

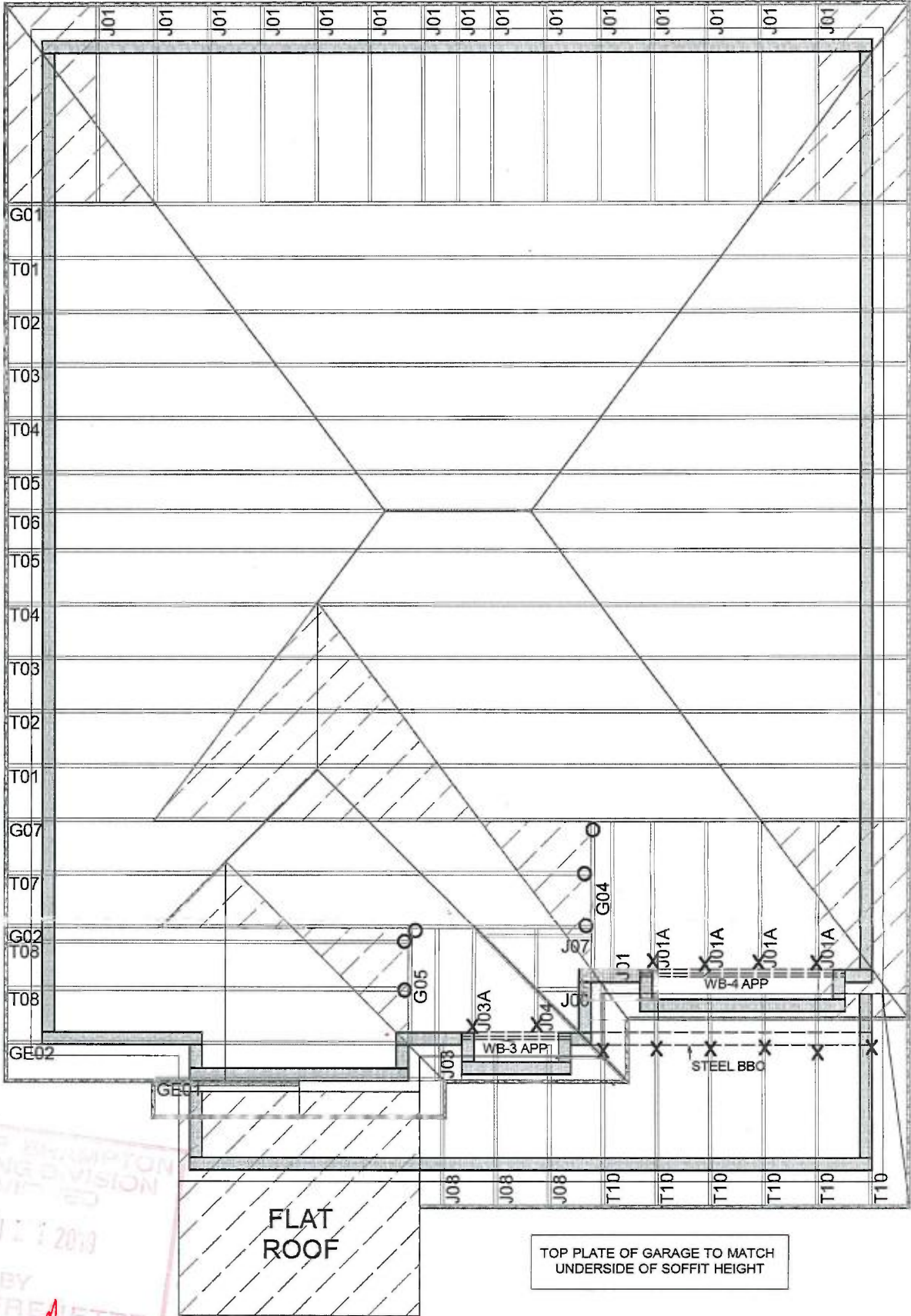
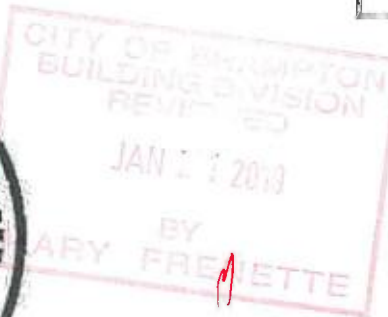


Layout based on plans provided by
JARDIN DESIGN GROUP INC
Dated April 26/18

This certification is for the engineering review of structural components shown on this drawing and is limited to the components supplied by KOTT for the loads and conditions shown on the calculation page of each component. The placement of these components on this layout is shown for reference only. Roof trusses must be installed according to manufacturer's guidelines at 24" C/C unless noted otherwise. Conventional framing shown on the layout must be designed and approved by the project engineer. Temporary and permanent bracing is to conform to BCSI guidelines.



September 14, 2018



For conventional framing roof / ceilings
framing shall conform to OBC.9.23

All work shall conform to the Ontario
Building Code O. Reg. 332/17 as amended

ROOF TRUSS LAYOUT

Lot 26



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS.
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	MILLWOOD 1 EL 3
Customer:	GREENPARK
Project:	MINNISALE HOMES
Location:	BRAMPTON
Drawn By:	DM
Date:	9/17/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS
MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTER THAT CROSS MEET
OVER TRUSSES TO BE 2X4 SPF @24" O/C WITH VERTICAL POST TO THE TRUSS BELOW EACH CROSS POINT.
VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT
AND ROWS OF BRACING DOES NOT EXCEED 6'.

18-413522-000-00 RR
Roof trusses

L&T 26

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

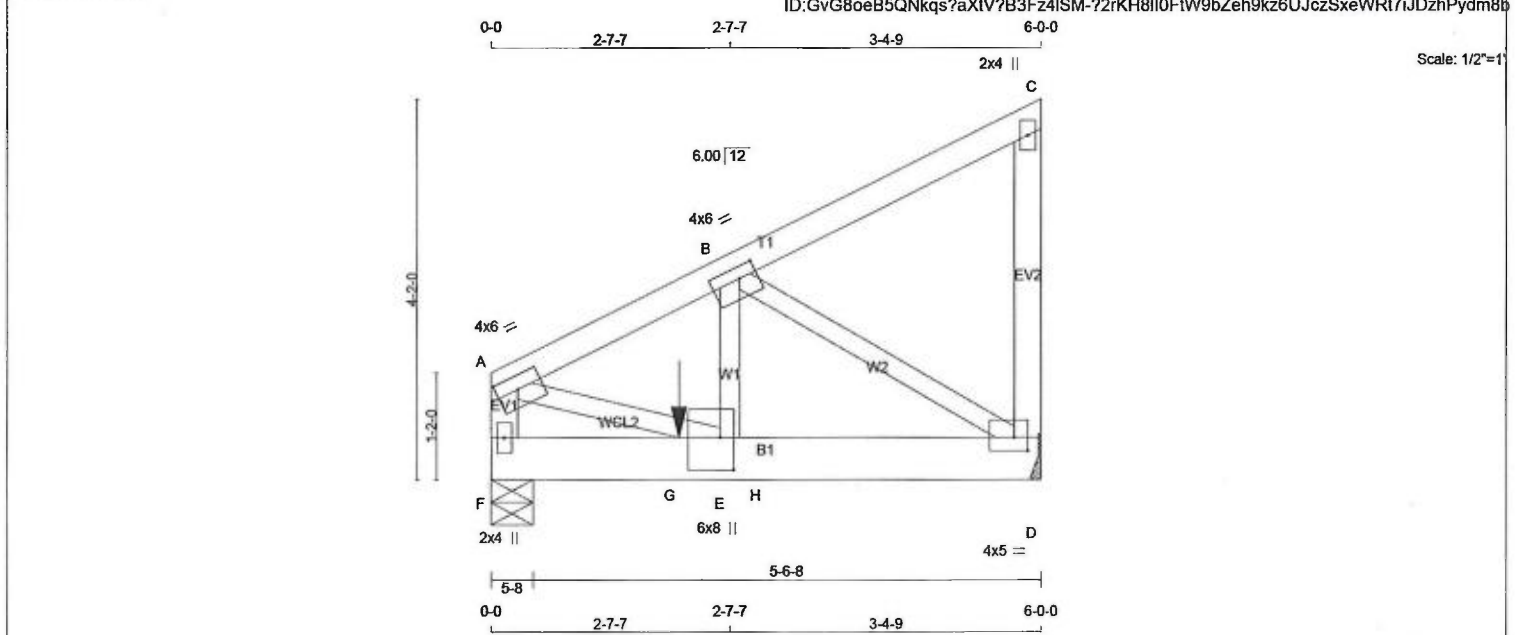
SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.





Version 8.210 S Mar 12 2018 M/Tek Industries, Inc. Fri Sep 14 10:04:08 2018 Page 1
ID: GvG8oeB5QNkqs?axIV7B3Fz4ISM-72rKH8l10fW9bZeh9kz6UJczSxeWR17iDzhPydm8b
Scale: 1/2"=1'
TOTAL WEIGHT = 29 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	4.0	6.0	1.75	2.75
B	TMVW-1	MT20	4.0	6.0	1.50	2.25
C	TMV+P	MT20	2.0	4.0		
D	BMVW-1	MT20	4.0	5.0	1.75	1.75
E	BMVW-1	MT20	6.0	8.0	4.25	1.75
F	BMV1+P	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1708.4 lbs FACTORED DOWN AT 2-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	1835 0	1835 0	0 0	MECHANICAL
F	1727 0	1727 0	5-8	2-15

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	DEAD	SOIL
D	1288	901 / 0	0 / 0	0 / 0
F	1212	848 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)
FR-TO		FR-TO				FR-TO			
A-B	-1938 / 0	-77.4	-77.4	0.13 (1)	4.71	E-B	0 / 1676		0.41 (1)
B-C	-7 / 0	-77.4	-77.4	0.13 (1)	10.00	B-D	-2045 / 0		0.54 (1)
D-C	-116 / 0	0.0	0.0	0.03 (1)	7.81	A-E	0 / 1818		0.45 (1)
F-A	-1467 / 0	0.0	0.0	0.16 (1)	6.71				
F-G	0 / 0	-17.5	-17.5	0.20 (1)	10.00				
G-E	0 / 0	-17.5	-17.5	0.20 (1)	10.00				
E-H	0 / 1741	-17.5	-17.5	0.28 (1)	10.00				
H-D	0 / 1741	-445.4	-445.4	0.28 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
G	2-0-12	-1708	-1708		BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.0	PSF
TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 3-0-0
START SPAN CARRIED = 20-6-0
END DISTANCE = 6-0-0
END SPAN CARRIED = 20-6-0
END WALL WIDTH = 5-8
APPLIED TO BACK SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.16/1.00 (A-F:1), BC=0.28/1.00 (D-E:1), WB=0.54/1.00 (B-D:1), SSI=0.99/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

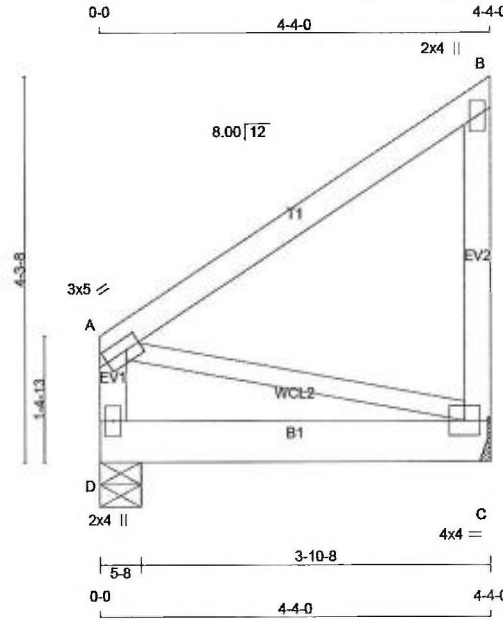
JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.52 (D) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:24.5

TOTAL WEIGHT = 22 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
A - B 2x4 DRY No.2
C - B 2x4 DRY No.2
D - A 2x4 DRY No.2
D - C 2x6 DRY No.2
ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A TMVW-1 MT20 3.0 5.0 1.50 2.00
B TMV+p MT20 2.0 4.0
C BMVW1-1 MT20 4.0 4.0
D BMV1+p MT20 2.0 4.0

DESGR.
SPF
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
C	781	0	781	0
D	781	0	781	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
JT	548	384 / 0 0 / 0 0 / 0 0 / 0 165 / 0 0 / 0
C	548	384 / 0 0 / 0 0 / 0 0 / 0 165 / 0 0 / 0
D	548	384 / 0 0 / 0 0 / 0 0 / 0 165 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	FACTORED	FACTORED	FACTORED	WEBS	FACTORED	FACTORED
	MEMB.	FORCE	VERT. LOAD	MAX	MEMB.	FORCE	MAX
		(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)
FR-TO			FROM TO		FR-TO		
A-B	0 / 0	-77.4	-77.4 0.27 (1)	10.00	A-C	0 / 0	0.00 (1)
C-B	-168 / 0	0.0	0.0 0.04 (1)	7.81			
D-A	-168 / 0	0.0	0.0 0.02 (1)	7.81			
D-C	0 / 0	-283.3	-283.3 0.49 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStd Girder
START DISTANCE = 0-0
START SPAN CARRIED = 13-8-0
END DISTANCE = 4-4-0
END SPAN CARRIED = 13-8-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/959$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/770$ (0.07")

CSI: TC=0.27/1.00 (A-B-1), BC=0.49/1.00 (C-D-1),
WB=0.00/1.00 (A-C-1), SS=0.41/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

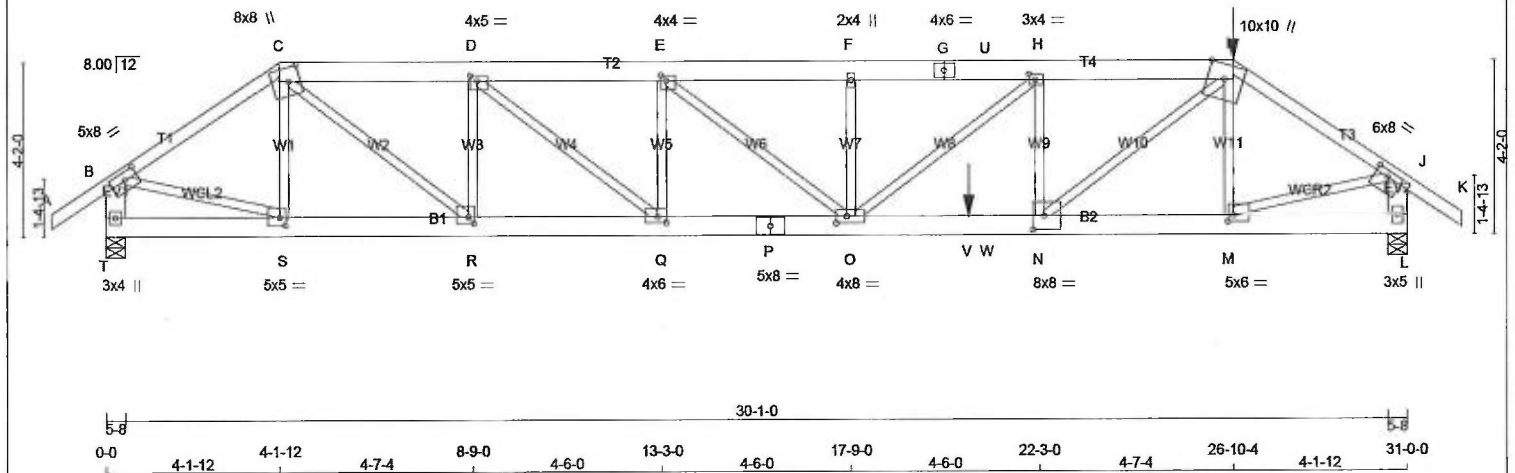
JSI GRIP= 0.15 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)



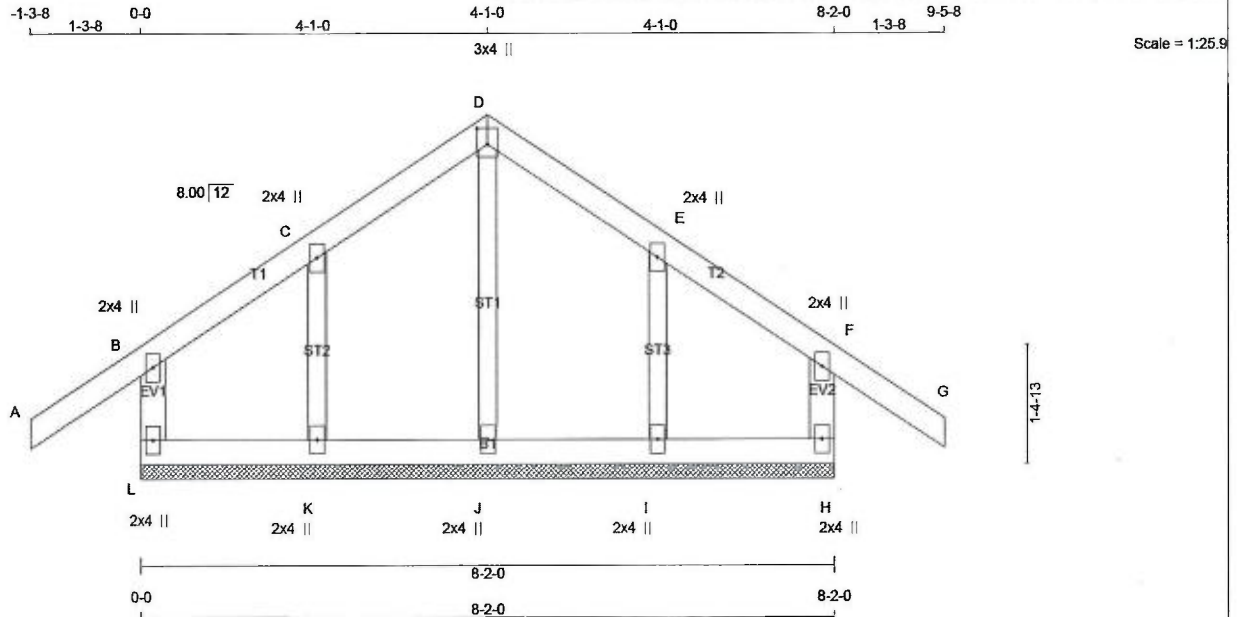
September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA				
N. L. G. A. RULES				DESIGNER										*** SPECIAL LOADS ANALYSIS ***				
CHORDS		SIZE	LUMBER	DESCR.	BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD		GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.			
							GROSS REACTION		GROSS REACTION		BRG		BRG		LOADS WERE DERIVED FROM USER INPUT			
A - C	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	NO FURTHER MODIFICATIONS WERE MADE					
C - G	2x6	DRY	No.2	SPF	T	2343	0	2343	0	0	5-8	2-9						
G - I	2x6	DRY	No.2	SPF	L	3442	0	3442	0	0	5-8	3-12						
I - K	2x4	DRY	No.2	SPF	UNFACTORED REACTIONS										SPECIFIED LOADS:			
T - B	2x6	DRY	No.2	SPF	1ST LCASE										TOP CH. LL = 23.3 PSF			
L - J	2x6	DRY	No.2	SPF	MAX/MIN										DL = 3.0 PSF			
T - P	2x6	DRY	No.2	SPF	JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	BOT CH. LL = 0.0 PSF					
P - L	2x6	DRY	2100F 1.8E	SPF	T	1642	1164 / 0	0 / 0	0 / 0	0 / 0	478 / 0	0 / 0	DL = 7.0 PSF					
ALL WEBS EXCEPT				SPF	L	2414	1695 / 0	0 / 0	0 / 0	0 / 0	719 / 0	0 / 0	TOTAL LOAD = 33.3 PSF					



TOTAL WEIGHT = 33 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS

SIZE

DRY

LUMBER

No.2

L - B

2x4

DRY

No.2

A - D

2x4

DRY

No.2

D - G

2x4

DRY

No.2

H - F

2x4

DRY

No.2

L - H

2x4

DRY

No.2

ALL WEBS

2x3

DRY

No.2

ALL GABLE WEBS

2x3

DRY

No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT

2-0-0 OC.

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

B

TMV+p

MT20

2.0

4.0

C

TMW+w

MT20

2.0

4.0

D

TTW+p

MT20

3.0

4.0

2.25

1.50

E

TMW+w

MT20

2.0

4.0

F

TMV+p

MT20

2.0

4.0

H

BMV1+p

MT20

2.0

4.0

I, J, K

I

BMW1+w

MT20

2.0

4.0

L

BMV1+p

MT20

2.0

4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS

MAX. FACTORED

FORCE

VERT. LOAD

LC1

MAX

WEBS

MAX. FACTORED

FORCE

MAX

MEMB.

(LBS)

(PLF)

CSI (LC)

UNBRAC

MEMB.

(LBS)

CSI (LC)

FR-TO

FROM

TO

LENGTH

FR-TO

L-B

-193 / 0

0.0

0.0

0.01 (1)

7.81

J-D

-149 / 0

0.04 (1)

A-B

0 / 29

-77.4

-77.4

0.10 (1)

10.00

K-C

-154 / 0

0.03 (1)

B-C

-18 / 0

-77.4

-77.4

0.04 (1)

6.25

I-E

-154 / 0

0.03 (1)

C-D

-11 / 0

-77.4

-77.4

0.04 (1)

6.25

D-E

-11 / 0

-77.4

-77.4

0.04 (1)

6.25

E-F

-18 / 0

-77.4

-77.4

0.04 (1)

6.25

F-G

0 / 29

-77.4

-77.4

0.10 (1)

10.00

H-F

-193 / 0

0.0

0.0

0.01 (1)

7.81

L-K

0 / 15

-17.5

-17.5

0.02 (4)

10.00

K-J

0 / 9

-17.5

-17.5

0.02 (4)

10.00

J-I

0 / 9

-17.5

-17.5

0.02 (4)

10.00

I-H

0 / 15

-17.5

-17.5

0.02 (4)

10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (F-G:1), BC=0.02/1.00 (H-I:4), WB=0.04/1.00 (D-J:1), SSI=0.07/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



September 14, 2018

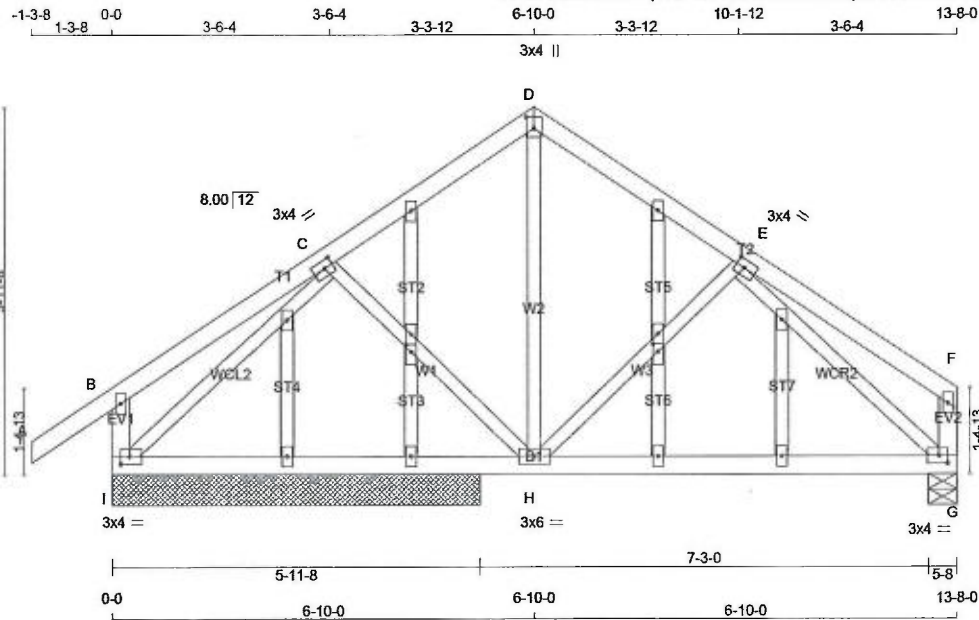
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Kott, Ottawa, Ontario, IM

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Version 8.210 S Mar 12 2018 MitTek Industries, Inc. Fri Sep 14 10:04:10 2018 Page 1



TOTAL WEIGHT = 68 lb (M/F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4 DRY	No.2
D - F	2x4 DRY	No.2
I - B	2x4 DRY	No.2
G - F	2x4 DRY	No.2
I - G	2x4 DRY	No.2

ALL WEBS EXCEPT 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	2.0	4.0		
C TMVWV-t	MT20	3.0	4.0	1.50	1.75
D TTW+p	MT20	3.0	4.0	2.25	1.50
E TMVWV-t	MT20	3.0	4.0	1.50	1.75
F TMV+p	MT20	2.0	4.0		
G BMVWV-t	MT20	3.0	4.0	1.50	1.75
H BMVWV-t	MT20	3.0	6.0		
I BMVWV-t	MT20	3.0	4.0	1.50	1.75
J NPW	MT20	2.0	4.0	1.50	1.00
J, K, L, M, N, O, P, Q, R, S					
J NPW	MT20	2.0	4.0		
Q NPW	MT20	2.0	4.0	1.50	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
I	754	0	754	0	5-11-8	10-15
G	648	0	648	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	527	382 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
G	455	318 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 29	-77.4	10.00	-138 / 7
B-C	0 / 16	-77.4	10.00	0 / 340
C-D	-509 / 0	-77.4	10.00	-138 / 6
D-E	-509 / 0	-77.4	10.00	-713 / 0
E-F	0 / 16	-77.4	10.00	-713 / 0
I-B	-208 / 0	0.0	7.81	
G-F	-102 / 0	0.0	7.81	
I-H	0 / 508	-17.5	10.00	
H-G	0 / 509	-17.5	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.14/1.00 (B-C:1), BC=0.27/1.00 (G-H:4), WB=0.26/1.00 (E-G:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

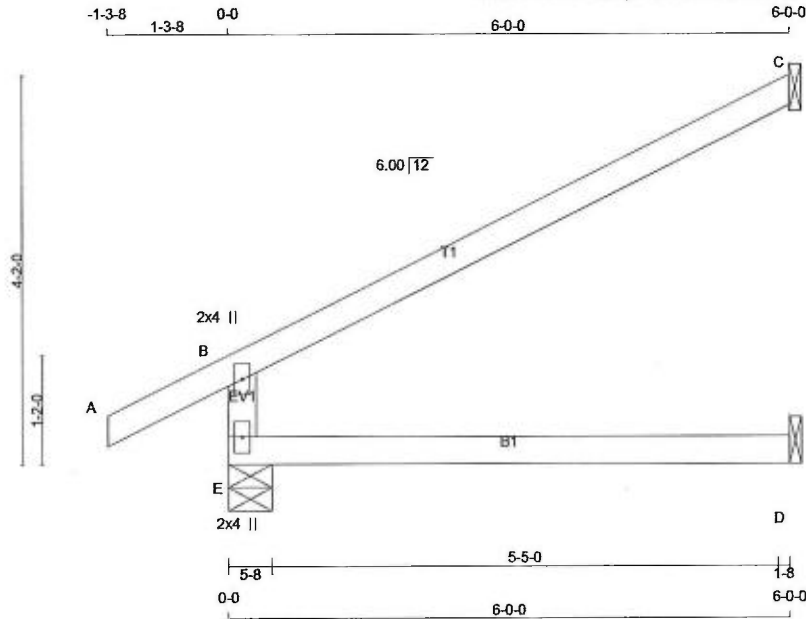
JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.25 (C) (INPUT = 1.00)



September 14, 2018

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Scale = 1:23.6

TOTAL WEIGHT = 16 X 17 = 274 lb (M)

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
JT	457	0	457	0	5-8	1-8
E	174	0	174	0	1-8	1-8
C	43	0	49	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	318	238 / 0	0 / 0	0 / 0	0 / 0	80 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS	
	MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)
E-B	-395 / 0	0.0	0.0	0.13 (4)	7.81	
A-B	0 / 23	-77.4	-77.4	0.10 (1)	10.00	
B-C	-26 / 0	-77.4	-77.4	0.47 (1)	6.25	
E-D	0 / 0	-17.5	-17.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 23.3 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.47/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (N/A:0), SSI=0.20/1.00 (B-C:1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2204 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
 JSI METAL= 0.09 (B) (INPUT = 1.00)

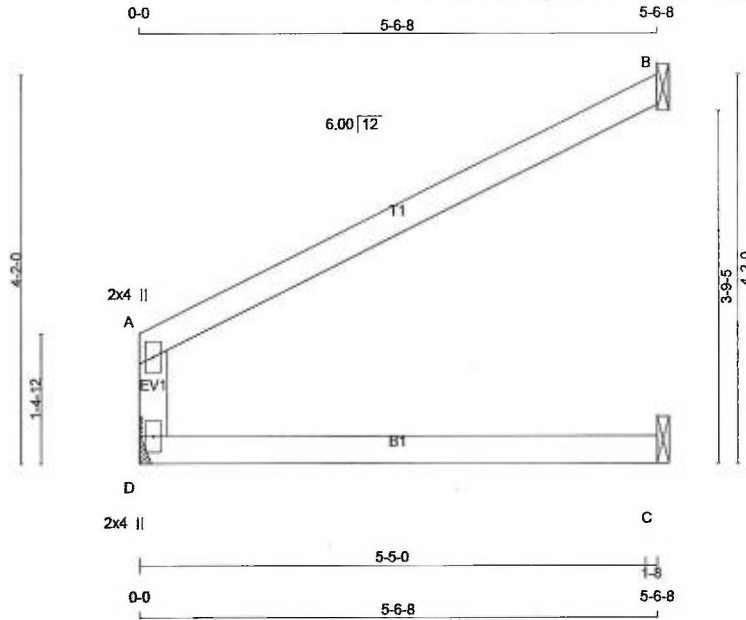


September 14, 2018

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Scale = 1:23.6

TOTAL WEIGHT = 4 X 15 = 60 lb

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	2.0	4.0	
D	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	263	0	263	0	0	0
B	197	0	197	0	1-8	1-8
C	66	0	66	0	1-8	1-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
D	184	129/0	0/0	0/0	0/0	55/0	0/0
B	134	116/0	0/0	0/0	0/0	18/0	0/0
C	50	13/0	0/0	0/0	0/0	37/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX.	FORCE	VERT. LOAD	MAX	MAX.	MEMB.	FORCE	MAX
FR-TO		(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
D-A	-232/0		0.0	0.0 0.12 (1)	7.81			
A-B	-8/1		-77.4	-77.4 0.34 (1)	10.00			
D-C	0/0		-17.5	-17.5 0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = 1/829 (0.08")

CSI: TC=0.34/1.00 (A-B:1), BC=0.17/1.00 (C-D:1),
WB=0.00/1.00 (n/a:0), SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

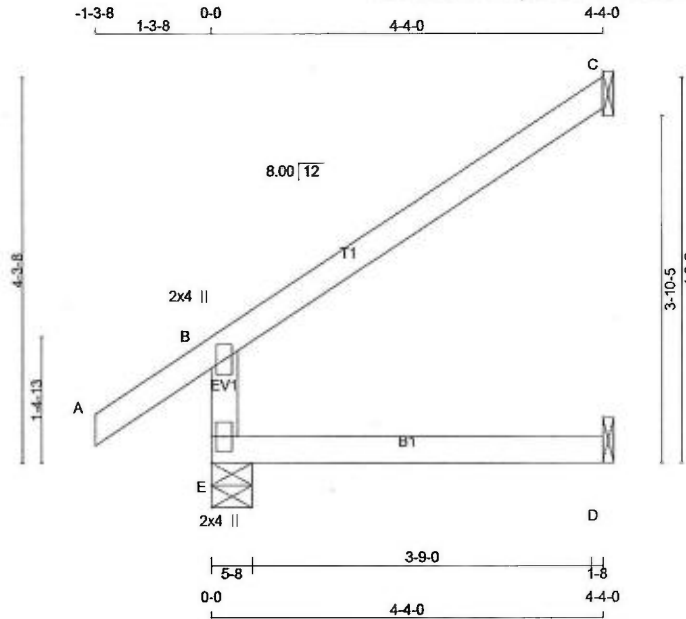
JSI GRIP= 0.13 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)



September 14, 2018

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TOTAL WEIGHT = 14 lb

LUMBER

N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0	
E	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	INPUT BRG IN-SX	REQD BRG IN-SX
E	359	0	359	0	5-8	1-8
C	126	0	126	0	1-8	1-8
D	32	0	36	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	249	190 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
C	85	78 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	26	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	
		VERT. LOAD LC1 (PLF)	MAX. PSI (LC)	FR-TO				
FR-TO		FROM	TO					
E-B	-316 / 0	0.0	0.0	0.06 (4)	7.81			
A-B	0 / 29	-77.4	-77.4	0.10 (1)	10.00			
B-C	-23 / 0	-77.4	-77.4	0.25 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.25/1.00 (B-C:1), BC=0.07/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

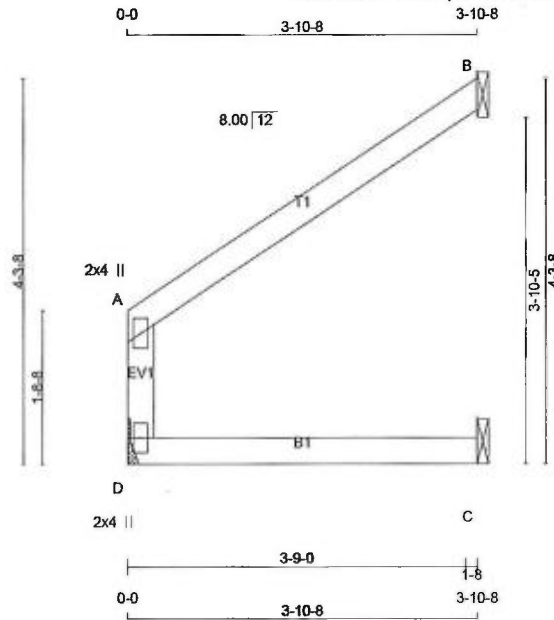
JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



September 14, 2018

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Scale = 1:24.5

TOTAL WEIGHT = 11 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	No.2	DESCR.	SPF	SPF
D - A	2x4	DRY	No.2						
A - B	2x4	DRY	No.2						
D - C	2x4	DRY	No.2						

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	184	184	0	0
B	139	139	0	0
C	45	45	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	129	90/0	0/0	0/0	0/0	0/0	39/0	0/0
B	95	82/0	0/0	0/0	0/0	0/0	13/0	0/0
C	34	8/0	0/0	0/0	0/0	0/0	26/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE MAX (LBS)
FR-TO				
D-A	-181/0	0.0	0.0 0.05 (1)	7.81
A-B	-8/0	-77.4	-77.4 0.16 (1)	10.00
D-C	0/0	-17.5	-17.5 0.08 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN.CIG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.16/1.00 (A-B:1), BC=0.08/1.00 (C-D:1), WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

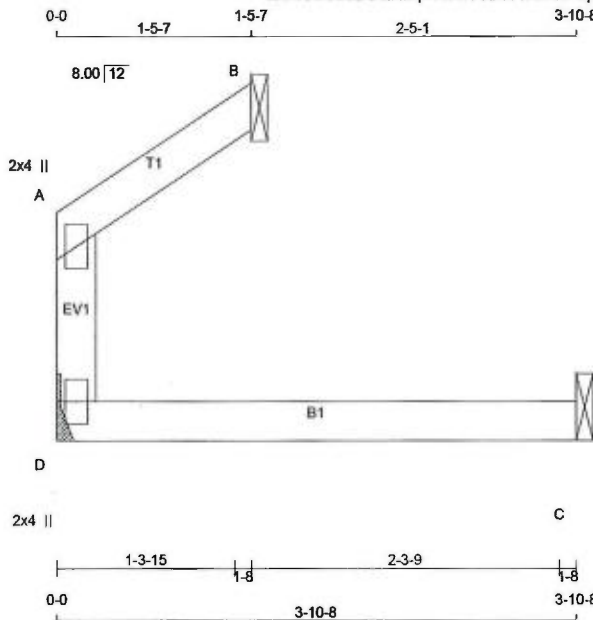
JSI GRIP= 0.10 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:16.5

TOTAL WEIGHT = 8 lb (M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
D - A	2x4	DRY	No.2	
A - B	2x4	DRY	No.2	
D - C	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0			
D	BMV1+p	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	86	86	0	0
B	63	63	0	0
C	31	34	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST CASE	MAX / MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	62	35 / 0	0 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
B	44	32 / 0	0 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	25	1 / 0	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CS1 (LC)	MAX	UNBRAC	MEMB.	FORCE (LBS)
FR-TO		FROM	TO		LENGTH	FR-TO	
D-A	-50 / 4	0.0	0.0	0.03 (4)	7.81		
A-B	0 / 5	-77.4	-77.4	0.04 (1)	10.00		
D-C	0 / 0	-17.5	-17.5	0.06 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.04/1.00 (A-B:1), BC=0.06/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SSI=0.05/1.00 (C-D:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1567 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.03 (A) (INPUT = 0.90)
JSI METAL= 0.01 (A) (INPUT = 1.00)



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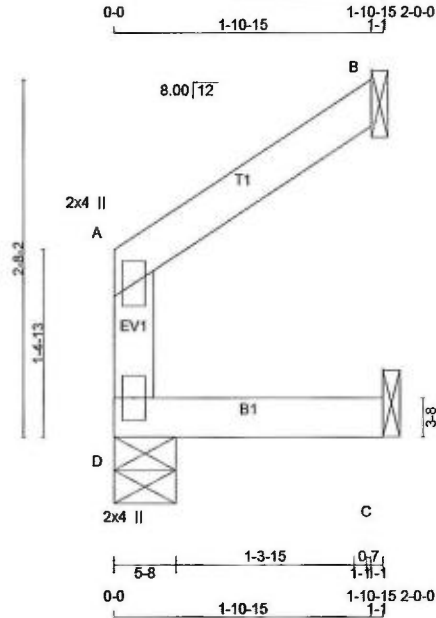
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Scale = 1:16.5

TOTAL WEIGHT = 6 lb (M/F)

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
D - A 2x4 DRY No.2
A - B 2x4 DRY No.2
D - C 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A TMV+p MT20 2.0 4.0
D BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
D	92	0	92	0	5-8	1-8
B	70	0	70	0	1-8	1-8
C	22	0	22	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37578H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	64	45/0	0/0	0/0	0/0	20/0	0/0
B	47	41/0	0/0	0/0	0/0	6/0	0/0
C	17	3/0	0/0	0/0	0/0	13/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
D-A	-78/0	0.0	0.0	0.01 (1)			
A-B	-2/0	-77.4	-77.4	0.04 (1)	10.00		
D-C	0/0	-17.5	-17.5	0.02 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.02/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SSI=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



September 14, 2018

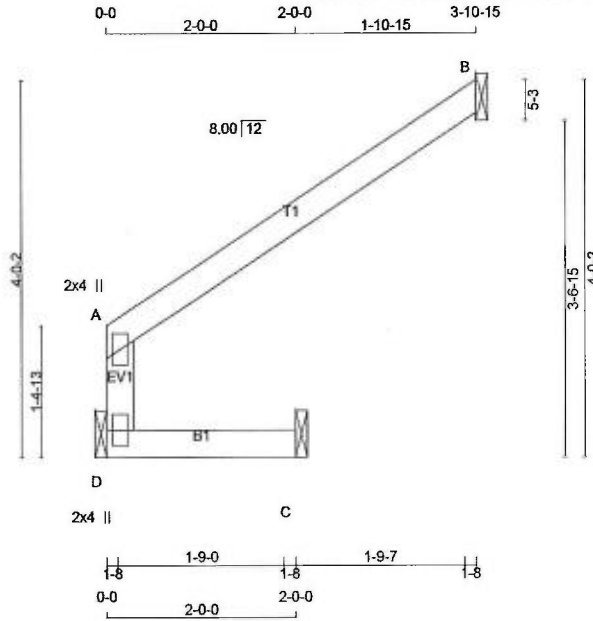
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TOTAL WEIGHT = 9 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
D - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
D - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
D	152	152	0	1-8
B	134	134	0	1-8
C	52	52	0	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D, B, C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
D	106	81/0	0/0	0/0	25/0	0/0
B	91	80/0	0/0	0/0	11/0	0/0
C	37	21/0	0/0	0/0	16/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO								
D-A	-169/0	0.0	0.0	0.08 (1)	7.81			
A-B	-10/0	-77.4	-77.4	0.15 (1)	10.00			
D-C	0/0	-17.5	-17.5	0.09 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN.C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/969$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/969$ (0.00")

CSI: TC=0.15/1.00 (A-B-1), BC=0.09/1.00 (C-D-1), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

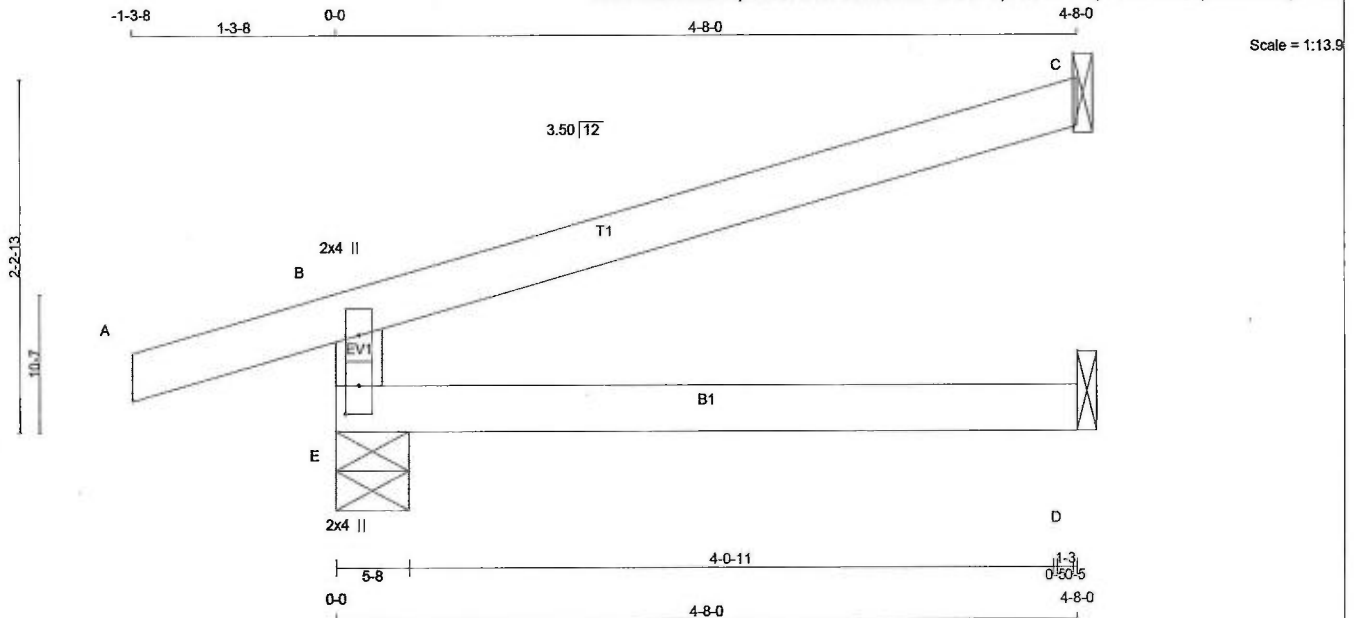
JSI GRIP= 0.11 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)



September 14, 2018

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
BEARINGS	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
E	377	0	377	0
C	135	0	135	0
D	33	0	37	0

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	262	198 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
C	92	82 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	27	0 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO		
E-B	-329 / 0	0.0 0.0	0.08 (4)	7.81		
A-B	0 / 14	-77.4 -77.4	0.10 (1)	10.00		
B-C	-13 / 0	-77.4 -77.4	0.29 (1)	6.25		
E-D	0 / 0	-17.5 -17.5	0.08 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH.	LL	PSF
	DL	23.3	PSF
	LL	3.0	PSF
	DL	0.0	PSF
	LL	7.0	PSF
	TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.29/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

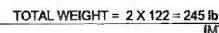
JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.06 (E) (INPUT = 1.00)



September 14, 2018

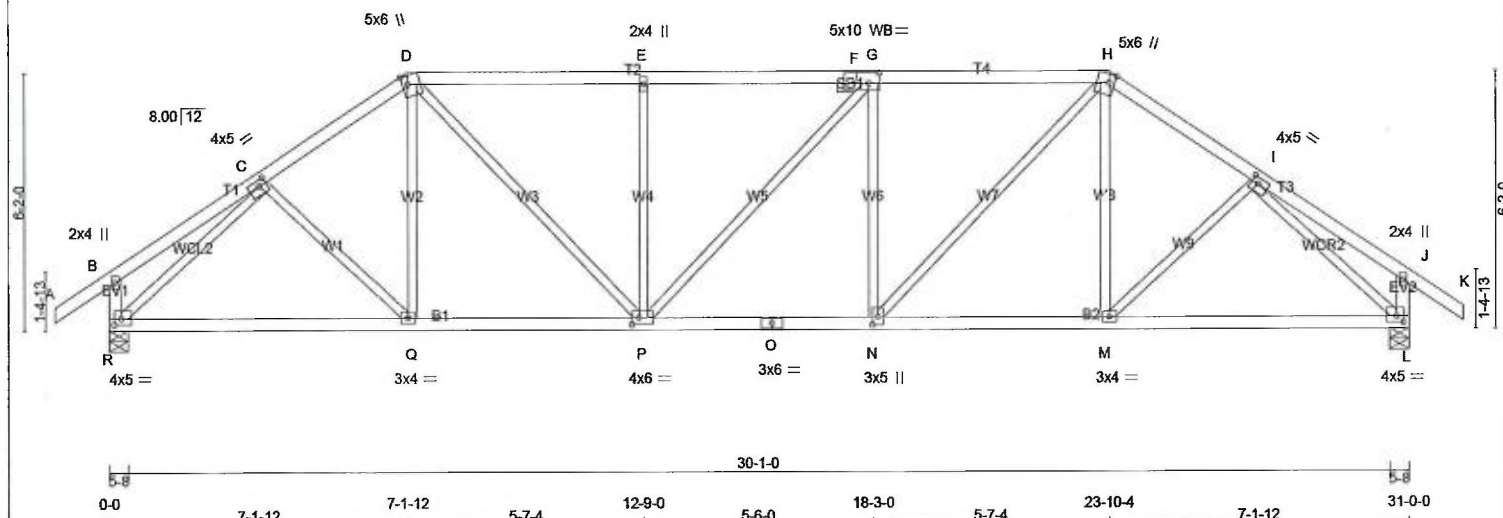
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IN THE DESIGN OF THIS COMPONENT.**





TOTAL WEIGHT = 2 X 133 = 266 LB

[M]

LUMBER	N. L. G. A. RULES				
CHORDS	SIZE	LUMBER			
A - D	2x4	DRY	No.2		
D - F	2x4	DRY	No.2		
F - H	2x4	DRY	No.2		
H - K	2x4	DRY	No.2		
K - B	2x4	DRY	No.2		
L - J	2x4	DRY	No.2		
R - O	2x4	DRY	No.2		
O - L	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2
EXCEPT SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-I	MT20	4.0	5.0	1.75	2.00
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMV+w	MT20	2.0	4.0		
F						
F	TSWW-I	MT20	5.0	10.0	3.00	3.00
G						
H	TTWW+m	MT20	5.0	6.0	2.50	1.50
I	TMWW-I	MT20	4.0	5.0	1.75	2.00
J	TMV+p	MT20	2.0	4.0		
L	BMVWI-I	MT20	4.0	5.0	1.75	2.00
M	BMVWI-I	MT20	3.0	4.0		
N	BMVW+I	MT20	3.0	5.0	2.25	1.50
O	BS-I	MT20	3.0	6.0		
P	BMVWWW-I	MT20	4.0	6.0	2.00	2.00
Q	BMVWI-I	MT20	3.0	4.0		
R	BMVWI-I	MT20	4.0	5.0	1.75	2.00

WB - INDICATES BLOCKING REQUIRED

SIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED BRG	INPUT BRG	REQRD BRG
JT	VERT HORZ	DOWN HORZ	UPLIFT IN-SX	IN-SX
R	1577 0	1577 0	0 5-8	1-12
L	1577 0	1577 0	0 5-8	1-12

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS					
JT	SNOW LIVE PERM.LIVE DEAD SOIL						
R	1104 786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0	
L	1104 786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS			WEBS		
	MAX. FACTORED MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED MEMB.	FORCE (LBS)
FR-TO			FROM TO			
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	C-Q	0 / 67
B-C	0 / 16	-77.4	-77.4 0.14 (1)	10.00	D-Q	0 / 103
C-D	-1693 / 0	-77.4	-77.4 0.18 (1)	4.94	D-P	0 / 763
D-E	-1922 / 0	-77.4	-77.4 0.41 (1)	4.44	P-E	-464 / 0
E-F	-1922 / 0	-77.4	-77.4 0.41 (1)	4.43	P-G	-2 / 0
F-G	-1922 / 0	-77.4	-77.4 0.41 (1)	4.43	N-G	-465 / 0
G-H	-1923 / 0	-77.4	-77.4 0.41 (1)	4.43	N-H	0 / 765
H-I	-1693 / 0	-77.4	-77.4 0.18 (1)	4.94	M-H	0 / 103
I-J	0 / 16	-77.4	-77.4 0.14 (1)	10.00	M-I	0 / 67
J-K	0 / 29	-77.4	-77.4 0.10 (1)	10.00	R-C	-1879 / 0
R-B	-216 / 0	0.0	0.0 0.02 (1)	7.81	I-L	-1879 / 0
L-J	-216 / 0	0.0	0.0 0.02 (1)	7.81		

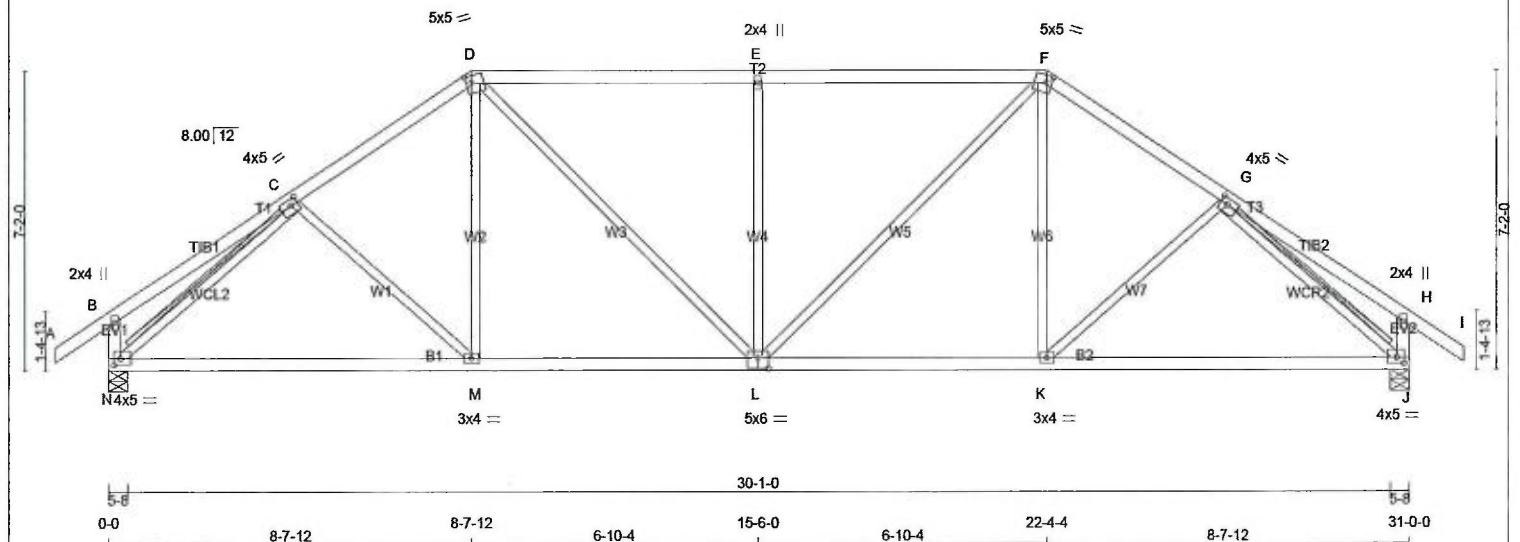
R-Q	0 / 1349	-17.5	-17.5 0.33 (1)	10.00
Q-P	0 / 1394	-17.5	-17.5 0.33 (1)	10.00
P-O	0 / 1923	-17.5	-17.5 0.36 (1)	10.00
O-N	0 / 1923	-17.5	-17.5 0.36 (1)	10.00
N-M	0 / 1394	-17.5	-17.5 0.33 (1)	10.00
M-L	0 / 1348	-17.5	-17.5 0.32 (1)	10.00



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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE		LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVp	MT20	2.0	4.0		
C	TMVW-I	MT20	4.0	5.0	1.75	2.00
D	TTWW-m	MT20	5.0	5.0	2.00	1.50
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	5.0	2.00	1.50
G	TMWW-I	MT20	4.0	5.0	1.75	2.00
H	TMVp	MT20	2.0	4.0		
J	TMVW-I	MT20	4.0	5.0	1.75	2.00
K	BMVW-I	MT20	3.0	4.0		
L	BSWWW-I	MT20	5.0	6.0	3.00	3.00
M	BMVW-I	MT20	3.0	4.0		
N	BMVW-I	MT20	4.0	5.0	1.75	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1577	0	1577	0	0	5-8	1-12
J	1577	0	1577	0	0	5-8	1-12

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	1ST LGASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-N, G-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (L/C)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (L/C)		
FR-TO		FROM	TO		FR-TO				
A-B	0/29	-77.4	-77.4	0.10 (1)	10.00	C-M	-55/44	0.03 (1)	
B-C	0/21	-77.4	-77.4	0.22 (1)	10.00	M-D	0/200	0.05 (4)	
C-D	-1648/0	-77.4	-77.4	0.21 (1)	4.98	D-L	0/524	0.12 (1)	
D-E	-177.0	-77.4	-77.4	0.53 (1)	4.46	L-E	-652/0	0.58 (1)	
E-F	-1726/0	-77.4	-77.4	0.53 (1)	4.46	L-F	0/524	0.12 (1)	
F-G	-1648/0	-77.4	-77.4	0.21 (1)	4.98	K-F	0/200	0.05 (4)	
G-H	0/21	-77.4	-77.4	0.22 (1)	10.00	K-G	-55/44	0.03 (1)	
H-I	0/29	-77.4	-77.4	0.10 (1)	10.00	N-C	-1891/0	0.50 (1)	
N-B	-236/0	0.0	0.0	0.02 (1)	7.81	G-J	-1891/0	0.50 (1)	
J-H	-236/0	0.0	0.0	0.02 (1)	7.81				
N-M	0/1393	-17.5	-17.5	0.39 (4)	10.00				
M-L	0/1355	-17.5	-17.5	0.40 (4)	10.00				
L-K	0/1355	-17.5	-17.5	0.40 (4)	10.00				
K-J	0/1393	-17.5	-17.5	0.39 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9, NBCC
2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (1.03")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL)= $L/360$ (1.03")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.20")

CSI: TC=0.53/1.00 (E-F:1), BC=0.40/1.00 (L-M:4),
WB=0.58/1.00 (E-L:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

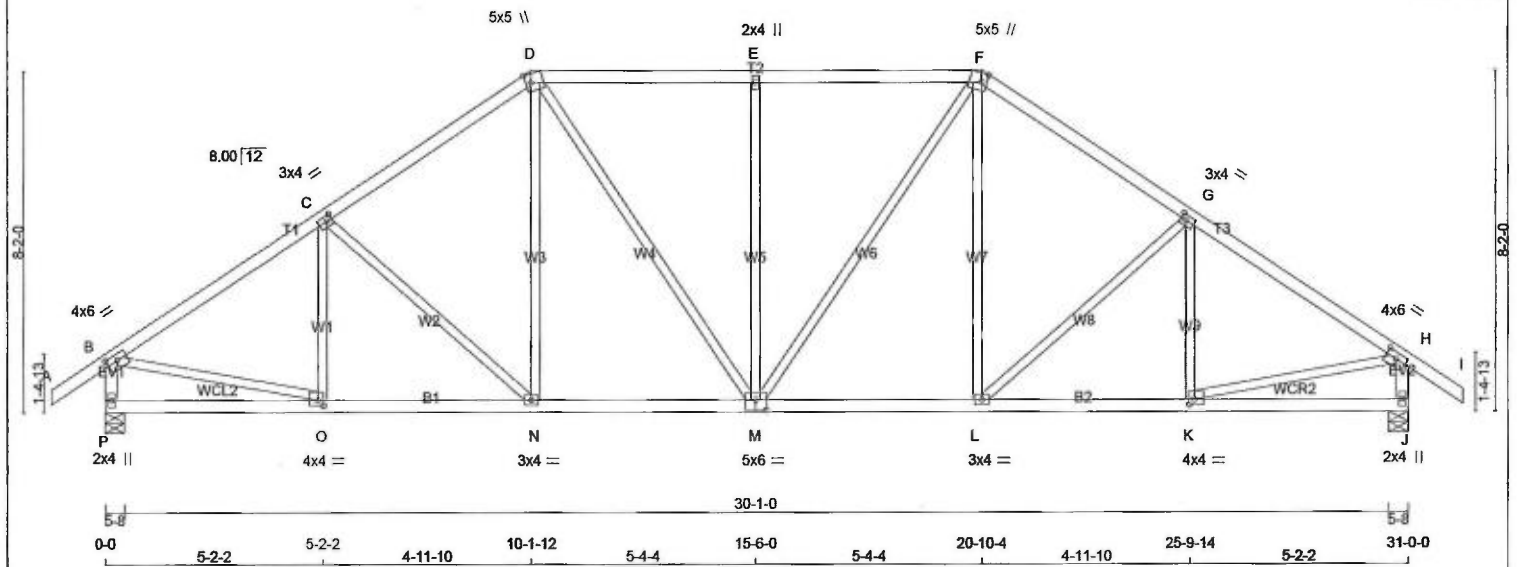
JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)



September 14, 2018

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TOTAL WEIGHT = 2 X 140 = 281 lb (M/P)

LUMBER			DESIGNER		
N. L. G. A. RULES			FACTORED		
CHORDS	SIZE	LUMBER	SPF	GROSS REACTION	MAXIMUM FACTORED
A - D	2x4	DRY	No.2	VERT	DOWN
D - F	2x4	DRY	No.2	HORZ	HORZ
F - I	2x4	DRY	No.2	UPLIFT	UPLIFT
P - B	2x4	DRY	No.2	IN-SX	IN-SX
J - H	2x4	DRY	No.2	2-11	2-11
P - M	2x4	DRY	No.2	5-8	5-8
M - J	2x4	DRY	No.2	2-11	2-11

ALL WEBS EXCEPT			UNFACTORED REACTIONS		
2x3	DRY	No.2	1ST LOSE	MAX/MIN COMPONENT REACTIONS	
			JT	COMBINED	SNOW
			P	1104	786 / 0
			J	1104	786 / 0

DRY: SEASONED LUMBER.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)
A-B	0 / 29	-77.4	O-C	-210 / 22	0.07 (1)
B-C	-1727 / 0	-77.4	C-N	-240 / 0	0.21 (1)
C-D	-1565 / 0	-77.4	N-D	0 / 252	0.06 (1)
D-E	-1485 / 0	-77.4	D-M	0 / 360	0.08 (1)
E-F	-1485 / 0	-77.4	M-E	-505 / 0	0.65 (1)
F-G	-1565 / 0	-77.4	M-F	0 / 360	0.08 (1)
G-H	-1727 / 0	-77.4	L-F	0 / 252	0.06 (1)
H-I	0 / 29	-77.4	L-G	-240 / 0	0.21 (1)
P-B	-1538 / 0	0.0	K-G	-210 / 22	0.07 (1)
J-H	-1538 / 0	0.0	B-O	0 / 1489	0.34 (1)
P-O	0 / 0	-17.5	K-H	0 / 1489	0.34 (1)
O-N	0 / 1458	-17.5			
N-M	0 / 1282	-17.5			
M-L	0 / 1282	-17.5			
L-K	0 / 1458	-17.5			
K-J	0 / 0	-17.5			

ALLOWABLE DEFL. (LL) = L/360 (1.03")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
 ALLOWABLE DEFL. (TL) = L/360 (1.03")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.12")

CSI: TC=0.31/1.00 (D-E-1), BC=0.29/1.00 (N-O-1), WB=0.65/1.00 (E-M-1), SS=0.20/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

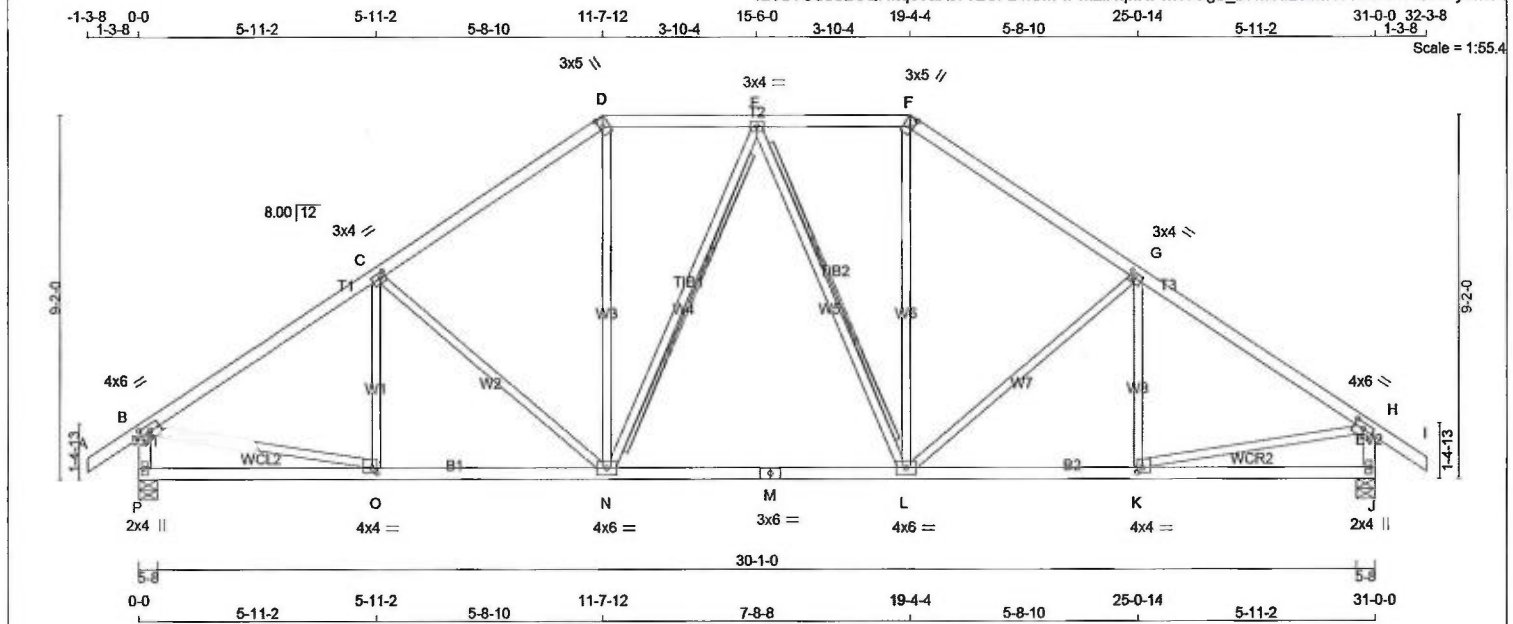
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
 JSI METAL= 0.55 (O) (INPUT = 1.00)



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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	6.0	1.75	3.00
C	TMVW-1	MT20	3.0	4.0	1.50	1.50
D	TTW+h	MT20	3.0	5.0	2.50	1.00
E	TMVW-1	MT20	3.0	4.0	1.50	1.50
F	TTW+h	MT20	3.0	5.0	2.50	1.00
G	TMVW-1	MT20	3.0	4.0	1.50	1.50
H	TMVW-1	MT20	4.0	6.0	1.75	3.00
J	BMV1+p	MT20	2.0	4.0	1.50	1.50
K	BMVW-1	MT20	4.0	4.0	1.50	1.50
L	BMVW-1	MT20	4.0	6.0	1.75	3.00
M	BS-1	MT20	3.0	6.0	1.50	1.50
N	BMVW-1	MT20	4.0	6.0	1.75	3.00
O	BMVW-1	MT20	4.0	4.0	1.50	1.50
P	BMV1+p	MT20	2.0	4.0	1.50	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
P	1577	0	1577	0
J	1577	0	1577	0

UNFACTORED REACTIONS

	1ST CASE	2ND CASE	3RD CASE	4TH CASE	5TH CASE	6TH CASE	7TH CASE	8TH CASE	9TH CASE	10TH CASE
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL			
P	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0			
J	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0			

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	FACTORED FORCE (LBS)	MEMB.	FACTORED FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 29	-77.4	-77.4	0.10 (1)	10.00
B-C	-1739 / 0	-77.4	-77.4	0.41 (1)	4.65
C-D	-1488 / 0	-77.4	-77.4	0.38 (1)	4.97
D-E	-1217 / 0	-77.4	-77.4	0.15 (1)	5.65
E-F	-1217 / 0	-77.4	-77.4	0.15 (1)	5.65
F-G	-1488 / 0	-77.4	-77.4	0.38 (1)	4.97
G-H	-1739 / 0	-77.4	-77.4	0.41 (1)	4.65
H-I	0 / 29	-77.4	-77.4	0.10 (1)	10.00
P-B	-1532 / 0	0.0	0.0	0.16 (1)	6.65
J-H	-1532 / 0	0.0	0.0	0.16 (1)	6.65
P-O	0 / 0	-17.5	-17.5	0.13 (4)	10.00
O-N	0 / 1472	-17.5	-17.5	0.33 (1)	10.00
N-M	0 / 1292	-17.5	-17.5	0.30 (1)	10.00
M-L	0 / 1292	-17.5	-17.5	0.30 (1)	10.00
L-K	0 / 1472	-17.5	-17.5	0.33 (1)	10.00
K-J	0 / 0	-17.5	-17.5	0.13 (4)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (1.03")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.16")

CSI: TC=0.41/1.00 (B-C:1), BC=0.33/1.00 (N-O:1), WB=0.41/1.00 (C-N:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

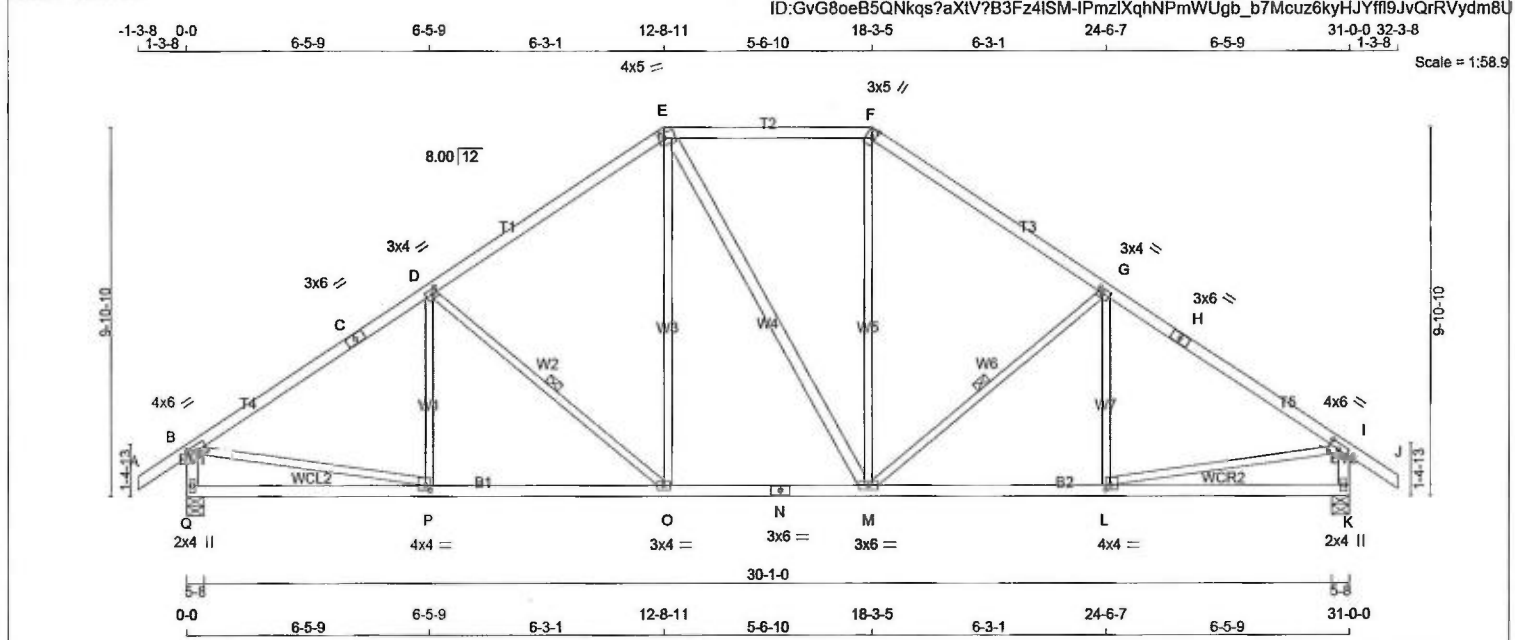
JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.55 (O) (INPUT = 1.00)



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TOTAL WEIGHT = 141 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	4.0	6.0	1.75	3.00
C	TS-1	MT20	3.0	6.0		
D	TMVW-1	MT20	3.0	4.0	1.50	1.50
E	TTVW-m	MT20	4.0	5.0	1.75	1.50
F	TTVW-h	MT20	3.0	5.0	2.50	1.00
G	TMVW-1	MT20	3.0	4.0	1.50	1.50
H	TS-1	MT20	3.0	6.0		
I	TMVW-1	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMVW-1	MT20	4.0	4.0	1.50	1.50
M	BMVW-1	MT20	3.0	6.0		
N	BS-1	MT20	3.0	6.0		
O	BMVW-1	MT20	3.0	4.0		
P	BMVW-1	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 1577	HORZ 0	DOWN 1577	HORZ 0
Q	VERT 1577	HORZ 0	DOWN 1577	HORZ 0
K	VERT 1577	HORZ 0	DOWN 1577	HORZ 0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
K	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1- 1x4 LATERAL BRACE(S) AT 1/2 LENGTH OF D-O, G-M. DBS = 20-0-0. CBF = 53 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"x3") SPIRAL NAILS: 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 29	-77.4 -77.4	0.10 (1)	10.00	P-D	-121 / 71	0.06 (1)
B-C	-1743 / 0	-77.4 -77.4	0.50 (1)	4.53	D-O	-426 / 0	0.20 (1)
C-D	-1743 / 0	-77.4 -77.4	0.50 (1)	4.53	O-E	0 / 374	0.08 (1)
D-E	-1420 / 0	-77.4 -77.4	0.45 (1)	4.96	E-M	0 / 1	0.00 (1)
E-F	-1157 / 0	-77.4 -77.4	0.33 (1)	5.53	M-F	0 / 375	0.08 (1)
F-G	-1421 / 0	-77.4 -77.4	0.45 (1)	4.96	M-G	-426 / 0	0.20 (1)
G-H	-1743 / 0	-77.4 -77.4	0.50 (1)	4.53	L-G	-122 / 71	0.06 (1)
H-I	-1743 / 0	-77.4 -77.4	0.50 (1)	4.53	B-P	0 / 1498	0.34 (1)
I-J	0 / 29	-77.4 -77.4	0.10 (1)	10.00	L-I	0 / 1498	0.34 (1)
Q-B	-1530 / 0	0.0 0.0	0.16 (1)	6.65			
K-I	-1530 / 0	0.0 0.0	0.16 (1)	6.65			
Q-P	0 / 0	-17.5 -17.5	0.18 (4)	10.00			
P-O	0 / 1478	-17.5 -17.5	0.33 (1)	10.00			
O-N	0 / 1156	-17.5 -17.5	0.25 (1)	10.00			
N-M	0 / 1156	-17.5 -17.5	0.25 (1)	10.00			
M-L	0 / 1477	-17.5 -17.5	0.32 (1)	10.00			
L-K	0 / 0	-17.5 -17.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 0.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.50/1.00 (B-D:1), BC=0.33/1.00 (O-P:1), WB=0.34/1.00 (B-P:1), SSI=0.20/1.00 (B-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

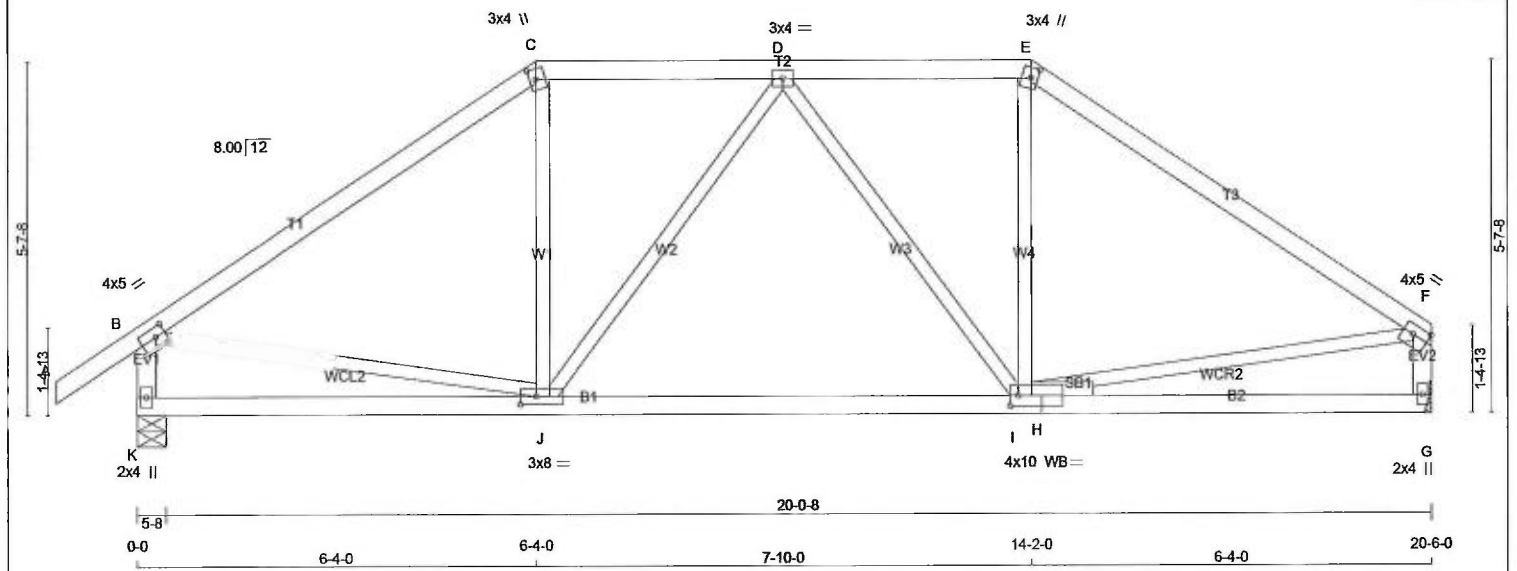
PLATE ROTATION TOL. = 5.0 Deg.

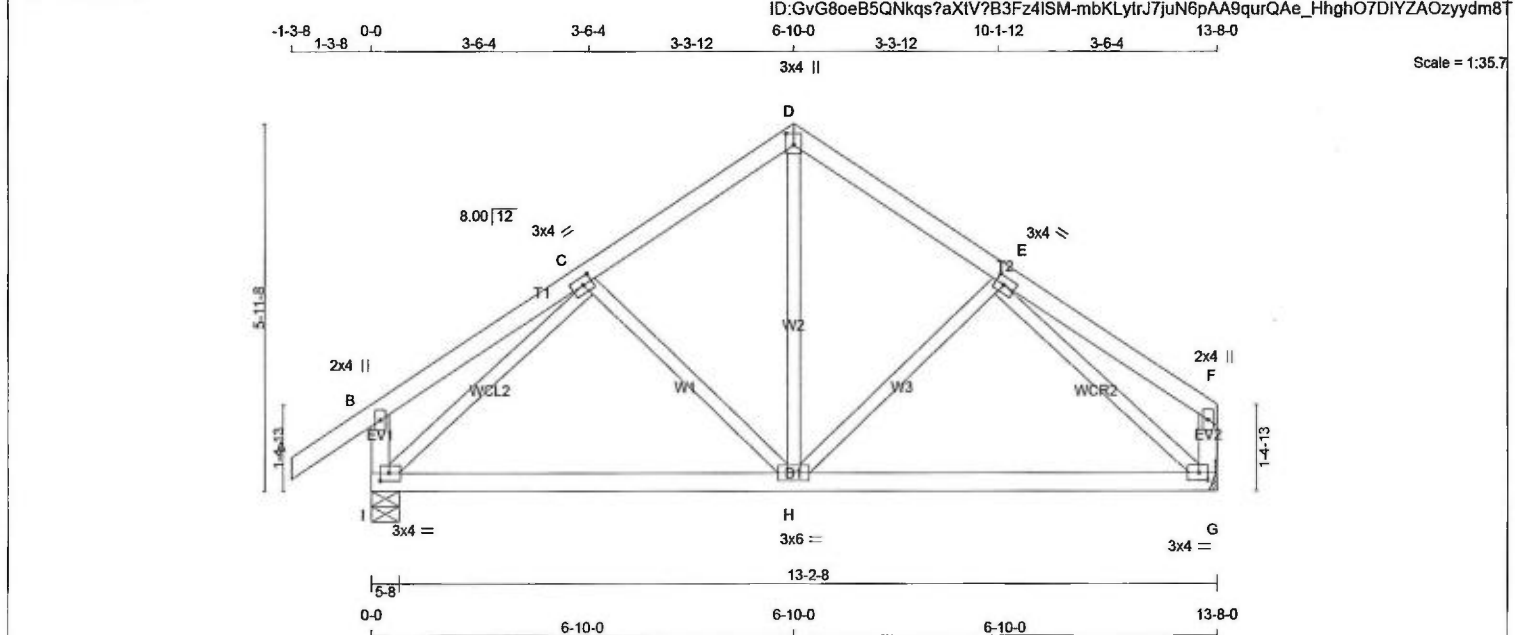
JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.55 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.







LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
I - B	2x4	DRY No.2
G - F	2x4	DRY No.2
I - G	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-1	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-1	MT20	3.0	4.0	1.50	1.75
F	TMV+p	MT20	2.0	4.0		
G	BMVW-1	MT20	3.0	4.0	1.50	1.75
H	BMVW-1	MT20	3.0	6.0		
I	BMVW-1	MT20	3.0	4.0	1.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG UPLIFT	REQD BRG
I	754	0	0	5-8
G	648	0	0	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	527	382 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
G	455	318 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.
A-B	0 / 29	-77.4	10.00	C-H	-138 / 7	0.05 (1)	
B-C	0 / 16	-77.4	10.00	H-D	0 / 340	0.08 (1)	
C-D	-509 / 0	-77.4	6.25	H-E	-138 / 6	0.05 (1)	
D-E	-509 / 0	-77.4	6.25	I-C	-713 / 0	0.26 (1)	
E-F	0 / 16	-77.4	10.00	E-G	-713 / 0	0.26 (1)	
I-B	-208 / 0	0.0	7.81				
G-F	-102 / 0	0.0	7.81				
I-H	0 / 508	-17.5	10.00				
H-G	0 / 509	-17.5	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.14/1.00 (B-C:1), BC=0.27/1.00 (G-H:4), WB=0.26/1.00 (E-G:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)

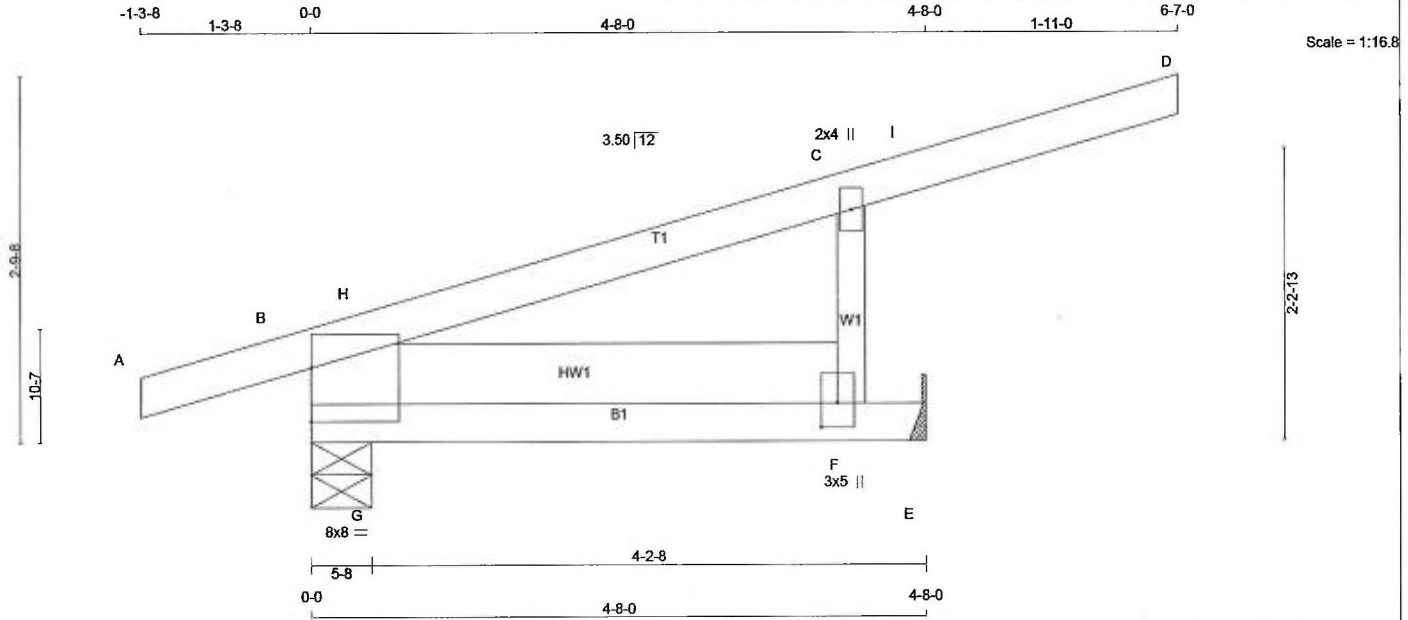
JSI METAL= 0.25 (E) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 6 X 23 = 139 lb [M]

LUMBER
N. L. G. A. RULES
CHORDS SIZE DRY LUMBER
A - D 2x4 DRY No.2
B - E 2x4 DRY No.2

REINFORCING MEMBERS
HW1 2x6 DRY No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBMW1-J	MT20	8.0	8.0	5.00	
C	TMMW+u	MT20	2.0	4.0		
F	BMWW+u	MT20	3.0	5.0	2.25	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	347	0	347	0	5-8	1-8
E	350	0	350	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	241	184 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
E	243	186 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	-7 / 0	-77.4	-77.4	0.09 (1)	10.00	F-C	-311 / 0	0.05 (1)	
B-H	-548 / 0	-77.4	-77.4	0.12 (1)	6.25	G-H	0 / 293	0.00 (1)	
H-C	-14 / 0	-77.4	-77.4	0.12 (1)	6.25	H-F	-385 / 0	0.05 (1)	
C-I	-28 / 0	-77.4	-77.4	0.35 (1)	6.25				
I-D	-28 / 0	-77.4	-77.4	0.35 (1)	6.25				
B-G	0 / 385	-17.5	-17.5	0.18 (1)	10.00				
G-F	0 / 385	-17.5	-17.5	0.33 (1)	10.00				
F-E	0 / 0	-17.5	-17.5	0.28 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.35/1.00 (C-D:1), BC=0.33/1.00 (F-G:1), WB=0.05/1.00 (F-H:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (F) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



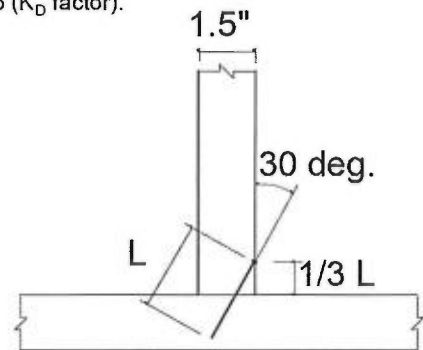
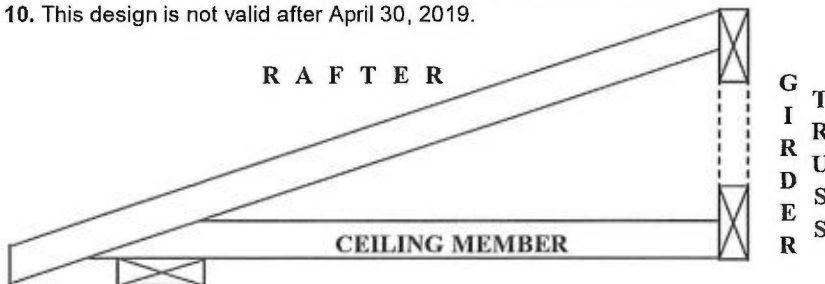
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9-3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to **wind** the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry (< 19% moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



TOE-NAIL INSTALLATION

Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

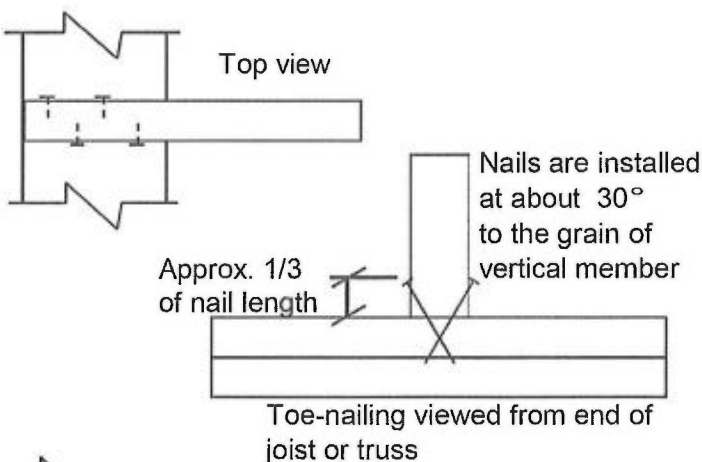
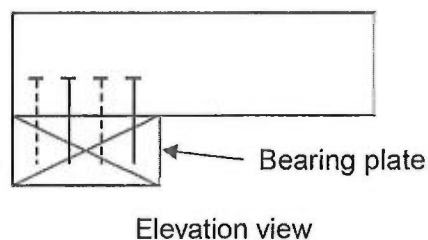
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

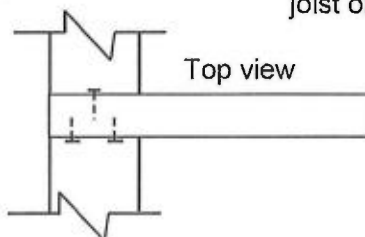
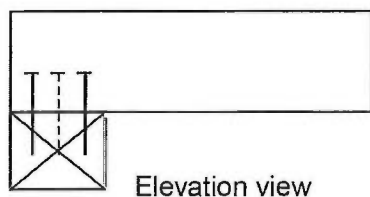
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



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April 26, 2017