

Products						
PlotID	Length	Product	Plies	Net Qty		
J1	16-00-00	9 1/2" NI-40x	1	2		
J2	14-00-00	9 1/2" NI-40x	1	35		
J2DJ	14-00-00	9 1/2" NI-40x	2	8		
J3	12-00-00	9 1/2" NI-40x	1	2		
J4	10-00-00	9 1/2" NI-40x	1	7		
J5	6-00-00	9 1/2" NI-40x	1	10		
J6	4-00-00	9 1/2" NI-40x	1	4		
J7	2-00-00	9 1/2" NI-40x	1	2		
B2	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B3	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B1	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B7	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B4	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B5	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		

C	Connector Summary					
Qty	Manuf	Product				
2	H1	IUS2.56/9.5				
11	H1	IUS2.56/9.5				
4	H1	IUS2.56/9.5				
5	H1	IUS2.56/9.5				
1	H2	HU310-2				
1	H3	HUS1.81/10				



FROM PLAN DATED: OCT 2017

BUILDER: GREENPARK HOMES

SITE: SECONDO VALES ESTATES

MODEL: HOLLAND 8E

ELEVATION: 1

LOT:

CITY: EAST GWILLIMBURY

SALESMAN: M D DESIGNER: AJ REVISION: Ibv

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND

INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F
REQ'D UNDER INTERIOR UNIFORM LOAD
BEARING WALLS. MULTIPLE SQUASH
BLOCKS REQ'D UNDER CONCENTRATED
LOADS. SEE FIGURE 1. CANTILEVERED
JOISTS INCLUDING CANT' OVER BRICK REQ.
I-JOIST BLOCKING ALONG BEARING AND
RIMBOARD CLOSURE AT ENDS. SEE
FIGURES 4 & 5 FOR REINFORCEMENT
REQUIREMENTS. FOR HOLES INCLUDING
DUCT CHASE AND FIELD CUT OPENINGS
SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE
APPLICATION AS PER O.B.C 9.30.6.

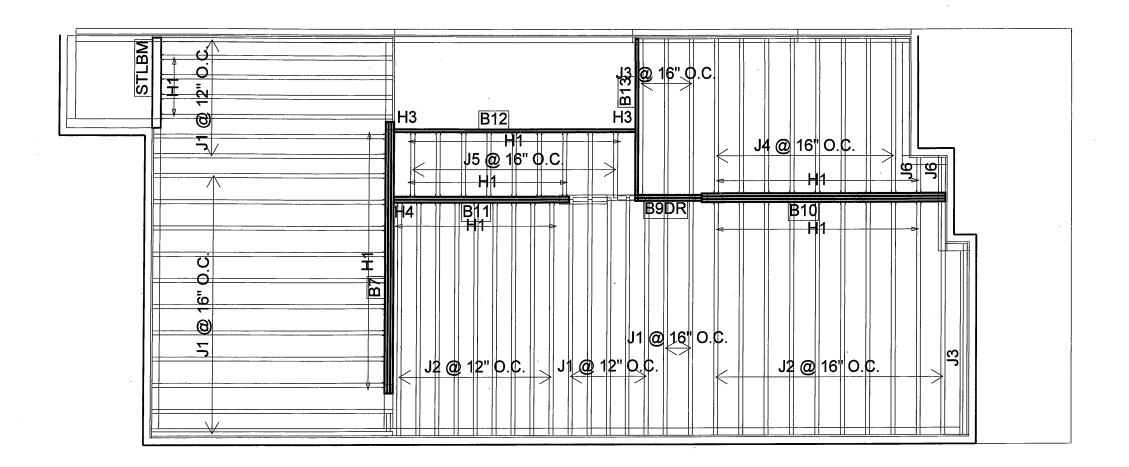
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft² TILED AREAS: 20 lb/ft

SUBFLOOR: 3/4" GLUED AND NAILED

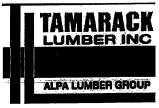
DATE: 2017-11-24

1st FLOOR



		Products		
PlotID	Length	Product	Plies	Net Qty
J1	14-00-00	9 1/2" NI-40x	1	25
J2	12-00-00	9 1/2" NI-40x	1	19
J3	10-00-00	9 1/2" NI-40x	1	4
J4	8-00-00	9 1/2" NI-40x	1	8
J5	4-00-00	9 1/2" NI-40x	1	9
J6	2-00-00	9 1/2" NI-40x	1	2
B12	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B7	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3
B13	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1
B11	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2
B9DR	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2

	Connector Summary						
Qty	Manuf	Product					
9	H1	IUS2.56/9.5					
16	H1	IUS2.56/9.5					
29	H1	IUS2.56/9.5					
4	H1	IUS2.56/9.5					
1	H3	HUS1.81/10					
∥ 1	H3	HUS1.81/10					
1	H4	HGUS410					
1							



FROM PLAN DATED: OCT 2017

BUILDER: GREENPARK HOMES

SITE: SECONDO VALES ESTATES

MODEL: HOLLAND 8E

ELEVATION: 1

LOT:

CITY: EAST GWILLIMBURY

SALESMAN: M D DESIGNER: AJ REVISION: Ibv

NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE FOR PROPER STORAGE AND** INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD **CUT OPENINGS** SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft² DEAD LOAD: 15.0 lb/ft² TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 2018-01-20

2nd FLOOR



BC CALC® Design Report

Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1st Floor\Flush Beams\B10(i2159)

Dry | 1 span | No cant.

January 20, 2018 10:23:52

Build 6215 Job name:

Address:

City, Province, Postal Code: EAS...URY

File name:

HOLLAND 8E.mmdl

Description: 1st Floor\Flush Beams\B10(i2159)

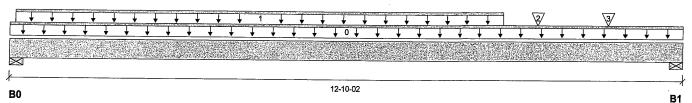
Specifier:

Designer: ΑJ

Customer: Code reports:

CCMC 12472-R

Company:



Total Horizontal Product Length = 12-10-02

Reaction Summary (Down / Unlift) (lbs)

	ininary (DOWILL O				
Bearing	Live	Dead	Snow	Wind	
B0, 3-1/2"	2,522 / 0	1,036 / 0			
B1, 4-3/8"	2,149 / 0	897 / 0			

Lo	ad Summary					Live	Dead	Snow	Wind	Tributary
Tag		Load Type	Ref.	Start	End	1.00	0.65	1.00	1.15	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	12-10-02		14			00-00-00
1,	Smoothed Load	Unf. Lin. (lb/ft)	L	00-01-04	09-05-04	405	152			n\a
2	-	Conc. Pt. (lbs)	L	10-01-04	10-01-04	522	195			n\a
3	-	Conc. Pt. (lbs)	L	11-05-04	11-05-04	369	138			n\a

Controls Summary	Factored Demand	Factored Resistance	Demand/ Resistance	Case	Location
Pos. Moment	15,329 ft-lbs	36,222 ft-lbs	42.3%	1	06-01-04
End Shear	4,639 lbs	17,356 lbs	26.7%	1	01-01-00
Total Load Deflection	L/382 (0.387")	n\a	62.9%	4	06-05-04
Live Load Deflection	L/538 (0.275")	n\a	66.9%	5	06-05-04
Max Defl.	0.387"	n\a	n\a	4	06-05-04
Span / Depth	15.6				

Bearing	g Supports	Dim. (LxW)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	3-1/2" x 5-1/4"	5,077 lbs	64.7%	22.6%	Unspecified
B1	Wall/Plate	4-3/8" x 5-1/4"	4,345 lbs	44.3%	15.5%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Nailing schedule applies to both sides of the member.



1612

DWG NO. TAM 5396 .18 STRUCTURAL COMPONENT ONLY





Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP

PASSED

1st Floor\Flush Beams\B10(i2159)

Dry | 1 span | No cant.

January 20, 2018 10:23:52

Build 6215

Job name:

Address:

BC CALC® Design Report

City, Province, Postal Code: EAS...URY

Customer:

Code reports:

CCMC 12472-R

File name:

HOLLAND 8E.mmdl

ΑJ

Description:

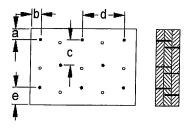
1st Floor\Flush Beams\B10(i2159)

Specifier:

Designer:

Company:

Connection Diagram



a minimum = 2"

b minimum = 3"

c = 2-1/4" d = 🦈

e minimum = 3"

Calculated Side Load = 441.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Nailing schedule applies to both sides of the member.

Nails Connectors are: 16d ?

3-1/2" ARDOX SPIRAL



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®

DWG NO. TAM \$396.98 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i1980)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:05

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i1980)

Specifier:

Designer: AJ

Company:

Misc:

◎	$\overline{\forall}$	2/	3/ 4/
		All Market States	
ј во	05-08-00		р В1

Total Horizontal P	roduct Len	gth = 05-08-00
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Reaction Summary (Down / Uplift) (lbs)						
Be aring	Live	De ad	Snow	Wind		
B0, 1-3/4"	684/0	368/0				
B1, 3-1/2"	855/0	468/0				

Load Summon				Live	Dead	Snow	Wind	Trib.
Load Summary Tag Description	Load Type	Load Type Ref. Start			0.65	1.00	1.15	
0 J2(i1963)	Conc. Pt. (lbs)	L 01-00-08	01-00-08	378	189			n/a
1 J2DJ(i2037)	Conc. Pt. (lbs)	L 02-05-08	02-05-08	405	202			n/a
2 J2(i2147)	Conc. Pt. (lbs)	L 03-08-08	03-08-08	372	186			n/a
3 J2(i2072)	Conc. Pt. (lbs)	L 05-00-08	05-00-08	384	192			n/a
4 E1(i510)	Conc. Pt. (lbs)	L 05-07-00	05-07-00		13			n/a

Domand/

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	2,357 ft-lbs	25,408 ft-lbs	9.3%	1	02-05-08
End Shear	1,476 lbs	11,571 lbs	12.8%	1	00-11-04
Total Load Defl.	L/999 (0.017")	n/a	n/a	4	02-09-04
Live Load Defl.	L/999 (0.011")	n/a	n/a	5	02-09-04
Max Defl.	0.017" [´]	n/a	n/a	4	02-09-04
Span / Depth	6.8	n/a	n/a		00-00-00

Beari	ng Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Post	1-3/4" x 3-1/2"	1,487 lbs	29.9%	19.9%	Unspecified
B1	Post	3-1/2" x 3-1/2"	1,867 lbs	18.8%	12.5%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086. CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWG NO. TAM 5397 -18 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i1980)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:05

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i198

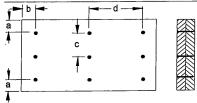
Specifier:

Designer: AJ

Company:

Misc:

Connection Diagram



a minimum = 2"

b minimum = 3"

c = 2-3/4"

Calculated Side Load = 577.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nails

Connectors are: 16d

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 5 3 9 7 - 18 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i2022)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:04

Build 5033

BC CALC® Design Report

Job Name: Address:

City, Province, Postal Code:EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i2022)

Specifier:

Designer: A

Company:

Misc:

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2000 000 000 000 000 000 000 000 000 00	₫
06-03-06 B	

Total Horizontal	Product	Length =	06-03-06
------------------	---------	----------	----------

Reaction Summary	(Down / Uplift) (lbs)	Dead	Snow	Wind	
B0, 3-1/2"	21 / 0	26 / 0			
B1, 2-3/8"	23 / 0	26 / 0			

Land Commons				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 FC1 Floor Material	Unf. Lin. (lb/ft)	L 00-03-08	06-03-06	7	4		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	94 ft-lbs	12,704 ft-lbs	0.7%	1	03-02-04
End Shear	45 lbs	5,785 lbs	0.8%	1	01-01-00
Total Load Defl.	L/999 (0.002")	n/a	n/a	4	03-02-04
Live Load Defl.	L/999 (0.001")	n/a	n/a	5	03-02-04
Max Defl.	0.002"	n/a	n/a	4	03-02-04
Span / Depth	7.5	n/a	n/a		00-00-00

Bearin	ng Supports	Dim. (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Post	3-1/2" x 1-3/4"	64 lbs	1.3%	0.9%	Unspecified
B1	Wall/Plate	2-3/8" x 1-3/4"	67 lbs	3%	1.3%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 5398-18 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i2099)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:06

BC CALC® Design Report



Build 5033 Job Name: Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

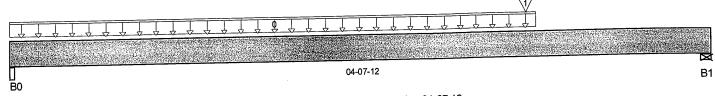
Description: Designs\Flush Beams\Basment\Flush Beams\B3(i2099)

Specifier:

Designer: AJ

Company:

Misc:



Total Horizontal	Product	Length =	04-07-12

Reaction Summary (Down /	Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 2-3/4" B1, 3-1/2"	237/0 466/0	132/0 251/0			

			Live	e Dead	Snow Wind	· I FID.
Load Summary	Load Type	Ref. Start	End 1.00	0.65	1.00 1.15	
Tag Description			00 00 00 50	27		n/a
0 FC1 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	03-06-00 53			
0 FC F100 Material	• •	1. 03-05-02	03-05-02 516	3 267		n/a
1 B4(i1978)	Conc. Pt. (lbs)	L 03-05-02	00-00-02 010			

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
	996 ft-lbs	12,704 ft-lbs	7.8%	1	03-05-02
Pos. Moment	1.006 lbs	5.785 lbs	17.4%	1	03-06-12
End Shear	L/999 (0.008")	n/a	n/a	4	02-05-06
Total Load Defl.	L/999 (0.005")	n/a	n/a	5	02-05-06
Live Load Defl.	0.008"	n/a	n/a	4	02-05-06
Max Defl. Span / Depth	5.4	n/a	n/a		00-00-00

D. avis	- « Sumporto	Dim. (L x W)	Demand	De man d/ Re sistance Su pport	De mand/ Re sistance Me mbe r	Material
B0 B1	n g Supports Beam Wall/Plate	2-3/4" x 1-3/4" 3-1/2" x 1-3/4"	520 lbs 1,012 lbs	20.2% 31%	8.9% 13.5%	Unspecified Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST® , BC RIM BOARD $^{\text{TM}}$, BC $^{\text{R}}$, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood



DWG NO. TAM 539 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i1978)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:05

Build 5033

Job Name:

Address: City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

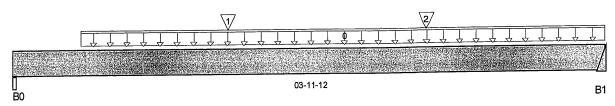
Description: Designs\Flush Beams\Basment\Flush Beams\B4(i1978)

Specifier:

Designer: A

Company:

Misc:



Total Horizontal Product Length = 03-11-12

		TOTAL TOTAL OTTAL TOTAL OTTAL	it Longtii oo					
Reaction Summary (Do	own / Uplift) (lbs) Live	De ad S	Snow	Wine	d			
B0, 5-1/2"	495/0	258/0						•
B1	531/0	275/0						
1.0				Live	Dead	Snow	Wind	Trib
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00	1.15	
0 STAIR	Unf. Lin. (lb/ft)	L 00-05-08	03-11-12	240	120			n/a
U UITAIN	J L (12/19)							n/a

01-05-04

02-09-04

01-05-04 87

02-09-04 91

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	986 ft-lbs	12,704 ft-lbs	7.8%	1	02-01-12
End Shear	649 lbs	5.785 lbs	11.2%	1	01-03-00
Total Load Defl.	L/999 (0.006")	n/a	n/a	4	02-01-12
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	02-01-12
Max Defl.	0.006"	n/a	n/a	4	02-01-12
Span / Depth	4.4	n/a	n/a		00-00-00

Conc. Pt. (lbs)

Conc. Pt. (lbs)

Beari	ng Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Beam	5-1/2" x 1-3/4"	1,065 lbs	20.7%	9.1%	Un specified
B1	Hanger	2" x 1-3/4"	1,140 lbs	n/a	26.7%	HUS1.81/10

Notes

1

J6(i2152)

J6(i2108)

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

Disclosure

44

46

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

n/a

n/a

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DWG NO. TAM 5 400 .PB STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i1996)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:06

Build 5033

Job Name:

Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

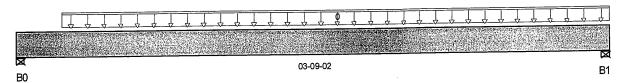
Description: Designs\Flush Beams\Basment\Flush Beams\B5(i1996)

Specifier:

Designer: AJ

Company:

Misc:



Total Horizontal Product Length = 03-09-02

Reaction Summary (Dov	vn / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 3-1/2"	14 / 0	16 / 0			
B1, 2-3/8"	16 / 0	17 / 0			

			Live	Dead	Snow wind	ITID.
Load Summary Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0 FC1 Floor Material	Unf. Lin. (lb/ft)	L 00-03-08	03-09-02 9	4		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	35 ft-lbs	12,704 ft-lbs	0.3%	1	01-11-02
End Shear	21 lbs	5,785 lbs	0.4%	1	01-01-00
Total Load Defl.	L/999 (0")	n/a	n/a	4	01-11-02
Live Load Defl.	L/999 (0")	n/a	n/a	5	01-11-02
Max Defl.	0"	n/a	n/a	4	01-11-02
Span / Depth	4.3	n/a	n/a		00-00-00

		- <i>u</i> 	D	De mand/ Resistance	Demand/ Resistance Member	Material
Beari	ng Supports	Dim . (L x W)	Demand	Support		
B0	Wall/Plate	3-1/2" x 1-3/4"	42 lbs	1.3%	0.6%	Un spe cified
B1	Wall/Plate	2-3/8" x 1-3/4"	45 lbs	2%	0.9%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

CONFORMS TO OBC 2012

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 540/ .18
STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6(i2151)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:06

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code:EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B6(i2151)

Specifier:

Designer: AJ

Company. Misc:

$\overline{\mathbb{Q}}$	$\overline{\forall}$	2/	3
		PROPERTY AND ADMINISTRATION OF THE PROPERTY OF	La L
⊠ (B0		04-05-12	B1

Total Horizontal Product Length = 04-05-12

Reaction Summary (Down / Uplift) (lbs)						
Be aring	Live	De ad	Snow	Wind	·	
B0, 2-3/8"	1,114/0	582/0				:
B1, 2-3/8"	721/0	382/0				

Lood Summon			Live	e Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	End 1.00	0.65	1.00 1.15	
0 J2(i2077)	Conc. Pt. (lbs)	L 00-01-14	00-01-14 627	7 317		n/a
1 -	Conc. Pt. (lbs)	L 01-05-14	01-05-14 452	2 226		n/a
2 -	Conc. Pt. (lbs)	L 02-09-14	02-09-14 452	2 226		n/a
3 -	Conc. Pt. (lbs)	L 04-01-14	04-01-14 297	7 148		n/a

Domand/

Demand/

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,513 ft-lbs	25,408 ft-lbs	6%	1	02-09-14
End Shear	1,035 lbs	11,571 lbs	8.9%	1	00-11-14
Total Load Defl.	L/999 (0.007")	n/a	n/a	4	02-02-14
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	02-02-14
Max Defl.	0.007"	n/a	n/a	4	02-02-14
Span / Depth	5.3	n/a	n/a		00-00-00

Bearing Supports		Dim . (L x W)	Demand	Resistance Support	Resistance Member	Material
B0	Wall/Plate	2-3/8" x 3-1/2"	2,399 lbs	54%	23.7%	Unspecified
B1	Wall/Plate	2-3/8" x 3-1/2"	1,558 lbs	35.1%	15.4%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6(i2151)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:06

BC CALC® Design Report

Build 5033

Job Name: Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

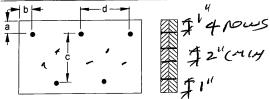
Description: Designs\Flush Beams\Basment\Flush Beams\B6(i215

Specifier:
Designer: AJ

Company:

Misc:

Connection Diagram D



a minimum = **(**" c = **?**-1/2" b minimum = 3" d = **(%)**

Calculated Side Load = 724.5 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Spice Nails

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM \$ 402 -1 STRUCTURAL COMPONENT ONLY



Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B6DR(i2157)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 10:15:58

V-7

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

Description: Designs\Dropped Beams\Basment\Dropped Beams\B6D

Specifier:

Designer: AJ

Company:

Misc:

·	V
	×
⊠ 04-0	01-08 B1
B0	

Total Horizontal	Product	Length	= 04-01-08
------------------	---------	--------	------------

Reaction Summary (Do	own / Uplift) (lbs)	Donad	Snow	Win	d		
Be aring	Live	De ad	SHOW	******	<u> </u>		
B0, 5-1/2"	423/0	222/0					
B1, 1-3/4"	874/0	464/0					
				Live	Dead	Snow Wind	Trib.

			Live	Dead	Snow Wina	I FID.
Load Summary Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0 STAIR	Unf. Lin. (lb/ft)	1 00-05-08	03-11-12 240	120		n/a
1 B3(i2155)	Conc. Pt. (lbs)	L 04-00-10		244		n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	856 ft-lbs	10,549 ft-lbs	8.1%	1	02-02-10
End Shear	500 lbs	5,785 lbs	8.6%	1	01-03-00
Total Load Defl.	L/999 (0.006")	n/a	n/a	4	02-02-10
Live Load Defl.	<u>∟/</u> 999 (0.004")	n/a	n/a	5	02-02-10
Max Defl.	0.006"	n/a	n/a	4	02-02-10
Span / Depth	4.6	n/a	n/a		00-00-00

				De mand/ Re sistance	Demand/ Resistance	
Beari	ing Supports	Dim.(LxW)	Demand	Support	Member	Material
B0 B1	Wall/Plate Wall/Plate	5-1/2" x 1-3/4" 1-3/4" x 1-3/4"	911 lbs 1,892 lbs	11.7% 76.1%	7.8% 50.6%	Unspecified Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 03-06-04, Bottom: 03-06-04.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM5403 STRUCTURAL COMPONENT ONLY



Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B7(i2015)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:03:51

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

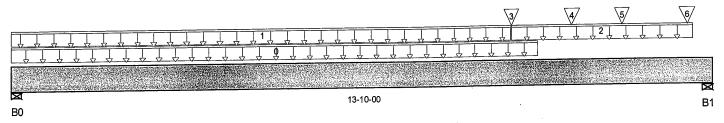
File Name: HOLLAND 8E.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\87(i2015)

Specifier:

Designer: AJ Company:

Misc:



Total Horizontal Product Length = 13-10-00

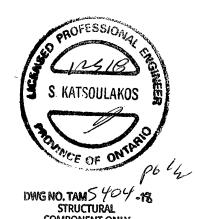
Reaction Summary (D	own / Uplift) (lbs) Live	De ad	Snow	Wind	·
B0, 5-1/2"	2,248 / 0	957/0			•
B1, 4-1/2"	3,853/0	1,694/0			

						Live	Dead	Snow	Wind	Trib.
	ad Summary g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	00-00-00	10-05-00	253	95			n/a
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-10-08	11	4			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	L	09-10-08	13-05-08	24	9			n/a
2	B11(i2000)	Conc. Pt. (lbs)	L	09-10-08	09-10-08	1,323	537			n/a
4	J1 (i2027)	Conc. Pt. (lbs)	L	11-01-00	11-01-00	289	108			n/a
5	J1(i2048)	Conc. Pt. (lbs)	L	12-01-00	12-01-00	247	93			n/a
-	51(12040)	Conc. Pt. (lbs)	L	13-04-00	13-04-00	1,405	648			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	17,051 ft-lbs	39,636 ft-lbs	43%	1	08-05-00
End Shear	5.370 lbs	17.356 lbs	30.9%	1	12-08-00
Total Load Defl.	L/323 (0.488")	0.656"	74.4%	4	07-01-00
Live Load Defl.	L/460 (0.343")	0.438"	78.3%	5	07-01-00
Max Defl.	0.488"	n/a	n/a	4	07-01-00
Span / Depth	16.6	n/a	n/a		00-00-00

Popri	ng Supports	Dim.(L x W)	De man d	De mand/ Re sistance Support	Demand/ Resistance Member	Material
B0	Wall/Plate	5-1/2" x 5-1/4"	4,568 lbs	29.6%	13%	Un specified
B1	Wall/Plate	4-1/2" x 5-1/4"	7,898 lbs	62.6%	27.4%	Un specified

Notes



COMPONENT ONLY

Page 1 of 2



Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B7(i2015)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:03:51

BC CALC® Design Report



Build 5033

Job Name:

Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B7(i201t

Specifier:

Designer: Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

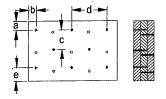
O86.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9

Connection Diagram



a minimum = 2"

c = 2-1/4"

b minimum = 3" d = 6 θ e minimum = 3"

Calculated Side Load = 487.5 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

Connectors are: 16d

A .

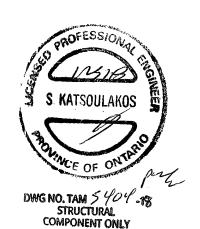
3-1/2" ARDOX SPIRAL

· Nails

Disclosure

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ALLJOIST® , BC RIM BOARD™, BCI® ,
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PLUS® , VERSA-RIM®,
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Products L.L.C.





Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B9DR(i2080)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:01

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B9D

Specifier:

Designer: AJ

Company:

Misc:

O	V	2/	3
⊠ B0		03-06-00	B1

Total Horizontal Product Length = 03-06-00

Reaction Summary (Down /	Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 6"	1,072 / 0	465/0			
B1, 4"	705/0	281/0			

Load Commons				e Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0 -	Conc. Pt. (lbs)	L 00-01-08	00-01-08 515	239		n/a
1 J1(i2059)	Conc. Pt. (lbs)	L 00-07-08	00-07-08 238	89		n/a
2 -	Conc. Pt. (lbs)	L 01-07-08	01-07-08 492	185		n/a
3 -	Conc. Pt. (lbs)	L 02-11-08	02-11-08 532	200	,	n/a

CONFORMS TO OBC 2012

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location	
Pos. Moment	844 ft-lbs	25,408 ft-lbs	3.3%	1	01-07-08	
End Shear	708 lbs	11,571 lbs	6.1%	1	01-03-08	
Total Load Defl	L/999 (0.001")	n/a	n/a	4	01-09-12	
Live Load Defl.	L/999 (0.001")	n/a	n/a	5	01-09-12	
Max Defl.	0.001"	n/a	n/a	4	01-09-12	
Span / Depth	3.5	n/a	n/a		00-00-00	

				Resistance	Resistance	
Bea	ring Supports	Dim.(LxW)	De man d	Support	Member	Material
B0	Wall/Plate	6" x 3-1/2"	2,189 lbs	12.8%	8.5%	Unspecified
B1	Wall/Plate	4" x 3-1/2"	1,410 lbs	12.4%	8.3%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-02-09, Bottom: 00-02-09.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9



DWG NO. TAM 5405 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B9DR(i2080)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:04:01

BC CALC® Design Report

Build 5033 Job Name:

Address: City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

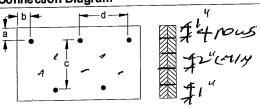
File Name: HOLLAND 8E.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B{

Specifier: Designer:

Company: Misc:

Connection Diagram



a minimum = ? b minimum = 3" d = 💯

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 5405 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11(i2000)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:03:54



BC CALC® Design Report

Build 5033 Job Name:

Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

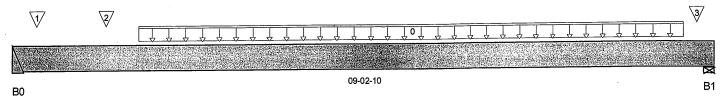
Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i2000)

Specifier:

Designer: AJ

Company:

Misc:



Total Horizontal Product Length = 09-02-10

Reaction Summary (Down / Uplift) (Ibs)								
Be aring	Live	De ad	Snow	Wind				
B0	1,336 / 0	543/0						
B1.6-1/4"	1,419/0	577/0						

	ad Cumman					Live	Dead	Snow	Wind	Trib.
Load Summary Tag Description		Load Type	Ref	f. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	01-07-14	08-09-14	299	112			n/a
1	J2(i1987)	Conc. Pt. (lbs)	L	00-03-14	00-03-14	184	69			n/a
2	,	Conc. Pt. (lbs)	L	01-02-12	01-02-12	329	123			n/a
3	J5(i1957)	Conc. Pt. (lbs)	Ĺ	08-11-14	08-11-14	96	36			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5,846 ft-lbs	25,408 ft-lbs	23%	1	04-03-14
End Shear	2,398 lbs	11,571 lbs	20.7%	1	07-10-14
Total Load Defl.	L/999 (0.11")	n/a	n/a	4	04-05-14
Live Load Defl.	L/999 (0.078")	n/a	n/a	5	04-05-14
Max Defl.	0.11" `	n/a	n/a	4	04-05-14
Span / Depth	10.9	n/a	n/a		00-00-00

				De mand/ Resistance		
Bearin	ng Supports	Dim.(L x W)	Demand	Support	Member	Material
B0	Hanger	2" x 3-1/2"	2,683 lbs	n/a	31.4%	HGUS410
B1	Wall/Plate	6-1/4" x 3-1/2"	2,849 lbs	24.4%	10.7%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

CONFORMS TO OBC 2012

Importance Factor: Normal Part code: Part 9



DWG NO. TAM 5406.12 STRUCTURAL COMPONENT ONLY



Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B11(i2000)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:03:54

BC CALC® Design Report



Build 5033

Job Name: Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

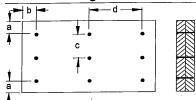
Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i20(

Specifier:

Designer: AJ Company.

Misc:

Connection Diagram



a minimum = 2" b minimum = 3" c = 2-3/4"

Calculated Side Load = 445.6 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Box Nails

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 5406-15 STRUCTURAL COMPONENT ONLY



Boise Cascade Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B12(i2139)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 09:03:56

Build 5033

Job Name:

Address: City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

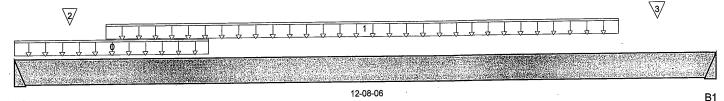
Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i2139)

Specifier:

Designer: AJ

Company:

Misc:



B0

Total Horizontal Product Length = 12-08-06

Reaction Summary (Down / Bearing	Uplift) (lbs)	De ad	Snow	Wind	
B0	1,163 / 0	559/0			
B1	535/0	245/0			

	C					Live	Dead	Snow	Wind	Trib.
Load Summary Tag Description		Load Type	Ref. Start E	En d	1.00	0.65	1.00	1.15		
<u></u>	STAIR	Unf. Lin. (lb/ft)	L	00-00-00	03-06-06	240	120			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	Ē	01-07-14	10-11-14	72	27			n/a
2	J5(i2079)	Conc. Pt. (lbs)	Ī.	00-11-14	00-11-14	91	34			n/a
3	.15(i1984)	Conc. Pt. (lbs)	Ĺ	11-07-14	11-07-14	88	33			n/a

	Factore d	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	4.586 ft-lbs	12,704 ft-lbs	36.1%	1	04-11-14
End Shear	1,949 lbs	5,785 lbs	33.7%	1	00-11-08
Total Load Defl.	L/413 (0.363")	0.624"	58.1%	4	05-11-14
Live Load Defl.	L/604 (0.248")	0.416"	59.6%	5	05-11-14
Max Defl.	0.363"	n/a	n/a	. 4	05-11-14
Span / Depth	15.8	n/a	n/a		00-00-00

Bearing Supports		Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material	
B0	Hanger	2" x 1-3/4"	2,444 lbs	n/a	57.2%	HUS1.81/10	
B1	Hanger	2" x 1-3/4"	1,110 lbs	n/a	26%	HUS1.81/10	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-00-00, Bottom: 00-00-00.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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CONFORMS TO OBC 2012



Boise Cascade Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B13(i2082)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

November 24, 2017 10:02:13

Build 5033

Job Name:

Address:

City, Province, Postal Code: EAST GWILLIMBURY,

Customer:

Code reports:

CCMC 12472-R

File Name: HOLLAND 8E.mmdl

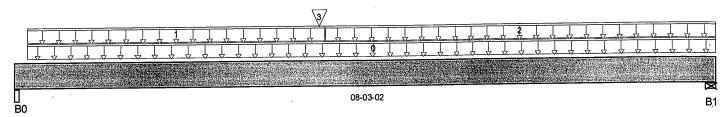
Description: Designs\Flush Beams\1st Floor\Flush Beams\B13(i2082`

Specifier:

Designer: AJ

Company:

Misc:



Total Horizontal Product Length = 08-03-02

Reaction Summary (Down	/ Uplift) (lbs) Live	De ad	Snow	Wind
B0, 3-1/2"	395/0	195/0		
B1, 2-3/8"	274/0	142/0		

	I O					Live	Dead	Snow	Wind	i rib.
	ad Summary g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
'n	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-01-12	08-03-02	4	2			n/a
1	FC2 Floor Material	Unf. Lin. (lb/ft)	Ĺ	00-01-12	03-07-12	22	8			n/a
2	FC2 Floor Material	Unf. Lin. (lb/ft)	Ē	03-07-12	08-03-02	3	1			n/a
3	B12(i2139)	Conc. Pt. (lbs)	L	03-06-14	03-06-14	542	249			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	2,450 ft-lbs	12,704 ft-lbs	19.3%	1	03-06-14
End Shear	781 lbs	5,785 lbs	13.5%	1	01-01-00
Total Load Defl.	L/999 (0.063")	n/a	n/a	4	03-11-13
Live Load Defl.	L/999 (0.042")	n/a	n/a	5	03-11-13
Max Defl.	0.063"	n/a	n/a	4	03-11-13
Span / Depth	10	n/a	n/a		00-00-00

Rearin	ng Supports	Dim.(L x W)	Demand	De mand/ Resistance Support	De mand/ Resistance Member	Material
B0	Beam	3-1/2" x 1-3/4"	837 lbs	12.5%	11.2%	Un specified
B1	Wall/Plate	2-3/8" x 1-3/4"	587 lbs	26.5%	11.6%	Un specified

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

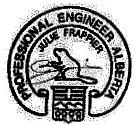
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CONFORMS TO OBC 2012



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







				Bare			1/2" Gyp	sum Ceiling	
Depth	Series			tre Spacing				tre Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-1 17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
L4"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
6"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	21 -5 22'-5"	20 - 6 21' - 5"	N/A
.0	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"		N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A N/A	24 -8 25'-4"	23'-5"	21'-9"	N/A
					IV/A	23 -4	23 -5	22'-4"	N/A

D	Mid-Span Blocking						Span Blocking a	nd 1/2" Gypsum	n Ceiling	
Depth	Series			tre Spacing		On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A	
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A	
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A	
	NI-70	19 '- 2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A	
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A	
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A	
11-7/8"	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A	
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A	
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A	
	NI-80	22 '- 9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A	
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A	
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A	
	NI-60	24'-0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A	
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A	
	NI-80	25' - 7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A	
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24' - 11"	23' - 9"	N/A	
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"		
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	24 -2 25'-2"	N/A	
	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"		N/A	
	NI-90x	29'-0"						25'-6"	N/A N/A	
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"		

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists. 3. Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			B	are		1/2" Gypsum Ceiling				
Depth	Series		On Cent	re Spacing			On Cen	tre Spacing		
		12"	16"	19.2"	24"	12"	16"	/ 19.2"	24"	
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"	
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"	
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"	
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"	
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"	
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"	
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"	
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"	
11-7/0	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"	
	NI-80	21'-1"	19' - 5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"	
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"	
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"	
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"	
14"	NI-70	23'-0"	21'-3"	20'-3"	19' - 2"	23'-8"	21'-11"	20'-10"	19'-9"	
	N1-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"	
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"	
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"	
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22'-9"	21'-6"	
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"	
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"	

		Mid-Span Blocking				Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling	
Depth	Series		On Cent	re Spacing		On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"	
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"	
	NI-80	20' - 3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"	
11-7/8"	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"	
11-7/0	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22 '- 3"	21'-2"	19'-9 "	
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21' - 5"	20'-0"	
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20' - 9"	
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"	
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23' - 8"	22'-4"	20'-10"	
14"	NI-70	26'-1"	24'-3"	23' - 2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"	
	NI-80	26'-6"	24'-7"	23' - 5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"	
	NI-90x	27' - 3"	25'-4"	24'-1"	22 '- 9"	27'-9"	25'-11"	24'-8"	23'-4"	
	NI-60	27' - 3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"	
16"	NI-70	28'-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"	
10	NI-80	29'-1"	27'-0"	25'-9"	24'-4"	29'-8"	27'-9"	26' - 5"	25'-0"	
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25'-8"	

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			E	Bare	· · · · · · · · · · · · · · · · · · ·	1	1/2" Gypsum Ceiling On Centre Spacing				
Depth	Series		On Cen	tre Spacing							
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A		
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A		
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A		
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A		
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A		
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A		
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A		
11-7/8"	NI-60	18'-4"	17'- 3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A		
11-7/0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A		
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A		
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A		
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A		
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19' - 7"	18'-9"	N/A		
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19' - 8"	N/A		
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A		
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A		
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A		
16"	NI-70	23'-6"	21'-9"	20' - 9"	N/A	24'-3"	22'-5"	21'-5"	N/A		
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A		
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A		

			Mid-Spa	n Blocking	Mid-S	Mid-Span Blocking and 1/2" Gypsum Ceiling					
Depth	Series		On Cent	re Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A		
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A		
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A		
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A		
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A		
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A		
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A		
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A		
	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A		
	NI-80	22' - 9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A		
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A		
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A		
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22 '- 5"	21'-0"	N/A		
14"	NI-70	25'-3"	23'-4"	22' - 3"	N/A	25'-10"	24'-0"	22'-9"	N/A		
	NI-80	25' - 7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A		
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A		
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A		
16"	NI-70	27'-9"	25 '- 8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A		
	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A		
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A		

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			E	Bare		1/2" Gypsum Ceiling				
Depth	Series		On Cent	tre Spacing				tre Spacing	* *	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"	
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"	
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"	
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"	
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"	
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"	
	Ni-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"	
11-7/8"	NI-60	19'-7"	18' - 2"	17'-5"	16' - 9"	20'-2"	18' - 9"	17'-11"	17'-1"	
11 7/0	NI-70	20'-9"	19'-2"	18'-3"	17' - 5"	21'-4"	19'-9"	18'-10"	17'-10"	
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"	
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"	
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"	
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"	
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"	
	NI-80	23' - 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"	
	NI-90x	24'-1"	22' - 3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"	
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"	
16"	NI-70	25' - 1"	23'-2"	22'-0"	20'-10"	25' - 9"	23'-10"	22'-9"	21'-6"	
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"	
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"	

			Mid-Spa	n Blocking		Mid-Span Blocking and 1/2" Gypsum Ceiling					
Depth	Series		On Cent	re Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"		
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"		
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"		
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"		
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"		
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"		
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"		
11-7/8"	NI-60	21'-9"	19' - 8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"		
11 7/0	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"		
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"		
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22' - 7"	21' - 3"	19'-7"		
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"		
	NI-60	24' - 9"	22' - 5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"		
14"	NI-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"		
	NI-80	26' - 6"	24'-7"	23' - 3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"		
	NI-90x	27'-3"	25'-4"	24'-1"	22'-4"	27' - 9"	25'-10"	24'-3"	22'-4"		
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"		
16"	NI-70	28'-8"	26'-8"	25' - 3"	23' - 4"	29'-3"	26'-11"	25'-3"	23'-4"		
10	NI-80	29'-1"	27'-0"	25'-9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"		
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28'-5"	26' -11 "	24'-10"		

^{1.} Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

^{2.} Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

^{3.} Minimum bearing length shall be 1-3/4 inches for the end bearings.

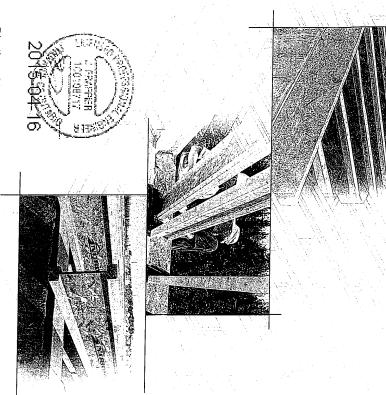
^{4.} Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

^{5.} This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

^{6.} Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

NSTALLATION GUIDE

FOR RESIDENTIAL FLOORS



Distributed by:



SAFETY AND CONSTRUCTION PRECAUTIONS

WARNING



N-C301 / November 2014

Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



concentrated loads from Once sheathed, do not over-stress I-joist with materials over unsheathed I-joists. Never stack building building materials.

Avoid Accidents by Following these Important Guidelines:

Lipoists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied Brace and nail each Lipist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When Lipists are applied continuous over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support. to prevent I-joist rollover or buckling.

- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails tastened to the top surface of each Hoist. Nail bracing over at least two I-joists. the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic Hoists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

Never install a damaged I-joist.

STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- 2. Store, stack, and handle I-joists vertically and level only.
- 3. Always stack and handle Ljoists in the upright position only.
- 4. Do not store I-joists in direct contact with the ground and/or flatwise
- Protect I-joists from weather, and use spacers to separate bundles.
- 7. When handling I-joists with a crane on the job site, take a few Bundled units should be kept intact until time of installation simple precautions to prevent damage to the I-joists and injury
- Pick 1-joists in bundles as shipped by the supplier
- ■Orient the bundles so that the webs of the I-joists are vertical.
- Pick the bundles at the 5th points, using a spreader bar if necessary.
- 8. Do not handle I-joists in a horizontal orientation.
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED 1-JOIST





MAXIMUM FLOOR SPANS

- . Maximum clear spans applicable to simple-span or live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L \pm or more of the adjacent span. 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. For multiple-span applications, the end spans shall be 40% multiple-span residential floor construction with a design
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive of gypsum and/or a row of blocking at mid-span. assumed. Increased spans may be achieved with the used Standard. No concrete topping or bridging element was shall meet the requirements given in CGBS-71.26
- 3. Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- 4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- 5. This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. SI units conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

SIMPLE AND MULTIPLE SPANS MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

Joist	Joist		Simple	spans			Multipl	e spans
Depth	Series		On centr	e spacing			On centr	e spacin
		12"	16"	19.2	24"	12"	16 ⁿ	19.2
	- -)(- -)(-	651	14:21	313'9" =	13:5		1514	a) 4:4[0
		16:3	15.4"	14-10	14111	177	1657	16.0
	10	7731	16:31	15-6	15:27	2.91	17.4	16-91
		16,111	16.0	15.5	15-6	1844	17.3	16-8
		181	1713	16:7	16-9	20-0	18-6	17.9
		19.6	18-0	17.4	17.6	21.6	19:17:	19-0
		2012	18:71	17-10		2213	202	9-3
		*20'4"	18:9"	17-11	18:01	22:5	20.9	19-10
		20:1	18:7:	17-10	17.11	22:2	20.6	19-8
		21.7	20-0	19'11	19:2	23.0	2211	20-0
		32	20.3	194	19.5	24.9	22.5	21.5
		20.7	20111	19-111	20'0	25.0	29-1	22:0
	34		20.8	19.9		24.7	22:9	21:9
		23.11*	22.1	2111	21.2	26.5	24-5) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1
		24.5	22.6	21:5	21-6	26:11	24-10	23-9
Name of the second	STOY OF STREET	24-0	72.4		21/210	27-3 F	25-2	24-0

4. Web stiffeners are required when the

brace the top flange of the I-joist sides of the hangers do not laterally Hangers should be selected based

and load capacity based on the

maximum spans.

on the joist depth,

flange width

2. All nailing must meet the hanger

to support I-joists.

manufacturer's recommendations.

I-JOIST HANGERS

Hangers shown illustrate the three

most commonly used metal hangers

ee	Series		CH COLL				On centre	Specine	
		12"	16"	19.2	24"	12"	16"	19.2	24"
		1.51	14:2:	13:9"	13-5	16:31	154	14:10	17.4
		16:35	1514	14-10		77.70	10.5	15-10	5
			16-11	15-6	15:77	10.75	77.4	16.01	7 0
		17.3°	16.3	115'-8"	15-9	18410	17-6	16-11	17.0
			0.0	15.5	15-6	1814	11713	. B-91	16'7"
		18-4	17.3	-16-7	16-9	20.3	18:9	18-0	18-1
		7-6	18-0	174	176	2116	19:11:	19:01	19:1:
	a k	2062	18.71	77.10		20107	20.2	9	194
		20'4"	18:9	17:11	18:01	22.5	20:9*	19-10	107
		20.5	81/	1/-10		22:2	20.6	19-8	19:4
		21:7	20.0	19:11	19:2	23/10	2211	21-11	5 Z
			20.3	19.4	1945	2413	22.5	21.6	21'-6"
		2017	20:11	19-11	20-0	25.0	23-1	22.0	20-10.
	. Y.	231.4	21.6	20-9	1910	24-7	22. 9	211-9	21-10
*		29.11	221	1.21:1" i	2112	26.5	24.5	23-3	234
		24-5	2246	21:5	21.6	26-11	24-10	23.9	23.9
			51 V	100	- 4 Part C 38	S. 71-0-01	7-C7	24-0	24-1

CCMC EVALUATION REPORT 13032-R

Top Mour

race Mount

WEB STIFFENERS

RECOMMENDATIONS:

- the stiffener and the flange is at the top. engineered applications with factored A bearing stiffener is required in all l-joist properties table found of the I-joist reactions greater than shown in the Construction Guide (C101).The gap between
- stiffener and flange is at the top. support, the top flange. The gap between the sides of the hanger do not extend up to, and the Lioist is supported in a hanger and the A bearing stiffener is required when
- and the flange is at the bottom. by the code. The gap between the stiffener adjusted for other load durations as permitted standard term load duration, and may be tip and the support. These values are for cantilever, anywhere between the cantilever between supports, or in the case of a than 2,370 lbs is applied to the top flange ■ A load stiffener is required at locations where a factored concentrated load greater

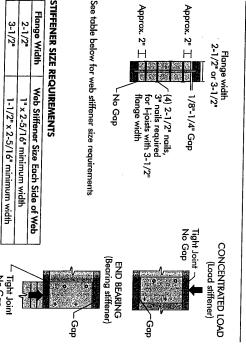
SI units conversion: 1 inch = 25.4 mm

3-1/2

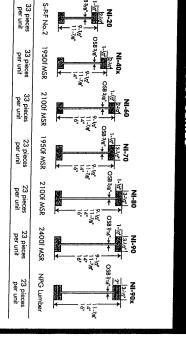
Tight Join

FIGURE 2

WEB STIFFENER INSTALLATION DETAILS



NORDIC I-JOIST SERIES



products to adhere to strict quality control procedures throughout the manutacturing process. Every phase of the operation, from forest to the Chantiers Chibougamau Ltd. harvests its own trees, which enables. Nortic finished product, reflects our commitment to quality.

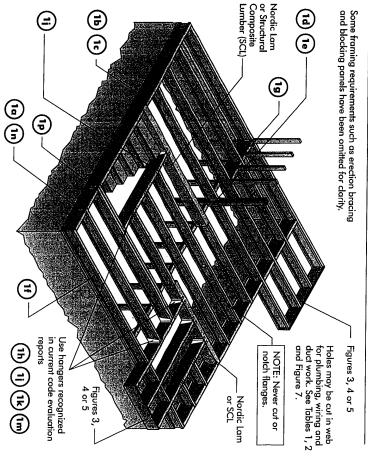
Nordic Engineered Wood I-joists use only finger-jointed back spruce longer span carrying capacity. lumber in their flanges, ensuring consistent quality, superior strength zing

015-04-16

INSTALLING NORDIC I-JOISTS

- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, லூல்க்லன்
- 2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment
- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple அள்ளில் கூறாமுர்
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings 20450246
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header
- 8. Concentrated loads greater than those that can normally be expected in residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the Ljoist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the
- 9. Never install Ljoists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or I-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below
- 12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I joist blocking panels or other engineered wood products – such as rim board – must be cut to fit between the I-joists, and an -joist-compatible depth selected.
- 13. Provide permanent lateral support of the bottom flange of all 1-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered 1-joists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used
- 14. If square-edge panels are used, edges must be supported between 1-joists with 2x4 blocking. Glue panels to blocking to underlayment layer is installed minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS FIGURE 1

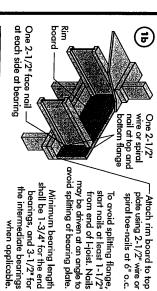


All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.

top plate per detail Attach I-joist to ⓓ NI blocking panel with same nailing plate (when used for lateral shear as required for transter, nail to 2-1/2" nails at bearing plate 6" o.c. to top decking)

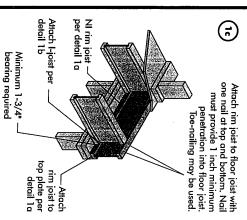
***************************************	NI Joists	Blocking Panel or Rim Joist
	3,300	Maximum Factored Uniform Vertical Load* (plf)

It shall not be used in the design of a bending member, inches or less and is based on standard term load duration such as joist, header, or ratter. For concentrated vertical The uniform vertical load is limited to a joist depth of 16



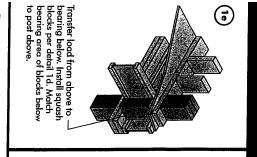
1-1/8" Rim Board Plus	Blocking Panel or Rim Joist
8,090	Maximum Factored Uniform Vertical Load* (pH)

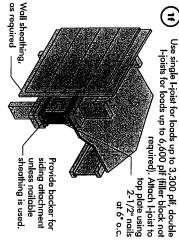
or less and is based on standard term load duration. It shall not be used in the design of a bending member, such as joist, header, or *The uniform vertical load is limited to a rim board depth of 16 inches rafter. For concentrated vertical load transfer, see detail 1d.



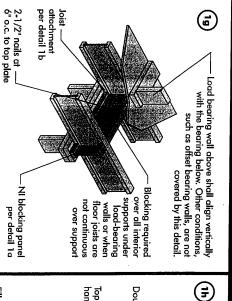
				_	
1-1/8"	2x Lumber		Pair	Squash block	a
1-1/8" Rim Board Plue	ber		Pair of Squash Blocks		NI or rim board blocking panel per detail 1a
1 200	5,500	3-1/2" wide	Maximum Factored Vertical per Pair of Squash Blocks (lbs)		`
000 7	8,500	5-1/2" wide	ored Vertical per h Blocks (lbs)		1/16" for 'squash blocks

Provide lateral bracing per detail 1a, 1b, or 1c	1 1/0 Mill board 1108
oracing per	100
detail 1a,	4,300
lb, or lc	-
	0,600





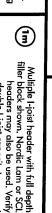
Rim board may be used in lieu of Ljoists. Backer is not required when rim board is used. Bracing per code shall be carried to the foundation.

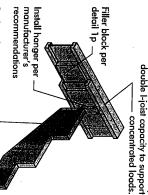




 \equiv

Nordic Lam or SCL

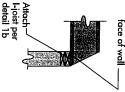




clinch when possible. Backer block attached per detail 1h. Nail with twelve 3" nails,

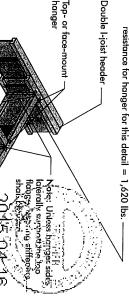
Maximum support capacity = 1,620 lbs





support, not shown for clarity. Note: Blocking required at bearing for lateral

additional 3" nails through the webs and filler block where the Before installing a backer block to a double I-joist, drive three Backer block (use if hanger load exceeds 360 lbs) backer block will fit. Clinch. Install backer tight to top flange. Use twelve 3" nails, clinched when possible. Maximum factored



per detail 1p Filler block (both sides for face-mount hangers) Backer block required 20个99年16

/erify double I-joist capacity to support concentrated loads For hanger capacity see hanger manufacturer's recommendations.

BACKER BLOCKS (Blocks must be long enough to permit required railing without splitting)

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	"	5-1/2"
3-1/2"	1-1/2"	7-1/4"

- better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard. Minimum grade for backer block material shall be S-P-F No. 2 or
- For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4"
- to lumber piece, of adjacent web. Lumber 2x4 min., extend block to face nails from each web Two 2-1/2" spiral NI blocking **(3**) board -One 2-1/2" nails at top and bottom flange One 2-1/2" nails one side only lumber piece īwo 2-1/2" nails from each web to I-joist blocking panel -2x4 min. (1/8" gap minimum)

2-1/2" nails at 6" o.c.

panel

- In some local codes, blocking is prescriptively required in for spacing of the blocking the starter joist. Where required, see local code requirements the first joist space (or first and second joist space) next to
- All nails are common spiral in this detail

Notes:

€

Filler block

stitteners shall be used

Note: Unless hanger sides laterally

beams, see the manufacturer's For nailing schedules for multiple

installed per manufacturer's recommendations

Top- or face-mount hanger -

support the top flange, bearing

support the top flange, bearing Note: Unless hanger sides laterally manufacturer's recommendations Top-mount hanger installed per _

- 1. Support back of I-joist web during nailing to prevent damage to web/flange connection
- 2. Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist
- 3. Filler block is required between joists for full length of span.
- 4. Nail joists together with two rows of 3" nails at 12 inches o.c. (clinched when possible) on each side of the double I-join are required. can be clinched, only two nails per foot Total of four nails per foot required. If na

Offset nails from opposite face by 6"

-1/8" to 1/4" gap between top flange The maximum factored load that may be using this detail is 860 lbf/ft. Verify double applied to one side of the double joist

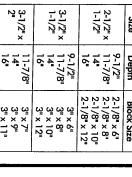
and filler block

FILLER BLOCK REQUIREMENTS FOR DOUBLE I-JOIST CONSTRUCTION

(

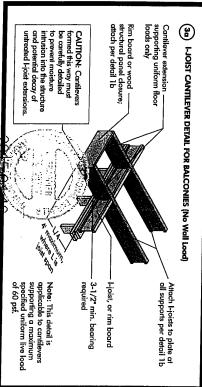
=				
ס	Flange Size	Joist Depth	Filler Block Size	
	2-1/2"×	9-1/2" 11-7/8"	2-1/8" × 6" 2-1/8" × 8"	
	1-1/2"	14 <u>.</u>	2-1/8" x 10" 2-1/8" x 12"	
₫.	3-1/2"×	9-1/2" 11-7/8"	သူ သူ × နူ ဧ	
₩.	1-1/2"	14" 16"	3" × 10"	
	3-1/2"×	11-7/8" 14"	3" x 7" 3" x 9"	
	١	ا و	3" × 11"	

opposite side. alternate on



line or 1/2 inch minimum gypsum ceiling Optional: Minimum 1x4 inch _____ strap applied to underside of joist at blocking attached to underside of joists

CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)



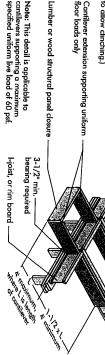
(y LUMBER CANTILEVER DETAIL FOR BALCONIES (No Wall Load)

Full depth backer block with 1/8" gap between block and top flange of I-joist. See detail 1 h. Nail with 2 rows of 3" nails at 6" o.c. and clinch.

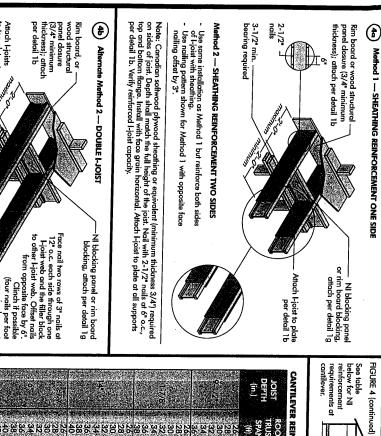
2x8 min. Nail to backer block and joist with 2 rows of 3" nails at 6" o.c. and clinch. (Cantilever nails may be to allow clinching.) used to attach backer block if length of nail is sufficient (Cantilever nails may be plate at all supports per detail 1b Attach I-joists to

floor loads only Cantilever extension supporting uniform

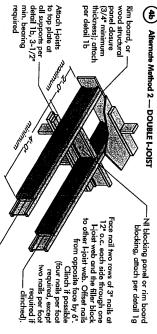
cantilevers supporting a maximum specified uniform live load of 60 psf Note: This detail is applicable to



CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" rails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.



Roof truss span

200 cantileve

> truss Girder Roof trusses

4 13'-0" maximum

Roof truss span

T 22-0 Jack trusses

maximum

requirements for a span of 26 ft. shall be permitted to trusses running parallel to the cantilevered floor joists, the I-joist reinforcement For hip roofs with the jack

(in.)	4 % TI	LL = 30 psf, DL = 15 psf JOIST SPACING (in.) 12 16 19.2 24)F LOADING (L LL = 40 psf, DI JOIST SPACII	(CTORED) 15 psf (in.)	LL = 50 psf, DI JOIST SPACII	5 psf
	286888 286888	(1) (2) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1		XXXX	N 2 2 X X X X X X X X X X X X X X X X X	××××
II.7/B	9 9 9 9 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	ZZZZZ	-zzz)		XX	<×××××
Latin in	88.588888 88.588888		Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z	₩ ₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩₩	ZZZZZZZ 	×===aaada>
6	8 2 2 3 2 3 6 4 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7	ŽZZZZŽ			ZZZZZ-	vx:
	38 40 42	ZZZZ	22 Z Z	7 2 2	471.69	<n n="" n<="" td=""></n>

- N = No reinforcement required.
 N is einforced with 3/4 wood structural penel on one side only.
 Penel on one side only.
 N is einforced with 3/4 wood structural penel on both sides, or double I-joist.
 Penel on both sides, or double I-joist.
 X = Try a deeper joist or closer spacing.
 Mosimum design load shall be: 15 pst froot both load, and 80 plf wall load. Wall load is based on 3/0 plf wall load. Wall load is based on 3/0.

Block Hoists together with filler blocks for the full length of the reinforcement. For Hoist flange widths greater than 3 inches place an additional row of 3° nails along the centreline of the reinforcing panel from each side. Clinch when possible.

- studs may be required.

 3. Table applies to joists 1.2* to 2.4* o.c. that meet the floor span requirements for a design live load of 40 pet and dead load of 15 pet, and a live load deflection limit of (/480, Use 12* o.c. requirements for lesser spacing. For larger openings, or multiple 3'.0" width openings spaced less than 6'.0" o.c., additional joists beneath the opening's cripple
- 4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge board, the Roof Truss Span is equivalent to the distance between the supporting walls as if a
- truss is used.

 Cantilevered joists supporting girder trusses or roof beams may require additional

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively
- 5 I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. The maximum size hole or the maximum depth of a duct chase opening that can between the top or bottom of the hole or opening and the adjacent l-joist flange. be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained
- Ģ The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- ٥. Where more than one hole is necessary, the distance between adjacent hole size of the largest square hole (or twice the length of the longest side of the langest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of edges shall exceed twice the diameter of the largest round hole or twice the Tables 1 and 2, respectively
- .7 A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to
- % A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- 10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Service of the control of the contro	Joist Depth	Joist Series	2	1	Î	t num d	listan 5	ce fro	m in: Ro	side f und l	ace a	5 H	ny supp neter (i 8-5/8	n.) ort to	ceni	ire o	of hole	
		1910	90					66	640						1;	320		1
		oje)	287					166	: 23	4				Ŧŧ	11			111
		je e	99					66			200		. 9					I
		lajai.	i 6											H	11			ΗŢ
		a) a (e	198								28	130	1 G	ii.		70		
9 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		(ele)	23	11.76							***			886	ę8			
		la/ara						o e je	i i i		1		60		ŦŃ			
		e) ai al	100						9006 8006	a de la				970 975	198 198			

Above table may be used for Lioist spacing of 24 inches on centre or less.

Hole location distance is measured from inside face of supports to centre of hole.

Distances in this chart are based on uniformly loaded joists.

Dreduced = Lactual × D

Where:

Lactual 11

₽¥

OPTIONAL:

The above table is based on the Hoists used at their maximum span. If the Hoists are placed at less than their full maximum span (see Maximum Ficor Spans). The minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

Dreduced =

Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications (fit. The reduced distance shall not be less than 6 inches from the face of the support to edge of the hole. The actual measured span distance between the inside faces of supports (ft)

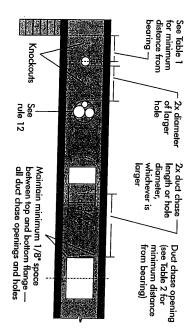
Span Adjustment Factor given in this table.

The minimum distance from the inside face of any support to centre of hole from this table actual is greater than 1, use 1 in the above calculation for Lactua

2015-04-1 ๑

FIGURE 7

FIELD-CUT HOLE LOCATOR



and may be ignored for purposes of calculating minimum distances A knockout is **NOT** considered a hole, may be utilized wherever it occurs

> spaced 15 inches on centre along the length of the I-joist. Where possible, it is are 1-1/2 inches in diameter, and are preferable to use knockouts instead of electrical or small plumbing lines. They for the contractor's convenience to install Knockouts are prescored holes provided



should be cut with a Holes in webs over-cut the web

Never drill, cut or

and then making the cuts between the holes is another good method to the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners For rectangular holes, avoid over-cutting ninimize damage to the I-joist

TABLE 2

DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only

Joist	Joist	Minimu	m distan	ce from in	side face	of any	upport t	o centre :	of openin	i firm
Depth	Series				Duct ch	ıase lenç	Jth (in.)			
	2847 (N. 127) (N. 127)	8	70	12	14	16	18	20	22	24
		51.4	5.8	4410	251	8.5	63.	19:9	Z-1	7.5
	d.	. (c.	5.9	6.2	6/7		166	866	8-3 3-2	8.9 6.8
		5.4	5.6	610	6.5	6.0		7.6	801 1	0.00
			56.6			7.5	7.9	9,3	8.91	24.0
	1		8	9-0		90		0 0 0 0		10.9
	17			8.0			9	0 0 0 6	10-1	
				815		9.2	8.8	10:1:	107	10
		8.9.	900	9.8	7.6		.;g-z:		0.21	12-8
		0.00	0.0	9.5	9510		10:8	11/2	11.7	12:3
			9 0 9 0 9 0		10-6	10:11	15	11.00	12.4	12K
	isl		0.8	- - - - - -		1251	12 6	13.2	4	1410
		100	10.5	3	9181-9 1918-9	12:1	2		13-8	1414
不是是现代的					i i	2-0		13-6	1412	14.70

12:10 12:2" "13:9" 14:4" 15:2"

. Above table may be used for I-joist spacing of 24 inches on centre or less.

Duct chase opening location distance is measured from inside face of supports to centre of opening.

The above table is based on simple-span joists only. For other applications, contact oyour local distributor,

Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of I/480. For other applications, contact your local distributor.

BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

FIGURE 5 (continued)

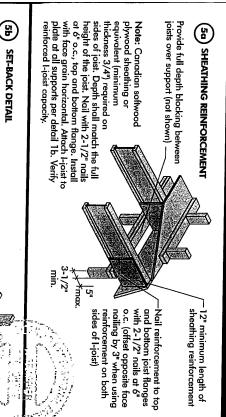
See table

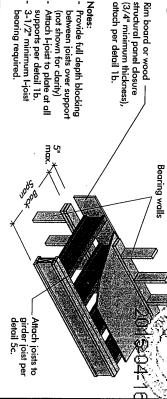
below for NI requirements at reinforcement

> Roof truss. span

> > 7 2'-0"

cantilever.





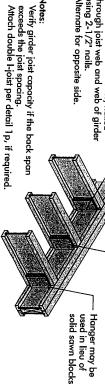
attach per detail 1b.

Rim board or wood

50 SET-BACK CONNECTION

Nail joist end using 3" bottom flanges. nails, toe-nail at top and

through joist web and web of girder using 2-1/2" nails. Alternate for opposite side. (2x6 S-P-F No. 2 or better) nailed Vertical solid sawn blocks –



1. N = No reinforcement required.

1 = NI reinforced with 3/4" wood structural

2. Maximum design load shall be: 15 psf roof X = Try a deeper joist or closer spacing. panel on one side only. 2 = NI reinforced with 3/4ⁿ wood structural wall load. Wall load is based on 3'-0" dead load, 55 psf floor total load, panel on both sides, or double I-joist.

maximum width window or door openings.

openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple For larger openings, or multiple 3'-0" width

studs may be required.

Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 pst and dead load of 15 pst, and a live load deflection limit of 1/480. Use 12" o.c. requirements for lesser spacing.

L-maximum -5" maximum cantilever Girder : Roof trusses Roof truss span <u></u> 13′−0″ maximum 2<u>1</u> - maximum cantilever Jack trusses 5" maximum

> the I-joist reinforcement the cantilevered floor joists, For hip roofs with the jack trusses running parallel to requirements for a span of 26 ft. shall be permitted to

BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

<u>7</u>			9 % 72 23	JOIST DEPTH (in.)
228 320 32 34 36 38 40	408 6/4 200 408 6/4 200	884 4 2 0 8 6	28 30 32 34	ROOF TRUSS SPAN (ff)
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NNNNN===	×22221	××××××××	××××	= 30 psf, DIST SPA
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******	×××××××	**************************************	××××××	psf) 24
zzzz	5 <u> Z</u>	プレルーニュ	××2225	ROOF L
××××××××	×××××	**************************************	×××××	OADING = 40 psf, OIST SPA
	<	****	X X X) (UNFAC DL = 15 CING (in 19.2
**************************************	**************************************	×××××	×××××	TORED) psf .) 24
222Z	, , , , , , , ,	-aaaax	××××2	1 2 _ L
××××××××××××××××××××××××××××××××××××××	××××××	·×××××	×××××	= 50 psi JOIST SP
******	××××××××××××××××××××××××××××××××××××××		*****	f, DL = 15 ACING (in 19.2
******	×××××××	·××××	×××××	psf .) 24

4. For conventional roof construction using a ridge beam, the Roof Truss Span column distance between the supporting walls as if a the supporting wall and the ridge beam.
When the roof is framed using a ridge board,
the Roof Truss Span is equivalent to the above is equivalent to the distance between russ is used

Cantilevered joists supporting girder trusses or oof beams may require additional reinforcing.

INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing
- Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- 3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single 1-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time a thinner line (1/8 inch) than used on I-joist flanges. before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying
- 8. Tap the second row of panels into place, using a block to protect groove edges
- Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common nail to assure accurate and consistent spacing.)
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations finished deck can be walked on right away and will carry construction loads without damage to the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the for cure time. (Warm weather acc elerates glue setting.) Use 2" ring- or screw-shank nails for panels

FASTENERS FOR SHEATHING AND SUBFLOORING(1)

24	20	16	Maximum Joist Spacing (in.)
3/4	5/8	5/8	Minimum Panel Thickness (in.)
2	2"	2"	No Common Wire or Spiral Nails
1-3/4"	1-3/4"	1-3/4"	iil Size and Ty Ring Thread Nails or Screws
2"	2	2	/pe Staples
6"	6"	6"	Maximum of Fast Edges
12"	12"	12"	a Spacing leners Interm. Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table
- 2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter.
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manufacturer.

Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5

IMPORTANT NOTE:

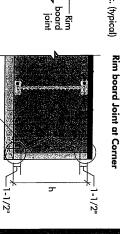
Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with

RIM BOARD INSTALLATION DETAILS

8 ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

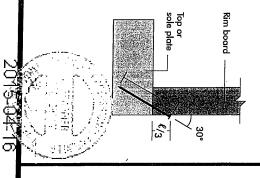
Rim board Joint Between Floor Joists 2-1/2" nails at 6"_o.c. (typical)





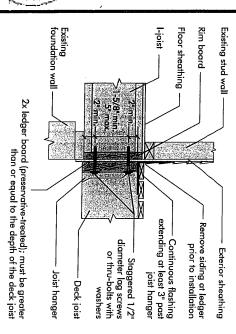
(B) TOE-NAIL CONNECTION AT RIM BOARD

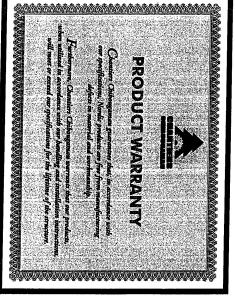
2-1/2" toe-nails at 6" o.c. (typical) —

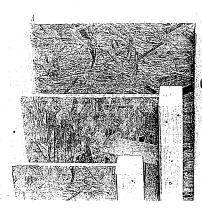


8 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL

Rim board joint -







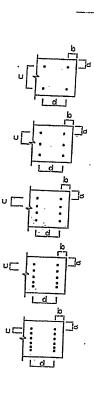
MICRO CITY

Engineering services inc.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

	CIVI LIEA	DED AND OF				
	LVL HEADER AND CONVENTIONAL LUMBER NAILING DETAILS					
	DETAIL NUMBER	NUMBER OF ROWS				
	. A	2	1 12			
	В	2	8			
	C	2	6			
	D	2	4			
A TOTAL STATES	1A	3	12			
4	1B	3	8			
•	1C	3	. 6			
•	1D	. 3:	4 .			
	2A	4	12			
	2B	4	8			
	2C	4	6			
	2D	4	4			
H	3A	5	12			
-	3B	5	. 8			
-	3C	5	6			
ŀ	3D	5	4			
╟	4A	6	12			
-	4B	6	8			
H	4C	6	6			
Ŀ	4D	6	4			



NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
 - (2) MINIMUM LUMBER END DISTANCE "b" = 2"
 - (3) MINIMUM NAIL ROW SPACING "c" = 2"
 - (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
 - (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
 - (6) DO NOT USE AIR-DRIVEN NAILS



DWG NO TÄNNIOO1. 14. STRUCTURAL COMPONENT ONLY TO BE USED ONLY WITH BEAM CALCS BEARING THE STAMP BELOWS

> PROVICE NAILING DETAIL Nº X'SEE 0W0 #TAM N1001-14