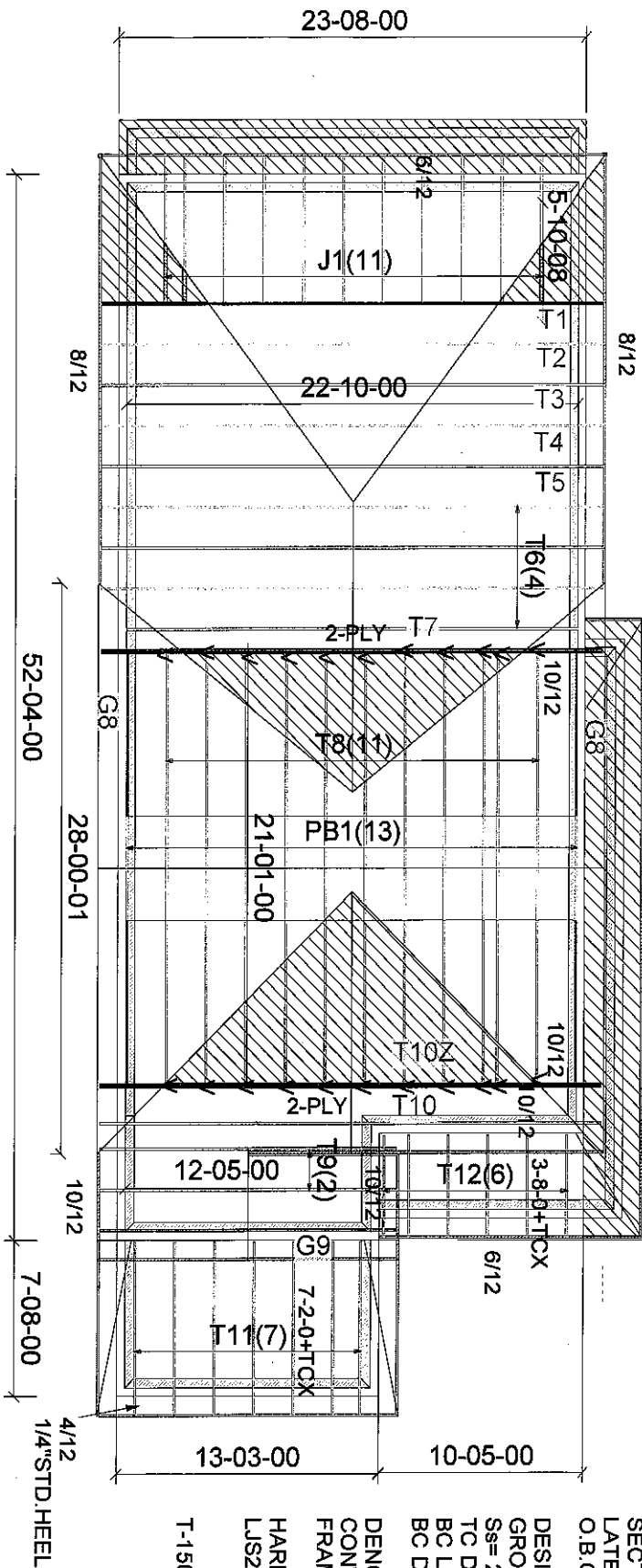




DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.3 kPa

TC DEAD	8 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF


 DENOTES
 CONVENTIONAL
 FRAMING

HARDWARE
LJS26DS(V)

T-150436

4/12
1/4"STD.HEEL

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TAMARACK
LUMBER INC.
A PLANTATION GROUP

Job Track: **45246**

Layout ID: **289742**

Plan Log: **95085**

Builder / Location:		GREENPARK HOMES / EAST GWILLIMBURY	
Project:		SECOND0 VALLEY ESTATES	
Date:	10/19/2017	Designer:	JG
		THESE DRAWINGS OR REDISTRIBUTE TOWNRACK ROOF	

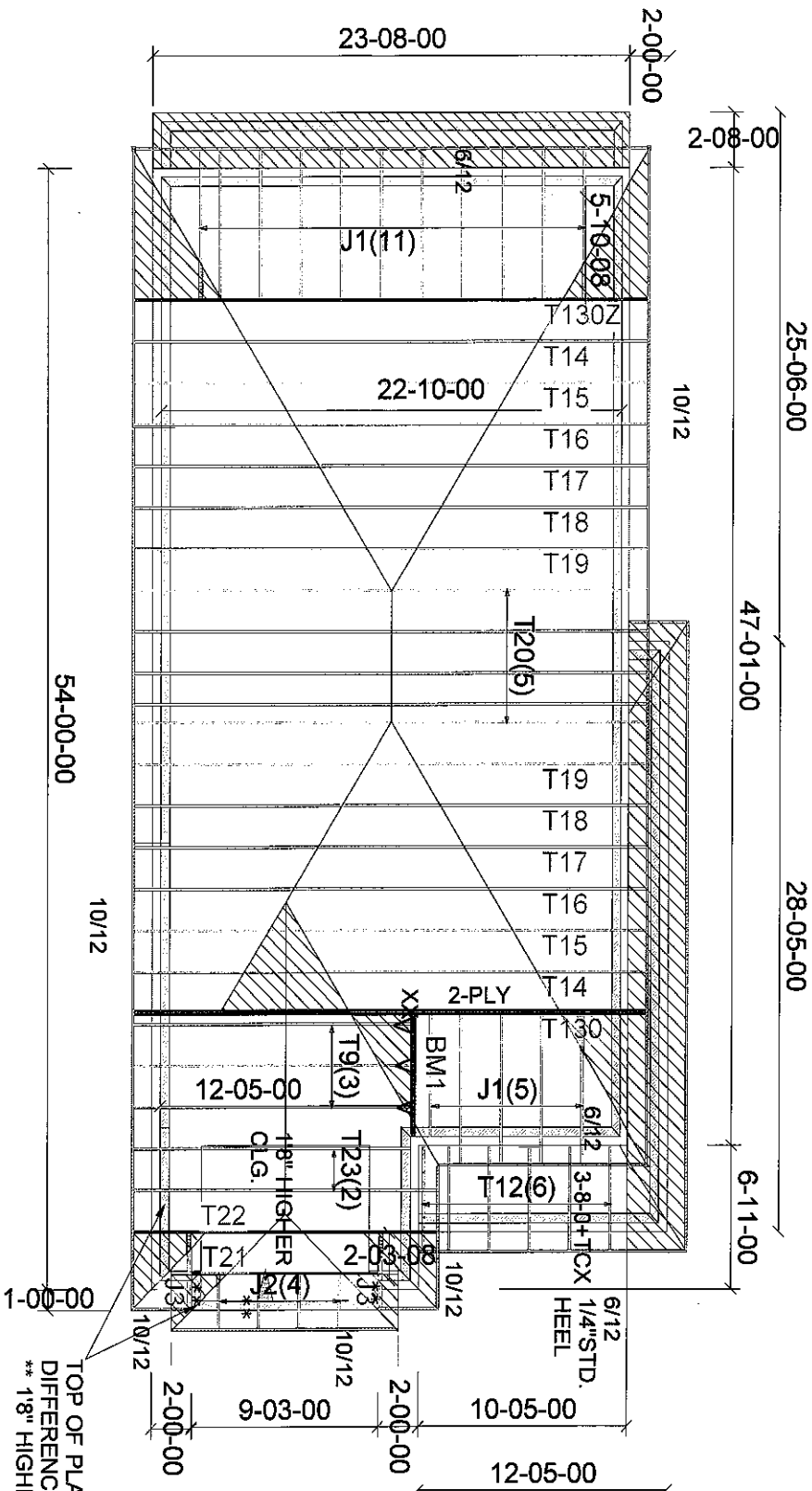
Model / Elevation:
NEWBERRY 2A / 1

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Milekay ver 7.5.0.

Milestone ver 7.5.0

THIS TRUSS DESIGNED FOR FLAT SOLAR PANEL ARRAY TO BE INSTALLED ALONG TOP CHORDS HAVING A COMBINED DEAD LOAD (SOLAR PANEL ARRAY) NOT EXCEEDING 5.0 PSF AND IN ACCORDANCE WITH CURRENT EDITION OF T.P.C. THIS IS A LOAD READY DECLARATION ONLY AND NOT AN INSTALLATION READY DECLARATION. A SEPARATE INSTALLATION DETAIL IS REQUIRED BY LICENSED PROFESSIONAL ENGINEER ONCE A SOLAR ARRAY LAYOUT IS ESTABLISHED, AND AVAILABLE ON A ROOF BY ROOF BASIS.



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN CONFORMS
WITH THE RELEVANT
SECTION OF THE
LATEST EDITION OF
O.B.C. PART 9

DESIGN LOADS:
GROUND SNOW LOAD
S_s = 2.3 kPa

TC DEAD 8 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LUS26DS(V)
HGUS26-2(XX)

BM1:2-2X10
T-150436

TOP OF PLATE 18"
DIFFERENCE
** 18" HIGHER TOP OF PLATE

mm, 1/64



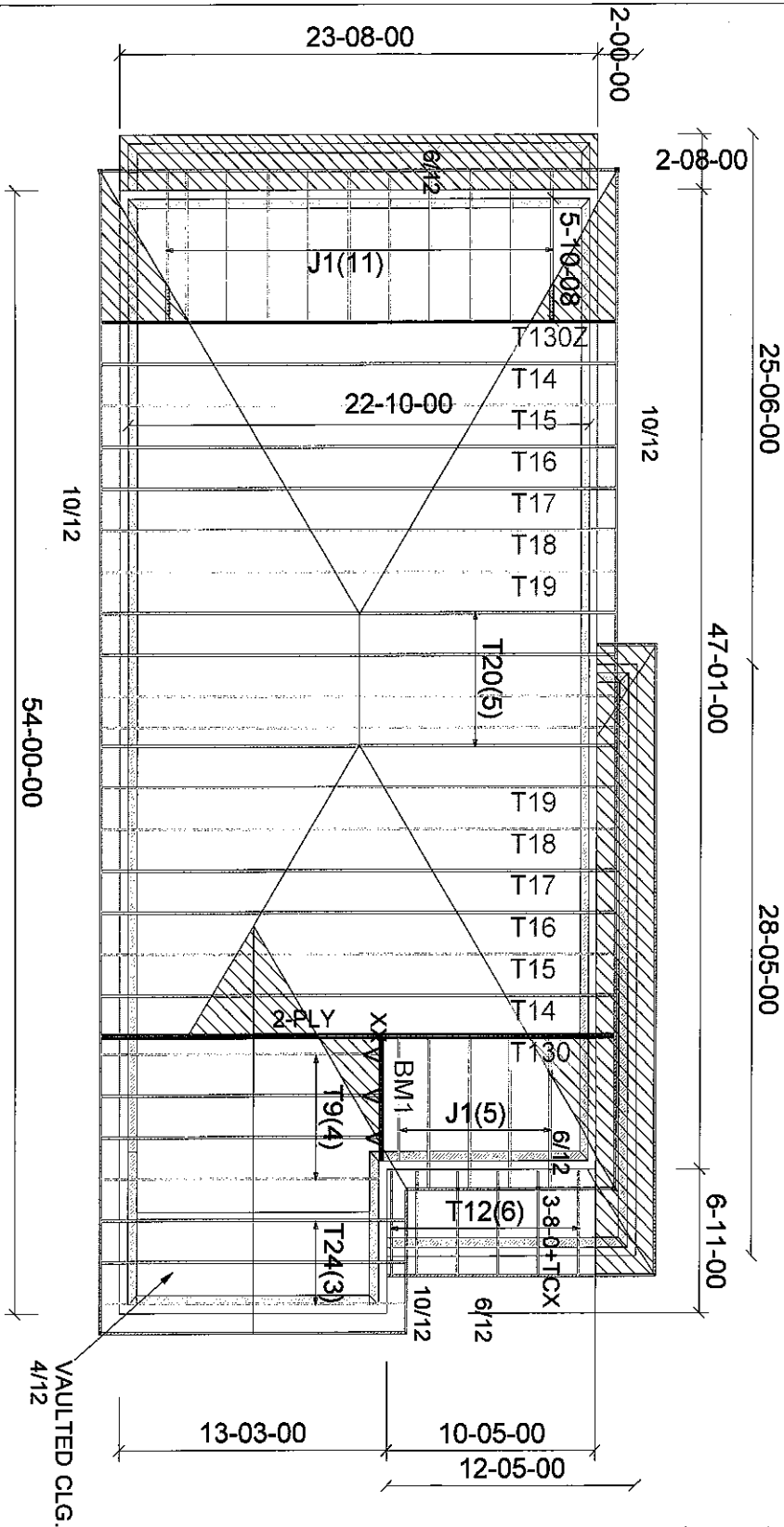
Job Track: **45246**
Layout ID: **289743**
Plan Log: **95085**

Builder / Location: **GREENPARK HOMES / EAST GWILLIMBURY**
Project: **SECONDO VALLEY ESTATES**
Date: **10/19/2017** Designer: **JGS/V**

Model / Elevation: **NEWBERRY 2A / 2**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

THIS TRUSS DESIGNED FOR FLAT SOLAR PANEL ARRAY TO BE INSTALLED ALONG TOP CHORDS HAVING A COMBINED DEAD LOAD (SOLAR PANEL ARRAY) NOT EXCEEDING 5.0 PSF AND IN ACCORDANCE WITH CURRENT EDITION OF TPIG. THIS IS A LOAD READY DECLARATION ONLY AND NOT AN INSTALLATION READY DECLARATION. A SEPARATE INSTALLATION DETAIL IS REQUIRED BY LICENSED PROFESSIONAL ENGINEER ONCE A SOLAR ARRAY LAYOUT IS ESTABLISHED, AND AVAILABLE ON A ROOF BY ROOF BASIS.



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN CONFORMS
WITH THE RELEVANT
SECTION OF THE
LATEST EDITION OF
O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
S_s = 2.3 kPa
TC DEAD 8 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES
CONVENTIONAL
FRAMING

HARDWARE
LS26DS(V)
HGUS26-2(XX)

BM1:2-2X10

T-150436

VAULTED CLG.
4/12

Millik



Job Track: **45246**
Layout ID: **289744**
Plan Log: **95085**

Builder / Location: **GREENPARK HOMES / EAST GWILLIMBURY**
Project: **SECONDO VALLEY ESTATES**
Date: **10/19/2017** Designer: **JG**

Model / Elevation: **NEWBERRY 2A / 3**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Millik ver 7.5.0



Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 45246 LAYOUT ID: 289742 LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALLEY EST SUB-BUILDER:
 MODEL: NEWBERRY 2A ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	8.00 0.00	22-10-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	106.95 65.33		
	1	T2 HIP	8.00 0.00	22-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	95.60 60.17		
	1	T3 HIP	8.00 0.00	22-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	101.14 63.33		
	1	T4 HIP	8.00 0.00	22-10-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	101.43 64.50		
	1	T5 HIP	8.00 0.00	22-10-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	106.78 66.33		
	4	T6 COMMON	8.00 0.00	22-10-00	09-00-02	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	392.64 248.00		
	1 2 Ply	T7 COMMON	8.00 0.00	22-10-00	09-00-02	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	260.74 159.34		
	11	T8 PIGGYBACK	10.00 0.00	21-01-00	10-00-00	2 X 4	2 X 4	00-00-00 00-00-00	03-04-02 03-04-02	1225.62 777.37		
	2	G8 PIGGYBACK	10.00 0.00	21-01-00	10-00-00	2 X 4	2 X 4	00-00-00 00-00-00	03-04-02 03-04-02	254.22 162.66		
	2	T9 COMMON	10.00 0.00	12-05-00	06-09-12	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	119.52 79.00		
	1	G9 COMMON	10.00 0.00	12-05-00	06-09-12	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	60.45 40.50		
	1	T10 COMMON	10.00 0.00	22-10-00	11-01-14	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	141.84 89.00		
	1 2 Ply	T10Z COMMON	10.00 0.00	22-10-00	11-01-14	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	288.34 180.68		
	7	T11 MONOPITCH	4.00 0.00	07-02-00	03-00-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-08-10	193.27 128.31		
	6	T12 MONOPITCH	6.00 0.00	03-08-00	02-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-04-12	102.72 66.00		
	13	PB1 PIGGYBACK	10.00 0.00	03-10-02	02-00-00	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	155.48 104.00		
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	184.69 117.37		

TOTAL # TRUSS= 67.00

TOTAL BFT OF ALL TRUSSES=

2471.89 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3891.43 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 45246	LAYOUT ID: 289742	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALLEY EST	SUB-BUILDER:	
MODEL: NEWBERRY 2A	ELEVATION: 1	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
22	Hangers	LJS26DS	

TOTAL # ITEMS= 22.00



Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 45246 LAYOUT ID: 289743 LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALLEY EST SUB-BUILDER:
 MODEL: NEWBERRY 2A ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	3	T9 COMMON	10.00 0.00	12-05-00	06-09-12	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	179.28 118.50		
	6	T12 MONOPITCH	6.00 0.00	03-08-00	02-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-04-12	102.72 66.00		
	2	T14 HIP	10.00 0.00	22-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	198.22 128.34		
	2	T15 HIP	10.00 0.00	22-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	195.74 123.34		
	2	T16 HIP	10.00 0.00	22-10-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	216.94 135.66		
	2	T17 HIP	10.00 0.00	22-10-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	217.30 139.34		
	2	T18 HIP	10.00 0.00	22-10-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	225.46 142.00		
	2	T19 HIP	10.00 0.00	22-10-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	236.58 150.66		
	5	T20 COMMON	10.00 0.00	22-10-00	11-01-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	576.80 358.35		
	1	T21 HIP GIRDER	10.00 0.00	09-03-00	02-08-09	2 X 4	2 X 6	00-00-00 00-00-00	01-03-08 01-03-08	40.91 26.33		
	1	T22 HIP GIRDER	10.00 0.00	12-05-00	06-00-09	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	74.73 51.00		
	2	T23 ROOF	10.00 0.00	12-05-00	06-09-12	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	128.58 89.66		
	1	T130 HIP GIRDER	10.00	22-10-00	04-01-04	2 X 6	2 X 6	01-03-08	01-07-11	272.98		
	2 Ply		0.00					01-03-08	01-07-11	168.00		
	1	T130Z HIP GIRDER	10.00 0.00	22-10-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	136.49 84.00		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		
	4	J2 JACK-OPEN	10.00 0.00	01-08-08	02-08-09	2 X 4	2 X 4	00-10-08 00-00-00	01-03-08 02-08-09	28.16 18.68		
	2	J3 JACK-OPEN	10.00 0.00	02-03-08	03-06-10	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-06-10	19.18 13.34		

TOTAL # TRUSS= 55.00

TOTAL BFT OF ALL TRUSSES=

1983.92 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3118.71 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 45246	LAYOUT ID: 289743	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALLEY EST	SUB-BUILDER:	
MODEL: NEWBERRY 2A	ELEVATION: 2	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
3	Hangers	LJS26DS	

TOTAL # ITEMS= 4.00



Delivery Shiplist

DATE	10/19/17
SALES REP	Mario

JOB TRACK: 45246 LAYOUT ID: 289744 LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALLEY EST SUB-BUILDER:
 MODEL: NEWBERRY 2A ELEVATION: 3

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	4	T9 COMMON	10.00 0.00	12-05-00	06-09-12	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	239.04 158.00		
	6	T12 MONOPITCH	6.00 0.00	03-08-00	02-10-00	2 X 4	2 X 4	01-03-08 00-00-00	00-06-12 02-04-12	102.72 66.00		
	2	T14 HIP	10.00 0.00	22-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	198.22 128.34		
	2	T15 HIP	10.00 0.00	22-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	195.74 123.34		
	2	T16 HIP	10.00 0.00	22-10-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	216.94 135.66		
	2	T17 HIP	10.00 0.00	22-10-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	217.30 139.34		
	2	T18 HIP	10.00 0.00	22-10-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	225.46 142.00		
	2	T19 HIP	10.00 0.00	22-10-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	236.58 150.66		
	5	T20 COMMON	10.00 0.00	22-10-00	11-01-14	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	576.80 358.35		
	3	T24 SCISSOR	10.00 4.00	12-05-00	06-09-12	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	177.06 118.50		
	1 2 Ply	T130 HIP GIRDER	10.00 0.00	22-10-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	272.98 168.00		
	1	T130Z HIP GIRDER	10.00 0.00	22-10-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	136.49 84.00		
	16	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	268.64 170.72		

TOTAL # TRUSS= 49.00

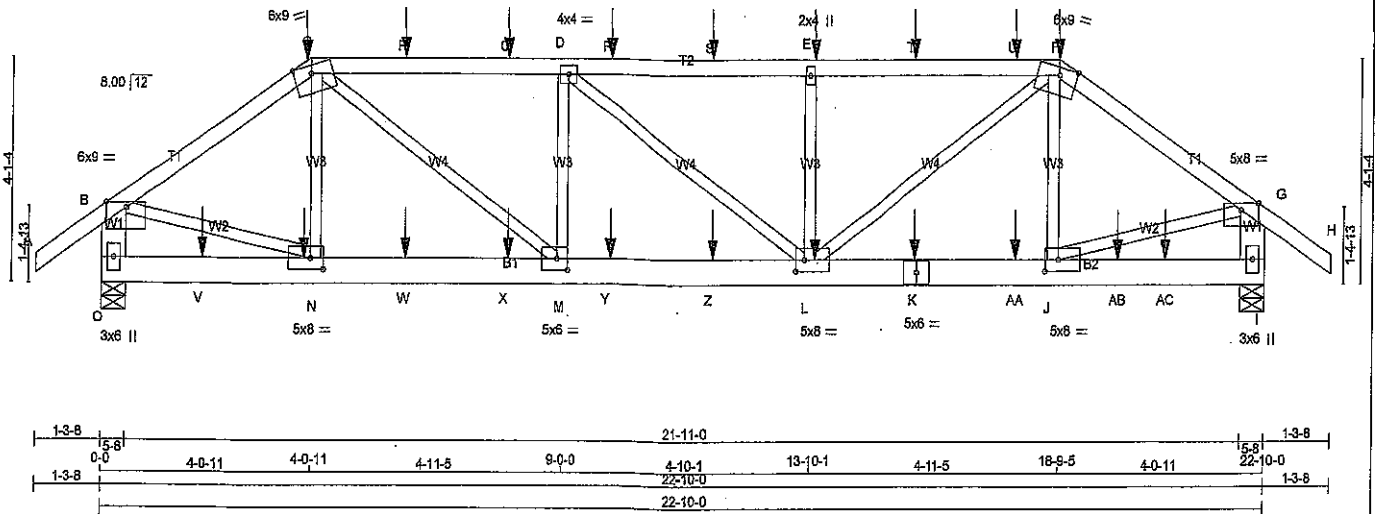
TOTAL BFT OF ALL TRUSSES=

1942.91 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3063.97 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	LJS26DS	
1	Hangers	HGUS26-2	

TOTAL # ITEMS= 4.00



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF	
C - F	2x4	DRY	1650F 1.5E	SPF	
F - H	2x4	DRY	1650F 1.5E	SPF	
O - B	2x6	DRY	No.2	SPF	
I - G	2x6	DRY	No.2	SPF	
O - K	2x6	DRY	No.2	SPF	
K - I	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	8.0	9.0	Edge		
C TMVW-m	MT20	8.0	9.0	1.75	4.00	
D TMVW-t	MT20	4.0	4.0			
E TMVW-w	MT20	2.0	4.0			
F TMVW-m	MT20	8.0	9.0	1.75	4.00	
G TMVW-p	MT20	5.0	8.0	Edge		
I BMV1+p	MT20	3.0	6.0			
J BMVW-t	MT20	5.0	8.0	2.50	3.00	
K BS-t	MT20	5.0	6.0			
L BMVW-t	MT20	5.0	8.0	2.50	2.00	
M BMVW-t	MT20	5.0	6.0	2.50	2.50	
N BMVW-t	MT20	5.0	8.0	2.50	3.00	
O BMV1+p	MT20	3.0	6.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 431.5 lbs FACTORED DOWN AT 4-0-11, 281.8 lbs FACTORED DOWN AT 18-9-5, 149.7 lbs FACTORED DOWN AT 5-11-4, 149.7 lbs FACTORED DOWN AT 7-11-4, 149.7 lbs FACTORED DOWN AT 9-11-4, 149.7 lbs FACTORED DOWN AT 11-11-4, 149.7 lbs FACTORED DOWN AT 13-11-4, AND 149.7 lbs FACTORED DOWN AT 15-11-4, AND 149.7 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, AND 69.9 lbs FACTORED DOWN AT 19-11-4, AND 69.9 lbs FACTORED DOWN AT 21-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
O	3027	0	0	5-8
I	2894	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
O	2410	1387 / 0 437 / 0 0 / 0 0 / 0 608 / 0 0 / 0
I	2397	1339 / 0 450 / 0 0 / 0 0 / 0 608 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC)	MEMB.	FORCE (LBS) MAX CSI (LC)
FR-TO		FROM TO	FR-TO	
A-B	0 / 47	-124.4 -124.4 0.15 (1)	10.00	N-C -424 / 91 0.11 (1)
B-C	-3361 / 0	-124.4 -124.4 0.44 (1)	3.90	C-M 0 / 2098 0.52 (1)
C-P	-4455 / 0	-124.4 -124.4 0.82 (1)	2.88	M-D -1028 / 0 0.26 (1)
P-Q	-4455 / 0	-124.4 -124.4 0.82 (1)	2.88	D-L -36 / 0 0.02 (1)
Q-D	-4455 / 0	-124.4 -124.4 0.82 (1)	2.88	L-E -1052 / 0 0.27 (1)
D-R	-4427 / 0	-124.4 -124.4 0.81 (1)	2.89	E-F 0 / 2110 0.52 (1)
R-S	-4427 / 0	-124.4 -124.4 0.81 (1)	2.89	J-F -382 / 121 0.10 (1)
S-E	-4427 / 0	-124.4 -124.4 0.81 (1)	2.89	B-N 0 / 2871 0.71 (1)
E-T	-4427 / 0	-124.4 -124.4 0.76 (1)	3.02	J-G 0 / 2828 0.70 (1)
T-U	-4427 / 0	-124.4 -124.4 0.76 (1)	3.02	
U-F	-4427 / 0	-124.4 -124.4 0.76 (1)	3.02	
F-G	-3311 / 0	-124.4 -124.4 0.44 (1)	3.93	
G-H	0 / 47	-124.4 -124.4 0.15 (1)	10.00	
O-B	-2863 / 0	0.0 0.0 0.21 (1)	6.07	
I-G	-2926 / 0	0.0 0.0 0.21 (1)	6.10	
O-V	0 / 0	-28.0 -28.0 0.11 (3)	10.00	
V-N	0 / 0	-28.0 -28.0 0.11 (3)	10.00	
N-W	0 / 2780	-28.0 -28.0 0.43 (1)	10.00	
W-X	0 / 2780	-28.0 -28.0 0.43 (1)	10.00	
X-M	0 / 2780	-28.0 -28.0 0.43 (1)	10.00	
M-Y	0 / 4455	-28.0 -28.0 0.70 (1)	10.00	
Y-Z	0 / 4455	-28.0 -28.0 0.70 (1)	10.00	
Z-L	0 / 4455	-28.0 -28.0 0.70 (1)	10.00	
L-K	0 / 2740	-28.0 -28.0 0.45 (1)	10.00	
K-A	0 / 2740	-28.0 -28.0 0.45 (1)	10.00	
AA-J	0 / 2740	-28.0 -28.0 0.45 (1)	10.00	
J-AB	0 / 0	-28.0 -28.0 0.13 (3)	10.00	
AB-AC	0 / 0	-28.0 -28.0 0.13 (3)	10.00	
AC-I	0 / 0	-28.0 -28.0 0.13 (3)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
C	4-0-11	-431	-431	FRONT	VERT	TOTAL
E	13-11-4	-150	-150	FRONT	VERT	TOTAL
F	18-9-5	-282	-282	FRONT	VERT	TOTAL
K	15-11-4	-40	-70	FRONT	VERT	TOTAL
L	13-11-4	-40	-70	FRONT	VERT	TOTAL
N	3-11-4	-40	-70	FRONT	VERT	TOTAL
P	5-11-4	-150	-150	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.76")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.16")
ALLOWABLE DEFL. (TL) = $L/360$ (0.76")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.27")

CSI: TC=0.82 (C-D:1), BC=0.70 (L-M:1), WB=0.71 (B-N:1), SS=0.49 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (G) (INPUT = 0.90)
JSI METAL = 0.58 (B) (INPUT = 1.00)

DWG NO. TAN 52-V-16 P1 P6 1/2
STRUCTURAL
COMPONENT ONLY
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	39938	DRWG NO.
254312	T1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	7-11-4	-150	-150	—	FRONT	VERT	TOTAL
R	9-11-4	-150	-150	—	FRONT	VERT	TOTAL
S	11-11-4	-150	-150	—	FRONT	VERT	TOTAL
T	15-11-4	-150	-150	—	FRONT	VERT	TOTAL
U	17-11-4	-150	-150	—	FRONT	VERT	TOTAL
V	1-11-4	-40	-70	—	FRONT	VERT	TOTAL
W	6-11-4	-40	-70	—	FRONT	VERT	TOTAL
X	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
Y	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
Z	11-11-4	-40	-70	—	FRONT	VERT	TOTAL
AA	17-11-4	-40	-70	—	FRONT	VERT	TOTAL
AB	19-11-4	-40	-70	—	FRONT	VERT	TOTAL
AC	20-10-12	-40	-70	—	FRONT	VERT	TOTAL



P622
DWG NO. TAM 521-1021
STRUCTURAL
COMPONENT ONLY



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
<u>BEARINGS</u>									
	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
O	1910	0	1910	0	0	5-8	2-1		
J	1910	0	1910	0	0	5-8	2-1		
<u>UNFACTORED REACTIONS</u>									
JT	1ST LCASE COMBINED	MAX MIN. SNOW	MAX MIN. LIVE	MAX MIN. PERM. LIVE	WIND	DEAD	SOIL		
O	1494	890 / 0	240 / 0	0 / 0	0 / 0	364 / 0	0 / 0		
J	1494	890 / 0	240 / 0	0 / 0	0 / 0	364 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S					W E B S				
MAX. FACTORED		VERT. LOAD	LC1	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MAX. FACTORED		MAX. CS1 (LC)	
MEMB.	FORCE (LBS)					MEMB.	FORCE (LBS)		
FR-TO		FROM	TO		FR-TO				
A-B	0/47	-124.4	-124.4	0.17 (1)	10.00	C-N	0/163	0.04 (3)	
B-C	0/18	-124.4	-124.4	0.12 (1)	10.00	N-D	0/211	0.05 (3)	
C-D	-1877/0	-124.4	-124.4	0.17 (1)	4.75	D-M	0/822	0.18 (1)	
D-E	-2181/0	-124.4	-124.4	0.89 (1)	3.31	M-E	-893/0	0.34 (1)	
E-F	-2181/0	-124.4	-124.4	0.89 (1)	3.31	M-F	0/822	0.18 (1)	
F-G	-1877/0	-124.4	-124.4	0.17 (1)	4.75	K-G	0/211	0.05 (3)	
G-H	0/18	-124.4	-124.4	0.12 (1)	10.00	K-G	0/163	0.04 (3)	
H-I	0/47	-124.4	-124.4	0.17 (1)	10.00	O-C	-2115/0	0.56 (1)	
O-B	-311/0	0.0	0.0	0.03 (1)	7.81	G-J	-2115/0	0.56 (1)	
J-H	-311/0	0.0	0.0	0.03 (1)	7.81				
O-N	0/1454	-28.0	-28.0	0.38 (2)	10.00				
N-M	0/1545	-28.0	-28.0	0.40 (2)	10.00				
M-L	0/1545	-28.0	-28.0	0.40 (2)	10.00				
L-K	0/1545	-28.0	-28.0	0.40 (2)	10.00				
K-J	0/1454	-28.0	-28.0	0.38 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

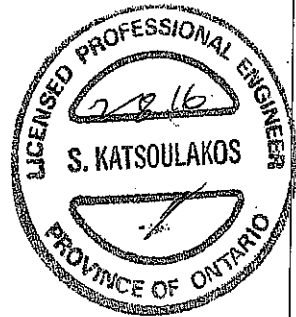
CSL: TC=0.89 (E-F:1), BC=0.40 (M-N:2), WB=0.56 (G-J:1), SSL=0.35 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS SEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

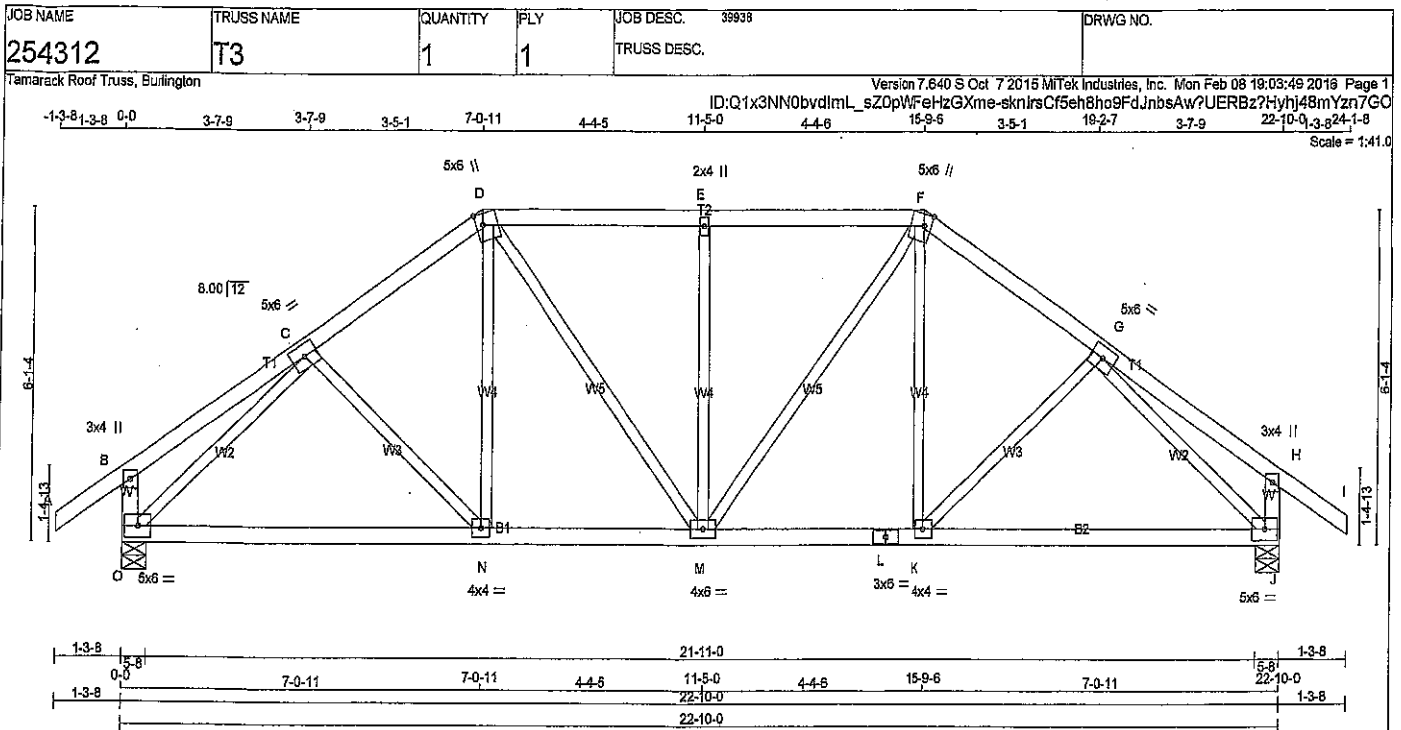
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PSI) (PSI)



DWG NO. TAM 5212-16 R1
STRUCTURAL
COMPONENT ONLY



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	SPF	SPF
A - D	2x4	DRY	No.2	SPF	SPF
D - F	2x4	DRY	No.2	SPF	SPF
F - I	2x4	DRY	No.2	SPF	SPF
O - B	2x4	DRY	No.2	SPF	SPF
J - H	2x4	DRY	No.2	SPF	SPF
O - L	2x4	DRY	No.2	SPF	SPF
L - J	2x4	DRY	No.2	SPF	SPF
ALL WEBS	2x3	DRY	No.2	SPF	SPF
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMV+p	MT20	5.0	6.0	
D	TTWW+m	MT20	5.0	6.0	2.50 1.50
E	TMV+p	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	6.0	2.50 1.50
G	TMV+p	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW+1	MT20	5.0	6.0	
K	BMVW+1	MT20	4.0	4.0	
L	BS-1	MT20	3.0	6.0	
M	BMVW+1	MT20	4.0	6.0	
N	BMVW+1	MT20	4.0	4.0	
O	BMVW+1	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
O	1910	0	1910	0	0	5-8	2-1		
J	1910	0	1910	0	0	5-8	2-1		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
O	1494	890 / 0	240 / 0	0 / 0	0 / 0	364 / 0	0 / 0		
J	1494	890 / 0	240 / 0	0 / 0	0 / 0	364 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

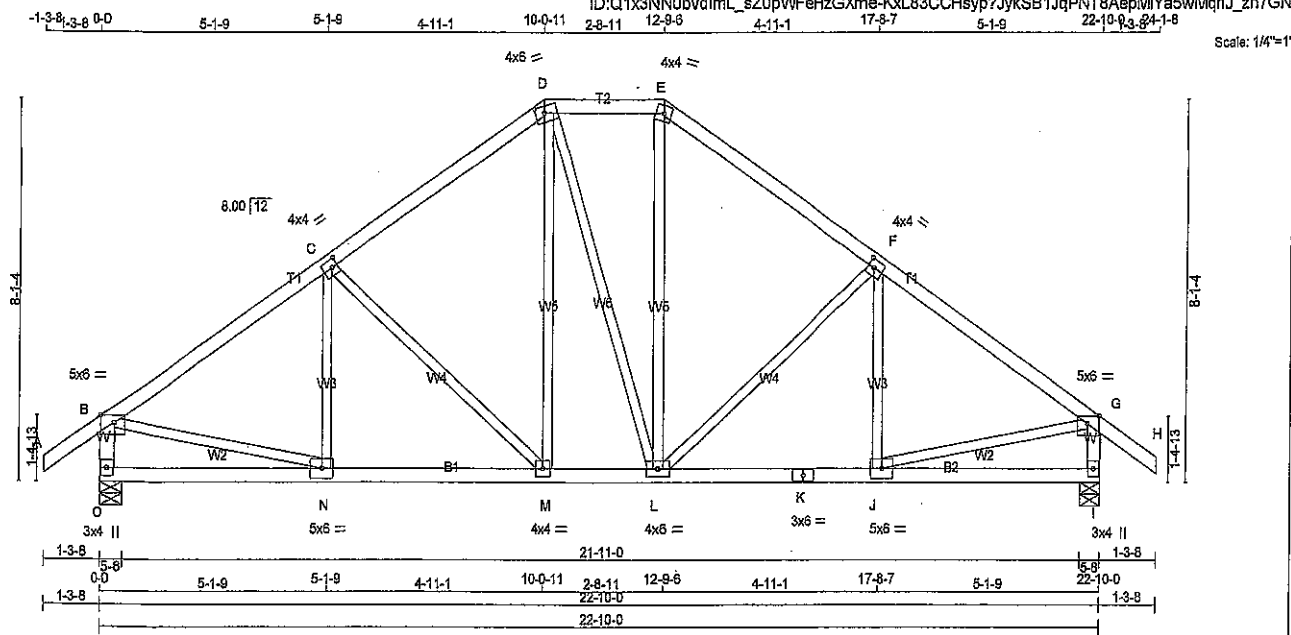
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00	C-N	-60 / 94
B-C	0 / 26	-124.4	-124.4	0.23 (1)	10.00	N-D	0 / 328
C-D	-1810 / 0	-124.4	-124.4	0.21 (1)	4.78	D-M	0 / 446
D-E	-1754 / 0	-124.4	-124.4	0.33 (1)	4.70	M-E	-658 / 0
E-F	-1754 / 0	-124.4	-124.4	0.33 (1)	4.70	M-F	0 / 446
F-G	-1810 / 0	-124.4	-124.4	0.21 (1)	4.79	K-G	0 / 328
G-H	0 / 26	-124.4	-124.4	0.23 (1)	10.00	K-F	-50 / 94
H-I	0 / 47	-124.4	-124.4	0.17 (1)	10.00	C-C	-2129 / 0
C-B	-342 / 0	0.0	0.0	0.04 (1)	7.81	G-J	-2129 / 0
J-H	-342 / 0	0.0	0.0	0.04 (1)	7.81		
O-N	0 / 1525	-28.0	-28.0	0.47 (2)	10.00		
N-M	0 / 1486	-28.0	-28.0	0.48 (2)	10.00		
M-L	0 / 1486	-28.0	-28.0	0.48 (2)	10.00		
L-K	0 / 1486	-28.0	-28.0	0.48 (2)	10.00		
K-J	0 / 1525	-28.0	-28.0	0.47 (2)	10.00		



TOTAL WEIGHT = 107 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	3.25
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	4.0	6.0	1.75	2.50
E	TTVW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	Edge	3.25
I	BMV1-p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	8.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT	REQRD		
GROSS REACTION		GROSS REACTION		BRG	BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1910	0	1910	0	0	5-8	2-11
I	1910	0	1910	0	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST LCASE		MAX MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
O	1494	890 / 0	240 / 0	0 / 0	0 / 0	364 / 0	0 / 0
I	1494	890 / 0	240 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED		
		VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH					
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00	N-C	-182 / 136	0.06 (1)	
B-C	-1925 / 0	-124.4	-124.4	0.46 (1)	4.36	C-M	-512 / 0	0.43 (1)	
C-D	-1551 / 0	-124.4	-124.4	0.43 (1)	4.79	M-D	0 / 436	0.10 (1)	
D-E	-1261 / 0	-124.4	-124.4	0.13 (1)	5.60	D-L	0 / 8	0.00 (1)	
E-F	-1553 / 0	-124.4	-124.4	0.43 (1)	4.79	L-E	0 / 441	0.10 (1)	
F-G	-1924 / 0	-124.4	-124.4	0.46 (1)	4.36	L-F	-509 / 0	0.43 (1)	
G-H	0 / 47	-124.4	-124.4	0.17 (1)	10.00	J-F	-186 / 133	0.06 (1)	
O-B	-1851 / 0	0.0	0.0	0.19 (1)	6.16	B-N	0 / 1671	0.38 (1)	
I-G	-1850 / 0	0.0	0.0	0.19 (1)	6.16	J-G	0 / 1670	0.38 (1)	
O-N	0 / 0	-28.0	-28.0	0.20 (3)	10.00				
N-M	0 / 1635	-28.0	-28.0	0.35 (2)	10.00				
M-L	0 / 1259	-28.0	-28.0	0.26 (1)	10.00				
L-K	0 / 1634	-28.0	-28.0	0.35 (2)	10.00				
K-J	0 / 1634	-28.0	-28.0	0.35 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.20 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/950 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/390 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.46 (B-C:1), BC=0.36 (M-N:2), WB=0.43 (C-M:1), CSI=0.25 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1657

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.80)
JSI METAL= 0.46 (K) (INPUT = 1.00)

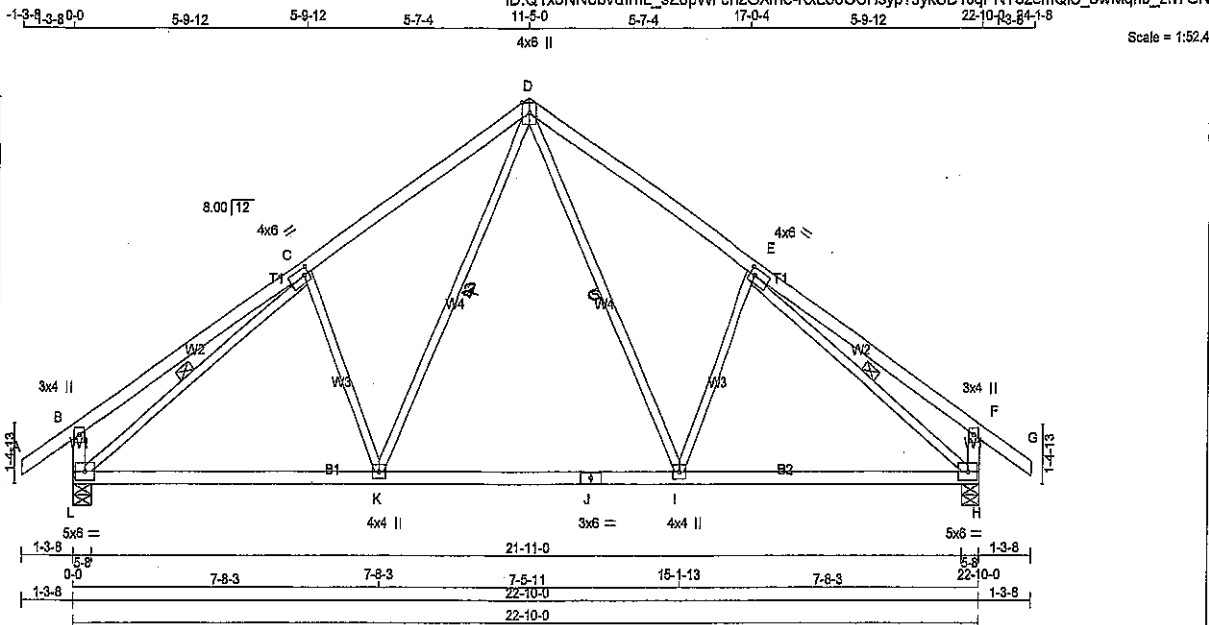


DWG NO. TAM 5215-18R1
STRUCTURAL COMPONENT ONLY

Temarack Roof Truss, Burlington

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TOTAL WEIGHT = 3 X 98 = 294 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
L	1810	0	1810	0	5-8
H	1810	0	1810	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN.	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
L	1484	880/0	240/0	0/0
H	1484	880/0	240/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1- 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H. DBS = 8'-0". CBF = 106 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/47	-124.4 -124.4	0.17 (1)	10.00	D-I	0/742	0.17 (1)
B-C	0/47	-124.4 -124.4	0.67 (1)	10.00	I-E	-488/0	0.23 (1)
C-D	-1766/0	-124.4 -124.4	0.58 (1)	4.33	K-D	0/742	0.17 (1)
D-E	-1766/0	-124.4 -124.4	0.58 (1)	4.33	C-K	-488/0	0.23 (1)
E-F	0/47	-124.4 -124.4	0.67 (1)	10.00	L-C	-2128/0	0.79 (1)
F-G	0/47	-124.4 -124.4	0.17 (1)	10.00	E-H	-2128/0	0.79 (1)
G-H	-439/0	0.0	0.05 (1)	7.81			
H-F	-439/0	0.0	0.05 (1)	7.81			
L-K	0/1615	-28.0	-28.0	0.54 (2)	10.00		
K-J	0/1137	-28.0	-28.0	0.50 (3)	10.00		
J-I	0/1137	-28.0	-28.0	0.50 (3)	10.00		
I-H	0/1615	-28.0	-28.0	0.54 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 240 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.76")
CALCULATED VERT. DEFL.(LL) = 1/899 (0.15")
ALLOWABLE DEFL.(TL) = 1/360 (0.76")
CALCULATED VERT. DEFL.(TL) = 1/599 (0.25")

CSI: TC=0.67 (B-C:1), BC=0.54 (K-L:2), WB=0.79 (C-L:1), SSI=0.29 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

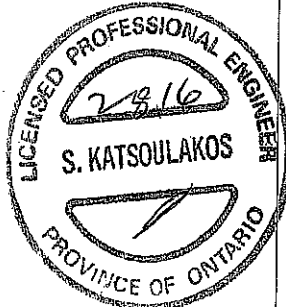
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354 1667	822 2284 1666	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.55 (E) (INPUT = 1.00)



DRWG NO. TAM 5216-16R1
STRUCTURAL
COMPONENT ONLY

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1578.2 lbs FACTORED DOWN AT
2-0-12, 1578.2 lbs FACTORED DOWN AT 4-0-12,
1578.2 lbs FACTORED DOWN AT 4-8-4, 1578.2
lbs FACTORED DOWN AT 8-9-4, 1578.2 lbs
FACTORED DOWN AT 10-9-4, 1578.2 lbs
FACTORED DOWN AT 12-9-4, 1578.2 lbs
FACTORED DOWN AT 14-9-4, 1578.2 lbs
FACTORED DOWN AT 16-9-4, AND 1578.2 lbs
FACTORED DOWN AT 18-9-4, AND 1578.2 lbs
FACTORED DOWN AT 20-9-4 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	18-9-4	-1578	-1578	---	BACK	VERT	TOTAL
P	4-0-12	-1578	-1578	---	BACK	VERT	TOTAL
R	2-0-12	-1578	-1578	---	BACK	VERT	TOTAL
S	4-9-4	-1578	-1578	---	BACK	VERT	TOTAL
T	6-9-4	-1578	-1578	---	BACK	VERT	TOTAL
U	8-9-4	-1578	-1578	---	BACK	VERT	TOTAL
V	10-9-4	-1578	-1578	---	BACK	VERT	TOTAL
W	12-9-4	-1578	-1578	---	BACK	VERT	TOTAL
X	14-9-4	-1578	-1578	---	BACK	VERT	TOTAL
Y	16-9-4	-1578	-1578	---	BACK	VERT	TOTAL
Z	20-9-4	-1578	-1578	---	BACK	VERT	TOTAL



DWG NO. TAM 5217-16R4
STRUCTURAL
COMPONENT - ONLY



[MIF]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	60.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/823 (0.31")

CSI: TC=0.42 (G-D:1), BC=0.48 (G-H:3), WB=0.58 (B-K:1), SS=0.25 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE	GRIP(DRY)		SHEAR SECTION	
	(PSI)		(PLI)	
	MAX	MIN	MAX	MIN
MT20	618	354	1667	822
			2284	1655

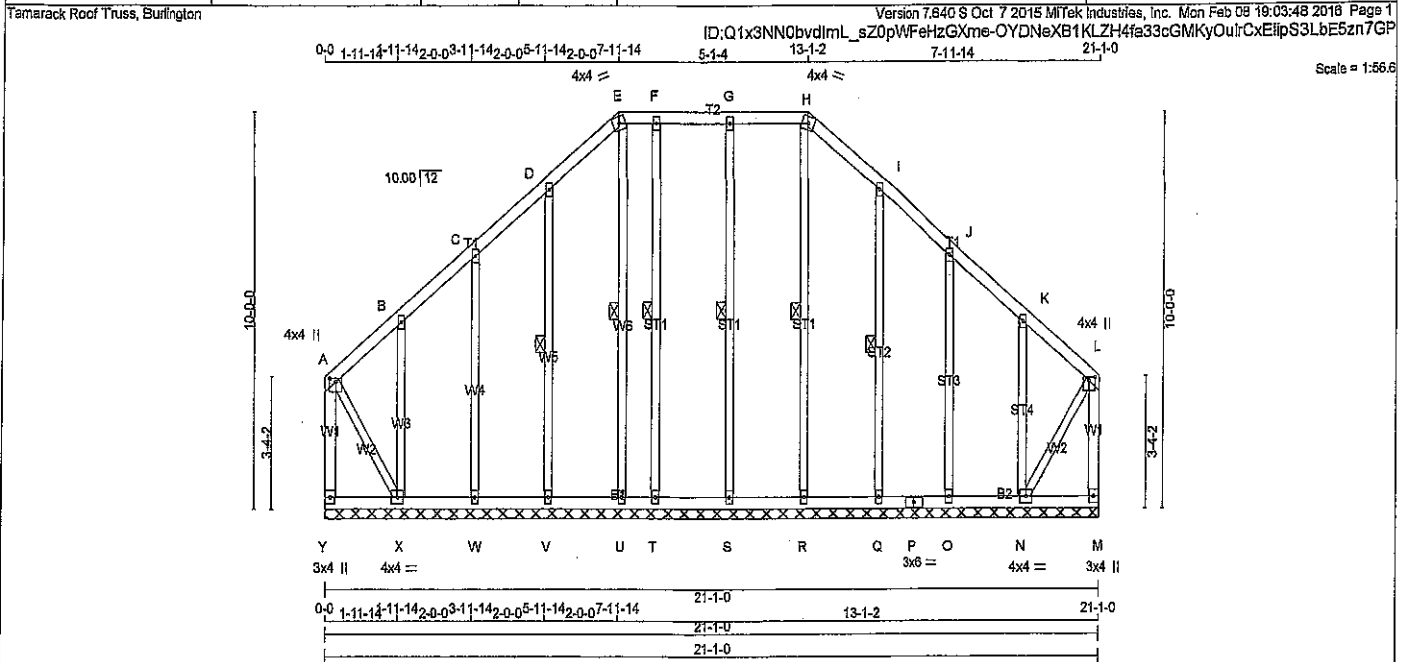
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

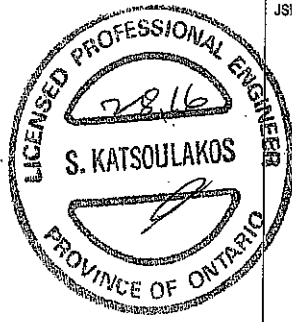
JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.51 (I) (INPUT = 1.00)

DWG NO. TAM 5218-16 R1
STRUCTURAL
COMPONENT ONLY

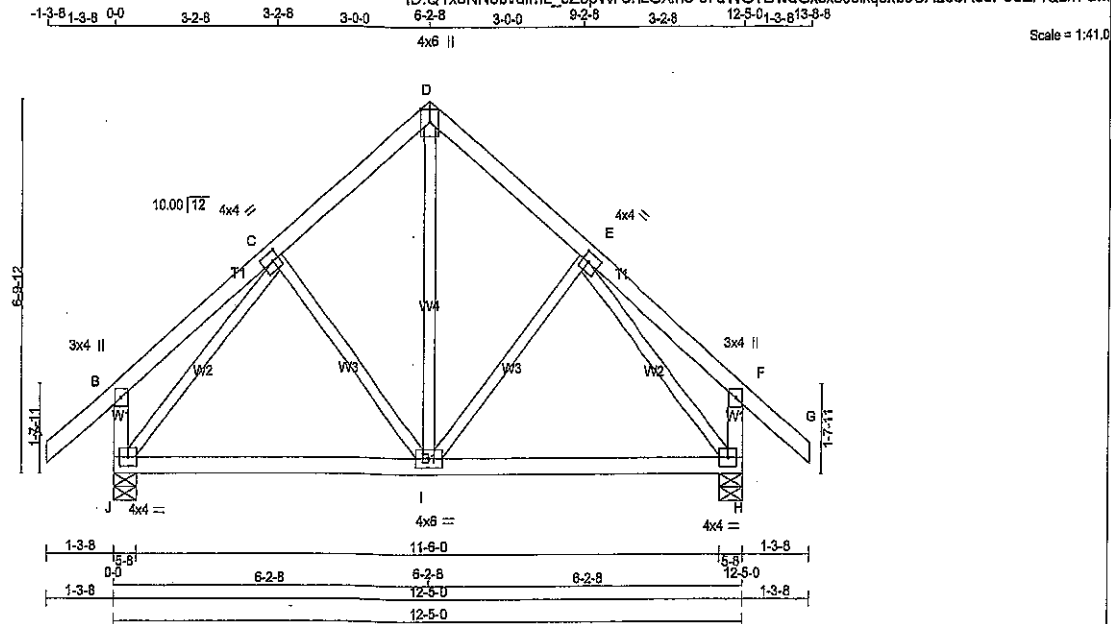




LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. Y - A 2x4 DRY No.2 SPF A - E 2x4 DRY No.2 SPF E - H 2x4 DRY No.2 SPF H - L 2x4 DRY No.2 SPF M - L 2x4 DRY No.2 SPF Y - P 2x4 DRY No.2 SPF P - M 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF ALL GABLE WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2'-0" OC.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 34.8 PSF DL = 8.0 PSF BOT CH. LL = 10.6 PSF DL = 7.0 PSF TOTAL LOAD = 60.3 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, SCBC 2012, ABC 2014 - CSA 089-09 - TPIC 2011 (55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD CSI: TC=0.07 (G-H-1), BC=0.03 (W-X-3), WB=0.20 (G-S-1), SS=0.12 (G-H-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.57 (H) (INPUT = 0.90) JSI METAL= 0.07 (I) (INPUT = 1.00)				PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TMWV+p MT20 4.0 4.0 1.00 2.00 B, C, D, F, G, I, J, K B TMWV+w MT20 2.0 4.0 E TTW-m MT20 4.0 4.0 H TTW-m MT20 4.0 4.0 L TMWV+p MT20 4.0 4.0 1.00 2.00 M BMV1+p MT20 3.0 4.0 N BMWV1-t MT20 4.0 4.0 O, Q, R, S, T, U, V, W O BMW1+w MT20 2.0 4.0 P BS-t MT20 3.0 6.0 X BMWV1-t MT20 4.0 4.0 Y BMV1+p MT20 3.0 4.0				LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED MEMB. FORCE (LBS) FACTORED VERT. LOAD LC1 (PLF) MAX. CS1 (LC) UNBRAC LENGTH FR-TO Y-A -151 / 0 0.0 0.0 0.03 (1) 7.81 A-B -14 / 0 -124.4 -124.4 0.07 (1) 6.25 B-C -26 / 0 -124.4 -124.4 0.07 (1) 6.25 C-D -15 / 0 -124.4 -124.4 0.06 (1) 6.25 D-E -29 / 0 -124.4 -124.4 0.06 (1) 6.25 E-F -9 / 0 -124.4 -124.4 0.04 (1) 10.00 F-G -9 / 0 -124.4 -124.4 0.07 (1) 10.00 G-H -9 / 0 -124.4 -124.4 0.07 (1) 10.00 H-I -31 / 0 -124.4 -124.4 0.07 (1) 6.25 I-J -16 / 0 -124.4 -124.4 0.07 (1) 6.25 J-K -27 / 0 -124.4 -124.4 0.07 (1) 6.25 K-L -15 / 0 -124.4 -124.4 0.07 (1) 6.25 M-L -153 / 0 0.0 0.0 0.03 (1) 7.81 Y-X 0 / 0 -28.0 -28.0 0.03 (3) 10.00 X-W 0 / 17 -28.0 -28.0 0.03 (3) 10.00 W-V 0 / 14 -28.0 -28.0 0.03 (3) 10.00 V-U 0 / 11 -28.0 -28.0 0.03 (3) 10.00 U-T 0 / 9 -28.0 -28.0 0.02 (3) 10.00 T-S 0 / 9 -28.0 -28.0 0.03 (3) 10.00 S-R 0 / 9 -28.0 -28.0 0.03 (3) 10.00 R-Q 0 / 11 -28.0 -28.0 0.03 (3) 10.00 Q-P 0 / 15 -28.0 -28.0 0.03 (3) 10.00 P-O 0 / 15 -28.0 -28.0 0.03 (3) 10.00 O-N 0 / 18 -28.0 -28.0 0.03 (3) 10.00 N-M 0 / 0 -28.0 -28.0 0.03 (3) 10.00				WEBS MAX. FACTORED MEMB. FORCE (LBS) MAX. CS1 (LC) UNBRAC LENGTH FR-TO R-H -195 / 0 0.13 (1) S-G -293 / 0 0.20 (1) T-F -200 / 0 0.14 (1) Q-I -275 / 0 0.12 (1) O-J -235 / 0 0.18 (1) N-K -275 / 0 0.10 (1) U-E -127 / 0 0.09 (1) V-D -273 / 0 0.12 (1) W-C -236 / 0 0.18 (1) X-B -275 / 0 0.10 (1) A-X 0 / 40 0.01 (1) N-L 0 / 41 0.01 (1)			
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DWG NO. TAM 5219-1621
STRUCTURAL COMPONENT ONLY



TOTAL WEIGHT = 2 X 60 = 120 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
J	1118	0	1118	0	5-8	1-8
H	1118	0	1118	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	COMBINED					
J	887	528 / 0	130 / 0	0 / 0	0 / 0	208 / 0
H	887	528 / 0	130 / 0	0 / 0	0 / 0	208 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)
FR-TO		FROM TO	FR-TO	FROM TO
A-B	0 / 55	-124.4 -124.4 0.17 (1)	I-D	0 / 483
B-C	0 / 28	-124.4 -124.4 0.19 (1)	I-E	-173 / 19
C-D	-643 / 0	-124.4 -124.4 0.15 (1)	C-I	-173 / 19
D-E	-643 / 0	-124.4 -124.4 0.15 (1)	J-C	-935 / 0
E-F	0 / 28	-124.4 -124.4 0.19 (1)	E-H	-935 / 0
F-G	0 / 55	-124.4 -124.4 0.17 (1)		
J-B	-320 / 0	0.0 0.0 0.03 (1)		
H-F	-320 / 0	0.0 0.0 0.03 (1)		
J-I	0 / 582	-28.0 -28.0 0.39 (3)		
I-H	0 / 582	-28.0 -28.0 0.39 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, ECBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")

CALCULATED VERT. DEFL.(LL) = L/899 (0.05")

ALLOWABLE DEFL.(TL) = L/360 (0.41")

CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.19 (E-F:1), BC=0.39 (H-I:3), WB=0.37 (C-J:1), SSI=0.16 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (E) (INPUT = 0.80)

JSI METAL= 0.34 (E) (INPUT = 1.00)



DRWG NO. **TAM 5220-16 R-1**

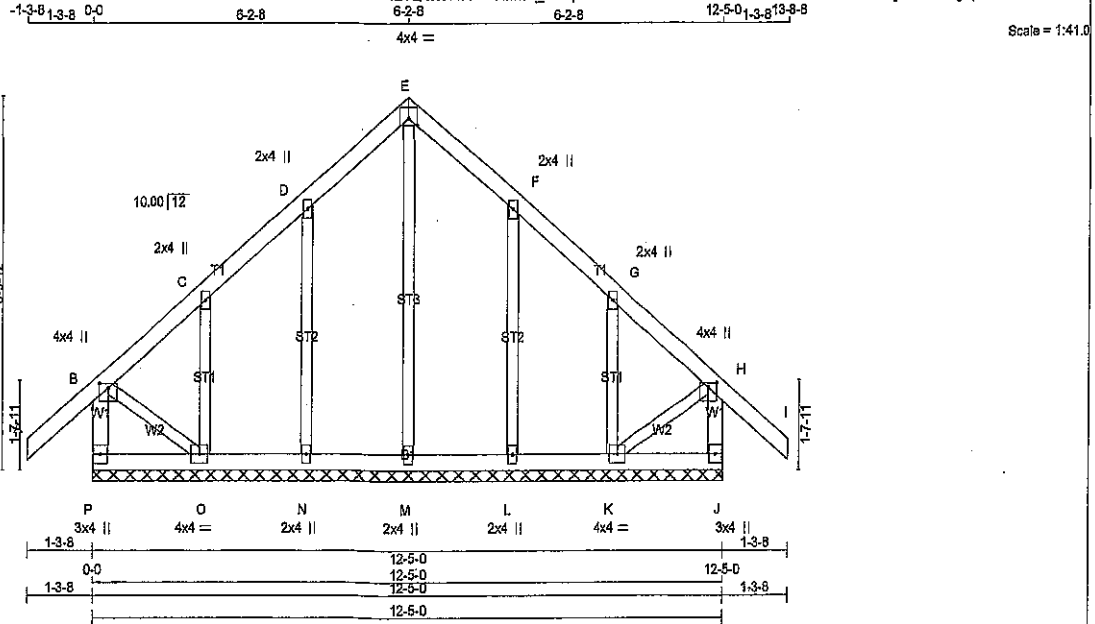
STRUCTURAL

COMPONENT ONLY

Tamareck Roof Truss, Burlington

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TOTAL WEIGHT = 60 LB

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
P-B	-331 / 0	0.0	0.0	0.04 (1)	7.81	M-E	-173 / 0 0.13 (1)		
A-B	0 / 55	-124.4	-124.4	0.17 (1)	10.00	N-D	-273 / 0 0.11 (1)		
B-C	-32 / 0	-124.4	-124.4	0.07 (1)	6.25	O-C	-276 / 0 0.08 (1)		
C-D	-38 / 0	-124.4	-124.4	0.07 (1)	6.25	L-F	-273 / 0 0.11 (1)		
D-E	-46 / 0	-124.4	-124.4	0.07 (1)	6.25	K-G	-276 / 0 0.08 (1)		
E-F	-46 / 0	-124.4	-124.4	0.07 (1)	6.25	B-O	0 / 43 0.01 (1)		
F-G	-38 / 0	-124.4	-124.4	0.07 (1)	6.25	K-H	0 / 43 0.01 (1)		
G-H	-32 / 0	-124.4	-124.4	0.07 (1)	6.25				
H-I	0 / 55	-124.4	-124.4	0.17 (1)	10.00				
J-H	-331 / 0	0.0	0.0	0.04 (1)	7.81				
P-O	0 / 0	-28.0	-28.0	0.04 (3)	10.00				
O-N	0 / 29	-28.0	-28.0	0.04 (3)	10.00				
N-M	0 / 23	-28.0	-28.0	0.03 (3)	10.00				
M-L	0 / 23	-28.0	-28.0	0.03 (3)	10.00				
L-K	0 / 29	-28.0	-28.0	0.04 (3)	10.00				
K-J	0 / 0	-28.0	-28.0	0.04 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17 (A-B:1), BC=0.04 (K-L:3), WB=0.13 (E-M:1), SSI=0.10 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

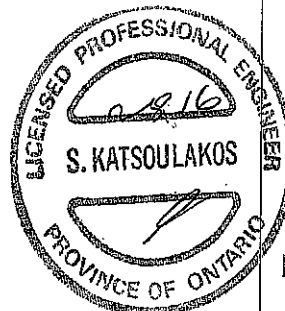
PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)

JSI METAL= 0.07 (C) (INPUT = 1.00)



DWG NO. TAM 5221-16 R4

STRUCTURAL

COMPONENT ONLY



TOTAL WEIGHT = 142 lb

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
<u>BEARINGS</u>									
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG			
	VERT	HORZ	DOWN	HORZ					
S	1912	0	1912	0	5-8		2-1		
L	1912	0	1912	0	5-8		2-1		
<u>UNFACTORED REACTIONS</u>									
JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL	
	COMBINED	SNOW	LIVE	PERM. LIVE					
S	1485	891 / 0	240 / 0	0 / 0	0 / 0	365 / 0	0 / 0		
L	1485	891 / 0	240 / 0	0 / 0	0 / 0	365 / 0	0 / 0		

DRY: SEASONED LUMBER.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERMETER CORNER JOINTS MUST BE Laterally Restrained.

2x3 DRY SPF No.2 T-BRACE AT H-P, D-P

FASTEN T AND H-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 95% OF WEB LENGTH.

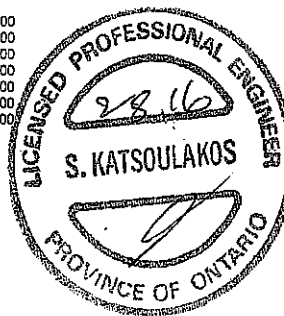
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	WEBS
--------	------

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 55	-124.4	-124.4	0.17 (1)	10.00	P-F	0 / 1173
B-C	-1681 / 0	-124.4	-124.4	0.26 (1)	4.91	P-H	-567 / 0
C-D	-1540 / 0	-124.4	-124.4	0.22 (1)	5.09	O-H	0 / 252
D-E	-1240 / 0	-124.4	-124.4	0.21 (1)	5.55	O-I	-184 / 0
E-F	-1240 / 0	-124.4	-124.4	0.21 (1)	5.55	M-I	-288 / 28
F-G	-1240 / 0	-124.4	-124.4	0.21 (1)	5.55	D-P	-567 / 0
G-H	-1240 / 0	-124.4	-124.4	0.21 (1)	5.55	Q-D	0 / 252
H-I	-1540 / 0	-124.4	-124.4	0.22 (1)	5.08	C-Q	-184 / 0
I-J	-1651 / 0	-124.4	-124.4	0.26 (1)	4.91	R-C	-288 / 28
J-K	0 / 55	-124.4	-124.4	0.17 (1)	10.00	B-R	0 / 1359
S-B	-1858 / 0	0.0	0.0	0.13 (1)	7.34	M-J	0 / 1359
L-J	-1858 / 0	0.0	0.0	0.13 (1)	7.34		
S-R	0 / 0	-28.0	-28.0	0.04 (3)	10.00		
R-Q	0 / 1298	-28.0	-28.0	0.12 (1)	10.00		
Q-P	0 / 1181	-28.0	-28.0	0.12 (1)	10.00		
P-O	0 / 1181	-28.0	-28.0	0.12 (1)	10.00		
O-N	0 / 1298	-28.0	-28.0	0.12 (1)	10.00		
N-M	0 / 1298	-28.0	-28.0	0.12 (1)	10.00		
M-L	0 / 0	-28.0	-28.0	0.04 (3)	10.00		



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	34.8	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	60.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIG 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.76")
CALCULATED VERT. DEFL.(LL)= $L/999$ (0.05")
ALLOWABLE DEFL.(TL)= $L/360$ (0.76")
CALCULATED VERT. DEFL.(TL)= $L/999$ (0.08")

CSI: TC=0.25 (B-C:1), BC=0.12 (Q-R:1), WB=0.68 (H-P:1), SSI=0.17 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

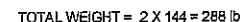
NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1687	822	2284 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRIP= 0.82 (M) (INPUT = 0.90)
JSI METAL= 0.32 (M) (INPUT = 1.00)

DWG NO. TAM 5222-16 N
STRUCTURAL
COMPONENT ONLY

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.96 (B) (INPUT = 1.00)

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
P	BMWWH	MT20	8.0	9.0		
Q	BMWWH	MT20	3.0	10.0		
R	BMWWH	MT20	6.0	9.0	4.50	1.75
S	BMV1+	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1578.2 lbs FACTORED DOWN AT 2-0-12, 1578.2 lbs FACTORED DOWN AT 4-0-12, 1578.2 lbs FACTORED DOWN AT 4-9-4, 1578.2 lbs FACTORED DOWN AT 6-9-4, 1578.2 lbs FACTORED DOWN AT 8-9-4, 1578.2 lbs FACTORED DOWN AT 10-9-4, 1578.2 lbs FACTORED DOWN AT 12-9-4, 1578.2 lbs FACTORED DOWN AT 14-9-4, 1578.2 lbs FACTORED DOWN AT 16-9-4, AND 1578.2 lbs FACTORED DOWN AT 18-9-4, AND 1578.2 lbs FACTORED DOWN AT 20-9-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	18-9-4	-1578	-1578	—	FRONT	VERT	TOTAL
R	4-0-12	-1578	-1578	—	FRONT	VERT	TOTAL
T	2-0-12	-1578	-1578	—	FRONT	VERT	TOTAL
U	4-9-4	-1578	-1578	—	FRONT	VERT	TOTAL
V	8-9-4	-1578	-1578	—	FRONT	VERT	TOTAL
W	8-9-4	-1578	-1578	—	FRONT	VERT	TOTAL
X	10-9-4	-1578	-1578	—	FRONT	VERT	TOTAL
Y	12-9-4	-1578	-1578	—	FRONT	VERT	TOTAL
Z	14-9-4	-1578	-1578	—	FRONT	VERT	TOTAL
AA	16-9-4	-1578	-1578	—	FRONT	VERT	TOTAL
AB	20-9-4	-1578	-1578	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER

2016

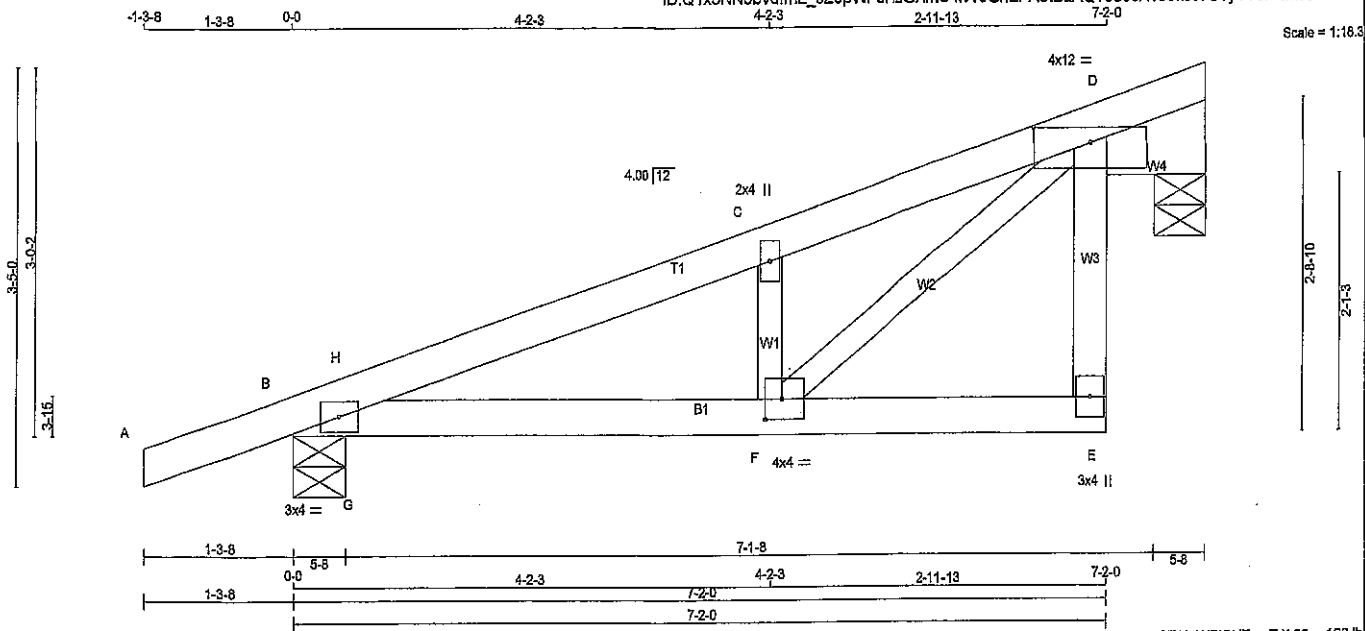
S. KATSOUKAKOS

PROVINCE OF ONTARIO

DWG NO. TAM 5223-16

STRUCTURAL

COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-H	MT20	3.0	4.0			
C	TMW+w	MT20	2.0	4.0			
D	TMVWW1-p	MT20	4.0	12.0	1.50	8.00	
E	BMV+p	MT20	3.0	4.0			
F	BMVWW-t	MT20	4.0	4.0	2.00	1.75	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
D	545	0	545	0
B	712	0	712	0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE
D	432	249 / 0	75 / 0
B	547	342 / 0	75 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)	MAX (LC)
A-B	0 / 25	-124.4	-124.4	0.16 (1)	10.00	F-C	-497 / 0	0.07 (1)
B-H	-874 / 0	-124.4	-124.4	0.05 (3)	6.25	F-D	0 / 1020	0.23 (1)
H-C	-838 / 0	-124.4	-124.4	0.18 (1)	6.25	G-H	-150 / 57	0.00 (1)
C-D	-851 / 0	-124.4	-124.4	0.16 (1)	6.25			
E-D	0 / 53	0.0	0.0	0.01 (3)	10.00			
B-G	0 / 809	-28.0	-28.0	0.27 (1)	10.00			
G-F	0 / 808	-28.0	-28.0	0.27 (1)	10.00			
F-E	0 / 0	-28.0	-28.0	0.09 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.24")
CALCULATED VERT. DEFL.(LL) = L/699 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.24")
CALCULATED VERT. DEFL.(TL) = L/699 (0.05")

CSI: TC=0.18 (C-H:1), BC=0.27 (F-G:1), WB=0.23 (D-F:1), SSI=0.20 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

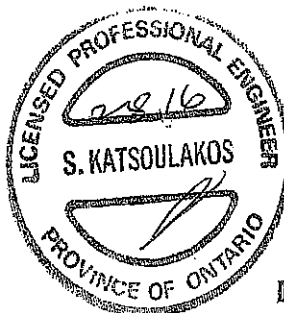
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

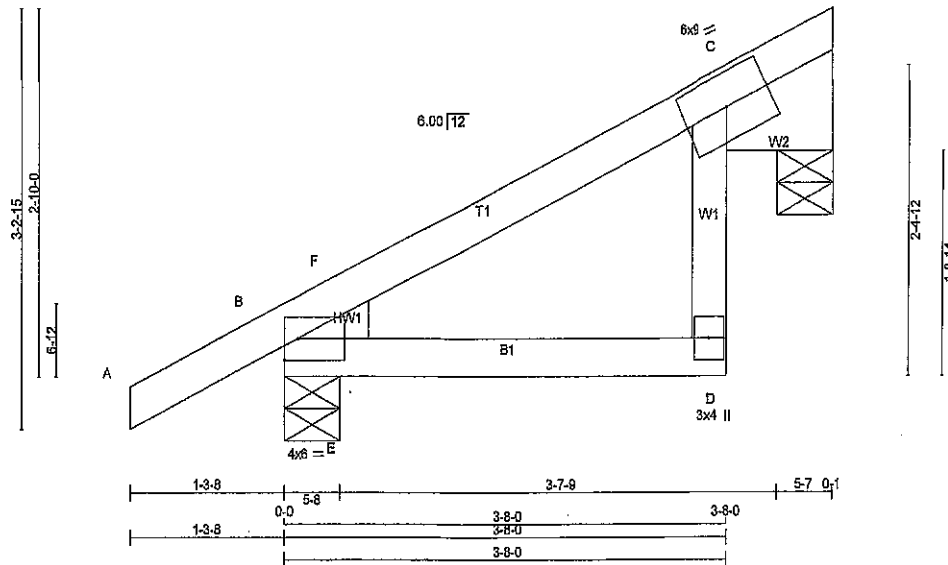
JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.31 (B) (INPUT = 1.00)



DRWG NO. TAW 5224-16 R-1
STRUCTURAL
COMPONENT ONLY

-1-3-8 1-3-8 0-0 3-8-0 3-8-0

Scale = 1:17.3



TOTAL WEIGHT = 6 X 17 = 103 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-t	MT20	4.0	6.0		Edge
C	TMBW1-t	MT20	6.0	9.0		
D	BMBVtp	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	REQD	HEEL
	GROSS REACTION	HORZ	GROSS REACTION	DOWN			
C	279	0	279	0	5-8 (5-7)	1-8	WEDGE
B	448	0	448	0	5-8	1-8	2x4 L

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
C	221	128 / 0	39 / 0	0 / 0	0 / 0	55 / 0	0 / 0
B	337	222 / 0	39 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	FACTORED		MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED	
			LC1	MAX			CS1 (LC)	MAX
FR-TO			FROM	TO		FR-TO		
A-B	0 / 25	-124.4	-124.4	0.16 (1)	10.00	E-F	-221 / 51	0.00 (1)
B-F	-53 / 0	-124.4	-124.4	0.06 (3)	6.25			
F-C	0 / 6	-124.4	-124.4	0.21 (1)	10.00			
D-C	0 / 93	0.0	0.0	0.02 (3)	10.00			
B-E	0 / 0	-28.0	-28.0	0.16 (1)	10.00			
E-D	0 / 0	-28.0	-28.0	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.21 (C-F:1), BC=0.17 (D-E:1), WB=0.00 (E-F:1), SSI=0.19 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

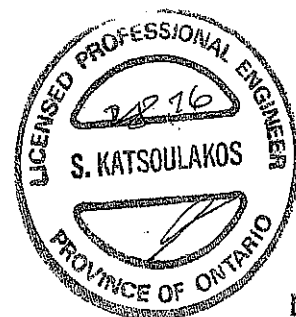
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

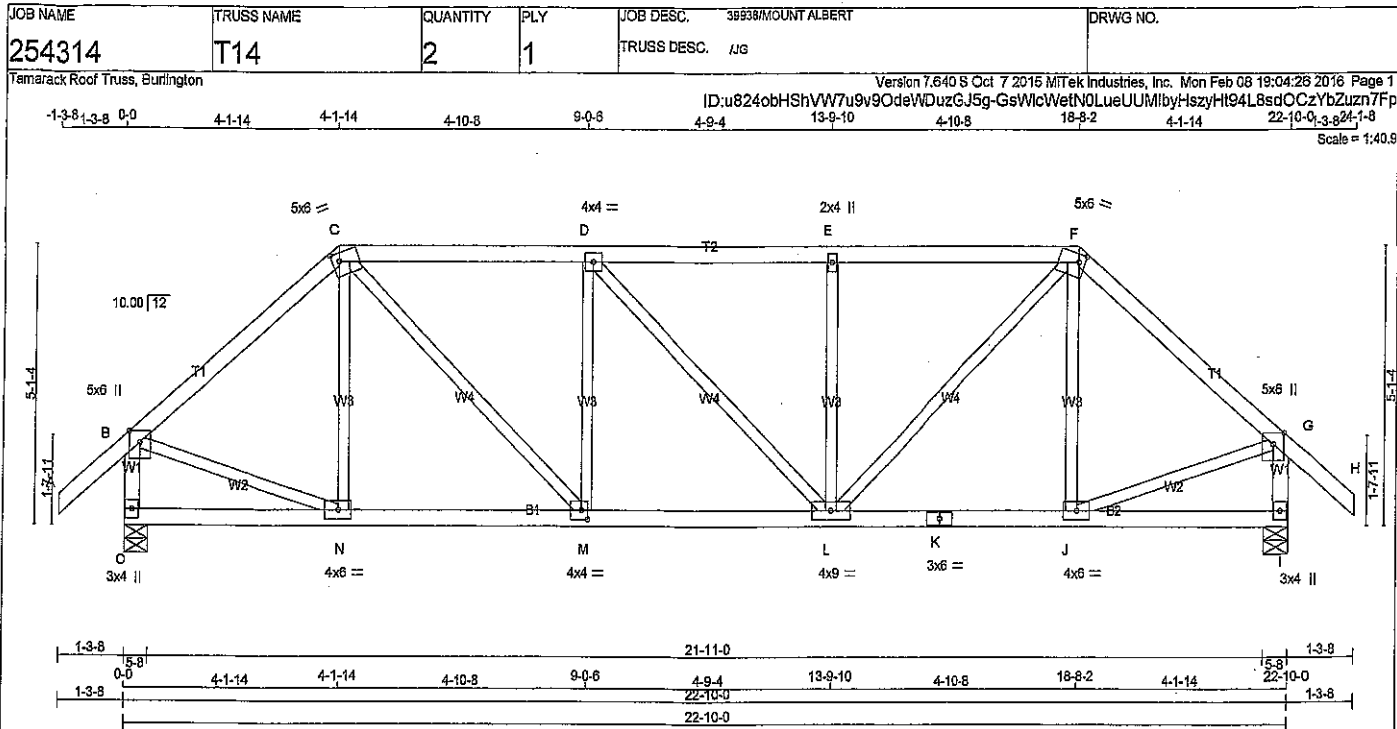
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.60)
JSI METAL= 0.05 (B) (INPUT = 1.00)



DWG NO. **TAM 522516 R1**
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMW-w	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	1.75	1.50
G	TMW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMW-w	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMW-w	MT20	4.0	9.0		
M	BMW-w	MT20	4.0	4.0	2.00	1.50
N	BMW-w	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
O	1912	0	1912	0
I	1912	0	1912	0

UNFACTORED REACTIONS

	1ST CASE	SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
O	1495	891/0	240/0	0/0	0/0	365/0	0/0
I	1495	891/0	240/0	0/0	0/0	365/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 4.21 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/55	-124.4 -124.4	0.17 (1)	10.00	N-C	-241/69	0.09 (1)
B-C	-1635/0	-124.4 -124.4	0.44 (1)	4.66	C-M	0/1110	0.25 (1)
C-D	-2040/0	-124.4 -124.4	0.47 (1)	4.21	M-D	-546/0	0.25 (1)
D-E	-2037/0	-124.4 -124.4	0.47 (1)	4.21	D-L	-3/0	0.00 (1)
E-F	-2037/0	-124.4 -124.4	0.47 (1)	4.22	L-E	-546/0	0.25 (1)
F-G	-1638/0	-124.4 -124.4	0.44 (1)	4.66	L-F	0/1107	0.25 (1)
G-H	0/55	-124.4 -124.4	0.17 (1)	10.00	J-F	-240/70	0.09 (1)
O-B	-1665/0	0.0	0.20 (1)	6.14	B-N	0/1310	0.29 (1)
I-G	-1666/0	0.0	0.20 (1)	6.14	J-G	0/1311	0.29 (1)
O-N	0/0	-28.0 -28.0	0.15 (3)	10.00			
N-M	0/1248	-28.0 -28.0	0.28 (1)	10.00			
M-L	0/2040	-28.0 -28.0	0.38 (1)	10.00			
L-K	0/1248	-28.0 -28.0	0.28 (1)	10.00			
K-J	0/1248	-28.0 -28.0	0.28 (1)	10.00			
J-I	0/0	-28.0 -28.0	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 068-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")

CALCULATED VERT. DEFL.(LL) = L/999 (0.06")

ALLOWABLE DEFL.(TL) = L/360 (0.76")

CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.47 (C-D:1), BC=0.38 (L-M:1), WB=0.29 (G-J:1), SI=0.28 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

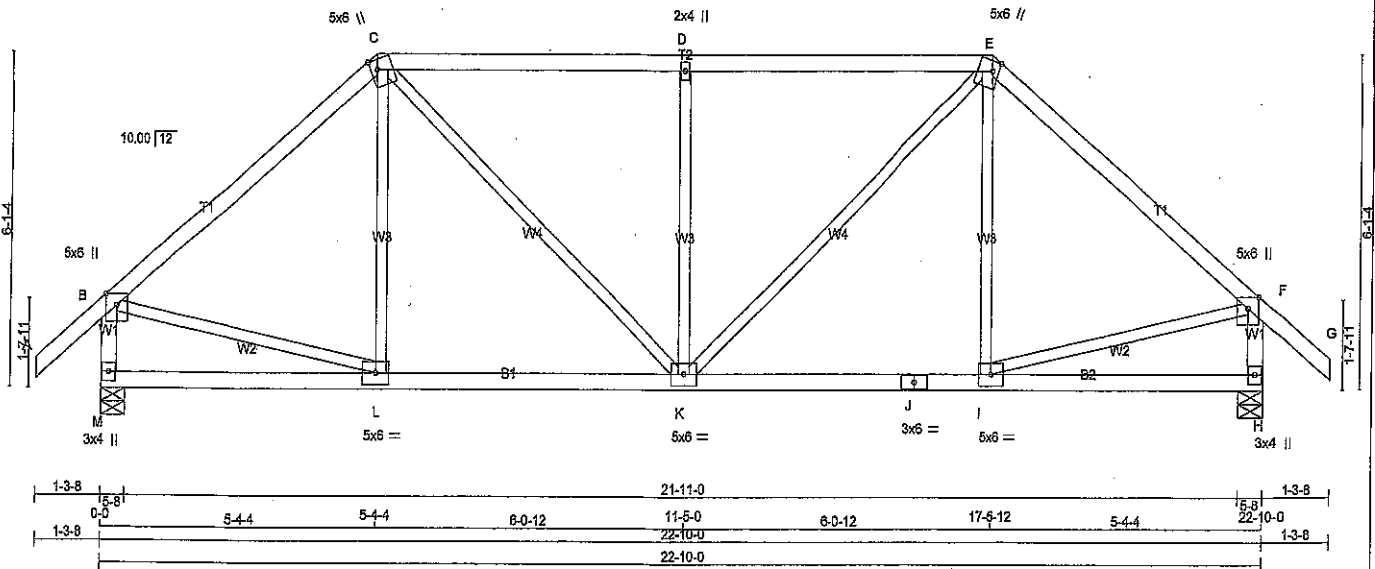
JSI GRIP= 0.87 (M) (INPUT = 0.90)

JSI METAL= 0.36 (K) (INPUT = 1.00)



DWG NO. TAW5229 - 1674

STRUCTURAL COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W.	LEN Y X
B	TMVW+p	MT20	5.0	6.0 Edge
C	TTVW+m	MT20	5.0	6.0 2.25 1.50
D	TMVW+w	MT20	2.0	4.0
E	TTVW+m	MT20	5.0	6.0 2.25 1.50
F	TMVW+p	MT20	5.0	6.0 Edge
H	BMV1+p	MT20	3.0	4.0
J	BMVW-t	MT20	5.0	6.0
K	BS-t	MT20	3.0	6.0
L	BMVW-t	MT20	5.0	6.0
M	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
M	1912	0	1912	0	0	5-8	2-11	5-8	2-11
H	1912	0	1912	0	0	5-8	2-11	5-8	2-11
UNFACTORED REACTIONS									
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS							
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
M	1495	891/0	240/0	0/0	0/0	395/0	0/0		
H	1495	891/0	240/0	0/0	0/0	395/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.44 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO			FR-TO				
A-B	0/55	-124.4 -124.4	0.17 (1)	10.00	L-C	-118/175	0.07 (1)	10.00	
B-C	-1632/0	-124.4 -124.4	0.77 (1)	4.14	C-K	0/782	0.18 (1)	10.00	
C-D	-1814/0	-124.4 -124.4	0.92 (1)	3.44	K-D	-923/0	0.54 (1)	10.00	
D-E	-1814/0	-124.4 -124.4	0.92 (1)	3.44	K-E	0/782	0.18 (1)	10.00	
E-F	-1632/0	-124.4 -124.4	0.77 (1)	4.14	I-E	-118/175	0.07 (1)	10.00	
F-G	0/55	-124.4 -124.4	0.17 (1)	10.00	B-L	0/1286	0.29 (1)	10.00	
M-B	-1852/0	0.0 0.0	0.20 (1)	6.16	I-F	0/1286	0.29 (1)	10.00	
H-F	-1852/0	0.0 0.0	0.20 (1)	6.16					
M-L	0/0	-28.0 -28.0	0.24 (3)	10.00					
L-K	0/1249	-28.0 -28.0	0.35 (2)	10.00					
K-J	0/1249	-28.0 -28.0	0.35 (2)	10.00					
J-I	0/1249	-28.0 -28.0	0.36 (2)	10.00					
I-H	0/0	-28.0 -28.0	0.24 (3)	10.00					

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 34.8 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 60.3 PSF
SPACING = 24.0 IN. C/C	

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TFC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.76")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.11")
 CSI: TC=0.92 (C-D:1), BC=0.36 (K-L:2), WB=0.54 (D-K:1), SS=0.37 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
MAX MIN	MAX MIN	MAX MIN	MAX MIN		
MT20	618 354	1687 822	2264 1656		

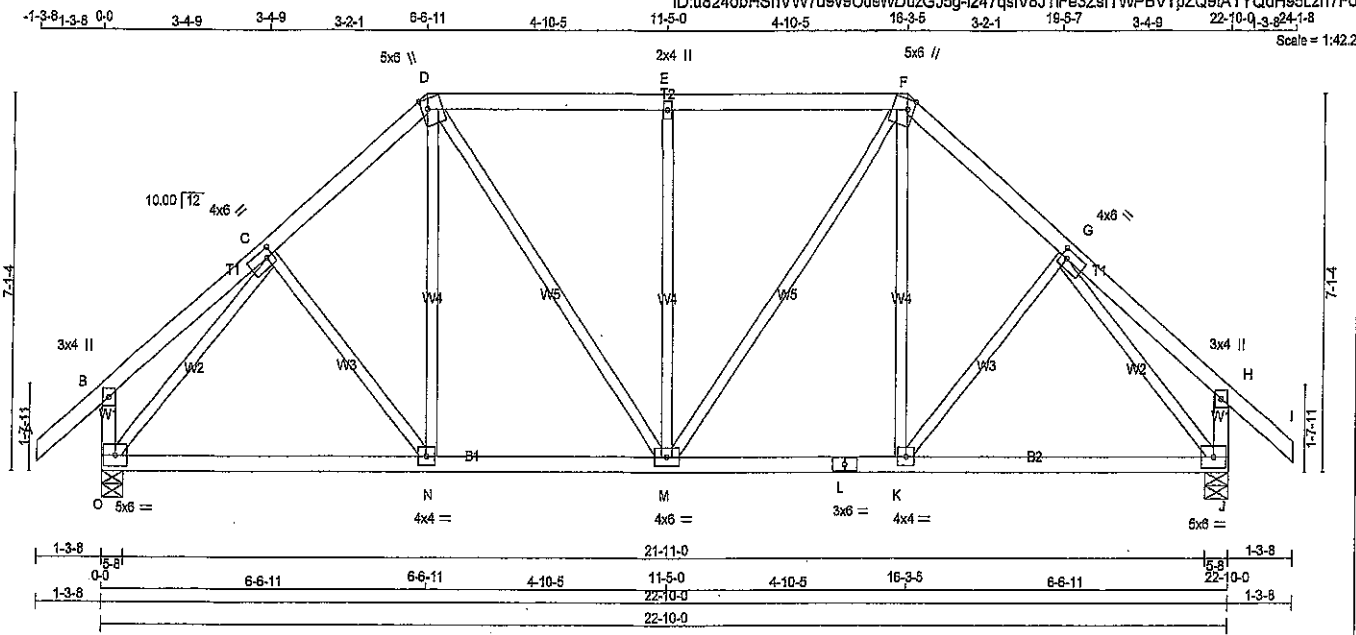
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
 JSI METAL= 0.33 (B) (INPUT = 1.00)



DRWG NO. TAM 5230-18 R-1
 STRUCTURAL
 COMPONENT ONLY



TOTAL WEIGHT = 2 X 108 = 217 lb (M/F)

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2 SPF
D - F	2x4	DRY	No.2 SPF
F - I	2x4	DRY	No.2 SPF
O - B	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
O - L	2x4	DRY	No.2 SPF
L - J	2x4	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
O	1912	0	1912	0	0	5-8
J	1912	0	1912	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
O	1495	891/0	240/0	0/0	0/0	365/0	0/0
J	1495	891/0	240/0	0/0	0/0	365/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)
FR-TO		FROM TO			FR-TO		
A-B	0/55	-124.4 -124.4	0.17 (1)	10.00	C-N	-11/86	0.02 (3)
B-C	0/28	-124.4 -124.4	0.20 (1)	10.00	N-D	0/288	0.07 (3)
C-D	-1611/0	-124.4 -124.4	0.18 (1)	5.05	D-M	0/508	0.11 (1)
D-E	-1511/0	-124.4 -124.4	0.40 (1)	4.88	M-E	-735/0	0.64 (1)
E-F	-1511/0	-124.4 -124.4	0.40 (1)	4.88	F-F	0/508	0.11 (1)
F-G	-1611/0	-124.4 -124.4	0.18 (1)	5.05	K-F	0/288	0.07 (3)
G-H	0/28	-124.4 -124.4	0.20 (1)	10.00	K-G	-11/86	0.02 (3)
H-I	0/55	-124.4 -124.4	0.17 (1)	10.00	C-C	-1937/0	0.83 (1)
O-B	-330/0	0.0 0.0	0.04 (1)	7.81	G-J	-1937/0	0.83 (1)
J-H	-330/0	0.0 0.0	0.04 (1)	7.81			
O-N	0/1219	-28.0 -28.0	0.40 (2)	10.00			
N-M	0/1215	-28.0 -28.0	0.41 (2)	10.00			
M-L	0/1215	-28.0 -28.0	0.41 (2)	10.00			
L-K	0/1215	-28.0 -28.0	0.41 (2)	10.00			
K-J	0/1219	-28.0 -28.0	0.40 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.76")
 CALCULATED VERT. DEFL.(LL) = $L/899$ (0.08")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.76")
 CALCULATED VERT. DEFL.(TL) = $L/899$ (0.14")

CSI: TC=0.40 (D-E:1), BC=0.41 (M-N:2), WB=0.83 (C-O:1), SS=0.29 (E-F:1)

DOE LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	822	2284	1656

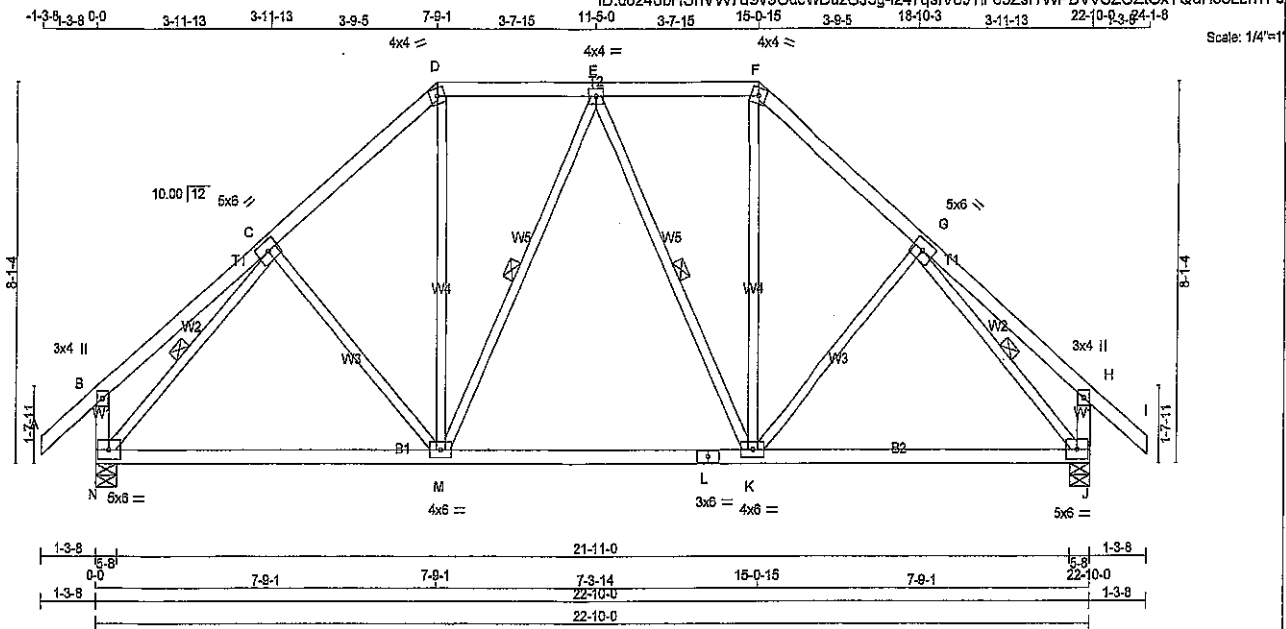
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.88 (G) (INPUT = 0.90)
 JSI METAL = 0.47 (G) (INPUT = 1.00)



DRG NO. TAW 5231-16 R
STRUCTURAL COMPONENT ONLY



TOTAL WEIGHT = 2 X 109 = 217 lb

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR. SPF

A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	8.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW-1	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-1	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW-1	MT20	5.0	6.0		
K	BMVW-1	MT20	4.0	6.0		
L	BS-1	MT20	3.0	6.0		
M	BMVW-1	MT20	4.0	6.0		
N	BMVW-1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	REQD BRG IN-SX
N	1912	0	5-8	2-1
J	1912	0	5-8	2-1

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	SNOW	MAX. MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1485	891 / 0	240 / 0	0 / 0	0 / 0	365 / 0	0 / 0
J	1485	891 / 0	240 / 0	0 / 0	0 / 0	365 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1- 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K. DBS = 20-0-0. CBF = 37 LBS.
 1- 2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J. DBS = 8-0-0. CBF = 96 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 55	-124.4 -124.4	0.17 (1)	C-M	-134 / 60	0.06 (1)	
B-C	0 / 35	-124.4 -124.4	0.29 (1)	M-D	0 / 601	0.14 (1)	
C-D	-1549 / 0	-124.4 -124.4	0.28 (1)	D-E	-289 / 0	0.15 (1)	
D-E	-1168 / 0	-124.4 -124.4	0.21 (1)	E-K	-289 / 0	0.15 (1)	
E-F	-1168 / 0	-124.4 -124.4	0.21 (1)	K-F	0 / 601	0.14 (1)	
F-G	-1549 / 0	-124.4 -124.4	0.26 (1)	G-K	-134 / 60	0.08 (1)	
G-H	0 / 35	-124.4 -124.4	0.29 (1)	N-C	-1925 / 0	0.49 (1)	
H-I	0 / 55	-124.4 -124.4	0.17 (1)	G-J	-1925 / 0	0.48 (1)	
I-B	-357 / 0	0.0 0.0	0.04 (1)				
J-H	-357 / 0	0.0 0.0	0.04 (1)				
N-M	0 / 1249	-28.0 -28.0	0.50 (3)				
M-L	0 / 1292	-28.0 -28.0	0.51 (3)				
L-K	0 / 1292	-28.0 -28.0	0.51 (3)				
K-J	0 / 1249	-28.0 -28.0	0.50 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.76")
 CALCULATED VERT. DEFL. (LL) = 1/999 (0.16")
 ALLOWABLE DEFL. (TL) = 1/360 (0.76")
 CALCULATED VERT. DEFL. (TL) = 1/999 (0.26")

CSI: TC=0.29 (B-C:1), BC=0.51 (K-M:3), WB=0.48 (C-N:1), 6SI=0.22 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

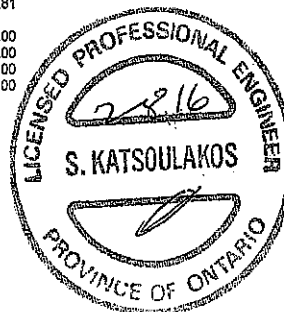
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (3) (INPUT = 0.90)
 JSI METAL= 0.51 (1) (INPUT = 1.00)

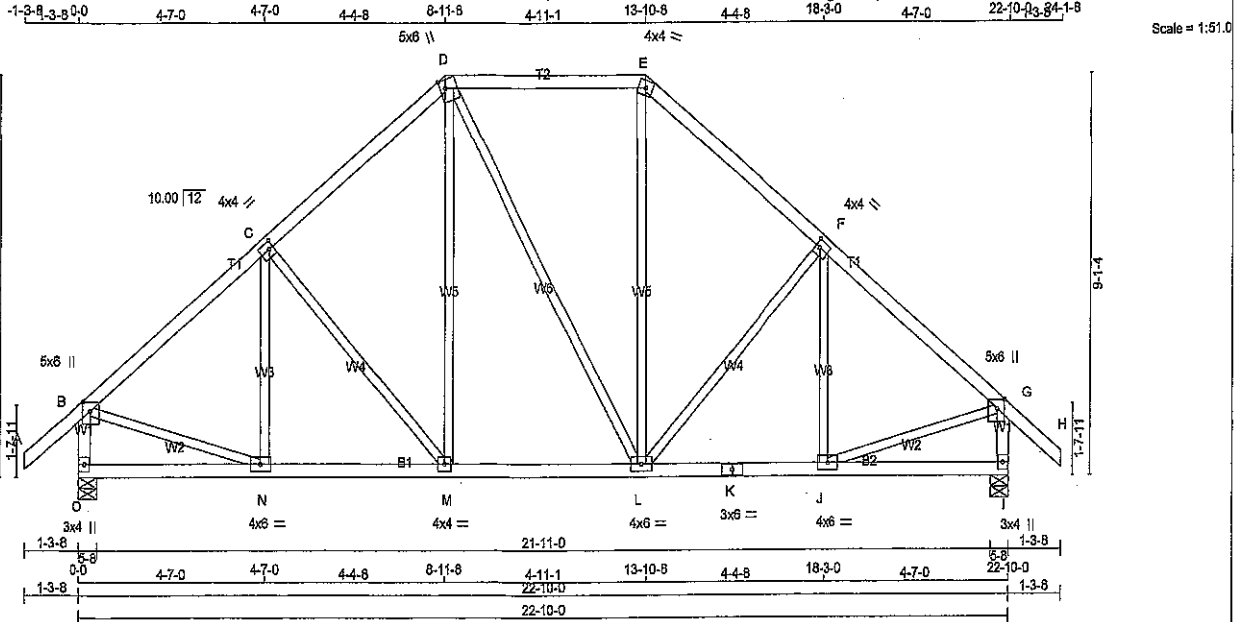


DWG NO. TAW 5232 1612-1
STRUCTURAL COMPONENT ONLY

Tamareck Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 113 = 226 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	UP	BRG	IN-SX	BRG
O	1912	0	0	1912	0	0	5-8	2-11	2-11
I	1912	0	0	1912	0	0	5-8	2-11	2-11

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
O	1495	891/0	240/0
I	1495	891/0	240/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
(LBS)	(LBS)	(LBS)	(LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0/55	N-C	-238/79
B-C	-1665/0	C-M	-354/0
C-D	-1451/0	M-D	0/393
D-E	-1084/0	D-L	0/2
E-F	-1452/0	L-E	0/395
F-G	-1664/0	L-F	-352/0
G-H	0/55	J-F	-238/77
O-B	-1658/0	B-N	0/1363
I-G	-1857/0	J-G	0/1363
O-N	0/0		
N-M	0/1311		
M-L	0/1083		
L-K	0/1311		
K-J	0/1311		
J-I	0/0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.08")

CSI: TC=0.40 (D-E:1), BC=0.28 (J-L:1), WB=0.31 (B-N:1), SSI=0.24 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

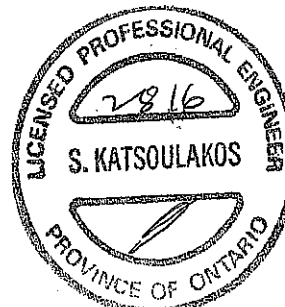
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

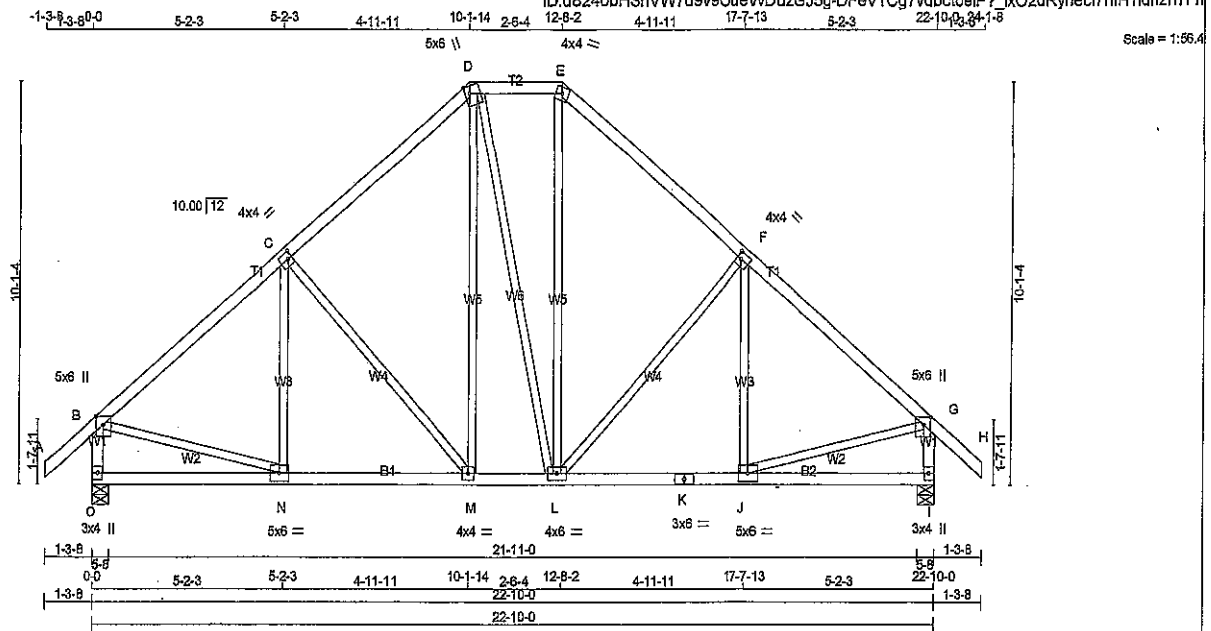
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.38 (K) (INPUT = 1.00)



DWG NO. TAW 5233-16P-1
STRUCTURAL
COMPONENT ONLY

JOB NAME 254314	TRUSS NAME T19	QUANTITY 2	PLY 1	JOB DESC. 39938/MOUNT ALBERT	DRWG NO.
TARNACK ROOF TRUSS, BURLINGTON		Version 7.640 S Oct 7 2015 Mitek Industries, Inc. Mon Feb 08 19:04:26 2016 Page 1		ID:u824obHShVW7u9v9OdeWVDuzGJ5g-DFeV1Cg7vdbctosIP?_lx02dRyneci7hfHildnzn7Fn	



TOTAL WEIGHT = 2 X 116 = 237 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORZ	JT	DOWN	GROSS REACTION	UP	BRG	IN-SX
O	1912	0	0	O	1912	0	0	5-8	2-11
I	1912	0	0	I	1912	0	0	5-8	2-11

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
O	1495	891 / 0	240 / 0	0 / 0	0 / 0
I	1495	891 / 0	240 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 55	N-C	-166 / 145
B-C	-1872 / 0	C-M	-479 / 0
C-D	-1355 / 0	M-D	0 / 441
D-E	-1007 / 0	D-L	0 / 9
E-F	-1357 / 0	L-E	0 / 451
F-G	-1670 / 0	L-F	-475 / 0
G-H	0 / 55	J-F	-172 / 141
O-B	-1853 / 0	B-N	0 / 1362
I-G	-1852 / 0	J-G	0 / 1361
O-N	0 / 0		
N-M	0 / 1321		
M-L	0 / 1005		
L-K	0 / 1320		
K-J	0 / 1320		
J-I	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BC9C 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.47 (B-C:1), BC=0.33 (M-N:2), WB=0.55 (C-M:1), SSI=0.24 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

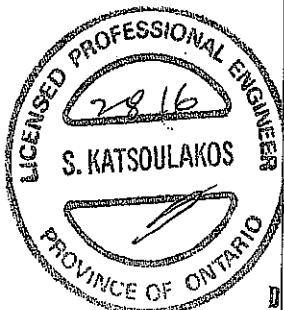
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1687	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.80)
JSI METAL= 0.87 (C) (INPUT = 1.00)

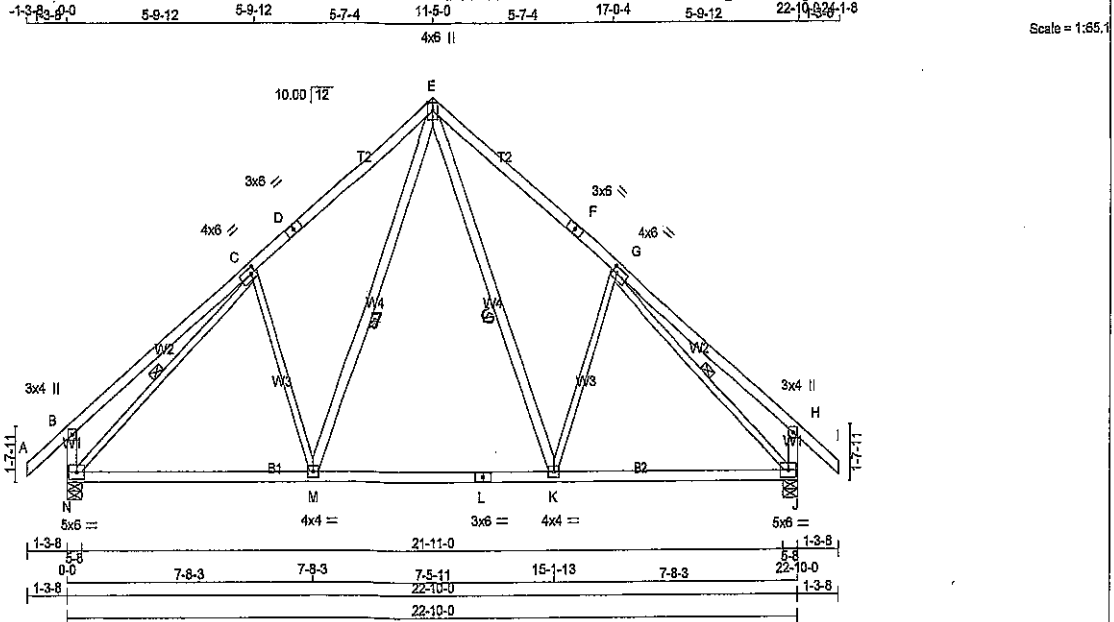


DRWG NO. YAM 5234-16 R1
STRUCTURAL COMPONENT ONLY

Tamarack Roof Truss, Burlington

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Scale = 1:85.1

TOTAL WEIGHT = 4 X 115 = 461 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
K - G	2x3	DRY	No.2
C - M	2x3	DRY	No.2
N - C	2x3	DRY	No.2
G - J	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	4.0	6.0	2.00	1.75
D TS-t	MT20	3.0	6.0		
E TTWW+p	MT20	4.0	6.0	Edge	
F TS-t	MT20	3.0	6.0		
G TMWW-t	MT20	4.0	6.0	2.00	1.75
H TMV+p	MT20	3.0	4.0		
J BMVW-t	MT20	5.0	6.0		
K BMWW-t	MT20	4.0	4.0		
L BS-t	MT20	3.0	6.0		
M BMWW-t	MT20	4.0	4.0		
N BMVW-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	1912	0	1912	0
J	1912	0	1912	0

UNFACTORED REACTIONS

	1ST CASE	MAX MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1465	891 / 0	240 / 0	0 / 0	0 / 0	365 / 0	0 / 0
J	1465	891 / 0	240 / 0	0 / 0	0 / 0	365 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1-2x3 DRY SPF No.2 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J. DBS = 8'-0". CBF = 85 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE. FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x6, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 55	-124.4 -124.4	0.17 (1)	10.00	E-K	0 / 726	0.12 (1)
B-C	0 / 55	-124.4 -124.4	0.67 (1)	10.00	K-G	-479 / 0	0.36 (1)
C-D	-1552 / 0	-124.4 -124.4	0.59 (1)	4.55	M-E	0 / 726	0.12 (1)
D-E	-1552 / 0	-124.4 -124.4	0.59 (1)	4.55	C-M	-479 / 0	0.36 (1)
E-F	-1552 / 0	-124.4 -124.4	0.59 (1)	4.55	N-C	-1902 / 0	0.87 (1)
F-G	-1552 / 0	-124.4 -124.4	0.59 (1)	4.55	G-J	-1902 / 0	0.87 (1)
G-H	0 / 55	-124.4 -124.4	0.67 (1)	10.00			
H-I	0 / 55	-124.4 -124.4	0.67 (1)	10.00			
I-J	-438 / 0	0.0	0.05 (1)	7.81			
J-H	-438 / 0	0.0	0.05 (1)	7.81			
N-M	0 / 1303	-28.0 -28.0	0.51 (3)	10.00			
M-L	0 / 914	-28.0 -28.0	0.49 (3)	10.00			
L-K	0 / 914	-28.0 -28.0	0.49 (3)	10.00			
K-J	0 / 1303	-28.0 -28.0	0.51 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 34.8	PSF
DL = 8.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 60.3	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-08
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.67 (G-H:1), BC=0.51 (J-K:3), WB=0.87 (C-N:1), SS=0.27 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

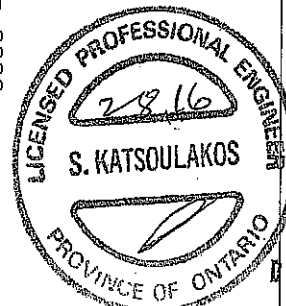
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

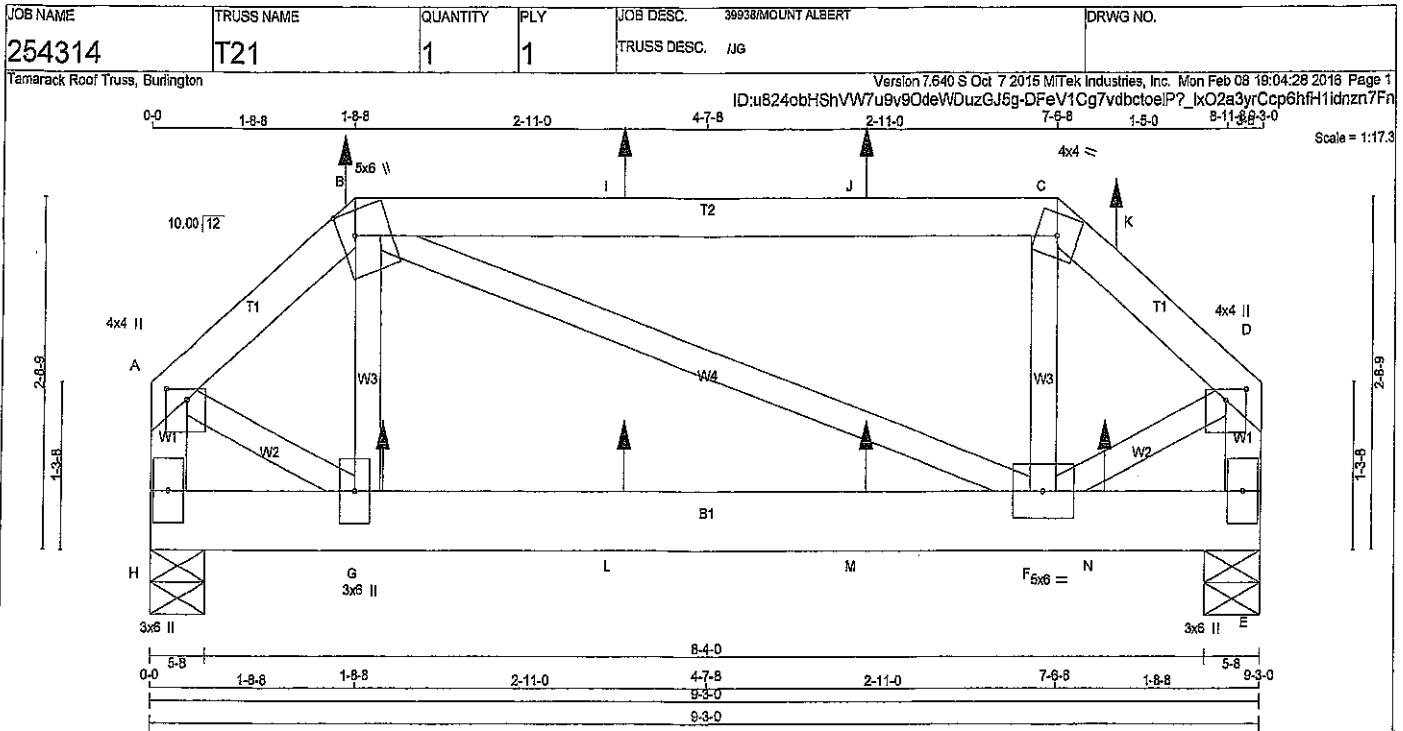
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.52 (C) (INPUT = 1.00)



DRWG NO. TAM 5235-16 R-1
STRUCTURAL COMPONENT ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - E	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW+p	MT20	4.0	4.0 1.00 2.00
B	TTWW+m	MT20	5.0	6.0 2.25 1.50
C	TTW-m	MT20	4.0	4.0
D	TMVW+p	MT20	4.0	4.0 1.00 2.00
E	BMV1+p	MT20	3.0	6.0
F	BMVWW-l	MT20	5.0	6.0
G	BMVWW-l	MT20	3.0	6.0
H	BMV1+p	MT20	3.0	6.0

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 43.8 lbs FACTORED UP AT 1-8-8, 43.8 lbs FACTORED UP AT 3-11-4, AND 43.8 lbs FACTORED UP AT 5-11-4, AND 43.8 lbs FACTORED UP AT 7-11-4 ON TOP CHORD, AND 11.1 lbs FACTORED UP AT 1-11-4, 11.1 lbs FACTORED UP AT 3-11-4, AND 11.1 lbs FACTORED UP AT 5-11-4, AND 11.1 lbs FACTORED UP AT 7-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
H	610	0	610	0	5-8
E	599	0	599	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERM. LIVE WIND DEAD SOIL
H	488	275/0	88/0 0/0 0/0 122/0 0/0
E	477	270/0	87/0 0/0 0/0 120/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1 MAX	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1 MAX
FR-TO	FORCE (LBS)	FROM TO	PLF (LC) CSI (LC)	FR-TO	FORCE (LBS)	FROM TO	PLF (LC) CSI (LC)
A-B	-495/0	-124.4 -124.4	0.07 (1)	G-B	-65/112	0.03 (3)	
B-I	-372/0	-124.4 -124.4	0.69 (1)	B-F	-3/0	0.00 (1)	
I-J	-372/0	-124.4 -124.4	0.69 (1)	F-C	-69/111	0.03 (3)	
J-C	-372/0	-124.4 -124.4	0.69 (1)	A-G	0/420	0.10 (1)	
C-K	-499/0	-124.4 -124.4	0.08 (1)	F-D	0/417	0.10 (1)	
K-D	-499/0	-124.4 -124.4	0.08 (1)				
H-A	-609/0	0.0 0.0	0.07 (1)				
E-D	-595/0	0.0 0.0	0.07 (1)				
H-G	0/0	-28.0 -28.0	0.07 (3)				
G-L	0/375	-28.0 -28.0	0.10 (2)				
L-M	0/375	-28.0 -28.0	0.10 (2)				
M-F	0/375	-28.0 -28.0	0.10 (2)				
F-N	0/0	-28.0 -28.0	0.07 (3)				
N-E	0/0	-28.0 -28.0	0.07 (3)				

FACTORED CONCENTRATED LOADS (LBS)				FACE	DIR.	TYPE
JT	LOC.	LC1	MAX. MAX.			
B	1-8-8	44	---	44	BACK	VERT
G	1-11-4	6	---	11	BACK	VERT
I	3-11-4	44	---	44	BACK	VERT
J	5-11-4	44	---	44	BACK	VERT
K	7-11-4	44	---	44	BACK	VERT
L	3-11-4	6	---	11	BACK	VERT
M	5-11-4	6	---	11	BACK	VERT
N	7-11-4	6	---	11	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA C88-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/899 (0.02")

CSI: TC=0.69 (B-C:1), BC=0.10 (F-G:2), WB=0.10 (A-G:1), SSI=0.28 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

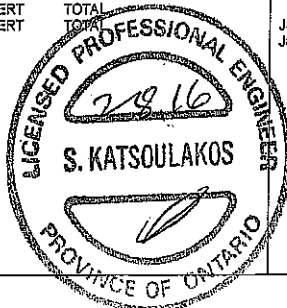
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MT20	618	354	1697
	622	2264	1656

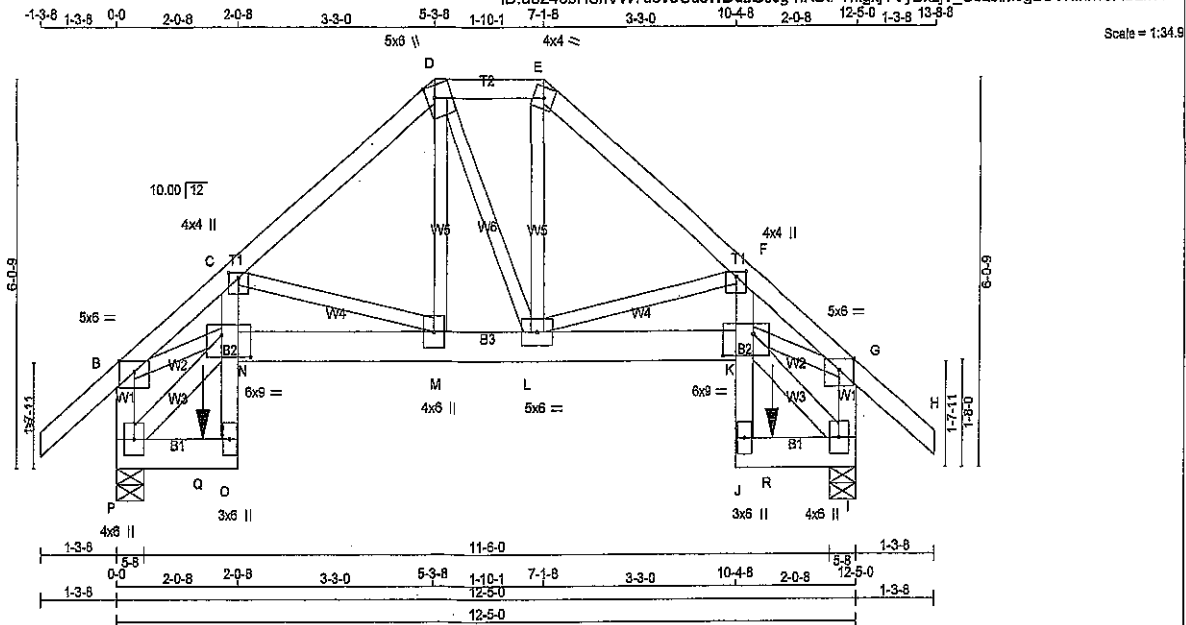
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.16 (G) (INPUT = 1.00)



DRWG NO. TAW 523 616 R4
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 75 lb

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
P - O	2x6	DRY	No.2
O - C	2x4	DRY	No.2
N - K	2x6	DRY	No.2
J - F	2x4	DRY	No.2
J - I	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
P - N	2x4	DRY	No.2
K - I	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW-p	MT20	4.0	4.0	1.00	2.00
D	TTVW-m	MT20	5.0	6.0	2.25	1.50
E	TTVW-m	MT20	4.0	4.0		
F	TMVW-p	MT20	4.0	4.0	1.00	2.00
G	TMVW-p	MT20	5.0	6.0	Edge	
I	BMVW1-p	MT20	4.0	6.0		
J	BMV-p	MT20	3.0	6.0		
K	BMVWW-I	MT20	6.0	9.0	4.00	6.00
L	BMVWW-I	MT20	5.0	6.0		
M	BMVWW-I	MT20	4.0	6.0		
N	BMVWW-I	MT20	6.0	9.0	4.00	6.00
O	BMV-p	MT20	3.0	6.0		
P	BMVW1-p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2.0 lbs FACTORED DOWN AT 1-5-4 AND 2.0 lbs FACTORED DOWN AT 10-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
P	1119	0	1119	0
I	1119	0	1119	0

UNFACTORED REACTIONS

1ST LCASE MAX/MIN COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	669	528 / 0	131 / 0	0 / 0	0 / 0	209 / 0	0 / 0
I	669	528 / 0	131 / 0	0 / 0	0 / 0	209 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 55	-124.4 -124.4	0.19 (1)	10.00	C-M	-788 / 0	0.20 (1)
B-C	-1785 / 0	-124.4 -124.4	0.22 (1)	4.74	M-D	0 / 250	0.06 (1)
C-D	-908 / 0	-124.4 -124.4	0.24 (1)	6.13	D-L	0 / 22	0.01 (2)
D-E	-701 / 0	-124.4 -124.4	0.08 (1)	6.25	L-E	0 / 272	0.07 (1)
E-F	-912 / 0	-124.4 -124.4	0.24 (1)	6.12	L-F	-782 / 0	0.20 (1)
F-G	-1785 / 0	-124.4 -124.4	0.22 (1)	4.74	P-N	-62 / 0	0.01 (1)
G-H	0 / 55	-124.4 -124.4	0.19 (1)	10.00	K-I	-62 / 0	0.01 (1)
P-B	-1050 / 0	0.0 0.0	0.12 (1)	7.64	B-N	0 / 1415	0.35 (1)
I-G	-1050 / 0	0.0 0.0	0.12 (1)	7.64	K-G	0 / 1414	0.35 (1)

P-Q	0 / 45	-28.0	-28.0	0.02 (3)	10.00
Q-O	0 / 45	-28.0	-28.0	0.02 (3)	10.00
O-N	0 / 48	0.0	0.0	0.12 (1)	10.00
N-C	0 / 481	0.0	0.0	0.20 (1)	10.00
N-M	0 / 1436	-28.0	-28.0	0.22 (1)	10.00
M-L	0 / 693	-28.0	-28.0	0.12 (1)	10.00
L-K	0 / 1436	-28.0	-28.0	0.21 (1)	10.00
J-K	0 / 48	0.0	0.0	0.12 (1)	10.00
K-F	0 / 472	0.0	0.0	0.20 (1)	10.00
J-R	0 / 45	-28.0	-28.0	0.02 (3)	10.00
R-I	0 / 45	-28.0	-28.0	0.02 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
Q	1-5-4	-1	-2	-	BACK	VERT	TOTAL
R	10-11-12	-1	-2	-	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.41")
 CALCULATED VERT. DEFL.(LL) = 1/999 (0.03")
 ALLOWABLE DEFL.(TL) = 1/360 (0.41")
 CALCULATED VERT. DEFL.(TL) = 1/999 (0.06")

CSI: TC=0.24 (E-F:1), BC=0.22 (M-N:1), WB=0.35 (B-N:1), SSI=0.15 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

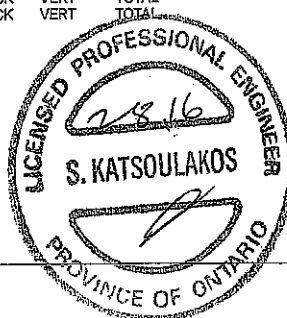
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (L) (INPUT = 0.50)
 JSI METAL= 0.31 (S) (INPUT = 1.00)

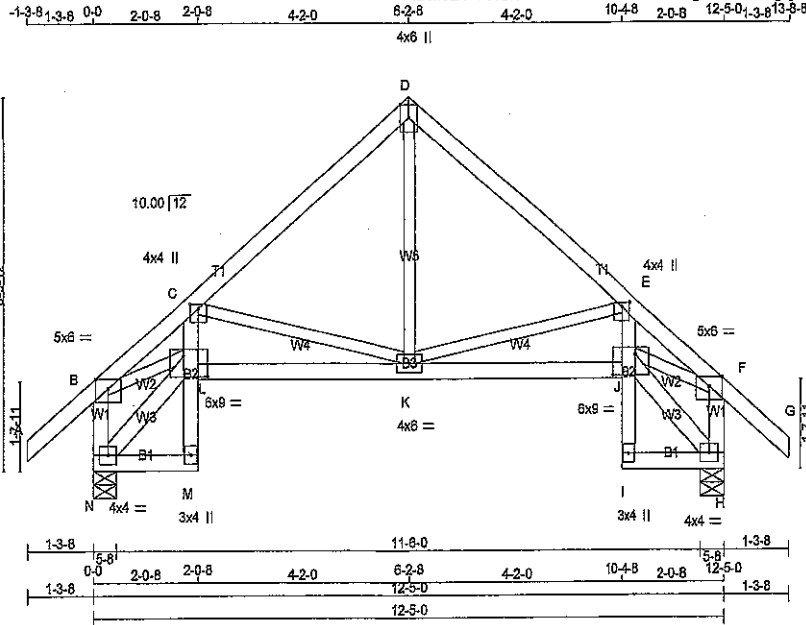


DRWG NO. TAN 5237-1612-1
STRUCTURAL COMPONENT ONLY

Tamarack Roof Truss, Burlington

Version 7.840 S Oct 7 2015 MITak Industries, Inc. Mon Feb 08 19:04:29 2016 Page 1

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TOTAL WEIGHT = 2 X 64 = 128 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
N - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - C	2x4	DRY	No.2
L - J	2x4	DRY	No.2
I - E	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - L	2x4	DRY	No.2
J - H	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B - TMVW+p	MT20	5.0	6.0	Edge	
C - TMVW+p	MT20	4.0	4.0	1.00	2.00
D - TTW+p	MT20	4.0	6.0	Edge	
E - TMVW+p	MT20	4.0	4.0	1.00	2.00
F - TMVW-p	MT20	5.0	6.0	Edge	
H - BMVW1-i	MT20	4.0	4.0		
I - BMV+p	MT20	3.0	4.0		
J - BMVWW-i	MT20	6.0	9.0	4.50	5.75
K - BMVWW-i	MT20	4.0	6.0		
L - BMVWW-i	MT20	6.0	9.0	4.50	5.75
M - BMV+p	MT20	3.0	4.0		
N - BMVW1-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
	FACTORED	MAXIMUM FACTORED	INPUT
	GROSS REACTION	GROSS REACTION	BRG
JT	VERT	DOWN	IN-SX
N	1118	0	5-8
H	1118	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
N	897	528 / 0	130 / 0
H	897	528 / 0	130 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO			
A-B	0 / 55	-124.4	-124.4
B-C	-1903 / 0	-124.4	-124.4
C-D	-800 / 0	-124.4	-124.4
D-E	-800 / 0	-124.4	-124.4
E-F	-1903 / 0	-124.4	-124.4
F-G	0 / 55	-124.4	-124.4
G-H	-1046 / 0	0.0	0.0
H-I	-1046 / 0	0.0	0.0
I-J	0 / 46	-28.0	-28.0
J-K	0 / 46	0.0	0.0
K-L	0 / 566	0.0	0.0
L-M	0 / 1555	-28.0	-28.0
M-N	0 / 1555	-28.0	-28.0
N-O	0 / 46	0.0	0.0
O-P	0 / 566	0.0	0.0
P-Q	0 / 46	-28.0	-28.0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.41")
CALCULATED VERT. DEFL. (TL) = L/999 (0.08")

CSI: TC=0.31 (D-E:1), BC=0.34 (J-K:1), WB=0.35 (F-J:1), SSI=0.18 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

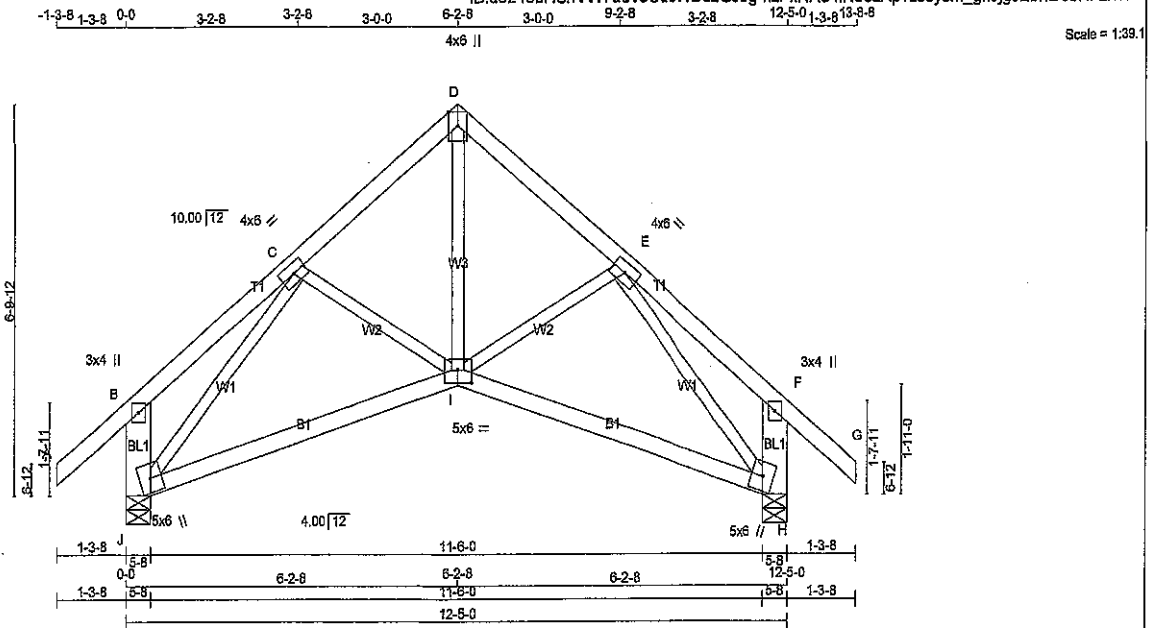
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)



DWG NO. TAM 5238-16 R-1
STRUCTURAL COMPONENT ONLY



TOTAL WEIGHT = 3 X 59 = 177 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x6	DRY	No.2	SPF	
H - F	2x6	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	

BEARING BLOCKS	DRY	No.2	SPF
BL1	2x6	DRY	No.2
BL1	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMK+p	MT20	3.0	4.0		
C	TMWV+I	MT20	4.0	6.0		
D	TTVW+p	MT20	4.0	6.0	Edge	
E	TMWV+I	MT20	4.0	6.0		
F	TMK+p	MT20	3.0	4.0		
H	BWKM1+m	MT20	5.0	6.0		
I	BBWVW+p	MT20	5.0	6.0	2.75	3.00
J	BWKM1+m	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
J	1118	0	1118	0	0	5-8	1-8	1-8	1-8
H	1118	0	1118	0	0	5-8	1-8	1-8	1-8

UNFACTORED REACTIONS		1ST LOSE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
J	867	628 / 0	130 / 0	0 / 0	0 / 0
H	867	628 / 0	130 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS		WEBS	
MEMB.	FORCE	MEMB.	FORCE
FR-TO	MAX. FACTORED	FR-TO	MAX. FACTORED
A-B	0 / 61	I-D	0 / 772
B-C	-15 / 0	I-E	-101 / 40
C-D	-871 / 0	C-I	-101 / 40
D-E	-871 / 0	J-C	-1157 / 0
E-F	-15 / 0	E-H	-1157 / 0
F-G	0 / 61		
J-B	-360 / 0		
H-F	-360 / 0		
J-I	0 / 776		
I-H	0 / 776		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2012, CBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (0.41")
 CALCULATED VERT. DEFL.(TL) = L/612 (0.18")

CSI: TC=0.21 (A-B:1), BC=0.41 (I-J:3), WB=0.44 (E-H:1), SI=0.14 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

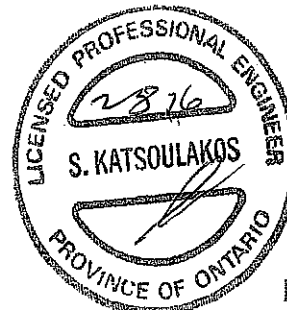
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PS)	SECTION (PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	616 354	1687 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

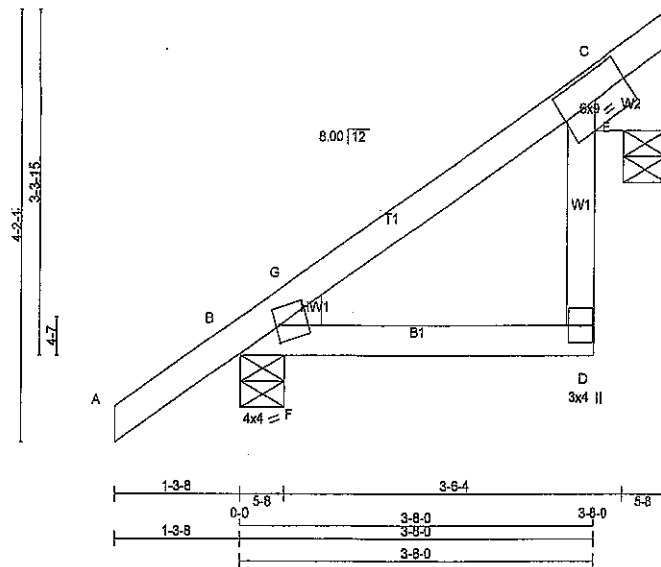
JSI GRIP= 0.80 (H) (INPUT = 0.80)
 JSI METAL= 0.27 (E) (INPUT = 1.00)



DWG NO. TAM 5239-16 R1
 STRUCTURAL
 COMPONENT ONLY

-1-3-8 1-3-8 0-0 3-8-0 3-8-0 9-4 4-8-4

Scale = 1:21.7



TOTAL WEIGHT = 6 X 18 = 106 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-m	MT20	4.0	4.0	2.00 0.50
C	TMVW1-t	MT20	6.0	9.0	
D	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D		HEEL	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
H(C)	243	0	0	243	0	0	0	5-8	1-8		
B	486	0	0	486	0	0	0	5-8	1-8	2x4 L	

UNFACTORED REACTIONS

1ST LCASE		MAX MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
H(C)	193	111/0	34/0	0/0	0/0	46/0	0/0				
B	367	239/0	43/0	0/0	0/0	84/0	0/0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C), B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/46	-124.4	-124.4 0.17 (1)	F-G	-189/42	0.00 (1)	
B-G	-80/0	-124.4	-124.4 0.05 (3)	C-H	-370/0	0.01 (1)	
G-C	-61/0	-124.4	-124.4 0.20 (1)	E-H	0/279	0.00 (1)	
D-E	0/94	0.0	0.0 0.16 (1)				
E-C	0/94	0.0	0.0 0.16 (1)				
B-F	0/53	-28.0	-28.0 0.19 (1)				
F-D	0/53	-28.0	-28.0 0.19 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CSI: TC=0.20 (C-G:1), BC=0.19 (B-F:1), WB=0.01 (C-H:1), SSI=0.17 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 822 2284 1656

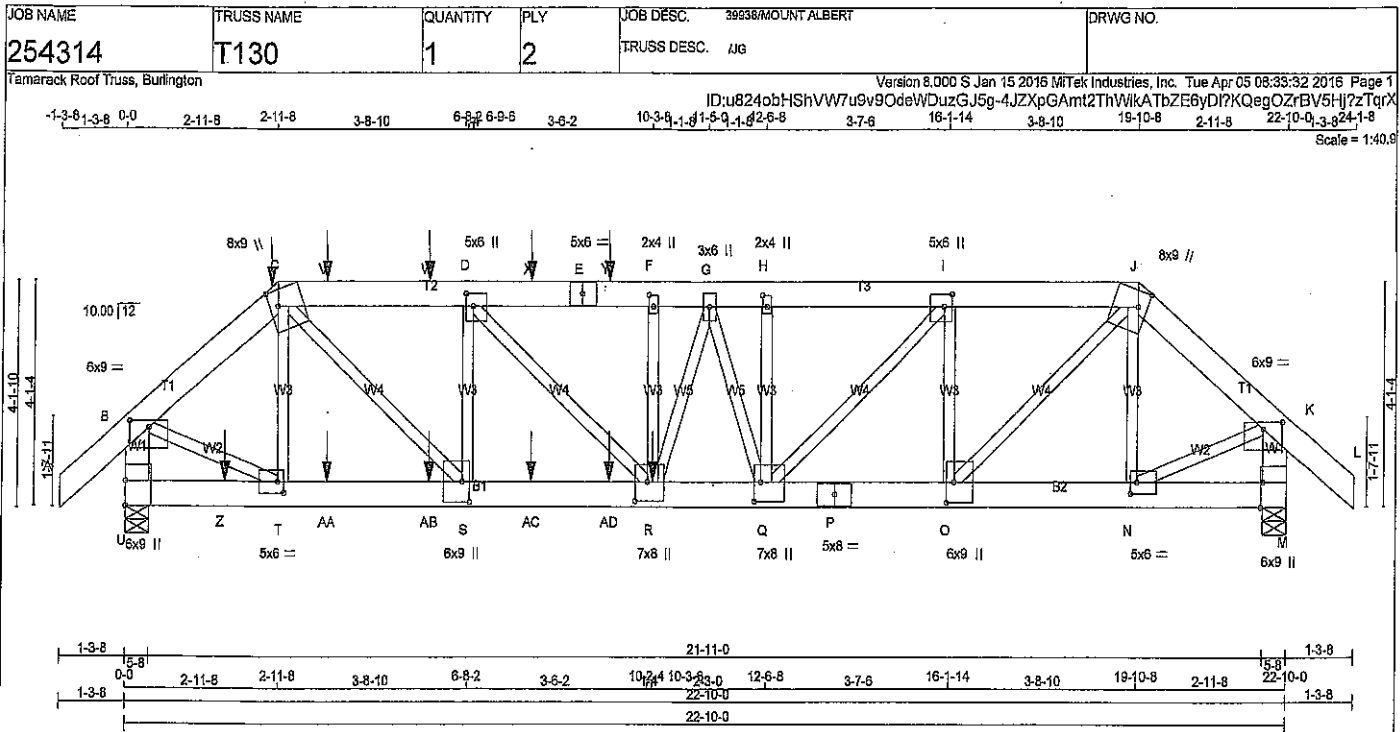
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. TAN/5133-16
STRUCTURAL
COMPONENT ONLY



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2
C - E	2x6	DRY	No.2
E - J	2x6	DRY	No.2
J - L	2x6	DRY	No.2
U - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
U - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2

ALL WEBS 2x6 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS # ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	2 12	SIDE(122.0)
C-E	2 12	SIDE(81.0)
E-J	2 12	SIDE(0.0)
J-L	2 12	TOP
U-B	2 12	TOP
M-K	2 12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P-U	2 12	SIDE(183.1)
P-M	2 12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	1 6	SIDE(425.6)
2x3	1 4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	8.0	9.0	1.50 4.50
C	TTWWH-m	MT20	8.0	9.0	3.50 2.00
D	TMVW-H	MT20	5.0	6.0	2.75 1.75
E	TS-t	MT20	5.0	6.0	
F	TMVW-w	MT20	2.0	4.0	2.50 1.00
G	TMVW-H	MT20	3.0	6.0	
H	TMVW-w	MT20	2.0	4.0	2.50 1.00
I	TMVW-H	MT20	5.0	6.0	2.75 1.75
J	TTWWH-m	MT20	8.0	9.0	3.50 2.00
K	TMVW-p	MT20	6.0	9.0	1.50 4.50
M	BMV1-H	MT20	6.0	8.0	Edge 0.50
N	BMVW-H	MT20	5.0	6.0	2.50 1.50
O	BMVW-H	MT20	6.0	9.0	4.50 1.75
P	BS-t	MT20	5.0	8.0	
Q	BMVW-H	MT20	7.0	8.0	4.25 1.50
R	BMVW-H	MT20	7.0	8.0	4.25 3.00
S	BMVW-H	MT20	6.0	9.0	4.50 1.75
T	BMVW-H	MT20	5.0	6.0	2.50 1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
M	3235	0	0	5-8
U	4002	0	0	5-8

UNFACTORED REACTIONS

	1ST LOOSE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	2513	1624 / 0	393 / 0	0 / 0	0 / 0	603 / 0	0 / 0
U	3113	1881 / 0	479 / 0	0 / 0	0 / 0	753 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL LOADING BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 57	-124.4 -124.4	0.05 (1)	10.00	T-C	-875 / 0	0.10 (1)
B-C	-3915 / 0	-124.4 -124.4	0.07 (1)	5.67	N-J	-708 / 0	0.08 (1)
C-V	-5831 / 0	-124.4 -124.4	0.14 (1)	4.85	B-T	0 / 3182	0.39 (1)
V-W	-5831 / 0	-124.4 -124.4	0.14 (1)	4.85	N-K	0 / 2515	0.31 (1)
W-D	-5831 / 0	-124.4 -124.4	0.14 (1)	4.85	R-F	-253 / 3	0.03 (1)
D-X	-7122 / 0	-124.4 -124.4	0.20 (1)	4.35	G-Q	-279 / 0	0.03 (1)
X-E	-7122 / 0	-124.4 -124.4	0.20 (1)	4.35	R-G	0 / 1100	0.14 (1)
E-Y	-7122 / 0	-124.4 -124.4	0.20 (1)	4.35	G-Q	-1352 / 0	0.17 (1)
Y-F	-7122 / 0	-124.4 -124.4	0.20 (1)	4.35	D-R	0 / 2117	0.28 (1)
F-G	-7122 / 0	-124.4 -124.4	0.12 (1)	4.43	C-S	0 / 3716	0.46 (1)
G-H	-6399 / 0	-124.4 -124.4	0.07 (1)	4.69	S-D	-2362 / 0	0.28 (1)
H-I	-6399 / 0	-124.4 -124.4	0.12 (1)	4.63	C-J	0 / 3255	0.40 (1)
I-J	-4679 / 0	-124.4 -124.4	0.10 (1)	5.27	Q-I	0 / 2442	0.30 (1)
J-K	-3094 / 0	-124.4 -124.4	0.06 (1)	6.21	C-I	-2183 / 0	0.26 (1)
K-L	0 / 57	-124.4 -124.4	0.05 (1)	10.00			
U-B	-3977 / 0	0.0	0.0	1.14 (1)	7.12		
M-K	-3219 / 0	0.0	0.0	0.12 (1)	7.72		
U-Z	0 / 0	-28.0	-28.0	0.04 (2)	10.00		
Z-T	0 / 0	-28.0	-28.0	0.04 (2)	10.00		
T-AA	0 / 2985	-28.0	-28.0	0.23 (1)	10.00		
AA-AB	0 / 2985	-28.0	-28.0	0.23 (1)	10.00		
AB-S	0 / 2985	-28.0	-28.0	0.23 (1)	10.00		
S-AC	0 / 5631	-28.0	-28.0	0.46 (1)	10.00		
AC-AD	0 / 5631	-28.0	-28.0	0.46 (1)	10.00		
AD-R	0 / 5631	-28.0	-28.0	0.46 (1)	10.00		
R-Q	0 / 5797	-28.0	-28.0	0.53 (1)	10.00		
Q-P	0 / 4679	-28.0	-28.0	0.34 (1)	10.00		
P-O	0 / 4679	-28.0	-28.0	0.34 (1)	10.00		
O-N	0 / 2343	-28.0	-28.0	0.18 (1)	10.00		
N-M	0 / 0	-28.0	-28.0	0.03 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.
C	2-11-8	-44	-49	FRONT	VERT
C	2-11-8	-224	-224	FRONT	VERT
R	10-3-8	-2333	-2333	BACK	VERT
V	3-11-4	-151	-151	BACK	VERT
W	5-11-4	-150	-150	BACK	VERT
X	7-11-4	-150	-150	BACK	VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 096-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.76")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.10")
ALLOWABLE DEFL. (TL) = 1/360 (0.76")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.16")

CSK: TC=0.20 (D-F:1), BC=0.53 (Q-R:1), WB=0.46 (C-S:1), SS=0.15 (D-F:1)

DO LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (FL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (N) (INPUT = 0.90)
JSI METAL= 0.51 (P) (INPUT = 1.00)

DRWG NO. TAM 15131-16

STRUCTURAL

COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
U	BMV1+t	MT20	6.0	9.0	5.50	

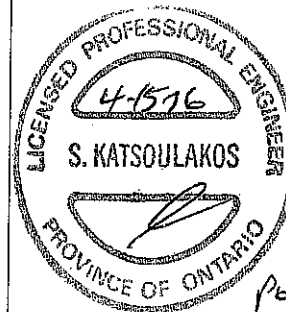
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

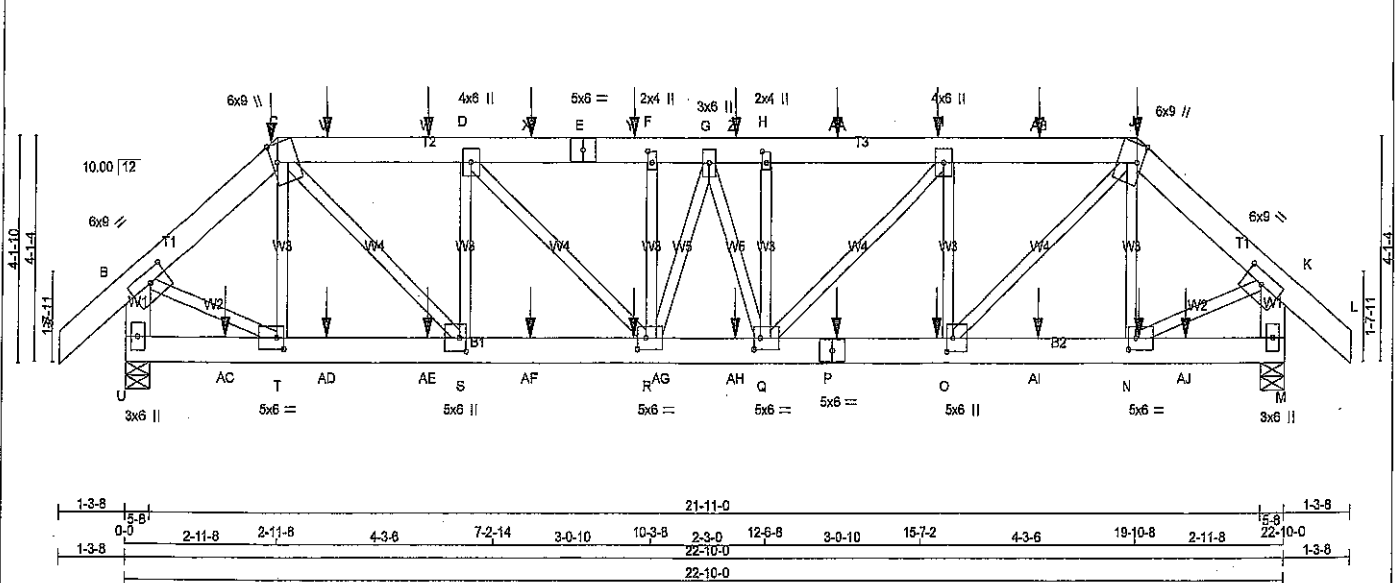
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 287.3 lbs FACTORED DOWN AT 2-11-8, 150.9 lbs FACTORED DOWN AT 3-11-4, 149.7 lbs FACTORED DOWN AT 5-11-4, AND 149.7 lbs FACTORED DOWN AT 7-11-4, AND 149.7 lbs FACTORED DOWN AT 9-5-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, AND 69.9 lbs FACTORED DOWN AT 9-5-4, AND 233.2 lbs FACTORED DOWN AT 10-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	9-8-4	-150	-150	-	BACK	VERT	TOTAL
Z	1-11-4	-40	-70	-	BACK	VERT	TOTAL
AA	3-11-4	-40	-70	-	BACK	VERT	TOTAL
AB	5-11-4	-40	-70	-	BACK	VERT	TOTAL
AC	7-11-4	-40	-70	-	BACK	VERT	TOTAL
AD	9-5-4	-40	-70	-	BACK	VERT	TOTAL



DWG NO. TAM 15131-16
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 138 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6 DRY	No.2	SPF
C - E	2x6 DRY	No.2	SPF
E - J	2x6 DRY	No.2	SPF
J - L	2x6 DRY	No.2	SPF
U - B	2x6 DRY	No.2	SPF
M - K	2x6 DRY	No.2	SPF
U - P	2x6 DRY	No.2	SPF
P - M	2x6 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP
JT	3292	0	0	5-8
M	3206	0	0	5-8
U	3206	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
JT	2591	1513 / 0 493 / 0 0 / 0 0 / 0 644 / 0 0 / 0
M	2521	1478 / 0 421 / 0 0 / 0 0 / 0 622 / 0 0 / 0
U		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.31 (H-I), BC=0.68 (Q-R-I), WB=0.63 (K-N-I), SSI=0.30 (C-D-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVEHEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1667 822 2284 1856

PLATE PLACEMENT TOL = 0.250 inches
PLATE PLACEMENT TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.70 (P) (INPUT = 1.00)

PLATES (table in inches)

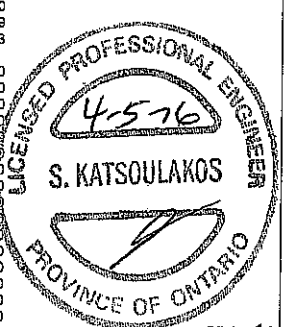
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.50 4.25
C	TTWW-h	MT20	6.0	9.0	4.00 1.25
D	TMVW-h	MT20	4.0	6.0	
E	TS-t	MT20	5.0	6.0	
F	TMVW-w	MT20	2.0	4.0	2.50 1.00
G	TMVW-h	MT20	3.0	6.0	
H	TMVW-w	MT20	2.0	4.0	2.50 1.00
I	TMVW-h	MT20	4.0	6.0	
J	TTWW-h	MT20	6.0	9.0	4.00 1.25
K	TMVW-h	MT20	6.0	9.0	2.50 4.25
M	BMV1-p	MT20	3.0	6.0	
N	BMVW-h	MT20	5.0	8.0	2.50 1.75
O	BMVW-h	MT20	5.0	8.0	3.00 1.50
P	BS-t	MT20	5.0	6.0	
Q	BMVW-w	MT20	5.0	6.0	2.50 1.50
R	BMVW-h	MT20	5.0	6.0	2.50 2.00
S	BMVW-h	MT20	5.0	6.0	3.00 1.50
T	BMVW-h	MT20	5.0	6.0	2.50 1.75
U	BMV1-p	MT20	3.0	6.0	

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					
A-B	0 / 57	-124.4 -124.4 0.10 (1)	10.00	T-C	-898 / 0 0.15 (1)
B-C	-3044 / 0	-124.4 -124.4 0.15 (1)	4.59	C-S	0 / 2352 0.58 (1)
C-V	-3694 / 0	-124.4 -124.4 0.29 (1)	4.05	S-D	-1478 / 0 0.35 (1)
V-W	-3694 / 0	-124.4 -124.4 0.28 (1)	4.05	C-I	-1442 / 0 0.35 (1)
W-D	-3694 / 0	-124.4 -124.4 0.29 (1)	4.05	C-J	0 / 2289 0.57 (1)
D-X	-4661 / 0	-124.4 -124.4 0.30 (1)	3.77	N-J	-644 / 0 0.15 (1)
X-E	-4661 / 0	-124.4 -124.4 0.30 (1)	3.77	B-T	0 / 2474 0.61 (1)
E-Y	-4661 / 0	-124.4 -124.4 0.30 (1)	3.77	N-K	0 / 2548 0.63 (1)
Y-F	-4661 / 0	-124.4 -124.4 0.30 (1)	3.77	R-F	-409 / 0 0.10 (1)
F-G	-4661 / 0	-124.4 -124.4 0.16 (1)	3.92	Q-H	-401 / 0 0.10 (1)
G-Z	-4661 / 0	-124.4 -124.4 0.16 (1)	3.92	D-R	0 / 946 0.23 (1)
Z-H	-4661 / 0	-124.4 -124.4 0.16 (1)	3.92	Q-I	0 / 910 0.23 (1)
H-AA	-4661 / 0	-124.4 -124.4 0.31 (1)	3.76	R-G	-139 / 0 0.04 (1)
AA-I	-4661 / 0	-124.4 -124.4 0.31 (1)	3.76	G-Q	-122 / 0 0.03 (1)
I-AB	-4025 / 0	-124.4 -124.4 0.29 (1)	4.02		
AB-J	-4025 / 0	-124.4 -124.4 0.29 (1)	4.02		
J-K	-3135 / 0	-124.4 -124.4 0.15 (1)	4.63		
K-L	0 / 57	-124.4 -124.4 0.10 (1)	10.00		
U-B	-3173 / 0	0.0 0.0 0.23 (1)	5.89		
M-K	-3257 / 0	0.0 0.0 0.23 (1)	5.83		
U-AC	0 / 0	-28.0 -28.0 0.08 (3)	10.00		
AC-T	0 / 0	-28.0 -28.0 0.08 (3)	10.00		
T-AD	0 / 2307	-28.0 -28.0 0.36 (1)	10.00		
AD-AE	0 / 2307	-28.0 -28.0 0.36 (1)	10.00		
AE-S	0 / 2307	-28.0 -28.0 0.36 (1)	10.00		
S-AF	0 / 3995	-28.0 -28.0 0.60 (1)	10.00		
AF-AG	0 / 3995	-28.0 -28.0 0.60 (1)	10.00		
AG-R	0 / 3995	-28.0 -28.0 0.60 (1)	10.00		
R-AH	0 / 4702	-28.0 -28.0 0.68 (1)	10.00		
AH-Q	0 / 4702	-28.0 -28.0 0.68 (1)	10.00		
Q-P	0 / 4025	-28.0 -28.0 0.61 (1)	10.00		
P-O	0 / 4025	-28.0 -28.0 0.61 (1)	10.00		
O-AI	0 / 2376	-28.0 -28.0 0.35 (1)	10.00		
AI-N	0 / 2376	-28.0 -28.0 0.35 (1)	10.00		
N-AJ	0 / 0	-28.0 -28.0 0.07 (3)	10.00		
AJ-M	0 / 0	-28.0 -28.0 0.07 (3)	10.00		

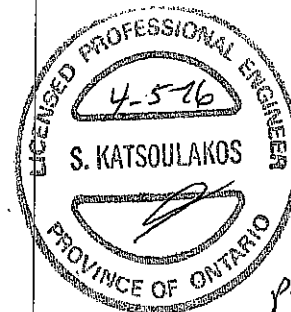
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 451.5 lbs FACTORED DOWN AT 2-11-8, 590.8 lbs FACTORED DOWN AT 19-10-8, 150.9 lbs FACTORED DOWN AT 3-11-4, 149.7 lbs FACTORED DOWN AT 5-11-4, 149.7 lbs FACTORED DOWN AT 7-11-4, 149.7 lbs FACTORED DOWN AT 9-11-4, 149.7 lbs FACTORED DOWN AT 11-11-4, 149.7 lbs FACTORED DOWN AT 13-11-4, 149.7 lbs FACTORED DOWN AT 15-11-4, 149.7 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, 69.9 lbs FACTORED DOWN AT 11-11-4, 69.9 lbs FACTORED DOWN AT 13-11-4, 69.9 lbs FACTORED DOWN AT 15-11-4, 69.9 lbs FACTORED DOWN AT 17-11-4, 69.9 lbs FACTORED DOWN AT 19-11-4, 69.9 lbs FACTORED DOWN AT 20-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

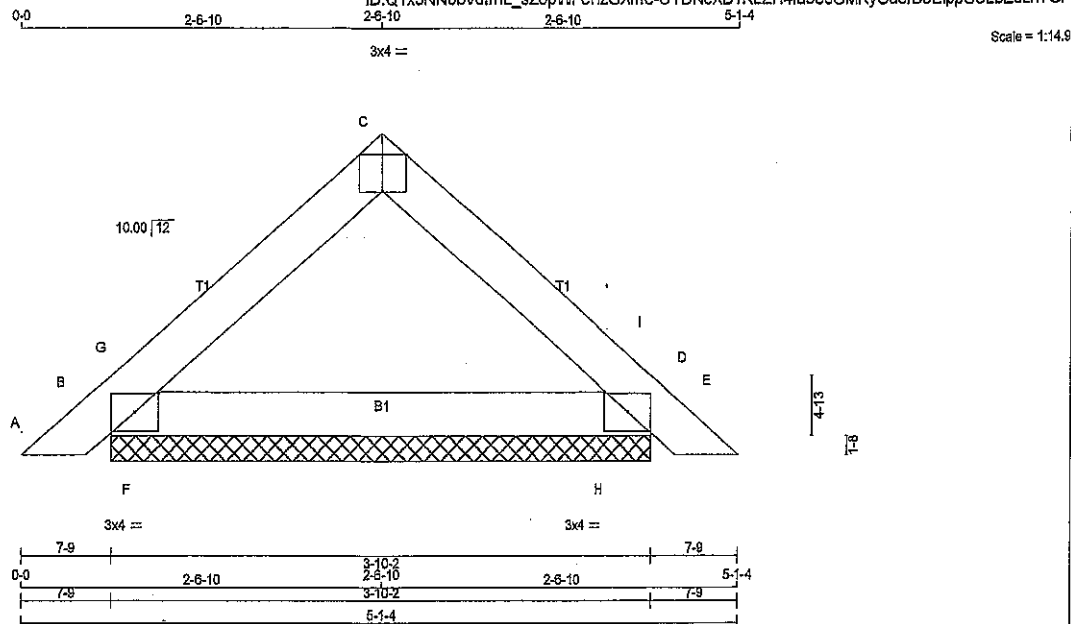


DRWG NO. TAW1513216
STRUCTURAL
COMPONENT ONLY

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR. TYPE
C	2-11-8	-184	-184	—	FRONT	VERT TOTAL
C	2-11-8	-44	-49	—	FRONT	VERT DEAD
C	2-11-8	-224	-224	—	FRONT	VERT SNOW
I	15-11-4	-150	-150	—	FRONT	VERT TOTAL
J	19-10-8	-323	-323	—	FRONT	VERT TOTAL
J	19-10-8	-44	-49	—	FRONT	VERT DEAD
J	19-10-8	-224	-224	—	FRONT	VERT SNOW
N	19-11-4	-40	-70	—	FRONT	VERT TOTAL
O	15-11-4	-40	-70	—	FRONT	VERT TOTAL
P	13-11-4	-40	-70	—	FRONT	VERT TOTAL
V	3-11-4	-151	-151	—	FRONT	VERT TOTAL
W	5-11-4	-150	-150	—	FRONT	VERT TOTAL
X	7-11-4	-150	-150	—	FRONT	VERT TOTAL
Y	9-11-4	-150	-150	—	FRONT	VERT TOTAL
Z	11-11-4	-150	-150	—	FRONT	VERT TOTAL
AA	13-11-4	-150	-150	—	FRONT	VERT TOTAL
AB	17-11-4	-150	-150	—	FRONT	VERT TOTAL
AC	1-11-4	-40	-70	—	FRONT	VERT TOTAL
AD	3-11-4	-40	-70	—	FRONT	VERT TOTAL
AE	5-11-4	-40	-70	—	FRONT	VERT TOTAL
AF	7-11-4	-40	-70	—	FRONT	VERT TOTAL
AG	9-11-4	-40	-70	—	FRONT	VERT TOTAL
AH	11-11-4	-40	-70	—	FRONT	VERT TOTAL
AI	17-11-4	-40	-70	—	FRONT	VERT TOTAL
AJ	20-10-12	-40	-70	—	FRONT	VERT TOTAL



DWG NO. TAM 15132-16
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REGRD BRG
	VERT	HORZ	DOWN	HORZ		
B	360	0	360	0	3-10-2	1-8
D	360	0	360	0	3-10-2	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	278	171 / 0	40 / 0	0 / 0	0 / 0	66 / 0	0 / 0
D	278	171 / 0	40 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			
		VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH			MAX. CSI (LC)			
FR-TO		FROM	TO			FR-TO					
A-B	0 / 19	-124.4	-124.4	0.03 (1)	10.00	F-G	0 / 182	0.00 (1)			
B-G	-289 / 0	-124.4	-124.4	0.06 (3)	6.25	H-I	0 / 182	0.00 (1)			
G-C	-177 / 0	-124.4	-124.4	0.08 (1)	6.25						
C-I	-177 / 0	-124.4	-124.4	0.08 (1)	6.25						
I-D	-289 / 0	-124.4	-124.4	0.06 (3)	6.25						
D-E	0 / 19	-124.4	-124.4	0.03 (1)	10.00						
B-F	0 / 143	-28.0	-28.0	0.04 (3)	10.00						
F-H	0 / 143	-28.0	-28.0	0.08 (2)	10.00						
H-D	0 / 143	-28.0	-28.0	0.04 (3)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 240 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSi: TC=0.08 (C-G:1), BC=0.06 (F-H:2), WB=0.00 (F-G:1), SI=0.10 (B-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

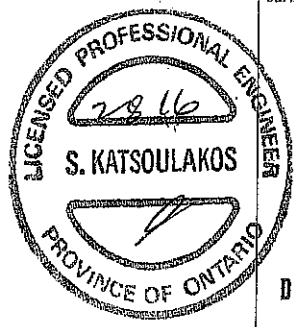
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1657

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.32 (D) (INPUT = 0.80)
JSI METAL= 0.07 (B) (INPUT = 1.00)



DWG NO. TAM 5226-10921
STRUCTURAL
COMPONENT ONLY

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

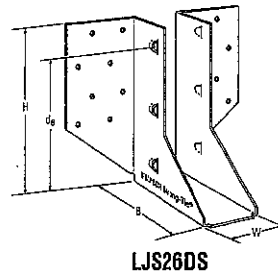
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

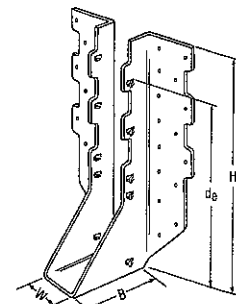
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

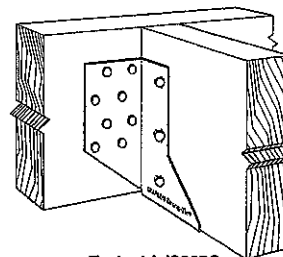
- See current catalogue for options



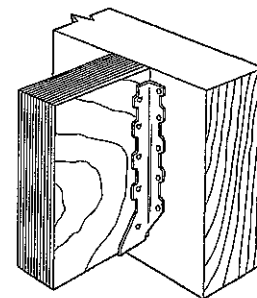
LJS26DS



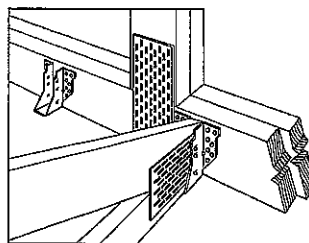
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



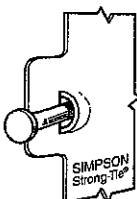
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

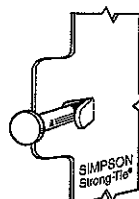
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 3/8	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 1/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/32	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 3/32	3	7 3/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_g is the distance from the seat of the hanger to the highest joist nail.

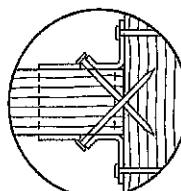


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

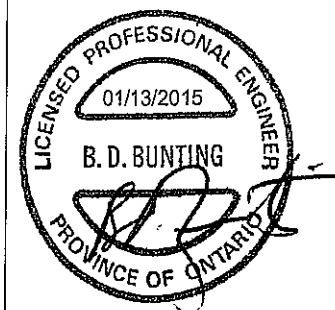
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

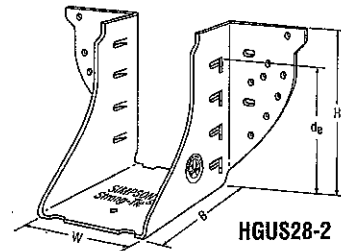
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

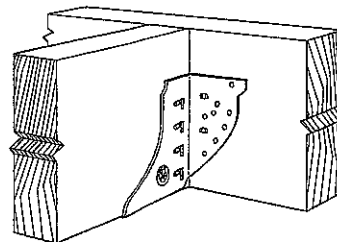
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

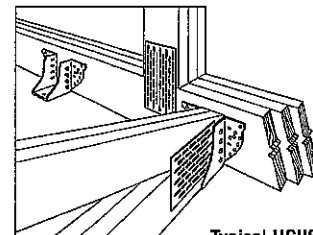
- See current catalogue for options



HGUS28-2



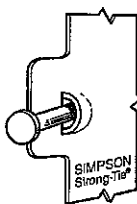
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

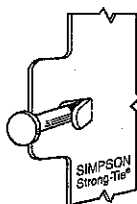
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d ₀ ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _v =1.00)	Uplift (K _u =1.15)	Normal (K _v =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 1/2	20-16d	8-16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/2	20-16d	8-16d	4385	8950	3100	6355
HGUS28	12	1 5/8	7 1/8	5	6 1/2	36-16d	12-16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/2	36-16d	12-16d	6070	12980	4310	9215
HGU210-2	12	3 1/8	9 1/8	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/2	46-16d	16-16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645

1. d₀ is the distance from the seat of the hanger to the highest joist nail.

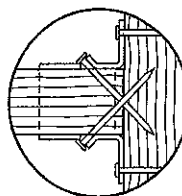


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

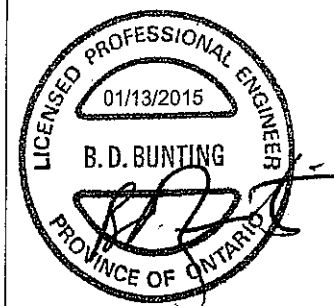
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2016. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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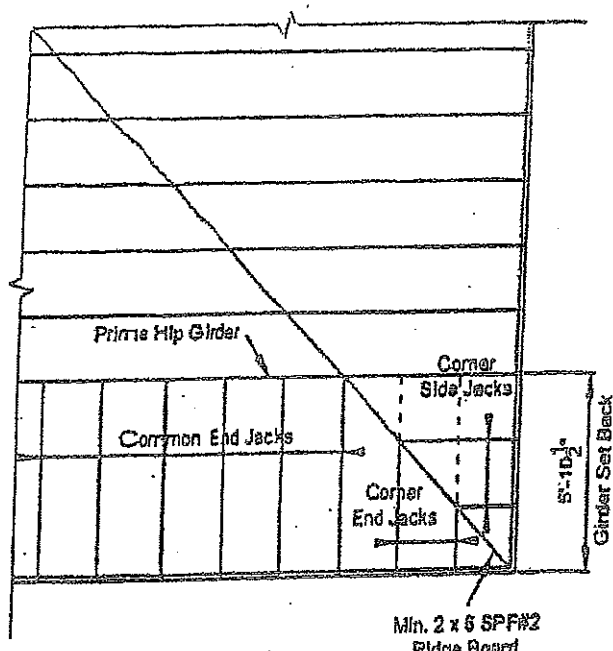
T-SPECHGUS15 1/15 exp. 12/16

800-999-5099

www.strongtie.com

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

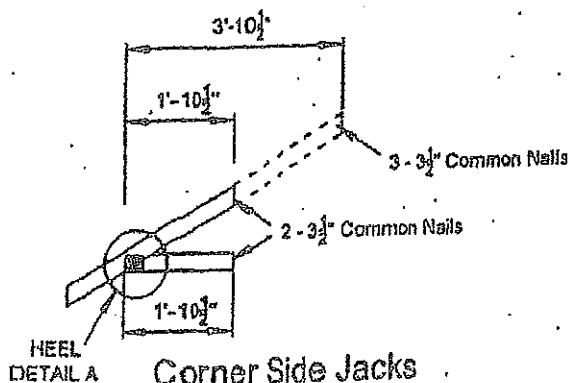
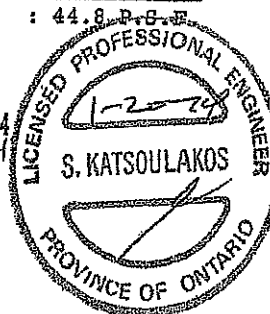
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

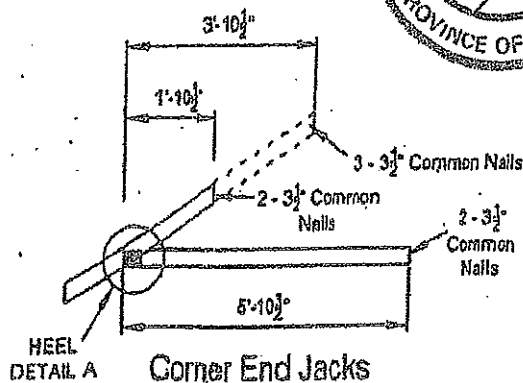
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

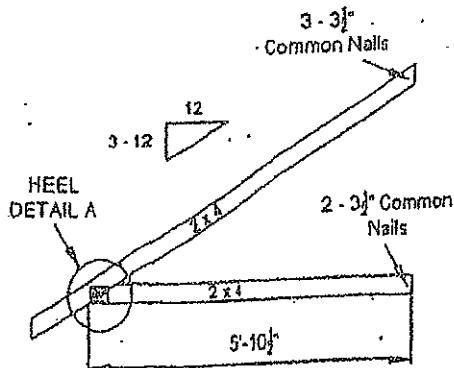
DWG NO T&M 3495.14
STRUCTURAL
COMPONENT ONLY



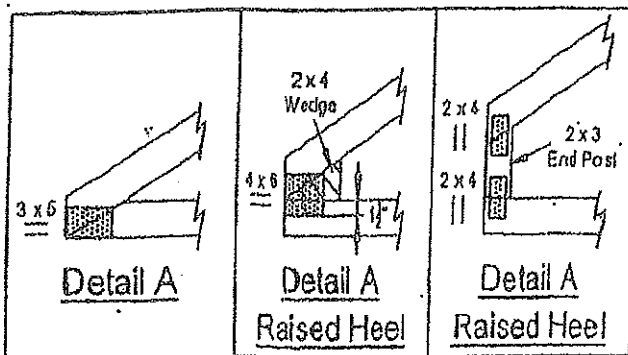
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

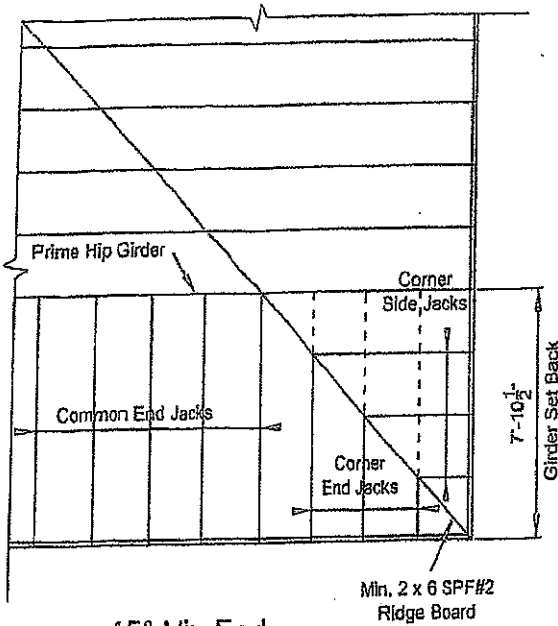
MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



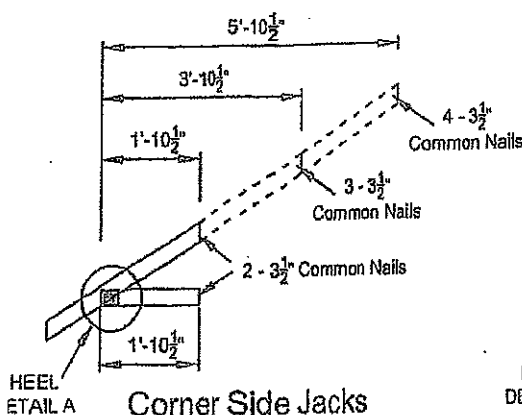
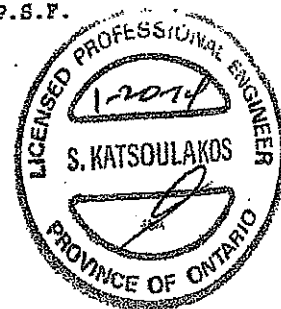
45° Hip End

DESIGN LOAD:

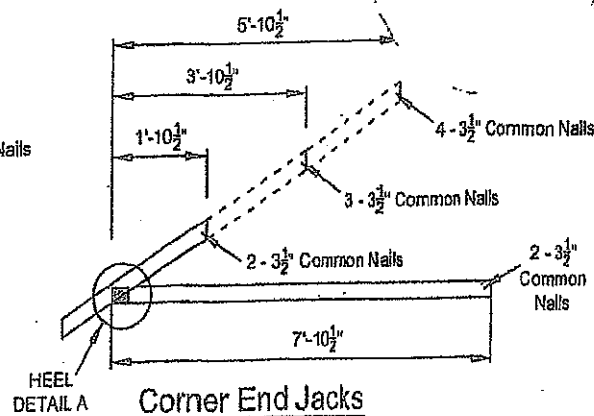
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

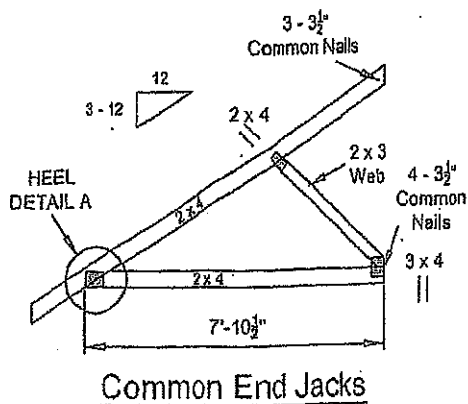
DWG NO TAM 3503.14
STRUCTURAL
COMPONENT ONLY



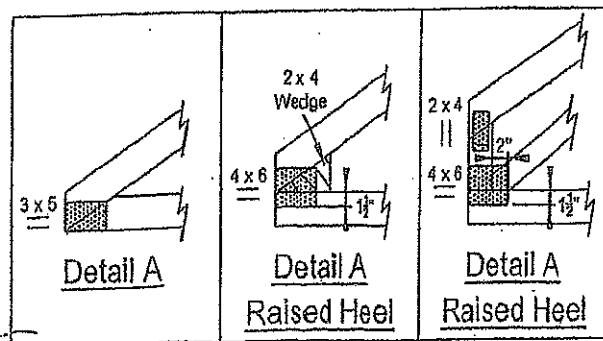
Corner Side Jacks



Corner End Jacks



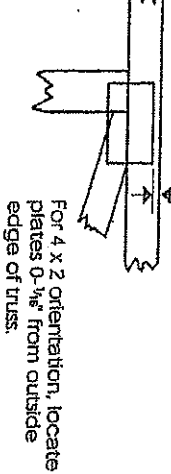
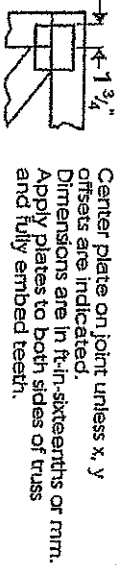
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

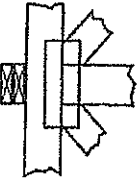
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, 1 or Eliminator bracing if indicated.

BEARING

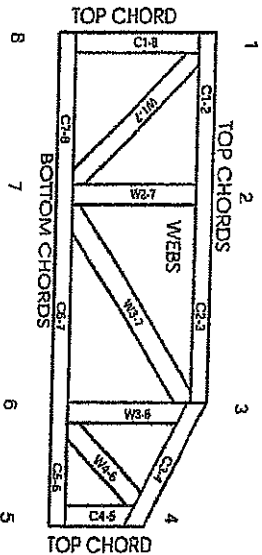


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPI-C: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

8-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11995-L, 10319-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet MTK-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

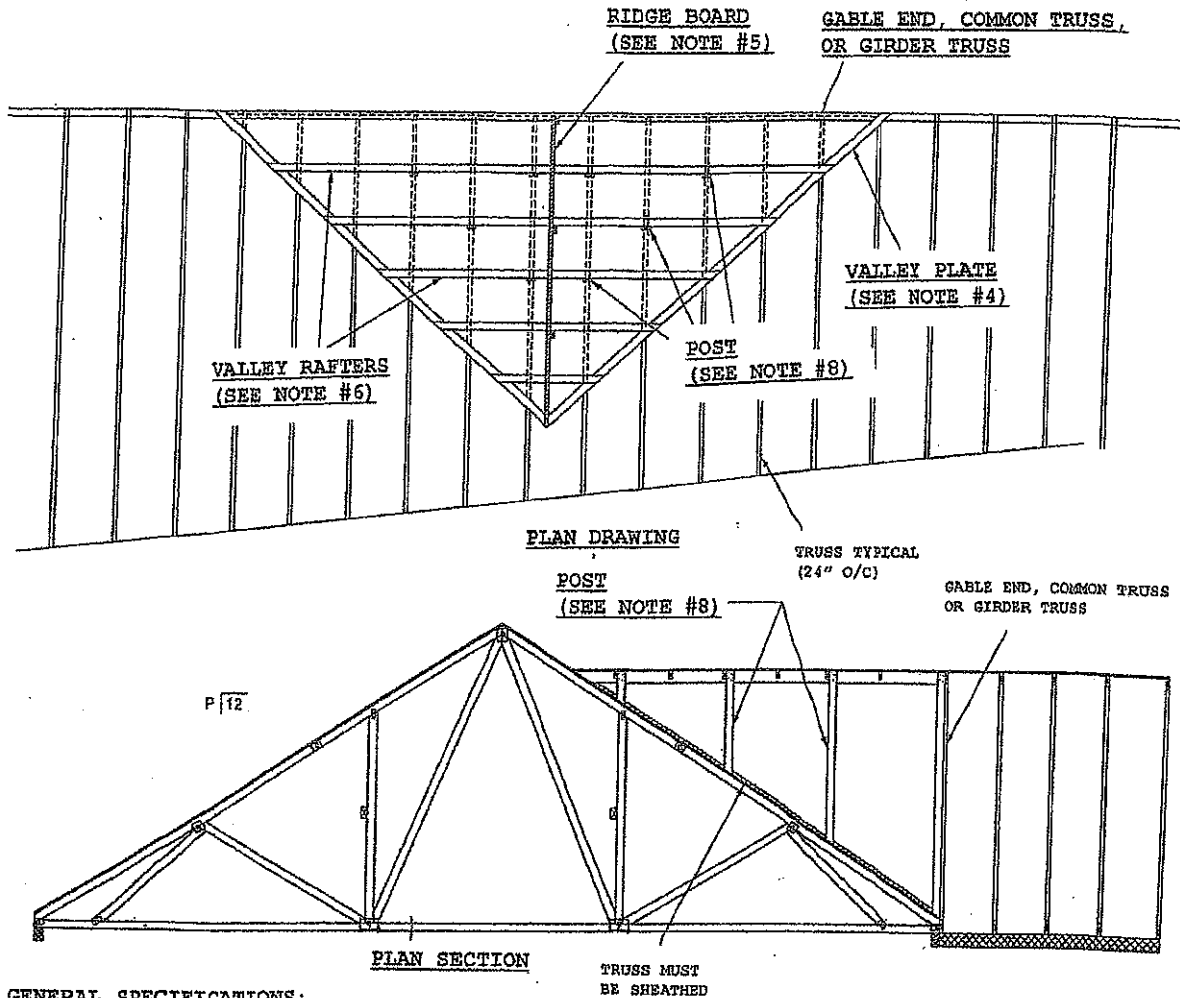
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative 1, 1, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Truss and wane at joint locations are regulated by TPI-C.
7. Design assumes trusses will be suitably protected from the environment in accord with TPI-C.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPI-C Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

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CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

TRUSS MUST
BE SHEATHED

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.).
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.).
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.).
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
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Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.