8	-1354	-1354	---	BACK	VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/380 (0.83")
CALCULATED VERT. DEFL.(LL)= L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/380 (0.83")
CALCULATED VERT. DEFL.(TL)= L/999 (0.29")

CSI: TC=0.42 (D-E:1), BC=0.78 (N-P:1), WB=0.76 (C-P:1), SSF=0.77 (N-P:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

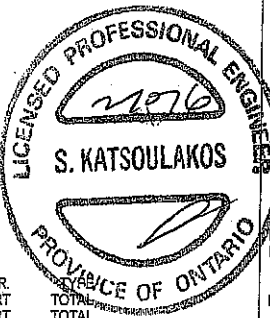
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

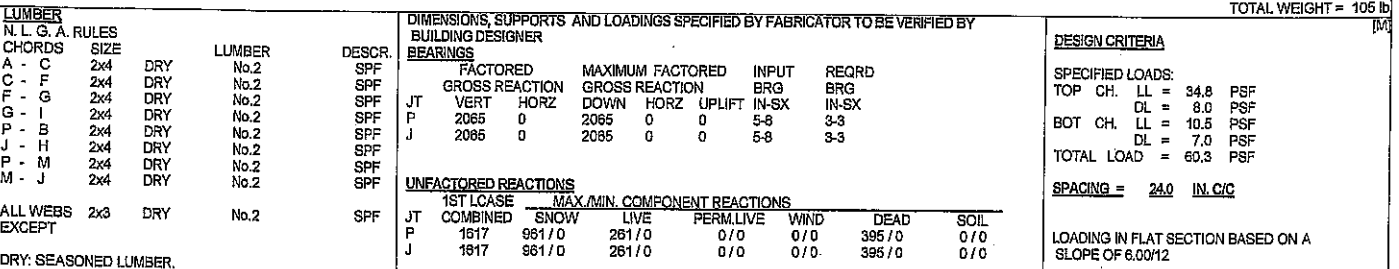
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.80)
JSI METAL= 0.83 (C) (INPUT = 1.00)



DRWG NO. TAN 2790-16 R1
STRUCTURAL
COMPONENT ONLY

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ID: ilzl_p3_Wr0kLwnxKQE1YUzEdWU-LX8FQVspCOY9RvAZB/F5_cZKvWcWAI?TdMsGzmXBD
-1-3-8-1-3-8 0-0 2-11-15 2-11-15 6-3-13 9-3-12 6-2-9 15-6-4 6-3-13 21-10-1 2-11-15 24-10-4 3-6-18
Scale = 1:44.4



Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0/61	-124.4	-124.4	0.17 (1)	10.00	Q-C	-398/4	0.14 (1)	
B-C	-1597/0	-124.4	-124.4	0.23 (1)	4.89	C-N	0/1740	0.39 (1)	
C-D	-2526/0	-124.4	-124.4	0.92 (1)	3.19	N-D	-842/0	0.29 (1)	
D-E	-2522/0	-124.4	-124.4	0.91 (1)	3.20	D-L	-5/0	0.01 (1)	
E-F	-2523/0	-124.4	-124.4	0.90 (1)	3.25	L-E	-938/0	0.29 (1)	
F-G	-2523/0	-124.4	-124.4	0.90 (1)	3.25	L-G	0/1734	0.39 (1)	
G-H	-1597/0	-124.4	-124.4	0.23 (1)	4.89	K-G	-397/4	0.14 (1)	
H-I	0/61	-124.4	-124.4	0.17 (1)	10.00	B-O	0/1252	0.28 (1)	
P-B	-2045/0	0.0	0.0	0.23 (1)	5.91	K-H	0/1252	0.28 (1)	
J-H	-2047/0	0.0	0.0	0.23 (1)	5.91				
P-O	0/0	-28.0	-28.0	0.18 (3)	10.00				
O-N	0/1114	-28.0	-28.0	0.33 (2)	10.00				
N-M	0/2526	-28.0	-28.0	0.51 (1)	10.00				
M-L	0/2526	-28.0	-28.0	0.51 (1)	10.00				
L-K	0/1114	-28.0	-28.0	0.33 (2)	10.00				
K-J	0/0	-28.0	-28.0	0.18 (3)	10.00				

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.83")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.83")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.20")

CSI: TC=0.92 (C-D:1), BC=0.51 (L-N:1), WB=0.39 (C-N:1), SSI=0.37 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

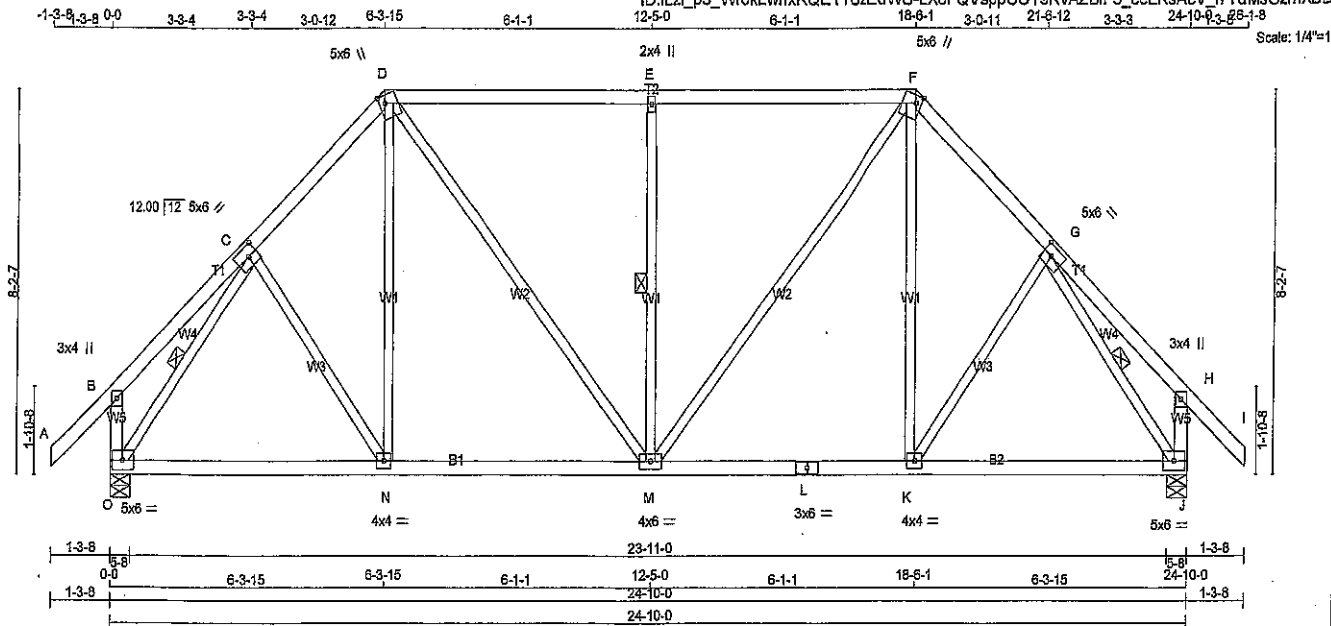
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.68 (M) (INPUT = 1.00)



DWG NO. TAM 2785-16R4
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 122 = 245 lb (M/F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	8.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
O	2065	0	2065	0	5-8	2-4
J	2065	0	2065	0	5-8	2-4

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
O	1617	961/0	261/0	0/0	0/0	395/0	0/0
J	1617	961/0	261/0	0/0	0/0	395/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M. DBS = 20-0-0. CBF = 116 LBS.
 1 - 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-J. DBS = 8-0-0. CBF = 100 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x6, 3 FOR 2x8, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/61	-124.4 -124.4	0.17 (1)	C-N	0/110	0.03 (3)	
B-C	0/30	-124.4 -124.4	0.19 (1)	N-D	0/267	0.08 (3)	
C-D	-1657/0	-124.4 -124.4	0.17 (1)	D-M	0/679	0.15 (1)	
D-E	-1562/0	-124.4 -124.4	0.63 (1)	M-E	-925/0	0.39 (1)	
E-F	-1562/0	-124.4 -124.4	0.63 (1)	F-F	0/879	0.15 (1)	
F-G	-1657/0	-124.4 -124.4	0.17 (1)	K-F	0/267	0.08 (3)	
G-H	0/30	-124.4 -124.4	0.19 (1)	K-G	0/110	0.03 (3)	
H-I	0/61	-124.4 -124.4	0.17 (1)	O-C	-2010/0	0.47 (1)	
O-B	-325/0	0.0	0.04 (1)	G-J	-2010/0	0.47 (1)	
J-H	-325/0	0.0	0.04 (1)				
O-N	0/1121	-28.0 -28.0	0.40 (2)				
N-M	0/1148	-28.0 -28.0	0.41 (2)				
M-L	0/1148	-28.0 -28.0	0.41 (2)				
L-K	0/1148	-28.0 -28.0	0.41 (2)				
K-J	0/1121	-28.0 -28.0	0.40 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.83")
 CALCULATED VERT. DEFL.(LL) = 1/699 (0.07")
 ALLOWABLE DEFL.(TL) = 1/360 (0.83")
 CALCULATED VERT. DEFL.(TL) = 1/699 (0.12")

CSI: TC=0.63 (D-E-1), BC=0.41 (M-N-2), WB=0.47 (C-O-1), SSI=0.37 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

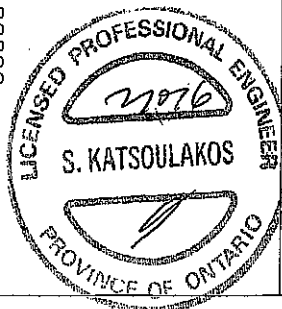
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

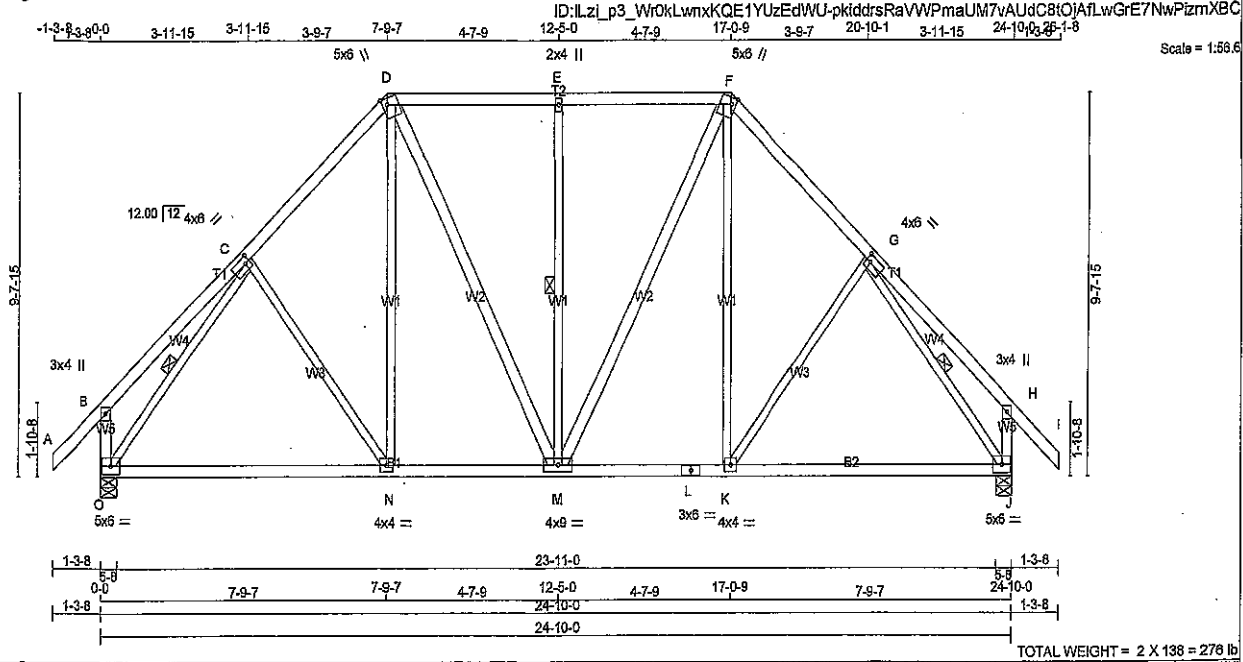
JSI GRIP = 0.89 (C) (INPUT = 0.90)
 JSI METAL = 0.49 (C) (INPUT = 1.00)



DRWG NO. TAW 278718 R4
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 254489	TRUSS NAME T15	QUANTITY 2	PLY 1	JOB DESC. 39938/MOUNT	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC. ALBERTAUG	

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TOTAL WEIGHT = 2 X 138 = 276 lb (M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
D - M	2x4	DRY	No.2	SPF	
M - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	1.50
D	TTVW+m	MT20	5.0	6.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTVW+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	6.0	2.00	1.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
O 2065 0	2065 0	5-8	2-4
J 2065 0	2065 0	5-8	2-4

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
O	1617	981/0 281/0 0/0 0/0 395/0 0/0
J	1617	981/0 281/0 0/0 0/0 395/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1- 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M. DBS = 20-0-0. CBF = 87 LBS.
1- 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-J. DBS = 60-0-0. CBF = 100 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x6, 3 FOR 2x8, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/61	-124.4 -124.4	0.17 (1)	C-N	-103/66	0.09 (1)	
B-C	0/40	-124.4 -124.4	0.30 (1)	D-M	0/398	0.09 (3)	
C-D	-1598/0	-124.4 -124.4	0.27 (1)	E-F	0/418	0.07 (1)	
D-E	-1286/0	-124.4 -124.4	0.35 (1)	F-G	-866/0	0.43 (1)	
E-F	-1286/0	-124.4 -124.4	0.35 (1)	G-H	0/418	0.07 (1)	
F-G	-1598/0	-124.4 -124.4	0.27 (1)	H-I	0/398	0.09 (3)	
G-H	0/40	-124.4 -124.4	0.30 (1)	I-J	-103/66	0.09 (1)	
H-I	0/61	-124.4 -124.4	0.17 (1)	J-K	-1980/0	0.59 (1)	
O-B	-357/0	0.0 0.0	0.04 (1)	K-L	-1980/0	0.59 (1)	
J-H	-357/0	0.0 0.0	0.04 (1)				
O-N	0/1159	-28.0 -28.0	0.51 (3)				
N-M	0/1101	-28.0 -28.0	0.52 (3)				
M-L	0/1101	-28.0 -28.0	0.52 (3)				
L-K	0/1101	-28.0 -28.0	0.52 (3)				
K-J	0/1159	-28.0 -28.0	0.51 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.83")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.16")
ALLOWABLE DEFL. (TL) = 1/390 (0.83")
CALCULATED VERT. DEFL. (TL) = 1/599 (0.26")

CSI: TC=0.35 (E-F:1), BC=0.52 (M-N:3), WB=0.59 (C-C:1), SS=0.28 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

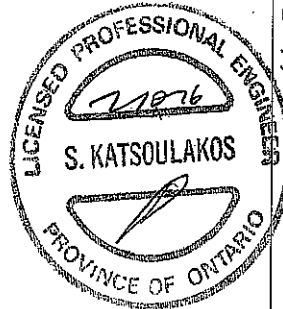
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

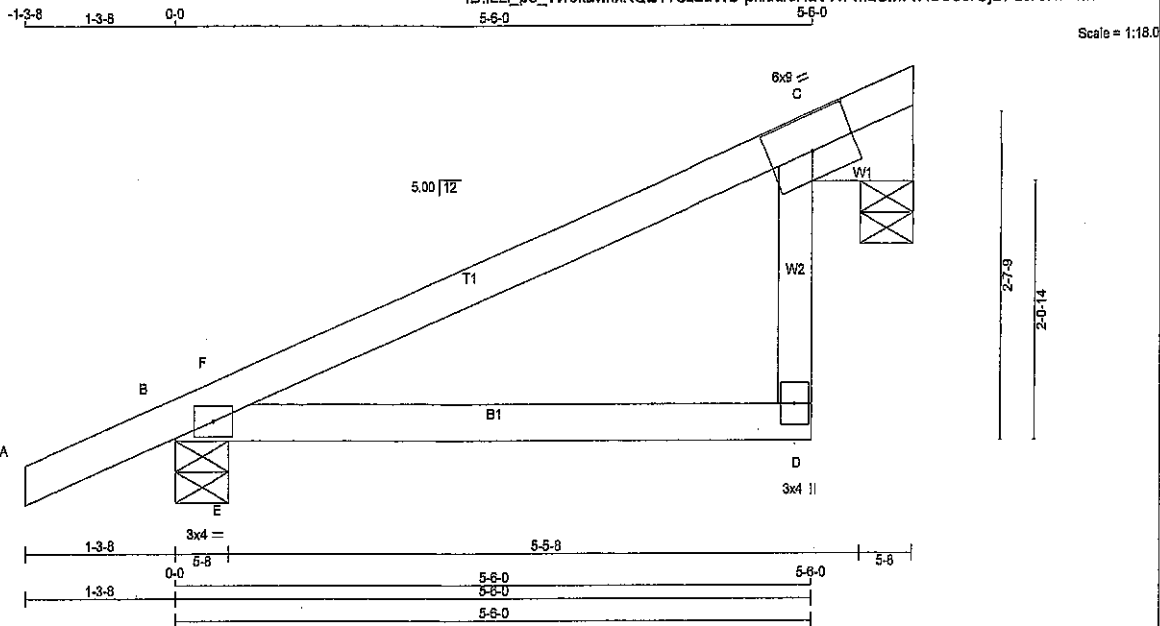
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.51 (L) (INPUT = 1.00)



DRWG NO. TAW 2788-16 P-1
STRUCTURAL COMPONENT ONLY

Tamarack Roof Truss, Burlington



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY No.2	SFF
D - C	2x4	DRY No.2	SFF
B - D	2x4	DRY No.2	SFF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMBVW1-I	MT20	6.0	9.0	3.25	4.50
D	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		GROSS REACTION		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX	IN-SX	IN-SX
C	418	0	418	0	0	5-8	5-8	5-8	5-8	5-8	5-8
B	587	0	587	0	0	5-8	5-8	5-8	5-8	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	332	191 / 0	58 / 0	0 / 0	0 / 0	83 / 0	0 / 0
B	447	285 / 0	58 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (LBS)	MEMB.	FORCE (LBS)	LC1 MAX (LBS)	CS1 (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 31	-124.4	-124.4	0.18 (1)	10.00	0.18 (1)	10.00
B-F	-70 / 51	-124.4	-124.4	0.09 (3)	6.25	0.09 (3)	6.25
F-C	-4 / 6	-124.4	-124.4	0.48 (1)	10.00	0.48 (1)	10.00
D-C	0 / 139	0.0	0.0	0.02 (2)	10.00	0.02 (2)	10.00
B-E	0 / 0	-28.0	-28.0	0.33 (1)	10.00	0.33 (1)	10.00
E-D	0 / 0	-28.0	-28.0	0.34 (1)	10.00	0.34 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/723 (0.09")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/420 (0.16")

CSI: TC=0.48 (C-F:1), BC=0.34 (D-E:1), WB=0.00 (E-F:1), SSI=0.39 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.52 (B) (INPUT = 0.80)
 JSI METAL=0.10 (B) (INPUT = 1.00)



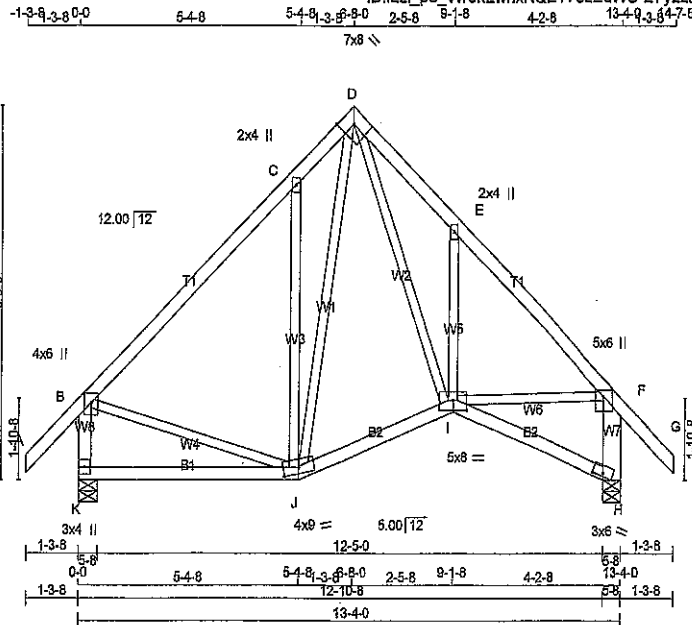
DWG NO. TAM 27076 R1
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 254503	TRUSS NAME T17	QUANTITY 1	PLY 1	JOB DESC. 36839/MOUNT TRUSS DESC. ALBERTA/JG	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:ILZl_p3_VwOkLwnxKQE1YuzEdWU-E7y2zJWJs5KkFuckHF88paN44VgC7LtofWSnlazmXAM



Scale = 1:51.3

TOTAL WEIGHT = 75 lb

LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2	D - G	2x4	DRY	No.2
K - B	2x4	DRY	No.2	K - B	2x4	DRY	No.2
H - F	2x6	DRY	No.2	H - F	2x6	DRY	No.2
K - J	2x4	DRY	No.2	K - J	2x4	DRY	No.2
J - I	2x4	DRY	No.2	J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2	I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2	ALL WEBS	2x3	DRY	No.2
EXCEPT				EXCEPT			
DRY: SEASONED LUMBER.				DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW+p	MT20	4.0	6.0	2.75 2.00
C	TMVW+w	MT20	2.0	4.0	
D	TTWW-I	MT20	7.0	8.0	Edge
E	TMVW+w	MT20	2.0	4.0	
F	TMVW+p	MT20	5.0	6.0	1.75 2.75
H	5VM1-I	MT20	3.0	6.0	0.75 3.00
I	BBWWV+p	MT20	5.0	8.0	2.75 4.00
J	BBWWV-m	MT20	4.0	9.0	1.75 4.50
K	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
K	1189	0	1189	0	0	5-8	1-8	1-8	1-8
H	1189	0	1189	0	0	5-8	1-8	1-8	1-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SOIL
K	923	561 / 0	140 / 0	0 / 0	0 / 0	222 / 0	0 / 0	923	0 / 0
H	923	561 / 0	140 / 0	0 / 0	0 / 0	222 / 0	0 / 0	923	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	LC1 MAX	MEMB.	FORCE (LBS)	MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM TO			FR-TO			FR-TO			
A-B	0 / 81	-124.4 -124.4	0.17 (1)	10.00	J-C	-866 / 0	0.59 (1)	J-C	-866 / 0	0.59 (1)	
B-C	-708 / 0	-124.4 -124.4	0.40 (1)	6.25	I-E	-512 / 0	0.15 (1)	I-E	-512 / 0	0.15 (1)	
C-D	-882 / 0	-124.4 -124.4	0.33 (1)	6.18	B-J	0 / 549	0.12 (1)	B-J	0 / 549	0.12 (1)	
D-E	-1094 / 0	-124.4 -124.4	0.20 (1)	5.82	I-F	0 / 753	0.17 (1)	I-F	0 / 753	0.17 (1)	
E-F	-1035 / 0	-124.4 -124.4	0.26 (1)	5.85	D-I	0 / 869	0.20 (1)	D-I	0 / 869	0.20 (1)	
F-G	0 / 81	-124.4 -124.4	0.17 (1)	10.00	J-D	0 / 457	0.10 (1)	J-D	0 / 457	0.10 (1)	
K-B	-1114 / 0	0.0 0.0	0.12 (1)	7.51							
H-F	-1130 / 0	0.0 0.0	0.09 (1)	7.81							
K-J	0 / 0	-28.0 -28.0	0.27 (3)	10.00							
J-I	0 / 474	-28.0 -28.0	0.17 (2)	10.00							
I-H	0 / 0	-28.0 -28.0	0.16 (3)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.10")

CSI: TC=0.40 (B-C:1), BC=0.27 (J-K:3), WB=0.58 (C-J:1), SS=0.21 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 622 2284 1658

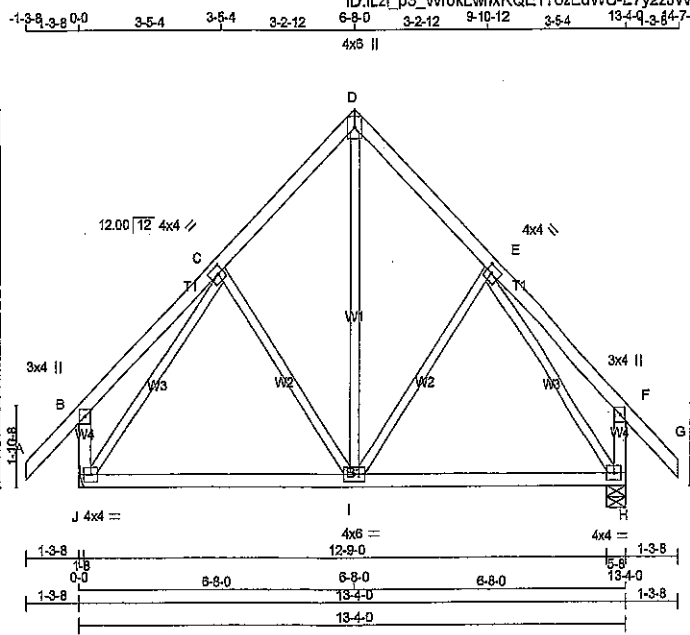
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.24 (F) (INPUT = 1.00)



DRG NO. TAM 2791-1621
STRUCTURAL COMPONENT ONLY



Scale = 1:50.8

TOTAL WEIGHT = 2 X 69 = 139 lb

LUMBER			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW-t	MT20	4.0	4.0 2.00 1.50
D	TTW+p	MT20	4.0	6.0
E	TMVW-t	MT20	4.0	4.0 2.00 1.50
F	TMV+p	MT20	3.0	4.0
H	BMVW-t	MT20	4.0	4.0
I	BMVW-t	MT20	4.0	6.0
J	BMVW-t	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	1189	0	1189	0	0
H	1189	0	1189	0	0

UNFACTORED REACTIONS		1ST LOASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE WIND DEAD SOIL
J	923	561/0	140/0	0/0 0/0 222/0 0/0
H	923	561/0	140/0	0/0 0/0 222/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/61	-124.4 -124.4	0.17 (1)	10.00	I-D	0/536	0.12 (1)
B-C	0/34	-124.4 -124.4	0.22 (1)	10.00	I-E	-187/14	0.11 (1)
C-D	-643/0	-124.4 -124.4	0.17 (1)	6.25	C-I	-187/14	0.11 (1)
D-E	-643/0	-124.4 -124.4	0.17 (1)	6.25	J-C	-854/0	0.55 (1)
E-F	0/34	-124.4 -124.4	0.22 (1)	10.00	E-H	-854/0	0.55 (1)
F-G	0/61	-124.4 -124.4	0.17 (1)	10.00			
J-B	-330/0	0.0 0.0	0.04 (1)	7.81			
H-F	-330/0	0.0 0.0	0.04 (1)	7.81			
J-I	0/539	-28.0 -28.0	0.44 (3)	10.00			
I-H	0/539	-28.0 -28.0	0.44 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 240 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (0.44")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.22 (B-C), BC=0.44 (H-I), WB=0.55 (C-J), SSI=0.18 (I-J)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

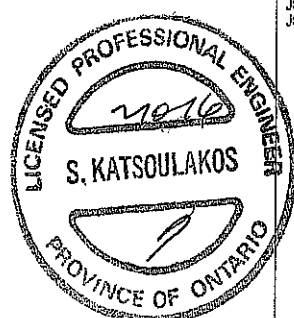
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	(FLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20 618 354	1667 822	2284 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.96 (C) (INPUT = 1.00)

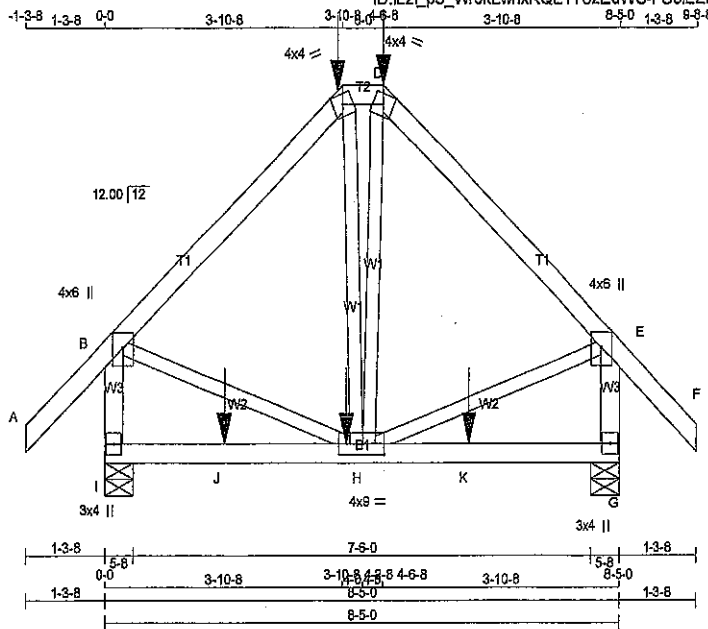


DRW NO. TAM 2792-16 R1
STRUCTURAL
COMPONENT ONLY

JOB NAME 254506	TRUSS NAME T19	QUANTITY 1	PLY 1	JOB DESC. 39935/MOUNT	TRUSS DESC. ALBERT/JG	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:34.2

TOTAL WEIGHT = 46 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	8.0	2.75	2.00
C	TTW-m	MT20	4.0	4.0	Edge	
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMVW+p	MT20	4.0	5.0	2.75	2.00
G	BMV+p	MT20	3.0	4.0		
H	BMVW+p	MT20	4.0	8.0	2.00	4.25
I	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 146.6 lbs FACTORED DOWN AT 3-10-8, AND 146.6 lbs FACTORED DOWN AT 4-6-8, AND 116.6 lbs FACTORED DOWN AT 3-10-8 ON TOP CHORD, AND 42.6 lbs FACTORED DOWN AT 1-11-4, AND 42.6 lbs FACTORED DOWN AT 3-11-4, AND 42.6 lbs FACTORED DOWN AT 5-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1062	0	1062	0	0	5-8	1-8
G	1060	0	1060	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	834	482 / 0	137 / 0	0 / 0	0 / 0	205 / 0	0 / 0
G	823	487 / 0	135 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 61	-124.4	-124.4	0.19 (1)	10.00	B-H	0 / 480 0.11 (1)
B-C	-615 / 0	-124.4	-124.4	0.37 (1)	6.25	H-E	0 / 453 0.11 (1)
C-D	-428 / 0	-124.4	-124.4	0.01 (1)	6.25	H-D	0 / 109 0.03 (3)
D-E	-805 / 0	-124.4	-124.4	0.37 (1)	6.25	C-H	-109 / 91 0.06 (1)
E-F	0 / 61	-124.4	-124.4	0.19 (1)	10.00		
I-B	-1007 / 0	0.0	0.0	0.12 (1)	7.77		
G-E	-898 / 0	0.0	0.0	0.12 (1)	7.80		
I-J	0 / 0	-28.0	-28.0	0.22 (3)	10.00		
J-H	0 / 0	-28.0	-28.0	0.22 (3)	10.00		
H-K	0 / 0	-28.0	-28.0	0.22 (3)	10.00		
K-G	0 / 0	-28.0	-28.0	0.22 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	3-10-8	-147	-147	--	FRONT	VERT	TOTAL
C	3-10-8	-117	-117	--	BACK	VERT	TOTAL
D	4-6-8	-147	-147	--	FRONT	VERT	TOTAL
H	3-11-4	-24	-43	--	BACK	VERT	TOTAL
J	1-11-4	-24	-43	--	BACK	VERT	TOTAL
K	5-11-4	-24	-43	--	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.25")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.25")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.37 (B-C:1), BC=0.22 (G-H:3), WB=0.11 (B-H:1), SSI=0.15 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1655

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (5) (INPUT = 0.90)
JSI METAL= 0.18 (5) (INPUT = 1.00)



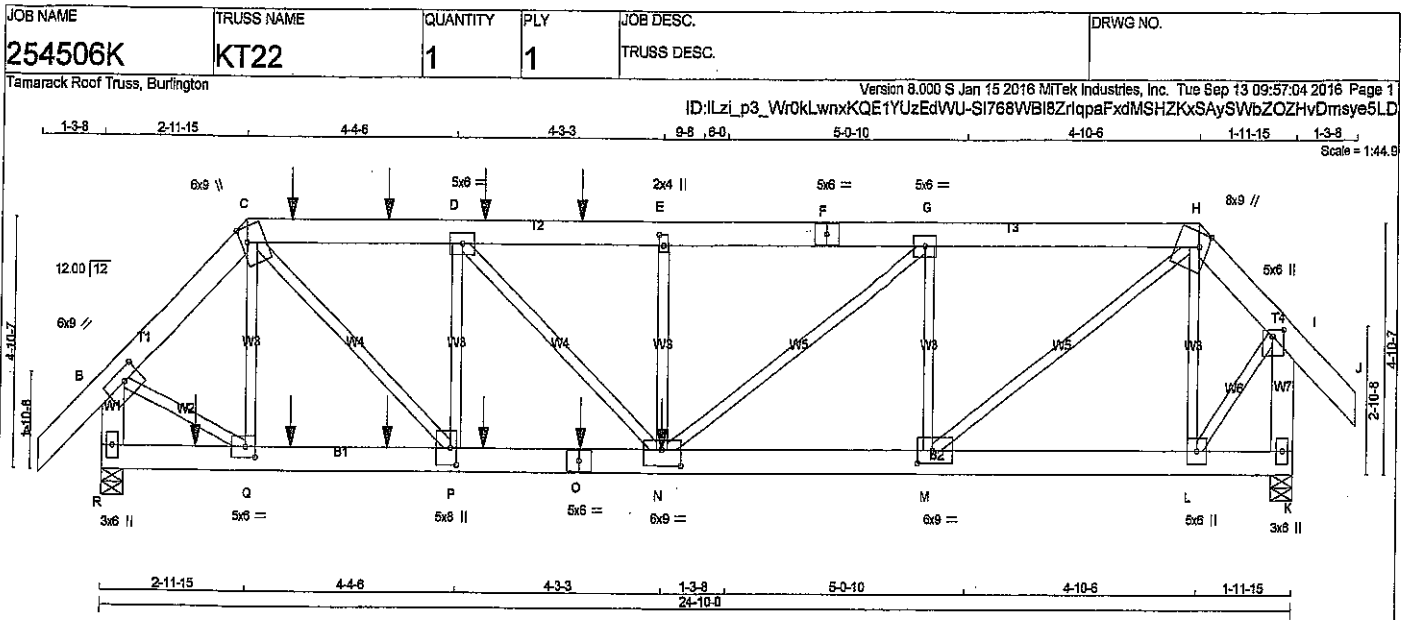
DWG NO. TAM 279316
STRUCTURAL
COMPONENT ONLY

Scale = 1:42.5

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.28 (F) (INPUT = 1.00)



DWG NO. TAM 4109416
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 148 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY No.2	SPF
C - F	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - J	2x6	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
R - O	2x6	DRY No.2	SPF
O - K	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.50	4.00
C	TTWW+m	MT20	6.0	9.0	3.50	1.50
D	TMVW-t	MT20	5.0	6.0		
E	TMVW-w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	8.0	9.0	Edge	2.25
I	TMVW+p	MT20	5.0	6.0	1.50	2.75
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	6.0	9.0	3.00	4.00
N	BMVW-w	MT20	6.0	9.0	3.75	4.50
O	BS-t	MT20	5.0	6.0		
P	BMVW-t	MT20	5.0	6.0	4.00	1.50
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 120.1 lbs FACTORED DOWN AT 3-11-4, 116.6 lbs FACTORED DOWN AT 5-11-4, AND 116.6 lbs FACTORED DOWN AT 7-11-4, AND 116.6 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 42.0 lbs FACTORED DOWN AT 1-11-4, 42.0 lbs FACTORED DOWN AT 3-11-4, 42.0 lbs FACTORED DOWN AT 5-11-4, 42.0 lbs FACTORED DOWN AT 7-11-4, AND 42.0 lbs FACTORED DOWN AT 9-11-4, AND 1382.8 lbs FACTORED DOWN AT 11-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 3233	DOWN 3233	0	5-8
R	HORZ 2885	HORZ 2885	0	5-8
K			0	3-14

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	2535	1500 / 0	414 / 0	0 / 0	0 / 0	621 / 0	0 / 0
K	2253	1347 / 0	357 / 0	0 / 0	0 / 0	548 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 84	-124.4 -124.4	0.10 (1)	Q-C	-842 / 0	0.20 (1)	
B-C	-2815 / 0	-124.4 -124.4	0.15 (1)	L-H	-1132 / 0	0.38 (1)	
C-S	-4042 / 0	-124.4 -124.4	0.33 (1)	S-Q	0 / 2147	0.53 (1)	
S-T	-4042 / 0	-124.4 -124.4	0.33 (1)	L-I	0 / 1785	0.44 (1)	
T-D	-4042 / 0	-124.4 -124.4	0.33 (1)	N-E	-815 / 0	0.20 (1)	
D-U	-5084 / 0	-124.4 -124.4	0.39 (1)	P-D	-1908 / 0	0.61 (1)	
U-V	-5084 / 0	-124.4 -124.4	0.39 (1)	C-P	0 / 2944	0.73 (1)	
V-E	-5084 / 0	-124.4 -124.4	0.39 (1)	D-N	0 / 1471	0.36 (1)	
E-F	-5084 / 0	-124.4 -124.4	0.47 (1)	M-G	-1827 / 0	0.58 (1)	
F-G	-5084 / 0	-124.4 -124.4	0.47 (1)	N-G	0 / 1662	0.42 (1)	
G-H	-3738 / 0	-124.4 -124.4	0.36 (1)	M-H	0 / 3244	0.80 (1)	
H-I	-1760 / 0	-124.4 -124.4	0.11 (1)				
I-J	0 / 84	-124.4 -124.4	0.10 (1)				
R-B	-3215 / 0	0.0	0.0				
K-I	-2920 / 0	0.0	0.0				
R-W	0 / 0	-28.0	-28.0				
W-Q	0 / 0	-28.0	-28.0				
Q-X	0 / 1959	-28.0	-28.0				
X-Y	0 / 1959	-28.0	-28.0				
Y-P	0 / 1959	-28.0	-28.0				
P-Z	0 / 4042	-28.0	-28.0				
Z-O	0 / 4042	-28.0	-28.0				
O-N	0 / 4042	-28.0	-28.0				
N-M	0 / 3739	-28.0	-28.0				
M-L	0 / 1170	-28.0	-28.0				
L-K	0 / 0	-28.0	-28.0				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
N	11-7-8	-1383	-1383		BACK	VERT	TOTAL
O	9-11-4	-24	-42		BACK	VERT	TOTAL
S	3-11-4	-120	-120		BACK	VERT	TOTAL
T	5-11-4	-117	-117		BACK	VERT	TOTAL
U	7-11-4	-117	-117		BACK	VERT	TOTAL
V	9-11-4	-117	-117		BACK	VERT	TOTAL
W	1-11-4	-24	-42		BACK	VERT	TOTAL
X	3-11-4	-24	-42		BACK	VERT	TOTAL
Y	5-11-4	-24	-42		BACK	VERT	TOTAL
Z	7-11-4	-24	-42		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.83")

CALCULATED VERT. DEFL. (LL) = $L/999$ (0.16")

ALLOWABLE DEFL. (TL) = $L/360$ (0.83")

CALCULATED VERT. DEFL. (TL) = $L/999$ (0.27")

CSI: TC=0.47 (E-G:1), BC=0.63 (N-P:1), WB=0.80 (H-M:1), SSI=0.30 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

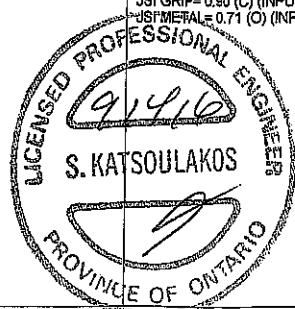
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	1667
	822	2284
	1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (C) (INPUT = 0.80)

JSI METAL= 0.71 (O) (INPUT = 1.00)



DRWG NO. TAM 4109616

STRUCTURAL

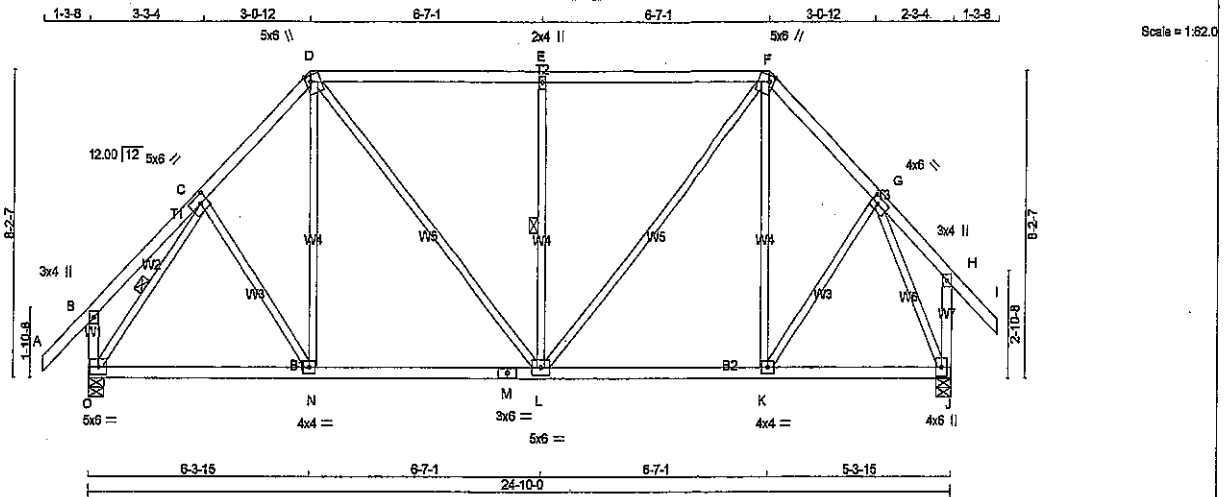
COMPONENT ONLY

JOB NAME 254506K	TRUSS NAME KT24	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:ILzI_p3_Wr0kLwnxKQE1YUzEdWU-Xuo2zmQlaP92XEEoGzArHaUSh80MG6d5ZDVeye5IK



TOTAL WEIGHT = 2 X 123 = 246 lb

LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA				
N. L. G. A. RULES															SPECIFIED LOADS:				
CHORDS		SIZE	LUMBER	DESCR.	BEARINGS										TOP CH. LL = 34.8 PSF				
A - D	2x4	DRY	No.2	SPF	FACTORED					MAXIMUM FACTORED					DL = 8.0 PSF				
D - F	2x4	DRY	1650F 1.5E	SPF	GROSS REACTION					GROSS REACTION					BOT CH. LL = 10.5 PSF				
F - I	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	DL = 7.0 PSF						
O - B	2x4	DRY	No.2	SPF	J	2065	0	2065	0	0	5-8	2-4	TOTAL LOAD = 60.3 PSF						
J - H	2x4	DRY	No.2	SPF	J	2065	0	2065	0	0	5-8	2-4							
O - M	2x4	DRY	No.2	SPF											SPACING = 24.0 IN. C/C				
M - J	2x4	DRY	No.2	SPF											LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12				
ALL WEBS EXCEPT					2x3 DRY No.2 SPF														
DRY: SEASONED LUMBER.																			
					UNFACTORED REACTIONS														
					1ST CASE														
					JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL							
					O	1817	961 / 0	261 / 0	0 / 0	0 / 0	395 / 0	0 / 0							
					J	1817	961 / 0	261 / 0	0 / 0	0 / 0	395 / 0	0 / 0							

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	5.0	6.0	2.50	2.50
D	TTWV+m	MT20	5.0	6.0	2.00	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TTWV+m	MT20	5.0	6.0	2.00	1.50
G	TMVW-t	MT20	4.0	6.0	2.00	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 - 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L. DBS = 18-0-0. CBF = 113 LBS.

1 - 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O. DBS = 8-0-0. CBF = 101 LBS.

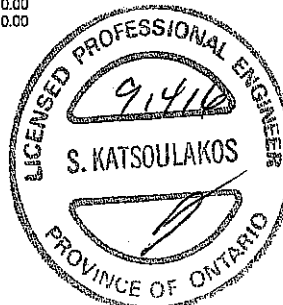
DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

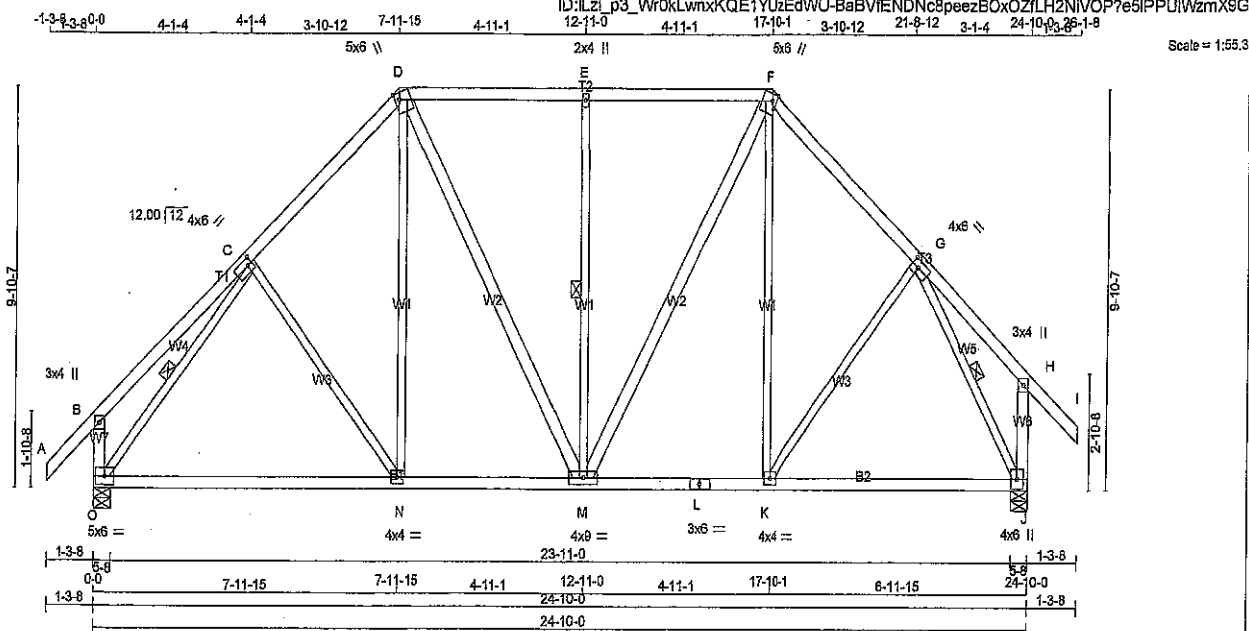
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/61	-124.4 -124.4	0.17 (1)	10.00	C-N	0/110	0.03 (3)
B-C	0/30	-124.4 -124.4	0.19 (1)	10.00	N-D	0/273	0.07 (3)
C-D	-1658/0	-124.4 -124.4	0.23 (1)	4.82	D-L	0/688	0.15 (1)
D-E	-1575/0	-124.4 -124.4	0.85 (1)	4.28	L-E	-1004/0	0.43 (1)
E-F	-1575/0	-124.4 -124.4	0.85 (1)	4.28	L-F	0/688	0.20 (1)
F-G	-1472/0	-124.4 -124.4	0.18 (1)	5.21	K-F	-117/132	0.15 (1)
G-H	0/32	-124.4 -124.4	0.14 (1)	10.00	K-G	0/371	0.08 (1)
H-I	0/61	-124.4 -124.4	0.17 (3)	10.00	O-C	-2011/0	0.47 (1)
O-B	-325/0	0.0 0.0	0.04 (1)	7.81	G-J	-1923/0	0.82 (1)
J-H	-264/0	0.0 0.0	0.04 (1)	7.81			
O-N	0/1121	-28.0 -28.0	0.40 (2)	10.00			
N-M	0/1148	-28.0 -28.0	0.42 (2)	10.00			
M-L	0/1148	-28.0 -28.0	0.42 (2)	10.00			
L-K	0/1019	-28.0 -28.0	0.35 (2)	10.00			
K-J	0/809	-28.0 -28.0	0.33 (2)	10.00			



DWG NO. TAM 41095-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 140 = 280 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS				
EXCEPT	SIZE	LUMBER	DESCR.	
D - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
O	2065	0	2065	0	5-8
J	2065	0	2065	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1817	961/0	261/0	0/0	0/0	395/0	0/0
J	1817	961/0	261/0	0/0	0/0	395/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1- 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M. DBS = 20'-0" . CBF = 93 LBS.
1- 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-J. DBS = 8'-0" . CBF = 99 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x6, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/161	C-N	-123/61
B-C	0/141	D-M	0/402
C-D	-1586/0	E-F	0/370
D-E	-1262/0	F-G	-743/0
E-F	-1262/0	G-H	0/550
F-G	-1485/0	H-I	0/240
G-H	0/143	I-J	0/183
H-I	0/161	J-K	-1967/0
I-J	-362/0	K-L	-1917/0
J-K	-300/0		
O-N	0/1163		
N-M	0/1092		
M-L	0/1010		
L-K	0/1010		
K-J	0/917		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-08
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/899 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/699 (0.29")

CSI: TC=0.40 (D-E-1), BC=0.54 (M-N-3), WB=0.61 (C-O-1), SS=0.30 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

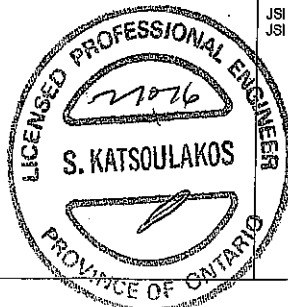
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (C) (INPUT = 0.80)
JSI METAL=0.49 (C) (INPUT = 1.00)



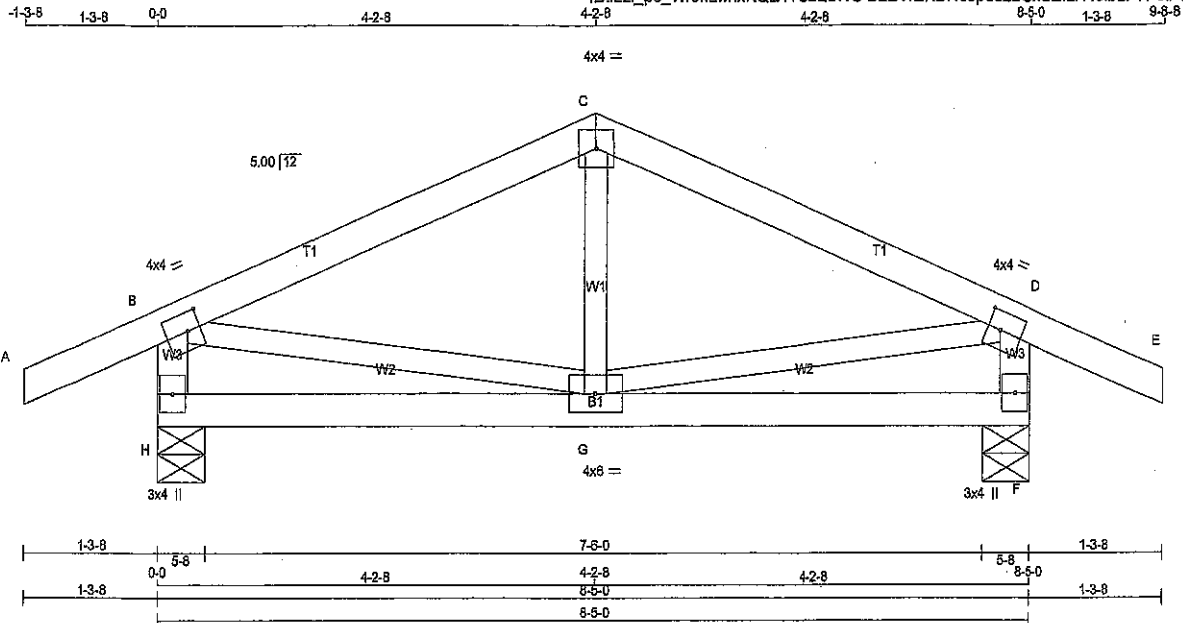
DRWG NO. YAM 2299-1621
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	59938/MOUNT	DRWG NO.
254506	T26	2	1	TRUSS DESC.	ALBERT/JG	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x8	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	4.0	2.00	1.50
C	TTW-p	MT20	4.0	4.0		
D	TMVW-1	MT20	4.0	4.0	2.00	1.50
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-1	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT
H	809	0	809	0	5-8
F	809	0	809	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	623	387/0	88/0	0/0	0/0	148/0	0/0
F	623	387/0	88/0	0/0	0/0	148/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX		MAX		FACTORED		MAX	
MEMB.	FORCE (LBS)	VERT.	LOAD	LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	MAX	MEMB.
FR-TO		FROM	TO	LENGTH	FR-TO		FR-TO		FROM	TO	LENGTH
A-B	0/32	-124.4	-124.4	0.16 (1)	10.00	G-C	-57/154	0.04 (3)			
B-C	-606/0	-124.4	-124.4	0.29 (1)	6.25	B-G	0/588	0.13 (1)			
C-D	-606/0	-124.4	-124.4	0.29 (1)	6.25	G-D	0/588	0.13 (1)			
D-E	0/32	-124.4	-124.4	0.16 (1)	10.00						
H-B	-761/0	0.0	0.0	0.08 (1)	7.81						
F-D	-761/0	0.0	0.0	0.08 (1)	7.81						
H-G	0/0	-28.0	-28.0	0.15 (3)	10.00						
G-F	0/0	-28.0	-28.0	0.15 (3)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-08
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.29 (B-C:1), BC=0.15 (G-H:3), WB=0.13 (D-G:1), SSI=0.19 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.82 (B) (INPUT = 0.80)
JSI METAL=0.29 (B) (INPUT = 1.00)



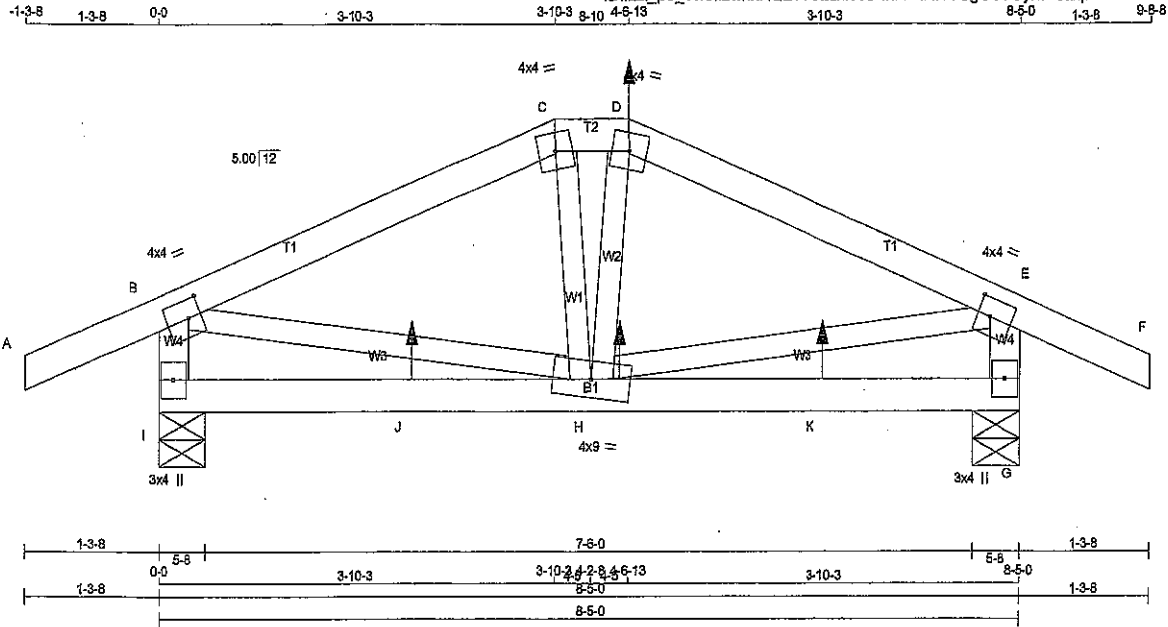
DRWG NO. TAM 2800-16P4
STRUCTURAL
COMPONENT ONLY

JOB NAME 254506	TRUSS NAME T27	QUANTITY 1	PLY 1	JOB DESC. 39938/AMOUNT	TRUSS DESC. ALBERT/JG	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 7.640 S Oct 7 2015 Mitek Industries, Inc. Wed Feb 10 14:25:34 2016 Page 1

ID:ILZl_p3_Vw0kLwnxKQE1YUzEdWU-fnlusaNr8vGgGocYcyfwoCZqEo6x88ZOF_391HyzmX9F



TOTAL WEIGHT = 34 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY No.2	SPF			
C - D	2x4 DRY No.2	SPF			
D - F	2x4 DRY No.2	SPF			
I - B	2x4 DRY No.2	SPF			
G - E	2x4 DRY No.2	SPF			
I - G	2x4 DRY No.2	SPF			
ALL WEBS EXCEPT	2x3 DRY No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-H	MT20	4.0	4.0	2.00	1.50
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-H	MT20	4.0	4.0	2.00	1.50
G	BMV1+p	MT20	3.0	4.0		
H	BMVWWV+MT20	MT20	4.0	9.0		
I	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 188.1 lbs FACTORED UP AT 4-6-13 ON TOP CHORD, AND 5.5 lbs FACTORED UP AT 2-5-12, AND 5.5 lbs FACTORED UP AT 4-5-12, AND 5.5 lbs FACTORED UP AT 6-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
I	772	0	772	0
G	766	0	766	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
I	695	369 / 0	88 / 0	0 / 0	0 / 0	141 / 0	0 / 0			
G	690	365 / 0	88 / 0	0 / 0	0 / 0	140 / 0	0 / 0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC1)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 32	-124.4	-124.4 0.17 (1)	B-H	0 / 544	0.13 (1)	10.00
B-C	-581 / 0	-124.4	-124.4 0.38 (1)	H-E	0 / 534	0.13 (1)	6.25
C-D	-528 / 0	-124.4	-124.4 0.02 (1)	E-F	-91 / 71	0.02 (7)	6.25
D-E	-570 / 0	-124.4	-124.4 0.38 (1)	F-G	0 / 91	0.02 (7)	6.25
E-F	0 / 32	-124.4	-124.4 0.17 (1)				10.00
F-G	-728 / 0	0.0	0.0 0.08 (1)				7.81
G-H	-720 / 0	0.0	0.0 0.08 (1)				7.81
H-I	0 / 0	-28.0	-28.0 0.18 (7)				10.00
I-J	0 / 0	-28.0	-28.0 0.18 (7)				10.00
J-K	0 / 0	-28.0	-28.0 0.18 (7)				10.00
K-L	0 / 0	-28.0	-28.0 0.18 (7)				10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE
D	4-6-13	70	---	168	BACK	VERT	TOTAL
H	4-5-12	3	---	6	BACK	VERT	TOTAL
J	2-5-12	3	---	6	BACK	VERT	TOTAL
K	6-5-12	3	---	6	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 34.8	PSF
	DL = 8.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 60.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA S806-09
- TPC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.26")
CALCULATED VERT. DEFL. (LL) = $L/998$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.26")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CSI: TC=0.36 (B-C:1), BC=0.16 (H-I:7), WB=0.13 (B-H:1), SSI=0.18 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1657
	822	2284	1656

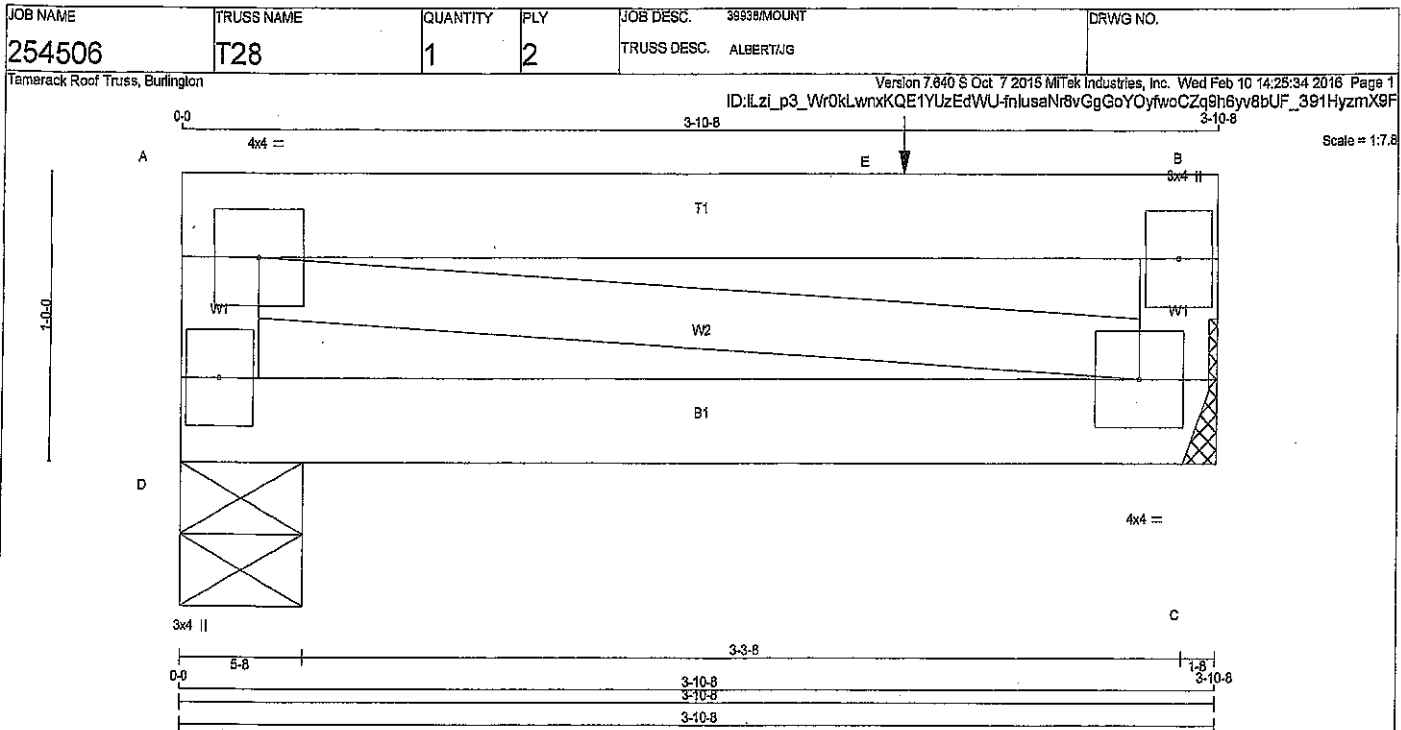
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.84 (H) (INPUT=0.80)
JSI METAL=0.23 (B) (INPUT=1.00)



DWG NO. TAM 2801-16H
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 12 = 25 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
D - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	2100F 1.8E	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
D-A 1 12		TOP
A-B 1 12		TOP
B-C 1 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
D-C 1 12		TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 8		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

HANGERS/NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1808.6 lbs FACTORED DOWN AT 2-8-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	788	0	788	0	0
C	1411	0	1411	0	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	819	384/0	103/0	0/0	0/0	152/0	0/0
C	1103	654/0	181/0	0/0	0/0	271/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
D-A	-734/0	0.0	0.0	0.04 (1)	7.81	A-C	0/0
A-E	0/0	-124.4	-124.4	0.68 (1)	10.00		
E-B	0/0	-124.4	-124.4	0.68 (1)	10.00		
C-B	-1355/0	0.0	0.0	0.07 (1)	7.81		
D-C	0/0	-28.0	-28.0	0.07 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	2-8-4	-1809	-1809	---	TOP	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 34.8	PSF
DL	= 8.0	PSF
BOT CH. LL	= 10.6	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 60.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.68 (A-B:1), BC=0.07 (C-D:3), WB=0.00 (A-C:1), SSI=0.83 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1567
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (C) (INPUT = 0.80)
JSI METAL= 0.10 (C) (INPUT = 1.00)



DWG NO. TAM 2802-10/14
STRUCTURAL
COMPONENT ONLY

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

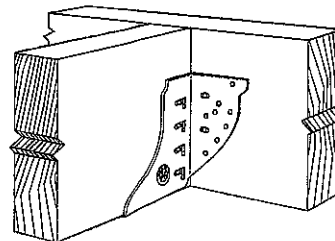
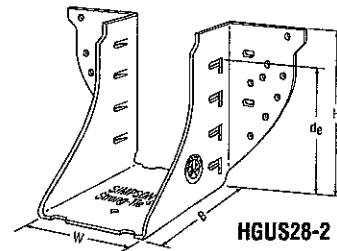
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

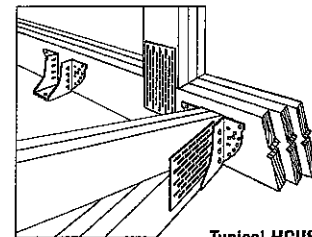
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



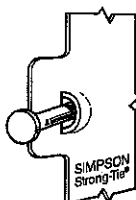
Typical HGUS Installation



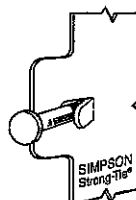
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _g =1.15)	Normal (K _g =1.00)	Uplift (K _g =1.15)	Normal (K _g =1.00)
HGUS26	12	1⅞	5⅞	5	4⅝	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6⅞	5⅞	4	4⅞	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1⅞	7⅞	5	6⅞	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3⅞	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4⅞	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6⅞	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGU210-3	12	4⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGU210-4	12	6⅞	9⅞	4	8⅞	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6⅞	10⅞	4	10⅞	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6⅞	12⅞	4	11⅞	66-16d	22-16d	10130	16400	7195	11645

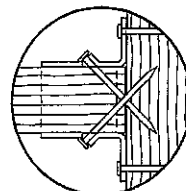
1. d_g is the distance from the seat of the hanger to the highest joist nail.



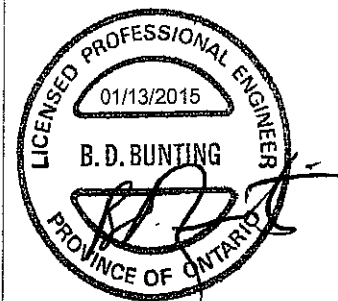
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

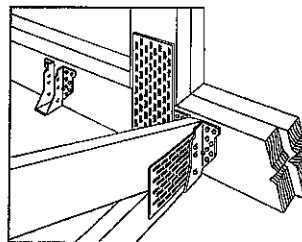
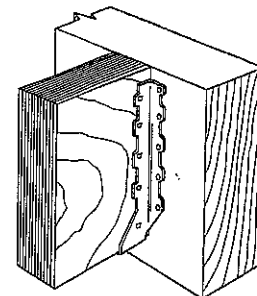
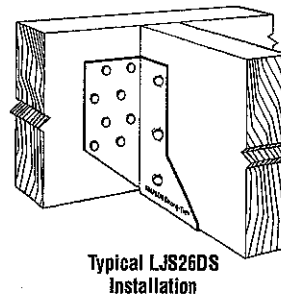
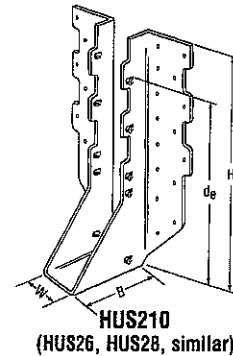
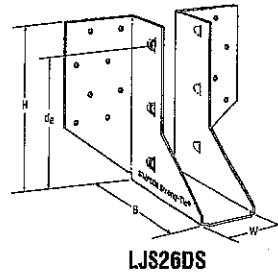
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%
No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

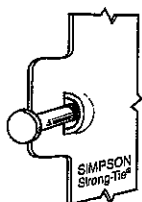
OPTIONS:

- See current catalogue for options

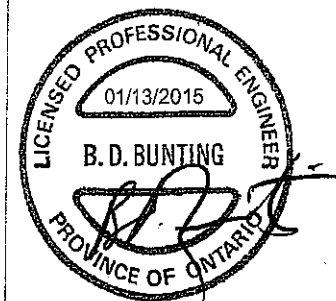
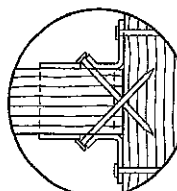
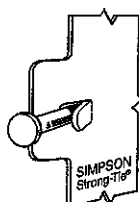


Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 5/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/8	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 3/8	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.



U.S. Patent
5,603,580



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPEC-HUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

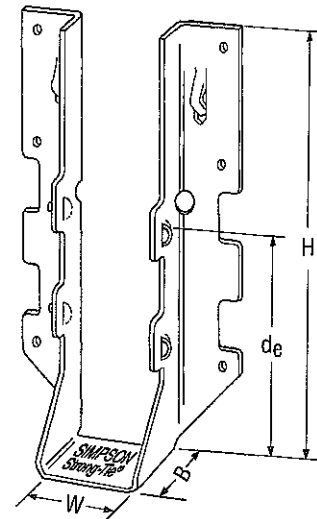
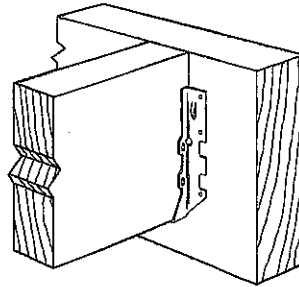
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- These hangers cannot be modified.

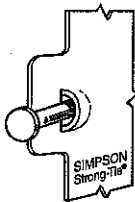
Typical LUS Installation



LUS28

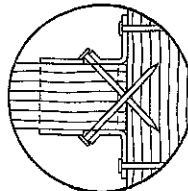
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 1/8	3 1/8	1 1/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/4	1 1/4	3 3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 9/16	2	3 1/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 1/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 1/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 13/16	1 1/4	3 3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.

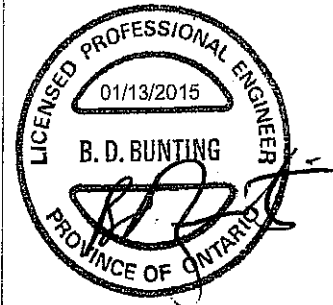


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

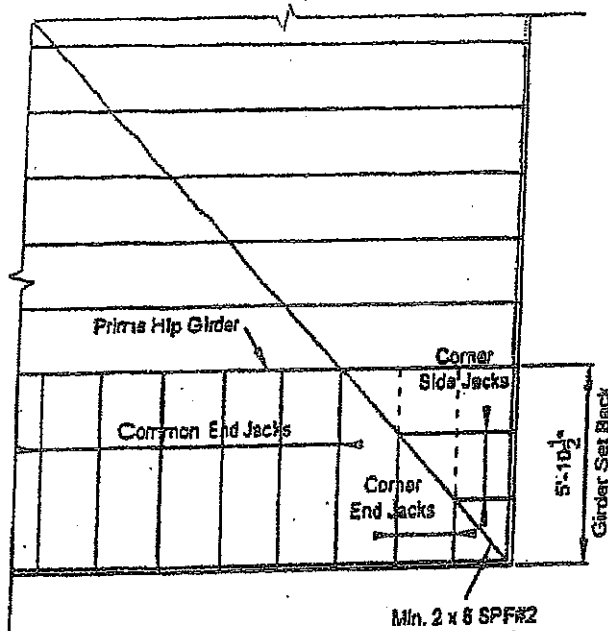
© 2015 Simpson Strong-Tie Company Inc.

T-SPECUS15-1/15 exp. 12/16

800-999-5099
www.strongtie.com

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

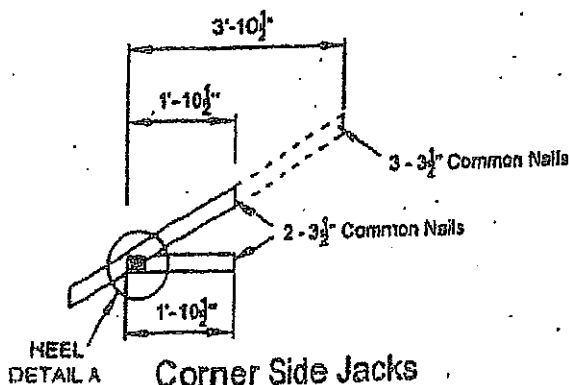
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

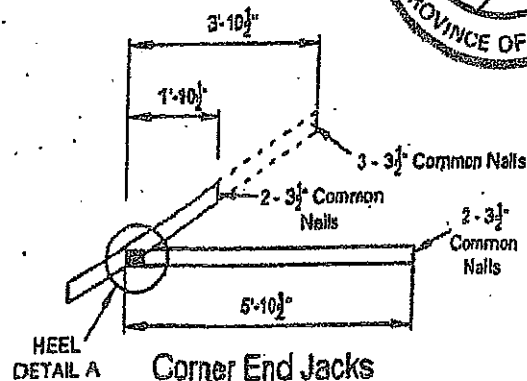
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

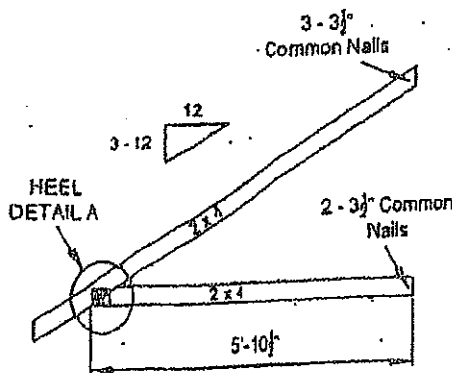
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STRUCTURAL
COMPONENT ONLY



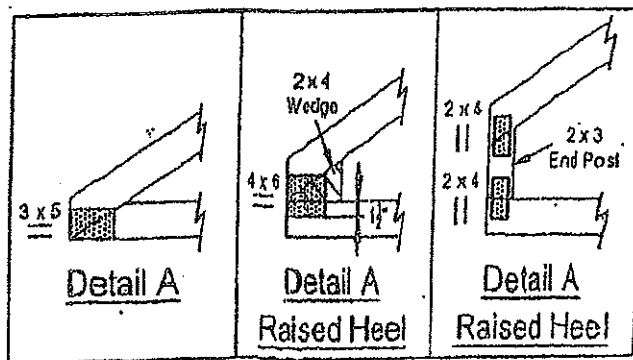
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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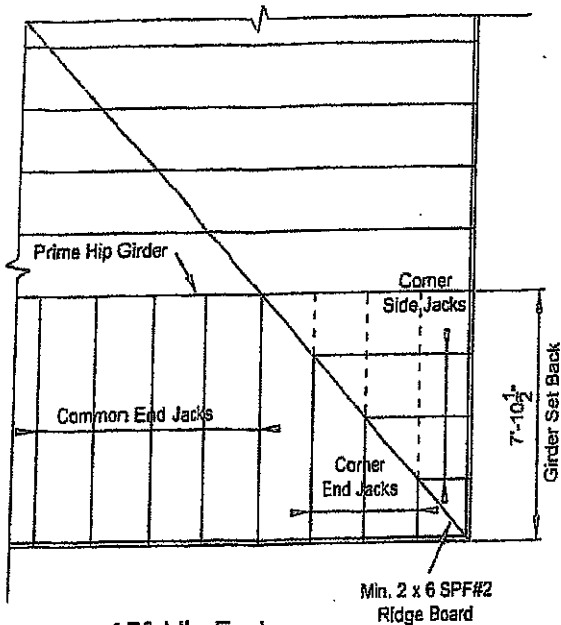
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

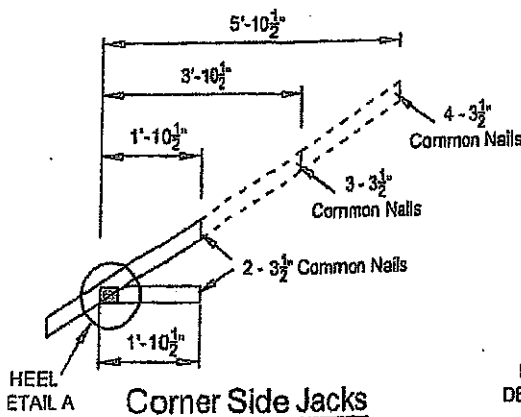
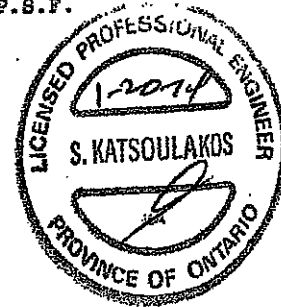
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

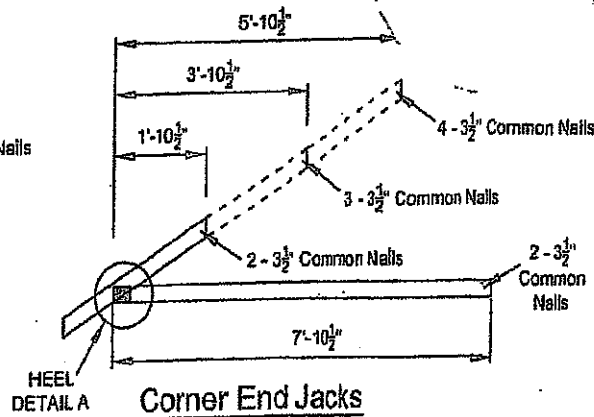


45° Hip End

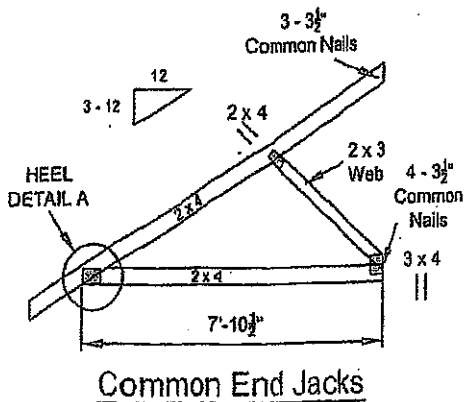
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STRUCTURAL
COMPONENT ONLY



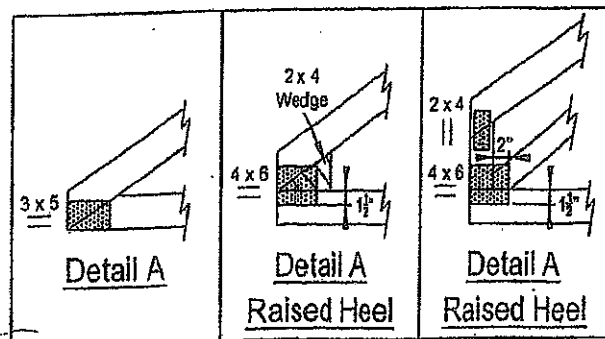
Corner Side Jacks



Corner End Jacks



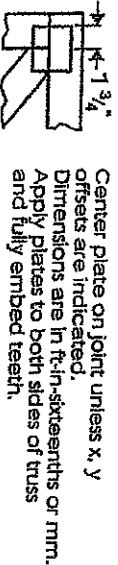
Common End Jacks



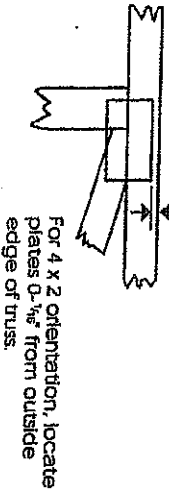
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



0-1/8"



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

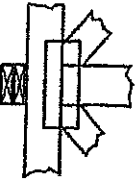
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



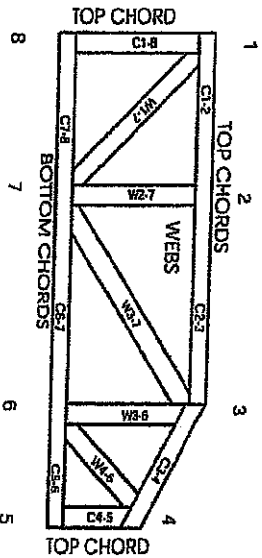
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in feet-sixteenths or millimeters (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10318-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet MIT-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

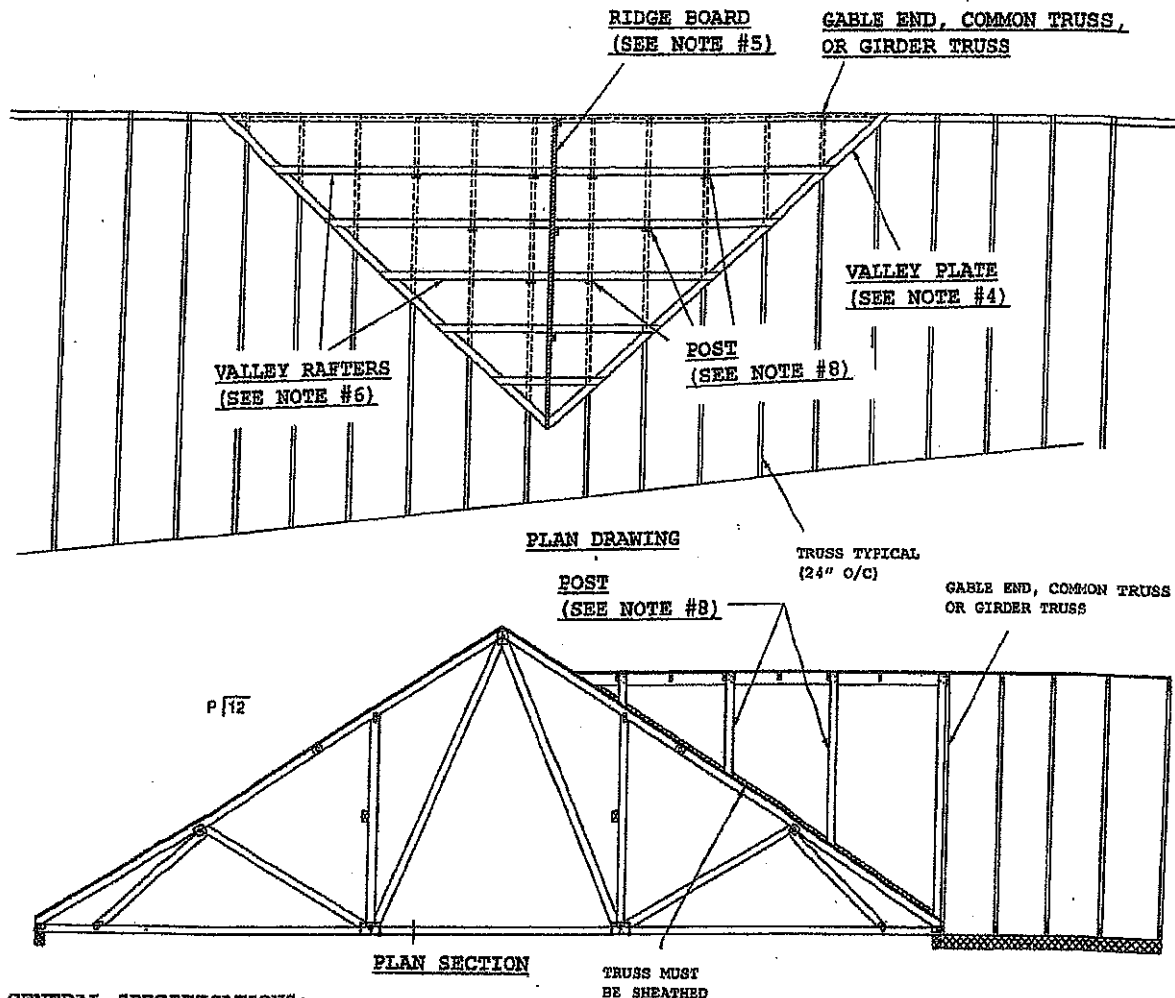
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Truss and ware at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOE CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

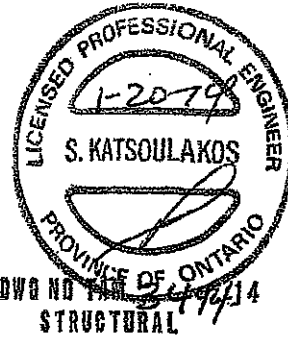
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

N0L 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.