

53-08-00

4-06-00

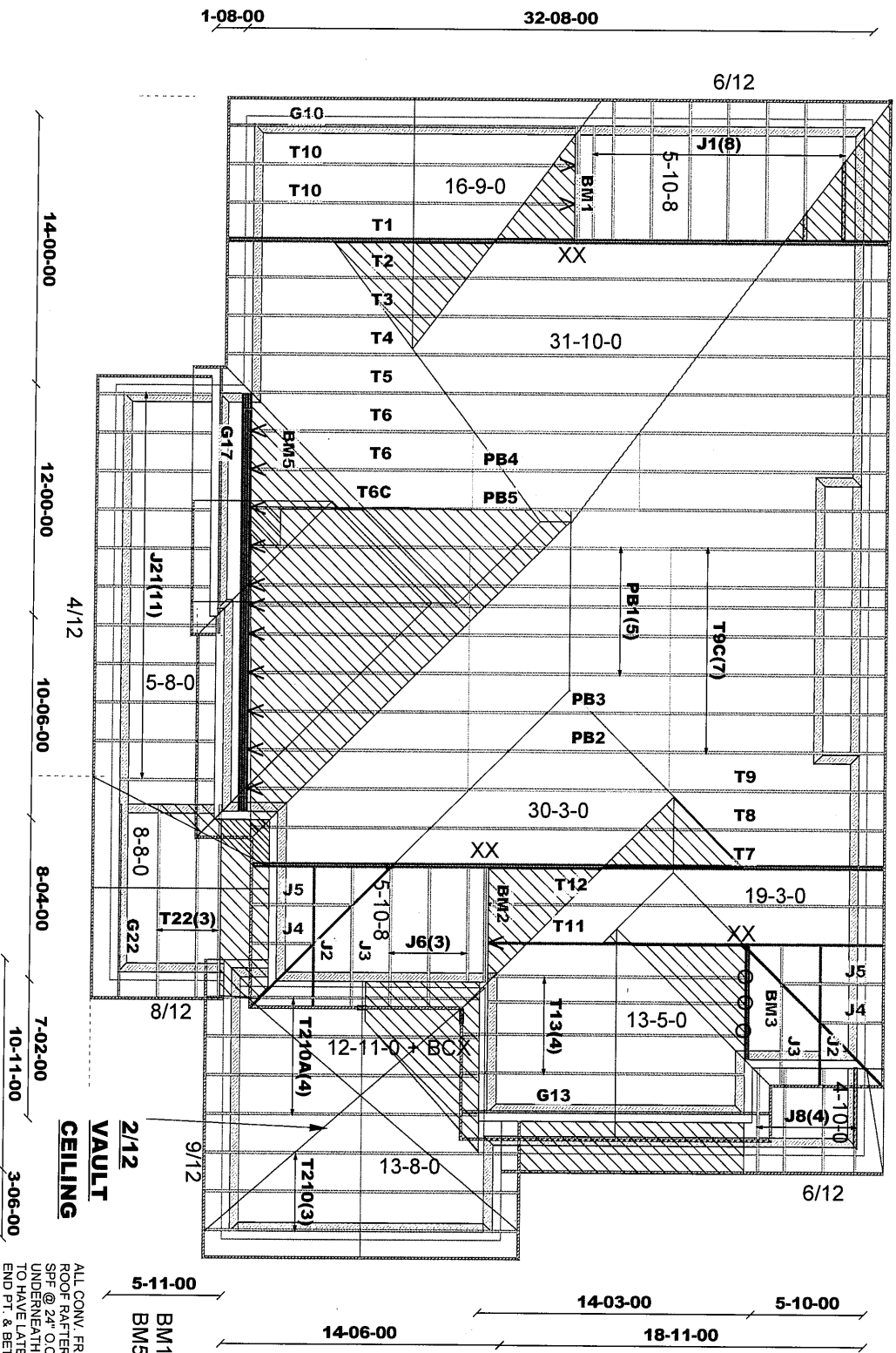
19-01-00

13-11-00

16-02-00

2-10-00

ROOF PITCH 8/12 UNLESS NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LUS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 36 PSF
TC DEAD 8 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170769

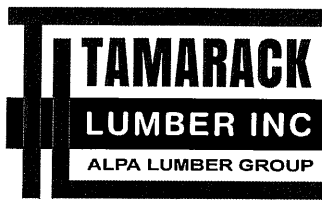
BM1, BM2, BM3 = 2-2X10
BM5 = 4PLY - 1 3/4" X 16" LV. 2.0E

2/12
VAULT
CEILING

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4
SPF @ 24" O.C. WITH A 2X4 VERT. POST TO THE TRUSSES
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M/L 401
SECOND VALES

Job Track:	45246	Builder / Location:	GREENPARK HOMES / EAST GWILLIMBURY	Model / Elevation:	GLENWAY 12A / 2 LOT 12
Layout ID:	289751	Project:	TRINAR HALL HOMES INC/SECOND VALES		
Plan Log:	95086	Date:	2018-11-06	Designer:	AC/S V
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.					



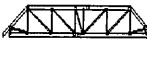
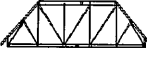
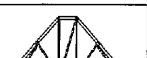
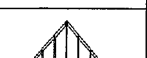
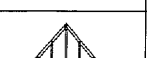
Delivery Shiplist

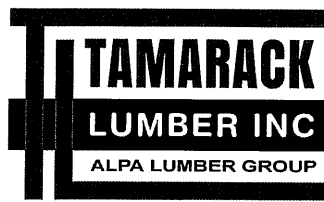
DATE	10/29/18
SALES REP	Mario

JOB TRACK: 45246 LAYOUT ID: 289751 LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALES ESTA SUB-BUILDER:
 MODEL: GLENWAY 12A ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T1 HIP GIRDER	8.00 0.00	31-10-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	341.02 208.00		
	2 Ply											
	1	T2 HIP	8.00 0.00	31-10-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	133.60 83.84		
	1	T3 HIP	8.00 0.00	31-10-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	145.16 89.33		
	1	T4 HIP	8.00 0.00	31-10-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	141.43 88.00		
	1	T5 HIP	8.00 0.00	31-10-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	149.48 92.33		
	2	T6 PIGGYBACK	8.00 0.00	31-10-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	328.20 206.00		
	1	T6C PIGGYBACK	8.00 0.00	31-10-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	164.10 103.00		
	1	T7 HIP GIRDER	8.00 0.00	30-03-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	299.94 186.34		
	2 Ply											
	1	T8 HIP	8.00 0.00	30-03-00	06-07-13	2 X 4	2 X 4	01-03-08 01-05-00	01-04-13 01-04-13	133.87 86.00		
	1	T9 PIGGYBACK	8.00 0.00	31-10-00	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-00	151.60 94.67		
	7	T9C PIGGYBACK	8.00 0.00	31-10-00	07-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-06-00	1061.20 662.69		
	2	T10 COMMON	8.00 0.00	16-09-00	06-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	148.80 91.34		
	1	G10 COMMON	8.00 0.00	16-09-00	06-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	76.54 49.17		
	1	T11 HIP GIRDER	8.00 0.00	19-03-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	98.99 60.83		
	1	T12 HIP	8.00 0.00	19-03-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	85.74 55.84		
	4	T13 COMMON	8.00 0.00	13-05-00	05-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	233.24 148.00		
	1	G13 COMMON	8.00 0.00	13-05-00	05-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	60.09 38.00		
	1	G17 COMMON	8.00 0.00	11-02-00	05-01-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	48.64 31.83		



Delivery Shiplist

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JOB TRACK:45246 LAYOUT ID: 289751 LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALES ESTA SUB-BUILDER:
 MODEL: GLENWAY 12A ELEVATION: 2

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	G22 COMMON	8.00 0.00	08-08-00	03-08-04	2 X 4	2 X 4	01-03-08 01-03-08	00-09-10 00-09-10	31.93 20.67		
	3	T22 COMMON	8.00 0.00	08-08-00	03-08-04	2 X 4	2 X 4	01-03-08 01-03-08	00-09-10 00-09-10	106.41 68.01		
	3	T210 SCISSOR	9.00 2.00	13-08-00	06-07-12	2 X 4	2 X 4	01-03-08 01-03-08	01-06-04 01-06-04	190.59 123.99		
	4	T210A SCISSOR	9.00 2.00	13-08-00	06-07-12	2 X 4	2 X 4	01-03-08 00-00-00	01-06-04 00-02-07	244.04 160.00		
	5	PB1 PIGGYBACK	8.00 0.00	09-00-02	03-04-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	134.45 87.50		
	1	PB2 PIGGYBACK	8.00 0.00	09-00-02	01-02-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	24.70 16.83		
	1	PB3 PIGGYBACK	8.00 0.00	09-00-02	02-06-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	29.61 18.83		
	1	PB4 PIGGYBACK	8.00 0.00	07-02-13	00-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	19.60 13.50		
	1	PB5 PIGGYBACK	8.00 0.00	07-02-13	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	21.68 14.67		
	8	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	134.32 85.36		
	2	J2 JACK-OPEN	8.00 0.00	05-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -04-01-01	01-04-13 00-03-08	24.46 14.66		
	2	J3 JACK-OPEN	8.00 0.00	05-10-08	03-11-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	29.94 18.66		
	2	J4 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	15.34 9.34		
	2	J5 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	20.82 13.34		
	3	J6 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	53.49 32.01		
	4	J8 JACK-OPEN	6.00 0.00	04-10-00	02-09-03	2 X 4	2 X 4	01-03-08 00-00-00	00-04-03 02-09-03	53.96 32.00		
	11	J21 JACK-OPEN	4.00 0.00	05-08-00	02-02-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-02-10	164.34 102.63		

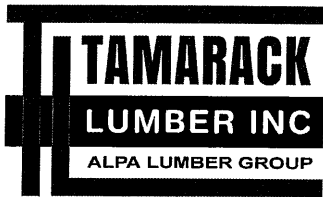
TOTAL # TRUSS= 85.00

TOTAL BFT OF ALL TRUSSES=

3207.21 BFT. TOTAL WEIGHT OF ALL TRUSSES= 5101.32 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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Delivery Shiplist

DATE	10/29/18
SALES REP	Mario

JOB TRACK: 45246	LAYOUT ID: 289751	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALES ESTA	SUB-BUILDER:	
MODEL: GLENWAY 12A	ELEVATION: 2	

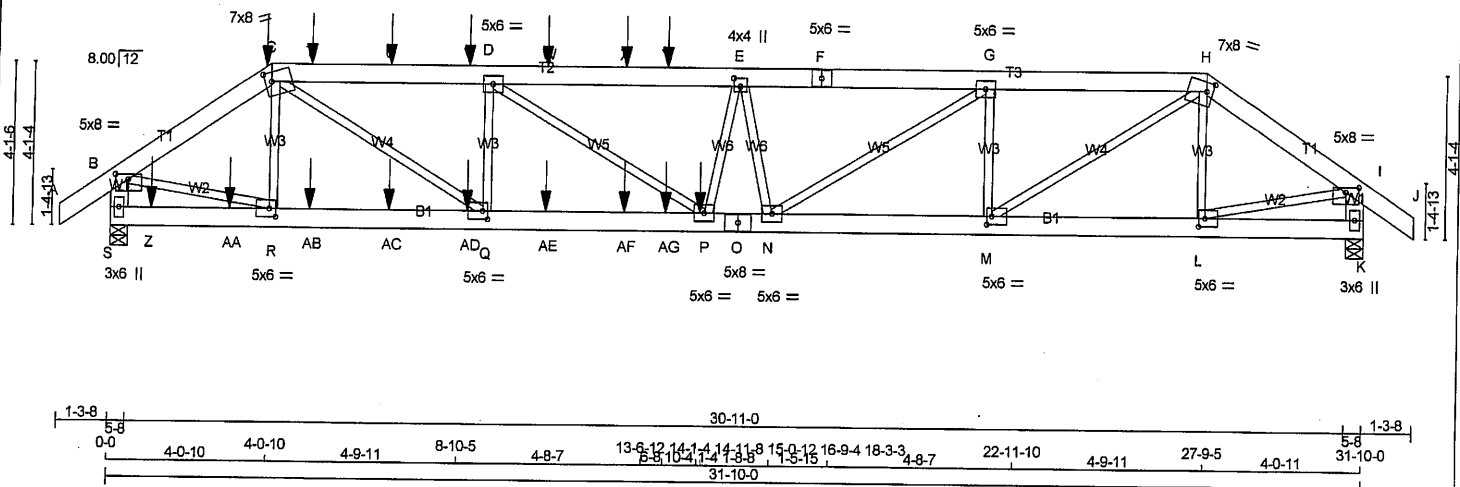
HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
3	Hangers	LUS24	
13	Hangers	LJS26DS	
4	Beam	1 3/4" x 16" LVL 2.0E	22-00-00

TOTAL # ITEMS= 23.00

JOB NAME 289750	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. 45246	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.210 S May 18 2018 MiTek Industries, Inc. Thu Nov 1 11:08:34 2018 Page 1
 ID: DvdDM7sBvDfd2U1ZhskATKyRpFFIya5sj8amqJtavKt5QqhS9KLalbyec2WPNWWSynNbcB
 -1-3-8 0-0 4-0-10 4-0-10 4-9-11 8-10-5 4-8-7 13-6-12 14-1-4 15-0-12 16-9-4 18-3-3 4-8-7 22-11-10 4-9-11 27-9-5 4-0-11 31-10-0 31-10-0
 Scale = 1:55.6



TOTAL WEIGHT = 2 X 168 = 336 lb [M]

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x6 DRY	No.2	SPF
C - F	2x6 DRY	No.2	SPF
F - H	2x6 DRY	No.2	SPF
H - J	2x6 DRY	No.2	SPF
S - B	2x6 DRY	No.2	SPF
K - I	2x6 DRY	No.2	SPF
S - O	2x6 DRY	No.2	SPF
O - K	2x6 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	SIDE (122.0)
C-F 2	12	SIDE (61.0)
F-H 2	12	TOP
H-J 2	12	TOP
S-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-O 2	12	SIDE (0.0)
O-K 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	4.00
C	TTWW-m	MT20	7.0	8.0	2.75	2.00
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.50	2.00
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	7.0	8.0	2.75	2.00
I	TMVW-p	MT20	5.0	8.0	1.50	4.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW-t	MT20	5.0	6.0	2.50	1.50
N	BMWW-t	MT20	5.0	6.0		
O	BS-t	MT20	5.0	8.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	1.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQ'D	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	VERT	HORZ	BRG	BRG
S	4929	0	0	4929	0	0	5-8	5-8
K	4005	0	0	4005	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
S	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	3854	2306 / 0	620 / 0	0 / 0	0 / 0	928 / 0	0 / 0
K	3121	1884 / 0	490 / 0	0 / 0	0 / 0	746 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4).

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 50	-127.8 -127.8	0.05 (1)	10.00	R-C	-793 / 0	0.09 (1)
B-C	-6000 / 0	-127.8 -127.8	0.15 (1)	4.72	C-Q	0 / 5398	0.67 (1)
C-T	-9465 / 0	-127.8 -127.8	0.40 (1)	3.66	Q-D	-2679 / 0	0.32 (1)
T-U	-9465 / 0	-127.8 -127.8	0.40 (1)	3.66	D-P	0 / 2419	0.30 (1)
U-V	-9465 / 0	-127.8 -127.8	0.40 (1)	3.66	N-G	0 / 3385	0.42 (1)
V-D	-9465 / 0	-127.8 -127.8	0.40 (1)	3.66	M-G	-2620 / 0	0.31 (1)
D-W	-11489 / 0	-127.8 -127.8	0.51 (1)	3.26	M-H	0 / 6612	0.62 (1)
W-X	-11489 / 0	-127.8 -127.8	0.51 (1)	3.26	L-H	872 / 0	0.08 (1)
X-Y	-11489 / 0	-127.8 -127.8	0.51 (1)	3.26	B-X	0 / 6612	0.08 (1)
Y-E	-11489 / 0	-127.8 -127.8	0.51 (1)	3.26	E-X	0 / 6612	0.08 (1)
E-F	-10975 / 0	-127.8 -127.8	0.38 (1)	3.48	P-X	0 / 6612	0.08 (1)
F-G	-10975 / 0	-127.8 -127.8	0.38 (1)	3.48	R-N	1728 / 0	0.21 (1)
G-H	-8144 / 0	-127.8 -127.8	0.25 (1)	4.05			
H-I	-4792 / 0	-127.8 -127.8	0.13 (1)	5.78			
I-J	0 / 50	-127.8 -127.8	0.05 (1)	10.00			
S-B	-4862 / 0	0.0 0.0	0.17 (1)	6.56			
K-I	-3971 / 0	0.0 0.0	0.14 (1)	7.13			
S-Z	0 / 0	-28.0 -28.0	0.08 (3)	10.00			
Z-AA	0 / 0	-28.0 -28.0	0.08 (3)	10.00			
AA-R	0 / 0	-28.0 -28.0	0.08 (3)	10.00			
R-AB	0 / 4957	-28.0 -28.0	0.37 (1)	10.00			
AB-AC	0 / 4957	-28.0 -28.0	0.37 (1)	10.00			
AC-AD	0 / 4957	-28.0 -28.0	0.37 (1)	10.00			
AD-Q	0 / 4957	-28.0 -28.0	0.37 (1)	10.00			
Q-AE	0 / 9465	-28.0 -28.0	0.73 (1)	10.00			
AE-AF	0 / 9465	-28.0 -28.0	0.73 (1)	10.00			
AF-AG	0 / 9465	-28.0 -28.0	0.73 (1)	10.00			
AG-P	0 / 9465	-28.0 -28.0	0.73 (1)	10.00			
P-O	0 / 11343	-28.0 -28.0	0.81 (1)	10.00			
O-N	0 / 11343	-28.0 -28.0	0.81 (1)	10.00			
N-M	0 / 8144	-28.0 -28.0	0.59 (1)	10.00			
M-L	0 / 3958	-28.0 -28.0	0.29 (1)	10.00			
L-K	0 / 0	-28.0 -28.0	0.05 (2)	10.00			

PROFESSIONAL
11/01/19
H. J. G. ALVES
100009024

PROVINCE OF ONTARIO

DWG NO. TAM 116120
STRUCTURAL
COMPONENT ONLY

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-10	-54	-54		FRONT	VERT	DEAD
C	4-0-11	-240	-240		FRONT	VERT	SNOW
P	14-11-8	-1488	-1488		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIAL LOADS:

TOP CH.	LL = 35.9 PSF
	DL = 8.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 61.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, EBCB 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(LL) = U/999 (0.27")
 ALLOWABLE DEFL.(TL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(TL) = U/868 (0.44")

CSI: TC=0.51/1.00 (D-E:1), BC=0.81/1.00 (N-P:1), WB=0.67/1.00 (C-Q:1), SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
 JSI METAL= 0.90 (O) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME 289750	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. 45246 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

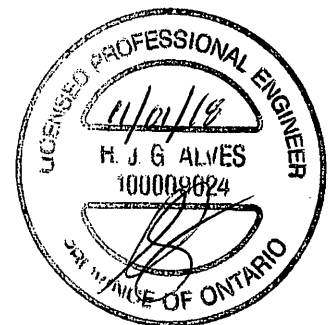
Version 8.210 S May 18 2018 MiTek Industries, Inc. Thu Nov 1 11:08:35 2018 Page 2
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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 427.5 lbs FACTORED DOWN AT 4-0-10,
153.8 lbs FACTORED DOWN AT 5-0-12, 153.8 lbs
FACTORED DOWN AT 7-0-12, 153.8 lbs
FACTORED DOWN AT 9-0-12, 153.8 lbs
FACTORED DOWN AT 11-0-12, AND 153.8 lbs
FACTORED DOWN AT 13-0-12, AND 153.8 lbs
FACTORED DOWN AT 14-1-4 ON TOP CHORD,
AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9
lbs FACTORED DOWN AT 3-0-12, 69.9 lbs
FACTORED DOWN AT 5-0-12, 69.9 lbs
FACTORED DOWN AT 7-0-12, 69.9 lbs
FACTORED DOWN AT 9-0-12, 69.9 lbs
FACTORED DOWN AT 11-0-12, 69.9 lbs
FACTORED DOWN AT 13-0-12, AND 69.9 lbs
FACTORED DOWN AT 14-1-4, AND 1943.2 lbs
FACTORED DOWN AT 14-11-8 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	5-0-12	-106	-106	—	FRONT	VERT	TOTAL
U	7-0-12	-106	-106	—	FRONT	VERT	TOTAL
V	9-0-12	-106	-106	—	FRONT	VERT	TOTAL
W	11-0-12	-106	-106	—	FRONT	VERT	TOTAL
X	13-0-12	-106	-106	—	FRONT	VERT	TOTAL
Y	14-1-4	-106	-106	—	FRONT	VERT	TOTAL
Z	1-0-12	-50	-50	—	FRONT	VERT	TOTAL
AA	3-0-12	-50	-50	—	FRONT	VERT	TOTAL
AB	5-0-12	-50	-50	—	FRONT	VERT	TOTAL
AC	7-0-12	-50	-50	—	FRONT	VERT	TOTAL
AD	9-0-12	-50	-50	—	FRONT	VERT	TOTAL
AE	11-0-12	-50	-50	—	FRONT	VERT	TOTAL
AF	13-0-12	-50	-50	—	FRONT	VERT	TOTAL
AG	14-1-4	-50	-50	—	FRONT	VERT	TOTAL

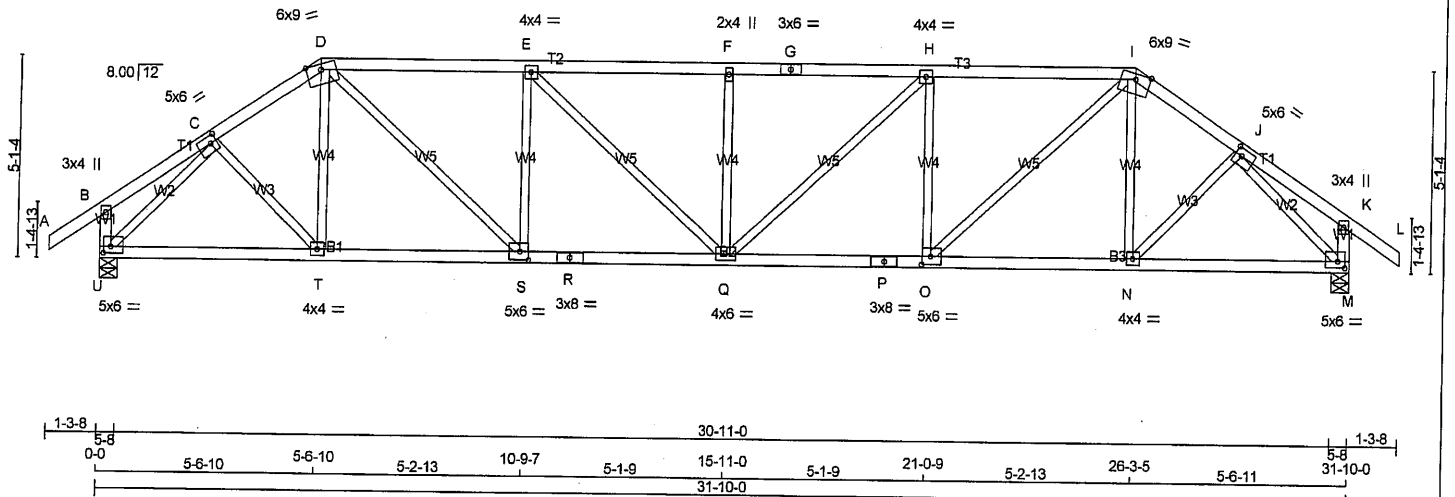


DWG NO. TAM: 1712096
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME 289750	TRUSS NAME T2	QUANTITY 1	PLY 1	JOB DESC. 45246	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MITek Industries, Inc. Thu Nov 1 10:48:10 2018 Page 1
ID:DvdDM7sBvDfd2U1ZhskATKyRpFf8TdNzBKo0l6YkpmekOKtfr1e4wL9IA15t3ztbYOyNbvJ
1-3-8 0-0 2-10-9 2-10-9 2-8-1 5-6-10 5-2-13 10-9-7 5-1-9 15-11-0 5-1-9 21-0-9 5-2-13 28-3-5 2-8-1 28-11-7 2-10-9 31-10-0 33-1-8
Scale = 1:55.8



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
U - R	2x4	DRY No.2	SPF
R - P	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWV-t	MT20	5.0	6.0	2.25	2.00
D	TTWW-m	MT20	6.0	9.0	Edge	
E	TMVWV-t	MT20	4.0	4.0		
F	TMVWV-t	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVWV-t	MT20	4.0	4.0		
I	TTWW-m	MT20	6.0	9.0	Edge	
J	TMVWV-t	MT20	5.0	6.0	2.25	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVWV-t	MT20	5.0	6.0	2.00	2.25
N	BMVWV-t	MT20	4.0	4.0		
O	BMVWV-t	MT20	5.0	6.0	2.50	2.75
P	BS-t	MT20	3.0	8.0		
Q	BMVWV-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	8.0		
S	BMVWV-t	MT20	5.0	6.0	2.50	2.75
T	BMVWV-t	MT20	4.0	4.0		
U	BMVWV-t	MT20	5.0	6.0	2.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
U	2655	0	0	5-8
M	2655	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	2076	1242 / 0	334 / 0	0 / 0	0 / 0	499 / 0	0 / 0
M	2076	1242 / 0	334 / 0	0 / 0	0 / 0	499 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 49	-127.8	-127.8	C-T	0 / 349	0.08	(1)
B-C	0 / 16	-127.8	-127.8	D-S	-75 / 128	0.03	(3)
C-D	-2895 / 0	-127.8	-127.8	E-Q	0 / 1854	0.42	(1)
D-E	-3755 / 0	-127.8	-127.8	F-R	-1118 / 0	0.43	(1)
E-F	-4154 / 0	-127.8	-127.8	G-H	0 / 547	0.12	(1)
F-G	-4154 / 0	-127.8	-127.8	H-I	-604 / 0	0.23	(1)
G-H	-4154 / 0	-127.8	-127.8	I-J	0 / 547	0.12	(1)
H-I	-3755 / 0	-127.8	-127.8	J-K	-1118 / 0	0.43	(1)
I-J	-2895 / 0	-127.8	-127.8	K-L	0 / 1854	0.42	(1)
J-K	0 / 16	-127.8	-127.8	L-M	-75 / 128	0.03	(3)
K-L	0 / 49	-127.8	-127.8	M-N	0 / 349	0.08	(1)
U-B	-323 / 0	0.0	0.0	N-J	-3120 / 0	0.83	(1)
M-K	-323 / 0	0.0	0.0	J-M	-3120 / 0	0.83	(1)
U-T	0 / 2144	-28.0	-28.0				
T-S	0 / 2388	-28.0	-28.0				
S-R	0 / 3755	-28.0	-28.0				
R-Q	0 / 3755	-28.0	-28.0				
Q-P	0 / 3755	-28.0	-28.0				
P-O	0 / 3755	-28.0	-28.0				
O-N	0 / 2388	-28.0	-28.0				
N-M	0 / 2144	-28.0	-28.0				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.9	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD				= 61.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/989 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/989 (0.39")
CSI: TC=0.87/1.00 (F-H:1), BC=0.67/1.00 (O-Q:1),
WB=0.83/1.00 (J-M:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

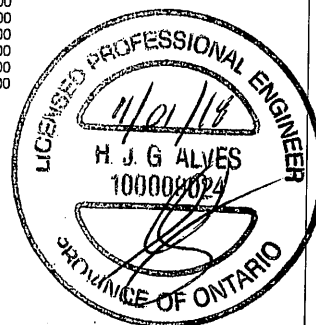
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667

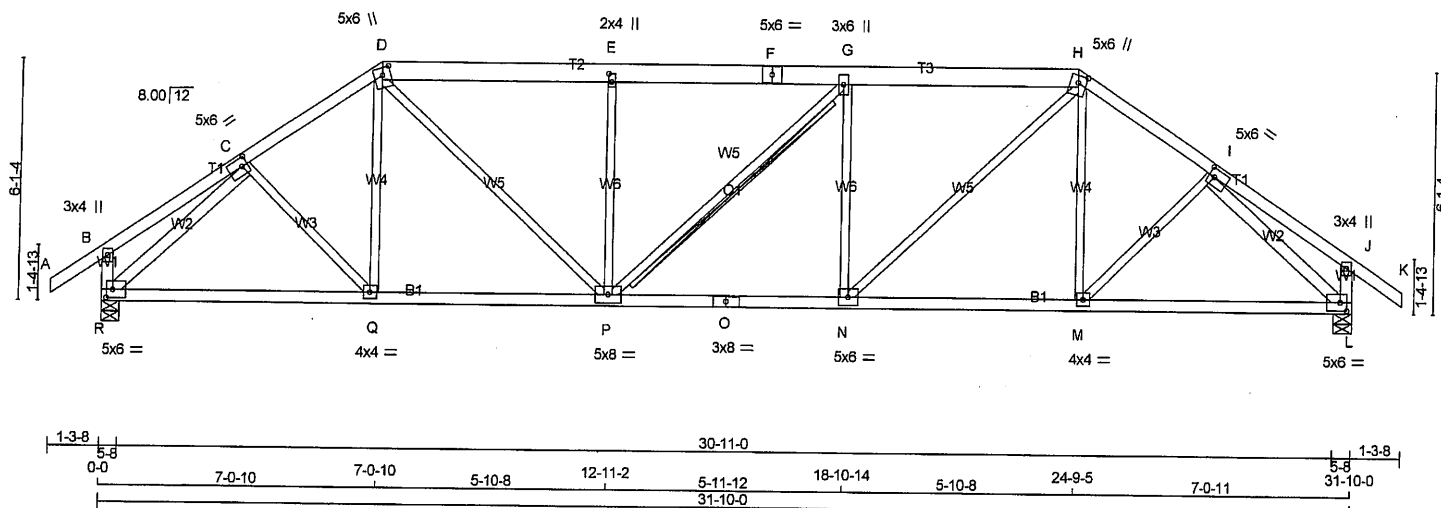
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (U) (INPUT = 0.90)
SI METAL = 0.98 (R) (INPUT = 1.00)



DWG NO. TAM 11812007
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 148 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	5.0	6.0	2.25	2.50
E	TMVW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMVW+t	MT20	3.0	6.0		
H	TTWW+m	MT20	5.0	6.0	2.25	2.50
I	TMVW+t	MT20	5.0	6.0	2.50	1.75
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.00
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	8.0		
P	BMVWW-t	MT20	5.0	8.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.50	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	DOWN	BRG	BRG
R	2655	0	5-8	5-8
L	2655	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2076	1242 / 0	334 / 0	0 / 0	0 / 0	499 / 0	0 / 0
L	2076	1242 / 0	334 / 0	0 / 0	0 / 0	499 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.78 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT G-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 49	-127.8 -127.8	0.17 (1)	10.00	C-Q	0 / 201	0.05 (3)
B-C	0 / 26	-127.8 -127.8	0.23 (1)	10.00	Q-D	0 / 233	0.06 (3)
C-D	-2884 / 0	-127.8 -127.8	0.35 (1)	3.78	M-H	0 / 233	0.06 (3)
D-E	-3401 / 0	-127.8 -127.8	0.34 (1)	4.32	M-I	0 / 200	0.05 (3)
E-F	-3401 / 0	-127.8 -127.8	0.35 (1)	4.30	R-C	-3185 / 0	0.87 (1)
F-G	-3401 / 0	-127.8 -127.8	0.35 (1)	4.30	I-L	-3185 / 0	0.87 (1)
G-H	-3403 / 0	-127.8 -127.8	0.34 (1)	4.30	P-G	-3 / 0	0.00 (1)
H-I	-2884 / 0	-127.8 -127.8	0.35 (1)	3.78	N-H	0 / 1425	0.32 (1)
I-J	0 / 26	-127.8 -127.8	0.23 (1)	10.00	D-P	0 / 1422	0.32 (1)
J-K	0 / 49	-127.8 -127.8	0.17 (1)	10.00	N-G	-840 / 0	0.45 (1)
R-B	-352 / 0	0.0	0.04 (1)	7.81	P-E	-837 / 0	0.45 (1)
L-J	-352 / 0	0.0	0.04 (1)	7.81			
R-Q	0 / 2282	-28.0	-28.0	0.54 (2)	10.00		
Q-P	0 / 2385	-28.0	-28.0	0.57 (2)	10.00		
P-O	0 / 3403	-28.0	-28.0	0.63 (1)	10.00		
O-N	0 / 3403	-28.0	-28.0	0.63 (1)	10.00		
N-M	0 / 2385	-28.0	-28.0	0.57 (2)	10.00		
M-L	0 / 2282	-28.0	-28.0	0.54 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 35.9	PSF
DL = 8.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 61.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.35/1.00 (C-D:1), BC=0.63/1.00 (N-P:1), WB=0.87/1.00 (C-R:1), SS=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

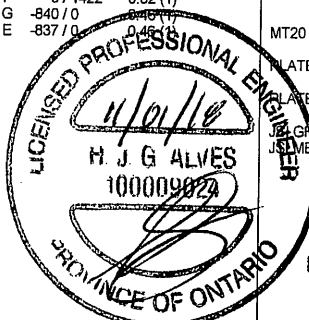
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

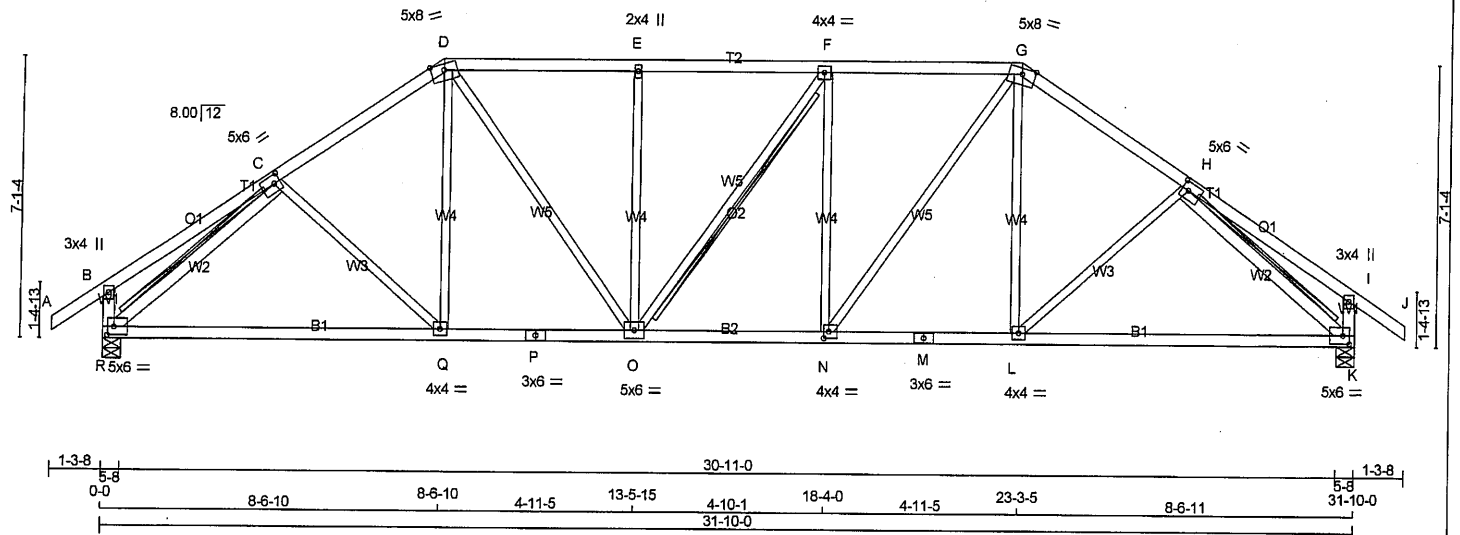
PLATE ROTATION TOL. = 5.0 Deg.

SEA GRIP= 0.90 (D) (INPUT = 0.90)
 JOINT METAL= 0.98 (O) (INPUT = 1.00)



DRWG NO. TAM 17812098
 STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	45246	DRWG NO.
289750	T4	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.210 S May 18 2018 MiTek Industries, Inc. Thu Nov 1 10:48:11 2018 Page 1		
				ID:DvdDM7sBvDfd2U1ZhskATKyRpFFcgAlAXLQn3EPLzGy2OuOFAL5lVvWwWA1Hdc94ryNbvI Scale = 1:55.8		



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	CTMMW+t	MT20	5.0	6.0	2.50 2.00
D	TTVVW-m	MT20	5.0	8.0	Edge
E	TMW+w	MT20	2.0	4.0	
F	CTMMW+t	MT20	4.0	4.0	
G	TTVVW-m	MT20	5.0	8.0	Edge
H	CTMMW+t	MT20	5.0	6.0	2.50 2.00
I	TMV+p	MT20	3.0	4.0	
J	BMVWV-t	MT20	5.0	6.0	2.50 2.00
L	BMWw+t	MT20	4.0	4.0	
M	BS-t	MT20	3.0	6.0	
N	BMWw+t	MT20	4.0	4.0	2.00 1.50
O	BMWwW-t	MT20	5.0	6.0	
P	BS-t	MT20	3.0	6.0	
Q	QBMWw+t	MT20	4.0	4.0	
R	BMVWV-t	MT20	5.0	6.0	2.50 2.00

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2655	0	2655	0	0	5-8	5-8
K	2655	0	2655	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2076	1242 / 0	334 / 0	0 / 0	0 / 0	499 / 0	0 / 0
K	2076	1242 / 0	334 / 0	0 / 0	0 / 0	499 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-R, H-K
2x3 DRY SPF No.2 T-BRACE AT F-O

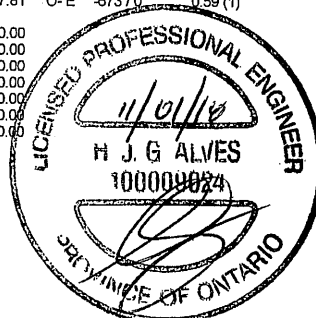
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0/49	-127.8	-127.8	0.17 (1)	10.00	C-Q	-70/120 0.04 (1)
B-C	0/34	-127.8	-127.8	0.36 (1)	10.00	Q-D	0/377 0.09 (3)
C-D	-2810/0	-127.8	-127.8	0.39 (1)	3.81	L-G	0/376 0.09 (3)
D-E	-2874/0	-127.8	-127.8	0.45 (1)	3.69	L-H	-70/120 0.04 (1)
E-F	-2874/0	-127.8	-127.8	0.42 (1)	3.72	R-C	-3203/0 0.65 (1)
F-G	-2876/0	-127.8	-127.8	0.45 (1)	3.69	H-K	-3203/0 0.65 (1)
G-H	-2809/0	-127.8	-127.8	0.39 (1)	3.81	O-F	-4/0 0.00 (1)
H-I	0/34	-127.8	-127.8	0.36 (1)	10.00	N-G	0/963 0.22 (1)
I-J	0/49	-127.8	-127.8	0.17 (1)	10.00	D-O	0/958 0.22 (1)
R-B	-387/0	0.0	0.0	0.04 (1)	7.81	N-F	-674/0 0.59 (1)
K-I	-387/0	0.0	0.0	0.04 (1)	7.81	O-E	-673/0 0.59 (1)
R-Q	0/2358	-28.0	-28.0	0.69 (2)	10.00		
Q-P	0/2310	-28.0	-28.0	0.70 (2)	10.00		
P-O	0/2310	-28.0	-28.0	0.70 (2)	10.00		
O-N	0/2876	-28.0	-28.0	0.55 (1)	10.00		
N-M	0/2310	-28.0	-28.0	0.70 (2)	10.00		
M-L	0/2310	-28.0	-28.0	0.70 (2)	10.00		
L-K	0/2357	-28.0	-28.0	0.69 (2)	10.00		



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	35.9	PSF
		DL =	8.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	61.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 933 (0.41")

CSI: TC=0.45/1.00 (D-E:1), BC=0.70/1.00 (O-Q:2),
WB=0.65/1.00 (C-R:1), SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)

	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

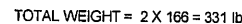
JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.79 (C) (INPUT = 1.00)

DWG NO. TAM 118120901
STRUCTURAL
COMPONENT ONLY

Scale = 1:57.0



DWG NO. TAM *77812100*
STRUCTURAL
COMPONENT ONLY



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT L

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

TOTAL LOAD CASES: (4)

D-R -541/A 0.58(1)
M-I-929/B 0.10(1)

LICENSED PROFESSIONAL ENGINEER

H. J. G. ALVES
100009024

MINISTER OF ONTARIO

SPECIFIED LOADS:

SPECIFIED LOADS:

TOP CH.	LL =	35.9	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		61.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.45/1.00 (H-I:1), BC=0.45/1.00 (N-P:1),
WB=0.58/1.00 (H-P:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION

	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

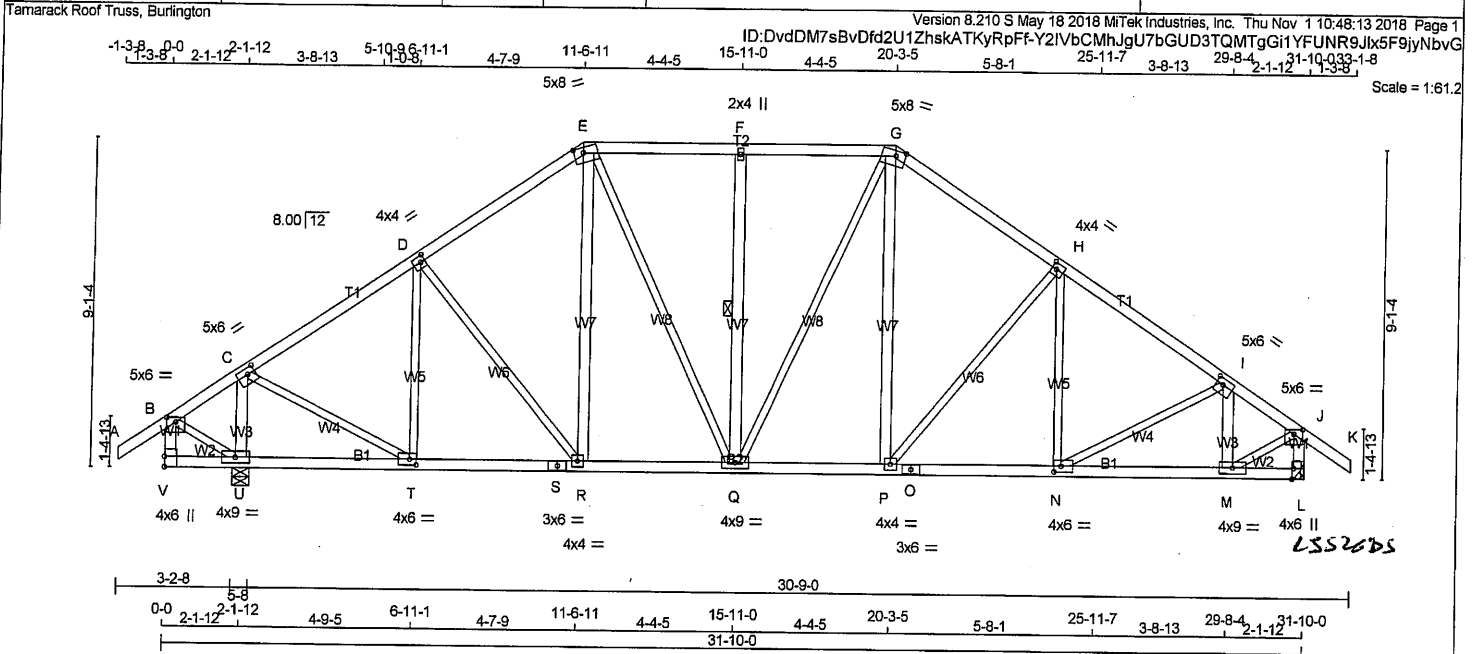
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.66 (O) (INPUT = 1.00)

DWG NO. TAM 71812101
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME T6C	QUANTITY 1	PLY 1	JOB DESC. 45246	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	5.0	6.0	2.00	2.50
D	TMVW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	5.0	8.0	1.75	3.25
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	3.25
H	TMVW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-t	MT20	5.0	6.0	2.00	2.50
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMV-t	MT20	4.0	6.0	Edge	0.50
M	BMVW-t	MT20	4.0	9.0		
N	BMVW-t	MT20	4.0	6.0	2.00	2.25
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	4.0	9.0		
R	BMVW-t	MT20	4.0	4.0		
S	BS-t	MT20	3.0	6.0		
T	BMVW-t	MT20	4.0	6.0	2.00	2.25
U	BMVW-t	MT20	4.0	9.0		
V	BMV-t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQD BRG
L	2463	0	2463	0	0	0
U	2846	0	2846	0	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT L.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS	DEAD	SOIL
L	1926	1152 / 0	310 / 0	0 / 0	0 / 0
U	2226	1332 / 0	358 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO						FR-TO				
A-B	0 / 49	-127.8	-127.8	0.17 (1)	10.00	R-E	0 / 190	0.03 (3)		
B-C	0 / 271	-127.8	-127.8	0.30 (1)	10.00	E-Q	0 / 650	0.15 (1)		
C-D	-1883 / 0	-127.8	-127.8	0.36 (1)	4.53	Q-F	-675 / 0	0.26 (1)		
D-E	-2007 / 0	-127.8	-127.8	0.37 (1)	4.41	Q-G	0 / 223	0.05 (1)		
E-F	-1928 / 0	-127.8	-127.8	0.35 (1)	4.50	P-G	0 / 568	0.09 (1)		
F-G	-1928 / 0	-127.8	-127.8	0.35 (1)	4.50	P-H	-571 / 0	0.62 (1)		
G-H	-2226 / 0	-127.8	-127.8	0.39 (1)	4.20	N-H	-22 / 174	0.04 (3)		
H-I	-2610 / 0	-127.8	-127.8	0.43 (1)	3.90	M-J	0 / 2103	0.47 (1)		
I-J	-2219 / 0	-127.8	-127.8	0.21 (1)	4.41	B-U	-207 / 0	0.03 (1)		
J-K	0 / 49	-127.8	-127.8	0.17 (1)	10.00	U-C	-2609 / 0	0.32 (1)		
V-B	-8 / 12	0.0	0.0	0.00 (3)	7.81	C-T	0 / 1989	0.45 (1)		
L-J	-2441 / 0	0.0	0.0	0.25 (1)	5.48	T-D	-805 / 0	0.46 (1)		
						D-R	0 / 96	0.02 (1)		
						M-I	-846 / 0	0.40 (1)		
V-U	0 / 0	-28.0	-28.0	0.13 (2)	10.00	N-I	0 / 248	0.08 (1)		
U-T	-185 / 0	-28.0	-28.0	0.10 (3)	6.25					
T-S	0 / 1576	-28.0	-28.0	0.32 (1)	10.00					
S-R	0 / 1576	-28.0	-28.0	0.32 (1)	10.00					
R-Q	0 / 1639	-28.0	-28.0	0.33 (1)	10.00					
Q-P	0 / 1829	-28.0	-28.0	0.36 (1)	10.00					
P-O	0 / 2186	-28.0	-28.0	0.42 (1)	10.00					
O-N	0 / 2186	-28.0	-28.0	0.42 (1)	10.00					
N-M	0 / 1878	-28.0	-28.0	0.37 (1)	10.00					
M-L	0 / 0	-28.0	-28.0	0.09 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 35.9	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 61.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.43/1.00 (H-I:1), BC=0.42/1.00 (N-P:1), WB=0.62/1.00 (H-P:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

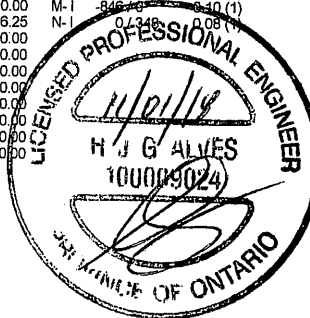
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.60 (C) (INPUT = 1.00)

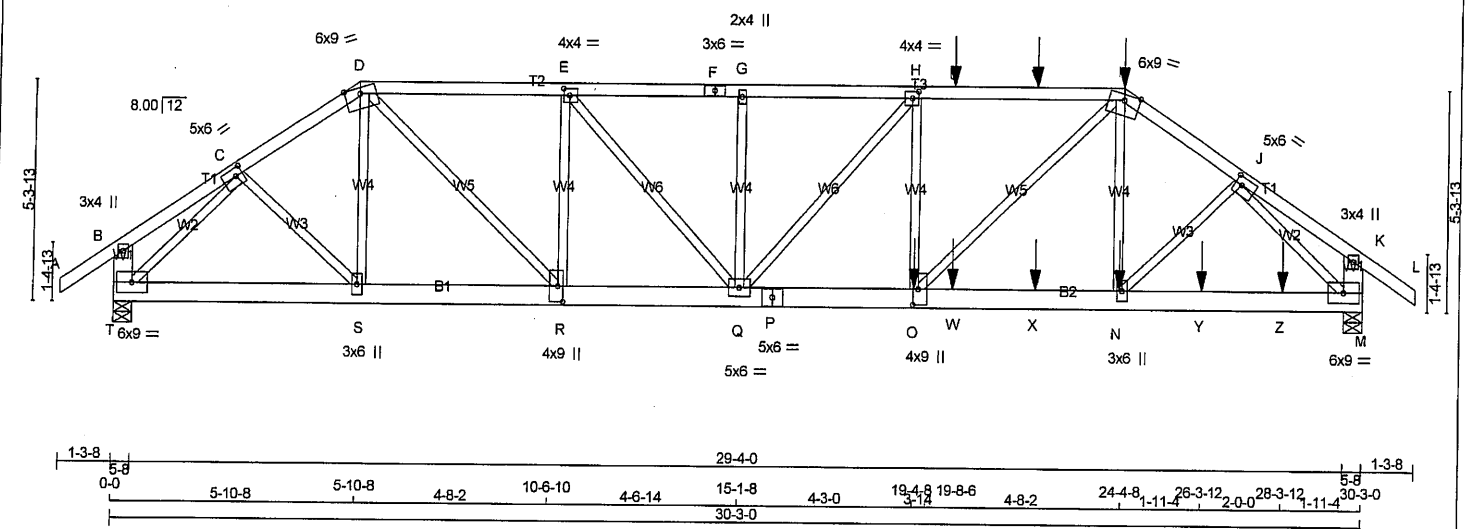
DWG NO. TAM 718/2102
STRUCTURAL
COMPONENT ONLY



JOB NAME 289750	TRUSS NAME T7	QUANTITY 1	PLY 2	JOB DESC. 45246	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 Mittek Industries, Inc. Thu Nov 1 10:48:14 2018 Page 1
ID:DvdDM7sBvDfd2U1ZhskATKyRpFf-1FstoYNJ4_c_CQ3PdAxb?toofyYJ6qlTzbrphAyNbvF
19-8-6 20-3-12 22-3-12 24-4-8 2-10-0 27-2-8 30-3-0 31-6-8
1-3-8 0-0 3-0-8 3-0-8 2-10-0 5-10-8 4-8-2 10-6-10 4-6-14 15-1-8 4-6-14 19-8-6 20-3-12 22-3-12 24-4-8 2-10-0 27-2-8 30-3-0 31-6-8
Scale = 1:53.1



TOTAL WEIGHT = 2 X 150 = 300 lb

LUMBER	SIZE	DRY	No.2	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-F 1	12	TOP
F-I 1	12	SIDE(61.0)
I-L 1	12	SIDE(61.0)
T-B 2	12	TOP
M-K 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
T-P 2	12	TOP
P-M 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
H-O 1	6	SIDE(45.7)
2x3 1	6	SIDE(444.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW-t	MT20	5.0	6.0	2.25	2.00
D TTWW-m	MT20	6.0	9.0	Edge	
E TMWW-t	MT20	4.0	4.0	2.00	1.75
F TS-t	MT20	3.0	6.0		
G TMW+w	MT20	2.0	4.0		
H TMWW-t	MT20	4.0	4.0	2.00	1.75
I TTWW-m	MT20	6.0	9.0	Edge	
J TMWW-t	MT20	5.0	6.0	2.25	2.00
K TMV+p	MT20	3.0	4.0		
M BMVW-t	MT20	6.0	9.0		
N BMWW-t	MT20	3.0	6.0		
O BMWW-t	MT20	4.0	9.0	4.50	1.50
P BS-t	MT20	5.0	6.0		
Q BMWWW-t	MT20	5.0	6.0		
R BMWW-t	MT20	4.0	9.0	4.50	1.50
S BMWW-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
T	3652	0	0	5-8
M	4970	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERMLIVE WIND DEAD SOIL
T	2842	1723 / 0 442 / 0 0 / 0 0 / 0 678 / 0 0 / 0
M	3879	2333 / 0 616 / 0 0 / 0 0 / 0 930 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
FR-TO	(LBS)	TO	FR-TO	(LBS)
A-B	0 / 49	-127.8 -127.8 0.09 (1)	C-S	0 / 550
B-C	0 / 16	-127.8 -127.8 0.07 (1)	S-D	-200 / 87
C-D	-4377 / 0	-127.8 -127.8 0.14 (1)	D-R	0 / 3127
D-E	-5842 / 0	-127.8 -127.8 0.41 (1)	R-E	-2073 / 0
E-F	-7036 / 0	-127.8 -127.8 0.45 (1)	E-Q	0 / 1830
F-G	-7036 / 0	-127.8 -127.8 0.45 (1)	Q-G	-414 / 0
G-H	-7036 / 0	-127.8 -127.8 0.54 (1)	Q-H	-1118 / 0
H-U	-7764 / 0	-127.8 -127.8 0.69 (1)	U-H	-66 / 167
U-V	-7764 / 0	-127.8 -127.8 0.69 (1)	Q-I	0 / 3664
V-I	-7764 / 0	-127.8 -127.8 0.69 (1)	N-I	-295 / 231
I-J	-6230 / 0	-127.8 -127.8 0.22 (1)	N-J	0 / 924
J-K	-2 / 12	-127.8 -127.8 0.08 (1)	T-C	-4576 / 0
K-L	0 / 49	-127.8 -127.8 0.09 (1)	J-M	-6380 / 0
T-B	-334 / 0	0.0 0.0 0.01 (1)		
M-K	-340 / 0	0.0 0.0 0.01 (1)		

T-S	0 / 3224	-28.0	-28.0	0.24 (1)	10.00
S-R	0 / 3616	-28.0	-28.0	0.26 (1)	10.00
R-Q	0 / 5842	-28.0	-28.0	0.42 (1)	10.00
Q-P	0 / 7764	-28.0	-28.0	0.57 (1)	10.00
P-O	0 / 7764	-28.0	-28.0	0.57 (1)	10.00
O-W	0 / 5155	-28.0	-28.0	0.41 (1)	10.00
W-X	0 / 5155	-28.0	-28.0	0.41 (1)	10.00
X-N	0 / 5155	-28.0	-28.0	0.41 (1)	10.00
N-Y	0 / 4497	-28.0	-28.0	0.34 (1)	10.00
Y-Z	0 / 4497	-28.0	-28.0	0.34 (1)	10.00
Z-M	0 / 4497	-28.0	-28.0	0.34 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TOTAL
I	24-4-8	-451	-451		BACK	VERT	TOTAL
N	24-3-12	-51	-51		BACK	VERT	TOTAL
O	19-4-8	-1844	-1844		BACK	VERT	TOTAL
U	20-3-12	-106	-106		BACK	VERT	TOTAL
V	22-3-12	-106	-106		BACK	VERT	TOTAL
W	20-3-12	-51	-51		BACK	VERT	TOTAL
X	22-3-12	-51	-51		BACK	VERT	TOTAL
Y	26-3-12	-51	-51		BACK	VERT	TOTAL
Z	28-3-12	-51	-54		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 35.9 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.01")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL) = L/360 (1.01")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.69/1.00 (H-I:1), BC=0.57/1.00 (O-Q:1),
WB=0.88/1.00 (J-M:1), SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.88 (J) (INPUT = 0.90)
SI METAL= 0.81 (J) (INPUT = 1.00)

DRWG NO. TAM: 118/2103
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME 289750	TRUSS NAME T7	QUANTITY 1	PLY 2	JOB DESC. 45246 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Thu Nov 1 10:48:14 2018 Page 2
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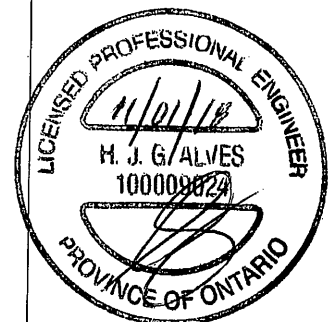
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMVW1-t	MT20	6.0	9.0		

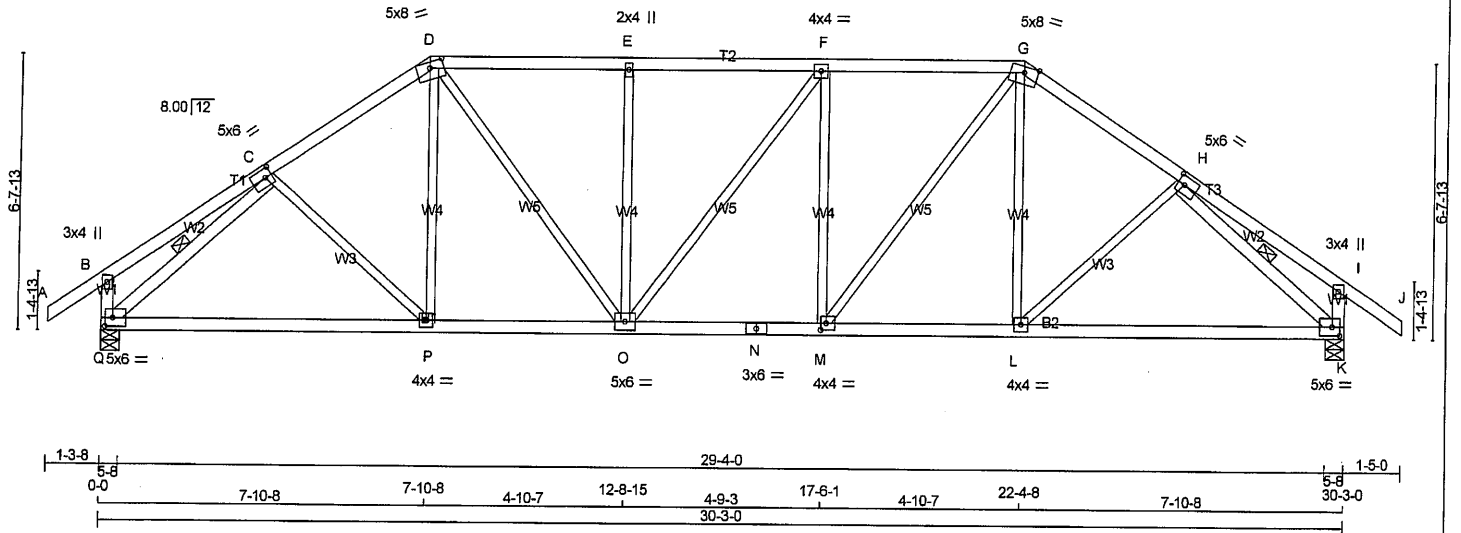
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 153.8 lbs. FACTORED DOWN AT 20-3-12, AND 153.8 lbs. FACTORED DOWN AT 22-3-12, AND 643.0 lbs. FACTORED DOWN AT 24-4-8 ON TOP CHORD, AND 2405.5 lbs. FACTORED DOWN AT 19-4-8, 71.7 lbs. FACTORED DOWN AT 20-3-12, 71.7 lbs. FACTORED DOWN AT 22-3-12, 71.7 lbs. FACTORED DOWN AT 24-3-12, AND 71.7 lbs. FACTORED DOWN AT 26-3-12, AND 75.8 lbs. FACTORED DOWN AT 28-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM **118/2103**
STRUCTURAL
COMPONENT ONLY **9/12**



TOTAL WEIGHT = 137 lb [M/F]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
Q - C	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.00
D	TTVW-m	MT20	5.0	8.0	1.75	4.00
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTVW-m	MT20	5.0	8.0	1.75	4.00
H	TMVW-t	MT20	5.0	6.0	2.50	2.00
I	TMV+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.25
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
Q	2531	0	2531	0	0	5-8	5-8	5-8	5-8
K	2547	0	2547	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1979	1185 / 0	318 / 0	0 / 0	0 / 0	478 / 0	0 / 0
K	1990	1194 / 0	318 / 0	0 / 0	0 / 0	478 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

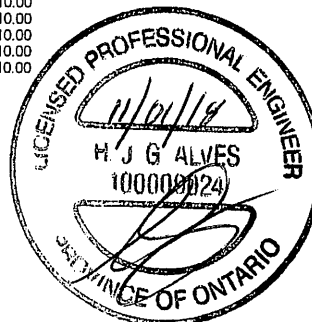
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. UNBRAC	MAX. CS (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO						
A-B	0 / 49	-127.8	-127.8	0.17 (1)	10.00	C-P	-16 / 131	0.03 (3)			
B-C	0 / 31	-127.8	-127.8	0.30 (1)	10.00	P-D	0 / 326	0.08 (3)			
C-D	-2661 / 0	-127.8	-127.8	0.32 (1)	3.97	L-G	0 / 326	0.08 (3)			
D-E	-2779 / 0	-127.8	-127.8	0.43 (1)	3.77	L-H	-16 / 131	0.03 (3)			
E-F	-2779 / 0	-127.8	-127.8	0.40 (1)	3.80	Q-C	-3017 / 0	0.48 (1)			
F-G	-2781 / 0	-127.8	-127.8	0.43 (1)	3.76	H-K	-3017 / 0	0.48 (1)			
G-H	-2661 / 0	-127.8	-127.8	0.32 (1)	3.97	M-G	0 / 973	0.22 (1)			
H-I	0 / 31	-127.8	-127.8	0.30 (1)	10.00	D-O	0 / 969	0.22 (1)			
I-J	0 / 53	-127.8	-127.8	0.20 (1)	10.00	M-F	-664 / 0	0.49 (1)			
Q-B	-371 / 0	0.0	0.0	0.04 (1)	7.81	O-E	-663 / 0	0.48 (1)			
K-I	-387 / 0	0.0	0.0	0.04 (1)	7.81	O-F	-4 / 0	0.01 (1)			
Q-P	0 / 2197	-28.0	-28.0	0.61 (2)	10.00						
P-O	0 / 2189	-28.0	-28.0	0.62 (2)	10.00						
O-N	0 / 2781	-28.0	-28.0	0.53 (1)	10.00						
N-M	0 / 2781	-28.0	-28.0	0.53 (1)	10.00						
M-L	0 / 2189	-28.0	-28.0	0.62 (2)	10.00						
L-K	0 / 2196	-28.0	-28.0	0.61 (2)	10.00						



DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 35.9 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 61.4 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.01")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
 ALLOWABLE DEFL.(TL) = L/360 (1.01")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.43/1.00 (D-E:1), BC=0.62/1.00 (O-P:2), WB=0.49/1.00 (F-M:1), SS=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

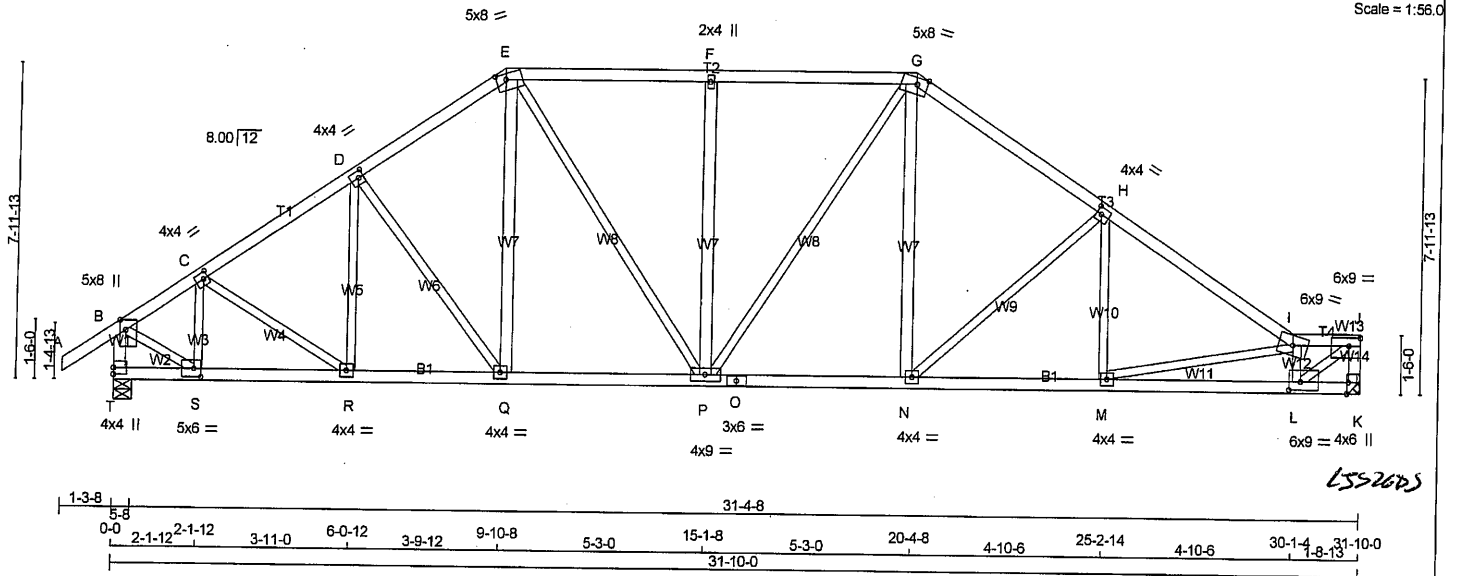
JSI GRIP= 0.89 (O) (INPUT = 0.90)
 JSI METAL= 0.80 (N) (INPUT = 1.00)

DWG NO. TAM 11812104
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 289750	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. 45246	DRWG NO.
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Tamarack Roof Truss, Burlington.

Version 8.210 S May 18 2018 M/Tek Industries, Inc. Thu Nov 1 10:48:15 2018 Page 1
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1-3-8 0-0 2-1-12 2-10-12 5-0-8 0-12 3-9-12 9-10-8 5-3-0 15-1-8 5-3-0 20-4-8 4-10-6 25-2-14 30-1-4 31-10-0
Scale = 1:56.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
T - B	2x4	DRY	No.2	SPF	
T - O	2x4	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
Q - E	2x4	DRY	No.2	SPF	
P - F	2x4	DRY	No.2	SPF	
N - G	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW+p	MT20	5.0	8.0	Edge	1.75
C, D, H					
C TMWV-t	MT20	4.0	4.0	2.00	1.50
E TTWW-m	MT20	5.0	8.0	1.75	3.00
F TMW+w	MT20	2.0	4.0		
G TTWW-m	MT20	5.0	8.0	2.00	3.25
I TTWW-m	MT20	6.0	9.0		
J TMWV-t	MT20	6.0	9.0	2.50	Edge
K BMV1+t	MT20	4.0	6.0	Edge	0.50
L BMWV-t	MT20	6.0	9.0	2.50	3.50
M, N, Q, R					
M BMWV-t	MT20	4.0	4.0		
O BS-t	MT20	3.0	6.0		
P BMWVW-t	MT20	4.0	9.0		
S BMWV-t	MT20	5.0	6.0	2.50	2.25
T BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
K	2479 0	2479 0	0	MECHANICAL
T	2655 0	2655 0	0	5-8 5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
K	COMBINED	SNOW	LIVE				
T	1955	1144 / 0	334 / 0	0 / 0	0 / 0	478 / 0	0 / 0
T	2076	1242 / 0	334 / 0	0 / 0	0 / 0	499 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	MAX	WEBS	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 49	-127.8	-127.8	0.17 (1)	10.00	Q-E	0 / 417
B-C	-2400 / 0	-127.8	-127.8	0.16 (1)	4.32	E-P	0 / 731
C-D	-2892 / 0	-127.8	-127.8	0.32 (1)	3.84	P-F	-816 / 0
D-E	-2677 / 0	-127.8	-127.8	0.30 (1)	3.99	P-G	0 / 431
E-F	-2620 / 0	-127.8	-127.8	0.54 (1)	3.70	N-G	0 / 727
F-G	-2620 / 0	-127.8	-127.8	0.54 (1)	3.70	N-H	-885 / 0
G-H	-2885 / 0	-127.8	-127.8	0.51 (1)	3.63	M-H	0 / 329
H-I	-3592 / 0	-127.8	-127.8	0.59 (1)	3.20	M-I	-329 / 0
I-J	-3145 / 0	-127.8	-127.8	0.15 (1)	3.85	L-I	-2314 / 0
K-J	-2419 / 0	0.0	0.0	0.25 (1)	5.50	L-J	0 / 3917
T-B	-2628 / 0	0.0	0.0	0.27 (1)	5.30	B-S	0 / 2258
						S-C	-930 / 0
T-S	0 / 0	-28.0	-28.0	0.06 (3)	10.00	C-R	0 / 477
S-R	0 / 2017	-28.0	-28.0	0.38 (1)	10.00	R-D	-152 / 77
R-Q	0 / 2418	-28.0	-28.0	0.46 (1)	10.00	D-Q	-357 / 0
Q-P	0 / 2208	-28.0	-28.0	0.43 (1)	10.00		
P-O	0 / 2377	-28.0	-28.0	0.46 (1)	10.00		
O-N	0 / 2377	-28.0	-28.0	0.46 (1)	10.00		
N-M	0 / 3027	-28.0	-28.0	0.57 (1)	10.00		
M-L	0 / 3346	-28.0	-28.0	0.67 (1)	10.00		
L-K	0 / 0	-28.0	-28.0	0.08 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 35.9 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.4 PSF

SPACING = 24.0 IN/C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.59/1.00 (H-I-1), BC=0.67/1.00 (L-M-1), WB=0.88/1.00 (J-L-1), SI=0.32/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.89 (J) (INPUT = 0.90)
ISI METAL= 0.60 (O) (INPUT = 1.00)

DWG NO. TAM 11812105
STRUCTURAL
COMPONENT ONLY

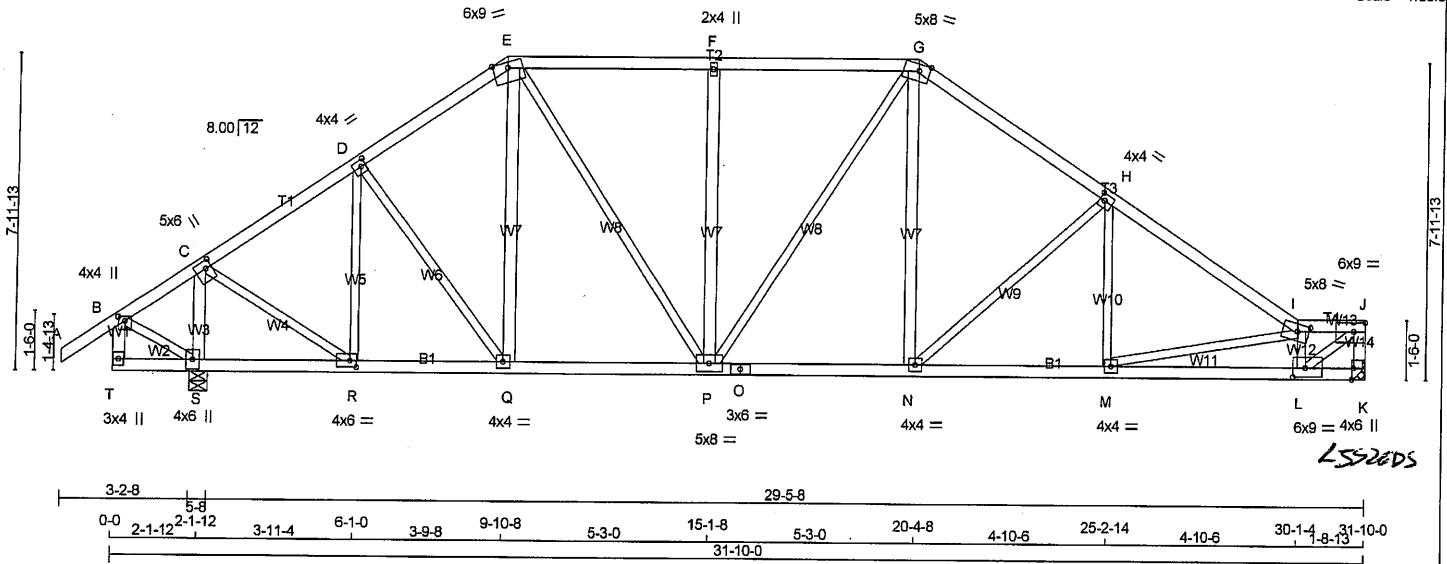
JOB NAME 289750	TRUSS NAME T9C	QUANTITY 7	PLY 1	JOB DESC. 45246	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Thu Nov 1 10:48:16 2018 Page 1

ID:DvdDM7sBvDfd2U1ZhskATKyRpFfzd_eDEPZcbSiSkDokbz35luAKmD?ZktnRvKwm2yNbvd

1-3-8 0-0 2-1-12 2-10-12 5-0-8 5-1-0 3-9-8 9-10-8 5-3-0 15-1-8 5-3-0 20-4-8 4-10-6 25-2-14 4-10-6 30-1-4 31-10-0 1-8-13
Scale = 1:55.8



TOTAL WEIGHT = 7 X 152 = 1066 lb
[MIF]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - J	2x4	DRY	No.2
K - J	2x4	DRY	No.2
T - B	2x4	DRY	No.2
T - O	2x4	DRY	No.2
O - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
Q - E	2x4	DRY	No.2
P - F	2x4	DRY	No.2
N - G	2x4	DRY	No.2
S - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.25	2.00
C	TMWW+u	MT20	5.0	6.0	2.25	1.75
D	TMWW+u	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	6.0	9.0	Edge	
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.00	3.25
H	TMWW+u	MT20	4.0	4.0	2.00	1.50
I	TTWW-m	MT20	5.0	8.0	2.25	3.50
J	TMWW+u	MT20	6.0	9.0	2.75	Edge
K	BMV+u	MT20	4.0	6.0	Edge	0.50
L	BMWW+u	MT20	6.0	9.0	2.75	3.75
M, N, Q						
M	BMWW+u	MT20	4.0	4.0		
O	BS+u	MT20	3.0	6.0		
P	BMWW+u	MT20	5.0	8.0		
R	BMWW+u	MT20	4.0	6.0	2.00	2.00
S	BMWW+u	MT20	4.0	6.0		
T	BMV+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
K	2287	0	2287	0
S	2846	0	2846	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
K	1805	1054 / 0	310 / 0
S	2226	1332 / 0	358 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

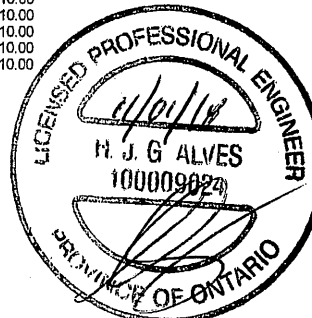
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO			
A-B	0 / 49	-127.8	-127.8 0.17 (1)	Q-E	-135 / 103	10.00	0.12 (1)
B-C	0 / 285	-127.8	-127.8 0.24 (1)	E-P	0 / 964	10.00	0.22 (1)
C-D	-1739 / 0	-127.8	-127.8 0.24 (1)	P-F	-819 / 0	4.83	0.71 (1)
D-E	-2029 / 0	-127.8	-127.8 0.25 (1)	F-G	0 / 200	4.53	0.05 (1)
E-F	-2203 / 0	-127.8	-127.8 0.51 (1)	G-H	0 / 727	4.03	0.12 (1)
F-G	-2203 / 0	-127.8	-127.8 0.51 (1)	H-I	-886 / 0	4.03	0.72 (1)
G-H	-2540 / 0	-127.8	-127.8 0.48 (1)	I-J	0 / 330	3.86	0.08 (3)
H-I	-3249 / 0	-127.8	-127.8 0.55 (1)	J-K	-343 / 0	3.40	0.15 (1)
I-J	-2890 / 0	-127.8	-127.8 0.13 (1)	K-L	-2117 / 0	4.01	0.30 (1)
J-K	-2232 / 0	0.0	0.0 0.23 (1)	L-M	0 / 3599	5.69	0.81 (1)
K-L	-2232 / 0	0.0	0.0 0.00 (3)	M-N	-231 / 0	7.81	0.04 (1)
L-M	-4 / 20	0.0	0.0 0.00 (3)	N-O	-2607 / 0	6.25	0.32 (1)
M-N	0 / 0	-28.0	-28.0 0.11 (2)	O-P	0 / 1968	10.00	0.44 (1)
N-O	-206 / 0	-28.0	-28.0 0.08 (2)	P-Q	-974 / 0	6.25	0.44 (1)
O-P	0 / 1453	-28.0	-28.0 0.31 (1)	Q-R	0 / 344	10.00	0.08 (1)
P-Q	0 / 1659	-28.0	-28.0 0.34 (1)	R-S			
Q-R	0 / 2090	-28.0	-28.0 0.41 (1)	S-T			
R-S	0 / 2090	-28.0	-28.0 0.41 (1)	T-U			
S-T	0 / 2741	-28.0	-28.0 0.52 (1)	U-V			
T-U	0 / 3074	-28.0	-28.0 0.62 (1)	V-W			
U-V	0 / 0	-28.0	-28.0 0.07 (1)	W-X			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 35.9 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.21")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/120 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.55/1.00 (H-I:1), BC=0.62/1.00 (L-M:1), WB=0.81/1.00 (J-L:1), SS=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.63 (I) (INPUT = 1.00)

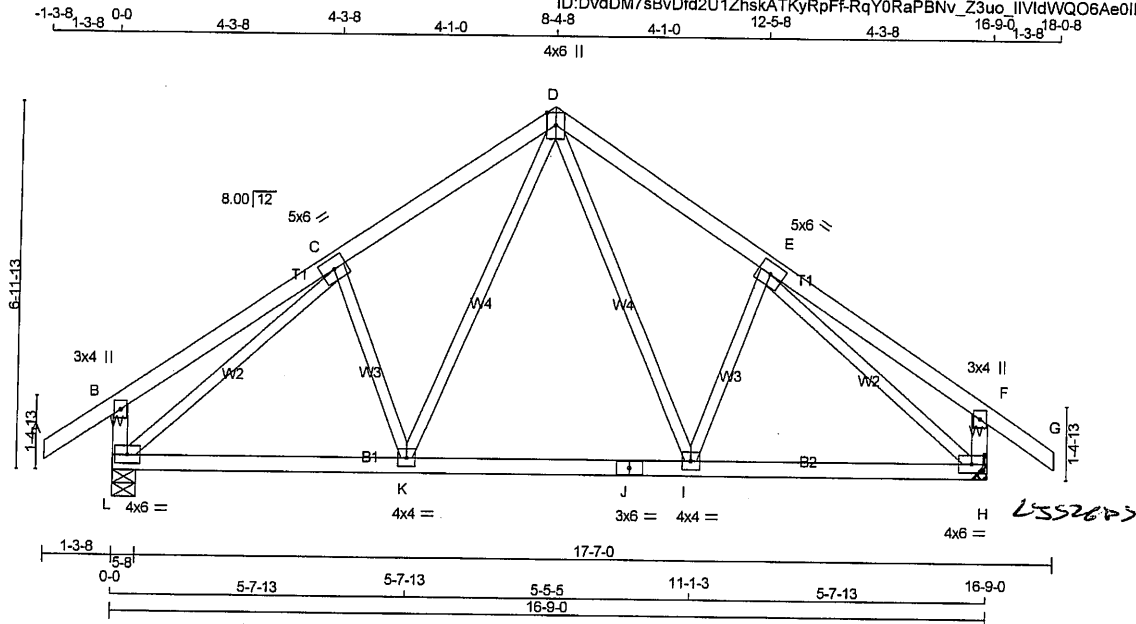
DWG NO. TAM 71812106
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME T10	QUANTITY 2	PLY 1	JOB DESC. 45246	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: DvdDM7sBvDfd2U1ZhskATKyRpFFRqYORaPBNV_Z3uo_IIVidWQO6Ae0IB7vgZ3TIUyNbvC

Scale = 1:42.0



TOTAL WEIGHT = 2 X 74 = 149 lb [M][F]

LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+I	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMVW+I	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW+I	MT20	4.0	6.0		
I	BMVW+I	MT20	4.0	4.0		
J	BS-I	MT20	3.0	6.0		
K	BMVW+I	MT20	4.0	4.0		
L	BMVW+I	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
L	1480	0	1480	0
H	1480	0	1480	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT H.

UNFACTORED REACTIONS

	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
L	1149	700/0	176/0	0/0
H	1149	700/0	176/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	CSI (LC)
FR-TO							FR-TO				
A-B	0/49	-127.8	-127.8	0.17 (1)	10.00		D-I	0/499	0.11 (1)		
B-C	0/34	-127.8	-127.8	0.36 (1)	10.00		I-E	-314/18	0.09 (1)		
C-D	-1241/0	-127.8	-127.8	0.29 (1)	5.43		K-D	0/499	0.11 (1)		
D-E	-1241/0	-127.8	-127.8	0.29 (1)	5.43		C-K	-314/18	0.09 (1)		
E-F	0/34	-127.8	-127.8	0.36 (1)	10.00		L-C	-1523/0	0.81 (1)		
F-G	0/49	-127.8	-127.8	0.17 (1)	10.00		E-H	-1523/0	0.81 (1)		
L-B	-379/0	0.0	0.0	0.04 (1)	7.81						
H-F	-379/0	0.0	0.0	0.04 (1)	7.81						
L-K	0/1118	-28.0	-28.0	0.31 (2)	10.00						
K-J	0/816	-28.0	-28.0	0.29 (3)	10.00						
J-I	0/816	-28.0	-28.0	0.29 (3)	10.00						
I-H	0/1118	-28.0	-28.0	0.31 (2)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 35.9	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD	=	61.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.56")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.05")
ALLOWABLE DEFL.(TL) = $L/360$ (0.56")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.08")

CSI: TC=0.36/1.00 (B-C:1), BC=0.31/1.00 (K-L:2), WB=0.81/1.00 (C-L:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)



DRWG NO. TAM 17412/107
STRUCTURAL
COMPONENT ONLY

Version 9.010.0-M-18-0010-NIT 1A 1-11-18 7:46 AM

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ID:DvdDM7sBvDfd2U1ZhskATKyRpFf-nWpUvTGfCDTGd2tqbnHU8_xO1KcpVwF8v9igtBvNbvC

Scale = 1:42 000



JSI GRIP= 0.46 (O) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)



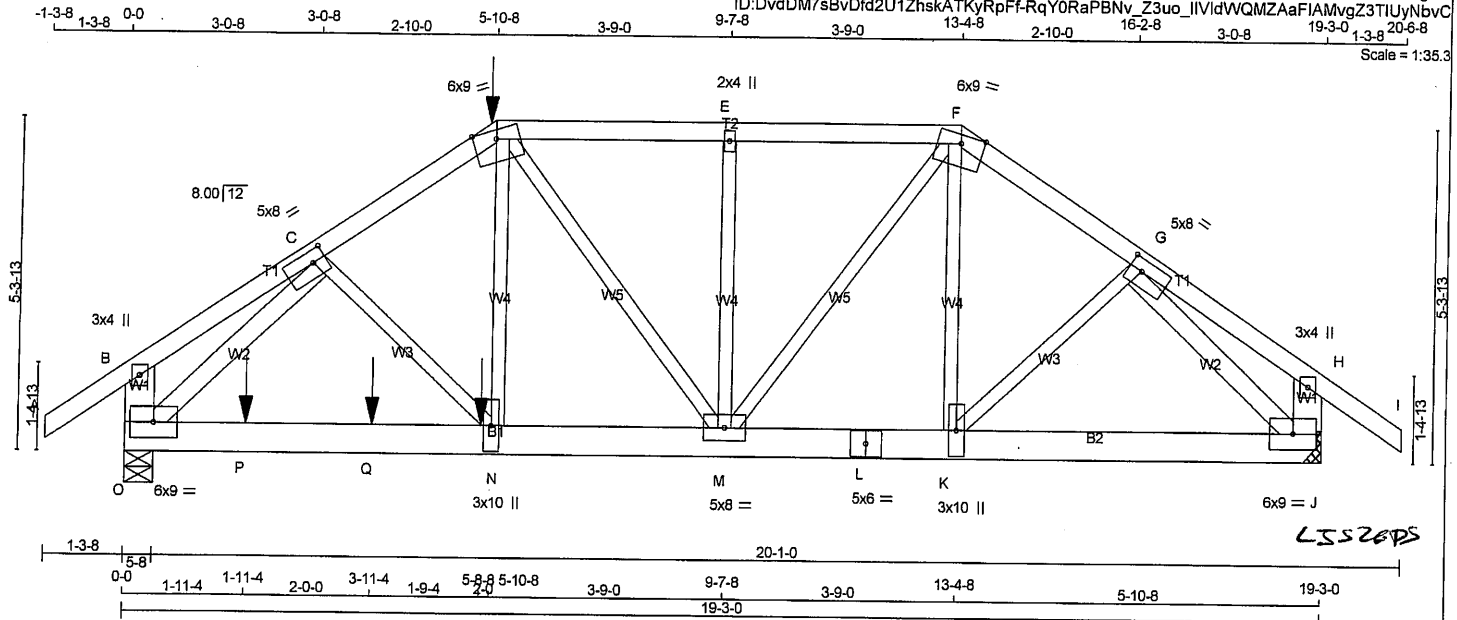
JOB NAME 289750	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. 45246	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Thu Nov 1 10:48:17 2018 Page 1

ID:DvdDM7sBvDfd2U1ZhskATKyRpFfRqYORaPBNv_Z3uo_IIVIdWQMZAaFIAMvgZ3TIUyNbvC

Scale = 1:35.3



LS526DS

TOTAL WEIGHT = 101 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
O - B	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
O - C	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+t	MT20	5.0	8.0	2.25	2.50
D	TTWW-m	MT20	6.0	9.0	Edge	
E	TMVW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	6.0	9.0	Edge	
G	TMVW+t	MT20	5.0	8.0	2.25	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	6.0	9.0		
K	BMVW+t	MT20	3.0	10.0		
L	BS-t	MT20	5.0	6.0		
M	BMVW+t	MT20	5.0	8.0		
N	BMVW+t	MT20	3.0	10.0		
O	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 479.8 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD, AND 75.8 lbs FACTORED DOWN AT 1-11-4, AND 71.7 lbs FACTORED DOWN AT 3-11-4, AND 2133.4 lbs FACTORED DOWN AT 5-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UP/LT	IN-SX
O	3546	0	3546	0
J	2496	0	2496	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	2746	1690 / 0	411 / 0	0 / 0	0 / 0	645 / 0	0 / 0
J	1927	1194 / 0	281 / 0	0 / 0	0 / 0	451 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 49	-127.8 -127.8	0.19 (1)	10.00	C-N	0 / 506	0.13 (1)
B-C	0 / 16	-127.8 -127.8	0.15 (1)	10.00	N-D	0 / 1950	0.48 (1)
C-D	-4145 / 0	-127.8 -127.8	0.40 (1)	3.11	D-M	-611 / 0	0.40 (1)
D-E	-3099 / 0	-127.8 -127.8	0.46 (1)	3.47	M-E	-561 / 0	0.23 (1)
E-F	-3099 / 0	-127.8 -127.8	0.46 (1)	3.47	M-F	0 / 1425	0.35 (1)
F-G	-2714 / 0	-127.8 -127.8	0.27 (1)	3.93	K-F	0 / 183	0.05 (3)
G-H	0 / 21	-127.8 -127.8	0.17 (1)	10.00	K-G	0 / 212	0.05 (1)
H-I	0 / 49	-127.8 -127.8	0.19 (1)	10.00	O-C	-4348 / 0	0.86 (1)
O-B	-334 / 0	0.0 0.0	0.02 (1)	7.81	G-J	-2957 / 0	0.58 (1)
J-H	-325 / 0	0.0 0.0	0.02 (1)	7.81			
P-Q	0 / 3064	-28.0 -28.0	0.55 (1)	10.00			
P-Q	0 / 3064	-28.0 -28.0	0.55 (1)	10.00			
Q-N	0 / 3064	-28.0 -28.0	0.55 (1)	10.00			
N-M	0 / 3469	-28.0 -28.0	0.49 (1)	10.00			
M-L	0 / 2236	-28.0 -28.0	0.34 (1)	10.00			
L-K	0 / 2236	-28.0 -28.0	0.34 (1)	10.00			
K-J	0 / 2083	-28.0 -28.0	0.34 (1)	10.00			

PROFESSION

11/10/12

H. J. G. AL

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1 MAX-	MAX+	FACE	DIR.
D	5-10-8	-338	-338	---	BACK VERT
N	5-8-8	-1626	-1626	---	BACK VERT
P	1-11-4	-51	-51	---	BACK VERT
Q	3-11-4	-51	-51	---	BACK VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 35.9	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD	=	61.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")
CSI: TC=0.46/1.00 (E-F:1), BC=0.55/1.00 (N-O:1), WB=0.86/1.00 (C-O:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
M 20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

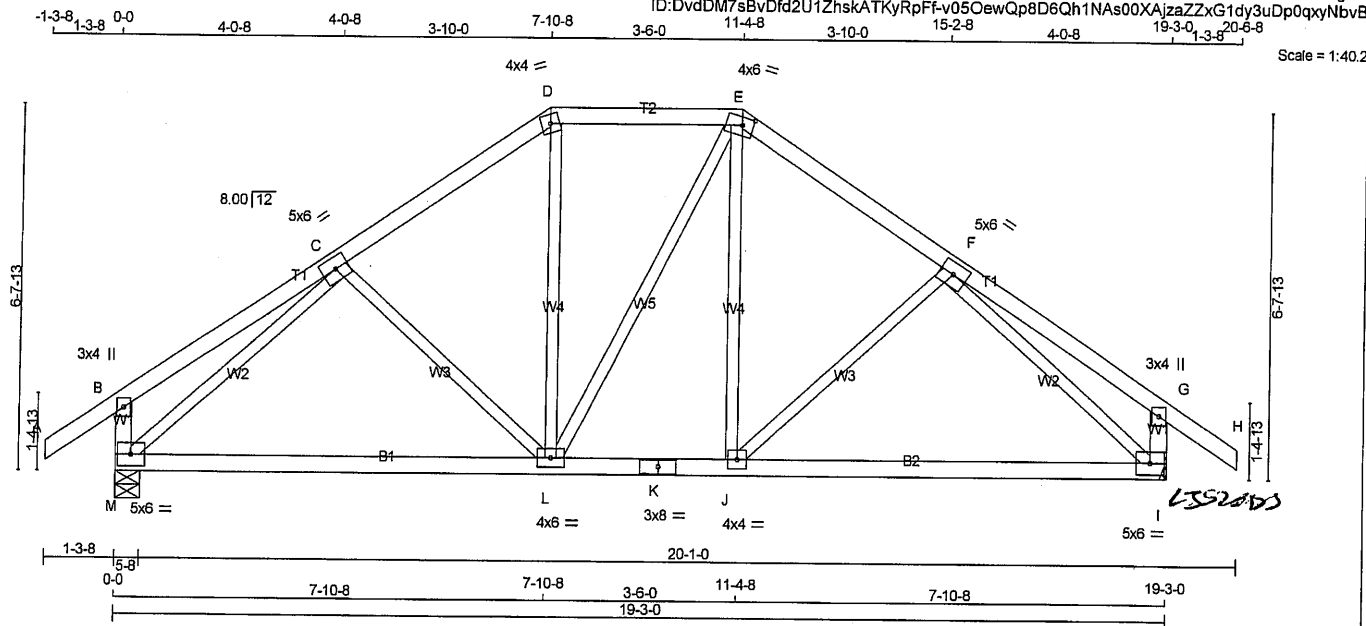
JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.82 (C) (INPUT = 1.00)

DWG NO. TAM T1812108
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: DvdDM7sBvDfd2U1ZhskATKyRpFf-v05OewQp8D6Qh1NAs00XAJzaZZxG1dy3uDP0qxyNbvB



TOTAL WEIGHT = 86 lb
[M][F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWV-m	MT20	4.0	6.0	1.75	2.50
F	TMVW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
M	1675	0	1675	0
I	1675	0	1675	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT I.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	1303	790 / 0	202 / 0	0 / 0	0 / 0	311 / 0	0 / 0	0 / 0
I	1303	790 / 0	202 / 0	0 / 0	0 / 0	311 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO				FR-TO			
A-B	0 / 49	-127.8	-127.8	0.17 (1)	10.00	C-L	-230 / 35
B-C	0 / 32	-127.8	-127.8	0.31 (1)	10.00	L-D	0 / 336
C-D	-1377 / 0	-127.8	-127.8	0.26 (1)	5.26	L-E	0 / 1
D-E	-1122 / 0	-127.8	-127.8	0.21 (1)	5.74	J-E	0 / 335
E-F	-1376 / 0	-127.8	-127.8	0.26 (1)	5.26	J-F	-230 / 35
F-G	0 / 32	-127.8	-127.8	0.31 (1)	10.00	M-C	-1767 / 0
G-H	0 / 49	-127.8	-127.8	0.17 (1)	10.00	F-I	-1766 / 0
M-B	-369 / 0	0.0	0.0	0.04 (1)	7.81		
I-G	-369 / 0	0.0	0.0	0.04 (1)	7.81		
M-L	0 / 1287	-28.0	-28.0	0.50 (2)	10.00		
L-K	0 / 1122	-28.0	-28.0	0.47 (3)	10.00		
K-J	0 / 1122	-28.0	-28.0	0.47 (3)	10.00		
J-I	0 / 1286	-28.0	-28.0	0.50 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 35.9	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 61.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 60.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.64")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.18")
ALLOWABLE DEFL. (TL) = $L/360$ (0.64")
CALCULATED VERT. DEFL. (TL) = $L/785$ (0.29")

CSI: TC=0.31/1.00 (F-G:1), BC=0.50/1.00 (I-J:2),
WB=0.84/1.00 (C-M:1), SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (E) (INPUT = 0.90)
JSI METAL= 0.63 (K) (INPUT = 1.00)



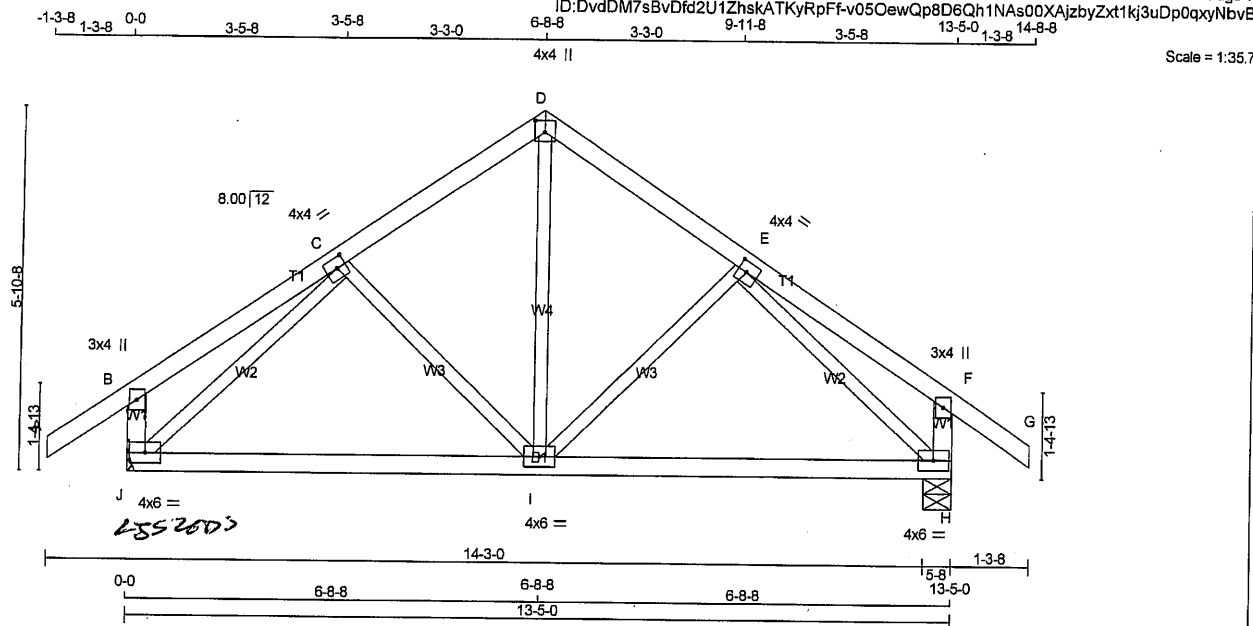
DRWG NO. TAM 71812/109
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME T13	QUANTITY 3	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: DvdDM7sBvDfd2U1ZhsKATKyRpF-v05OewQp8D6Qh1NAs00XajzbyZxt1kj3uDp0qxyNbvB



LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - G	2x4 DRY	No.2	SPF
J - B	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWV-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMVWV-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMVWV-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
J	1220	0	1220	0	0	5-8	5-8		
H	1220	0	1220	0	0	5-8	5-8		

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
J	945	581/0	141/0	0/0	223/0	0/0
H	945	581/0	141/0	0/0	223/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	MAX. (LC)
FR-TO		FROM TO		FR-TO			
A-B	0/49	-127.8 -127.8	0.17 (1)	I-D	0/542	0.12 (1)	
B-C	0/27	-127.8 -127.8	0.22 (1)	I-E	-222/19	0.08 (1)	
C-D	-817/0	-127.8 -127.8	0.18 (1)	C-I	-222/19	0.08 (1)	
D-E	-817/0	-127.8 -127.8	0.18 (1)	J-C	-1148/0	0.40 (1)	
E-F	0/27	-127.8 -127.8	0.22 (1)	E-H	-1148/0	0.40 (1)	
F-G	0/49	-127.8 -127.8	0.17 (1)				
J-B	-341/0	0.0 0.0	0.04 (1)				
H-F	-341/0	0.0 0.0	0.04 (1)				
J-I	0/816	-28.0 -28.0	0.47 (3)				
I-H	0/816	-28.0 -28.0	0.47 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 35.9 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 61.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.45")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.45")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.11")

CSI: TC=0.22/1.00 (E-F:1), BC=0.47/1.00 (H-I:3),
WB=0.40/1.00 (E-H:1), SSI=0.18/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

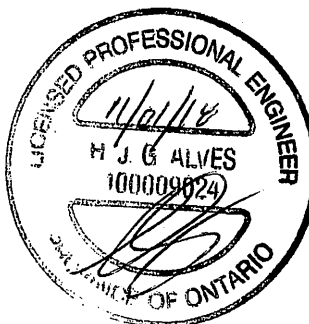
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

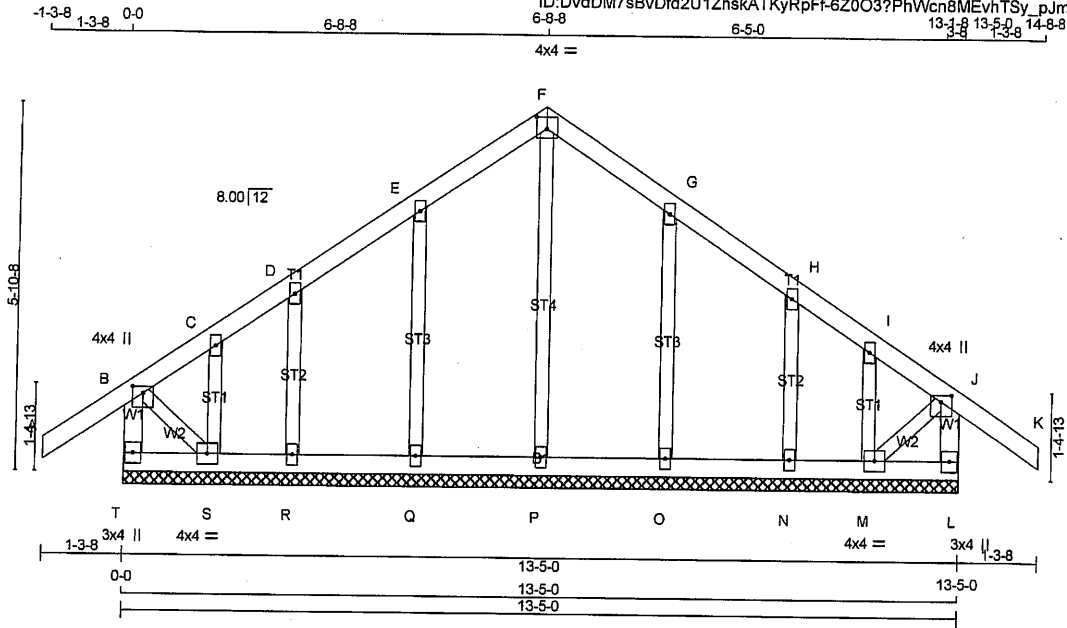


DRWG NO. TAM 11012110
STRUCTURAL
COMPONENT ONLY

DWG NO. TAM 7312111
STRUCTURAL
COMPONENT ONLY

JOB NAME 289751	TRUSS NAME G13	QUANTITY 1	PLY 1	JOB DESC. 45246	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: DvdDM7sBvDfd2U1ZhskATKyRpFf-6Z0O3?PhWcn8MEvhTSy_pJmeNg04T_?BckYiaQyNbsd
13-1-8 13-5-0 14-8-8
Scale = 1:35.3



TOTAL WEIGHT = 60 lb
[M]

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER			
CHORDS					
T - B	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
F - K	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT 2-0-0 OC.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO		FR-TO			
T-B	-387 / 0	0.0 0.0	0.04 (1)	P-F	-200 / 0	0.10 (1)	
A-B	0 / 49	-127.8 -127.8	0.17 (1)	Q-E	-296 / 0	0.09 (1)	
B-C	-79 / 0	-127.8 -127.8	0.16 (1)	R-D	-222 / 0	0.04 (1)	
C-D	-9 / 0	-127.8 -127.8	0.04 (1)	S-C	-45 / 0	0.01 (1)	
D-E	-11 / 0	-127.8 -127.8	0.08 (1)	O-G	-296 / 0	0.09 (1)	
E-F	-26 / 0	-127.8 -127.8	0.08 (1)	N-H	-222 / 0	0.04 (1)	
F-G	-11 / 0	-127.8 -127.8	0.08 (1)	M-I	-45 / 0	0.01 (1)	
G-H	-11 / 0	-127.8 -127.8	0.08 (1)	B-S	0 / 28	0.01 (1)	
H-I	-9 / 0	-127.8 -127.8	0.04 (1)	M-J	0 / 28	0.01 (1)	
I-J	-79 / 0	-127.8 -127.8	0.16 (1)				
J-K	0 / 49	-127.8 -127.8	0.17 (1)				
L-J	-387 / 0	0.0 0.0	0.04 (1)				
T-S	0 / 0	-28.0 -28.0	0.01 (3)				
S-R	0 / 21	-28.0 -28.0	0.02 (3)				
R-Q	0 / 15	-28.0 -28.0	0.03 (3)				
Q-P	0 / 9	-28.0 -28.0	0.03 (3)				
P-O	0 / 9	-28.0 -28.0	0.03 (3)				
O-N	0 / 15	-28.0 -28.0	0.03 (3)				
N-M	0 / 21	-28.0 -28.0	0.02 (3)				
M-L	0 / 0	-28.0 -28.0	0.01 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 35.9 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (J-K:1), BC=0.03/1.00 (N-O:3), WB=0.10/1.00 (F-P:1), SS=0.12/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

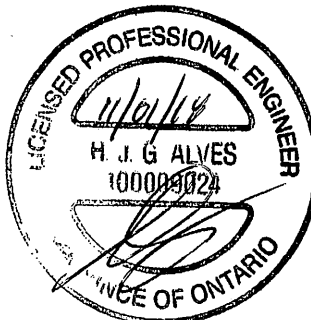
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (E) (INPUT = 1.00)

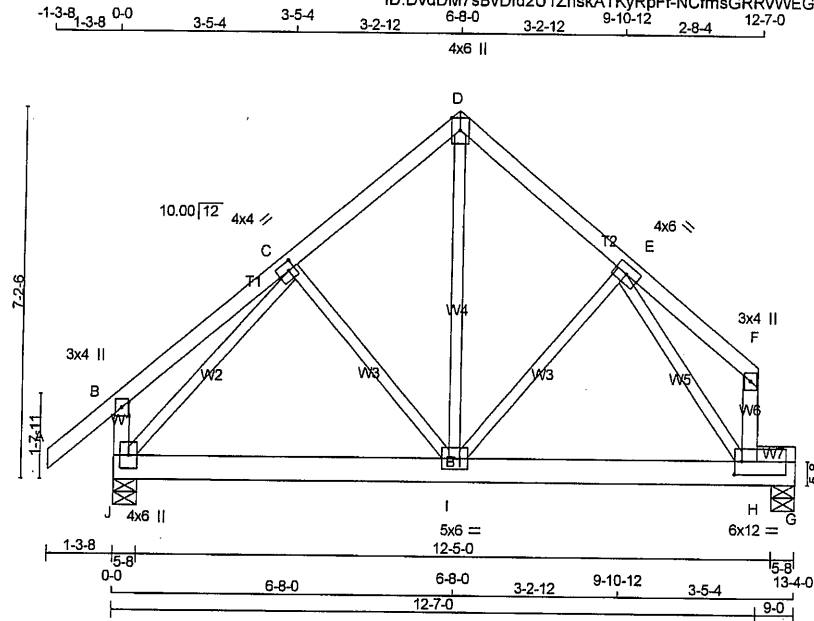


DRWG NO. TAM 176/2115
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME T14A	QUANTITY 3	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

ID: DvdDM7sBvDfd2U1ZhskATKyRpFF-NCfmsGRRvWEGJByMQjXmixWmYzG0mBHC7tYaMNYNbva
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Scale = 1:42.9

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
J - B	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		
J - G	2x6 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	6.0	12.0	3.00	1.75
I	BMVW-t	MT20	5.0	6.0		
J	BMVW-t+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
G	945	0	945	0
J	1213	0	1213	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN	COMPONENT REACTIONS					
G	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
G	755	427 / 0	140 / 0	0 / 0	0 / 0	188 / 0	0 / 0	
J	939	577 / 0	140 / 0	0 / 0	0 / 0	222 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX. FACTORED	MEMB.	MAX. FACTORED	MAX. FACTORED
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 57	-127.8	-127.8	0.18 (1)	10.00	C-I	-208 / 13
B-C	0 / 31	-127.8	-127.8	0.22 (1)	10.00	I-D	0 / 569
C-D	-743 / 0	-127.8	-127.8	0.23 (1)	6.25	J-C	-1066 / 0
D-E	-737 / 0	-127.8	-127.8	0.19 (1)	6.25	I-E	-41 / 71
E-F	0 / 33	-127.8	-127.8	0.18 (1)	10.00	E-H	-1061 / 0
J-B	-339 / 0	0.0	0.0	0.04 (1)	7.81		
H-F	-112 / 0	0.0	0.0	0.01 (1)	7.81		
J-I	0 / 681	-28.0	-28.0	0.27 (2)	10.00		
I-H	0 / 574	-28.0	-28.0	0.54 (1)	10.00		
H-G	0 / 0	-28.0	-28.0	0.29 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 35.9	PSF
	DL = 8.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 61.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 60.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.23/1.00 (C-D:1), BC=0.54/1.00 (H-I:1), VB=0.45/1.00 (C-J:1), SSI=0.35/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

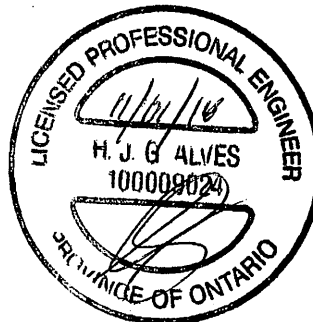
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

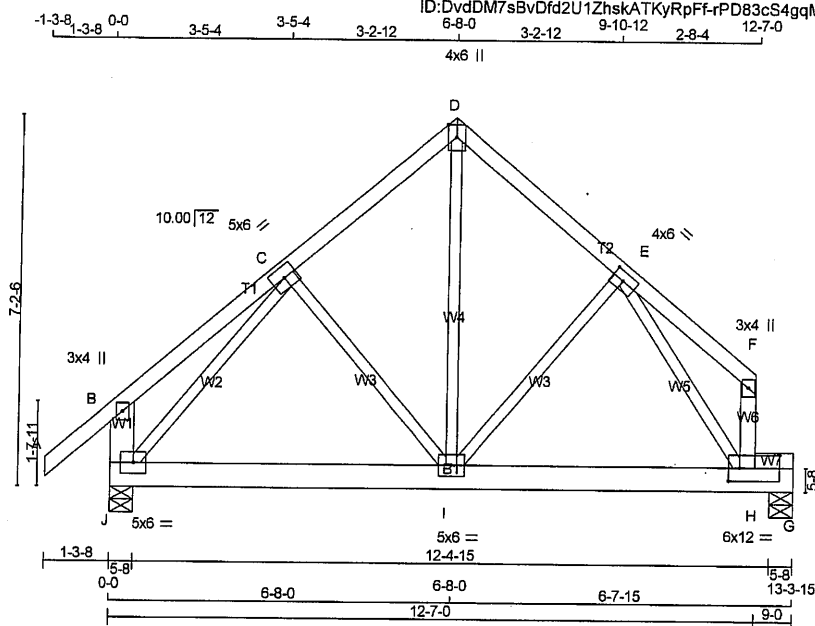


DWG NO. TAM **TB12112**
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME T15A	QUANTITY 1	PLY 2	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Thu Nov 1 10:48:20 2018 Page 1
ID: DvdDM7sBvDfd2U1ZhsKATKyRpFf-rPD83cS4gqM7wLXZzR2?F82vANWVaVbjLMX17vpyNbv9



Scale = 1:42.9

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1 12		SIDE(88.7)
D-F 1 12		SIDE(88.7)
H-F 1 12		TOP
J-B 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J-G 2 12		SIDE(49.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		
H-G 1 6		SIDE(49.0)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMWW-t	MT20	4.0	6.0	2.00 2.75
F	TMV+p	MT20	3.0	4.0	
H	BMVWW-t	MT20	6.0	12.0	3.00 2.75
I	BMVWW-t	MT20	5.0	6.0	
J	BMVW-t	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	2691	0	2691	0
J	3440	0	3440	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	2148	1282 / 0	420 / 0	0 / 0	0 / 0	446 / 0	0 / 0	0 / 0
J	2657	1733 / 0	420 / 0	0 / 0	0 / 0	505 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.58 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			MEMB.		
A-B	0 / 159	-358.8	-358.8	0.27 (1)	10.00	C-I	-579 / 87
B-C	0 / 88	-358.8	-358.8	0.35 (1)	10.00	I-D	0 / 1640
C-D	-2114 / 0	-358.8	-358.8	0.37 (1)	5.58	I-E	-114 / 227
D-E	-2097 / 0	-358.8	-358.8	0.30 (1)	5.72	J-C	-3021 / 0
E-F	0 / 90	-358.8	-358.8	0.28 (1)	10.00	E-H	-3002 / 0
J-B	-953 / 0	0.0	0.0	0.04 (1)	7.81		
H-F	-325 / 0	0.0	0.0	0.02 (1)	7.81		
J-I	0 / 1931	-84.0	-84.0	0.43 (2)	10.00		
I-H	0 / 1631	-84.0	-84.0	0.83 (1)	10.00		
H-G	0 / 0	-84.0	-84.0	0.45 (1)	10.00		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 35.9 PSF
	DL = 8.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 61.4 PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.44")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.44")
CALCULATED VERT. DEFL.(TL)= L/833 (0.19")

CSI: TC=0.37/1.00 (C-D:1), BC=0.83/1.00 (H-I:1), WB=0.63/1.00 (C-J:1), SSI=0.55/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

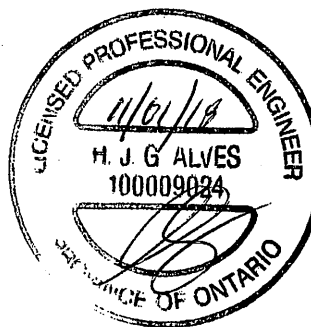
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

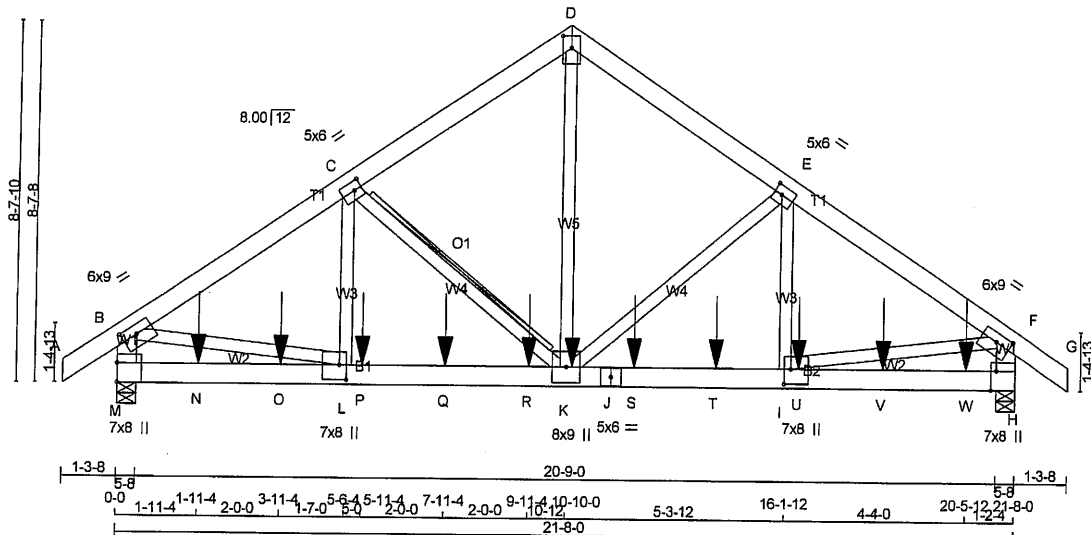
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.37 (C) (INPUT = 1.00)

DWG NO. TAM 11612113
STRUCTURAL
COMPONENT ONLY





TOTAL WEIGHT = 3 X 139 = 417 lb

LUMBER	SIZE	DRY	No.2	DESCR.
CHORDS	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-D 2 12		TOP
D-G 2 12		TOP
M-B 2 12		TOP
H-F 2 12		TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
M-J 2 6		SIDE (689.6)
J-H 2 6		SIDE (692.3)
WEBS: (0.122"x3") SPIRAL NAILS		
2x4 1 6		
E-I 1 5		
C-L 1 5		

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	6.0	9.0	2.50	4.25
C TMWW-t	MT20	5.0	6.0	2.50	2.25
D TTW-p	MT20	5.0	6.0	3.50	2.50
E TMVW-t	MT20	5.0	6.0	2.50	2.25
F TMVW-t	MT20	6.0	9.0	2.50	4.25
H BMV1+t	MT20	7.0	8.0	Edge 1.50	
I BMWW-t	MT20	7.0	8.0	4.25	2.00
J BS-t	MT20	5.0	6.0		
K BMWW-t	MT20	8.0	9.0		
L BMWW-t	MT20	7.0	8.0	4.25	2.00
M BMV1+t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
M	14677	0	14677	0
H	15016	0	15016	0

UNFACTORED REACTIONS

JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	11449	6895	0	1811	0	0/0	2743	0
H	11746	7021	0	1897	0	0/0	2828	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.45 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT C-K

FASTEN T AND L-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
A-B	0/50	-127.8	-127.8 0.03 (1)	K-D	0/13826	0.74 (1)
B-C	-17705/0	-127.8	-127.8 0.32 (1)	L-E	-4657/0	0.97 (1)
C-D	-13074/0	-127.8	-127.8 0.21 (1)	M-F	0/4698	0.25 (1)
D-E	-13075/0	-127.8	-127.8 0.21 (1)	N-G	-5084/0	0.31 (1)
E-F	-17315/0	-127.8	-127.8 0.31 (1)	O-H	0/5196	0.28 (1)
F-G	0/50	-127.8	-127.8 0.03 (1)	P-I	0/14924	0.80 (1)
G-H	-12747/0	0.0	0.0 0.27 (1)	Q-J	0/14596	0.78 (1)
H-I	-12478/0	0.0	0.0 0.27 (1)			
M-N	0/0	-28.0	-28.0 0.45 (1)			
N-O	0/0	-28.0	-28.0 0.45 (1)			
O-L	0/0	-28.0	-28.0 0.45 (1)			
L-P	0/14731	-28.0	-28.0 0.64 (1)			
P-Q	0/14731	-28.0	-28.0 0.64 (1)			
Q-R	0/14731	-28.0	-28.0 0.64 (1)			
R-K	0/14731	-28.0	-28.0 0.64 (1)			
K-J	0/14407	-28.0	-28.0 0.60 (1)			
J-S	0/14407	-28.0	-28.0 0.60 (1)			
S-T	0/14407	-28.0	-28.0 0.60 (1)			
T-I	0/14407	-28.0	-28.0 0.60 (1)			
I-U	0/0	-28.0	-28.0 0.37 (1)			
U-V	0/0	-28.0	-28.0 0.37 (1)			
V-W	0/0	-28.0	-28.0 0.37 (1)			
W-H	0/0	-28.0	-28.0 0.37 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE
K	10-11-12	-1770	-1770	---	BACK	VERT
N	1-11-4	-2041	-2041	---	BACK	VERT
O	3-11-4	-2041	-2041	---	BACK	VERT
P	5-11-4	-1891	-1891	---	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.9	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.72")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
 ALLOWABLE DEFL.(TL) = L/360 (0.72")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.32/1.00 (B-C:1), BC=0.64/1.00 (K-L:1), WB=0.97/1.00 (E-K:1), SSI=0.92/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	822
	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
 JSI METAL= 1.00 (F) (INPUT = 1.00)

DWG NO. TAM 17612 114
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 289750	TRUSS NAME T16	QUANTITY 1	PLY 3	JOB DESC. 45246 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2626.6 lbs FACTORED DOWN AT 1-11-4, 2626.6 lbs FACTORED DOWN AT 3-11-4, 2434.7 lbs FACTORED DOWN AT 5-11-4, 2259.4 lbs FACTORED DOWN AT 7-11-4, 2259.4 lbs FACTORED DOWN AT 9-11-4, 2259.4 lbs FACTORED DOWN AT 10-11-12, 2259.4 lbs FACTORED DOWN AT 12-5-12, 2259.4 lbs FACTORED DOWN AT 14-5-12, 2259.4 lbs FACTORED DOWN AT 16-5-12, AND 2259.4 lbs FACTORED DOWN AT 18-5-12, AND 2451.3 lbs FACTORED DOWN AT 20-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	7-11-4	-1770	-1770	—	BACK	VERT	TOTAL
R	9-11-4	-1770	-1770	—	BACK	VERT	TOTAL
S	12-5-12	-1770	-1770	—	BACK	VERT	TOTAL
T	14-5-12	-1770	-1770	—	BACK	VERT	TOTAL
U	16-5-12	-1770	-1770	—	BACK	VERT	TOTAL
V	18-5-12	-1770	-1770	—	BACK	VERT	TOTAL
W	20-5-12	-1920	-1920	—	BACK	VERT	TOTAL

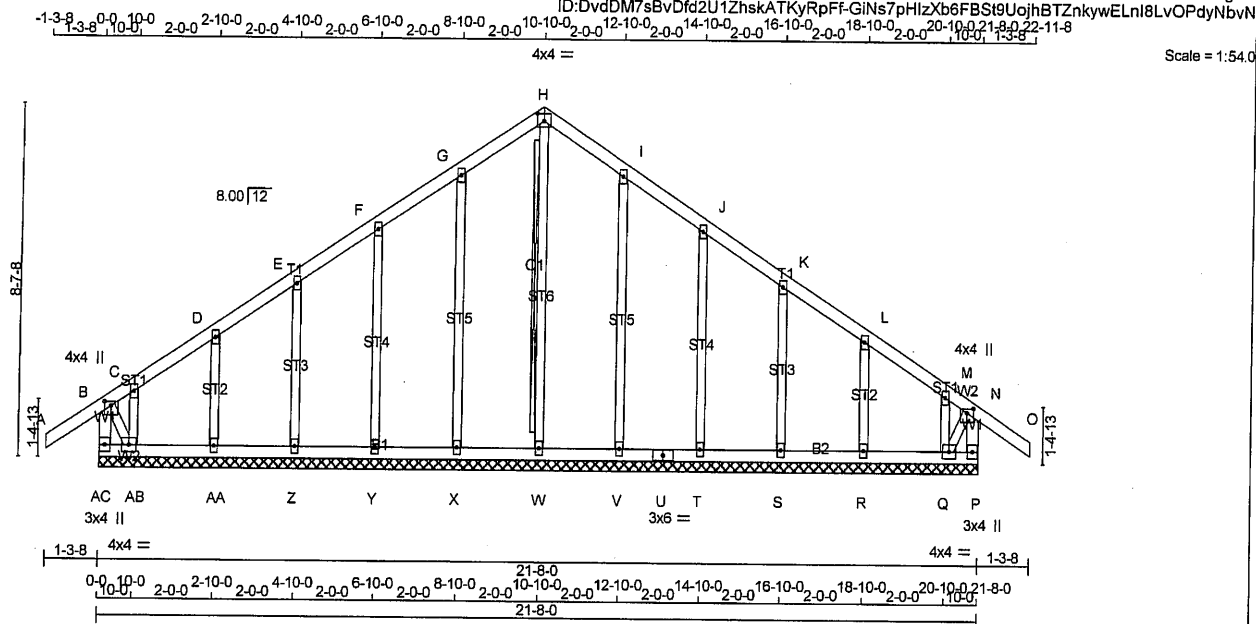


DWG NO. TAM **11/01/14**
STRUCTURAL
COMPONENT ONLY **2/2**

JOB NAME 289750	TRUSS NAME G16	QUANTITY 1	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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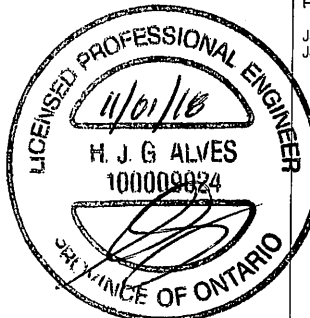
Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MiTek Industries, Inc. Thu Nov 1 10:48:06 2018 Page 1
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Scale = 1:54.0

LUMBER N. L. G. A. RULES CHORDS SIZE A - H 2x4 DRY No.2 H - O 2x4 DRY No.2 AC- B 2x4 DRY No.2 P - N 2x4 DRY No.2 AC- U 2x4 DRY No.2 U - P 2x4 DRY No.2 ALL WEBS 2x3 DRY No.2 ALL GABLE WEBS 2x3 DRY No.2 DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2-0-0 OC.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 35.9 PSF DL = 8.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 61.4 PSF SPACING = 24.0 IN./C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS OVERHANG NOT TO BE ALTERED OR CUT OFF. (55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD CS1: TC=0.17/1.00 (N-O:1), BC=0.03/1.00 (Q-R:3), WB=0.27/1.00 (I-V:1), SSI=0.12/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.50 (AB) (INPUT = 0.90) JSI METAL= 0.07 (G) (INPUT = 1.00)				PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMWV+p MT20 4.0 4.0 1.25 2.00 C, D, E, F, G, I, J, K, L, M C TMWV+w MT20 2.0 4.0 H TTW+p MT20 4.0 4.0 2.25 2.00 N TMWV+p MT20 4.0 4.0 1.25 2.00 P BMV1+p MT20 3.0 4.0 Q BMWV1-t MT20 4.0 4.0 R, S, T, V, W, X, Y, Z, AA R BMV1+w MT20 2.0 4.0 U BS-t MT20 3.0 6.0 AB BMWV1-t MT20 4.0 4.0 AC BMV1+p MT20 3.0 4.0				LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) WEBS MAX. FACTORED FORCE (LBS) MEMB. MAX. FACTORED FORCE (LBS) FR-TO MAX. FACTORED FORCE (LBS) FROM MAX. FACTORED FORCE (LBS) TO MAX. FACTORED FORCE (LBS)				BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 2x3 DRY SPF No.2 T-BRACE AT H-W FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW				DESIGN CRITERIA TOTAL WEIGHT = 102 lb [M]			
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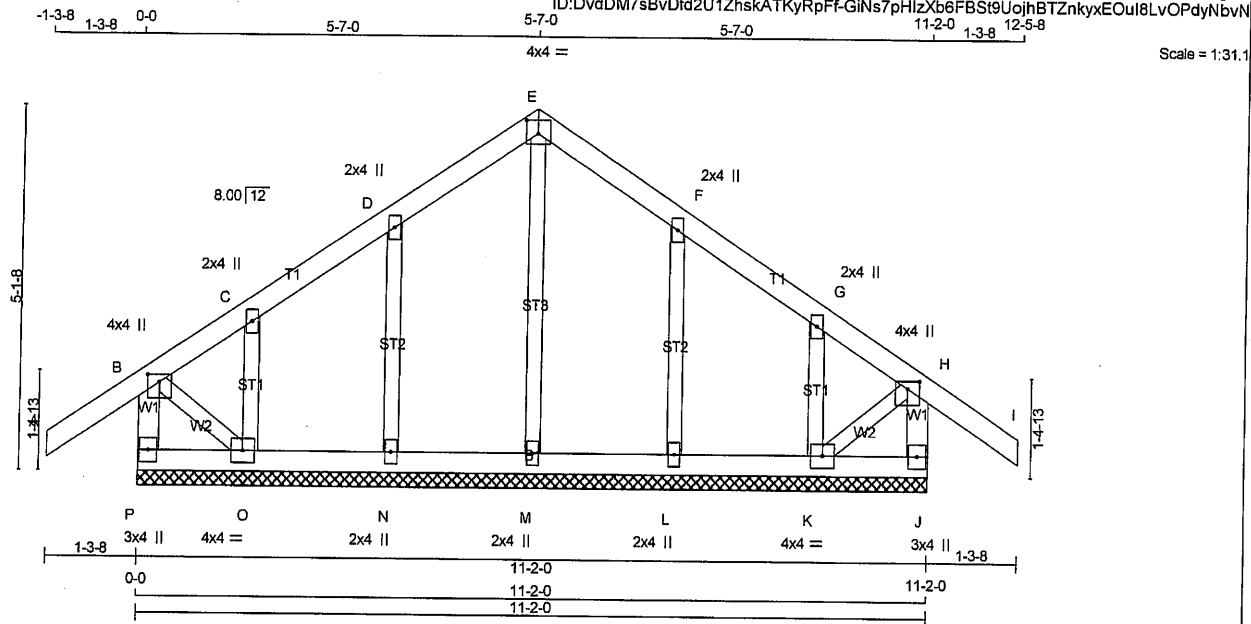


DWG NO. TAM 1712084
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME G17	QUANTITY 1	PLY 1	JOB DESC. 45246	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:DvdDM7sBvDfd2U1ZnskATKyRpFf-GiNs7pHlzXb6FBSt9UojhBTZnkyxEOu8LvOPdyNbvN



TOTAL WEIGHT = 49 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-370 / 0	0.0	0.04 (1)	M-E	-197 / 0	0.07 (1)	
A-B	0 / 49	-127.8	-127.8 0.17 (1)	N-D	-310 / 0	0.07 (1)	
B-C	-64 / 0	-127.8	-127.8 0.16 (1)	O-C	-124 / 0	0.02 (1)	
C-D	-4 / 0	-127.8	-127.8 0.08 (1)	L-F	-310 / 0	0.07 (1)	
D-E	-25 / 0	-127.8	-127.8 0.08 (1)	K-G	-124 / 0	0.02 (1)	
E-F	-25 / 0	-127.8	-127.8 0.08 (1)	B-O	0 / 24	0.01 (1)	
F-G	-4 / 0	-127.8	-127.8 0.08 (1)	K-H	0 / 24	0.01 (1)	
G-H	-64 / 0	-127.8	-127.8 0.16 (1)				
H-I	0 / 49	-127.8	-127.8 0.17 (1)				
J-H	-370 / 0	0.0	0.04 (1)				
P-O	0 / 0	-28.0	-28.0 0.02 (3)				
O-N	0 / 15	-28.0	-28.0 0.03 (3)				
N-M	0 / 7	-28.0	-28.0 0.03 (3)				
M-L	0 / 7	-28.0	-28.0 0.03 (3)				
L-K	0 / 15	-28.0	-28.0 0.03 (3)				
K-J	0 / 0	-28.0	-28.0 0.02 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 35.9	PSF
DL = 8.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 61.4	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (H-I:1), BC=0.03/1.00 (K-L:3), VB=0.07/1.00 (E-M:1), SSI=0.11/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)

JSI METAL= 0.08 (D) (INPUT = 1.00)



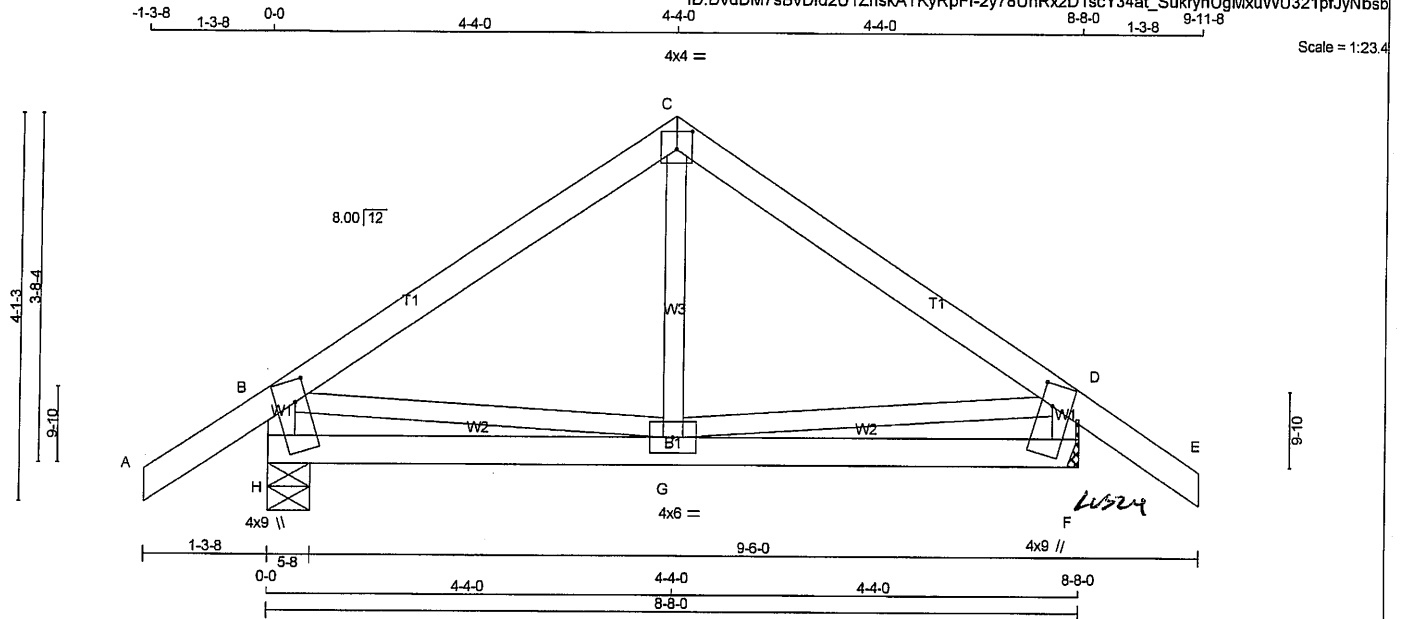
DWG NO. TAM 11812090
STRUCTURAL
COMPONENT ONLY

JOB NAME 289751	TRUSS NAME T22	QUANTITY 3	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:DvdDM7sBvDfd2U1ZhskATKyRpPf-2y78UhRxD21scY34at_SukryhUgMxuVU321pfJyNbsb



TOTAL WEIGHT = 3 X 35 = 106 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, D, F, H						
B						
C	TTW-p	MT20	4.0	4.0	2.25	2.00
F	TMBMVW1**m	MT20	4.0	9.0	2.75	1.50
G	BMVWW-l	MT20	4.0	6.0		
H	TMBMVW1**m	MT20	4.0	9.0	2.75	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	850	0	850	0	5-8	5-8
F	850	0	850	0	MECHANICAL	- 2x52

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT F.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
H	653	410 / 0	91 / 0	0 / 0	0 / 0	152 / 0	0 / 0
F	653	410 / 0	91 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 49	-127.8	-127.8	0.17 (1)	10.00	G-C	0 / 213 0.05 (3)	
B-C	-542 / 0	-127.8	-127.8	0.31 (1)	6.25	B-G	0 / 454 0.10 (1)	
C-D	-542 / 0	-127.8	-127.8	0.31 (1)	6.25	G-D	0 / 454 0.10 (1)	
D-E	0 / 49	-127.8	-127.8	0.17 (1)	10.00			
H-B	-803 / 0	0.0	0.0	0.08 (1)	7.81			
F-D	-803 / 0	0.0	0.0	0.08 (1)	7.81			
H-G	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
G-F	0 / 0	-28.0	-28.0	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.9	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.29")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.29")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CSI: TC=0.31/1.00 (C-D:1), BC=0.17/1.00 (G-H:3),
WB=0.10/1.00 (B-G:1), SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

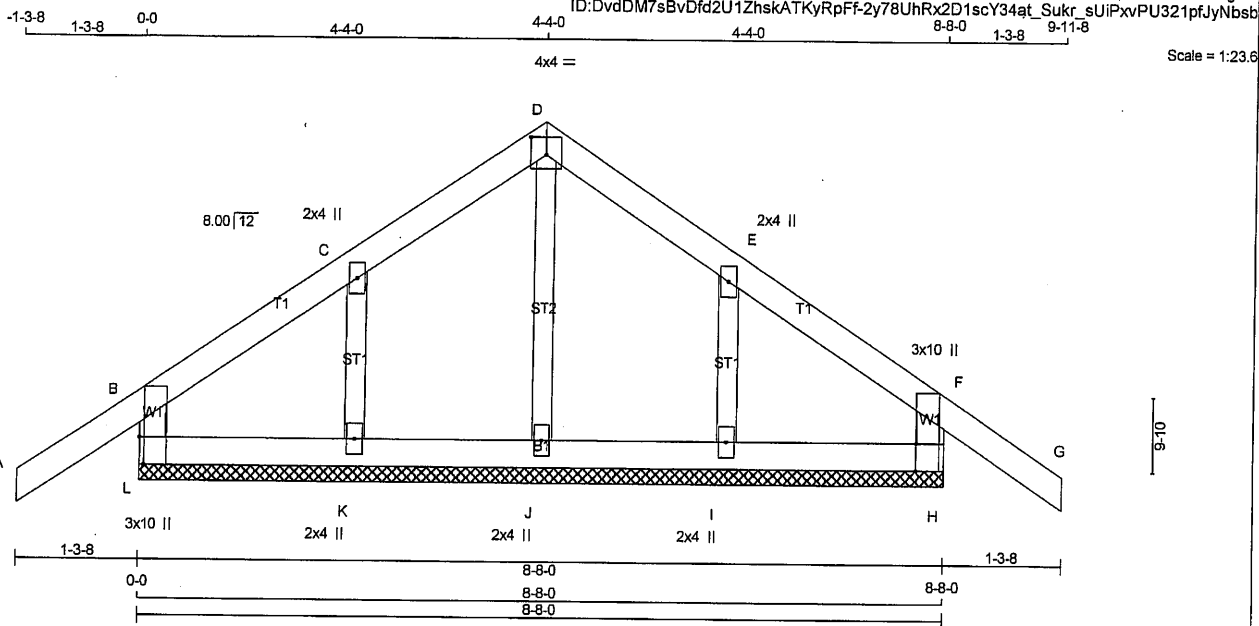
JSI GRIP= 0.81 (H) (INPUT = 0.90)
JSI METAL= 0.31 (F) (INPUT = 1.00)



DRWG NO. TAM 1812116
STRUCTURAL
COMPONENT ONLY

JOB NAME 289751	TRUSS NAME G22	QUANTITY 1	PLY 1	JOB DESC. 45246	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: DvdDM7sBvDfd2U1ZhskATKyRpFf-2y78UhRx2D1scY34at_Sukr_sUIPvPU321pfJyNbsb



Scale = 1:23.6

TOTAL WEIGHT = 32 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B, F, H, L					
C	TMW+w	MT20	2.0	4.0	
D	TTW-p	MT20	4.0	4.0	2.25 2.00
E	TMW+w	MT20	2.0	4.0	
F	TMBMV1+p	MT20	3.0	10.0	Edge
I, J, K					
I	BMW1+w	MT20	2.0	4.0	
L	TMBMV1+p	MT20	3.0	10.0	Edge 0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM TO		FR-TO			
L-B	-355 / 0	0.0	0.0 0.02 (3)	J-D	-212 / 0	0.05 (1)	
A-B	0 / 49	-127.8	-127.8 0.17 (1)	K-C	-268 / 0	0.04 (1)	
B-C	-59 / 0	-127.8	-127.8 0.08 (1)	I-E	-268 / 0	0.04 (1)	
C-D	-45 / 0	-127.8	-127.8 0.07 (1)				
D-E	-45 / 0	-127.8	-127.8 0.07 (1)				
E-F	-59 / 0	-127.8	-127.8 0.08 (1)				
F-G	0 / 49	-127.8	-127.8 0.17 (1)				
H-F	-355 / 0	0.0	0.0 0.02 (3)				
L-K	0 / 47	-28.0	-28.0 0.04 (3)				
K-J	0 / 36	-28.0	-28.0 0.04 (3)				
J-I	0 / 36	-28.0	-28.0 0.04 (3)				
I-H	0 / 47	-28.0	-28.0 0.04 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 35.9 PSF
DL = 8.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 61.4 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1), BC=0.04/1.00 (H-I:3), WB=0.05/1.00 (D-J:1), SS=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

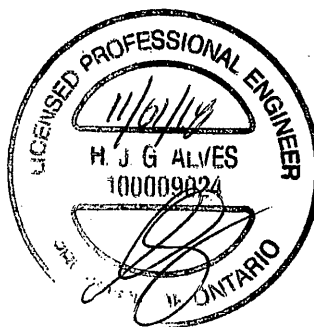
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (C) (INPUT = 0.90)

JSI METAL= 0.07 (F) (INPUT = 1.00)



DRWG NO. TAM 11/12/17
STRUCTURAL
COMPONENT ONLY

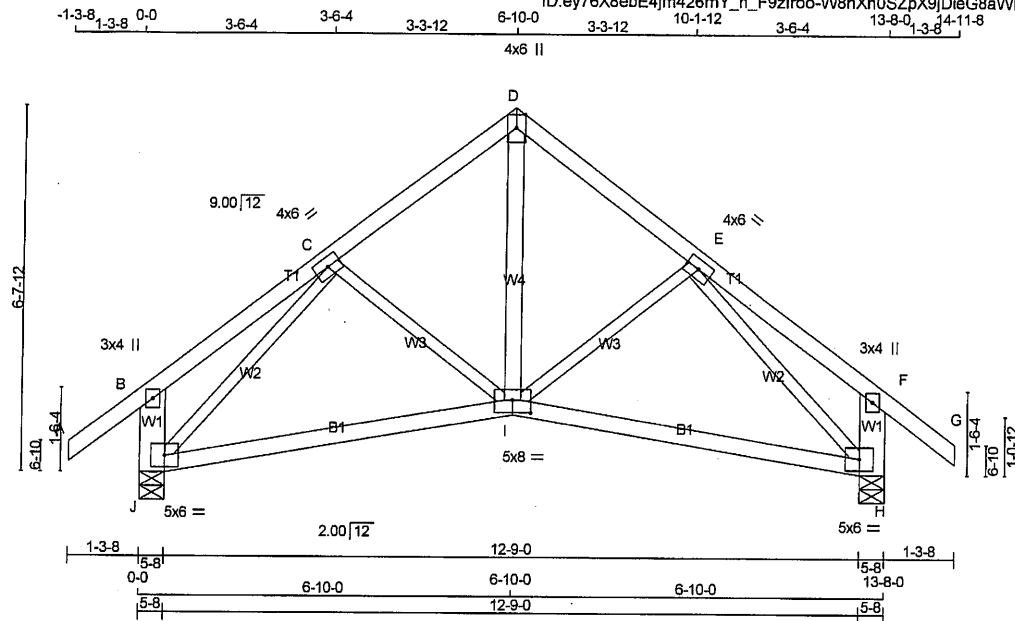
JOB NAME 289751	TRUSS NAME T210	QUANTITY 3	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 Mitek Industries, Inc. Thu Nov 1 10:51:05 2018 Page 1

ID:ey76X8ebE4jm426mY_n_F9zlr0o-VW8hXh0SZpX9jDieG8aVhRxO8huxegFgelimMBlyNbsa

Scale = 1:40.3



TOTAL WEIGHT = 3 X 64 = 191 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWV-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWV-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BVMW1-p	MT20	5.0	6.0		
I	BBWVW-p	MT20	5.0	8.0	2.75	4.00
J	BVMW1-p	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		GROSS REACTION		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX		
J	1241	0	1241	0	0	5-8	5-8				
H	1241	0	1241	0	0	5-8	5-8				

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS						
J	961	590 / 0	144 / 0	0 / 0	0 / 0	227 / 0	0 / 0		
H	961	590 / 0	144 / 0	0 / 0	0 / 0	227 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FACTORED MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 53	-127.8 -127.8	0.17 (1)	I-D	0 / 697	10.00	0.11 (1)
B-C	0 / 29	-127.8 -127.8	0.23 (1)	I-E	-184 / 22	10.00	0.06 (1)
C-D	-894 / 0	-127.8 -127.8	0.18 (1)	C-I	-184 / 22	6.25	0.06 (1)
D-E	-894 / 0	-127.8 -127.8	0.18 (1)	J-C	-1259 / 0	6.25	0.49 (1)
E-F	0 / 29	-127.8 -127.8	0.23 (1)	E-H	-1259 / 0	10.00	0.49 (1)
F-G	0 / 53	-127.8 -127.8	0.17 (1)				
J-B	-345 / 0	0.0 0.0	0.02 (1)				
H-F	-345 / 0	0.0 0.0	0.02 (1)				
J-I	0 / 849	-28.0 -28.0	0.48 (3)				
I-H	0 / 849	-28.0 -28.0	0.48 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.9	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	61.4	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.46")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.15")
ALLOWABLE DEFL. (TL) = $L/360$ (0.46")
CALCULATED VERT. DEFL. (TL) = $L/653$ (0.25")

CSI: TC=0.23/1.00 (E-F:1), BC=0.48/1.00 (I-J:3),
WB=0.49/1.00 (E-H:1), SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)

JSI METAL= 0.30 (E) (INPUT = 1.00)



DRWG NO. TAM 1812119
STRUCTURAL
COMPONENT ONLY

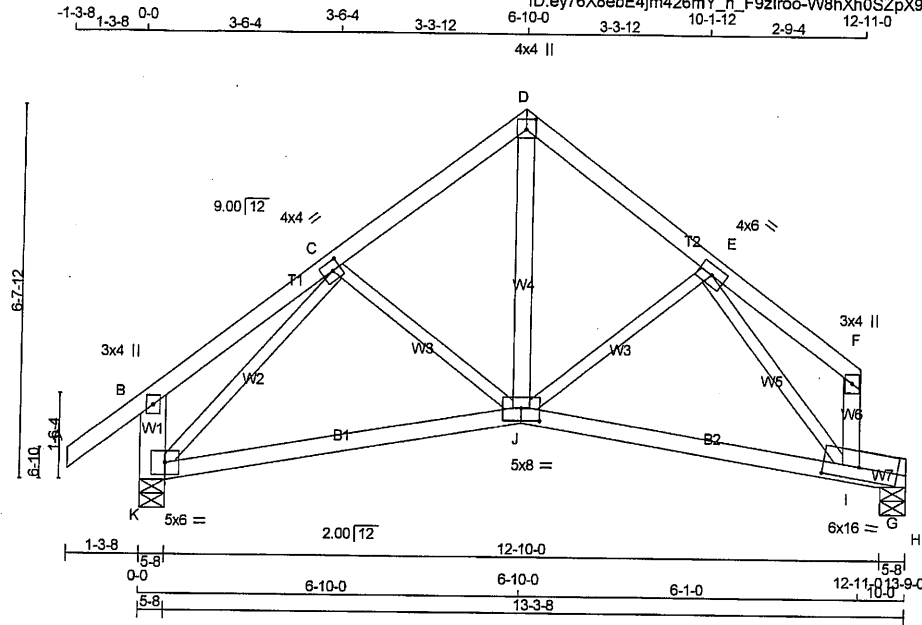
JOB NAME 289751	TRUSS NAME T210A	QUANTITY 4	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:ey76X8ebE4jm426mY_n_F9zlr0o-VW8hXh0SZpX9jDieG8aVhRxO8eUwrgFEelimMBlyNbsa

Scale = 1:39.4



TOTAL WEIGHT = 4 X 61 = 244 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - D 2x4 DRY No.2

D - F 2x4 DRY No.2

K - B 2x6 DRY No.2

I - F 2x4 DRY No.2

K - J 2x4 DRY No.2

J - H 2x4 DRY No.2

I - H 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

J - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES

B TMV+p MT20 3.0 4.0

C TMVW-t MT20 4.0 4.0 2.00 1.75

D TTW+p MT20 4.0 4.0 2.25 2.00

E TMVW-t MT20 4.0 6.0

F TMV+p MT20 3.0 4.0

I BMVWVm MT20 6.0 16.0 2.50 7.75

J BBWVW-p MT20 5.0 8.0 2.75 4.00

K BVMV1-p MT20 5.0 6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
K	1182	0	1182	0
H	1137	0	1137	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	915	563 / 0	136 / 0	0 / 0	0 / 0	216 / 0	0 / 0
H	897	524 / 0	153 / 0	0 / 0	0 / 0	220 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX
FR-TO						FR-TO		
A-B	0 / 53	-127.8	-127.8	0.17 (1)	10.00	J-D	0 / 575	0.09 (1)
B-C	0 / 30	-127.8	-127.8	0.23 (1)	10.00	J-E	-49 / 60	0.02 (1)
C-D	-800 / 0	-127.8	-127.8	0.18 (1)	6.25	C-J	-204 / 13	0.06 (1)
D-E	-794 / 0	-127.8	-127.8	0.17 (1)	6.25	K-C	-1168 / 0	0.46 (1)
E-F	0 / 31	-127.8	-127.8	0.19 (1)	10.00	E-I	-1123 / 0	0.36 (1)
K-B	-344 / 0	0.0	0.0	0.02 (1)	7.81			
I-F	-118 / 0	0.0	0.0	0.01 (1)	7.81			
K-J	0 / 787	-28.0	-28.0	0.47 (3)	10.00			
J-I	0 / 664	-28.0	-28.0	0.38 (3)	10.00			
I-G	0 / 368	-157.1	-157.1	0.54 (1)	10.00			
G-H	-1137 / 0	0.0	0.0	0.08 (1)	6.25			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.9	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	61.4	PSF	

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/999 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/660 (0.25")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/120 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/120 (0.00")

CSI: TC=0.23/1.00 (B-C:1), BC=0.54/1.00 (G-I:1),
WB=0.46/1.00 (C-K:1), SSI=0.42/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

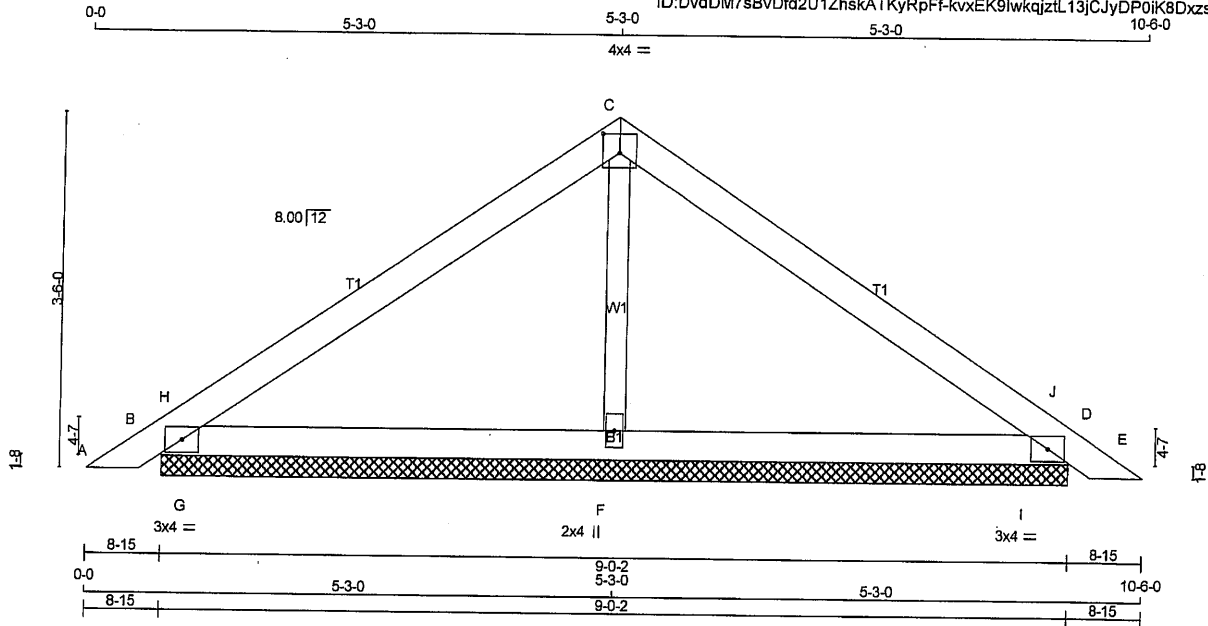
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DRWG NO. TAM 17812120
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME PB1	QUANTITY 5	PLY 1	JOB DESC. 45246	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.210 S May 18 2018 MITek Industries, Inc. Thu Nov 1 10:48:07 2018 Page 1
ID:DvdDM7sBvDfd2U1ZhsKATKyRpFf-kvxEK9lwqjztL13jCjyDP0ik8DxzsdRM?exx3yNbvM
Scale = 1:21.8



TOTAL WEIGHT = 5 X 27 = 134 lb
[M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
B	529 0	529 0	0 0	9-0-2	9-0-2
D	529 0	529 0	0 0	9-0-2	9-0-2
F	516 0	516 0	0 0	9-0-2	9-0-2

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	397	263 / 0	45 / 0	0 / 0	0 / 0	89 / 0	0 / 0	0 / 0
D	397	263 / 0	45 / 0	0 / 0	0 / 0	89 / 0	0 / 0	0 / 0
F	429	216 / 0	99 / 0	0 / 0	0 / 0	114 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED	VERT. LOAD	LC1 MAX	MEMB.	MAX. FACTORED
	(LBS)	(PLF)	CSI (LC)		(LBS)
FR-TO		FROM TO		FR-TO	
A-B	0 / 22	-127.8 -127.8	0.04 (1)	F-C	-223 / 0
B-H	-97 / 0	-127.8 -127.8	0.16 (1)	G-H	-666 / 7
H-C	-248 / 0	-127.8 -127.8	0.31 (1)	I-J	-666 / 7
C-J	-248 / 0	-127.8 -127.8	0.31 (1)		0.00 (1)
J-D	-97 / 0	-127.8 -127.8	0.16 (1)		
D-E	0 / 22	-127.8 -127.8	0.04 (1)		
B-G	0 / 194	-28.0 -28.0	0.30 (1)		
G-F	0 / 194	-28.0 -28.0	0.30 (1)		
F-I	0 / 194	-28.0 -28.0	0.30 (1)		
I-D	0 / 194	-28.0 -28.0	0.30 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 35.9 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 61.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (C-J:1), BC=0.30/1.00 (F-I:1), WB=0.04/1.00 (C-F:1), SSI=0.50/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

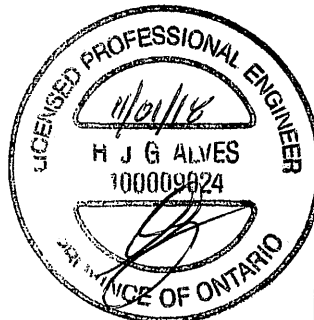
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

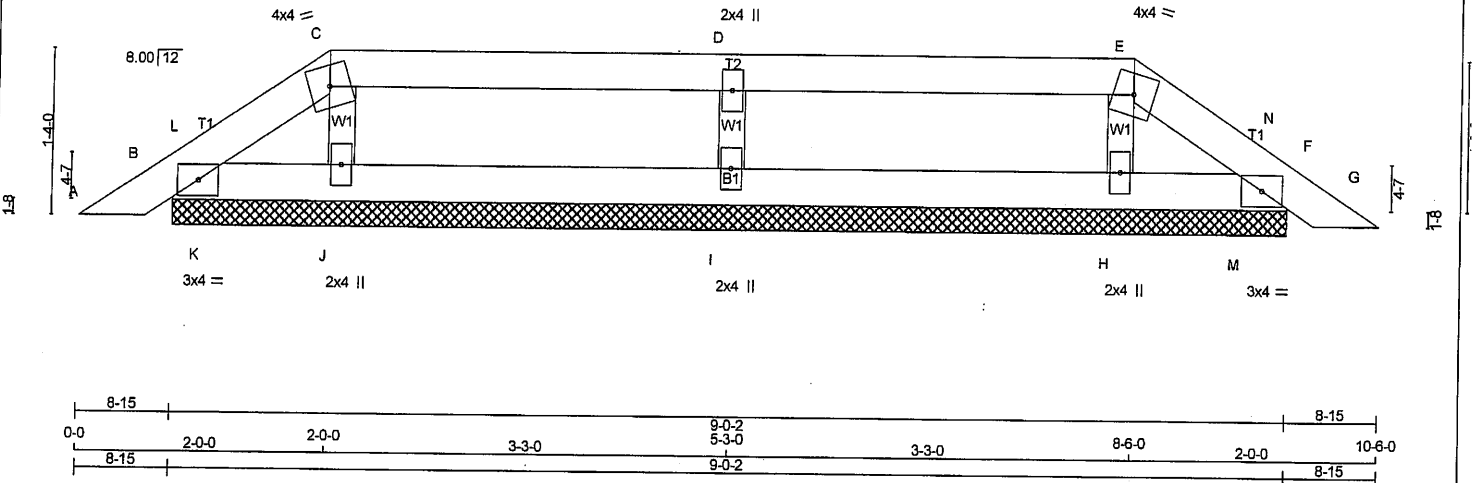
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)



DWG NO. TAM 17012091
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-w	MT20	2.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMB1-I	MT20	3.0	4.0		
H	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	194	0	194	0	9-0-2	9-0-2
F	194	0	194	0	9-0-2	9-0-2
J	288	0	288	0	9-0-2	9-0-2
M	288	0	288	0	9-0-2	9-0-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS			
		SNOW	LIVE	PERM. LIVE	WIND
B	137	106 / 0	5 / 0	0 / 0	27 / 0
F	137	106 / 0	5 / 0	0 / 0	27 / 0
J	239	121 / 0	55 / 0	0 / 0	64 / 0
M	239	121 / 0	55 / 0	0 / 0	64 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB. FR-TO	WEBS	
						MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
A-B	0 / 22	-127.8	-127.8	0.04 (1)	10.00	J-C	-194 / 0
B-L	-41 / 0	-127.8	-127.8	0.00 (3)	6.25	I-D	-518 / 0
L-C	-42 / 0	-127.8	-127.8	0.02 (1)	6.25	K-E	-194 / 0
C-D	-11 / 0	-127.8	-127.8	0.23 (1)	6.25	K-L	-80 / 0
D-E	-11 / 0	-127.8	-127.8	0.23 (1)	6.25	M-N	-80 / 0
E-N	-42 / 0	-127.8	-127.8	0.02 (1)	6.25		
N-F	-41 / 0	-127.8	-127.8	0.00 (3)	6.25		
F-G	0 / 22	-127.8	-127.8	0.04 (1)	10.00		
B-K	0 / 34	-28.0	-28.0	0.03 (1)	10.00		
K-J	0 / 34	-28.0	-28.0	0.05 (3)	10.00		
J-I	0 / 12	-28.0	-28.0	0.07 (3)	10.00		
I-H	0 / 12	-28.0	-28.0	0.07 (3)	10.00		
H-M	0 / 34	-28.0	-28.0	0.05 (3)	10.00		
M-F	0 / 34	-28.0	-28.0	0.03 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.9	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	61.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (D-E:1), BC=0.07/1.00 (I-J:3), WB=0.07/1.00 (D-I:1), SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

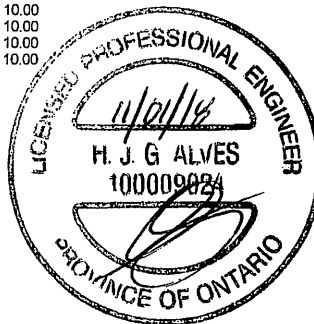
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

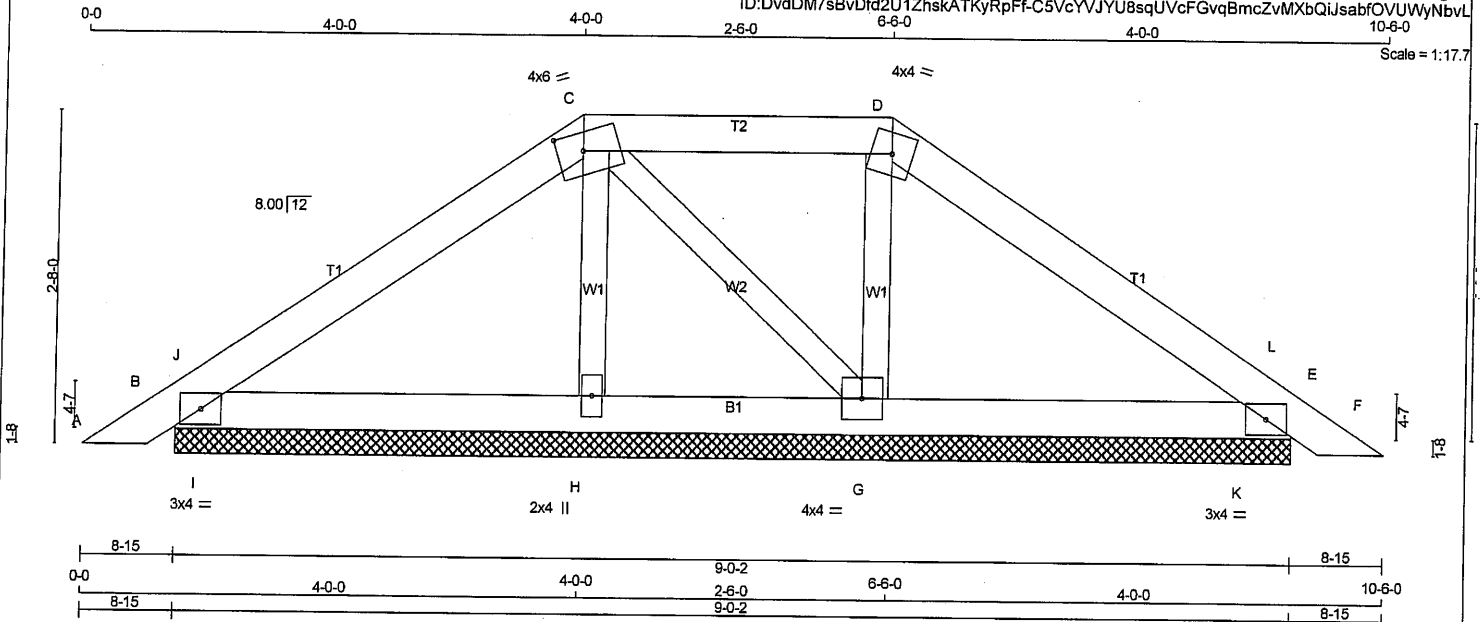


DRWG NO. TAM1712092
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME PB3	QUANTITY 1	PLY 1	JOB DESC. 45246	DRWG NO.
Tamarack Roof Truss, Burlington					

ID: DvdDM7sBvDfd2U1ZhskATKyRpFf-C5VcYVJYU8sqUVcFGvqBmcZvMXbQiJsabfOVUWYNbvL

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TOTAL WEIGHT = 30 lb

LUMBER					N. L. G. A. RULES	
CHORDS	SIZE		LUMBER	DESCR.		
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
B - E	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW-m	MT20	4.0	6.0	1.75 2.50
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMWW1-t	MT20	4.0	4.0	
H	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
B	415	0	415	0	0	9-0-2	9-0-2	9-0-2	9-0-2
E	389	0	389	0	0	9-0-2	9-0-2	9-0-2	9-0-2
H	334	0	334	0	0	9-0-2	9-0-2	9-0-2	9-0-2
G	435	0	435	0	0	9-0-2	9-0-2	9-0-2	9-0-2

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
B	312	207/0	35/0	0/0	0/0	69/0	0/0		
E	294	193/0	35/0	0/0	0/0	66/0	0/0		
H	274	143/0	60/0	0/0	0/0	72/0	0/0		
G	344	200/0	59/0	0/0	0/0	84/0	0/0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/22	-127.8 -127.8	0.04 (1)	H-C	-211/0	0.03 (1)	
B-J	-94/0	-127.8 -127.8	0.05 (1)	C-G	-55/0	0.01 (1)	
J-C	-163/0	-127.8 -127.8	0.17 (1)	G-D	-274/0	0.04 (1)	
C-D	-80/0	-127.8 -127.8	0.14 (1)	I-J	-318/32	0.00 (1)	
D-L	-118/0	-127.8 -127.8	0.17 (1)	K-L	-322/31	0.00 (1)	
L-E	-74/0	-127.8 -127.8	0.05 (1)				
E-F	0/22	-127.8 -127.8	0.04 (1)				
B-I	0/130	-28.0 -28.0	0.15 (1)				
I-H	0/130	-28.0 -28.0	0.15 (1)				
H-G	0/120	-28.0 -28.0	0.09 (1)				
G-K	0/92	-28.0 -28.0	0.15 (1)				
K-E	0/92	-28.0 -28.0	0.15 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.9	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	61.4	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (D-L:1), BC=0.15/1.00 (H-I:1), WB=0.04/1.00 (D-G:1), SSI=0.25/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

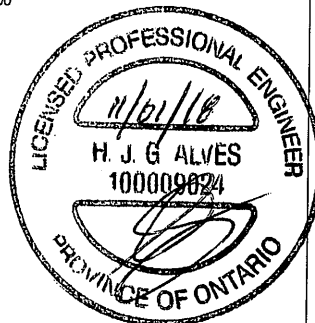
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1657
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



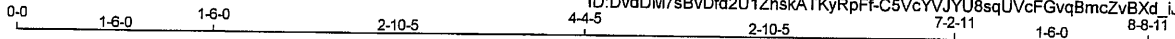
DWG NO. TAM 11612093
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME PB4	QUANTITY 1	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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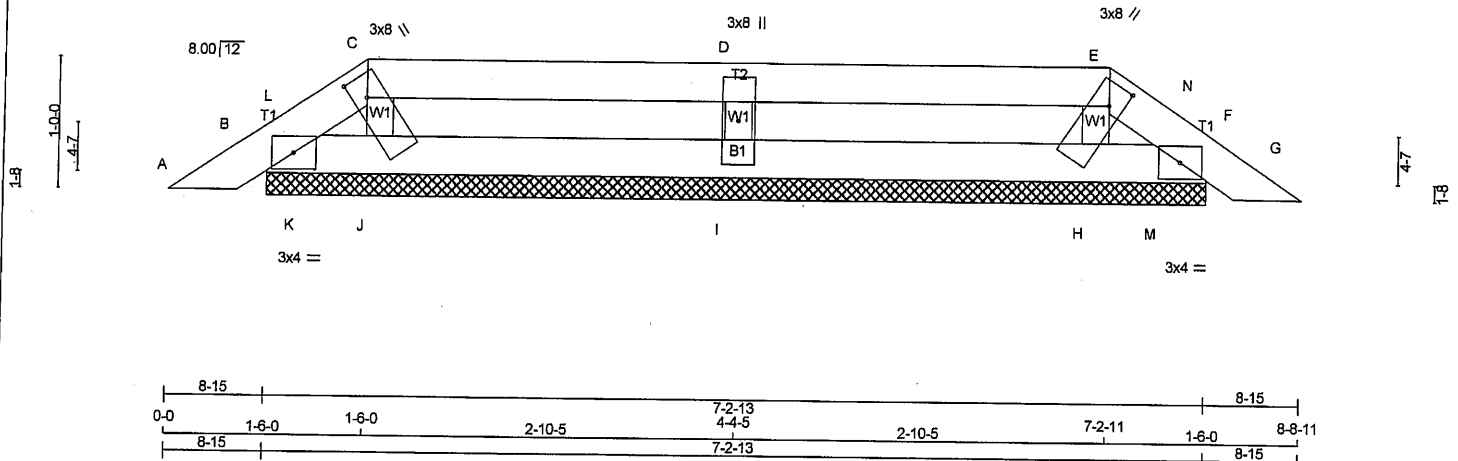
Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MTek Industries, Inc. Thu Nov 1 10:48:08 2018 Page 1

ID: DvdDM7sBvDfd2U1ZhskATKyRpFf-C5VcYVJYU8sqUVcFGvqBmcZvBXd_iYabfOVUWYnBvL



Scale = 1:16.9



TOTAL WEIGHT = 20 lb [M]

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E, H, I, J						
C	TTBW1*+h	MT20	3.0	8.0	2.00	1.25
D	TMBMW1+H	MT20	3.0	8.0		
E	TTBW1*+h	MT20	3.0	8.0	2.00	1.25
F	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
JT		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
B	184	0	184	0	0	7-2-13	7-2-13		
F	184	0	184	0	0	7-2-13	7-2-13		
J	195	0	195	0	0	7-2-13	7-2-13		
I	539	0	539	0	0	7-2-13	7-2-13		
H	195	0	195	0	0	7-2-13	7-2-13		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	124	106/0	0/-3	0/0	0/0	21/0	0/0
F	124	106/0	0/-3	0/0	0/0	21/0	0/0
J	171	73/0	49/0	0/0	0/0	49/0	0/0
I	416	257/0	61/0	0/0	0/0	98/0	0/0
H	171	73/0	49/0	0/0	0/0	49/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/22	-127.8 -127.8	0.04 (1)	J-C	-133/0	0.02 (1)	
B-L	-71/0	-127.8 -127.8	0.04 (1)	I-D	-457/0	0.06 (1)	
L-C	-39/0	-127.8 -127.8	0.01 (1)	H-E	-133/0	0.02 (1)	
C-D	0/2	-127.8 -127.8	0.18 (1)	K-L	-23/0	0.00 (1)	
D-E	0/2	-127.8 -127.8	0.18 (1)	M-N	-23/0	0.00 (1)	
E-N	-39/0	-127.8 -127.8	0.01 (1)				
N-F	-71/0	-127.8 -127.8	0.04 (1)				
F-G	0/22	-127.8 -127.8	0.04 (1)				
B-K	0/23	-28.0 -28.0	0.01 (1)				
K-J	0/23	-28.0 -28.0	0.04 (3)				
J-I	-1/0	-28.0 -28.0	0.05 (3)				
I-H	-1/0	-28.0 -28.0	0.05 (3)				
H-M	0/23	-28.0 -28.0	0.04 (3)				
M-F	0/23	-28.0 -28.0	0.01 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 35.9 PSF
	DL	= 8.0 PSF
BOT CH.	LL	= 10.5 PSF
	DL	= 7.0 PSF
TOTAL LOAD	=	61.4 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-D:1), BC=0.05/1.00 (I-J:3), WB=0.06/1.00 (D-I:1), SI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

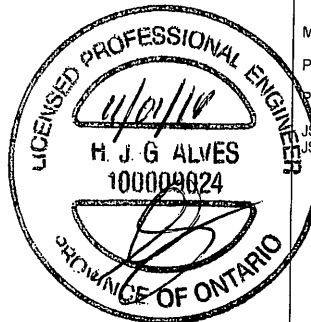
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.21 (C) (INPUT = 0.90)
JS METAL= 0.06 (D) (INPUT = 1.00)



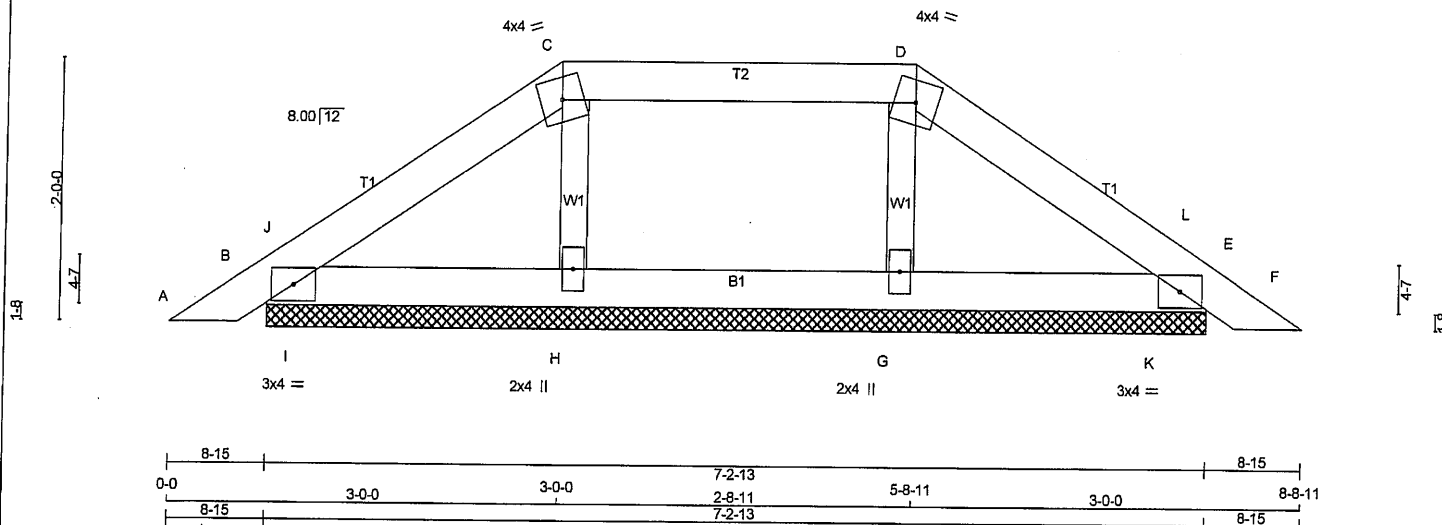
DWG NO. TAM 17812094
STRUCTURAL
COMPONENT ONLY

JOB NAME 289750	TRUSS NAME PB5	QUANTITY 1	PLY 1	JOB DESC. 45246	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 MTEK Industries, Inc. Thu Nov 1 10:48:09 2018 Page 1
ID:DvdDM7sBvDfd2U1ZhskATKyRpPf-gH3_lrJAFS_h6fBRqdLQJq54BxyoRmEkqJ720yyNbvK
8-8-11

Scale = 1:16.9



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

C - D 2x4 DRY

D - F 2x4 DRY

B - E 2x4 DRY

ALL WEBS 2x3 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

No.2

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION		INPUT BRG	REQD BRG
			DOWN	HORZ		
B	302	0	302	0	7-2-13	7-2-13
E	302	0	302	0	7-2-13	7-2-13
H	347	0	347	0	7-2-13	7-2-13
G	347	0	347	0	7-2-13	7-2-13

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	224	153/0	22/0	0/0	0/0	49/0	0/0
E	224	153/0	22/0	0/0	0/0	49/0	0/0
H	279	154/0	54/0	0/0	0/0	71/0	0/0
G	279	154/0	54/0	0/0	0/0	71/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO	FR-TO	
A-B	0/22	-127.8 -127.8 0.04 (1)	H-C	-248/0 0.04 (1)
B-J	-74/0	-127.8 -127.8 0.01 (1)	G-D	-248/0 0.04 (1)
J-C	-89/0	-127.8 -127.8 0.08 (1)	I-J	-167/10 0.00 (1)
C-D	-55/0	-127.8 -127.8 0.16 (1)	K-L	-167/10 0.00 (1)
D-L	-89/0	-127.8 -127.8 0.08 (1)		
L-E	-74/0	-127.8 -127.8 0.01 (1)		
E-F	0/22	-127.8 -127.8 0.04 (1)		
B-I	0/72	-28.0 -28.0 0.08 (1)		
I-H	0/72	-28.0 -28.0 0.08 (1)		
H-G	0/65	-28.0 -28.0 0.08 (1)		
G-K	0/72	-28.0 -28.0 0.08 (1)		
K-E	0/72	-28.0 -28.0 0.08 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	35.9	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	61.4	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS TRUSS COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 50.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 35.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (C-D-1), BC=0.08/1.00 (E-K-1), WB=0.04/1.00 (D-G-1), SSI=0.14/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

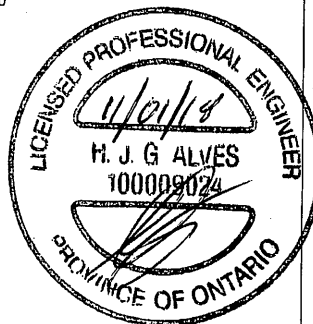
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



DRWG NO. TAM 11/01/14
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

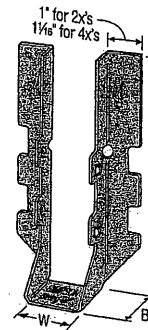
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

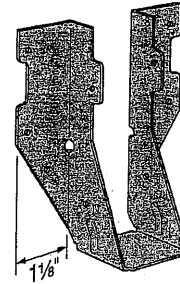
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



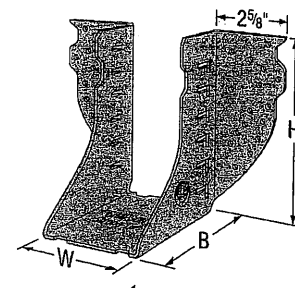
LUS28



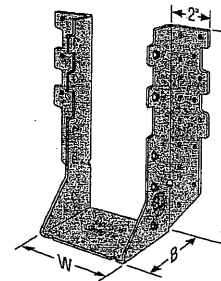
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)



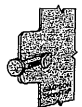
HGUS28-2



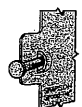
HHUS210-2



Double-Shear
Nailing
Top View

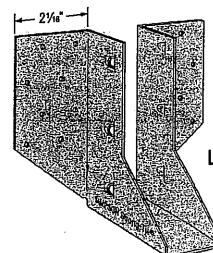
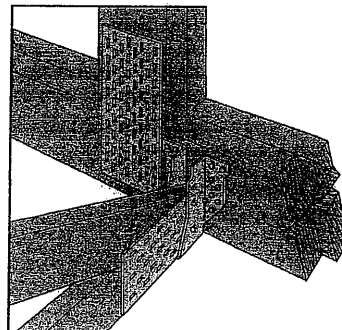


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options information on pp. 125–127.

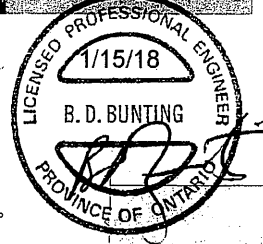
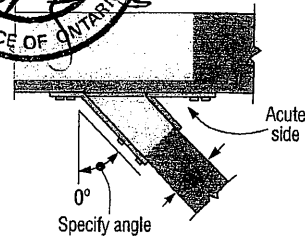
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value

**SIMPSON**
Strong-Tie**Top View HHUS Hanger
Skewed Right**(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).**Standard and Double-Shear Joist Hangers (cont.)**

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance				
		W	H	B	d _a ³	Header	Joist	D-Fir-L		S-P-F		
								Uplift	Normal	Uplift	Normal	
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	
								lb.	lb.	lb.	lb.	
								kN	kN	kN	kN	
Single 2x Sizes												
SS	LUS24	18	1 1/8	3 1/4	1 3/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
									316	723	287	514
SS	LU24L	22	1 1/8	3	1 3/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
									160	454	142	322
SS	LU26L	22	1 1/8	5	1 3/4	4 3/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
									520	714	287	507
SS	LUS26	18	1 1/8	4 3/4	1 3/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
									632	965	574	725
SS	HUS26	16	1 1/8	5 3/4	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
									1130	2197	920	1724
SS	LJS26DS	18	1 1/8	5	3 1/2	4 3/4	(16) 16d	(6) 16d	2055	4265	1460	4115
									914	1897	649	1831
SS	HGUS26	12	1 1/8	5 3/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
									1196	2351	1196	2535
SS	LU28L	20	1 1/8	6 3/4	1 3/4	5 3/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
									607	972	454	689
SS	LUS28	18	1 1/8	6 3/4	1 3/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
									632	1121	574	796
SS	HUS28	16	1 1/8	7 3/4	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345
									1604	2386	1190	1933
SS	HGUS28	12	1 1/8	7 3/4	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	6900
									1474	3419	1474	3073
SS	LU210L	20	1 1/8	8	1 3/4	7 3/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
									507	1110	454	787
SS	LUS210	18	1 1/8	7 1/4	1 3/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
									632	1239	574	983

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

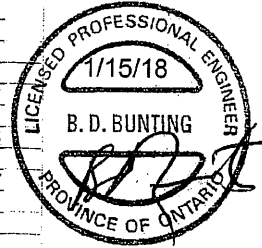
Face-Mount Hangers

SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

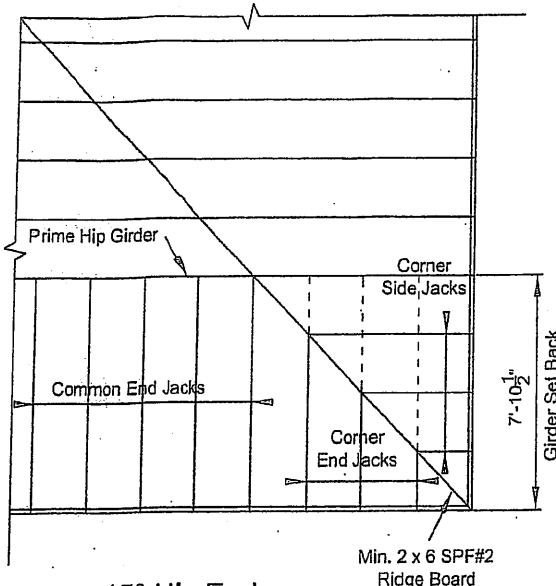
These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _a ¹	Header	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
SS LUS24-2	18	3 3/16	3 3/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435
								3.71	8.99	2.62	6.38
SS LUS26-2	18	3 3/16	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3 3/16	5 3/8	3	3 1/16	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3 3/16	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
SS LUS28-2	18	3 3/16	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3 3/16	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS28-2	12	3 3/16	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS210-2	18	3 3/16	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3 3/16	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3 3/16	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4 15/16	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4 15/16	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4 15/16	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6 3/16	5 3/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6 3/16	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6 1/8	8 3/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.37	18.84	32.07
HGUS210-4	12	6 3/16	9 3/8	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.14	21.60	46.26
HGUS212-4	12	6 3/16	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6 3/16	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80
4x Sizes											
SS LUS46	18	3 3/16	4 3/4	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
SS HHUS46	14	3 3/16	5 1/4	3	3 1/16	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS46	12	3 3/16	5 1/4	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
SS LUS48	18	3 3/16	6 3/4	2	3 3/8	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
SS HHUS48	14	3 3/16	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.77	11.90	28.22
HGUS48	12	3 3/16	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS410	18	3 3/16	8 3/4	2	5 3/8	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3 3/16	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3 3/16	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3 3/16	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.95	32.00	51.80



Plated Truss Connectors

See footnotes on p. 258.



45° Hip End

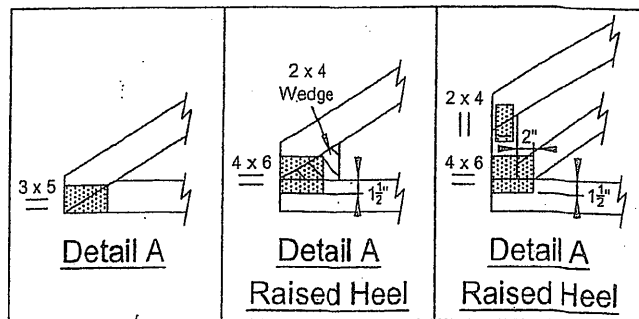
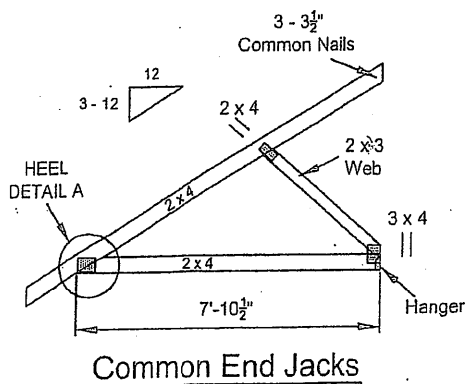
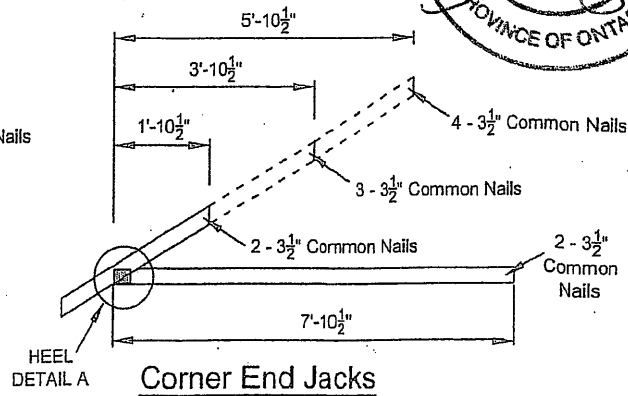
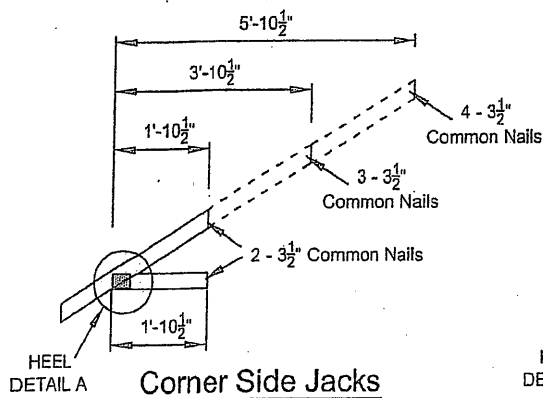
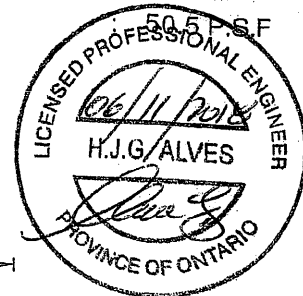
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

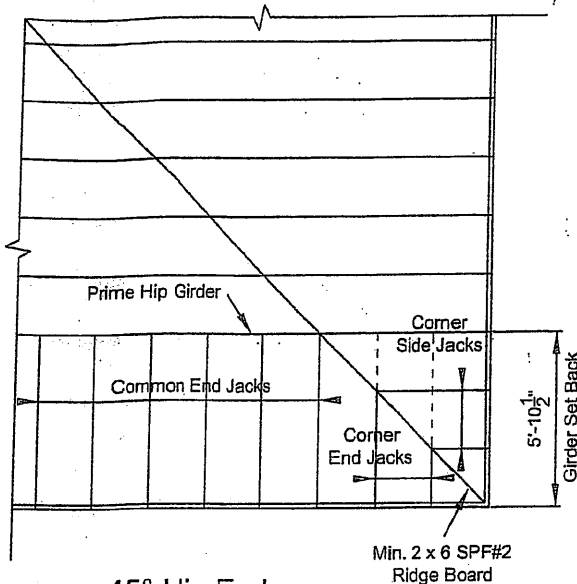
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

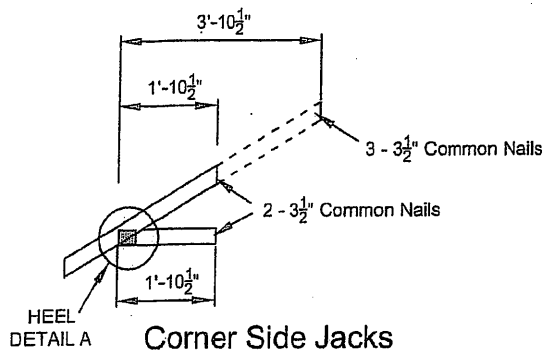
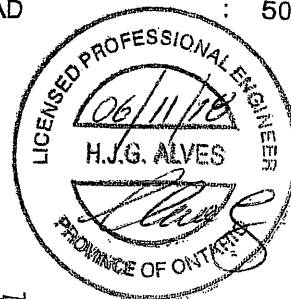
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

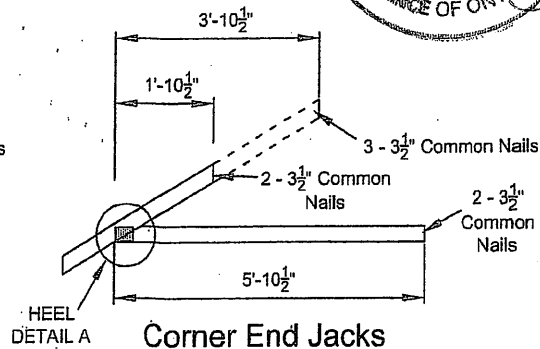
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

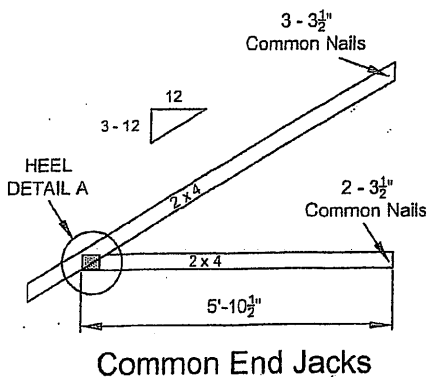
TOTAL LOAD : 50.5 P.S.F



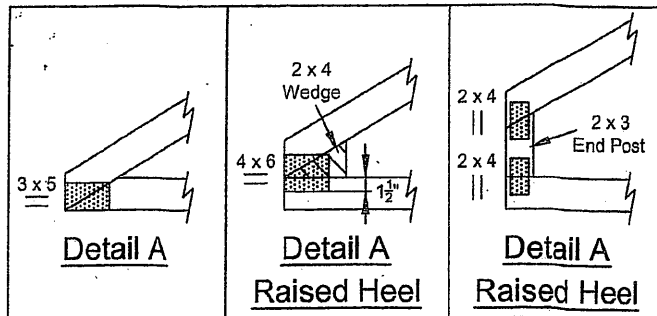
Corner Side Jacks



Corner End Jacks



Common End Jacks



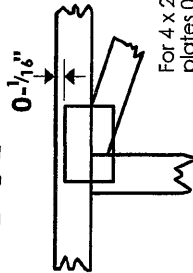
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



* Plate location details available in MiTek software or upon request.

PLATE SIZE

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

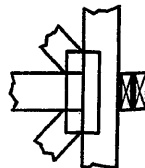
4 X 4

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



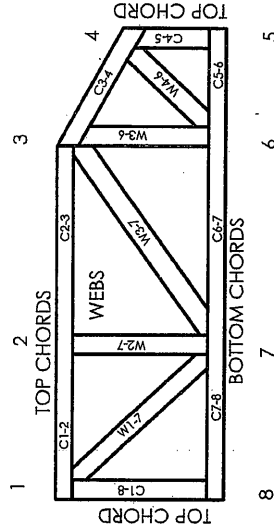
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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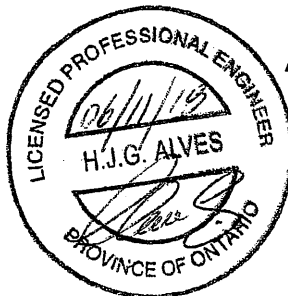
MiTek
POWER TO PERFORM™

MiTek Engineering Reference Sheet: MIL-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900219

Feb 09, 2018

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

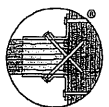
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp.20–24.

Installation:

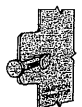
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

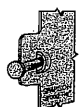
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



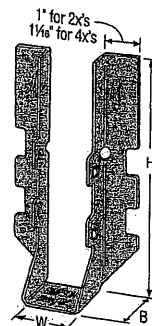
Double-Shear
Nailing
Top View



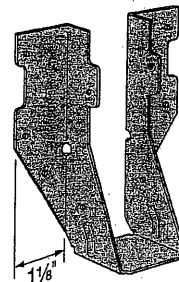
Double-Shear
Nailing
Side View;
Do not
bend tab



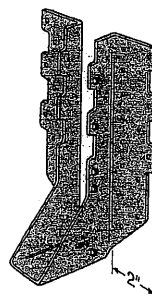
Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



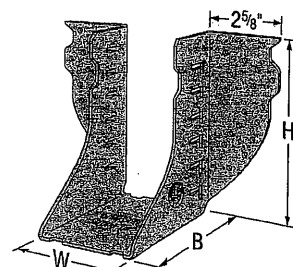
LUS28



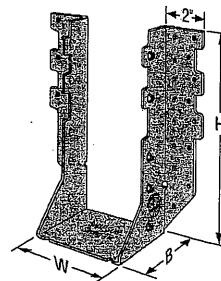
LU26L



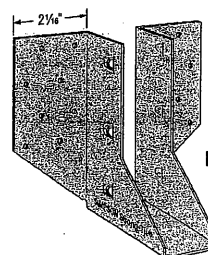
HUS210
(HUS26, HUS28,
and HHUS similar)



HGUS28-2



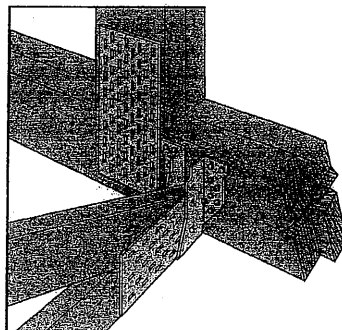
HHUS210-2



LJS26DS

Typical HUS26
Installation
with Reduced
Heel Height

(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options information on pp. 125–127.

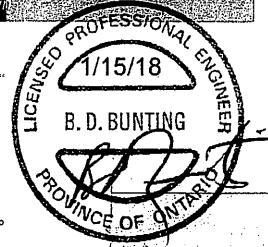
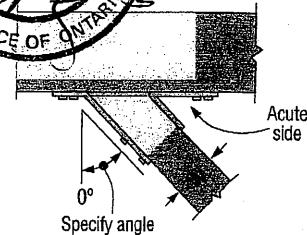
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value

**SIMPSON
Strong-Tie®**

**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance				
		W	H	B	d _e ³	Header	Joist	D-Fir-L		S-P-F		
								Uplift	Normal	Uplift	Normal	
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)	
lb.	lb.	lb.	lb.									
kN	kN	kN	kN									
Single 2x Sizes												
SS	LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
	LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
SS	LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
	LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
SS	HUS26	16	1 1/8	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2065	3875
	LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4265	1460	4115
SS	HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
	LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
SS	LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	1420	2520	1290	1790
	HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3605	5365	2675	4345
SS	HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	6900
	LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
SS	LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

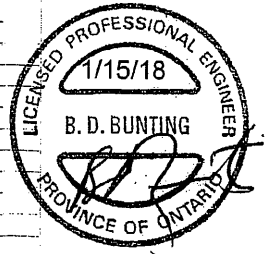
Face-Mount Hangers

SIMPSON
Strong-Tie®

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance					
		W	H	B	d _a ³	Header	Joist	D-Fir-L		S-P-F			
								Uplift	Normal	Uplift	Normal		
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)		
lb.	lb.	lb.	lb.										
kN	kN	kN	kN										
Double 2x Sizes													
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	590	1435		
								3.71	8.99	2.62	6.38		
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920		
								7.65	11.54	6.87	8.54		
HHUS26-2	14	3 3/8	5 3/8	3	3 3/8	(14) 16d	(6) 16d	2850	7335	2065	5205		
								12.68	32.63	9.20	23.15		
HGUS26-2	12	3 3/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
								19.51	39.81	13.83	28.27		
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575		
								7.65	14.79	6.87	11.45		
HHUS28-2	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345		
								16.75	39.77	11.90	28.22		
HGUS28-2	12	3 3/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215		
								27.00	57.74	19.17	40.99		
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195		
								11.48	20.02	10.32	14.21		
HHUS210-2	14	3 3/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000		
								20.77	42.47	18.84	31.14		
HGUS210-2	12	3 3/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270		
								30.43	62.34	21.60	45.69		
Triple 2x Sizes													
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
								19.51	39.81	13.83	28.27		
HGUS28-3	12	4 1/8	7 1/4	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215		
								27.00	57.74	19.17	40.99		
HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865		
								20.77	43.02	18.84	30.54		
HGUS210-3	12	4 1/8	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400		
								30.43	65.14	21.60	46.26		
Quadruple 2x Sizes													
HGUS26-4	12	6 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
								19.51	39.81	13.83	28.27		
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215		
								27.00	57.74	19.17	40.99		
HHUS210-4	14	6 1/8	8 3/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210		
								20.77	45.17	18.84	32.07		
HGUS210-4	12	6 1/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400		
								30.43	65.14	21.60	46.26		
HGUS212-4	12	6 1/8	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645		
								33.98	66.70	24.13	47.35		
HGUS214-4	12	6 1/8	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645		
								45.06	72.95	32.00	51.80		
4x Sizes													
LUS46	18	3 3/8	4 3/4	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	1920		
								7.65	11.54	6.87	8.54		
HHUS46	14	3 3/8	5 1/4	3	3 3/8	(14) 16d	(6) 16d	2540	7335	2065	5205		
								11.30	32.63	9.20	23.15		
HGUS46	12	3 3/8	5 1/4	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355		
								19.51	39.81	13.83	28.27		
LUS48	18	3 3/8	6 3/4	2	3 3/8	(6) 16d	(4) 16d	1720	3325	1545	2575		
								7.65	14.79	6.87	11.45		
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8940	2675	6345		
								16.75	39.77	11.90	28.22		
HGUS48	12	3 3/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215		
								27.00	57.74	19.17	40.99		
LUS410	18	3 3/8	8 3/4	2	5 3/8	(8) 16d	(6) 16d	2580	4500	2320	3195		
								11.48	20.02	10.32	14.21		
HGUS410	12	3 3/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270		
								30.43	62.34	21.60	45.69		
HGUS412	12	3 3/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645		
								33.98	66.70	24.13	47.35		
HGUS414	12	3 3/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645		
								45.06	72.95	32.00	51.80		



Plated Truss Connectors

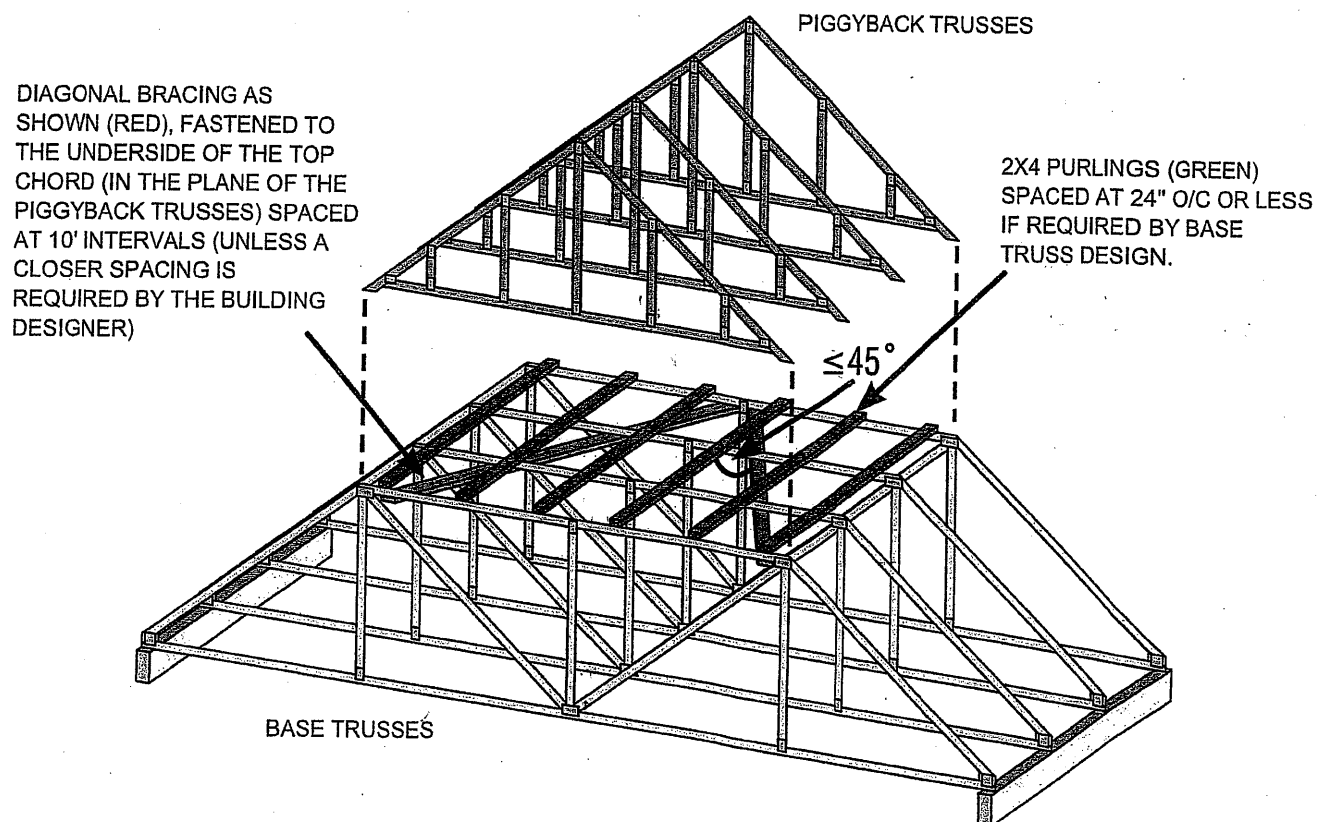
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

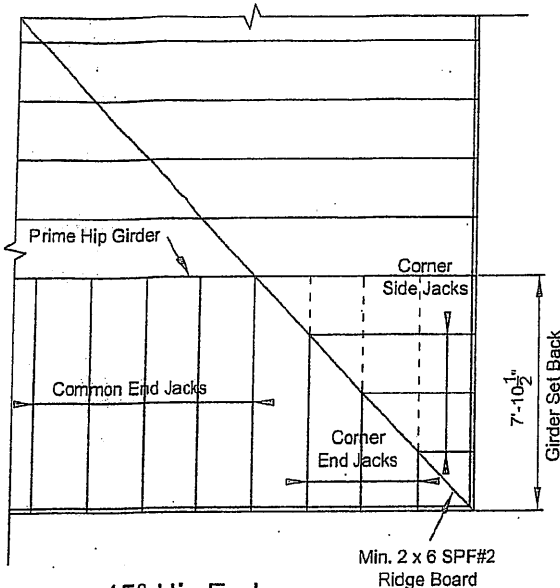
Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



45° Hip End

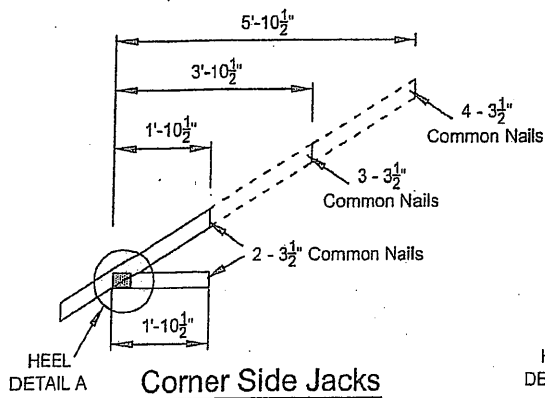
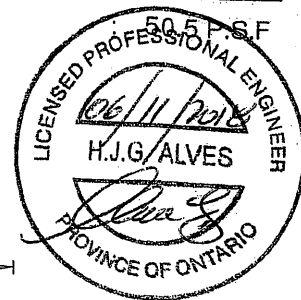
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

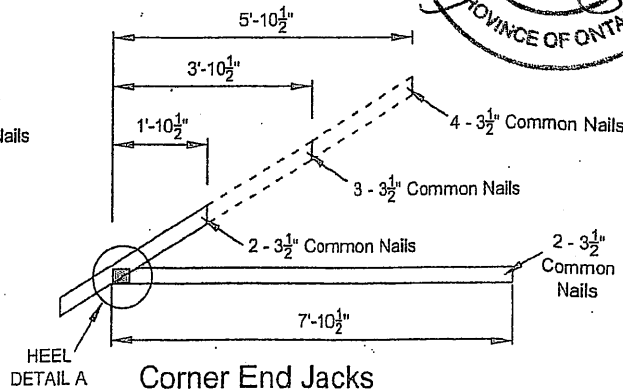
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

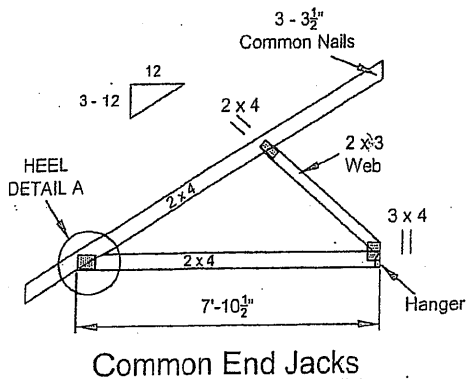
TOTAL LOAD



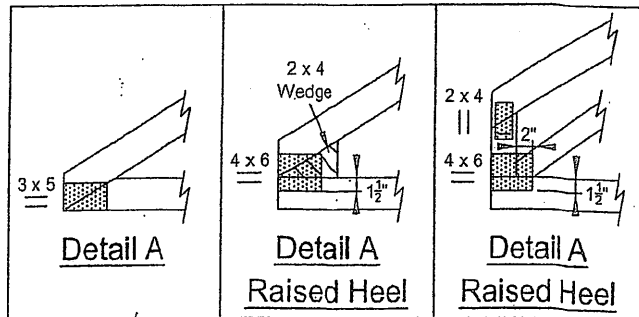
Corner Side Jacks



Corner End Jacks

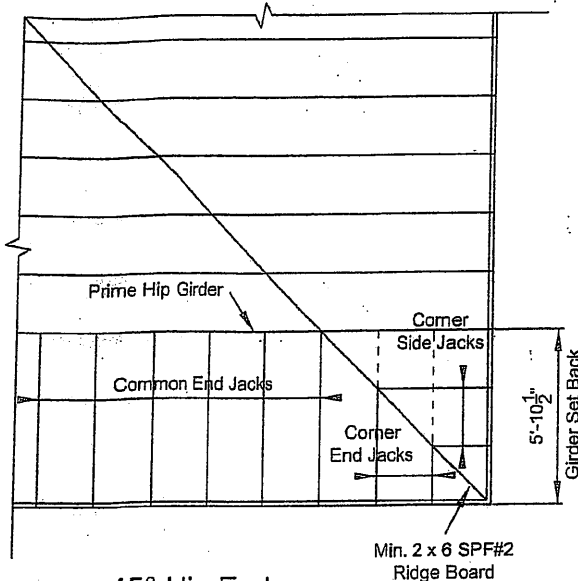


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

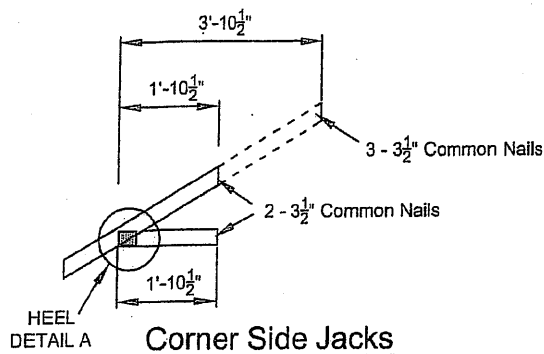
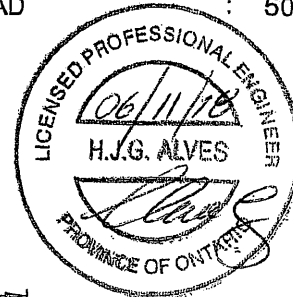
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

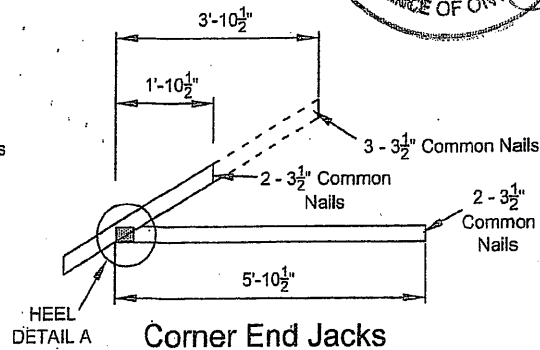
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

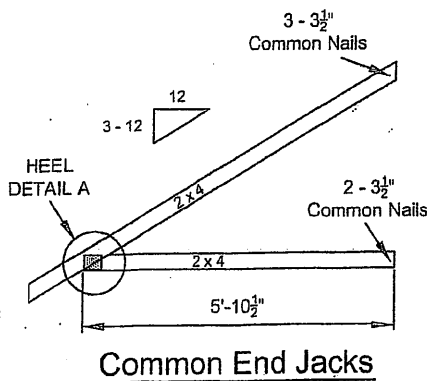
TOTAL LOAD : 50.5 P.S.F.



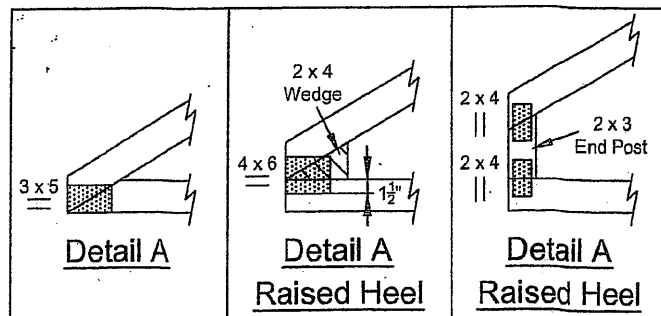
Corner Side Jacks



Corner End Jacks



Common End Jacks



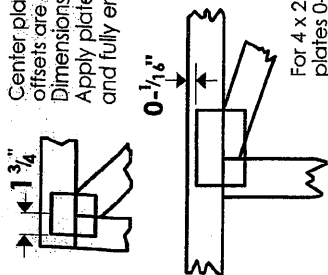
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



*Plate location details available in Mitek software or upon request.

PLATE SIZE

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

4 X 4

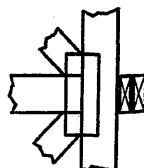
LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.



BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

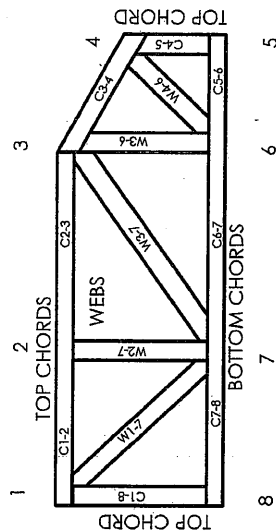


Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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Mi

Mitek

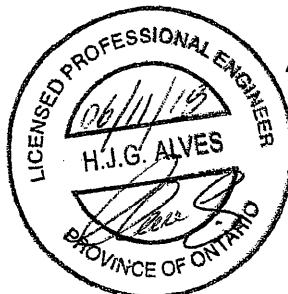
POWER TO PERFORM.™

Mitek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300215

Feb 09, 2018